ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 0 278
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID:1TB18228Z0124134501

Truss Fabricator: Anderson Truss Company

Job Identification: 7-212--OWNER BUILDER Josh Campbell -- 386-984-6589-C

Truss Count: 26

Model Code: Florida Building Code 2004 and 2006 Supplement

Truss Criteria: ANSI/TPI-2002 (STD) /FBC

Engineering Software: Alpine Software, Version 7.36.

Minimum Design Loads: Roof - 40.0 PSF @ 1.25 Duration

Floor - N/A

Wind - 110 MPH ASCE 7-02 - Closed

Notes:

 Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1

2. As shown on attached drawings; the drawing number is preceded by: HCUSR8228

Details: A11015EE-GBLLETIN-BRCLBSUB-

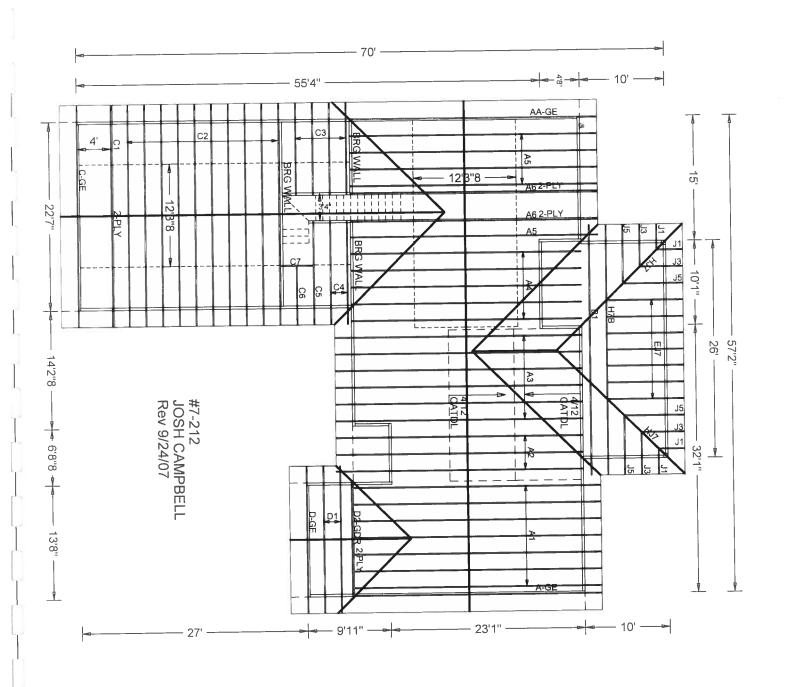
#	Ref Description	Drawing#	Date
1	40812A5	07267010	09/24/07
2	40813A1	07267017	09/24/07
3	40814 A2	07267018	09/24/07
4	40815A3	07267016	09/24/07
5	40816A6	07267012	09/24/07
6	40817 AA - GE	07267001	09/24/07
7	40818A-GE	07267013	09/24/07
8	40819A4	07267014	09/24/07
9	40820 H7B	07267001	09/24/07
10	40821 B1	07267011	09/24/07
11	40822C2	07267005	09/24/07
12	40823C1	07267015	09/24/07
13	40824C-GE	07267007	09/24/07
14	40825 D1	07267002	09/24/07
15	40826 D-GE	07267025	09/24/07
16	40827 D2 - GDR	07267004	09/24/07
17	40828J1	07267023	09/24/07
18	40829HJ7	07267020	09/24/07
19	40830 J3	07267022	09/24/07
20	40831 J5	07267021	09/24/07
21	40832EJ7	07267024	09/24/07
22	40833C3	07267008	09/24/07
23	40834C6	07267009	09/24/07
24	40835C7	07267006	09/24/07
25	40836-=C5	07267003	09/24/07
26	40837 C4	07267019	09/24/07

Seal Date: 09/24/2007

-Truss Design Engineer-

1950 Marley Drive Haines City, FL 33844





JOB DESCRIPTION:: OWNER BUILDER
/: Josh Campbell

Top chord 2x4 SP #2 Dense :T2, T3 2x8 SP SS:
Bot chord 2x8 SP SS :B3 2x4 SP #2 Dense:
Webs 2x4 SP #3

Calculated horizontal deflection is 0.15" due to live load and 0.27" due to dead load.

Collar-tie braced with continuous lateral bracing at 24" OC. rigid ceiling.

0

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

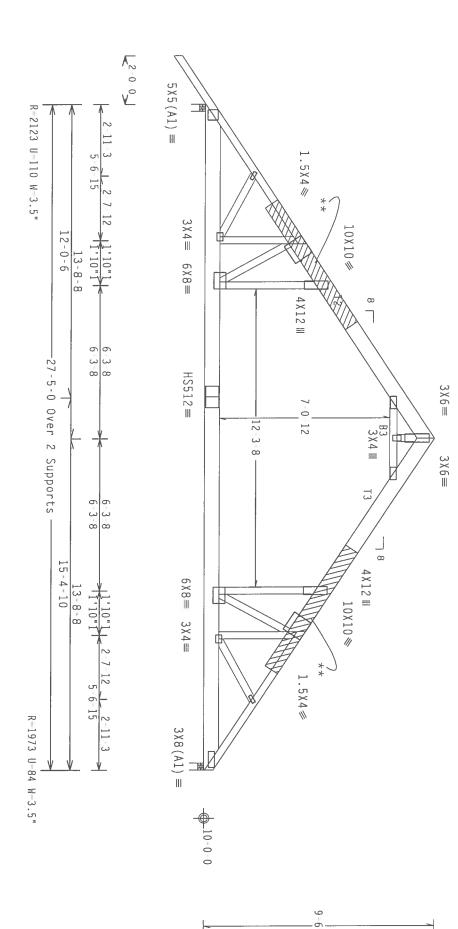
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 7-6-12 to 19-10 4.

(1) 2x8x(6-0-0) TRIMMED TO FIT SP #SS OR BETTER SCAB. ATTACH ONE SCAB TO ONE FACE OF TRUSS USING 0.128"X3.0" NAILS @ 4" OC STAGGERED THROUGHOUT, WITHOUT SPLITTING THE LUMBER.

4X12(R) III



N

ITW Building Components Group, Inc. Haines City, FL 33844

F1 Carificate of Authorization 4 667 TYP. ALPINE 20 Gauge HS, ,₩ave **IMPORTANT**FURNISH A COPY OF THIS DESIGN TO THE TRISLALIATION CONTRACTOR. THE BCG, THC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN COMPORNANCE WITH FP: OR FLABELATING, HANDLING, SHAPPING, HISALLING A BRACHING OF TRUSSES, DOTSION COMPORES, MITH APPLICABLE PROPERTIES, BEACH SHOULDE, SHEPPING, THISALLING A BRACHE OF THE APPLY PHALES TO EACH SHOULDE, DESIGN CONTROL OF THE ALE AND THE APPLY PHALES TO EACH SACE OF TRUSS AND, DINCESS OTHER SET LOCATED ON THIS DESIGN, POSITION OF BRANDED SHOW, ANY DISPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A OF THE TOOS SEC.3. A SEAL ON THIS DESIGN. POSITION OF THE ANY DISPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX AS OF THE TOOS SEC.3. A SEAL ON THIS DESIGN. POSITION OF THE PLAYS COMPONENT OF THE SHALL BE AND THE TOOS SEC.3. A SEAL ON THIS DESIGN. POSITION OF THE PLAYS COMPONENT OF THE SHALL BE PER ANNEX AS OF THE TOOS SEC.3. **MARRIBO** HRISSIS RIGHIBE CYRECHE CARE IN FARRICATION, HARISTED BY FIT (TRUSS PLATE INSTALLING AND BRACING. REFER TO REST (BRILLING COMPOUNT SAFETY INFORMATION), PUBLISHED BY FIT (TRUSS PLATE INSTITUTE, 218 MORTH LE STREET, SUITE JEZ, ALEXANDRIA, VA, 22214) AND HEAR (HOUD TRUSS COUNCEL OF MARRICA. 6300 CHILERRISE LAWE, MADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING INEST FUNCTIONS. UNLESS CHILDICATED OF CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED ATTACHED REGION ESTALL HAVE BUILDING DESIGNER PER ANSI/1PT 1 SEC Cq/RT=1.00(1.25)/0(0) 7.36.0424.12 BC LL BC DL TC DL SPACING DUR.FAC. IC LL TOT.LD. FL/4/-/-/R/-24.0" 40.0 20.0 10.0 PSF 10.0 PSF 1.25 0.0 PSF PSF PSF

SEQN

HC-ENG

DF/DF 49012

JRFF-

1TB18778Z01

REF

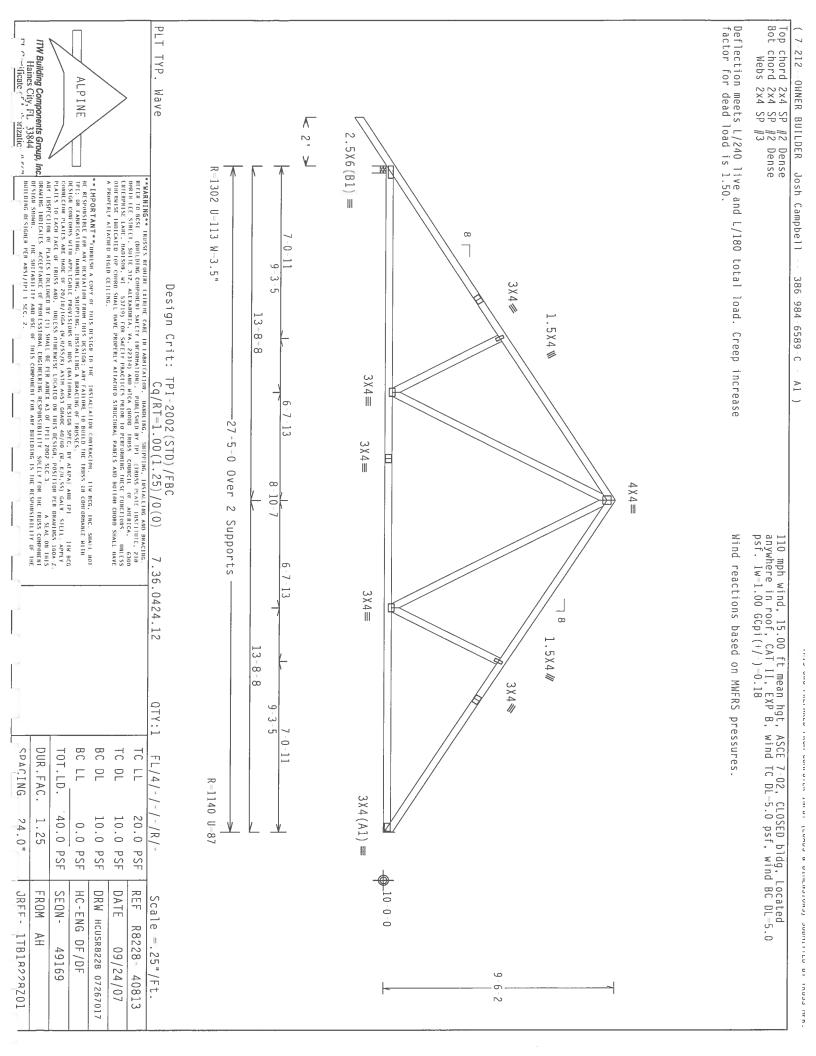
09/24/07

Scale = .25"/Ft. EF R8228= 40812

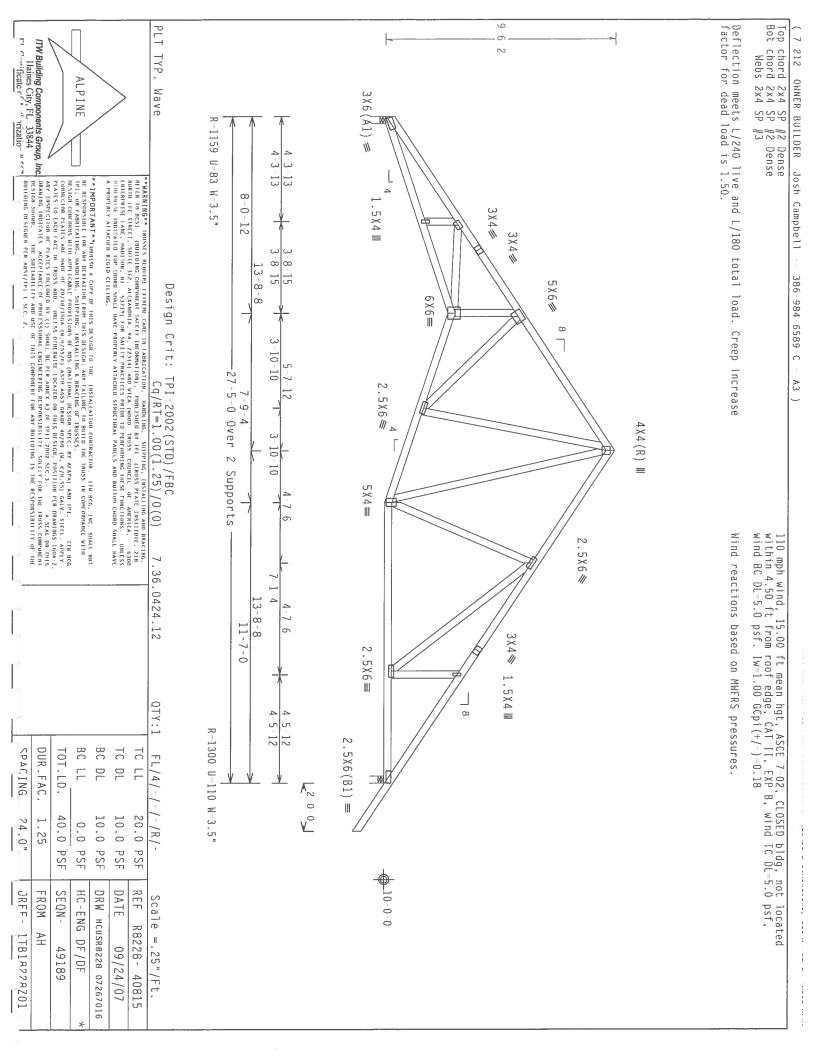
DRW HCUSR8228 07267010

Design Crit:

