

DATE 12/28/2005

Columbia County Building Permit

PERMIT

This Permit Expires One Year From the Date of Issue

000023991

APPLICANT GERMAN SANCHEZ PHONE 352-665-7529
ADDRESS P BOX 140565 GAINESVILLE FL 32614
OWNER EDIX INVESTMENT PHONE 352-371-6100
ADDRESS 10985 S. US HWY 441 LAKE CITY FL 32025
CONTRACTOR SAN JUAN CONSTRUCTION PHONE 352-665-7529
LOCATION OF PROPERTY 441 S, AT MASON CITY ON THE LEFT

TYPE DEVELOPMENT SFD,UTILITY ESTIMATED COST OF CONSTRUCTION 74900.00
HEATED FLOOR AREA 1498.00 TOTAL AREA 2294.00 HEIGHT 25.00 STORIES 1
FOUNDATION CONCRETE WALLS FRAMED ROOF PITCH 6/12 FLOOR SLAB
LAND USE & ZONING A-3 MAX. HEIGHT 35
Minimum Set Back Requirments: STREET-FRONT 30.00 REAR 25.00 SIDE 25.00
NO. EX.D.U. 0 FLOOD ZONE XPS DEVELOPMENT PERMIT NO.

PARCEL ID 22-5S-17-09328-008 SUBDIVISION MASON CITY S/D PARCEL 2
LOT 2 BLOCK PHASE UNIT TOTAL ACRES

Culvert Permit No. Culvert Waiver Contractor's License Number CBC025206
DOT APPROVED 05-1182-N BK JH Y
Driveway Connection Septic Tank Number LU & Zoning checked by Approved for Issuance New Resident

COMMENTS: FLOOR 1 FOOT ABOVE THE ROAD
DOT APPROVAL RECIEVED

Check # or Cash 13425

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power date/app. by Foundation date/app. by Monolithic date/app. by
Under slab rough-in plumbing date/app. by Slab date/app. by Sheathing/Nailing date/app. by
Framing date/app. by Rough-in plumbing above slab and below wood floor date/app. by
Electrical rough-in date/app. by Heat & Air Duct date/app. by Peri. beam (Lintel) date/app. by
Permanent power date/app. by C.O. Final date/app. by Culvert date/app. by
M/H tie downs, blocking, electricity and plumbing date/app. by Pool date/app. by
Reconnection date/app. by Pump pole date/app. by Utility Pole date/app. by
M/H Pole date/app. by Travel Trailer date/app. by Re-roof date/app. by

BUILDING PERMIT FEE \$ 375.00 CERTIFICATION FEE \$ 11.47 SURCHARGE FEE \$ 11.47
MISC. FEES \$ 0.00 ZONING CERT. FEE \$ 50.00 FIRE FEE \$ 0.00 WASTE FEE \$
FLOOD DEVELOPMENT FEE \$ FLOOD ZONE FEE \$ 25.00 CULVERT FEE \$ TOTAL FEE 472.94
INSPECTORS OFFICE CLERKS OFFICE

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE. PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

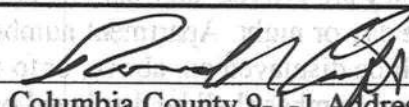
COLUMBIA COUNTY 9-1-1 ADDRESSING

263 NW Lake City Ave. * P. O. Box 1787 * Lake City, FL 32056-1787

PHONE: (386) 758-1125 * FAX: (386) 758-1365 * Email: ron_croft@columbiacountyfla.com

Addressing Maintenance

To maintain the Countywide Addressing Policy you must make application for a 9-1-1 Address at the time you apply for a building permit. The established standards for assigning and posting numbers to all principal buildings, dwellings, businesses and industries are contained in Columbia County Ordinance 2001-9. The addressing system is to enable Emergency Service Agencies to locate you in an emergency, and to assist the United States Postal Service and the public in the timely and efficient provision of services to residents and businesses of Columbia County.

DATE ISSUED: October 24, 2005**ENHANCED 9-1-1 ADDRESS:****10985 S US HIGHWAY 441 (LAKE CITY, FL 32025)****Addressed Location 911 Phone Number: NOT AVAIL.****OCCUPANT NAME: NOT AVAIL.****OCCUPANT CURRENT MAILING ADDRESS:****PROPERTY APPRAISER PARCEL NUMBER: 22-5S-17-09328-008****Other Contact Phone Number (If any):****Building Permit Number (If known):****Remarks: IDENTIFIED BY REQUESTER AS LOT 2****Address Issued By:**
Columbia County 9-1-1 Addressing / GIS Department

NOTICE: THIS ADDRESS WAS ISSUED BASED ON LOCATION INFORMATION RECEIVED FROM THE REQUESTER. SHOULD, AT A LATER DATE, THE LOCATION INFORMATION BE FOUND TO BE IN ERROR, THIS ADDRESS IS SUBJECT TO CHANGE.

**COLUMBIA COUNTY
9-1-1 ADDRESSING
APPROVED**

COLUMBIA COUNTY 9-1-1 ADDRESSING

263 NW Lake City Ave. * P. O. Box 1787 * Lake City, FL 32056-1787

PHONE: (386) 758-1125 * FAX: (386) 758-1365 * Email: ron_croft@columbiacountyfla.com

Posting of Address Numbers in accordance with Ordinance 2001-9, Section 5:

- A. Principal Buildings (residence, apartment building or "In Town" business) shall display the assigned address number made of Arabic numerals not less than 3 inches in height and 1 ½ inches in width of a contrasting color to the background on which affixed, as near to the front entrance as possible and practical so that the number is visible and legible from the sidewalk (if any), the public or private way on which the principal building fronts and the opposite side of the public or private way, day or night.
- B. Private Lane and Long Driveways: for any principal building (residence, apartment building or business) (except malls or shopping centers) located so that the address number is not clearly legible and visible from the public or private way, shall post an additional set of numbers at the intersection of the driveway to the principal building at the public or private way. The additional address number shall be made up of Arabic numerals not less than 3 inches in height and 1-1/2 inches in width. Numbers shall be contrasting in color with the background on which they are affixed, visible day or night, and placed upon a post or other structure which displays the number so it is visible and legible to emergency services personnel approaching from either direction along the public or private way.
- C. Industrial and Commercial Structures in Low Density Areas: All industrial and commercial structures located in low-density development areas (areas in which small residential style address numbers are not visible from the public or private way) shall display address numbers of not less than 10 inches in height. The numbers shall contrast in color with the background on which they are affixed and shall be visible and legible day or night from the public or private way. When possible, the number shall be displayed beside or over the main entrances of the structure.
- D. Apartment Buildings and High-Rises: All apartment buildings and high-rises style principal buildings shall display address numbers above or to the side of the primary entrance to the Addressed location. Numbers shall contrast with the color of the background to which they are affixed, and shall be at least 6 inches in height and visible and legible day or night. Apartment numbers for individual units within the complex shall be displayed on, above, or to the side of the doorway of each unit. Assigned number shall be displayed on each separate front entrance in the case of a principal building which is occupied by more than one business or family dwelling unit.
- E. Any different numbers, which might be mistaken for or confused with the numbers assigned in accordance with the "Numbering System", shall be removed upon proper display of the assigned address number.
- F. The responsibility of placement and maintenance of the building address numbers is that of the occupant or property owner.

Columbia County Building Permit Application

Stewart

Revised 9-23-0

For Office Use Only Application # 050995 Date Received 9/30/05 By JW Permit # 23991
Application Approved by - Zoning Official BLK Date 22.12.05 Plans Examiner OK JTH Date 12-29-05
Flood Zone X P-SUB-1 Development Permit N/A Zoning A-3 Land Use Plan Map Category A-3
Comments Signed Ed. McArthur - NO Section 2.3.1 Legal Non-conforming lot
NEED NOT APPROVE Copy of Records DEF. 9.4 + 352.380.0542 Record

Applicants Name German Sanchez Phone 352-665-7529
Address P.O. Box 140565 Gainesville, FL 32614
Owners Name Edix Investment. Phone 352-371-6100
911 Address 10985 S.W. Hwy 441 (Lake City) FL 32025
Contractors Name San Juan Const. Co. Inc. Phone 352-665-7529
Address P.O. Box 140565 Gainesville, FL 32614
Fee Simple Owner Name & Address _____
Bonding Co. Name & Address None
Architect/Engineer Name & Address Ricardo Cavalino & Asso. 225 E. 5th Ave Gville
Mortgage Lenders Name & Address Mercantile Bank Gville, FL

Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progressive Energy

Property ID Number 22-55-17-09328-012 Estimated Cost of Construction 89,880.00

Subdivision Name MASON CITY S/O (Parcel # 2) Lot 2 Block 1 Unit 1 Phase 1

Driving Directions From I-75 Exit 414 take 441/41 north to Mason City
Lot is 2.7 miles from the exit to the front of the lot
Lot is on Right side of 441/41 when heading North

Type of Construction Frame Const. Number of Existing Dwellings on Property 0

Total Acreage _____ Lot Size _____ Do you need a Culvert Permit or Culvert Waiver or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 60' Side 58'-2" Side 61'-1" Rear 18'16"

Total Building Height 25' Number of Stories 1 Heated Floor Area 1,498 SF Roof Pitch 6/12
Porches 268 GARAGE 529 TOTAL 2,294

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Owner Builder or Agent (Including Contractor)
San Juan Const. Co. Inc.

STATE OF FLORIDA
COUNTY OF COLUMBIA

Sworn to (or affirmed) and subscribed before me
this 29 day of Sept. 2005
Personally known ✓ or Produced Identification _____

Contractor Signature
Contractors License Number CBC-025206
Competency Card Number _____
NOTARY STAMP/SEAL

Notary Signature

Gloria E. Sanchez
GLORIA E. SANCHEZ
MY COMMISSION # DD264629
EXPIRES: November 03, 2007

f

**COLUMBIA COUNTY BUILDING DEPARTMENT
CHECKLIST FOR PERMITTING**

Application # _____

✓	Notarized completed Building Permit Application	
	Notes:	
N/A	If an Owner Builder, signed Disclosure Statement	
	Notes:	
✓	Recorded Deed or a Notarized Affidavit (form from the Building Dept.)	
	Notes:	
OK	Approved and Signed Site Plan from Environmental Health on the septic	
	Notes:	
✓	Site plan with actual distances of the structure to each property line	
	Notes:	
	911 Address form, Contact 386.752.8787 for an appointment	Monday 2:30
	Notes:	
✓	Residential or Commercial Checklist completed	
	Notes:	
✓	Driving directions including all road names	
	Notes:	
✓	Well information (on plans or letter from the well driller)	
	Notes:	
	Before the 1st inspection Recorded Notice of Commencement signed by owner	
	Notes:	
✓	2 sets of plans (blueprints)	
	Notes:	
✓	2 sets of sealed truss engineering	
	Notes:	
✓	2 sets of energy code & manual J	
	Notes:	
✓	2 sets of engineering packets including specs on windows, doors, roof and etc.	
	Notes:	

FLORIDA ENERGY EFFICIENCY CODE
FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs
Residential Whole Building Performance Method A

Project Name:	EDI-Model Niya "1498"	Builder:	Ed Dix
Address:	Lot: , Sub: , Plat:	Permitting Office:	Columbia Co.
City, State:	,	Permit Number:	23981
Owner:		Jurisdiction Number:	221000
Climate Zone:	North		

1. New construction or existing	New	12. Cooling systems	
2. Single family or multi-family	Single family	a. Central Unit	Cap: 30.0 kBtu/hr
3. Number of units, if multi-family	1		SEER: 12.00
4. Number of Bedrooms	3	b. N/A	
5. Is this a worst case?	Yes	c. N/A	
6. Conditioned floor area (ft²)	1498 ft²	13. Heating systems	
7. Glass area & type	Single Pane Double Pane	a. Electric Heat Pump	Cap: 27.0 kBtu/hr
a. Clear glass, default U-factor	0.0 ft² 160.0 ft²		HSPF: 7.00
b. Default tint	0.0 ft² 0.0 ft²	b. N/A	
c. Labeled U or SHGC	0.0 ft² 0.0 ft²	c. N/A	
8. Floor types		14. Hot water systems	
a. Slab-On-Grade Edge Insulation	R=0.0, 172.0(p) ft	a. Electric Resistance	Cap: 50.0 gallons
b. N/A			EF: 0.88
c. N/A		b. N/A	
9. Wall types		c. Conservation credits	
a. Frame, Wood, Exterior	R=13.0, 980.0 ft²	(HR-Heat recovery, Solar	
b. Frame, Wood, Adjacent	R=13.0, 180.0 ft²	DHP-Dedicated heat pump)	
c. N/A		15. HVAC credits	
d. N/A		(CF-Ceiling fan, CV-Cross ventilation,	
e. N/A		HF-Whole house fan,	
10. Ceiling types		PT-Programmable Thermostat,	
a. Under Attic	R=30.0, 1498.0 ft²	MZ-C-Multizone cooling,	
b. Under Attic	R=19.0, 100.0 ft²	MZ-H-Multizone heating)	
c. N/A			
11. Ducts			
a. Sup: Unc. Ret: Unc. AH: Garage	Sup. R=6.0, 81.0 ft		
b. N/A			

Glass/Floor Area: 0.11

Total as-built points: 22646
Total base points: 23912

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: SUNCOAST INSULATORS

DATE: 3-10-05

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT:

DATE:

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL:

DATE:



SUMMER CALCULATIONS
Residential Whole Building Performance Method A - Details

ADDRESS: Lot: , Sub: , Plat: , , ,

PERMIT #:

BASE				AS-BUILT							
GLASS TYPES .18 X Conditioned X BSPM = Points Floor Area				Type/SC	Overhang Ornt Len Hgt Area X SPM X SOF = Points						
.18	1498.0	20.04	5403.6	Double, Clear	W	2.0	6.0	57.0	38.52	0.85	1865.2
				Double, Clear	E	2.0	6.0	43.0	42.06	0.85	1533.9
				Double, Clear	N	2.0	6.0	45.0	19.20	0.90	777.7
				Double, Clear	S	2.0	6.0	15.0	35.87	0.78	417.5
				As-Built Total:							160.0
WALL TYPES Area X BSPM = Points				Type	R-Value Area X SPM = Points						
Adjacent	180.0	0.70	126.0	Frame, Wood, Exterior			13.0	980.0	1.50	1470.0	
Exterior	980.0	1.70	1666.0	Frame, Wood, Adjacent			13.0	180.0	0.60	108.0	
Base Total:				1160.0		1792.0		As-Built Total:		1160.0	1578.0
DOOR TYPES Area X BSPM = Points				Type	Area X SPM = Points						
Adjacent	18.0	2.40	43.2	Exterior Insulated				36.0	4.10	147.6	
Exterior	36.0	6.10	219.6	Adjacent Insulated				18.0	1.60	28.8	
Base Total:				54.0		262.8		As-Built Total:		54.0	176.4
CEILING TYPES Area X BSPM = Points				Type	R-Value Area X SPM X SCM = Points						
Under Attic	1498.0	1.73	2591.5	Under Attic			30.0	1498.0	1.73 X 1.00	2591.5	
				Under Attic			19.0	100.0	2.34 X 1.00	234.0	
Base Total:				1498.0		2591.5		As-Built Total:		1598.0	2825.5
FLOOR TYPES Area X BSPM = Points				Type	R-Value Area X SPM = Points						
Slab	172.0(p)	-37.0	-6364.0	Slab-On-Grade Edge Insulation			0.0	172.0(p)	-41.20	-7086.4	
Raised	0.0	0.00	0.0								
Base Total:				-6364.0		As-Built Total:		172.0	-7086.4		
INFILTRATION Area X BSPM = Points				Area X SPM = Points							
1498.0 10.21 15294.6				1498.0 10.21 15294.6							

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: , Sub: , Plat: , , ,

PERMIT #:

BASE				AS-BUILT							
Summer Base Points: 18980.5				Summer As-Built Points: 17382.3							
Total Summer Points	X	System Multiplier	= Cooling Points	Total Component	X	Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Cooling Points	
18980.5		0.4266	8097.1	17382.3		1.000	(1.090 x 1.147 x 1.00)	0.284	1.000	6180.9	
				17382.3		1.00	1.250	0.284	1.000	6180.9	

WINTER CALCULATIONS
Residential Whole Building Performance Method A - Details

ADDRESS: Lot: , Sub: , Plat: , , ,

PERMIT #:

BASE				AS-BUILT							
GLASS TYPES											
.18 X Conditioned X BWPM = Points Floor Area				Type/SC	Overhang Ornt Len Hgt			Area X WPM X WOF = Points			
.18	1498.0	12.74	3435.2	Double, Clear	W	2.0	6.0	57.0	20.73	1.04	1232.0
				Double, Clear	E	2.0	6.0	43.0	18.79	1.06	857.0
				Double, Clear	N	2.0	6.0	45.0	24.58	1.00	1111.2
				Double, Clear	S	2.0	6.0	15.0	13.30	1.26	251.0
				As-Built Total:						160.0	3451.2
WALL TYPES Area X BWPM = Points				Type	R-Value			Area X WPM = Points			
Adjacent	180.0	3.60	648.0	Frame, Wood, Exterior	13.0			980.0	3.40	3332.0	
Exterior	980.0	3.70	3626.0	Frame, Wood, Adjacent	13.0			180.0	3.30	594.0	
Base Total:		1160.0	4274.0	As-Built Total:			1160.0			3926.0	
DOOR TYPES Area X BWPM = Points				Type				Area X WPM = Points			
Adjacent	18.0	11.50	207.0	Exterior Insulated				36.0	8.40	302.4	
Exterior	36.0	12.30	442.8	Adjacent Insulated				18.0	8.00	144.0	
Base Total:		54.0	649.8	As-Built Total:			54.0			446.4	
CEILING TYPESArea X BWPM = Points				Type	R-Value			Area X WPM X WCM = Points			
Under Attic	1498.0	2.05	3070.9	Under Attic	30.0			1498.0	2.05 X 1.00	3070.9	
				Under Attic	19.0			100.0	2.70 X 1.00	270.0	
Base Total:		1498.0	3070.9	As-Built Total:			1598.0			3340.9	
FLOOR TYPES Area X BWPM = Points				Type	R-Value			Area X WPM = Points			
Slab	172.0(p)	8.9	1530.8	Slab-On-Grade Edge Insulation	0.0			172.0(p)	18.80	3233.6	
Raised	0.0	0.00	0.0								
Base Total:			1530.8	As-Built Total:			172.0			3233.6	
INFILTRATION Area X BWPM = Points											
		1498.0	-0.59				1498.0 -0.59 -883.8				

WINTER CALCULATIONS
Residential Whole Building Performance Method A - Details

ADDRESS: Lot: , Sub: , Plat: , , ,

PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned X BWPM = Points Floor Area				Type/SC	Overhang Ornt Len Hgt		Area X WPM X WOF = Points					
.18	1498.0	12.74	3435.2	Double, Clear	W	2.0	6.0	57.0	20.73	1.04	1232.0	
				Double, Clear	E	2.0	6.0	43.0	18.79	1.06	857.0	
				Double, Clear	N	2.0	6.0	45.0	24.58	1.00	1111.2	
				Double, Clear	S	2.0	6.0	15.0	13.30	1.26	251.0	
				As-Built Total:				160.0				3451.2
WALL TYPES Area X BWPM = Points				Type	R-Value		Area X WPM = Points					
Adjacent	180.0	3.60	648.0	Frame, Wood, Exterior	13.0		980.0	3.40	3332.0			
Exterior	980.0	3.70	3626.0	Frame, Wood, Adjacent	13.0		180.0	3.30	594.0			
Base Total:		1160.0	4274.0	As-Built Total:				1160.0	3926.0			
DOOR TYPES Area X BWPM = Points				Type			Area X WPM = Points					
Adjacent	18.0	11.50	207.0	Exterior Insulated			36.0	8.40	302.4			
Exterior	36.0	12.30	442.8	Adjacent Insulated			18.0	8.00	144.0			
Base Total:		54.0	649.8	As-Built Total:				54.0	446.4			
CEILING TYPESArea X BWPM = Points				Type	R-Value		Area X WPM X WCM = Points					
Under Attic	1498.0	2.05	3070.9	Under Attic	30.0		1498.0	2.05 X 1.00	3070.9			
				Under Attic	19.0		100.0	2.70 X 1.00	270.0			
Base Total:		1498.0	3070.9	As-Built Total:				1598.0	3340.9			
FLOOR TYPES Area X BWPM = Points				Type	R-Value		Area X WPM = Points					
Slab	172.0(p)	8.9	1530.8	Slab-On-Grade Edge Insulation	0.0		172.0(p)	18.80	3233.6			
Raised	0.0	0.00	0.0									
Base Total:			1530.8	As-Built Total:				172.0	3233.6			
INFILTRATION Area X BWPM = Points								Area X WPM = Points				
		1498.0	-0.59					1498.0	-0.59	-883.8		

WINTER CALCULATIONS
Residential Whole Building Performance Method A - Details

ADDRESS: Lot: , Sub: , Plat: , , ,	PERMIT #:
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BASE				AS-BUILT							
Winter Base Points: 12076.9				Winter As-Built Points: 13514.3							
Total Winter Points	X	System Multiplier	= Heating Points	Total Component	X	Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points	
12076.9		0.6274	7577.0	13514.3		1.00	(1.069 x 1.169 x 1.00)	0.487	1.000	8227.0	
				13514.3		1.00	1.250	0.487	1.000	8227.0	

Code Compliance Checklist
Residential Whole Building Performance Method A - Details

ADDRESS: Lot: , Sub: , Plat: , , ,

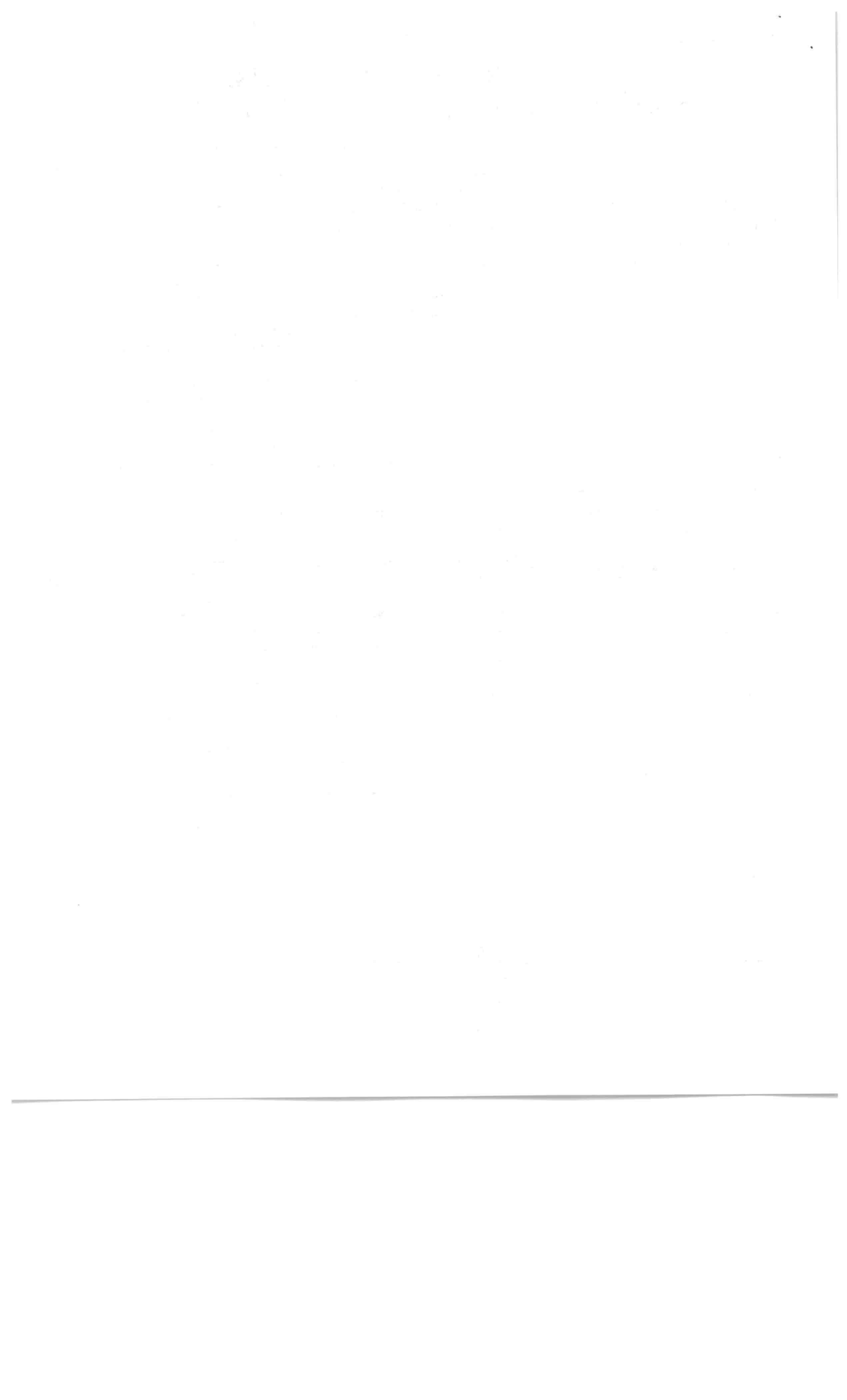
PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum: .3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 6-12. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	



ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 84.0

The higher the score, the more efficient the home.

, Lot: , Sub: , Plat: , , ,

1. New construction or existing	New	12. Cooling systems	
2. Single family or multi-family	Single family	a. Central Unit	Cap: 30.0 kBtu/hr
3. Number of units, if multi-family	1		SEER: 12.00
4. Number of Bedrooms	3	b. N/A	
5. Is this a worst case?	Yes	c. N/A	
6. Conditioned floor area (ft ²)	1498 ft ²		
7. Glass area & type	Single Pane Double Pane	13. Heating systems	
a. Clear - single pane	0.0 ft ² 160.0 ft ²	a. Electric Heat Pump	Cap: 27.0 kBtu/hr
b. Clear - double pane	0.0 ft ² 0.0 ft ²		HSPF: 7.00
c. Tint/other SHGC - single pane	0.0 ft ² 0.0 ft ²	b. N/A	
d. Tint/other SHGC - double pane		c. N/A	
8. Floor types		14. Hot water systems	
a. Slab-On-Grade Edge Insulation	R=0.0, 172.0(p) ft	a. Electric Resistance	Cap: 50.0 gallons
b. N/A			EF: 0.88
c. N/A		b. N/A	
9. Wall types		c. Conservation credits	
a. Frame, Wood, Exterior	R=13.0, 980.0 ft ²	(HR-Heat recovery, Solar	
b. Frame, Wood, Adjacent	R=13.0, 180.0 ft ²	DHP-Dedicated heat pump)	
c. N/A		15. HVAC credits	
d. N/A		(CF-Ceiling fan, CV-Cross ventilation,	
e. N/A		HF-Whole house fan,	
10. Ceiling types		PT-Programmable Thermostat,	
a. Under Attic	R=30.0, 1498.0 ft ²	MZ-C-Multizone cooling,	
b. Under Attic	R=19.0, 100.0 ft ²	MZ-H-Multizone heating)	
c. N/A			
11. Ducts			
a. Sup: Unc. Ret: Unc. AH: Garage	Sup. R=6.0, 81.0 ft		
b. N/A			

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: _____

Date: _____

Address of New Home: _____

City/FL Zip: _____



**NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStarTM designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.*

EnergyGauge® (Version: FLRCPB v3.30)

*Does this need
to go
in both
permits*

**FAX
MEMORANDUM**

*17 Total Sheets
FAXED*

MEMORANDUM

FLORIDA DEPARTMENT OF TRANSPORTATION

To: Mr. John Kerce, Dept. Director
Columbia Co. Building & Zoning Dept.
Fax No: 904-758-2160

From: Neil E. Miles, FDOT Permits Coord.
Date: 12-16-05 **Fax No. 904-961-7180**
Attention: In-House Staff

() Sign and return. () For your files. () Please call me. **(XX)** FYI () For Review

REF: Notice of Driveway Access Review / Inspected On: 12-16-05

PROJECT: Inspection of Existing Access Connection for current FDOT Compliance

PROPT. OWNER: EDIX Investments, Inc

PRIOR PROPERTY OWNER: Vernal L. Huffines

PROPOSED: Inspection of existing FDOT Access

LANDOWNER'S MAILING ADDRESS: 720 NW 23rd Ave. Gainesville, FL 32609

COUNTY PARCEL ID No: 22-55-17-09328-013 / **CONTRACTOR:** San Juan Construction Co.

FDOT Permit No: None Required

Contractor's Phone No. 352-665-7529 / **Engineer:** N/A

Lots/Parcel's That Relate to this inspection: 9 through 13 and 20 through 24 only

Subdivision Name: Mason City, Block 3

Mr. Kerce:

Per our inspection this property is currently gaining access by way of a legally permitted connection point that meets current FDOT access standards. The new driveway connection was inspected today and has passed FDOT inspection.

Please accept this as our notice of same and lift any Columbia County Permit restrictions that may have applied to this site due to State FDOT Access compliances.

If further information is required on this project please do not hesitate to contact this office for additional access permitting information details. My office number is 961-7193 or 961-7180.

Sincerely,

Neil E. Miles
Neil Miles

Access Permits Coordinator

It's great to have folks like you to work with, thanks again for your assistance!

Merry Christmas

DEC-16-2005 FRI 01:08 PM Bosshardt Realty 23rd av FAX NO. 3522409101

P. 02

COLUMBIA CO CLERK CT

PAGE 01

11/21/2005 08:06 3867581337

WARRANTY DEED
HONOR. TO INDIVID.

Return to: (enclosed in stamped envelope)

Name: Chicago Title-Gainesville

Attn: Lynn Francis

Address: 2321C NW 41st ST, Gainesville, 32608

File Number: 05-433-12

This instrument prepared by:

Name: Chicago Title-Gainesville

Attn: Lynn Francis

Address: 2321C NW 41st ST, Gainesville, 32608

Property Address/Parcel Identification

Parcel Number(s): R05328-008 AND R05333-000

Grantor(s) S.S. # (s)

Inst: 2005023221 Date: 09/29/2005 Time: 08:24

Doc Stamp-Deed: 405.00

PC, P. Dewitt Cason, Columbia County R:1059 P:2175

SPACE ABOVE THIS LINE FOR PROCESSING DATA

SPACE ABOVE THIS LINE FOR RECORDING DATA

This Warranty Deed, Made and executed this 12 SEPTEMBER 2005

by VERNAL L. HUFFINES, AKA VERNALL HUFFINES, whose address is 624 INDIANA STREET, WITCHITA FALLS, TEXAS 76301 hereinafter called the Grantor,

to EDIX INVESTMENTS, INC., whose post office address is hereinafter called the Grantee.

(Wherever used herein the terms "Grantor" and "Grantee" include all the parties to this instrument and the heirs, legal representatives, and assigns of individuals, and the successors and assigns of corporations, wherever the context so admits or requires)

Witnesseth, That the Grantor, for and in consideration of the sum of \$10.00 (Ten and 00/100 Dollars) and other valuable considerations, receipt whereof is hereby acknowledged, hereby grants, bargains, sells, aliens, remises, releases, conveys and confirms unto the Grantee, all that certain land, situate in Columbia County, State of Florida, viz:

LOTS 9, 10, 11, 12, 13, 20, 21, 22, 23 & 24, BLOCK 3 OF MASON CITY AS PER PLAT THEREOF AS RECORDED IN PLAT BOOK 1, PAGE 31 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA, TOGETHER WITH: BEGIN AT THE SW CORNER OF LOT 2 IN BLOCK 3 OF MASON CITY AS PER PLAT THEREOF RECORDED IN PLAT BOOK 1, PAGE 31 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA AND RUN THENCE N 88°30'42" E., ALONG THE SOUTH LINE OF SAID LOT 2 AND LOT 31 IN SAID BLOCK 3, 294.80 FEET TO THE SE CORNER OF SAID LOT 31; THENCE N 03°55'06" W., ALONG THE EAST LINE OF SAID BLOCK 3, 85.38 FEET, THENCE S 86°11'41" W., 294.54 FEET TO A POINT ON THE EASTERLY RIGHT-OF-WAY LINE OF U.S. HIGHWAY # 41; THENCE S 03°55'06" E., ALONG SAID EASTERLY RIGHT-OF-WAY LINE, 73.65 FEET TO THE POINT OF BEGINNING, BEING ALL OF LOTS 2 AND 31 AND A PART OF LOT 1 AND 32, BLOCK 3 OF MASON CITY AS PER PLAT THEREOF AS RECORDED IN PLAT BOOK 1, PAGE 31 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.

ALSO: LOTS 3, 4, 29 & 30 IN BLOCK 3 OF MASON CITY, AS PER PLAT THEREOF RECORDED IN PLAT BOOK 1, PAGE 31 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.

THIS PROPERTY IS NOT THE HOMESTEAD OF THE GRANTOR Together, with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

To Have and to Hold, the same in fee simple forever.

1 of 3

Oked
NEIL
Parcel #2Not
Inspected
Parcel #1

DEC-16-2005 FRI 01:06 PM Bosshardt Realty 23rd av FAX NO. 3522409101

P. 03

11/21/2005 08:06 3867581337

COLUMBIA CO CLERK CT

PAGE 02

WARRANTY DEED
FROM IND. TO INDIVID.

Inst: 2005023821 Date: 09/28/2005 Time: 08:24

Doc Stamp-Deed : 408.00

DC, P. Dewitt Cason, Columbia County B: 1059 P: 2180

SPACE ABOVE THIS LINE FOR RECORDING DATA

And the Grantor hereby covenants with said Grantee that the Grantor is lawfully seized of said land in fee simple; that the Grantor has good right and lawful authority to sell and convey said land; and hereby warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever; and that said land is free of all encumbrances, except taxes accruing subsequent to December 31, 2004.

In Witness Whereof, the said Grantor has signed and sealed these presents the day and year first above written.

Randy Holley
Witness

Randy Holley
Printed name

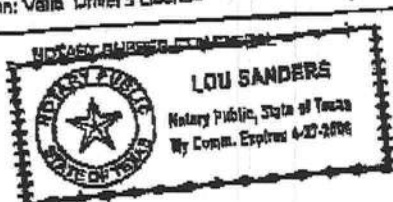
Lou Sanders
Witness

Lou Sanders
Printed name

Vernal L. Huffines
VERNAL L. HUFFINES, AKA VERNAL HUFFINES

STATE OF Texas
COUNTY OF Chilton

This foregoing instrument was acknowledged before me this 28th DAY OF SEPTEMBER 2005 by VERNAL L. HUFFINES, AKA VERNAL HUFFINES. (Check one) ☐ Said person(s) to me personally known to me. ☒ Said person(s) provided the following type of Identification: Valid Driver's License



Lou Sanders
Notary Signature
Lou Sanders
Print Name

**ALTA COMMITMENT
CHICAGO TITLE INSURANCE COMPANY
SCHEDULE A**

Office File Number:
05-433-12

Effective Date: August 20, 2005

1. Policy or Policies to be issued:

ALTA OWNER'S POLICY, Form B (amended 10/17/92)
with Florida Modifications
Proposed Insured:

\$ 58,000.00

EDIX INVESTMENTS, INC.

ALTA LOAN POLICY (amended 10/17/92)
with Florida Modifications
Proposed Insured:

\$

371-2020

2. The estate or interest in the land described or referred to in this Commitment and covered herein is a fee simple, and title thereto is at the effective date hereof vested in:

VERNAL L. HUFFINES, A/K/A VERNALL HUFFINES

3. The Land is described as follows:

LOTS 9, 10, 11, 12, 13, 20, 21, 22, 23 & 24, BLOCK 3 OF MASON CITY AS PER PLAT THEREOF AS RECORDED IN PLAT BOOK 1, PAGE 31 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.

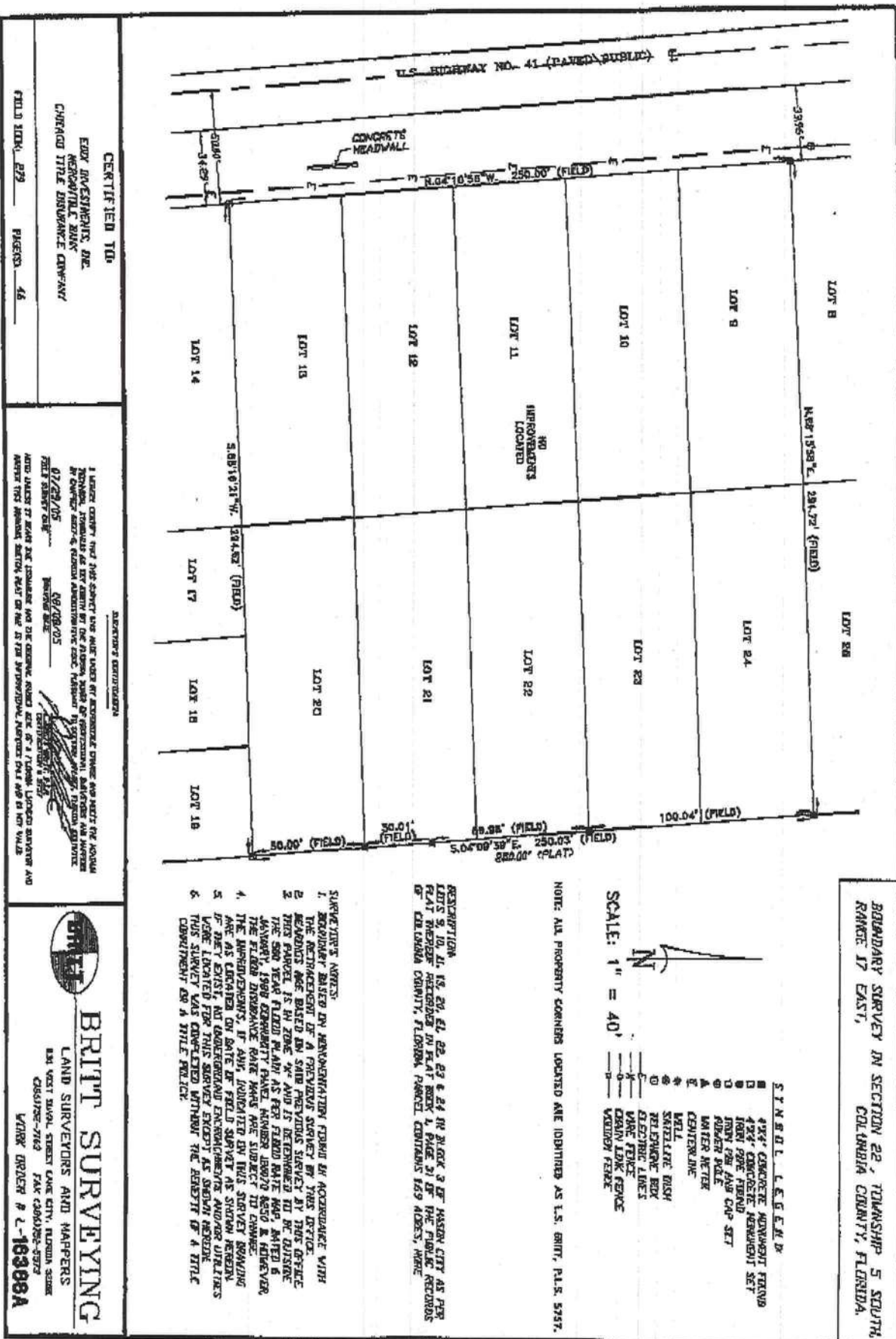
TOGETHER WITH: BEGIN AT THE SW CORNER OF LOT 2 IN BLOCK 3 OF MASON CITY AS PER PLAT THEREOF RECORDED IN PLAT BOOK 1, PAGE 31 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA AND RUN THENCE N 88°30'42" E., ALONG THE SOUTH LINE OF SAID LOT 2 AND LOT 31 IN SAID BLOCK 3, 294.80 FEET TO THE SE CORNER OF SAID LOT 31; THENCE N 03°55'06" W., ALONG THE EAST LINE OF SAID BLOCK 3, 85.36 FEET, THENCE S 86°11'41" W., 294.54 FEET TO A POINT ON THE EASTERLY RIGHT-OF-WAY LINE OF U.S. HIGHWAY #41; THENCE S 03°55'06" E., ALONG SAID EASTERLY RIGHT-OF-WAY LINE, 73.85 FEET TO THE POINT OF BEGINNING. BEING ALL OF LOTS 2 AND 31 AND A PART OF LOT 1 AND 32, BLOCK 3 OF MASON CITY AS PER PLAT THEREOF AS RECORDED IN PLAT BOOK 1, PAGE 31 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.

NOTE: This Commitment consists of insert pages labeled in Schedule A, Schedule B- Section 1, and Schedule B-Section 2. This Commitment is of no force and effect unless all schedules are included, along with any Rider pages incorporated by reference in the insert pages.

Parcel #
2



Parcel #1
10985 S. Hwy 441



Columbia County Property Appraiser J. Dayla Crews, CFA - Lake City, Florida - 386-758-1083		0 140 280 420 ft	
PARCEL: 22-5S-17-09328-013 - VACANT (000000)			
LOTS 3,4,9,10,11,12,13,20,21, 22,23,24,29 & 30 BLOCK 3 MASON CITY & ALSO BEG AT SW			
Name: EDIX INVESTMENTS INC	LandVal \$37,050.00		
Site:	BldgVal \$0.00		
Mail: 720 NW 23RD AVE	ApprVal \$37,050.00		
GAINESVILLE, FL 32609	JustVal \$37,050.00		
Sales Info 9/12/2005 \$59,000.00 V / Q	Assd \$37,050.00		
	Exmpt \$0.00		
	Taxable \$37,050.00		

This information, GIS Map Updated: 12/8/2005, was derived from data which was compiled by the Columbia County Property Appraiser Office solely for the governmental purpose of property assessment. This information should not be relied upon by anyone as a determination of the ownership of property or market value. No warranties, expressed or implied, are provided for the accuracy of the data herein, it's use, or it's interpretation. Although it is periodically updated, this information may not reflect the data currently on file in the Property Appraiser's office. The assessed values are NOT certified values and therefore are subject to change before being finalized for ad valorem assessment purposes.

Columbia County Property Appraiser

DB Last Updated: 12/8/2005

2006 Proposed Values

Parcel: 22-5S-17-09328-013

[Back Record](#) [Property Card](#) [Interactive GIS Map](#) [Print](#)

Owner & Property Info

Search Result: 1 of 1

Owner's Name	EDIX INVESTMENTS INC
Site Address	
Mailing Address	720 NW 23RD AVE GAINESVILLE, FL 32609
Brief Legal	LOTS 3,4,9,10,11,12,13,20,21, 22,23,24,29 & 30 BLOCK 3 MASON CITY & ALSO BEG AT SW

Use Desc. (code)	VACANT (000000)
Neighborhood	22517.00
Tax District	3
UD Codes	
Market Area	02
Total Land Area	2.850 ACRES

Property & Assessment Values

Mkt Land Value	cnt: (1)	\$37,050.00
Ag Land Value	cnt: (0)	\$0.00
Building Value	cnt: (0)	\$0.00
XFOB Value	cnt: (0)	\$0.00
Total Appraised Value		\$37,050.00

Just Value	\$37,050.00
Class Value	\$0.00
Assessed Value	\$37,050.00
Exempt Value	\$0.00
Total Taxable Value	\$37,050.00

Sales History

Sale Date	Book/Page	Inst. Type	Sale Vimp	Sale Qual	Sale RCode	Sale Price
9/12/2005	1059/2179	WD	V	Q		\$58,000.00

Building Characteristics

Bldg Item	Bldg Desc	Year Blt	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value
NONE						

Extra Features & Out Buildings

Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
NONE						

Land Breakdown

Lnd Code	Desc	Units	Adjustments	Eff Rate	Lnd Value
000000	VAC RES (MKT)	2.850 AC	1.00/1.00/1.00/1.00	\$13,000.00	\$37,050.00

Columbia County Property Appraiser

DB Last Updated: 12/8/2005

1 of 1

Disclaimer

This information was derived from data which was compiled by the Columbia County Property Appraiser's Office solely for the government purpose of property assessment. The information shown is a work in progress and should not be relied upon by anyone as a determination of the ownership of property or market value. No warranties, expressed or implied, are provided for the accuracy of the data herein, it's use, or it's interpretation. Although it is periodically updated, this information may not reflect the data currently on file in the Property Appraiser's Office. The assessed values are NOT CERTIFIED values and therefore are subject to change before finalized for ad-valorem assessment purposes.

[Scroll to Top](#)

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WARRANTY DEED
INDIVID. TO INDIVID.

Return to: (enclose self-addressed stamped envelope)

Name: Chicago Title-Gainesville

Attn: Lynn Francis

Address: 2831C NW 41st ST, Gainesville, 32606

File Number: 05-433-12

This Instrument Prepared by:

Name: Chicago Title-Gainesville

Attn: Lynn Francis

Address: 2831C NW 41st ST, Gainesville, 32606

Property Appraisers Parcel Identification

Folio Number(s): R09328-008 AND R09333-000

Grantee(s) S.S. # (s)

Inst: 2005023821 Date: 09/28/2005 Time: 08:24

Doc Stamp-Deed : 406.00

DC, P. DeWitt Cason, Columbia County B: 1059 P: 2179

SPACE ABOVE THIS LINE FOR PROCESSING DATA

SPACE ABOVE THIS LINE FOR RECORDING DATA

This Warranty Deed, Made and executed this 12 SEPTEMBER 2005by VERNAL L HUFFINES, A/K/A VERNALL HUFFINES, whose address is 624 INDIANA STREET,
WITCHITA FALLS, TEXAS 76301

hereinafter called the Grantor,

to EDIX INVESTMENTS, INC., whose post office address is

hereinafter called the Grantee.

(Wherever used herein the terms "Grantor" and "Grantee" include all the parties to this instrument and the heirs, legal representatives, and assigns of individuals, and the successors and assigns of corporations, wherever the context so admits or requires)

Witnesseth, That the Grantor, for and in consideration of the sum of \$10.00 (Ten and 00/100 Dollars) and other valuable considerations, receipt whereof is hereby acknowledged, hereby grants, bargains, sells, aliens, remises, releases, conveys and confirms unto the Grantee, all that certain land, situate in Columbia County, State of Florida, viz:LOTS 9, 10, 11, 12, 13, 20, 21, 22, 23 & 24, BLOCK 3 OF MASON CITY AS PER PLAT THEREOF AS RECORDED IN PLAT BOOK 1, PAGE 31 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.
TOGETHER WITH: BEGIN AT THE SW CORNER OF LOT 2 IN BLOCK 3 OF MASON CITY AS PER PLAT THEREOF RECORDED IN PLAT BOOK 1, PAGE 31 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA AND RUN THENCE N 88°30'42" E., ALONG THE SOUTH LINE OF SAID LOT 2 AND LOT 31 IN SAID BLOCK 3, 294.80 FEET TO THE SE CORNER OF SAID LOT 31; THENCE N 03°55'06" W., ALONG THE EAST LINE OF SAID BLOCK 3, 85.36 FEET, THENCE S 86°11'41" W., 294.54 FEET TO A POINT ON THE EASTERLY RIGHT-OF-WAY LINE OF U.S. HIGHWAY #41; THENCE S 03°55'06" E., ALONG SAID EASTERLY RIGHT-OF-WAY LINE, 73.65 FEET TO THE POINT OF BEGINNING. BEING ALL OF LOTS 2 AND 31 AND A PART OF LOT 1 AND 32, BLOCK 3 OF MASON CITY AS PER PLAT THEREOF AS RECORDED IN PLAT BOOK 1, PAGE 31 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.

ALSO:

LOTS 3, 4, 29 & 30 IN BLOCK 3 OF MASON CITY, AS PER PLAT THEREOF RECORDED IN PLAT BOOK 1, PAGE 31 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.

THIS PROPERTY IS NOT THE HOMESTEAD OF THE GRANTOR**Together**, with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.**To Have and to Hold**, the same in fee simple forever.

WARRANTY DEED
INDIVID. TO INDIVID.

Inst:2005023821 Date:09/28/2005 Time:08:24
 Doc Stamp-Deed : 406.00
 DC,P.Dewitt Cason,Columbia County B:1059 P:2180

SPACE ABOVE THIS LINE FOR RECORDING DATA

And the Grantor hereby covenants with said Grantee that the Grantor is lawfully seized of said land in fee simple; that the Grantor has good right and lawful authority to sell and convey said land; and hereby warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever; and that said land is free of all encumbrances, except taxes accruing subsequent to December 31, 2004.

In Witness Whereof, the said Grantor has signed and sealed these presents the day and year first above written.

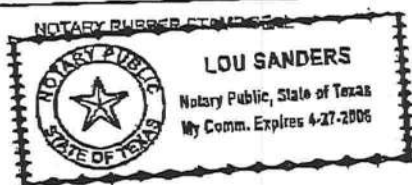
Randy Holley
 Witness
Randy Holley
 Printed name

Lou Sanders
 Witness
Lou Sanders
 Printed name

Vernal L. Huffines
 VERNAL L. HUFFINES, A/K/A VERNALL HUFFINES

STATE OF Texas
 COUNTY OF Wichita

This foregoing instrument was acknowledged before me this 12th DAY OF SEPTEMBER 2005 by VERNAL L. HUFFINES, A/K/A VERNALL HUFFINES. (Check one:) ☐ Said person(s) is/are personally known to me. ☒ Said person(s) provided the following type of identification: Valid Driver's License



Lou Sanders
 Notary Signature
Lou Sanders
 Print Name



STATE OF FLORIDA
DEPARTMENT OF BUSINESS AND
PROFESSIONAL REGULATION

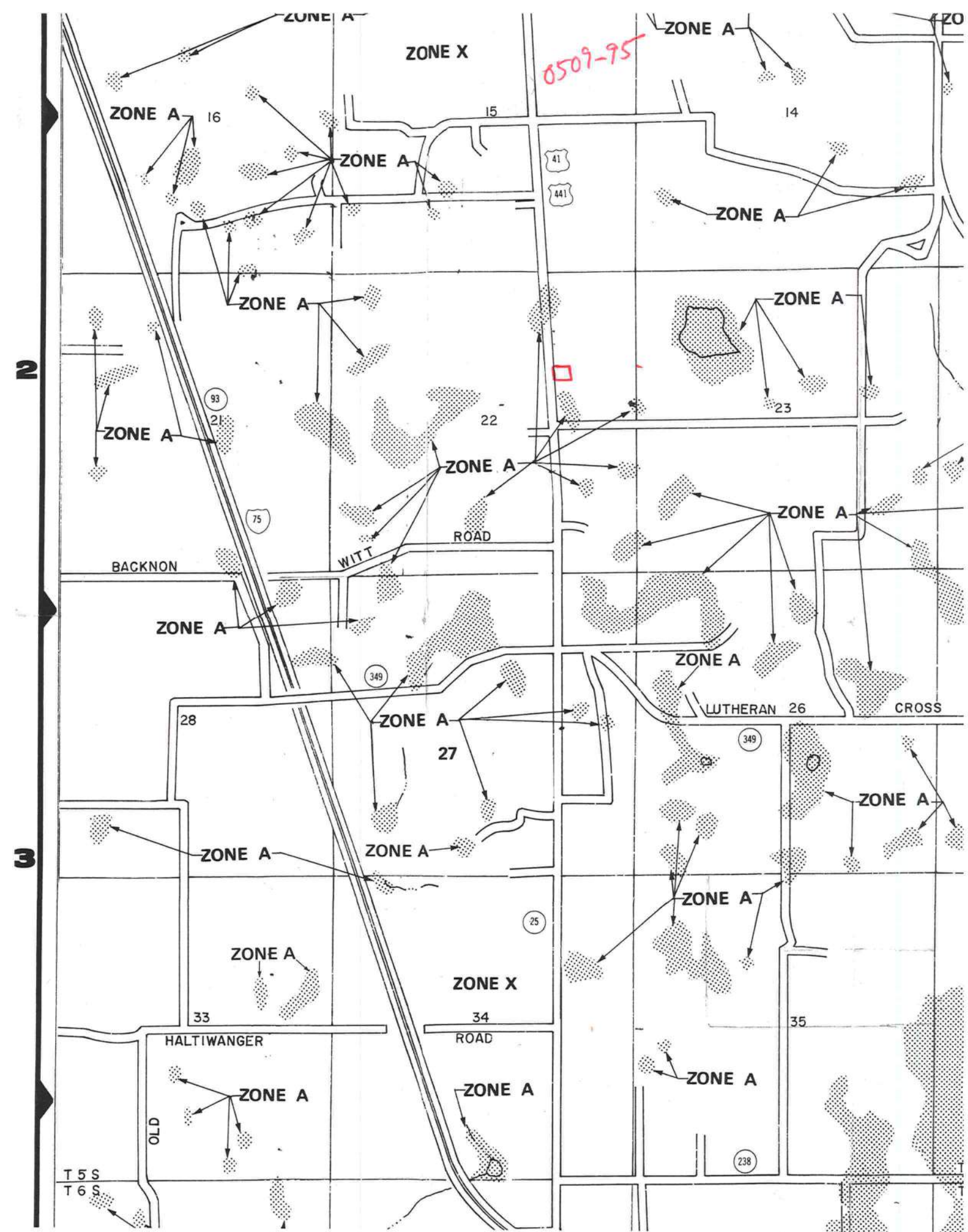
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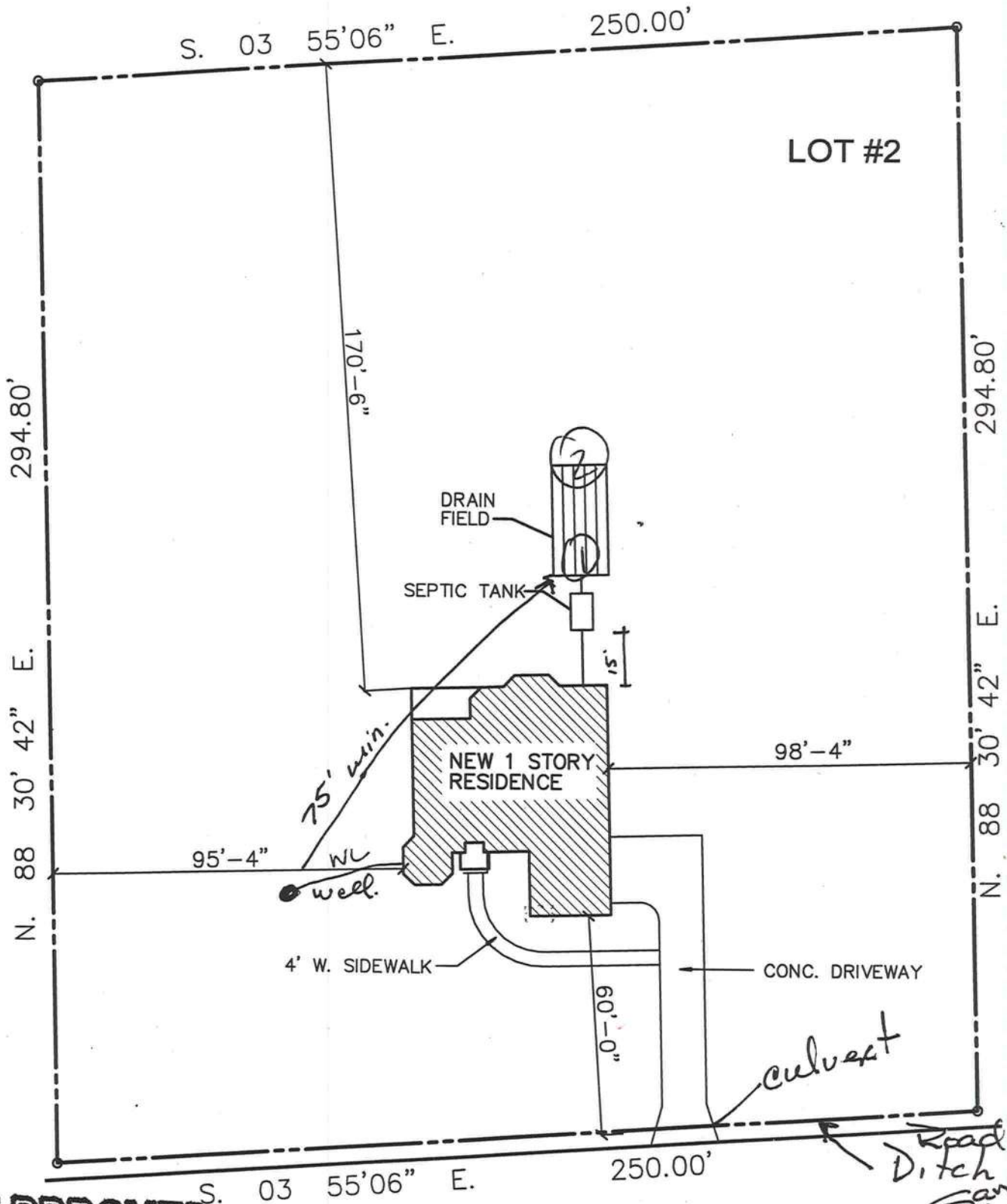
CBC025206

08/23/04 040176084

CERTIFIED BUILDING CONTRACTOR
SANCHEZ, GERMAN
SAN JUAN CONSTRUCTION CO INC

IS CERTIFIED under the provisions of Ch. 489 FS.
Expiration date: AUG 31, 2006 L04082301722





APPROVED

Sallie G 12-2 2005

SITE PLAN

1"=40'-0"



German Sanchez
11-17-05

DATE	REVISION
6-18-2005	

STEWART 10985 S. US HW 441
 LOT #2
 S.R. 441, LAKE CITY, FLORIDA

RICARDO CAVALLINO AND ASSOCIATES, INC.
 ARCHITECTURE-PLANNING AA 002865
 22 SE FIFTH AVENUE GAINESVILLE, FL 32601
 352-377-1751 (O)
 352-377-1765 (F)



WARRANTY DEED
INDIVID. TO INDIVID.

Return to: (enclose self-addressed stamped envelope)

Name: Chicago Title-Gainesville

Attn.: Lynn Francis

Address: 2631C NW 41st ST, Gainesville, 32606

File Number: 05-433-12

This Instrument Prepared by:

Name: Chicago Title-Gainesville

Attn.: Lynn Francis

Address: 2631C NW 41st ST, Gainesville, 32606

Property Appraisers Parcel Identification

Folio Number(s): R09328-008 AND R09333-000

Grantee(s) S.S. # (s)

SPACE ABOVE THIS LINE FOR PROCESSING DATA

SPACE ABOVE THIS LINE FOR RECORDING DATA

This Warranty Deed, Made and executed this 2 SEPTEMBER 2005

by VERNAL L. HUFFINES, A/K/A VERNALL HUFFINES, whose address is 624 INDIANA STREET,
WITCHITA FALLS, TEXAS 76301

hereinafter called the Grantor,

to EDIX INVESTMENTS, INC., whose post office address is

hereinafter called the Grantee.

