

ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 0 278
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID: ITB18228Z0124134501

Truss Fabricator: Anderson Truss Company
Job Identification: 7-212--OWNER BUILDER Josh Campbell -- 386-984-6589-C
Truss Count: 26
Model Code: Florida Building Code 2004 and 2006 Supplement
Truss Criteria: ANSI/TPI-2002(STD)/FBC
Engineering Software: Alpine Software, Version 7.36.
Minimum Design Loads: Roof - 40.0 PSF @ 1.25 Duration
Floor - N/A
Wind - 110 MPH ASCE 7-02 -Closed



Seal Date: 09/24/2007

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. As shown on attached drawings; the drawing number is preceded by: HCUSR8228

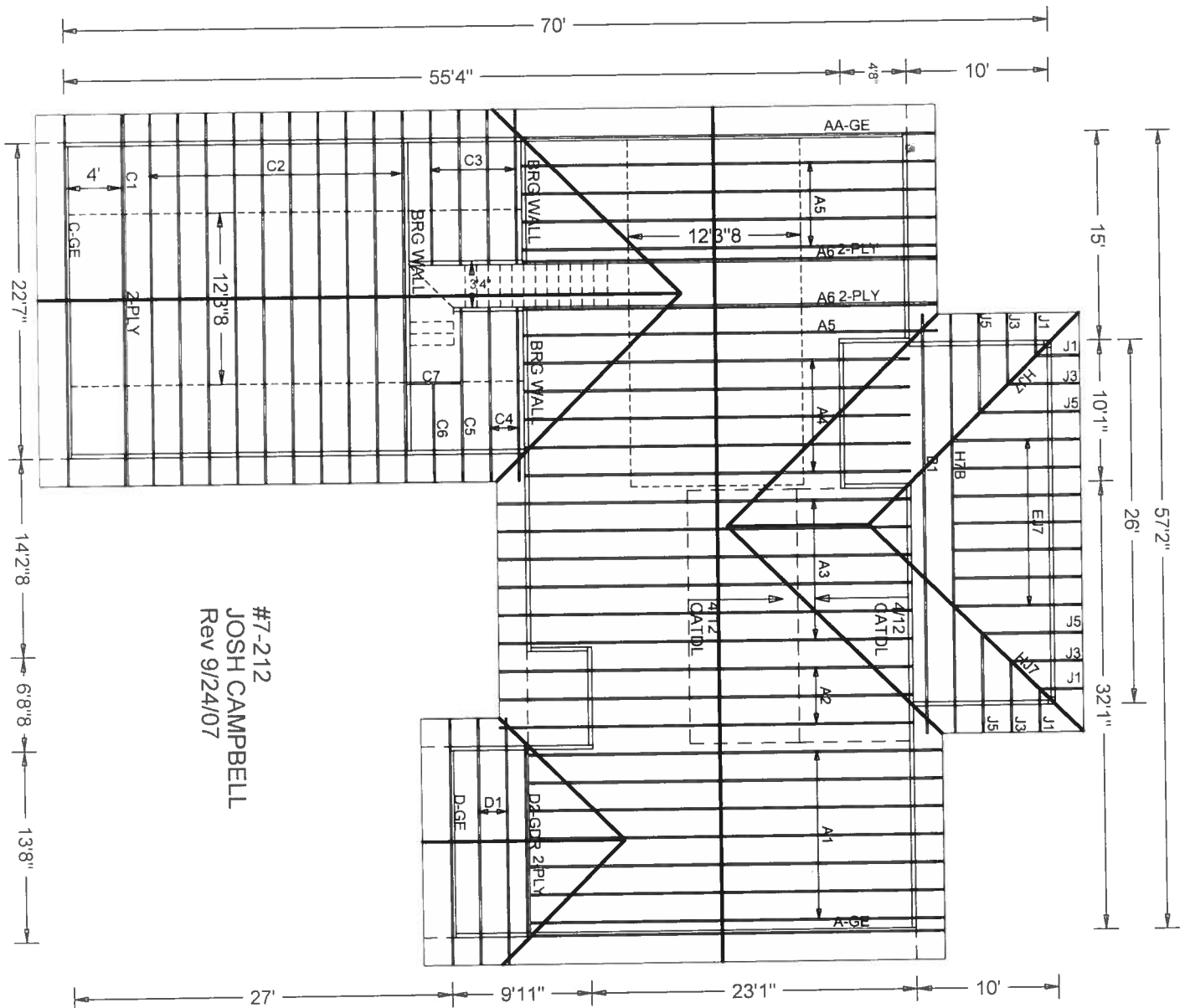
-Truss Design Engineer-

1950 Marley Drive
Haines City, FL 33844

Details: A11015EE-GBLLETIN-BRCLBSUB-

#	Ref	Description	Drawing#	Date
1	40812--A5		07267010	09/24/07
2	40813--A1		07267017	09/24/07
3	40814--A2		07267018	09/24/07
4	40815--A3		07267016	09/24/07
5	40816--A6		07267012	09/24/07
6	40817--AA-GE		07267001	09/24/07
7	40818--A-GE		07267013	09/24/07
8	40819--A4		07267014	09/24/07
9	40820--H7B		07267001	09/24/07
10	40821--B1		07267011	09/24/07
11	40822--C2		07267005	09/24/07
12	40823--C1		07267015	09/24/07
13	40824--C-GE		07267007	09/24/07
14	40825--D1		07267002	09/24/07
15	40826--D-GE		07267025	09/24/07
16	40827--D2-GDR		07267004	09/24/07
17	40828--J1		07267023	09/24/07
18	40829--HJ7		07267020	09/24/07
19	40830--J3		07267022	09/24/07
20	40831--J5		07267021	09/24/07
21	40832--EJ7		07267024	09/24/07
22	40833--C3		07267008	09/24/07
23	40834--C6		07267009	09/24/07
24	40835--C7		07267006	09/24/07
25	40836--C5		07267003	09/24/07
26	40837--C4		07267019	09/24/07





JOB DESCRIPTION:: OWNER BUILDER
/: Josh Campbell

JOB NO:

7-212

PAGE NO:

1 OF 1

110 mph wind, 15.00 ft mean hgt. ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCPI (V/V_0)=0.18

Wind reactions based on MIFRS pressures.

BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 7-6-12 to 19-10-4.

** (1) 2x8x(6 0 0) TRIMMED TO FIT SP #SS OR BETTER SCAB. ATTACH ONE SCAB TO ONE FACE OF TRUSS USING 0.128"x3.0" NAILS @ 4" OC STAGGERED THROUGHOUT, WITHOUT SPLITTING THE LUMBER.



REF	R8228 - 40812
DATE	09/24/07

ITW Building Components Group, Inc.

James City, FL 33844

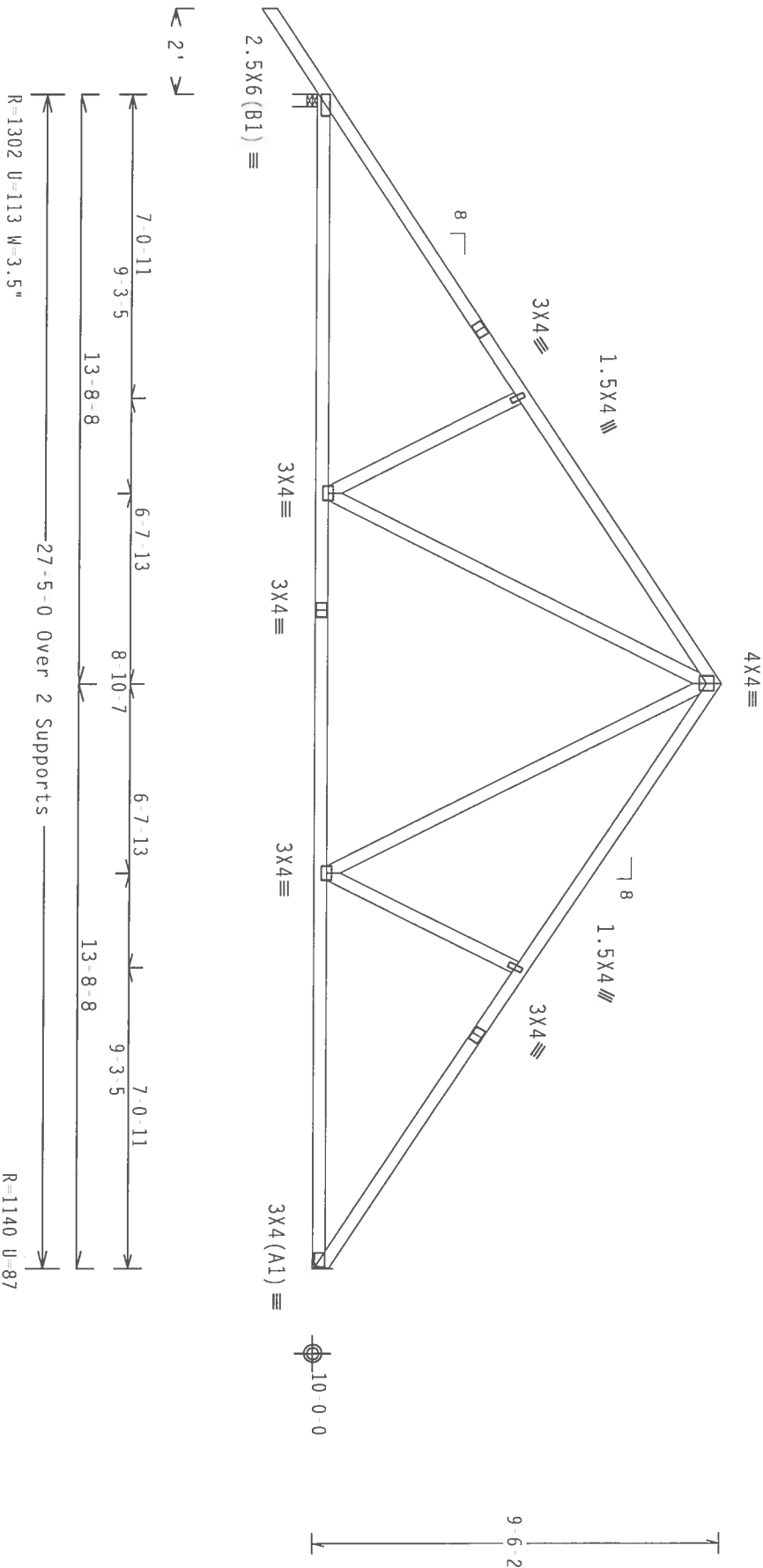
El coefficiente de variación es el

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt., ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ $GCP1(+/-)=0.18$

Wind reactions based on MMFRS pressures.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

7.36.0424.12

QTY:1 FL/4/-/-/R/-

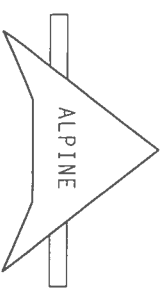
Scale = .25"/ft.

****WARNING**** TRUSSES, BEAMS, BRIDGES, EXTERIOR CEILING, HANDLING, SHIPMENT, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI, INC., 11100 W. 11TH AVE., SUITE 312, ALEXANDRIA, VA, 22304 AND WEA GOOD TRUSS COUNCIL OF AMERICA, 6800 ENTERPRISE LANE, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED FOR GOOD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPMENT, INSTALLING & BRACING OF TRUSSES.

DESIGN CONDITIONS WITH APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC., BY AISC) AND TPI. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPMENT, INSTALLING & BRACING OF TRUSSES.

