

GENERAL NOTES

FABRICATION SHALL BE IN ACCORDANCE WITH ELITE STRUCTURES STANDARD PRACTICES AND IN COMPLIANCE WITH THE APPLICABLE SECTIONS, RELATING TO DESIGN REQUIREMENTS AND ALLOWABLE STRESSES OF THE LATEST EDITION OF THE 'AWS STRUCTURAL WELDING CODE D 1.1 AND D1.3'

MATERIALS	ASTM DESIGNATION	MIN. YIELD STRENGTH
HOT ROLLED STEEL SHAPES	A572	FY = 50 KSI
STEEL PIPES	A500	FY = 42 KSI
STRUCTURAL TUBING	A500	FY = 42 KSI
STRUCTURAL STEEL WEB PLATE	A572/A501	FY = 50 KSI
STRUCTURAL STEEL FLANGE PLATES/BARS	A572/A501	FY = 50 KSI
COLD FORM LIGHT GAUGE	A572/A501	FY = 50, 55 KSI
ROOF AND WALL SHEETS	A792/A583	FY = 50, 60 KSI
CABLE BRACE	A475 - TYPE 1	EXTRA HIGH STRENGTH
ROD BRACE	A36	FY = 36 KSI
WALL SECTIONS	A36	FY = 36 KSI

MACHINE BOLTS & NUTS	A307	MIN. TENSILE STRENGTH
HIGH STRENGTH BOLTS (1" DIA. & LESS)	A325 - TYPE 1	FU = 60 KSI
HIGH STRENGTH BOLTS (1" TO 1-1/2")	A325 - TYPE 1	FU = 120 KSI
ANCHOR BOLTS (IF SUPPLIED)	F1554 - GR6	FU = 105 KSI

CLOSURE STRIPS ARE FURNISHED ONLY IF NOTED ON SHIPPING DOCUMENTS

INSIDE - UNDER ROOF PANELS AT EAVE
OUTSIDE - BETWEEN END WALL PANELS AND RAKE TRIM
UNDER CONTINUOUS RIDGE VENT SKIRTS

ERECTION NOTE:

ALL BRACING, STRAPPING, & BRIDGING SHOWN AND PROVIDED BY ELITE STRUCTURES FOR THIS BUILDING IS REQUIRED AND SHALL BE INSTALLED BY THE ERECTOR AS A PERMANENT PART OF THE STRUCTURE. IF ADDITIONAL BRACING IS REQUIRED FOR STABILITY DURING ERECTION, IT SHALL BE THE ERECTOR'S RESPONSIBILITY TO DETERMINE THE AMOUNT OF SUCH BRACING AND TO PROVIDE AND INSTALL AS NEEDED.

ERECTION AND UNLOADING NOT BY ELITE STRUCTURES

SHORTAGES

ANY CLAIMS OR SHORTAGES BY BUYERS MUST BE MADE TO ELITE STRUCTURES AT TIME OF DELIVERY, OR SUCH CLAIMS SHALL BE CONSIDERED WAIVED BY THE CUSTOMER AND DISALLOWED IF MATERIAL IS SHOWN DELIVERED

CORRECTIONS OF ERRORS AND REPAIRS (MBMA 6.10)

CLAIMS FOR CORRECTIONS OF ALLEGED MISFITS WILL BE DISALLOWED UNLESS ELITE STRUCTURES SHALL HAVE RECEIVED PRIOR NOTICE THEREOF AND ALLOWED REASONABLE INSPECTION OF SUCH MISFITS. THE CORRECTION OF MINOR MISFITS BY THE USE OF DRIFT PINS TO DRAW THE COMPONENTS INTO LINE, MODERATE AMOUNTS OF REPAIRING, CHIPPING AND CUTTING, AND THE REPLACEMENT OF MINOR SHORTAGES OF MATERIAL ARE A NORMAL PART OF ERECTION AND ARE NOT SUBJECT TO CLAIM.

A-325 BOLT TIGHTENING REQUIREMENTS

ALL BOLTED JOINTS WITH A325 OR TYPE 1 BOLTS ARE SPECIFIED AS PRETENSIONED JOINTS IN ACCORDANCE WITH THE SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS, JUNE 30, 2004. PRETENSIONING METHODS, INCLUDING TURN-OF-NUT, CALIBRATED WRENCH, TIGHT OFF TYPE TENSION CONTROL, BOLTS OR DIRECT TENSION INDICATOR ARE NOT REQUIRED. INSTALLATION INSPECTION REQUIREMENTS FOR SNUG TIGHT (SPECIFICATION FOR STRUCTURAL JOINTS 9.1) IS SUGGESTED.

REFERENCE: STEEL CONSTRUCTION MANUAL
AMERICAN INSTITUTE OF STEEL CONSTRUCTION INC.
THIRTEENTH EDITION (AISC)

FLORIDA PRODUCT APPROVAL NUMBER - ELITE STRUCTURES 26 GAUGE PBR PANEL

WALL PANEL FL27502

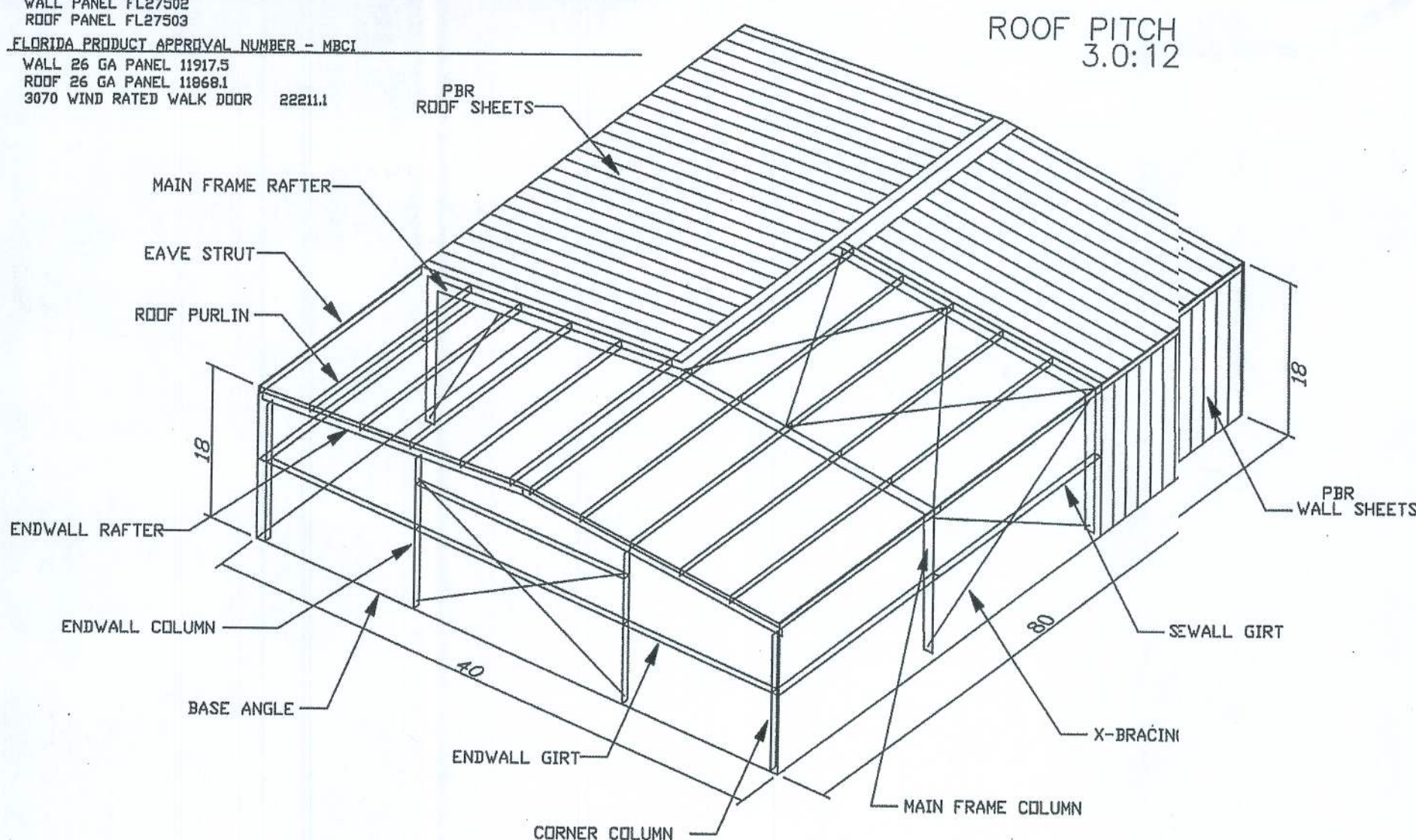
ROOF PANEL FL27503

FLORIDA PRODUCT APPROVAL NUMBER - HBCI

WALL 26 GA PANEL 11917.5

ROOF 26 GA PANEL 11968.1

3070 WIND RATED WALK DOOR 22211.1



BUILDING DRAWING FOR ILLUSTRATION ONLY



401 OLD QUITMAN I.
ADEL, GA. 31620
229-896-7569



ELITE-STRUCTURES.COM

CONSTRUCTION MANUAL AVAILABLE ONLINE
WWW.ELITE-STRUCTURES.COM

PROJECT: SHEP'S TRUCKING
PROJECT MANAGER: BRANDON HARPER
LOCATION: LAKE CITY, FL.

BUILDING DESCRIPTION:
40' x 80' x 18'

LOCAL BUILDING CODE: FBC 220
ENCLOSURE TYPE: Closed
BUILDING RISK CATEGORY: II - Normal
LOAD REDUCTION USED: Yes
LIVE LOAD TO FRAMES: 12
LIVE LOAD TO PURLINS: 20.00
DEAD LOAD: 2.500
COLLATERAL LOAD: 0
SNOW LOAD (GROUND): 0
SNOW LOAD (ROOF): 0
WIND SPEED: 119
WIND EXPOSURE: B
SEISMIC ZONE: B
SEISMIC IMPORTANCE: 1.00
SPECTRAL RESPONSE S_s: 0.09
SPECTRAL RESPONSE S₁: 0.05
RAIN INTENSITY: 5 yr. 10
25 yr. 11.0000

DESIGN SPECIFICATIONS
FOR
JOB ID: 11996

BUYER/CUSTOMER RESPONSIBILITIES

APPROVAL OF ELITE STRUCTURES DRAWINGS AND CALCULATIONS INDICATES THAT ELITE STRUCTURES HAS CORRECTLY INTERPRETED AND APPLIED THE CONTRACT DOCUMENTS. THIS APPROVAL CONSTITUTES THE CONTRACTOR/OWNER ACCEPTANCE OF THE ELITE STRUCTURES DESIGN CONCEPTS, ASSUMPTIONS, AND LOADING (SECTION 4 AISC CODE 9TH EDITION AND MBMA 3.3.3)

ONCE THE BUYER/END USER CUSTOMER HAS SIGNED ELITE STRUCTURES APPROVAL PACKAGE AND THE PROJECT IS RELEASED FOR FABRICATION, CHANGES SHALL BE BILLED TO THE BUYER/END USER CUSTOMER INCLUDING MATERIAL, ENGINEERING AND OTHER COSTS. AN ADDITIONAL FEE MAY BE CHARGED IF THE PROJECT MUST BE MOVED FROM THE FABRICATION AND SHIPPING SCHEDULE.