(Wherever used herein the terms "Grantor" and "Grantee" include all the parties to this instrument and the heirs, legal representatives, and assigns of individuals, and the successors and assigns of corporations, wherever the context so admits or requires)

Witnesseth, That the Grantor, for and in consideration of the sum of \$10.00 (Ten and 00/100 Dollars) and other valuable considerations, receipt whereof is hereby acknowledged, hereby grants, bargains, sells, aliens, remises, releases, conveys and confirms unto the Grantee, all that certain land, situate in Columbia County, State of Florida, viz:

LOTS 9, 10, 11, 12, 13, 20, 21, 22, 23 & 24, BLOCK 3 OF MASON CITY AS PER PLAT THEREOF AS RECORDED IN PLAT BOOK 1, PAGE 31 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.
TOGETHER WITH: BEGIN AT THE SW CORNER OF LOT 2 IN BLOCK 3 OF MASON CITY AS PER PLAT THEREOF RECORDED IN PLAT BOOK 1, PAGE 31 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA AND RUN THENCE N 88°30'42" E., ALONG THE SOUTH LINE OF SAID LOT 2 AND LOT 31 IN SAID BLOCK 3, 294.80 FEET TO THE SE CORNER OF SAID LOT 31; THENCE N 03°55'06" W., ALONG THE EAST LINE OF SAID BLOCK 3, 85.36 FEET, THENCE S 86°11'41" W., 294.54 FEET TO A POINT ON THE EASTERLY RIGHT-OF-WAY LINE OF U.S. HIGHWAY # 41; THENCE S 03°55'06" E., ALONG SAID EASTERLY RIGHT-OF-WAY LINE, 73.65 FEET TO THE POINT OF BEGINNING. BEING ALL OF LOTS 2 AND 31 AND A PART OF LOT 1 AND 32, BLOCK 3 OF MASON CITY AS PER PLAT THEREOF AS RECORDED IN PLAT BOOK 1, PAGE 31 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.

ALSO:

LOTS 3, 4, 29 & 30 IN BLOCK 3 OF MASON CITY, AS PER PLAT THEREOF RECORDED IN PLAT BOOK 1, PAGE 31 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.

THIS PROPERTY IS NOT THE HOMESTEAD OF THE GRANTOR

Together, with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

To Have and to Hold, the same in fee simple forever.

SPACE ABOVE THIS LINE FOR RECORDING DATA

And the Grantor hereby covenants with said Grantee that the Grantor is lawfully seized of said land in fee simple; that the Grantor has good right and lawful authority to sell and convey said land; and hereby warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever; and that said land is free of all encumbrances, except taxes accruing subsequent to December 31, 2004.

In Witness Whereof, the said Grantor has signed and sealed these presents the day and year first above written.

Randy Holley
Witness

Randy Holley
Printed name

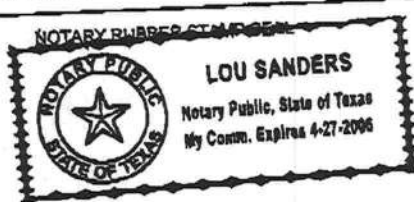
Lou Sanders
Witness

Lou Sanders
Printed name

Vernal L. Huffines
VERNAL L. HUFFINES, A/K/A VERNAL L. HUFFINES

STATE OF Texas
COUNTY OF Wichita

This foregoing instrument was acknowledged before me this 12th DAY OF SEPTEMBER 2005 by VERNAL L. HUFFINES, A/K/A VERNAL L. HUFFINES. (Check one:) ☐ Said person(s) is/are personally known to me. ☒ Said person(s) provided the following type of identification: Valid Driver's License



Lou Sanders
Notary Signature
Lou Sanders
Print Name

**ALTA COMMITMENT
CHICAGO TITLE INSURANCE COMPANY
SCHEDULE A**

Office File Number:
05-433-12

Effective Date: August 20, 2005

1. Policy or Policies to be issued:

ALTA OWNER'S POLICY, Form B (amended 10/17/92)
with Florida Modifications \$ 58,000.00
Proposed Insured:

EDIX INVESTMENTS, INC.

ALTA LOAN POLICY (amended 10/17/92)
with Florida Modifications \$
Proposed Insured:

371-2020

2. The estate or interest in the land described or referred to in this Commitment and covered herein is a fee simple, and title thereto is at the effective date hereof vested in:

VERNAL L. HUFFINES, A/K/A VERNALL HUFFINES

3. The Land is described as follows:

LOTS 9, 10, 11, 12, 13, 20, 21, 22, 23 & 24, BLOCK 3 OF MASON CITY AS PER PLAT THEREOF AS RECORDED IN PLAT BOOK 1, PAGE 31 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.

TOGETHER WITH: BEGIN AT THE SW CORNER OF LOT 2 IN BLOCK 3 OF MASON CITY AS PER PLAT THEREOF RECORDED IN PLAT BOOK 1, PAGE 31 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA AND RUN THENCE N 88°30'42" E., ALONG THE SOUTH LINE OF SAID LOT 2 AND LOT 31 IN SAID BLOCK 3, 294.80 FEET TO THE SE CORNER OF SAID LOT 31; THENCE N 03°55'06" W., ALONG THE EAST LINE OF SAID BLOCK 3, 85.36 FEET, THENCE S 86°11'41" W., 294.54 FEET TO A POINT ON THE EASTERLY RIGHT-OF-WAY LINE OF U.S. HIGHWAY # 41; THENCE S 03°55'06" E., ALONG SAID EASTERLY RIGHT-OF-WAY LINE, 73.65 FEET TO THE POINT OF BEGINNING. BEING ALL OF LOTS 2 AND 31 AND A PART OF LOT 1 AND 32, BLOCK 3 OF MASON CITY AS PER PLAT THEREOF AS RECORDED IN PLAT BOOK 1, PAGE 31 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.

NOTE: This Commitment consists of insert pages labeled in Schedule A, Schedule B- Section 1, and Schedule B-Section 2. This Commitment is of no force and effect unless all schedules are included, along with any Rider pages incorporated by reference in the insert pages.

A.L.T.A. COMMITMENT
Chicago TITLE INSURANCE COMPANY
SCHEDULE A, continued

Office File Number
05-433-12

ALSO:

LOTS 3, 4, 29 & 30 IN BLOCK 3 OF MASON CITY, AS PER PLAT THEREOF RECORDED IN
PLAT BOOK 1, PAGE 31 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.

** TOTAL PAGE.03 **

DAMPIER SEPTIC TANK CO., INC.
PORTA POTTI'S & WELLS
7030 NW 23RD WAY
GAINESVILLE, FL 32653
PH. (352) 375-8873 • (352) 378-2859
FAX (352) 378-9945

Date: 11/17/2005

To: Edwin Dix

From: Freddie Dampier
Dampier Septic Tank, Inc.

Well specifications for Lot 1 & Lot 2
State Road 441, Columbia County

- 1 HP Jacuzzi Submersible Pump (model #1S418-10)
- 1 ¼" drop pipe from pump to tank
- 82 gallon equivalent diaphragm tank
- 1 ¼" cycle stop valve
- ¾" hose bib
- 1 ¼" ball valve (for supply line)
- Black steel casing

Ridgway Roof Truss Company

(Trusses and Prefabricated Building Components)

Mailing: P.O. Box 1309 – Gainesville, Florida 32602

Physical: 235 SW 11th Place – Gainesville, Florida 32601

Telephone: (352) 376-4436
FAX: (352) 371-3316

Email: Sales@RidgwayTruss.com
www.RidgwayTruss.com

WARNING

**THESE TRUSSES MUST BE
HANDLED ACCORDING TO
BCSI-B1 SUMMARY SHEETS**

SEE TABLE OF CONTENTS

SEALED **DESIGN MANUAL**

PROJECT NAME: SR Lot 441- Lot 1 Lakecity

JOB NUMBER: 05-1062

CONTRACTOR: Edix Investments

DATE: 11/15/05

REVISIONS: _____

COMMENTS: _____

62-1000

RIDGWAY ROOF TRUSS COMPANY

P.O. BOX 1309 GAINESVILLE, FL 32602, TELEPHONE: (352) 376-4436 FAX: (352) 371-3316

Job: **SR 441 Lot 1 Lakecity**

Customer: **Edix Investments**

Jurisdiction: **Alachua**

Structural Eng. of Record: none

License #: none

building designer

Address of Eng. Of Record: none

SIGNATURE of Building Designer: *single family home*

I, Gary G. Dounson, Florida Professional Engineer No.35054, hereby certify that I am the truss engineer of record with responsibility for the design of the metal plate connected wood trusses listed herein. The design of these trusses have been performed under my responsible supervision, direction and control and are in accordance with the National Design Standard for Metal Plate Connected Wood Truss Construction (ANSI TPI 1). Building Designer of Record responsibilities shall be required in Article 2.2 of the aforementioned TPI 1 document and shall include but are not limited to the design and detailing of truss supports, anchorage, and permanent truss bracing.

Engineering Program is **Robbins Engineering Online Plus**

Building code and chapter: **ANSI/ASCE 7-98**

Live load: **20 lbs**

Dead load: **17 lbs**

Roof

Wind speed: **110 mph**

Design criteria: **TPI 1-95**

Mean Roof Height: **25'**

Exposure: **B**

Enclosure: **Enclosed**

Category: **II**

#	Mark	Run Date	#	Mark	Run Date	#	Mark	Run Date	#	Mark	Run Date	#	Mark	Run Date
1	R1	11/14/05	29	V1	11/14/05	57			85			107		
2	R2	11/14/05	30	V2	11/14/05	58			86			108		
3	R3	11/14/05	31	V3	11/14/05	59			87			109		
4	R4	11/14/05	32	V4	11/14/05	60			88			110		
5	R5	11/14/05	33	V5	11/14/05	61			89			111		
6	R6	11/14/05	34			62			90			112		
7	R7	11/14/05	35			63			91			113		
8	R8	11/14/05	36			64			92			114		
9	R9	11/14/05	37			65			93			115		
10	R10	11/14/05	38			66			94			116		
11	R11	11/14/05	39			67			95			117		
12	R12	11/14/05	40			68			96			118		
13	R13	11/14/05	41			69			97			119		
14	R14	11/14/05	42			70			98			120		
15	R15	11/14/05	43			71			99			121		
16	R16	11/14/05	44			72			100			122		
17	R17	11/14/05	45			73			101			123		
18	R18	11/14/05	46			74			102			124		
19	R19	11/14/05	47			75			103			125		
20	R20	11/14/05	48			76			104			126		
21	R21	11/14/05	49			77			105			127		
22	R22	11/14/05	50			78			106			128		
23	R23	11/14/05	51			79			Delegated Engineer (Truss Designer) GARY DOUNSON, PE 35054 GARY DOUNSON & ASSOCIATES, INC. 2830 NW 41st STREET SUITE D GAINESVILLE, FL 32606 (352) 375-8593 CA 5201 11/15/2005					
24	R24	11/14/05	52			80								
25	R25	11/14/05	53			81								
26	R26	11/14/05	54			82								
27	R27	11/14/05	55			83								
28	R3A	11/14/05	56			84								

This Package includes 33 individual, dated Truss Design Drawings. With my Seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003 section 5 of the Florida Board of Professional Engineers.

Note: The seal on this index sheet indicates acceptance of professional engineering responsibility solely for the Truss Design Drawings listed above and attached. The suitability and use of each component for any particular building is the responsibility of the Building Designer, per ANSI/TPI 1-1995 Section 2.

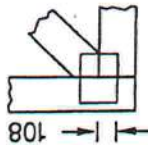
Gary Dounson
11-15-05

TABLE OF CONTENTS

1. GENERAL NOTES	
2. WTCA 1-1995 (STANDARD RESPONSIBILITIES IN THE DESIGN PROCESS INVOLVING METAL PLATE CONNECTED WOOD TRUSSES)	
ENGINEERING (INSERT)	
BLUEPRINT OF TRUSS PLACEMENT PLAN (INSERT)	
6. BCSI-B1 SUMMARY SHEET	
HANDLING, INSTALLING & BRACING INFORMATION	
8. STANDARD CHORD & WEB REPAIRS	
11. EXAMPLES OF PERMANENT WEB BRACING	

ROBBINS ENG. GENERAL NOTES & SYMBOLS

PLATE LOCATION



Center plates on joints unless otherwise noted in plate list or on drawing. Dimensions are given in inches (i.e. 1 1/2" or 1.5") or IN-16ths (i.e. 108).

PLATE SIZE



The first dimension is the width measured perpendicular to slots. The second dimension is the length measured parallel to slots.

LATERAL BRACING

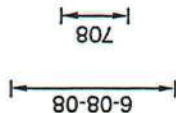
1 x4 continuous lateral bracing attached with (2) 8d nails each member where indicated or 2x4 "T" or "L" brace stiffener if applicable nailed flat to edge of web with 12d nails spaced 8" o.c. "T" or "L" brace must be extended at least 90% of web length.

PLATE ORIENTATION



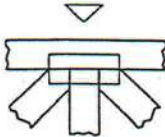
When shown, indicates direction of slots in connector plate.

DIMENSIONS



All dimensions are shown in FT-IN-SX (i.e. 6' 8 1/2" or 6-08-08). Dimensions less than one foot are shown in IN-SX only (i.e. 708).

BEARING



When truss is designed to bear on multiple supports, interior bearing locations should be marked on the truss. Interior support or temporary shoring must be in place before erecting this truss. If necessary, shim bearings to assure solid contact with truss.

ROBBINS LOCK connector plates (20 ga. galv. steel ASTM A653 SS Grade 40) shall be applied on both faces of truss at each joint. Center the plates, unless shown otherwise by circles (o) or dimensions. No loose knots or wanes in plate contact area. Splice only where shown. Overall spans assume 4" bearings at each end, unless indicated otherwise. Cutting and fabrication shall be performed on equipment which produces snug-fitting joints and plates. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication and these designs are not applicable for use with fire retardant lumber. This design was prepared in accordance with "National Design Specifications for Stress - Grade Lumber and Its Fastenings" (AFPA), "Design Specifications for Light Metal Plate Connected Wood Trusses" (TPI), and HUD Design Criteria for

FURNISH A COPY OF THIS DESIGN TO ERECTION CONTRACTOR. IT IS THE RESPONSIBILITY OF BUILDING DESIGNER TO REVIEW THIS DRWG. & VERIFY THAT DATA INCLUDING DIM. & LOADS CONFORM TO ARCH. PLAN/SPECS & FAB. TRUSS LAYOUTS.



CORPORATE HEADQUARTERS
P.O. Box 280055
Tampa, FL 33682-0055
800-282-1299 • Fax: 813-971-6117

Developed by the WTCA Engineering Review Committee
in cooperation with the Truss Plate Institute

Standard Responsibilities in the Design Process Involving Metal Plate Connected Wood Trusses

WTCA 1-1995



Wood Truss Council of America
5937 Meadowood Dr., Ste. 14 • Madison, WI 53711-4125 • 608/274-3329 (fax)

The Wood Truss Council of America publishes standard practice materials prepared and edited by knowledgeable authors from the construction industry to give as much assistance as possible to owners, architects, engineers, contractors, building officials, and others involved in the metal plate connected wood truss industry. The competence of the authors ensures accurate and authoritative information in regard to the subject matter covered, but, of course, neither the Wood Truss Council of America, nor the authors make either express or implied warranties in regard to the use of the materials.

1.0 SCOPE AND DEFINITIONS

- 1.1 This standard defines the design responsibilities of the individuals and organizations involved in the preparation, submittal, review and approval of each Truss Design Drawing and Truss Placement Plan associated with the use of metal plate connected wood trusses. These guidelines are presented as industry standard practice. The guidelines are not intended to preclude alternate provisions as agreed upon by the parties involved.

1.2 The following definitions shall apply:

1.2.1 "Architect" shall mean the individual registered architect responsible for the architectural design of the structure and who produces the architectural drawings included in the Construction Design Documents.

1.2.2 "Building Designer" is the individual or organization having responsibility for the overall building or structure design in accordance with the state's statutes and regulations governing the professional registration and certification of architects or engineers. This responsibility includes but is not limited to foundation design, structural member sizing, load transfer, bearing conditions, and the structure's compliance with the applicable building codes. Also referred to as registered architect or engineer, building designer, and registered building designer, but hereinafter will be referred to as Building Designer.

1.2.3 "Construction Design Documents" are the architectural drawings, structural drawings, mechanical drawings, electrical drawings, and addenda which set forth the overall design of the structure and issued by the Building Designer.

1.2.4 "Contractor" shall mean the individual or organization responsible for the field storage, handling, and installation of trusses including, but not limited to, temporary bracing, permanent

1.2.5 "Engineer-of-Record" shall mean the registered professional engineer responsible for the structural design of the structure and who produces the structural drawings included in the Construction Design Documents.

1.2.6 "Owner" shall mean the individual or organization for whom the structure is designed.

1.2.7 "Truss" is an individual metal plate connected wood structural component manufactured by the Truss Manufacturer.

1.2.8 "Truss Designer" is the design professional, individual or organization, having responsibility for the design of metal plate connected wood trusses. This responsibility shall be in accordance with the state's statutes and regulations governing the professional registration and certification of architects or engineers. Also referred to as truss engineer, design engineer, registered engineer, and engineer, but hereinafter will be referred to as Truss Designer.

1.2.9 "Truss Design Drawing" shall mean the graphic depiction of an individual Truss prepared by the Truss Designer.

1.2.10 "Truss Manufacturer" shall mean an individual or organization regularly engaged in the manufacturing of Trusses.

1.2.11 "Truss Placement Plan" is the drawing identifying the location assumed for each Truss based on the Truss Manufacturer's interpretation of the Construction Design Documents.
- 3

2.0 OWNER RESPONSIBILITIES

- 2.1

Directly or through its representatives, which may include the Contractor and/or Building Designer; (a) review and approve each Truss Design Drawing; (b) review and approve the Truss Placement Plan; (c) resolve and approve all design issues arising out of the preparation of each
- Truss Design Drawing and Truss Placement Plan; and (d) coordinate the return of each approved Truss Design Drawing and Truss Placement Plan to the Truss Manufacturer prior to truss manufacturing.

3.0 BUILDING DESIGNER RESPONSIBILITIES

- 3.1

Design a structure suitable to ensure that the intended function of each Truss is not affected by adverse influences including, but not limited to, moisture, temperature, corrosive chemicals and gases;
- 3.2

Prepare the Construction Design Documents, showing all trussed areas, which must provide as a minimum the following:
- 3.2.1

All truss orientations and locations;
- 3.2.2

Information to fully determine all truss profiles;
- 3.2.3

Adequate support of the Truss and all truss bearing conditions;
- 3.2.4

Permanent bracing design for the structure including the Trusses, except as provided in 3.4 and 6.2.12.
- 3.2.5

The location, direction and magnitude of all dead and live loads applicable to each Truss including, but not limited to, loads attributable to: roof, floor, partition, mechanical, fire sprinkler, attic, storage, wind, snow drift and seismic;
- 3.2.6

All Truss anchorage designs required to resist uplift, gravity, and lateral loads;
- 3.2.7

Allowable vertical and horizontal deflection criteria;
- 3.2.8

Proper transfer of design loads affecting the Truss; and
- 3.2.9

Adequate connections between Truss and non-Truss components, except as noted in Section 6.2.9.

4.0 CONTRACTOR RESPONSIBILITIES

- 4.1

Provide to the Truss Manufacturer the Construction Design Documents and all revisions and supplements thereto.
- 4.2

Review and approve the Truss Placement Plan and each Truss Design Drawing for conformance with the requirements and intent of the Construction Design Documents, and the effect of the Truss Placement Plan and each Truss Design Drawing on other trades involved in the construction of the structure and the effect of the other trades on the Trusses.
- 4.3

Coordinate the review, approval and return of each Truss Design Drawing and the Truss Placement Plan by the Owner and Building Designer.
- 4.4

Provide the approved Truss Design Drawings, approved Truss Placement Plans, and any supplemental information provided by the Truss Manufacturer to the individual or organization responsible for the installation of the Trusses.
- 4.5

Comply with the field storage, handling, installation, permanent bracing, anchorage, connections and field assembly requirements of the Construction Design Documents.
- 4.6

Determine and install the temporary bracing for the structure, including the Trusses.

WTCA is committed to promoting the common interests of all engaged in the manufacture of wood trusses and related components to ensure growth, continuity, and increased professionalism in our industry. Fundamental to this is promoting the safe, economic, and structurally sound use of trusses in all applications.

Wood Truss Council of America's Objective

7.1 Any party who cuts or damages a truss shall be responsible for securing the engineering required for the repair and for subsequent costs.

7.0 OTHER RESPONSIBILITIES

- 6.2.5 Adjustments to lumber and metal connector plate design values for conditions of use;
- 6.2.4.6 Controlling wind and earthquake loads;
- 6.2.4.5 Concentrated loads and their points of application; and
- 6.2.4.4 Bottom chord dead load;
- 6.2.4.3 Bottom chord live load;
- 6.2.4.2 Top chord dead load;
- 6.2.4.1 Top chord live load (including snow loads);
- 6.2.4 Design loads as applicable:
- 6.2.3 Required bearing widths;
- 6.2.2 Location of all joints;
- 6.2.1 Slope or depth, span and spacing;
- 6.2 For each Truss Design Drawing, set forth as a minimum the following:
 - 6.1 Prepare the Truss Design Drawings in conformance with the requirements set forth in the latest approved edition of ANSI/TPI 1-1995 "National Design Standard for Metal Plate Connected Wood Truss Construction."
- 6.2.7 Metal connector plate type, size, thickness or gage, and the dimensioned location of each metal connector plate except where symmetrical location relative to the joint interface;
- 6.2.8 Lumber size, species, and grade for each member;
- 6.2.9 Connection Requirements for:
 - (a) Truss to Truss girder;
 - (b) Truss ply to ply; and
 - (c) Field splices;
- 6.2.10 Calculated deflection ratio and/or maximum deflection for live and total load;
- 6.2.11 Maximum axial compression forces in the Truss members to enable the Building Designer to design the size, connections and anchorage of the permanent continuous lateral bracing. Forces may be shown on the Truss Design Drawing or on supplemental documents; and
- 6.2.12 Required permanent Truss member bracing location.

6.0 TRUSS DESIGNER RESPONSIBILITIES

- 5.1 Communicate the design criteria from the Construction Design Documents to the Truss Designer.
- 5.2 Where required by the Construction Design Documents, prepare the Truss Placement Plan, providing as a minimum the location assumed for each Truss based on the Truss Manufacturer's interpretation of the Construction Design Documents.
- 5.4 Manufacture the Trusses in accordance with the final approved Truss Design Drawings using the quality criteria for Metal Plate Connected Wood Trusses established by the ANSI/TPI 1-1995 "National Design Standard for Metal Plate Connected Wood Truss Construction."
- 5.3 Submit to the Contractor the Truss Placement Plan, as may be required, and each Truss Design Drawing for review and approval.

5.0 TRUSS MANUFACTURER RESPONSIBILITIES

Plus 9 Wind Load Case(s)	Plus 1 UBC LT Load Case(s)	jt	React	Uplift	Size	Req'd
			Lbs	Lbs	In-Sx	In-Sx
		A	2867	983	4- 0	1-11
		D	2781	943	4- 0	1-10
					HZ =	HZ =
						74

	CS1	-Size-	-----Lumber-----
TC	0.84	2x 4	SP-#2
BC	0.60	2x 6	SP-#2
WB	0.43	2x 4	SP-#3

Brace truss as follows:

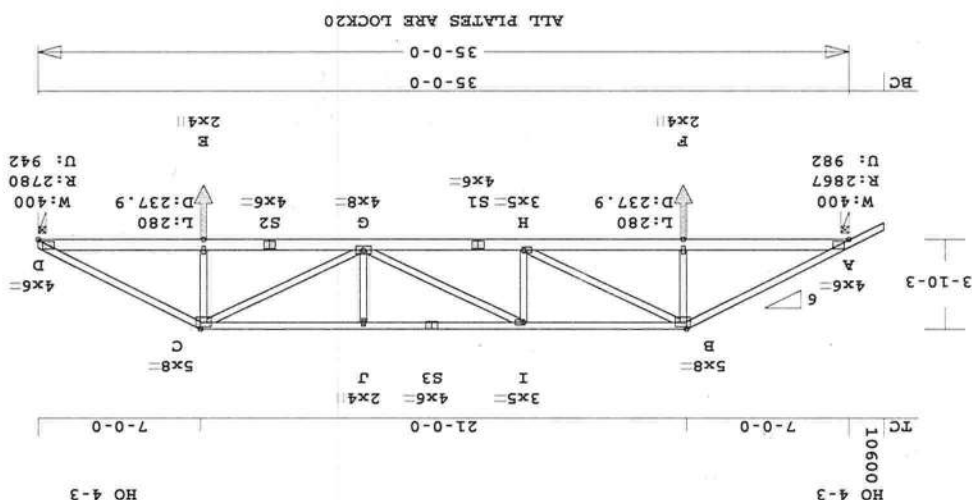
From
To

```
Online Plus -- Version 18.0.014
RUN DATE: 14-NOV-05
*****
* 2-PLY Truss
*****
```

Plates for each ply each face.	TO BE MANUFACTURED BY	ROBBINS ENGINEERING, INC.	LOCK 20 ga, Gross Area	Plate -	RHS 20 ga, Gross Area	Plt Size	X	Y	USI
						4.0	6.0	6.0	0.72
						5.0	8.0	8.0	0.93
						3.0	5.0	5.0	0.52
						4.0	6.0	6.0	0.93
						2.0	4.0	4.0	0.44
						5.0	8.0	8.0	0.93
						4.0	6.0	6.0	0.72
						3.0	5.0	5.0	0.52
						4.0	6.0	6.0	0.93
						4.0	6.0	6.0	0.74
						4.0	6.0	6.0	0.63
						4.0	6.0	6.0	0.56
						4.0	6.0	6.0	0.66
						2.0	4.0	4.0	0.44

	Top Chords	Bottom Chords	
A-B	0.35	5661 C	0.06
B-I	0.83	7508 C	0.25
I-S3	0.62	7490 C	0.11
S3-J	0.61	7490 C	0.11
J-C	0.84	7490 C	0.25
C-D	0.35	5666 C	0.06
A-F	0.38	5079 T	0.33
F-H	0.44	5054 T	0.33
H-S1	0.56	7508 T	0.50
S1-G	0.60	7508 T	0.50
G-S2	0.45	5057 T	0.33
S2-E	0.44	5057 T	0.33
E-D	0.38	5083 T	0.33
F-B	0.11	816 T	0.05
B-H	0.43	2730 T	0.05
H-I	0.00	868 C	0.12
I-G	0.00	45 T	0.10
G-J	0.08	872 C	0.06
J-C	0.43	2707 T	0.11
E-C	0.11	822 T	0.12
TL Defl -0.25"	In SI-G	L/999	
TL Defl -0.47"	In SI-G	L/872	
Shear // Grain	In B-I	0.29	

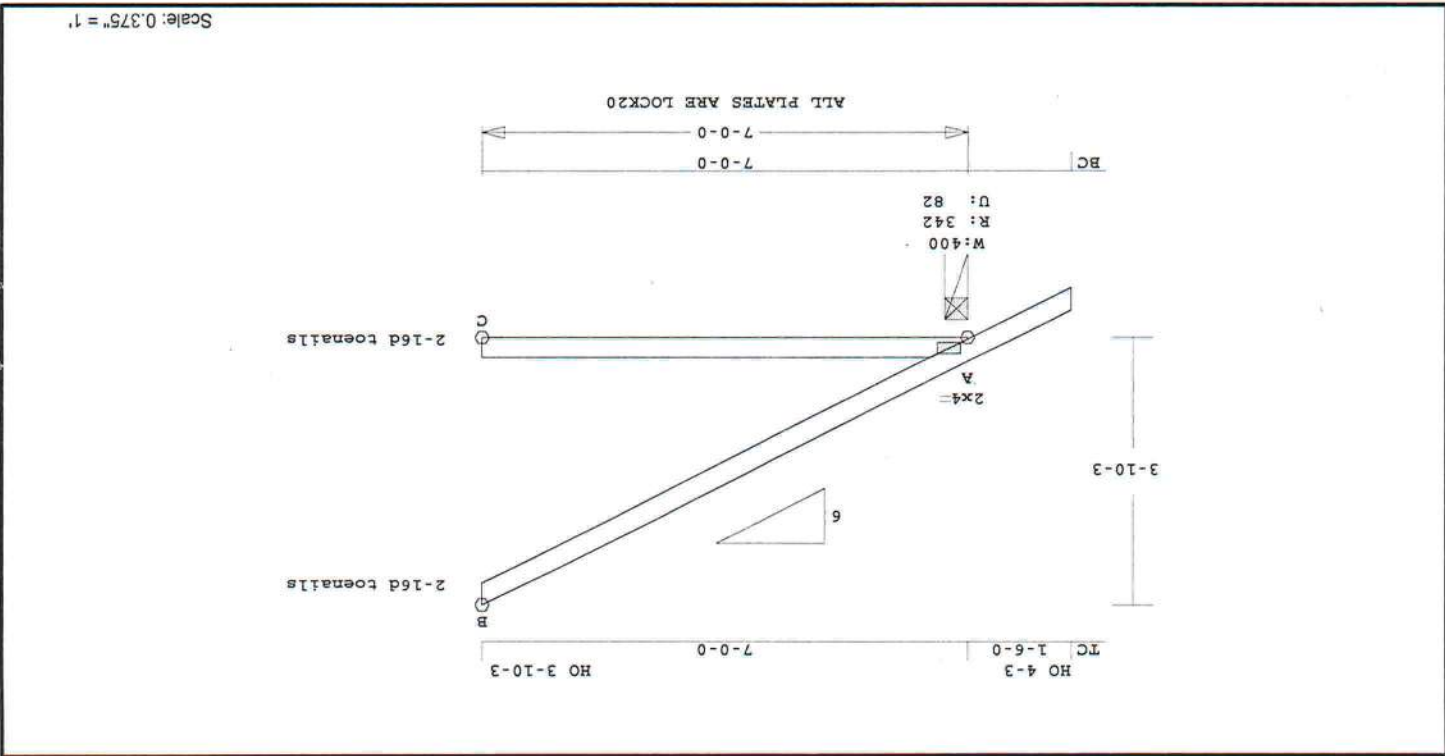
FRAMES MANUFACTURED BY:
 RIDGWAY ROOF TRUSS
 Analysis Conforms To:
 PRC2004
 Girder
 Step Down Hip
 Framing
 King Jacks
 Jack Open Faced
 S setbacks 7 - 0 - 0
 2 COMPLETE TRUSSES REQUIRED.
 Fasten together in staggered
 pattern. (1/2" bolts - OR -
 SPS screws - OR - 10d nails
 as each layer is applied.)
 ----Spacing (in)----
 Rows Nails Screws Bolts
 TC 1 12 24 0
 BC 2 12 24 0
 WB 1 4 4
 Plus clusters of nails where
 shown.
 OH Loading
 Soffit pft 2.0
 Design checked for 10 pft non-
 concurrent LL on BC.
 Prevent truss rotation at all
 bearing locations.
 Wind Loads - ANSI / ASCE 7-02
 Truss is designed as
 Components and Claddings*
 for Exterior zone location.
 Wind Speed:
 Mean Roof Height: 25-0
 Exposure Category: B
 Occupancy Factor : 1.00
 Building Type: Enclosed
 TC Dead Load:
 BC Dead Load:
 Max comp. force 7508 lbs
 Quality Control Factor 1.25
 FABRICATOR NOTES:
 1. Delegated Engineer (Truss
 Designer)
 Gary Dounson, PE 35054
 Gary Dounson & Associates,
 Inc.
 2830 NW 41st Street Suite I
 Gainesville, FL 32606
 CA 5201 (352) 375-8593



SR 441 - LOT 1 LAKECITY

Job	051062	Mark	RI	Quan	1*2P	Type	HIPP	Span	3500000	6	P1-H1	Left OH	1 - 6 - 0	0	Right OH	Engineering
-----	--------	------	----	------	------	------	------	------	---------	---	-------	---------	-----------	---	----------	-------------

Job	051062	Mark	R2	Quan	30	Type	JACK	Span	70000	Pl-HI	6	Left OH	1-6-0	Right OH	0	Engineering
-----	--------	------	----	------	----	------	------	------	-------	-------	---	---------	-------	----------	---	-------------



Online Plus -- Version 18.0.014
 RUN DATE: 14-NOV-05

CSI -Size- ---Lumber---
 TC 0.53 2x 4 SP-#2
 BC 0.42 2x 4 SP-#2

Brace truss as follows:
 O.C. From To
 TC Cont. 0-0-0 7-0-0
 BC Cont. 0-0-0 7-0-0

Loading Live Dead (psf)

TC 20.0 7.0
 BC 0.0 10.0
 Total 20.0 17.0
 Spacing 24.0"

Lumber Duration Factor 1.25
 Plate Duration Factor 1.25
 TC Fb=1.15 Fc=1.10 Ft=1.10
 BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 8 Wind Load Case(s)
 Plus 1 UBC LL Load Case(s)

Ut React Uplift Size Req'd
 Lbs In-Sx In-Sx
 A 342 82 4-0 1-0 Hz = 156
 B 177 146 3-8 1-0 Hz = 84
 C 129 0 3-8 1-0 Hz = 84

Member CSI P Lbs Axl-CSI-Bnd
 A-B 0.53 165 C 0.00 0.53
 ---Top Chords---

For proper installation of
 toe-nails, refer to the 2001
 National Design Specification
 (NDS) for Wood Construction

NOTES:
 Trusses Manufactured by:
 RIDGWAY ROOF TRUSS
 Analysis Conforms To:
 FBC2004
 OH Loading
 Soffit psf 2.0
 Design checked for 10 psf non-

concurrent LL on BC.
 Wind Loads - ANSI / ASCE 7-02
 Truss is designed as
 Components and Claddings*
 for Exterior zone location.
 Wind Speed:
 110 mph
 Mean Roof Height: 25-0
 Exposure Category: B

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-----Bottom Chords-----
 A - C 0.42 83 T 0.00 0.42
 Occupancy Factor : 1.00
 Building Type: Enclosed
 TC Dead Load: 4.0 psf
 BC Dead Load: 6.0 psf
 Max comp. force 165 lbs
 Quality Control Factor 1.25

Plates for each ply each face.
 ATL CONNECTOR PLATES
 TO BE MANUFACTURED BY
 ROBBINS ENGINEERING, INC.
 Plate - LOCK 20 Ga, Gross Area
 Plate - RHS 20 Ga, Gross Area
 Ut Type Plt Size X Y JSI
 A LOCK 2.0x 4.0 Ctr Ctr 0.69

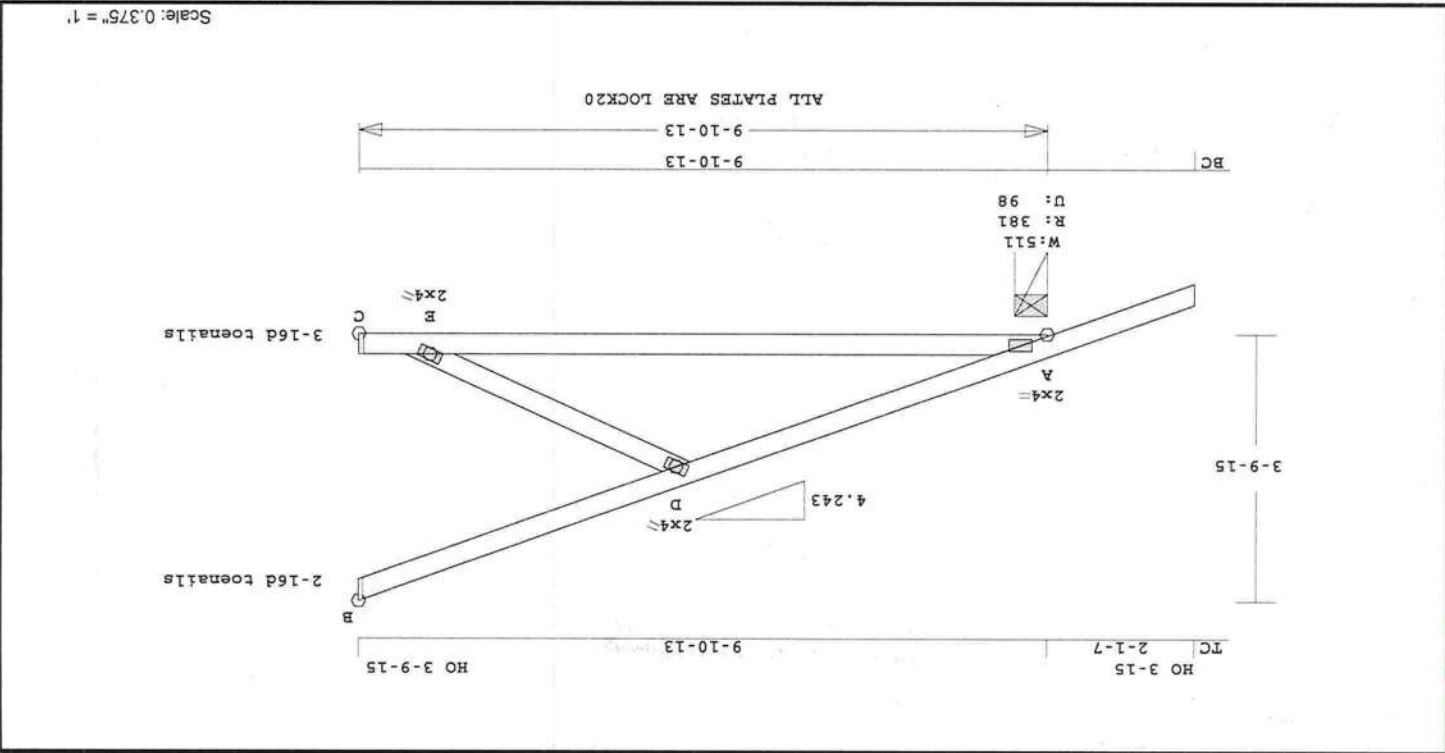
FABRICATOR NOTES:
 I. Delegated Engineer (Truss
 Designer)
 Gary Dounson, PE 35054
 Gary Dounson & Associates,
 Inc.
 2830 NW 41st Street Suite D
 Gainesville, FL 32606
 (352)375-8593
 CA 5201

Trusses Manufactured by:
 RIDGWAY ROOF TRUSS
 Analysis Conforms To:
 FBC2004
 OH Loading
 Soffit psf 2.0
 Design checked for 10 psf non-

concurrent LL on BC.
 Wind Loads - ANSI / ASCE 7-02
 Truss is designed as
 Components and Claddings*
 for Exterior zone location.
 Wind Speed:
 110 mph
 Mean Roof Height: 25-0
 Exposure Category: B

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Job	051062	Mark	R3	Quan	6	Type	KING	Span	91013	Pl-HI	4.243	Left OH	2-1-7	Right OH	0	Engineering
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Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 51.2 LBS

Tr React Uplift Size Req'd

Notes:

Trusses Manufactured by:

RIDGEWAY ROOF TRUSS

Analysis Conforms To:

FB2004

Girder King Jack

Loading TC and BC

Setback 7-0-0

OH Loading

Soffit psf 2.0

Design checked for 10 psf non-

concurrent LT on BC.

Use properly rated hangers for

loads framing into girder

truss.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as

Components and Claddings*

for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 25-0

Exposure Category: B

Occupancy Factor: 1.00

Building Type: Enclosed

TC Dead Load:

BC Dead Load:

Max comp. force

501 lbs

Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss

Designer)

Gary Dounson, PE 35054

Inc.

2830 NW 41st Street Suite D

Gainesville, FL 32606

(352)375-8593

CA 5201

Online Plus -- Version 18.0.014 A

RUN DATE: 14-NOV-05

CSI -Size- -Lumber-

TC 0.45 2x 4 SP-#2

BC 0.72 2x 4 SP-#2

WB 0.16 2x 4 SP-#3

Brace truss as follows:

O.C. From To

TC Cont. 0-0-0 9-10-13

BC Cont. 0-0-0 9-10-13

Loading Live Dead (psf)

TC 20.0 7.0

BC 0.0 10.0

Total 20.0 17.0

Spacing

Lumber Duration Factor 1.25

Plate Duration Factor 1.25

Plate - LIVE Dead From To

TC V 40 14 0.0 9.9

BC V 0 20 0.0 9.9

TC V -40 -14 0.0 9.9

BC V 0 -20 0.0 9.9

Plus 8 Wind Load Case(s)

Plus 1 UBC LT Load Case(s)

Plates for each ply each face.

ATL CONNECTOR PLATES

TO BE MANUFACTURED BY

ROBBINS ENGINEERING, INC.

Plate - LOCK 20 Ga, Gross Area

Plate - RHS 20 Ga, Gross Area

LT Type Plt Size X Y JSI

A LOCK 2.0x 4.0 Ctr 0.83

D LOCK 2.0x 4.0 Ctr 0.37

E LOCK 2.0x 4.0 Ctr 0.39

For proper installation of

toe-nails, refer to the 2001

National Design Specification

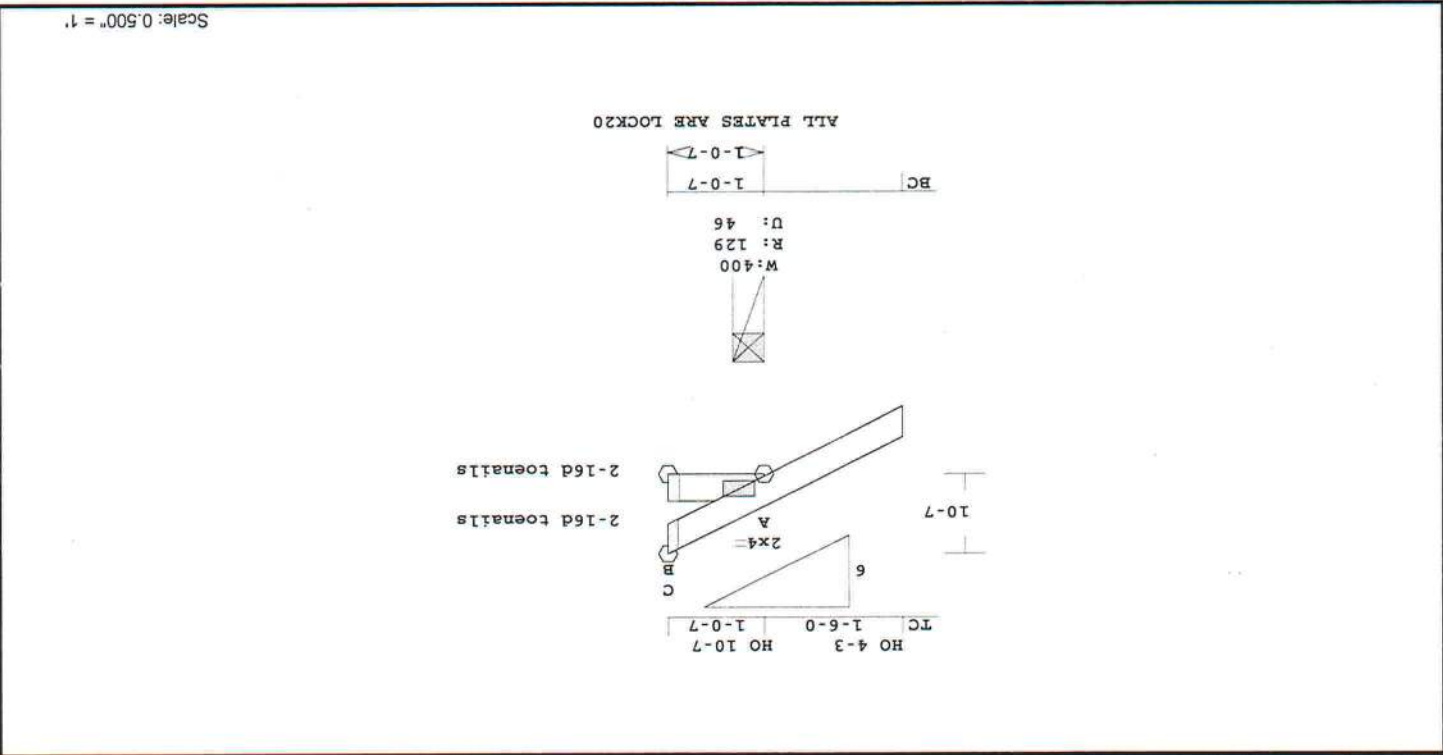
(NDS) for Wood Construction

Robbins Engineering, Inc./Online Plus™

Version 18.0.014 Engineering - Portal

11/15/2005 8:06:58 AM Page 1

Job	051062	Mark	R4	Quan	16	Type	JACK1	Span	10007	Pl-H1	6	Left OH	1-6-0	Right OH	0	Engineering
SR 441 - LOT 1 LAKECITY																



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 7.6 LBS

-----Bottom Chords-----
A - C 0.01 10 T 0.00 0.01

Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 21 lbs
Quality Control Factor 1.25

FABRICATOR NOTES:

Plates for each ply each face.

1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates,
Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.

Brace truss as follows:

CSI -Size- ---Lumber---
TC 0.01 2x 4 SP-#2
BC 0.01 2x 4 SP-#2

Online Plus -- Version 18.0.014
RUN DATE: 14-NOV-05

O.C.
From To
TC Cont. 0-0-0 1-0-7
BC Cont. 0-0-0 1-0-7

Loading Live Dead (psf)
TC 20.0 7.0
BC 0.0 10.0
Total 20.0 17.0 37.0

Spacing
Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Pb=1.15 Fc=1.10 Ft=1.10
BC Pb=1.10 Fc=1.10 Ft=1.10

Plus 8 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

rt React Uplift Size Req'd
lbs In-Sx In-Sx
A 129 47 3-8 1-0
B 19 16 1-8 1-0
C 14 2 1-8 1-0
Hz = 20
Hz = 11

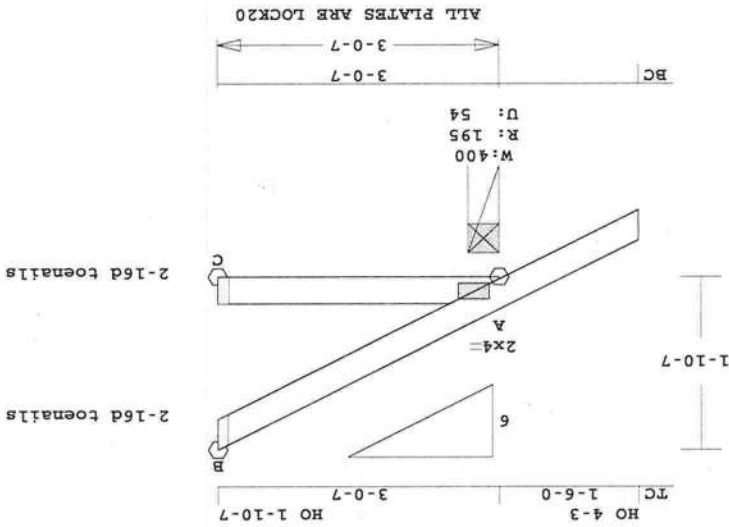
Membr CSI F lbs Axl-CSI-Bnd
-----Top Chords-----
A - B 0.01 21 C 0.00 0.01

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-
concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B

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Job	051062	Mark	R5	Quan	16	Type	JACK1	Span	30007	PI-H1	6	Left OH	1-6-0	Right OH	0	Engineering
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SR 441 - LOT 1 LAKECITY



Scale: 0.500" = 1'

Robbins Engineering, Inc./Online Plus[™] APPROX. TRUSS WEIGHT: 15.6 LBS

-----Bottom Chords-----
 A - C 0.10 35 T 0.00 0.10
 Occupancy Factor : 1.00
 Building Type: Enclosed
 TC Dead Load: 4.0 psf
 BC Dead Load: 6.0 psf
 Max comp. force 90 lbs
 Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss Designer)
 Gary Dounson, PR 35054
 Gary Dounson & Associates,
 Inc.
 2830 NW 41st Street Suite D
 Gainesville, FL 32606
 (352)375-8593
 CA 5201

Plates for each ply each face.

ALL CONNECTOR PLATES
 TO BE MANUFACTURED BY
 ROBBINS ENGINEERING, INC.
 Plate - LOCK 20 ga, Gross Area
 Plate - RHS 20 ga, Gross Area
 Ut Type Plt Size X Y JSI
 A LOCK 2.0x 4.0 Ctr Ctr 0.65

Brace truss as follows:

O.C. From To
 TC Cont. 0-0-0 3-0-7
 BC Cont. 0-0-0 3-0-7

Loading Live Dead (psf)

TC 20.0 7.0
 BC 0.0 10.0
 Total 20.0 17.0
 Spacing 24.0"

Lumber Duration Factor 1.25
 Plate Duration Factor 1.25
 TC Pb=1.15 Fc=1.10 Ft=1.10
 BC Pb=1.10 Fc=1.10 Ft=1.10

Plus 8 Wind Load Case(s)
 Plus 1 UBC LL Load Case(s)

Ut React Uplift Size Req'd

Lbs In-Sx In-Sx
 A 195 54 4-0 1-0
 B 80 67 3-8 1-0
 C 55 0 3-8 1-0

Hz = 36

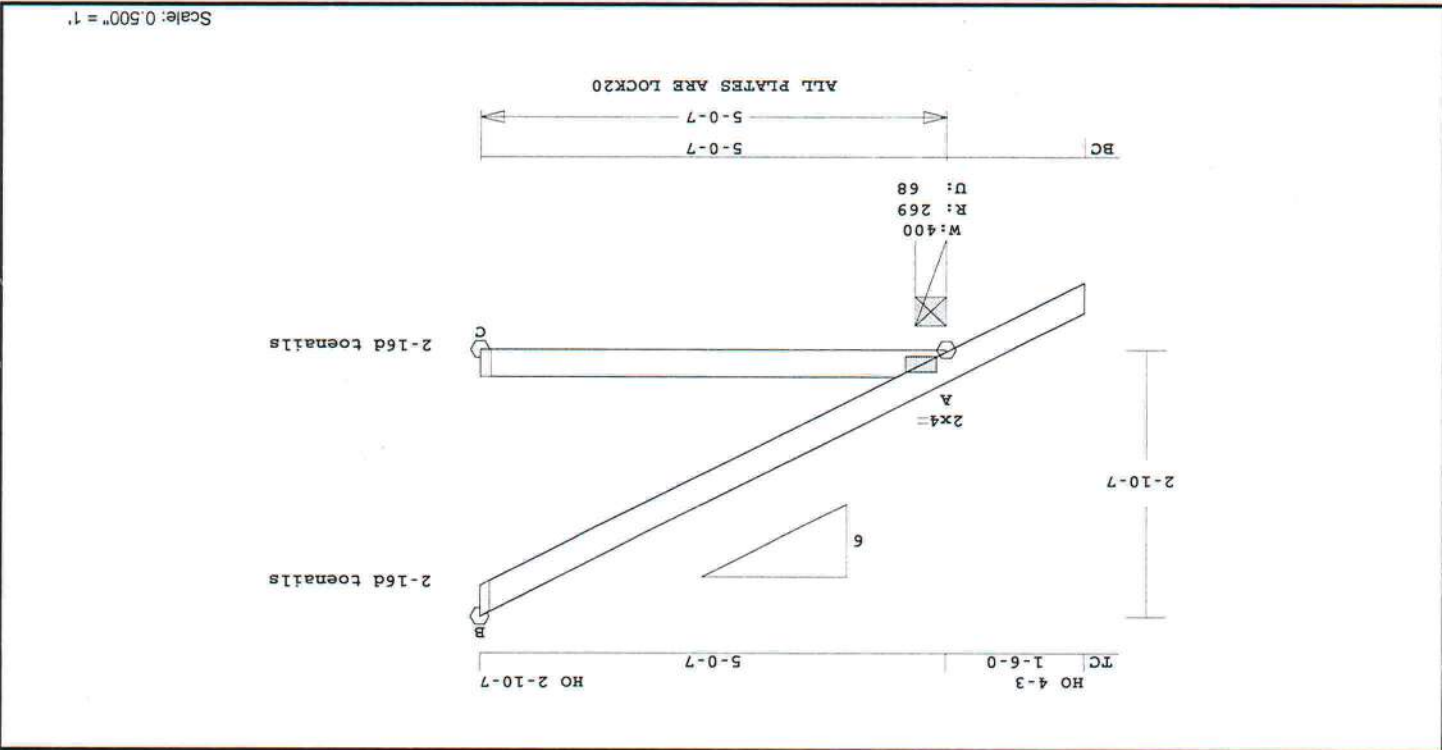
Member CSI F lbs Axl-CSI-Bnd
 -----Top Chords-----
 A-B 0.11 90 C 0.00 0.11

For proper installation of
 toe-nails, refer to the 2001
 National Design Specification
 (NDS) for Wood Construction

Trusses Manufactured by:
 RIDGWAY ROOF TRUSS
 Analysis Conforms To:
 FBC2004
 OH Loading
 Soffit psf 2.0
 Design checked for 10 psf non-concurrent LL on BC.
 Wind Loads - ANSI / ASCE 7-02
 Truss is designed as
 Components and Claddings*
 for Exterior zone location.
 Wind Speed: 110 mph
 Mean Roof Height: 25-0
 Exposure Category: B

Job	051062	Mark	R6	Quan	12	Type	JACK1	Span	50007	Pl-HI	6	Left OH	1 - 6 - 0	Right OH	0	Engineering
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SR 441 - LOT 1 LAKECITY



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 23.6 LBS

-----Bottom Chords-----
 A - C 0.31 59 T 0.00 0.31
 Occupancy Factor : 1.00
 Building Type: Enclosed
 TC Dead Load: 4.0 psf
 BC Dead Load: 6.0 psf
 Max comp. force 144 Lbs
 Quality Control Factor 1.25

CSI - Size - Lumber-----
 TC 0.36 2x 4 SP-#2
 BC 0.31 2x 4 SP-#2

Brace truss as follows:
 O.C. From To
 TC Cont. 0 - 0 - 0 5 - 0 - 7
 BC Cont. 0 - 0 - 0 5 - 0 - 7

Loading Live Dead (psf)
 TC 20.0 7.0
 BC 0.0 10.0
 Total 20.0 17.0
 Spacing 24.0"

Lumber Duration Factor 1.25
 Plate Duration Factor 1.25
 TC Fb=1.15 Fc=1.10 Ft=1.10
 BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 8 Wind Load Case(s)
 Plus 1 UBC TL Load Case(s)

UT React Uplift Size Req'd
 Lbs In-Sx In-Sx
 A 269 69 4- 0 1- 0
 Hz = 111
 B 129 107 3- 8 1- 0
 C 93 0 3- 8 1- 0
 Hz = 60

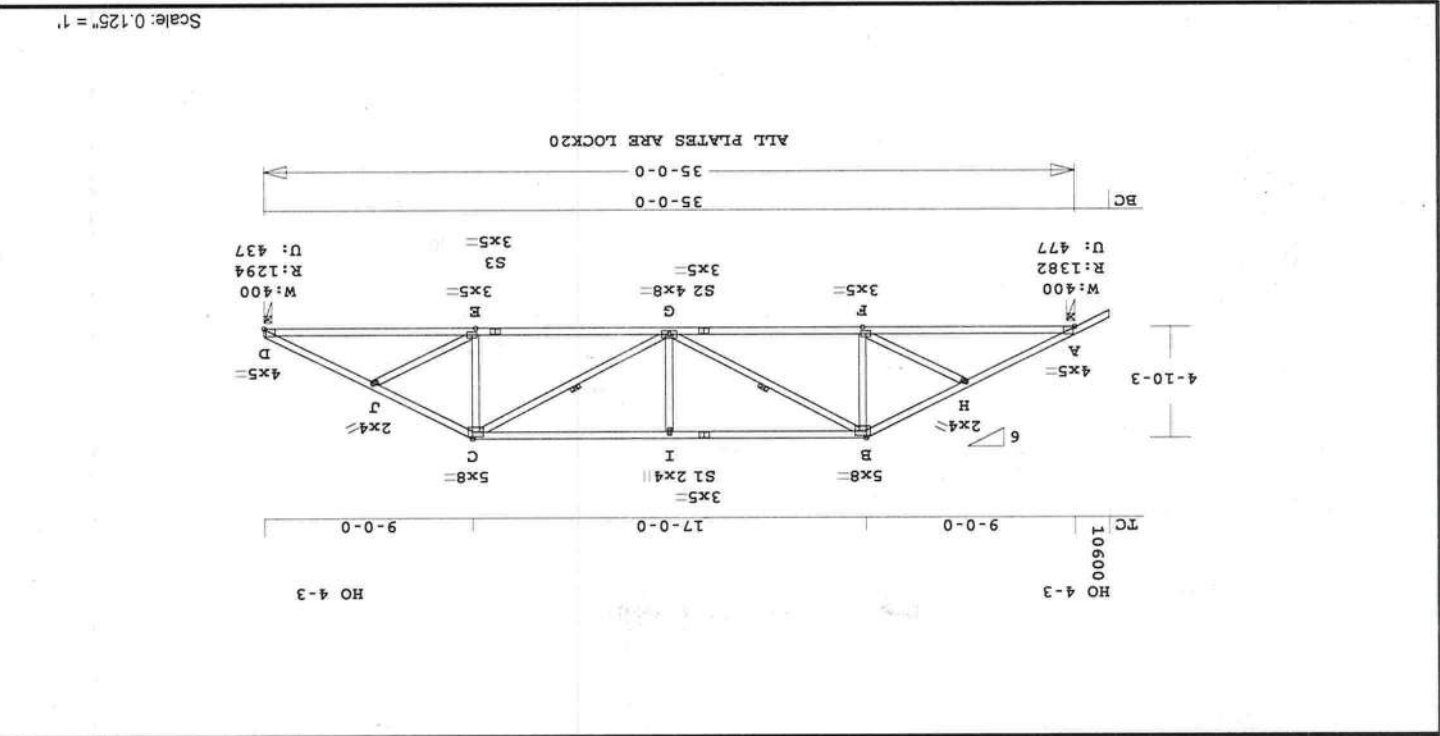
Member CSI F Lbs Ax1-CST-Bnd
 -----Top Chords-----
 A - B 0.36 144 C 0.00 0.36

NOTES:
 Trusses Manufactured by:
 RIDGWAY ROOF TRUSS
 Analysis Conforms To:
 FBC2004
 OH Loading
 Soffit psf 2.0
 Design checked for 10 psf non-concurrent LL on BC.
 Wind Loads - ANSI / ASCE 7-02
 Truss is designed as
 Components and Claddings*
 for Exterior zone location.
 Wind Speed: 110 mph
 Mean Roof Height: 25-0
 Exposure Category: B

FABRICATOR NOTES:
 1. Delegated Engineer (Truss Designer)
 Gary Dounson, PR 35054
 Gary Dounson & Associates, Inc.
 2830 NW 41st Street Suite D
 Gainesville, FL 32606
 (352)375-8593
 CA 5201

Job	051062	Mark	R7	Quan	1	Type	HIPP	Span	350000	Pl-HI	6	Left OH	1-6-0	Right OH	0	Engineering
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SR 441 - LOT 1 LAKECITY



Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 216.9 LBS

B-S1	0.79	2553	C	0.17	0.62	E	LOCK	3.0x	5.0	Ctr	0.51
S1-I	0.74	2553	C	0.04	0.70						
I-C	0.79	2553	C	0.17	0.62						
C-J	0.30	2136	C	0.02	0.28						
J-D	0.27	2358	C	0.14	0.13						
-----Bottom Chords-----											
A-F	0.60	2104	T	0.20	0.40						
F-S2	0.59	1917	T	0.19	0.40						
S2-G	0.52	1917	T	0.32	0.20						
G-S3	0.52	1917	T	0.20	0.40						
S3-E	0.59	1917	T	0.19	0.40						
E-D	0.60	2104	T	0.20	0.40						
-----Webs-----											
H-F	0.08	277	T								
F-B	0.10	383	T								
B-G	0.23	720	T								
G-I	0.16	512	C								
I-C	0.23	720	T								
C-G	0.23	720	T								
E-C	0.10	383	T								
E-J	0.08	277	T								
TL Defl -0.16" in S2-G L/999											
TL Defl -0.34" in G-S3 L/999											
Shear // Grain in S1-I 0.33											
Plates for each ply each face.											
ALL CONNECTOR PLATES TO BE MANUFACTURED BY ROBBINS ENGINEERING, INC.											
Plate - LOCK 20 Ga, Gross Area											
Plate - RHS 20 Ga, Gross Area											
Jt Type Plt Size X Y JSI											
A	LOCK	4.0x	5.0	Ctr	0.1	0.74					
H	LOCK	2.0x	4.0	Ctr	0.44						
B	LOCK	5.0x	8.0	Ctr	-0.1	0.93					
S1	LOCK	3.0x	5.0	Ctr	0.89						
I	LOCK	2.0x	4.0	Ctr	0.44						
C	LOCK	5.0x	8.0	Ctr	-0.1	0.93					
J	LOCK	2.0x	4.0	Ctr	0.44						
D	LOCK	4.0x	5.0	Ctr	0.1	0.74					
F	LOCK	3.0x	5.0	Ctr	0.51						
S2	LOCK	3.0x	5.0	Ctr	0.74						
G	LOCK	4.0x	8.0	Ctr	0.45						
S3	LOCK	3.0x	5.0	Ctr	0.91						

Online Plus -- Version 18.0.014
RUN DATE: 14-NOV-05

CSI -Size- ---Lumber---

TC	0.79	2x	4	SP-#2
BC	0.60	2x	4	SP-#2
WB	0.23	2x	4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0-0-0	35-0-0
BC Cont.	0-0-0	35-0-0

WB 1 rows CLB on B -G
WB 1 rows CLB on G -C
Attach CLB with (2)-10d nails
at each web.