CONSTRUCTION PLANS ARE MADE OF 20/10/10GA (W/1/5/5) ASH 6053 GRADE 40/60 (W/1/5/5) GALV STEEL. APPLY A SEAL ON THIS DRAWING INDICATES ACCEPTABLE AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



ITW Building Components Group, Inc.
Haines City, FL 33844
Phone: 888.222.2222
Fax: 888.222.2222

TC LL	20.0 PSF	REF R8228-40813
TC DL	10.0 PSF	DATE 09/24/07
BC DL	10.0 PSF	DRW HCUR8228 07267017
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 49169

DUR.FAC.	1.25	FROM AH
SPACING	24.0"	URFF- 1TB1R22R201

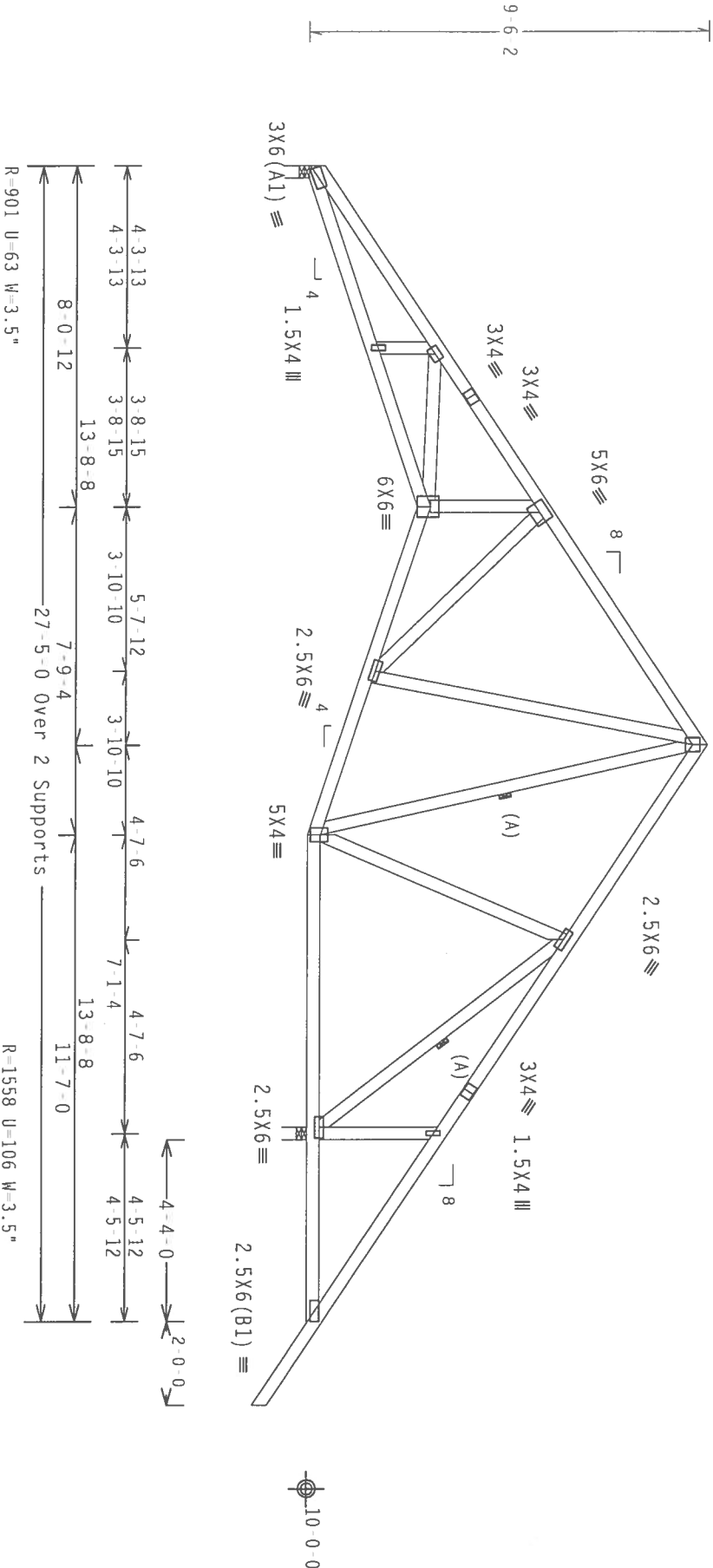
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

(A) Continuous lateral bracing equally spaced on member.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCPI(+/-)=0.18

Wind reactions based on MWFRS pressures.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

7.36.0424.12

QTY:1 FL/4/-/-/R/-

Scale = .25"/ft.

****WARNING**** TRUSSES REQUIRE EXTENSIVE CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST AVAILABLE MANUFACTURER'S INSTRUCTIONS AND SPECIFICATIONS FOR ALL MATERIALS. THE TRUSS COMPONENTS ARE NOT TO BE USED FOR ANY OTHER PURPOSES. THE TRUSS COMPONENTS ARE NOT TO BE USED FOR ANY OTHER PURPOSES. THE TRUSS COMPONENTS ARE NOT TO BE USED FOR ANY OTHER PURPOSES.

****IMPORTANT**** TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR.

ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
Tel: 888.444.4444

TC LL	20.0 PSF	REF	R8228 - 40814
TC DL	10.0 PSF	DATE	09/24/07
BC DL	10.0 PSF	DRW	HCUSR8228 07267018
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	49195
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JRFF-	1TB18228201

Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.

Wind reactions based on MWFRS pressures.



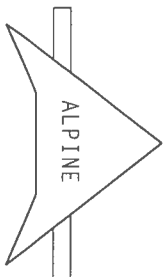
Design Crit: TPI-2002(STD)/FBC

$$Cq/RT=1.00(1.25)/0(0)$$

7.36.0424.12

QTY:1 FL/4/-/-/-/R/-/

Scale = .25"/Ft.



ITW Building Components Group, Inc.
Haines City, FL 33844
Telephone: 800/368-7222

[illegible]

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. IBM BCG, INC. SHALL NOT

IP1; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

PLATES TO EACH FACE OF BRASS AND INMITS OTHERWIST LOCATED ON THIS DESIGN POSITION PER DRAWINGS 1604-1-1

DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT

BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

TC LL	20.0 PSF	REF	R8228- 40815
TC DL	10.0 PSF	DATE	09/24/07
BC DL	10.0 PSF	DRW	HCUSR8228 07267016
BC LL	0.0 PSF	HC-ENG DF/DF	*
TOT.LD.	40.0 PSF	SEQN-	49189
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JRFF-	1TB1R22RZ01

its)

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Nailing Schedule: (12d Common (0.148"x3.25",_min)._nails)
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs      : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails
in each row to avoid splitting.
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Use equal spacing between rows and stagger nails in each row to avoid splitting.

Calculated horizontal deflection is 0.10" due to live load and 0.19" due to dead load.

4X12(R) III collar tie braced with continuous lateral bracing at 24" OC.

Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.



REF	R8228	40816
DATE	09/24/07	

Dillon et al. • Cerebellar Atrophy in Schizophrenia

וְכִי יִשְׁמַע ה' / וְכִי יִשְׁמַע ה'

SEON - 49202

FROM AH

1055 1TB1032

Top chord 2x4 SP #2 Dense : T3, T4 2x8 SP SS:
Bot chord 2x8 SP SS : B3 2x4 SP #2 Dense:
Webs 2x4 SP #3

Collar-tie braced with continuous lateral bracing at 24" OC. on rigid ceiling.

+ MEMBER TO BE Laterally Braced For Horizontal Wind Loads.
Bracing System To Be Designed And Furnished By Others.

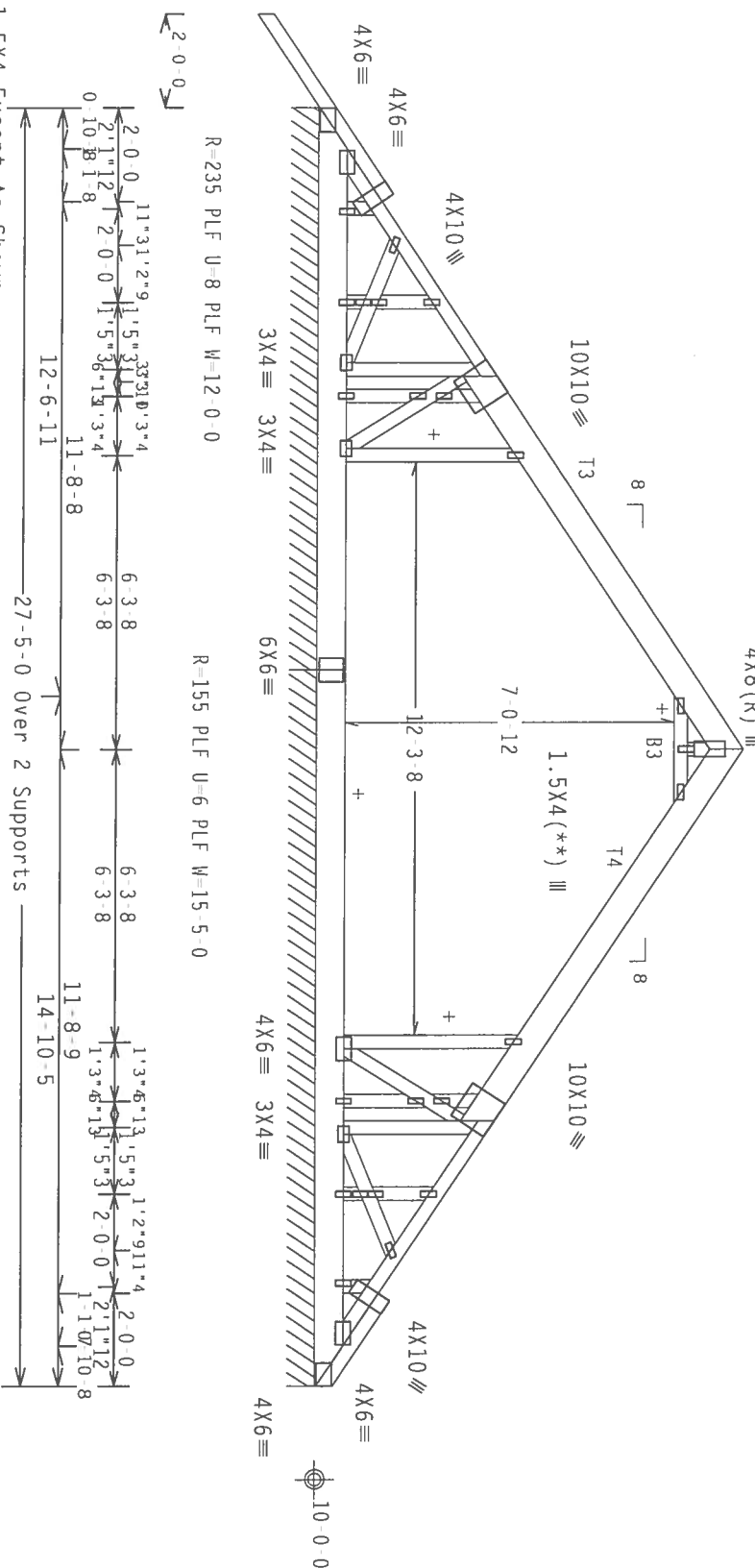
See DWGS A11015EE0207 & GBLLETIN0207 for more requirements.

Wind reactions based on MFRS pressures.

Truss spaced at 24.0" OC designed to support 2-0.0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.

The building designer is responsible for the design of the roof and ceiling diaphragms, gable end shear walls, and supporting shear walls. Shear walls must provide continuous lateral restraint to the gable end. All connections to be designed by the building designer.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)

 $Cq/RT=1.00(1.25)/0(0)$

QTY:1 FL/4/-/-/-/R/-

Scale = .25" / Ft.

WARNING: THESE BENDING EXPERIENCE CAN INFLUENCE HANDLING, SHIPPING, INSTALLING AND BRACING REFER TO DESIGN (INCLUDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE (FIRMS) SAFETY INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA 22314, AND WICKA WOOD TRUSS COMPANY OF AMERICA, 6300 GLENNDALE DRIVE, HANOVER, VA 52719 FOR SAFETY PRACTICES RELATIVE TO REPAIRING THESE STRUCTURES. UNDESIRABLE OUTSTANDING INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERTY ATTACHED TOP CHORD CELLING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT

TP1: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

CONNECTOR PLATES ARE MADE OF 20/18/16GA (W, H/SS/K) ASIM A653 GRADE 40/60 (W, K/H,SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF BRUSH AND BRUSSES ATTACHED LOCATED ON THE DESIGN POSITION PER DRAWINGS AND

DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SEAL FOR THE THIS COMPONENT AND INSPECTION OF PLATE FOLLOWED BY (1) SHALL BE PER ANNEA 3 OF IP11 2002 SEC.3. A SEAL ON THE

BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ITW Building Components Group, Inc.
Haines City, FL 33844
Tel: 800-451-7121

TC LL	20.0 PSF	REF	R8228 - 40817
TC DL	10.0 PSF	DATE	09/24/07
BC DL	10.0 PSF	DRW	HCUSR8228 07267001
BC LL	0.0 PSF	HC-ENG	SSB/AP
TOT.LD.	40.0 PSF	SEQN-	51256
DUR.FAC.	1.25	FROM	AH
SPACING	SFF ABOVE	JRFF-	1TB18278Z01

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Truss spaced at 24.0" OC designed to support 2-0-0 top chord
outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must
not be cut or notched.

See DWGS A11015EE0207 & GBULLETIN0207 for more requirements.