THE BUYER/END USER CUSTOMER IS RESPONSIBLE FOR OVERALL PROJECT COORDINATION, ALL INTERFACE, COMPATIBILITY AND DESIGN CONSIDERATIONS CONCERNING ANY MATERIAL NOT FURNISHED BY ELITE STRUCTURES ARE TO BE CONSIDERED AND COORDINATED BY THE BUYER/END USER CUSTOMER. SPECIFIC DESIGN CRITERIA CONCERNING THIS INTERFACE BETWEEN MATERIALS MUST BE FURNISHED BEFORE RELEASE FOR FABRICATION OR ELITE STRUCTURES ASSUMPTIONS WILL GOVERN (SECTION 4 AND COMMENTARY, AISC CODE OF STANDARD 9TH EDITION)

IT IS THE RESPONSIBILITY OF THE BUYER/END CUSTOMER TO INSURE THAT ELITE STRUCTURES PLANS COMPLIES WITH THE APPLICABLE REQUIREMENTS OF ANY GOVERNING BUILDING AUTHORITIES. THE SUPPLYING OF SEALED ENGINEERING DATA AND DRAWINGS FOR THE METAL BUILDING SYSTEM DOES NOT IMPLY OR CONSTITUTE AN AGREEMENT THAT ELITE STRUCTURES OR ITS DESIGN ENGINEERS ARE ACTING AS THE ENGINEER OF RECORD OR DESIGN PROFESSIONAL FOR A CONSTRUCTION PROJECT. THESE DRAWINGS ARE SEALED ONLY TO CERTIFY THE DESIGN OF THE STRUCTURAL COMPONENTS FURNISHED BY ELITE STRUCTURES

THE BUYER/END USER CUSTOMER IS RESPONSIBLE FOR SETTING OF ANCHOR BOLTS AND ERECTION OF THE STEEL IN ACCORDANCE WITH ELITE STRUCTURES 'FOR CONSTRUCTION' DRAWINGS ONLY. TEMPORARY SUPPORTS SUCH AS GUYS, BRACES, FALSEWORK, CRIBBING OR OTHER ELEMENTS REQUIRED FOR ERECTION OPERATION SHALL BE DETERMINED, FURNISHED AND INSTALLED BY ERECTOR. NO ITEMS SHOULD BE PURCHASED FROM A PRELIMINARY SET OF DRAWINGS, INCLUDING ANCHOR BOLTS, USE ONLY FINAL 'FOR CONSTRUCTION' DRAWINGS FOR THE USE.

ELITE STRUCTURES IS RESPONSIBLE FOR THE DESIGN OF THE ANCHOR BOLTS TO PERMIT THE TRANSFER OF FORCES BETWEEN THE BASE PLATE AND ANCHOR BOLT IN SHEAR, BEARING AND TENSION, BUT IS NOT RESPONSIBLE FOR THE TRANSFER OF ANCHOR BOLT FORCES TO THE CONCRETE OR THE ADEQUACY OF THE ANCHOR BOLT IN RELATION TO THE CONCRETE.

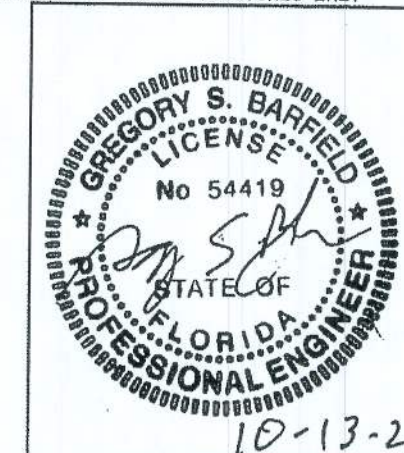
UNLESS OTHERWISE PROVIDED IN THE ORDER DOCUMENT, ELITE STRUCTURES DOES NOT DESIGN AND IS NOT RESPONSIBLE FOR THE DESIGN, MATERIAL AND CONSTRUCTION OF THE FOUNDATION OR FOUNDATION ELEMENTS. THE END USER CUSTOMER SHOULD OBTAIN HURDLES THAT ASSURE PROVISIONS ARE MADE IN THE FOUNDATION DESIGN FOR LOADS IMPOSED BY COLUMN REACTIONS, OTHER IMPOSED LOADS, AND BEARING CAPACITY OF THE SOIL AND OTHER CONDITIONS OF THE BUILDING SITE.

NORMAL ERECTION OPERATIONS INCLUDE THE CORRECTIONS OF MINOR MISFITS BY MODERATE AMOUNTS OF REPAIRING, CHIPPING, WELDING OR CUTTING, AND THE DRAWING OF ELEMENTS INTO LINE THROUGH THE USE OF DRIFT PINS. ERRORS WHICH CANNOT BE CORRECTED BY FOREGOING MEANS OR WHICH REQUIRE MAJOR CHANGES IN MEMBERS CONFIGURATION ARE TO REPORT IMMEDIATELY TO ELITE STRUCTURES BY THE BUYER/END USER CUSTOMER TO ENABLE WHEREVER IS RESPONSIBLE EITHER TO CORRECT THE ERROR OR TO APPROVE THE MOST EFFICIENT AND ECONOMIC METHOD OF CORRECTION TO BE USED BY OTHERS. (SECTION 7 AISC CODE OF STANDARD 9TH EDITION)

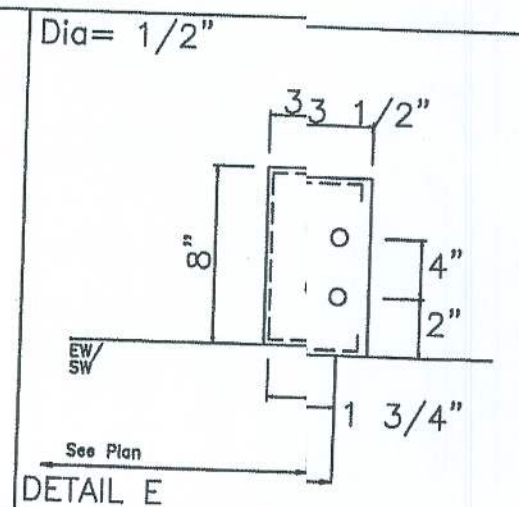
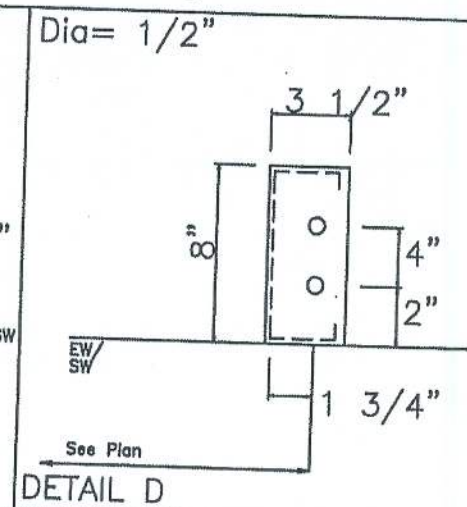
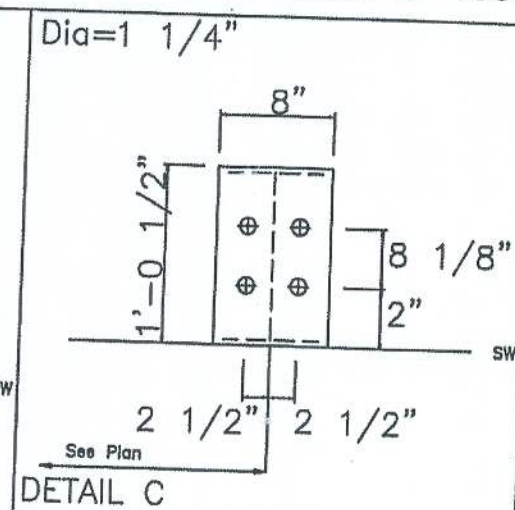
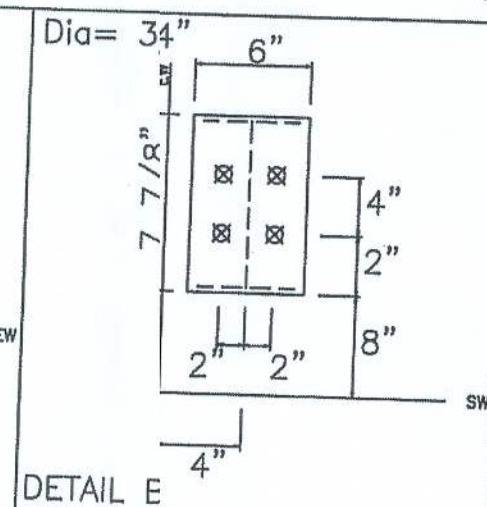
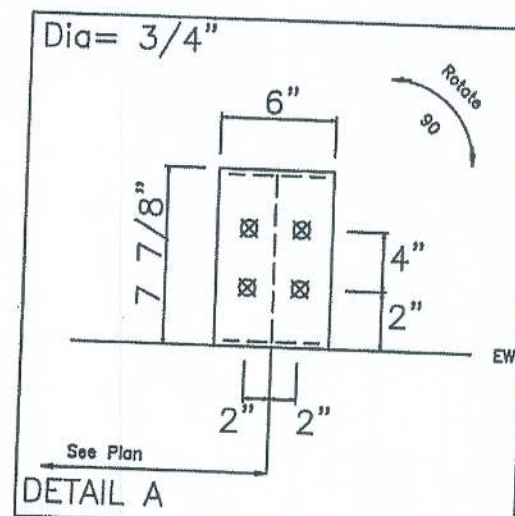
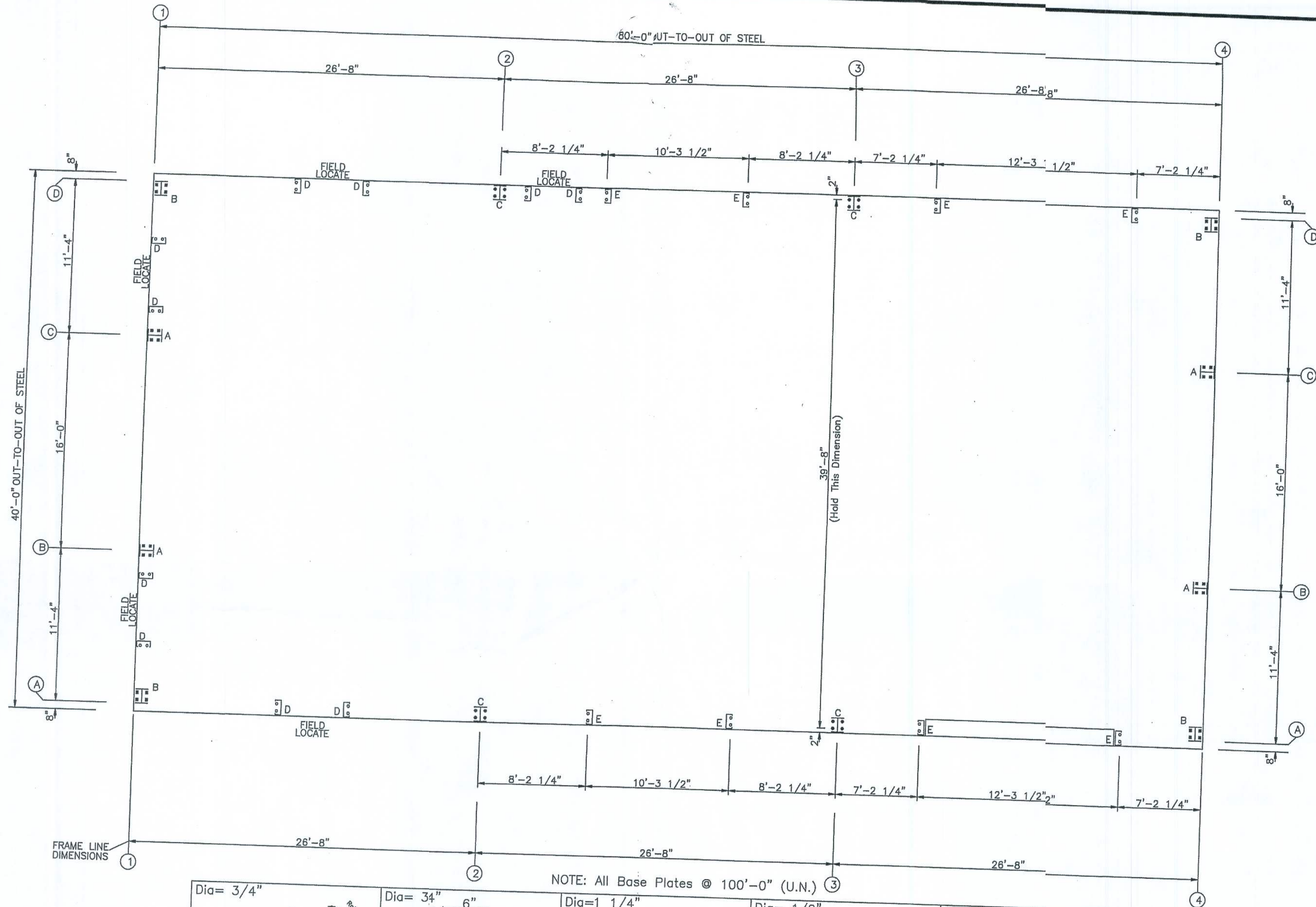


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ENGINEERING SEAL
THIS CERTIFICATION COVERS PARTS MANUFACTURED AND DELIVERED BY ELITE STRUCTURES ONLY



SEALING OF THESE DRAWINGS DOES NOT IMPLY OR CONSTITUTE THAT ELITE STRUCTURES ENGINEER IS THE ENGINEER OF RECORD OR THE DESIGN PROFESSIONAL FOR THE THIS PROJECT. ONLY THE DESIGN OF THE METAL BUILDING SYSTEM AS FURNISHED BY ELITE STRUCTURES IS INCLUDED. FOUNDATIONS ANALYSIS, ELECTRICAL AND MECHANICAL SYSTEMS, AND/OR OTHER PARTS SUPPLIED BY ANYONE OTHER THAN ELITE STRUCTURES ARE SPECIFICALLY EXCLUDED, NO INSPECTION OR SUPERVISION IS IMPLIED.