TPI-2002 (STD) /FBC



9 Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 PLT TYP. Wave Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50\,.$ (2) 7-212--OWNER BUILDER Josh Campbell Haines City, FL 33844
History FL 33844
Ft Conficate of Anni prization in Confication in Conficat 2 Continuous lateral bracing equally spaced on member ALPINE 3X6(A1) ≡ =9014-3 0 = 63-13 DESIGN CHUTOMES MITH APPLICANTE PROVISIONS OF HIDS (MATIONAL DESIGN SPIC, BY ATRAY) AND TREE. HIP ACC. COUNT CLUBE TEATES ARE LANGE OF ZOTALFIGA, MY HISSEN, ASIA MASS GRADE MOTEO, MY JUSS GAIN STEEL, APPLY PLATES TO EACH FACE OF TRUSS AND, UNICESS OTHERWISE LOCATED ON THIS DESIGN, POSITION FER DRAWHINGS 160A-Z. ANY THREFCETION OF PLATES TOLLOWED BY (1) SMALL BE FER ANNEX AS OF THE 2002 SEC. 3. A SEAL ON THIS DRAWHING HOSTACKES ACCEPTANCE OF PROTESSIONAL CHOINTERING RESPONSIBILITY SOFTIER SECONDARY OF THE BRISS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT TORS ANY BUILDING IS THE RESPONSIBILITY OF THE **WARNING** IRUSETS REQUIRE EXTRUME CARE IN CABRICATION, MANDLING, SHIPPING, INSTALLING AND BRACING, RELER TO BEST. (BULLDING COMPOUND SAFLY INFORMATION), PUBLISHING BAY PI (CHUSS PLAIE HESTITUE, 218 HORS). ALEXANDRAN, VA, 22134) AND MICA (400D TRUSS COUNCIL OF AUTHLEA, 630D CHIERCA, MAISSON, MI 55379) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNITES OTHERWISE INDIVALED FOR CHORD SMALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SMALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SMALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SMALL HAVE BUILDING DESIGNER PER ANSI/IPE I SEC ₩=3.5" ∞ 3X4/ 0 .5X4 III 12 3X4# 8 8 13-8-8 -15 386-984-6589-C 5×6/ Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/0(0) €X6≡ 10 10 27-5-0 A2) 2.5X6₩ 9 0ver 4X4(R) III 10 \sim -10 Supports 5 X 4 ≡ Ξ 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED within 4.50 ft from roof edge, CAT II, EXP B, wind wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18 Wind reactions based on MWFRS pressures 2.5X6 7.36.0424.12 α R=1558 U=106 W=3.5' 11-7-0 A 3 X 4 // $2.5 \times 6 =$ 1.5X4 III 00 4-5-4-5-12 4-4-0--12 $2.5 \times 6 (B1) =$ BC DL TC DL TC LL SPACING DUR.FAC. TOT.LD. FL/4/-/-/R/-24.0" 40.0 1.25 10.0 PSF 10.0 PSF 20.0 PSF 0.0 bldg, not located TC DL=5.0 psf, PSF PSF JRFF-SEQN-DATE REF FR OM HC-ENG DRW HCUSR8228 07267018 10-0-0 Scale = .25"/ft. R8228- 40814 1TB18778Z01 DF / DF 49195 09/24/07



Wind reactions based on MWFRS pressures 110 mph wind, 15.00 ft mean hgt, ASCE anywhere in roof, CAT II, EXP B, wind psf. iw=1.00 GCpi(+/-)=0.18 Top chord 2x4 SP #2 Dense :T2, Bot chord 2x8 SP SS :B3 2x4 SP Webs 2x4 SP #3 BC attic room floor loading: LL = psf; from 7-6-12 to 19-10-4. Trusses to be spaced at 32.5" OC maximum ī 1 Haines City, FL 33844

"Ticate" vizatio lieu of structural panels use purlins to TYP. ALPINE 20 Gauge HS K2-0-0 V $5X5(A1) \equiv$,₩а∨е **IMPORTANT**FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. IT B BCG, INC. SHALL NOT OR RESPONSIBLY FOR ANY DEVIATION FROM THIS OESIGN: ANY FAILURE TO BUILD THE FRUSS IN CONTROMANCE WITH FPL; OR LARRIAGING, MANDING, SHEPPIRG, HISTALLING, A BRACHING OF FRUSSES, DESIGN CONTROL OF THE PROPERTY OF THE PROP **MARNING** HUSSES REQUERE EXPRENE CARE IN FARRECATION, HANDELING, SHIPPING, HISTALLING AND BRACING.

RETER TO RESE
(RUHLDING COMPONENT SAFETY INFORMATION), DUBLISHED BY FPT (TRUSS PLATE INSTITUTE, 218

HORRILLE STREET, SWITE JEZ, ALEXANDRIA, VA. ZEZIJA) AND MICA (MOND 18USS COUNCEL OF AMERICA. 6300

THIERRESE, LANE, HADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING HUSS, FUNCTIONS, UNLESS

OTHERRISE HOLICATIO FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARILES AND BOTTOM CHORD SHALL HAVE

A PROPERLY ATTACHED REGID CELLING. =2878 U=149 BUILDING DESIGNER PER ANSI/IPI I SEC T3 2x8 SP : #2 Dense: 1.5X4 // 5 9 40.00 psf; 15 W=3.5"7-02, CLOSED bldg, Located TC DL=5.0 psf, wind BC DL=5.0 SS: $3 \times 4 \equiv$ Design Crit: 10X10 / brace 12-0-6 DL 10"I 13-8-8 €X8= TC @ 10 .00 4 X 1 2 III 24" ∞ TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/0(0) 000 00 ယ် ထ ထ -27 - 5 HS512≡ 3 X 6≡ 0 4X12(R) || Collar-tie braced with continuous lateral bracing at 0ver 0 12 12 83 3×4 **■** ώ 2 œ 3 X 6≡ Supports Nailing Schedule: (12d Common_(0.148"x3.25",_min.)_nails)
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails due to dead load Calculated horizontal deflection is 0.10" due to live load and 0.19 in each row to avoid splitting I COMPLETE 7.36.0424.12 Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. 6-3-8 5-4-10 4X12 III €X8≡ TRUSSES 13-8-8 1 10"1 10X10₩ 3 \ 4 == REQUIRED 1.5X4 BC LL BC DL TC DL G TC LL SPACING DUR.FAC. TOT.LD. FL/4/-/-/R/-6 15 R=2676 U=113 W=3.5" $3 \times 8 (A1) =$ 32.5" 1.25 40.0 20.0 10.0 PSF 10.0 PSF 0.0 PSF PSF PSF 24" DATE FROM SEQN-REF 10 JRFF-HC-ENG DF/DF DRW HCUSR8228 07267012 Scale = .25"/Ft. 0 R8228 1TB18228Z01 49202 09/24/07 40816 9 ~

Top chord 2x4 SP #2 Dense :T3, Bot chord 2x8 SP SS :B3 2x4 SP Webs 2x4 SP #3 Note: All Plates Are 1.5X4 Except See DWGS Al1015EE0207 & GBLLETIN0207 for more requirements Collar tie braced with continuous lateral rigid ceiling. 110 mph wind, 15.00 ft mean hgt, ASCE 7.02, anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18 (7-212--OWNER BUILDER Josh Campbell --Haines City, FL 33844
Haines City, FL 33844 9 MEMBER TO BE LATERALLY BRACED BRACING SYSTEM TO BE DESIGNED attic room floor loading: LL 6-12 to 19-10-4. TYP. ALPINE Wave nizatic " " " ** IMPORTANT** URBIES A COPY OF THIS MESTAGE TO THE INSTALLATION CONTRACTOR. THE BLOCK THAC. SHALL HOT BE RESPONSIBLE FOR MAY DEVIATION FROM THIS DESIGN, AND FALLER TO BOILD THE BROSS HE COMPORMANCE WITH PIECE OF TARRESTS.

BY SARRICATHED, MANDLING, SHIPPING, LIMSALEHRA & BRACLING OF TRASSES.

BY SARRICATHED, MANDLING, SHIPPING, THAS LELIER & BRACLING OF TRASSES.

BY SARRICATE OF TARRESTS AND THE SHIPPING, THE SARRICAN STATEMENT OF THE STATEMENT OF THE SARRICAN STATEMENT OF THE SAR **WARNING** TRUSSES REQUIRE EXTREME CARE IN FARRICATION. INABELING, SHIPPING, INSTALLING AND BRACING. RELER TO BEST. (BUILDING COMPONENT SAFELY INFORMATION), PUBLISHED BY FET (TRUSS PLATE INSTITUTE, ZIB MORTH LLE STREIT, SHITE JIS, ALEXANDRIA, VA. 22314) AND HEGA (HOOD TRUSS COUNCELS OF ARREICA, 6300 CHILBRUSS LANE, HANTSON, H. 53719) FOR SAFELY PRACTICES PRIOR TO PERFORMING IN SE FUNCTIONS. UNLESS OTHERWISE LANDSON, H. 53719) FOR SAFELY PRACTICES PRIOR TO PERFORMING IN SE FUNCTIONS. UNLESS OTHERWISE LANDSON FOR CHORD SMALL MAYE PROPERTY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SMALL MAYE PROPERTY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SMALL MAYE **1**2-0-0 ✓ 4 X 6 == T4 2x8 SP SS: #2 Dense: F OR AND 0 4 X 6 ≡ 10 40.00 psf; DL R=235 PLF U=8 PLF W=12-0-0 HORIZONTAL WIND LOADS. FURNISHED BY OTHERS. 8 As Shown. 386-984-6589-C 4X10₩ bracing Design Crit: CLOSED bldg, Located =5.0 psf, wind BC at 10.00 psf; from 1,5,33,310,3,4 3 X 4 ≡ 24" 10X10 @ 36"131'3"4 12-6-11 TPI-2002 (STD) /FBC AA-GE) Cq/RT=1.00(1.25)/0(0) 3 X 4 = 11-8-8 œ BY ATKAN) AND IPI. IIH BCG
(H. K/H.SS) GALV. SIEEL. APPLY
GH. POSITION PER DBAHINGS 160A Z.
DOZ SEC. J. A SEAL ON THIS
SOLELY FOR THE TRUSS COMPONENT
LING IS THE RESPONSIBILITY OF THE 9 6 ىل ယ် 27-5-0 8 8 =9 X 9 4 X 8 (R) R=155 PLF U=6 PLF W=15-5 7 0ver The building designer is responsible for the design of the roof and ceiling diaphragms, gable end shear walls, and supporting shear walls. Shear walls must provide continuous lateral restraint to the Gable end. All connections to be designed by the building designer. Truss spaced at 24.0" OC designed to support 2-0-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched. plate Wind reactions based on MWFRS pressures (**) 1 plate(s) require special positioning. Refer to s plate plot details for special positioning requirements Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. Ö 2 ВЗ 7.36.0424.12 ú 1.5X4(**) III N ά Supports T 4 6 5 ယ် ώ ά ά ∞ 14 - 10 - 5ά 4 X 6 = 1'3"4"131'5"31 2-0-0 1'3"6"13 | 5"3 | 1'2"911"4 0X10 / 3 \ 4 ≡ BC DL BC LL TC DL TC LL SPACING DUR.FAC. TOT.LD. FL/4/-/-/-/R/-SFF Refer to scaled 1.25 40 10.0 PSF 10.0 20.0 0.0 ABOVE 0 PSF PSF PSF PSF 2-0-0 4X10 / 1"12 JRFF-FROM SEQN DATE REF DRW HCUSR8228 07267001 HC-ENG 4 X 6 = Scale = 4 X 6 ≡ R8228-10-0-1TB18228Z01 SSB/AP 51256 09/24/07 .25"/Ft. 40817 0

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF, FLOOR AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS. AND SUPPORTING SHEAR WALLS. DIAPHRAGMS AND SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS ARE TO BE PROVIDED BY THE BUILDING DESIGNER. Truss spaced at 24.0" OC designed to support 2-0-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched. Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 Note: All Plates Are 1.5X4 Except As Shown. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. See DWGS All015EE0207 & GBLLETIN0207 for more requirements 7-212--OWNER BUILDER Josh Campbell TYP. ALPINE Wave 4 X 4 ≡ Λ R = 1922 4X10(R)3※4 ● 0 PLF U=25 4-114-7-2 $4 \times 4 =$ **IMPORTANT**FURBLES A COPY OF THIS DESIGN TO THE THISTALLATION CONTRACTOR. THE BCG, TMC. SMALL NOT BLE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE BRUSS IN CONTORNANCE WITH IP: OR FARELSTATURG, MANUEL REOVISIONS OF THOS CS. A BRACING OF TRUSSES, DESIGN CONTRORS WITH APPLICABLE REOVISIONS OF THOS CS. (MATIONAL DESIGN SECC. BY ATRA) AND IP!. THE BCG CONNECTION PLAITS ARL HADE OF 70/19/166A (M.H/SS/K) ASTH A653 GRADE 40/60 (M. K/M.SS) GALV. SIEEL. APPLY PLAIES TO FACH FACE OF TRUSS AND. UNILSS OTHERSISE LOCATED ON THIS DESIGN, POSITION PER DRAWNINGS 166A-Z. ANY THIS COLINIA CONTROLLS ACCEPTANCE OF A SEA, ON THIS DESIGN. ASTHER ANY THIS COLINIA COLOR FOR THE THIS COCREGIONAL THE SHEET OF THE THIS COCREGIONAL THE SHEET OF TH **MARNING** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, INNOCING, SHIPPING, INSTALLING AND BRACING, REFER TO BEST. (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY FIFE (TRUSS PLATE INSTITUTE, 218 AND HEALT SUITE 312, ALEXANDRIA, VA, 27314) AND WICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 UNLERPRISE LAND, MADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE, FUNCTIONS, UNLESS OTHERWISE HOUGHLY AND SMALL HAVE PROPERLY ATTACHED STRUCTURAL PARIELS AND BOTTOM CHORD SMALL HAVE A PROPERLY ATTACHED RIGID CETCING. PLF W=27 386-984-6589-C Design Crit: ά 8 ထ TPI-2002 (STD) /FBC A - GE Cq/RT=1.00(1.25)/0(0)27-5-0 Over Continuous Support 3×4≡ $\widehat{\mathbb{A}}$ $4 \times 4 \equiv$ 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18 (A) 1x4~#3 or better "L" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" 0C. Wind reactions based on MWFRS pressures 7.36.0424.12 11-8-9 8 BC LL BC DL TC DL DUR.FAC. TC LL SPACING TOT.LD. FL/4/-/-/R/-3X4// 1-7-01-4-114 $4 \times 4 =$ SFF 4X10(R) / 1.25 40. 20.0 10.0 PSF 10.0 PSF 0.0 ABOVE . PSF PSF $4 \times 4 \equiv$ PSF ₩10-0-JRFF-FROM DATE REF HC-ENG SEQN-DRW HCUSR8228 07267013 Scale = .25"/ft. R8228-AH 1TB18228Z01 DF/DF 09/24/07 49255 9 40818 -15 REV

