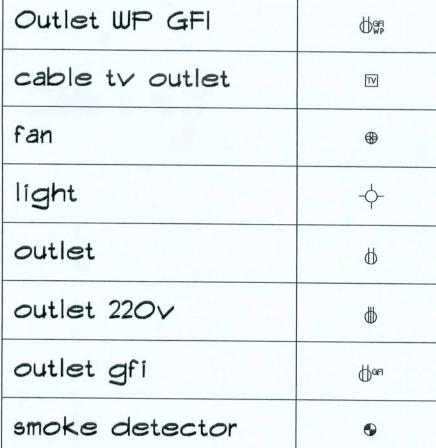


ELECTRICAL	SYMBOL
ceiling fan globe 1	
ceiling globe light	0
chandelier	<u> </u>
double spotlight	<b>Q</b> D
fluorescent fixture	
pot light	0
vanity bar light	<u> </u>
wall sconce	오
electrical panel	11

AC Disconnect

switch

telephone

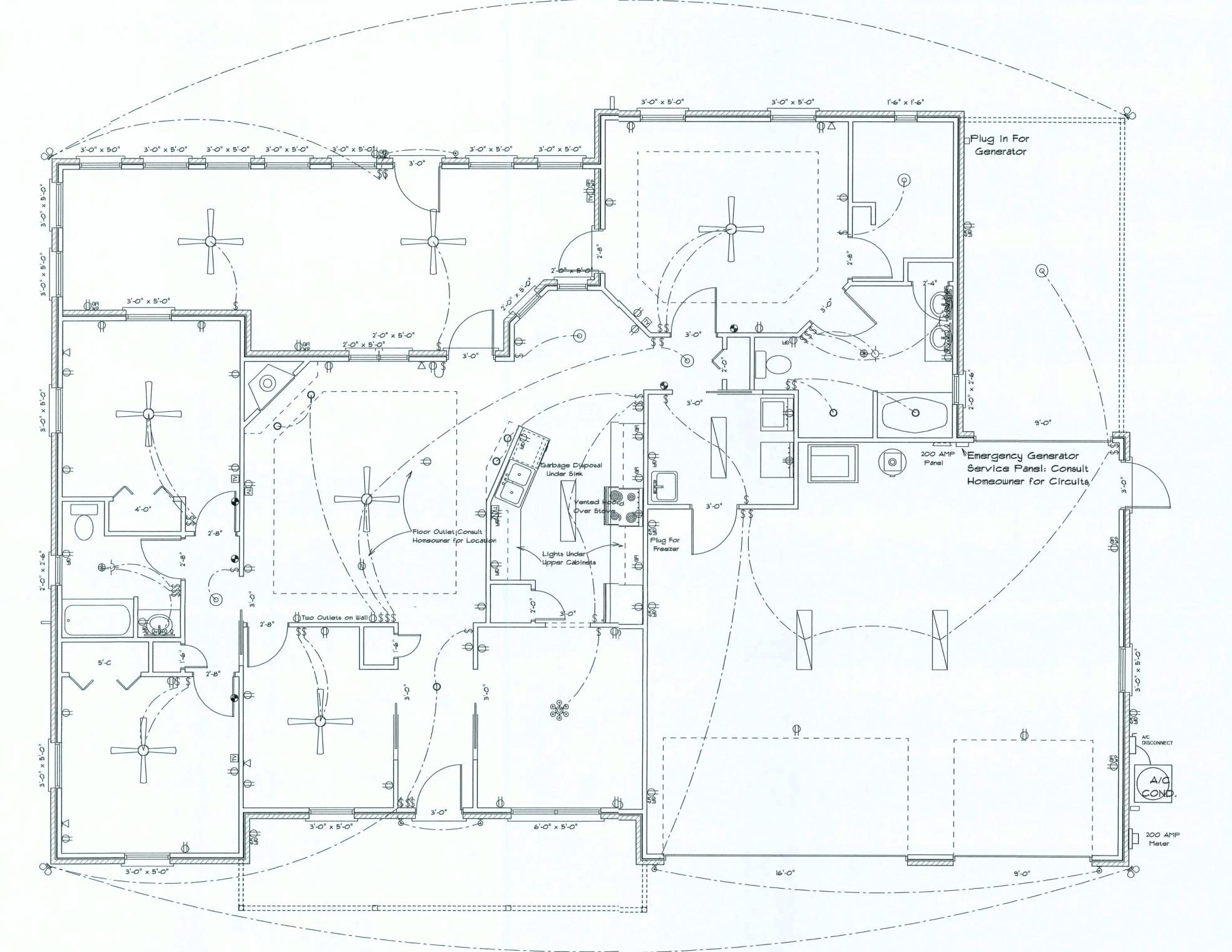


 $\nabla$ 



# Electrical Plan Notes:

- E-1 Wire all appliances, HVAC units and other equiptment per manufactures specifications.
- E-2 Consult the owner for the number or seperate telephone lines to be installed. Owner is responsible for all overages not noted on plan.
- E-3 All installations shall be per national code.
- E-4 All smoke detectors shall be 120v with battery back-up of the photoelectric type, and shall be interlocked together. Install inside and near all bedrooms.
- E-5 Telephone, television and other low voltage devices or outlets shall be as per the owners directions and in accordance with applicable sections of the National Electric Codes latest edition. Owner is responsible for all overages not noted on plan.