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SCALE: N.T.S. DATE: 9-6-21
DRAWN BY: R WALKER SALES: BRANDON HARPER
BUILDING SIZE: 40'X80'X18'

REVISION	DESCRIPTION

TITLE/LOCATION
SHEPS TRUCKING
LAKE CITY, FL.

PLAN:
ANCHOR BOLT PL²AN

JOB # 11996-AB
SHEET 1 OF 10



10-13-21

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column Reactions(k)						Bolt(in) Qty	Dia	Base Plate(in)			Grout (in)
		Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin			Width	Length	Thick	
2*	D	3	3.7	5.0	6	-4.6	-3.9	4	1.250	8.000	12.50	1.000	0.0
		1	2.5	9.1	4	-4.0	-6.3						
2*	A	7	4.6	-3.9	2	-3.7	5.0	4	1.250	8.000	12.50	1.000	0.0
		1	-2.5	9.1	5	4.0	-6.3						

2* Frame lines: 2 3

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	Dead		Live		Wind Left1		Wind Right1		Wind Left2		Wind Right2	
		Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
2*	D	0.5	2.2	1.9	6.9	-7.3	-12.7	3.8	-5.2	-1	-8.6	2.9	-1.0
2*	A	-0.5	2.2	-1.9	6.9	-3.8	-5.2	7.3	-12.7	-9	-1.0	8.1	-8.6

Frame Line	Column Line	Wind Long1		Wind Long2		Seismic Left		Seismic Right	
		Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
2*	D	1.4	-8.6	0.5	-7.7	-0.3	-0.2	0.3	0.2
2*	A	-0.5	-7.7	-1.4	-8.6	-0.3	0.2	0.3	-0.2

2* Frame lines: 2 3

ENDWALL COLUMN: BASIC COLUMN REACTIONS (k)

Frm Line	Col Line	Dead Vert	Live Vert	Wind Left1		Wind Right1		Wind Left2		Wind Right2		Wind Press Horiz	Wind Suct Horiz	Wd Lsg1 Vert	Wind Long2 Vert	Seis Left Vert	Seis Right Vert
				Vert	Vert	Vert	Vert	Vert	Vert	Vert	Vert						
1	D	0.6	2.4	-1.8	-1.1	-1.0	-0.4	0.0	0.0	-8	-1.3	0.0	0.0	-1	-1.3	0.0	0.1
1	C	0.8	3.7	-4.5	-3.2	-3.3	-1.9	-2.4	2.7	-0	-2.4	0.0	0.0	-1	-2.4	0.0	-0.1
1	B	0.8	3.7	-3.2	-4.5	-1.9	-3.3	-2.4	2.7	-4	-4.0	-0.1	0.0	-1	-4.0	0.0	0.0
4	A	0.6	2.4	-1.1	-1.8	-0.4	-1.0	0.0	0.0	-3	-1.8	0.1	0.0	-1	-1.8	0.0	0.0
4	B	0.8	3.7	-4.5	-3.2	-3.3	-1.9	-2.4	2.7	-0	-2.4	0.0	0.0	-1	-2.4	0.0	0.1
4	C	0.8	3.7	-3.2	-4.5	-1.9	-3.3	-2.4	2.7	-4	-4.0	-0.1	0.0	-1	-4.0	0.0	-0.1
4	D	0.6	2.4	-1.1	-1.8	-0.4	-1.0	0.0	0.0	-3	-1.8	0.1	0.0	-1	-1.8	0.0	0.0

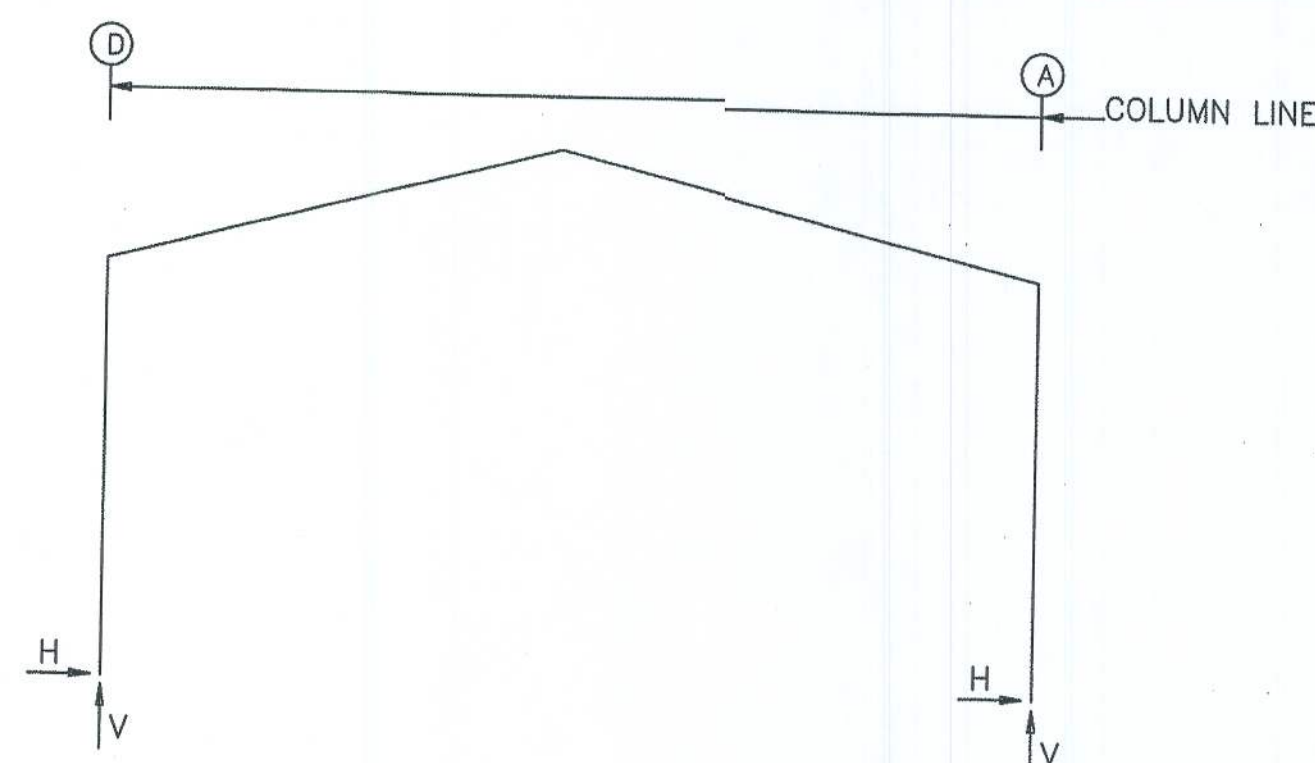
ENDWALL COLUMN: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column Reactions(k)						Bolt(in) Qty	Dia	Base Plate(in)			Grout (in)
		Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin			Width	Length	Thick	
1	D	8	0.0	-0.8	8	0.0	-0.8	4	0.750	6.000	7.875	375	0.0
		1	0.0	3.0									
1	C	10	1.6	-2.2	11	-1.5	-1.9	4	0.750	6.000	7.875	375	0.0
		1	0.0	4.5	10	1.6	-2.2						
1	B	12	1.6	-2.2	13	-1.5	-1.9	4	0.750	6.000	7.875	375	0.0
		1	0.0	4.5	12	1.6	-2.2						
1	A	9	0.0	-0.8	9	0.0	-0.8	4	0.750	6.000	7.875	375	0.0
		1	0.0	3.0									
4	A	8	0.0	-0.8	8	0.0	-0.8	4	0.750	6.000	7.875	375	0.0
		1	0.0	3.0									
4	B	10	1.6	-2.2	11	-1.5	-1.9	4	0.750	6.000	7.875	375	0.0
		1	0.0	4.5	10	1.6	-2.2						
4	C	12	1.6	-2.2	13	-1.5	-1.9	4	0.750	6.000	7.875	375	0.0
		1	0.0	4.5	12	1.6	-2.2						
4	D	9	0.0	-0.8	9	0.0	-0.8	4	0.750	6.000	7.875	375	0.0
		1	0.0	3.0									

ANCHOR BOLT SUMMARY

Qty	Locate	Dia (in)	Type	Proj (in)
36	Jamb	1/2"	GR36	1.50
32	Endwall	3/4"	GR36	2.50
16	Frame	1 1/4"	GR36	3.50

FRAME LINES: 2 3



NOTES FOR REACTIONS

Building reactions are based on the following building data:

Width (ft)	=	40.0
Length (ft)	=	80.0
Eave Height (ft)	=	18.0/ 18.0
Roof Slope (rise/12)	=	3.0/ 3.0
Dead Load (psf)	=	2.5
Collateral Load (psf)	=	0.0
Roof Live Load (psf)	=	20.0
Frame Live Load (psf)	=	12.0
Wind Speed (mph)	=	119.0
Wind Code	=	FBC 20 (IBC 18)
Exposure	=	B
Closed/Open	=	C
Importance Wind	=	1.00
Importance Seismic	=	1.00
Seismic Zone	=	B
Seismic Coeff (Fa*Gs)	=	0.14

ID Description

1	Dead+Collateral+Live
2	Dead+Collateral+0.75Live+0.45Wind_Left1
3	Dead+Collateral+0.75Live+0.45Wind_Right1
4	0.6Dead+0.6Wind_Left1
5	0.6Dead+0.6Wind_Right1
6	0.6Dead+0.6Wind_Left2
7	0.6Dead+0.6Wind_Right2
8	0.6Dead+0.6Wind_Long1L
9	0.6Dead+0.6Wind_Long2L
10	0.6Dead+0.6Wind_Left1+0.6Wind_Suction
11	0.6Dead+0.6Wind_Pressure+0.6Wind_Long1L
12	0.6Dead+0.6Wind_Right1+0.6Wind_Suction
13	0.6Dead+0.6Wind_Pressure+0.6Wind_Long2L

BUILDING BRACING REACTIONS

Loc	Wall Line	Col Line	Reactions(k)				Panel Shear (lb/ft)		Note
			Horiz	Vert	Horiz	Vert	Wind	Seis	
L-EW	1								(i)
F-SW	A								(i)
R-EW	4								(i)
B-SW	D								(i)

(i) Bracing in roof to rigid frame
(j) Weak axis bending is used

RIGID FRAME REACTIONS: WEAK AXIS BENDING

Loc	Wall Line	Col Line	Reactions(k, f-k)			Load ID
			Horiz	AB_Vert	Moment	
F-SW	A	2	1.8	56.2	30.4	Wind
F-SW	A	2	0.5	14.4	7.8	Seismic
F-SW	A	3	1.8	56.2	30.4	Wind
F-SW	A	3	0.5	14.4	7.8	Seismic
B-SW	D	3	1.8	56.2	30.4	Wind
B-SW	D	3	0.5	14.4	7.8	Seismic
B-SW	D	2	1.8	56.2	30.4	Wind
B-SW	D	2	0.5	14.4	7.8	Seismic

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SCALE: N.T.S.	DATE: 9-6-21
DRAWN BY: R. WALKER	SALES: BRANDON HARPER
BUILDING SIZE: 40'X80'X18'	

REVISION

DESCRIPTION

TITLE/LOCATION

SHEPS TRUCKING
LAKE CITY, FL.