Bot NOTE: THIS TRUSS MUST BE INSTALLED AS SHOWN. IT CAN NOT BE FLIPPED END FOR END. Deflection meets L/240 live and L/180 total load. factor for dead load is $1.50\,.$ Collar tie braced with continuous lateral bracing at 24 $^{\circ}$ OC. rigid ceiling. Calculated horizontal deflection due to dead load. 7-212--OWNER BUILDER Josh Campbell --1 Haines City, FL 33844
'ificate' '' prizatic' '' chord 2x4 SP #2 Dense :T2, chord 2x8 SP SS :B3 2x4 SP Webs 2x4 SP #3 TYP. ALPINE 20 Gauge HS, Wave 3X8(A1) =-11-4 -4-8 1.5X4// ** IMPORTANT** "BUBLISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG. HC. SHALL NOT BE RESONASHE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN COM ORMANCE WITH THE TO METABLISH THE AMBLISH. SHIPPING. INSTALLING A BRACING OF TRUSSES.

DESIGN CONFICURE PLAITS ARE MORE OF TO/10/1604 (M.1/1/55/) OATH MOSS JEADE APORE OF METAPA) AND THE CONTROL OF TRUSSES.

APPLY A PROPERTY OF THE APPLY OATH ASS JEADE APORE OF METAPA SHALL SHEEL APPLY OATH ASS JEADE APORE OF METAPA. **MARNING** IRUSKIS REQUIRE EXTREME CARL IN FABRICATION, IMABLING, SHIPPING, INSTALLING AND BRACING. RETER TO ESSI. (BUILDING COMPONENT SACETY INFORMATION), PUBLISHED BY TP: (TRUSS PLATE INSTITUTE, 218 MORTH LEE STREET, SUITE 127. ALEXANDRIA, NA. 222.14) AND MICHA (MODO TRUSS COUNCIL OF AMERICA. 6300 CHILERRISE LADE, HADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OFFERNISE LADECALED FOR CHORDS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANICLS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED REGION CHORD SHALL HAVE 15 DRAHING INDICATES ACCEPTANCE OF PRINTESS
DESIGN SHOWN. THE SUITABILITY AND USE
BUILDING DESIGNER PIR ANSI/TPI 1 SEC. 2. PHAIRS TO FACH FACE OF TRUSS AND, HURLESS ONDERHISE LOCATED ON THIS DESIGN, POSITION ANY INSPECTION OF PLATES FOR CHORDEN VI, SINCH ES FOR ABOVE AS OF TRI TROS SEC. 3. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL EMGINEERING RESPONSIBILLITY SOLECY FOR THE STATE OF THE PROFESSIONAL EMGINEERING RESPONSIBILLITY SOLECY FOR THE PROFESSIONAL EMGINEERING RESPONSIBILITY SOLECY FOR THE PROFESSIONAL EMGINEERING RESPONSIBILITY FOR THE PROFESSION 0 5 X 6 (k) T3 2x8 SP #2 Dense: is 0.17" due to live load and 0.31" R=2394 U=76 W=3.5" 10X14 / THE SUITABILITY AND USE OF THIS 2-0-6 11'8"5 - 9 - I 6X8(**) **≡** 13-8-8 SS: 4X12 III 386-984-6589-C Design Crit: Creep increase 6 ယ 3-8 27 TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/0(0) A4) 5-0 HS512≡ 3X6≡ 4X12(R) ₩ 0ver 12 12 83 3 X 4 III ယ် BY ATRAY AND TEL. THE BGG
) (M. K/M.SS) GALV. SIEEL APPLY
GH. POSITION FER BRANTHES 160A-Z.
A STAL ON THIS
SOLELY FOR THE TRUSS COMPOREM.
HIGHS IS THE RESPONSIBILITY OF THE 2 Supports ά 6 ىب 000 Wind reactions based on MWFRS pressures 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18 (**) 1 plate(s) require special positioning. Refer
plot details for special positioning requirements. BC attic room floor loading: LL = 40.00 psf; DL 7-6-12 to 19-10-4. 7.36.0424.12 8 15-4-10 4X12 III **8** X 8 ≡ 13-8-8 1,10,1 10X10 / 3 X 4≡ 12 1.5X4 9 15 R=1570 U=71 W=3.5" 3X8(A1) =SPACING BC DL TC DL TC LL DUR.FAC TOT.LD. FL/4/-/-/R/-40.0 10.0 20.0 24.0" 10.0 PSF 1.25 0.0 10.00 psf; from 0.1 PSF PSF PSF PSF scaled plate REF FROM SEQN-DATE JRFF-HC-ENG DF/DF DRW HCUSR8228 07267014 Scale = .25"/Ft. R8228- 40819 1TB18228Z01 49270 09/24/07 9 9

In 0C. Top chord 2x4 SP #2 Dense :T2 2x6 SP #2: Bot chord 2x6 SP #2 Webs 2x4 SP #3 Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50\,.$ 7-212--OWNER BUILDER Josh Campbell Haines City, FL 33844

'ificate' '' vizzatic' " '' lieu of structural panels use purlins to brace all flat TC @ TYP. ALPINE ₩ave **★** 2 · 5X5(A1) =V_ R=2256 U=485 W=3.5* 8 **IMPORTANT*** URWISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN FARLING TO BUILD THE TRUSS IN COMPORMANCE WITH PI. OR FARRICATION, HANDLING, SHAPPING, HISTALLING A BRACING OF FRUSSES.

DESIGN CONTRONES WITH APPLICABLE PROVISIONS OF DDS (MATIONAL DESIGN SPEC, BY AFATA) AND TPI. THE BCG CONNECTOR PLATES ARE HADE OF ZO/IB/JAGA, CHAIRSYN, ASH HASS AGRAIN AGS AGRAIN AGS AGRAIN AND THE RESPONSIBLE APPLY PLATES TO CALL FOR THATES AND DUES OF THIS DESIGN, POSITION PER DEAAHORS AND ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX AS OF THIS ZOLLY FOR THE THATS COMPONENT BESIGN SHAPPLY AND THE SECOND OF THE THAT SOLLY FOR THE THATS COMPONENT BESIGN SHAPPLY AND THE SULFACE OF THE THATS COMPONENT BESIGN SHAPPLY AND THE SULFACE AND THE THATS COMPONENT BESIGN SHAPPLY AND THE SULFACE AND THE THATS COMPONENT BESIGN SHAPPLY AND THE SULFACE AND THE THATS COMPONENT BESIGN SHAPPLY AND THE SULFACE AND THE THATS COMPONENT BESIGN SHAPPLY AND THE SULFACE AND THE THATS COMPONENT BESIGN SHAPPLY AND THE SULFACE AND THE THATS COMPONENT BESIGN SHAPPLY AND THE SULFACE AND THE THATS COMPONENT BESIGN SHAPPLY AND THE SULFACE AND THE THATS COMPONENT BESIGN SHAPPLY AND THE SULFACE AND THE THAT SHAPPLY AND THE SULFACE SHAPPLY AND THE SULFACE SHAPPLY AND THE SULFACE AND THE THAT SHAPPLY AND THE SULFACE SHAPPLY AND TH **HARNING** RUSSES REDUIRE EXTREME CARE IN FARRICATION. HANDLING, SHIPPING, HISTALLING AND BRACING. RETER TO BEST. (BUILDING COMPONENT SAFETY HEODRALION). PUBLISHED BY FIT (RUSS PLATE INSTITUTE, 210 HORNI HE STREET, SUITE 312, ALEXANDRIA, VA. Z2214) AND HEAVE (AND BUSSE CONNICLE OF ARREIGA. 6300 LINTERPISE LANE, HADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING INESE FUNCTIONS. HHEESS DIHERISE HOLGATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARIELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGHD CELLING. 0-0--0-0-386-984-6589-C Design Crit: 7X8= 3 X 4 Ⅲ TPI-2002(STD)/FBC н7в Cq/RT=1.00(1.25)/0(0)26-0-0 24" 12 $4 \times 4 \equiv$ 0ver 1.5X4 III 3 X 7 = 2-0-0 2 Supports 110 mph wind, 15.00 ft mean hgt, ASCE 7 02, PART._ENC. bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. lw=1.00 GCpi(+/-)=0.55 #1 hip supports 7-0-0 jacks with no webs Wind reactions based on MWFRS pressures ò 7.36.0424.12 3 X 4 III 7 X 8 ≢ 7-0-0 7-0-0 8 R=2256 U=485 W=3.5" BC LL BC DL TC DL DUR.FAC. TOT.LD. TC LL FL/4/-/-/R/-5X5(A1) = κ 2 40.0 1.25 10.0 PSF 10.0 PSF 20.0 PSF 0.0 PSF ¥_ PSF SEQN-DATE REF FROM HC-ENG DRW HCUSR8228 07267001 Scale = .25"/Ft. _10-0-R8228- 40820 DF / DF 49158 09/24/07

BUILDING DESIGNER PER ANSI/IPI 1 SEC. 2.

SPACING

SFF

ABOVE

JRFF-

1TB18228Z01

Fop Bot PLT TYP. In lieu of structural panels use purlins to brace all flat TC @ 0C. 7-212 OWNER BUILDER Josh Campbell Haines City, FL 33844 chord 2x4 SP #2 Dense chord 2x4 SP #2 Dense Webs 2x4 SP #3 ALPINE Wave K 2' V $2.5 \times 6 (B1) =$ R=1230 U=258 W=3.5 **IMPORTANT** FURBLEI A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, THG. SMALL HOT BE RESPONSIBLE FOR ANY OPENATION FROM THIS OFSIGN, ANY TAILURE TO BUILD THE TRUSS IN CONTRIBUNCE WITH PIL. OR FARRESTING, MANDIAGE, SHIPPING, INSTALLING AS MEADED FRUSSES.

BYSIGN CONTRIBUTES WITH APPLICABLE PROPERSONS OF THIS OSCILLA SPEECE SPEEC, BY AFREYA AND THE THE BEST OF THE AREA OF THE SPEECE SPEECE SPEECE OF THE SPEECE **WARNING** IRUSSES REQUIRE TXIRLUE CARE HI FARRICATION, HANDLING, SHIPPING, HISTAILIRG AND BRACING.

RETER TO BEST. (BUILDING COMPONEN) SAFETY HATOMANION, PUBLISHED BRY FET (TRUSS PLATE INSTITUTE, 219

HORH LEE SHEET, SHIE 312, ALIXANDHA, VA, 22314) AND HICA (MODD TRUSS COUNCIL OF AMERICA, 6300

ERHTREREISE LARE, MADISON, MI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING HIESE FUNCTIONS. UNITESS

OHNEMENSE HOLDSKALDE HOP FORMOS SHALL HAVE PROPERTY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE

A PROPERTY ATTACHED REGID CELLING. 8 ω 1.5X4// 9-0-0 9 0 - 0 -386 984 6589 C Design Crit: 9 4 X 4 ≡ TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/0(0) 3 X 7 ≡ B1) -26 24 " 0-0 3 X 4 ≡ Over 2 Supports 8-0-0 3 X 4 ≡ 0 0 Ô Wind reactions based on MWFRS pressures 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. iw=1.00 GCpi(+/-)=0.55 Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. 0 7.36.0424.12 3 X 7 = 4 X 4 == 1.5X4# 9-0-0 9-0-0 ω R=1230 U=258 W=3.5" BC LL BC DL TC DL TC LL DUR.FAC. SPACING TOT.LD. $2.5 \times 6 (B1) =$ FL/4/-/-/-/R/-8 V √ 5 24.0" 1.25 40.0 10.0 20.0 10.0 PSF 0.0 PSF PSF PSF PSF DATE REF JRFF-FROM SEQN-HC-ENG DF/DF DRW HCUSR8228 07267011 Scale = .25"/Ft. 0 R8228- 40821 1TB18778Z01 49163 09/24/07 9

Top chord 2x4 SP #2 Dense :T2, T3 2x8 SP Bot chord 2x8 SP SS :B2 2x4 SP #2 Dense: Webs 2x4 SP #3 P Collar tie braced with continuous lateral bracing at 24" rigid ceiling. End Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. D1 7 Haines City, FL 33844

Haines City, FL 37844

The Haines City of American Haines City of Haines verticals not exposed to wind 212 -- OWNER BUILDER Josh Campbell --TYP. ALPINE 20 Gauge HS, Wave 2-0-0 3 X 4 Ⅲ 4×6個 _W =1919PALIES DE EACH FACE OF TRUSS AND, UNLESS OHICRRISE LOCALED ON THIS DESIGN, POSTITI ANY TREVECTION OF PALIES ALCHOED BY (1) SWALL BY FER ANDEX AS OF THI 2002 SEC. A DRANTHE HIDICALLS ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY I DESIGN SHOWN THE SHITARILITY AND USE. OF THIS COMPONENT FOR ANY BUILDING IS THE BHILDING DESIGNER PER AUSLIPH I SEC. 2. **IMPORTANT***URBUSU A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, THC, SHALL NOT URL RESPONSIBLE FOR ANY DEVIATION FROM THIS PESSON, ANY FAILURE TO BRILD THE TRUSS IN COMPORTANCE WITH THE CORE CAMPLEATING. UNBILLING. INSTALLING & BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF MOS (MATIONAL DESIGN SPEC, BY AFRICA) AND THI.