- E-6 Electrical contractor shall be responssible for the design and sizing of electrical service and circuits.
- E-7 Entry of service (underground or overhead) to to be determined by contractor agreement.
- E-8 All bedroom receptacles shall be AFCI (arc fault circuit interrupter).
- E-9 All outlets to be located above base flood elevation.
- E-10 All exterior GFI outlets shall be weatherproof.
- E-11 Overcurrent Protection device shall be installed on the exterior of structures to serve as a disconnecting means. Conductors used from theexterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equiptment ground.



Electrical Plan

Consult Homeowner:

- 1. Themastatic Control for Attic Fan.
- 2. Set up for Emergency Generator Circuits

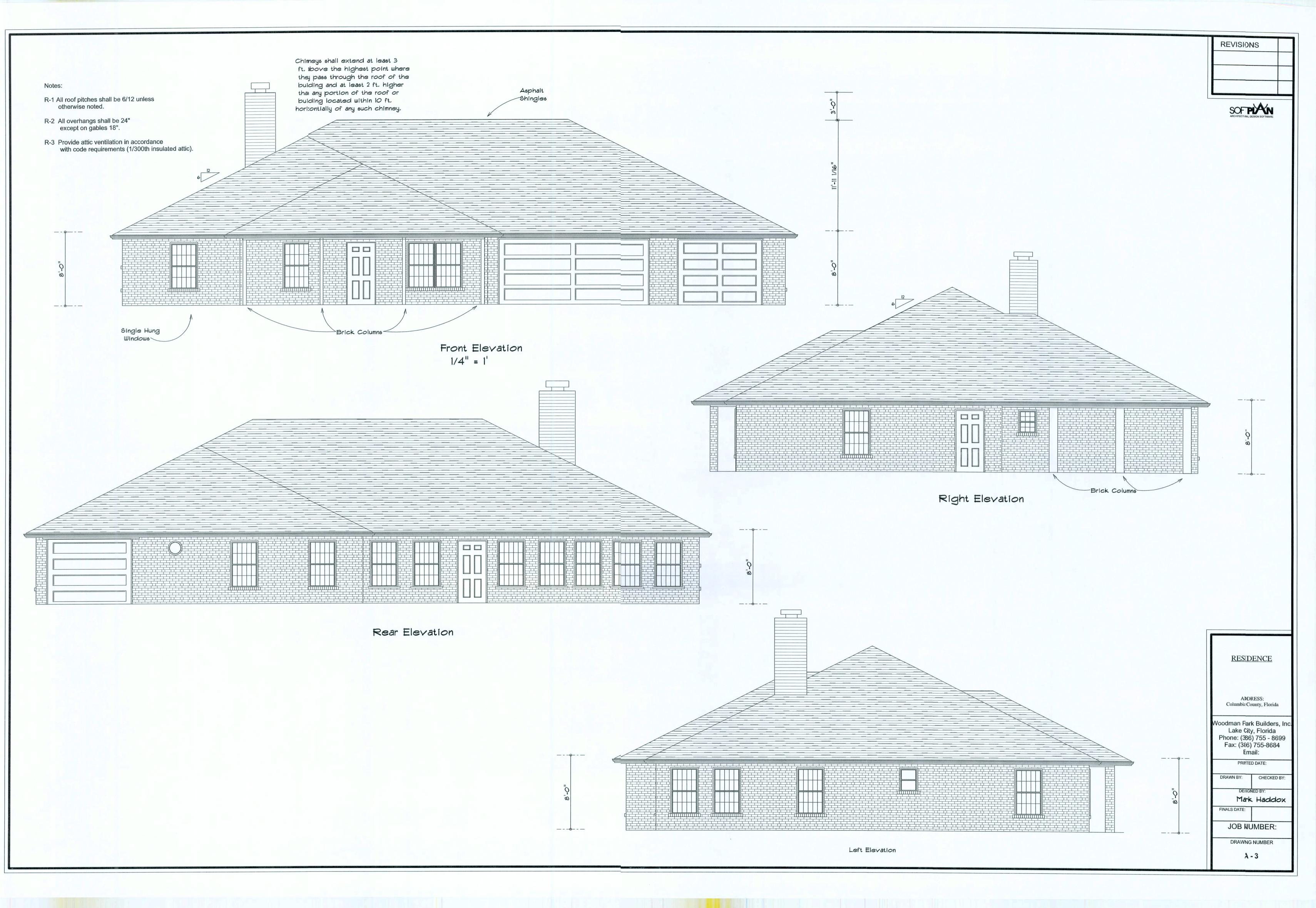
3. Lights Under Kitchen Cabinets.