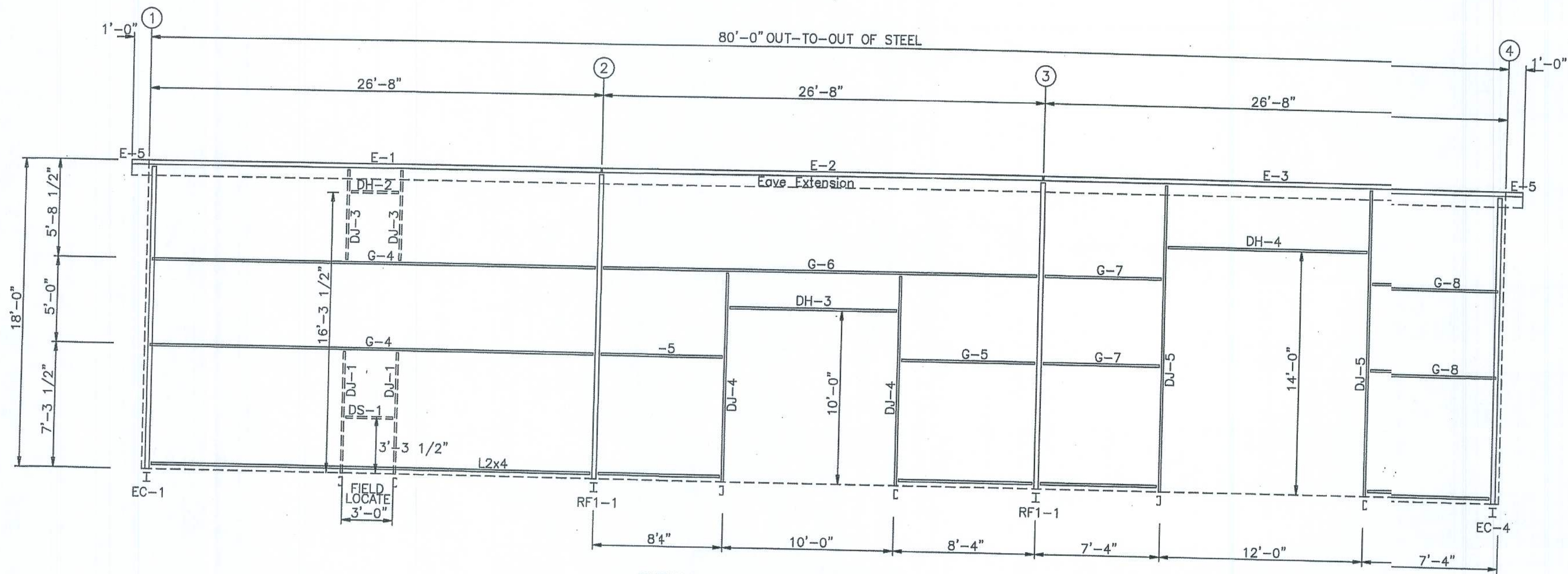
PLAN:

COLUMN REACTIONNS

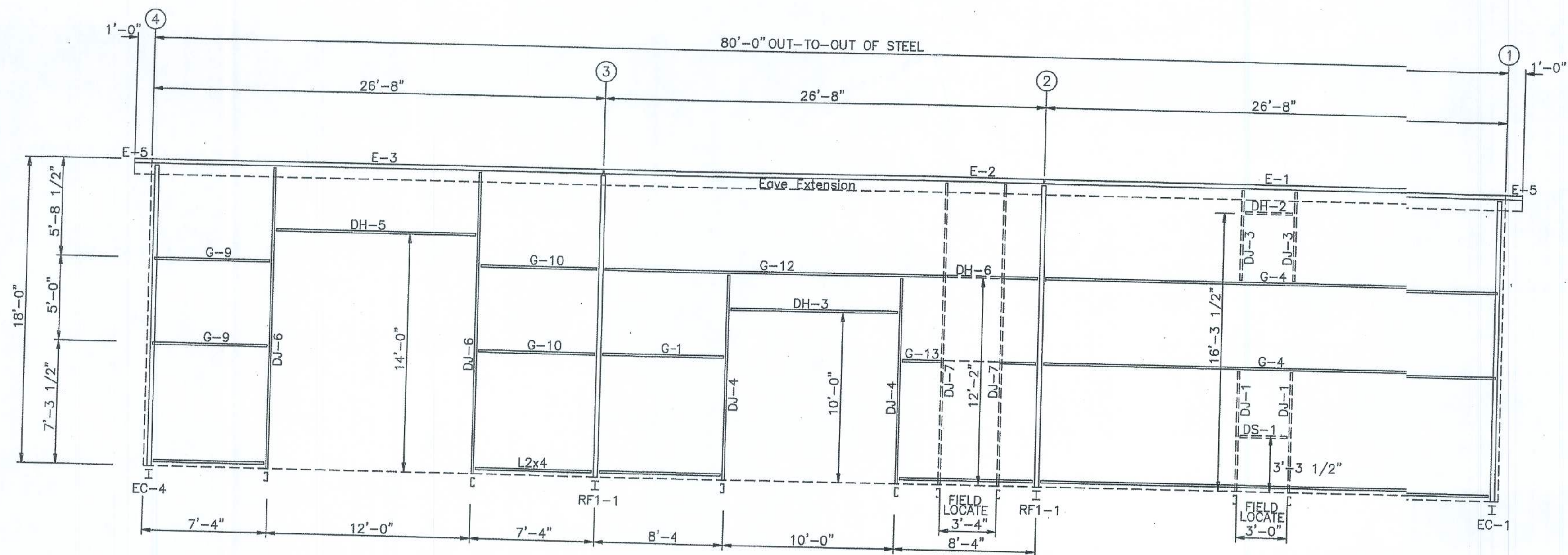
JOB # 11996-CR

SHEET 2 OF 10

10-13-21



SIEWALL FRAMING: FRAME LINE A



SIDEWALL FRAMING: FRAME LINE D

MEMBER TABLE FRAME LINE A & D		
MARK	PART	LENGTH
DJ-1	8X25C14	7'-3 1/2"
DJ-3	8X25C14	5'-2 1/4"
DJ-4	8X25C14	12'-3 1/2"
DJ-5	8X35C14	17'-5 3/4"
DJ-6	8X35C14	17'-5 3/4"
DJ-7	8X25C14	17'-5 3/4"
DH-2	8X25C14	3'-0"
DH-3	8X25C14	9'-11 1/2"
DH-4	8X35C14	11'-11 1/2"
DH-5	8X35C14	11'-11 1/2"
DH-6	8X25C14	3'-3 1/2"
DS-1	8X25C14	3'-0"
E-1	8E214L3	25'-11 1/2"
E-2	8E214L3	26'-3 1/2"
E-3	8E214L3	25'-11 1/2"
E-5	8E214L3	1'-1 1/2"
G-4	8X25C12	26'-3 1/2"
G-5	8X35C12	7'-9"
G-6	8X35C12	25'-11 1/2"
G-7	8X25C16	6'-8"
G-8	8X25C16	7'-0"
G-9	8X25C16	7'-0"
G-10	8X25C16	6'-8"
G-11	8X25C12	7'-9"
G-12	8X25C12	25'-11 1/2"
G-13	8X25C12	7'-9"

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SCALE: N.T.S. DATE: 9-6-21
DRAWN BY: R WALKER SALES: BRANDON HARPER
BUILDING SIZE: 40'X80'X18'

REVISION

NO.	DESCRIPTION

TITLE/LOCATION

SHEPS TRUCKING
LAKE CITY, FL.

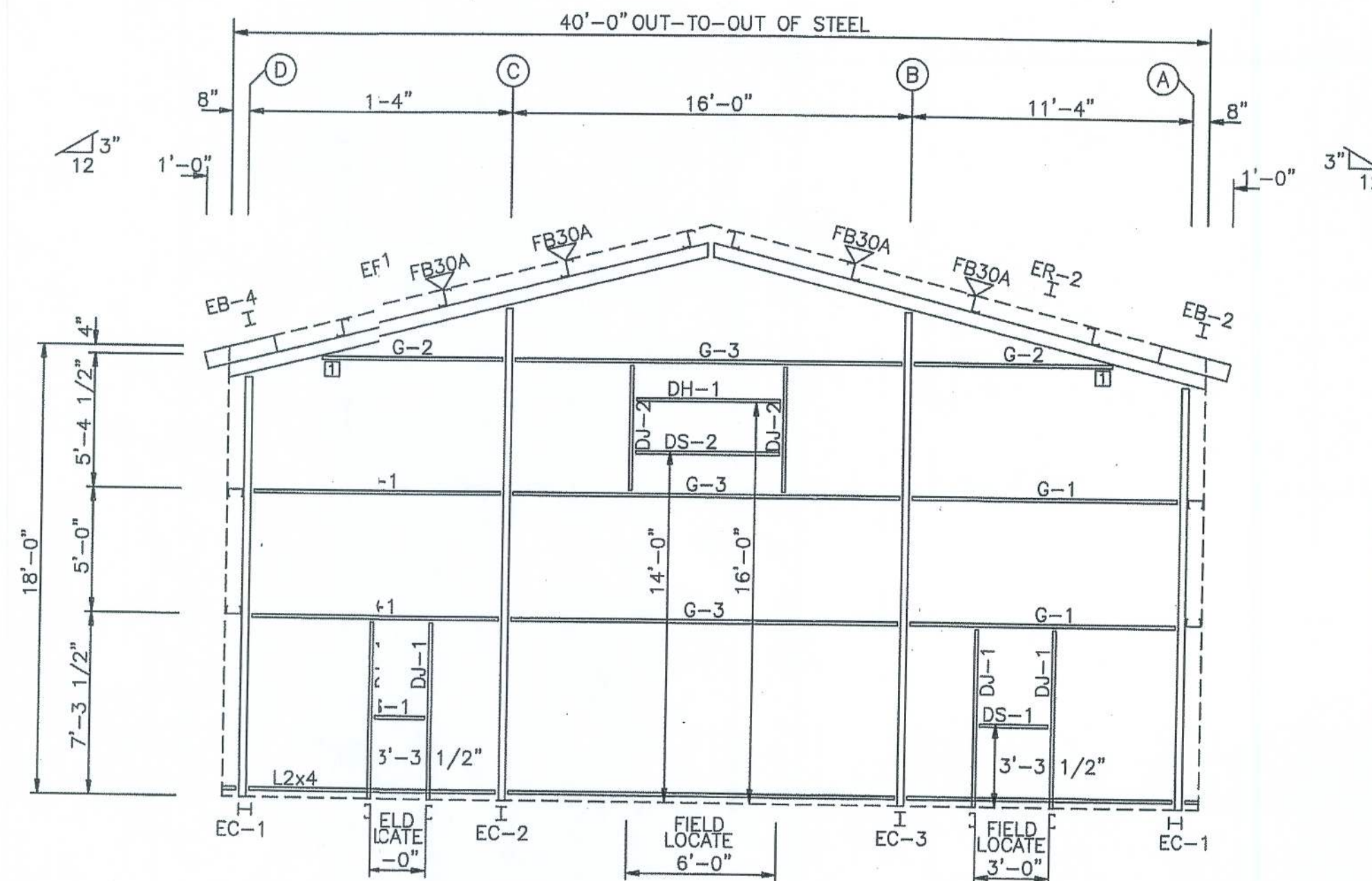
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SIDEWALL FRAMING

