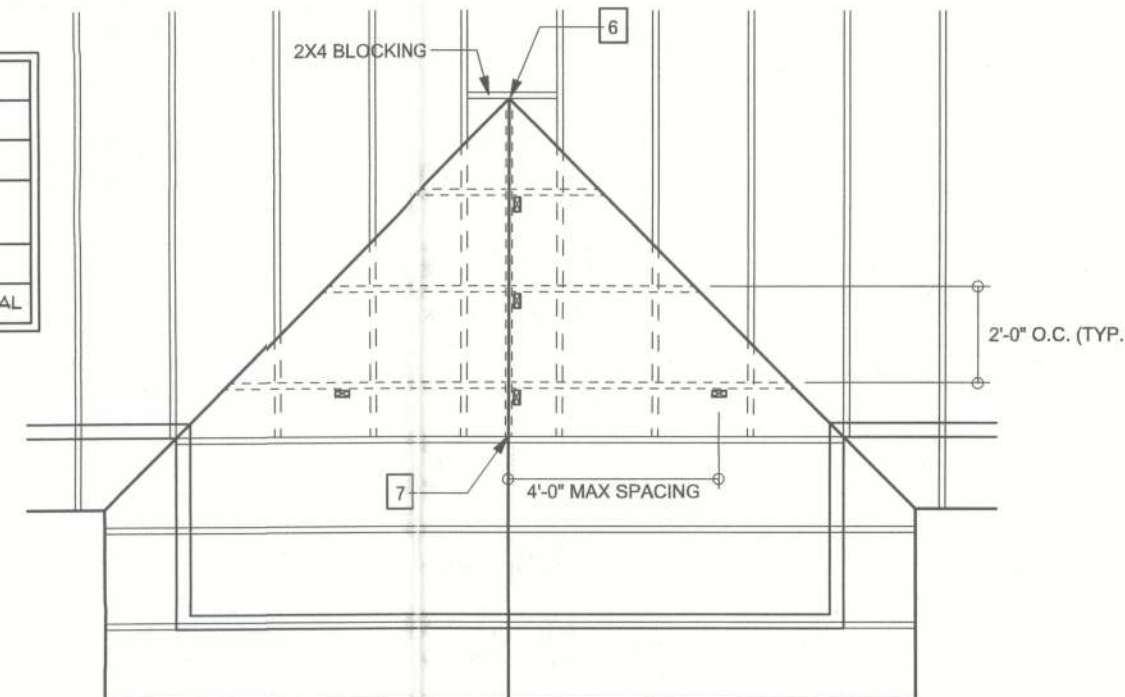


STRUCTURAL PLAN

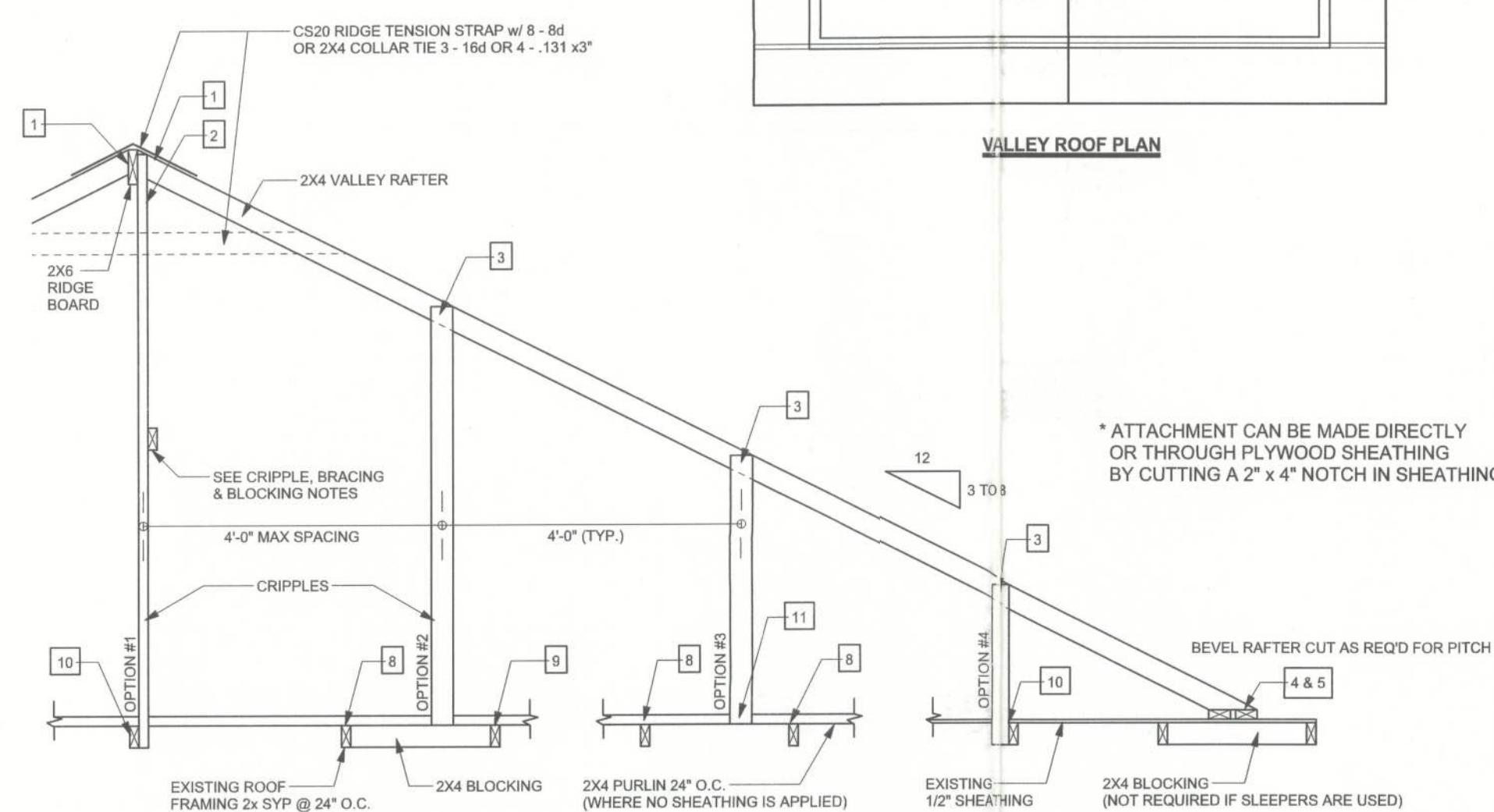
SCALE: 1/4" = 1'-0"

LUMBER SIZE & GRADE MINIMUM REQUIREMENTS

RIDGE BOARD	2X6 SYP #2
RAFTER SPANS 20'-0" OR LESS	2X4 SYP #2
FURLINS / LATERAL BRACING	2X4 SPF #2
SLEEPERS	2X (WIDTH OF RAFTER BEAT CUT) SPF #3 OR 2 PARALLEL 2X4 SPF #3
CRIPPLES & BLOCKING	2X4 SPF #2 OR BETTER
TRUSSES BELOW	SEE TRUSS DESIGN - SOUTHERN PINE MATERIAL



VALLEY ROOF PLAN



SECTION CUT PARALLEL TO VALLEY RAFTER

RETROFIT ROOF OVER FRAMING & BRACING DETAIL

SCALE: N.T.S.