Loading	Live	Dead	(psf)
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TC	20.0	7.0
BC	0.0	10.0
Total	20.0	17.0

Spacing
Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

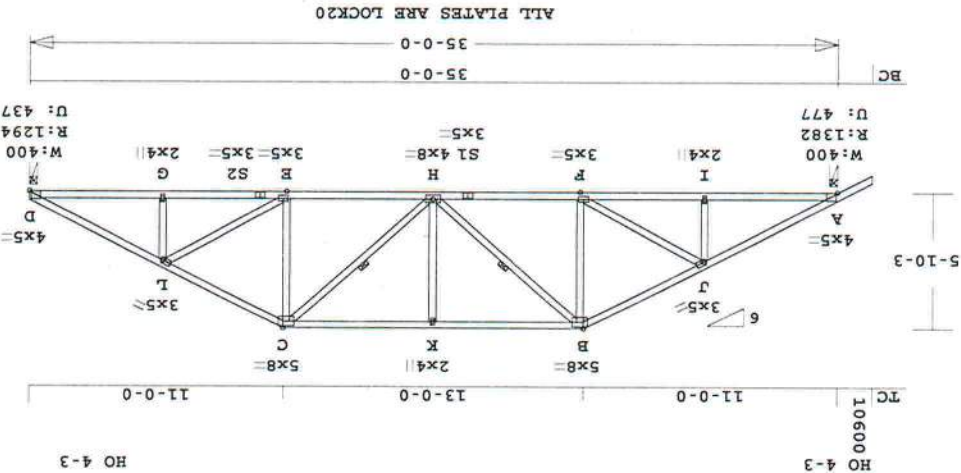
Jt	React	Uplift	Size	Req'd
A	1382	478	4-0	1-10

D	1295	438	4-0	1-8
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Membr	CSI	P	Lbs	AxI	CSI-Bnd
A-H	0.27	2358	C	0.14	0.13
H-B	0.30	2136	C	0.02	0.28

Job	051062	Mark	R8	Quan	1	Type	HIPP	Span	350000	Pl-HI	6	Left OH	1-6-0	Right OH	0	Engineering
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SR 441 - LOT 1 LAKECITY



Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 232.9 LBS

B - K	0.42	2057	C	0.02	0.40
K - C	0.42	2057	C	0.02	0.40
C - L	0.34	1963	C	0.12	0.22
L - D	0.36	2356	C	0.14	0.22
A - I	0.42	2105	T	0.35	0.07
I - F	0.45	2105	T	0.35	0.10
F - S1	0.39	1753	T	0.29	0.10
S1 - H	0.41	1753	T	0.29	0.12
H - E	0.41	1753	T	0.29	0.12
E - S2	0.45	2105	T	0.35	0.10
S2 - G	0.41	2105	T	0.35	0.06
G - D	0.42	2105	T	0.35	0.07

CSI - Size - Lumber -
 TC 0.42 2x 4 SP-#2
 BC 0.45 2x 4 SP-#2
 WB 0.21 2x 4 SP-#3
 Brace truss as follows:
 To From
 TC Cont. 0-0-0 35-0-0
 BC Cont. 0-0-0 35-0-0
 WB 1 rows CLB on B - H
 WB 1 rows CLB on H - C
 Attach CLB with (2)-10d nails
 at each web.

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	
Spacing			24.0"

Plus 9 Wind Load Case(s)
 Plus 1 UBC LL Load Case(s)
 Jt React Uplift Size Req'd
 Lbs In-Sx In-Sx
 1382 478 4-0 1-10
 1295 438 4-0 1-8
 Hz = -118
 Hz = 119
 Member CSI P Lbs Ax1-CSI-Bnd
 A - J 0.36 2356 C 0.14 0.22
 J - B 0.34 1964 C 0.12 0.22

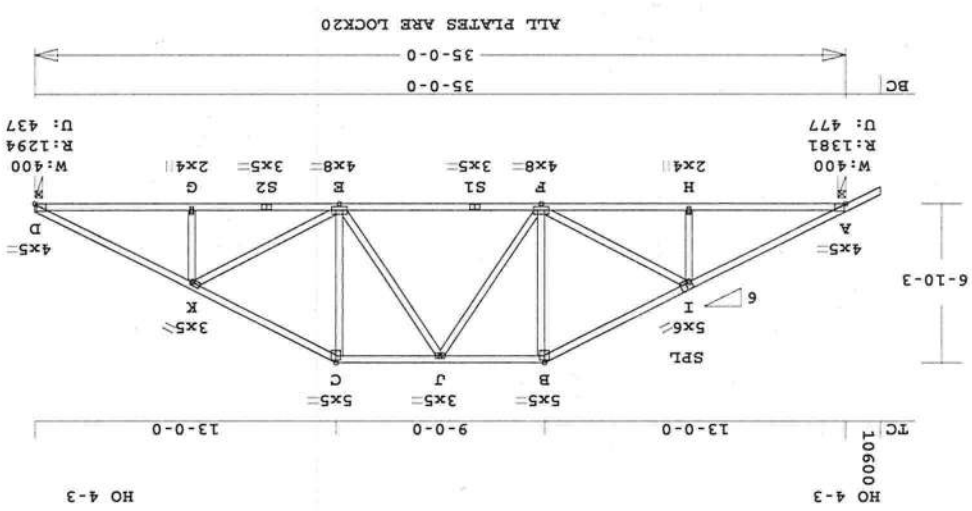
Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 232.9 LBS

B - K	0.42	2057	C	0.02	0.40
K - C	0.42	2057	C	0.02	0.40
C - L	0.34	1963	C	0.12	0.22
L - D	0.36	2356	C	0.14	0.22
A - I	0.42	2105	T	0.35	0.07
I - F	0.45	2105	T	0.35	0.10
F - S1	0.39	1753	T	0.29	0.10
S1 - H	0.41	1753	T	0.29	0.12
H - E	0.41	1753	T	0.29	0.12
E - S2	0.45	2105	T	0.35	0.10
S2 - G	0.41	2105	T	0.35	0.06
G - D	0.42	2105	T	0.35	0.07

Notes:
 Trusses Manufactured by:
 RIDGWAY ROOF TRUSS
 Analysis Conforms To:
 FBC2004
 OH Loading
 Soffit psf 2.0
 Design checked for 10 psf non-concurrent LL on BC.
 Wind Loads - ANSI / ASCE 7-02
 Truss is designed as
 Components and Claddings*
 for Exterior zone location.
 Wind Speed:
 110 mph
 Mean Roof Height: 25-0
 Exposure Category: B
 Occupancy Factor: 1.00
 Building Type: Enclosed
 TC Dead Load:
 4.0 psf
 BC Dead Load:
 6.0 psf
 Max comp. force
 2356 lbs
 Quality Control Factor 1.25
 FABRICATOR NOTES:
 1. Delegated Engineer (Truss Designer)
 Gary Dounson, PE 35054
 Gary Dounson & Associates,
 Inc.
 2830 NW 41st Street Suite D
 Gainesville, FL 32606
 (352)375-8593
 CA 5201

Job	051062	Mark	R9	Quan	1	Type	HIPP	Span	350000	Pl-H1	6	Left OH	1-6-0	Right OH	0	Engineering
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SR 441 - LOT 1 LAKECITY



Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 231.9 LBS

Online Plus -- Version 18.0.014

RUN DATE: 14-NOV-05

CSI - Size - Lumber

TC 0.47 2x 4 SP-#2
BC 0.55 2x 4 SP-#2
WB 0.38 2x 4 SP-#3

Brace truss as follows:

O.C. From To
TC Cont. 0-0-0 35-0-0
BC Cont. 0-0-0 35-0-0

Loading Live Dead (psf)

TC 20.0 7.0
BC 0.0 10.0
Total 20.0 17.0

Spacing 24.0"
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

TC FB=1.15 FC=1.10 Ft=1.10
BC FB=1.10 FC=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Ut React Uplift Size Req'd
Lbs In-Sx In-Sx
A 1382 478 4-0 1-10
D 1295 438 4-0 1-8

Membr CSI P Lbs Axl-CSI-Bnd
-----Top Chords-----
A -I 0.47 2315 C 0.14 0.33
I -B 0.44 1813 C 0.11 0.33
B -J 0.24 1617 C 0.12 0.12
J -C 0.24 1617 C 0.12 0.12
C -K 0.44 1813 C 0.11 0.33

K -D 0.47 2315 C 0.14 0.33	Bottom Chords
A -H 0.45 2074 T 0.34 0.11	
H -F 0.55 2074 T 0.21 0.34	
F -S1 0.51 1699 T 0.17 0.34	
S1 -E 0.51 1699 T 0.17 0.34	
E -S2 0.55 2074 T 0.21 0.34	
S2 -G 0.42 2074 T 0.34 0.08	
G -D 0.45 2074 T 0.34 0.11	
-----Webs-----	
H -I 0.06 237 T	
I -F 0.38 518 C	
F -B 0.16 529 T	
B -J 0.13 148 C	
J -E 0.13 148 C	
E -C 0.16 529 T	
C -K 0.38 518 C	
K -G 0.06 237 T	
LT Defl -0.14" in SI-E I/999	
TL Defl -0.33" in SI-E I/999	
Shear // Grain in A -I 0.21	
Plates for each ply each face.	
ALU CONNECTOR PLATES	
TO BE MANUFACTURED BY	
ROBBINS ENGINEERING, INC.	
Plate - LOCK 20 Ga, Gross Area	
Plate - RHS 20 Ga, Gross Area	
Jt Type Plt Size X Y JSI	
A LOCK 4.0x 5.0 Ctr 0.1 0.74	
I LOCK 5.0x 6.0 Ctr 0.3-0.7 0.63	
B LOCK 5.0x 5.0 Ctr 0.2 0.99	
J LOCK 3.0x 5.0 Ctr 0.2 0.99	
C LOCK 5.0x 5.0 Ctr 0.2 0.99	
K LOCK 3.0x 5.0 Ctr 0.57	
D LOCK 4.0x 5.0 Ctr 0.1 0.74	
H LOCK 2.0x 4.0 Ctr 0.44	
F LOCK 4.0x 8.0 Ctr 0.45	
S1 LOCK 3.0x 5.0 Ctr 0.84	
E LOCK 4.0x 8.0 Ctr 0.45	
G LOCK 2.0x 4.0 Ctr 0.74	

FABRICATOR NOTES:

1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D Gainesville, FL 32606
(352) 375-8593
CA 5201

Truss is designed as Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor: 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 2315 Lbs
Quality Control Factor 1.25

Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02

NOTES:

Trusses Manufactured by: RIDGWAY ROOF TRUSS

Analysis Conforms To: FBC2004

OH Loading

Soffit psf 2.0

Design checked for 10 psf non-concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as Components and Claddings* for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 25'-0

Exposure Category: B

Occupancy Factor : 1.00

Building Type: Enclosed

TC Dead Load: 4.0 psf

BC Dead Load: 6.0 psf

Max comp. force 2272 lbs

Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss Designer)

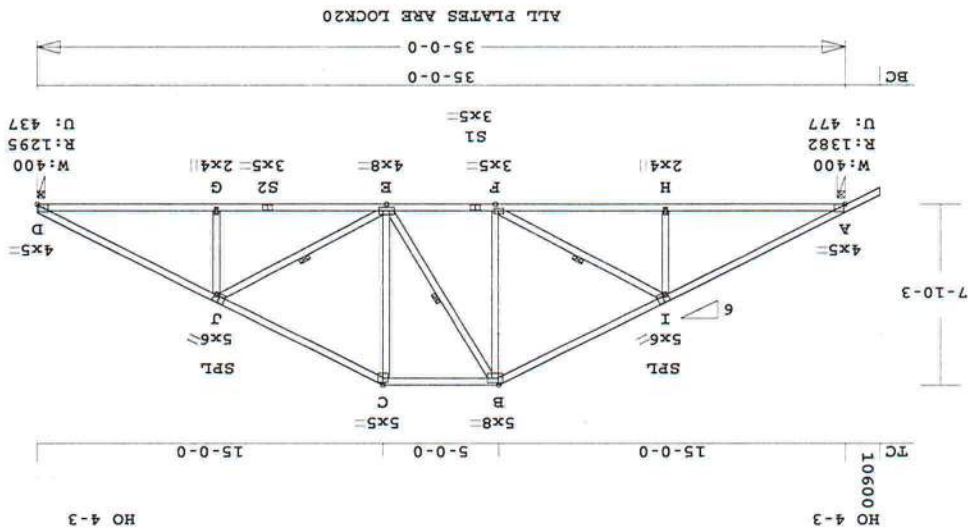
Gary Dounson, PE 35054

Gary Dounson & Associates, Inc.

2830 NW 41st Street Suite D Gainesville, FL 32606

(352)375-8593

CA 5201



Scale: 0.125" = 1'

Job	051062	Mark	R10	Quan	1	Type	HIPP	Span	350000	6	P1-H1	Left OH	1-6-0	Right OH	0	Engineering
SR 441 - LOT 1 LAKECITY																

	Plus	9 Wind Load Case(s)
	Plus	1 UBC LL Load Case(s)
UT	React	Uplift
A	Lbs	In-Sx
C	HZ	= -384
		4- 0 1- 1
		HZ = -140
Member	CSI	P Lbs Axl-CSI-Bnd
-----Top Chords-----		
A-I	0.25	2270 C 0.06 0.19

	Dead	Live	Dead	Live	Dead	Live
TC	7.0	20.0	0.0	10.0	17.0	37.0
BC	7.0	20.0	0.0	10.0	17.0	37.0
Spacing						
Lumber Duration Factor	1.25					
Plate Duration Factor	1.25					
TC Fb=1.15 Fc=1.10 Ft=1.10						
BC Fb=1.10 Fc=1.10 Ft=1.10						

TC Cont.
0-0-0 0-0-0 28-11-11
BC Cont.
0-0-0 0-0-0 28-11-11
WB 1 rows CLB on J-M
WB 1 rows CLB on M-K
Attach CLB with (2) -10d nails
at each web.

TC	0.43	2x 4	SP-#2
BC	0.45	2x 4	SP-#2
WB	0.28	2x 4	SP-#3

Online Plus -- Version 18.0.014
RUN DATE: 14-NOV-05

plates for each ply each face.

TO BE MANUFACTURED BY
ROBINS ENGINEERING, INC.
plate - LOCK 20 ga, Gross Area
plate - RHS 20 ga, Gross Area

ut Type Plt Size X Y JSI
A LOCK 4.0x 5.0 Ctr 0.14 0.74
I LOCK 3.0x 5.0 Ctr 0.17 0.57
J LOCK 5.0x 6.0 0.3-0.7 0.63
K LOCK 5.0x 8.0 Ctr 1.6 0.85
O LOCK 5.0x 6.0-0.3-0.7 0.63
L LOCK 3.0x 5.0 Ctr 0.17 0.57
C LOCK 4.0x 5.0 Ctr 0.1 0.74

LT Defl - 0.11" in SI-M	TL Defl - 0.23" in G-SI	Shear // Grain in L-C
666 L/999	666 L/999	0.31

1	BR	I	H	0.05	215	T
1	BR	I	G	0.24	428	C
		J	G	0.10	385	T
		J	M	0.17	609	C
		O	M	0.28	899	T
		K	M	0.14	511	C
		K	E	0.09	299	T
		L	E	0.14	571	T
		L	D	0.02	78	T

	Webs	
A-H	0.42	2030 T
H-G	0.43	2030 T
G-S1	0.36	1655 T
S1-M	0.38	1655 T
M-E	0.38	1585 T
E-S2	0.40	1702 T
S2-D	0.40	1702 T
D-C	0.45	1702 T
	0.17	0.28

	I	J	-J	0	-0	K	-K	L	-L	T	-T	C	-C	0.43	0.36	0.33	0.27	0.26	0.37
Bottom Chords	1848	1848	C	0.01	0.01	1365	C	0.01	0.01	1365	C	0.01	0.01	1907	C	0.06	0.06	0.36	0.37

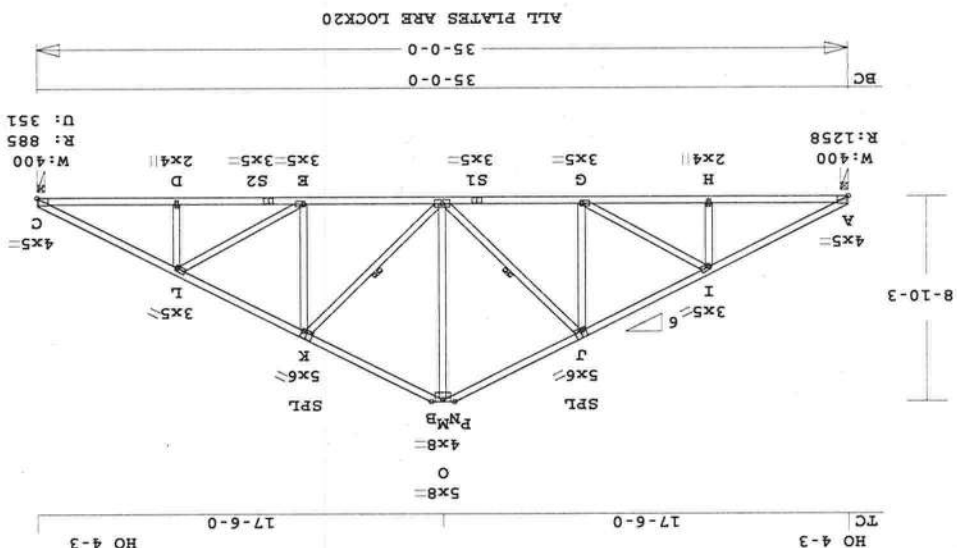
Robbins Engineering, Inc./Online Plus[®] APPROX. TROSS WEIGHT: 240.2 LBS

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite I Gainesville, FL 32606
(352) 375-8593
CA 5201

provide connection to bearing for 385 lbs Horiz Reaction Design checked for 10 psf non-concurrent LL on BC. Wind Loads - - ANSI / ASCE 7-02 Truss is designed as Components and Claddings* for Exterior zone location. Wind Speed: 110 mph Mean Roof Height: 25-0 Exposure Category: B Occupancy Factor : 1.00 Building Type: Enclosed TC Dead load: 4.0 psf BC Dead load: 6.0 psf Max comp. force 2270 lbs Quality Control Factor 1.25

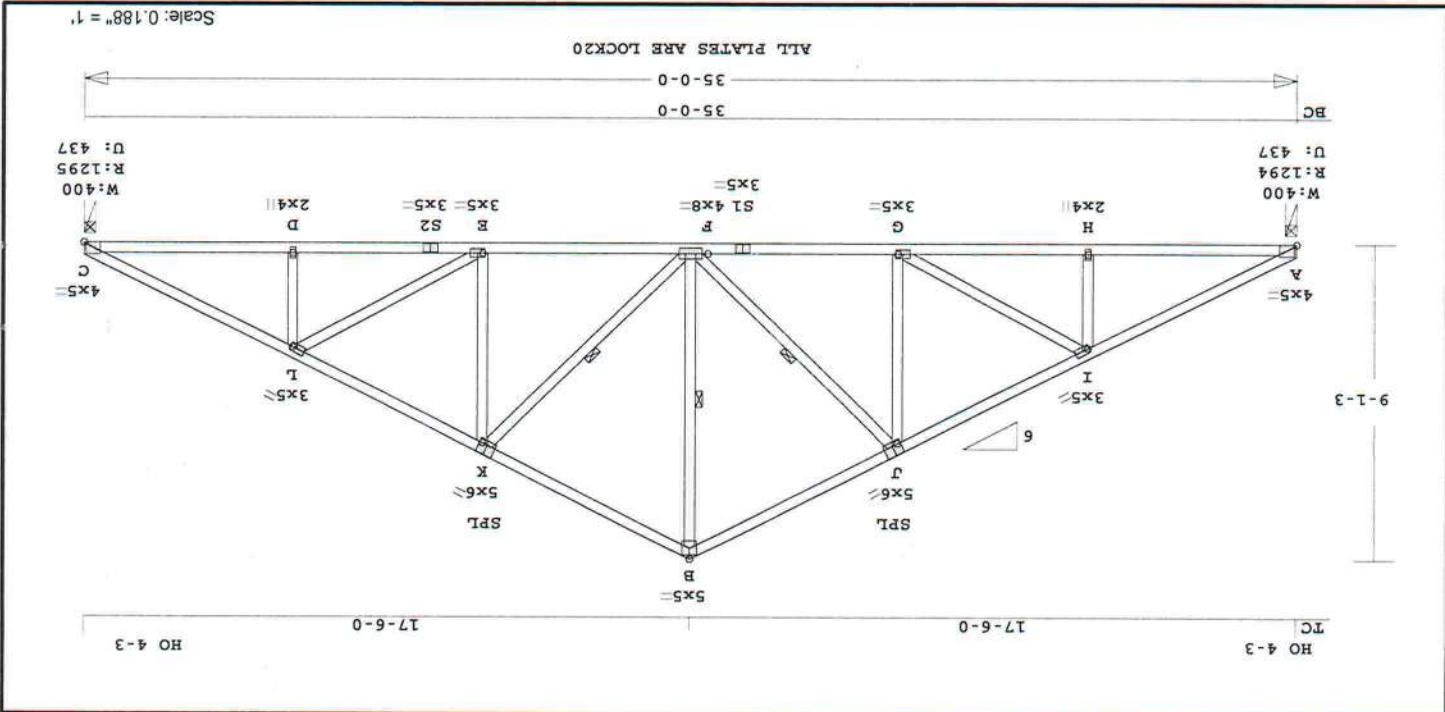
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004

	H	S1	M	E	S2	D
LQCK	2.0x	4.0	4.0	4.0	4.0	2.0x
LQCK	3.0x	5.0	5.0	5.0	5.0	3.0x
LQCK	3.0x	5.0	8.0	5.0	5.0	3.0x
LQCK	3.0x	5.0	5.0	5.0	5.0	3.0x
LQCK	2.0x	4.0	4.0	4.0	4.0	2.0x



Job	051062	Mark	R11	Quan	1	Type	TR	Span	350000	P1-H1	6	Left OH	0	Right OH	0	Engineering
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Job	051062	Mark	R12	Quan	6	Type	TR	Span	350000	Pl-H1	6	Left OH	0	Right OH	0	Engineering
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U-B	0.38	1444	C	0.11	0.27	F	LOCK	4.0x	8.0	Ctr	Ctr	0.42
B-K	0.38	1444	C	0.11	0.27	E	LOCK	3.0x	5.0	Ctr	Ctr	0.51
K-L	0.40	1929	C	0.13	0.27	S2	LOCK	3.0x	5.0	Ctr	Ctr	0.74
L-C	0.37	2348	C	0.15	0.22	D	LOCK	2.0x	4.0	Ctr	Ctr	0.44

Robbins Engineering, Inc./Online Plus" APPROX. TRUSS WEIGHT: 240.6 LBS

Online Plus -- Version 18.0.014
RUN DATE: 14-NOV-05

CSI -Size- ---Lumber---

TC	0.40	2x 4	SP-#2
BC	0.44	2x 4	SP-#2
WB	0.31	2x 4	SP-#3

Brace truss as follows:

TC	Cont.	0-0-0	35-0-0
BC	Cont.	0-0-0	35-0-0

WB	1 rows	CLB on J	F
WB	1 rows	CLB on F	B
WB	1 rows	CLB on F	K

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	

Spacing	24.0"
Lumber Duration Factor	1.25
Plate Duration Factor	1.25

Plus 9 Wind Load Case(s)	Plus 1 UBC LL Load Case(s)
UT React Uplift Size Req'd	Lbs In-Sx

A	1295	438	4-0	1-8	HZ =	-190
C	1295	438	4-0	1-8	HZ =	191

Membr	CSI	P	Lbs	Ax1-CSI-Bnd
-----Top Chords-----				

A-I	0.37	2348	C	0.15	0.22
I-J	0.40	1929	C	0.13	0.27

ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
UT Type Plt Size X Y JSI

Plates for each ply each face.

UT React Uplift Size Req'd
Lbs In-Sx
A 1295 438 4-0 1-8
H Z = -190

Membr CSI P Lbs Ax1-CSI-Bnd
-----Top Chords-----
A-I 0.37 2348 C 0.15 0.22
I-J 0.40 1929 C 0.13 0.27

Robbins Engineering, Inc./Online Plus" APPROX. TRUSS WEIGHT: 240.6 LBS

Run Date: 14-Nov-05

CSI -Size- ---Lumber---

TC	0.40	2x 4	SP-#2
BC	0.44	2x 4	SP-#2
WB	0.31	2x 4	SP-#3

Brace truss as follows:

TC	Cont.	0-0-0	35-0-0
BC	Cont.	0-0-0	35-0-0

WB 1 rows CLB on J F
WB 1 rows CLB on F B
WB 1 rows CLB on F K
Attach CLB with (2)-10d nails
at each web.

Loading Live Dead (psf)
TC 20.0 7.0
BC 0.0 10.0
Spacing 24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Pb=1.15 Fc=1.10 Ft=1.10
BC Pb=1.10 Fc=1.10 Ft=1.10
Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)
UT React Uplift Size Req'd
Lbs In-Sx
A 1295 438 4-0 1-8
H Z = -190
C 1295 438 4-0 1-8
H Z = 191
Membr CSI P Lbs Ax1-CSI-Bnd
-----Top Chords-----
A-I 0.37 2348 C 0.15 0.22
I-J 0.40 1929 C 0.13 0.27

UT React Uplift Size Req'd
Lbs In-Sx
A 1295 438 4-0 1-8
H Z = -190

Membr CSI P Lbs Ax1-CSI-Bnd
-----Top Chords-----
A-I 0.37 2348 C 0.15 0.22
I-J 0.40 1929 C 0.13 0.27

Robbins Engineering, Inc./Online Plus" APPROX. TRUSS WEIGHT: 240.6 LBS

Run Date: 14-Nov-05

CSI -Size- ---Lumber---

TC	0.40	2x 4	SP-#2
BC	0.44	2x 4	SP-#2
WB	0.31	2x 4	SP-#3

Brace truss as follows:

TC	Cont.	0-0-0	35-0-0
BC	Cont.	0-0-0	35-0-0

WB 1 rows CLB on J F
WB 1 rows CLB on F B
WB 1 rows CLB on F K
Attach CLB with (2)-10d nails
at each web.

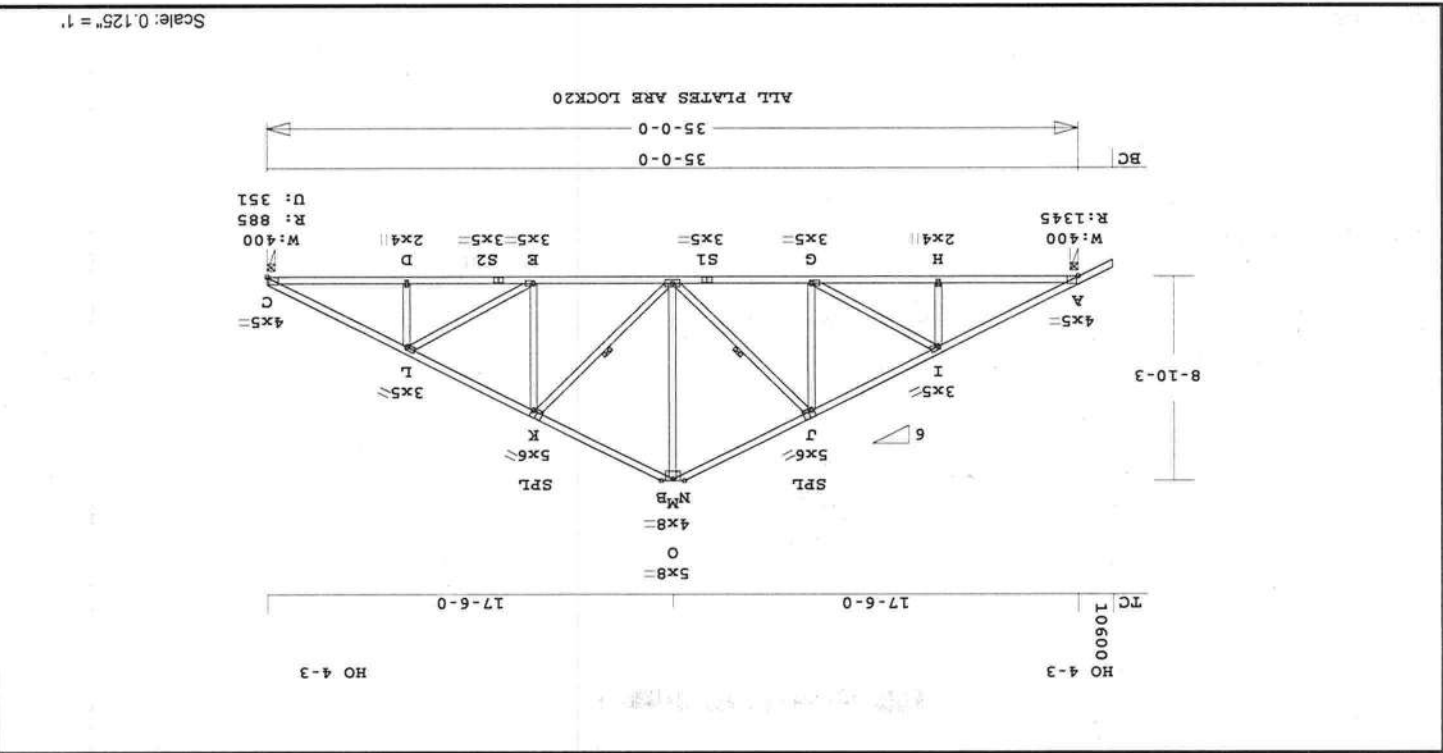
Loading Live Dead (psf)
TC 20.0 7.0
BC 0.0 10.0
Spacing 24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Pb=1.15 Fc=1.10 Ft=1.10
BC Pb=1.10 Fc=1.10 Ft=1.10
Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)
UT React Uplift Size Req'd
Lbs In-Sx
A 1295 438 4-0 1-8
H Z = -190
C 1295 438 4-0 1-8
H Z = 191
Membr CSI P Lbs Ax1-CSI-Bnd
-----Top Chords-----
A-I 0.37 2348 C 0.15 0.22
I-J 0.40 1929 C 0.13 0.27

UT React Uplift Size Req'd
Lbs In-Sx
A 1295 438 4-0 1-8
H Z = -190
C 1295 438 4-0 1-8
H Z = 191
Membr CSI P Lbs Ax1-CSI-Bnd
-----Top Chords-----
A-I 0.37 2348 C 0.15 0.22
I-J 0.40 1929 C 0.13 0.27

Job	051062	Mark	R13	Quan	1	Type	TR	Span	350000	Pl-HI	6	Left OH	1-6-0	Right OH	0	Engineering
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SR 441 - LOT 1 LAKECITY



Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 243.4 LBS

Online Plus -- Version 18.0.014 K - L 0.36 1760 C 0.00 0.36
L - C 0.43 1907 C 0.06 0.37
J - O 0.27 1365 C 0.01 0.26
O - K 0.33 1367 C 0.00 0.33
M - LOCK 4.0x 8.0 Ctr 0.42
E - LOCK 3.0x 5.0 Ctr 0.51
S2 - LOCK 3.0x 5.0 Ctr 0.74
D - LOCK 2.0x 4.0 Ctr 0.44

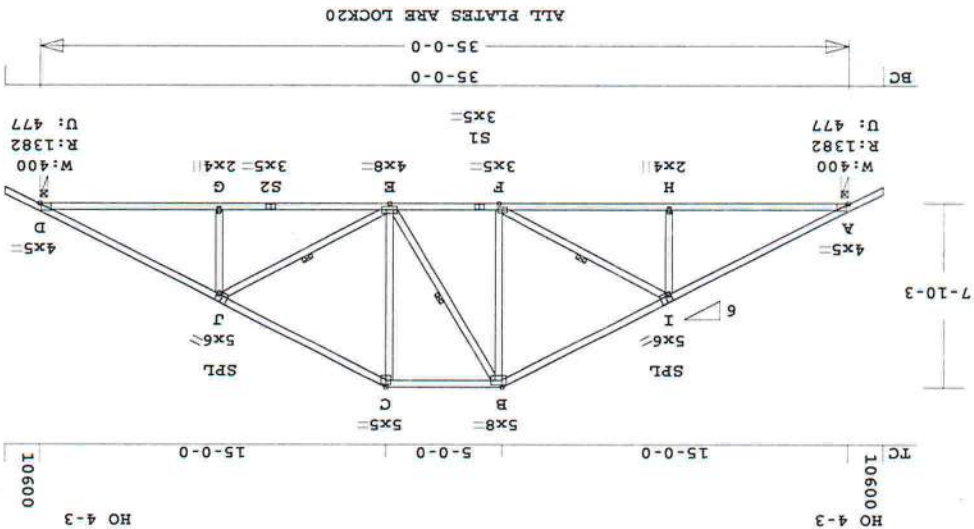
CS1 - Size - Lumber
TC 0.43 SP-#2
BC 0.45 2x 4 SP-#2
WB 0.28 2x 4 SP-#3
Brace truss as follows:
O.C. From To
TC Cont. 0-0-0 28-11-11
BC Cont. 0-0-0 28-11-11
WB 1 rows CLB on J - M
WB 1 rows CLB on M - K
Attach CLB with (2)-10d nails
at each web.
Loading Live Dead (psf)
TC 20.0
BC 0.0
Total 20.0
Spacing 24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Pb=1.15 Fc=1.10 Ft=1.10
BC Pb=1.10 Fc=1.10 Ft=1.10
Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)
JT React Uplift Size Req'd
Lbs In-Sx In-Sx
A 1345 0 4-0 1-9
C 886 352 4-0 1-1
Hz = -384
I - J 0.27 1848 C 0.01 0.26
G - LOCK 3.0x 5.0 Ctr 0.57
K - LOCK 5.0x 6.0-0.3-0.7 0.63
O - LOCK 5.0x 8.0 Ctr-1.6 0.85
J - LOCK 5.0x 6.0 0.3-0.7 0.63
I - LOCK 3.0x 5.0 Ctr 0.57
A - LOCK 4.0x 5.0 Ctr 0.1 0.74
JT Type Plt Size X JSI
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plates for each ply each face.
ALL CONNECTOR PLATES
SHEAR // Grain in L - C 0.31
TL Defl -0.23" in G - S1 L/999
LT Defl -0.11" in S1-M L/999

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates,
Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352) 375-8593
CA 5201

Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor: 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 2270 Lbs
Quality Control Factor 1.25

Job	Mark	Quan	Type	Span	Pl-HI	Left OH	Right OH	Engineering
051062	R14	1	HIPP	350000	6	1-6-0	1-6-0	



A-I	0.59	C	0.14	0.45	S2 LOCK	3.0x	5.0	CTR	CTR	0.74
I-B	0.56	C	0.11	0.45	G LOCK	2.0x	4.0	CTR	CTR	0.44

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 233.4 LBS

Online Plus -- Version 18.0.014
 RUN DATE: 14-NOV-05
 B - C 0.27 1467 C 0.11 0.16
 C - J 0.56 1649 C 0.11 0.45
 J - D 0.59 2272 C 0.14 0.45

-----Bottom Chords-----

CSI - Size - Lumber

Trusses Manufactured by:

NOTES:

CS1	Size	Bottom Chords	Trusses Manufactured by:
TC	2x 4	A-H 0.53	RIDGWAY ROOF TRUSS
BC	2x 4	H-F 0.53	Analysis Conforms To:
WB	2x 4	F-S1 0.34	FBC2004

Brace truss as follows:		From	To
S1-E	0.35	1462 T	0.24
E-S2	0.45	2039 T	0.34
S2-G	0.52	2039 T	0.34
OH Loading			0.11
Soffit psf 2.0			0.11
Design checked for 10 psf non-			0.18

MB 1 rows	CLB on I - F	H - I	0.08	317 T	Truss is designed as
BC Cont.	0 - 0 - 0 35 - 0 - 0				Wind Loads - ANSI / ASCE 7-02
TC Cont.	0 - 0 - 0 35 - 0 - 0	G - D	0.52	2039 T	concurrent LT on BC.
				0.34 0.18	

WB 1 rows CLB on B - E	I - F	0.19	646 C	1 Br	Components and Claddings*
WB 1 rows CLB on E - J	F - B	0.13	447 T		for Exterior zone location,
Attach CLB with (2) -10d nails	B - E	0.03	115 T	1 Br	Wind Speed: 110 mph

at each web.		Loading		Live		Dead (psf)	
E-C	0.14	444	T	E-J	0.19	647	C
G-J	0.08	316	T	E-J	0.19	647	C
				I Br			
Mean Roof Height: 25-0				Exposure Category: B			
				Occupancy Factor: 1.00			

TC	20.0	7.0				
BC	0.0	10.0	LT Defl -0.11" in SI-E	L/999	TC Dead Load:	4.0 psf
Total	20.0	17.0	TL Defl -0.24" in H -R	L/999	BC Dead Load:	6.0 psf

Spacing	24.0"	Shear // Grain in A - I	0.24	Max comp. force	2272 lbs
Lumber Duration Factor	1.25			Quality Control Factor	1.25
Plate Duration Factor	1.25	Plates for each ply each face.			

TC	Fb=1.15	Fc=1.10	Ft=1.10	ALU CONNECTOR PLATES	TO BE MANUFACTURED BY	1. Delegated Engineer (Truss Designer)
BC	Fb=1.10	Fc=1.10	Ft=1.10	ROBINS ENGINEERING, INC.		

Plus 9 Wind Load Case(s)	Plate - LOCK 20 Ga, Gross Area	Gary Dounson, PE 35054
Plus 1 UBC TL Load Case(s)	Plate - RHS 20 Ga, Gross Area	Gary Dounson & Associates, Inc.
	rt Type Plt Size X Y JSI	

ut	React Uplift	Size Req'd	Lbs In-Sx	Lbs In-Sx
A	LOCK	4.0x	5.0	0.1 0.74
I	LOCK	5.0x	6.0	0.3-0.7 0.63
B	LOCK	5.0x	8.0	0.1 0.93

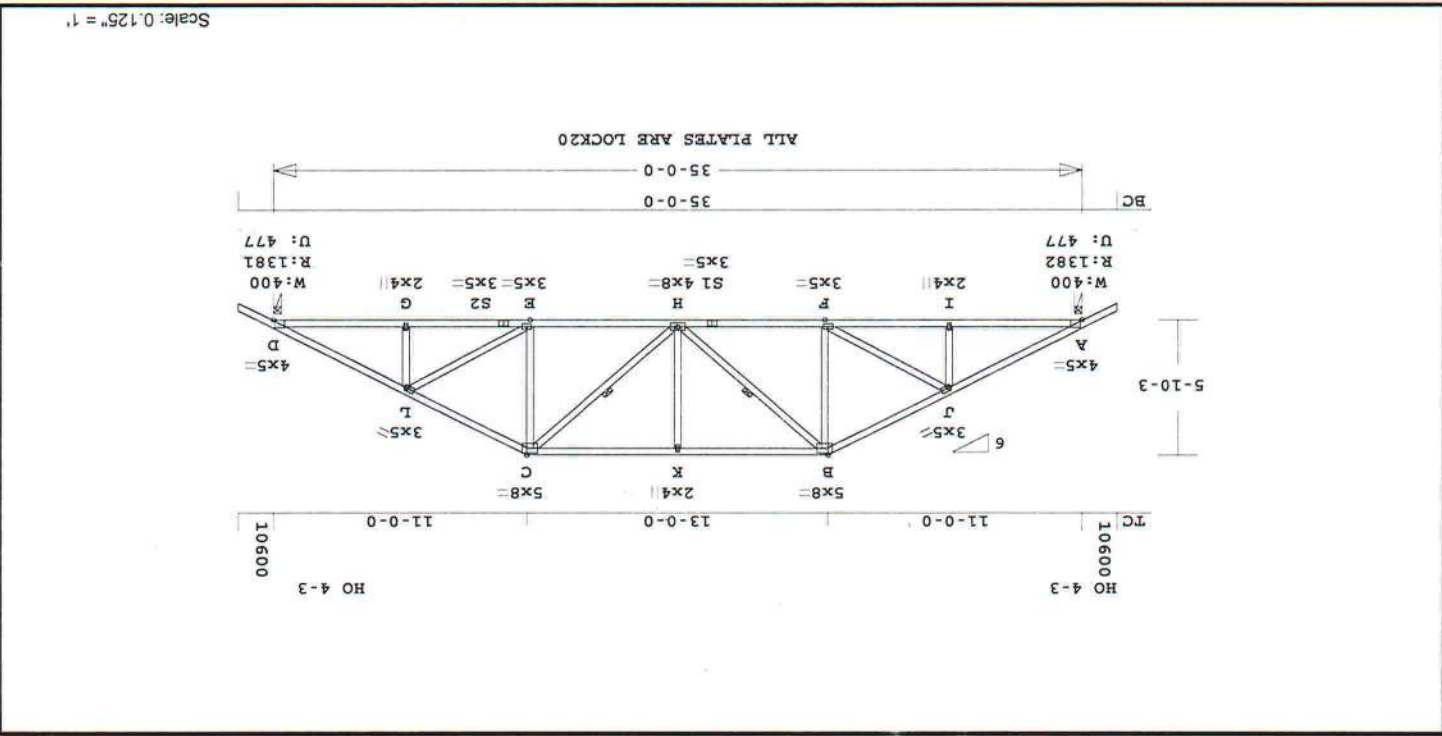
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352) 375-8593

Case	Frequency	Phase	Amplitude	Time	Location	Depth	Width	Height	Volume	Weight	Value	Cost	Benefit	Risk	Impact	Score
A	1382	478	4-0	1-10	C LOCK	5.0x	5.0	Ctrl-0.2	0.99							
			Hz =	-163	J LOCK	5.0x	6.0	-0.3-0.7	0.63							
D	1382	478	4-0	1-10	D LOCK	4.0x	5.0	Ctrl 0.1	0.74							

Member	CSI	P Lbs	Ax1-CSI-Bnd			
				Hz =	164	
			H LOCK	2.0x	4.0	Ctr Ctr 0.44
			F LOCK	3.0x	5.0	Ctr Ctr 0.51
			S1 LOCK	3.0x	5.0	Ctr Ctr 0.74

-----Top Chords----- E LOCK 4.0x 8.0 Ctr Ctr 0.45

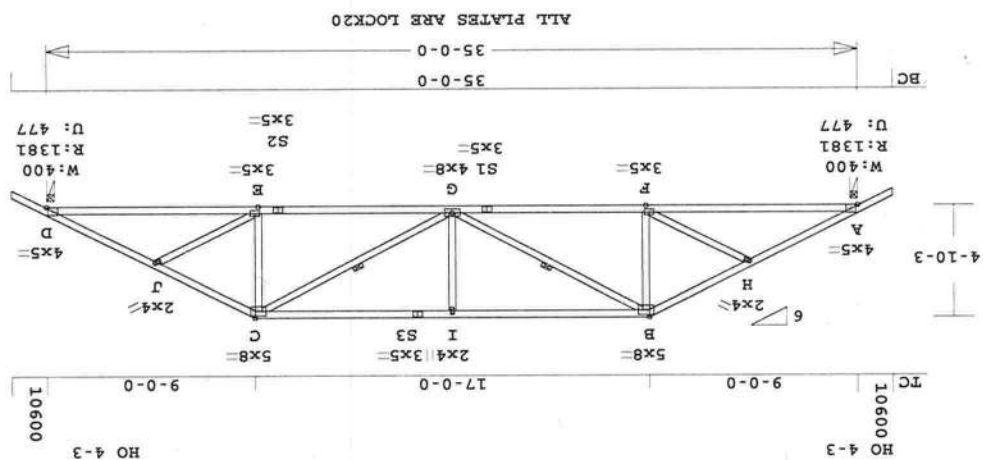
Job	051062	SR 441 - LOT 1 LAKECITY
Mark	R16	
Quan	1	
Type	HIPP	
Span	350000	
Pl-H1	6	
Left OH	1- 6- 0	
Right OH	1- 6- 0	
Engineering		



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 236.1 LBS

Online Plus -- Version 18.0.014
RUN DATE: 14-NOV-05
CSI - Size - Lumber -
TC 0.42 2x 4 SP-#2
BC 0.45 2x 4 SP-#2
WB 0.21 2x 4 SP-#3
Brace truss as follows:
To From
TC Cont. 0-0-0 35-0-0
BC Cont. 0-0-0 35-0-0
WB 1 rows CLB on B-H
WB 1 rows CLB on H-C
Attach CLB with (2)-10d nails
at each web.
Loading Live Dead (psf)
TC 20.0 7.0
BC 0.0 10.0
Total 20.0 17.0
Spacing 24.0"
Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10
Plus 9 Wind Load Case(s)
Plus 1 UBC LT Load Case(s)
Jt React UpLft Size Req'd
Lbs In-Sx In-Sx
A 1382 478 4- 0 1-10 Hz = -118
D 1382 478 4- 0 1-10 Hz = 119
Member CSI P Lbs Axl-CSI-Bnd
-----Top Chords-----
A-U 0.36 2356 C 0.14 0.22
J-B 0.34 1964 C 0.12 0.22
F LOCK 3.0x 5.0 Ctr Ctr 0.51
I LOCK 2.0x 4.0 Ctr Ctr 0.44
D LOCK 4.0x 5.0 Ctr Ctr 0.74
L LOCK 3.0x 5.0 Ctr Ctr 0.57
C LOCK 5.0x 8.0 Ctr-0.1 0.93
K LOCK 2.0x 4.0 Ctr Ctr 0.44
B LOCK 5.0x 8.0 Ctr-0.1 0.93
J LOCK 3.0x 5.0 Ctr Ctr 0.57
A LOCK 4.0x 5.0 Ctr 0.1 0.74
Ut Type Plt Size X Y JSI
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
ROBBINS ENGINEERING, INC.
TO BE MANUFACTURED BY
ALL CONNECTOR PLATES
Plates for each ply each face.
LT Defl -0.13" in SI-H L/999
TL Defl -0.26" in H-E L/999
Shear // Grain in B-K 0.25
Plates for each ply each face.

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSSES
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-
concurrent LB on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor: 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 2356 Lbs
Quality Control Factor 1.25
FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates,
Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201



Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 220.0 LBS

Trusses Manufactured by:	0.62	0.17	0.79	2553 C	0.17	0.62
RIDGWAY ROOF TRUSS	0.28	0.02	0.30	2136 C	0.02	0.28
Analysts Conforms To:	0.13	0.14	0.27	2358 C	0.14	0.13

Online Plus -- Version 18.0.014
RUN DATE: 14-NOV-05

CS1 -Size- ---Lumber---

WB	0.23	2x 4	SP-#3
BC	0.60	2x 4	SP-#2
TC	0.79	2x 4	SP-#2

Brace truss as follows:

O.C.
From
To
0 - 0 - 0
0 35 - 0 - 0
TC Cont.

BC Conf. 0 - 0 - 0 35 - 0 - 0
WB 1 rows CLB on B - G

WB 1 rows CLB on G - C
Attach CLB with (2) -10d nails

at each web.

TC	20.0	7.0	(psf)
Dead			
Live			
Load			

BC	0.0	10.0	
Total	20.0	17.0	37.0

Spacing 24.0"
Lumber Duration Factor 1.25

Plate Duration Factor 1.25
TC Rb=1.15 Fc=1.10 Ft=1.10

BC FB=1.10 FC=1.10 FT=1.10

Plus 9 Wind Load Case(s)

Plus 1 UBC LT Load Case(s)

It React Uplift Size Req'd Lbs Lbs In-Sx In-Sx

A	1382	478	4 - 0	1-10	Hz = -96
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D	1382	478	4 - 0	1-10	HZ = 97
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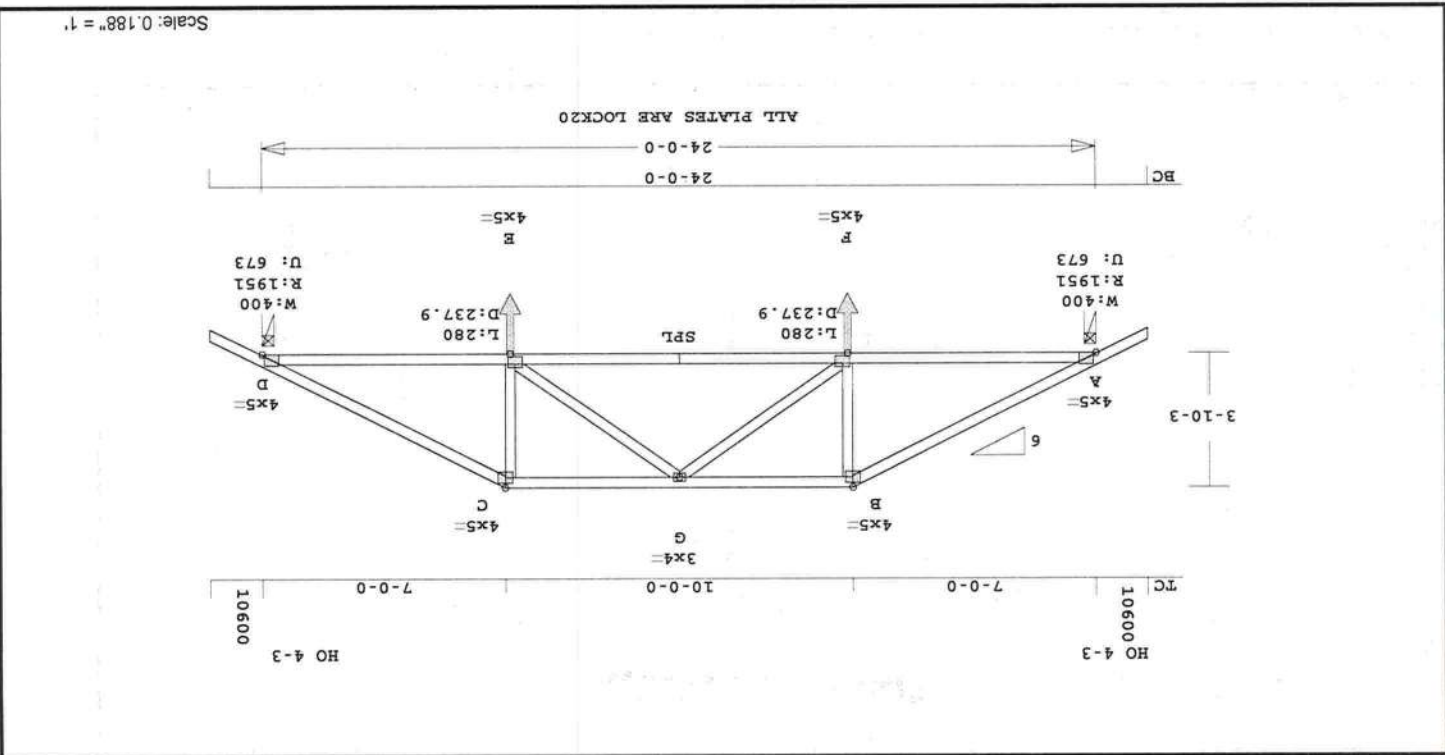
Membr CSI P lbs Ax1-CS1-Bnd

```
-----Top Chords-----
A - H 0.27 2358 C 0.14 0.13
```

H-B	0.30	2136	C	0.02	0.28
B-I	0.79	2553	C	0.17	0.62

Robbins Engineering, Inc./Online Plus™ © 1996-2005 Version 18.0.014 Engineering - Portrait 11/15/2005 8:07:04 AM Page 1

Job	051062	Mark	R19	Quan	1	Type	HIP	Span	240000	Pl-H1	6	Left OH	1-6-0	Right OH	1-6-0	Engineering
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Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 134.7 LBS

Trusses Manufactured by: RIDGWAY ROOF TRUSS

Analysis Conforms To: FBC2004

Girder Step Down Hip

Framing King Jacks

Jack Open Faced

Setback 7'-0" 0

OH Loading

Soffit psf 2.0

Design checked for 10 psf non-concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as Components and Claddings* for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 25'-0

Exposure Category: B

Occupancy Factor: 1.00

Building Type: Enclosed

TC Dead Load: 4.0 psf

BC Dead Load: 6.0 psf

Max comp. force 3600 Lbs

Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss Designer)

Gary Dounson, PE 35054

Gary Dounson & Associates, Inc.

2830 NW 41st Street Suite D Gainesville, FL 32606

(352)375-8593

CA 5201

Online Plus -- Version 18.0.014

RUN DATE: 14-NOV-05

CSI -Size- ---Lumber---

TC 0.70 2x 4 SP-#2

BC 0.75 2x 4 SP-2400

WB 0.37 2x 4 SP-#3

Brace truss as follows:

O.C.

From To

TC Cont. 0-0-0 24-0-0

BC Cont. 0-0-0 24-0-0

Loading Live Dead (psf)

TC 20.0 7.0

BC 0.0 10.0

Total 20.0 17.0

Spacing 24.0"

Lumber Duration Factor 1.25

Plate Duration Factor 1.25

TC Fb=1.00 Fc=1.00 Ft=1.00

BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Glider Loading

Lumber Duration Factor 1.25

Plate Duration Factor 1.25

Plt - Live Dead To

TC V 40 14 0.0' 24.0'

BC V 0 20 0.0' 24.0'

TC V 50 18 7.0' 17.0'

BC V 0 25 16.9' 7.1'

BC V 280 238 16.9' 7.1'

Plus 9 Wind Load Case(s)

Plus 1 UBC LL Load Case(s)

JT React Uplift Size Req'd

A 1952 673 4-0 1-10

NOTES:

Plates for each ply each face.

ALU CONNECTOR PLATES

TO BE MANUFACTURED BY

ROBBINS ENGINEERING, INC.