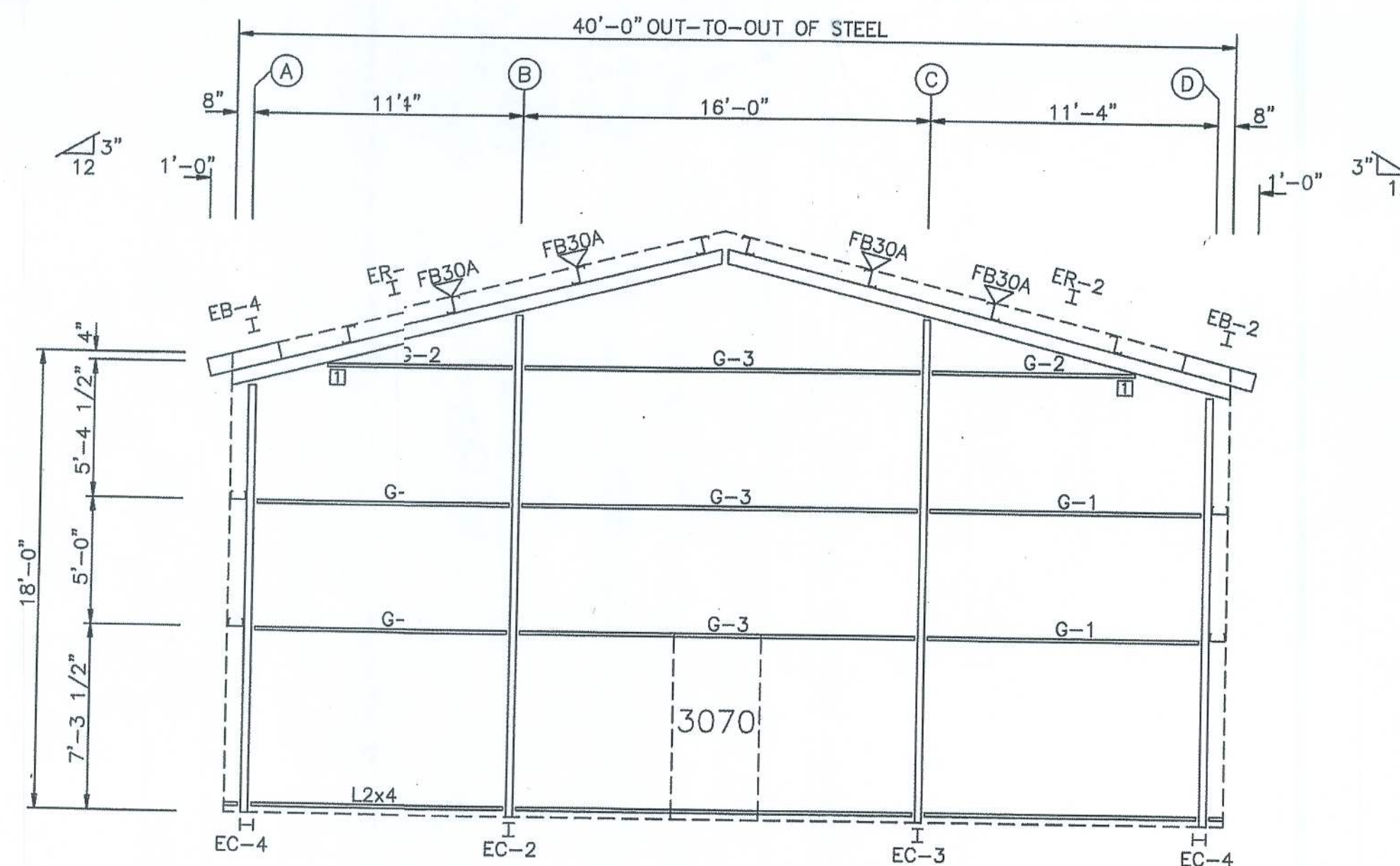
JOB # 11996-SF

SHEET 3 OF 10

10-13-21



ENDWALL FRAMING: FRAME LINE 1



ENDWALL FRAMING: FRAME LINE 4

BOLT TABLE FRAME LINE 1 & 4				
LOCATION	QUAN	TYPE	DIA	LENGTH
Cor Column/Raf	4	A325	3/4"	3"
ER-1/ER-2	8	A325	3/4"	3"
Int Column/Raf	4	A325	1/2"	1 1/2"
ER-1/EB-4	4	A325	3/4"	3"
ER-2/EB-2	4	A325	3/4"	3"

MEMBER TABLE FRAME LINE 1 & 4		
MARK	PART	LENGTH
EB-2	W8X10	1'-8 1/4"
EB-4	W8X10	1'-8 1/4"
EC-1	W8X10	17'-7 3/4"
EC-2	W8X10	19'-8 1/8"
EC-3	W8X10	19'-8 1/8"
EC-4	W8X10	17'-7 3/4"
ER-1	W8X10	19'-5"
ER-2	W8X10	19'-5"
DJ-1	8X25C14	7'-3 1/2"
DJ-2	8X25C14	5'-4 1/2"
DH-1	8X25C14	5'-11 1/2"
DS-1	8X25C14	3'-0"
DS-2	8X25C14	6'-0"
G-1	8X25C16	10'-5 5/8"
G-2	8X25C16	7'-8 1/4"
G-3	8X25C16	15'-7 1/2"

FLANGE BRACE TABLE FRAME LINE 1 & 4		
VID	MARK	LENGTH
1	FB30A	2'-6"

CONNECTION PLATES FRAME LINE 1 & 4	
VID	MARK/PART
1	b1

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P.E. # 54419



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ADEL, GA 31620

SCALE: N.T.S. DATE: 9-6-21
DRAWN BY: R. WALKER SALES: BRANDON HARPER
BUILDING SIZE: 40'X80'X18'

REVISION
DESCRIPTION

TITLE/LOCATION

SHEPS TRUCKING
LAKE CITY, FL.

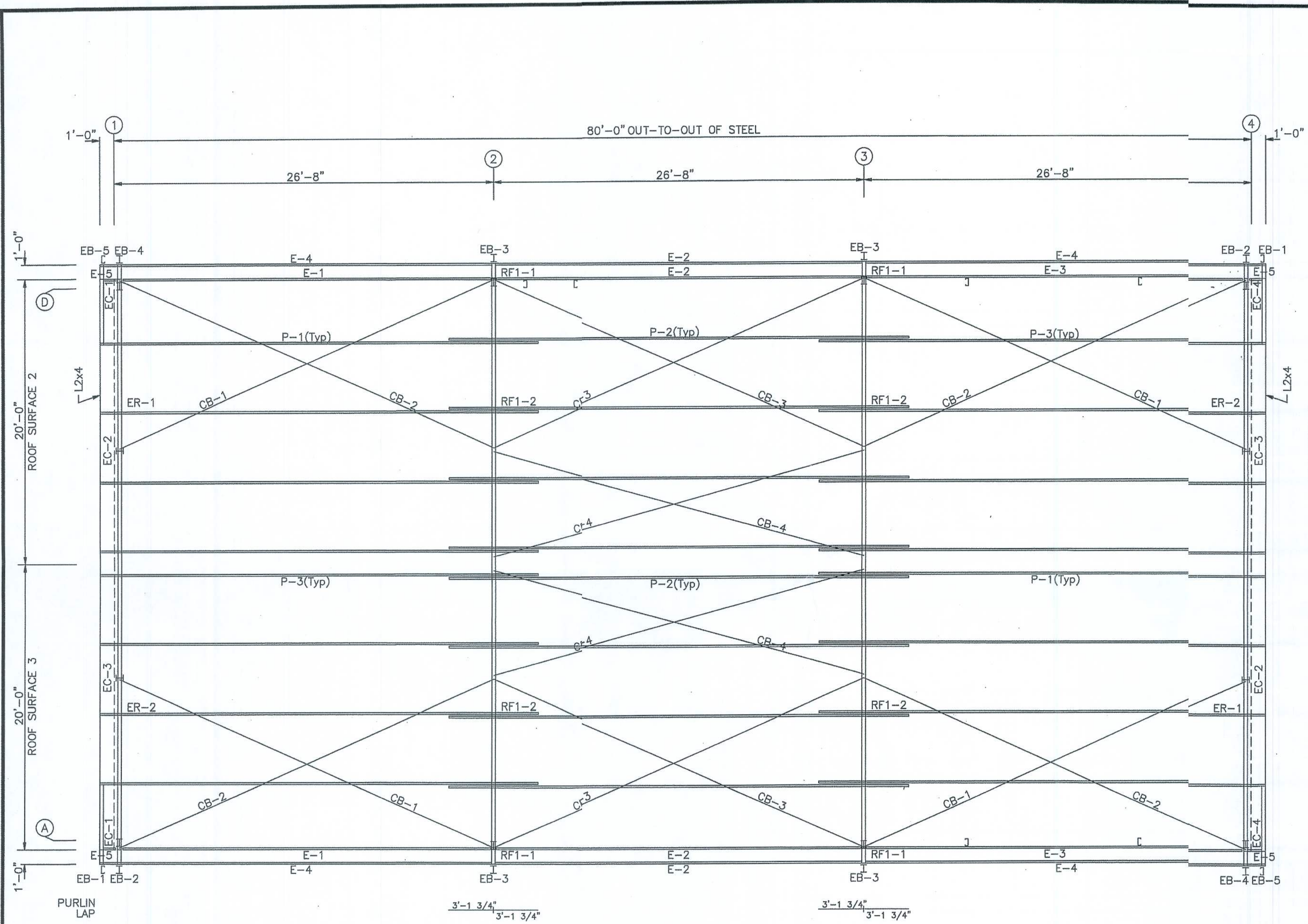
PLAN:

ENDWALL FRAMING

JOB # 11996-EF

SHEET 4 OF 10

10-13-21



EXTENSION/CANOPY BOLTS				
ROOF PLAN				
MARK	QUAN	TYPE	DIA	LENGTH
EB-2	4	A325	1/2"	1 1/2"
EB-3	4	A325	1/2"	1 1/2"
EB-4	4	A325	1/2"	1 1/2"

ANGLE TABLE			
ROOF PLAN			
◇ ID	MARK	LENGTH	DETAIL
	L2x4	20'-0"	ANGLE 36

MEMBER TABLE		
ROOF PLAN		
MARK	PART	LENGTH
EB-1	8X2CH16	5'-0 1/2"
EB-2	W8X10	1'-8 1/4"
EB-3	W8X10	1'-8 1/4"
EB-4	W8X10	1'-8 1/4"
EB-5	8X2CH16	5'-0 1/2"
P-1	8X25Z12	30'-9 1/2"
P-2	8X25Z16	32'-11 1/2"
P-3	8X25Z12	30'-9 1/2"
E-1	8E214L3	25'-11 1/2"
E-2	8E214L3	26'-3 1/2"
E-3	8E214L3	25'-11 1/2"
E-4	8E214L3	27'-5 1/2"
E-5	8E214L3	1'-1 1/2"
CB-1	1/4_CBL	29'-6"
CB-2	1/4_CBL	28'-11"
CB-3	1/4_CBL	29'-10"
CB-4	1/4_CBL	28'-6"

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ADEL, GA 31620

SCALE: N.T.S. DATE: 9-6-21
DRAWN BY: R WALKER SALES: BRANDON HAER
BUILDING SIZE: 40'X80'X18'