VALLEY ROOF PLAN MEMBER LEGEND

- TRUSS
- TRUSS UNDER VALLEY FRAMING
- VALLEY RAFTER OR RIDGE
- CRIPPLE

CRIPPLES 4'-0" O.C. FOR 20 psf (TL) AND 10 psf (TD) (TYP. SHINGLE ROOF) MAX

CONNECTION REQUIREMENT NOTES

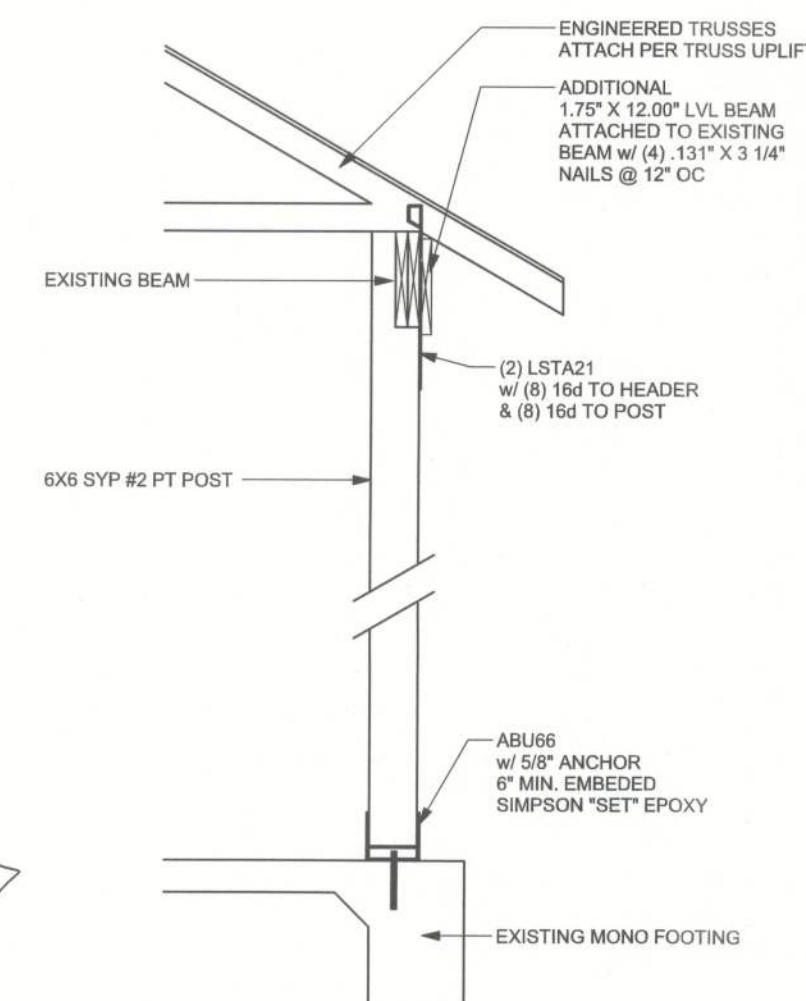
1 2X4 RAFTERS TO RIDGE	3 - 16d OR 6 - .131 x 3" TOE NAILS
2 CRIPPLE TO RIDGE	3 - 16d OR 6 - .131 x 3" FACE NAILS
3 CRIPPLE TO RAFTERS	3 - 16d OR 6 - .131 x 3" FACE NAILS
4 RAFTER TO SLEEPER OR BLOCKING	6 - 16d OR 12 - .131 x 3" TOE NAILS
5 SLEEPER TO TRUSS	4 - 16d OR 8 - .131 x 3" FACE NAILS EACH TRUSS
6 RIDGE BOARD TO ROOF BLOCK	3 - 16d OR 6 - .131 x 3" TOE NAILS
7 RIDGE BOARD TO TRUSS	3 - 16d OR 6 - .131 x 3" TOE NAILS
8 PURLIN TO TRUSS (TYP.)	3 - 16d OR 6 - .131 x 3" NAILS
9 PURLIN TO TRUSS (IF CRIPPLE IS ATTACHED TO PURLIN)	4 - 16d OR 8 - .131 x 3" NAILS
10 TRUSS TO BLOCKING	3 - 16d OR 6 - .131 x 3" END NAILS
11 CRIPPLE TO TRUSS	3 - 16d OR 6 - .131 x 3" FACE NAILS
12 CRIPPLE TO PURLIN	3 - 16d OR 6 - .131 x 3" FACE NAILS

GENERAL NOTES

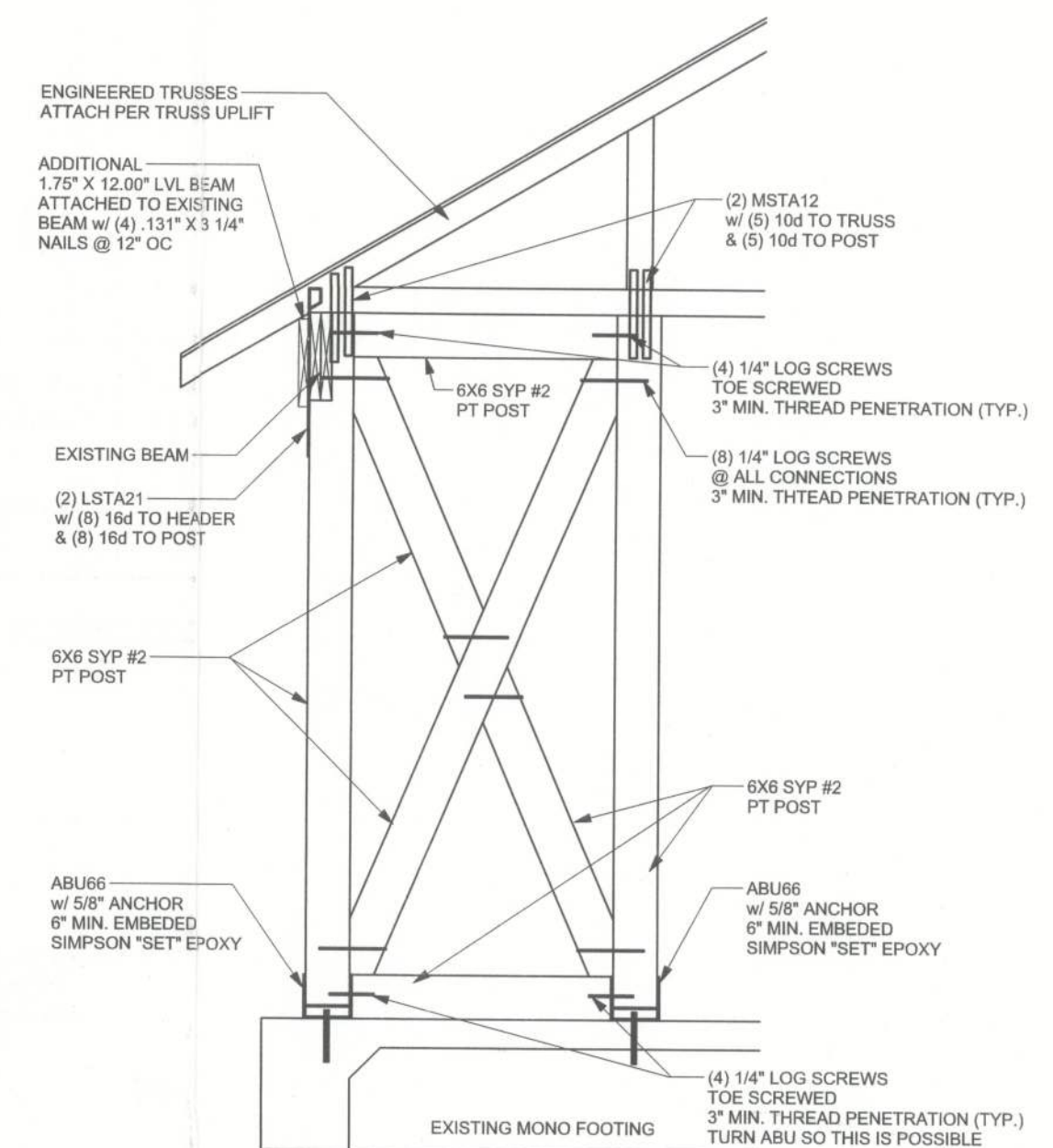
- MAXIMUM RAFTER SPANS: 6'-0" FOR 2X4, 9'-0" FOR 2X6 SPF #2 OR SYP #2.
- MAXIMUM ROOF AREA PER SUPPORT: 1602 IN ZONES 2 & 3, 2482 IN ZONE 1. (EXAMPLE: 4'-0" O.C. X 4'-0" SPAN = 1602 OR 2'-0" X 8'-0" SPAN = 1602)
- PURLINS REQUIRED 2'-0" O.C. IF EXISTING SHEATHING IS REMOVED. PURLINS SHOULD OVERLAP SHEATHING ONE TRUSS SPACING MINIMUM. IN CASES THAT THIS IS IMPRACTICAL, OVERLAP SHEATHING A MINIMUM OF 6", AND NAIL UPWARDS THROUGH SHEATHING INTO PURLIN WITH A MINIMUM OF 6 - 3d COMMON WIRE NAILS.
- THIS DRAWING APPLIES TO VALLEYS WITH THE FOLLOWING CONDITIONS:
 - SPANS (DISTANCES BETWEEN HEELS) 4'-0" OR LESS
 - MAXIMUM VALLEY HEIGHT: 14'-0" OR LESS
 - MAXIMUM WIND SPEED: 130 MPH
 - MAXIMUM MEAN ROOF HEIGHT: 30 FEET
 - MAXIMUM TOTAL LOADING: 40 psf
 - MEETS FBC 2010/ASCE 7-10 WIND REQUIREMENTS
 - EXPOSURE CATEGORY "C", I = 1.0, Kd = 1.0
 - ENCLOSED BUILDING

CRIPPLE, BRACING, & BLOCKING NOTES

- 2X4 CONTINUOUS LATERAL BRACE (CLB) MIN. IS REQUIRED FOR CRIPPLES 5'-0" TO 10'-0" LONG NAILED w/ 2 - 10d NAILS OR 2X4 1" OR SCAB BRACE NAIL TO FLAT EDGE OF CRIPPLE WITH 8d NAILS @ 8" O.C. 1" OR SCAB MUST BE 90% OF CRIPPLE LENGTH. CRIPPLES OVER 10'-0" LONG REQUIRE TWO CLBs OR BOTH FACES w/ 1" OR SCAB. USE STRESS GRADED LUMBER & BOX OR COMMON NAILS.
- NARROW EDGE OF CRIPPLE CAN FACE RIDGE OR RAFTER, AS LONG AS THE PROPER NUMBER OF NAILS ARE INSTALLED INTO RIDGE BOARD.
- INSTALL BLOCKING UNDER RAFTER IF SLEEPERS ARE NOT USED.
- INSTALL BLOCKING UNDER CRIPPLES IF CRIPPLES FALL BETWEEN LOWER TRUSS TOP CHORDS AND LATERAL BRACING IS NOT USED.
- APPLY ALL NAILING IN ACCORDANCE TO NDS-1997 SECTION 12. NAILS ARE COMMON WIRE NAILS UNLESS NOTED OTHERWISE.