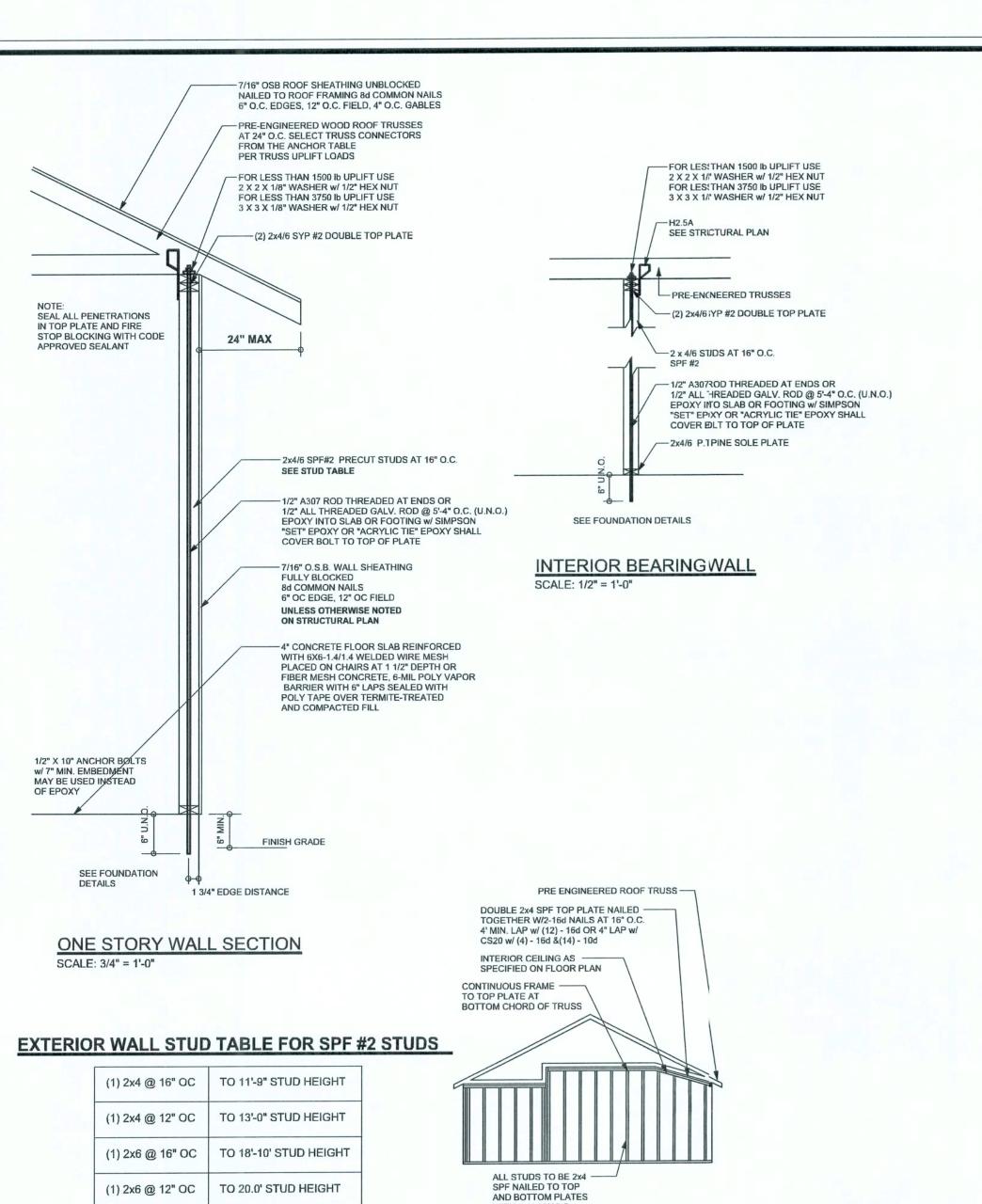
ColumbiaCounty, Florida Woodman Park Builders, Inc Lake City, Florida Phone: (386) 755 - 2411 Fax: (386) 755-8684 PRINCED DATE: DRAWN BY: CHECKED BY: Mark Haddox FINALS DATE: JOB NUMBER: DRAWING NUMBER A - 2