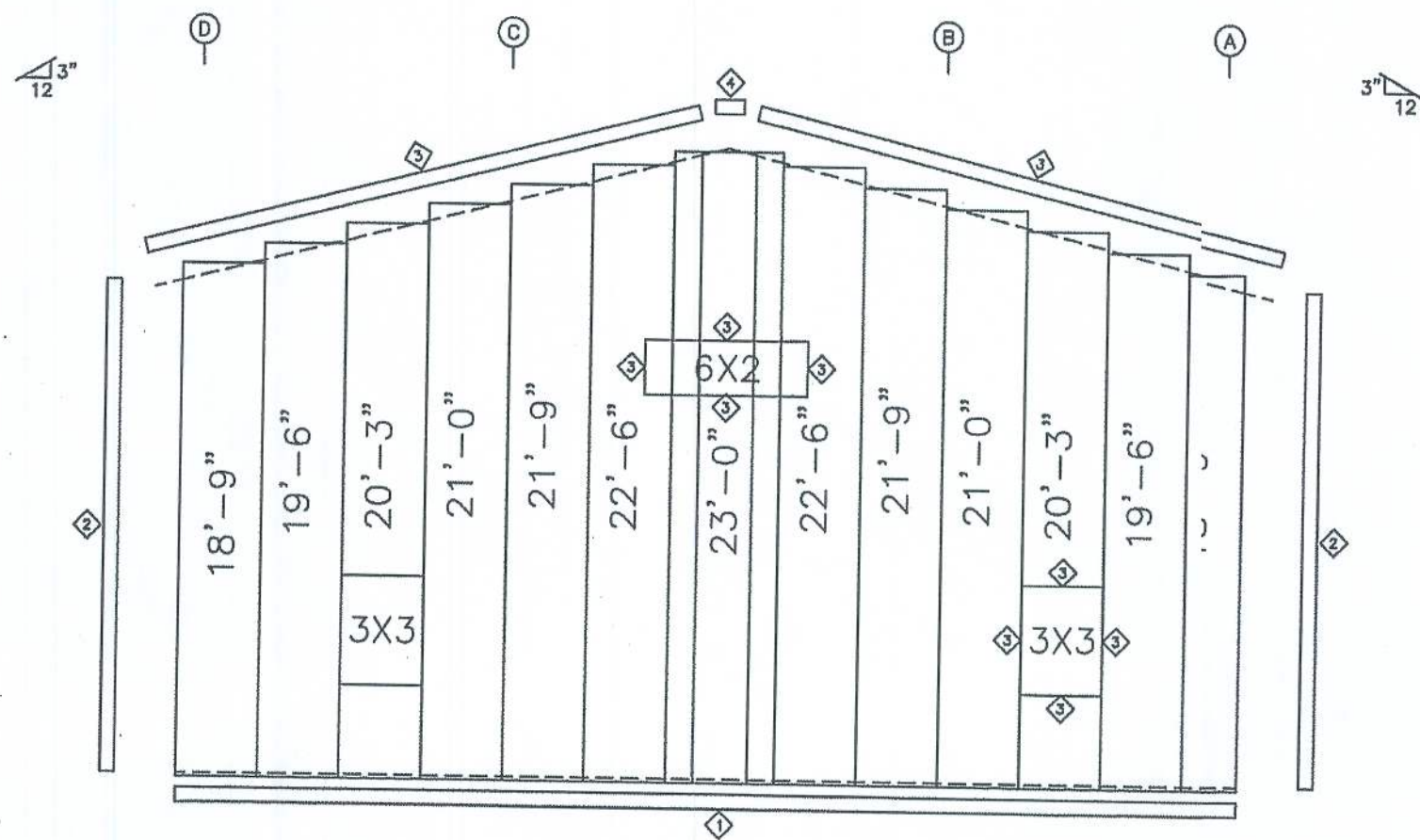
REVISION	
DESCRIPTION	DATE

TITLE/LOCATION
SHEPS TRUCKING
LAKE CITY, FL.

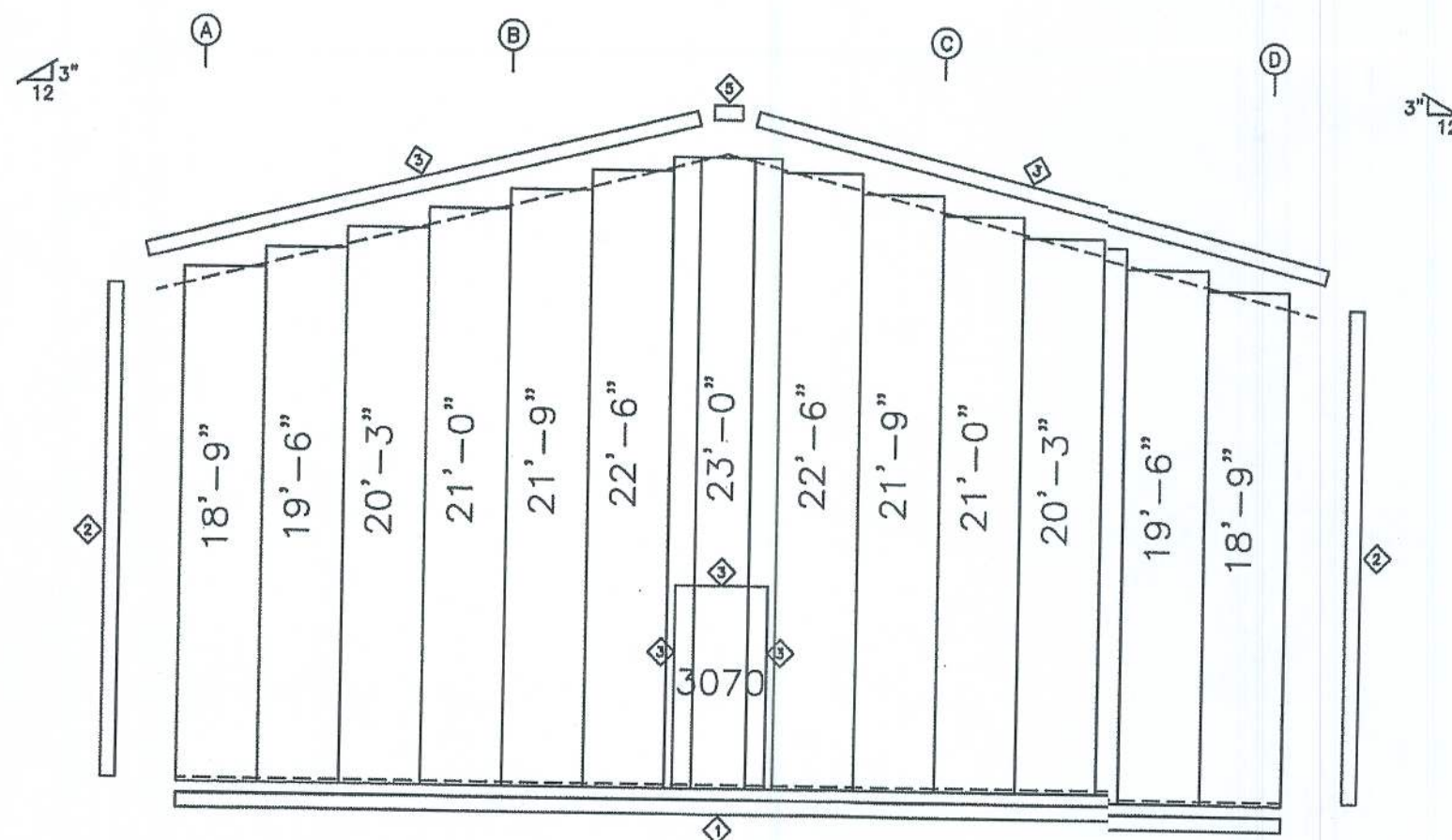
PLAN:
ROOF FRAMMING

JOB # 11996-RF
SHEET 5 OF 10

10-13-21

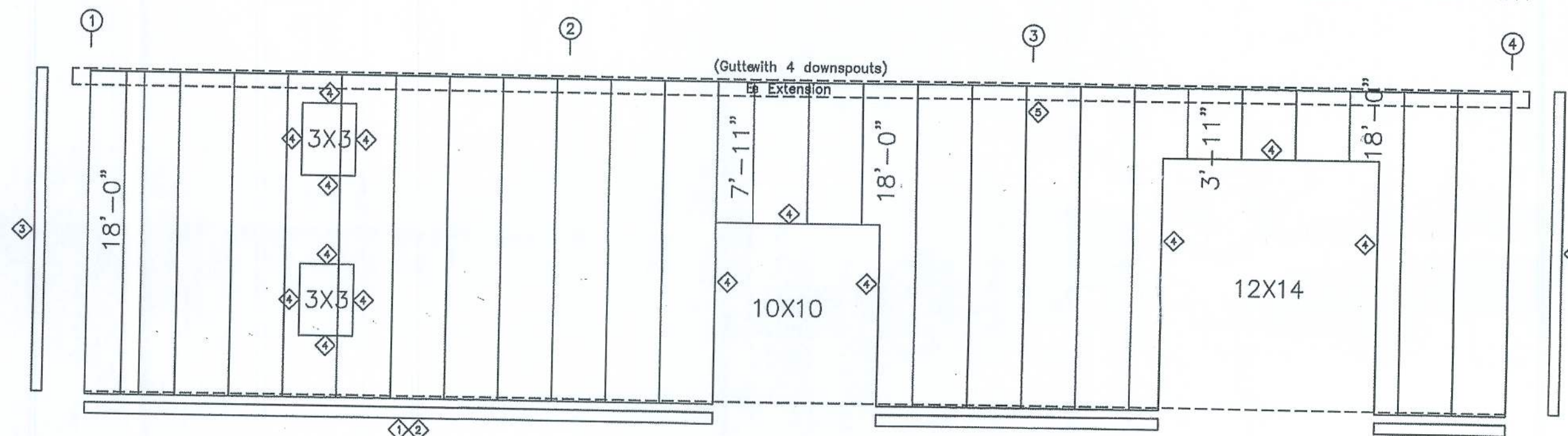


ENDWALL SHEETING & TRIM: FRAME LINE 1
PANELS: 26 Ga. PBR - NEED COLOR



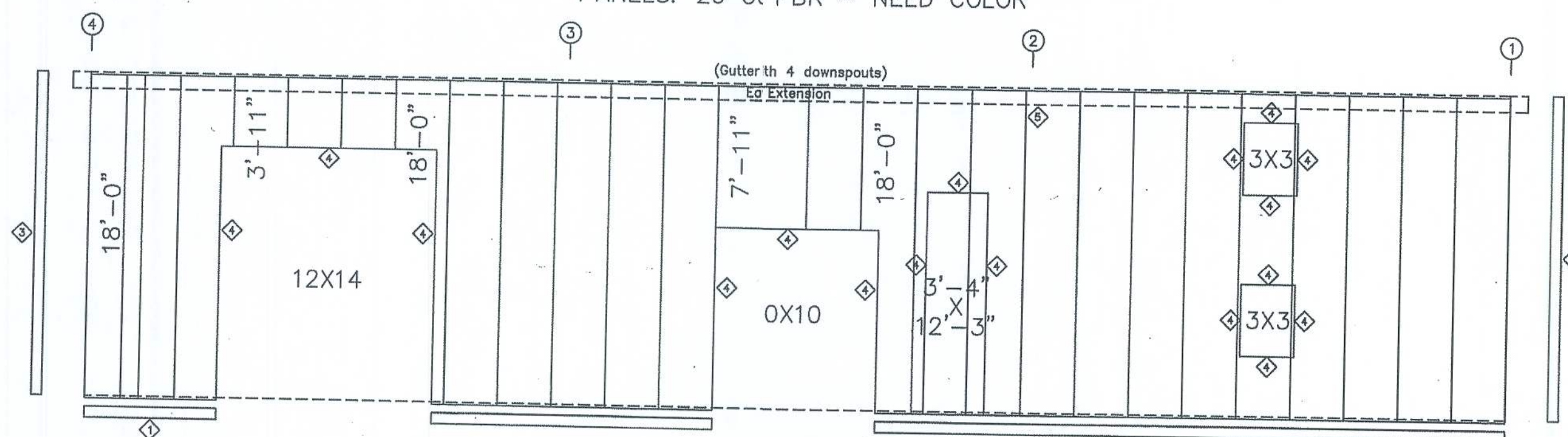
ENDWALL SHEETING & TRIM: FRAME LINE 4
PANELS: 26 Ga. PBR - NEED COLOR

TRIM TABLE FRAME LINE 1 & 4		
ID	PART	DETAIL
1	E-108	TRIM_43
2	FL-830	TRIM_47
3	FL-26	TRIM_49
4	E-149	TRIM_28
5	E-149	TRIM_28



