

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

RE: 2332916 - EDGLEY - NELSON RES.

MiTek USA, Inc.

6904 Parke East Blvd. Tampa, FL 33610-4115

Site Information:

Customer Info: Edgley Const. Project Name: Nelson Res Model: Custom

Lot/Block: N/A

Subdivision: N/A

Address: TBD Pinemount Rd, N/A

City: Columbia Cty

State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name:

License #:

Address:

City:

State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2017/TPI2014

Design Program: MiTek 20/20 8.2

Wind Code: ASCE 7-10

Wind Speed: 130 mph

Roof Load: 37.0 psf

Floor Load: N/A psf

This package includes 10 individual, Truss Design Drawings and 0 Additional Drawings. With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No.	Seal#	Truss Name	Date
1	T20094749	T01	4/29/20
2	T20094750	T01G	4/29/20
2 3 4	T20094751	T02	4/29/20
4	T20094752	T02G	4/29/20
5 6 7	T20094753	T03	4/29/20
6	T20094754	T04	4/29/20
7	T20094755	T04G	4/29/20
8	T20094756	T05	4/29/20
9	T20094757	T06	4/29/20
10	T20094758	T06G	1/20/20



The truss drawing(s) referenced above have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Builders FirstSource-Jacksonville.

Truss Design Engineer's Name: Velez, Joaquin

My license renewal date for the state of Florida is February 28, 2021.

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

April 29,2020

Job Truss Truss Type Qty EDGLEY - NELSON RES. T20094749 2332916 T01 Common Job Reference (optional) 8.240 s Mar 9 2020 MiTek Industries, Inc. Wed Apr 29 12:50:50 2020 Page 1 Builders FirstSource Jacksonville, FL - 32244, ID:H14GiFDt0QwCPd2wj9FV1GySYUT-co\_xX7KEiKUdiUm4v\_Q6ZlyfmQhkH6dsYZwzkXzLry3 14-1-9 19-10-0 21-10-0 -2-0-0 2-0-0 9-11-0 2-0-0

Scale = 1:38.2

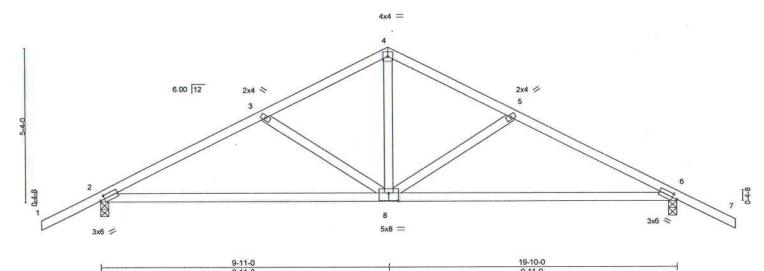


Plate Offset	ts (X,Y)- [	2:0-1-15,0-1-8], [6:0-1-1	9-11-0 5,0-1-8], [8:0-4-	0,0-3-4]						-11-0		
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
	20.0	Plate Grip DOL	1.25	TC	0.41	Vert(LL)	-0.16	8-14	>999	240	MT20	244/190
CDL	7.0	Lumber DOL	1.25	BC	0.90	Vert(CT)	-0.33	8-14	>731	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.22	Horz(CT)	0.03	6	n/a	n/a		
	10.0	Code FBC2017/T	PI2014	Matri	x-MS	1 12					Weight: 91 lb	FT = 20%

BRACING-

TOP CHORD

**BOT CHORD** 

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3

REACTIONS. (size) 2=0-3-8, 6=0-3-8

Max Horz 2=125(LC 12)

Max Uplift 2=-343(LC 12), 6=-343(LC 13) Max Grav 2=842(LC 1), 6=842(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-1166/635, 3-4=-897/498, 4-5=-897/498, 5-6=-1166/635

BOT CHORD 2-8=-415/1009, 6-8=-432/1009

WEBS 4-8=-262/575, 5-8=-323/304, 3-8=-323/304

### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=343, 6=343.



Structural wood sheathing directly applied or 5-0-13 oc purlins.

Rigid ceiling directly applied or 8-11-0 oc bracing.

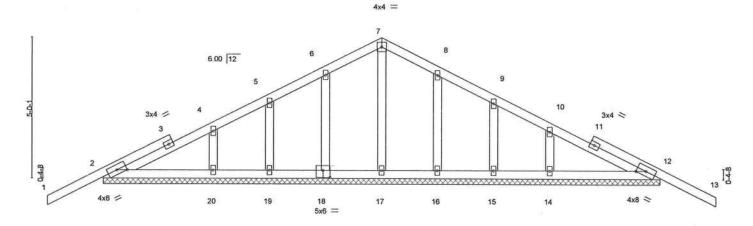
MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

April 29,2020



Job	Truss	Truss Type	Qty	Ply	EDGLEY - NELSON RES.	
2332916	T01G	Common Supported Gable	1	1	Joh Defenses (astisses)	T20094750
Builders FirstSource,	Jacksonville, FL - 32244,		ID:H14GiEDIC		Job Reference (optional)   9 2020 MiTek Industries, Inc.   Wed Apr 29   V1GySYUT-4   YJkTLtTecUKeLGSixL6yVsz	
-2-0-0		9-11-0	10.1114011 010	anor azinjor	19-10-0	21-10-0 ,
2-0-0		9-11-0			9-11-0	2-0-0

Scale = 1:39.5



		-				19-10-0 19-10-0						
Plate Offse	ts (X,Y)-	[2:0-4-0,0-1-15], [12:0-4-0	0,0-1-15], [18:0	-3-0,0-3-0]		19-10-0						
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.31	Vert(LL)	-0.02	13	n/r	120	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.10	Vert(CT)	-0.02	13	n/r	120	- India Part	
BCLL	0.0	Rep Stress Incr	YES	WB	0.05	Horz(CT)	0.00	12	n/a	n/a		
BCDL	10.0	Code FBC2017/TI	PI2014	Matri	x-S	1400040040000000				C.SFX.7853.	Weight: 104 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

OTHERS 2x4 SP No.3

**BRACING-**

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. All bearings 19-10-0.

(lb) - Max Horz 2=-118(LC 13)

Max Uplift All uplift 100 lb or less at joint(s) 18, 19, 16, 15 except 2=-111(LC 12), 12=-132(LC 13),

20=-117(LC 12), 14=-122(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 17, 18, 19, 20, 16, 15, 14 except 2=263(LC 23), 12=264(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

### NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 18, 19, 16, 15 except (jt=lb) 2=111, 12=132, 20=117, 14=122.



Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

April 29,2020

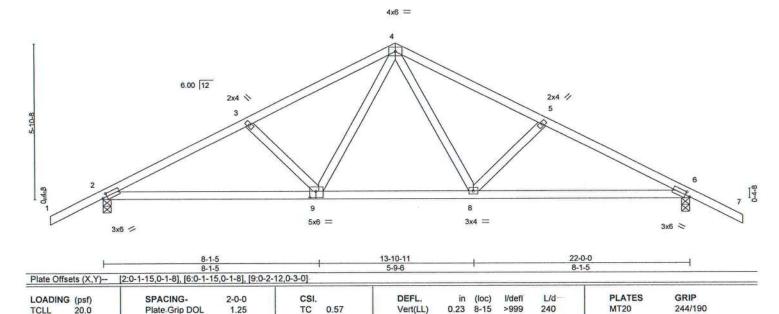
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITP1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



EDGLEY - NELSON RES. Qty Ply Job Truss Truss Type T20094751 T02 Common 2332916 Job Reference (optional) 8.240 s Mar 9 2020 MiTek Industries, Inc. Wed Apr 29 12:50:52 2020 Page 1 Builders FirstSource Jacksonville, FL - 32244, ID:H14GiFDt0QwCPd2wj9FV1GySYUT-YA6hypLVEykLyowS0PSafA1zeDSHlxH80tP4oQzLry1 22-0-0 5-6-2 11-0-0 24-0-0 2-0-0 5-5-14 2-0-0

Scale = 1:41.7



LUMBER-

TCDL

BCLL

BCDL

TOP CHORD 2x4 SP No.2 2x4 SP No.2 **BOT CHORD** WEBS 2x4 SP No.3

7.0

0.0

10.0

BRACING-

Vert(CT)

Horz(CT)

-0.21 8-15

0.04

>999

TOP CHORD **BOT CHORD**  Structural wood sheathing directly applied or 4-9-10 oc purlins.

Weight: 105 lb

FT = 20%

Rigid ceiling directly applied or 4-2-12 oc bracing.

180

n/a

REACTIONS.

2=0-3-8, 6=0-3-8 (size) Max Horz 2=-137(LC 13)

Max Uplift 2=-399(LC 9), 6=-399(LC 8) Max Grav 2=922(LC 1), 6=922(LC 1)

Lumber DOL

Rep Stress Incr

Code FBC2017/TPI2014

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-1383/1703, 3-4=-1178/1621, 4-5=-1170/1609, 5-6=-1382/1700 TOP CHORD

**BOT CHORD** 

2-9=-1390/1193, 8-9=-835/797, 6-8=-1409/1193

WEBS

4-8=-693/408, 5-8=-313/363, 4-9=-696/412, 3-9=-311/360

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat: II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

BC

WB 0.53

Matrix-MS

0.57

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

1.25

YES

- 4) \*This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=399, 6=399.



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April 29,2020

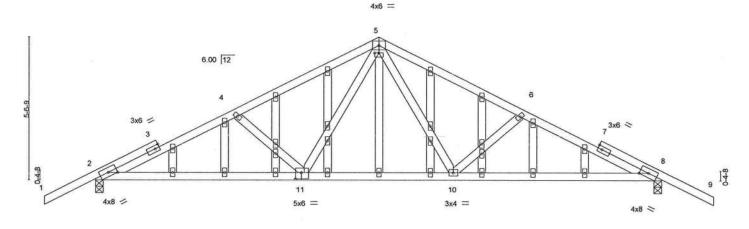
MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSITP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job		Truss	Truss Type	Qty	Ply	EDGLEY - NELSON RES.	)38433233333333
2332916		T02G	GABLE	1	1		T20094752
						Job Reference (optional)	
Builders Fi	irstSource,	Jacksonville, FL - 32244,			8.240 s Ma	r 9 2020 MiTek Industries, Inc. Wed Apr 2	9 12:50:54 2020 Page 1
				ID:H14GiFDt0Qw	CPd2wj9F\	/1GySYUT-UZDSMVNImZ_3B54r8qU2kb7	DJ18PDrxRTBuAtlzLry?
	-2-0-0	5-6-2	11-0-0	i i	16-5-14	22-0-0	24-0-0
	2-0-0	5-6-2	5-5-14		5-5-14	5-6-2	2-0-0

Scale = 1:43.0



		r	8-1-5		1	13-10-11				2	2-0-0	- 2
			8-1-5			5-9-6				8	3-1-5	
Plate Offse	ets (X,Y)-	[2:0-4-0,0-1-15], [5:0-2-0,	0-0-0], [8:0-4-0	0,0-1-15], [11	:0-2-12,0-3-0	0]						
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.95	Vert(LL)	0.22	10-39	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.53	Vert(CT)	-0.19	10-39	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.52	Horz(CT)	0.04	8	n/a	n/a		
BCDL	10.0	Code FBC2017/T	PI2014	Matri	x-MS	100000000000000000000000000000000000000					Weight: 146 lb	FT = 20%

BRACING-

TOP CHORD

**BOT CHORD** 

LUMBER-

TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x4 SP No.2

2x4 SP No.3 WEBS

2x4 SP No.3 **OTHERS** 

REACTIONS. (size) 2=0-3-8, 8=0-3-8

Max Horz 2=130(LC 12)

Max Uplift 2=-397(LC 9), 8=-397(LC 8) Max Grav 2=919(LC 1), 8=919(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. TOP CHORD 2-4=-1438/1827, 4-5=-1237/1688, 5-6=-1228/1674, 6-8=-1437/1824

**BOT CHORD** 

2-11=-1548/1310, 10-11=-869/818, 8-10=-1573/1310 WEBS 4-11=-371/458, 5-11=-747/444, 5-10=-742/439, 6-10=-375/463

### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=397, 8=397,



Structural wood sheathing directly applied or 2-2-0 oc purlins.