Plate - LOCK 20 Ga, Gross Area

Plate - RHS 20 Ga, Gross Area

JT Type Plt Size X X JSI

A LOCK 4.0x 5.0 Ctr 0.1 0.92

B LOCK 4.0x 5.0 Ctr 0.93

G LOCK 3.0x 4.0 Ctr 0.47

C LOCK 4.0x 5.0 Ctr 0.93

D LOCK 4.0x 5.0 Ctr 0.1 0.92

F LOCK 4.0x 5.0 Ctr 0.8 0.90

E LOCK 4.0x 5.0 Ctr 0.8 0.90

LT Defl -0.34" in F -S1 L/820

TL Defl -0.74" in F -S1 L/377

Shear // Grain in B -G 0.45

F -B 0.37 1176 T

F -G 0.31 587 C

G -B 0.31 587 C

E -C 0.37 1176 T

-----Bottom Chords-----

A -F 0.70 3217 T 0.15 0.55

F -S1 0.75 3691 T 0.16 0.59

S1-E 0.75 3691 T 0.16 0.59

E -D 0.70 3217 T 0.15 0.55

-----Top Chords-----

A -B 0.56 3600 C 0.24 0.32

B -G 0.70 3217 C 0.08 0.62

G -C 0.70 3217 C 0.08 0.62

C -D 0.56 3600 C 0.24 0.32

Member CSI P Lbs Axl-CSI-Bnd

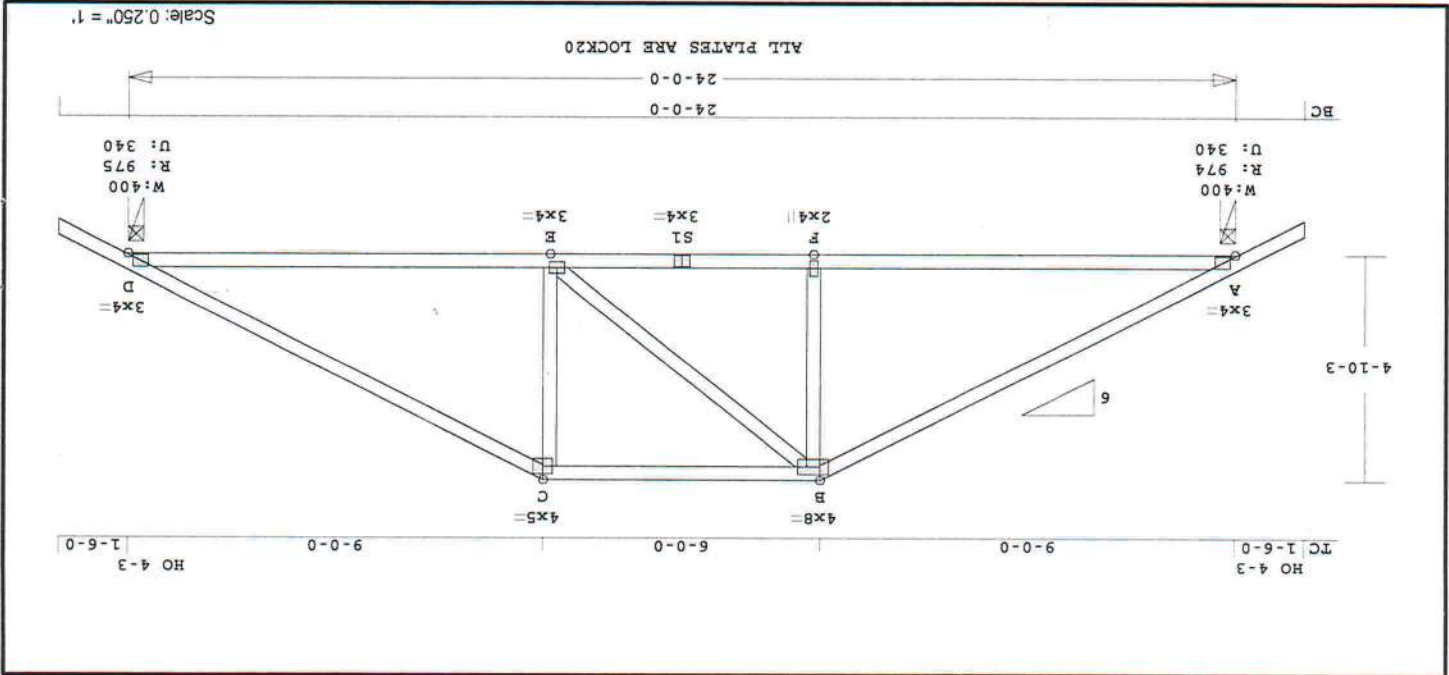
HZ = -66

HZ = 1-10

HZ = 68

Job	051062	Mark	R20	Quan	1	Type	HIPP	Span	240000	Pl-H1	6	Left OH	1-6-0	Right OH	1-6-0	Engineering
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SR 441 - LOT 1 LAKECITY



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 130.7 LBS

Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25'-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1283 lbs
Quality Control Factor 1.25

FABRICATOR NOTES:

I. Delegated Engineer (Truss Designer)

Gary Dounson, PE 35054

Gary Dounson & Associates, Inc.

2830 NW 41st Street Suite D

Gainesville, FL 32606

(352) 375-8593

CA 5201

Brace truss as follows:
O.C. From To
TC Cont. 0-0-0 24-0-0
BC Cont. 0-0-0 24-0-0

CSI - Size - Lumber
TC 0.78 2x 4 SP-#2
BC 0.56 2x 4 SP-#2
WB 0.08 2x 4 SP-#3

Online Plus -- Version 18.0.014
RUN DATE: 14-NOV-05

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 130.7 LBS

Bottom Chords

A - F 0.56 1148 T 0.19 0.37

F - SI 0.48 1144 T 0.19 0.29

SI - E 0.47 1144 T 0.19 0.28

E - D 0.56 1147 T 0.19 0.37

-----Webs-----

F - B 0.08 301 T

B - E 0.07 105 T

E - C 0.07 297 T

LT Defl -0.14" in A - F L/999

TL Defl -0.33" in E - D L/836

Shear // Grain in A - B 0.30

Plates for each ply face.

ATL CONNECTOR PLATES

TO BE MANUFACTURED BY

ROBBINS ENGINEERING, INC.

Plate - LOCK 20 Ga, Gross Area

Plate - RHS 20 Ga, Gross Area

UT Type Plt Size X Y JSI

A LOCK 3.0x 4.0 Ctr Ctr 0.83

B LOCK 4.0x 8.0 Ctr Ctr 0.93

C LOCK 4.0x 5.0 Ctr Ctr 0.93

D LOCK 3.0x 4.0 Ctr Ctr 0.83

F LOCK 2.0x 4.0 Ctr Ctr 0.40

SI LOCK 3.0x 4.0 Ctr Ctr 0.69

E LOCK 3.0x 4.0 Ctr Ctr 0.56

Plus 9 Wind Load Case(s)
Plus 1 UBC LT Load Case(s)

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Lumber Duration Factor 1.25
Plate Duration Factor 1.25

Spacing
Total 24.0"
BC 20.0
TC 17.0

Loading Live Dead (psf)
TC 20.0 7.0
BC 0.0 10.0

IT React Uplift Size Req'd
Lbs In-Sx In-Sx
975 340 4-0 1-2

Hz = -86
Hz = 87

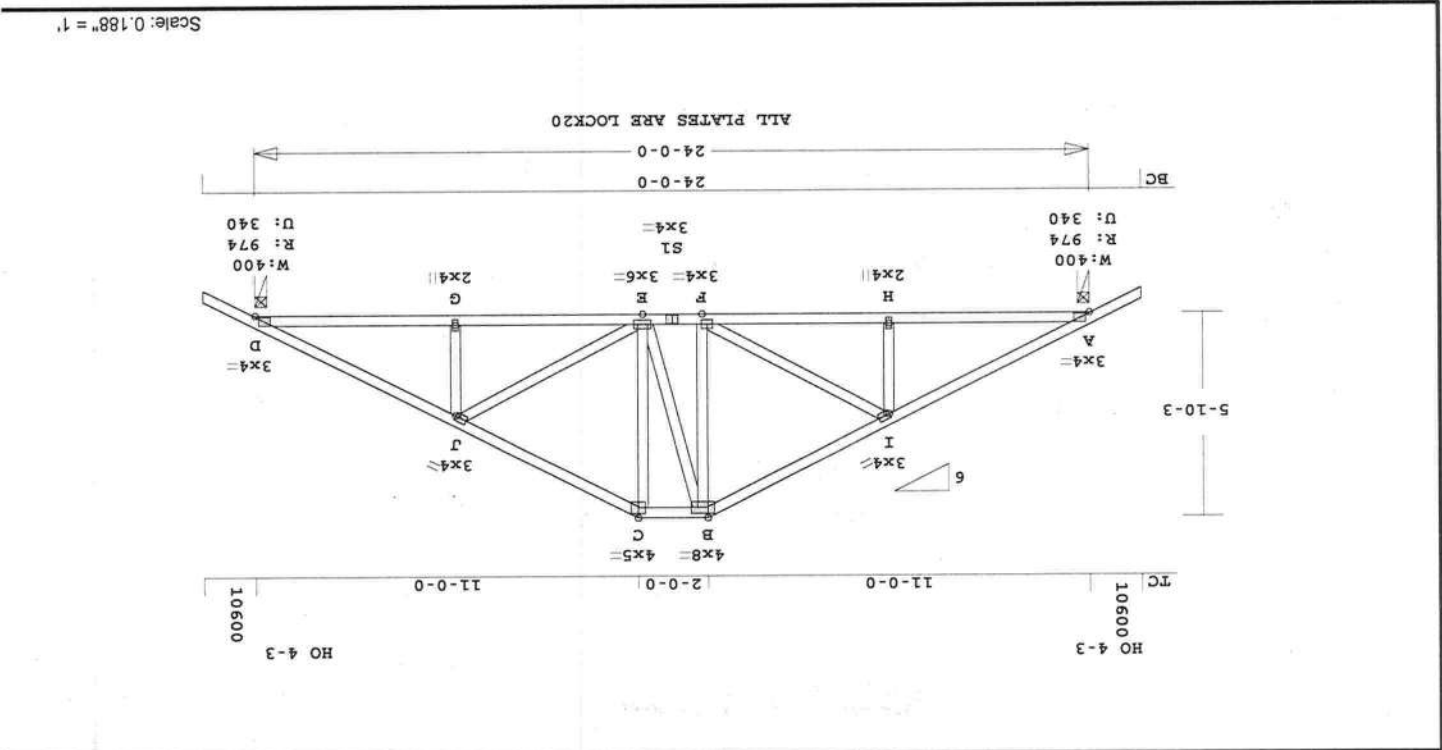
Analysis Conforms To:
FBC2004

OH Loading
Soffit psf 2.0

Design checked for 10 psf non-concurrent LT on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as

Job	051062	Mark	R21	Quan	1	Type	HIPP	Span	240000	Pl-H1	6	Left OH	1-6-0	Right OH	1-6-0	Engineering
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SR 441 - LOT 1 LAKECITY



Scale: 0.188" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 164.5 LBS

I-B 0.33 1061 C 0.08 0.25
 B-C 0.17 943 C 0.08 0.09
 C-J 0.33 1062 C 0.08 0.25
 J-D 0.35 1500 C 0.10 0.25

-----Bottom Chords-----

A-H 0.30 1345 T 0.22 0.08
 H-F 0.30 1345 T 0.22 0.08
 F-S1 0.20 938 T 0.15 0.05
 S1-E 0.21 938 T 0.15 0.06
 E-G 0.29 1345 T 0.22 0.07
 G-D 0.30 1345 T 0.22 0.08

-----Webs-----

H-I 0.06 228 T
 I-F 0.24 458 C
 F-B 0.09 287 T
 B-E 0.03 81 T
 E-C 0.08 278 T
 C-J 0.24 457 C
 G-J 0.06 225 T

LT Defl -0.05" in H -F L/999
 TL Defl -0.11" in H -F L/999
 Shear // Grain in A -I 0.18

PLATES FOR EACH PLY EACH FACE.
 ALL CONNECTOR PLATES
 TO BE MANUFACTURED BY
 ROBINS ENGINEERING, INC.

PLATE - LOCK 20 Ga, Gross Area
 Plate - RHS 20 Ga, Gross Area
 Jt Type Plt Size X Y JSI
 A LOCK 3.0x 4.0 Ctr 0.83
 I LOCK 3.0x 4.0 Ctr 0.62
 B LOCK 4.0x 8.0 Ctr 0.93
 C LOCK 4.0x 5.0 Ctr 0.93
 J LOCK 3.0x 4.0 Ctr 0.62
 D LOCK 3.0x 4.0 Ctr 0.83
 H LOCK 2.0x 4.0 Ctr 0.40
 F LOCK 3.0x 4.0 Ctr 0.56

Member CSI P Lbs Axl-CST-Bnd
 A-I 0.35 1500 C 0.10 0.25

-----Top Chords-----

Plus 9 Wind Load Case(s)
 Plus 1 UBC LL Load Case(s)

Jt React Uplift Size Req'd
 Lbs In-Sx In-Sx
 A 975 340 4- 0 1- 2
 D 975 340 4- 0 1- 2
 Hz = -106
 Hz = 107

Lumber Duration Factor 1.25
 Plate Duration Factor 1.25
 TC Pb=1.15 Fc=1.10 Ft=1.10
 BC Pb=1.10 Fc=1.10 Ft=1.10

Spacing
 Total 20.0 17.0 37.0
 BC 0.0 10.0
 TC 20.0 7.0
 Loading Live Dead (psf)
 BC Cont. 0- 0- 0 24- 0- 0
 TC Cont. 0- 0- 0 24- 0- 0
 O.C. From To

Brace truss as follows:
 TC 0.35 2x 4 SP-#2
 BC 0.30 2x 4 SP-#2
 WB 0.24 2x 4 SP-#3

CSI -Size- -Lumber-
 RUN DATE: 14-NOV-05
 Online Plus -- Version 18.0.014

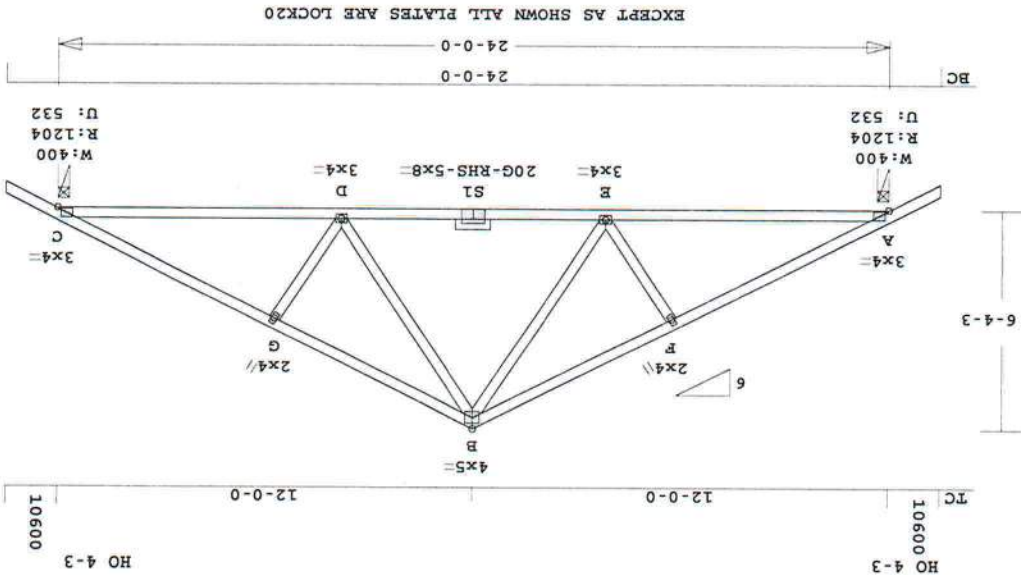
NOTES:

Trusses Manufactured by:
 RIDGWAY ROOF TRUSS
 Analysis Conforms To:
 FBC2004
 OH Loading
 Soffit psf 2.0
 Design checked for 10 psf non-
 concurrent LL on BC.
 Wind Loads - ANSI / ASCE 7-02
 Truss is designed as
 Components and Claddings*
 for Exterior zone location.
 Wind Speed: 110 mph
 Mean Roof Height: 25-0
 Exposure Category: B
 Occupancy Factor: 1.00
 Building Type: Enclosed
 TC Dead Load: 4.0 psf
 BC Dead Load: 6.0 psf
 Max comp. force 1500 lbs
 Quality Control Factor 1.25

FABRICATOR NOTES:
 1. Delegated Engineer (Truss
 Designer)
 Gary Dounson, PR 35054
 Gary Dounson & Associates,
 Inc.
 2830 NW 41st Street Suite D
 Gainesville, FL 32606
 (352) 375-8593
 CA 5201

Job	051062	Mark	R22	Quan	4	Type	TR	Span	240000	Pl-HI	6	Left OH	1-6-0	Right OH	1-6-0	Engineering
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SR 441 - LOT 1 LAKECITY



Scale: 0.188" = 1'

Online Plus -- Version 18.0.014
 RUN DATE: 14-NOV-05

Lbs In-Sx In-Sx
 A 1204 533 4- 0 1- 7 Hz = -115
 C 1204 533 4- 0 1- 7 Hz = 116

CSI -Size- ---Lumber---

TC 0.46 2x 4 SP-#2
 BC 0.90 2x 4 SP-#2
 WB 0.40 2x 4 SP-#3
 PB --- 2x 4 SP-#3

Brace truss as follows:

O.C. From To
 TC Cont. 0- 0- 0 24- 0- 0
 BC Cont. 0- 0- 0 24- 0- 0

Loading Live Dead (psf)

TC 20.0 7.0
 BC 0.0 10.0
 Total 20.0 17.0
 Spacing 24.0"

Lumber Duration Factor 1.25
 Plate Duration Factor 1.25
 plf - Live Dead From To

Load Case # 1 Standard Loading
 BC Fb=1.00 Fc=1.00 Ft=1.00
 TC Fb=1.00 Fc=1.00 Ft=1.00

Plate - Live Dead From To
 TC V 40 14 0.0' 24.0'
 BC V 0 20 0.0' 24.0'

Plus 9 Wind Load Case(s)
 Plus 1 UBC LL Load Case(s)

ut React Uplift Size Req'd
 C LOCK 3.0x 4.0 Ctr Ctr 0.89
 G LOCK 2.0x 4.0 Ctr Ctr 0.37

NOTES:

Trusses Manufactured by:

RIDGWAY ROOF TRUSS

Analysis Conforms To:

FBC2004

OH Loading

Softfit psf 2.0

Design checked for 10 psf non-

concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as

Components and Claddings*

for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 25-0

Exposure Category: B

Occupancy Factor: 1.00

Building Type: Enclosed

TC Dead Load: 4.0 psf

BC Dead Load: 6.0 psf

Max comp. force 1985 lbs

Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss

Designer)

Gary Dounson, PE 35054

Gary Dounson & Associates,

Inc.

