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Alpine, an ITW Company 155 Harlem Ave North Building, 4th Floor Glenview, IL 60025 Phone: (800)755-6001 www.alpineitw.com

Florida Certificate of Product Approval #FL 1999 02/23/2023

COA #0 278

No. 70861

Site Information:	Page 1:
Customer: W. B. Howland Company, Inc.	Job Number: 23-8964
Job Description: Aaron Marc Company (Nickelson) - Lot 1 Church St	
Address: CHURCH ST	

Job Engineering Criteria:							
Design Code: FBC 7th Ed. 2020 Res.	IntelliVIEW Version: 22.02.00						
	JRef #: 1XNG2150013						
Wind Standard: ASCE 7-16 Wind Speed (mph): 130	Design Loading (psf): 40.00						
Building Type: Closed							

This package contains general notes pages, 12 truss drawing(s) and 3 detail(s).

Item	Drawing Number	Truss
1	054.23.1550.09267	A01
3	054.23.1550.26247	A03
5	054.23.1550.43880	A05
7	054.23.1551.30477	B01
9	054.23.1551.36280	HJ01
11	054.23.1551.59830	J03
13	A14015ENC160118	
15	GBLLETIN0118	

Item	Drawing Number	Truss
2	054.23.1550.17090	A02
4	054.23.1550.30177	A04
6	054.23.1551.12243	A06
8	054.23.1551.33457	B02
10	054.23.1551.52450	J02
12	054.23.1552.04873	J04
14	BRCLBSUB0119	

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed and detailed by the Building Designer.

Connector Plate Information:

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for of all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for of all load cases.

Max Web CSI= Maximum bending and axial Combined Stress Index for Webs for of all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment. W = Width of non-hanger bearing, in inches.

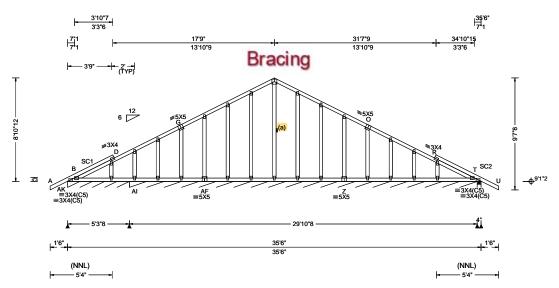
Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

References:

- 1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
- 2. ICC: International Code Council; www.iccsafe.org.
- 3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; www.alpineitw.com.
- 4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
- 5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www. sbcacomponents.com.

SEQN: 692831 GABL Ply: 1 Job Number: 23-8964 Cust: R 215 JRef: 1XNG2150013 T2 Qty: 1 FROM: CDM Aaron Marc Company (Nickelson) - Lot 1 Church St DrwNo: 054.23.1550.09267 Truss Label: A01 KD / WHK 02/23/2023



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.001 K 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.003 B 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.005 R
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.007 R
NCBCLL: 10.00	Mean Height: 15.00 ft TCDL: 5.0 psf	Building Code:	Creep Factor: 2.0
Soffit: 2.00	BCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.272
Load Duration: 1.25	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max BC CSI: 0.058
Spacing: 24.0 "	C&C Dist a: 3.55 ft	Rep Fac: Yes	Max Web CSI: 0.159
'	Loc. from endwall: Any	FT/RT:20(0)/10(0)	
	GCpi: 0.18	Plate Type(s):	
	Wind Duration: 1.60	WAVE	VIEW Ver: 22.02.00.0914.12
	•		

▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity /Rh /Rw /U /RL AK*91 /51 /51 AI* 78 /42 /-/15 301 /197 /33 Wind reactions based on MWFRS AK Brg Wid = 63.5 Min Req = -Brg Wid = 358 Min Req = Brg Wid = 4.0 Min Req = 1.5 (Truss) Bearings AK, AI, & T are a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #3; Stack Chord: SC1 2x4 SP #2; Stack Chord: SC2 2x4 SP #2;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 2X4 except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

The overall height of this truss excluding overhang is



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WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

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Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec. 2. For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

SEQN: 692834 COMN Ply: 1 Job Number: 23-8964 Cust: R 215 JRef: 1XNG2150013 T1 FROM: CDM Aaron Marc Company (Nickelson) - Lot 1 Church St DrwNo: 054.23.1550.17090 Qty: 5 Truss Label: A02 KD / WHK 02/23/2023 6'3"5 12'0"3 17'9" 23'5"13 29'2"11 35'6" 6'3"5 5'8"13 5'8"13 5'8"13 5'8"13 6'3"5 =5<u>X</u>5 Bracing **∌**4X6

B ≡3X4(A1)		M M M 3X4 =6X6	=3X4	K ≡5X5	=3X4	≡3)	H (6(A1)
<u> </u>	- 5'5"8	\		30'0"8 —			→
-1 '6"- -	5'5"8 5'5"8	1'11 <u>"</u> 10 7'5"2	6'10"10 14'3"11	6'10"10 21'2"5	6'10"10 28'0"14	7'5"2 35'6"	1'6"

Loading Criteria (psf) Wind Criteria Snow Criteria (Pg,Pf in P		Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.100 F 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.183 F 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.034 E
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.062 E
NCBCLL: 10.00	Mean Height: 15.00 ft TCDL: 5.0 psf	Building Code:	Creep Factor: 2.0
Soffit: 2.00	BCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.546
Load Duration: 1.25	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max BC CSI: 0.700
Spacing: 24.0 "	C&C Dist a: 3.55 ft	Rep Fac: Yes	Max Web CSI: 0.598
'	Loc. from endwall: Any	FT/RT:20(0)/10(0)	
	GCpi: 0.18	Plate Type(s):	
Wind Duration: 1.60		WAVE	VIEW Ver: 22.02.00.0914.12
Lumbar			

▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /Rh /Rw /U 190 /-151 /78 1980 /-/1020 /299 /-1432 /820 /231 Wind reactions based on MWFRS Brg Wid = 4.0 Min Req = 1.5 (Truss) Brg Wid = 4.0Min Req = 2.0 (Truss) Brg Wid = 4.0 Min Req = 1.7 (Truss) Bearings B, N, & H are a rigid surface. Members not listed have forces less than 375# **Maximum Top Chord Forces Per Ply (lbs)** Chords Tens.Comp. Chords Tens. Comp.