Rigid ceiling directly applied or 4-2-3 oc bracing.

Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

April 29,2020

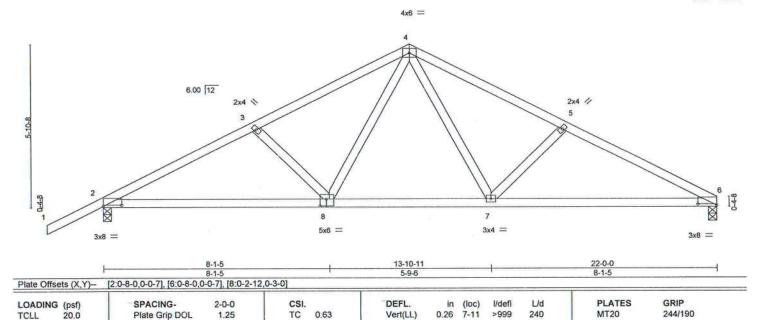
🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Eracing indicated is to prevent buckling of individual truss web and/of chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



EDGLEY - NELSON RES. Truss Type Qty Job Truss T20094753 T03 Common 2332916 Job Reference (optional) 8.240 s Mar 9 2020 MiTek Industries, Inc. Wed Apr 29 12:50:55 2020 Page 1 Jacksonville, FL - 32244. Builders FirstSource ID:H14GiFDt0QwCPd2wj9FV1GySYUT-yInqaqONXt7vpFe1hX0HGofS4RToyInbirekPkzLry\_ 22-0-0 5-6-2 2-0-0 5-6-2 11-0-0 5-5-14 16-5-14 5-5-14

Scale = 1:39.8

FT = 20%



LUMBER-

TCDL

BCLL

BCDL

2x4 SP No.2 TOP CHORD **BOT CHORD** 2x4 SP No.2 WEBS 2x4 SP No.3

7.0 0.0 \*

10.0

BRACING-

Vert(CT)

Horz(CT)

-0.22

0.03

7-11

6

>999

n/a

TOP CHORD **BOT CHORD**  Structural wood sheathing directly applied or 4-7-15 oc purlins.

Weight: 102 lb

Rigid ceiling directly applied or 3-11-10 oc bracing.

180

n/a

REACTIONS.

(size) 6=0-3-8, 2=0-3-8 Max Horz 2=157(LC 12)

Max Uplift 6=-368(LC 8), 2=-401(LC 9) Max Grav 6=809(LC 1), 2=927(LC 1)

Lumber DOL

Rep Stress Incr

Code FBC2017/TPI2014

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-1393/1721, 3-4=-1188/1638, 4-5=-1196/1641, 5-6=-1400/1736

**BOT CHORD** 

2-8=-1474/1203, 7-8=-900/808, 6-7=-1493/1226

WEBS

4-7=-715/432, 5-7=-330/379, 4-8=-690/411, 3-8=-311/361

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

BC

WB 0.54

Matrix-MS

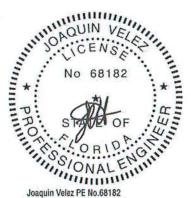
0.58

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

1.25

YES

- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 6=368, 2=401.



Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

April 29,2020

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Job Truss Truss Type Qty Ply EDGLEY - NELSON RES. T20094754 T04 2332916 Common Job Reference (optional) 8.240 s Mar 9 2020 MiTek Industries, Inc. Wed Apr 29 12:50:56 2020 Page 1 ID:H14GiFDt0QwCPd2wj9FV1GySYUT-RyLCnA0?lAFmQPDDFFXWp0Cgjqlfhk4kxVNHwBzLrxz Jacksonville, FL - 32244, Builders FirstSource, 12-0-0 5-11-0 30-0-0 17-10-8 5-10-8 2-0-0 Scale = 1:57.6 4x4 = 6.00 12 3x4 = 3x4 > 3x6 > 2x4 // 17 10 18 19 13 12 11 3x6 = 3x4 = 3x6 || 3x4 = 3x8 = 3x6 > 6-1-0 12-0-0 20-9-6 5-11-0 8-9-6 Plate Offsets (X,Y)-[7:0-1-15,0-1-8] LOADING (psf) SPACING-2-0-0 CSI. DEFL. PLATES L/d GRIP in (loc) I/defl TCLL 20.0 Plate Grip DOL 1.25 TC 0.44 Vert(LL) -0.19 >999 240 9-11 MT20 244/190 TCDL 7.0 1.25 0.86 Lumber DOL BC Vert(CT) -0.34 9-11 >999 180 BCLL 0.0 \* WB 0.60 Horz(CT) Rep Stress Incr YES 0.05 n/a n/a BCDL Code FBC2017/TPI2014 10.0 Matrix-MS Weight: 174 lb FT = 20% LUMBER-BRACING-TOP CHORD 2x4 SP No.2 TOP CHORD Structural wood sheathing directly applied or 3-9-12 oc purlins, **BOT CHORD** 2x4 SP No.2 except end verticals. WEBS 2x4 SP No.3 **BOT CHORD** Rigid ceiling directly applied or 6-7-1 oc bracing. WEBS 1 Row at midpt 2-11, 4-11 REACTIONS. (size) 13=0-3-8, 7=0-3-8 Max Horz 13=-316(LC 13)

Max Uplift 13=-375(LC 12), 7=-498(LC 13)

Max Grav 13=1101(LC 1), 7=1216(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 1-2=-1064/598, 2-3=-1070/704, 3-4=-1068/703, 4-6=-1753/976, 6-7=-1983/1048,

TOP CHORD

1-13=-1044/606 12-13=-75/307, 11-12=-279/907, 9-11=-488/1308, 7-9=-794/1719

**BOT CHORD** 2-12=-344/269, 3-11=-383/625, 4-11=-618/473, 4-9=-197/512, 6-9=-310/320, WEBS

1-12=-493/981

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 13=375, 7=498.



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EDGLEY - NELSON RES. Job Truss Type Qty Truss T20094755 T04G GABLE 2332916 Job Reference (optional) Builders FirstSource, Jacksonville, FL - 32244, 8.240 s Mar 9 2020 MiTek Industries, Inc. Wed Apr 29 12:50:57 2020 Page 1 ID:H14GiFDt0Q:wCPd2wj9FV1GySYUT-v8va?WPd3UNd2ZoQpy2lMDltSEGoQlhu997rTdzLrxy 30-0-0 2-0-0 Scale = 1:56.7 4x4 = 6.00 12 10 11 3x6 ≥ 12 13 3x4 = 14 15 3x4 > 16 17 18 14 4x8 = 19 32 31 30 29 28 27 26 25 24 23 22 21 20 5x6 = 30-0-0 Plate Offsets (X,Y)-- [17:0-4-0,0-2-1], [25:0-3-0,0-3-0] LOADING (psf) **PLATES** GRIP SPACING-2-0-0 CSI. DEFL in I/defl L/d (loc) -0.02 244/190 Plate Grip DOL 0.31 Vert(LL) 120 MT20 20.0 1.25 TC 18 TCLL n/r BC -0.02 120 1.25 0.10 TCDL 7.0 Lumber DOL Vert(CT) 18 n/r 0.13 WB 0.01 17 0.0 Rep Stress Incr Horz(CT) n/a BCLL YES n/a Code FBC2017/TPI2014 Weight: 210 lb FT = 20% Matrix-S BCDL 10.0 LUMBER-BRACING-

TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x4 SP No.2 2x4 SP No.3 WEBS **OTHERS** 2x4 SP No.3 TOP CHORD **BOT CHORD** 

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

1 Row at midpt

REACTIONS. All bearings 30-0-0.

Max Horz 32=-300(LC 13) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 27, 28, 29, 30, 25, 24, 23, 22, 21, 20, 17 except 32=-152(LC

13), 31=-239(LC 12), 19=-116(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 32, 26, 27, 28, 29, 30, 31, 25, 24, 23, 22, 21, 20, 19 except

17=268(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 5-6=-92/266, 6-7=-112/323, 7-8=-112/323, 8-9=-92/266

31-32=-82/298 **BOT CHORD** 

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 27, 28, 29, 30, 25, 24, 23, 22, 21, 20, 17 except (jt=lb) 32=152, 31=239, 19=116.



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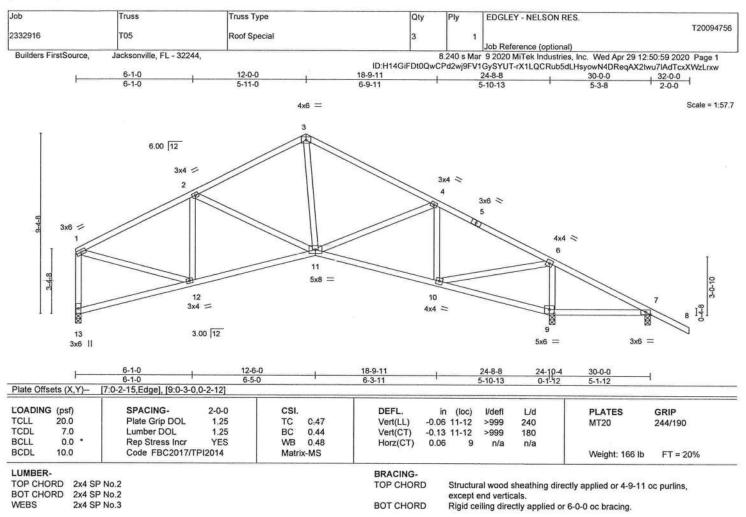
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ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





REACTIONS.