2830 NW 41st Street Suite D

Gainesville, FL 32606

(352)375-8593

CA 5201

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Robbins Engineering, Inc./Online PlusSM APPROX. TRUSS W
Lbs In-Sx In-Sx
A 1204 533 4- 0 1- 7 Hz = -115
C 1117 493 4- 0 1- 5 Hz = 116
Online Plus -- Version 18.0.014
RUN DATE: 14-NOV-05

```

EXCEPT AS SHOWN ALL PLATES ARE 10

BC 24-0-0



A diagram of a beam of length 30 ft, supported by a pin at the left end, a roller at 10 ft, and a roller at 20 ft. A triangular load starts at 0 lb/ft at the left end and increases linearly to 300 lb/ft at the right end. The beam is divided into three segments of 10 ft each by the supports.

6-4-3

10660 12-0-0

HO 4-3

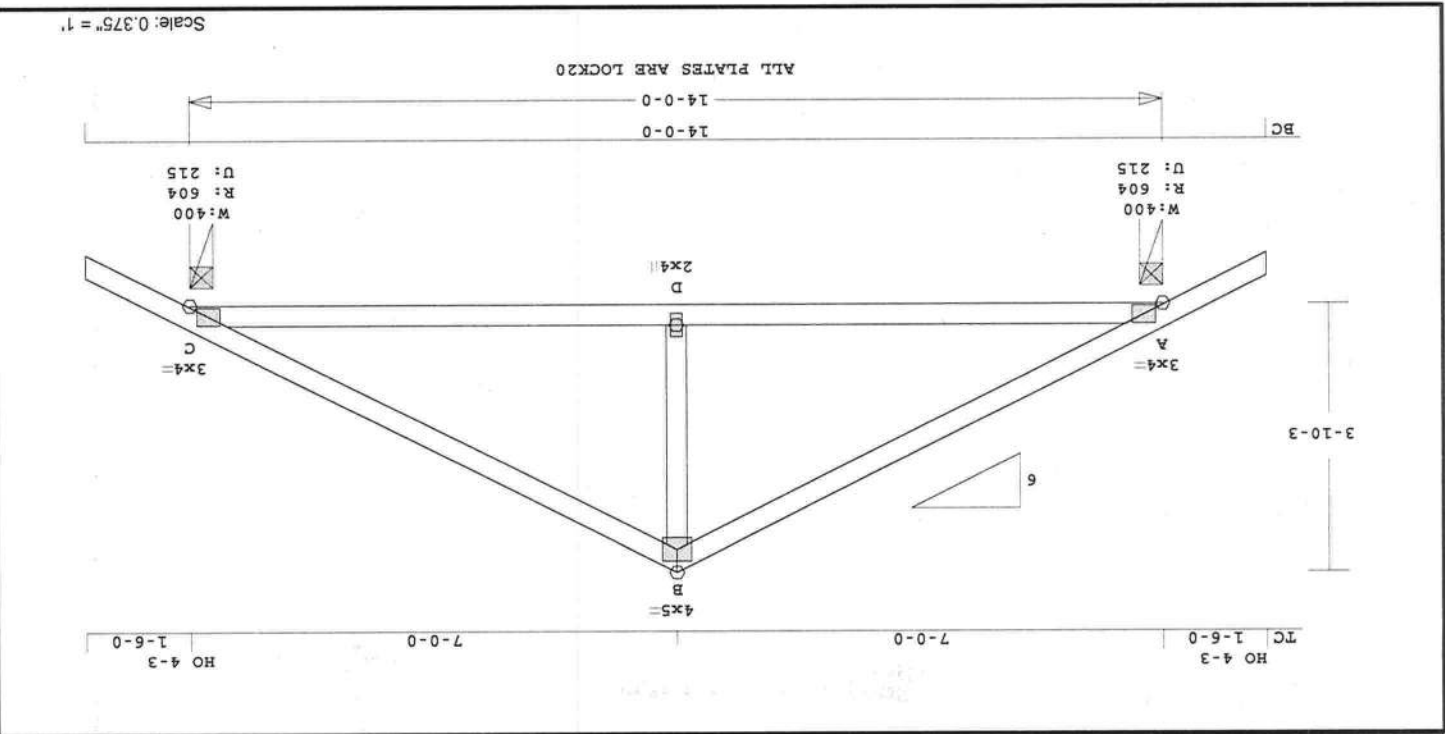
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051062	R23	1	TR	240000	6
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Job	Male	Female	Age	TA	TD
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Job	051062	Mark	R25	Quan	1	Type	TR	Span	140000	Pl-HI	6	Left OH	1-6-0	Right OH	1-6-0	Engineering
-----	--------	------	-----	------	---	------	----	------	--------	-------	---	---------	-------	----------	-------	-------------



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 69.0 LBS

Online Plus -- Version 18.0.014 A-D 0.36 582 T 0.09 0.27

Bottom Chords-----

B-C 0.34 645 C 0.05 0.29

LT Defl -0.04" in A-D L/999

TL Defl -0.08" in A-D L/999

Shear // Grain in A-B 0.25

Plates for each ply each face.

ALL CONNECTOR PLATES

TO BE MANUFACTURED BY

ROBBINS ENGINEERING, INC.

Plate - LOCK 20 Ga, Gross Area

Plate - RHS 20 Ga, Gross Area

Jt Type Plt Size X Y JSI

A LOCK 3.0x 4.0 Ctr 0.67

B LOCK 4.0x 5.0 Ctr 0.51

C LOCK 3.0x 4.0 Ctr 0.67

D LOCK 2.0x 4.0 Ctr 0.38

NOTES:

Trusses Manufactured by:

RIDGWAY ROOF TRUSS

Analysis Conforms To:

RBC2004

OH Loading

Soffit psf 2.0

Design checked for 10 psf non-

concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as

Components and Claddings*

For Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 25-0

CSI -Size- ----Lumber----

TC 0.34 2x 4 SP-#2

BC 0.36 2x 4 SP-#2

WB 0.08 2x 4 SP-#3

Brace truss as follows:

O.C. From To

TC Cont. 0-0-0 14-0-0

BC Cont. 0-0-0 14-0-0

Loading Live Dead (psf)

TC 20.0 7.0

BC 0.0 10.0

Total 20.0 17.0

Spacing 24.0"

Lumber Duration Factor 1.25

Plate Duration Factor 1.25

TC Fb=1.15 Fc=1.10 Ft=1.10

BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)

Plus 1 UBC LL Load Case(s)

Jt React Uplift Size Req'd

Lbs In-Sx In-Sx

A 605 215 4-0 1-0

Hx = -66

C 605 215 4-0 1-0

Hx = 67

Membr CSI P Lbs AXI-CSI-Bnd

-----Top Chords-----

A-B 0.34 645 C 0.06 0.28

Brace truss as follows:

	O.C.	From	To
TC Cont.	0 - 0 - 0	0 - 0 - 0	5 - 0 - 0
BC Cont.	0 - 0 - 0	0 - 0 - 0	5 - 0 - 0

	Live	Dead	(psf)
Loading	20.0	7.0	
TC	20.0		
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"
Number Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15			Fc=1.10 Ft=1.10
BC Fb=1.10			Fc=1.10 Ft=1.10

TC	0.35	2x	4	SP-#2
BC	0.31	2x	4	SP-#2

Online Plus -- Version 18.0.014
RUN DATE: 14-NOV-05

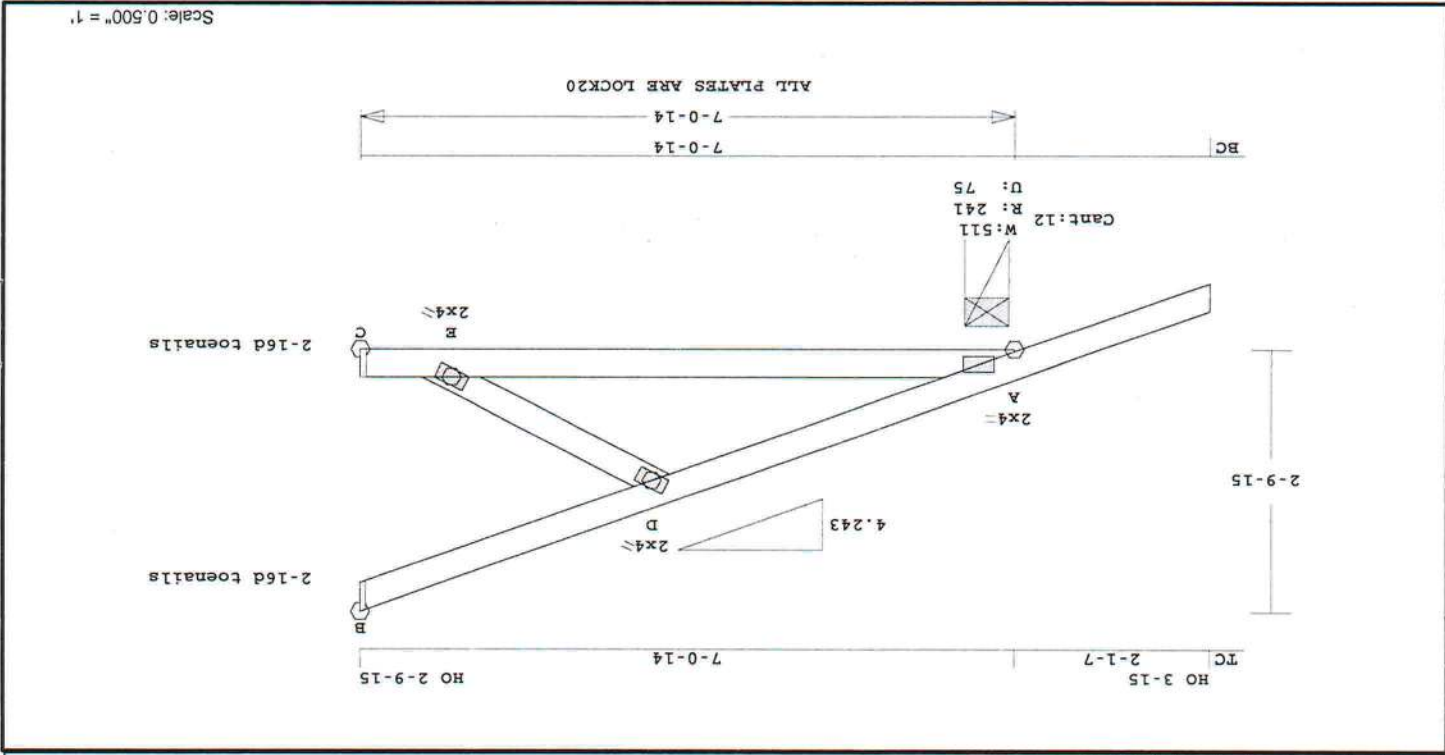
Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS W

Job	051062	Mark	R27	Quan	Type	JACK	50000	Span	P1-H1	6
SR 441 - LOT 1 LAKECITY										

Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load:
BC Dead Load:
Max comp. force 143 lbs
Quality Control Factor 1.25

Scale: 0.500" = 1'

Job	051062	Mark	R3A	Quan	2	Type	KING	Span	70014	Pl-H1	4.243	Left OH	2-1-7	Right OH	0	Engineering
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Robbins Engineering, Inc./Online Plus[®] APPROX. TRUSS WEIGHT: 37.4 LBS

Tr React Uplift Size Req'd

NOTES:

Trusses Manufactured by:

RIDGWAY ROOF TRUSS

Analysis Conforms To:

FB2004

Girder King Jack

Loading TC and BC

Setback 5-0-0

OH Loading

Soffit psf 2.0

Design checked for 10 psf non-

concurrent LL on BC.

Use properly rated hangers for

loads framing into girder

truss.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as

Components and Claddings*

for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 25-0

Exposure Category: B

Occupancy Factor: 1.00

Building Type: Enclosed

TC Dead Load: 4.0 psf

BC Dead Load: 6.0 psf

Max comp. force 248 lbs

Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss

Designer)

Gary Dounson, PE 35054

Inc.

2830 NW 41st Street Suite D

Gainesville, FL 32606

(352)375-8593

CA 5201

Online Plus -- Version 18.0.014 A

RUN DATE: 14-NOV-05

CSI -Size- -Lumber-

TC 0.16 2x 4 SP-#2

BC 0.24 2x 4 SP-#2

WB 0.06 2x 4 SP-#3

Brace truss as follows:

O.C. From To

TC Cont. 0-0-0 7-0-14

BC Cont. 0-0-0 7-0-14

Loading Live Dead (psf)

TC 20.0 7.0

BC 0.0 10.0

Spacing

Lumber Duration Factor 1.25

Plate Duration Factor 1.25

Plate Duration Factor 1.25

TC Fb=1.00 Fc=1.00 Ft=1.00

BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Girder Loading

Lumber Duration Factor 1.25

Plate Duration Factor 1.25

Plate - LIVE Dead From To

TC V 40 14 0.0'

BC V 0 20 0.0'

TC V -40 -14 0.0'

BC V 0 -20 0.0'

0 8

7.1'

7.1'

plus 8 Wind Load Case(s)

plus 1 UBC LL Load Case(s)

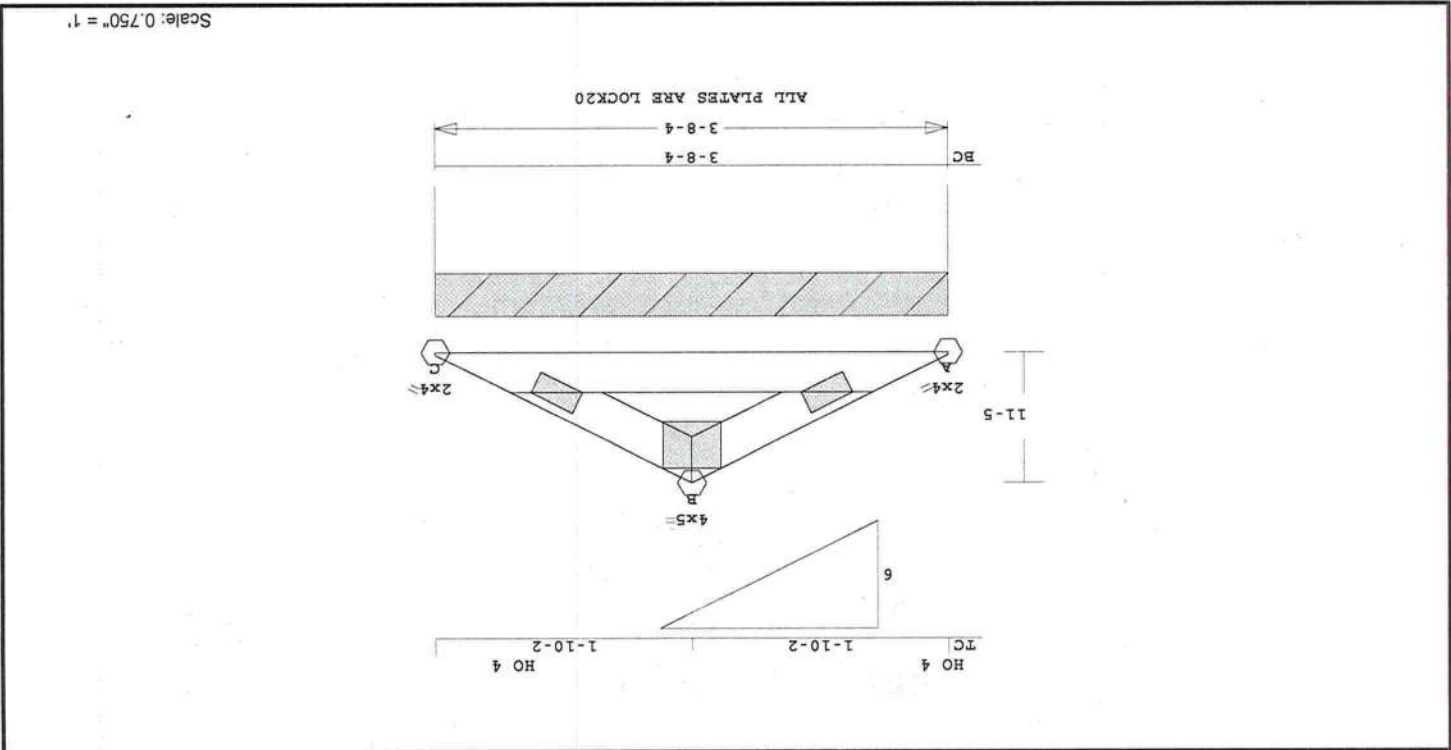
For proper installation of

toe-nails, refer to the 2001

National Design Specification

(NDS) for Wood Construction

Job	051062	SR 441 - LOT 1 LAKECITY
Mark	VI	
Quan	2	
Type	VL	
Span	30804	
Pl-HI	6	
Left OH	0	
Right OH	0	
Engineering		



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 12.4 LBS
 Designer) Gary Dounson, PE 35054
 Gary Dounson & Associates,
 Inc.
 2830 NW 41st Street Suite D
 Gainesville, FL 32606
 (352)375-8593
 CA 5201

Plates for each ply each face.
 ALL CONNECTOR PLATES
 TO BE MANUFACTURED BY
 ROBBINS ENGINEERING, INC.
 Plate - LOCK 20 Ga, Gross Area
 Plate - RHS 20 Ga, Gross Area
 Ut Type Plt Size X Y JSI
 A LOCK 2.0x 4.0 Ctr 0.65
 B LOCK 4.0x 5.0 Ctr-1.3 0.33
 C LOCK 2.0x 4.0 Ctr 0.65
 NOTES:
 Trusses Manufactured by:
 RIDGWAY ROOF TRUSS
 Analysis Conforms To:
 FBC2004
 Design checked for 10 psf non-
 concurrent LL on BC.
 Wind Loads - ANSI / ASCE 7-02
 Truss is designed as
 Components and Claddings*
 for Exterior zone location.
 Wind Speed: 110 mph
 Mean Roof Height: 25-0
 Exposure Category: B
 Occupancy Factor : 1.00
 Building Type: Enclosed
 TC Dead Load: 4.0 psf
 BC Dead Load: 6.0 psf
 Max comp. force 51 lbs
 Quality Control Factor 1.25

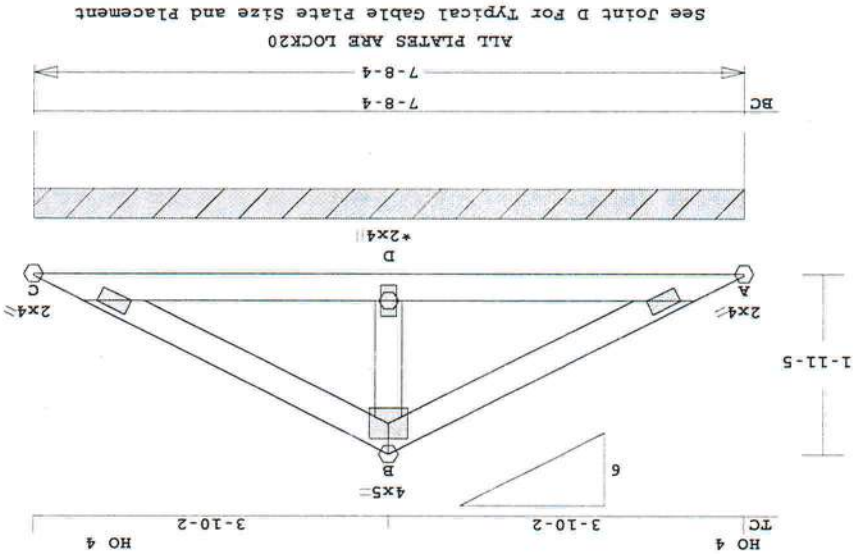
FABRICATOR NOTES:
 1. Delegated Engineer (Truss)

Plus 9 Wind Load Case(s)
 Plus 1 UBC LL Load Case(s)
 Ut React Uplift Size Req'd
 Lbs In-Sx In-Sx
 Cont. Brg 0-0-0 to 3-8-4
 187 63 Hz = 11
 Membri CSI P Lbs AxI-CSt-Bnd
 -----Top Chords-----
 A-B 0.01 96 T 0.01 0.00
 B-C 0.01 96 T 0.01 0.00
 A-C 0.01 0 T 0.00 0.01

Brace truss as follows:
 O.C.
 From To
 TC Cont. 0-0-0 3-8-4
 BC Cont. 0-0-0 3-8-4
 Loading Live Dead (psf)
 TC 20.0 7.0
 BC 0.0 10.0
 Total 20.0 17.0
 Spacing 24.0"
 Lumber Duration Factor 1.25
 Plate Duration Factor 1.25
 TC Pb=1.15 Pc=1.10 Ft=1.10
 BC Pb=1.10 Pc=1.10 Ft=1.10

Job	051062	Mark	V2	Quan	2	Type	VL	Span	70804	Pl-HI	6	Left OH	0	Right OH	0	Engineering
-----	--------	------	----	------	---	------	----	------	-------	-------	---	---------	---	----------	---	-------------

SR 441 - LOT 1 LAKECITY



Scale: 0.500" = 1'

Robbins Engineering, Inc./Online Plus[®] APPROX. TRUSS WEIGHT: 30.9 LBS

BC Dead Load: 6.0 psf

Max comp. force 107 lbs

Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss Designer)

Gary Dounson, PE 35054

Gary Dounson & Associates, Inc.

2830 NW 41st Street Suite D Gainesville, FL 32606

(352)375-8593

CA 5201

LT Defl 0.00" in A - D L/999

TL Defl 0.00" in A - D L/999

Shear // Grain in A - B 0.15

Plates for each ply each face.

ALL CONNECTOR PLATES TO BE MANUFACTURED BY

ROBBINS ENGINEERING, INC.

Plate - LOCK 20 Ga, Gross Area

Plate - RHS 20 Ga, Gross Area

LT Type Plt Size X Y JSI

A LOCK 2.0x 4.0 Ctr 0.71

B LOCK 4.0x 5.0 Ctr 0.43

C LOCK 2.0x 4.0 Ctr 0.71

D LOCK 2.0x 4.0 Ctr 0.00

NOTES:

Trusses Manufactured by: RIDGWAY ROOF TRUSS

Analysis Conforms To: RBC2004

Design checked for 10 psf non-concurrent TL on BC.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as Components and Claddings* for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 25-0

Exposure Category: B

Occupancy Factor : 1.00

Building Type: Enclosed

TC Dead Load: 4.0 psf

CSI -Size- ---Lumber---

TC 0.10 2x 4 SP-#2

BC 0.04 2x 4 SP-#2

GW 0.04 2x 4 SP-#3

Brace truss as follows:

O.C. From To

TC Cont. 0-0-0 7-8-4

BC Cont. 0-0-0 7-8-4

Loading Live Dead (psf)

TC 20.0 7.0

BC 0.0 10.0

Total 20.0 17.0

Spacing 24.0"

Lumber Duration Factor 1.25

Plate Duration Factor 1.25

TC Fb=1.15 Fc=1.10 Ft=1.10

BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)

Plus 1 UBC TL Load Case(s)

Member CSI P Lbs Ax1-CS1-Bnd

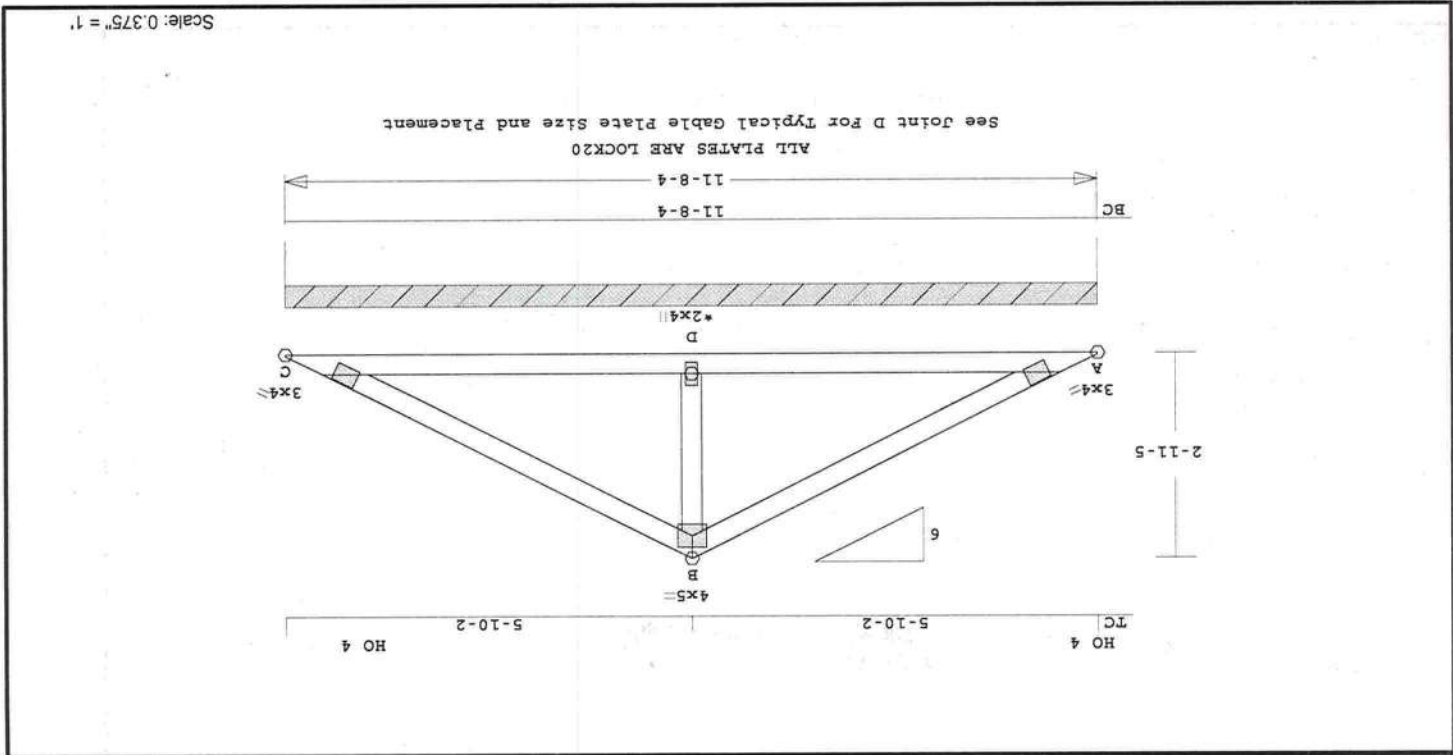
Top Chords-----

A-B 0.10 88 T 0.01 0.09

B-C 0.10 88 T 0.01 0.09

Bottom Chords-----

Job	051062	Mark	V3	Quan	2	Type	VL	Span	110804	Pl-HI	6	Left OH	0	Right OH	0	Engineering
-----	--------	------	----	------	---	------	----	------	--------	-------	---	---------	---	----------	---	-------------



Robbins Engineering, Inc./Online Plus[™] APPROX. TRUSS WEIGHT: 48.8 LBS

A - D 0.13 0 T 0.00 0.13 D - C 0.13 0 T 0.00 0.13 D - B 0.06 250 T

BC Dead Load: 6.0 psf Max comp. force 175 lbs Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss Designer)

Gary Dounson, PE 35054

Gary Dounson & Associates, Inc.

2830 NW 41st Street Suite D

Gainesville, FL 32606

(352) 375-8593

CA 5201

CSI - Size - Lumber -

TC 0.28 2x 4 SP-#2

BC 0.13 2x 4 SP-#2

GW 0.06 2x 4 SP-#3

Brace truss as follows:

O.C. From To

TC Cont. 0 - 0 - 0 11 - 8 - 4

BC Cont. 0 - 0 - 0 11 - 8 - 4

Loading Live Dead (psf)

TC 20.0 7.0

BC 0.0 10.0

Total 20.0 17.0

Spacing 24.0"

Lumber Duration Factor 1.25

Plate Duration Factor 1.25

TC Fb=1.15 Fc=1.10 Ft=1.10

BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)

Plus 1 UBC LL Load Case(s)

React Uplift Size Req'd

Lbs In-Sx In-Sx

Cont. Brg 0 - 0 - 0 to 11 - 8 - 4

779 263 Hz = 51

Membr CSI P Lbs AXI-CXI-Bnd

-----Top Chords-----

A - B 0.28 143 T 0.01 0.27

B - C 0.28 143 T 0.01 0.27

-----Bottom Chords-----

NOTES:

Trusses Manufactured by:

RIDGWAY ROOF TRUSS

Analysis Conforms To:

FB2004

Design checked for 10 psf non-concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as

Components and Claddings* for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 25-0

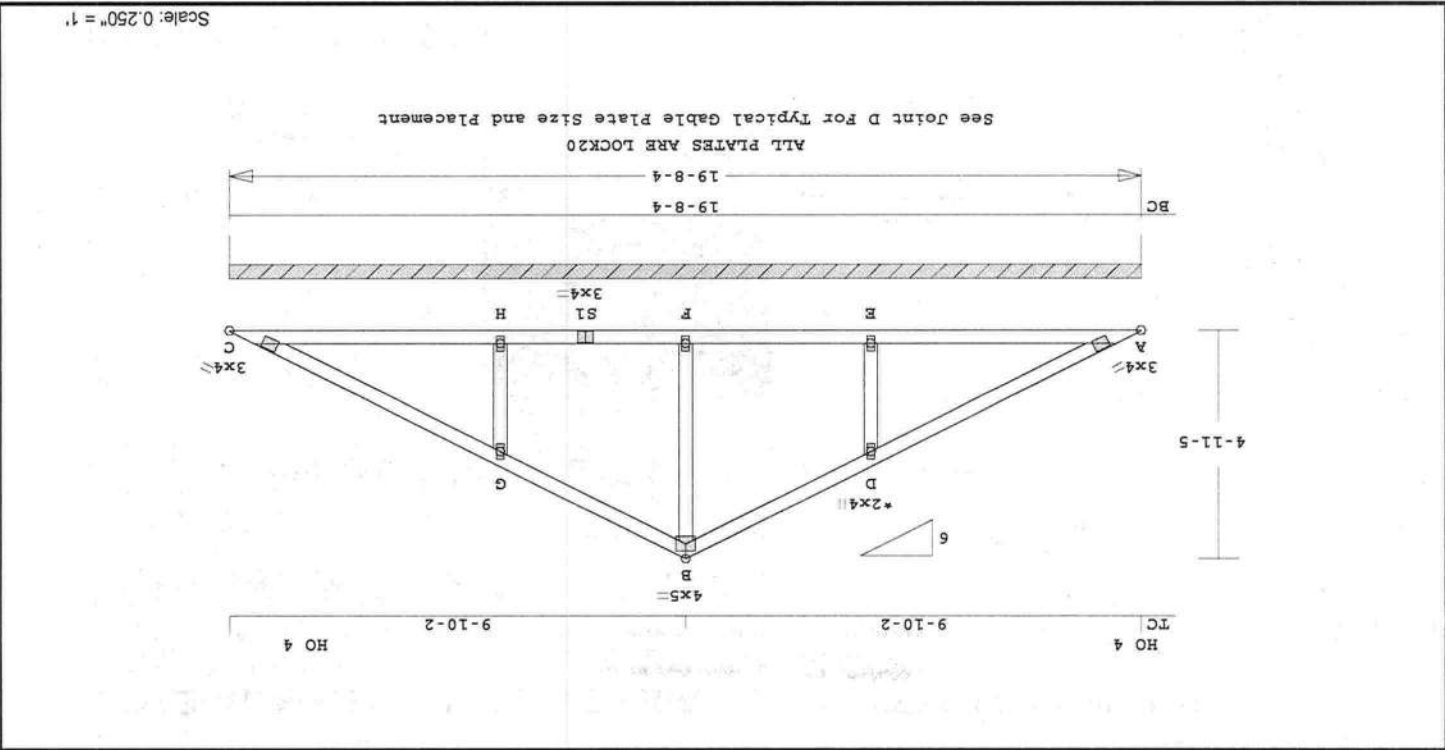
Exposure Category: B

Occupancy Factor : 1.00

Building Type: Enclosed

TC Dead Load: 4.0 psf

Job	051062	Mark	V5	Quan	1	Type	VL	Span	190804	Pl-HI	6	Left OH	0	Right OH	0	Engineering
-----	--------	------	----	------	---	------	----	------	--------	-------	---	---------	---	----------	---	-------------



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 93.7 LBS

Online Plus -- Version 18.0.014 A - E 0.11 0 T 0.00 0.11
 RUN DATE: 14-NOV-05
 CSI - Size - Lumber -
 TC 0.19 2x 4 SP-#2
 BC 0.11 2x 4 SP-#2
 GW 0.07 2x 4 SP-#3

Brace truss as follows:

O.C. From To
 TC Cont. 0 - 0 - 0 19 - 8 - 4
 BC Cont. 0 - 0 - 0 19 - 8 - 4

Loading Live Dead (psf)

TC 20.0 7.0
 BC 0.0 10.0
 Total 20.0 17.0
 Spacing 24.0"

Lumber Duration Factor 1.25
 Plate Duration Factor 1.25
 TC Fb=1.15 Fc=1.10 Ft=1.10
 BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
 Plus 1 UBC LL Load Case(s)

It React Uplift Size Req'd
 Lbs In-Sx In-Sx
 Cont. Brq 0 - 0 - 0 to 19 - 8 - 4
 1371 463 Hz = 90

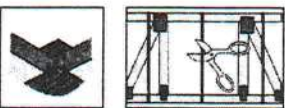
Member CSI P Lbs Axl-Csi-Bnd
 -----Top Chords-----
 A - D 0.18 46 C 0.00 0.18
 D - B 0.19 136 T 0.01 0.18
 B - G 0.19 136 T 0.01 0.18

BCSI-B1 SUMMARY SHEET 1 of 2

GENERAL NOTES

Trusses are not marked in any way to identify the frequency or location of temporary bracing. Follow the recommendations for handling, installing and temporary bracing of trusses. Refer to BCSI J-03 Guide to Good Practice for Connected Wood Trusses for more detailed information.

Truss Design Drawings may specify locations of permanent bracing for more information. All other members secondary (webs) for more information. El resto de miembros secundarios (webs) para mayor información. El diseñador del Edificio.



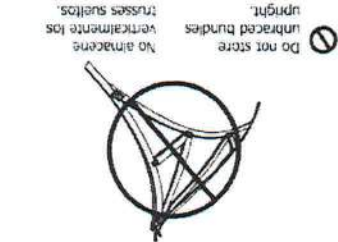
HANDLING — MANEJO

⚠ The consequences of improper handling, installing and bracing may be a collapse of the structure, or worse, serious personal injury or death.
El resultado de un manejo, instalación y arrioste inadecuados, puede ser la caída de la estructura o aun peor, muertes o heridos.

Use special care in windy weather or near power lines and airports. Use special cuidado en días ventosos o cerca de aeropuertos.

Check bracing prior to moving bundles. Revisar los empacques antes de mover los paquetes de trusses.

Avoid lateral bending. — Evite la flexión lateral.



⚠ Bundles stored on the ground for one week or more should be raised by blocking at 8' to 10' on center.
Los paquetes almacenados en la tierra por una semana o mas deben ser elevados con bloques a cada 8 o 10 pies.

⚠ For long term storage, cover bundles to prevent moisture gain but allow for ventilation.
Para almacenamiento por mayor tiempo, cubra los paquetes para prevenir aumento de humedad pero permita ventilación.

BRACING — ARRIOSTRE

⚠ Refer to BCSI-B2 Summary Sheet - Truss Installation and Temporary Bracing for more information.
Vea el resumen BCSI-B2 - Instalación de Trusses y Arrioste Temporal para mayor información.

⚠ Do not walk on unbraced trusses.
No camine en trusses sueltos.

⚠ Local ground braces for first truss directly in line with all rows of top chord temporary lateral bracing.
Coloque los arriostres de tierra para el primer truss directamente en línea con los temporales de la cuerda superior.

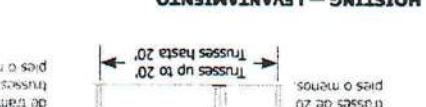
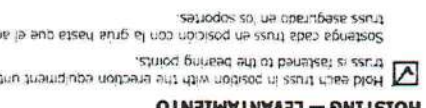
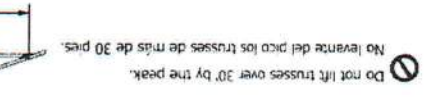
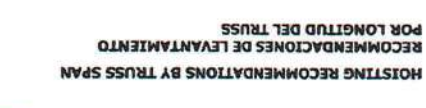
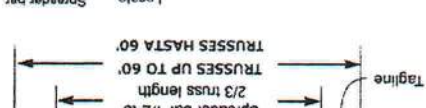
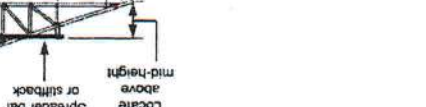
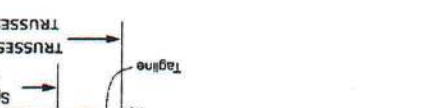
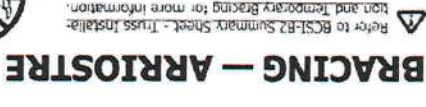
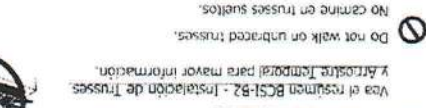
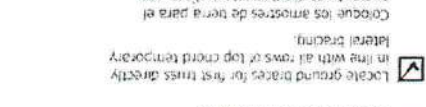
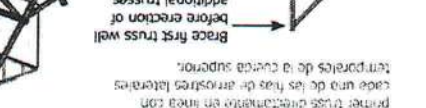
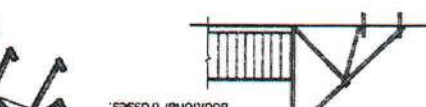
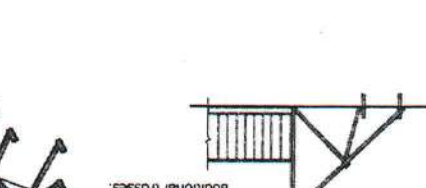
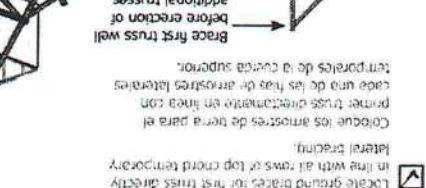
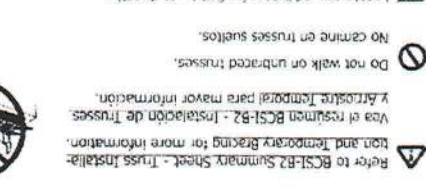
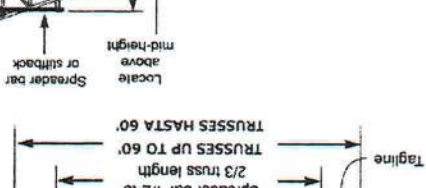
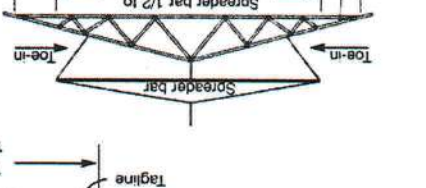
⚠ Brace first truss well before erection of additional trusses.
Cada uno de las filas de arriostres laterales debe ser instalado en línea con el primer truss.



HOISTING RECOMMENDATIONS BY TRUSS SPAN

⚠ Do not lift trusses over 30' by the peak.
No levante del pico los trusses de más de 30 pies.

⚠ Hold each truss in position with the erection equipment until temporary bracing is installed and truss is fastened to the bearing points.
Sostenga cada truss en posición con la grúa hasta que el arrioste temporal esté instalado y el truss asegurado en los soportes.

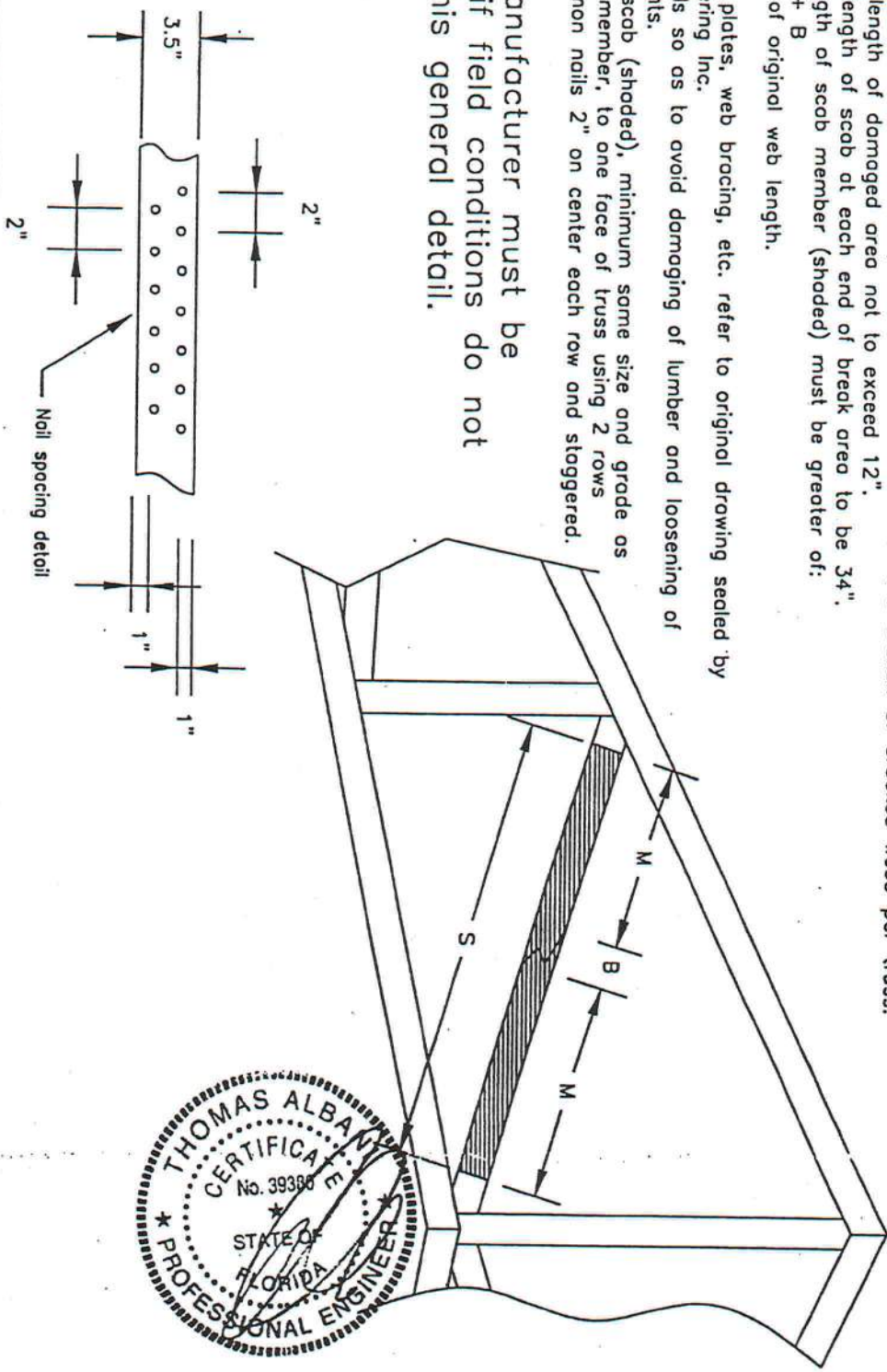


- General detail for repair of broken webs that meet the following conditions.
- * Web size and grade must be maximum 2x4, #3, standard, or stud grade of any species.
 - * Truss must be single ply.
 - * No more than one crack or break per web and no more than two broken or cracked webs per truss.
 - B = Maximum length of damaged area not to exceed 12".
 - M = Minimum length of scab at each end of break area to be 34".
 - S = Overall length of scab member (shaded) must be greater of:
 - S = 68" + B
 - S = 80% of original web length.

For all lumber, plates, web bracing, etc. refer to original drawing sealed by Robbins Engineering Inc.

- 1) Apply all nails so as to avoid damaging of lumber and loosening of plates at joints.
- 2) Attach 2x4 scab (shaded), minimum same size and grade as original web member, to one face of truss using 2 rows of 10d common nails 2" on center each row and staggered.

Truss manufacturer must be notified if field conditions do not match this general detail.



ROBBINS

Engineering Inc.

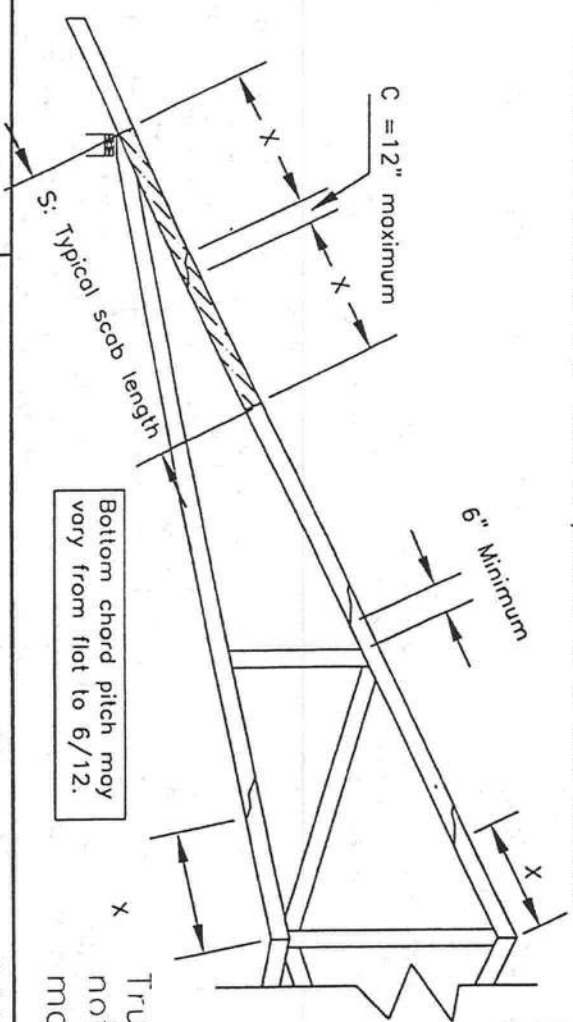
P.O.Box 280033, Tampa, FL 33682

ROBBINS LOCK connector plates (20 90 gph, steel-ASTM A36 S5 Grade 40) Robbins Mfg. Co. bears no responsibility for the reaction of trusses, which are subject to change in design, material, or construction. The manufacturer of the truss is responsible for the design, material, and construction of the truss. The manufacturer of the truss is responsible for the design, material, and construction of the truss. The manufacturer of the truss is responsible for the design, material, and construction of the truss.

IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER AND TRUSS FAB. TO REVIEW THIS DRAW. & VERIFY THAT DATA INCLUDING DIM. & LOADS CONFORM TO ARCH. PLAN/SPEC'S & F.W. TRUSS LAYOUTS.

Designed By: MG
Checked By: TAA
Date: 5/23/02
Dwg. No: GD-41

TOTAL NUMBER OF NAILS ON EACH SIDE OF BREAK		.X (INCHES)	MAXIMUM FORCE (LBS.) AT 33% LOAD DURATION	
2X4 S/P	2X6 S/P		2X4 S/P	2X6 S/P
13	20	24"	1924	2886
17	26	30"	2501	3752
21	32	36"	3079	4618
25	38	42"	3656	5484
29	44	48"	4233	6349



x Truss manufacturer must be notified if field conditions do not match this general detail.

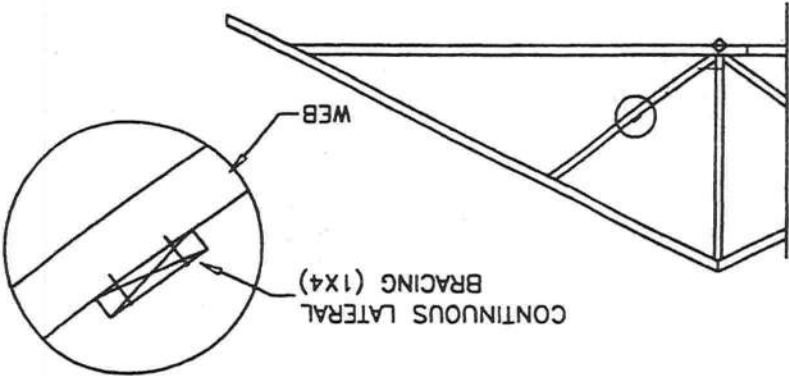
P.O.Box 280055, Tampa, FL 33682

[illegible]

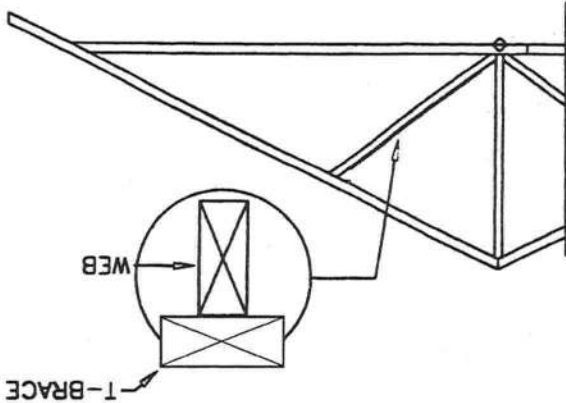
FURNISH A COPY OF THIS DESIGN TO ERECTION CONTRACTOR
IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER AND TRUSS FAB. TO REVIEW THIS DRAWG. &
VERIFY THAT DATA INCLUDING DIM. & LOADS CONFORM TO ARCH. PLAN/SPECS & FAB. TRUSS LAYOUTS

Dwg. No: GD-62-33

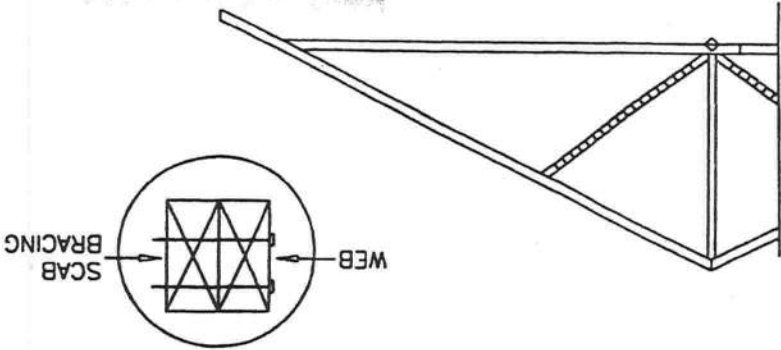
PERMANENT WEB BRACING



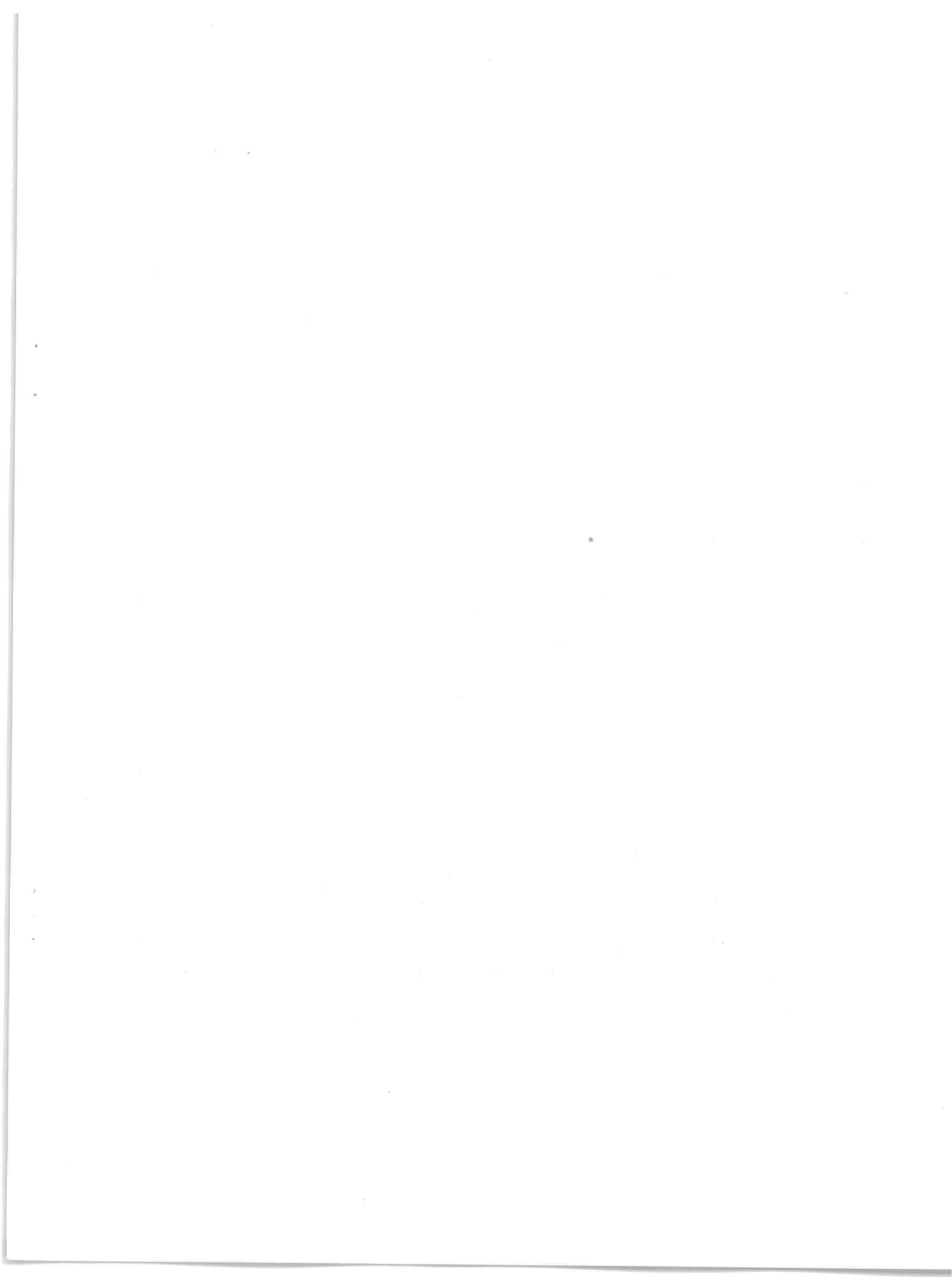
CONTINUOUS LATERAL BRACING
1x4 #3 HEM-FIR OR
BETTER CONTINUOUS LATERAL
BRACING TO BE EQUALLY SPACED.
ATTACH WITH (2) 8d NAILS.
BRACING MATERIAL TO BE SUPPLIED
AND ATTACHED AT BOTH ENDS
TO A SUITABLE SUPPORT BY
ERECTION CONTRACTOR.



T-BRACE
THESE DETAILS APPLY TO 1.5" WIDE WOOD
TRUSSES.
• USE A 2x4 T-BRACE IF THE TRUSS DESIGN
SPECIFIES ONE LATERAL BRACE (MID POINT
OF WEB).
• USE A 2x6 T-BRACE IF THE TRUSS DESIGN
SPECIFIES TWO LATERAL BRACES (AT THE
THIRD POINTS OF THE WEB).
• USE A CONTINUOUS PIECE FOR THE
T-BRACE, OF THE SAME GRADE AS THE WEB
AND COVERING AT LEAST 90% OF THE WEB
LENGTH.
• CENTER THE T-BRACE ON THE WEB AND
FASTEN WITH 10d COMMON NAILS SPACED 4"
ON CENTER.



SCAB BRACE SAME SIZE, GRADE,
AND LENGTH AS WEB MEMBER.
ATTACH WITH 10d NAILS @ 4" O.C.
BRACING MATERIAL TO BE SUPPLIED
BY ERECTION CONTRACTOR.



PROJECT NAME: SR Lot 441 - Lot 1 Lakecity
JOB NUMBER: 05-1062
CONTRACTOR: Edix Investments
DATE: 11/15/05
REVISIONS:
COMMENTS:

SEALED
DESIGN MANUAL

**THESE TRUSSES MUST BE
HANDLED ACCORDING TO
BCSI-B1 SUMMARY SHEETS
SEE TABLE OF CONTENTS**

WARNING

Telephone: (352) 376-4436
FAX: (352) 371-3316
Email: Sales@RidgwayTruss.com
www.RidgwayTruss.com
Mailing: P.O. Box 1309 - Gainesville, Florida 32602
Physical: 235 SW 11th Place - Gainesville, Florida 32601
(Trusses and Prefabricated Building Components)

Ridgway Roof Truss Company

1000



RIDGWAY ROOF TRUSS COMPANY

P.O. BOX 1309 GAINESVILLE, FL 32602, TELEPHONE: (352) 376-4436 FAX: (352) 371-3316

Job: SR 441 Lot 1 Lakecity

Customer: Edix Investments

Jurisdiction: Alachua

Structural Eng. of Record: none

License #: none

building designer

Address of Eng. Of Record: none

SIGNATURE of Building Designer:

single family home

I, Gary G. Dounson, Florida Professional Engineer No. 35054, hereby certify that I am the truss engineer of record with responsibility for the design of the metal plate connected wood trusses listed herein. The design of these trusses have been performed under my responsible supervision, direction and control and are in accordance with the National Design Standard for Metal Plate Connected Wood Truss Construction (ANSI TPI 1). Building Designer of Record responsibilities shall be required in Article 2.2 of the aforementioned TPI 1 document and shall include but are not limited to the design and detailing of truss supports, anchorage, and permanent truss bracing.

Building code and chapter: ANSI/ASCE 7-98

Design criteria: TPI 1-95

Category: II

Enclosure: Enclosed

Exposure: B

Roof

Dead load: 17 lbs

Wind speed: 110 mph

Live load: 20 lbs

Mean Roof Height: 25'

#	Mark	Run Date	#	Mark	Run Date	#	Mark	Run Date	#	Mark	Run Date	#	Mark	Run Date	#	Mark	Run Date
1	R1	11/14/05	29	V1	11/14/05	57			85			107					
2	R2	11/14/05	30	V2	11/14/05	58			86			108					
3	R3	11/14/05	31	V3	11/14/05	59			87			109					
4	R4	11/14/05	32	V4	11/14/05	60			88			110					
5	R5	11/14/05	33	V5	11/14/05	61			89			111					
6	R6	11/14/05	34			62			90			112					
7	R7	11/14/05	35			63			91			113					
8	R8	11/14/05	36			64			92			114					
9	R9	11/14/05	37			65			93			115					
10	R10	11/14/05	38			66			94			116					
11	R11	11/14/05	39			67			95			117					
12	R12	11/14/05	40			68			96			118					
13	R13	11/14/05	41			69			97			119					
14	R14	11/14/05	42			70			98			120					
15	R15	11/14/05	43			71			99			121					
16	R16	11/14/05	44			72			100			122					
17	R17	11/14/05	45			73			101			123					
18	R18	11/14/05	46			74			102			124					
19	R19	11/14/05	47			75			103			125					
20	R20	11/14/05	48			76			104			126					
21	R21	11/14/05	49			77			105			127					
22	R22	11/14/05	50			78			106			128					
23	R23	11/14/05	51			79											
24	R24	11/14/05	52			80											
25	R25	11/14/05	53			81											
26	R26	11/14/05	54			82											
27	R27	11/14/05	55			83											
28	R3A	11/14/05	56			84											

This Package includes 33 individual, dated Truss Design Drawings. With my Seal

affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index

sheet conforms to 61G15-31.003 section 5 of the Florida Board of Professional Engineers.

Note: The seal on this index sheet indicates acceptance of professional engineering responsibility solely for the Truss Design Drawings listed above and attached. The suitability and use of each component for any particular building is the responsibility of the Building Designer, per ANSI/TPI 1-1995 Section 2.

11-15-05

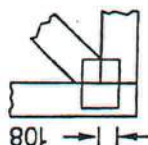


TABLE OF CONTENTS

1. GENERAL NOTES
2. WTCA 1-1995 (STANDARD RESPONSIBILITIES IN THE
DESIGN PROCESS INVOLVING METAL PLATE CONNECTED
WOOD TRUSSES)
ENGINEERING (INSERT)
BLUEPRINT OF TRUSS PLACEMENT PLAN (INSERT)
6. BCSI-B1 SUMMARY SHEET
HANDLING, INSTALLING & BRACING INFORMATION
8. STANDARD CHORD & WEB REPAIRS
11. EXAMPLES OF PERMANENT WEB BRACING

ROBBINS ENG. GENERAL NOTES & SYMBOLS

PLATE LOCATION



Center plates on joints unless otherwise noted in plate list or on drawing. Dimensions are given in inches (i.e. 1 1/2" or 1.5") or IN-16ths (i.e. 108).

PLATE SIZE



The first dimension is the width measured perpendicular to slots. The second dimension is the length measured parallel to slots.

LATERAL BRACING

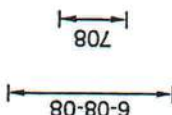
1x4 continuous lateral bracing attached with (2) 8d nails each member where indicated or 2x4 "T" or "L" brace stiffener if applicable nailed flat to edge of web with 12d nails spaced 8" o.c. "T" or "L" brace must be extended at least 90% of web length.

PLATE ORIENTATION

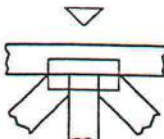


When shown, indicates direction of slots in connector plate.

DIMENSIONS



All dimensions are shown in FT-IN-SX (i.e. 6' 8" 1/2" or 6-08-08). Dimensions less than one foot are shown in IN-SX only (i.e. 708).



BEARING

When truss is designed to bear on multiple supports, interior bearing locations should be marked on the truss. Interior support or temporary shoring must be in place before erecting this truss. If necessary, shim bearings to assure solid contact with truss.

ROBBINS LOCK connector plates (20 ga. galv. steel ASTM A653 SS Grade 40) shall be applied on both faces of truss at each joint. Center the plates, unless shown otherwise by circles (o) or dimensions. No loose knots or wanes in plate contact area. Splice only where shown. Overall spans assume 4" bearings at each end, unless indicated otherwise. Cutting and fabrication shall be performed on equipment which produces snug-fitting joints and plates. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication and these designs are not applicable for use with fire retardant lumber. This design was prepared in accordance with "National Design Specifications for Stress - Grade Lumber and Its Fastenings" (AFPA), "Design Specifications for Light Metal Plate Connected Wood Trusses" (TPI), and HUD Design Criteria for

Trussed Rafters. Robbins Eng. Co. bears no responsibility for the erection of trusses, field bracing or permanent truss bracing. Refer to HIB-91 as published by the Truss Plate Institute, 583 D'Oroff Drive, Suite 200, Madison, Wisconsin 53719. Persons erecting trusses are cautioned to seek professional advice concerning proper erection bracing to prevent toppling and "dominoing". Care should be taken to prevent damage during fabrication, storage, shipping and erection. Top and bottom chords shall be adequately braced in the absence of sheathing or rigid ceiling, respectively. It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead loads imposed by the structure and the live loads imposed by the local building code or historical climatic records.

FURNISH A COPY OF THIS DESIGN TO ERECTION CONTRACTOR. IT IS THE RESPONSIBILITY OF BUILDING DESIGNER TO REVIEW THIS DRWG. & VERIFY THAT DATA INCLUDING DIM. & LOADS CONFORM TO ARCH. PLAN/SPECS & FAB. TRUSS LAYOUTS.



CORPORATE HEADQUARTERS

P.O. Box 280055
Tampa, FL 33682-0055
800-282-1299 • Fax: 813-971-6117

Developed by the WTCA Engineering Review Committee
in cooperation with the Truss Plate Institute

Standard Responsibilities in the Design Process Involving Metal Plate Connected Wood Trusses

WTCA 1-1995

Wood Truss Council of America
5937 Meadowood Dr., Ste. 14 • Madison, WI 53711-4125 • 608/274-3329 (fax)



The Wood Truss Council of America publishes standard practice materials prepared and edited by knowledgeable authors from the construction industry to give as much assistance as possible to owners, architects, engineers, contractors, building officials, and others involved in the metal plate connected wood truss industry. The competence of the authors ensures accurate and authoritative information in regard to the subject matter covered, but, of course, neither the Wood Truss Council of America, nor the authors make either express or implied warranties in regard to the use of the materials.

1.0 SCOPE AND DEFINITIONS

- 1.1

This standard defines the design responsibilities of the individuals and organizations involved in the preparation, submittal, review and approval of each Truss Design Drawing and Truss Placement Plan associated with the use of metal plate connected wood trusses. These guidelines are presented as industry standard practice. The guidelines are not intended to preclude alternate provisions as agreed upon by the parties involved.
- 1.2

The following definitions shall apply:
- 1.2.1

"Architect" shall mean the individual registered architect responsible for the architectural design of the structure and who produces the architectural drawings included in the Construction Design Documents.
- 1.2.2

"Building Designer" is the individual or organization having responsibility for the overall building or structure design in accordance with the state's statutes and regulations governing the professional registration and certification of architects or engineers. This responsibility includes but is not limited to foundation design, structural member sizing, load transfer, bearing conditions, and the structure's compliance with the applicable building codes. Also referred to as registered architect or engineer, building designer, and registered building designer, but hereinafter will be referred to as Building Designer.
- 1.2.3

"Construction Design Documents" are the architectural drawings, structural drawings, mechanical drawings, electrical drawings, and any other drawings, specifications, and addenda which set forth the overall design of the structure and issued by the Building Designer.
- 1.2.4

"Contractor" shall mean the individual or organization responsible for the field storage, handling, and installation of trusses including, but not limited to, temporary bracing, permanent
- 1.2.5

"Engineer-of-Record" shall mean the registered professional engineer responsible for the structural design of the structure and who produces the structural drawings included in the Construction Design Documents.
- 1.2.6

"Owner" shall mean the individual or organization for whom the structure is designed.
- 1.2.7

"Truss" is an individual metal plate connected wood structural component manufactured by the Truss Manufacturer.
- 1.2.8

"Truss Designer" is the design professional, individual or organization, having responsibility for the design of metal plate connected wood trusses. This responsibility shall be in accordance with the state's statutes and regulations governing the professional registration and certification of architects or engineers. Also referred to as truss engineer, design engineer, registered engineer, and engineer, but hereinafter will be referred to as Truss Designer.
- 1.2.9

"Truss Design Drawing" shall mean the graphic depiction of an individual Truss prepared by the Truss Designer.
- 1.2.10

"Truss Manufacturer" shall mean an individual or organization regularly engaged in the manufacturing of Trusses.
- 1.2.11

"Truss Placement Plan" is the drawing identifying the location assumed for each Truss based on the Truss Manufacturer's interpretation of the Construction Design Documents.

2.0 OWNER RESPONSIBILITIES

- 2.1
- Directly or through its representatives, which may include the Contractor and/or Building Designer, (a) review and approve each Truss Design Drawing; (b) review and approve the Truss Placement Plan; (c) resolve and approve all design issues arising out of the preparation of each

Truss Design Drawing and Truss Placement Plan; and (d) coordinate the return of each approved Truss Design Drawing and Truss Placement Plan to the Truss Manufacturer prior to truss manufacturing.

3.0 BUILDING DESIGNER RESPONSIBILITIES

- 3.1
- Design a structure suitable to ensure that the intended function of each Truss is not affected by adverse influences including, but not limited to, moisture, temperature, corrosive chemicals and gases;
- 3.2
- Prepare the Construction Design Documents, showing all trussed areas, which must provide as a minimum the following:
- 3.2.1
- All truss orientations and locations;
- 3.2.2
- Information to fully determine all truss profiles;
- 3.2.3
- Adequate support of the Truss and all truss bearing conditions;
- 3.2.4
- Permanent bracing design for the structure including the Trusses, except as provided in 3.4 and 6.2.12.
- 3.2.5
- The location, direction and magnitude of all dead and live loads applicable to each Truss including, but not limited to, loads attributable to: roof, floor, partition, mechanical, fire sprinkler, attic, storage, wind, snow drift and seismic;
- 3.2.6
- All Truss anchorage designs required to resist uplift, gravity, and lateral loads;
- 3.2.7
- Allowable vertical and horizontal deflection criteria;
- 3.2.8
- Proper transfer of design loads affecting the Truss; and
- 3.2.9
- Adequate connections between Truss and non-Truss components, except as noted in Section 6.2.9.

Review and approve the Truss Placement Plan and each Truss Design Drawing for conformance with the requirements and intent of the Construction Design Documents, the effect of each Truss Design Drawing and Truss Placement Plan on other parts of the structure, and the effect of the structure on each Truss.

Specify permanent lateral bracing where indicated by the Truss Designer on the Truss Design Drawings, to prevent buckling of the individual truss members due to design loads. The Building Designer shall specify how the permanent lateral bracing is to be anchored or restrained to prevent lateral movement if all truss members, so braced, buckle together. This shall be accomplished by: (a) anchorage to solid end walls; (b) permanent diagonal bracing in the plane of the web members; or (c) other means when demonstrated by the Building Designer to provide equivalent bracing.

4.0 CONTRACTOR RESPONSIBILITIES

- 4.1
- Provide to the Truss Manufacturer the Construction Design Documents and all revisions and supplements thereto.
- 4.2
- Review and approve the Truss Placement Plan and each Truss Design Drawing for conformance with the requirements and intent of the Construction Design Documents, and the effect of the Truss Placement Plan and each Truss Design Drawing on other trades involved in the construction of the structure and the effect of the other trades on the Trusses.
- 4.3
- Coordinate the review, approval and return of each Truss Design Drawing and the Truss Placement Plan by the Owner and Building Designer.
- 4.4
- Provide the approved Truss Design Drawings, approved Truss Placement Plans, and any supplemental information provided by the Truss Manufacturer to the individual or organization responsible for the installation of the Trusses.
- 4.5
- Comply with the field storage, handling, installation, permanent bracing, anchorage, connections and field assembly requirements of the Construction Design Documents.
- 4.6
- Determine and install the temporary bracing for the structure, including the Trusses.

5.0 TRUSS MANUFACTURER RESPONSIBILITIES

- 5.1 Communicate the design criteria from the Construction Design Documents to the Truss Designer.
- 5.2 Where required by the Construction Design Documents, prepare the Truss Placement Plan, providing as a minimum the location assumed for each Truss based on the Truss Manufacturer's interpretation of the Construction Design Documents.
- 5.4 Manufacture the Trusses in accordance with the final approved Truss Design Drawings using the quality criteria for Metal Plate Connected Wood Trusses established by the ANSI/TPI 1-1995 "National Design Standard for Metal Plate Connected Wood Truss Construction."

6.0 TRUSS DESIGNER RESPONSIBILITIES

- 6.1 Prepare the Truss Design Drawings in conformance with the requirements set forth in the latest approved edition of ANSI/TPI 1-1995 "National Design Standard for Metal Plate Connected Wood Truss Construction."
 - 6.2 For each Truss Design Drawing, set forth as a minimum the following:
 - 6.2.1 Slope or depth, span and spacing;
 - 6.2.2 Location of all joints;
 - 6.2.3 Required bearing widths;
 - 6.2.4 Design loads as applicable:
 - 6.2.4.1 Top chord live load (including snow loads);
 - 6.2.4.2 Top chord dead load;
 - 6.2.4.3 Bottom chord live load;
 - 6.2.4.4 Bottom chord dead load;
 - 6.2.4.5 Concentrated loads and their points of application; and
 - 6.2.4.6 Controlling wind and earthquake loads;
 - 6.2.5 Adjustments to lumber and metal connector plate design values for conditions of use;
- 6.2.6 Each reaction force and direction;
- 6.2.7 Metal connector plate type, size, thickness or gage, and the dimensioned location of each metal connector plate except where symmetrically located relative to the joint interface;
- 6.2.8 Lumber size, species, and grade for each member;
- 6.2.9 Connection Requirements for:
 - (a) Truss to Truss girder;
 - (b) Truss ply to ply; and
 - (c) Field spllices;
- 6.2.10 Calculated deflection ratio and/or maximum deflection for live and total load;
- 6.2.11 Maximum axial compression forces in the Truss members to enable the Building Designer to design the size, connections and anchorage of the permanent continuous lateral bracing. Forces may be shown on the Truss Design Drawing or on supplemental documents; and
- 6.2.12 Required permanent Truss member bracing location.

7.0 OTHER RESPONSIBILITIES

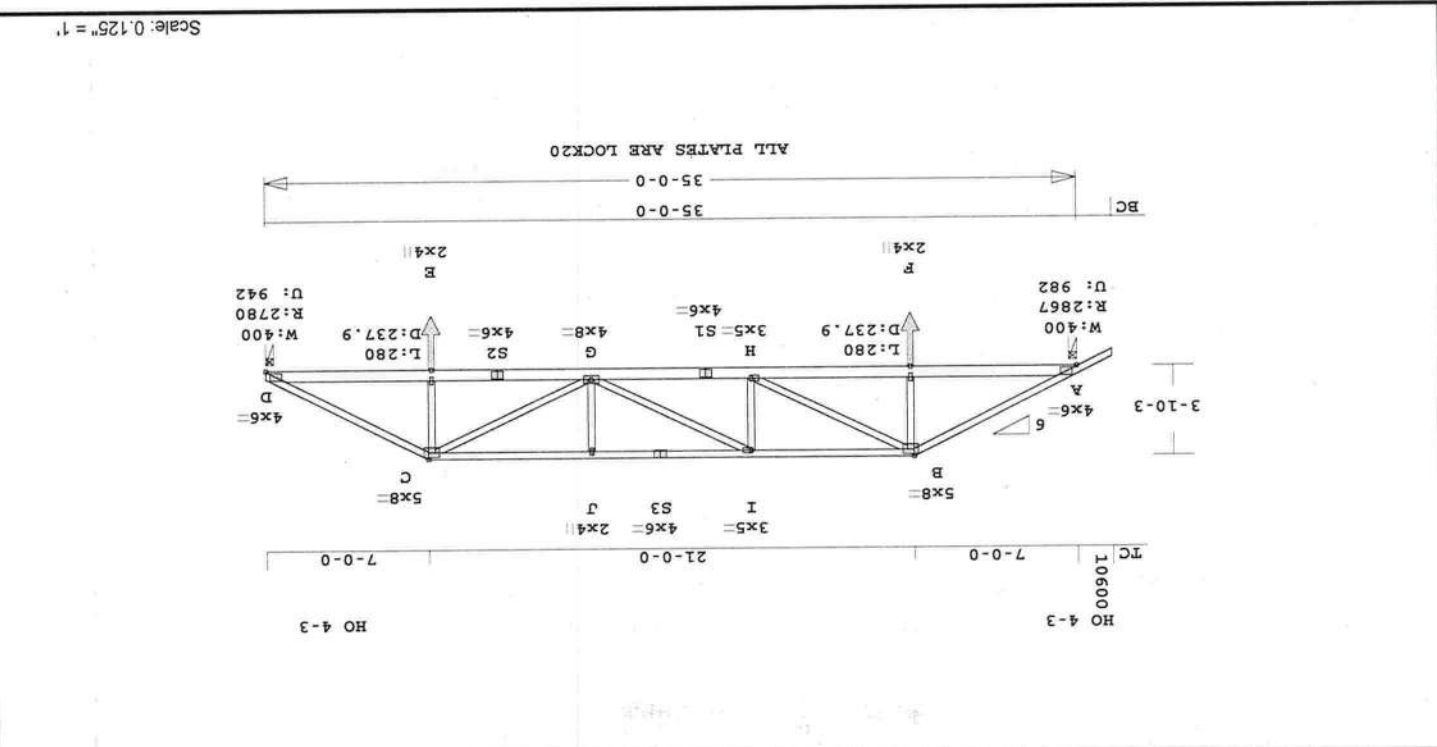
- 7.1 Any party who cuts or damages a truss shall be responsible for securing the engineering required for the repair and for subsequent costs.

Wood Truss Council of America's Objective

WTCA is committed to promoting the common interests of all engaged in the manufacture of wood trusses and related components to ensure growth, continuity, and increased professionalism in our industry. Fundamental to this is promoting the safe, economic, and structurally sound use of trusses in all applications.

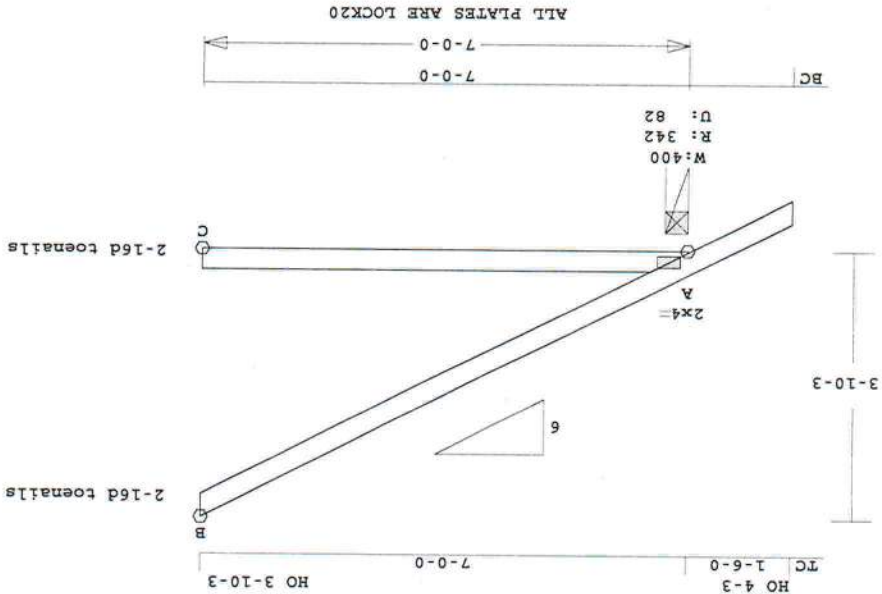
Job	051062	Mark	RI	Quan	1*2P	Type	HIPP	Span	350000	Pl-HI	6	Left OH	1-6-0	Right OH	0	Engineering
-----	--------	------	----	------	------	------	------	------	--------	-------	---	---------	-------	----------	---	-------------

SR 441 - LOT 1 LAKECITY



Job	051062	Mark	R2	Quan	30	Type	JACK	Span	70000	Pl-HI	6	Left OH	1-6-0	Right OH	0	Engineering
-----	--------	------	----	------	----	------	------	------	-------	-------	---	---------	-------	----------	---	-------------

SR 441 - LOT 1 LAKECITY



Scale: 0.375" = 1'

Occupancy Factor : 1.00
 Building Type: Enclosed
 TC Dead Load: 4.0 psf
 BC Dead Load: 6.0 psf
 Max comp. force 165 lbs
 Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss

Designer)

Gary Dounson, PE 35054

Gary Dounson & Associates,

Inc.

2830 NW 41st Street Suite D

Gainesville, FL 32606

(352)375-8593

CA 5201

Online Plus -- Version 18.0.014

RUN DATE: 14-NOV-05

CSI - Size - Lumber -

TC 0.53 2x 4 SP-#2
 BC 0.42 2x 4 SP-#2

Brace truss as follows:

O.C. From To
 TC Cont. 0-0-0 7-0-0
 BC Cont. 0-0-0 7-0-0

Loading Live Dead (psf)

TC 20.0 7.0
 BC 0.0 10.0
 Total 20.0 17.0
 Spacing 24.0"

Lumber Duration Factor 1.25

Plate Duration Factor 1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
 BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 8 Wind Load Case(s)
 Plus 1 UBC LL Load Case(s)

UT React Uplift Size Req'd

Lbs In-Sx In-Sx

A 342 82 4-0 1-0
 B 177 146 3-8 1-0
 C 129 0 3-8 1-0
 Hz = 84

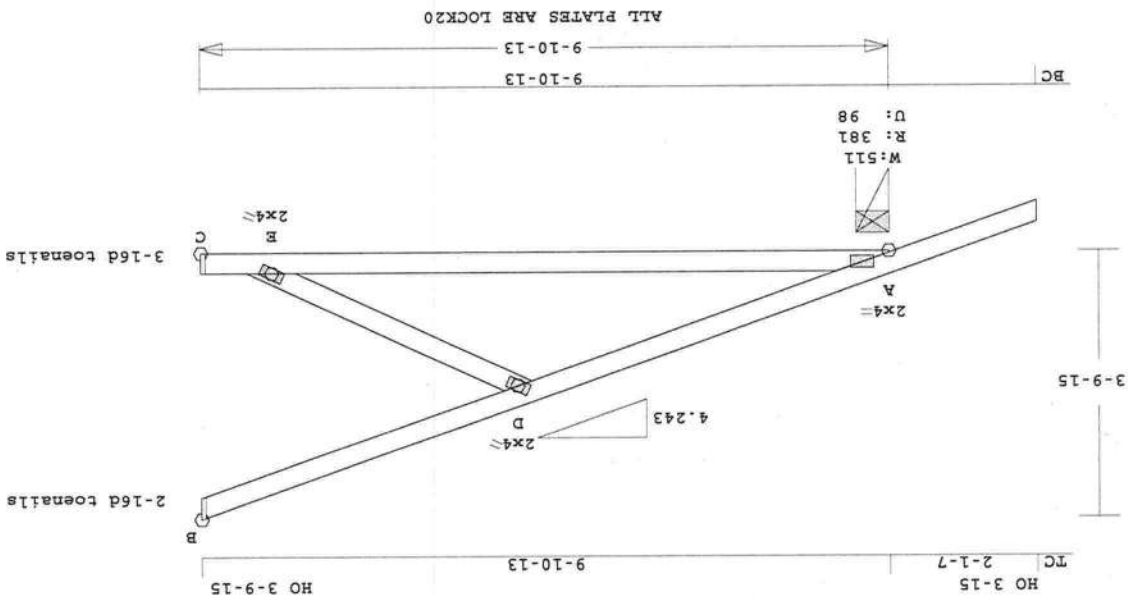
Membr CSI F Lbs Axl-CSI-Bnd

-----Top Chords-----
 A - B 0.53 165 C 0.00 0.53

OH Loading
 Soffit psf 2.0
 Design checked for 10 psf non-
 concurrent LL on BC.
 Wind Loads - ANSI / ASCE 7-02
 Truss is designed as
 Components and Claddings*
 for Exterior zone location.
 Wind Speed: 110 mph
 Mean Roof Height: 25-0
 Exposure Category: B

Job	051062	Mark	R3	Quan	6	Type	KING	Span	91013	Pl-HI	4.243	Left OH	2-1-7	Right OH	0	Engineering
-----	--------	------	----	------	---	------	------	------	-------	-------	-------	---------	-------	----------	---	-------------

SR 441 - LOT 1 LAKECITY



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 51.2 LBS

Notes: Trusses Manufactured by: RIDGWAY ROOF TRUSS

Analysis Conforms To: FBG2004

Girder King Jack Loading TC and BC

Setback 7-0-0

OH Loading

Soffit psf 2.0

Design checked for 10 psf non-concurrent TL on BC.

Use properly rated hangers for loads framing into girder

Wind Loads - ANSI / ASCE 7-02 Truss is designed as

Components and Claddings* for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 25-0

Exposure Category: B

Occupancy Factor: 1.00

Building Type: Enclosed

TC Dead Load: 4.0 psf

BC Dead Load: 6.0 psf

Max comp. force: 501 lbs

Quality Control Factor 1.25

FABRICATOR NOTES:

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At React Uplift Size Req'd

Lbs In-Sx In-Sx

382 99 5-11 1-0

Hz = 167

210 171 3-8 1-0

335 49 3-8 1-0

Hz = 90

Member CSI P Lbs Axl-Csi-Bnd

-----Top Chords-----

A-D 0.45 453 C 0.00 0.45

D-B 0.40 125 C 0.00 0.40

-----Bottom Chords-----

A-E 0.72 472 C 0.03 0.69

E-C 0.34 89 T 0.00 0.34

-----Webs-----

D-E 0.16 518 T

LT Defl -0.29" in A - E L/380

TL Defl -0.61" in A - E L/181

Shear // Grain in E - C 0.40

Plates for each ply each face.

ATL CONNECTOR PLATES

TO BE MANUFACTURED BY

ROBBINS ENGINEERING, INC.

Plate - LOCK 20 Ga, Gross Area

Plate - RHS 20 Ga, Gross Area

LT Type Plt Size X Y JSI

A LOCK 2.0x 4.0 Ctr Ctr 0.83

D LOCK 2.0x 4.0 Ctr Ctr 0.37

E LOCK 2.0x 4.0 Ctr Ctr 0.39

For proper installation of

toenails, refer to the 2001

National Design Specification

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WB 0.16 2x 4 SP-#3

TC 0.45 2x 4 SP-#2

BC 0.72 2x 4 SP-#2

-----Lumber-----

CSI -Size-

Run Date: 14-NOV-05

Online Plus -- Version 18.0.014 A

382 99 5-11 1-0

Hz = 167

210 171 3-8 1-0

335 49 3-8 1-0

Hz = 90

Member CSI P Lbs Axl-Csi-Bnd

-----Top Chords-----

A-D 0.45 453 C 0.00 0.45

D-B 0.40 125 C 0.00 0.40

-----Bottom Chords-----

A-E 0.72 472 C 0.03 0.69

E-C 0.34 89 T 0.00 0.34

-----Webs-----

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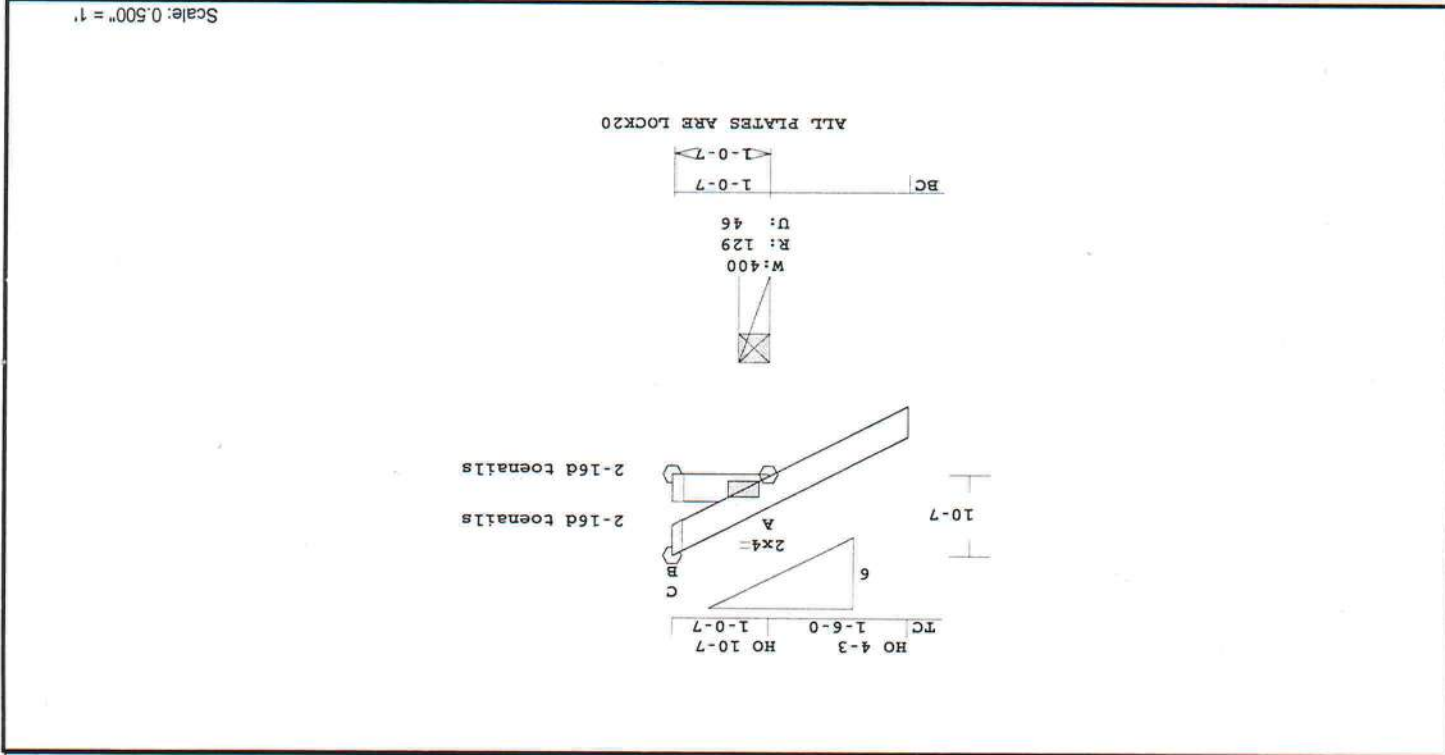
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Gary Dounson & Associates, Inc.

2830 NW 41st Street Suite D

Job	051062	Mark	R4	Quan	16	Type	JACK1	Span	10007	Pl-H1	6	Left OH	1-6-0	Right OH	0	Engineering
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Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 7.6 LBS

Scale: 0.500" = 1'

Occupancy Factor : 1.00
 Building Type: Enclosed
 TC Dead Load: 4.0 psf
 BC Dead Load: 6.0 psf
 Max comp. force 21 lbs
 Quality Control Factor 1.25

FABRICATOR NOTES:
 1. Delegated Engineer (Truss Designer)
 Gary Dounson, PR 35054
 Gary Dounson & Associates, Inc.
 2830 NW 41st Street Suite D
 Gainesville, FL 32606
 (352)375-8593
 CA 5201

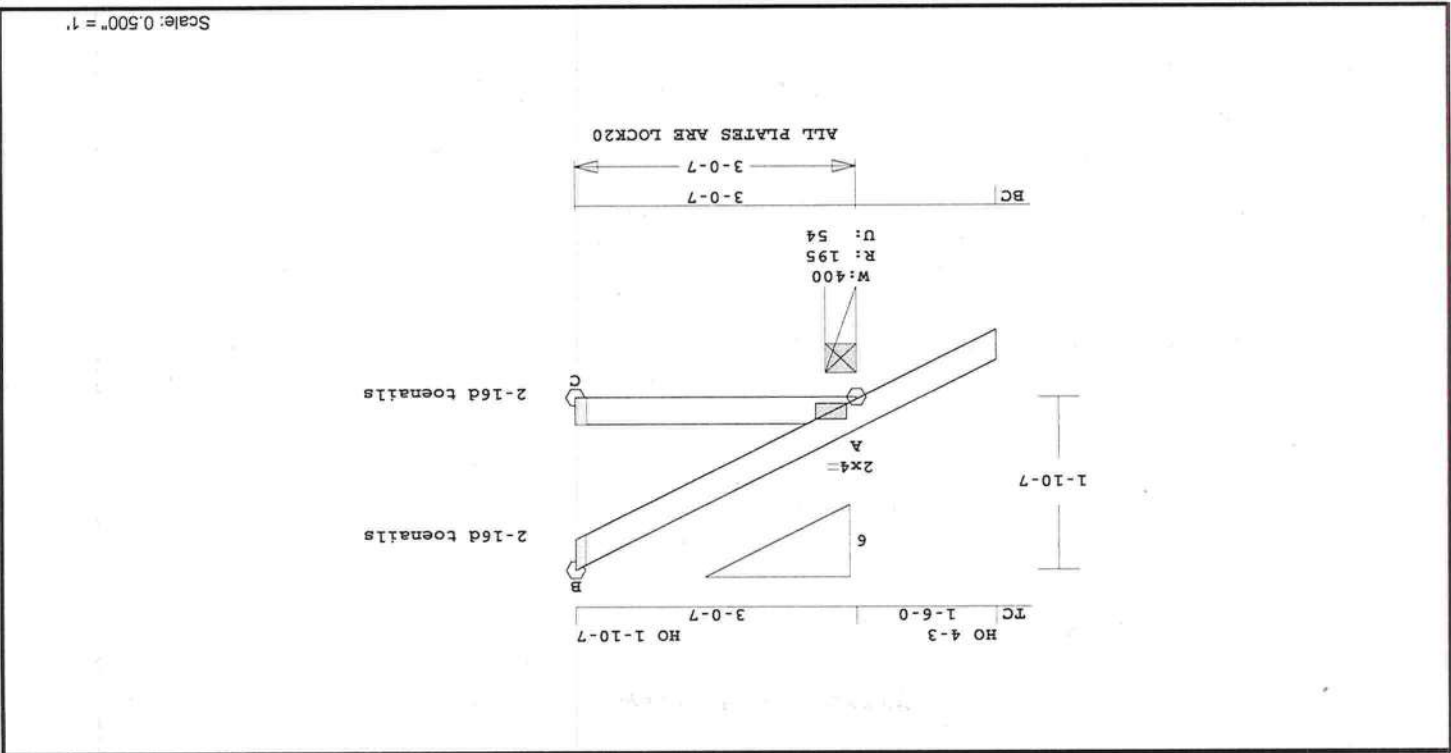
NOTES:
 Trusses Manufactured by:
 RIDGWAY ROOF TRUSSES
 Analysis Conforms To:
 FBC2004
 OH Loading
 Soffit psf 2.0
 Design checked for 10 psf non-concurrent LL on BC.
 Wind Loads - ANSI / ASCE 7-02
 Truss is designed as
 Components and Claddings*
 for Exterior zone location.
 Wind Speed: 110 mph
 Mean Roof Height: 25-0
 Exposure Category: B

Brace truss as follows:
 O.C. From To
 TC Cont. 0-0-0 1-0-7
 BC Cont. 0-0-0 1-0-7
 Loading Live Dead (psf)
 TC 20.0 7.0
 BC 0.0 10.0
 Total 20.0 17.0
 Spacing 24.0"
 Lumber Duration Factor 1.25
 Plate Duration Factor 1.25
 TC Pb=1.15 Fc=1.10 Ft=1.10
 BC Pb=1.10 Fc=1.10 Ft=1.10

Plates for each ply each face.
 ATL CONNECTOR PLATES
 TO BE MANUFACTURED BY
 ROBINS ENGINEERING, INC.
 Plate - LOCK 20 Ga, Gross Area
 Plate - RHS 20 Ga, Gross Area
 Ut Type Plt Size X Y JSI
 A LOCK 2.0x 4.0 Ctr Ctr 0.65

Plus 8 Wind Load Case(s)
 Plus 1 UBC LL Load Case(s)
 Ut React Uplift Size Req'd
 Lbs In-Sx In-Sx
 A 129 47 3-8 1-0 Hz = 20
 B 19 16 1-8 1-0 Hz = 20
 C 14 2 1-8 1-0 Hz = 11
 Membr CSI F Lbs Axl-CST-Bnd
 A-B 0.01 21 C 0.00 0.01
 A-B 0.01

Job	051062	SR 441 - LOT 1 LAKECITY
Mark	R5	
Quan	16	Type
Span	30007	PL-H1
Left OH	1-6-0	Right OH
0		Engineering



Robbins Engineering, Inc./Online Plus[®] APPROX. TRUSS WEIGHT: 15.6 LBS

-----Bottom Chords-----

A - C 0.10 35 T 0.00 0.10

LT Defl 0.00" in A - C L/999

TL Defl 0.00" in A - C L/999

Shear // Grain in A - B 0.13

Plates for each ply each face.

ALU CONNECTOR PLATES

TO BE MANUFACTURED BY

ROBBINS ENGINEERING, INC.

Plate - LOCK 20 Ga, Gross Area

Plate - RHS 20 Ga, Gross Area

rt Type Plt Size X Y JSI

A LOCK 2.0x 4.0 Ctr Ctr 0.65

Online Plus -- Version 18.0.014

RUN DATE: 14-NOV-05

CSI -Size- ---Lumber---

TC 0.11 2x 4 SP-#2

BC 0.10 2x 4 SP-#2

Brace truss as follows:

O.C. From To

TC Cont. 0-0-0 3-0-7

BC Cont. 0-0-0 3-0-7

Loading Live Dead (psf)

TC 20.0 7.0

BC 0.0 10.0

Total 20.0 17.0

Spacing 24.0"

Lumber Duration Factor 1.25

Plate Duration Factor 1.25

TC Pb=1.15 Fc=1.10 Ft=1.10

BC Pb=1.10 Fc=1.10 Ft=1.10

Plus 8 Wind Load Case(s)

Plus 1 UBC LL Load Case(s)

Ut React Uplift Size Req'd

Lbs In-Sx In-Sx

A 195 54 4-0 1-0

B 80 67 3-8 1-0

C 55 0 3-8 1-0

Hz = 66

Wind Loads - ANSI / ASCE 7-02

concurrent LL on BC.

Truss is designed as

Components and Claddings*

for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 25-0

Exposure Category: B

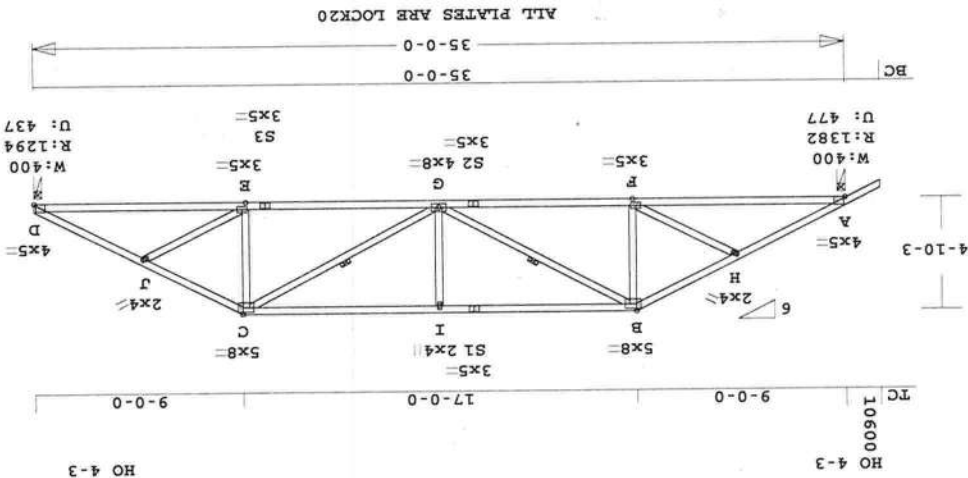
A - B 0.11 90 C 0.00 0.11

-----Top Chords-----

Member CSI P Lbs Axl-CST-Bnd

Job	051062	Mark	R7	Quan	1	Type	HIPP	Span	350000	Pl-H1	6	Left OH	1-6-0	Right OH	0	Engineering
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SR 441 - LOT 1 LAKECITY



Robbins Engineering, Inc./Online Plus[™] APPROX. TRUSS WEIGHT: 216.9 LBS

B - S1 0.79 2553 C 0.17 0.62
 S1-I 0.74 2553 C 0.04 0.70
 I - C 0.79 2553 C 0.17 0.62
 C - J 0.30 2136 C 0.02 0.28
 J - D 0.27 2358 C 0.14 0.13

-----Bottom Chords-----

A - F 0.60 2104 T 0.20 0.40
 F - S2 0.59 1917 T 0.19 0.40
 S2-G 0.52 1917 T 0.32 0.20
 G - S3 0.59 1917 T 0.19 0.40
 S3-E 0.59 1917 T 0.19 0.40
 E - D 0.60 2104 T 0.20 0.40

-----Webs-----

H - F 0.08 277 T
 F - B 0.10 383 T
 B - G 0.23 720 T
 G - I 0.16 512 C
 I - C 0.23 720 T
 C - G 0.10 383 T
 G - J 0.23 720 T
 J - U 0.08 277 T

LT Defl -0.16" in S2-G I/999
 TL Defl -0.34" in G -S3 I/999
 Shear // Grain in S1-I 0.33

Plates for each ply each face.

ALU CONNECTOR PLATES
 TO BE MANUFACTURED BY
 ROBBINS ENGINEERING, INC.

Plate - LOCK 20 Ga, Gross Area
 Plate - RHS 20 Ga, Gross Area

Jt Type Plt Size X Y JSI
 A LOCK 4.0x 5.0 Ctr 0.1 0.74
 H LOCK 2.0x 4.0 Ctr 0.44
 B LOCK 5.0x 8.0 Ctr-0.1 0.93
 S1 LOCK 3.0x 5.0 Ctr 0.89
 I LOCK 2.0x 4.0 Ctr 0.44
 C LOCK 5.0x 8.0 Ctr-0.1 0.93
 J LOCK 2.0x 4.0 Ctr 0.44
 D LOCK 4.0x 5.0 Ctr 0.1 0.74
 F LOCK 3.0x 5.0 Ctr 0.51
 S2 LOCK 3.0x 5.0 Ctr 0.74
 G LOCK 4.0x 8.0 Ctr 0.45
 S3 LOCK 3.0x 5.0 Ctr 0.91

Member CSI P Lbs AXI-CSI-Bnd
 -----Top Chords-----
 A - H 0.27 2358 C 0.14 0.13
 H - B 0.30 2136 C 0.02 0.28

Online Plus -- Version 18.0.014
 RUN DATE: 14-NOV-05

CSI -Size- -Lumber-
 TC 0.79 2x 4 SP-#2
 BC 0.60 2x 4 SP-#2
 WB 0.23 2x 4 SP-#3

Brace truss as follows:
 O.C.
 From To
 TC Cont. 0-0-0 35-0-0
 BC Cont. 0-0-0 35-0-0
 WB 1 rows CLB on B - G
 WB 1 rows CLB on G - C
 Attach CLB with (2)-10d nails
 at each web.

Loading Live Dead (psf)
 TC 20.0 7.0
 BC 0.0 10.0
 Total 20.0 17.0
 Spacing 24.0"

Lumber Duration Factor 1.25
 Plate Duration Factor 1.25
 TC Fb=1.15 Fc=1.10 Ft=1.10
 BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
 Plus 1 UBC LL Load Case(s)

Jt React Uplift Size Req'd
 Lbs In-Sx In-Sx
 A 1382 478 4-0 1-10
 D 1295 438 4-0 1-8
 Hz = -96
 Hz = 97

NOTES:

Trusses Manufactured by:

RIDGEWAY ROOF TRUSSES

Analysis Conforms To:

FBC2004

OH Loading

Design checked for 10 psf non-concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as

Components and Claddings*
 for Exterior zone location.
 Wind Speed:
 110 mph

Mean Roof Height: 25-0

Exposure Category: B

Occupancy Factor: 1.00

Building Type: Enclosed

TC Dead Load:
 4.0 psf

BC Dead Load:
 6.0 psf

Max comp. force 2553 Lbs

Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss Designer)

Gary Dounson, PE 35054

Gary Dounson & Associates, Inc.

2830 NW 41st Street Suite D Gainesville, FL 32606

(352)375-8593

CA 5201

[illegible]

LC	20.0	LC	20.0	LC	20.0	LC	20.0
TC	7.0	TC	7.0	TC	7.0	TC	7.0
HC	0.0	HC	0.0	HC	0.0	HC	0.0
Total	20.0	Total	20.0	Total	20.0	Total	20.0
Spacing	24.0"	Spacing	24.0"	Spacing	24.0"	Spacing	24.0"
Lumber Duration Factor	1.25	Lumber Duration Factor	1.25	Lumber Duration Factor	1.25	Lumber Duration Factor	1.25
Plate Duration Factor	1.25	Plate Duration Factor	1.25	Plate Duration Factor	1.25	Plate Duration Factor	1.25
TC Fb=1.15	Fc=1.10	TC Fb=1.15	Fc=1.10	TC Fb=1.15	Fc=1.10	TC Fb=1.15	Fc=1.10
HC Fb=1.10	Fc=1.10	HC Fb=1.10	Fc=1.10	HC Fb=1.10	Fc=1.10	HC Fb=1.10	Fc=1.10

Brace truss as follows:

O.C.	From	To
TC Cont.	0 - 0	0 35 - 0 - 0
BC Cont.	0 - 0	0 35 - 0 - 0
WB 1 rows	CLB on B - H	
WB 1 rows	CLB on H - C	
Attach CLB	with (2) 10d nails	at each web.

TC	0.42	2x	4	SP-#2
BC	0.45	2x	4	SP-#2
WB	0.21	2x	4	SP-#3

CS1 -size- ----lumber

Online Plus -- Version 18.0.014
RUN DATE: 14-NOV-05

[illegible]

KOBBIUS Engineering, Inc./Online Plus[®] APPROX. TRUSS WEIGHT: 232.9 LBS

NOTES:

Trusses Manufactured by:

RIDGWAY ROOF TRUSS

Analysis Conforms To:

FBC2004

OH Loading

Soffit pft 2.0

Design checked for 10 pft non-concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as

Components and Claddings*

for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 25'-0"

Exposure Category: B

Occupancy Factor : 1.00

Building Type: Enclosed

TC Dead Load: 4.0 pft

BC Dead Load: 6.0 pft

Max comp. force 2356 lbs

Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss Designer)

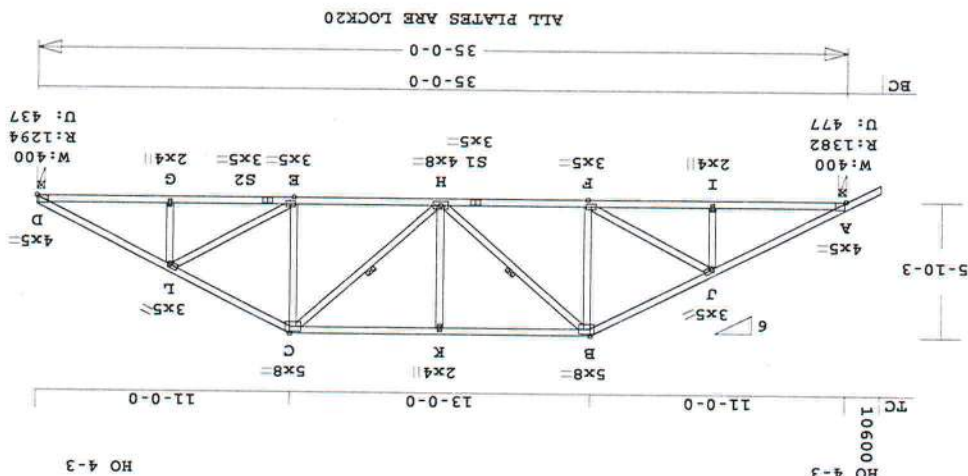
Gary Dounson, PE 35054

Gary Dounson & Associates, Inc.

2830 NW 41st Street Suite D Gainesville, FL 32606

(352) 375-8593

CA 5201

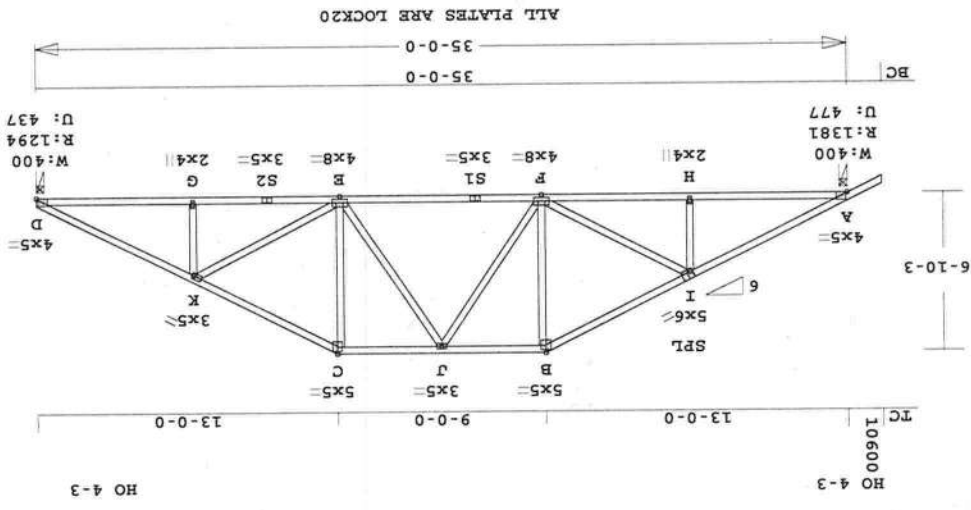


Scale: 0.125" = 1'

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
051062	R8	1	HIPP	350000	6	1 - 6 - 0	0	

Job	051062	Mark	R9	Quan	1	Type	HIPP	Span	350000	Pl-HI	6	Left OH	1-6-0	Right OH	0	Engineering
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SR 441 - LOT 1 LAKECITY



Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 231.9 LBS

NOTES:
 Trusses Manufactured by:
 RIDGWAY ROOF TRUSS
 Analysis Conforms To:
 FBC2004
 OH Loading
 Design checked for 10 psf non-concurrent LL on BC.
 Wind Loads - ANSI / ASCE 7-02
 Truss is designed as
 Components and Claddings*
 for Exterior zone location.
 Wind Speed:
 Mean Roof Height: 25'-0
 Exposure Category: B
 Occupancy Factor: 1.00
 Building Type: Enclosed
 TC Dead Load:
 BC Dead Load:
 Max comp. force
 2315 lbs
 Quality Control Factor 1.25

FABRICATOR NOTES:
 1. Delegated Engineer (Truss Designer)
 Gary Dounson, PE 35054
 Gary Dounson & Associates, Inc.
 2830 NW 41st Street Suite 1
 Gainesville, FL 32606
 (352) 375-8593
 CA 5201

Online Plus -- Version 18.0.014
 RUN DATE: 14-NOV-05

CSI - Size - Lumber -
 TC 0.47 2x 4 SP-#2
 BC 0.55 2x 4 SP-#2
 WB 0.38 2x 4 SP-#3

Brace truss as follows:
 O.C. From To
 TC Cont. 0-0-0 35-0-0
 BC Cont. 0-0-0 35-0-0

Loading Live Dead (psf)
 TC 20.0 7.0
 BC 0.0 10.0
 Total 20.0 17.0
 Spacing 24.0"

Lumber Duration Factor 1.25
 Plate Duration Factor 1.25
 TC Pb=1.15 Fc=1.10 Ft=1.10
 BC Pb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
 Plus 1 UBC LL Load Case(s)

Jt React Uplift Size Req'd
 lbs In-Sx In-Sx
 A 1382 478 4-0 1-10
 D 1295 438 4-0 1-8
 Hz = 142

Member CSI P lbs Axl-CSI-Bnd
 -----Top Chords-----
 A-I 0.47 2315 C 0.14 0.33
 I-B 0.44 1813 C 0.11 0.33
 B-J 0.24 1617 C 0.12 0.12
 J-C 0.24 1617 C 0.12 0.12
 C-K 0.44 1813 C 0.11 0.33

Bottom Chords
 K-D 0.47 2315 C 0.14 0.33
 D-L 0.45 2074 T 0.34 0.11
 L-H 0.55 2074 T 0.21 0.34
 H-F 0.55 2074 T 0.21 0.34
 F-S1 0.51 1699 T 0.17 0.34
 S1-E 0.51 1699 T 0.17 0.34
 E-S2 0.55 2074 T 0.21 0.34
 S2-G 0.42 2074 T 0.34 0.08
 G-D 0.45 2074 T 0.34 0.11

Web
 H-I 0.06 237 T
 I-F 0.38 518 C
 F-B 0.16 529 T
 B-U 0.13 148 C
 U-E 0.13 148 C
 E-C 0.16 529 T
 C-K 0.38 518 C
 K-G 0.06 237 T

LT Defl -0.14" in S1-E L/999
 TL Defl -0.33" in S1-E L/999
 Shear // Grain in A-I 0.21

Plates for each ply each face.
 ALL CONNECTOR PLATES
 TO BE MANUFACTURED BY
 ROBINS ENGINEERING, INC.
 Plate - LLOCK 20 Ga, Gross Area
 Plate - RHS 20 Ga, Gross Area
 Jt Type Plt Size X Y JSI
 A LOCK 4.0x 5.0 Ctr 0.1 0.74
 I LOCK 5.0x 6.0 0.3-0.7 0.63
 B LOCK 5.0x 5.0 Ctr-0.2 0.99
 J LOCK 3.0x 5.0 Ctr 0.49
 C LOCK 5.0x 5.0 Ctr-0.2 0.99
 K LOCK 3.0x 5.0 Ctr 0.57
 D LOCK 4.0x 5.0 Ctr 0.1 0.74
 H LOCK 2.0x 4.0 Ctr 0.44
 F LOCK 4.0x 8.0 Ctr 0.45
 S1 LOCK 3.0x 5.0 Ctr 0.84
 E LOCK 4.0x 8.0 Ctr 0.45
 S2 LOCK 3.0x 5.0 Ctr 0.74
 G LOCK 2.0x 4.0 Ctr 0.44