COMMICTOR PLATES ARE MADE OF 20/18/18GA (H.H/SS/K) ASH A653 GRADE 40/40 (H.K/H.SS) GAV. SEEL, APPLY **WARNING** RUSSIS RIQUER EXERCIE CARE IN FABRICATION. HANDLING, SHIPPING, INSTALLING AND BRACING, RETER TO BEST. (RUHICING COMPOUND SAFELY INTORNATION). PUBLISHED BY IT! (RUSS PLATE INSTITUTE, 210 HORH LLE SHEEL, SHIEL 317, ALEXANDRA, VA, 22314) AND HICA (MODD TRUSS COUNCIL OF AMERICA, 6300 ELHIERPRISE LANE, MANISON, HI 53719) FOR SAFELY PRACTICES PRIOR TO PERFORMING HESE CHUCTIONS. DHUESS OTHERHIS, HOLGALID HOR LONDS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PABELS AND BOTTOM CHORD SHALL HAVE A PROPERTY ATTACHED STRUCTURAL PABELS AND BOTTOM CHORD SHALL HAVE A PROPERTY ATTACHED STRUCTURAL PABELS AND BOTTOM CHORD SHALL HAVE 0 = 9315 15 3 X 6 ≡ 10X10 / pressure W=3.5'10" 9-0-4X10(R) III 3 X 9 Ⅲ SS: 386-984-6589-C Design Crit: ω œ 6 6 ىٰ ώ ·22-7-0 Over 2 Supports ά ά TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/0(0) 3 X 4 ≡ C2 20 2-11-9 0 4X10(R) Ⅲ 1.5X4 III 12 12 HS512≡ ىٰ ά SOLELY FOR THE TRUSS COMPONENT NG IS THE RESPONSIBILITY OF THE 11-6-Y AFAPA) AND IPI. IIH BCG 3. K/H.SS) GALV. STEEL. APPLY , POSITION PER DRAHINGS 166A-Z. > SEC.3. A SEAL ON THIS SOFFY FOR THE FRAME COMMERCE 3-91 \Box 6 9 ٺ ىل ۵ 00 BC attic room floor loading: LL = 40.00 psf; $5 \cdot 1 \cdot 12$ to $17 \cdot 5 \cdot 4$. Wind reactions based on MWFRS pressures 110 mph wind, 15.00 ft mean hgt, ASCE 7–02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/-)=0.18 œ 7.36.0424.12 4X10(R) III 3X9 Ⅲ 9-0-7 17,10,1 3 X 6 ≡ 10X10 / 3-1-15 R=1919 U=93 W=3.5" 4 X 6 // ₩ L2-0-0-V 3 X 4 III BC DL BC LL SPACING DUR.FAC. TC DL IC LL TOT.LD. FL/4/-/-/-/R/-10 40.0 10.0 20.0 10.0 24.0" 1.25 0.0 10.00 psf; from PSF PSF PSF PSF PSF DATE REF JRFF-FROM SEQN-DRW HCUSR8228 07267005 HC-ENG Scale = .25"/Ft. R8228-1TB18228Z01 DF / DF 9 49056 09/24/07 -10 40822 12

Top Bot 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/-)=0.18 Wind reactions based on MWFRS pressures. PLT TYP. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. Collar-tie braced with continuous lateral bracing at 24" OC In lieu of structural panels use purlins to brace TC @ 24" 3 Haines City, FL 33844 chord 2x4 SP #2 Dense :T2, T3 2x8 SP chord 2x8 SP SS :B2 2x4 SP #2 Dense: Webs 2x4 SP #3 ficate ALPINE 20 Gauge HS, Wave L2 0 - 0 V 3 X 4 Ⅲ 4×6/ _ M **IMPORTANT** THRHISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE RIGG, INC. SHALL HOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. AND FAILURE FOR DULID THE BRISES HE COMPORMANCE MITH. PPL: OR FARREATHE, MANDLING, SHIPPHG, INSTALLING A BRACHE OF TRUSSES.

DISTAL COMPORTS WITH APPLICABLE PROVISIONS OF HOS (MALIGA & BRACHE) OF TRUSSES.

DISTAL COMPORTS WITH APPLICABLE PROVISIONS OF HOS (MALIGA BRACHE) OF HESSES, AND FAIL APPLY PARTS TO FACH FACE OF TRUSS AND. UNITSS OTHERWIST LOCATEO ON HIS DESIGN. POSITION FUR BRAHMES IGAN Z. ANY INSPECTION OF PLATES TOLOHOUR BY VIT SHALL HE FER ARMER AS OF FULL 2000 SEC. 3. AS ALOU HIS BRAHME HIDDORY SECTION OF THE STATABLE 3843 U=186 W=3.5 **WARNING** IRUSSES RIQUIRE EXTREME CARE IN FARRICATION, HANDLING, SHIPPING, HISTALLING AND BRACING METER TO BEST. (BUILDING COMPONENT SAFETY HUROMATION), PHOLSTED DRY TEL (TRUSS PLATE INSTITUTE, 21M HORSH LEE STREET, SHIFT 1375, AFEXNORAL, VA, 223-134) AND META, (MODO) TRUSS, COUNCIL OF AMERICA, 6300 ENTERCRAS LAND. AND SON, MI 557-19) FOR SAFETY PRACTICES PRIOR TO PETERBHING HUSE FUNCTIONS. HHLESS OFHERMENTS HOROMACHED HOW CHORD SHALL HAME PROPERLY ATTACHED STRUCTURAL PAHELS AND BOTTOM CHORD SHALL HAME PROPERLY ATTACHED STRUCTURAL PAHELS AND BOTTOM CHORD SHALL HAME. 3-1-15 -15 $3 \times 6 \equiv$ 10X10 / ['10" 9-0-7 4X10(R) III 3 X 9 Ⅲ SS: Design Crit: 11-3-8 6 6 ىٰ 22-7-0 Over 2 œ ά TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/0(0) 3 X 4 ≡ 2-11-9 0 4X10(R) Ⅲ 1.5X4 III 12 IN SPEC, BY AFRAY, AND IP!. IH BGG
CADIGO (H. KUH, SS) CALV. SITEL, APPLY
IS DESIGN, POSITION PER DRAWHRGS 160A-2,
IPPL 2002 SIC. 3, A SEAL ON THIS
IDILITY SOFTEY FOR HIE HASS COMPONENT
Y BUILDING IS THE RESPONSIBILLITY OF THE 12 HS512= ت ω Supports 1-6 9 | IJ 9 w ىب ω ω BC attic room floor loading: LL = 40.00 psf; $5 \cdot 1 \cdot 12$ to $17 \cdot 5 \cdot 4$. Nailing Schedule: (12d Common_(0.148"x3.25",_min.)_nails)
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @11.25" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting. Trusses to be spaced End verticals not exposed to wind pressure ω COMPLETE 7.36.0424.12 4X10(R) Ⅲ 3 X 9 III 9-0-7 ★ 10 1 3 X 6 ≡ 10X10/ TRUSSES 3-1-15 at 48.0" R=3843 U=186 W=3.5 4×6/ 00 L2-0-0V 3 X 4 III REQUIRED maximum BC LL BC DL TC LL SDACING TC DL DUR.FAC. TOT.LD. FL/4/-/ 40.0 10.0 10.0 20.0 18.0" 1.25 '-/R/-0.0 IJ 10.00 psf; from PSF PSF PSF PSF PSF DATE REF JRFF SEQN FROM HC-ENG DRW HCUSR8228 07267015 Scale = .25"/Ft. R8228- 40823 11B18228Z01 DF / DF 9 09/24/07 49061 -10--12

Deflection factor for Bot THE ROOF, FLOOR AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS. DIAPHRAGMS AND SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS ARE TO BE PROVIDED BY THE BUILDING DESIGNER. Note: All Plates Are 1.5X4 Except As Shown. PLT TYP. Fasten rated sheathing to one face of this frame Wind reactions SPECIAL LOADS t chord 2x4 SP t chord 2x8 SP Webs 2x4 SP -212--OWNER BUILDER BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF From From From From From From rom From From LUMBER ALPINE Wave meets L/240 live and L/180 total load. Creep increase dead load is 1.50. 240 240 40 10 128 128 128 128 128 10 40 DUR.FAC. 28 PLF at 28 PLF at based PLF PLF PLF ||2 Dense ||3 :M1, M13 a a a a 라 <u>a</u> t a ۵ 0n Josh Campbell 0.00 5.15 12.00 17.44 MWFRS pressures. . 25 **IMPORTANT**FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE NEG. THE C. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM HIS DESIGN, ANY FAILURE TO BHILD THE TRUSS IN COMPORMANCE HITH PI; OR LARREACHING, HANDLING, SHIPPIG, HISTALLING & BRACLING OF TRUSSES, OR AND FIT OF THE PI; OR LARREACHING THE PI; OR SHIPPICARLE PROVISIONS OF HIS SHIPPICARLE SPECE, BY ALLASS ONLY SHIPPICARLE APPLY THAT IS DESIGNATION FOR THE PI AND THE **MARNING** HRUSE'S REDIEM EXTREME CARE IN FABRICATION. HANDLING. SHIPPING, INSTALLING AND BRACING. BELLE TO BEST. (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY FIP (TRUSS PLATE INSTITUTE, ZIB UDDRING LEE STREET, SUNTE JEZ, ALEXANDRAL, NA, 22-214) AND WILLE STREET, SUNTE OF AMERICA. 6.300
FIRLEWRISE LAME, MADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INJECTION TO RECORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PAWELS AND BOITON CHORD SHALL HAVE A PROPERLY ATTACHED ATTACHED STRUCTURAL PAWELS AND BOITON CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PAWELS AND BOITON CHORD SHALL HAVE WANTS TO FARM FACE OF TRUSS AND, UNRESS OMMERISE LOCALED ON THE BESIGN, POSTI BRANING HOUTCATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY DRAWING HOUTCATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY .00 2×4 to tο 0.1 tο to to 0.1 to SP . DUR. 128 P' 128 F 240 240 128 40 128 0 12 PLF PLF Dense 386-984-6589-C а Design Crit: .=1.25) t 1.84 t 11.29 t 20.75 24.58 22.58 24.58 0.00 5.15 12.00 17.44 R=659 U=291 W=3.5" L2-0-0 THIS COMPONENT FOR 4X10(R) ₩ T#I-2002 (STD) /FBC C-GE Cq/RT=1.00(1.25)/0(0) 2-0-0 ANY BUILDING 3×4// 4X10 œ R=279 PLF U=30 PLF W=22-3-8 HE RESPONSIBILITY OF THE SEAL ON THIS 9 with 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18 (A) 1x4 with 8d Truss spaced at 48.0" OC designed to support 2 0 0 top chord must outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must See DWGS Al1015EE0207 & GBLLETIN0207 for more requirements In lieu of not be cut or notched. End verticals not exposed to wind pressure Trusses to be spaced at 48.0" OC maximum. 7.36.0424.12 #3 or better "L" brace. 80% length of web member. Attach Box or Gun (0.113"x2.5",min.)nails @ 6" OC. 22-7-0 Over 3 X 4 III structural panels use purlins to 4 X 4 = €X6= 2 (A)Supports 2X4 III QTY:1 3 X 4 III BC DL BC LL TC DL DUR.FAC. TC LL SPACING TOT.LD. FL/4/-/ brace 333 40 10.0 10.0 20.0 1.25 -/R/-0.0 ABOVF 0 TC @ 24" 0C PSF PSF PSF PSF PSF 3X4// 8 DATE REF JRFF-SEQN-FROM DRW HCUSR8228 07267007 HC-ENG 2-0-0 Scale = 4X10(R) / 4 X 10 (R) R8228 1TB18228Z01 AH M13 DF / DF 49129 09/24/07 . 25 " -/ft. 9 40824 o ė 0-0-