⊕9'1"2

B - C	671 - 219	E-F	781 - 1665
C - D	296 - 466	F-G	922 - 2285
D-F	669 - 1344	G - H	867 - 2414

Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #3;

(a) Continuous lateral restraint equally spaced on

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is

THIS TRUSS MUST BE INSTALLED AS SHOWN AND NOT END FOR END.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	s Tens.Comp.		Chords	Tens. Comp		
3 - N	315	- 552	K-J	1630	- 394	
M - L	1051	- 216	J - H	2087	- 631	
K	1097	- 148				

Maximum Web Forces Per Ply (lbs)

vvebs	rens.Comp.	vvebs	rens. (Jomp.
N-C	704 - 1915	E-K	886	- 339
C - M	1168 - 254	K-F	411	- 585
M - D	385 - 1226	F-J	572	- 228
C - M	1168 - 254	K-F	411	- 58



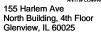
Flored Carrier at Product Approval #FL 1999

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FROM: CDM Aaron Marc Company (Nickelson) - Lot 1 Church St DrwNo: 054.23.1550.26247 Qty: 1 Truss Label: A03 KD / WHK 02/23/2023 6'3"5 12'0"3 17'9" 23'5"13 29'2"11 35'6" 6'3"5 5'8"13 5'8"13 5'8"13 5'8"13 6'3"5 =5<u>X</u>5 =3X4 K ≡5X5 =3X4 5'3"8 29'10"8 7'5"2 6'10"10 6'10"10 6'10"10 7'5"2 7'5"2 14'3"11 21'2"5 28'0"14 35'6'

Loading Criteria (psf) Wind Criteria		Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.123 K 999 240	
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.249 K 999 180	
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.046 H	
Des Ld: 40.00	EXP: C Kzt: NA Mean Height: 15.00 ft		HORZ(TL): 0.093 H	
NCBCLL: 10.00	TCDL: 5.0 psf	Building Code:	Creep Factor: 2.0	
Soffit: 2.00	BCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.415	
Load Duration: 1.25	MWFRS Parallel Dist: h/2 to h	TPI Std: 2014	Max BC CSI: 0.764	
Spacing: 24.0 "	C&C Dist a: 3.55 ft	Rep Fac: Yes	Max Web CSI: 0.463	
	Loc. from endwall: not in 9.00 ft	FT/RT:20(0)/10(0)		
	GCpi: 0.18	Plate Type(s):		
	Wind Duration: 1.60	WAVE	VIEW Ver: 22.02.00.0914.12	
Lumber				

Job Number: 23-8964

▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ /Rh /Rw /U /RL В 1258 /740 /245 /273 В* /46 71 /8 /905 1491 /-/266 Wind reactions based on MWFRS Brg Wid = 4.0 Min Req = 1.5 (Truss) Brg Wid = 63.5 Min Req = -Brg Wid = 4.0 Min Req = 1.8 (Truss) Bearings B, B, & H are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Cust: R 215 JRef: 1XNG2150013 T3

546 - 1816 B - C E-F 469 - 2041 C-D 515 - 1911 F-G 591 - 2404 D-E 535 - 1749 - 2534 G-H 546

Wind

SEQN: 692837

COMN

Ply: 1

Wind loads based on MWFRS with additional C&C

Bot chord: 2x4 SP #2; B1 2x4 SP M-31;

Wind loading based on both gable and hip roof types.

Additional Notes

Top chord: 2x4 SP #2;

Webs: 2x4 SP #3;

The overall height of this truss excluding overhang is

THIS TRUSS MUST BE INSTALLED AS SHOWN AND NOT END FOR END.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.		Chords	Tens. Comp.		
B - M	3501	- 625	K-J	1758	- 254	
M - L	1617	- 231	J - H	2193	- 384	
L-K	1296	- 100				

Maximum Web Forces Per Ply (lbs) Webs Tens Comp

	rono.comp.	******	rono. Comp.	
L-E E-K	541 - 147 707 - 179		252 - 569 535 - 115	



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WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

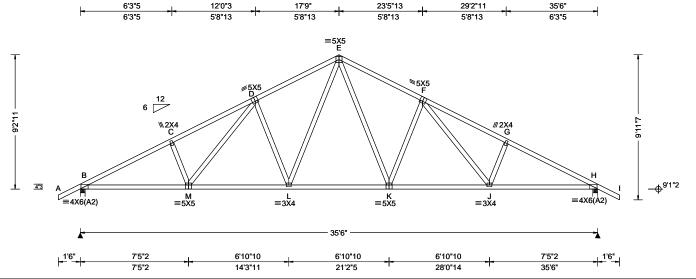
IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

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SEQN: 692840 COMN Ply: 1 Job Number: 23-8964 Cust: R 215 JRef: 1XNG2150013 T4 FROM: CDM Aaron Marc Company (Nickelson) - Lot 1 Church St DrwNo: 054.23.1550.30177 Qty: 13 Truss Label: A04 KD / WHK 02/23/2023



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria	4
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.55 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.181 L 999 240 VERT(CL): 0.334 L 999 180 HORZ(LL): 0.071 H HORZ(TL): 0.130 H Creep Factor: 2.0 Max TC CSI: 0.459 Max BC CSI: 0.844 Max Web CSI: 0.470 VIEW Ver: 22.02.00.0914.12	
Lumber				ì

▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /Rh /Rw / U /RL В 1731 /-/940 /276 /273 /940 /-1731 /276 Wind reactions based on MWFRS Brg Wid = 4.0Min Reg = 2.0 (Truss) Brg Wid = 4.0 Min Req = 2.0 (Truss) Bearings B & H are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. 573 - 3053 C - D 618 - 2923 F-G 618 - 2924 D-E 574 - 2325 G-H 572 - 3055

Top chord: 2x4 SP #2;

Bot chord: 2x4 SP #2; Webs: 2x4 SP #3;

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is

Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

B - M 2655 - 410 K - J 2216 - 278 2216 - 281 M - L J - H 2656 - 407 L-K 1687 - 124

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens.	Comp.
——— М - D	547 - 115	E-K	882	- 180
D-L	253 - 576	K-F	252	- 577
L-E	885 - 179	F-J	550	- 113



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SEQN: 692843 COMN Ply: 1 Job Number: 23-8964 Cust: R 215 JRef: 1XNG2150013 T5 DrwNo: 054.23.1550.43880 FROM: CDM Aaron Marc Company (Nickelson) - Lot 1 Church St Qty: 2 Truss Label: A05 KD / WHK 02/23/2023 6'3"5 12'0"3 23'5"13 29'2"11 35'6" 6'3"5 5'8"13 5'8"13 5'8"13 6'3"5 Bracing K ≡5X5 M ≡5X5 =3X4(A1) =3X4 =3X4 14'8"

6'6"5

21'2"5

20'10"