```

Online Plus -- Version 18.0.014
RUN DATE: 14-NOV-05
CSI -Size- ----Lumber-----
TC 0.59 2x 4 SP-#2
BC 0.53 2x 4 SP-#2
WB 0.19 2x 4 SP-#3
Brace truss as follows:
O.C.
From
To
TC Cont. 0-0 0 35- 0-0
BC Cont. 0-0 0 35- 0-0
WB 1 rows CLB on I -F
WB 1 rows CLB on B -E
WB 1 rows CLB on E -J
Attach CLB with (2) -10d nails
at each web.
Loading Live Dead (psf)
TC 20.0 0.0
BC 0.0 10.0
Total 20.0 17.0
Spacing 37.0
Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10
Plus 9 Wind Load Case(s)
Plus 1 UBC LT Load Case(s)
Jt React Uplift Size Req'd
Lbs In-Sx In-Sx
A 1382 478 4- 0 1-10
D 1295 438 4- 0 1- 8
Hz = -163
Hz = 164
Member CSI P Lbs Axl-CST-Bnd
-----Top Chords-----

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Online Plus -- Version 18.0.014
RUN DATE: 14-NOV-05

A-I	0.59	2272	C	0.14	0.45
I-B	0.56	1649	C	0.11	0.45
B-C	0.27	1467	C	0.11	0.16
C-J	0.56	1649	C	0.11	0.45
J-D	0.59	2272	C	0.14	0.45
-----Bottom Chords-----					
A-H	0.53	2038	T	0.34	0.19
H-F	0.53	2038	T	0.34	0.19
F-S1	0.34	1462	T	0.24	0.10
S1-E	0.35	1462	T	0.24	0.11
E-S2	0.45	2039	T	0.34	0.11
S2-G	0.52	2039	T	0.34	0.18
G-D	0.52	2039	T	0.34	0.18
-----Webs-----					
H-I	0.08	317	T		
I-F	0.19	646	C		
F-B	0.13	447	T		
B-E	0.03	115	T		
E-C	0.14	444	T		
C-J	0.19	647	C		
G-J	0.08	316	T		

LT Defl	-0.11"	in SI-E			
TL Defl	-0.24"	in H-F			
Shear	// Grain	in A-I			
				0.24	L/999
Plates for each ply face.					
ALL CONNECTOR PLATES					
TO BE MANUFACTURED BY					
ROBBINS ENGINEERING, INC.					
Plate - LOCK 20 Ga, Gross Area					
Plate - RHS 20 Ga, Gross Area					
Plate -					
LT Defl					
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Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 230.3 LBS

S2 LOCK	3.0x	5.0	CTR	CTR	0.74
G LOCK	2.0x	4.0	CTR	CTR	0.44

NOTES:

RIDGWAY ROOF TRUSS
Analysis conforms To:

NO Loading
Softfit psf 2.0

concurrent LT on BC.
Wind Loads - ANSI / ASCE 7-02

*Components and Claddings

Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B

Occupancy Factor : 1.00
Building Type: Enclosed
TC Data Load

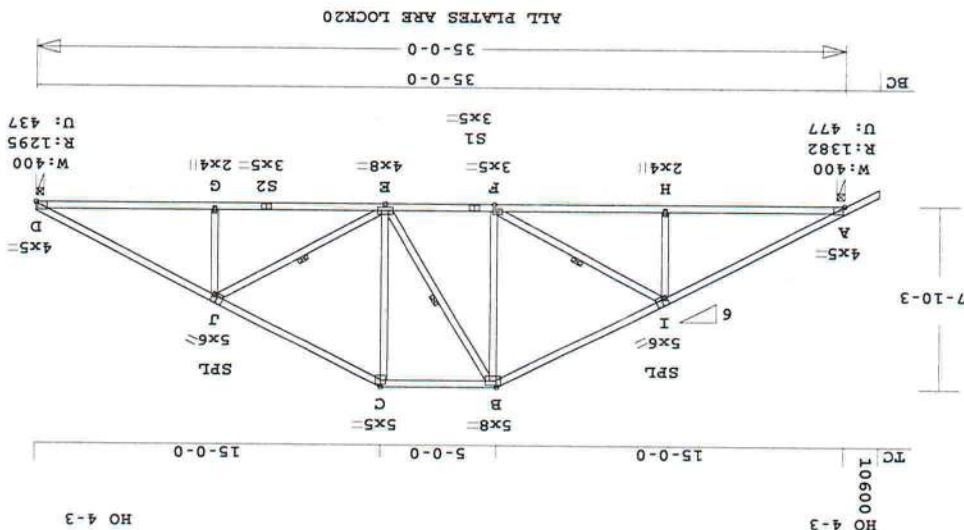
BC Dead Load: 6.0 psf
Max comp. force 2272 lbs

FABRICATOR NOTES:

Designer)
Gary Dounson, PE 35054

2830 NW 41st Street Suite 1
Inc.

CA 5201
(352) 375-8593
GAINESVILLE, FL 32608



Scale: 0.125" = 1'

Job	051062	R10	1	Type	Span	6	1-6-0	Right OH	Engineering
SR 441 - LOT 1 LAKECITY									

Robbins Engineering, Inc./Online Plus SM APPROX. TRUSS WEIGHT		Online Plus -- Version 18.0.014		RUN DATE: 14-NOV-05	
I-J	0.27	1848	C	0.01	0.26
J-O	0.27	1365	C	0.01	0.26
O-K	0.33	1367	C	0.00	0.33
K-L	0.36	1760	C	0.00	0.36
L-M	0.42	1807	C	0.06	0.37

NOTES:

Trusses Manufactured by: RIDGWAY ROOF TRUSSES

Analysis Conforms To: FBC2004

Provide connection to bearing for 385 lbs Horiz Reaction Design checked for 10 psf non-concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02 Truss is designed as Components and Claddings* for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 25'-0"

Exposure Category: B

Occupancy Factor: 1.00

Building Type: Enclosed

TC Dead Load: 4.0 psf

BC Dead Load: 6.0 psf

Max comp. force 2270 lbs

Quality Control Factor 1.25

FABRICATOR NOTES:

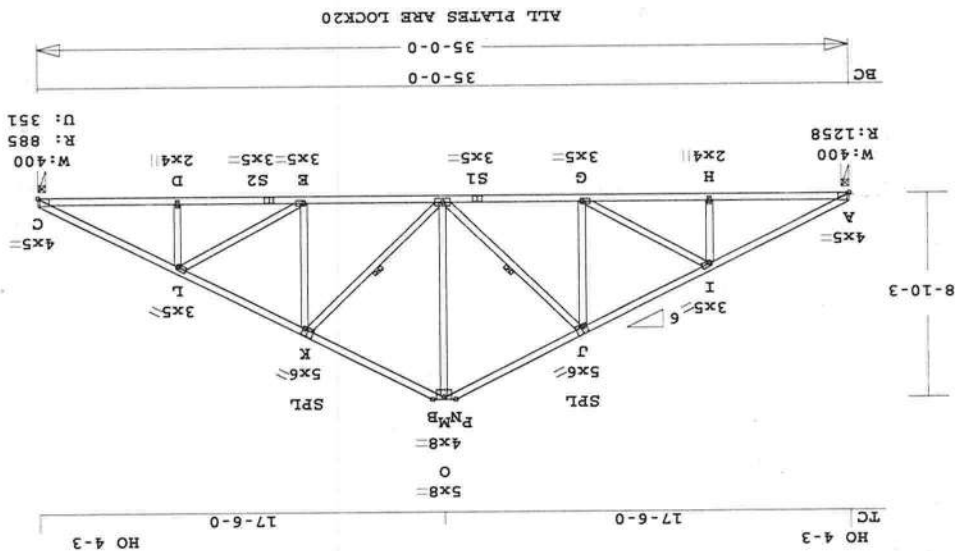
1. Delegated Engineer (Truss Designer)

Gary Downson, PE 35054

Gary Downson & Associates, Inc.

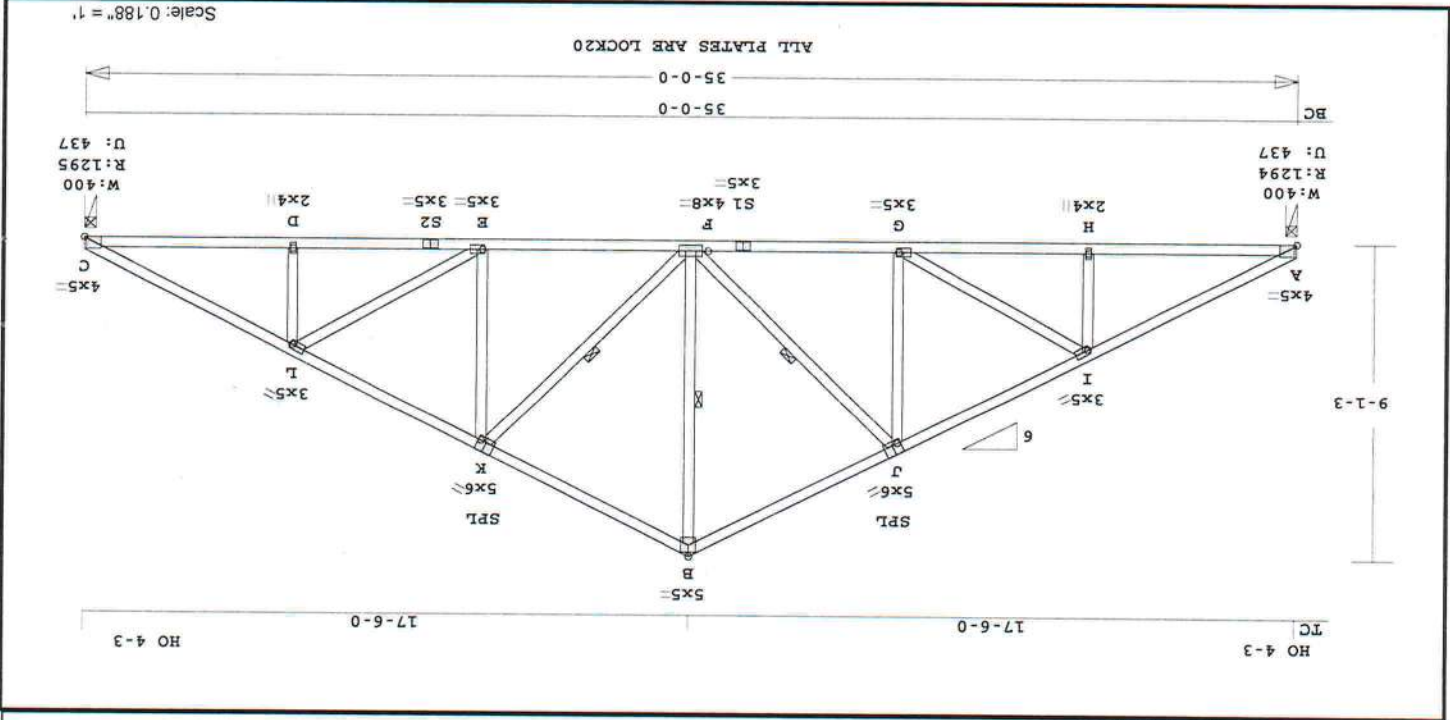
2830 NW 41st Street Suite D Gainesville, FL 32606

CA 5201 (352) 375-8593



Job	051062	Mark	R12	Quan	6	Type	TR	Span	350000	P1-H1	6	Left OH	0	Right OH	0	Engineering
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SR 441 - LOT 1 LAKECITY



Robbins Engineering, Inc./Online Plus[™] APPROX. TRUSS WEIGHT: 240.6 LBS

Notes:
Trusses Manufactured by:
RIDGWAY ROOF TRUSSES
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent TL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor: 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 2348 lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A LOCK 4.0x 5.0 Ctr 0.1 0.74
I LOCK 3.0x 5.0 Ctr 0.57
J LOCK 5.0x 6.0 Ctr 0.7 0.63
B LOCK 5.0x 5.0 Ctr 0.66
K LOCK 5.0x 6.0-0.3-0.7 0.63
L LOCK 3.0x 5.0 Ctr 0.57
C LOCK 4.0x 5.0 Ctr 0.1 0.74
H LOCK 2.0x 4.0 Ctr 0.44
G LOCK 3.0x 5.0 Ctr 0.51
S1 LOCK 3.0x 5.0 Ctr 0.74

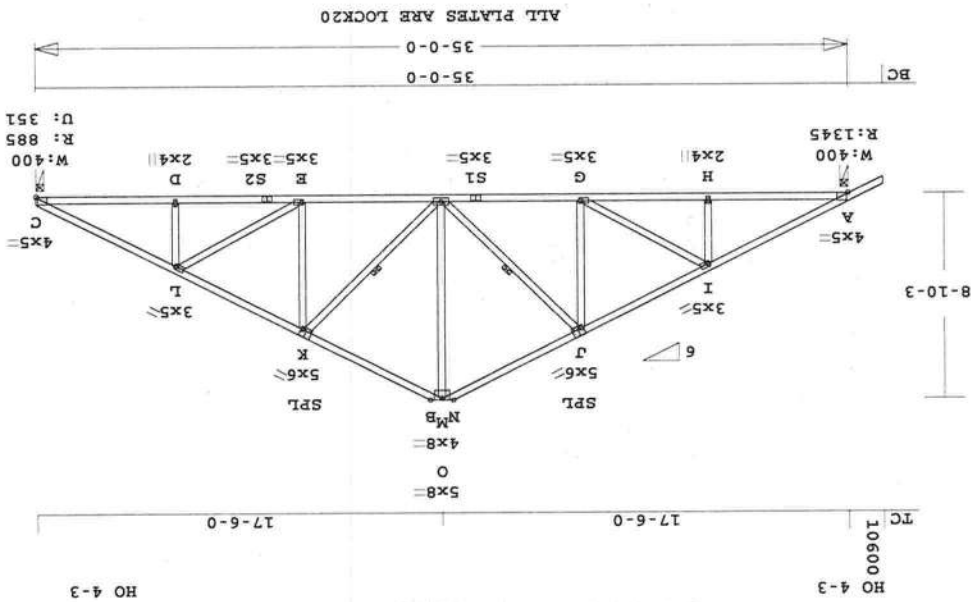
Plates for each ply each face.
LT Defl -0.12" in SI-F L/999
TL Defl -0.23" in F-E L/999
Shear // Grain in J-B 0.21

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)
Jt React Uplift Size Req'd
Lbs In-Sx In-Sx
A 1295 438 4-0 1-8
C 1295 438 4-0 1-8
Hz = 191

Membr CSI P Lbs Axl-CSI-Bnd
A-I 0.37 2348 C 0.15 0.22
I-J 0.40 1929 C 0.13 0.27

Job	051062	Mark	R13	Quan	1	Type	TR	Span	350000	Pl-HI	6	Left OH	1-6-0	Right OH	0	Engineering
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SR 441 - LOT 1 LAKECITY



Online Plus -- Version 18.0.014
 RUN DATE: 14-NOV-05
 CSI - Size - Lumber -
 TC 0.43 2x 4 SP-#2
 BC 0.45 2x 4 SP-#2
 WB 0.28 2x 4 SP-#3
 Brace truss as follows:
 O.C. From To
 TC Cont. 0-0-0 28-11-11
 BC Cont. 0-0-0 28-11-11
 WB 1 rows CLB on J-M
 WB 1 rows CLB on M-K
 Attach CLB with (2)-10d nails
 at each web.
 Loading Live Dead (psf)
 TC 20.0 7.0
 BC 0.0 10.0
 Total 20.0 17.0
 Spacing 24.0"
 Lumber Duration Factor 1.25
 Plate Duration Factor 1.25
 TC Fb=1.15 Fc=1.10 Ft=1.10
 BC Fb=1.10 Fc=1.10 Ft=1.10
 Plus 9 Wind Load Case(s)
 Plus 1 UBC LL Load Case(s)
 Jt React Uplift Size Req'd
 A 1345 Lbs In-Sk In-Sk
 C 886 352 4-0 1-1
 Hz = -384
 Hz = -140
 Membr CSI P Lbs Ax1-CSI-Bnd
 A-I 0.25 2270 C 0.06 0.19
 I-J 0.27 1848 C 0.01 0.26

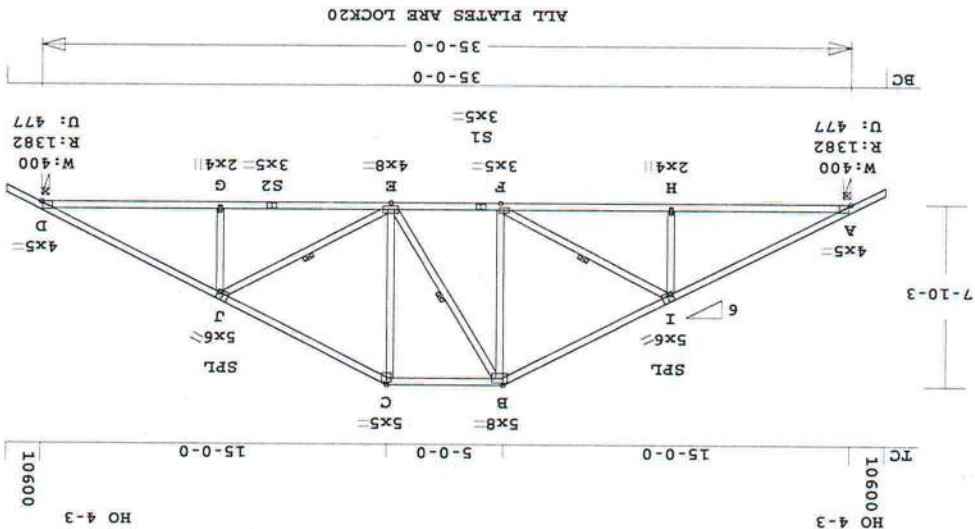
Robbins Engineering, Inc./Online Plus™ @ 1996-2005 Version 18.0.014 Engineering - Porrait 11/15/2005 8:07:03 AM Page 1
 Plates for each ply each face.
 ALL CONNECTOR PLATES
 TO BE MANUFACTURED BY
 ROBBINS ENGINEERING, INC.
 Plate - LOCK 20 ga, Gross Area
 Plate - RHS 20 ga, Gross Area
 Jt Type Plt Size X JSI
 A LOCK 4.0x 5.0 Ctr 0.1 0.74
 I LOCK 3.0x 5.0 Ctr 0.1 0.57
 J LOCK 5.0x 6.0 0.3-0.7 0.63
 K LOCK 5.0x 8.0 Ctr-1.6 0.85
 O LOCK 5.0x 6.0 0.3-0.7 0.63
 L LOCK 3.0x 5.0 Ctr 0.1 0.57
 C LOCK 4.0x 5.0 Ctr 0.1 0.74
 H LOCK 2.0x 4.0 Ctr 0.44
 G LOCK 3.0x 5.0 Ctr 0.51

NOTES:
 Trusses Manufactured by:
 RIDGWAY ROOF TRUSS
 Analysis Conforms To:
 FB2004
 OH Loading
 Soffit psf 2.0
 Provide connection to bearing
 for 385 lbs Horiz Reaction
 Design checked for 10 psf non-
 concurrent LL on BC.
 Wind Loads - ANSI / ASCE 7-02
 Truss is designed as
 Components and Claddings*
 for Exterior zone location.
 Wind Speed:
 110 mph
 Mean Roof Height: 25-0
 Exposure Category: B
 Occupancy Factor: 1.00
 Building Type: Enclosed
 TC Dead Load: 4.0 psf
 BC Dead Load: 6.0 psf
 Max comp. force 2270 lbs
 Quality Control Factor 1.25
 FABRICATOR NOTES:
 1. Delegated Engineer (Truss
 Designer)
 Gary Dounson, PE 35054
 Gary Dounson & Associates,
 Inc.
 2830 NW 41st Street Suite D
 Gainesville, FL 32606
 (352)375-8593
 CA 5201

Scale: 0.125" = 1"

Job	051062	Mark	R14	Quan	1	Type	HIPP	Span	350000	Pl-HI	6	Left OH	1-6-0	Right OH	1-6-0	Engineering
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SR 441 - LOT 1 LAKECITY



Online Plus -- Version 18.0.014
RUN DATE: 14-NOV-05

CSI -Size- Lumber-----

TC 0.59 2x 4 SP-#2
BC 0.53 2x 4 SP-#2
WB 0.19 2x 4 SP-#3

Brace truss as follows:

TC Cont. 0-0-0 35-0-0
BC Cont. 0-0-0 35-0-0

WB 1 rows CLB on I-F
WB 1 rows CLB on B-E
WB 1 rows CLB on E-J

Attach CLB with (2)-10d nails
at each web.

Loading Live Dead (psf)
TC 20.0
BC 0.0

Spacing
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Ut React Uplift Size Req'd
Lbs In-Sx In-Sx
A 1382 478 4-0 1-10

Member CSI P Lbs Axl-Csi-Bnd
D 1382 478 4-0 1-10

-----Top Chords-----
Hz = 164

A-I	0.59	2272	C	0.14	0.45
I-B	0.56	1649	C	0.11	0.45
B-C	0.27	1467	C	0.11	0.16
C-J	0.56	1649	C	0.11	0.45
J-D	0.59	2272	C	0.14	0.45
-----Bottom Chords-----					
A-H	0.53	2038	T	0.34	0.19
H-F	0.53	2038	T	0.34	0.19
F-S1	0.34	1462	T	0.24	0.10
S1-E	0.35	1462	T	0.24	0.11
E-S2	0.45	2039	T	0.34	0.11
S2-G	0.52	2039	T	0.34	0.18
G-D	0.52	2039	T	0.34	0.18
-----Webs-----					
H-I	0.08	317	T		
I-F	0.19	646	C		
F-B	0.13	447	T		
B-E	0.03	115	T		
E-C	0.14	444	T		
C-J	0.19	647	C		
J-D	0.08	316	T		
Plates for each ply each face.					
TO BE MANUFACTURED BY					
ROBBINS ENGINEERING, INC.					
Plate - LOCK 20 Ga, Gross Area					
Plate - RHS 20 Ga, Gross Area					
Ut Type Plt Size X JSI					
A	LOCK	4.0x	5.0	Ctr	0.1 0.74
I	LOCK	5.0x	6.0	0.3-0.7	0.63
B	LOCK	5.0x	8.0	Ctr	0.1 0.93
C	LOCK	5.0x	5.0	Ctr	0.2 0.99
J	LOCK	5.0x	6.0	0.3-0.7	0.63
D	LOCK	4.0x	5.0	Ctr	0.1 0.74
H	LOCK	2.0x	4.0	Ctr	0.44
F	LOCK	3.0x	5.0	Ctr	0.51
S1	LOCK	3.0x	5.0	Ctr	0.74
E	LOCK	4.0x	8.0	Ctr	0.45

NOTES:

Trusses Manufactured by:

RIDGWAY ROOF TRUSS

Analysis Conforms To:

FB22004

OH Loading

Soffit pft 2.0

Design checked for 10 pft non-concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as

Components and Claddings* for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 25-0

Exposure Category: B

Occupancy Factor: 1.00

Building Type: Enclosed

TC Dead Load: 4.0 pft

BC Dead Load: 6.0 pft

Max comp. Force 2272 Lbs

Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss Designer)

Gary Dounson, PE 35054

Gary Dounson & Associates, Inc.

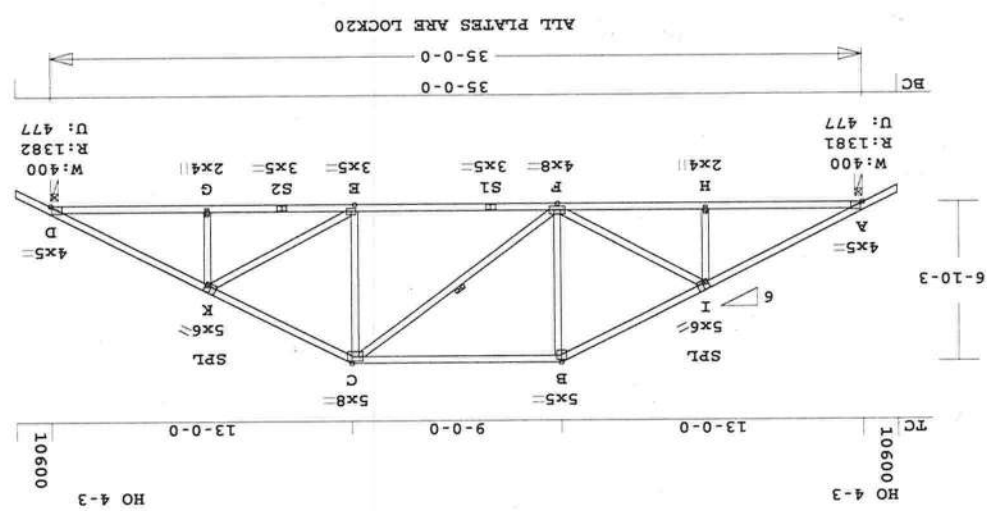
2830 NW 41st Street Suite D Gainesville, FL 32606

(352)375-8593

CA 5201

Scale: 0.125" = 1'

Job	051062	SR 441 - LOT 1 LAKECITY
Mark	RIS	
Quan	1	
Type	HIPP	
Span	350000	
Pl-HI	6	
Left OH	1 - 6 - 0	
Right OH	1 - 6 - 0	
Engineering		



Robbins Engineering, Inc./Online plus* APPROX. TRUSS WEIGHT: 226.4 LBS
B - C 0.88 1637 C 0.07 0.81 NOTES:

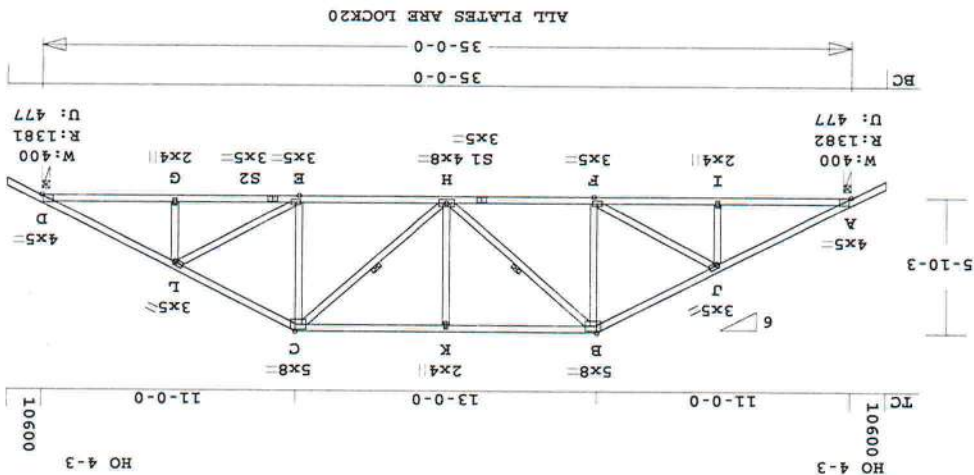
Trusses Manufactured by:		RIDGEWAY ROOF TRUSS		Analysis Conforms To:		FBC2004		OH Loading		Soffit psf 2.0		Design checked for 10 psf non-concurrent LL on BC.		Wind Loads - ANSI / ASCE 7-02		Truss is designed as		Components and Claddings*		for Exterior zone location.		Wind Speed: 110 mph		Mean Roof Height: 25-0		Exposure Category: B		Occupancy Factor: 1.00		Building Type: Enclosed	
C-K	0.44	1823	C	0.01	0.43																										
-----Bottom Chords-----																															
A-H	0.46	2065	T	0.34	0.12																										
H-F	0.55	2065	T	0.34	0.21																										
F-S1	0.50	1631	T	0.16	0.34																										
S1-E	0.49	1631	T	0.16	0.33																										
E-S2	0.54	2064	T	0.21	0.33																										
S2-G	0.43	2064	T	0.34	0.09																										
G-D	0.46	2064	T	0.34	0.12																										
-----Webs-----																															
H-I	0.06	237	T																												
I-F	0.36	485	C																												
F-B	0.12	478	T																												
F-C	0.04	116	T																												
E-C	0.12	477	T																												
E-K	0.36	484	C																												

CSI -Size- ---Lumber--- TC 0.88 2x 4 SP-#2 BC 0.55 2x 4 SP-#2 WB 0.36 2x 4 SP-#3 Brace truss as follows: From To TC Cont. 0-0-0 35-0-0 BC Cont. 0-0-0 35-0-0 WB 1 rows CLB on F-C Attach CLB with (2)-10d nails at each web.																															
Online Plus -- Version 18.0.014 RUN DATE: 14-NOV-05																															

Loading	Live	Dead	(psf)	G - K	0.06	239 T
TC	20.0	7.0				
BC	0.0	10.0				
Total	20.0	17.0	37.0			
Spacing			24.0"			
Lumber Duration Factor	1.25					
Plate Duration Factor	1.25					
TC Fb=1.15	Fc=1.10	Ft=1.10				
BC Fb=1.10	Fc=1.10	Ft=1.10				
Plus 9 Wind Load Case(s)						
Plus 1 UBC LT Load Case(s)						
at React Upflift Size Req'd						
I LOCK	5.0x	6.0	0.3-0.7	0.63		
A LOCK	4.0x	5.0	Ctr 0.1	0.74		
at Type Plt Size X Y JSI						
Plate - RHS	20 ga,	Gross Area				
Plate - LOCK	20 ga,	Gross Area				
ROBBINS ENGINEERING, INC.						
TO BE MANUFACTURED BY						
ATL CONNECTOR PLATES						
Plates for each ply each face.						
Shear // Grain in B - C			0.30			
LT Defl - 0.14" in SI-E	L/999					
TL Defl - 0.34" in SI-E	L/999					
Max comp. force						
2307 lbs						
Quality Control Factor	1.25					
TC Dead Load:						
4.0 psf						
BC Dead Load:						
6.0 psf						
Max comp. force						
2307 lbs						
Quality Control Factor	1.25					
FABRICATOR NOTES:						
1. Delegated Engineer (Truss Designer)						
Gary Dounson, PE 35054						
Gary Dounson & Associates, Inc.						
2830 NW 41st Street Suite D Gainesville, FL 32606						
(352)375-8593						
CA 5201						

[illegible]

Job	051062	SR 441 - LOT 1 LAKECITY
Mark	R16	
Quan	1	
Type	HIPP	
Span	350000	
Pl-H1	6	
Left OH	1-6-0	
Right OH	1-6-0	
Engineering		



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 236.1 LBS

Online Plus -- Version 18.0.014
RUN DATE: 14-NOV-05

CSI - Size - Lumber

TC 0.42 2x 4 SP-#2
BC 0.45 2x 4 SP-#2
WB 0.21 2x 4 SP-#3

Brace truss as follows:

TC Cont. 0-0-0 35-0-0
BC Cont. 0-0-0 35-0-0
WB 1 rows CLB on B-H
WB 1 rows CLB on H-C
Attach CLB with (2)-10d nails

at each web.

Loading Live Dead (psf)

TC 20.0 7.0
BC 0.0 10.0
Total 20.0 17.0

Spacing
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC TL Load Case(s)

Jt React Uplift Size Req'd

A 1382 478 4-0 1-10
D 1382 478 4-0 1-10

Member CSI P Lbs Axl-CSI-Bnd
A-J 0.36 2356 C 0.14 0.22
J-B 0.34 1964 C 0.12 0.22

Top Chords
D LOCK 2.0x 4.0 Ctr Ctr 0.44
I LOCK 2.0x 4.0 Ctr Ctr 0.44
F LOCK 3.0x 5.0 Ctr Ctr 0.51
S1 LOCK 3.0x 5.0 Ctr Ctr 0.74
H LOCK 4.0x 8.0 Ctr Ctr 0.42
E LOCK 3.0x 5.0 Ctr Ctr 0.51
S2 LOCK 3.0x 5.0 Ctr Ctr 0.74
G LOCK 2.0x 4.0 Ctr Ctr 0.44

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 ga, Gross Area
Plate - RHS 20 ga, Gross Area
Jt Type Plt Size X Y JSI
A LOCK 4.0x 5.0 Ctr 0.1 0.74
J LOCK 3.0x 5.0 Ctr 0.57
B LOCK 5.0x 8.0 Ctr 0.1 0.93
K LOCK 2.0x 4.0 Ctr Ctr 0.44
C LOCK 5.0x 8.0 Ctr 0.1 0.93
L LOCK 3.0x 5.0 Ctr Ctr 0.57
D LOCK 4.0x 5.0 Ctr 0.1 0.74
I LOCK 2.0x 4.0 Ctr Ctr 0.44
F LOCK 3.0x 5.0 Ctr Ctr 0.51

LT Defl -0.13" in S1-H L/999
TL Defl -0.26" in H-B L/999
Shear // Grain in B-K 0.25

Web-
I-J 0.05 202 T
J-F 0.21 393 C
F-B 0.10 371 T
B-H 0.12 400 T
H-K 0.18 393 C
H-C 0.12 400 T
E-C 0.10 371 T
E-L 0.21 394 C
G-L 0.05 202 T

Bottom Chords
A-I 0.42 2105 T 0.35 0.07
I-F 0.45 2105 T 0.35 0.10
F-S1 0.39 1753 T 0.29 0.10
S1-H 0.41 1753 T 0.29 0.12
H-E 0.41 1753 T 0.29 0.12
E-S2 0.45 2105 T 0.35 0.10
S2-G 0.41 2105 T 0.35 0.06
G-D 0.42 2105 T 0.35 0.07

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS

Analysis Conforms To:
FBC2004

OH Loading
Soffit psf 2.0

Design checked for 10 psf non-
concurrent TL on BC.

Wind Loads - ANSI / ASCE 7-02
Truss is designed as

Components and Claddings*
for Exterior zone location.

Wind Speed:
Mean Roof Height: 25-0

Exposure Category: B
Occupancy Factor: 1.00

Building Type: Enclosed
TC Dead Load:
BC Dead Load:

Max comp. force 2356 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:

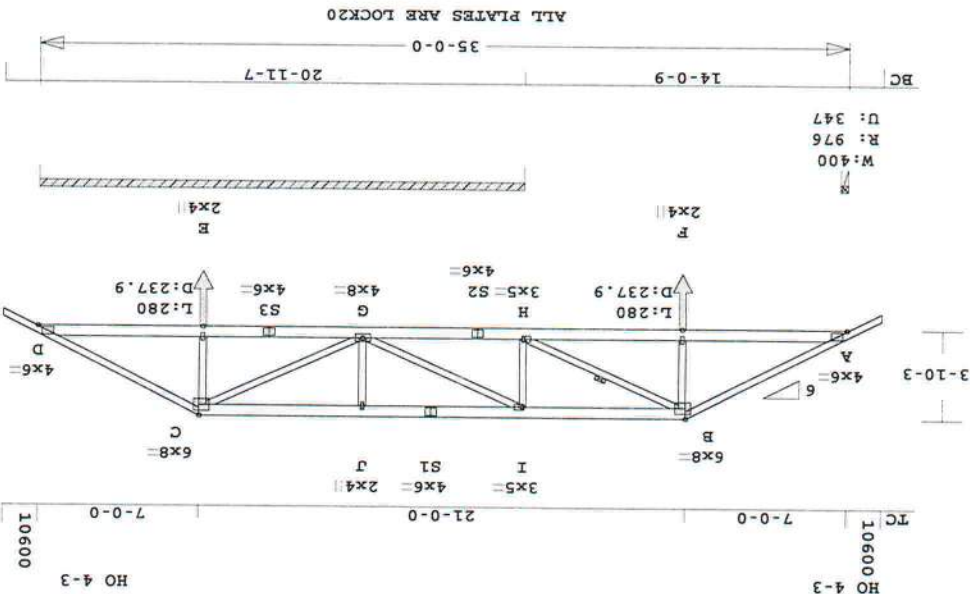
1. Delegated Engineer (Truss
Designer)
Gary Downson, PE 35054
Gary Downson & Associates,
Inc.

2830 NW 41st Street Suite D
Gainesville, FL 32606

(352)375-8593

CA 5201

Job	051062	SR 441 - LOT 1 LAKECITY
Mark	R18	
Quan	1	HIP
Type	Span	350000
Pl-H1	6	
Left OH	1-6-0	
Right OH	1-6-0	
Engineering		



Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 258.3 LBS

Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004

Girder Step Down Hip
Framing King Jacks
Open Faced
Setback 7'-0" 0
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.

Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25'-0"

Exposure Category: B
Occupancy Factor: 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1464 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D Gainesville, FL 32606
(352)375-8593
CA 5201

Cont. Brg 14'-0" 9 to 35'-0" 0
4758 1618 Hz = 74

Member CSI P Lbs AXI-CST-Bnd
Top Chords

A-B 0.58 1464 C 0.01 0.57
B-I 0.70 160 T 0.00 0.70
I-S1 0.70 112 T 0.00 0.70
S1-J 0.50 112 T 0.00 0.50
J-C 0.50 112 T 0.00 0.50
C-D 0.34 233 T 0.03 0.31

Bottom Chords
A-F 0.33 1325 T 0.17 0.16
F-H 0.46 1298 T 0.17 0.29
H-S2 0.34 0 T 0.00 0.34
S2-G 0.15 0 T 0.00 0.15
G-S3 0.15 0 T 0.00 0.15
S3-E 0.15 0 T 0.00 0.15
E-D 0.22 74 C 0.00 0.22

Plates for each ply each face.
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Size X JSI

LT Defl - 0.06" in A - F L/999
TL Defl - 0.11" in A - F L/999
Shear // Grain in B - I 0.45

Plates for each ply each face.
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Size X JSI

Plates for each ply each face.
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Size X JSI

Plates for each ply each face.
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Size X JSI

Online Plus -- Version 18.0.014
RUN DATE: 14-NOV-05

CSI - Size - Lumber

TC 0.58 2x 4 SP-#2
EX B-S1 2x 6 SP-#2
EX S1-C 2x 6 SP-#2
BC 0.46 2x 6 SP-#2
WB 0.35 2x 4 SP-#3

Brace truss as follows:
O.C. From To
TC Cont. 0-0-0 35-0-0
BC Cont. 0-0-0 35-0-0
WB 1 rows CLB on B-H
Attach CLB with (2)-10d nails
at each web.

Loading Live Dead (psf)
TC 20.0 7.0
BC 0.0 10.0
Total 20.0 17.0
Spacing 24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

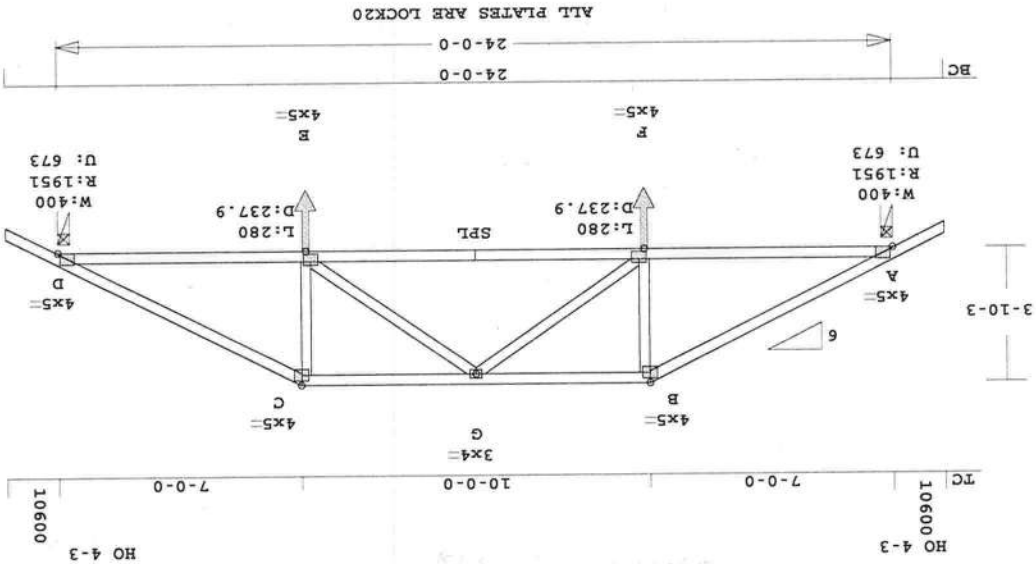
Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

Plf - Live Dead From To
TC V 40 14 0.0 35.0
BC V 0 20 0.0 35.0
TC V 50 18 7.0 28.0
BC V 25 7.1 27.9

Plf - Live Dead From To
TC V 40 14 0.0 35.0
BC V 0 20 0.0 35.0
TC V 50 18 7.0 28.0
BC V 25 7.1 27.9

Plf - Live Dead From To
TC V 40 14 0.0 35.0
BC V 0 20 0.0 35.0
TC V 50 18 7.0 28.0
BC V 25 7.1 27.9

Job	051062	SR 441 - LOT 1 LAKECITY
Mark	R19	
Quan	1	HIPP
Type	240000	Span
PI-HI	6	
Left OH	1-6-0	
Right OH	1-6-0	
Engineering		



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 134.7 LBS

Trusses Manufactured by: RIDGWAY ROOF TRUSSES

Analysis Conforms To: FBC2004

Glirder Step Down Hip

Framing King Jacks

Jack Open Faced

Setback 7'-0"-0

OH Loading

Soffit psf 2.0

Design checked for 10 psf non-concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as Components and Claddings* for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 25'-0

Exposure Category: B

Occupancy Factor: 1.00

Building Type: Enclosed

TC Dead Load: 4.0 psf

BC Dead Load: 6.0 psf

Max comp. force 3600 lbs

Quality Control Factor 1.25

Hz = -66

1952 673 4-0 1-10

Hz = 68

Online Plus -- Version 18.0.014

RUN DATE: 14-NOV-05

CSI - Size - Lumber

TC 0.70 2x 4 SP-#2

BC 0.75 2x 4 SP-2400

WB 0.37 2x 4 SP-#3

Brace truss as follows:

O.C. From To

TC Cont. 0-0-0 24-0-0

BC Cont. 0-0-0 24-0-0

Loading Live Dead (psf)

TC 20.0 7.0

BC 0.0 10.0

Total 20.0 17.0

Spacing 24.0"

Lumber Duration Factor 1.25

Plate Duration Factor 1.25

TC Fb=1.00 Fc=1.00 Ft=1.00

BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Girder Loading

Lumber Duration Factor 1.25

Plate Duration Factor 1.25

Plf - Live Dead From To

TC V 40 14 0.0' 24.0'

BC V 280 238 16.9' CL-LB

Plus 9 Wind Load Case(s)

Plus 1 UBC LL Load Case(s)

Ut React Uplift Size Req'd

Lbs In-Sx In-Sx

A 1952 673 4-0 1-10

NOTES:

Plates for each ply each face.

ALL CONNECTOR PLATES TO BE MANUFACTURED BY ROBBINS ENGINEERING, INC.

Plate - LOCK 20 Ga, Gross Area

Plate - RHS 20 Ga, Gross Area

Ut Type Plt Size X Y JSI

A LOCK 4.0x 5.0 Ctr 0.1 0.92

B LOCK 4.0x 5.0 Ctr 0.93

G LOCK 3.0x 4.0 Ctr 0.47

C LOCK 4.0x 5.0 Ctr 0.93

D LOCK 4.0x 5.0 Ctr 0.1 0.92

F LOCK 4.0x 5.0 Ctr 0.8 0.90

E LOCK 4.0x 5.0 Ctr 0.8 0.90

SI 3.00

LT Defl -0.34" in F -SI L/820

TL Defl -0.74" in F -SI L/377

Shear // Grain in B -G 0.45

E -C 0.37 1176 T

G -E 0.31 587 C

F -G 0.31 587 C

F -B 0.37 1176 T

-----Webs-----

E -D 0.70 3217 T 0.15 0.55

SI-E 0.75 3691 T 0.16 0.59

F -SI 0.75 3691 T 0.16 0.59

-----Bottom Chords-----

A -F 0.70 3217 T 0.15 0.55

C -D 0.56 3600 C 0.24 0.32

G -C 0.70 3217 C 0.08 0.62

-----Top Chords-----

Member CSI F Lbs AXI-CSI-Bnd

FABRICATOR NOTES:

1. Delegated Engineer (Truss Designer)

Gary Dounson, PE 35054

2830 NW 41st Street Suite I Gainesville, FL 32606

(352)375-8593

CA 5201

ut	React	Uplift	Size	Req'd
A	Lbs	340	4 - 0	In-Sx
D	Lbs	975	340	In-Sx
	HZ =		-86	
	HZ =		4 - 0	1-2
				87
Member	CSI	P Lbs	AxI-CSt-Bnd	
-----Top Chords-----				
A - B	0.78	1283	C	0.06 0.72
B - C	0.36	1147	C	0.09 0.27
C - D	0.78	1281	C	0.06 0.72

Plus	9 Wind Load Case(s)
Plus	1 UBC TL Load Case(s)

Loadings	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"
Lumber Duration Factor	1.25		
Plate Duration Factor	1.25		
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Brace truss as follows:

	O.C.	From	To
TC Cont.	0 - 0 - 0	24 - 0 - 0	
BC Cont.	0 - 0 - 0	24 - 0 - 0	

CSI	Size	TC	BC	WB
SP-#2	2x 4	0.78	0.56	0.08
SP-#2	2x 4			
SP-#3	2x 4			

Online Plus -- Version 18.0.014
RUN DATE: 14-NOV-05

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSSES
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-
concurrent TL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as

LOCK	3.0X	4.0	CTR	CTR	0.56
E	LOCK	3.0X	4.0	CTR	CTR
S1	LOCK	3.0X	4.0	CTR	CTR
					0.69

Plates for each ply each face.