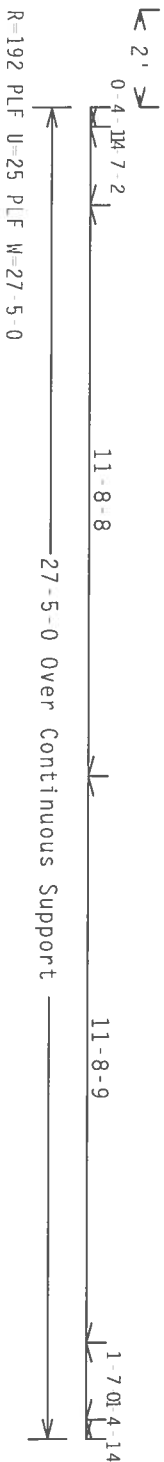
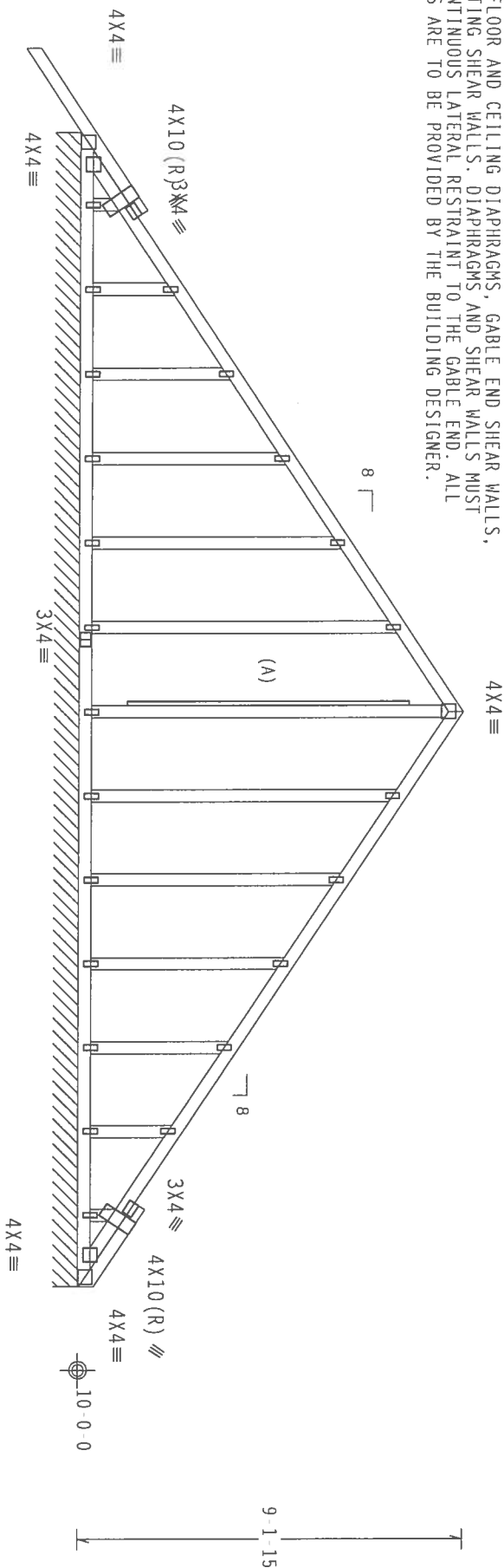
Deflection meets L/240 live and L/180 total load. Creep increase
factor for dead load is 1.50.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF
THE ROOF, FLOOR AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS,
AND SUPPORTING SHEAR WALLS. DIAPHRAGMS AND SHEAR WALLS MUST
PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL
CONNECTIONS ARE TO BE PROVIDED BY THE BUILDING DESIGNER.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located
anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0
psf. $I_w=1.00$ $G_{CPI}(+/-)=0.18$

Wind reactions based on MWFRS pressures.

(A) 1x4 #3 or better "L" brace, 80% length of web member. Attach
with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" OC.



Note: All Plates Are 1.5X4 Except As Shown.
Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

7.36.0424.12

QTY:1 FL/4/-/-/R/-

Scale = .25"/ft.

****WARNING**** TRUSSES SHOWN FOR THE PURPOSE OF ILLUSTRATION. THE FABRICATOR, MANUFACTURER, SHIPPER, INSTALLER AND BRACER, SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE FABRICATOR, MANUFACTURER, SHIPPER, INSTALLER AND BRACER. THE FABRICATOR, MANUFACTURER, SHIPPER, INSTALLER AND BRACER SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
www.alpinebuilding.com

TC LL	20.0 PSF	REF	R8228-40818
TC DL	10.0 PSF	DATE	09/24/07
BC DL	10.0 PSF	DRW	HCSR8228 07267013
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT. LD.	40.0 PSF	SEQN	49255 REV
DUR. FAC.	1.25	FROM	AH
SPACING	SFF ABOVE	JRFF	1TB1R2R201

Top chord 2x4 SP #2 Dense ; T2, T3 2x8 SP SS;
Bot chord 2x8 SP SS ; B3 2x4 SP #2 Dense;
Webs 2x4 SP #3

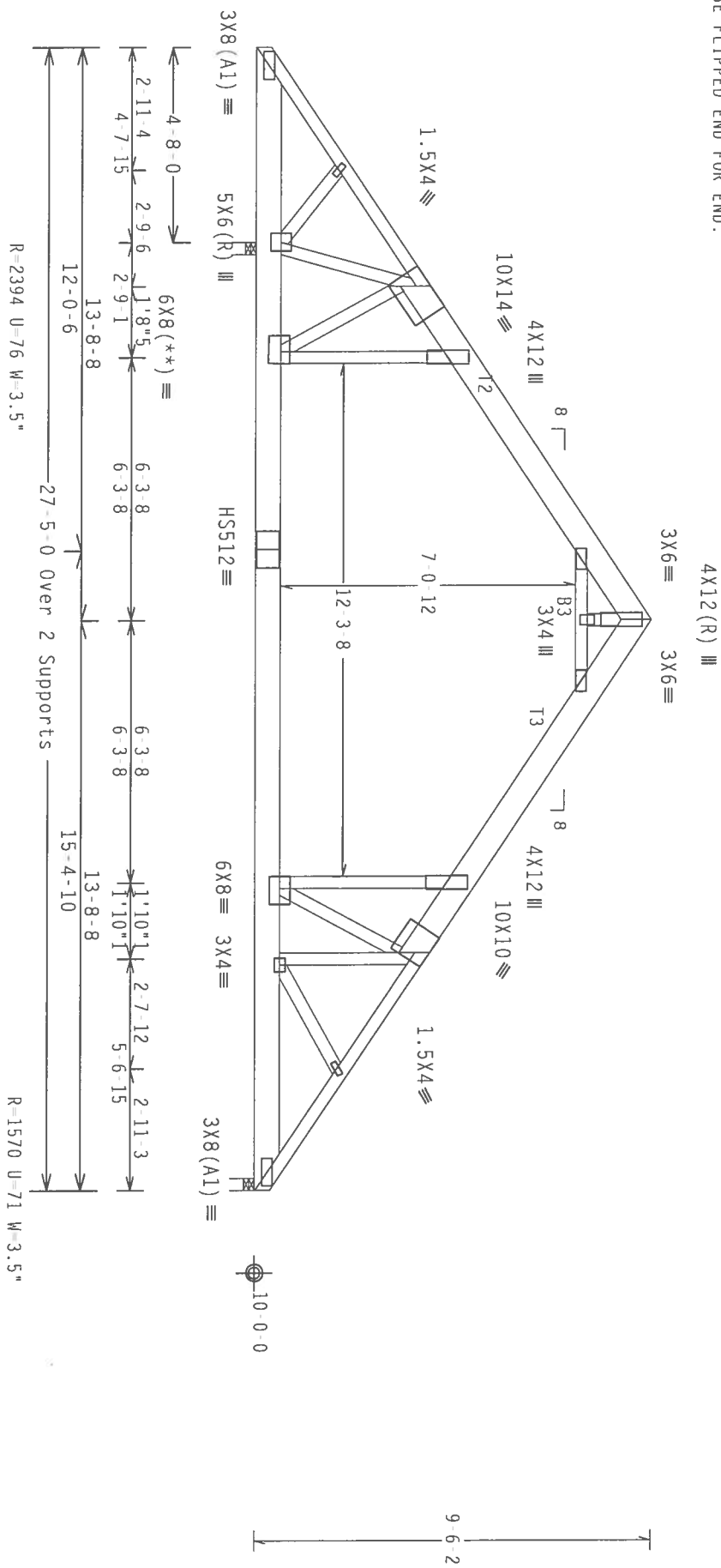
Calculated horizontal deflection is 0.17" due to live load and 0.31" due to dead load.

Collar tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

NOTE: THIS TRUSS MUST BE INSTALLED AS SHOWN. IT CAN NOT BE FLIPPED END FOR END.

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ $G_{cpi}(+/-)=0.18$
Wind reactions based on MWFRS pressures.
BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 7-6-12 to 19-10-4.



PLT TYP. 20 Gauge HS,Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

7.36.0424.12

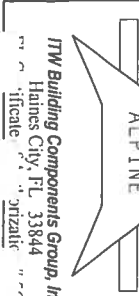
QTY:1 FL/4/-/-/R/-

Scale = .25"/Ft.

WARNING TRUSSES REMOVED EXISTING GABLE FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO DCSI BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI TRUSS PLANT DIRECTOR, 2100 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304 AND WICA (WOOD TRUSS) COUNCIL OF AMERICA, UNLESS OTHERWISE INDICATED. FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC (1989) DESIGN SPEC., BY AREA) AND TPI. ITW BCG CONDUCTOR PLATES ARE MADE OF 70/30 ZINC ALLOY (ALUMINUM) WITH A MINIMUM TENSILE STRENGTH OF 60,000 PSI. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 100A-2, 100B-2, 100C-2, 100D-2, 100E-2, 100F-2, 100G-2, 100H-2, 100I-2, 100J-2, 100K-2, 100L-2, 100M-2, 100N-2, 100O-2, 100P-2, 100Q-2, 100R-2, 100S-2, 100T-2, 100U-2, 100V-2, 100W-2, 100X-2, 100Y-2, 100Z-2. A SEAL OR THIS DRAWING SHALL BE AFFIXED TO THE TRUSS COMPONENT. THE SEAL SHALL BE AFFIXED TO THE TRUSS COMPONENT IN A MANNER THAT THE SEAL IS NOT REMOVED WITHOUT THE TRUSS BEING DESTROYED. THE SEAL SHALL BE AFFIXED TO THE TRUSS COMPONENT IN A MANNER THAT THE SEAL IS NOT REMOVED WITHOUT THE TRUSS BEING DESTROYED.



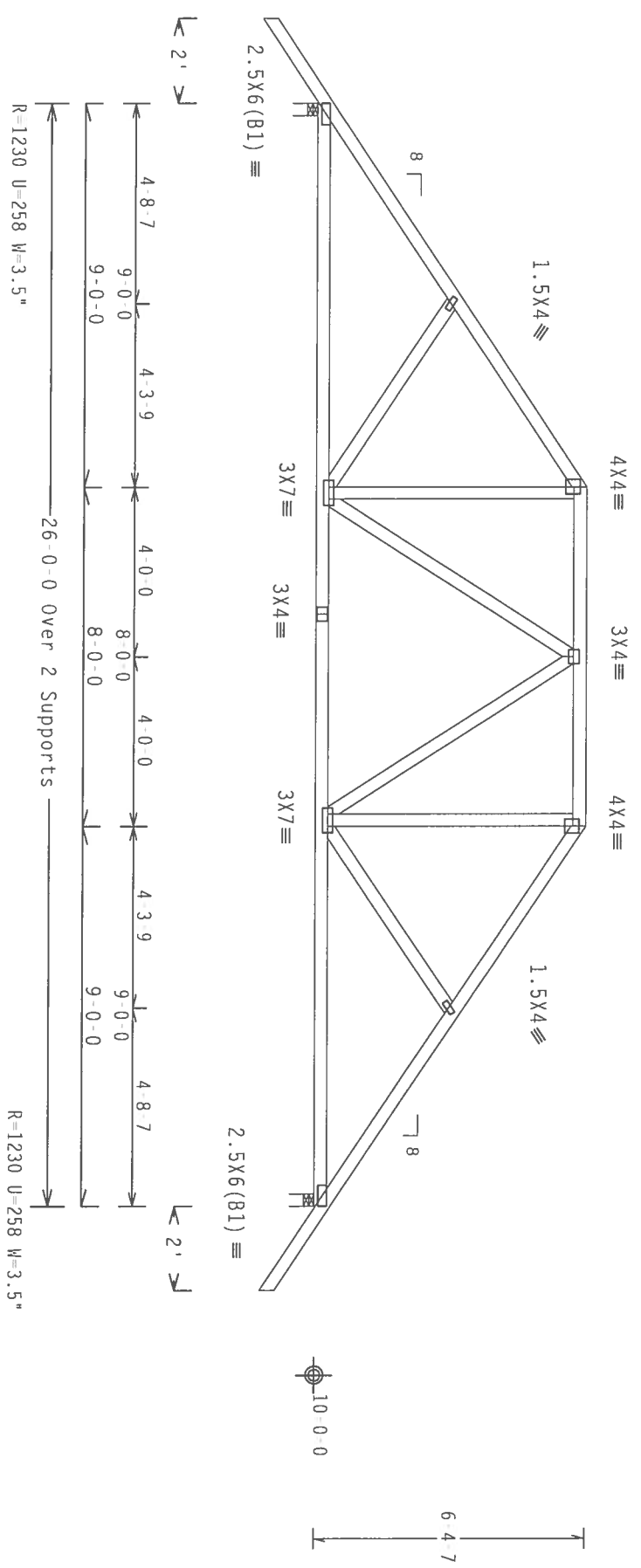
ITW Building Components Group, Inc.
Haines City, FL 33844
Telephone: 888.222.2222
Fax: 888.222.2222
Website: www.itwbc.com

TC LL	20.0 PSF	REF	R8228 - 40819
TC DL	10.0 PSF	DATE	09/24/07
BC DL	10.0 PSF	DRW	HCUSR8228 07267014
BC LL	0.0 PSF	HC-ENG DF/DF	
TOT.LD.	40.0 PSF	SEQN-	49270
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JRFF-	1TB18228Z01

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC

Cq/RT=1.00(1.25)/0(0)

7.36.0424.12

QTY:1

FL/4/-/-/R/-

Scale = .25"/ft.