6'10"10

28'0"14

Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
1.0220.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.041 J 999 240
DCLL. 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.080 J 999 180
	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.012 E
Dec I d: 10 00	EXP: C Kzt: NA Mean Height: 15.00 ft		HORZ(TL): 0.025 E
NCBCLL: 10.00	TCDL: 5.0 psf	Building Code:	Creep Factor: 2.0
Soffit: 2.00	BCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.538
	MWFRS Parallel Dist: h/2 to h	TPI Std: 2014	Max BC CSI: 0.506
Spacing: 24.0 "	C&C Dist a: 3.55 ft	Rep Fac: Yes	Max Web CSI: 0.646
	Loc. from endwall: not in 4.50 ft	FT/RT:20(0)/10(0)	
	GCpi: 0.18	Plate Type(s):	
	Wind Duration: 1.60	WAVE	VIEW Ver: 22.02.00.0914.12

7'2"14

▲ N	laximu	ım Reac	tions (lb	s)			
	G	ravity		No	n-Grav	/ity	
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL	
В	525	/-	/-	/325	/87	/273	
L	2159	/-	/-	/1046	/322	/-	
Н	878	/-	/-	/572	/144	/-	
Wir	nd read	tions ba	sed on M	IWFRS			
В	Brg V	Vid = 4.0	Min R	eq = 1.5	(Truss	s)	
L	Brg V	Vid = 4.0	Min R	eq = 2.2	(Truss	s)	
Н	Brg V	Vid = 4.0	Min R	eq = 1.5	(Truss	s)	
Bea	arings I	B, L, & H	are a rig	gid surfac	e.		
Mei	mbers	not listed	have fo	rces less	than 3	375#	
Maximum Top Chord Forces Per Ply (lbs)							
Cho	ords T	ens.Cor	np. C	Chords	Tens.	Comp.	

<u> 1'6"</u>

7'5"2

35'6

F-G B - C - 1096 83 267 D-E 664 0 G-H 222 - 1225 E-F 222

Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

K-J 1028 - 96 547 .I - H

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. 0	Comp.
M - D	646 - 120	E-K	911	- 207
D - L	278 - 643	K-F	278	- 634
L-E	245 - 1364	F-J	610	- 118

Lumber

Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #3;

(a) Continuous lateral restraint equally spaced on

7'5"2

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is

THIS TRUSS MUST BE INSTALLED AS SHOWN AND NOT END FOR END.



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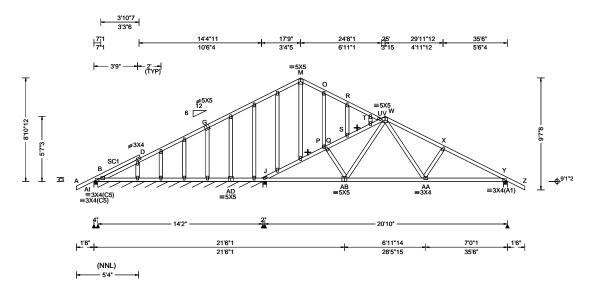
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SEQN: 692871 GABL Ply: 1 Job Number: 23-8964 Cust: R 215 JRef: 1XNG2150013 T11 Qty: 1 FROM: CDM Aaron Marc Company (Nickelson) - Lot 1 Church St DrwNo: 054.23.1551.12243 Page 1 of 2 Truss Label: A06 KD / WHK 02/23/2023



Loading	Criteria (psf)	Wind Criteria	Snow Criteria	(Pg,F	Pf in PSF)	Defl/CSI Cr	iteria			
TCLL:	20.00	Wind Std: ASCE 7-16	Pg: NA Ct: I	NA	CAT: NA	PP Deflection	on in	loc L	/defl	L/#
TCDL:	10.00	Speed: 130 mph	Pf: NA		Ce: NA	VERT(LL):	0.05	4 R	999	240
BCLL:	0.00	Enclosure: Closed	Lu: NA Cs:	: NA		VERT(CL):	0.10	9 R	999	180
BCDL:	10.00	Risk Category: II	Snow Duration	n: NA		HORZ(LL):	0.02	0 S	-	-
Des Ld:	40.00	EXP: C Kzt: NA Mean Height: 15.00 ft				HORZ(TL):	0.04	0 S	-	-
NCBCLL:	10.00	TCDL: 5.0 psf	Building Code:	:		Creep Facto	or: 2.0)		
Soffit:	2.00	BCDL: 5.0 psf	FBC 7th Ed. 20	020 R	es.	Max TC CS	I: C	.312		
Load Dura	ation: 1.25	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	4		Max BC CS	l: 0).511		
Spacing: 2	24.0 "	C&C Dist a: 3.55 ft	Rep Fac: Varie	es by l	_d Case	Max Web C	SI: C).178		
		Loc. from endwall: not in 4.50 ft	FT/RT:20(0)/10	0(0)						
		GCpi: 0.18	Plate Type(s):							
		Wind Duration: 1.60	WAVE			VIEW Ver: 2	22.02	.00.09	14.12	2

Lumber

Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #3; Stack Chord: SC1 2x4 SP #2;

Special Loads

(Lumber	Dur Fac =1	25 / Plate [Our Fac =1.3	25)
TC: From	62 plf at	-1.50 to	62 plf at	7.06
TC: From	31 plf at	7.06 to	31 plf at	11.06
TC: From	62 plf at	11.06 to	62 plf at	37.00
BC: From	4 plf at	-1.50 to	4 plf at	0.00
BC: From	20 plf at	0.00 to	20 plf at	7.06
BC: From	10 plf at	7.06 to	10 plf at	11.06
BC: From	20 plf at	11.06 to	20 plf at	35.50
BC: From	4 plf at	35.50 to	4 plf at	37.00
TC: 322 lb	Conc. Load	at 7.06	•	
TC: 440 lb	Cana Land	-4 0 00 44	06	

19 lb Conc. Load at 9.06,11.06 143 lb Conc. Load at 7.06 89 lb Conc. Load at 9.06,11.06 BC: BC:

Plating Notes

All plates are 2X4 except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind loads and reactions based on MWFRS.

Wind loading based on both gable and hip roof types.

Blocking

Blocking reinforcement required to prevent buckling of members over the bearings: Bearing 3 located at 14.5' (blocking >= 29.54" if used)

Member to be laterally braced for horizontal wind loads. bracing system to be desiged and furnished by others.