(size) 13=0-3-8, 9=0-3-8, 7=0-3-8

Max Horz 13=-316(LC 13)

Max Uplift 13=-314(LC 12), 9=-444(LC 13), 7=-139(LC 8) Max Grav 13=862(LC 1), 9=1350(LC 1), 7=155(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD

1-2=-1041/515, 2-3=-1009/538, 3-4=-1071/547, 4-6=-997/543, 6-7=-106/457, 1-13=-817/442

BOT CHORD 12-13=-91/322, 11-12=-300/917, 10-11=-226/871, 9-10=-436/253, 7-9=-347/206 WEBS

2-12=-316/239, 3-11=-216/545, 4-11=-71/270, 4-10=-417/241, 6-10=-432/1264,

6-9=-1134/621, 1-12=-376/882

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Bearing at joint(s) 13 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 13=314, 9=444, 7=139.



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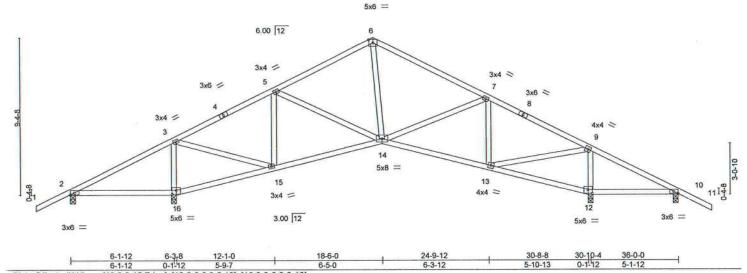
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MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only with MTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



EDGLEY - NELSON RES. Truss Type Qty Job Truss T20094757 T06 Roof Special 2332916 Job Reference (optional) 8.240 s Mar 9 2020 MiTek Industries, Inc. Wed Apr 29 12:51:00 2020 Page 1 ID:H14GiFDI0QwCPd2wj9FV1GySYUT-JjbjdYSWLPICv0X?U5bSzsNLJSDodbaKs7LV3yzLrxv Builders FirstSource, Jacksonville, FL - 32244, 36-0-0 2-0-0 12-1-0 5-9-7 24-9-12 30-8-8 18-0-0 6-9-1 5-10-13 2-0-0

Scale = 1:65.8



LOADING	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.47	Vert(LL)	0.10	16-19	>726	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.40	Vert(CT)	0.09	16-19	>876	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.44	Horz(CT)	0.04	12	n/a	n/a		
BCDL	10.0	Code FBC2017/T	PI2014	Matri	x-MS						Weight: 188 lb	FT = 20%

LUMBER-

WEBS

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No.3 BRACING-

TOP CHORD **BOT CHORD**  Structural wood sheathing directly applied or 5-2-3 oc purlins.

Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. All bearings 0-3-8

(lb) -Max Horz 2=210(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) except 2=-168(LC 8), 16=-529(LC 12), 12=-456(LC 13),

10=-135(LC 8)

All reactions 250 lb or less at joint(s) 2, 10 except 16=1322(LC 1), 12=1273(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown

TOP CHORD

2-3=-177/432, 3-5=-799/494, 5-6=-895/519, 6-7=-949/529, 7-9=-927/533, 9-10=-94/420 2-16=-302/207, 15-16=-378/230, 14-15=-215/683, 13-14=-216/806, 12-13=-399/253,

**BOT CHORD** 

10-12=-315/206

WEBS

3-16=-1107/617, 3-15=-338/1041, 5-15=-406/231, 6-14=-193/464, 7-14=-108/261,

7-13=-374/235, 9-13=-419/1163, 9-12=-1067/610

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 168 lb uplift at joint 2, 529 lb uplift at joint 16, 456 lb uplift at joint 12 and 135 lb uplift at joint 10.



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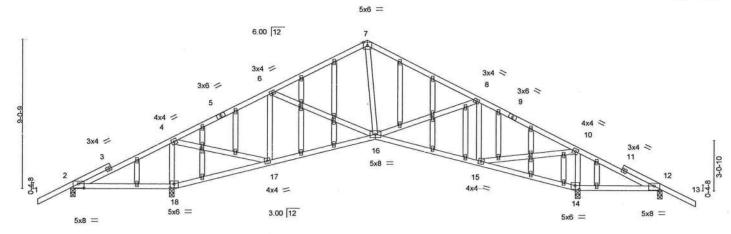
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Ansirtpi1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type		Qty	Ply	EDGLEY - NELSON RES.		2 mg art ar 200 mg 10 mg
2332916	T06G	GABLE		1	1			T20094758
						Job Reference (optional)		
Builders FirstSource,	Jacksonville, FL - 32244,					er 9 2020 MiTek Industries, Inc. 1		
				ID:H14GiFDt0Qw	CPd2wj9F1	V1GySYUT-F6iT2ETmt0?w8KhN	cVew3HShmFv35T	YdJRqc8qzLrxt
L-2-0-0 I	6-3-8	12-1-0	18-0-0	24-9	3-12	30-8-8	36-0-0	, 38-0-0
2-0-0	6-3-8	5-9-8	5-11-0	6-9	-12	5-10-12	5-3-8	2-0-0

Scale = 1:67.3



	1	6-3-8	12-1-0	18-6-0		24-9-12	(313)	30-8-8	36-0-0	100
		6-3-8	5-9-8	6-5-0		6-3-12	1	5-10-12	5-3-8	
Plate Offse	ets (X,Y)-	[2:0-4-0,0-3-1], [12:0-4-0, ,0-1-0]	0-3-1], [14:0-3-0,0	0-2-12], [18:0-3-0,0-2-12	J, [19:0-1-13,0-1-0]	, [22:0-1-13,	0-1-0], [2	7:0-1-9,0-1-0], [	36:0-1-11,0-1-0], [43	:0-1-9
LOADING	(psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP-
TCLL	20.0	Plate Grip DOL	1.25	TC 0.47	Vert(LL)	0.07 18-50	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC 0.42	Vert(CT) -	0.13 16-17	>999	180		
BCLL	0.0	Rep Stress Incr	YES	WB 0.48	Horz(CT)	0.05 14	n/a	n/a		
BCDL	10.0	Code FBC2017/TI	212014	Matrix-MS	5. 4				Weight: 245 lb	FT = 20%

AAI	D	E	D	

TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x4 SP No.2

WEBS 2x4 SP No.3

**OTHERS** 2x4 SP No.3

### BRACING-

TOP CHORD **BOT CHORD**  Structural wood sheathing directly applied or 5-0-15 oc purlins.

Rigid ceiling directly applied or 6-0-0 oc bracing.

### All bearings 0-3-8. REACTIONS.

Max Horz 2=203(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) except 2=147(LC 8), 12=-122(LC 13), 18=-562(LC 12),

14=-462(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 2, 12 except 18=1361(LC 1), 14=1247(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-4=-263/536, 4-6=-819/426, 6-7=-927/489, 7-8=-987/496, 8-10=-999/518,

10-12=-138/440

**BOT CHORD** 2-18=-415/313, 17-18=-502/365, 16-17=-210/703, 15-16=-223/874, 14-15=-433/305,

12-14=-341/253

4-18=-1126/662, 4-17=-448/1171, 6-17=-416/263, 7-16=-166/494, 8-16=-131/269,

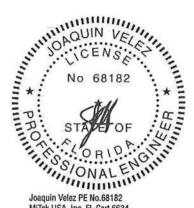
8-15=-350/232, 10-15=-471/1252, 10-14=-1041/612

### NOTES-

WEBS

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 147 lb uplift at joint 2, 122 lb uplift at joint 12, 562 lb uplift at joint 18 and 462 lb uplift at joint 14.



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April 29,2020

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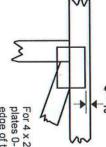


### Symbols

# PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y and fully embed teeth. Apply plates to both sides of truss Dimensions are in ft-in-sixteenths. offsets are indicated.



For  $4 \times 2$  orientation, locate plates 0-  $^{1}$ <sub>16</sub>" from outside edge of truss.

00

connector plates. required direction of slots in This symbol indicates the

Plate location details available in MiTek 20/20 software or upon request.

### PLATE SIZE

4 × 4

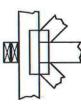
the length parallel to slots. to slots. Second dimension is width measured perpendicular The first dimension is the plate

## LATERAL BRACING LOCATION



output. Use T or I bracing if indicated. by text in the bracing section of the Indicated by symbol shown and/or

### BEARING



Min size shown is for crushing only. number where bearings occur. reaction section indicates joint (supports) occur. Icons vary but Indicates location where bearings

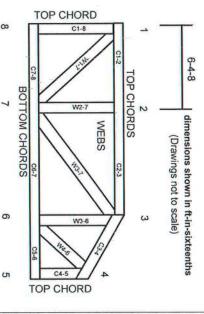
### Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.

DSB-89

Installing & Bracing of Metal Plate Guide to Good Practice for Handling, Building Component Safety Information Connected Wood Trusses. Design Standard for Bracing.

## Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

## PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988 ER-3907, ESR-2362, ESR-1397, ESR-3282

truss unless otherwise shown. Trusses are designed for wind loads in the plane of the

section 6.3 These truss designs rely on lumber values established by others. Lumber design values are in accordance with ANSI/TPI 1

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MiTek Engineering Reference Sheet: MII-7473 rev. 10/03/2015

# General Safety Notes

## Damage or Personal Injury Failure to Follow Could Cause Property

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- Provide copies of this truss design to the building designer, erection supervisor, property owner and
- Cut members to bear tightly against each other.

G

- Place plates on each face of truss at each locations are regulated by ANSI/TPI 1. joint and embed fully. Knots and wane at joint
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the camber for dead load deflection. responsibility of truss fabricator. General practice is to
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing. or less, if no ceiling is installed, unless otherwise noted
- Connections not shown are the responsibility of others
- Do not cut or alter truss member or plate without prior approval of an engineer
- Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.

### T-BRACE / I-BRACE DETAIL WITH 2X BRACE ONLY

MII-T-BRACE 2

MiTek USA, Inc. Page 1 of 1



Note: T-Bracing / I-Bracing to be used when continuous lateral bracing is impractical. T-Brace / I-Brace must cover 90% of web length.

Note: This detail NOT to be used to convert T-Brace / I-Brace webs to continuous lateral braced webs.

A MiTek Affiliate		
1	Nailing Pattern	
T-Brace size	Nail Size	Nail Spacing
2x4 or 2x6 or 2x8	10d (0.131" X 3")	6" o.c.