**RESIDENCE** 

**Butler Residence** Lot 22 Turkey Run

ALDRESS:





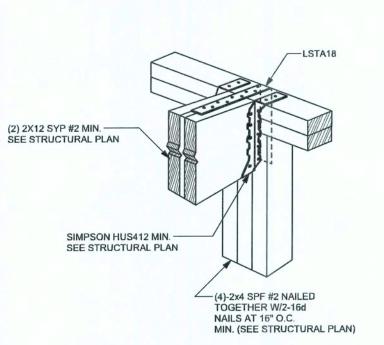
WITH 2-16d NAILS CONTINUOUS FRAME TO **CEILING DIAPHRAGM DETAL** 

-NON-SUPPORTIVE

2X4 LADDER BEAM

SCALE: N.T.S.

SUPPORTIVE .



EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.

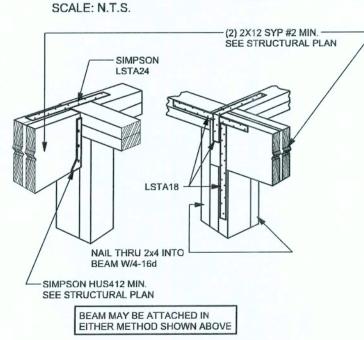
**BEAM MID-WALL CONNECTION DETAIL** 

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B,

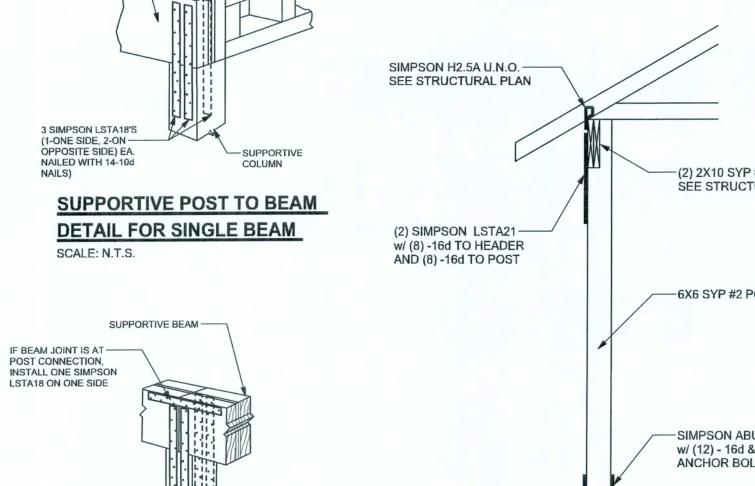
EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B.

STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING

LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING.



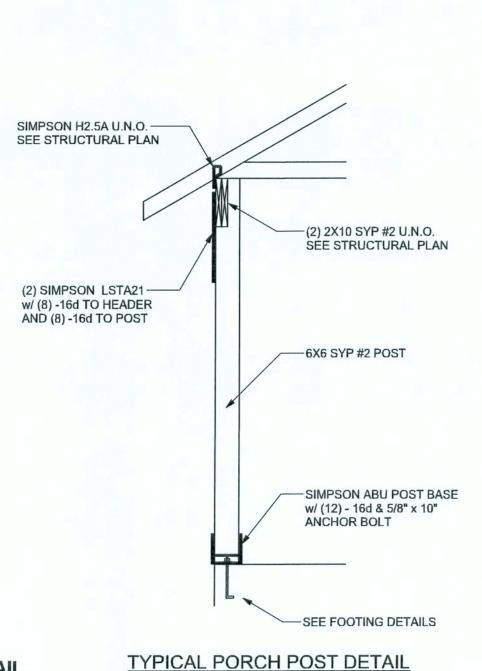
**BEAM CORNER CONNECTION. DETAIL** SCALE: N.T.S.



SUPPORTIVE CENTER POST TO BEAM DETAIL

4-SIMPSON LSTA18

(2-ONE SIDE,2-ON OTHER SIDE)



## **ANCHOR TABLE**

**OBTAIN UPLIFT REQUIREMENTS FROM TH** 

JPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LGT2	14 -16d	14 -16d	
		HEAVY GIRDER TIEDOWNS*			TO FOUNDATION
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED RO 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED RO 12" EMBEDMENT
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED RO 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED RO 12" EMBEDMENT
		STUD STRAP CONNECTOR*			TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
		STUD ANCHORS*	TO STUDS		TO FOUNDATION
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 - 16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		
< 3335	< 3335	HPAHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		1/2" AB
< 2320	< 2320	ABU88	18 - 16d		2-5/8" AB