(TYP.) POST



END POST DETAIL

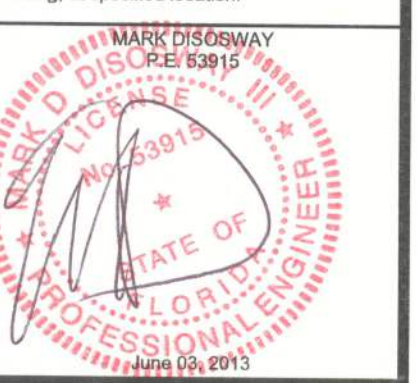
WINDLOAD ENGINEER: Mark Disoway,
PE No. 53915, POB 868, Lake City, FL
32066, 386-754-5419

DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, 2010 Florida Building Code Residential to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.



MTM Construction Services, Inc.

Mark Cady
Renovation

ADDRESS:
2249 SW Little Rd.
Lake City, FL 32024

Mark Disoway P.E.
P.O. Box 868
Lake City, Florida 32056
Phone: (386) 754 - 5419
Fax: (386) 269 - 4871

PRINTED DATE:
June 03, 2013

DRAWN BY: STRUCTURAL BY:

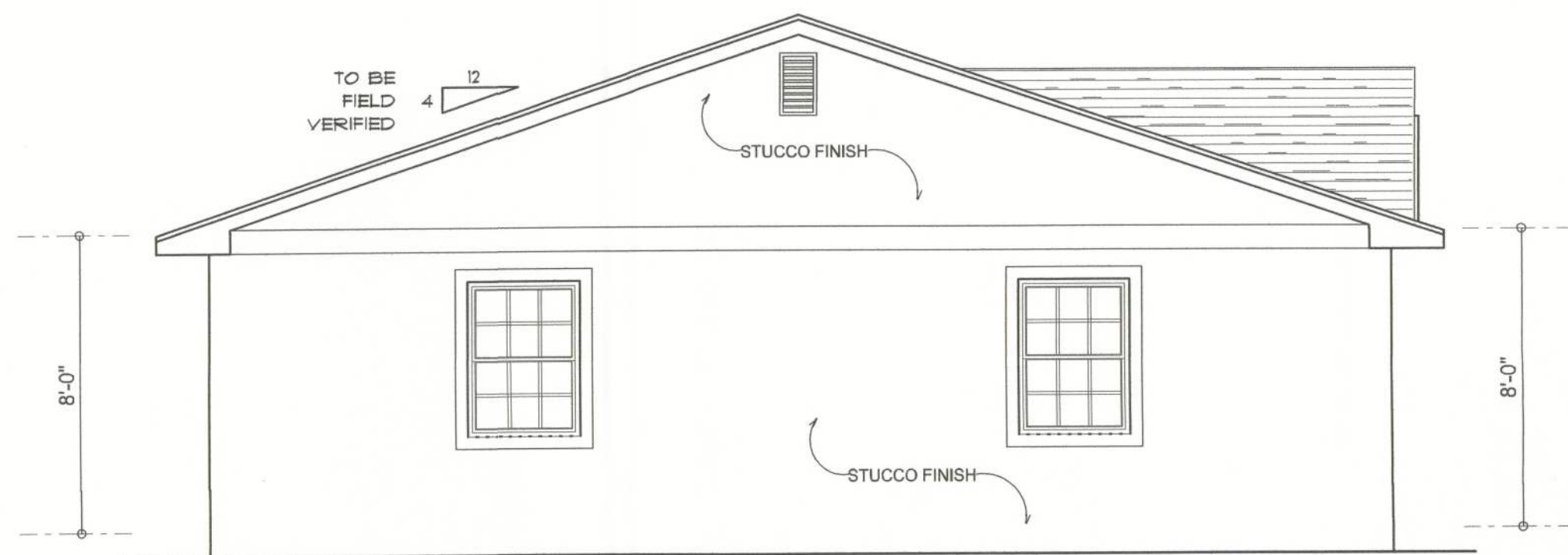
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JOB NUMBER:
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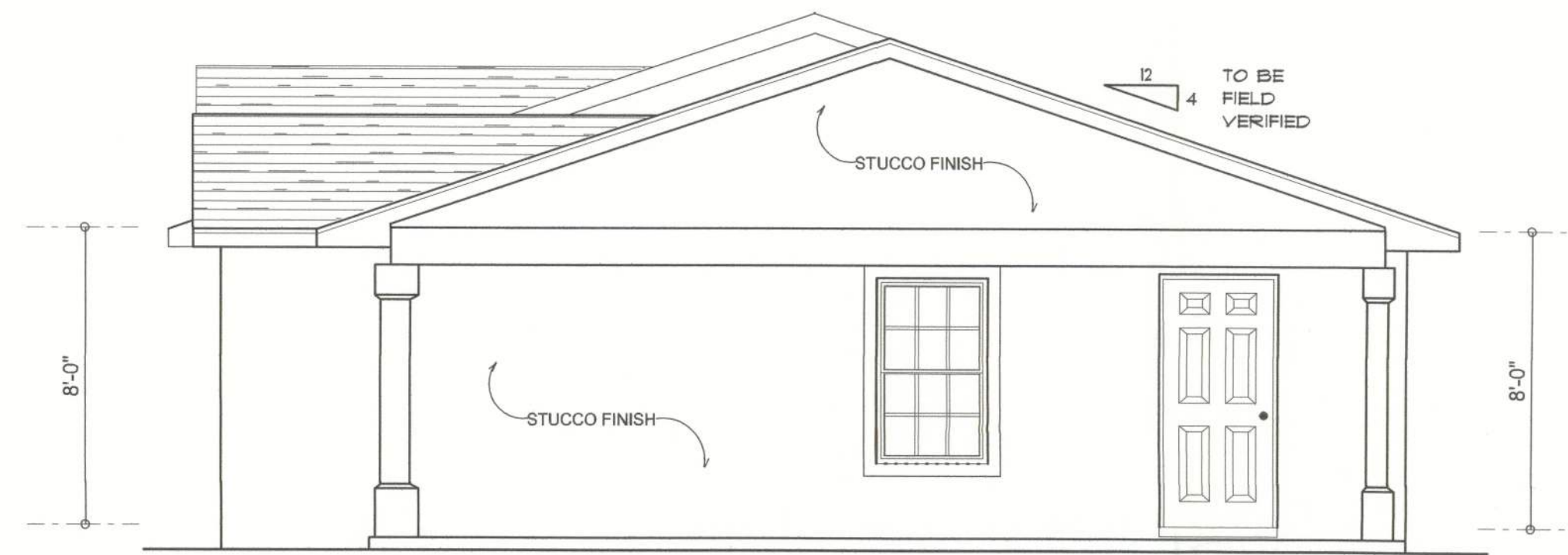
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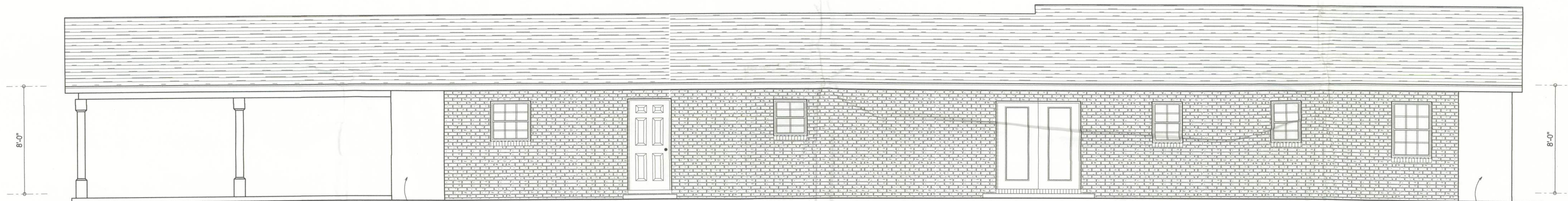
OF 2 SHEETS