Note: Nail along entire length of T-Brace / I-Brace (On Two-Ply's Nail to Both Plies)

WEB	Nails SPACING  T-BRACE	
		M
Nails	Section Detail	

Nails	
Web	I-Brace
Nails	

Web

T-Brace

	Brace Size for One-Ply Truss			
	Specified Continuous Rows of Lateral Bracing			
Web Size	1	2		
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace		
2x6	2x6 T-Brace	2x6 I-Brace		
2x8	2x8 T-Brace	2x8 I-Brace		

	Brace Size for Two-Ply Truss				
	Specified Rows of La	Continuous iteral Bracing			
Web Size	1	2			
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace			
2x6	2x6 T-Brace	2x6 I-Brace			
2x8	2x8 T-Brace	2x8 I-Brace			

T-Brace / I-Brace must be same species and grade (or better) as web member.



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### WEB BRACING RECOMMENDATIONS

### MII-WEBBRACE

MiTek USA, Inc.

Page 1 of 1



				MAX	KIMUM TR	USS WEB	FORCE (	lbs.)(See	note 7)	
BRACE .		24"	O.C.			48"	O.C.		72"	O.C.
DATOLL	BF	ACING MA	ATERIAL 1	TYPE	BR	ACING MA	ATERIAL T	TYPE	BRACING N	IATERIAL TYPE
	Α	В	C	D	Α	B	C	l D	С	D
10'-0"	1610	1886	1886	2829						
12'-0"	1342	1572	1572	2358		3143	3143	4715	4715	7074
14'-0"	1150	1347	1347	2021						
16'-0"	1006	1179	1179	1768		2358	2358	3536		
18'-0"	894	1048	1048	1572					3143	4715
20'-0"	805	943	943	1414		1886	1886	2829		

<sup>\*</sup>Bay size shall be measured in between the centers of pairs of diagonals.

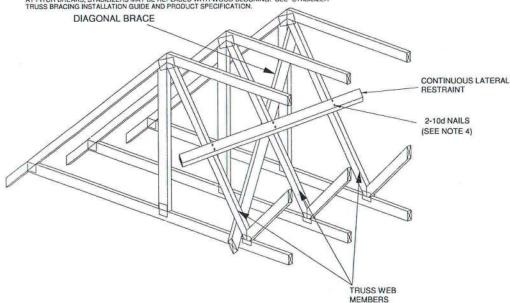
TYPE	BRACING MATERIALS	
	1 X 4 IND. 45 SP	
Α	-OR-	
	1 X 4 #2 SRB (DF, HF, SPF)	
В	2 X 3 #3, STD, CONST (SPF, DF, HF, OR SP)	
С	2 X 4 #3, STD, CONST (SPF, DF, HF, OR SP)	
D	2 X 6 #3 OR BETTER (SPF, DF, HF, OR SP)	

### **GENERAL NOTES**

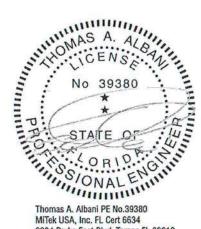
- DIAGONAL BRACING IS REQUIRED TO TRANSFER THE CUMULATIVE LATERAL BRACE FORCE INTO THE ROOF AND/OR CELING DIAPHRAGM. THE DIAPHRAGM IS TO BE DESIGNED BY A QUALIFIED PROFESSIONAL.
- 2. THESE CALCULATIONS ARE BASED ON LATERAL BRACE CARRYING 2% OF THE WEB FORCE.
- DIAGONAL BRACING MATERIAL MUST BE SAME SIZE AND GRADE OR BETTER, AS THE LATERAL BRACE MATERIAL, AND SHALL BE INSTALLED IN SUCH A MANNER THAT IT INTERSECTS WEB MEMBERS AT APPROX. 45 DEGREES AND SHALL BE INALED AT EACH END AND EACH INTERMEDIATE TRUSS WITH 2-8d (0.131"x2.5") FOR 1x4 BRACES, 2-10d (0.131"x3") FOR 2x3 and 2x4 BRACES, AND 3-10d (0.131"x3") FOR 2x6 BRACES.
- CONNECT LATERAL BRACE TO EACH TRUSS WITH 2-8d (0.131"X2.5") NAILS FOR 1x4-LATERAL BRACES, 2-10d (0.131"X3") NAILS FOR 2x3 and 2x4 LATERAL BRACES, AND 3-10d (0.131"X3") FOR 2x6 LATERAL BRACES.
- LATERAL BRACE SHOULD BE CONTINUOUS AND SHOULD OVERLAP AT LEAST ONE TRUSS SPACE FOR CONTINUITY
- FOR ADDITIONAL GUIDANCE REGARDING DESIGN AND INSTALLATION OF BRACING, CONSULT DSB-89 TEMPORARY BRACING OF METAL PLATE CONNECTED WOOD TRUSSES AND BOSI 1
  GUIDE TO GOOD PRACTICE FOR HANDLING, INSTALLING & BRACING OF METAL PLATE CONNECTED
  WOOD TRUSSES, JOINTLY PRODUCED BY WOOD TRUSS COUNCIL OF AMERICA and TRUSS PLATE INSTITUTE,
  www.sbcindustry.com and www.tpinst.org
- REFER TO SPECIFIC TRUSS DESIGN DRAWING FOR WEB MEMBER FORCE. TABULATED VALUES ARE BASED ON A DOL. = 1.15

### FOR STABILIZERS:

FOR A SPACING OF 24" O.C. ONLY, MITEK "STABILIZER" TRUSS BRACING SYSTEMS CAN BE SUBSTITUTED FOR TYPE A, B, C AND D BRACING MATERIAL. DIAGONAL BRACING FOR STABILIZERS ARE TO BE PROVIDED AT BAY SIZE INDICATED ABOVE. WHERE DIAPHRAGM BRACING IS REQUIRED AT PITCH BREAKS, STABILIZERS MAY BE REPLACED WITH WOOD BLOCKING. SEE "STABILIZER" TRUSS BRACING INSTALLATION GUIDE AND PRODUCT SPECIFICATION.



This information is provided as a recommendation to assist in the requirement for permanent bracing of the individual truss web members. Additional bracing may still be required for the stability of the overall roof system. The method shown here is just one method that can be used to provide stability against web buckling.



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### SCAB-BRACE DETAIL

### MII-SCAB-BRACE

(R)

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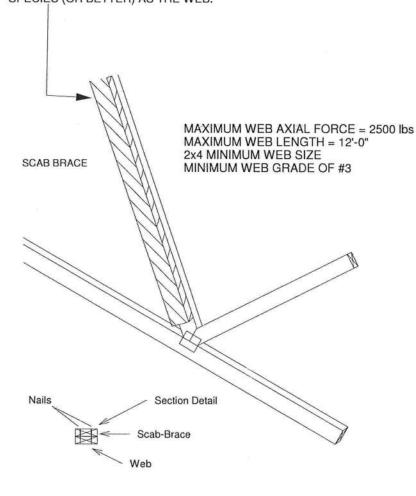
MiTek USA, Inc.

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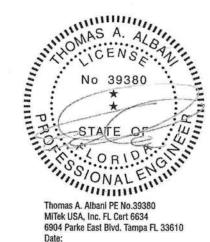
Note: Scab-Bracing to be used when continuous lateral bracing at midpoint (or T-Brace) is impractical. Scab must cover full length of web +/- 6".

\*\*\* THIS DETAIL IS NOT APLICABLE WHEN BRACING IS \*\*\* REQUIRED AT 1/3 POINTS OR I-BRACE IS SPECIFIED.

APPLY 2x\_ SCAB TO ONE FACE OF WEB WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 6" O.C. SCAB MUST BE THE SAME GRADE, SIZE AND SPECIES (OR BETTER) AS THE WEB.



Scab-Brace must be same species grade (or better) as web member.



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### STANDARD REPAIR TO REMOVE END VERTICAL (RIBBON NOTCH VERTICAL)

MII-REP05

MiTek USA, Inc. Page 1 of 1



1. THIS IS A SPECIFIC REPAIR DETAIL TO BE USED ONLY FOR ITS ORIGINAL INTENTION, THIS REPAIR DOES NOT IMPLY THAT THE REMAINING PORTION OF THE TRUSS IS UNDAMAGED. THE ENTIRE TRUSS SHALL BE INSPECTED TO VERIFY THAT NO FURTHER REPAIRS ARE REQUIRED. WHEN THE REQUIRED REPAIRS ARE PROPERLY APPLIED, THE TRUSS WILL BE CAPABLE OF SUPPORTING THE LOADS INDICATED. THE LOADS INDICATED.

THE LOADS INDICATED.

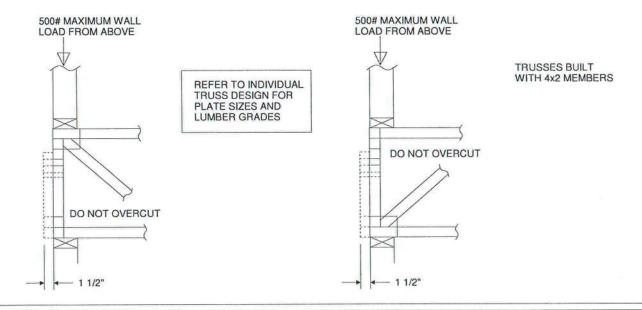
2. ALL MEMBERS MUST BE RETURNED TO THEIR ORIGINAL POSITIONS BEFORE APPLYING REPAIR AND HELD IN PLACE DURING APPLICATION OF REPAIR.

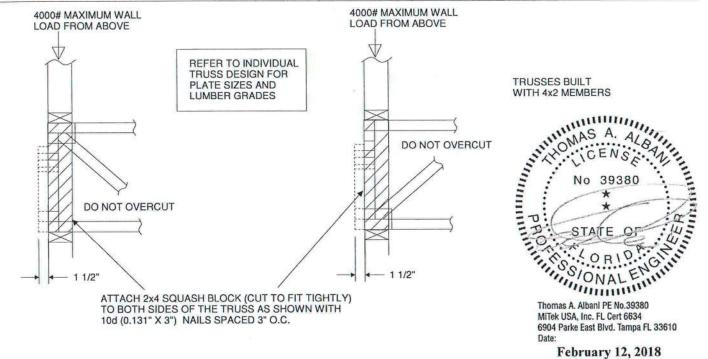
3. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID SPLITTING OF THE WOOD.

4. LUMBER MUST BE CUT CLEANLY AND ACCURATELY AND THE REMAINING WOOD MUST BE UNDAMAGED.

5. THIS REPAIR IS TO BE USED FOR SINGLE PLY TRUSSES IN THE 4X. ORIENTATION ONLY.

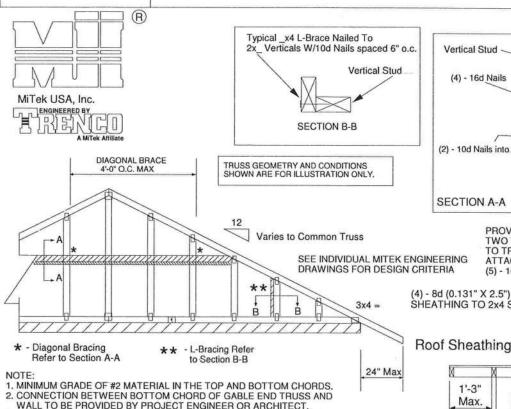
6. CONNECTOR PLATES MUST BE FULLY IMBEDDED AND UNDISTURBED.