****WARNING**** TRUSSES REQUIRE EXTERIOR CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO DCSE (CONCRETE COMPONENT) SAFETY INFORMATION, PUBLISHED BY TPI (TRUSS PLANT INSTITUTE, 6300 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6000 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI, OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF BCS (NATIONAL DESIGN SPEC. BY AIA/PA) AND TPI. THE BCG PLANTS IN ALABAMA, ARIZONA, CALIFORNIA, COLORADO, FLORIDA, GEORGIA, ILLINOIS, INDIANA, KANSAS, MISSISSIPPI, MISSOURI, MONTANA, NEBRASKA, NEVADA, NEW HAMPSHIRE, NEW JERSEY, NEW YORK, NORTH CAROLINA, NORTH DAKOTA, OHIO, OKLAHOMA, PENNSYLVANIA, RHODE ISLAND, SOUTH CAROLINA, SOUTH DAKOTA, TEXAS, UTAH, VERMONT, VIRGINIA, WISCONSIN, WYOMING. ANY INSPECTION OF TRUSSES AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION FOR DRAWINGS T604.2. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEER'S RESPONSIBILITY FOR THE DESIGN. THE DESIGN SHOWN THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ITW Building Components Group, Inc.
Haines City, FL 33844
(800) 451-1111 ext. 222

ALPINE
ITW Building Components Group, Inc.
Haines City, FL 33844
(800) 451-1111 ext. 222

TC LL	20.0 PSF	REF	R8228-40821
TC DL	10.0 PSF	DATE	09/24/07
BC DL	10.0 PSF	DRW	HCUSR8228 07267011
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT. LD.	40.0 PSF	SEQN-	49163
DUR. FAC.	1.25	FROM	AH
SPACING	24.0"	JRFF-	1TB18728Z01

Top chord 2x4 SP #2 Dense :T2, T3 2x8 SP SS:
Bot chord 2x8 SP SS :B2 2x4 SP #2 Dense:
Webs 2x4 SP #3

End verticals not exposed to wind pressure.

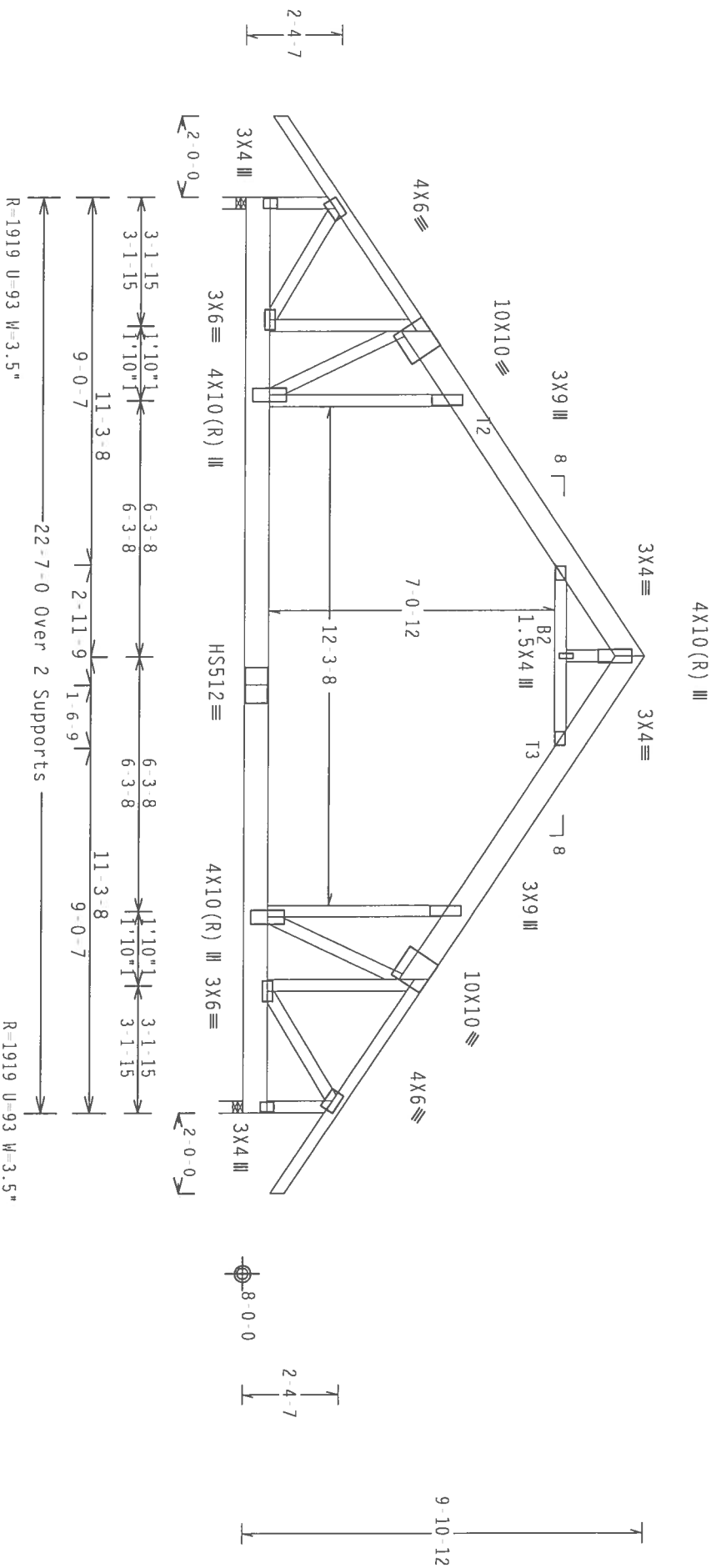
Collar tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL-5.0 psf, wind BC DL-5.0 psf. $I_w=1.00$ $G_{cpl}(+/-)=0.18$

Wind reactions based on MMFRS pressures.

BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 5-1-12 to 17-5-4.



PLT TYP. 20 Gauge HS.Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

7.36.0424.12

QTY:1 FL/4/-/-/R/-

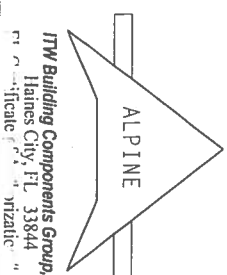
Scale = .25"/ft.

****WARNING**** TRUSSES REQUIRE EXTERIOR CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO DECS CONSULTING COMPONENT SAFETY INFORMATION) PUBLISHED BY THE TRUSS ASSOCIATION, 600 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA 22314 AND AISC, "GOOD PRACTICES FOR TRUSS CONSTRUCTION", UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING A BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF THIS QUALITY DESIGN SPEC. BY ACPA AND TPI. THE BCG CONNECTION PLATES ARE MADE OF 20/18/16GA (W/H/S/S) ASH A653 GRADE 40/50 (K/PL/SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS TIGA-2.

DESIGN OF TRUSSES FOLLOWED BY TPI SHALL BE PER AISC 301 AND TPI 2002 SEC.3. A SEAL ON THIS DRAWING INDICATES THE SUITABILITY AND USE OF THIS CONNECTION FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AISC/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228-40822
TC DL	10.0 PSF	DATE	09/24/07
BC DL	10.0 PSF	DRW	HCUSR8228 07267005
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN	49056
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JRFF	1TB1R22RZ01

Nailing Schedule: (12d Common (0.148"x3.25", min.)_nails)

Use equal spacing between rows and stagger nails in each row to avoid splitting.

End verticals not exposed to wind pressure.

Trusses to be spaced at 48.0" OC maximum.

BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 5.1.12 to 17.5.4.

5-1-12 to 17-5-4.



REF	R8228 - 40823
DATE	09/24/07

DRW HCUSR8228 07267015

HC-ENG DF/DF

SEQN - 49061

FROM AH

ALL	LOW
1	1
2	2
3	3
4	4
5	5
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100	100

JKH- LIB18228201

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT 11, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCPI (+/-)=0.18

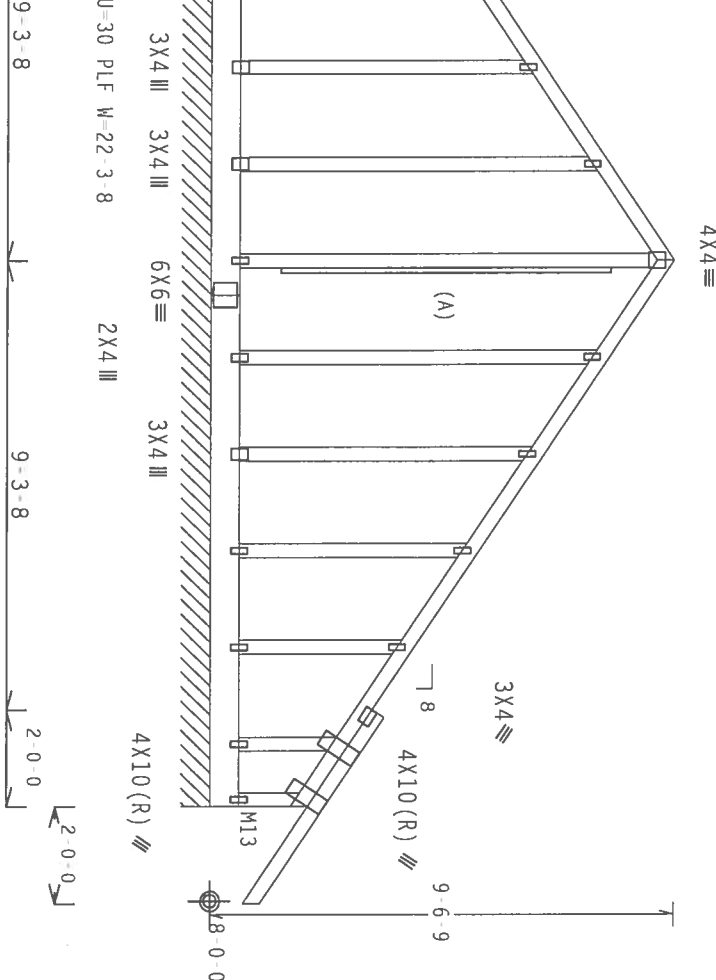
End verticals not exposed to wind pressure.

Truss spaced at 48.0" OC designed to support 2-0-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

(A) 1x4 #3 or better "L" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" OC.

In lieu of structural panels use purlins to brace TC @ 24" OC.

Trusses to be spaced at 48.0" OC maximum.

 $\Delta \chi^2 = 1$ 

22-7-0 Over 2 Supports

 $Cq/RT=1.00(1.25)$

7.36.0424.12 QTY:1

QTY:1 FL/4/-/-/-/R/

Scale = .25"/Ft.