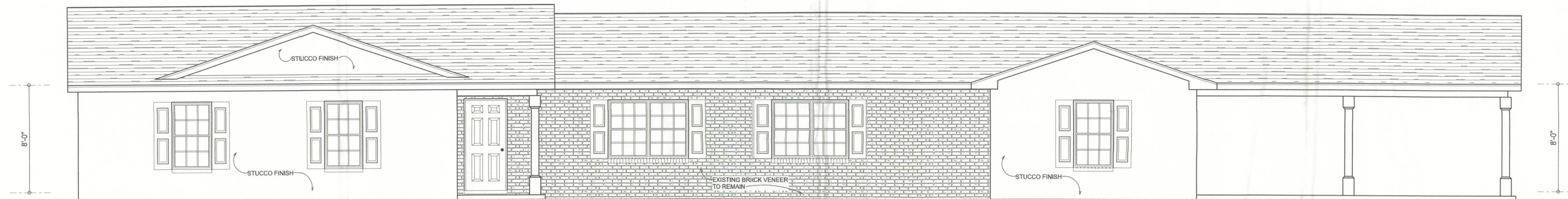
LEFT ELEVATION
SCALE: 1/4" = 1'-0"



RIGHT ELEVATION
SCALE: 1/4" = 1'-0"



REAR ELEVATION
SCALE: 1/4" = 1'-0"



FRONT ELEVATION
SCALE: 1/4" = 1'-0"



Wm C Myers

REVISIONS
May 14, 2012

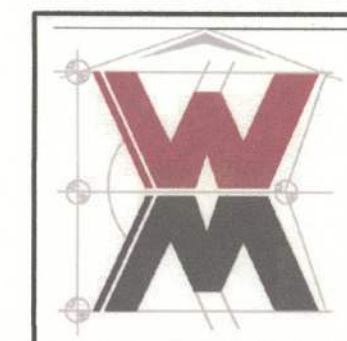
SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

EXTERIOR ELEVATIONS
SCALE: 1/4" = 1'-0"

TYPICAL WALL SECTION
SCALE: 1" = 1'-0"

A RESIDENCE RENOVATION FOR:
MARK CADY
PROJECT ADDRESS: 2249 SW LITTLE ROAD, LAKE CITY FLORIDA 32024
MTM CONSTRUCTION SERVICES, INC.
LAKE CITY, FLORIDA

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will@willmyers.net



JOB NUMBER
120504

SHEET NUMBER
A1
OF 3 SHEETS

REVISIONS
May 14, 2012

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

RENOVATION PLAN
SCALE: 1/4" = 1'-0"

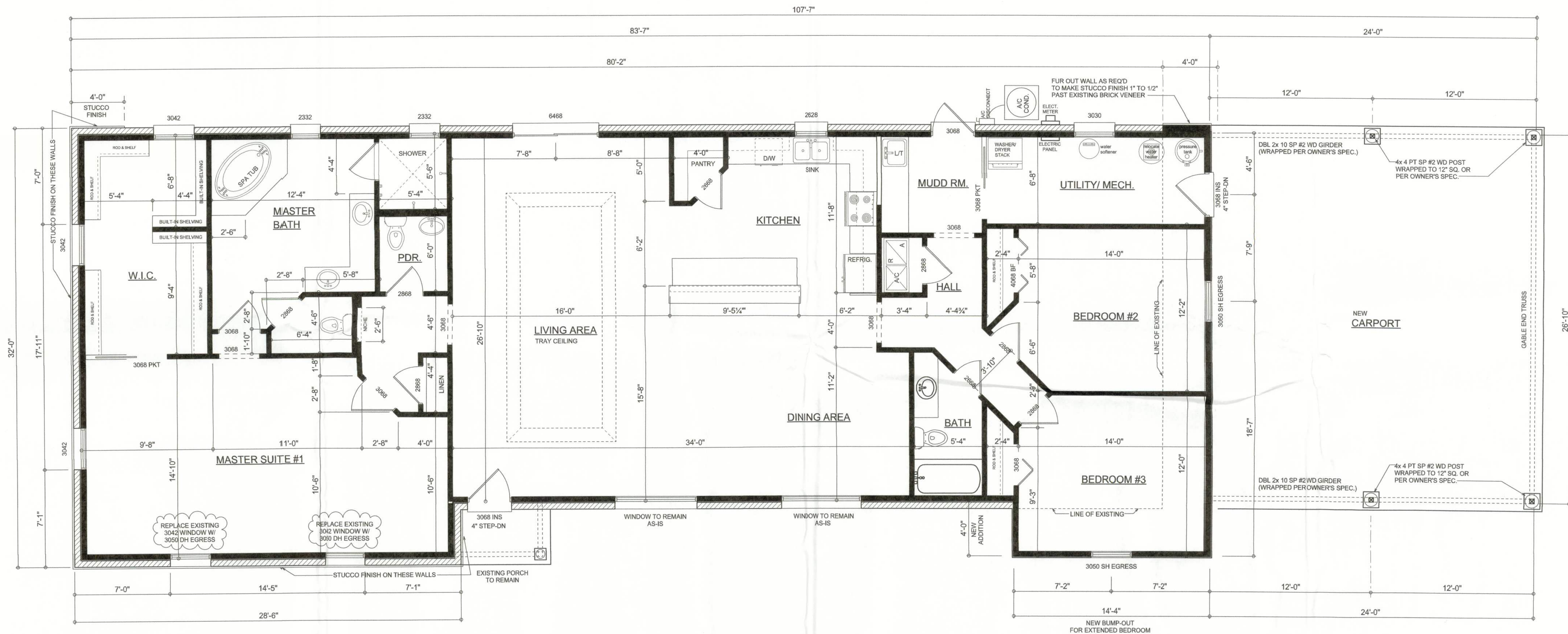
A RESIDENCE RENOVATION FOR:
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PROJECT ADDRESS: 2249 SW LITTLE ROAD, LAKE CITY FLORIDA 32024
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120504

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A.2
OF 3 SHEETS



PROPOSED RENOVATIONS
SCALE: 1/4" = 1'-0"

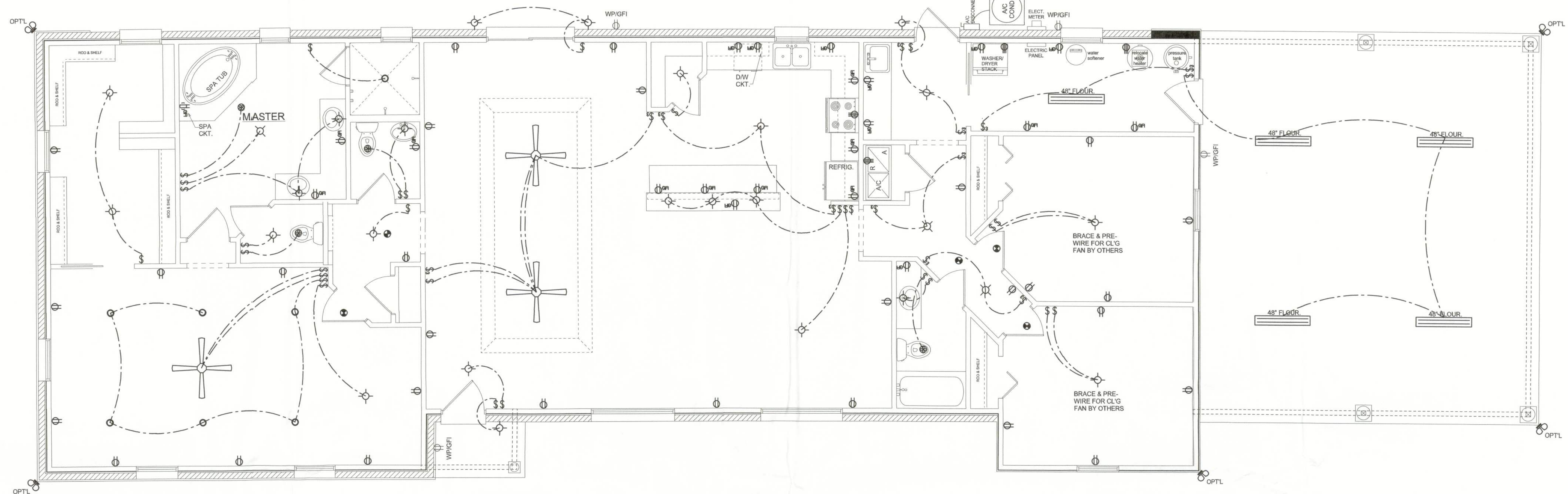
WALL LEGEND

- EXISTING NORWEGIAN BRICK EXTERIOR WALL WITH NEW 2x4 INTERIOR STUD WALL.
- EXISTING 2x4 INTERIOR STUD WALL.
- NEW 2x4 EXTERIOR WALL (STUCCO FINISH)
- NEW 2x4 INTERIOR STUD WALL.