				GENERAL NOTES:
				CHILITAL HOILD.
TRUSS				TRUSSES: TRUSSES SHALL BE DESIGNED BY A ELOPIDA LICENSED ENGINEER IN ASSESSE
				TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDAY FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEM
TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS	PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIET AND REACTION
H5A	3-8d	3-8d		ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MAN AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BU
H5	1.01	101		THE ABOVE REQUIREM
нэ	4-8d	4-8d		SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTING
H4	4-8d	4-8d		INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD EN- REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MI
H3	4-8d	4-8d		CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.
H2.5	5-8d	5-8d		SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN
H2.5A	5-8d	5-8d		FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET
H6	8-8d	8-8d		GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS
	0-00	0-00		VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE
H8	5-10d, 1 1/2"	5-10d, 1 1/2"		MINISTER COMPRESSION OF PERSON OF PE
H14-1	13-8d	12-8d, 1 1/2"		CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.
H14-2	15-8d	12-8d, 1 1/2"		WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORG
				[W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB: SUPPORTED WITH.
H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"		MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.
H10-2	6-10d	6-10d		FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REIN
H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"		FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUE
				PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. St.
H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"		TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OF
MTS24C	7-10d 1 1/2"	7-10d 1 1/2"		
HTS24	12-10d 1 1/2"	12-10d 1 1/2"		CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL B
2 - HTS24				ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. 1 WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO B
2-111024				CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUB

BE CUT IN THE LENGTH / NDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 \* DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"0C INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2"  $\times$  2"  $\times$  9/64"; WITH 5/8" BOLTS TO BE 3"  $\times$  3"  $\times$  9/64"; WITH 3/4" BOLTS TO BE 3"  $\times$  3"  $\times$  9/64"; WITH 7/8" BOLTS TO BE 3"  $\times$  3"  $\times$  5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

### **BUILDER'S RESPONSIBILITY**

_	
	THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.
	CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
	PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.
	PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.
	VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL

### **ROOF SYSTEM DESIGN**

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLIC LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1

# **DESIGN DATA**

BUILDII	NG IS NOT IN THE HIGH VELOCITY HUR	RICANE ZONE				
	NG IS NOT IN THE WIND-BORNE DEBRIS					
1.) BA	SIC WIND SPEED = 110 MPH					
2.) WI	ND EXPOSURE = B					
	ND IMPORTANCE FACTOR = 1.0					
	ILDING CATEGORY = II					
	OOF ANGLE = 10-45 DEGREES					
	AN ROOF HEIGHT = <30 FT					
	TERNAL PRESSURE COEFFICIENT = N/A	A (ENCLOSED B	IIII DI	NG)		
	OMPONENTS AND CLADDING DESIGN V				D201	2(2))
0., 00	ONE THE AND CEADDING DEGICITY	VIND FILESSON	L3 (1)	ADLL	Nou I.	2(2))
	~	Zone	Effect	tive W	ind Are	ea (ft2)
4			-	0	_	100
		1	-	-21.8		-18.1
K		2 2 O'bra	19.9	-25.5	18.1	-21.8
5	2 1 2	2 O'hg	10 0	-40.6 -25.5	19.1	-40.6 -21.8
3	4 2 2 5	3 O'hg	10.0	-68.3	10.1	-42.4
	13 4	4	21.8	-23.6	18.5	-20.4
. *	55	5	21.8	-29.1	18.5	-22.6
	The state of the s	Doors	& Wind	lows	21.8	-29.1
4			st Cas			
K			5, 10			
5	2 /32	8x7 Gar			19.5	-22.9
4	4 2 4 5	16x7 Ga	arage L	1000	18.5	-21.0
	333					
DESIGN	LOADS					
	40 PSF (ALL OTHER DWELLING ROO	MS)				
	30 PSF (SLEEPING ROOMS)					
	30 PSF (ATTICS WITH STORAGE)					
	10 PSF (ATTICS WITHOUT STORAGE,	<3:12)				
	TO THE PROPERTY OF THE PROPERT	· 0.12)				
ROOF						
ROOF	20 PSF (FLAT OR <4:12) 16 PSF (4:12 TO <12:12)					

STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

SOIL BEARING CAPACITY 1000PSF

PE No.53915, POI 868, Lake City, FL 32056, 386-754-5419

REVISIONS

SOFTPILAN

MENSIONS: Stated dimensionssupercede scaled nensions. Referall questions to Mark Disosway, PE. for resolution Do not proceed without clarification

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examined this plan and that the applicable portions of the plar, relating to wind engineer comply with section R301.2.1, florida building code residential 2004, to the best of my

LIMITATION: This design is valid for one

building, at specified location. MAFK DISOSWAY P.E. 53915

Woodman Park Builder

ADDRESS: Lot 21, Turkey Run LakeCity, Florida

Columbia County

Butler Residence

Mark Disosway P.E. P.C. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

> PRNTED DATE: August 30, 2006 STRUCTURAL BY

FINALS DATE:

David Disosway

30 / Aug / 06 JOB NUMBER:

608293 DRAWING NUMBER

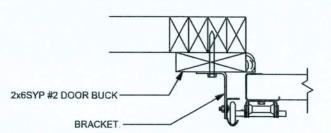
OF3 SHEETS

**GRADE & SPECIES TABLE** 

		Fb (psi)	E (10 <sup>6</sup> psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2900	2.0
PSL	PARALAM	2900	2.0

2x6 SYP #2 GARAGE DOOR BUCK ATTACHMENT ATTACH GARAGE DOOR BUCK TO STUD PACK AT EACH SIDE OF DOOR OPENING WITH 3/8"x4" LAG SCREWS w/ 1" WASHER LAG SCREWS MAY BE COUNTERSUNK. HORIZONTAL JAMBS DO NOT TRANSFER LOAD. CENTER LAG SCREWS OR STAGGER 16d NAILS OR (2) ROWS OF .131 x 3 1/4" GN PER TABLE BELOW:

OR WIDTH	3/8" x 4" LAG	16d STAGGER	(2) ROWS OF .131 x 3 1/4" GN
8' - 10'	24" O.C.	5" O.C.	5" O.C.
11' - 15'	18" O.C.	4" O.C.	4" O.C.
16' - 18'	16" O.C.	3" O.C.	3" O.C.



**GARAGE DOOR BUCK INSTALLATION DETAIL** 

FOR LESS THAN 1500 Ib UPLIFT USE

FOR LESS THAN 3750 Ib UPLIFT USE

-NAIL SHEATHING TO HEADER AND TOP PLATE WITH 8d AT 3" O.C. FOR UPLIFT

— SP4/6 @ 48" O.C. (U.N.O.) /——(7) .131 x 3 1/4" GUN NAILS

**TOE NAILED THRU HEADER** 

INTO KING STUD

2 X 2 X 1/8" WASHER

3 X 3 X 1/8" WASHER

CRIPPLES IF REQUIRED

(5) .131 x 3 1/4" GUN NAILS

TOE NAILED THRU SILL-

INTO JACK STUD U.N.O.

TYPICAL STRAPPING (U.N.O.)

(1) 2X6 SPF #2 SILL UP TO 7'-6" U.N.O.

(2) 2X4 SPF #2 SILL UP TO 7'-8" U.N.O.

(1) 2X4 SPF #2 SILL UP TO 5'-1" U.N.O.

(FOR: 120 MPH, 10'-0" WALL HEIGHT U.N.O.)

TYPICAL 1 STORY HEADER STRAPING DETAIL

(SEE STRUCTURAL PLAN)