SIDEWALL SHEETING & TRIM: FRAME LINE A
PANELS: 26 Ga. PBR - NEED COLOR

TRIM TABLE FRAME LINE A & D		
ID	PART	DETAIL
11	E-108	TRIM_43
22	E-108	TRIM_43
33	FL-830	TRIM_47
44	FL-26	TRIM_49
55	E-134	TRIM_26



SIDEWALL SHEETING & TRIM: FRAME LINE D
PANELS: 26 Ga. PBR - NEED COLOR

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ADEL, GA 31620

SCALE: N.T.S. DATE: 9-6-21
DRAWN BY: R. WALKER SALES: BRANDON HAER
BUILDING SIZE: 40'X80'X18'

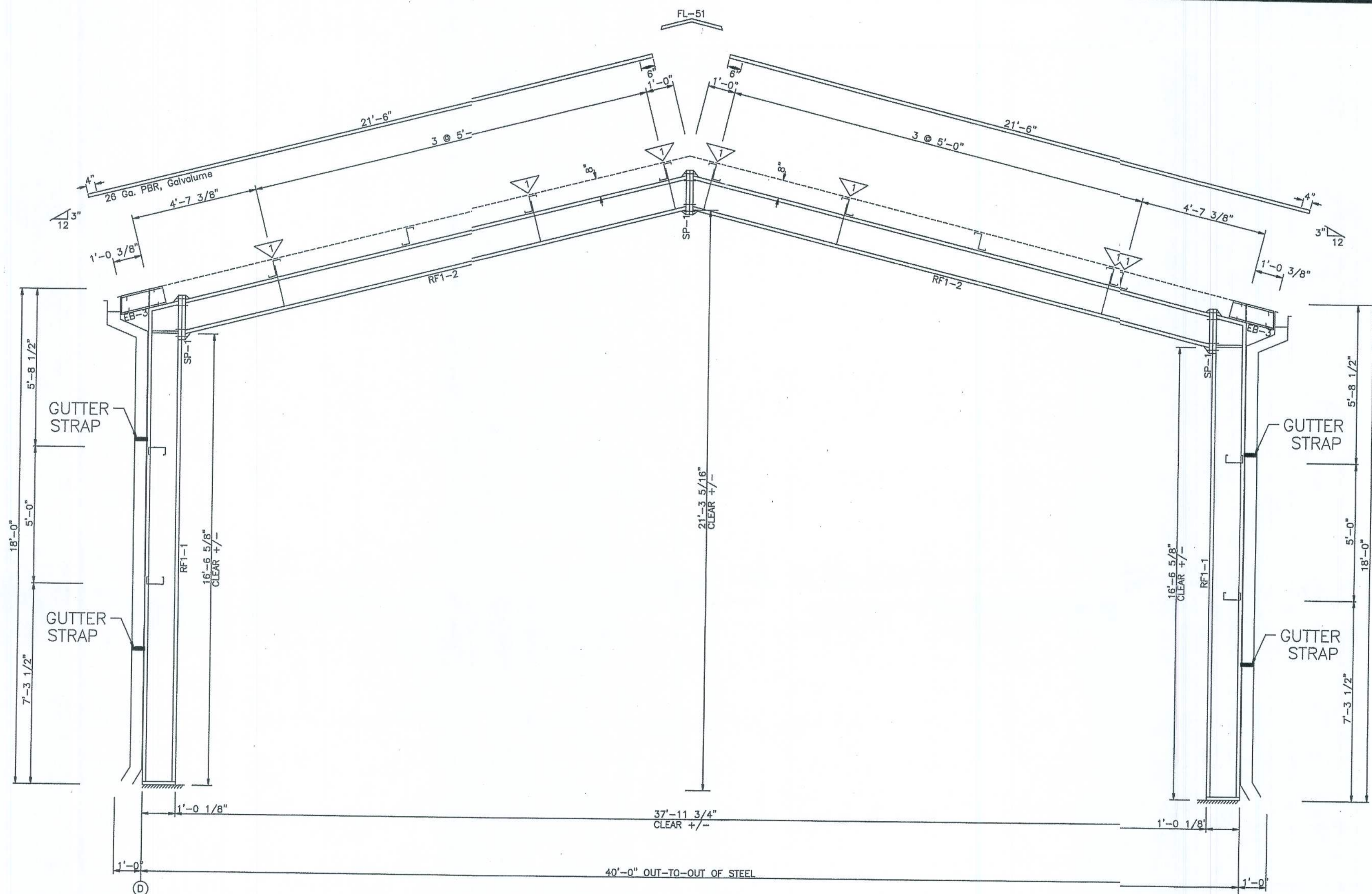
REVISION	
DATE	DESCRIPTION

TITLE/LOCATION
SHEPS TRUCKING
LAKE CITY, FL.

PLAN:
WALL SHEETING

JOB # 11996-WS
SHEET 6 OF 10

10-13-21



RIGID FRAME ELEVATION: FRAME LINE 2 3

MEMBER TABLE						
Mark	Weight	Web Depth Start/End	Web Pl. Thick	Leith	Outside Flange W x Thk x Length	Inside Flange W x Thk x Length
RF1-1	467	11.5/11.5	0.188	70	8 x 5/16" x 120.0	8 x 5/16" x 120.0
RF1-2	370	11.5/11.5	0.188	110	8 x 1/4" x 86.5	8 x 1/4" x 74.0
EB-3	25	W12X16	0.250	15	8 x 1/4" x 12.1	
		W8X10				

NOTE: NO ADDITIONAL LOADS APPLIED
FUTURE ADDITIONS SHALL BE
SELF SUPPORTING

FLANGE BRACE TABLE FRAME LINE 2 3						
∇ ID	SIDES	MARK	LENGTH	OFF. T	DETAIL	CLIP2
1	1	FB31A	2'-7"	2'-		

SPICE BOLT TABLE						
Mark	Qty	Top	Bot	Int	Type	Dia Length
SP-1	4	4	0	A325	0.750	3.00

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10-13-21

GENERAL NOTES:
BOLT TIGHTENING - ALL BOLTED JOINTS WITH A325-08 TYPE 1 BOLTS ARE SPECIFIED AS SNUG-TIGHTENED JOINTS IN ACCORDANCE WITH THE SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS, JUNE 30, 2004, PRETENSIONING METHODS, INCLUDING TURN-OF-NUT, CALIBRATED WRENCH, TWIST OFF TYPE TENSION CONTROL. BOLTS OR DIRECT TENSION INDICATOR ARE NOT REQUIRED, INSTALLATION INSPECTION REQUIREMENTS FOR SNUG TIGHT (SPECIFICATION FOR STRUCTURAL JOINTS 9.1) IS SUGGESTED.

REFERENCE: STEEL CONSTRUCTION MANUAL
AMERICAN INSTITUTE OF STEEL CONSTRUCTION INC.
THIRTEENTH EDITION (AISC)



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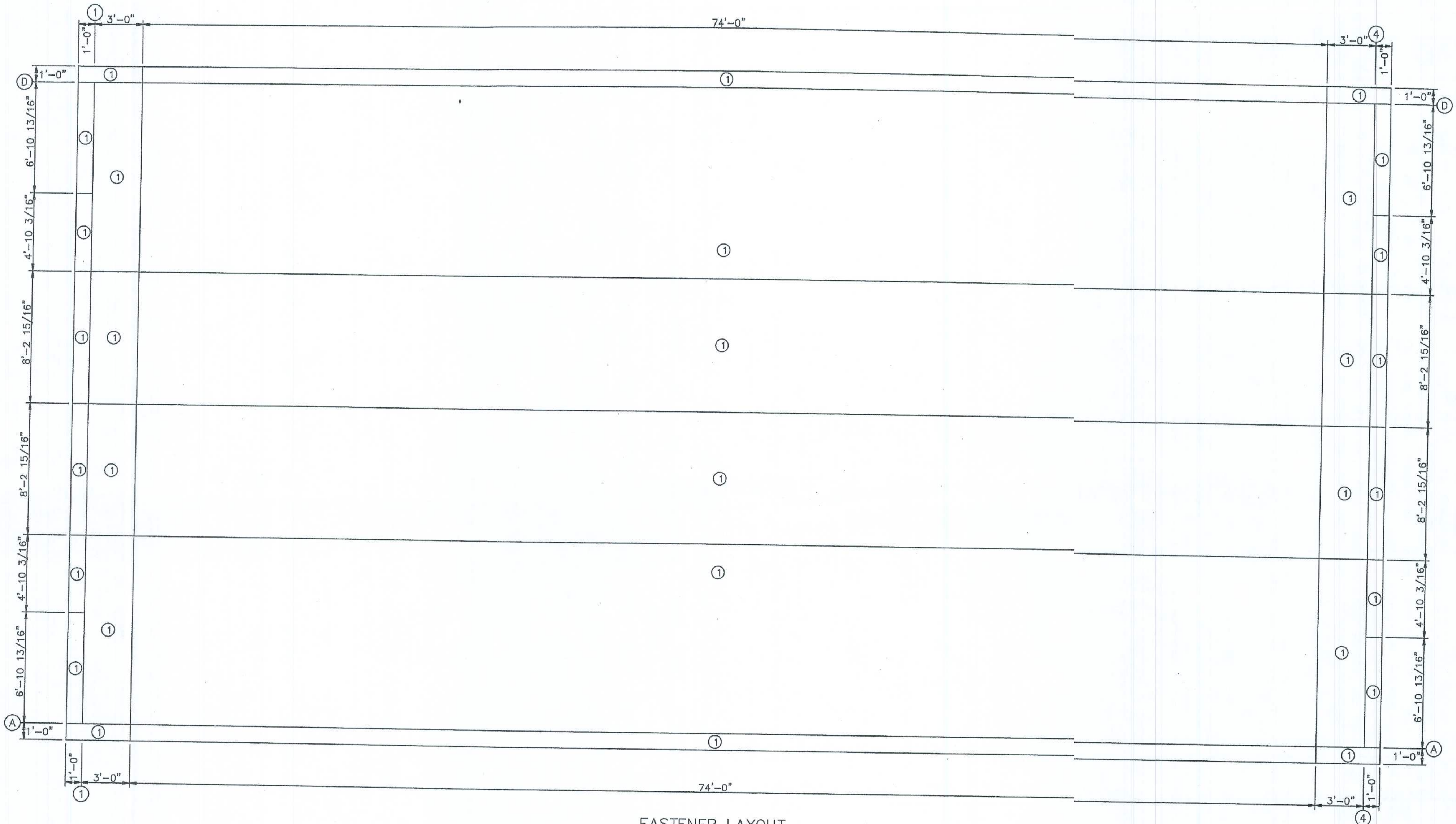
SCALE: N.T.S. DATE: 9-8-21
DRAWN BY: R WALKER SALES: BRANDON HARP
BUILDING SIZE: 40'X80'X18'

REVISION	
DATE	DESCRIPTION

TITLE/LOCATION
SHEPS TRUCKING
LAKE CITY, FL.