)	Loc	R+	/ R-	/ Rh	/ Rw	/U	-
)	ΑI	315	/-	/-	/-	/93	,
	B*	115	/-	/-	/-	/23	,
	J	930	/-	/-	/-	/86	,
	Υ	968	/-	/-	/-	/196	,
	Wir	nd rea	ctions b	ased on I	MWFRS		
	ΑI	Brg \	Nid = 4	.0 Min	Req = 1.5	(Trus	s)
	В	Brg \	Nid = 1	70 Min l	Req = -		
	J	Brg \	Nid = 4	.0 Min l	Req = 1.5	(Trus	s)
	Υ	Brg \	Nid = 4	.0 Min l	Req = 1.5	(Trus	s)
	Bea	arings	AI, B, J	l, & Y are	a rigid su	rface.	
							_

▲ Maximum Reactions (lbs), or *=PLF

Non-Gravity

/RL /-/-

/-

Gravity

Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Chords Tens.Comp. Tens. Comp.

J - P	261 - 1410	U - V	170	- 1149
P-Q	199 - 1227	V - W	175	- 1138
Q - S	194 - 1215	W - X	231	- 1292
S - U	180 - 1180	X - Y	290	- 1444

Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. J -AB 1207 - 237 AA-Y 1231 - 234 AB-AA 833 - 169

Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. 441 W -AA 469 - 24



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SEQN: 692871 GABL Ply: 1 Job Number: 23-8964 Cust: R 215 JRef: 1XNG2150013 T11 FROM: CDM DrwNo: 054.23.1551.12243 Qty: 1 Aaron Marc Company (Nickelson) - Lot 1 Church St Page 2 of 2 Truss Label: A06 KD / WHK 02/23/2023

Additional Notes

See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

The overall height of this truss excluding overhang is 8-10-12.



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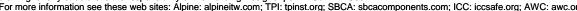
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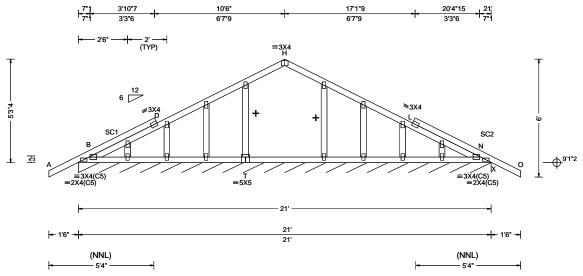
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SEQN: 692852 GABL Ply: 1 Job Number: 23-8964 Cust: R 215 JRef: 1XNG2150013 T8 FROM: CDM Aaron Marc Company (Nickelson) - Lot 1 Church St DrwNo: 054.23.1551.30477 Qty: 1 Truss Label: B01 KD / WHK 02/23/2023



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.004 H 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.008 H 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.002 L
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.003 I
NCBCLL: 10.00	Mean Height: 15.00 ft TCDL: 5.0 psf	Building Code:	Creep Factor: 2.0
Soffit: 2.00	BCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.276
l	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max BC CSI: 0.095
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Fac: Yes	Max Web CSI: 0.051
	Loc. from endwall: Any	FT/RT:20(0)/10(0)	
	GCpi: 0.18	Plate Type(s):	
	Wind Duration: 1.60	WAVE	VIEW Ver: 22.02.00.0914.12
Lumber			

▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ /R /Rh /Rw /U /RL /47 X* 92 /-/-Wind reactions based on MWFRS X Brg Wid = 252 Min Req = Bearing B is a rigid surface. Members not listed have forces less than 375#

Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #3; Stack Chord: SC1 2x4 SP #2;

Stack Chord: SC2 2x4 SP #2;

Plating Notes

All plates are 2X4 except as noted.

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

The overall height of this truss excluding overhang is 5-3-4.

+ Member to be laterally braced for horizontal wind loads. bracing system to be desiged and furnished by others.



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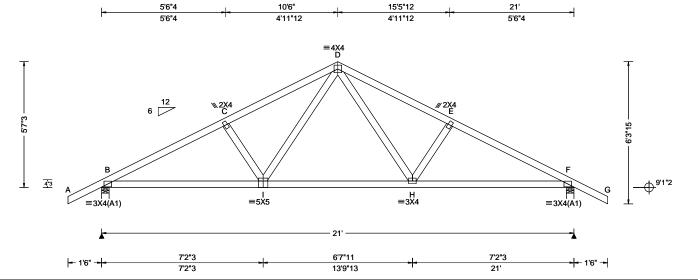
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SEQN: 692846 COMN Ply: 1 Job Number: 23-8964 Cust: R 215 JRef: 1XNG2150013 T7 FROM: CDM DrwNo: 054.23.1551.33457 Qty: 8 Aaron Marc Company (Nickelson) - Lot 1 Church St Truss Label: B02 KD / WHK 02/23/2023



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria	4
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA	PP Deflection in loc L/defl L/# VERT(LL): 0.045 H 999 240 VERT(CL): 0.091 H 999 180 HORZ(LL): 0.018 F	E
Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18	Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s):	-HORZ(TL): 0.036 F Creep Factor: 2.0 Max TC CSI: 0.255 Max BC CSI: 0.507 Max Web CSI: 0.171	
	Wind Duration: 1.60	WAVE	VIEW Ver: 22.02.00.0914.12] [
Lumber				-

	▲ Maxii	mum Rea	ections (I	lbs)			
		Gravity		Non-Gravity			
	Loc R-	+ /R-	/ Rh	/ Rw	/ U	/ RL	
	B 965	5 /-	/-	/584	/173	/173	
	F 965	5 /-	/-	/584	/173	/-	
	Wind re	actions b	ased on I	MWFRS			
	B Bro	Wid = 4	.0 Min	Reg = 1.5	(Trus	s)	
	F Bro	Wid = 4	.0 Min	Reg = 1.5	ī (Trus	s)	
	Bearing	sB&Fa	re a rigid	surface.	•	,	
			_	orces less	s than 3	375#	
	Maximu	um Top (Chord Fo	rces Per	Ply (lb	s)	
				Chords		•	
	B-C	620 -	1433	D-E	623	- 1268	
_	C-D	623 -		E-F	620	- 1434	

Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2: Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is

Maximum Bot Chord Forces Per Ply (lbs)

Cilolus	rens.comp.		Cilolus	i ciis. v	Jonep.	
B-I	1222	- 455	H-F	1222	- 431	
L-H	832	- 211				