ALL CONNECTOR PLATES	
TO BE MANUFACTURED BY	
ROBBINS ENGINEERING, INC.	
Plate - LOCK 20 ga, Gross Area	Plate - RHS 20 ga, Gross Area
ut Type	Plt Size
A LOCK	3.0x 4.0 Ctr Ctr
B LOCK	4.0x 8.0 Ctr Ctr
C LOCK	4.0x 5.0 Ctr Ctr

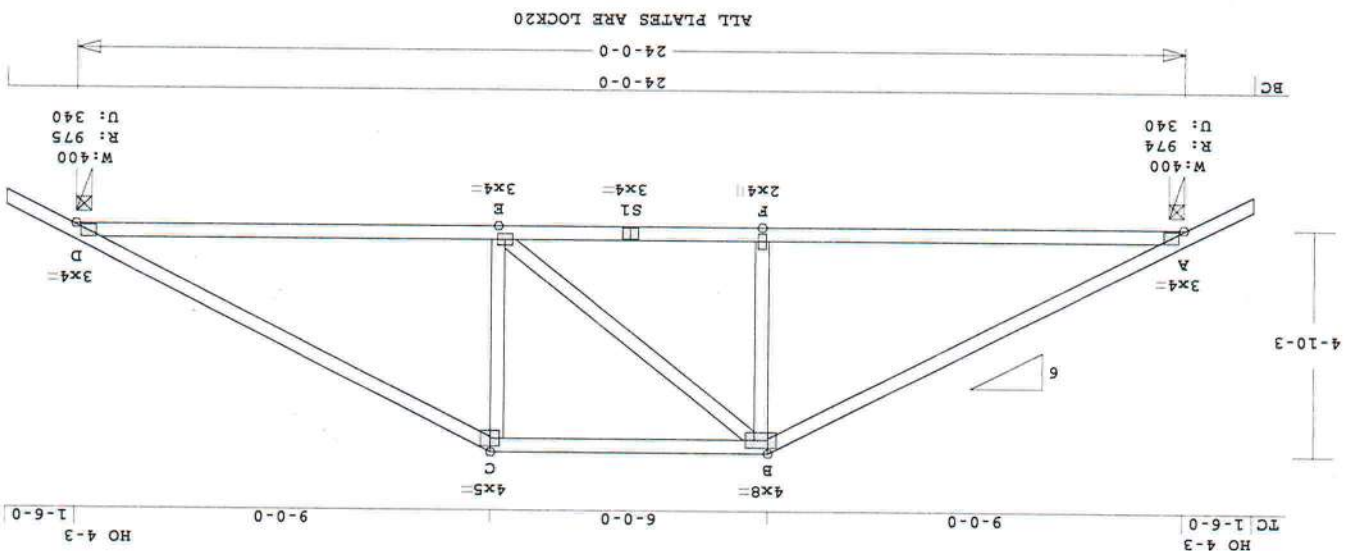
0.30	Shear // Grain in A - B
L/836	TL Defl -0.33" in E - D
L/999	TL Defl -0.14" in A - F

F-B	0.08	301	T
B-E	0.07	105	T
E-C	0.07	297	T

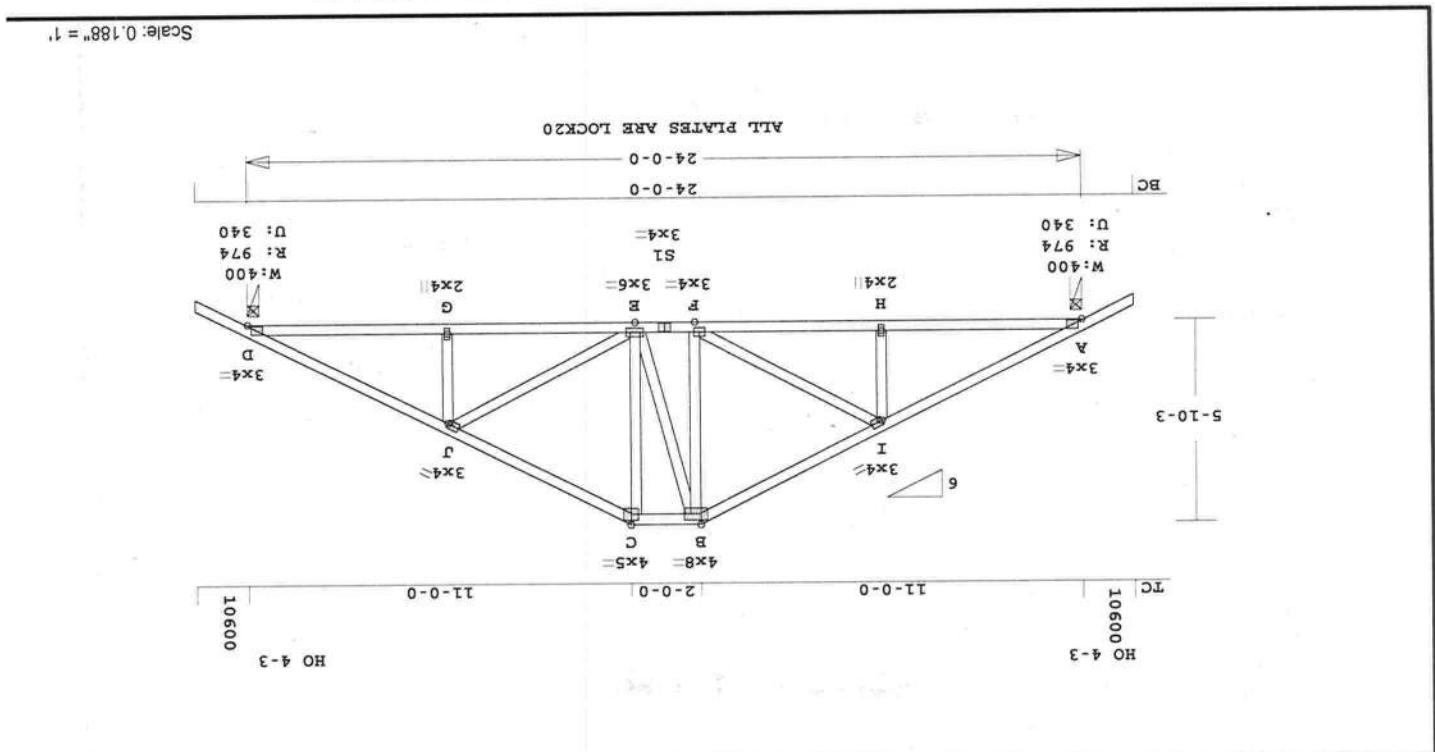
F-S1	0.48	1144	T	0.19	0.29
S1-E	0.47	1144	T	0.19	0.28

Robbins Engineering, Inc./Online Plus[®] APPROX. TRUSS WEIGHT: 130.7 LBS

Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1283 lbs
Quality Control Factor 1.25



Job	051062	Mark	R20	Quan	1	Type	HIPP	Span	240000	Pl-HI	6	Left OH	1- 6- 0	Right OH	1- 6- 0	Engineering
SR 441 - LOT 1 LAKECITY																



Robbins Engineering, Inc./Online Plus™									
APPROX. TRUSS WEIGHT: 164.5 LBS									
I-B	0.33	1061	C	0.08	0.25	S1	LOCK	3.0x 4.0	CTR 0.62
B-C	0.17	943	C	0.08	0.09	E	LOCK	3.0x 6.0	CTR 0.70
C-J	0.33	1062	C	0.08	0.25	G	LOCK	2.0x 4.0	CTR 0.40

[illegible]

TC Conf.	BC Conf.	H-I	I-F	F-B	287 T	Design checked for 10 per hour concurrent LL on BC.	Wind loads - ANSI / ASCE 7-02
0-0-0 24-0-0	0-0-0 24-0-0	0.06	0.24	0.09	228 T		
					458 C		
					287 T		

Total	20.0	17.0	37.0	G-J 0.06	225 T	Wind speed:	110 mph
Spacing			24.0"			Mean roof height:	25-0
Lumber Duration Factor	1.25					Exposure Category:	B
LT Defl - 0.05" in H - F	L/999						

Plus 9 Wind Load Case(s)
 ALL CONNECTOR PLATES
 TO BE MANUFACTURED BY
 Plates for each ply each face.
 BC Dead Load: 8.0 psf
 Max comp. force 1500 lbs
 Quality Control Factor 1.25

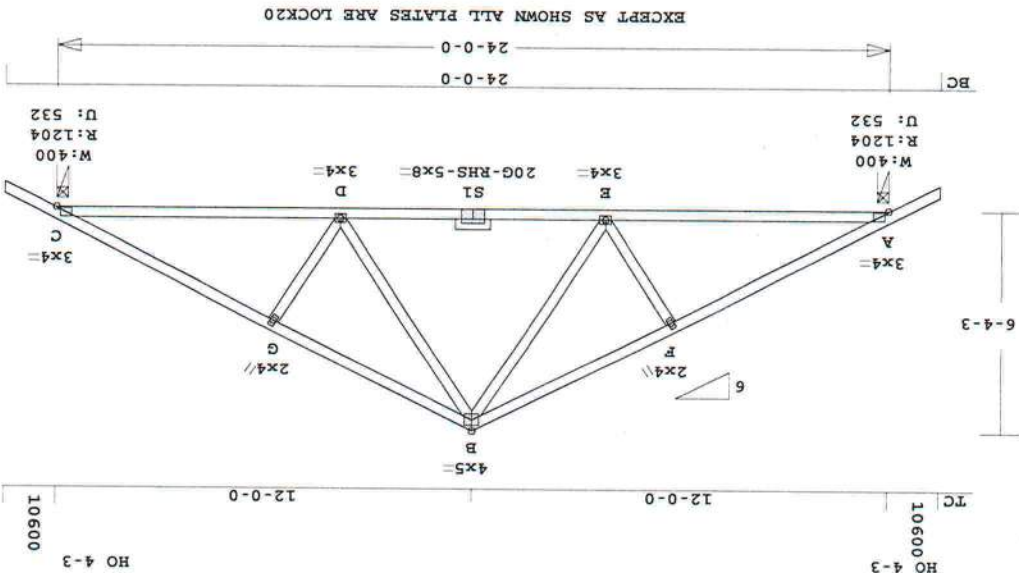
Lbs	In-Sx	In-Sx	Jt Type	Pit Size	X	Y	Z	UJI	
A	975	340	4-0	1-2	A LOCK	3.0X	4.0	Ctr Ctr	0.83
					I LOCK	3.0X	4.0	Ctr Ctr	0.62

Gary Donson & Associates,
Gary Donson, PE 35054
Designer)

Member	CSI P Lbs	AxI-CSt-Bnd	D LOCK	3.0X 4.0 Ctr	0.83	(352) 3/5-8595	CA 5201
-----Top Chords-----			H LOCK	2.0X 4.0 Ctr	0.40		
A-I	0.35	1500 C	0.10	0.25	F LOCK	3.0X 4.0 Ctr	0.56

Job	051062	Mark	R22	Quan	4	Type	TR	Span	240000	Pl-HI	6	Left OH	1-6-0	Right OH	1-6-0	Engineering
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SR 441 - LOT 1 LAKECITY



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 143.9 LBS
 Lbs In-Sk In-Sk 533 4-0 1-7 Hz = -115
 Lbs In-Sk In-Sk 533 4-0 1-7 Hz = 116
 E LOCK 3.0x 4.0 Ctr Ctr 0.52
 SI RHS 5.0x 8.0 Ctr Ctr 0.95
 D LOCK 3.0x 4.0 Ctr Ctr 0.52

CSI -Size- Lumber-
 TC 0.46 2x 4 SP-#2
 BC 0.90 2x 4 SP-#2
 WB 0.40 2x 4 SP-#3
 PB --- 2x 4 SP-#3

Brace truss as follows:
 O.C. From To
 TC Cont. 0-0-0 24-0-0
 BC Cont. 0-0-0 24-0-0

Loading Live Dead (psf)
 TC 20.0 7.0
 Spacing 24.0"
 Lumber Duration Factor 1.25
 Plate Duration Factor 1.25
 TC Fb=1.00 Fc=1.00 Ft=1.00
 BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Standard Loading
 Lumber Duration Factor 1.25
 Plate Duration Factor 1.25
 plf - Live Dead From To
 TC V 40 0.0' 24.0'
 BC V 0 0.0' 24.0'
 BC V 50 8.2' 15.8'

Plus 9 Wind Load Case(s)
 Plus 1 UBC LL Load Case(s)
 JC React Uplift Size Req'd

Plates for each ply each face.
 ALL CONNECTOR PLATES
 TO BE MANUFACTURED BY
 ROBBINS ENGINEERING, INC.
 Plate - LOCK 20 Ga, Gross Area
 Plate - RHS 20 Ga, Gross Area
 Jt Type Plt Size X Y JSI
 A LOCK 3.0x 4.0 Ctr Ctr 0.89
 F LOCK 2.0x 4.0 Ctr Ctr 0.38
 B LOCK 4.0x 5.0 Ctr Ctr 0.64
 G LOCK 2.0x 4.0 Ctr Ctr 0.37
 C LOCK 3.0x 4.0 Ctr Ctr 0.89

Shear // Grain in E-SI 0.41
 TL Defl -0.25" in SI-D L/999
 TL Defl -0.35" in SI-D L/799
 F-E 0.07 311 T
 E-B 0.40 789 T
 B-D 0.40 789 T
 D-G 0.07 311 T

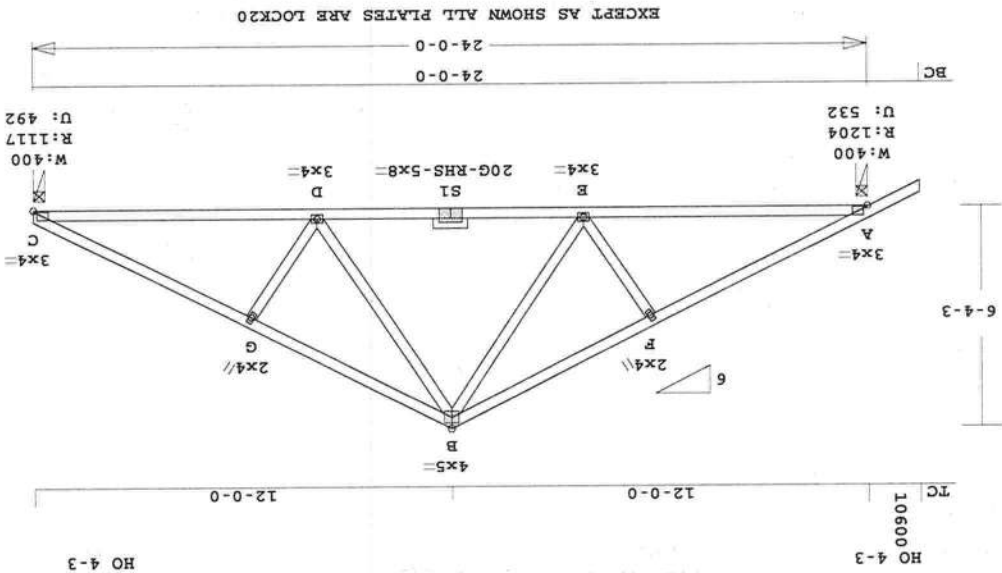
Member CSI P Lbs Axl-CSI-Bnd
 A-F 0.46 1985 C 0.19 0.27
 F-B 0.46 1818 C 0.19 0.27
 B-G 0.46 1818 C 0.19 0.27
 G-C 0.46 1985 C 0.19 0.27
 A-E 0.90 1776 T 0.32 0.58
 E-SI 0.84 1188 T 0.21 0.63
 SI-D 0.84 1188 T 0.21 0.63
 D-C 0.90 1776 T 0.32 0.58

Notes:
 Trusses Manufactured by:
 RIDGWAY ROOF TRUSSES
 Analysis Conforms To:
 FBC2004
 OH Loading
 Soffit psf 2.0
 Design checked for 10 psf non-concurrent LL on BC.
 Wind Loads - ANSI / ASCE 7-02
 Truss is designed as
 Components and Claddings*
 for Exterior zone location.
 Wind Speed: 110 mph
 Mean Roof Height: 25-0
 Exposure Category: B
 Occupancy Factor: 1.00
 Building Type: Enclosed
 TC Dead Load: 4.0 psf
 BC Dead Load: 6.0 psf
 Max comp. force 1985 lbs
 Quality Control Factor 1.25

FABRICATOR NOTES:
 I. Delegated Engineer (Truss Designer)
 Gary Dounson, PE 35054
 Gary Dounson & Associates, Inc.
 2830 NW 41st Street Suite D
 Gainesville, FL 32606
 (352) 375-8593
 CA 5201

Job	051062	Mark	R23	Quan	1	Type	TR	Span	240000	PI-HI	6	Left OH	1-6-0	Right OH	0	Engineering
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SR 441 - LOT 1 LAKECITY



Scale: 0.188" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 140.7 LBS

Lbs In-Sk In-Sk Lbs In-Sk In-Sk

A 1204 533 4-0 1-7

C 1117 493 4-0 1-5

HZ = -115

HZ = 116

Notes:

Trusses Manufactured by:

RIDGEWAY ROOF TRUSSES

Analysis Conforms To:

FB2004

OH Loading

Soffit psf 2.0

Design checked for 10 psf non-

concurrent TL on BC.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as

Components and Claddings*

for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 25-0

Exposure Category: B

Occupancy Factor: 1.00

Building Type: Enclosed

TC Dead Load: 4.0 psf

BC Dead Load: 6.0 psf

Max comp. force 1985 Lbs

Quality Control Factor 1.25

CSI -Size- -Lumber-

TC	0.46	2x 4	SP-#2
BC	0.90	2x 4	SP-#2
WB	0.40	2x 4	SP-#3
PB	---	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0-0-0	24-0-0
BC Cont.	0-0-0	24-0-0

Loading Live Dead (psf)

TC	20.0	7.0
BC	0.0	10.0
Total	20.0	17.0

Spacing	24.0"
Lumber Duration Factor	1.25
Plate Duration Factor	1.25

TC	Rb=1.00	Rc=1.00	Rt=1.00
BC	Rb=1.00	Rc=1.00	Rt=1.00

Load Case # 1 Standard Loading

Plates for each ply each face.

ATL CONNECTOR PLATES

TO BE MANUFACTURED BY

ROBBINS ENGINEERING, INC.

Plt Size X Y JSI

A LOCK 3.0x 4.0 Ctr Ctr 0.89

F LOCK 2.0x 4.0 Ctr Ctr 0.38

B LOCK 4.0x 5.0 Ctr Ctr 0.64

G LOCK 2.0x 4.0 Ctr Ctr 0.37

C LOCK 3.0x 4.0 Ctr Ctr 0.89

ut React Uplift Size Req'd

Plus	9 Wind Load Case(s)
Plus	1 UBC TL Load Case(s)

CSI	-Size-	-----Lumber----			
TC	0.38	2x 4	SP-#2		
BC	0.32	2x 4	SP-#2		
WB	0.11	2x 4	SP-#3		
Brace truss as follows:					
O.C.					
TC Cont.		From		To	
BC Cont.		0 - 0 - 0 14 - 0 - 0		0 - 0 - 0 14 - 0 - 0	
Loading					
Live		Dead		(psf)	
TC	20.0	7.0			
BC	0.0	10.0			
Total	20.0	17.0		37.0	
Spacing					
Lumber Duration Factor		1.25			
Plate Duration Factor		1.25			
TC Fb=1.00		Fc=1.00		Ft=1.00	
BC Fb=1.00		Fc=1.00		Ft=1.00	
Load Case # 1 Girder Loading					
Lumber Duration Factor		1.25			
Plate Duration Factor		1.25			
Plf - Live		Dead		To	
TC V	40	14		0.0'	
BC V	0	20		0.0'	
TC V	30	11		5.0'	
BC V	0	15		5.1'	
BC V	133	113		5.1'	
BC V	133	113		5.1'	
CL-LB	8.9'	CL-LB		8.9'	

Online Plus -- Version 14-NOV-05

Uplift	React	Uplift	Size	Req'd
A	Lbs	960	337	4-0 1-2
D	Lbs	960	337	4-0 1-2
Hz = 48				
-----Top Chords-----				
A-B	0.37	1508	C	0.16 0.21
B-C	0.32	1352	C	0.16 0.16
C-D	0.38	1510	C	0.16 0.22
-----Bottom Chords-----				
A-F	0.30	1352	T	0.24 0.06
F-E	0.32	1337	T	0.24 0.08
E-D	0.29	1352	T	0.24 0.05
-----Webs-----				
F-B	0.11	395	T	
B-E	0.01	61	T	
E-C	0.11	396	T	

Membr CSI F Lbs Axl-CSI-Bnd				

Plates for each ply each face.				
ALU CONNECTOR PLATES				
TO BE MANUFACTURED BY				
ROBINS ENGINEERING, INC.				
Plate - LOCK 20 Ga, Gross Area				
Plate - RHS 20 Ga, Gross Area				
Upl Type Plt Size X Y JSI				
A	LOCK	3.0x	4.0	Ctr Ctr 0.68
B	LOCK	4.0x	8.0	Ctr Ctr 0.75
C	LOCK	4.0x	5.0	Ctr Ctr 0.75
D	LOCK	3.0x	4.0	Ctr Ctr 0.68
E	LOCK	2.0x	4.0	Ctr Ctr 0.43
F	LOCK	3.0x	4.0	Ctr Ctr 0.45

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 78.3 LBS

NOTES:

Trusses Manufactured by:
RIDGEWAY ROOF TRUSS
Analysis Conforms To:

FBC2004
Gilder Step Down Hip

Framing King Jacks
Jack Open Faced
Setback 5 - 0 - 0

OH Loading
Softit psf 2.0

design checked for 10 psi non-concurrent TL on BC.
Wind Loads - ANSI / ASCE 7-02

Components and Claddings*
For Exterior zone location.

Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor: 1.00

Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf

BC Head Load: 6.0 psi
Max comp. force 1510 lbs
Quality Control Factor 1.25

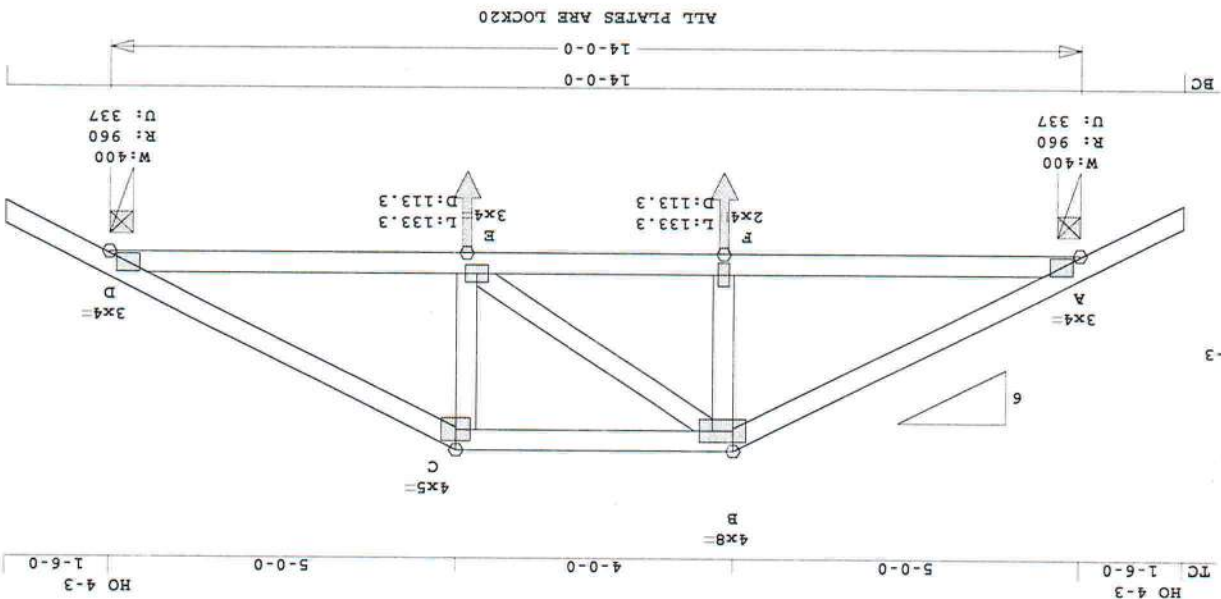
FABRICATOR NOTES:
1. Delegated Engineer (Truss

Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates,

INC.
2830 NW 41st Street Suite D
Gainesville, FL 32606

(352) 375-8593

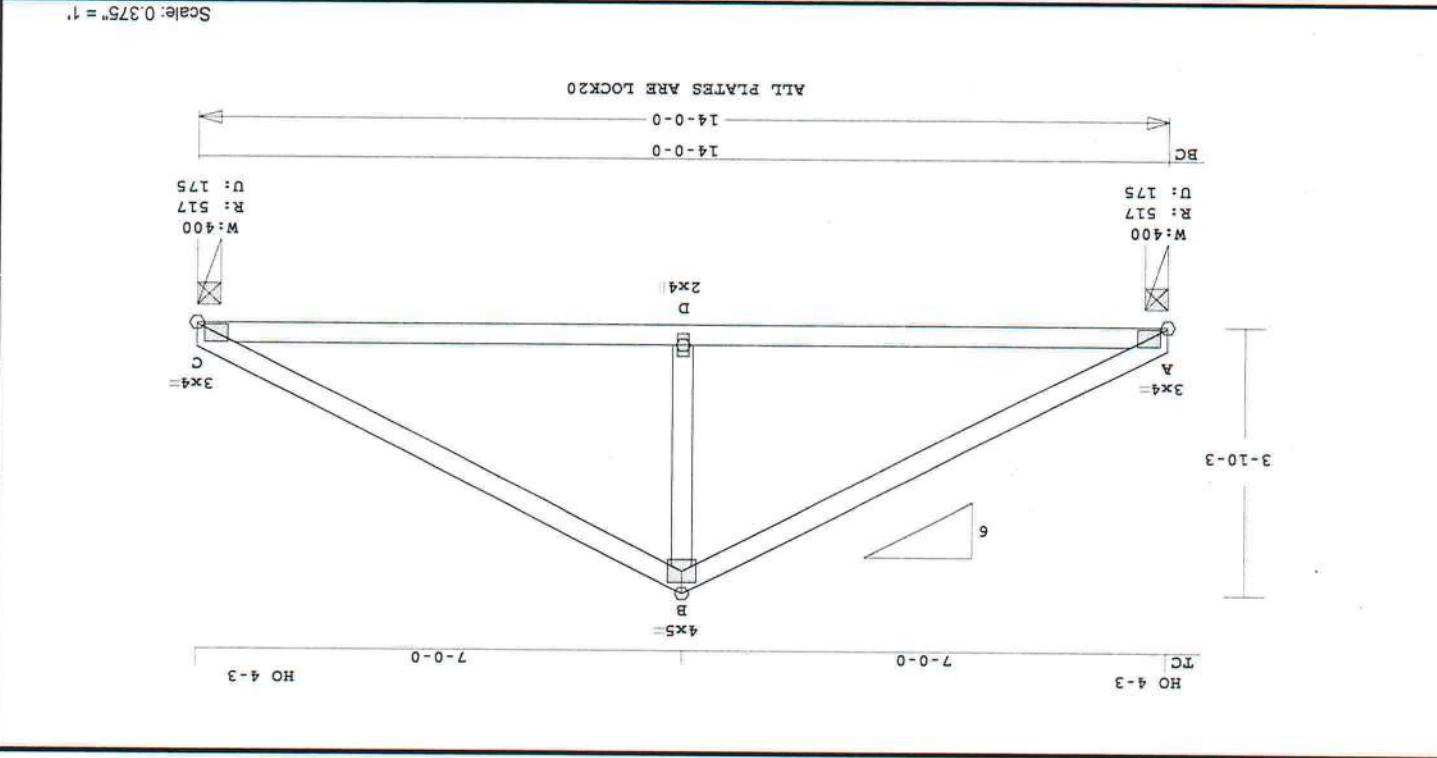
CA 5201



Scale: 0.375" = 1'

Job	051062	Mark	R24	Quan	1	Type	HIPP	Span	140000	Pl-HI	6	Left OH	1- 6- 0	Right OH	1- 6- 0	Engineering
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Job	051062	SR 441 - LOT 1 LAKECITY
Mark	R26	
Quan	1	
Type	TR	
Span	14'000	
Pl-HI	6	
Left OH	0	
Right OH	0	
Engineering		



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 62.7 LBS

Buildng Type: Enclosed

TC Dead Load: 4.0 psf

BC Dead Load: 6.0 psf

Max comp. force 645 lbs

Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss Designer)

Gary Dounson, PE 35054

Gary Dounson & Associates, Inc.

2830 NW 41st Street Suite D Gainesville, FL 32606

(352)375-8593

CA 5201

LT Defl -0.04" in A-D L/999

TL Defl -0.08" in A-D L/999

Shear // Grain in A-B 0.25

Plates for each ply each face.

TO BE MANUFACTURED BY

ROBBINS ENGINEERING, INC.

Plate - LOCK 20 Ga, Gross Area

Plate - RHS 20 Ga, Gross Area

ut Type Plt Size X Y JSI

A LOCK 3.0x 4.0 Ctr 0.67

B LOCK 4.0x 5.0 Ctr 0.51

C LOCK 3.0x 4.0 Ctr 0.67

D LOCK 2.0x 4.0 Ctr 0.38

CSI -Size- ---Lumber---

TC 0.34 2x 4 SP-#2

BC 0.36 2x 4 SP-#2

WB 0.08 2x 4 SP-#3

Brace truss as follows:

O.C. From To

TC Cont. 0-0-0 14-0-0

BC Cont. 0-0-0 14-0-0

Loading Live Dead (psf)

TC 20.0 7.0

BC 0.0 10.0

Total 37.0

Spacing 24.0"

Lumber Duration Factor 1.25

Plate Duration Factor 1.25

TC Fb=1.15 Fc=1.10 Ft=1.10

BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)

Plus 1 UBC LT Load Case(s)

ut React Uplift Size Req'd

A 518 175 4-0 1-0

C 518 175 4-0 1-0

HZ = -66

HZ = 67

Membr CSI P Lbs Axl-Csi-Bnd

-----Top Chords-----

A-B 0.34 645 C 0.06 0.28

NOTES:

TRUSSES MANUFACTURED BY:

RIDGWAY ROOF TRUSS

Analysis Conforms To:

FB02004

Design checked for 10 psf non-

concurrent LT on BC.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as

Components and Claddings*

for Exterior zone location.

Wind Speed: 110 mph

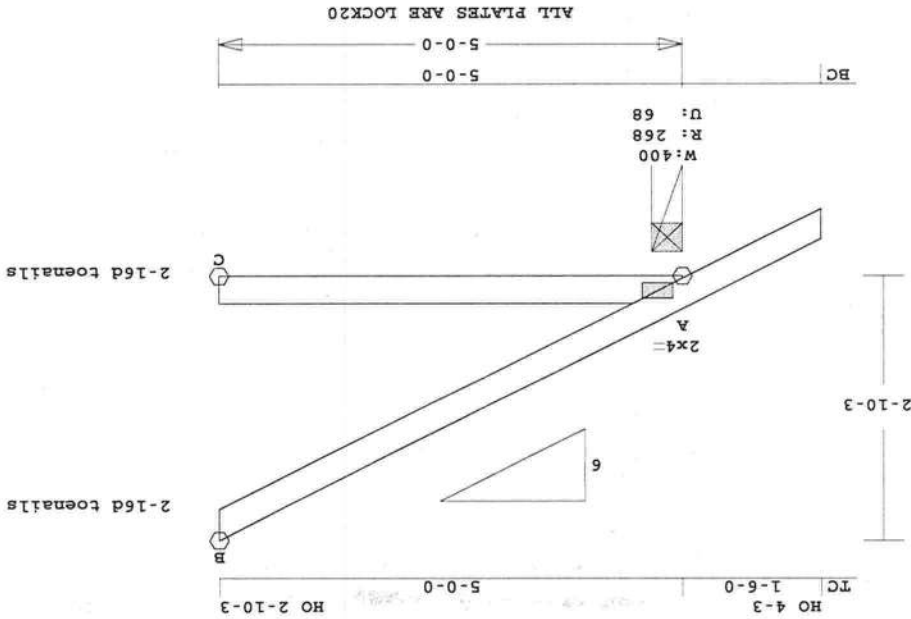
Mean Roof Height: 25-0

Exposure Category: B

Occupancy Factor : 1.00

Job	051062	Mark	R27	Quan	3	Type	JACK	Span	50000	Pl-HI	6	Left OH	1 - 6 - 0	Right OH	0	Engineering
-----	--------	------	-----	------	---	------	------	------	-------	-------	---	---------	-----------	----------	---	-------------

SR 441 - LOT 1 LAKECITY



Scale: 0.500" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 23.4 LBS

-----Bottom Chords-----
A - C 0.31 59 T 0.00 0.31
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 143 lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Inc.
2830 NW 41st Street Suite I
Gainesville, FL 32606
(352)375-8593
CA 5201

Plates for each ply each face.
ALT CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 ga, Gross Area
Plate - RHS 20 ga, Gross Area
TC Cont. 0 - 0 - 0 5 - 0 - 0
BC Cont. 0 - 0 - 0 5 - 0 - 0
Loading Live Dead (psf)
TC 20.0 7.0
BC 0.0 10.0
Total 20.0 17.0
Spacing 24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 8 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B

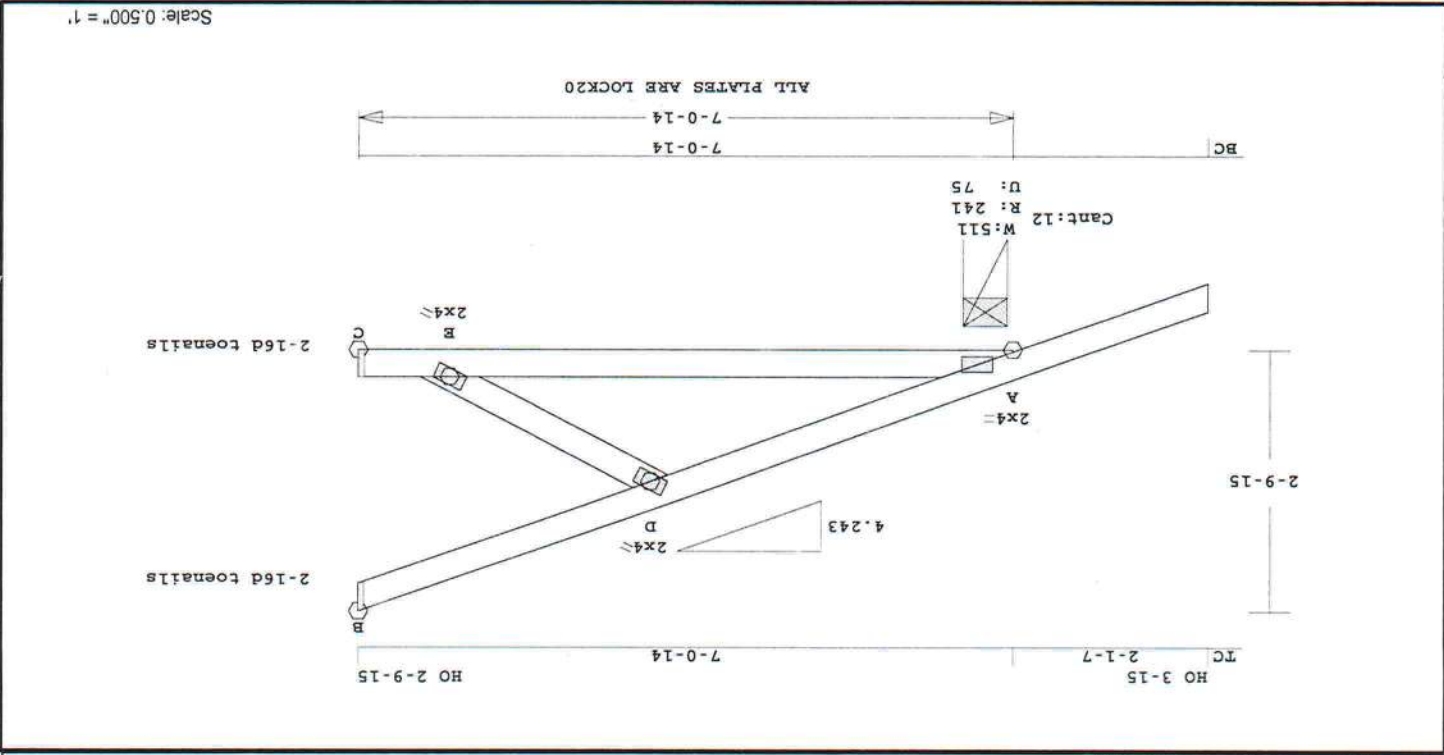
Notes:
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B

Online Plus -- Version 18.0.014
RUN DATE: 14-NOV-05
CSI - Size - Lumber -
TC 0.35 2x 4 SP-#2
BC 0.31 2x 4 SP-#2
Brace truss as follows:
O.C. From To
TC Cont. 0 - 0 - 0 5 - 0 - 0
BC Cont. 0 - 0 - 0 5 - 0 - 0
Loading Live Dead (psf)
TC 20.0 7.0
BC 0.0 10.0
Total 20.0 17.0
Spacing 24.0"

Plus 8 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)
Truss React Uplift Size Req'd
Lbs In-Sx In-Sx
A 268 68 4 - 0 1 - 0
B 128 106 3 - 8 1 - 0
C 92 0 3 - 8 1 - 0
Hz = 111
Hz = 111
Member CSI P Lbs Axl-CST-Bnd
A - B 0.35 143 C 0.00 0.35

Job	051062	Mark	R3A	Quan	2	Type	KING	Span	70014	Pl-HI	4.243	Left OH	2 - 1 - 7	Right OH	0	Engineering
-----	--------	------	-----	------	---	------	------	------	-------	-------	-------	---------	-----------	----------	---	-------------



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 37.4 LBS

of React Uplift Size Req'd

Trusses Manufactured by:

RIDGWAY ROOF TRUSSES

Analysis Conforms To:

FBC2004

Girder King Jack

Loading TC and BC

Setback 5 - 0 - 0

OH Loading

soffit psf 2.0

Design checked for 10 psf non-

concurrent LT on BC.

Use properly rated hangers for

loads framing into girder

truss.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as

Components and Claddings*

for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 25-0

Exposure Category: B

Occupancy Factor: 1.00

Building Type: Enclosed

TC Dead Load: 4.0 psf

BC Dead Load: 6.0 psf

Max comp. force: 248 lbs

Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss

Designer)

Gary Dounson, PE 35054

Inc.

2830 NW 41st Street Suite D

Gainesville, FL 32606

(352)375-8593

CA 5201

Online Plus -- Version 18.0.014 A

RUN DATE: 14-NOV-05

CSI - Size - Lumber -

TC 0.16 2x 4 SP-#2

BC 0.24 2x 4 SP-#2

WB 0.06 2x 4 SP-#3

Brace truss as follows:

O.C. From To

TC Cont. 0 - 0 - 0 7 - 0 - 14

BC Cont. 0 - 0 - 0 7 - 0 - 14

-----Bottom Chords-----

D - B 0.16 76 C 0.00 0.16

A - D 0.13 188 C 0.00 0.13

-----Top Chords-----

Membr CSI P Lbs Axl-CSt-Bnd

D - E 0.06 279 T

-----Webs-----

E - C 0.15 42 T 0.00 0.15

A - E 0.24 248 C 0.01 0.23

LT Defl -0.05" in A - E L/999

TL Defl -0.10" in A - E L/728

Shear // Grain in E - C 0.18

Plates for each ply each face.

ALL CONNECTOR PLATES

TO BE MANUFACTURED BY

ROBBINS ENGINEERING, INC.

Plate - LOCK 20 ga, Gross Area

Plate - RHS 20 ga, Gross Area

Plate - Live Dead From To

TC V 40 14 0.0' 7.1'

BC V 0 20 0.0' 7.1'

TC V -40 -14 0.0' 7.1'

BC V 0 -20 0.0' 7.1'

Plus 8 Wind Load Case(s)

Plus 1 UBC LT Load Case(s)

(NDS) for Wood Construction

For proper installation of

to-nails, refer to the 2001

National Design Specification

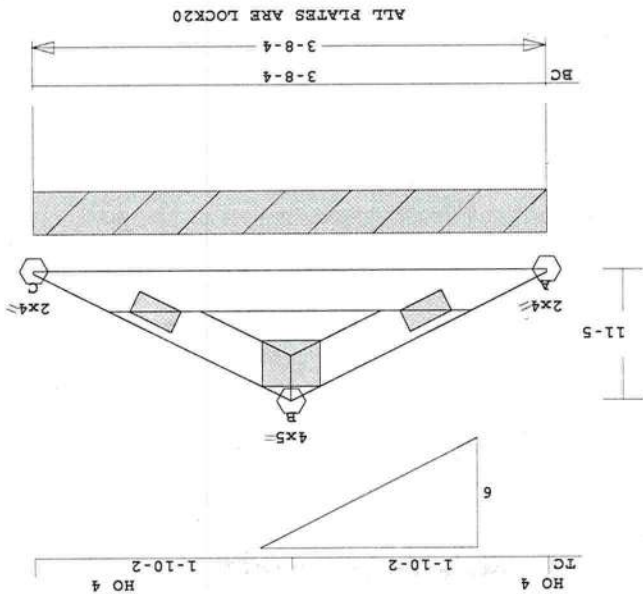
Inc./Online Plus™

Robbins Engineering, Inc./Online Plus™

Version 18.0.014 Engineering - Portal 11/15/2005 8:07:10 AM Page 1

Job	051062	Mark	VI	Quan	2	Type	VL	Span	30804	Pl-HI	6	Left OH	0	Right OH	0	Engineering
-----	--------	------	----	------	---	------	----	------	-------	-------	---	---------	---	----------	---	-------------

SR 441 - LOT 1 LAKECITY



Scale: 0.750" = 1'

Robbins Engineering, Inc./Online Plus[™] APPROX. TRUSS WEIGHT: 12.4 LBS
 Designer) Gary Dounson, PE 35054
 Gary Dounson & Associates, Inc.
 2830 NW 41st Street Suite 1
 Gainesville, FL 32606
 (352)375-8593
 CA 5201

NOTES:
 Trusses Manufactured by:
 RIDGWAY ROOF TRUSS
 Analysis Conforms To:
 FBC2004
 Design checked for 10 psf non-concurrent LL on BC.
 Wind Loads - ANSI / ASCE 7-02
 Truss is designed as
 Components and Claddings*
 for Exterior zone location.
 Wind Speed: 110 mph
 Mean Roof Height: 25'-0"
 Exposure Category: B
 Occupancy Factor: 1.00
 Building Type: Enclosed
 TC Dead Load: 4.0 psf
 BC Dead Load: 6.0 psf
 Max comp. force 51 Lbs
 Quality Control Factor 1.25

TO BE MANUFACTURED BY
 ROBBINS ENGINEERING, INC.

Plate - LOCK 20 Ga, Gross Area
 Plate - RHS 20 Ga, Gross Area

From To
 TC Cont. 0-0-0 3-8-4
 BC Cont. 0-0-0 3-8-4

Loading Live Dead (psf)
 TC 20.0 7.0
 BC 0.0 10.0
 Total 20.0 17.0

Spacing
 Lumber Duration Factor 1.25
 Plate Duration Factor 1.25

TC Pb=1.15 Fc=1.10 Ft=1.10
 BC Pb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
 Plus 1 UBC LL Load Case(s)

React Uplift Size Req'd
 Lbs Lbs In-Sx In-Sx
 Cont. Brg 0-0-0 to 3-8-4

Member CSI P Lbs Axl-CST-Bnd
 Top Chords-----
 A-B 0.01 96 T 0.01 0.00
 B-C 0.01 96 T 0.01 0.00
 A-C 0.01 0 T 0.00 0.01

FABRICATOR NOTES:
 1. Delegated Engineer (Truss

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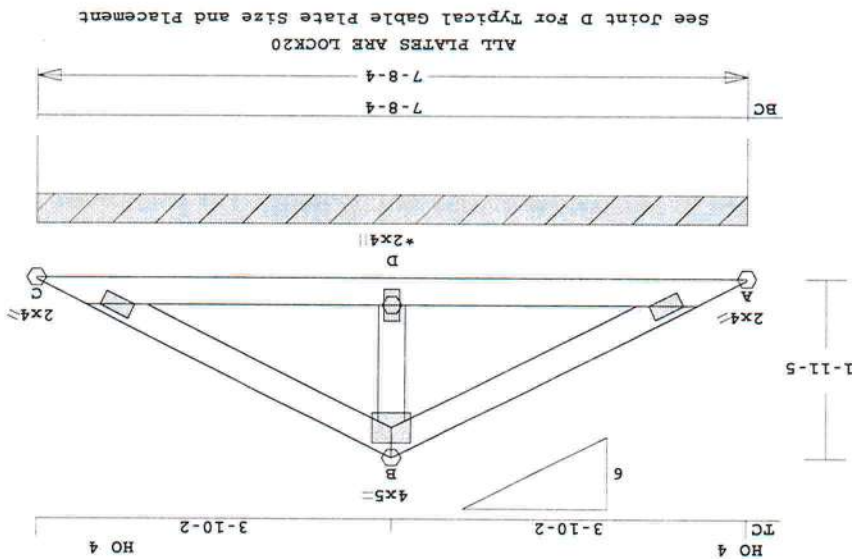
CSI -Size-	SP-#3	SP-#2	SP-#2	SP-#4	SP-#4	SP-#4
GW	0.04	2x 4	2x 4	2x 4	2x 4	2x 4
BC	0.04	2x 4	2x 4	2x 4	2x 4	2x 4
TC	0.10	2x 4	2x 4	2x 4	2x 4	2x 4
-----Lumber-----						
Brace truss as follows:						
O.C.						
From						
To						
TC Cont.						
BC Cont.						
0 - 0 - 0						
0 - 0 - 0						
7 - 8 - 4						
7 - 8 - 4						
Loading						
Live						
Dead (psf)						
20.0						
7.0						
BC						
0.0						
10.0						
Total						
20.0						
17.0						
37.0						
Spacing						
24.0"						
Lumber Duration Factor						
1.25						
Plate Duration Factor						
1.25						
TC Fb=1.15						
FC=1.10						
Ft=1.10						
BC Fb=1.10						
FC=1.10						
Ft=1.10						
Plus 9 Wind Load Case(s)						
Plus 1 UBC IL Load Case(s)						
UT React Uplift Size Req'd						
Lbs						
In-Sx In-Sx						
Cont. Brg						
0 - 0 - 0						
7 - 8 - 4						
483						
163						
Hz =						
31						
Member						
CSI						
P Lbs						
Ax1-End						
-----Top Chords-----						
A - B						
0.10						
88 T						
0.01						
0.09						
B - C						
0.10						
88 T						
0.01						
0.09						
-----Bottom Chords-----						

Online Plus -- Version 18.0.014
RUN DATE: 14-NOV-05

Quality Control Factor		Quality Control Factor	
107 lbs	6.0 psf	107 lbs	6.0 psf
Max comp. force	BC Dead Load:	Max comp. force	BC Dead Load:
0.04	0.04	0.04	0.04
0 T	0 T	0 T	0 T
0.04	0.04	0.04	0.04
Gable Webs-----		Gable Webs-----	
D-B 0.04	174 T	D-B 0.04	174 T

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 30.9 LBS

Scale: 0.500" = 1'



ALL PLATES ARE LOCK20

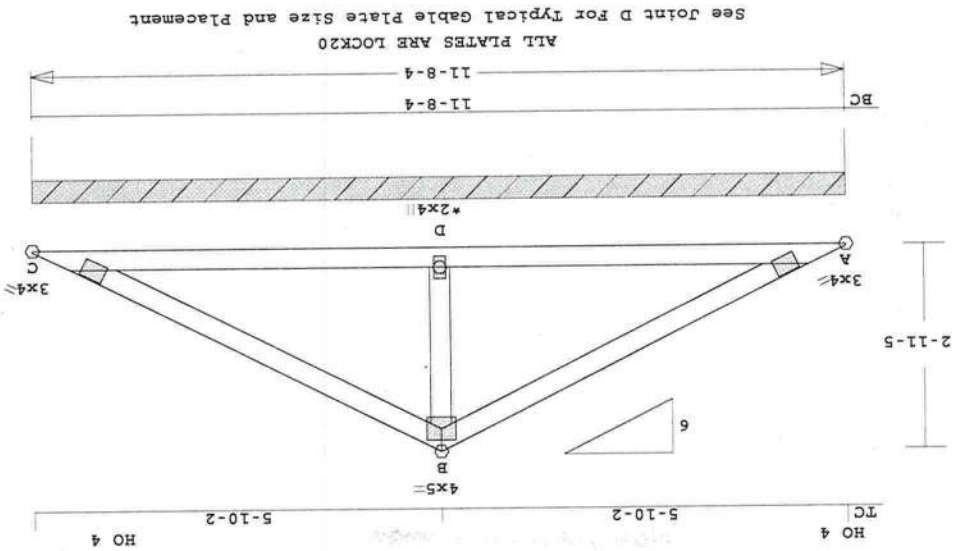
See Joint D For Typical Gable Plate Size and Placement

SR 441 - LOT 1 LAKECITY

Job	051062	Mark	V2	Quan	2	Type	VL	Span	70804	P1-H1	6	Left OH	0	Right OH	0	Engineering
-----	--------	------	----	------	---	------	----	------	-------	-------	---	---------	---	----------	---	-------------

Job	051062	Mark	V3	Quan	2	Type	VL	Span	110804	Pl-HI	6	Left OH	0	Right OH	0	Engineering
-----	--------	------	----	------	---	------	----	------	--------	-------	---	---------	---	----------	---	-------------

SR 441 - LOT 1 LAKECITY



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus[™] APPROX. TRUSS WEIGHT: 48.8 LBS
 A-D 0.13 0 T 0.00 0.13
 D-C 0.13 0 T 0.00 0.13
 D-B 0.06 250 T
 Online Plus -- Version 18.0.014
 RUN DATE: 14-NOV-05

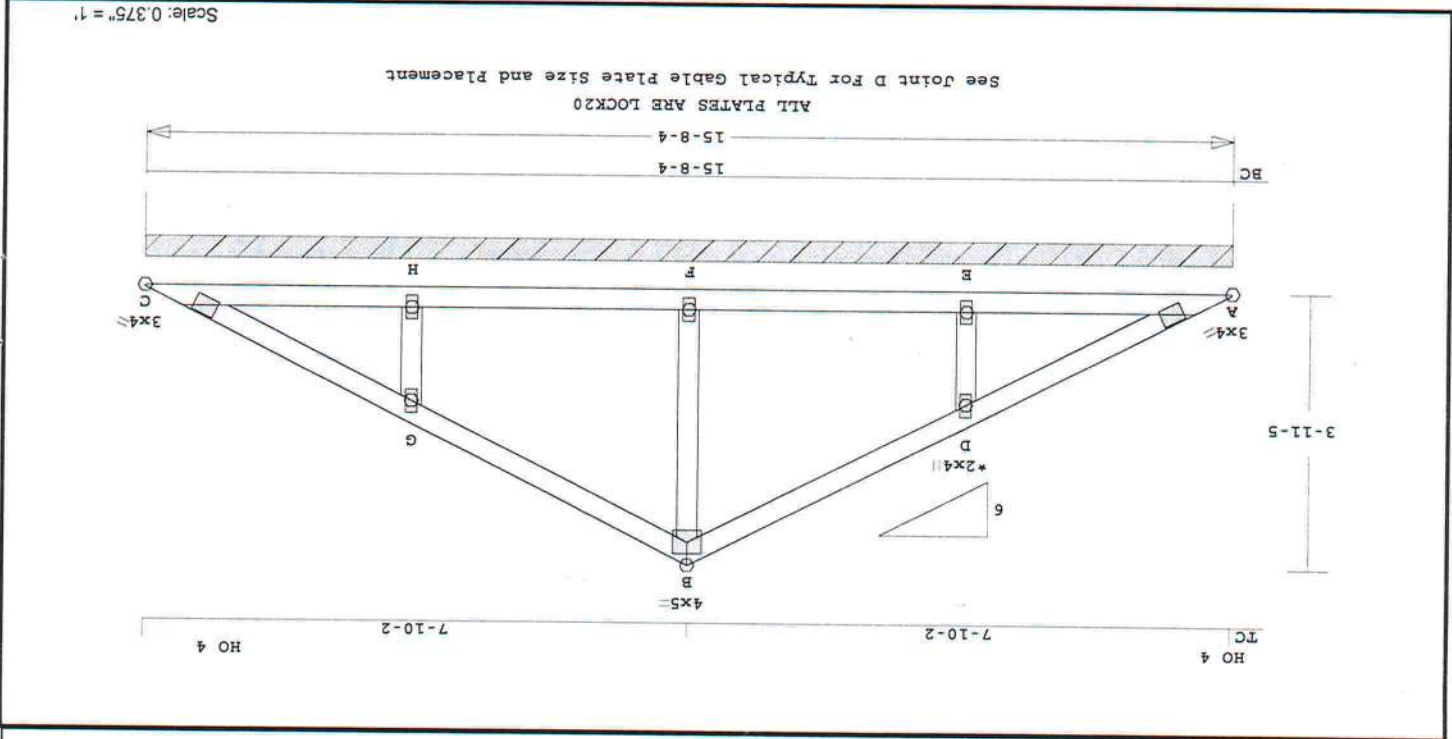
CSI -Size- ---Lumber-
 TC 0.28 2x 4 SP-#2
 BC 0.13 2x 4 SP-#2
 GW 0.06 2x 4 SP-#3
 Brace truss as follows:
 O.C.
 TC Cont. 0-0-0 11-8-4
 BC Cont. 0-0-0 11-8-4
 Loading Live Dead (psf)
 TC 20.0 7.0
 BC 0.0 10.0
 Total 20.0 17.0
 Spacing 24.0"
 Lumber Duration Factor 1.25
 Plate Duration Factor 1.25
 TC Pb=1.15 Fc=1.10 Ft=1.10
 BC Pb=1.10 Fc=1.10 Ft=1.10
 Plus 9 Wind Load Case(s)
 Plus 1 UBC TL Load Case(s)

UT React Uplift Size Req'd
 Lbs In-Sx In-Sx
 Cont. Brg 0-0-0 to 11-8-4
 779 263 Hz = 51
 Membri CSI P Lbs AXI-CSI-Bnd
 ---Top Chords-
 A-B 0.28 143 T 0.01 0.27
 B-C 0.28 143 T 0.01 0.27
 ---Bottom Chords-
 Notes:
 Trusses Manufactured by:
 RIDGWAY ROOF TRUSS
 Analysis Conforms To:
 FBC2004
 Design checked for 10 psf non-concurrent LL on BC.
 Wind Loads - ANSI / ASCE 7-02
 Truss is designed as
 Components and Claddings*
 for Exterior zone location.
 Wind Speed: 110 mph
 Mean Roof Height: 25-0
 Exposure Category: B
 Occupancy Factor: 1.00
 Building Type: Enclosed
 TC Dead Load: 4.0 psf

FABRICATOR NOTES:
 1. Delegated Engineer (Truss Designer)
 Gary Dounson, PE 35054
 Gary Dounson & Associates, Inc.
 2830 NW 41st Street Suite D
 Gainesville, FL 32606
 (352)375-8593
 CA 5201

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Job	051062	Mark	V4	Quan	1	Type	VL	Span	150804	Pl-HI	6	Left OH	0	Right OH	0	Engineering
-----	--------	------	----	------	---	------	----	------	--------	-------	---	---------	---	----------	---	-------------



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 72.0 LBS

Online Plus -- Version 18.0.014

RUN DATE: 14-NOV-05

CS1 - Size - Lumber -

TC 0.15 2x 4 SP-#2

BC 0.09 2x 4 SP-#2

GW 0.07 2x 4 SP-#3

Brace truss as follows:

O.C. From To

TC Cont. 0-0-0 15-8-4

BC Cont. 0-0-0 15-8-4

LT Defl 0.00" in E-F L/999

TL Defl -0.01" in E-F L/999

Shear // Grain in D-B 0.15

Plates for each ply each face.

ALU CONNECTOR PLATES

TO BE MANUFACTURED BY

ROBBINS ENGINEERING, INC.

Plate - LOCK 20 Ga, Gross Area

Plate - RHS 20 Ga, Gross Area

Uf Type Plt Size X Y JSI

A LOCK 3.0x 4.0 Ctr 0.69

D LOCK 2.0x 4.0 Ctr 0.00

B LOCK 4.0x 5.0 Ctr 0.53

G LOCK 2.0x 4.0 Ctr 0.00

C LOCK 3.0x 4.0 Ctr 0.69

E LOCK 2.0x 4.0 Ctr 0.00

F LOCK 2.0x 4.0 Ctr 0.00

H LOCK 2.0x 4.0 Ctr 0.00

NOTES:

Trusses Manufactured by:

RIDGWAY ROOF TRUSS

Analysis Conforms To:

FBC2004

Design checked for 10 psf non-concurrent LL on BC.

CS1 - Size - Lumber -

TC 0.15 2x 4 SP-#2

BC 0.09 2x 4 SP-#2

GW 0.07 2x 4 SP-#3

Brace truss as follows:

O.C. From To

TC Cont. 0-0-0 15-8-4

BC Cont. 0-0-0 15-8-4

LT Defl 0.00" in E-F L/999

TL Defl -0.01" in E-F L/999

Shear // Grain in D-B 0.15

Plates for each ply each face.

ALU CONNECTOR PLATES

TO BE MANUFACTURED BY

ROBBINS ENGINEERING, INC.

Plate - LOCK 20 Ga, Gross Area

Plate - RHS 20 Ga, Gross Area

Uf Type Plt Size X Y JSI

A LOCK 3.0x 4.0 Ctr 0.69

D LOCK 2.0x 4.0 Ctr 0.00

B LOCK 4.0x 5.0 Ctr 0.53

G LOCK 2.0x 4.0 Ctr 0.00

C LOCK 3.0x 4.0 Ctr 0.69

E LOCK 2.0x 4.0 Ctr 0.00

F LOCK 2.0x 4.0 Ctr 0.00

H LOCK 2.0x 4.0 Ctr 0.00

NOTES:

Trusses Manufactured by:

RIDGWAY ROOF TRUSS

Analysis Conforms To:

FBC2004

Design checked for 10 psf non-concurrent LL on BC.

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 72.0 LBS

Online Plus -- Version 18.0.014

RUN DATE: 14-NOV-05

CS1 - Size - Lumber -

TC 0.15 2x 4 SP-#2

BC 0.09 2x 4 SP-#2

GW 0.07 2x 4 SP-#3

Brace truss as follows:

O.C. From To

TC Cont. 0-0-0 15-8-4

BC Cont. 0-0-0 15-8-4

LT Defl 0.00" in E-F L/999

TL Defl -0.01" in E-F L/999

Shear // Grain in D-B 0.15

Plates for each ply each face.

ALU CONNECTOR PLATES

TO BE MANUFACTURED BY

ROBBINS ENGINEERING, INC.