### Standard Gable End Detail

### MII-GE130-D-SP



MiTek USA, Inc. Page 1 of 2 Vertical Stud DIAGONAL BRACE (4) - 16d Nails 16d Nails Spaced 6" o.c. (2) - 10d Nails into 2x6 2x6 Stud or 2x4 No.2 of better Typical Horizontal Brace Nailed To 2x\_ Verticals w/(4)-10d Nails SECTION A-A 2x4 Stud

> PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SPF BLOCK

WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.

3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.

4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH, GRADES: 1x4 SRB OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C. 5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF

DIAPHRAM AT 4'-0" O.C.

6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)

GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.

THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES

10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.

NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

d Max. ▶	NAILS
	***
Diag. Brace / at 1/3 points if needed	2x6 DIAG ATTACHE NAILS AN TO BLOC
End Wall	

1'-3"

SONAL BRACE SPACED 48" O.C. ED TO VERTICAL WITH (4) -16d ND ATTACHED CKING WITH (5) - 10d NAILS.

(2) - 10d NAILS

Trusses @ 24" o.c.

Minimum Stud Size Species	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
and Grade			Maximu	m Stud Le	ngth	
2x4 SP No. 3 / Stud	12" O.C.	3-9-13	4-1-1	5-9-6	7-1-3	11-5-7
2x4 SP No. 3 / Stud	16" O.C.	3-5-4	3-6-8	5-0-2	6-10-8	10-3-13
2x4 SP No. 3 / Stud	24" O.C.	2-9-11	2-10-11	4-1-1	5-7-6	8-5-1

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING EXPOSURE D ASCE 7-98, ASCE 7-02, ASCE 7-05 130 MPH ASCE 7-10 160 MPH DURATION OF LOAD INCREASE: 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.

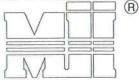


### Standard Gable End Detail

### MII-GE130-SP

Page 1 of 2

MiTek USA, Inc.



MiTek USA, Inc. ENGINEERED BY

Typical \_x4 L-Brace Nailed To Verticals W/10d Nails spaced 6" o.c. Vertical Stud SECTION B-B

3x4 =

TRUSS GEOMETRY AND CONDITIONS SHOWN ARE FOR ILLUSTRATION ONLY.

Vertical Stud DIAGONAL (4) - 16d Nails 16d Nails Spaced 6" o.c. (2) - 10d Nails into 2x6 -2x6 Stud or 2x4 No.2 of better Typical Horizontal Brace Nailed To 2x\_ Verticals w/(4)-10d Nails SECTION A-A 2x4 Stud

> PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH

(5) - 10d NAILS.

SHEATHING TO 2x4 STD SPF BLOCK

Roof Sheathing

(4) - 8d (0.131" X2.5") NAILS MINIMUM, PLYWOOD

Varies to Common Truss SEE INDIVIDUAL MITEK ENGINEERING DRAWINGS FOR DESIGN CRITERIA \*\*

DIAGONAL BRACE 4'-0" O.C. MAX

- Diagonal Bracing Refer to Section A-A

- L-Bracing Refer \*\* to Section B-B

### NOTE

1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS. 2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND

WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.

3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY, CONSULT BLDG, ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.

4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 1x4 SRB OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.

5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF

DIAPHRAM AT 4'-0" O.C. 6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)

7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240. 8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.

9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.

10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.

11. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

CONTRACTOR OF THE PARTY OF THE	The state of the s			
24" Max	N D	1	W V /	4
	1'-3" Max.	(2) - 10d NAILS		(2) - 10d NAILS
		/*/	_X	
			7 rus	ses @ 24" o.d
Diag. B		\/ ATTA	CHED TO VER	CE SPACED 48" O.C. TICAL WITH (4) -16d
at 1/3 p if neede			S AND ATTACH LOCKING WITH	ED I (5) - 10d NAILS.
End	Wall			ITAL BRACE CTION A-A)

Minimum Stud Size	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
Species and Grade			Maximu	m Stud Le	ngth	
2x4 SP No. 3 / Stud	12" O.C.	4-0-7	4-5-6	6-3-8	8-0-15	12-1-6
2x4 SP No. 3 / Stud	16" O.C.	3-8-0	3-10-4	5-5-6	7-4-1	11-0-1
2x4 SP No. 3 / Stud	24" O.C.	3-0-10	3-1-12	4-5-6	6-1-5	9-1-15

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

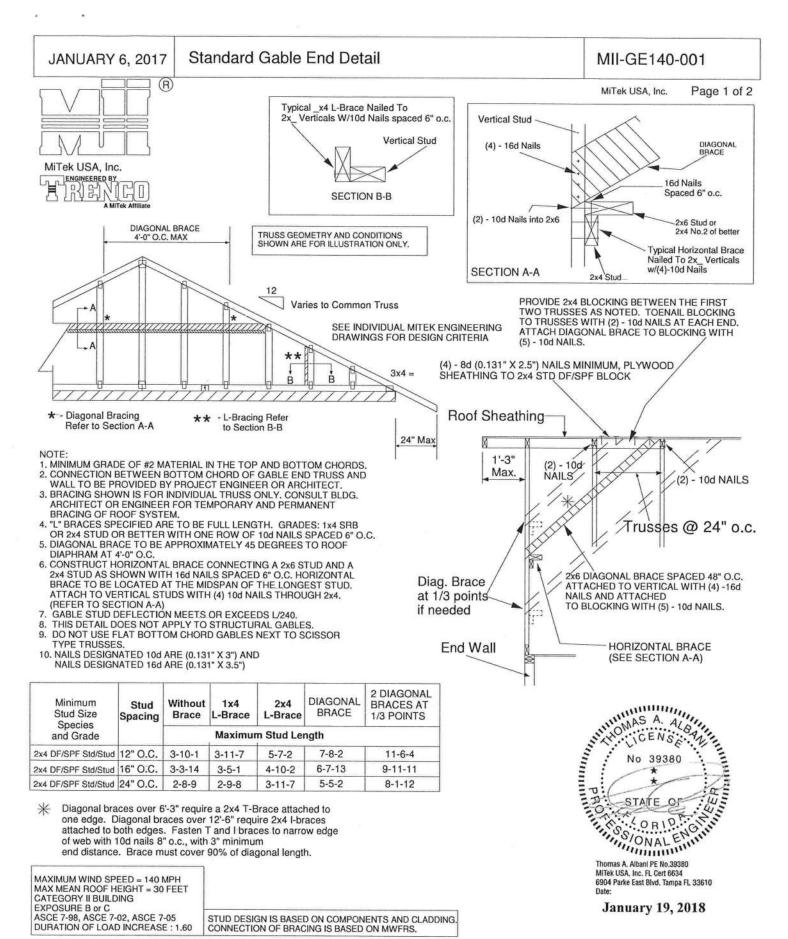
MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING EXPOSURE B or C ASCE 7-98, ASCE 7-02, ASCE 7-05 130 MPH ASCE 7-10 160 MPH

DURATION OF LOAD INCREASE: 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.



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### Standard Gable End Detail

### MII-GE170-D-SP

TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH

(2) - 10d NAILS

Trusses @ 24" o.c.

(5) - 10d NAILS.

SHEATHING TO 2x4 STD SPF BLOCK

Roof Sheathing

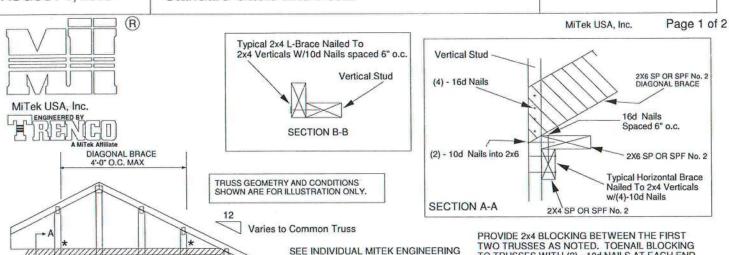
1'-0'

Max.

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD,

10d

NAILS



DRAWINGS FOR DESIGN CRITERIA

24" Max

Diag. Brace

at 1/3 points

End Wall

if needed

3x4 =

Diagonal Bracing Refer to Section A-A

L-Bracing Refer to Section B-B

\*\*

B

NOTE:

1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG.

ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.

4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH, SPF or SP No.3 OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C. 5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF

DIAPHRAM AT 4'-0" O.C.

6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 AND A 2x4 AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST GABLE STUD. ATTACH TO VERTICAL GABLE STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.

8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES

9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.

10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.

11. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Minimum Stud Size	Stud Spacing	Without Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS		
Species and Grade	14.5	Maximum Stud Length					
2x4 SP No. 3 / Stud	12" O.C.	3-9-7	5-8-8	6-11-1	11-4-4		
2x4 SP No. 3 / Stud	16" O.C.	3-4-12	4-11-15	6-9-8	10-2-3		
2x4 SP No. 3 / Stud	24" O.C.	2-9-4	4-0-7	5-6-8	8-3-13		
2x4 SP No. 2	12" O.C.	3-11-13	5-8-8	6-11-1	11-11-7		
2x4 SP No. 2	16" O.C.	3-7-7	4-11-5	6-11-1	10-10-5		
2x4 SP No. 2	24" O.C.	3-1-15	4-0-7	6-3-14	9-5-14		

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 6" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length. T or I braces must be 2x4 SPF No. 2 or SP No. 2.

MAX MEAN ROOF HEIGHT = 30 FEET **EXPOSURE D** ASCE 7-10 170 MPH

**DURATION OF LOAD INCREASE: 1.60** 

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.

No 39380

RESTAILE OF THE STAIL OF THE STAIL

2x6 DIAGONAL BRACE SPACED 48" O.C. ATTACHED TO VERTICAL WITH

BLOCKING WITH (5) -10d NAILS.

(4) -16d NAILS, AND ATTACHED TO

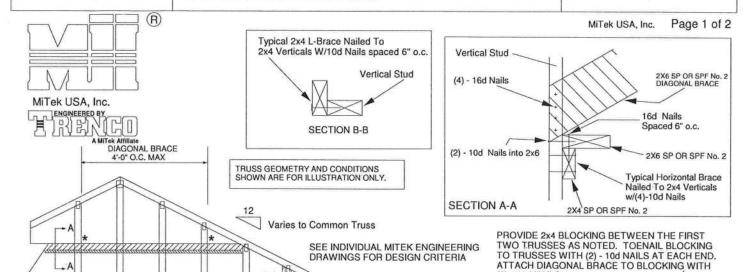
HORIZONTAL BRACE

(SEE SECTION A-A)

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### Standard Gable End Detail

### MII-GE180-D-SP



3x4 =

24" Max

- Diagonal Bracing Refer to Section A-A

- L-Bracing Refer to Section B-B

\*\*

MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
 CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.