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. PLT TYP. Wave 7 212 -- OWNER BUILDER Josh Campbell --Haines City, FL 33844
The ificate of the internal incomponents Group, Inc. ALPINE 2X4(A1) =**IMPORTANT**FURBLE IO ACOPY OF THIS DESIGN TO THE INSTALLATIO CONTRACTOR. THE DESCRIPTION AND DEVALUE OF THE DESIGN TO THE INSTALLATION CONTRACTOR. THEN DESIGN THEN DESIGN THE TALLER OF BRILL TO BRILLD THE TRUSS IN COMPORMACE WITH THE OR LOWER CHICK, AND DESIGN CONTRACT THE ADVENTURE OF HIS ARCHARD AND THE OFFICE OF AFARY AND THE APPLICABLE PROPYSIONS OF HIS SUMITIONAL DESIGN SPEC. BY AFARY AND THE APPLICABLE PROPYSIONS OF HIS SUMITIONAL DESIGN SPEC. BY AFARY AND THE APPLICABLE APPLICABLE OF THE AFARY AND THE SPECIAL APPLICABLE OF THE APPLICABLE OF THE AFARY AND THE SPECIAL OF THE AFARY AND THE SPECIAL THE AFARY AND THE SPECIAL THE SPECIAL OF THE AFARY AND THE SPECIAL **HARNING** TRUSKIS BEQUIRE EXTREME CARE IN FABRICATION, IMABILIDE, SHEPPING, HISTALLING AND BRACING.
RETER TO BEST. (MULTICHIC COMPONENT SAFETY INFORMATION), PUBLISHED BY PET (TRUSK PLATE INSTITUTE, ZHAMBRIA, PA, ZZZIA) AND NICA (MOOD TRUSK SCHUREL OF AMERICA, 6300 ERHIERPISE LAHE, MADISON, NI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING HEST FUNCTIONS. UNLESS CHHICKLYELD HAD SON, NI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING HEST FUNCTIONS. UNLESS CHHICKLYELD HAD FORD STALL HAVE PROPERTY ATTACHED STRUCTURAL PARTLE AND BOTTON CHOOD STALL HAVE A PROPERTY ATTACHED REAL PROPERTY ATTACHED STRUCTURAL PARTLE AND BOTTON CHOOD STALL HAVE A PROPERTY ATTACHED RESIDENCE. MM =712 ω 99≔0 W = 3.5 " 386-984-6589-C Design Crit: 6 -6-10-0--10-0 13-8-0 TPI=2002(STD)/FBC Cq/RT=1.00(1.25)/0(0) D1) Over 2 1.5X4 III 4 X 4 = Supports 110 mph wind, 15.00 ft mean hgt, anywhere in roof, CAT II, EXP B, psf. Iw=1.00 GCpi(+/-)=0.18 Wind reactions based on MWFRS pressures. 6 - 10 - 0-6-10-0 8 7.36.0424.12 R=712 U=66 W=3.5" $2X4(A1) \equiv$ MD **←**2-0-0-> QTY:1 ASCE 7-02, CLOSED bldg, Located wind TC DL=5.0 psf, wind BC DL=5.0 $^{\circ}$ BC LL ВС TC DL DUR.FAC. TOT.LD. TC LL SPACING FL/4/-/-/-/R/-0 10-0-0 40.0 PSF 20.0 PSF 24.0" 1.25 10.0 PSF 10.0 PSF 0.0 PSF REF JRFF-DATE FROM SEQN-HC-ENG DRW HCUSR8228 07267002 Scale = .375"/Ft. R8228 40825 1TB18228Z01 DF / DF 09/24/07 49103

7-02, CLOSED bldg, Located TC DL=5.0 psf, wind BC DL=5.0

Top chord 2x4 SP Bot chord 2x4 SP Webs 2x4 SP #2 Dense #2 Dense #3

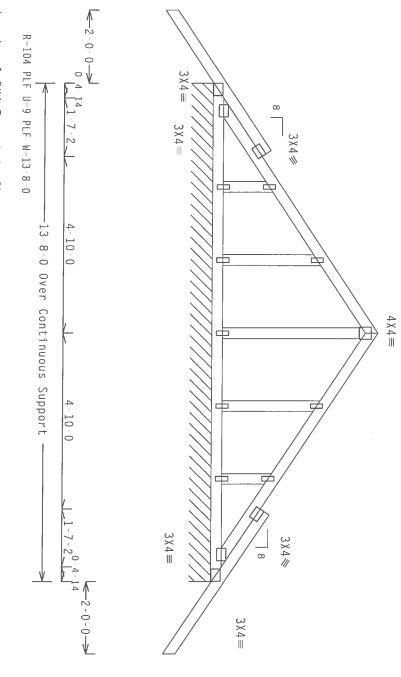
See DWGS A11015EE0207 & GBLLETIN0207 for more requirements

Wind reactions based on MWFRS pressures

110 mph wind, 15.00 ft mean hgt, ASCE anywhere in roof, CAT II, EXP B, wind psf. Iw=1.00 GCpi(+/-)=0.18

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50.\,$

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF, FLOOR AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS. AND SUPPORTING SHEAR WALLS. DIAPHRAGMS AND SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS ARE TO BE PROVIDED BY THE BUILDING DESIGNER.



→ 10-0-0

6-15

Note: All Plates Are 1.5X4 Except As Shown.

TYP.

Wave **WARNING** TRUSSES REQUIRE EXTREME CAME IN FABRICATION, HANDLING, SHIPPING, HISTAILING AND BRACING, REFER TO BEST (BUILDING COMPONEN SAFELY INFORMATION), PUBLISHED BY IP I (RRUSS PLATE INSTITUTE, 218 FORMILLET SHEET, SHIFT 132. ALEXANDRIA, VA, 72314) AND HICA (MODD IRUSS COUNCIL OF AMERICA, GODD ENTERPRISE LANE, MADISON, HI 53719) FOR SAFELY PRACITIES REFOR TO PERFORHING HIGST FUNCTIONS. UNILYSS OF THE PROPERTY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE Design Crit: TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/0(0)

7.36.0424.12

A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT Greater a copy of this design to the Installation contractor. The reg, inc. shall not be restouring the printing from this design, any lating to build the fails in componance with fire controlling simplying, simplying, and line as the fails of the fails in componance with fire to be said the fails of the fails of the fails of the fails of the fails in componance with fire controlling simply (mailour) (sign spec, by afra) and pt.

110 scale controlling this are that of the fails and the fail should design by fails only steel, apply plates to face fails and the fail face of the fails and the fails and the fails of the fails and the fails an

FI C-dificate

Haines City, FL 33844

"The inficate of the prization in form

ALPINE

SPACING DUR.FAC. 24.0" FROM JRFF-AH 1TB18228Z01

BC LL BC DL

0.0 PSF PSF

HC-ENG

DF/DF

TOT.LD.

40.0

SEQN-

49118

REV

TC DL

10.0 PSF 20.0

DATE REF

09/24/07

40826

10.0 PSF

DRW HCUSR8228 07267025

TC LL

PSF

FL/4/-/-/-/R/-

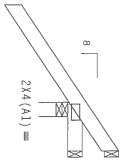
Scale =.375"/Ft. R8228-

Top chord 2x4 SP Bot chord 2x4 SP #2 Dense #2 Dense

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. lw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures



10 8 15

R R 118 U-93 32 U=31



Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/0(0)

7.36.0424.12

QTY:1

FL/4/-/-/R/-

Scale = .5"/Ft.

R8228 40828

BC DL

10.0 PSF 10.0 PSF 20.0 PSF

DRW HCUSR8228 07267023

TC DL TC LL

DATE REF

09/24/07

BC LL

0.0 PSF PSF

HC-ENG

DF/DF 49133

SPACING

24.0"

JRFF-

1TB18228Z01

DUR.FAC. TOT.LD.

40.0 1.25

SEQN-

FROM

ΤYΡ.

₩ave

WARNING IRUSSIS REQUIRE EXTREME CARE IN FARRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BAY FIT (ERUSS PLATE INSTITUTE, ZIB HORTH LEE STREIT, SHITE LIST, ALEXANDRIA, VA, 22314) AND HICA (MODD TRUSS COUNCIL OF AMERICA, 6300 ITHIGENEYS LANT, MADISHN, HI 53719) FOR SAFETY PRACTICES PRIOR TO PEFFORNING HUSE FUNCTIONS. DHLESS OHNERIST HORTSCHED BY CHARLES HAD SHALL HAVE PROPERTY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE PROPERTY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE PROPERTY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE

Haines City, FL 33844

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ALPINE

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 PLT TYP. Wave Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50\,.$ Hipjack supports 7-0-0 setback jacks with no webs 7-212--OWNER BUILDER Josh Campbell Haines City, FL 33844 ALPINE **IMPORTANT*** TURBISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BEG, INC. SHALL NOT DER RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN: ANY FAILURE TO BUILD THE REMSS IN COMPORMANCE WITH PP: OR LARREATION, AND DIG. SHEPPIG. HISALING A BRACHE OF TRUSSES; DESIGN SPEC. BY ATATA, AND THIS DESIGN CONTROTES PLAIS ARE ALOR OF 70/184/166A (M.14/58/), ASTH ACCE OF TRUSS. AND THIS DESIGN SPEC. BY ATATA, AND THE STORMALLY AND THE SPECIAL BY A SECOND SEC **#ARNIHOR** HUNSYS KHOHIRE LYBEME CARE IN CARRICATION, HANDLING, SHIPPING, INSTALLING AND BRACHG.
RETER TO BEST (BUILDING COMPOURIN SAFTY IN MORMATION), PUBLISHED BY FIT (TRUSS PLAIE INSTITUTE, 2000
HORRIL LEE STREIT, SUITE 317, ALEXANDRIA, VA. 2231) AND WICA (MODD TRUSS COUNCIL OF ARTERIA, 5300
EINERBRISE LAME, MADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THEST FUNCTIONS. UNITESS
OTHERBRISE LIDICATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE
A PROPERLY ATTACHED RIGID CETLING. -2-9-15 386-984-6589-C Design Crit: 2X4(A1) =MD =553 5.66 U=144 W=4.95" TPI-2002 (STD) /FBC HJ7) Cq/RT=1.00(1.25)/0(0) Ò ò -9-10-13 Over -14 -14 -10 - 131.5X4 III 3×4€ 3 Supports 110 mph wind, 15.00 ft mean hgt, anywhere in roof, CAT II, EXP B, psf. Iw=1.00 GCpi(+/-)=0.55 Wind reactions based on MWFRS pressures 7.36.0424.12 4 - 1 - 10 -15 $3 \times 4 \equiv$ 2<u>7</u> R=353 U=15 R=264 U=112 QTY:1ASCE 7-02, PART._ENC. bldg, Located wind TC DL=5.0 psf, wind BC DL=5.0 $\,$ BC LL BC DL TC DL TC LL DUR.FAC. TOT.LD. FL/4/-/-/-/R/-5 N 1.25 40.0 20.0 10.0 PSF 10.0 PSF **⊕**_10-0-0 0.0 PSF 14-8-4 PSF PSF DATE REF FROM SEQN-DRW HCUSR8228 07267020 HC-ENG Scale = .375"/Ft. R8228- 40829 DF/DF 49153 09/24/07

SPACING

SEE ABOVE

JRFF-

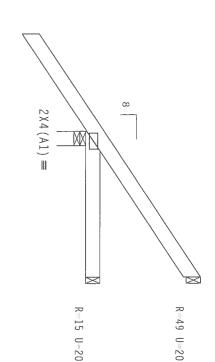
1TB18228Z01

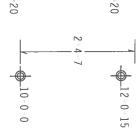
Bot chord 2x4 SP #2 Dense chord 2x4 SP #2 Dense

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures







Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/0(0)

7.36.0424.12

QTY:1

FL/4/-/-/-/R/-

BC LL BC DL

0.0 PSF

HC-ENG DF/DF

10.0 PSF 10.0 PSF

DRW HCUSR8228 07267022

TC DL

IC LL

20.0

PSF

REF DATE

R8228- 40830

09/24/07

Scale = .5"/Ft.

DUR.FAC.

TOT.LD.

40.0

PSF

49138

SPACING

24.0" 1.25

JRFF-FROM SEQN-

1TB18228Z01

PLT

TYP. Wave

MARNING RUSSYS REQUIRE EXTRINE EARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING, REFER TO BEST (BUILDING CHAMBULT SAFETY INFORMATION), PUBLISHED BY TP) (TRUSS PLATE INSTITUTE, 21B UNDER LES AREET, SHITE ATS. ALEXANDRIA, NA, 22314) AND MICKA (MODO TRUSS COUNCIL OF MARBICA. 6300 INHERRIY LANH, MADISON, H. 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING HESS FUNCTIONS. UNLESS OTHERWISE INDICATED TO PERMORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PAHELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PAHELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED ATTACHED STRUCTURAL PAHELS AND BOTTOM CHORD SHALL HAVE

IMPORTANTTURNISH A COPY OF HIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY ENVIATION FROM HIS DESIGN. ANY FAILURE TO BHILD HE TRUSS IN COMPORANCE WITH FPI; OR FARRICALING, MANDING, SHIPPING, HAYALLING A BRACHEG OF TRUSSES. DESIGN AND FPI. DESIGN COMPORTS WITH APPLICABLE PROPYSIONS OF HIS SECONDICTION FOR A FARRY AND FPI. THE BOYSIONS OF HIS SECONDICTION FOR A FARRY AND FPI. THE BOYSIONS OF HIS SECONDICTION FOR A FARRY AND FPI. THE BOYSIONS OF HIS SECONDICTION FOR A FARRY AND FPI. APPLY PLATES TO EACH FACE OF TRUSS, AND. HHILSS OTHERNISC LOCATED ON HIS DESIGN, POSITION FRE BRAHINGS 160A Z. ANY HISPECTION OF PALES FOLLOWID BY CT) SHALL BE FER ANIEX AS OF FPI 2002 SEC. 3. A SEAL ON THIS DESIGN AND FREE STATEMENT OF PROFESSIONAL ENGINEERING RESPONSIBILITY OR THE BRUSS COMPORENT DESIGN SHORM. HE SHIFABLLITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE

1

ALPINE

PLT Top chord 2x4 Bot chord 2x4 Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. Haines City, FL 33844

"Ificate " 'nizatic " " " 7-212--OWNER BUILDER Josh Campbell TYP. ALPINE Wave SP #2 Dense #2 Dense ** IMPORTANT** BURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE MEG. THC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. FOR FAILURE TO BUILD THE TRUES IN COMPORMANCE WITH FP: OR TANK TOACHTO. THE PROVISIONS OF HIS SECLED OF TRUSSES.