Maximum Web Forces Per Ply (lbs)

vebs	rens.Comp.	webs	Tens. Comp.	
- D	448 - 189	D - H	449 - 188	



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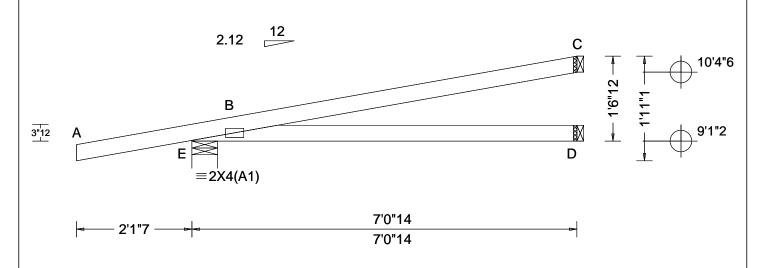
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SEQN: 692866 HIP_ Ply: 1 Job Number: 23-8964 Cust: R 215 JRef: 1XNG2150013 T17 FROM: CDM DrwNo: 054.23.1551.36280 Qty: 1 Aaron Marc Company (Nickelson) - Lot 1 Church St Truss Label: HJ01 KD / WHK 02/23/2023



TCDL: 10.00 Speed: 130 mph BCLL: 0.00 Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf Spacing: 24.0 " C&C Dist a: 3.00 ft Loc. from endwall: NA Ff: NA Ce: NA VERT(LL): NA VERT(LL)	Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
I Wiembers not listed have lorces less than 373	TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.010 B HORZ(TL): 0.018 B Creep Factor: 2.0 Max TC CSI: 0.639 Max BC CSI: 0.360	Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL E 299 /- /- /- /75 /- D 85 /- /- /26 /- /- C 203 /- /- /- /76 /- Wind reactions based on MWFRS E Brg Wid = 5.7 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = -
Wind Duration: 1.60 WAVE WIEW Ver: 22.02.00.0914.12		GCpi: 0.18	Plate Type(s):	VIEW Ver: 22 02 00 0914 12	Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2;

Hipjack supports 5-0-0 setback jacks with no webs.

Wind loads and reactions based on MWFRS.

Wind loading based on both gable and hip roof types.

The overall height of this truss excluding overhang is



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WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

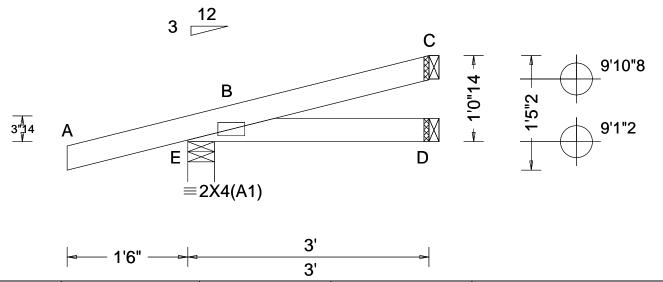
IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

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Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec. 2.

155 Harlem Ave North Building, 4th Floor Glenview, IL 60025

SEQN: 692857 JACK Ply: 1 Job Number: 23-8964 Cust: R 215 JRef: 1XNG2150013 T14 DrwNo: 054.23.1551.52450 FROM: CDM Qty: 2 Aaron Marc Company (Nickelson) - Lot 1 Church St Truss Label: J02 KD / WHK 02/23/2023



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): NA
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): NA
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.001 B
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.001 B
NCBCLL: 10.00	Mean Height: 15.00 ft TCDL: 5.0 psf	Building Code:	Creep Factor: 2.0
Soffit: 2.00	BCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.252
Load Duration: 1.25	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max BC CSI: 0.058
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Fac: Yes	Max Web CSI: 0.000
	Loc. from endwall: Any	FT/RT:20(0)/10(0)	
	GCpi: 0.18	Plate Type(s):	
	Wind Duration: 1.60	WAVE	VIEW Ver: 22.02.00.0914.12
Lumber			

▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /Rh /Rw /U /RL 255 /145 /41 D 46 /-/26 59 /27 Wind reactions based on MWFRS Brg Wid = 4.0 Min Req = 1.5 (Truss) Brg Wid = 1.5 Min Req = -Brg Wid = 1.5 Min Req = -Bearing E is a rigid surface. Members not listed have forces less than 375#

Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2;

Wind loads based on MWFRS with additional C&C

Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 1-0-14.



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SEQN: 692868 MONO Ply: 1 Job Number: 23-8964 Cust: R 215 JRef: 1XNG2150013 T13 DrwNo: 054.23.1551.59830 FROM: CDM Qty: 1 Aaron Marc Company (Nickelson) - Lot 1 Church St Truss Label: J03 KD / WHK 02/23/2023 10'4"6 3" 4 D \equiv 2X4(A1) **|||2X4** 5' 1'6" -5 Loading Criteria (psf) Wind Criteria Snow Criteria (Pg,Pf in PSF) Defl/CSI Criteria ▲ Maximum Reactions (lbs) Gravity Non-Gravity on in loc L/defl L/# Loc R+ /Rh /Rw /U /RL NA NA Ε 323 0.004 B 143 /-/-/40 /-D

TCLL: 2	20.00	Wind Std: ASCE 7-16	Pg: NA	Ct: NA	CAT: NA	PP Deflection
TCDL:	10.00	Speed: 130 mph	Pf: NA		Ce: NA	VERT(LL): 1
BCLL:	0.00	Enclosure: Closed	Lu: NA	Cs: NA		VERT(CL): 1
BCDL:	10.00	Risk Category: II EXP: C Kzt: NA	Snow Dui	ration: NA		HORZ(LL):
Des Ld: 4	40.00	Mean Height: 15.00 ft				HORZ(TL):
NCBCLL: 1	10.00	TCDL: 5.0 psf	Building C	Code:		Creep Factor
Soffit:	2.00	BCDL: 5.0 psf	FBC 7th E	Ed. 2020 I	Res.	Max TC CSI:
Load Durat	tion: 1.25	MWFRS Parallel Dist: 0 to h/2	TPI Std:	-		Max BC CSI:
Spacing: 2	4.0 "	C&C Dist a: 3.00 ft	Rep Fac:	Varies by	Ld Case	Max Web CS
		Loc. from endwall: not in 4.50 ft	FT/RT:20			
		GCpi: 0.18	Plate Typ	e(s):		
		Wind Duration: 1.60	WAVE			VIEW Ver: 22
Lumbar		·				

0.008 B or: 2.0 0.295 0.214 SI: 0.067 22.02.00.0914.12

322 Wind reactions based on MWFRS Brg Wid = 4.0 Min Req = 1.5 (Truss) Brg Wid = 1.5 Min Req = -Brg Wid = 1.5 Min Req = -Bearing E is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #3;

Special Loads

--(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25) 61 plf at 4 plf at 20 plf at TC: From -1.50 to -1.50 to 61 plf at 4 plf at 5.00 BC: From 0.00 BC: From 0.00 to 20 plf at 203 lb Conc. Load at 5.00 85 lb Conc. Load at 5.00

Wind

Wind loads and reactions based on MWFRS. Right end vertical not exposed to wind pressure. Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 1-6-14.