3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY, CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT

BRACING OF ROOF SYSTEM.

4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH, SPF or SP No.3
OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.

5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF

DIAPHRAM AT 4'-0" O.C. 6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 AND A 2x4 AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST GABLE STUD. ATTACH TO VERTICAL GABLE STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.

THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.

DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.

10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.
NAILS DESIGNATED 10d ARE (0.131" X 3") AND

NAILS DESIGNATED 16d ARE (0.131" X 3.5")

		1'-0" Max.
		A
at	ag. Brac 1/3 poin needed	
ONAL	End W	/all

(5) - 10d NAILS.

SHEATHING TO 2x4 STD SPF BLOCK

Roof Sheathing

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD

- 10d (2) - 10 NAILS

> 2x6 DIAGONAL BRACE SPACED 48" O.C. ATTACHED TO VERTICAL WITH (4) -16d NAILS, AND ATTACHED TO BLOCKING WITH (5) -10d NAILS.

(2) - 10d NAILS

Trusses @ 24" o.c.

HORIZONTAL BRACE (SEE SECTION A-A)

Minimum Stud Size Species	Stud Spacing	Without Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS		
and Grade		Maximum Stud Length					
2x4 SP No. 3 / Stud	12" O.C.	3-7-12	5-4-11	6-2-1	10-11-3		
2x4 SP No. 3 / Stud	16" O.C.	3-2-8	4-8-1	6-2-1	9-7-7		
2x4 SP No. 3 / Stud	24" O.C.	2-7-7	3-9-12	5-2-13	7-10-4		
2x4 SP No. 2	12" O.C.	3-10-0	5-4-11	6-2-1	11-6-1		
2x4 SP No. 2	16" O.C.	3-5-13	4-8-1	6-2-1	10-5-7		
2x4 SP No. 2	24" O.C.	3-0-8	3-9-12	6-1-1	9-1-9		

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 6in o.c., with 3in minimum end distance. Brace must cover 90% of diagonal length. T or I braces must be 2x4 SPF No. 2 or SP No. 2.

MAX MEAN ROOF HEIGHT = 30 FEET EXPOSURE D ASCE 7-10 180 MPH DURATION OF LOAD INCREASE : 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.



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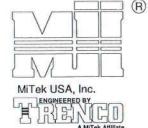
MiTek USA, Inc. Page 1 of 1

MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E MAX MEAN ROOF HEIGHT = 30 FEET MAX TRUSS SPACING = 24 " O.C. CATEGORY II BUILDING

EXPOSURE B or C ASCF 7-10

DURATION OF LOAD INCREASE: 1.60

DETAIL IS NOT APPLICABLE FOR TRUSSES TRANSFERING DRAG LOADS (SHEAR TRUSSES). ADDITIONAL CONSIDERATIONS BY BUILDING ENGINEER/DESIGNER ARE REQUIRED.



A - PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING. SHALL BE CONNECTED TO EACH PURLIN

DIRECTIONS AND:

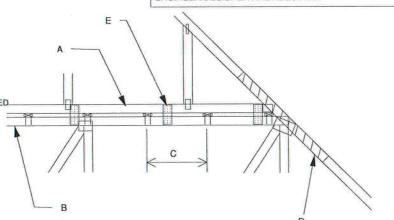
DIRECTIONS AND:

1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR

2. WIND SPEED OF 116 MPH TO 160 MPH WITH A MAXIMUM
PIGGYBACK SPAN OF 12 ft.

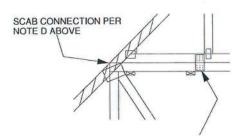
E - FOR WIND SPEEDS BETWEEN 126 AND 160 MPH, ATTACH

MITEK 3X8 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 72" O.C. W/ (4) (0.131" X 1.5") NAILS PER MEMBER. STAGGER NAILS FROM OPPOSING FACES. ENSURE 0.5" EDGE DISTANCE. (MIN. 2 PAIRS OF PLATES REQ. REGARDLESS OF SPAN)

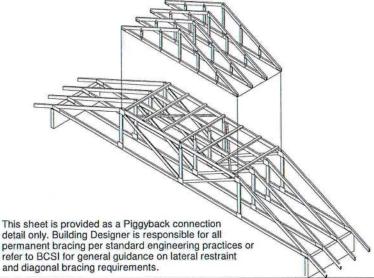


### WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

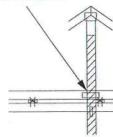
REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH Nail-On PLATES AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.



FOR ALL WIND SPEEDS, ATTACH MITEK 3X6 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 48" O.C. W/ (4) (0.131" X 1.5") PER MEMBER. STAGGER NAILS FROM OPPOSING FACES ENSURE 0.5" EDGE DISTANCE.



VERTICAL WEB TO EXTEND THROUGH **BOTTOM CHORD** OF PIGGYBACK



FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL.

AS SHOWN IN DELTAIL.
ATTACH 2 x \_\_\_ x 4\*-0" SCAB TO EACH FACE OF
TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS
SPACED 4" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH
VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.)

(MINIMUM 2X4)
THIS CONNECTION IS ONLY VALID FOR A MAXIMUM CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS GREATER THAN 4000 LBS.

FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS, NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS. CONCENTRATED LOAD MUST BE APPLIED TO BOTH

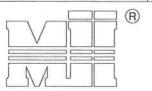
THE PIGGYBACK AND THE BASE TRUSS DESIGN.



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### STANDARD PIGGYBACK TRUSS CONNECTION DETAIL

MII-PIGGY-ALT 7 - 10



MiTek USA, Inc.

ENGINEERED BY 

MiTek USA, Inc. Page 1 of 1 MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E

MAX MEAN ROOF HEIGHT = 30 FEET MAX TRUSS SPACING = 24 " O.C. CATEGORY II BUILDING EXPOSURE B or C ASCE 7-10 **DURATION OF LOAD INCREASE: 1.60** 

DETAIL IS NOT APPLICABLE FOR TRUSSES TRANSFERING DRAG LOADS (SHEAR TRUSSES). ADDITIONAL CONSIDERATIONS BY BUILDING ENGINEER/DESIGNER ARE REQUIRED.

A - PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
SHALL BE CONNECTED TO EACH PURLIN
WITH (2) 0(0.131" X.9.5") TOE-NAILED.
B - BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
C - PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24" O.C.
UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING.
CONNECT TO BASE TRUSS WITH (2) (0.131" X 3.5") NAILS EACH.
D - 2 X \_\_ X 4"-0" SCAB, SIZE TO MATCH TOP CHORD OF
PIGGYBACK TRUSS, MIN GRADE #2, ATTACHED TO ONE FACE, CENTERED ON
INTERSECTION, WITH (2) ROWS OF (0.131" X 3") NAILS @ 4" O.C.
SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING
IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH
DIRECTIONS AND:

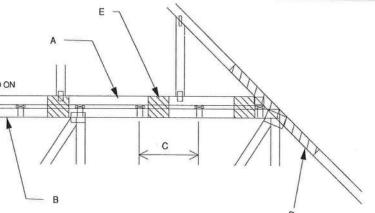
IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH DIRECTIONS AND:

1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR

2. WIND SPEED OF 116 MPH TO 160 MPH WITH A MAXIMUM PIGGYBACK SPAN OF 12f.

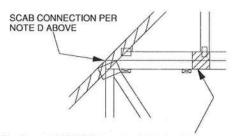
E - FOR WIND SPEED IN THE RANGE 126 MPH - 160 MPH ADD 9" x 9" x 1/2" PLYWOOD (or 7/16" OSB) GUSSET EACH SIDE AT 48" O.C. OR LESS. ATTACH WITH

3 - 6d (0.113" X 2") NAILS INTO EACH CHORD FROM EACH SIDE (TOTAL - 12 NAILS)

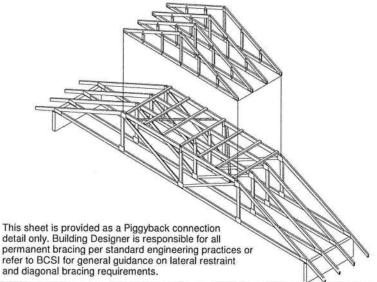


### WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

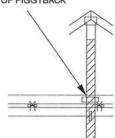
REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH PLYWOOD GUSSETS AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.



7" x 7" x 1/2" PLYWOOD (or 7/16" OSB) GUSSET EACH SIDE AT 24" O.C. ATTACH WITH 3 - 6d (0.113" X 2") NAILS INTO EACH CHORD FROM EACH SIDE (TOTAL - 12 NAILS)



VERTICAL WEB TO EXTEND THROUGH **BOTTOM CHORD** OF PIGGYBACK



FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

1) VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL

AS SHOWN IN DETAIL.

ATTACH 2 x x 4-0" SCAB TO EACH FACE OF
TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS

SPACED 4" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH
VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.) (MINIMUM 2X4)

THIS CONNECTION IS ONLY VALID FOR A MAXIMUM CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS GREATER THAN 4000 LBS.

FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS, NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS. CONCENTRATED LOAD MUST BE APPLIED TO BOTH

THE PIGGYBACK AND THE BASE TRUSS DESIGN.



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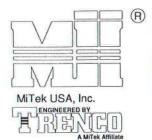
January 19, 2018

### STANDARD REPAIR DETAIL FOR BROKEN CHORDS, WEBS AND DAMAGED OR MISSING CHORD SPLICE PLATES

### MII-REP01A1

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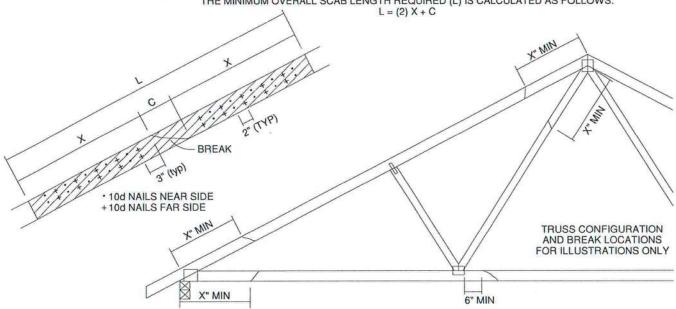


OTAL NUMBER OF NAILS EACH SIDE OF BREAK *		X INCHES	MAXIMUM FORCE (lbs) 15% LOAD DURATION								
			SP		DF		SPF		HF		
2x4	2x6		2x4	2x6	2x4	2x6	2x4	2x6	2x4	2x6	
20	30	24"	1706	2559	1561	2342	1320	1980	1352	2028	
26	39	30"	2194	3291	2007	3011	1697	2546	1738	2608	
32	48	36"	2681	4022	2454	3681	2074	3111	2125	3187	
38	57	42"	3169	4754	2900	4350	2451	3677	2511	3767	
44	66	48"	3657	5485	3346	5019	2829	4243	2898	4347	

\* DIVIDE EQUALLY FRONT AND BACK

ATTACH 2x\_ SCAB OF THE SAME SIZE AND GRADE AS THE BROKEN MEMBER TO EACH FACE OF THE TRUSS (CENTER ON BREAK OR SPLICE) WITH 10d (0.131" X 3") NAILS (TWO ROWS FOR 2x4, THREE ROWS FOR 2x6) SPACED 4" O.C. AS SHOWN. STAGGER NAIL SPACING FROM FRONT FACE AND BACK FACE FOR A NET 0-2-0 O.C. SPACING IN THE MAIN MEMBER. USE A MIN. 0-3-0 MEMBER END DISTANCE.