DESIGN CONTROLLES AND THE PROVISIONS OF HIS GRAIDONAL DESIGN SPEEL BY ATAPA) AND THE COMMICTION PLATES AND THE PROVISIONS OF HIS GRAIDONAL DESIGN SPEEL, BY ATAPA) AND THE SECLE. APPLY PLATES TO EACH FACE OF TRUSS AND. UNLESS OBJECTA SPEEL BOAD TO FOR U.K.M., SS) GALV. SITEL. APPLY PLATES TO EACH FACE OF TRUSS AND. UNLESS OBJECTANCE LOCATION OF HIS DESIGN, POSITION FER DRAWHINGS 160A 2. ANY INSPECTION OF PLATES TOLLOWED BY C1) SHALL BE FER AIMER X 3 OF FPIT 2002 SEC. 3. AS S.A. ON HITS DESIGN SHOULD BE SECREPARCE OF POSITIONAL CHARLES HER REPORTSHILLTY SOLITY ON THE TRUSS COMPONENT OF THE SOURCE SHOWN. THE SUITABLETTY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE RUSPONSIBILITY OF THE RUSPONSIBILITY OF THE SUITABLE PROVIDED OF THE SUITABLE PROVI **MARNING** HRISES, BEDURK LYRENE CARE IN LARRIGATION, HANDLING, SHIPPING, INSTALLING AND BRACING. RELLA TO BEST (RULLDING COMPORED) SAFLY HEROMATION, PUBLISHED BY TEL (1805 FLAIE INSTITUTE, 218 MOUTH LEE SHREE, SHITE 312, ALLXANDRIA, VA. 22313) AND MICA (MODO TRUSS COUNCIL O MERICA, 6300 CHICENES) LANE, MADSSON, HI 53719) FOR SAFLY PRACTICES PRIOR TO PREFERRING HIGHEST FUNCTIONS. HHEESS OTHERNIST HOUGHAUTH OF MERICA, 1807 BY A PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE -2-0-0-386 984 6589 C Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/0(0) 2X4(A1) =MI 8 387 U-47 W-3.5" -5-0-0 Over д5 3 Supports Wind reactions based on MWFRS pressures 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55 7.36.0424.12 R-48 U-18 R-124 U-44 ىب 8 •13-4-15 QTY:1 10-0-0 BC DL BC LL TC DL SPACING DUR.FAC. TC LL TOT.LD. FL/4/-/-/R/-ממוז מובה זיוו מו לבמוחמ פ מזוורוומזמנומ! מממוזוובת מו וממסס ווועו 24.0" 1.25 40.0 10.0 PSF 10.0 PSF 20.0 PSF 0.0 PSF PSF DATE REF JRFF-FROM SEQN-HC-ENG DRW HCUSR8228 07267021 Scale = .5"/Ft. R8228- 40831 1TB18228Z01 АН DF / DF 09/24/07 49142

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. Top chord 2x4 SP Bot chord 2x4 SP 7 212 OWNER BUILDER Josh Campbell 2 Haines City, FL 33844

History FL 33844

Finance City, FL 33844 TYP. ALPINE Wave #2 Dense **IMPORTANT**FIRMISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BEGG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN FOR TALLER TO BHILD THE BRASS IN COMPORANCE WITH PILE OR FARRECATHG. HANDLING, SUPPLIE, INSTALLING A BRACEING OF TRUSSES.

BYSIGH COMPORIS WITH APPLICABLE PROVISIONS OF HIDS (MALIDRAM DESIGN SPEE, BY ATAPA) AND IPT. ITH BEGCCOMMICTION PALES ARE TO POLITICALLY APPLY PALES TO EACH FACE OF TRUSS, AND. HIMESON DESIGN FOR THIS DESIGN POSITION FOR BRAHINGS ISON-Z. PALES OF THIS DESIGN POSITION PALES FOLIOHOR BY CONTROL FOR PALES AND THIS DESIGN POSITION PALES FOLIOHORS BY CONTROL FOR PALES AND THE STATE OF THE TRUSS COMPORTED BY THE THIS DESIGN FOR THE TRUSS COMPORTED THE THIS DESIGN FOR THE THI **#ARRING** IRUSCES REQUIRE LYKREME CARE IN FARRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. RELIER TO BEST. (BULEDING COMPONENT SALETY INFORMATION), PUBLISHOD BY FPI (TRUSS PLATE INSTITUTE, 218 NORTH LE SIREE, SUITE 312, ALEXANDRIA, NA, 25214) ARD METAC (HOOD TRUSS COUNCIL OF AMERICA. 6300 INLERRISE LAME, MADISON, H. 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING HEESE FUNCTIONS. DUICESS OTHERWISE HOLGALID INC EUROPE SHALL HAVE PROPERTY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE A PROPERTY ATTACHED REGID CETALING. 2X4(A1) =386 984 6589 C Design Crit: \overline{z} MM 461 œ U-52 W-3.5" -7-0-0 Over TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/0(0) EJ7) 3 Supports Wind reactions based on MWFRS pressures 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL-5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55 7.36.0424.12 R 188 U 65 R-77 U-28 5 0 10-0-0 **→**14 8 15 BC LL BC DL TC DL TC LL SPACING DUR.FAC TOT.LD. FL/4/-/-/R/-40.0 20.0 24.0" 10.0 PSF 10.0 PSF 1.25 0.0 PSF PSF PSF JRFF-DATE REF SEQN-FROM HC-ENG DRW HCUSR8228 07267024 Scale = .375"/Ft. R8228- 40832 1TB18778Z01 DF / DF 49146 09/24/07

Top Bot chord 2x4 SP #2 Dense :T2 2x8 SP SS: chord 2x8 SP SS Webs 2x4 SP #3

End verticals not exposed to wind pressure

Collar tie braced with continuous lateral bracing at 24" rigid ceiling.

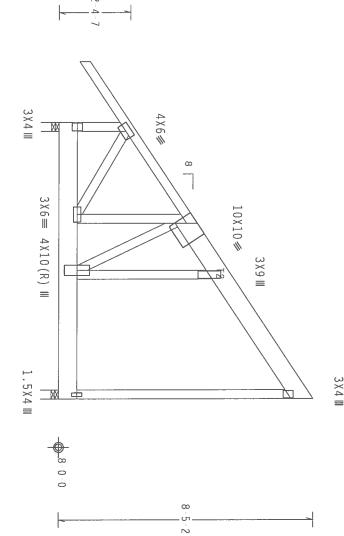
00

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED within 4.50 ft from roof edge, CAT II, EXP B, wind wind BC DL=5.0 psf. IW=1.00 GCpi(+/-)=0.18 bldg, not located TC DL=5.0 psf,

Wind reactions based on MWFRS pressures

BC attic room floor loading: LL = 40.00 psf; 5-1-12 to 8-9-8. DL 11 10.00 psf; from

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50\,.$





Haines City, FL 33844
F' 'Italia (''' rizatio "'' ALPINE **IMPORTANT**FURMISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SMALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN COMPORMANCE WITH FPI; OR FARRE CAPITAG, MANDILIG, SHEPPIG, HISTALLING A BRACING OF TRUSSES, DESIGN CONTROLS WITH APPLICANCE PROVISIONS OF DDS (MATIONAL DESIGN SECE, BY AFENA) AND TPI. IN BCG CONNECTOR PRAILS ARE HODE OF 20/10/1666 (H.M./SS/R) ASIM A653 GRADE 40/60 (H.K./M.SS) GALV. SILELL APPLY PLAILS TO CAULT CACE OF TRUSS AND. UNICES OHIERISE LOCALED ON THIS DESIGN, POSITION PER DRAHENS IGOA.Z. ANY HISPECTION OF PLAIES FOLLOWED BY (I) SMALL HE PER ANNEX AS OF THIS 2002 SEC. 3. A SEAL ON THIS BRAING, INDICALES, ACCEPTANCE OF PROFESSIONAL INGUITERING RESPONSIBILITY SOLELY FOR HE TRUSS COMPONENT DESIGNATION OF THE CONTROLS ASSESSMENT OF THE CONTROL OF THE CONTROL OF THE CONTROLS ASSESSMENT OF THE CONTROL OF THE CO **WARNING** INUSSES RIQUIRE CENTRE CARE IN FARRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING.
METER TO BEST (BUILDING COMPORED) SAFETY INFORMATION, PUBLISHED BY TPT (FRISS PLATE INSTITUTE, 218
MORTH (IT STREEL, SUITE 312, ALEXANDRA, VA, 22314) AND MICA (MOOD TRISS COUNCIL OF AMERICA, 6300
EURERPEISE LANE, MADISON, MI 53715) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS
OHIGHNISE HOLDENALDED THE CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE
A PROPERLY ATTACHED RIGID CELLING.

BC DL

10.0 PSF 10.0 PSF

DRW HCUSR8228 07267008

TC DL TC LL

BC LL

0.0 PSF

HC-ENG

DF/DF

.

PSF

SEQN-

49099

SPACING

24.0" 1.25 40.

JRFF-FROM

1TB18228Z01

AH

DUR.FAC. TOT.LD. PLT TYP.

Wave

Design Crit:

TPI-2002 (STD) /FBC

Cq/RT=1.00(1.25)/0(0)

7.36.0424.12

FL/4/=/-/-/R/-

20.0

PSF

REF

Scale = .3125"/Ft. R8228- 40833

DATE

09/24/07

BUILDING DESIGNER PER ANSI/IPI 1 SEC. 2.

l op Bot End PLT TYP. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. 7-212--OWNER BUILDER Josh Campbell)) Haines City, FL 33844
F' 'Incate e' rizatio verticals not exposed to wind pressure. chord 2x4 SP #2 Dense chord 2x4 SP #2 Dense Webs 2x4 SP #3 ALPINE Wave **IMPORTANT**FURBLE IA COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG. THE SHALL HOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY TALLINE TO BUILD THE TRUSS IN COMPORMANCE WITH PPI; OR FARELETHING, ANNOLING. SHIPPIG., HISALLING A BRACKLING OF TRUSSES.

DESIGN CONTRINS WITH APPLICABLE PROPERSIONS OF THIS SHALLING A SHEAR PROPERSION SPEC. BY ATARA) AND TPI. HIT BCC CONTRICTOR PLATES ARE HADE OF ZO/EU/BCAGA (M. 1955X) ASHA ASSO GRADE 40/60 (M. ZHL.SS) GAV STEEL, APPLY PLATES TO EACH FACE OF TRUSS, AND. HIMLES OTHERNISE LOCATED ON HIS DESIGN, POSITION FOR BRAHMES 150A. Z.

ANY INSPECTION OF PLATES TOLCHOURD BY CT) SHALLE EFFR ANITY AS OF TPI. 7007 SEC. 3.

AS SEA ON THIS DESIGNED, ACCEPTANCE OF PROPESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN.

HIS SULFABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE **MARNING** RUSSYS KRONINE EXTREME CARE IN FARRICATION, IMADEING, SHIPPING, INSTALLING AND BRACING, BELLER DE REST. (BULDING CHOPPONENT SALTY INFORMATION), PUBLISHED BY THE (RUSS PLATE INSTITUTE, 218 HORRILLE SIREET, SULIE, IZ, ALEXANDRIA, VA. ZZZIA) AND HICA (HODD TRUSS COUNCIL OF AMERICA, 6300 CHICEPRISE LANE, IMDISON, HI 53/19) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATION OF ORDER SAME HAVE PROPERLY ATTACHED STRUCTURAL PARECS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CELLING. -2-0-0-386-984-6589-C Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/0(0) 1.5X4 III 3X4# R=376 W=3.5" 5-0-4 6 Over 4-10-8 00 2 Supports -12 ά R=184 U=79 $3 \times 4 \equiv$ 1.5X4 III 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18 Wind reactions based on MWFRS pressures 7.36.0424.12 8-0-0 G -10 BC DL TC DL SDACING TC LL DUR.FAC. TOT.LD. FL/4/-/-/R/-24.0" 40.0 20.0 10.0 PSF 10.0 PSF 0.0 PSF PSF PSF JRFF-FROM SEQN-DATE REF HC-ENG DRW HCUSR8228 07267009 Scale = .5"/Ft. R8228- 40834 11B18228201 DF / DF 49095 09/24/07

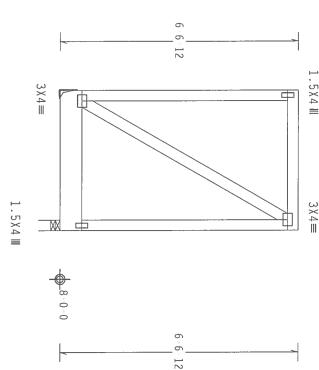
212 OWNER BUILDER Josh Campbell 386 984 6589 C C7

Top Bot chord 2x4 SP #2 Dense chord 2x8 SP #1 Dense Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. lw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures

Truss must be installed as shown with top chord ф



End verticals not exposed to wind pressure

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

The TC of this truss shall be braced with attached spans at 24" in lieu of structural sheathing.

THE BUILDING DESIGNER SHALL EVALUATE AND APPROVE LOAD MAGNITUDES AND LOCATIONS AS SHOWN ("SPECIAL LOADS").TRUSS ENGINEER & FABRICATOR ARE NOT RESPONSIBLE FOR LOAD MAGNITUDES AND LOCATIONS

Haines City, FL 33844
Ft Carificate of Authorization we for TYP. ALPINE Wave **IMPORTANT***| UBRISH A COPY OF THIS DESIGN TO THE THISTALLATION CONTRACTOR. THE BCG. THE SHALL HOT BE RESPONSIBLE FOR ANY DEPLATION FROM THIS DESIGN. ANY FAILURE TO BRILLE TO BRILLE HE RUSS IN COMPORMANCE WITH PHYSICAL REAL HOTELS ARE HADDELING. SHEPPING. IFSTALLING A BRACHE OF TRUSSES.