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155 Harlem Ave North Building, 4th Floor Glenview, IL 60025

SEQN: 692873 MONO Ply: 1 Job Number: 23-8964 Cust: R 215 JRef: 1XNG2150013 T10 Aaron Marc Company (Nickelson) - Lot 1 Church St DrwNo: 054.23.1552.04873 FROM: CDM Qty: 3 Truss Label: J04 KD / WHK 02/23/2023 10'4"6 3" 4 D \equiv 2X4(A1) **|||2X4** 5' 5 Mind Crit ▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /Rh /Rw /U /RL Ε 323 /179 /56

Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Deti/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): NA
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): NA
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.004 B
Des Ld: 40.00	EXP: C Kzt: NA Mean Height: 15.00 ft		HORZ(TL): 0.008 B
NCBCLL: 10.00	TCDL: 5.0 psf	Building Code:	Creep Factor: 2.0
Soffit: 2.00	BCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.333
Load Duration: 1.25	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max BC CSI: 0.199
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Fac: Yes	Max Web CSI: 0.083
	Loc. from endwall: Any	FT/RT:20(0)/10(0)	
	GCpi: 0.18	Plate Type(s):	
	Wind Duration: 1.60	WAVE	VIEW Ver: 22.02.00.0914.12
Lumber			

89 /-/49 /48 119 /49 Wind reactions based on MWFRS Brg Wid = 4.0 Min Req = 1.5 (Truss) Brg Wid = 1.5 Min Req = -Brg Wid = 1.5 Min Req = -Bearing E is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

The overall height of this truss excluding overhang is



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Gable Stud Reinforcement Detail

ASCE 7-16: 140 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Dr: 120 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00

Dr: 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00

Or: 100 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure D. Kzt = 1.00

Spacing Species Grade Spacing Species Spacing Species Grade Spacing Species Species							•			. 					
Spacing Species Grade Spacing Species Spacing Species Spacing Spacing Species Spacing S				Brace	No	(1) 1×4 "L	" Brace *	(1) 2×4 "L	" Brace *	(2) 2×4 L	Brace **	(1) 2×6 *L	" Brace *	(2) 2×6 L	Brace **
SPF #1 / #2 4' 3' 7' 7' 7' 8' 6' 8' 10' 10' 1' 10' 6' 13' 4' 13' 10' 14' 0' 14'	_					Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
The Stud 4' 1' 6' 7' 7' 1' 8' 6' 8' 10' 10' 1' 10' 6' 13' 4' 13' 10' 14' 0' 14'	1		CDE	#1 / #2	4′ 3″	7′ 3″	7' 7"	8′ 7 ″	8′ 11″	10′ 3″	10′ 8 ″	13′ 6″	14' 0"	14' 0"	14′ 0″
He Standard 4 1 5 8 6 0 7 7 8 6 8 10 11 10 6 13 4 13 10 14 0 14		1.7	12 L L	#3	4′ 1″	6′ 7 ″	7′ 1″	8′ 6 ″	8′ 10 ″	10′ 1″	10′ 6″	13′ 4″	13′ 10 ″	14′ 0″	14′ 0″
SP	21	_	ᅵᆸᆮ	Stud	4′ 1″	6′ 7 ″	7′ 0 ″	8′ 6 ″	8′ 10 ″	10′ 1″	10′ 6″	13′ 4″	13′ 10 ″	14′ 0″	14′ 0 ″
SP	>	0	1 11	Standard											
The color of the	ĮΨĮ				4′ 6 ″	7′ 4″		8′ 8 ″	9′ 0″	10′ 4″	10′ 9 ″	13′ 8 ″	14′ 0″	14' 0"	
Standard	$ \bot $		125					8′ 7 ″		10′ 3″			14′ 0″	14′ 0″	
Standard 4' 0' 5' 3' 5' 7' 7' 0' 7' 6' 9' 6' 10' 2' 11' 0' 11' 10' 14' 0		4	l	#3		6′ 0 ″		7′ 11″							
Standard 4' 0' 5' 3' 5' 7' 7' 0' 7' 6' 9' 6' 10' 2' 11' 0' 11' 10' 14' 0		N	IDF L	Stud					8′ 6 ″			12′ 5 ″	13′ 4″		
H		. –		Standard											
HF Stud 4' 8' 8' 1' 8' 6' 9' 8' 10' 1' 11' 7' 12' 1' 14' 0'	1.91		CDE		4′ 11″	8′ 4″	8′ 8 ″	9′ 10″	10′ 3″	11′ 8″	12′ 2 ′	14′ 0″	14′ 0″	14′ 0″	14′ 0″
Studard 4'8' 8'1' 8'6' 9'8' 10'1' 11'7' 12'1' 14'0' 14'0' 14'0' 14'0' 14'0' Standard 4'8' 6'11' 7'5' 9'3' 9'11' 11'7' 12'1' 14'0' 14'0' 14'0' 14'0' 14'0' #1 5'1' 8'5' 8'9' 9'11' 10'4' 11'18' 12'2' 14'0' 14'0' 14'0' 14'0' 14'0' #2 4'11' 8'4' 8'8' 9'10' 10'3' 11'8' 12'2' 14'0' 14'0' 14'0' 14'0' #3 4'9' 7'4' 7'9' 9'9' 10'2' 11'8' 12'1' 14'0' 14'0' 14'0' 14'0' Standard 4'8' 6'5' 6'10' 8'7' 9'2' 11'8' 12'1' 14'0' 14'0' 14'0' 14'0' Standard 4'8' 6'5' 9'10' 10'3' 11'8' 12'1' 14'0' 14'0' 14'0' 14'0' Standard 5'1' 9'0' 9'4' 10'8' 11'1' 12'9' 13'3' 14'0' 14'0' 14'0' 14'0' Standard 5'1' 8'0' 8'6' 10'8' 11'1' 12'9' 13'3' 14'0' 14'0' 14'0' 14'0' #1 5'8' 9'3' 9'8' 10'11' 11'4' 13'0' 13'6' 14'0' 14'0' 14'0' 14'0' Standard 5'1' 8'0' 8'6' 10'8' 11'1' 12'9' 13'3' 14'0' 14'0' 14'0' 14'0' #1 5'8' 9'3' 9'8' 10'11' 11'4' 13'0' 13'6' 14'0' 14'0' 14'0' 14'0' Standard 5'1' 8'5' 9'0' 10'9' 11'2' 12'10' 13'4' 14'0' 14'0' 14'0' 14'0' Standard 5'1' 7'5' 7'11' 9'11' 10'7' 12'9' 13'3' 14'0' 14'0' 14'0' 14'0' 14'0'	+>		12LL	#3									14′ 0″		
Standard	;	U		Stud								14′ 0″	14′ 0″		
SP	1 1	Ō	1 11										14′ 0″		
Hardward	1 🖑 1				5′ 1 ″	8′ 5 ″	8′ 9 ″	9′ 11″	10′ 4″	11′ 10″	12′ 4″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
The late of the	/		125		4′ 11″	8′ 4″	8′ 8 ″	9′ 10″	10′ 3″	11′ 8″	12′ 2 ″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
Standard 4' 8' 6' 5' 6' 10' 8' 7' 9' 2' 11' 7' 12' 1' 13' 6' 14' 0' 14' 0' 14' 0' 14' 0' Standard 4' 8' 6' 5' 6' 10' 10' 11' 3' 11' 8' 13' 5' 14' 0' 14' 0' 14' 0' 14' 0' STANDARD 5' 1' 9' 0' 9' 4' 10' 8' 11' 1' 12' 9' 13' 3' 14' 0' 14' 0' 14' 0' 14' 0' Standard 5' 1' 9' 0' 9' 4' 10' 8' 11' 1' 12' 9' 13' 3' 14' 0' 14' 0' 14' 0' 14' 0' Standard 5' 1' 8' 0' 8' 6' 10' 8' 11' 1' 12' 9' 13' 3' 14' 0' 14' 0' 14' 0' 14' 0' Standard 5' 1' 8' 0' 8' 6' 10' 8' 11' 1' 12' 9' 13' 3' 14' 0' 14' 0' 14' 0' 14' 0' Standard 5' 1' 8' 0' 8' 6' 10' 8' 11' 1' 12' 9' 13' 3' 14' 0' 14' 0' 14' 0' 14' 0' Standard 5' 1' 8' 0' 8' 6' 10' 10' 11' 11' 4' 13' 0' 13' 6' 14' 0' 14' 0' 14' 0' 14' 0' Standard 5' 3' 8' 5' 9' 0' 10' 9' 11' 2' 12' 10' 13' 4' 14' 0' 14' 0' 14' 0' 14' 0' Standard 5' 1' 7' 5' 7' 11' 9' 11' 10' 7' 12' 9' 13' 3' 14' 0' 14' 0' 14' 0' 14' 0' 14' 0' Standard 5' 1' 7' 5' 7' 11' 9' 11' 10' 7' 12' 9' 13' 3' 14' 0' 14' 0' 14' 0' 14' 0' 14' 0'			l	#3											
Standard 4' 8' 6' 5' 6' 10' 8' 7' 9' 2' 11' 7' 12' 1' 13' 6' 14' 0' 14'	ا له ا	16	IDF L	Stud											
Second	_			Standard											
HF Stud 5' 1' 9' 0' 9' 4' 10' 8' 11' 1' 12' 9' 13' 3' 14' 0' 1	2		CDE												
X O Standard 5' 1' 8' 0' 8' 6' 10' 8' 11' 1' 12' 9' 13' 3' 14' 0'	Ø		727	#3											
X 3tandard 5 / 8 / 9 / 3 / 9 / 8 / 10 / 11 / 11 / 4 / 13 / 0 / 13 / 6 / 14 / 0 / 14 /	101	\cup	ᅵᆸᄃ	Stud											
SP #1 5 8' 9' 3' 9' 8' 10' 11' 11' 4' 13' 0' 13' 8' 14' 0'		Ō	1 11												
SP #2 5'5' 9'2' 9'6' 10'10' 11'3' 12'11' 13'5' 14'0' 1	$ \times $														
Stud 5' 3' 8' 5' 9' 0' 10' 9' 11' 2' 12' 10' 13' 4' 14' 0'			125												
Standard 5' 1' 7' 5' 7' 11' 9' 11' 10' 7' 12' 9' 13' 3' 14' 0' 14' 0' 14' 0' 14' 0'	۱۲		L												
Standard 5' 1' 7' 5' 7' 11' 9' 11' 10' 7' 12' 9' 13' 3' 14' 0' 14' 0' 14' 0' 14' 0'	-	1,	IDF L	Stud											
		·		Standard	5′ 1 ′	7′ 5″	7′ 11″	9′ 11 ″			13′ 3″	14′ 0″	14′ 0″	14′ 0″	14′ 0″