AREA SUMMARY

LIVING AREA	2533	S.F.
ENTRY PORCH AREA	21	S.F.
GARAGE AREA	628	S.F.
TOTAL AREA	3,182	S.F.

W.C.M.



ELECTRICAL PLAN
SCALE: 1/4" = 1'-0"

ELECTRICAL LEGEND	
	CEILING FAN (PRE-WIRE FOR LIGHT (IT))
	DOUBLE SECURITY LIGHT
	RECESSED CAN LIGHT
	BATH EXHAUST FAN
	LIGHT FIXTURE
	DUPLEX OUTLET (AFCI & TAMPER RESISTANT)
	220v OUTLET
	GFI DUPLEX OUTLET (PER NEC 406.8)
	TELEVISION JACK
	TELEPHONE JACK
	SMOKE / CARBON MONOXIDE DETECTOR (see note below)
	WALL SWITCH
	3 WAY WALL SWITCH
	WATER PROOF GFI OUTLET
	2 OR 4 TUB FLUORESCENT FIXTURE

NOTE:
ALL INTERIOR RECEPTACLES SHALL BE AFCI
(ARC FAULT CIRCUIT INTERRUPT) PER NEC 210.12 & TAMPER RESISTANT PER
NEC 406.11

ALL SMOKE DETECTORS BE A COMBO SMOKE & CARBON MONOXIDE DETECTOR
AND SHALL HAVE BATTERY BACKUP POWER
AND ALL WIRED TOGETHER SO IF ANY ONE UNIT IS ACTUATED THEY
ALL ACTIVATE.

THE ELECTRICAL SERVICE OVERCURRENT PROTECTION DEVICE SHALL BE
INSTALLED ON THE EXTERIOR OF STRUCTURES TO SERVE AS A DISCONNECT MEANS.
CONDUCTORS USED FROM THE EXTERIOR DISCONNECTING MEANS TO A PANEL OR SUB
PANEL SHALL HAVE FOUR-WIRE CONDUCTORS, OF WHICH ONE CONDUCTOR
SHALL BE USED AS AN EQUIPMENT GROUND

IT IS THE LICENSED ELECTRICAL CONTRACTORS RESPONSIBILITY TO INSURE THAT ALL
WORK PERFORMED AND EQUIPMENT INSTALLED MEETS OR EXCEEDS THE 2008 NATIONAL
ELECTRIC CODE AND ALL OTHER LOCAL CODES AND ORDINANCES.

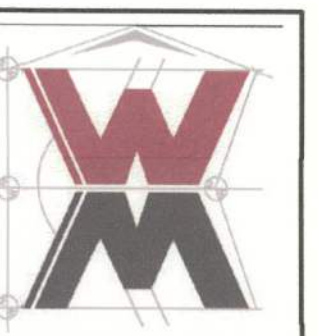
REVISIONS
May 14, 2012

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

RENOVATION PLAN
SCALE: 1/4" = 1'-0"

A RESIDENCE RENOVATION FOR:
MARK CADY
PROJECT ADDRESS: 2249 SW LITTLE ROAD, LAKE CITY FLORIDA 32024
MTM CONSTRUCTION SERVICES, INC.
LAKE CITY, FLORIDA

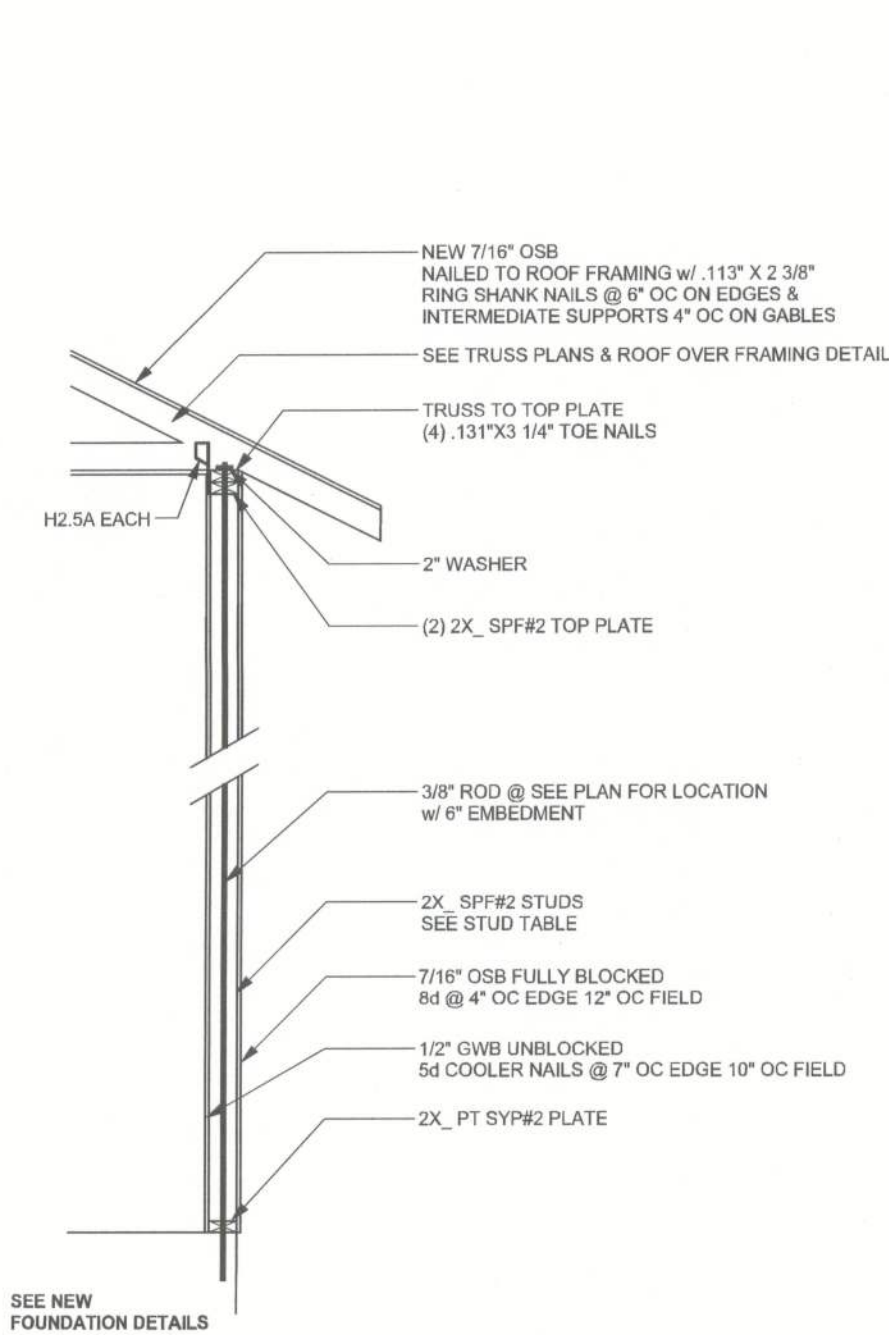
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will@willmyers.net



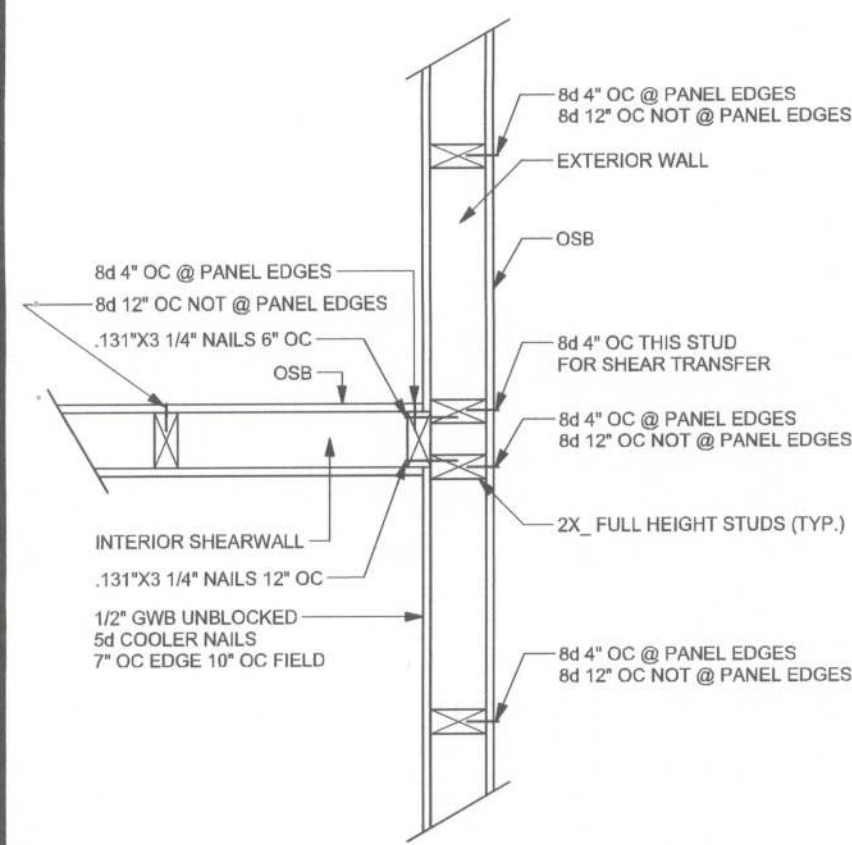
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120504