Plate - LOCK 20 Ga, Gross Area

Plate - RHS 20 Ga, Gross Area

Uf Type Plt Size X Y JSI

A LOCK 3.0x 4.0 Ctr 0.69

D LOCK 2.0x 4.0 Ctr 0.00

B LOCK 4.0x 5.0 Ctr 0.53

G LOCK 2.0x 4.0 Ctr 0.00

C LOCK 3.0x 4.0 Ctr 0.69

E LOCK 2.0x 4.0 Ctr 0.00

F LOCK 2.0x 4.0 Ctr 0.00

H LOCK 2.0x 4.0 Ctr 0.00

NOTES:

Trusses Manufactured by:

RIDGWAY ROOF TRUSS

Analysis Conforms To:

FBC2004

Design checked for 10 psf non-concurrent LL on BC.

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 72.0 LBS

Online Plus -- Version 18.0.014

RUN DATE: 14-NOV-05

CS1 - Size - Lumber -

TC 0.15 2x 4 SP-#2

BC 0.09 2x 4 SP-#2

GW 0.07 2x 4 SP-#3

Brace truss as follows:

O.C. From To

TC Cont. 0-0-0 15-8-4

BC Cont. 0-0-0 15-8-4

LT Defl 0.00" in E-F L/999

TL Defl -0.01" in E-F L/999

Shear // Grain in D-B 0.15

Plates for each ply each face.

ALU CONNECTOR PLATES

TO BE MANUFACTURED BY

ROBBINS ENGINEERING, INC.

Plate - LOCK 20 Ga, Gross Area

Plate - RHS 20 Ga, Gross Area

Uf Type Plt Size X Y JSI

A LOCK 3.0x 4.0 Ctr 0.69

D LOCK 2.0x 4.0 Ctr 0.00

B LOCK 4.0x 5.0 Ctr 0.53

G LOCK 2.0x 4.0 Ctr 0.00

C LOCK 3.0x 4.0 Ctr 0.69

E LOCK 2.0x 4.0 Ctr 0.00

F LOCK 2.0x 4.0 Ctr 0.00

H LOCK 2.0x 4.0 Ctr 0.00

NOTES:

Trusses Manufactured by:

RIDGWAY ROOF TRUSS

Analysis Conforms To:

FBC2004

Design checked for 10 psf non-concurrent LL on BC.

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 72.0 LBS

Online Plus -- Version 18.0.014

RUN DATE: 14-NOV-05

CS1 - Size - Lumber -

TC 0.15 2x 4 SP-#2

BC 0.09 2x 4 SP-#2

GW 0.07 2x 4 SP-#3

Brace truss as follows:

O.C. From To

TC Cont. 0-0-0 15-8-4

BC Cont. 0-0-0 15-8-4

LT Defl 0.00" in E-F L/999

TL Defl -0.01" in E-F L/999

Shear // Grain in D-B 0.15

Plates for each ply each face.

ALU CONNECTOR PLATES

TO BE MANUFACTURED BY

ROBBINS ENGINEERING, INC.

Plate - LOCK 20 Ga, Gross Area

Plate - RHS 20 Ga, Gross Area

Uf Type Plt Size X Y JSI

A LOCK 3.0x 4.0 Ctr 0.69

D LOCK 2.0x 4.0 Ctr 0.00

B LOCK 4.0x 5.0 Ctr 0.53

G LOCK 2.0x 4.0 Ctr 0.00

C LOCK 3.0x 4.0 Ctr 0.69

E LOCK 2.0x 4.0 Ctr 0.00

F LOCK 2.0x 4.0 Ctr 0.00

H LOCK 2.0x 4.0 Ctr 0.00

NOTES:

Trusses Manufactured by:

RIDGWAY ROOF TRUSS

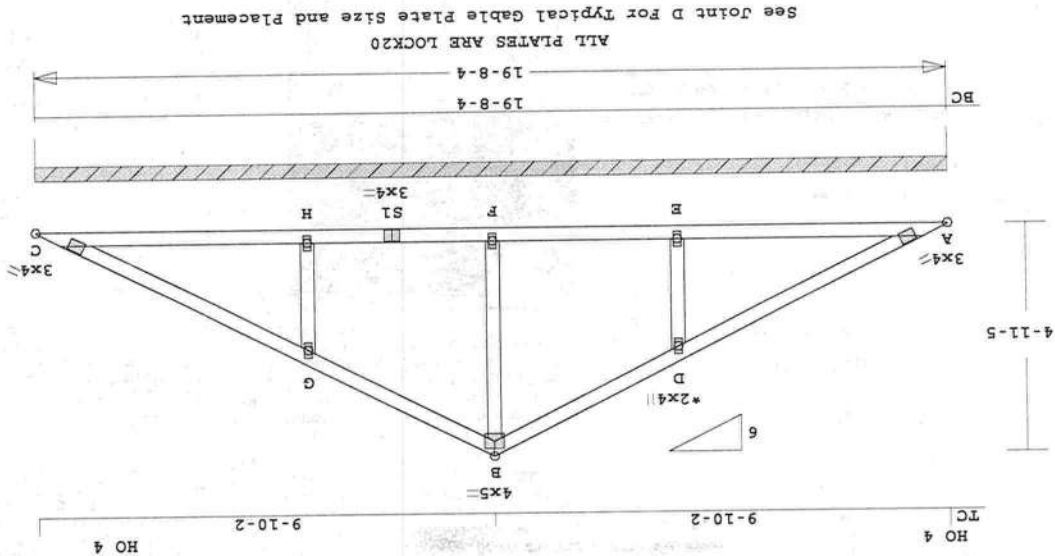
Analysis Conforms To:

FBC2004

Design checked for 10 psf non-concurrent LL on BC.

Job	051062	Mark	V5	Quan	1	Type	VL	Span	190804	Pl-H1	6	Left OH	0	Right OH	0	Engineering
-----	--------	------	----	------	---	------	----	------	--------	-------	---	---------	---	----------	---	-------------

SR 441 - LOT 1 LAKECITY



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 93.7 LBS

G - C 0.18 46 C 0.00 0.18

Bottom Chords

A - E 0.11 0 T 0.00 0.11

E - F 0.11 0 T 0.00 0.11

F - S1 0.05 0 T 0.00 0.05

S1 - H 0.09 0 T 0.00 0.09

H - C 0.10 0 T 0.00 0.10

-----Gable Webs-----

E - D 0.07 314 T

F - B 0.04 143 C

H - G 0.07 314 T

LT Defl -0.01" in H - C L/999

TL Defl -0.02" in H - C L/999

Shear // Grain in A - D 0.17

Plates for each ply each face.

ALL CONNECTOR PLATES

TO BE MANUFACTURED BY

ROBBINS ENGINEERING, INC.

Plate - LOCK 20 Ga, Gross Area

Plate - RHS 20 Ga, Gross Area

UT Type Plt Size X Y JSI

A LOCK 3.0x 4.0 Ctr Ctr 0.76

D LOCK 2.0x 4.0 Ctr Ctr 0.00

B LOCK 4.0x 5.0 Ctr Ctr 0.58

G LOCK 2.0x 4.0 Ctr Ctr 0.00

C LOCK 3.0x 4.0 Ctr Ctr 0.76

E LOCK 2.0x 4.0 Ctr Ctr 0.00

F LOCK 2.0x 4.0 Ctr Ctr 0.00

S1 LOCK 3.0x 4.0 Ctr Ctr 0.57

H LOCK 2.0x 4.0 Ctr Ctr 0.00

Notes:

Trusses Manufactured by:

RIDGWAY ROOF TRUSS

Analysis Conforms To:

FBC2004

Robbins Engineering, Inc./Online Plus™

Version 18.0.014 Engineering - Portal 11/15/2005 8:07:11 AM Page 1

Online Plus -- Version 18.0.014

RUN DATE: 14-NOV-05

CS1 - Size - Lumber

TC 0.19 2x 4 SP-#2

BC 0.11 2x 4 SP-#2

GW 0.07 2x 4 SP-#3

Brace truss as follows:

To From

TC Cont. 0 - 0 - 0 19 - 8 - 4

BC Cont. 0 - 0 - 0 19 - 8 - 4

Loading Live Dead (psf)

TC 20.0 7.0

BC 0.0 10.0

Total 20.0 17.0

Spacing 24.0"

Lumber Duration Factor 1.25

Plate Duration Factor 1.25

TC Fb=1.15 Fc=1.10

BC Fb=1.10 Fc=1.10

plus 9 Wind Load Case(s)

plus 1 UBC LL Load Case(s)

UT React Uplift Size Req'd

Lbs In-Sx In-Sx

Cont. Brg 0 - 0 - 0 to 19 - 8 - 4

1371 463 Hz = 90

Member CSI P Lbs AXI-CSI-Bnd

-----Top Chords-----

A - D 0.18 46 C 0.00 0.18

D - B 0.19 136 T 0.01 0.18

B - G 0.19 136 T 0.01 0.18

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Version 18.0.014 Engineering - Portal 11/15/2005 8:07:11 AM Page 1

NOTAS GENERALES

The drawings may specify locations of Truss Design Drawings on individual compression members. Refer to the BCS-173 Summary Sheet - Web Member Bracing/Weld Connections for more information. All other members secondary (Web) per major information. The Designer of the Building Design is responsible for the responsibility

Los dibujos de diseño de los trusses pueden especificar las ubicaciones de los miembros permanentes en los miembros individuales en compresión. Vea la hoja Resumen BCS-173 para más información sobre los miembros secundarios (web) por la mayor información. El Diseñador del Edificio es responsable de la responsabilidad

 The consequences of improper handling, installing and bracing may be a collapse of the structure, or worse, serious personal injury or death.

Banding and furr plates have sharp edges. Wear gloves when handling and safety glasses when cutting banding.

HANDLING — MANEJO

A

No permita mas
de 3 pulgadas de
pendido por cada 10
pies de tramo.

Use special care in
windy weather or
near power lines or
airports.

Utilice cuidado
especial en días
ventosos o cerca de
líneas eléctricas y de
aeropuertos.

Avoid lateral bending. — Evite la flexión lateral.

unbraced bundles upright.



BRACING – ARRIOSTRE

See el resumen BCS-B2 - Instalación de Trusses y Arroscure Temporal para mayor información.

 Do not walk on unbraced trusses.
No camine en trusses sueltos.

 Locate ground braces for first truss directly in line with all rows of top chord temporary bracing.

Coque los amortecedores en línea con
primer truss directamente en línea con
cada una de las truss laterales
temporales de la cuerda superior.

Brace first truss well
before erection of
additional trusses.

Top Chord Temporary Lateral Bracing (TCLB)

The diagram illustrates a roof truss system with the following labels and dimensions:

- TRUSSES UP TO AND OVER 60'**: Dimension for the main span of the trusses.
- TRUSSES HASTA Y SOBRE 60'**: Dimension for the main span of the trusses (Spanish text).
- Tagline**: A line extending from the peak of the truss.
- 3/4 truss length**: Dimension for the length of the truss section.
- Spreader bar 2/3 to**: Dimension for the spreader bar.
- 10' o.c.**: Dimension for the spacing of the trusses.
- or sillback**: Dimension for the sillback.
- mid-height**: Dimension for the mid-height of the truss.
- above**: Dimension for the height above the truss.

Diagram illustrating the components and dimensions of a roof truss system:

- Trusses up to 60°**: Indicated by arrows pointing to the roof structure.
- Trusses up to 60°**: Indicated by arrows pointing to the roof structure.
- 2/3 truss length**: Dimension for the spreader bar.
- Trusses up to 60°**: Indicated by arrows pointing to the roof structure.
- Spreader bar**: The central horizontal bar.
- Toe-in**: Indicated by arrows pointing towards the center.
- Toe-out**: Indicated by arrows pointing away from the center.
- Tagline**: Indicated by an arrow pointing to the right side of the truss.

Diagram illustrating the truss structure and dimensions:

- Tagline
- TRUSSES UP TO 30'
- TRUSSES HASTA 30'
- Approx. 1/2 truss length
- 60' or less

HOISTING RECOMMENDATIONS BY TRUSS SPAN
RECOMMENDACIONES DE LEVANTAMIENTO
POR LONGITUD DEL TRUSS

Do not lift trusses over 50' of the peak.
No levantar del pico los trusses de mas de 30 pies.

Sostenga cada truss en posición con la grúa hasta que el armazón temporal esté instalado y el truss asegurado en los soportes.

HOISTING — LEVANTAMIENTO

HAND ERECTION — LEVANTAMIENTO A MANO

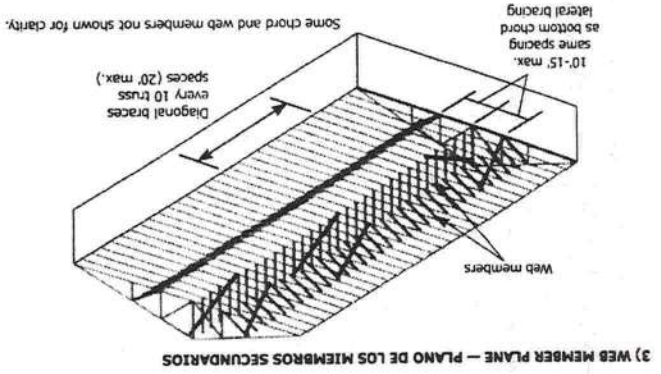
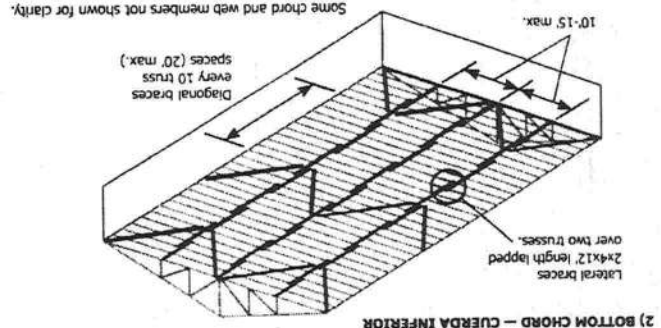
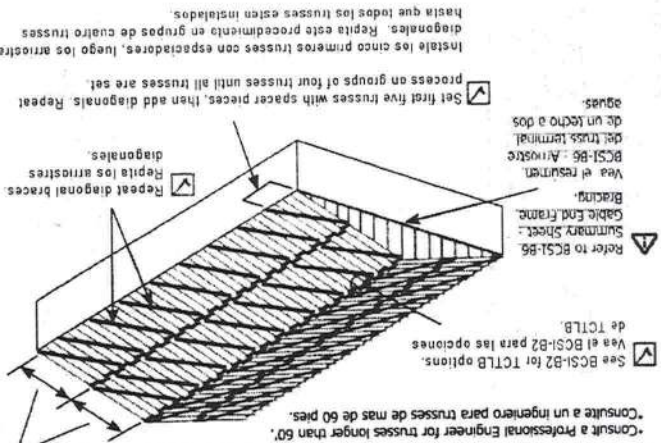
BCSI-B1 SUMMARY SHEET 2 of 2

BRACING FOR THREE PLANES OF ROOF
EL ARRIOSTRE EN TRES PLANOS DE TECHO

✓ This bracing method is for all trusses except 3x2 and 4x2 parallel chord trusses. Este método de arrioste es para todo trusses excepto trusses de cuerdas paralelas 3x2 y 4x2.

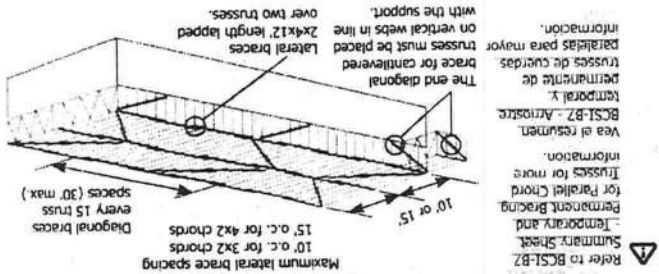
1) TOP CHORD — CUERDA SUPERIOR

Truss Span	Top Chord Temporary Lateral Bracing (TCLB) Spacing
Up to 30'	10' o.c. max.
Hasta 30 pies	10' o.c. max.
30' to 45'	8' o.c. max.
45' to 60'	6' o.c. max.
60' to 80'	4' o.c. max.
60' to 80' max.	4' o.c. max.



DIAGONAL BRACING IS VERY IMPORTANT
EL ARRIOSTRE DIAGONAL ES MUY IMPORTANTE

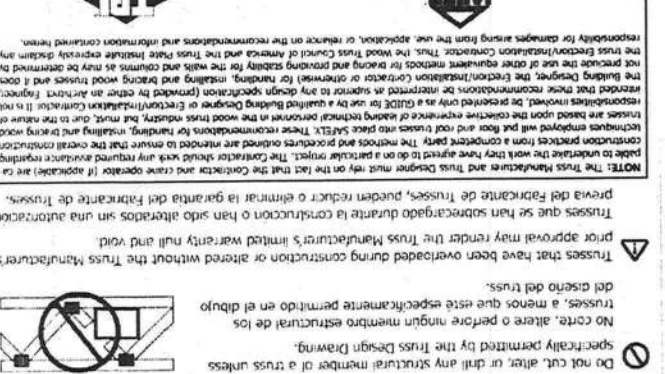
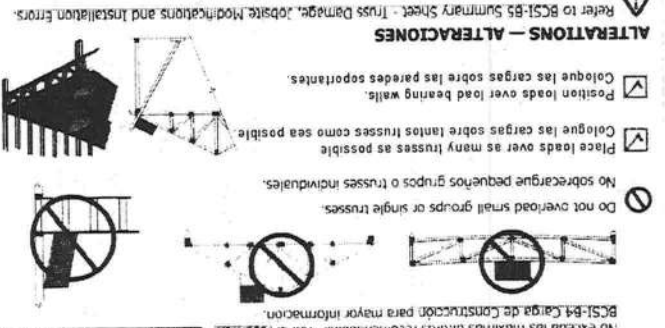
BRACING FOR 3x2 AND 4x2 PARALLEL CHORD TRUSSES
EL ARRIOSTRE PARA TRUSSES DE CUERDAS PARALELAS 3x2 Y 4x2



INSTALLING — INSTALACION

Max. Truss Length	Max. Bow	Length	Max. Bow	Length	Max. Bow
14.6'	7/8"	14.6'	7/8"	14.6'	7/8"
16.7'	1"	16.7'	1"	16.7'	1"
18.8'	1-1/8"	18.8'	1-1/8"	18.8'	1-1/8"
20.8'	1-1/4"	20.8'	1-1/4"	20.8'	1-1/4"
22.9'	1-3/8"	22.9'	1-3/8"	22.9'	1-3/8"
25.0'	1-1/2"	25.0'	1-1/2"	25.0'	1-1/2"
29.2'	1-3/4"	29.2'	1-3/4"	29.2'	1-3/4"
33.3'	2"	33.3'	2"	33.3'	2"

Material	Height (ft)	Height (m)
Gypsum Board	12'	3.7
Plywood or OSB	15'	4.6
Asphalt Shingles	2 bundles	8'
Concrete Block	8'	2.4
Clay Tile	3-4 tiles high	8'



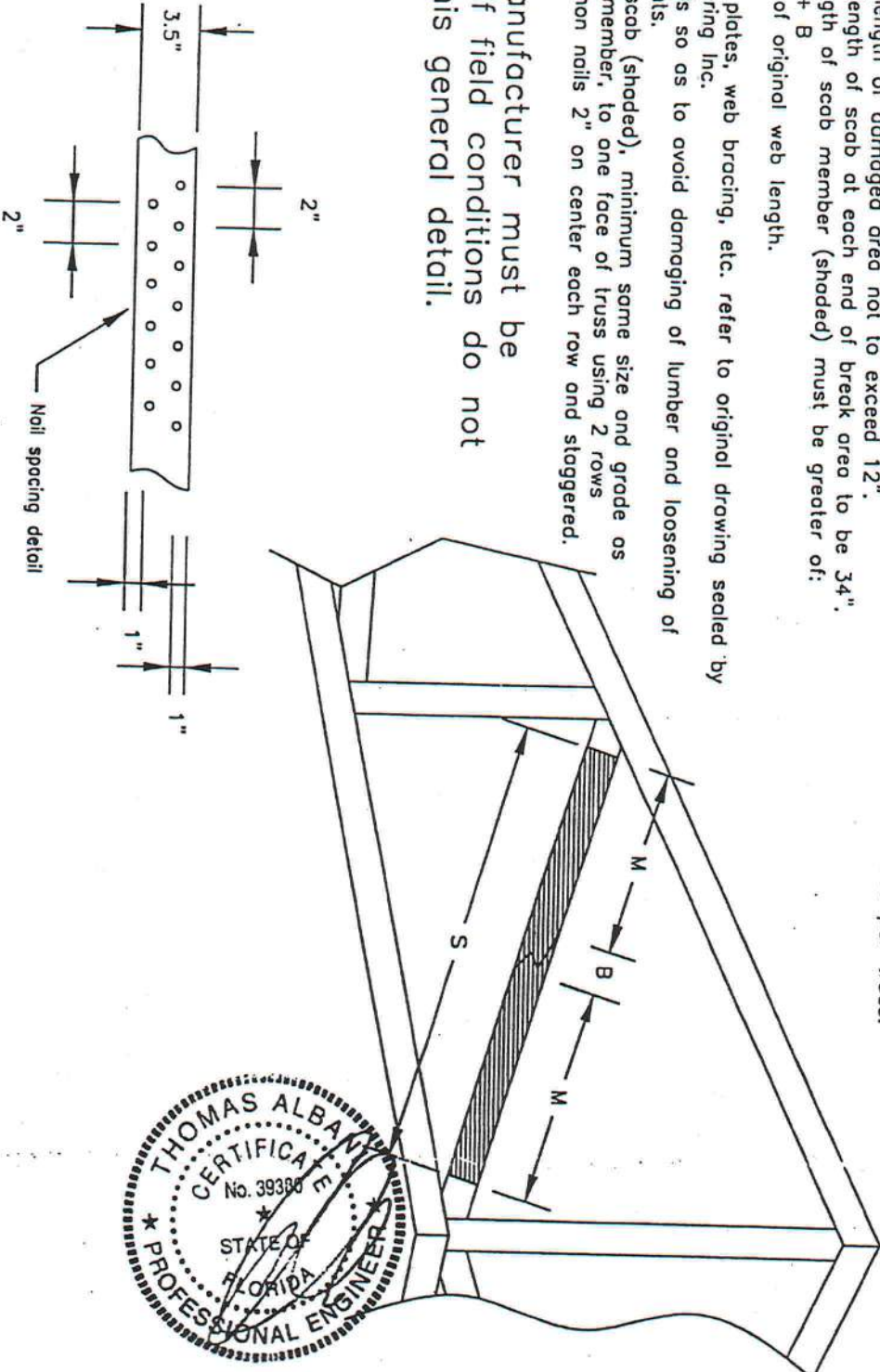
WOOD TRUSS COUNCIL OF AMERICA
WOOD TRUSS INSTITUTE

$S = 68'' + B$
 $S = 80\%$ of original web length.

- 1) Apply all nails so as to avoid damaging of lumber and loosening of plates at joints.

2) Attach 2x4 scab (shodded), minimum same size and grade as original web member, to one face of truss using 2 rows of 10d common nails 2" on center each row and staggered.

Truss manufacturer must be notified if field conditions do not match this general detail.



A circular professional engineer seal for Thomas Albany, State of Florida, No. 39385. The seal features the text "THOMAS ALBANY" at the top, "CERTIFICATE" in the center, "No. 39385" below it, "STATE OF FLORIDA" below that, and "PROFESSIONAL ENGINEER" at the bottom. The seal is surrounded by a dotted border and has a signature across it.

ROBBINS
Engineering Inc.

P.O.Box 280055, Tampa, FL 33688

Roberts v. U.S., 40 Fed. Cl. 696, 698, 40-1519 (A.C.S. 35 Code 40) (1963), 107-2 USTC ¶9531, 30 AFTR2d 63-6087 (Ct. Cl. 1963), cert. denied, 374 U.S. 830 (1963), 1963-2 CB 323, 324, 325, 326, 327, 328, 329, 330, 331, 332, 333, 334, 335, 336, 337, 338, 339, 340, 341, 342, 343, 344, 345, 346, 347, 348, 349, 350, 351, 352, 353, 354, 355, 356, 357, 358, 359, 360, 361, 362, 363, 364, 365, 366, 367, 368, 369, 370, 371, 372, 373, 374, 375, 376, 377, 378, 379, 380, 381, 382, 383, 384, 385, 386, 387, 388, 389, 390, 391, 392, 393, 394, 395, 396, 397, 398, 399, 400, 401, 402, 403, 404, 405, 406, 407, 408, 409, 410, 411, 412, 413, 414, 415, 416, 417, 418, 419, 420, 421, 422, 423, 424, 425, 426, 427, 428, 429, 430, 431, 432, 433, 434, 435, 436, 437, 438, 439, 440, 441, 442, 443, 444, 445, 446, 447, 448, 449, 450, 451, 452, 453, 454, 455, 456, 457, 458, 459, 460, 461, 462, 463, 464, 465, 466, 467, 468, 469, 470, 471, 472, 473, 474, 475, 476, 477, 478, 479, 480, 481, 482, 483, 484, 485, 486, 487, 488, 489, 490, 491, 492, 493, 494, 495, 496, 497, 498, 499, 500, 501, 502, 503, 504, 505, 506, 507, 508, 509, 510, 511, 512, 513, 514, 515, 516, 517, 518, 519, 520, 521, 522, 523, 524, 525, 526, 527, 528, 529, 530, 531, 532, 533, 534, 535, 536, 537, 538, 539, 540, 541, 542, 543, 544, 545, 546, 547, 548, 549, 550, 551, 552, 553, 554, 555, 556, 557, 558, 559, 560, 561, 562, 563, 564, 565, 566, 567, 568, 569, 570, 571, 572, 573, 574, 575, 576, 577, 578, 579, 580, 581, 582, 583, 584, 585, 586, 587, 588, 589, 590, 591, 592, 593, 594, 595, 596, 597, 598, 599, 600, 601, 602, 603, 604, 605, 606, 607, 608, 609, 610, 611, 612, 613, 614, 615, 616, 617, 618, 619, 620, 621, 622, 623, 624, 625, 626, 627, 628, 629, 630, 631, 632, 633, 634, 635, 636, 637, 638, 639, 640, 641, 642, 643, 644, 645, 646, 647, 648, 649, 650, 651, 652, 653, 654, 655, 656, 657, 658, 659, 660, 661, 662, 663, 664, 665, 666, 667, 668, 669, 670, 671, 672, 673, 674, 675, 676, 677, 678, 679, 680, 681, 682, 683, 684, 685, 686, 687, 688, 689, 690, 691, 692, 693, 694, 695, 696, 697, 698, 699, 700, 701, 702, 703, 704, 705, 706, 707, 708, 709, 710, 711, 712, 713, 714, 715, 716, 717, 718, 719, 720, 721, 722, 723, 724, 725, 726, 727, 728, 729, 730, 731, 732, 733, 734, 735, 736, 737, 738, 739, 740, 741, 742, 743, 744, 745, 746, 747, 748, 749, 750, 751, 752, 753, 754, 755, 756, 757, 758, 759, 760, 761, 762, 763, 764, 765, 766, 767, 768, 769, 770, 771, 772, 773, 774, 775, 776, 777, 778, 779, 780, 781, 782, 783, 784, 785, 786, 787, 788, 789, 790, 791, 792, 793, 794, 795, 796, 797, 798, 799, 800, 801, 802, 803, 804, 805, 806, 807, 808, 809, 810, 811, 812, 813, 814, 815, 816, 817, 818, 819, 820, 821, 822, 823, 824, 825, 826, 827, 828, 829, 830, 831, 832, 833, 834, 835, 836, 837, 838, 839, 840, 841, 842, 843, 844, 845, 846, 847, 848, 849, 850, 851, 852, 853, 854, 855, 856, 857, 858, 859, 860, 861, 862, 863, 864, 865, 866, 867, 868, 869, 870, 871, 872, 873, 874, 875, 876, 877, 878, 879, 880, 881, 882, 883, 884, 885, 886, 887, 888, 889, 890, 891, 892, 893, 894, 895, 896, 897, 898, 899, 900, 901, 902, 903, 904, 905, 906, 907, 908, 909, 910, 911, 912, 913, 914, 915, 916, 917, 918, 919, 920, 921, 922, 923, 924, 925, 926, 927, 928, 929, 930, 931, 932, 933, 934, 935, 936, 937, 938, 939, 940, 941, 942, 943, 944, 945, 946, 947, 948, 949, 950, 951, 952, 953, 954, 955, 956, 957, 958, 959, 960, 961, 962, 963, 964, 965, 966, 967, 968, 969, 970, 971, 972, 973, 974, 975, 976, 977, 978, 979, 980, 981, 982, 983, 984, 985, 986, 987, 988, 989, 990, 991, 992, 993, 994, 995, 996, 997, 998, 999, 1000.

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Designed By: MG
Checked By: TAA
Date: 5/23/02
Dwg. No: GD-41

TOTAL NUMBER OF NAILS ON EACH SIDE OF BREAK		.X (INCHES)	MAXIMUM FORCE (LBS) AT 33% LOAD DURATION	
2X4 SYP	2X6 SYP		2X4 SYP	2X6 SYP
13	20	2 1/4"	1924	2886
17	26	3 0"	2501	3752
21	32	3 6"	3079	4618
25	38	4 2"	3656	5484
29	44	4 8"	4233	6349

ROBBINS

Engineering Inc.

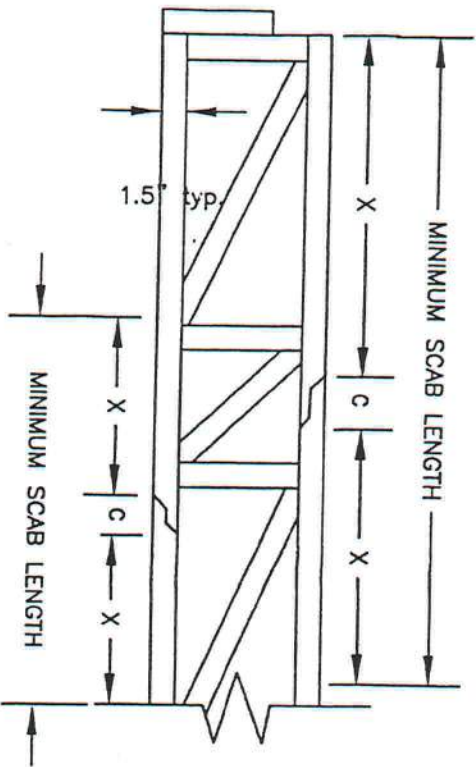
P.O.Box 260055, Tampa, FL 33602

ROBBINS LOCK connects (220 of each, steel-*STW 4453 S5 Grade 40*) bolts can be applied on both faces of flange of top joint. Center the plates using the shown dimensions by circles (6) or dimensions. Unless otherwise indicated by (6) or (10) or (12) or (14) or (16) or (18) or (20) or (22) or (24) or (26) or (28) or (30) or (32) or (34) or (36) or (38) or (40) or (42) or (44) or (46) or (48) or (50) or (52) or (54) or (56) or (58) or (60) or (62) or (64) or (66) or (68) or (70) or (72) or (74) or (76) or (78) or (80) or (82) or (84) or (86) or (88) or (90) or (92) or (94) or (96) or (98) or (100) or (102) or (104) or (106) or (108) or (110) or (112) or (114) or (116) or (118) or (120) or (122) or (124) or (126) or (128) or (130) or (132) or (134) or (136) or (138) or (140) or (142) or (144) or (146) or (148) or (150) or (152) or (154) or (156) or (158) or (160) or (162) or (164) or (166) or (168) or (170) or (172) or (174) or (176) or (178) or (180) or (182) or (184) or (186) or (188) or (190) or (192) or (194) or (196) or (198) or (200) or (202) or (204) or (206) or (208) or (210) or (212) or (214) or (216) or (218) or (220) or (222) or (224) or (226) or (228) or (230) or (232) or 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(1310) or (1

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Designed By: MC
Checked By: TAA
Rev. Date: 12/01/03
Dwg. No: GD-62-33

face of truss at the same location. Minimum end distance of 3" must be provided for all members.



TOTAL NUMBER OF NAILS AT EACH END OF	X (INCHES)	MAXIMUM AXIAL FORCE IN CHORD MEMBER (LBS.)
3	16	384
6	24	768
8	30	1024
10	36	1280
12	42	1536
14	48	1792
16	54	2048
18	60	2304
20	66	2560
22	72	2816

Truss manufacturer must be notified if field conditions do not match this general detail.

ROBBINS

Engineering Inc.

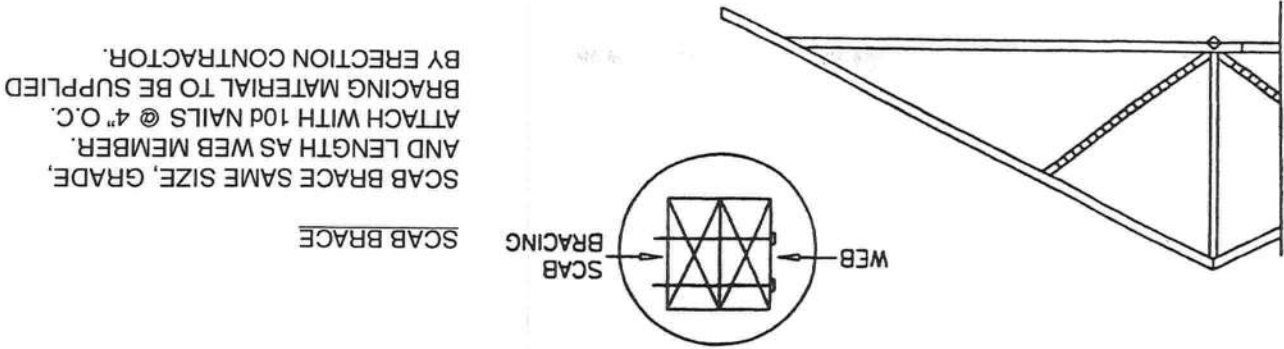
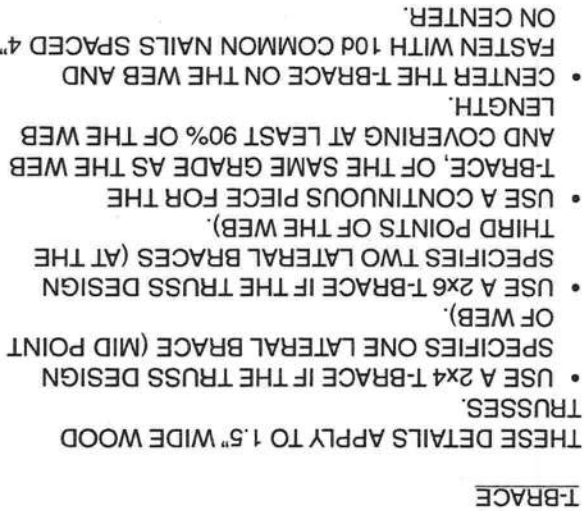
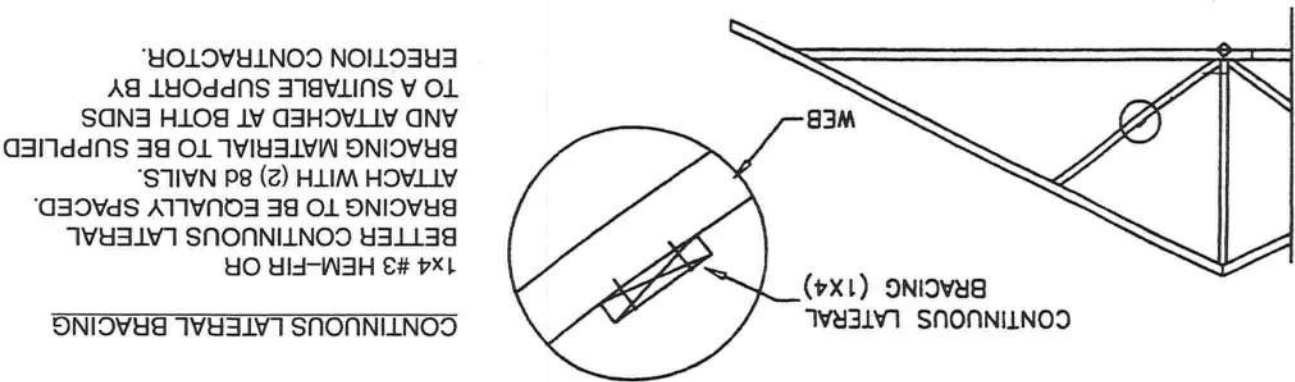
P.O.Box 280055, Tampa, FL 33682

[illegible]

FURNISH A COPY OF THIS DESIGN TO ERECTION CONTRACTOR
IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER AND TRUSS FAB. TO REVIEW THIS DRWG. &
VERIFY THAT DATA INCLUDING DIM. & LOADS CONFORM TO ARCH. PLAN/SPECS & FAB. TRUSS LAYOUTS

Designed By: MG
Checked By: TAA
Rev. Date: 12/09/03
Dwg. No: GD-62-FI

PERMANENT WEB BRACING



REPORT ON IN-PLACE DENSITY TESTS

Columbia Conference +

1. The first part of the report is a general description of the project and its objectives. This section should be written in a clear and concise manner, using simple language that is easy to understand. It should also include a brief overview of the project's history and the current status of the work.

2. The second part of the report is a detailed description of the project's progress. This section should be written in a more technical and detailed manner, using specific data and figures to support the claims. It should also include a discussion of the challenges faced during the project and the solutions that were implemented.

3. The third part of the report is a conclusion and a summary of the project's findings. This section should be written in a clear and concise manner, using simple language that is easy to understand. It should also include a brief overview of the project's history and the current status of the work.

4. The fourth part of the report is a list of references and a bibliography. This section should be written in a clear and concise manner, using simple language that is easy to understand. It should also include a brief overview of the project's history and the current status of the work.

DRY
DEN.

MAX.
DEN.

$\frac{3}{8}$ MAX.
DEN.

MOIST.
8

OPT.
MOIST.

of maximum density.

NOTE: The below tests DO/DO NOT meet the minimum _____ % compaction requirements

TYPE OF TEST: ASTM D-2922

DATE TESTED:

90-81-1

COURSE:

2

DEPTH OF TEST:

1-0

AREA TESTED: + 11 & prop 6105 pack.

10985 S. US Hwy 491

Columbia County

PROJECT

1650000 # + wia

4475 S.W. 35th Terrace • Gainesville, Florida 32608 • (352) 372-3392



UNIVERSAL ENGINEERING SCIENCES

Consultants In: Geotechnical Engineering • Environmental Sciences • Construction Materials Testing

REPORT ON IN-PLACE DENSITY TESTS



23991

NOTICE OF COMMENCEMENT FORM
COLUMBIA COUNTY, FLORIDA

***THIS DOCUMENT MUST BE RECORDED AT THE COUNTY
CLERKS OFFICE BEFORE YOUR FIRST INSPECTION.***

THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

Tax Parcel ID Number _____

1. Description of property: (legal description of the property and street address or 911 address)

See attached legal description. 10985 N. Hwy 441, Lot 2

Inst: 2006001912 Date: 01/26/2006 Time: 09:28

J. P. Dewitt Cason, Columbia County B: 1072 P: 282

2. General description of improvement:

Single Family Dwelling

3. Owner Name & Address

Edix Investments, Inc.

720 NW 23rd Ave, Gville, FL 32609

4. Name & Address of Fee Simple Owner (if other than owner):

Owner

5. Contractor Name

San Juan Construction

Phone Number (352) 665-7529

Address

P.O. Box 140515, Gville, FL 32614

6. Surety Holders Name

Phone Number _____

Amount of Bond

Merchante Bank

Phone Number (352) (904) 538-371

Address

Jacksonville, FL

(Attn: Karen Batts)

8. Persons within the State of Florida designated by the Owner upon whom notices or other documents may be served as provided by section 718.13 (1)(a) 7; Florida Statutes:

Name Edwin B. Dix

Phone Number (352) 494-4420

Address 720 NW 23rd Ave, Gville, FL 32609

9. In addition to himself/herself the owner designates

German Sanchez

of

San Juan Construction to receive a copy of the Lienor's Notice as provided in Section 713.13 (1) -

(a) 7. Phone Number of the designee

(352) 665-7529

10. Expiration date of the Notice of Commencement (the expiration date is 1 (one) year from the date of recording.

1/24/06

NOTICE AS PER CHAPTER 713, Florida Statutes:

The owner must sign the notice of commencement and no one else may be permitted to sign in his/her stead.

Signature of Owner

[Signature]

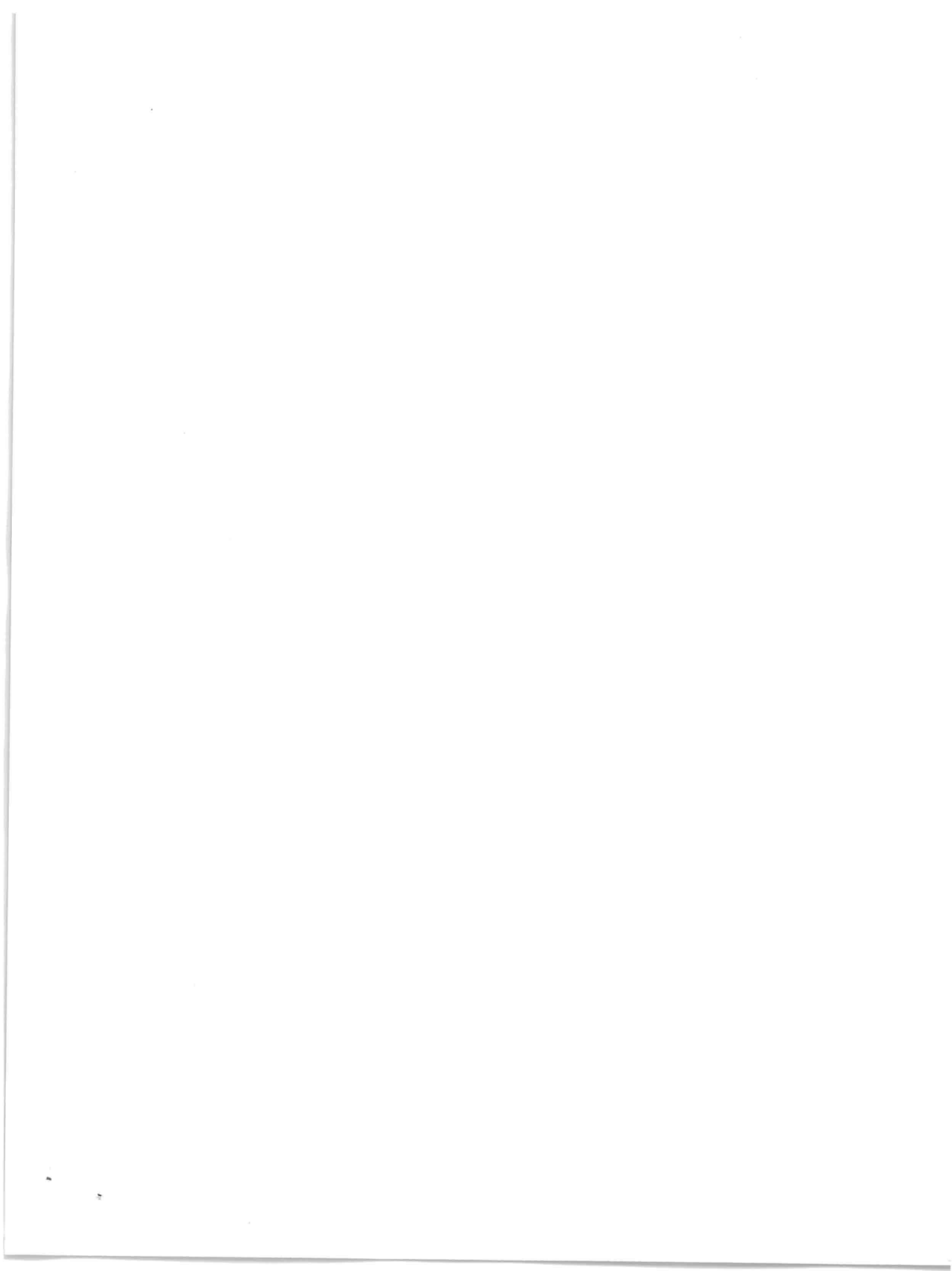


Beth Godwin
MY COMMISSION # DD189590 EXPIRES
March 10, 2007
BONDED THRU TROY FAIR INSURANCE, INC.

NOTARY STAMP/SEAL

Signature of Notary
[Signature]
Beth Godwin

Sworn to (or affirmed) and subscribed before
day of January 24th, 2006



11/21/2005 08:05 3867581337

406.00
18.50
WARRANTY DEED
RETURN TO WPMO.

Return to: (enclose self-addressed stamped envelope)

Name: Chicago Title-Gainesville

Attn: Lynn Francis

Address: 2831C NW 41st ST, Gainesville, 32608

File Number: 05-433-12

This instrument Prepared by:

Name: Chicago Title-Gainesville

Attn: Lynn Francis

Address: 2831C NW 41st ST, Gainesville, 32608

Property Appraisers Parcel Identification

Folio Number(s): R08328-008 AND R09333-000

Grantor(s) S.S. # (s)

Inst: 2005023821 Date: 09/28/2005 Time: 08:24
Doc Stamp-Deed: 406.00
DC, P. Dewitt Cason, Columbia County B: 1059 P: 2179

Inst: 2006001912 Date: 01/26/2006 Time: 09:28
DC, P. Dewitt Cason, Columbia County B: 1072 P: 283

This Warranty Deed, Made and executed this 12 SEPTEMBER 2005
by VERNAL L HUFFINES, AKA VERNAL HUFFINES, whose address is 624 INDIANA STREET,
WITCHITA FALLS, TEXAS 76301
to EDIX INVESTMENTS, INC., whose post office address is
hereinafter called the Grantor,
hereinafter called the Grantee.

(Whichever used herein the terms "Grantor" and "Grantee" include all the parties to this instrument and the heirs, legal representatives, and assigns of individuals, and the successors and assigns of corporations, wherever the context so admits or requires)

Witnesseth, That the Grantor, for and in consideration of the sum of \$10.00 (Ten and 00/100 Dollars) and other valuable considerations, receipt whereof is hereby acknowledged, hereby grants, bargains, sells, alien, remises, releases, conveys and confirms unto the Grantee, all that certain land, situate in Columbia County, State of Florida, viz:

LOTS 9, 10, 11, 12, 13, 20, 21, 22, 23 & 24, BLOCK 3 OF MASON CITY AS PER PLAT THEREOF AS RECORDED IN PLAT BOOK 1, PAGE 31 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA, TOGETHER WITH: BEGIN AT THE SW CORNER OF LOT 2 IN BLOCK 3 OF MASON CITY AS PER PLAT IN PLAT BOOK 1, PAGE 31 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA, AND RUN THENCE N 88°30'42" E., ALONG THE SOUTH LINE OF SAID LOT 2 AND LOT 31 IN SAID BLOCK 3, 294.80 FEET TO THE SE CORNER OF SAID LOT 31; THENCE N 03°55'06" W., ALONG THE EAST LINE OF SAID BLOCK 3, 85.36 FEET, THENCE S 86°11'41" W., 294.54 FEET TO A POINT ON THE EASTERLY RIGHT-OF-WAY LINE OF U.S. HIGHWAY # 41; THENCE S 03°55'06" E., ALONG SAID EASTERLY RIGHT-OF-WAY LINE, 73.65 FEET TO THE POINT OF BEGINNING, BEING ALL OF LOTS 2 AND 31 AND A PART OF LOT 1 AND 32, BLOCK 3 OF MASON CITY AS PER PLAT THEREOF AS RECORDED IN PLAT BOOK 1, PAGE 31 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.

ALSO: LOTS 3, 4, 29 & 30 IN BLOCK 3 OF MASON CITY, AS PER PLAT THEREOF RECORDED IN PLAT BOOK 1, PAGE 31 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.

THIS PROPERTY IS NOT THE HOMESTEAD OF THE GRANTOR
Together, with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

To Have and to Hold, the same in fee simple forever.

Parcel #1

Parcel #2

WARRANTY DEED
INDIVID. TO INDIVID.

Inst:2005023821 Date:03/28/2005 Time:08:24
Doc Stamp-Deed : 406.00
DC, P. Dewitt Cason, Columbia County B:1059 P:2180

SPACE ABOVE THIS LINE FOR RECORDING DATA

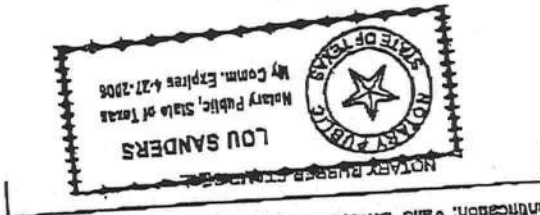
And the Grantor hereby covenants with said Grantee that the Grantor is lawfully seized of said land in fee simple; that the Grantor has good right and lawful authority to sell and convey said land; and hereby warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever; and that said land is free of all encumbrances, except taxes accruing subsequent to December 31, 2004.

In Witness Whereof, the said Grantor has signed and sealed these presents the day and year first above written.

Vernal L. Huffines
VERNAL L. HUFFINES, A/K/A VERNAL HUFFINES

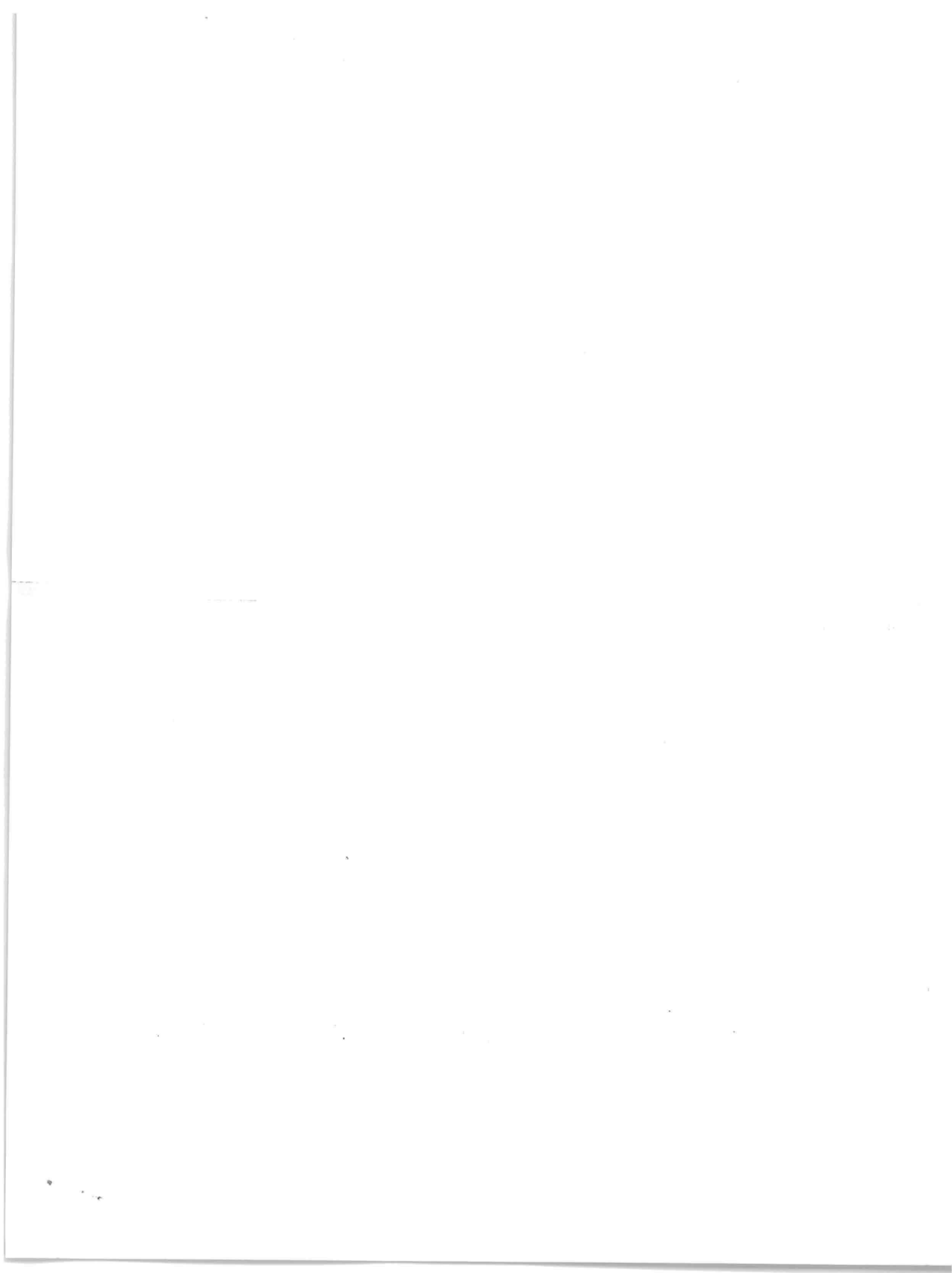
Randy Holley
Witness
Randy Holley
Printed name
Lou Sanders
Witness
Lou Sanders
Printed name

STATE OF Texas
COUNTY OF Whith
This foregoing instrument was acknowledged before me this 12th DAY OF SEPTEMBER 2005 by VERNAL L. HUFFINES, A/K/A VERNAL HUFFINES, (Check one): ☐ Said person(s) is/are personally known to me. ☒ Said person(s) provided the following type of identification: Valid Driver's License



Lou Sanders
Notary Signature
Lou Sanders
Print Name

Inst:2006001912 Date:01/26/2006 Time:09:28
DC, P. Dewitt Cason, Columbia County B:1072 P:284



4

- 4







Table with 4 columns: NO, DATE, REVISION, APPROVAL. Includes rows for 1/10/01, 1/10/01, and 1/10/01.

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Table with 4 columns: NO, DATE, REVISION, APPROVAL. Includes rows for 1/10/01, 1/10/01, and 1/10/01.

Table with 4 columns: OHD DOOR SERIES No., GENIE DOOR SERIES No., PAN STEEL THICKNESS, and other specifications.

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Table with 4 columns: OHD DOOR SERIES No., GENIE DOOR SERIES No., PAN STEEL THICKNESS, and other specifications.

NOTES: 1. TESTED IN ACCORDANCE WITH DASHA 108.

2. FOUR SECTION 7" TALL DOOR SHOWN. 8" DOORS HAVE 5 SECTIONS.

3. SECTION HEIGHTS OF 20.88, 19.00 AND 16.75 ARE AVAILABLE AND MAY BE USED IN COMBINATION TO ACHIEVE VARIOUS DOOR HEIGHTS.

4. EMBOSSED PATTERN OF 14.5 X 20.4 SHOWN. ALTERNATE PATTERNS OF 12.5 X 43.4 AND 12.5 X 20.4 MAY BE USED.

5. ARMBRUTE/REVOLUTION COUNTERBALANCE SHOWN. TORSION AND EXTENSION COUNTERBALANCE SYSTEMS AVAILABLE. 405964-0002 REQUIRED AT TRACK INTERFACE ON TORSION AND EXTENSION SYSTEMS.

6. USE THIS BRACKET 405964-0002 ON 8" HIGH DOORS ONLY.

7. LOCATE 400650 STRUTS AS SHOWN. VERTICAL PLACEMENT CAN VARY PLUS/MINUS 3 INCHES.

8. JAMB DETAIL TO BE IN ACCORDANCE WITH DRAWING 409783.

9. JAMB BRACKET ATTACHING HARDWARE SHALL BE AS FOLLOWS: JAMB 5/16 X 1-3/4" WOOD JAMB, 5/16 X 1-1/2" CONCRETE BLOCK, 5/16 X 1-1/2" EMBEDMENT - CONCRETE BLOCK.

10. USE 805911-0001 AB POINT SCREWS FOR ALL COMPONENT INSTALLATION UNLESS OTHERWISE NOTED.

11. JAMB DETAIL TO BE IN ACCORDANCE WITH DRAWING 409783.

12. JAMB BRACKET ATTACHING HARDWARE SHALL BE AS FOLLOWS: JAMB 5/16 X 1-3/4" WOOD JAMB, 5/16 X 1-1/2" CONCRETE BLOCK, 5/16 X 1-1/2" EMBEDMENT - CONCRETE BLOCK.

13. USE 805911-0001 AB POINT SCREWS FOR ALL COMPONENT INSTALLATION UNLESS OTHERWISE NOTED.

14. JAMB DETAIL TO BE IN ACCORDANCE WITH DRAWING 409783.

15. JAMB BRACKET ATTACHING HARDWARE SHALL BE AS FOLLOWS: JAMB 5/16 X 1-3/4" WOOD JAMB, 5/16 X 1-1/2" CONCRETE BLOCK, 5/16 X 1-1/2" EMBEDMENT - CONCRETE BLOCK.

16. USE 805911-0001 AB POINT SCREWS FOR ALL COMPONENT INSTALLATION UNLESS OTHERWISE NOTED.

17. JAMB DETAIL TO BE IN ACCORDANCE WITH DRAWING 409783.

18. JAMB BRACKET ATTACHING HARDWARE SHALL BE AS FOLLOWS: JAMB 5/16 X 1-3/4" WOOD JAMB, 5/16 X 1-1/2" CONCRETE BLOCK, 5/16 X 1-1/2" EMBEDMENT - CONCRETE BLOCK.

19. USE 805911-0001 AB POINT SCREWS FOR ALL COMPONENT INSTALLATION UNLESS OTHERWISE NOTED.

20. JAMB DETAIL TO BE IN ACCORDANCE WITH DRAWING 409783.

21. JAMB BRACKET ATTACHING HARDWARE SHALL BE AS FOLLOWS: JAMB 5/16 X 1-3/4" WOOD JAMB, 5/16 X 1-1/2" CONCRETE BLOCK, 5/16 X 1-1/2" EMBEDMENT - CONCRETE BLOCK.

17'-0" SHOWN (8'-0" WALL)

DOOR WIDTH

COUNTERBALANCE

COUNTERBALANCE SHAFT

ANCHOR BRACKET

DETAIL E 82.2

DETAIL D ZN A4.2

DETAIL B ZN D4.2

DETAIL C ZN B4.2

INTERIOR ELEVATION

DETAIL D ZN A4.2

DETAIL E 82.2

DETAIL B ZN D4.2

DETAIL C ZN B4.2

5.0

9.0

12.5

54.0 7" DOOR

52.8 8" DOOR

35.8

22.5

5.0

13.2

8 (TYPICAL ALL LOCATIONS)

SHAFT & (VARIES)

405964-0002 SCAB BRACKET

046450-0003 JAMB BRACKET

405964-0001 SCAB BRACKET

405964-0002 SCAB BRACKET

046450-0003 JAMB BRACKET

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