Bracing Group Species and Grades: Group A: Spruce-Pine-Fir Hem-Fir #1 / #2 Standard #2 Stud #3 Stud #3 Standard Douglas Fir-Larch Southern Pine*** #3 Stud Stud Standard Standard Group B Hem-Fir #1 & Btr Douglas Fir-Larch Southern Pine*** #1 #1 #2

1x4 Braces shall be SRB (Stress-Rated Board) **For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards, Group B values may be used with these grades.

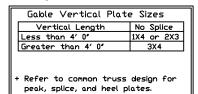
Gable Truss Detail Notes: Wind Load deflection criterion is L/240.

Provide uplift connections for 55 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2'0" overhang, or 12" plywood overhang.

Attach "L" braces with 10d (0.128"x3.0" min) nails. * For (1) "L" brace: space nails at 2" o.c. in 18" end zones and 4" o.c. between zones. ₩ ¥For (2) "L" braces: space nails at 3" o.c. in 18" end zones and 6" o.c. between zones.

"L" bracing must be a minimum of 80% of web member length.



Refer to the Building Designer for conditions not addressed by this detail.

> |DATE 01/26/2018

ASCE7-16-GAB14015

Symm C Gable Truss Diagonal brace option: vertical length may be doubled when diagonal brace is used. Connect diagonal brace for 450# at each end. Max web "L" Brace End total length is 14'. Zones, typ. 2x4 DF-L #2 or better diagonal brace; single Vertical length shown or double cut in table above. (as shown) at upper end. Continuous Bearing Connect diagonal at Refer to chart shove for max gable ventical length. midpoint of vertical web.

VARNINGI READ AND FOLLOW ALL NOTES ON THIS DRAVINGI
****IMPORTANT*** FURNISH THIS DRAVING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

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COA #0.278 02/23/2023

MAX, TOT, LD, 60 PSF

Florida Certificate of Product Approxal #Flat 1999

24.0"

CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforcement or scab reinforcement.