SHEET NUMBER
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OF 3 SHEETS

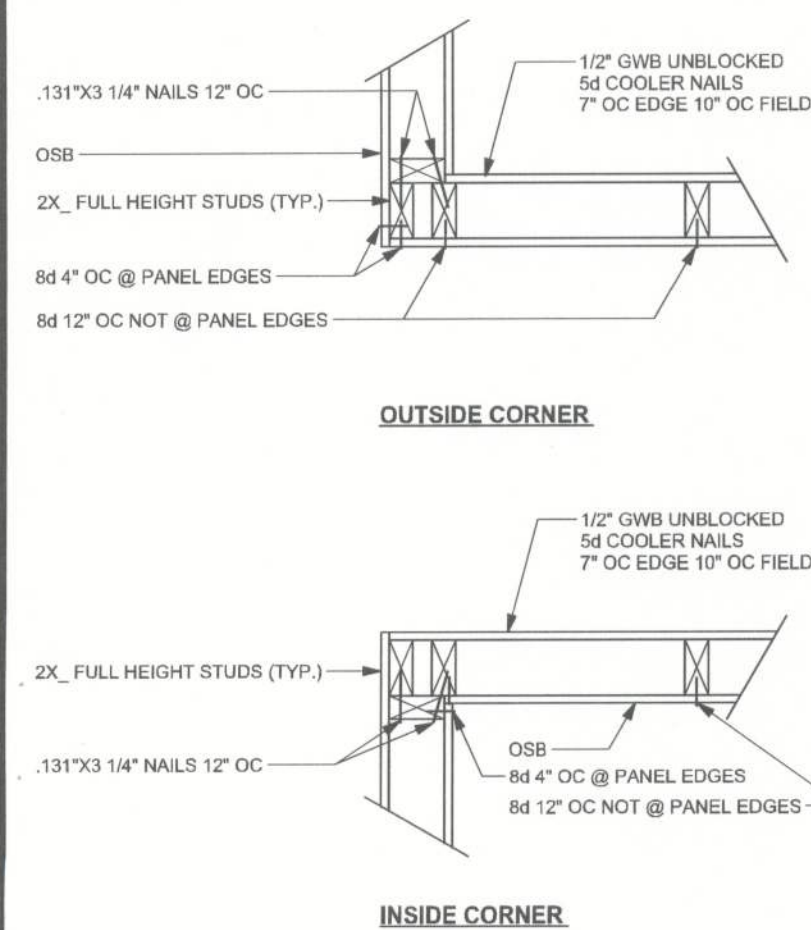
Wm C. Myers



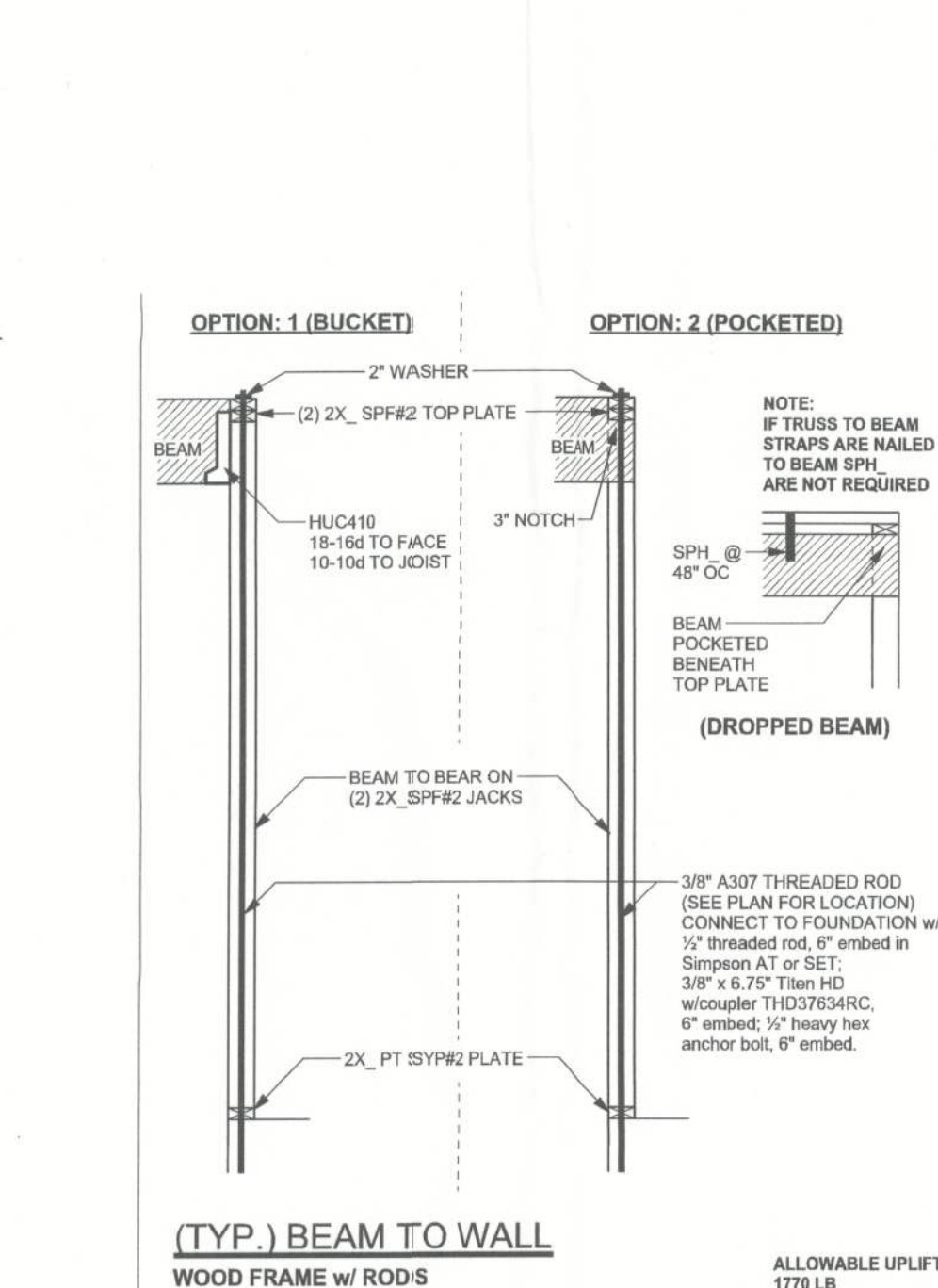
(TYP.) EXTERIOR WALL
@ NEW BUMP-OUT FOR EXTENDED BEDROOM