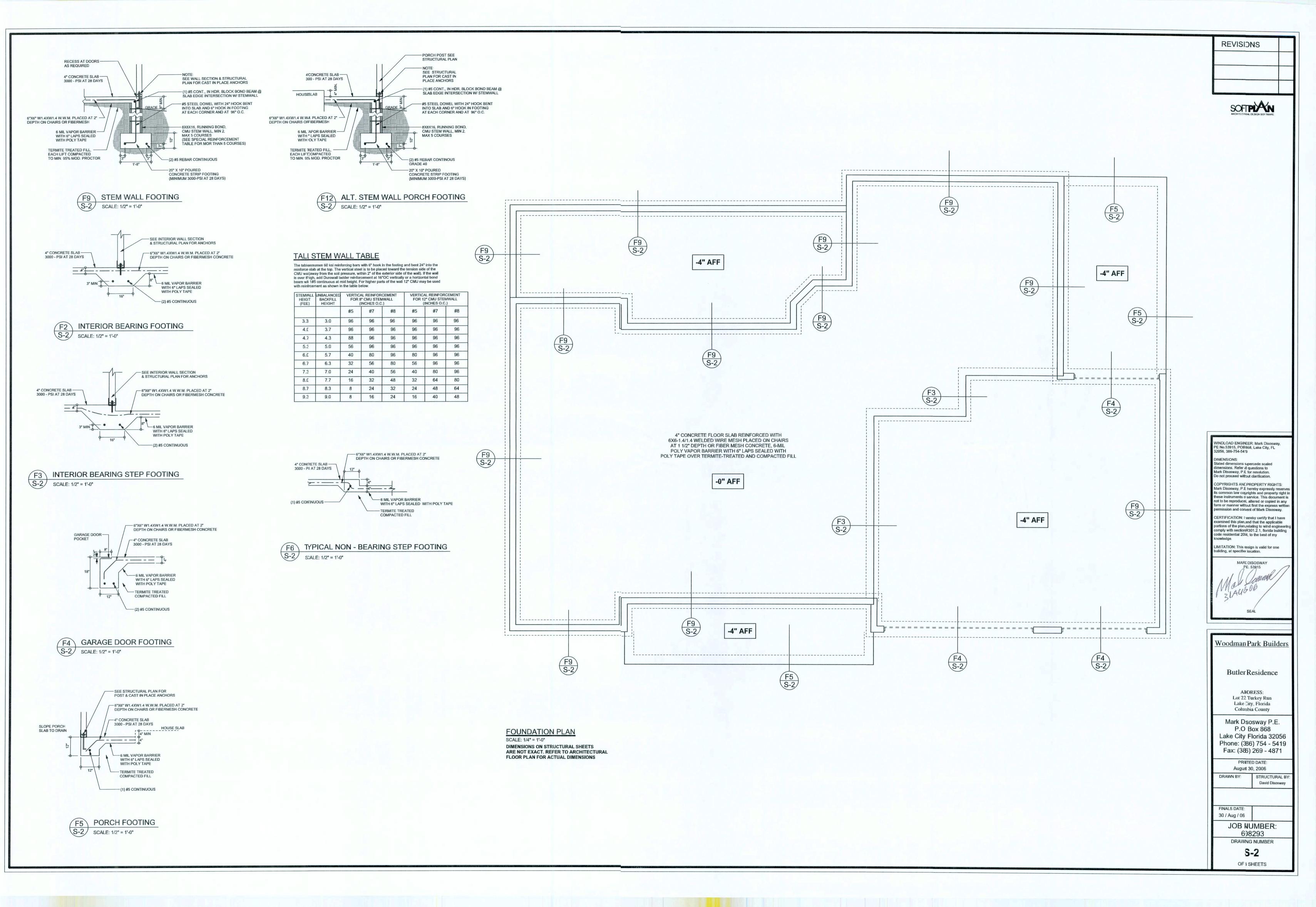
IF TRUSS TO WALL STRAPS ARE NAILED

TO THE HEADER THE SP4/6 @ 48" O.C.