THE LENGTH OF THE BREAK (C) SHALL NOT EXCEED 12". (C=PLATE LENGTH FOR SPLICE REPAIRS) THE MINIMUM OVERALL SCAB LENGTH REQUIRED (L) IS CALCULATED AS FOLLOWS:



THE LOCATION OF THE BREAK MUST BE GREATER THAN OR EQUAL TO THE REQUIRED X DIMENSION FROM ANY PERIMETER BREAK OR HEEL JOINT AND A MINIMUM OF 6" FROM ANY INTERIOR JOINT (SEE SKETCH ABOVE)

### DO NOT USE REPAIR FOR JOINT SPLICES

- 1. THIS REPAIR DETAIL IS TO BE USED ONLY FOR THE APPLICATION SHOWN. THIS REPAIR DOES NOT IMPLY THAT THE REMAINING PORTION OF THE TRUSS IS UNDAMAGED. THE ENTIRE TRUSS SHALL BE INSPECTED TO VERIFY THAT NO FURTHER REPAIRS ARE REQUIRED. WHEN THE REQUIRED REPAIRS ARE PROPERLY APPLIED, THE TRUSS WILL BE CAPABLE OF SUPPORTING THE LOADS INDICATED.

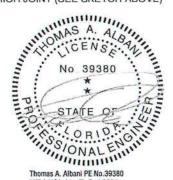
  2. ALL MEMBERS MUST BE RETURNED TO THEIR ORIGINAL POSITIONS BEFORE APPLING REPAIR
- AND HELD IN PLACE DURING APPLICATION OF REPAIR.
- AND HELD IN PLACE DURING APPLICATION OF HEPAIR.

  3. THE END DISTANCE, EDGE DISTANCE AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.

  4. WHEN NAILING THE SCABS, THE USE OF A BACKUP WEIGHT IS RECOMMENDED TO AVOID LOOSENING OF THE CONNECTOR PLATES AT THE JOINTS OR SPLICES.

  5. THIS REPAIR IS TO BE USED FOR SINGLE PLY TRUSSES IN THE 2x\_ ORIENTATION ONLY.

  6. THIS REPAIR IS LIMITED TO TRUSSES WITH NO MORE THAN THREE BROKEN MEMBERS.



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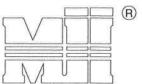
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### LATERAL TOE-NAIL DETAIL

MII-TOENAIL SP

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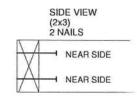
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NOTES:

- TOE-NAILS SHALL BE DRIVEN AT AN ANGLE OF 45 DEGREES WITH THE MEMBER AND MUST HAVE FULL WOOD SUPPORT. (NAIL MUST BE DRIVEN THROUGH AND EXIT AT THE BACK CORNER OF THE MEMBER END AS SHOWN.
- THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
- ALLOWABLE VALUE SHALL BE THE LESSER VALUE OF THE TWO SPECIES FOR MEMBERS OF DIFFERENT SPECIES.

### THIS DETAIL APPLICABLE TO THE THREE END DETAILS SHOWN BELOW

VIEWS SHOWN ARE FOR ILLUSTRATION PURPOSES ONLY



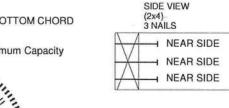
OE-NAIL SINGLE SHEAR VALUES PER NDS 2001 (lb/nail) DIAM. DF SPF SPF-S HF .131 69.9 88.0 80.6 68.4 LONG 59.7 .135 63.4 93.5 85.6 74.2 72.6 .162 108.8 99.6 86.4 84.5 73.8 3.5 LONG .128 74.2 57.6 50.3 67.9 58.9 75.9 69.5 60.3 59.0 51.1 .131 3.25" 81.4 64.6 63.2 .148 74.5 52.5

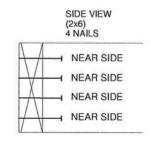
VALUES SHOWN ARE CAPACITY PER TOE-NAIL. APPLICABLE DURATION OF LOAD INCREASES MAY BE APPLIED.

### EYAMDI E.

(3) - 16d (0.162" X 3.5") NAILS WITH SPF SPECIES BOTTOM CHORD

For load duration increase of 1.15: 3 (nails) X 84.5 (lb/nail) X 1.15 (DOL) = 291.5 lb Maximum Capacity

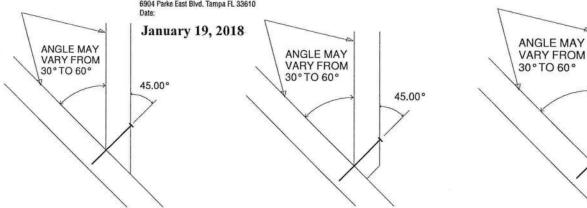




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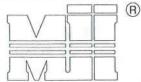


### TRUSSED VALLEY SET DETAIL

### MII-VALLEY HIGH WIND1

MiTek USA, Inc.

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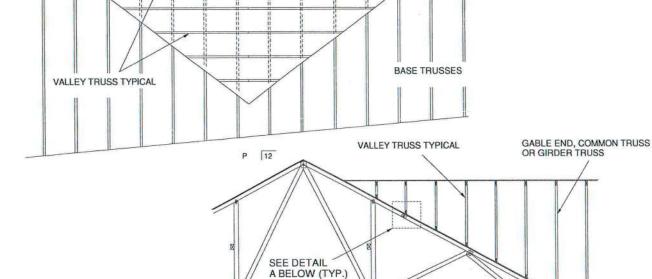
MiTek USA, Inc.

PRINCIPERED BY

GABLE END, COMMON TRUSS OR GIRDER TRUSS

### **GENERAL SPECIFICATIONS**

- 1. NAIL SIZE 10d (0.131" X 3")
- 2. WOOD SCREW = 3" WS3 USP OR EQUIVALENT DO NOT USE DRYWALL OR DECKING TYPE SCREW
- 3. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND SECURE PER DETAIL A
- 4. BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS.
- 5. BASE TRUSS SHALL BE DESIGNED WITH A PURLIN SPACING
- EQUILIVANT TO THE RAKE DIMENSION OF THE VALLEY TRUSS SPACING.
- 6. NAILING DONE PER NDS 01
- 7. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.



SECURE VALLEY TRUSS W/ ONE ROW OF 10d NAILS 6" O.C.

ATTACH 2x4 CONTINUOUS NO.2 SP TO THE ROOF W/TWO USP WS3 (1/4" X 3") WOOD SCREWS INTO EACH BASE TRUSS. **DETAIL A** (NO SHEATHING)

N.T.S.

WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05, 146 MPH WIND DESIGN PER ASCE 7-10 160 MPH MAX MEAN ROOF HEIGHT = 30 FEET ROOF PITCH = MINIMUM 3/12 MAXIMUM 6/12

CATEGORY II BUILDING

EXPOSURE C

WIND DURATION OF LOAD INCREASE: 1.60 MAX TOP CHORD TOTAL LOAD = 50 PSF MAX SPACING = 24" O.C. (BASE AND VALLEY) MINIMUM REDUCED DEAD LOAD OF 6 PSF

ON THE TRUSSES



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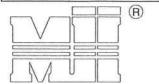
January 19, 2018

### TRUSSED VALLEY SET DETAIL

MII-VALLEY HIGH WIND2

MiTek USA, Inc.

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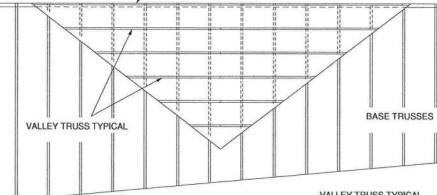


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GABLE END, COMMON TRUSS OR GIRDER TRUSS

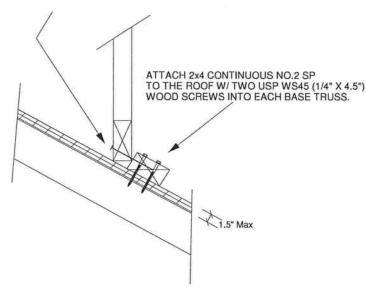
### GENERAL SPECIFICATIONS

- 1. NAIL SIZE 10d (0.131" X 3")
  2. WOOD SCREW = 4.5" WS45 USP OR EQUILIVANT
  3. INSTALL SHEATHING TO TOP CHORD OF BASE TRUSSES.
  4. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND SECURE TO BASE TRUSSES AS PER DETAIL A
  5. BRACE VALLEY WBBS IN ACCORDANCE WITH THE
- INDIVIDUAL DESIGN DRAWINGS.
- 6. NAILING DONE PER NDS-01
- 7. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.



GABLE END, COMMON TRUSS VALLEY TRUSS TYPICAL OR GIRDER TRUSS SEE DETAIL A BELOW (TYP.)

SECURE VALLEY TRUSS W/ ONE ROW OF 10d NAILS 6" O.C.



WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH WIND DESIGN PER ASCE 7-10 160 MPH MAX MEAN ROOF HEIGHT = 30 FEET ROOF PITCH = MINIMUM 3/12 MAXIMUM 6/12 CATEGORY II BUILDING EXPOSURE C WIND DURATION OF LOAD INCREASE: 1.60 MAX TOP CHORD TOTAL LOAD = 50 PSF MAX SPACING = 24" O.C. (BASE AND VALLEY) MINIMUM REDUCED DEAD LOAD OF 6 PSF ON THE TRUSSES

No 39380

STATE OF UNITED ON AL ENGINEER A. ALBORDA CO. A

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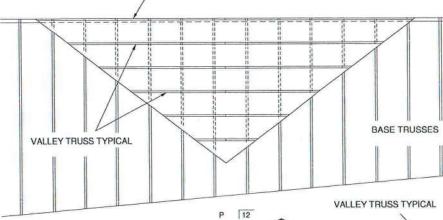
MiTek USA, Inc.