BRACING
SUITE, 218
A., 6300
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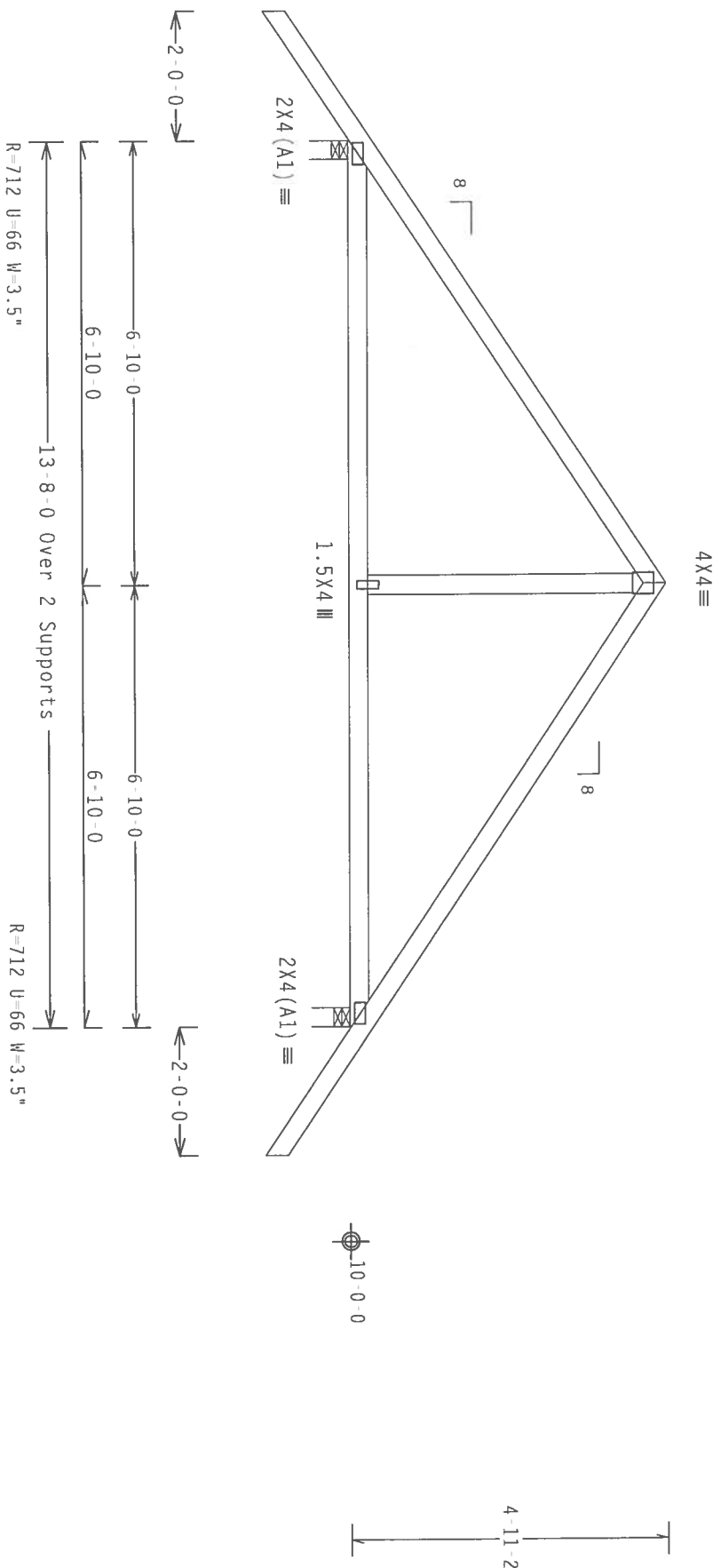
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TC DL	10.0 PSF	DATE	09/24/07
BC DL	10.0 PSF	DRW	HCSUR8228 07267007
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEON-	49129
DUR.FAC.	1.25	FROM	AH
SPACING	SFF ABOVE	JRFF-	1TB18228Z01

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ $GCP1(+/-)=0.18$

Wind reactions based on MWFRS pressures.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
 $Cq/RT=1.00(1.25)/0(0)$

7.36.0424.12

QTY:1 FL/4/-/-/-/R/-

Scale = .375"/Ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. BEFORE BEING USED, THE TRUSS MUST BE INSPECTED FOR ANY DEFECTS OR DAMAGE. THE TRUSS MUST BE PROTECTED FROM EXCESSIVE MOISTURE, CORROSION, AND OTHER FACTORS THAT MAY WEAKEN THE TRUSS. THE TRUSS MUST BE USED IN ACCORDANCE WITH THE DESIGN SPECIFICATIONS AND THE MANUFACTURER'S INSTRUCTIONS. THE TRUSS MUST BE USED IN ACCORDANCE WITH THE DESIGN SPECIFICATIONS AND THE MANUFACTURER'S INSTRUCTIONS. THE TRUSS MUST BE USED IN ACCORDANCE WITH THE DESIGN SPECIFICATIONS AND THE MANUFACTURER'S INSTRUCTIONS.

ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
www.alpinebuilding.com

TC LL	20.0 PSF	REF	R8228-40825
TC DL	10.0 PSF	DATE	09/24/07
BC DL	10.0 PSF	DRW	HCUSR8228 07267002
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	49103
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JRFF-	1TB1R22R201

Top Chord 2x4 SP #2 Dense
Bot Chord 2x4 SP #2 Dense
Webs 2x4 SP #3

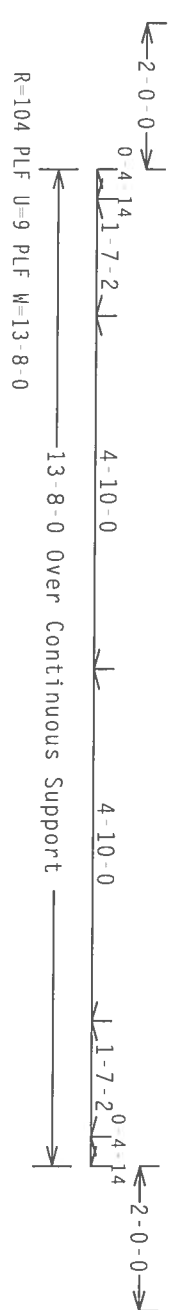
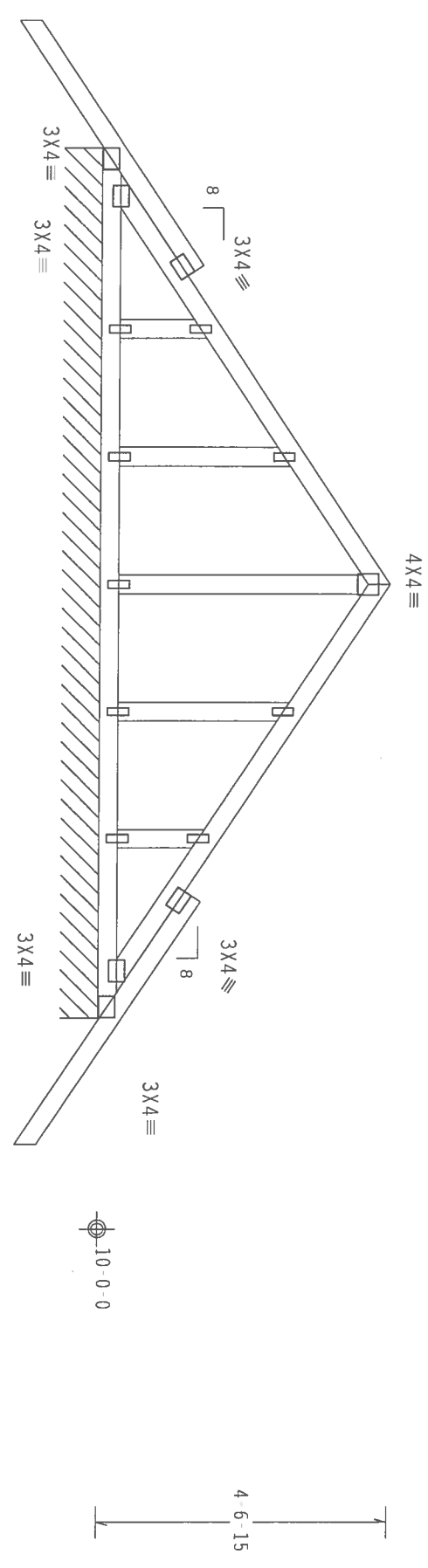
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located
anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0
psf, $I_w=1.00$ $G_{CPI}(\text{r})=0.18$

See DWGS A11015EE0207 & GBLLETIN0207 for more requirements.

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF, FLOOR AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS. DIAPHRAGMS AND SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS ARE TO BE PROVIDED BY THE BUILDING DESIGNER.



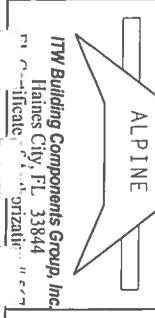
Note: All Plates Are 1.5X4 Except As Shown.

PLT TYP. Wave Design Crit: TPI 2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0) 7.36.0424.12

QTY:1 FL/4/-/-/R/-

Scale = .375"/Ft.

****WARNING**** BRUSSES, RIGID, EXTERIOR, CASE, IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING, REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY THE MANUFACTURER, AND THE BUILDING DESIGNER. THE BUILDING DESIGNER SHALL BE RESPONSIBLE FOR THE DESIGN OF THE TRUSS SYSTEM, INCLUDING THE DESIGN OF THE TRUSS MEMBERS, JOINTS, AND CONNECTIONS. THE BUILDING DESIGNER SHALL BE RESPONSIBLE FOR THE DESIGN OF THE TRUSS SYSTEM, INCLUDING THE DESIGN OF THE TRUSS MEMBERS, JOINTS, AND CONNECTIONS. THE BUILDING DESIGNER SHALL BE RESPONSIBLE FOR THE DESIGN OF THE TRUSS SYSTEM, INCLUDING THE DESIGN OF THE TRUSS MEMBERS, JOINTS, AND CONNECTIONS.



****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR.

TC LL	20.0 PSF	REF	R8228 - 40826
TC DL	10.0 PSF	DATE	09/24/07
BC DL	10.0 PSF	DRW	HCUSR8228 07267025
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	49118 REV
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JRFF-	1TB1R228Z01

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART. ENC. bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCPI(+/-)=0.55

Wind reactions based on MIFRS pressures.

 $Cq/RT=1.00(1.25)/0(0)$

QTY:1

Scale = .375"/Ft.

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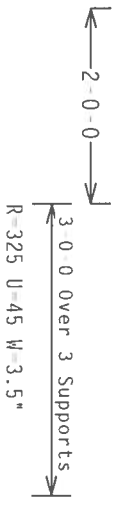
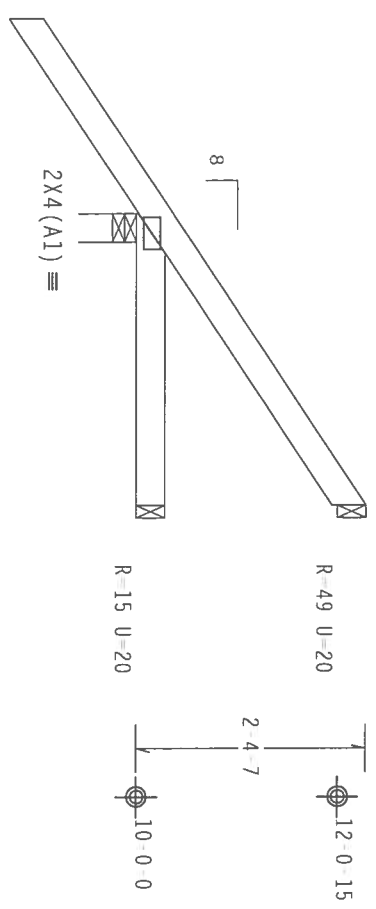
TC LL	20.0 PSF	REF	R8228- 40829
TC DL	10.0 PSF	DATE	09/24/07
BC DL	10.0 PSF	DRW	HCU8R8228 07267020
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	49153
DUR.FAC.	1.25	FROM	AH
SPACING	SFF ABOVE	DRFF-	1TB1822RZ01

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART. ENC. bldg. located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ $G_{CPI}(+/-)=0.55$

Wind reactions based on MWFRS pressures.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

7.36.0424.12

QTY:1 FL/4/-/-/R/-

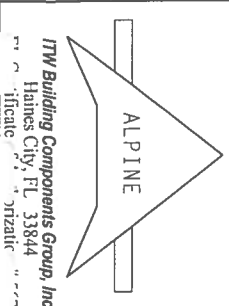
Scale = .5"/ft.

TC LL		20.0 PSF	REF	R8228-40830
TC DL		10.0 PSF	DATE	09/24/07
BC DL		10.0 PSF	DRW	HCSR8228 07267022
BC LL		0.0 PSF	HC-ENG	DF/DF
TOT.LD.		40.0 PSF	SEQN-	49138
DUR.FAC.		1.25	FROM	AH
SPACING		24.0"	DRFF-	1TB1R22R201

****WARNING**** BRUSSES REQUIRE EXTERNAL FORCE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST PRACTICES (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 100, ALEXANDRIA, VA, 22304) AND STEEL (STEEL TRUSS CONDUCT OF AMERICA, 6300 WILSON AVENUE, SUITE 100, CHICAGO, IL 60631) FOR ADDITIONAL INFORMATION. BRUSSES SHALL BE PROPERLY ATTACHED TO THE TRUSS CHORDS AND BOTTOM CHORDS. BRUSSES SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORDS SHALL HAVE A PROPERLY ATTACHED RIGID CELLING.

****IMPORTANT**** THROUGHOUT A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR, THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF BRUSSES.