DISIGN CONFORM SHIP APPLICABLE PROVISIONS OF HOS (MALIDAL DESIGN REEC, BY ATRAY) AND PH. THE ROCCOMPLICION FAILURE AND HITESS AND SHIPSSAY, ASTH ASS) GADE ALOO, HE KENTESS GAVE. SIFTL, APPLY AND LIST OF ANTESS AND HITESS OF ANY INSPECTION OF PLATES FOLLING BY (1) SAUL HE FIR AND SHIPS DESIGN POSITION PER BRAHINGS HOA Z. ANY INSPECTION OF PLATES FOLLING BY (1) SAUL HE FIR AND SHIPT TOOK SEG. ANY INSPECTION OF PLATES FOLLING BY (1) SAUL HE FIR AND SHIPT TOOK SEG. ANY INSPECTION OF PLATES FOLLING BY (1) SAUL HE FIR AND SHIPT TOOK SEG. THE RUSS COMPONENT HE SHIPT SHIPT TOOK SEG. BY ALL HE FIR AND SHIPT SHI **WARNING** INJECTE BEQUIRE CARETHE CARE IN FARRICATION, FROMELISHED BY THE (TRUSS PLATE ING AND BRACHGE, REFER TO BESS! (BUILDING COMPONENT SAFETY INFOMENTION), PROFESSIONED BY THE (TRUSS PLATE INSTITUTE, 728 MORTH LE STREE, SUITE 317. ALEXANDRIA, VA. ZZ314) AND WICA (MOND TRUSS COUNCIL OF AMERICA, 6300 ERRESSENDRIANE, MADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDIVIALED TO PROBE SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE Design Crit: TPI=2002(STD)/FBC Cq/RT=1.00(1.25)/0(0) 7.36.0424.12 QTY:1 BC DL SPACING BC LL TC DL TC LL DUR.FAC. TOT.LD. FL/4/=/ 20.0 1.25 40. 10.0 10.0 PSF -/R/-0.0 . PSF PSF PSF PSF DATE REF FROM SEQN HC-ENG DRW HCUSR8228 07267006 Scale = .375"/Ft. R8228- 40835 DF / DF 09/24/07 49090

24.0"

JREF-

1.TB18228Z01

3 10-0

3-6-0ver 2

2 8

Supports

8 4

244

 \overline{z}

-246 U-57 W-3.5"

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED within 4.50 ft from roof edge, CAT II, EXP B, wind wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

End verticals not exposed to wind pressure.

due Calculated horizontal deflection is 0.15" due to dead load. 0 live load and 0.45

(A) Continuous lateral bracing equally spaced on

Collar tie braced with continuous lateral bracing rigid ceiling. at 24" 0C. 9

2 X 4 III Deflection meets L/240 live and L/180 total load. Creep factor for dead load is 1.50.

SPECIAL LOADS

From

(LUMBER

DUR.FAC

PLATE

DUR.FAC

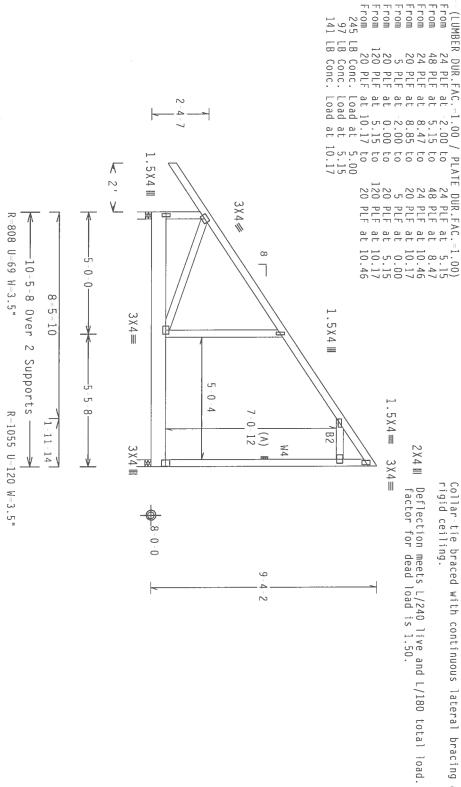
24 48

at

From From

to ťο 0.1 THE BUILDING DESIGNER SHALL EVALUATE AND APPROVE LOAD MAGNITUDES AND LOCATIONS AS SHOWN ("SPECIAL LOADS").TRUSS ENGINEER & FABRICATOR ARE NOT RESPONSIBLE FOR LOAD MAGNITUDES AND LOCATIONS

Wind reactions based on MWFRS pressures



MARNING HUNSYS REQUIRE EXTREMÉ CARE IN FARRICATION, HANDING, SHIPPING, INSTALLING AND BRACING. REFER TO REST (BUILDING COMPOURLY SAFETY INVENTION), PUBLISHED BY TPI (TRUSS PLAIE INSTITUTE, ZIB MORTH LEE STREET, SUITE 317, ALEXANDRÍA, VA. ZZIJA) AND MICA (HODD TRUSS COUNCIL OF MATRICA. 6300 CHILERRUSE, LAME, MADISON, H. 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNIESS COUNCIL OF MATRICA. 6300 CHILERRUSE INDICATOR TO DE CONTROL SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARILES AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGHD CHORD SHALL HAVE 7.36.0424.12

Design Crit:

TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/0(0)

TYP.

Wave

IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE REG. THE SHALL HOT OUR RESPONSIBLE FOR ANY DEVIATION FROM HILS DESIGN. ANY FALLER FOR SHILL HE RUSS IN COMFORMANCE WITH FPL. OR FLABERCALING. JUNIUM LIGG. SUPPINE, LISTALLING A BRACLING OF ENISSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF MUS (MATIONAL DESIGN SPEC, BY AFRA) AND FPL.

CONNECTOR PLAIES ARE HADE OF POPUNITIONS OF MUS (MATIONAL DESIGN SPEC, BY AFRA) AND FPL.

PA ANY HISTS OF LACIT FACE OF TRUSS AND. MULESS OHIGHNEYSE (DOCATED ON THIS OTSIGN, POSITION PER DRAMBINGS HOAZ ANY HISTS OF LACIT FACE OF TRUSS AND MULESS OF THE AMPLEX AND THE SOFT AND THE STANDARD OF THE RUSS COMPONENT OF PARTS FOLLOWED BY (1) SHALL BE FER AMPLEX AS OF FPL-2002 SEC.3.

ANY HISTSCHION OF PARTS FOLLOWED BY (1) SHALL BE FER AMPLEX AS OF FPL-2002 SEC.3.

A SEAN ON THIS DESIGN SHOWN. THE SHITABLLITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER FER AMSIJAPL I SEC. 2. OF MUS (MATIONAL DESIGN SPEC, BY ATRA) AND DEL.

HYSSYN ASTH AGS3 GRADE 40/60 (H. K/H.SS) GALVE STELL APPLY

SHURRISE LOCALED ON DITS DESIGN, POSITION PER DRAWHIGS 160A Z.

A SEAL ON THIS

MALL BE FER ANKEX AS OF TPIT 7002 SEC.3. A SEAL ON THIS

MALL MOLITER HUR EXPONISHILLY SOLETY OR HE NUSS COMPORATO

THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILLITY OF THE

> BC LL BC DL TC

PSF

DF / DF 49279

C

DL

10.0 20.0

PSF

DATE REF

09/24/07 40836

07267003

PSF

10.0 PSF 0.0

FL/4/-/-/-/R/-

Scale = .25"/Ft.

R8228-

SPACING DUR.FAC. TOT.LD.

SFF

ABOVE

JRFF-

1TB18228Z01

1.25 40

FROM

Æ

0

PSF

SEQN-HC-ENG DRW HCUSR8228

Haines City, FL 33844
Ft Constitute of Authorization at a constitution of the constitution at a constitution of the constituti

ALPINE

212 OWNER BUILDER Josh Campbell --386-984-6589-C C4)

Top chord 2x4 SP #2 Dense chord 2x8 SP SS :B2 2x4 SP Webs 2x4 SP #3 :W4 2x4 SP

#2 Dense: #2 Dense:

End

verticals not exposed to wind

pressure.

Calculated horizontal deflection is 0.21" due to live load and 0.34" due to dead load.

BC attic room floor loading: LL = 40.00 psf; DL = $5 \cdot 1 \cdot 12 \cdot to \cdot 10 \cdot 2 \cdot 0$. 10.00 psf; from

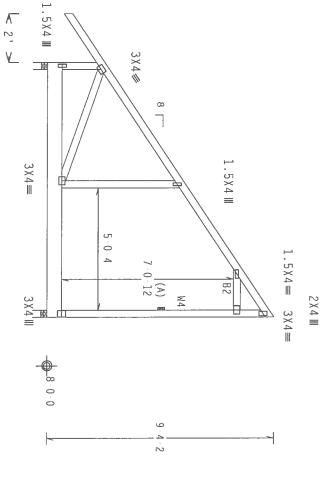
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi (+/-)=0.18

Wind reactions based on MWFRS pressures.

(A) Continuous lateral bracing equally spaced on member.

Collar-tie braced with continuous lateral bracing at 24" rigid ceiling.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50\,.$





-0-0-

5

MARNING IRUSES REQUIRE FXTRETHE CARE IN FABRICATION, INAMBLING, SHIPPING, INSTALLING AND BRACING, REFER TO BEST (BUILDING COMPONENT SAFETY HERBRACING), PUBLISHED BY THE (TRUSS PLATE INSTITUTE, ZIB HORTH LEE STREET, SUHIE JIZ ALEXANDRIA, NA, 22313) AND MICHA (MODO TRUSS COUNCIL OF ANH REA. 6300 CHITERPRIS LAME, HADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNITESS OFFICENCY TO THE CONTROL OF THE CONTROL Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/0(0)7.36.0424.12

QTY:1

FL/4/-/-/-/R/

20.0 10.0 PSF

PSF

REF

R8228- 40837

Scale = .25"/Ft.

DATE

09/24/07

BC DL TC DL 10 11

10.0 PSF

DRW HCUSR8228 07267019

PLT

TYP.

Wave

Haines City, FL 33844 **IMPORTANT**TURBLISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVALUOUS FOR THE SESSION AS THE ASSESSION OF THE SESSION OF TH

2

ALPINE

SPACING

24.0"

JRFF-FROM SEQN-

1TB18228Z01

DUR.FAC TOT.LD.

1.25

40.0

0.0

PSF PSF

HC-ENG

DF / DF 49287

ASCE 7-02: 110 MPH WIND SPEED, 15 MEAN HEIGHT, ENCLOSED, 1.00, **EXPOSURE** \bigcirc

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		1	2	,,		0	. (J.			1	6	,,		0	. (7.			2	4	"		0	. (<u> </u>		SPACING	CABI
			j	υ. '\)	TIT	I I	UTI	<u> </u>			1	ζ. '\)	TTT	口 叮	UTI	Ω] .j		<u> </u>		<u></u>)	TIT		U U U	2	SPACING SPECIES	2X4
	STANDARD	STUD	#3	#2	#1	STANDARD	STUD	#3	#1 / #2	STANDARD	STUD	#3	#2	#1	STANDARD	STUD	#3	#1 / #2	STANDARD	STUD	#3	#2	#1	STANDARD	STUD	#3	#1 / #2	GRADE	BRACE
	4' 11"	5' 0"	"	5, 3 <u>"</u>	5,	4' 9"	4' 9"	4' 9"	4' 11"	4' 5"	4' 6"	4' 6"	4' 9"	4' 10"	4' 4"	4' 4"	4' 4"	4' 5"	3' 10"	4' 0"	4'0"	4' 2"	4' 3"	3' 9"	3' 9"	3' 9"	3' 10"	BRACES	NO.
	7' 5"	8 5"	8, 5,	8, 5,	8	7' 3"	8, 5,	8, 2,	8' 5"	6, 5,	7' 6"	7' 7"	7' 8"	7' 8"	6' 4"	7' 4"	7' 4"	7' 8"	5' 3"	6' 1"	6' 2"	6'8"	6' 8"	5' 2"	6' 0"	6'0"	6' 8"	GROUP A	(1) 1X4 "L"
	7' 5"	8' 7"	8' 5"	9' 1"	9' 1"	7' 3"	8, 5,	8, 2,	8' 8"	6' 5"	7' 6"	7' 7"	8' 3"	8' 3"	6' 4"	7' 4"	7' 4"	7' 10"	5' 3"	6'1"	6' 2"	7' 2"	7' 2"	5' 2"	6'0"	6'0"	6' 10"	GROUP B	" BRACE *
	9' 10"	10, 0,	10' 0"	10' 0"	10' 0"	9' 7"	10' 0"	10' 0"	10' 0"	8' 6"	9' 1"	9' 1"	9' 1"	9' 1"	8' 4"	9' 1"	9' 1"	9' 1"	6' 11"	7' 11"	7' 11"	7' 11"	7' 11"	6' 9"	7' 11"	7' 11"	7' 11"	GROUP A	(1) 2X4 "L"
S MMAS	9' 10"	10' 6"	10' 6"	10′9″	10' 9"	9' 7"	10' 0"	10' 0"	10' 3"	8' 6"	9' 6"	9' 6"	9' 9"	9' 9"	8' 4"	9' 1"	9' 1"	9' 4"	6' 11"	8'0"	8' 1"	8' 6"	8' 6"	6' 9"	7' 11"	7' 11"	8' 1"	GROUP B	" BRACE *
₹. ¥.	11' 11"	11' 11"	11' 11"	11' 11"	11' 11"	11' 11"	11' 11"	11' 11"	11' 11"	10' 10"	10' 10"	10' 10"	10' 10"	10' 10"	10' 10"	10' 10"	10' 10"	10' 10"	9' 4"	9' 5"	9′ 5″	9' 5"	9' 5"	9' 1"	9' 5"	9' 5"	9' 5"	GROUP A	(2) 2X4 "L"
	12' 3"	12' 6"	12' 6"	12' 10"	12' 10"	11' 11"	11' 11"	11' 11"	12' 3"	11' 1"	11' 4"	11' 4"	11' 8"	11' 8"	10' 10"	10' 10"	10' 10"	11' 1"	9' 4"	9' 11"	9' 11"	10' 2"	10' 2"	9' 1"	9' 5"	9' 5"	9' 8"	GROUP B	" BRACE **
	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"	12' 11"	14' 0"	14' 0"	14' 0"		12' 5"	- 1		12' 5"	- 1	12' 3"	12' 4"	12' 5"	GROUP A	(1) 2X6 "L"
	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14'0"	14' 0"	14' 0"	14' 0"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"	12' 11"		14' 0"	14' 0"		12' 6"		13' 5"			12' 3"	12' 4"	12' 9"	GROUP B	BRACE *
	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		14' 0"			14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		14' 0"	14' 0"		14' 0"		14' 0"	14' 0"	14' 0"	GROUP A	(2) 2X6 "L"
	14' 0"	14' 0"	14' 0"	14' 0"	14′0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14′0″	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	GROUP B	BRACE **

DOUGLAS FIR-LARCH

SOUTHERN PINE #3

STANDARD

STANDARD

aurs

GROUP B: HEM-FIR #1 & BTR #1

SPRUCE-PINE-FIR
#1 / #2 STANDARD

HEM-FIR STUD

#3

STUD

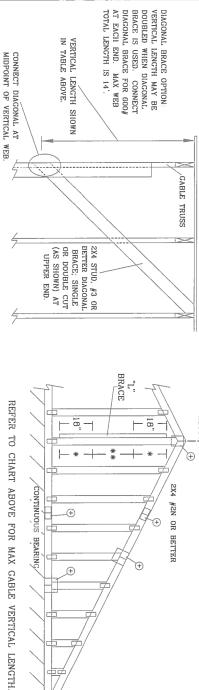
#3

STANDARD

BRACING GROUP SPECIES AND GRADES:

GROUP

A:



	ABOUT! L. * - * - * - + + - +	JI MMAS
CONTINUOUS BEARING	2X4 #2N OR BETTER	
RING	⊕ // ⊕	

GABLE TRUSS DETAIL NOTES:

SOUTHERN PINE

DOUGLAS FIR-LARCH

#2

#2

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG. PROVIDE UPLIFT CONNECTIONS FOR 80 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD). LIVE LOAD DEFLECTION CRITERIA IS L/240.