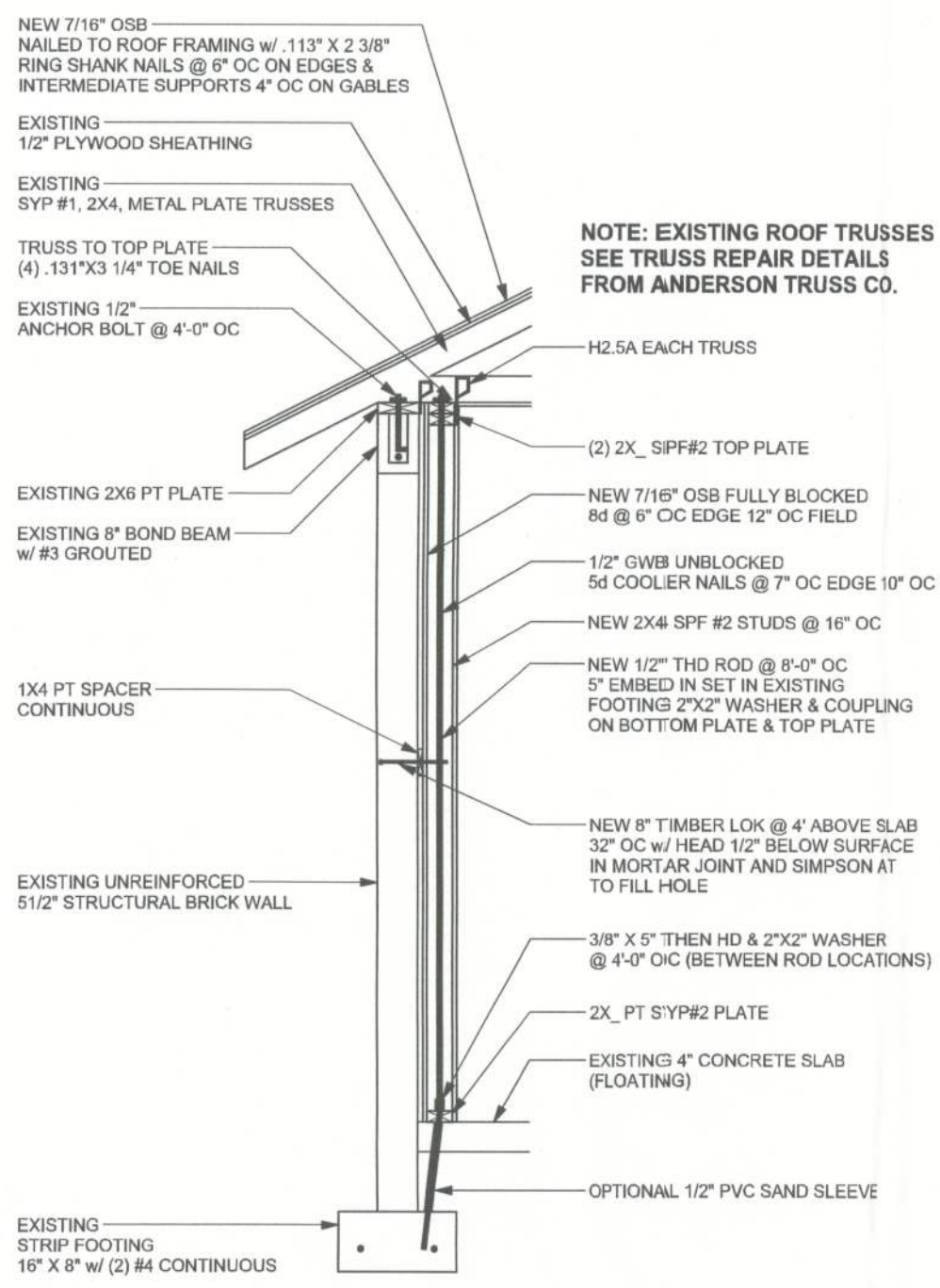
(TYP.) INTERSECTING WALL FRAMING
WOOD FRAME



(TYP.) CORNER FRAMING
WOOD FRAME

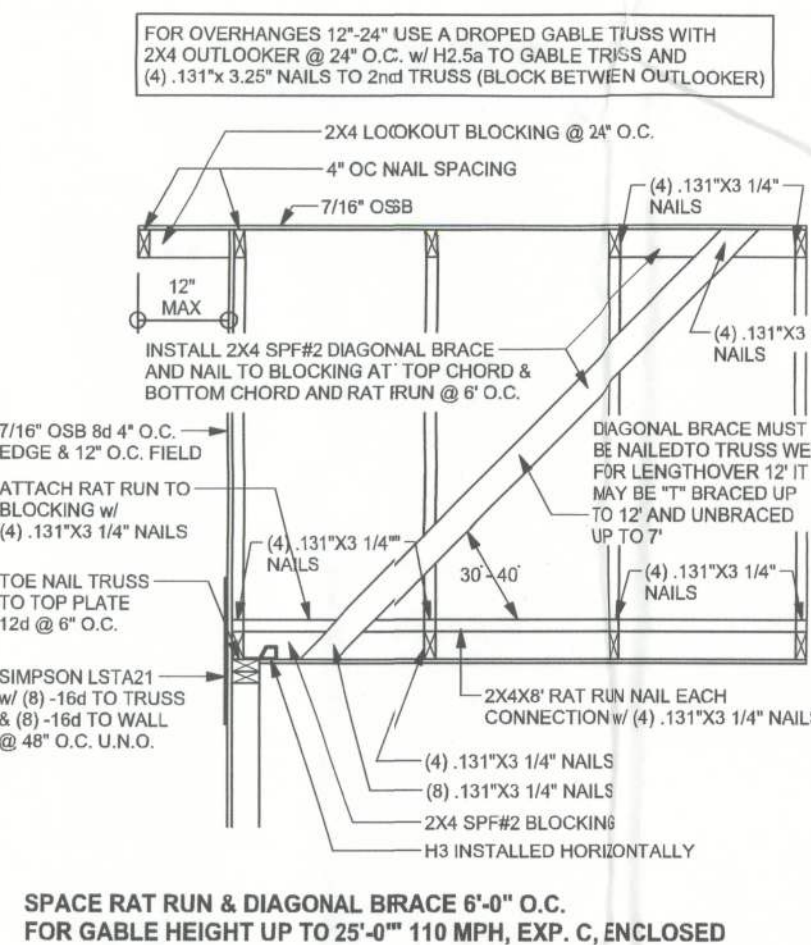
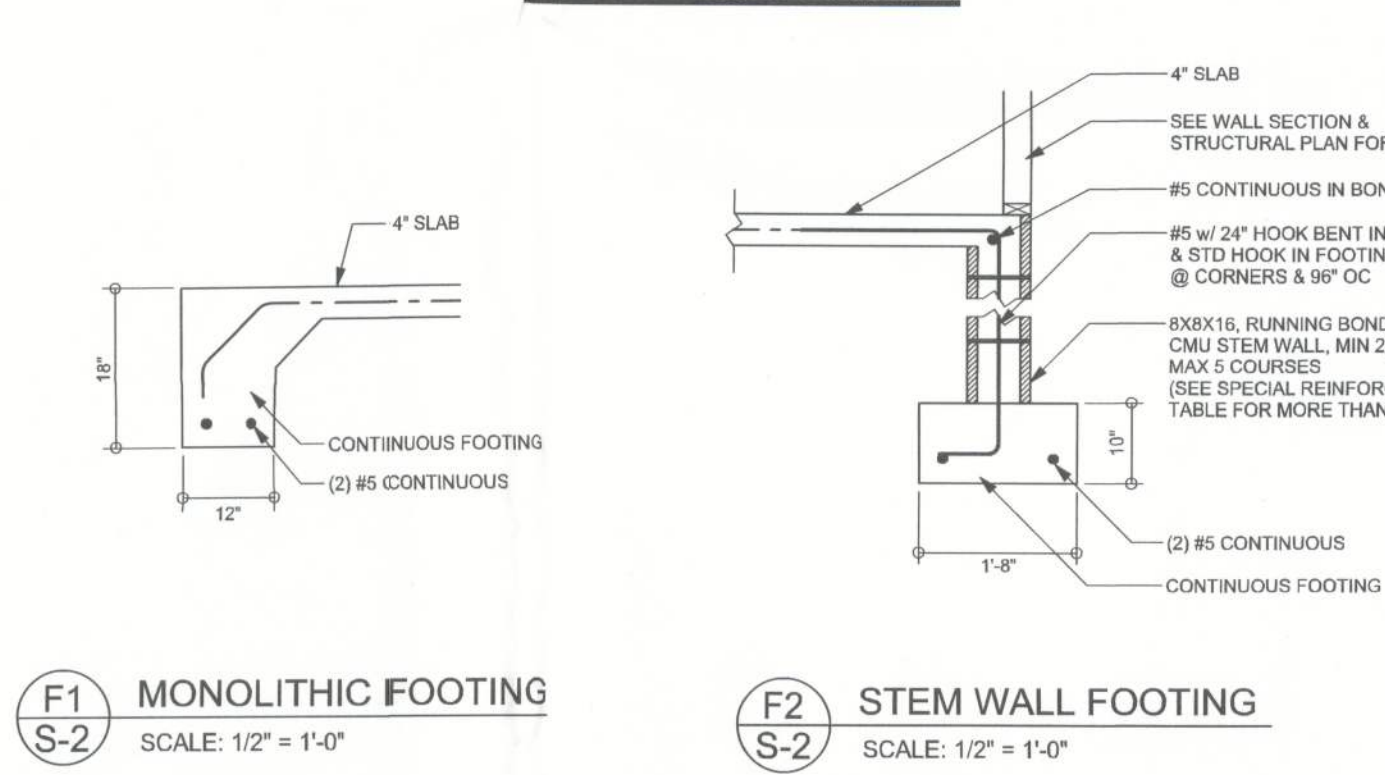