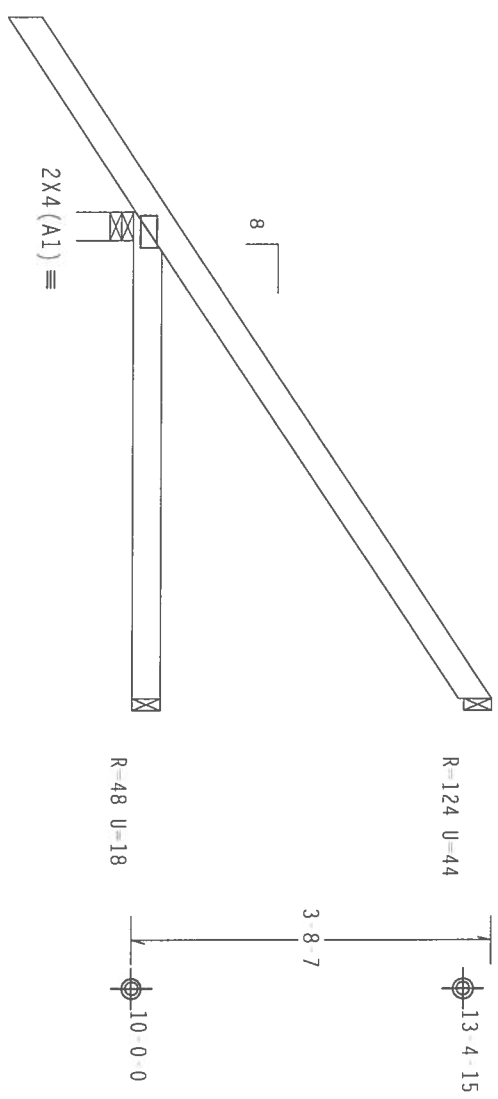
DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC. BY AISC) AND TPI. THE BCG CONSTRUCTION PLATES ARE MADE OF 20/18/16GA (GALV/SS/K) ASTM A653 GRADE 40/60 (N. K/H/SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI 2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT BUILDING DESIGNER. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART. ENC. bldg, not located within 4.50 ft from roof edge, CAT 11, EXP 8, wind TC DL=5.0 psf, wind BC DL=5.0 psf. 1w=1.00 GCPI(+/-)=0.55
Wind reactions based on MFRS pressures.



2'-0'-0" →
→ 5'-0'-0" Over 3 Supports →
R=387 U=47 W=3.5"

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

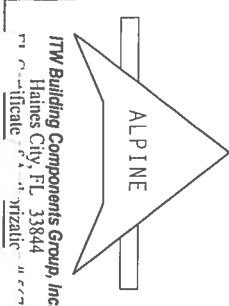
7.36.0424.12

QTY:1

FL/4/-/-/R/-

Scale = .5"/ft.

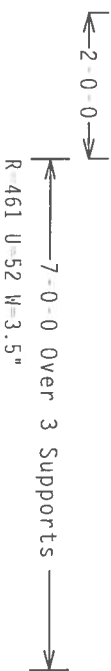
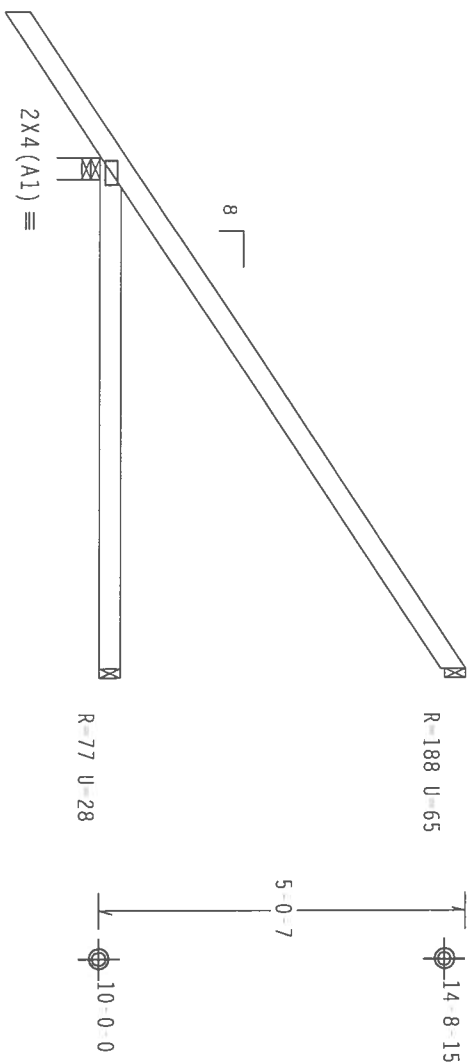
WARNING BRUSSES REMOVED EXTERIOR GABLE END FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST (BUILDING COMPONENT SAFETY INFORMATION) PUBLISHED BY TPI BRUSSES INSTALL INSTRUCTIONS TO HONOLULU STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD BRUSSES CONNECT OR AMERICAN CORD ENTERPRISE, LAKE MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.		**IMPORTANT** TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE BRUSSES IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF BRUSSES. DESIGN COMPLIANCE WITH APPLICABLE PROVISIONS OF 2005 NATIONAL DESIGN SPEC. BY AIA/PA AND TPI. ITW BCG CONSTRUCTION PLANS ARE MADE OF 20/10/16GA (40/55/PS) ASH/60S GRADE 40/60 (4. K/11.55) GALV. STEEL. APPLY ANY INSPECTION OF ALL BRUSSES AND BRUSSES OTHERWISE LOCATED ON THIS DESIGN. POSITION PER DRAWINGS 160A Z. A SPECIAL INSPECTION OF ALL BRUSSES AND BRUSSES SHALL BE REQUIRED. THE DESIGNER SHALL BE RESPONSIBLE FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.	
TC LL	20.0 PSF	REF	R8228-40831
TC DL	10.0 PSF	DATE	09/24/07
BC DL	10.0 PSF	DRW	HCUSR8228 07267021
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT. LD.	40.0 PSF	SEQN-	49142
DUR. FAC.	1.25	FROM	AH
SPACING	24.0"	JRFF-	1TB1R22RZ01



Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART. ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, lw=1.00 gcpl(+/-)=0.55
Wind reactions based on MWFRS pressures.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

7.36.0424.12

QTY:1 FL/4/-/-/R/-

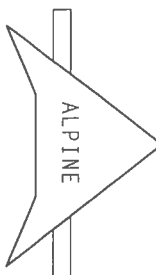
Scale =.375"/Ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPANIES' SAFETY INFORMATION, PUBLISHED BY TPI TRUSS MANUFACTURING, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304 AND WEA (WOOD TRUSS) COUNCIL OF AMERICA. GOOD INTERPRETATION OF THE DESIGN SHALL BE THE RESPONSIBILITY OF THE ENGINEER. THE ENGINEER SHALL BE RESPONSIBLE FOR THE DESIGN OF THE TRUSS AND THE BUILDING DESIGN SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED FIELD CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN OR FABRICATING, HANDLING, SHIPPING, INSTALLING A BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AIA (NATIONAL DESIGN SPEC.), BY AREA) AND TPI. THE BCG CONSTRUCTION PLANS ARE MADE OF 20/10/10 (40/55/5) ASH 4053 GRADE 40/60 (W, K/H/55) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE NOTED OR THIS DESIGN, POSITION PER DRAWINGS 100A-E.

INDICATION OF PLATE LOCATION AND SIZE SHALL BE PER AREA) AS OF TPI 2002 SEC. 3. A SEAL ON THIS DRAWING SHALL BE THE RESPONSIBILITY OF THE ENGINEER. THE ENGINEER SHALL BE RESPONSIBLE FOR THE DESIGN OF THE TRUSS AND THE BUILDING DESIGN SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED FIELD CEILING.



ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
Tel: 888-333-3333
Fax: 888-333-3333

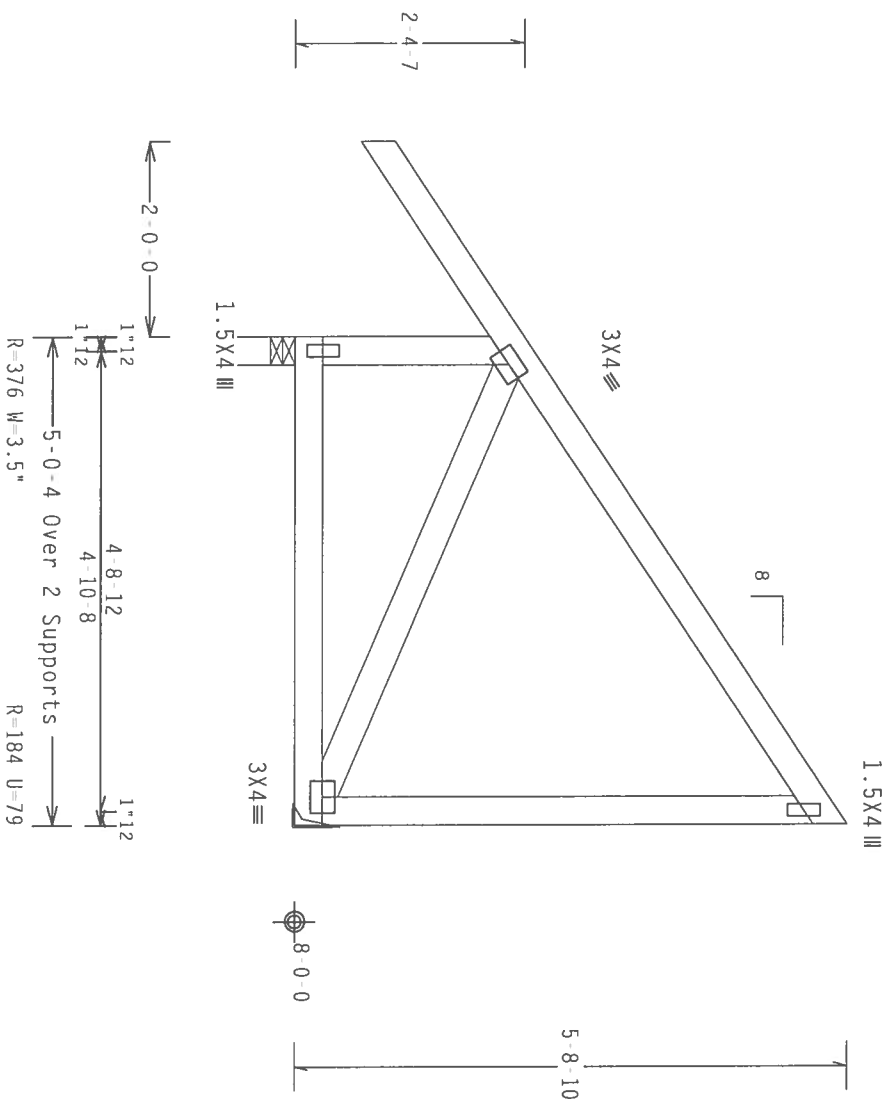
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TC DL	10.0 PSF	DATE	09/24/07
BC DL	10.0 PSF	DRW	HCUSR8228 07267024
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN	49146
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JRFF	1TB18728Z01

Top	chord	2x4	SP	#2	Dense
Bot	chord	2x4	SP	#2	Dense
	webs	2x4	SP	#3	

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 Gcpi (+/-)=0.18

Wind reactions based on MWFRS pressures.

Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

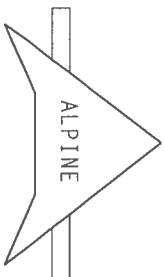
Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)

$$Cq/RT=1.00(1.25)/0(0)$$

7.36.0424.12

QTY:1 FL/4/-/-/-/R/-

Scale = .5"/Ft.



ITW Building Components Group, Inc.
Haines City, FL 33844

WARNING: PRIOR TO CHILDREN'S ENTRY CARE IN PARTICIPATION, HANDLING, SHIPPING, INSTALLING AND BRACING, REFER TO BEST AVAILABLE COMPROMISE SAFETY INFORMATION. PUBLISHED BY THE (FROSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND MICA (GOOD TRUSS COMPANY OF AMERICA, 65000 UNIVERSITY LANE, SUITE 500, W-53179) FOR SAFETY PRACTICES, PRIOR TO GOOD TRUSS FUNCTIONS. UNLESS OTHERWISE INDICATED, NO CHILD SHALL HAVE PROPERLY ATTACHED STRAIGHTENAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED MID CHORD.

***IMPORTANT** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT

IPJ; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

PLATES TO EACH FACE OF TRUSS AND: WELDS QUITRUST LOCATED ON THIS DESIGN. POSITION PER DRAWINGS 160A-2. CONNECTION PLATES ARE MADE OF 20/18/15GA (W.H/SS/K) WITH A653 GRADE 40/60 (W. K/H.SS) GALV. STEEL. APPLY

DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT

BUILDING DESIGNER PER ANSI/ISO 1 SEC. 2.

TC LL	20.0 PSF	REF	R8228- 40834
TC DL	10.0 PSF	DATE	09/24/07
BC DL	10.0 PSF	DRW	HCUSR8228 07267009
BC LL	0.0 PSF	HC-ENG DF/DF	*
TOT.LD.	40.0 PSF	SEQN-	49095
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JRFF-	1TB18229Z01

Top chord 2x4 SP #2 Dense
Bot chord 2x8 SP #1 Dense
Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7 02, CLOSED bldg, located
anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0
psf. $I_w=1.00$ $G C p i (+/-)=0.18$

Wind reactions based on MMFRS pressures.

Truss must be installed as shown with top chord up.