ATTACH EACH "L" BRACE WITH IOD NAILS.

FOR (I) "L" BRACE: SPACE NAILS AT 2" O.C.

IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.

** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C.

IN 18" END ZONES AND 6" O.C. BETWEEN ZONES. MEMBER LENGTH. "L" BRACING MUST BE A MINIMUM OF 80% OF WEB

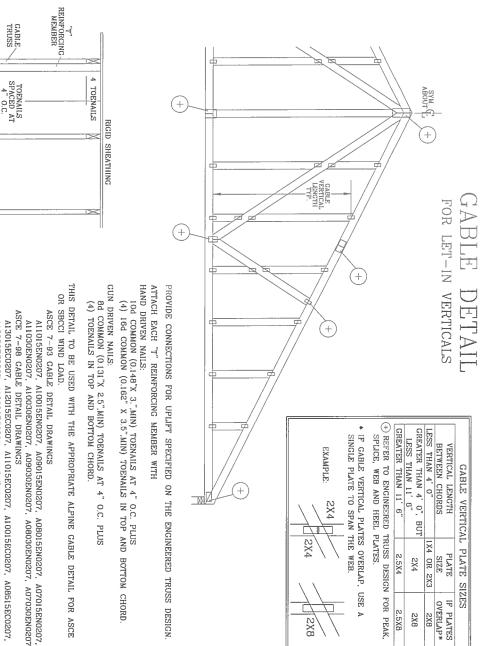
SROUP, INC.	
######################################	**ALVARIHIG** TRUSSES REQUIRE EXTREME CARE IN FABRICATHIG, HANDLING, SHIPPING, INSTALLING AND BRACING, REFER TO BESS (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TRI CRUSS PLATE INSTITUTE, 218 NORTH LEE STR. SUITE 32, ALEXANDRIA, VA. 22314) AND WTGA VYDD TRUSS CODANCIL OF AHERICA, 6300 ENTERPRISE LN, HADISON, VI 537199 FOR SAFETY PRACTICES PRIDE TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHARD SHALL HAVE PRIPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE PRIPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL
Sep	
24	
Sep 24 '07	

ITW BUILDING COMPONENTS GF POMPANO BEACH, FLORID

ALPINE

 Λ

MAX.	MAX.				
MAX. SPACING 24.0"	MAX. TOT. LD. 60 PSF				
ING	LD.				
24	60				
1.0"	PSF				
		-ENG	DRWG	DATE	REF
			DRWG A11015EE0207	DATE 2/23/07	ASCE7-02-GAB11015



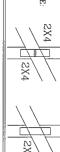
SPIJCE WEB AND HEEL PLATES	+) REFER TO ENGINEERED TRUSS DESIGN FOR PEAK,	GREATER THAN 11' 6"	LESS THAN 11' 6"	LESS THAN 4' 0"	BETWEEN CHORDS	VERTICAL LENGTH	GABLE VERTICAL PLATE SIZES
PLATES	TRUSS DESIGN	2.5X4	2X4	1X4 OR 2X3	SIZE	PLATE	L PLATE SE
	FOR PEAK,	2.5X8	2X8	2X8	OVERLAP*	IF PLATES	ZES

TOENAIL

TOENAIL

2X4 "T" REINFORCING MEMBER

2X6 "T" REINFORCING MEMBER



2X8 APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR SBCCI WIND LOAD. 2X4 "L" BRACE, GROUP A, OBTAINED FROM THE MAXIMUM ALLOWABLE "T" REINFORCED GABLE VERTICAL LENGTH IS 14° FROM TOP TO BOTTOM CHORD.

WEB LENGTH INCREASE W/ "T" BRACE

	_						_				_							_		_	
30 FT	70 MPH	15 FT	70 MPH	30 FT	80 MPH	15 FT	80 MPH	30 FT	90 MPH	15 FT	90 MPH	30 FT	100 MPH	15 FT	100 MPH	30 FT	110 MPH	15 FT	110 MPH	AND MRH	WIND SPEED
2x6	2x4	2x6	2x4	2x6	2x4	2 x 6	2 x4	2x6	2x4	2x6	2x4	2x6	2x4	2x6	2x4	2x6	2x4	2x6	2x4	MBR. SIZE	"T" REINF.
10 %	2 01	0 %	0 %	20 %	20 %	10 %	10 %	30 %	10 %	20 %	20 %	40 %	2 01	30 %	10 %	50 %	10 %	40 %	2,01	20001	SBCCI
30 %	20 %	20 %	20 %	40 %	10 %	30 %	20 %	50 %	2, 01	40 %	2, 01	40 %	10 %	50 %	10 %	50 %	2 01	50 %	2 01	2000	A SC F
				_	_																

EXAMPLE:

A08530EC0207

GABLE VERTICAL = 24" O.C. SP #3 MAXIMUM "T" REINFORCED GABLE VERTICAL LENGTH $1.10 \times 6' \ 7'' = 7' \ 3''$ "T" BRACE INCREASE (FROM ABOVE) = 10% = 1.10 (1) 2X4 "L" BRACE LENGTH = 6' 7" MEAN ROOF HEIGHT = 30 FT ASCE WIND SPEED = 100 MPH "T" REINFORCING MEMBER SIZE = 2X4

THIS DRAWING REPLACES DRAWINGS GAB98117 876,719 & HC26294035

		Sep 24 '07				
MAX SPACING 24.0"	DUR. FAC. ANY	MAX TOT. LD. 60 PSF				
			-ENG DLJ/KAR	DRWG GBLLE	DATE 2/23/07	REF LET-IN VERT
			AR	GBLLETIN0207	07	N VERT



TOENAILS

SEE APPROPRIATE ALPINE GABLE DETAIL (ASCE OR SBCCI

A13015E50207, A12015E50207, A11015E50207, A10015E50207, A08515E50207, A13030E50207, A12030E50207, A11030E50207, A10030E50207, A08530E50207

A08530E50207

A08530EE0207

ASCE 7-05 GABLE DETAIL DRAWINGS

A13030EE0207, A12030EE0207, A11030EE0207, A10030EE0207,

A13015EE0207, A12015EE0207, A11015EE0207, A10015EE0207, A08515EE0207,

ASCE 7-02 GABLE DETAIL DRAWINGS

A13030EC0207, A12030EC0207, A11030EC0207, A10030EC0207,

WIND LOAD) FOR MAXIMUM UNREINFORCED GABLE

VERTICAL LENGTH

CEILING

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BOSI GBUILDING COMPRIENT SAFETY INFORMATION), PUBLISHED BY TPI CIRUSS PLATE INSTITUTE, 218 MORTH LEE STR., SUITE 312, ALEXANDRIA, VA. 22314) AND WITCA CYOIDD INUSS COUNCIL MACRICA, 6300 ENTERPRISE LH, HAIDSON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNIESS OTHERWISE INDICATED. TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL

WHIPER-MAINE FURNISH CORY OF THIS DESIGN TO INSTALLATION COMPROCEDR TTY BEG, INC., SHALL MID BE RESPONSUIBLE FOR ANY DEVLATION FOR THIS DESIGN, ANY FAILURE TO BUILD THE FRESS. IN COMPROMENT THIS DESIGN, ANY FAILURE BUILD THE FRESS. IN COMPROMENT SHIP IN COMPROMENT THIS DESIGN, ANY FAILURE BUILD THE FRESS. IN DESIGN COMPROMENT FOR THIS CAMPOINT OF THIS COMPONENT FOR ANY BUILDING DESIGNER, PER MASSIVER FOR THIS COMPONENT FOR ANY BUILDING DESIGNER, PER MASSIVER FOR THIS COMPONENT FOR ANY BUILDING DESIGNER, PER MASSIVER FOR THIS COMPONENT FOR ANY BUILDING DESIGNER, PER MASSIVER FOR THE FOR

VERTICAL SPECIES, GRADE AND SPACING) FOR (1) TO CONVERT FROM "L" TO "T" REINFORCING MEMBERS, MULTIPLY "T" FACTOR BY LENGTH (BASED ON GABLE

WEB BRACE SUBSTITUTION

BRACING METHOD IS DESIRED THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON AN ALPINE TRUSS DESIGN BUT AN ALTERNATIVE WEB

NOTES

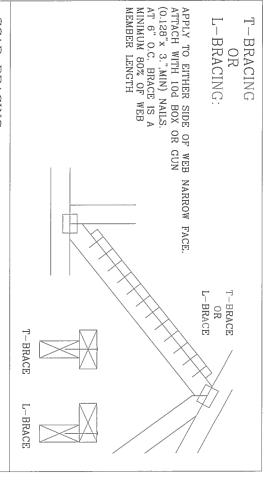
BRACING. THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB

ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE. FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE BRACING

2-2X6(*)	2X6	2 ROWS	2X8
1-2X8	2X6	1 ROW	2X8
2-2X4(*)	2X6	2 ROWS	2X6
1-2X6	2X4	1 ROW	2X6
2-2X4	2X6	2 ROWS	OR
1-2X4	2X4	1 ROW	2X3 OR 2X4
SCAB BRACE	T OR L-BRACE	BRACING	SIZE
E BRACING	ALTERNATIVE BRACING	SPECIFIED CLB	WEB MEMBER

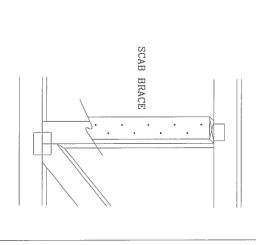
T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

* CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH FACE OF WEB.



SCAB BRACING:

(0.128"x 3.",MIN) NAILS. AT 6" O.C. BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH APPLY SCAB(S) TO WIDE FACE OF WEB. NO MORE THAN (1) SCAB PER FACE. ATTACH WITH 10d BOX OR GUN



THIS DRAWING REPLACES DRAWING 579,640

PSF REF

CLB SUBST.

TC

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ITW BUILDING COMPONENTS GROUP, INC. POMPANO BEACH, FLORIDA	1	<u> </u>	AI PINE	<u>\</u>	>
USSIGN, POSITION PER DIRAYMOS IGGA-Z. ANY INSPECTION OF PLATES ANDEX AS OF THE 1-2002 SEC. 3. A SEA, ON HIS BRAVING INDICATE: ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESI USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF ANY BUILDING IS THE RESPONSIBILITY OF ANY OF THE PROPERTY OF THE	GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS O	CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALI	**IMPORTANT** FURNISH COPY OF THIS DESIGN TO INSTALLATION CON NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAIL	FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID	INSTITUTE, 218 MORTH LEE STR., SUITE 312, ALEXANDRIA, VA. 22314) AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACI

LARHING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST (BUILDING COMPONENT SAFETY HAVROHATION), PUBLISHED BY TET CIRUSS PLATE INSTITUTE, 218 MURTH LEE STR., SUITE 312, ALEXANDRIA, VA. 22314) AND VTCA CAVODD TRUSS COUNCIL, MARCICA, 6300 ENTERPRISE LN, HADISON, VI 53719) FOR SAFETY PACTICES PRIDE TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHARD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL

COMPRACTOR. IT W BCG, INC., Y FAILURE TO BUILD THE TRUSS. 'ALLING & BRACING OF TRUSSS. BESSIGN SPEC. BY AF&PA) ANH ASTH A653 GRADE 40/66 (W/K SC OTHERWISE LOCATED ON TH FES FOLLOWED BY (1) SHALL E VIES ACCEPTANCE OF PROFESS. OF THE BUILDING DESIGNED

R, PER	SSIDNAL SSIDNAL	AND TPI. W,K/H,SS) THIS	SSES.	2001	OCTURAL OF
		Sep 24 '07			
SPACING	DUR. FAC.	TOT. LD. P	BC LL P	BC DL P	TC DL P
		PSF	PSF -EN	PSF DRWG	PSF DAT
			-ENG MLH/KAR	VG BRCLBSUB0207	DATE 2/23/07
)207	

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