(TYP.) WALL CONNECTIONS
ONE STORY WOOD FRAME



(TYP.) EXTERIOR WALL
@ EXISTING STRUCTURAL BRICK LOCATIONS

NEW FOOTING DETAILS



(TYP.) GABLE BRACING DETAIL
WOOD FRAME

ANCHOR TABLE

OBTAI UPIT REQUIREMENTS FROM TRUSS MANUFACTURERS ENGINEERING

TRUSS CONNECTOR	UPLIFT SYP	UPLIFT SPF	F1 SYP	F2 SYP	F1 SPF	F2 SPF	TO RAFTER/TRUSS	TO PLATES
H5	455	265	115	200	100	170	4-8d x 1 1/2"	4-8d x 1 1/2"
H3	415	290	125	160	105	140	4-8d x 1 1/2"	4-8d x 1 1/2"
H2.5	415	365	150	130	130	130	5-8d x 1 1/2"	5-8d x 1 1/2"
H2.5A	480	480	110	110	110	110	5-8d x 1 1/2"	5-8d x 1 1/2"
H6	950	820					8-8d	8-8d
H8	745	565					5-10d x 1 1/2"	5-10d x 1 1/2"
H14-1	1465	1050	515	265	480	245	12-8d x 1 1/2"	13-8d
H14-2	1465	1050	515	265	480	245	12-8d x 1 1/2"	13-8d
H10	990	850	585	525	505	450	8-8d x 1 1/2"	8-8d x 1 1/2"
H10-2	760	655	455	395	390	340	6-10d	6-10d
H16	1470	1265					2-10d x 1 1/2"	10-10d x 1 1/2"
H16-2	1470	1265					2-10d x 1 1/2"	10-10d x 1 1/2"
LTS12-LTS20	1000	620					6-10d x 1 1/2"	6-10d x 1 1/2"
MTS12-MTS30	1000	860					7-10d x 1 1/2"	7-10d x 1 1/2"
HTS16-HTS30	1450	1245					12-10d x 1 1/2"	12-10d x 1 1/2"
HEAVY GIRDER TIEWDOWS								TO FOUNDATION
LG12	2050	1785	700	170	700	170	14-16d	14-16d
LG13-SDS2.5	3685	2655	795	410	795	410	12-SDS 1/4" x 2 1/2"	26-16dS
LG14-SDS3	4060	3860	2000	675	2000	675	12-SDS 1/4" x 3"	36-16dS
MG1	3965	3330					22-10d	5/8" ANCHOR
HGT-2	10980	6485					16-10d	2-5/8" ANCHOR
HGT-3	10530	9035					16-10d	2-5/8" ANCHOR
HGT-4	9250	9250					16-10d	2-5/8" ANCHOR
STUD STRAP CONNECTOR								TO STUDS
SSP DOUBLE TOP PLATE	435	435					3-16d	4-10d
SSP SINGLE SILL PLATE	455	420					1-10d	4-10d
DSP DOUBLE TOP PLATE	825	825					6-10d	8-10d
DSP SINGLE SILL PLATE	825	800					2-10d	8-10d
SP1	585	535					4-10d	6-10d
SP2	1065	805					6-10d	6-10d
SP4	885	760					8-10d x 1 1/2"	10-10d x 1 1/2"
SPH	1240	1065					10-10d x 1 1/2"	10-10d x 1 1/2"
SP6	885	760					6-10d x 1 1/2"	10-10d x 1 1/2"
SPH6	1240	1065					10-10d x 1 1/2"	10-10d x 1 1/2"
LSTA18	1235	1110					14-10d	16-10d
LSTA21	1235	1235					16-10d	16-10d
CS20	1030	1030					14-10d	16-10d
CS16	1705	1705					22-10d	22-10d
STUD ANCHORS								TO FOUNDATION
LTT19	1350	1305					8-16d	1/2" ANCHOR
LTT131	2310	2310					18-10d x 1 1/2"	5/8" ANCHOR
HD2A	2775	2570					2-5/8" BOLTS	5/8" ANCHOR
HTT16	4175	3685					18-16d	5/8" ANCHOR
HTT22	5260	5250					32-16d	5/8" ANCHOR
ABU44	2200	2200					12-16d	5/8" ANCHOR
ABU66	2300	2300					12-16d	5/8" ANCHOR
ABU88	2320	2320					18-16d	2-5/8" ANCHOR

(1) w/ INSTALLATION OF 4-16dS OPTIONAL NAIL HOLES
(2) FOR SYP GIRDER & SPF STUDS

MASONRY NOTES:

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602), THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

	ACI530.1-02 Section	Specific Requirements
1.4A	Compressive strength	8" block bearing walls Fm = 1500 psi
2.1	Mortar	ASTM C 270, Type N, UNO
2.2	Grout	ASTM C 476, admixtures require approval
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block.
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 60, Fy = 60 ksi, Lap splices min 48 bar dia. (30" for #5)
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class 950, 0.60 oz/ft ² or 304SS
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft ² or 304SS
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 10'-1" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 11'-2" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 15'-7" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 17'-3" STUD HEIGHT

GRADE & SPECIES TABLE

		Fb (psi)	E (10 ⁶ psi)
2x6	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LVL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	1600	1.9
PSL	PARALAM	2900	2.0

THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.208A. EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 12" GYP INTERIOR RESISTING INTERIOR ZONE WINDLOADS, 130 MPH, EXPOSURE C. STUD DEFLECTION LIMIT H/240 NOT OK FOR SOME BRITTLE FINISHES. STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 16" O.C. x 9.8 = 12.8" O.C.)

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE 2010 FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS. TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END, 2X6 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN.

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVIDES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, Fc = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 8" x 8" W1 x W1.4, Fy = 60 KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT, FIBER LENGTH 1 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIO OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12 FT. DO NOT CUT W/M OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, Fy = 60 KSI, ALL LAP SPICES 40" DB (2' FOR #6 BARS), UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GULAM BEAMS: GLB 24F-V3SP, Fb = 2.4kl, E = 1800kl, UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALC.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS. 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (1.5"), 8dC PANEL EDGES, 12dC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY, 4dC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBERS FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: 4-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64", WITH 5/8" BOLTS TO BE 3" x 3" x 9/64", WITH 3/4" BOLTS TO BE 2" x 2" x 9/64", WITH 7/8" BOLTS TO BE 3" x 3" x 9/64", UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH 2010 FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH 2010 FBCR, SECTION R031.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF 2010 FBCR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

DESIGN DATA

WIND LOADS PER 2010 FLORIDA BUILDING CODE RESIDENTIAL, SECTION R301.2.1	
(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS, MEAN ROOF HEIGHT)	
BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE	
BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION	
1.) BASIC WIND SPEED = 130 MPH, (3 SEC GUST, 33 FT, EXP. C)	
2.) WIND EXPOSURE = C, BUILDER MUST FIELD VERIFY	
3.) TOPOGRAPHIC FACTOR = 1.0, BUILDER MUST FIELD VERIFY	
4.) RISK CATEGORY = II, (MRI = 700 YR)	
5.) ROOF ANGLE = 7-45 DEGREES	
6.) MEAN ROOF HEIGHT = <30 FT	
7.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)	
8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))	

Zone	Effective Wind Area (ft ²)	
1	10	
2	39 -43	
3	39 -100	
4	43 -46	
5	43 -57	
Garage Door		
2010 FBCR, Table R301.2.4(4)		
8x7 Garage Door	37	-42
16x7 Garage Door	36	-40

DESIGN LOADS	
FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)
	30 PSF (SLEEPING ROOMS)
	30 PSF (ATTICS WITH STORAGE)
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)
ROOF	20 PSF (FLAT OR <4:12)
	16 PSF (4:12 TO <12:12)
	12 PSF (12:12 AND GREATER)
STAIRS	40 PSF (ONE & TWO FAMILY DWELLINGS)
SOIL BEARING CAPACITY	1000PSF
(NOT IN FLOOD ZONE (BUILDER TO VERIFY))	

REVISIONS

NO.	DESCRIPTION

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

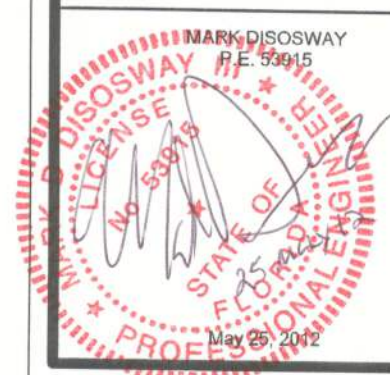
WINDLOAD ENGINEER: Mark Dicosway
PE No. 53915, POB 868, Lake City, FL
32056, 386-754-5419

DIMENSIONS:
Shaded dimensions supersede scaled dimensions. Refer all questions to Mark Dicosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with Section R301.2.1, 2010 Florida Building Code Residential, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.



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Renovation

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FINALS DATE:
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S-1

OF 2 SHEETS