SPECIAL LOADS

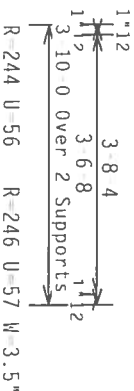
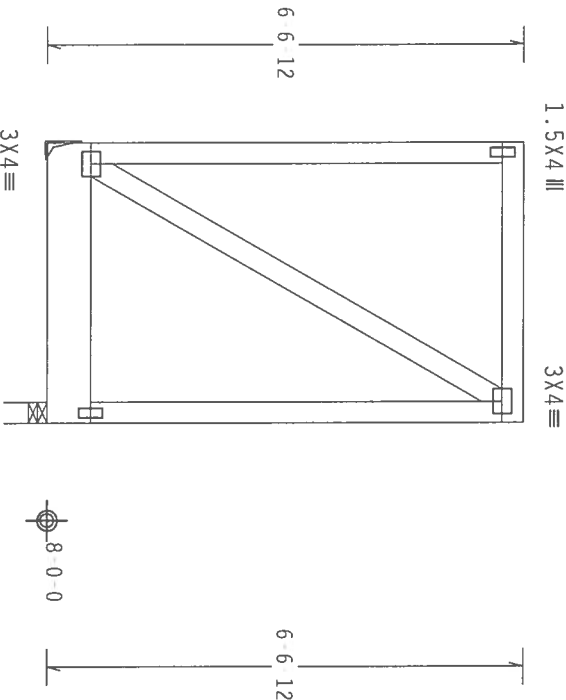
----- (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 60 PLF at 0.00 to 60 PLF at 3.83
BC - From 20 PLF at 0.00 to 20 PLF at 3.83
BC - 184 LB Conc. Load at 1.94

End verticals not exposed to wind pressure.

Deflection meets L/240 live and L/180 total load. Creep increase
factor for dead load is 1.50.

The TC of this truss shall be braced with attached spans at 24" OC
in lieu of structural sheathing.

THE BUILDING DESIGNER SHALL EVALUATE AND APPROVE LOAD MAGNITUDES
AND LOCATIONS AS SHOWN ("SPECIAL LOADS"). TRUSS ENGINEER &
FABRICATOR ARE NOT RESPONSIBLE FOR LOAD MAGNITUDES AND LOCATIONS.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

7.36.0424.12

QTY:1

FL/4/-/-/R/-

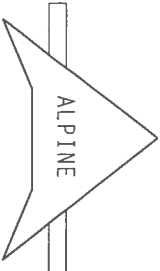
Scale = .375"/ft.

****WARNING**** TRUSSES BEARING EXTERIOR LOADS IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING.
AFTER THESE TRUSSES ARE DELIVERED TO THE PROJECT, THEY MUST BE PROTECTED BY THE CONTRACTOR FROM ALL
HAIL, ICE, SNOW, RAIN, WIND, AND OTHER WEATHER CONDITIONS. THE TRUSS CONTRACTOR SHALL BE RESPONSIBLE FOR
ENTERPRISE LANE, MADISON, WI, 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS
OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE
A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT
BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH
THIS DESIGN SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.

DESIGN CONDITIONS WITH APPLICABLE PROVISIONS OF AISC (ADDITIONAL DESIGN SPEC. BY AISC) AND TPI. THE BCG
CONNECTION PLATES ARE MADE OF 70/18/16GA (W/0.55%) ASH 6053 GRADE 40/60 (W/ 0.55% GALV. STEEL. APPLY
PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A Z.

ANY INSPECTION OF PLATES FURNISHED BY (1) SHALL BE PERFORMED AS OF TPI 2002 SEC. 3. A SEAL ON THIS
DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SOCIETY FOR THE TRUSS COMPONENT
DESIGNER SHALL BE RESPONSIBLE FOR THE USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE
BUILDING DESIGNER PER AISC/TPI 1 SEC. 2.



ITW Building Components Group, Inc.
Haines City, FL 33844
For specification information visit us at
www.alpinebuilding.com

TC LL	20.0 PSF	REF	R8228- 40835
TC DL	10.0 PSF	DATE	09/24/07
BC DL	10.0 PSF	DRW	HCUSR8228 07267006
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT. LD.	40.0 PSF	SEQN-	49090
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TB1R22RZ01

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP 8, wind TC DL 5.0 psf, wind BC DL 5.0 psf. $I_w=1.00$ $G_{CPI} (+/-)=0.18$

End verticals not exposed to wind pressure.

(A) Calculated horizontal deflection is 0.15" due to live load and 0.45" due to dead load.

(A) Continuous lateral bracing equally spaced on member.

rigid ceiling.

factor for dead load is 1.50.

 $R=808 \quad U=69 \quad W=3.5^{\text{a}}$

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)

QTY:1 FL/4/-/-/-/R/-

Scale = .25" / Ft.

OTHERWISE INDICATED TOP CHORD SHALL
A PROPERLY ATTACHED RIGID CEILING.

ALPINE

James City, FL 23044
 F1 Certificate of Authorization - 4/1/77

BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

SPACING SFF ABOVE

JRFF- 1TB18228Z01

Top	chord	2x4	SP	#2	Dense
Bot	chord	2x8	SP	SS	:B2 2x4 SP
	webs	2x4	SP	#3	:W4 2x4 SP
					#2 Dense:

Calculated horizontal deflection is 0.21" due to live load and 0.34" due to dead load.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT 11, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ Gcpi (+/-)=0.18

Wind reactions based on MMFRS pressures.

(A) Continuous lateral bracing equally spaced on member.

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: $TPI-2002(STD)/FBC$
 $Cq/RT=1.00(1.25)$

 $Cq/RT=1.00(1.25)/0(0)$

7.36.0424.12

QTY:1 FL/4/-/-/-/R/-

Scale = .25"/Ft.

WARNING: THE FOLLOWING EXISTING CASE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BROCKING REFER TO BECI (BUILDING COMPOSITE) SAFETY INFORMATION, PUBLISHED BY THE FRISS PLASTIC INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA., 22314, AND MICA (MODULAR INGRESS CONTROL) OF AMERICA, 65000 INTERSTATE LAKE, MADISON, AL., 35179 FOR SAFETY PRACTICES PRIOR TO RECONSTRUCTING THESE STRUCTURES. UNDESIRABLE, OCCASIONALY INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844

MAX GABLE VERTICAL LENGTH														
CABLE SPECIES	2X4 VERTICAL BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE •				(1) 2X4 "L" BRACE •				(1) 2X6 "L" BRACE •		(2) 2X6 "L" BRACE ••	
			GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B		
24" O.C.	SPF	#1 / #2	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 5"	9' 8"	12' 5"	12' 9"	14' 0"	14' 0"	
		#3	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"	
		STUD	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 3"	12' 3"	14' 0"	14' 0"	
	HF	STANDARD	3' 9"	5' 2"	5' 2"	6' 9"	6' 9"	9' 1"	9' 1"	10' 7"	10' 7"	14' 0"	14' 0"	
		#1	4' 3"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"	
		#2	4' 2"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"	
	DFL	#3	4' 0"	6' 2"	6' 2"	7' 11"	8' 1"	9' 5"	9' 11"	12' 5"	12' 8"	14' 0"	14' 0"	
		STUD	4' 0"	6' 1"	6' 1"	7' 11"	8' 0"	9' 5"	9' 11"	12' 5"	12' 8"	14' 0"	14' 0"	
		STANDARD	3' 10"	5' 3"	5' 3"	6' 11"	6' 11"	9' 4"	9' 4"	10' 10"	10' 10"	14' 0"	14' 0"	
	16" O.C.	SPF	#1 / #2	4' 5"	7' 8"	7' 10"	9' 1"	9' 1"	10' 10"	11' 1"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	4' 4"	7' 4"	7' 4"	7' 4"	9' 1"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"
			STUD	4' 4"	7' 4"	7' 4"	7' 4"	9' 1"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"
HF		STANDARD	4' 4"	6' 4"	6' 4"	8' 4"	8' 4"	10' 10"	10' 10"	12' 11"	12' 11"	14' 0"	14' 0"	
		#1	4' 10"	7' 8"	8' 3"	9' 1"	9' 9"	10' 10"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"	
		#2	4' 9"	7' 8"	8' 3"	9' 1"	9' 9"	10' 10"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"	
DFL		#3	4' 6"	7' 7"	7' 7"	9' 1"	9' 6"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"	
		STUD	4' 6"	7' 6"	7' 6"	9' 1"	9' 6"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"	
		STANDARD	4' 5"	6' 5"	6' 5"	8' 6"	8' 6"	10' 10"	11' 1"	13' 3"	13' 3"	14' 0"	14' 0"	
12" O.C.		SPF	#1 / #2	4' 11"	8' 5"	8' 8"	10' 0"	10' 3"	11' 11"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	4' 9"	8' 5"	8' 5"	10' 0"	10' 0"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
			STUD	4' 9"	8' 5"	8' 5"	10' 0"	10' 0"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
	HF	STANDARD	4' 9"	7' 3"	7' 3"	9' 7"	9' 7"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"	
		#1	5' 4"	8' 5"	9' 1"	10' 0"	10' 9"	11' 11"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"	
		#2	5' 3"	8' 5"	9' 1"	10' 0"	10' 9"	11' 11"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"	
	DFL	#3	5' 0"	8' 5"	8' 5"	10' 0"	10' 6"	11' 11"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	
		STUD	5' 0"	8' 5"	8' 5"	10' 0"	10' 6"	11' 11"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	
		STANDARD	4' 11"	7' 5"	7' 5"	9' 10"	9' 10"	11' 11"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"	

CABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS $L/240$.

PROVIDE UPLIFT CONNECTIONS FOR 80 PLF OVER
CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0"

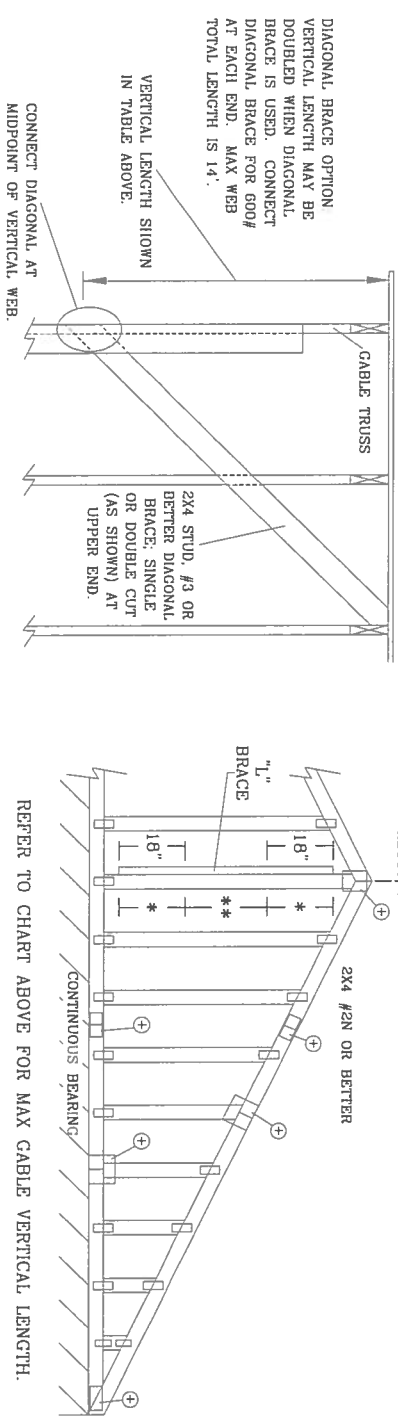
PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.

IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.

IN 18" END ZONES AND 6" O.C. BETWEEN ZONES
** FOR (2) L BRACES: SPACE NAILS AT 3' O.C.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.



GABLE VERTICAL PLATE SIZES	
VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4
GREATER THAN 11' 6"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR
PEAK, SPLICE, AND HEEL PLATES.

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.



ITW BUILDING COMPONENTS GROUP, INC.
POMPANO BEACH, FLORIDA

WARNING TRUSSES REQUIRE EXTENSIVE CARE. IF FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TP1 TRUSS PLATE INSTITUTE, 2108 NORTH LEE STR., SUITE 312, ALEXANDRIA, VA 22304 AND VITA CLOUD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719 FOR SAFETY PRECAUTIONS PRIOR TO REUPRISING THESE TRUSSES. TRUSSES MUST BE USED AS INDICATED. TWO CHORD SHALL BE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. (TV, BCG, INT) SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TP1, OR FABRICATING, HANDLING, SHIPPING, INSTALLING, OR BRACING OF TRUSSES. DESIGN CONFORMANCE WITH APPLICABLE PROVISIONS OF NDS NATIONAL DESIGN SPEC. BY AIA/PD AND TP1 TV, BCG CONNECTOR PLATES ARE MADE OF 20D/16GA. C.V./HSS/VS ASTM A453 GRADE 40/60 C.V./K/1.55S. DESIGNATION PER DRAWING 1604-2. FOLD OVER TOP OF TRUSS TO ALLOW ACCESS TO BOTTOM CHORD PER DESIGN. POSITION PER DRAWING 1604-2.

WARNING TP1 1-2008 SEC. 3, A STEEL, THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER. PER ANSI/TP1 1 SEC. 2.