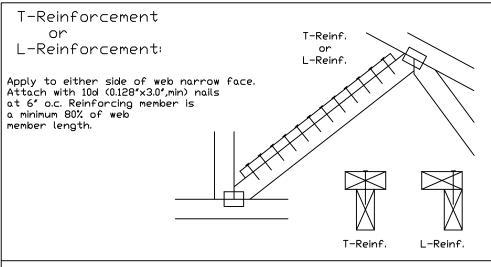
Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

Use scabs instead of L- or T- reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

Web Member	Specified CLR	Alternative Reir		
Size	Restraint	T- or L- Reinf.		
2x3 or 2x4	1 row	2×4	1-2×4	
2x3 or 2x4	2 rows	2×6	2-2×4	
2×6	1 row	2×4	1-2×6	
2×6	2 rows	2×6	2-2×4(米)	
2×8	1 row	2×6	1-2×8	
2×8	2 rows	2×6	2-2×6(*)	

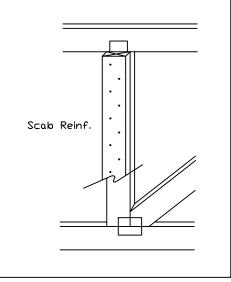
T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

(**) Center scab on wide face of web. Apply (1) scab to each face of web.



Scab Reinforcement:

Apply scab(s) to wide face of web. No more than (1) scab per face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



VARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING
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	TC DL	PSF	DATE	01/02/19
	BC DL	PSF	DRWG	BRCLBSUB0119
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	TOT. LD.	PSF		
	DUR. FAC.			
t 1	SPACING 1999			



Gable Detail For Let-in Verticals Gable Truss Plate Sizes Refer to appropriate Alpine gable detail for minimum plate sizes for vertical studs. (+) Refer to Engineered truss design for peak, splice, web, and heel plates. *If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web. Gable Vertical Length \ typ. Example:

Provide connections for uplift specified on the engineered truss design.

Attach each "T" reinforcing member with

End Driven Nails:

10d Common (0.148"x 3.", min) Nails at 4" o.c. plus

(4) nails in the top and bottom chords.

10d Common (0.148"x3".min) Toenails at 4" o.c. plus

(4) toenails in the top and bottom chords.

This detail to be used with the appropriate Alpine gable detail for ASCE wind load.

ASCE 7-05 Gable Detail Drawings

A13015051014, A12015051014, A11015051014, A10015051014, A14015051014, A13030051014, A12030051014, A11030051014, A10030051014, A14030051014

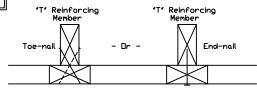
ASCE 7-10 & ASCE 7-16 Gable Detail Drawings

A11515ENC100118, A12015ENC100118, A14015ENC100118, A16015ENC100118, A18015ENC100118, A20015ENC100118, A20015END100118, A20015PED100118, A11530ENC100118, A12030ENC100118, A14030ENC100118, A16030ENC100118, A18030ENC100118, A20030ENC100118, A20030END100118, A20030PED100118, \$11515ENC100118, \$12015ENC100118, \$14015ENC100118, \$16015ENC100118, \$18015ENC100118, \$20015ENC100118, \$20015END100118, \$20015FED100118,

\$11530ENC100118, \$12030ENC100118, \$14030ENC10018, \$4030ENC100118 \$18030ENC100118, \$20030ENC100118, \$20030EN3100118, \$20030PED100118

See appropriate Alpine gable detail for maximum any eleforces gaine ver

"T" Reinforcement Attachment Detail



To convert from "L" to "T" reinforcing members, multiply "T" increase by length (based on appropriate Alpine gable detail).

Maximum allowable "T" reinforced gable vertical length is 14' from top to bottom chord.

"T" reinforcing member material must match size, specie, and grade of the "L" reinforcing member.

Web Length Increase w/ "T" Brace|

"T" Reinf.	"T"			
Mbr. Size	Increase			
2×4	30 %			
2x6	20 %			

Example:

ASCE 7-10 Wind Speed = 120 mph Mean Roof Height = 30 ft, Kzt = 1.00 Gable Vertical = 24°o.c. SP #3 "T" Reinforcing Member Size = 2x4

"T" Brace Increase (From Above) = 30% = 1.30 (1) 2x4 "L" Brace Length = 8' 7"

Maximum "T" Reinforced Gable Vertical Length $1.30 \times 8' \ 7'' = 11' \ 2''$

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COA #0 278 02/23/2023

IREF LET-IN VERT DATE 01/02/2018 DRWG GBLLETIN0118

MAX. TOT. LD. 60 PSF DUR. FAC. ANY

24.0"

Rigid Sheathing

Ceiling

4 Nails

Nails

Spaced At

4 Nails

Reinforcing

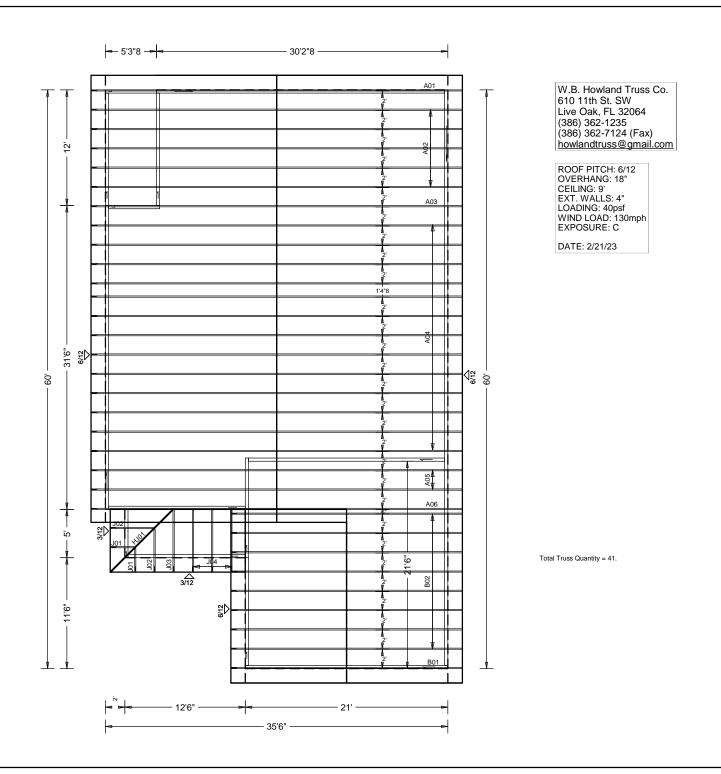
Member

Gable

Truss

155 Harlem Ave North Building, 4th Floor Glenview, IL 60025

Florida Certificate of Product A MAXal SPAIODNG





JOB #: 23-8964

Job Name: Aaron Marc Company (Nicke Customer: Contractor Designer: Fill in later ADDRESS: CHURCH ST

SALESMAN: Fill in later : <Not Found>

JOB NO:

23-8964

PAGE NO: 1 OF 1