INTO KING STUD

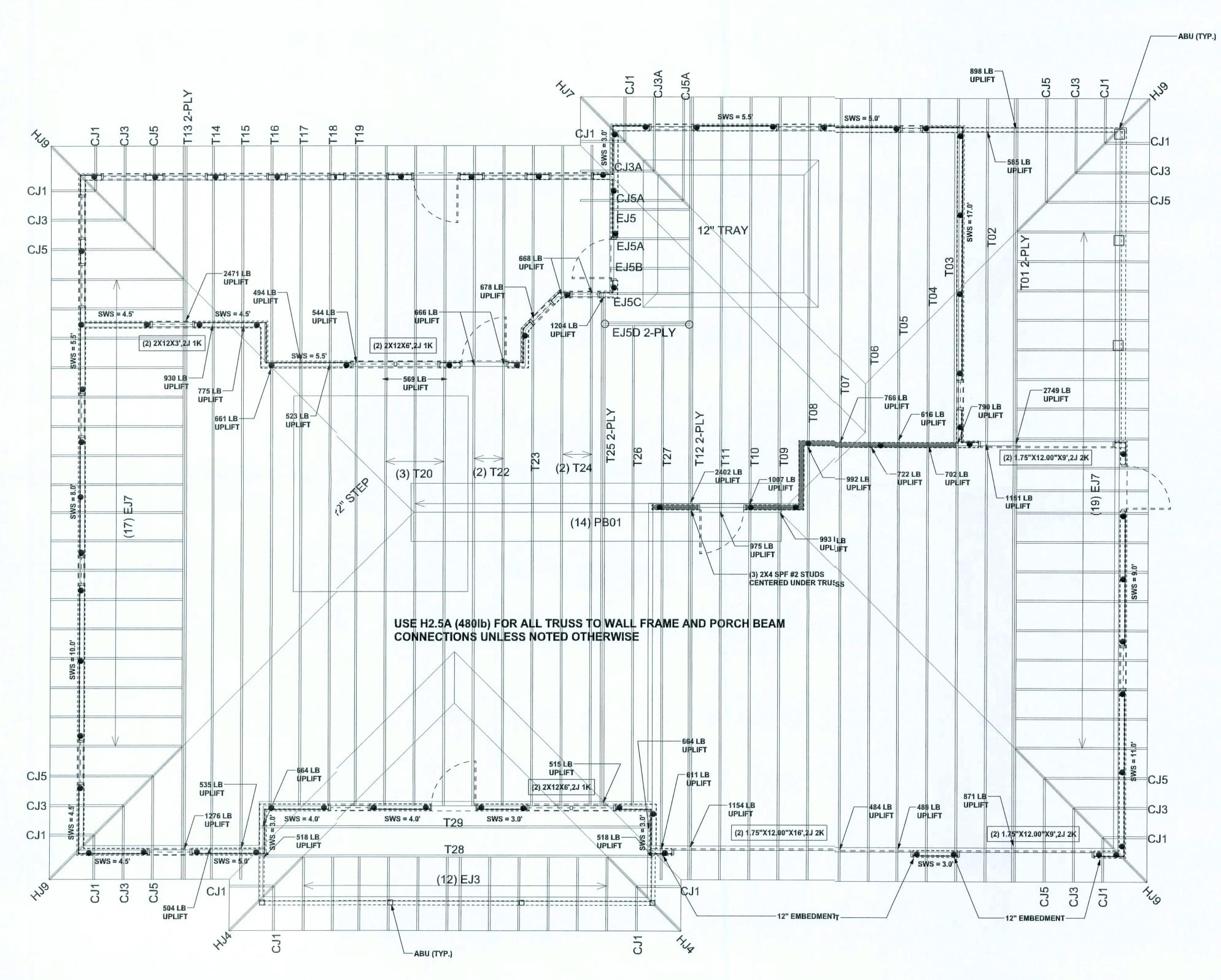
(7) .131 x 3 1/4" GUN NAILS -

TOE NAILED THRU HEADER



REVISIONS

SOFTPIN ARCHITECTURAL DISIGN SOFTWARE



STRUCTURAL PLAN SCALE: 1/4" = 1'-0"

# STRUCTURAL PLAN NOTES

SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP#2 (U.N.O.)

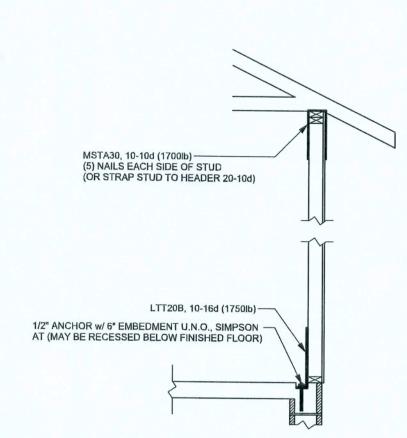
SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)

SN-3

DIMENSIONS ON STRUCTURAL SHEETS
ARE NOT EXACT. REFER TO ARCHITECTURAL
FLOOR PLAN FOR ACTUAL DIMENSIONS

PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS.

LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED

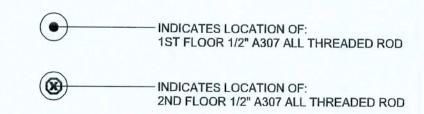


ALTERNATE WALL TIE CONNECTION WHERE
THREADED ROD CANNOT BE PLACED IN WALL
SCALE: 1/2" = 1'-0"

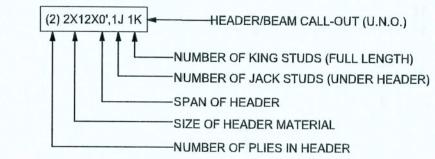
### WALL LEGEND

SWS = 0.0'	40T FL 00D EVERDIOD WAY
	1ST FLOOR EXTERIOR WALL
SWS = 0.0'	2ND FLOOR EXTERIOR
	ZND FLOOR EXTERIOR
IBW	1ST FLOOR INTERIOR BEARING WALLS
2	SEE DETAILS ON SHEET S-1
IBW	2ND FLOOR INTERIOR BEARING WALLS
	SEE DETAILS ON SHEET S-1

## THREADED ROD LEGEND



# **HEADER LEGEND**



TOTAL SHEAR WALL SEGMENTS

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

REQUIRED ACTUAL
TRANSVERSE 38.3' 74.0'
LONGITUDINAL 36.5' 48.5'

SEAL

WINDLOAD ENGINEER Mark Disosway,

PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for esolution.

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CERTIFICATION: I herely certify that I have examined this plan, and hat the applicable portions of the plan, relating to wind engineering comply with section R30 .2.1, florida building code residential 2004, tothe best of my knowledge.

MARK DISOSWAY P.E. 5)919

LIMITATION: This desigr is valid for one building, at specified location.

form or manner without first the express writte permission and consent if Mark Disosway.

Do not proceed without carification.

Woodman Park Builders

Butler Residence

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PRINTEL DATE:
August 30 2006

DRAWN BY: STRUCTURAL BY:

David Disosway

FINALS DATE: 30 / Aug / 06

JOB NUMBER: 608293 DRAWING NUMBER

> S-3 OF 3 SHEETS

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY: BUILDERS FIRST SOURCE JOB #L207901