SEP 24 '07

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF ASCE7-02-CAB1015

DATE 2/23/07

DRWG A11015EE0207

-ENG

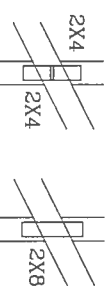
Diagram illustrating the arrangement of cables and the vertical length of the cables (GABLE VERTICAL LENGTH TYP.) for a bridge structure. The diagram shows a central vertical line and a horizontal line labeled "SYM. ABOUT". The cables are shown as diagonal lines connecting the bridge deck to the central vertical line. The diagram is labeled "EXAMPLE: 2" and includes a note: "* IF GABLE VERTICAL SINGLE PLATE TO SPICE, WEB AND".

CABLE VERTICAL PLATE SIZES		
VERTICAL LENGTH BETWEEN CHORDS	PLATE SIZE	IF PLATES OVERLAP*
LESS THAN 4' 0"	1X4 OR 2X3	2XB
GREATER THAN 4' 0" BUT LESS THAN 11' 6"	2X4	2XB
GREATER THAN 11' 6"	2.5X4	2.5XB

④ REFER TO ENGINEERED CROSS DESIGN FOR PEAK, SPLICE, WEB AND HEEL PLATES.

* IF GABLE VERTICAL PLATES OVERLAP, USE A SINGLE PLATE TO SPAN THE WEB.

EXAMPLE:



TO CONVERT FROM "L" TO "I" REINFORCING MEMBERS, MULTIPLY "I" FACTOR BY LENGTH (BASED ON GABLE VERTICAL SPECIES, GRADE AND SPACING) FOR (1) 2X4 "L" BRACE, GROUP A, OBTAINED FROM THE APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR SBCCI WIND LOAD.

MAXIMUM ALLOWABLE "T" REINFORCED CABLE VERTICAL LENGTH IS 14' FROM TOP TO BOTTOM CHORD.

WIND SPEED AND MRH	T ¹ REINF. MR. SIZE	SBCI	ASCE
110 MPH	2x4	10 %	10 %
15 FT	2x6	40 %	50 %
110 MPH	2x4	10 %	10 %
30 FT	2x6	50 %	50 %
100 MPH	2x4	10 %	10 %
15 FT	2x6	30 %	50 %
100 MPH	2x4	10 %	10 %
30 FT	2x6	40 %	40 %
90 MPH	2x4	20 %	10 %
15 FT	2x6	20 %	10 %
90 MPH	2x4	10 %	10 %
30 FT	2x6	30 %	50 %
80 MPH	2x4	10 %	20 %
15 FT	2x6	10 %	30 %
80 MPH	2x4	20 %	10 %
30 FT	2x6	20 %	40 %
70 MPH	2x4	0 %	20 %
15 FT	2x6	0 %	20 %
70 MPH	2x4	10 %	20 %
30 FT	2x6	10 %	30 %

EXAMPLE:

ASCE WIND SPEED = 100 MPH

MEAN ROOF HEIGHT = 30 FT

GABLE VERTICAL = 24" O.C. SP #3

"T" REINFORCING MEMBER SIZE = 2X4

"I" BRACE INCREASE (FROM ABOVE),

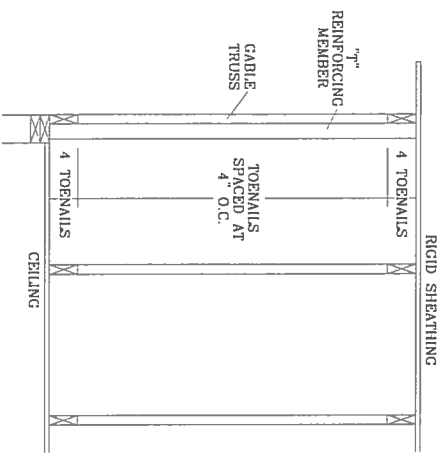
(1) 2X4 L BRACE LENGTH = 6'

MINIMUM 1 REINFORCED GABLE VENT
110 x 6' 7" = 7' 3"

1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50	51	52	53	54	55	56	57	58	59	60	61	62	63	64	65	66	67	68	69	70	71	72	73	74	75	76	77	78	79	80	81	82	83	84	85	86	87	88	89	90	91	92	93	94	95	96	97	98	99	100	101	102	103	104	105	106	107	108	109	110	111	112	113	114	115	116	117	118	119	120	121	122	123	124	125	126	127	128	129	130	131	132	133	134	135	136	137	138	139	140	141	142	143	144	145	146	147	148	149	150	151	152	153	154	155	156	157	158	159	160	161	162	163	164	165	166	167	168	169	170	171	172	173	174	175	176	177	178	179	180	181	182	183	184	185	186	187	188	189	190	191	192	193	194	195	196	197	198	199	200	201	202	203	204	205	206	207	208	209	210	211	212	213	214	215	216	217	218	219	220	221	222	223	224	225	226	227	228	229	230	231	232	233	234	235	236	237	238	239	240	241	242	243	244	245	246	247	248	249	250	251	252	253	254	255	256	257	258	259	260	261	262	263	264	265	266	267	268	269	270	271	272	273	274	275	276	277	278	279	280	281	282	283	284	285	286	287	288	289	290	291	292	293	294	295	296	297	298	299	300	301	302	303	304	305	306	307	308	309	310	311	312	313	314	315	316	317	318	319	320	321	322	323	324	325	326	327	328	329	330	331	332	333	334	335	336	337	338	339	340	341	342	343	344	345	346	347	348	349	350	351	352	353	354	355	356	357	358	359	360	361	362	363	364	365	366	367	368	369	370	371	372	373	374	375	376	377	378	379	380	381	382	383	384	385	386	387	388	389	390	391	392	393	394	395	396	397	398	399	400	401	402	403	404	405	406	407	408	409	410	411	412	413	414	415	416	417	418	419	420	421	422	423	424	425	426	427	428	429	430	431	432	433	434	435	436	437	438	439	440	441	442	443	444	445	446	447	448	449	450	451	452	453	454	455	456	457	458	459	460	461	462	463	464	465	466
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[illegible]

THIS DRAWING REPLACES DRAWINGS GAB98117 876,719 & HC26294035



PROVIDE CONNECTIONS FOR UPLIFT SPECIFIED ON THE ENGINEERED TRUSS DESIGN.

ATTACH EACH "I" REINFORCING MEMBER WITH
HAND DRIVEN NAILS:
10d COMMON (0.148"x 3 "MIN) TOENAILS AT 4" O.C. PLUS
(4) 16d COMMON (0.162" X 3.5".MIN) TOENAILS IN TOP AND BOTTOM CHORD.

GUN DRIVEN NAILS:
8d COMMON (0.131"x 2.5".MIN) TOENAILS AT 4" O.C. PLUS
(4) TOENAILS IN TOP AND BOTTOM CHORD.

THIS DETAIL TO BE USED WITH THE APPROPRIATE ALPINE GABLE DETAIL FOR ASCE
OR SBCCI WIND LOAD.

GUN DRIVEN NAILS:
8d COMMON (0.131"X 2.5",MIN) TOENAILS AT 4" O.C. PLUS
(4) TOENAILS IN TOP AND BOTTOM CHORD.

THIS DETAIL TO BE USED WITH THE APPROPRIATE ALPINE CABLE DETAIL FOR ASCE OR SBCCI WIND LOAD.

ASCE 7-93 GABLE DETAIL DRAWINGS

A110015E0207, A10015E0207, A09015E0207, A08015E0207, A07015E0207, A10030E0207, A10030E0207, A09030E0207, A08030E0207, A07030E0207

ASCE 7-98 GABLE DETAIL DRAWINGS

A13015E0C0207, A12015E0C0207, A11015E0C0207, A10015E0C0207, A08515E0C0207, A10030E0C0207, A12030E0C0207, A11030E0C0207, A10030E0C0207, A08530E0C0207

ASCE 7-02 GABLE DETAIL DRAWINGS

A13015E0207, A12015E0207, A11015E0207, A10015E0207, A08515E0207, A10030E0207, A12030E0207, A11030E0207, A10030E0207, A08530E0207

ASCE 7-05 GABLE DETAIL DRAWINGS

A13015E0207, A12015E0207, A11015E0207, A10015E0207, A08515E0207, A10030E0207, A12030E0207, A11030E0207, A10030E0207, A08530E0207

SEE APPROPRIATE ALPINE GABLE DETAIL (ASCE OR SBCCI WIND LOAD) FOR MAXIMUM UNREINFORCED CABLE VERTICAL LENGTH.

TRUSSES REQUIRE EXTREME CARE IN FACTURING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 210 NORTH LEE STR., SUITE 312, WESTLAKE, VA 22149 AND WFLA WOOD TRUSS COUNCIL OF AMERICA, 6400 ENTERPRISE BL., MIAMI 33157, FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE OPERATIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL MEMBERS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID JOINT.

ALPINE

ITW BUILDING COMPONENTS GROUP, INC.
POMPANO BEACH, FLORIDA

Sep 24 '07

REF	LET-IN VERT
DATE	2/23/07
DRWG	GBLETTIN0207
-ENG	DLJ/KAR
MAX TOT. LD.	60 PSF
DUR. FAC.	ANY
MAX SPACING	24.0"

CLB WEB BRACE SUBSTITUTION

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON AN ALPINE TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

NOTES:

THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB BRACING.

ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE. FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE BRACING.

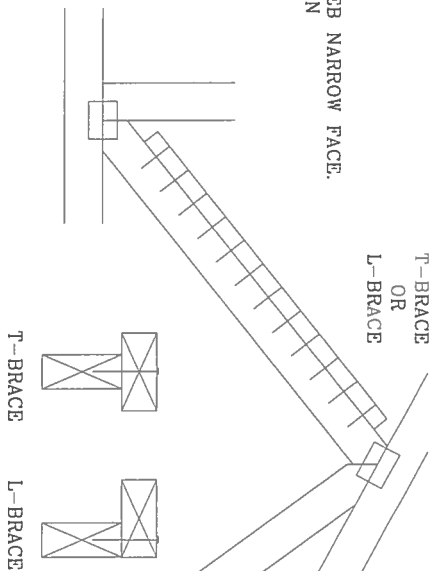
WEB MEMBER SIZE	SPECIFIED CLB BRACING	T OR L-BRACE	ALTERNATIVE BRACING SCAB BRACE
2X3 OR 2X4	1 ROW	2X4	1-2X4
2X3 OR 2X4	2 ROWS	2X6	2-2X4
2X6	1 ROW	2X4	1-2X6
2X6	2 ROWS	2X6	2-2X4(*)
2X8	1 ROW	2X6	1-2X8
2X8	2 ROWS	2X6	2-2X6(*)

T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

(*) CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH FACE OF WEB.

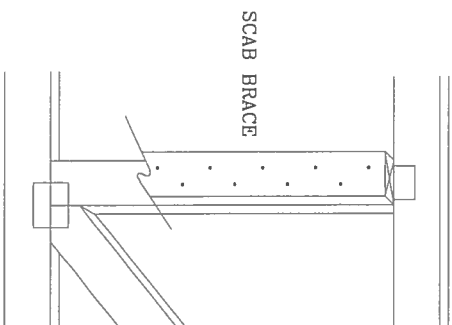
T-BRACING OR L-BRACING:

APPLY TO EITHER SIDE OF WEB NARROW FACE. ATTACH WITH 10d BOX OR GUN (0.128" x 3." MIN) NAILS. AT 6" O.C. BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH

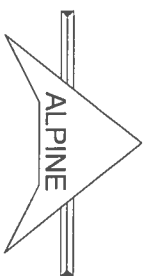


SCAB BRACING:

APPLY SCAB(S) TO WIDE FACE OF WEB. NO MORE THAN (1) SCAB PER FACE. ATTACH WITH 10d BOX OR GUN (0.128" x 3." MIN) NAILS. AT 6" O.C. BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH



THIS DRAWING REPLACES DRAWING 579.640



TRUSS BUILDING COMPONENTS GROUP, INC.
POMPAHO BEACH, FLORIDA

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 218 NORTH LEE STR., SUITE 312, ALEXANDRIA, VA. 22314 AND VICA GOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, HADISON, VI 52719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. TTV, BEG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSSES IN CONFORMANCE WITH TPI, OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY ACPA) AND TPI. ALL STEEL, APPLICABLE PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS PER DESIGN, SHALL BE 16d BOX OR GUN (0.128" x 3." MIN) NAILS. ALL BOLTS SHALL BE 1/2" DIA. A308. ALL WELDING SHALL BE PER ANSI/A3 OF TPI 1-2002 SEC. 3.160. SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF THE SUITABILITY AND ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSI/TPI 1 SEC. 2.

Sep 24 '07

TC LL	PSF	REF	CLB SUBST.
TC DL	PSF	DATE	2/23/07
BC DL	PSF	DRWG	BRCBLSUB0207
BC LL	PSF	-ENG	MLH/KAR
TOT. LD.	PSF		
DUR. FAC.			
SPACING			