ENGINEERED BY 国别剧 (40

GABLE END, COMMON TRUSS OR GIRDER TRUSS

### **GENERAL SPECIFICATIONS**

- NAIL SIZE 16d (0.131" X 3.5")
   INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND SECURE PER DETAIL A
- 3. BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS.
- 4. BASE TRUSS SHALL BE DESIGNED WITH A PURLIN SPACING
- EQUILIVANT TO THE RAKE DIMENSION OF THE VALLEY TRUSS SPACING.
- 5. NAILING DONE PER NDS 01
- 6. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.
- 7. ALL LUMBER SPECIES TO BE SP.

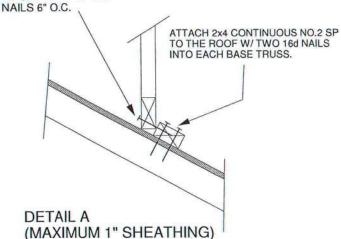


GABLE END, COMMON TRUSS

OR GIRDER TRUSS SEE DETAIL A BELOW (TYP.)

SECURE VALLEY TRUSS W/ ONE ROW OF 16d

N.T.S.



WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 120 MPH WIND DESIGN PER ASCE 7-10 150 MPH MAX MEAN ROOF HEIGHT = 30 FEET ROOF PITCH = MINIMUM 3/12 MAXIMUM 10/12 CATEGORY II BUILDING EXPOSURE C OR B

WIND DURATION OF LOAD INCREASE: 1.60 MAX TOP CHORD TOTAL LOAD = 60 PSF MAX SPACING = 24" O.C. (BASE AND VALLEY) MINIMUM REDUCED DEAD LOAD OF 4.2 PSF ON THE TRUSSES

No 39380

No 39380

No 39380

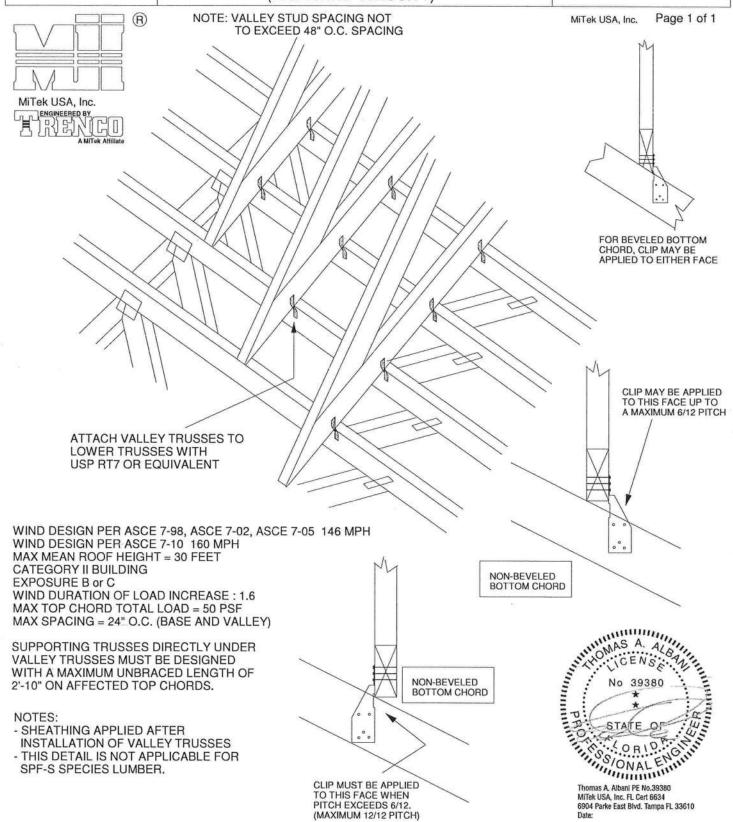
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### TRUSSED VALLEY SET DETAIL (HIGH WIND VELOCITY)

MII-VALLEY

January 19, 2018





### Standard Gable End Detail

### MII-GE146-001



Page 1 of 2



MiTek USA, Inc. ENGINEERED BY (40

Diagonal Bracing

Refer to Section A-A

Typical \_x4 L-Brace Nailed To 2x\_ Verticals W/10d Nails spaced 6" o.c. Vertical Stud SECTION B-B

DIAGONAL BRACE 4'-0" O.C. MAX TRUSS GEOMETRY AND CONDITIONS SHOWN ARE FOR ILLUSTRATION ONLY. Varies to Common Truss SEE INDIVIDUAL MITEK ENGINEERING DRAWINGS FOR DESIGN CRITERIA 3x4 =

L-Bracing Refer

to Section B-B

Vertical Stud DIAGONAL BRACE (4) - 16d Nails 16d Nails Spaced 6" o.c. (2) - 10d Nails into 2x6 2x6 Stud or 2x4 No.2 of better Typical Horizontal Brace Nailed To 2x\_ Verticals w/(4)-10d Nails SECTION A-A

> PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SP BLOCK

Roof Sheathing

1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG.

ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT

BRACING OF ROOF SYSTEM.

4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES:
2x4 No 3/STUD SP OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.

5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF

DIAPHRAM AT 4'-0" O.C. 6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)

GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.

8, THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.

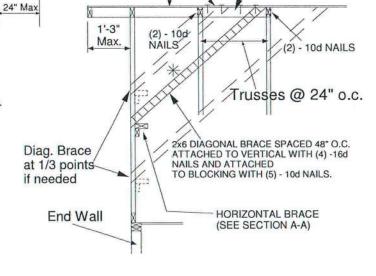
10. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Minimum Stud Size	Stud Spacing	Without Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS			
Species and Grade		Maximum Stud Length						
2x4 SP No 3/Stud	12" O.C.	3-11-3	6-8-0	7-2-14	11-9-10			
2x4 SP No 3/Stud	16" O.C.	3-6-14	5-9-5	7-1-13	10-8-11			
2x4 SP No 3/Stud	24" O.C.	3-1-8	4-8-9	6-2-15	9-4-7			

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of web with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAXIMUM WIND SPEED = 146 MPH MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING EXPOSURE B or C ASCE 7-98, ASCE 7-02, ASCE 7-05 **DURATION OF LOAD INCREASE: 1.60** 

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.





6904 Parke East Blvd. Tampa FL 33610

January 19, 2018

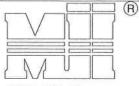
### **OCTOBER 5, 2016**

### REPLACE BROKEN OVERHANG

MII-REP13B

MiTek USA, Inc.

Page 1 of 1



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TRUSS CRITERIA:

LOADING: 40-10-0-10 **DURATION FACTOR: 1.15** SPACING: 24" O.C. TOP CHORD: 2x4 OR 2x6 PITCH: 4/12 - 12/12

HEEL HEIGHT: STANDARD HEEL UP TO 12" ENERGY HEEL

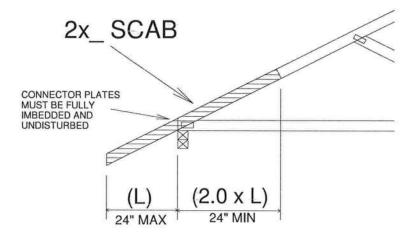
END BEARING CONDITION

### NOTES:

1. ATTACH 2x\_ SCAB (MINIMUM NO.2 GRADE SPF, HF, SP, DF) TO ONE FACE OF TRUSS WITH TWO ROWS OF 10d (0.131" X 3") SPACED 6" O.C.
2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE-SUCH

AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.

3. WHEN NAILING THE SCABS, THE USE OF A BACKUP WEIGHT IS RECOMMENDED TO AVOID LOOSENING OF THE CONNECTOR PLATES AT THE JOINTS OR SPLICES.



### **IMPORTANT**

This detail to be used only with trusses (spans less than 40') spaced 24" o.c. maximum and having pitches between 4/12 and 12/12 and total top chord loads not exceeding 50 psf. Trusses not fitting these criteria should be examined individually.

REFER TO INDIVIDUAL TRUSS DESIGN FOR PLATE SIZES AND LUMBER GRADES



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

### LATERAL BRACING RECOMMENDATIONS

MII-STRGBCK

MiTek USA, Inc.

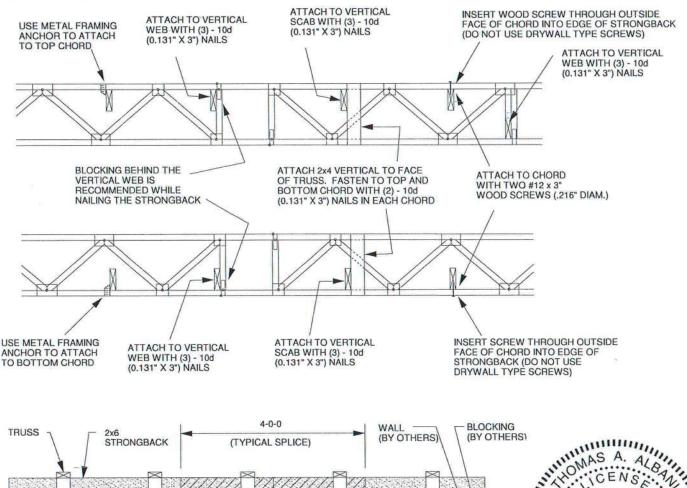
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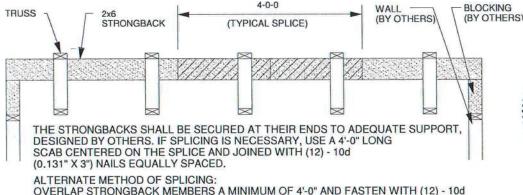


TO MINIMIZE VIBRATION COMMON TO ALL SHALLOW FRAMING SYSTEMS, 2x6 "STRONGBACK" IS RECOMMENDED, LOCATED EVERY 8 TO 10 FEET ALONG A FLOOR TRUSS.

NOTE 1: 2X6 STRONGBACK ORIENTED VERTICALLY MAY BE POSITIONED DIRECTLY UNDER THE TOP CHORD OR DIRECTLY ABOVE THE BOTTOM CHORD. SECURELY FASTENED TO THE TRUSS USING ANY OF THE METHODS ILLUSTRATED BELOW.

NOTE 2: STRONGBACK BRACING ALSO SATISFIES THE LATERAL BRACING REQUIREMENTS FOR THE BOTTOM CHORD OF THE TRUSS WHEN IT IS PLACED ON TOP OF THE BOTTOM CHORD, IS CONTINUOUS FROM END TO END, CONNECTED WITH A METHOD OTHER THAN METAL FRAMING ANCHOR, AND PROPERLY CONNECTED, BY OTHERS, AT THE ENDS.





ALTERNATE METHOD OF SPLICING: OVERLAP STRONGBACK MEMBERS A MINIMUM OF 4'-0" AND FASTEN WITH (12) - 10d (0,131" X 3") NAILS STAGGERED AND EQUALLY SPACED. (TO BE USED ONLY WHEN STRONGBACK IS NOT ALIGNED WITH A VERTICAL) No 39380

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