PLAN:
CROSS SECTION

JOB # 11996-XS
SHEET 8 OF 10



FASTENER LAYOUT
(Minimum Fastener Spacing)

FASTENER TABLE		
○ ID	SCREW PART	SPACE (in)
1	12x1.25	12.0

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SCALE: N.T.S. DATE: 9-6-21
DRAWN BY: R WALKER SALES: BRANDON HPER
BUILDING SIZE: 40'X80'X18'

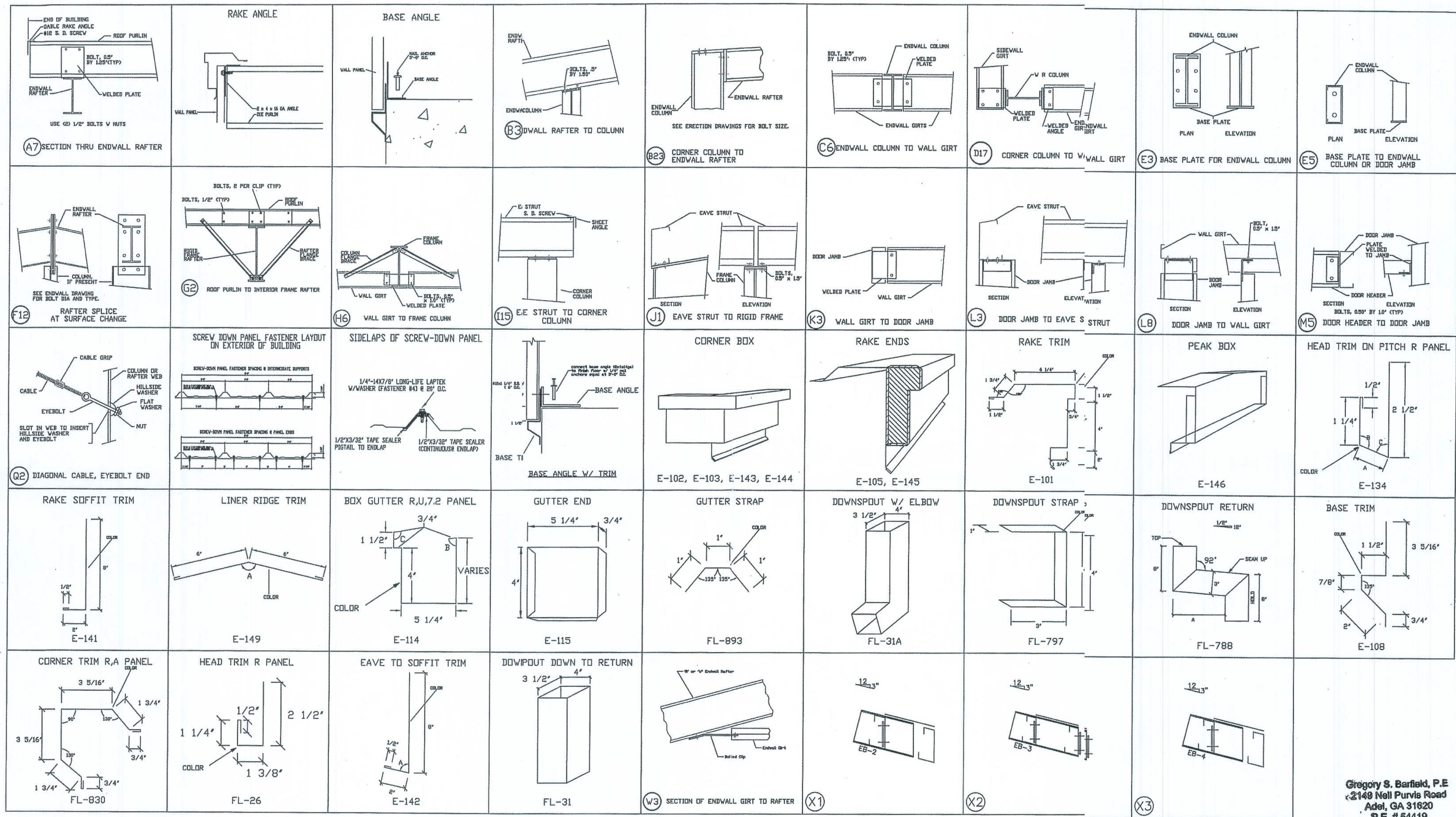
REVISION	
DATE	DESCRIPTION

TITLE/LOCATION
SHEPS TRUCKING
LAKE CITY, FL.

PLAN:
FASTENER LAYOUT

JOB # 11996-FL
SHEET 9 OF 10

10-13-21



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P.E. # 54419



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ADEL, GA 31620

SCALE: N.T.S. DATE: 9-6-21
DRAWN BY: R. WALKER SALES: BRANDON HARP
BUILDING SIZE: 40'X80'X18'

REVISION
DESCRIPTION

TITLE/LOCATION

SHEPS TRUCKING
LAKE CITY, FL.

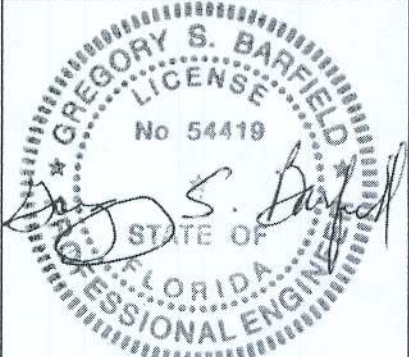
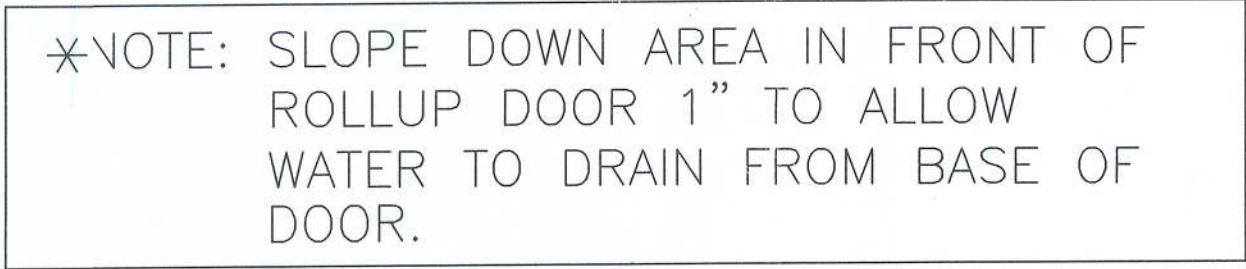
PLAN:

DETAIL PAGE

JOB # 11996-DP

SHEET 10 OF 10

10-13-21

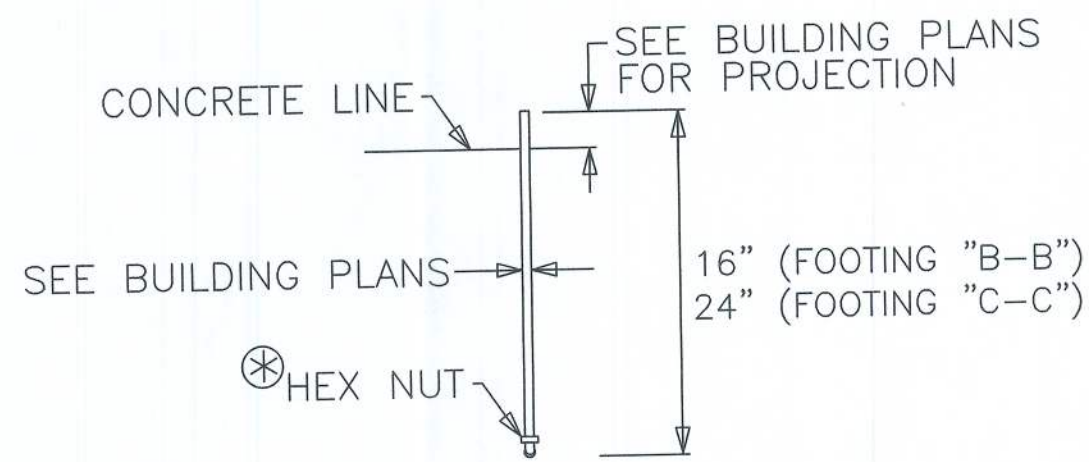


FOUNDATION PLANS		NO.	REVISION	DATE
SHEPS TRUCKING LIKE CITY, FL.				
JOB #: GSB-1968				

GREGORY S. BARFIELD, PE

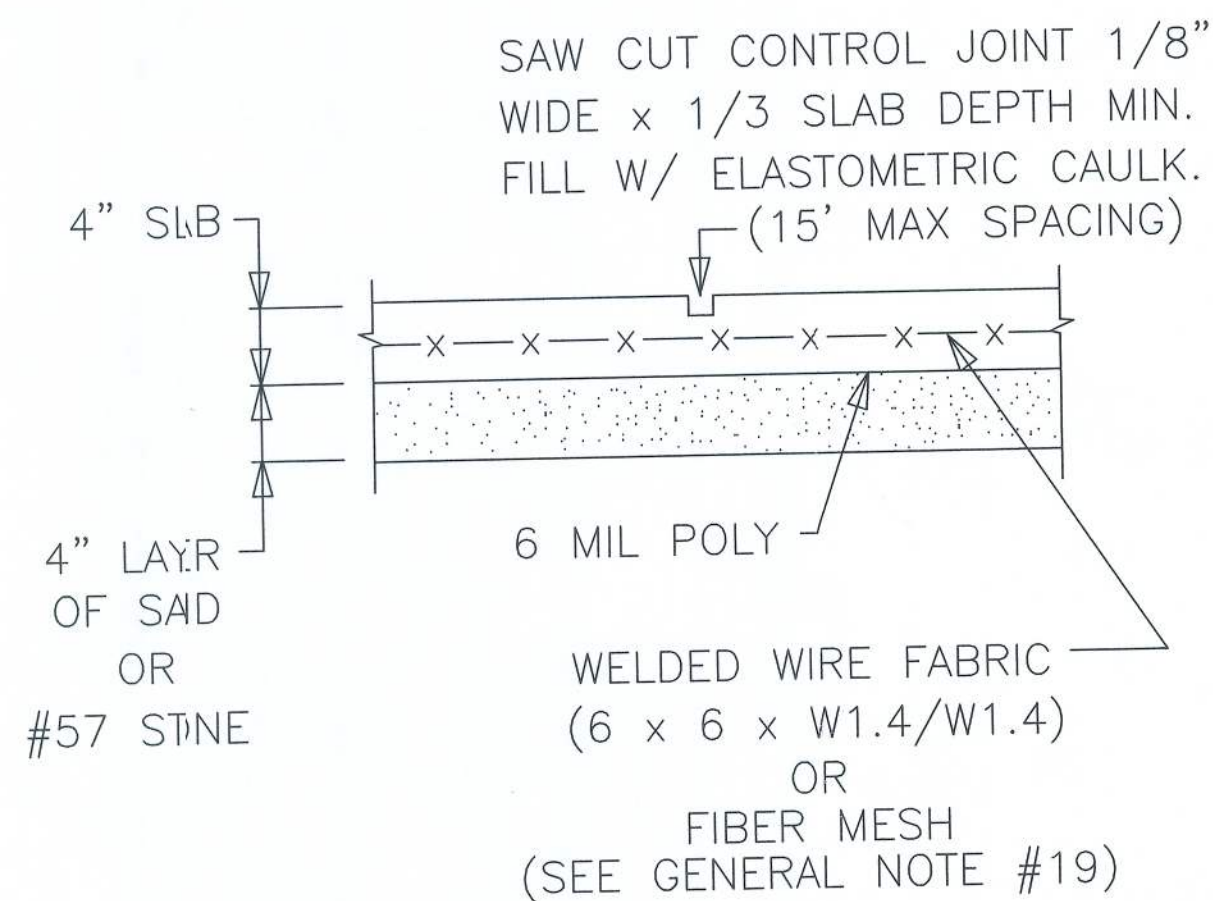


2149 NELL PURVIS ROAD
ADEL, GEORGIA 31620

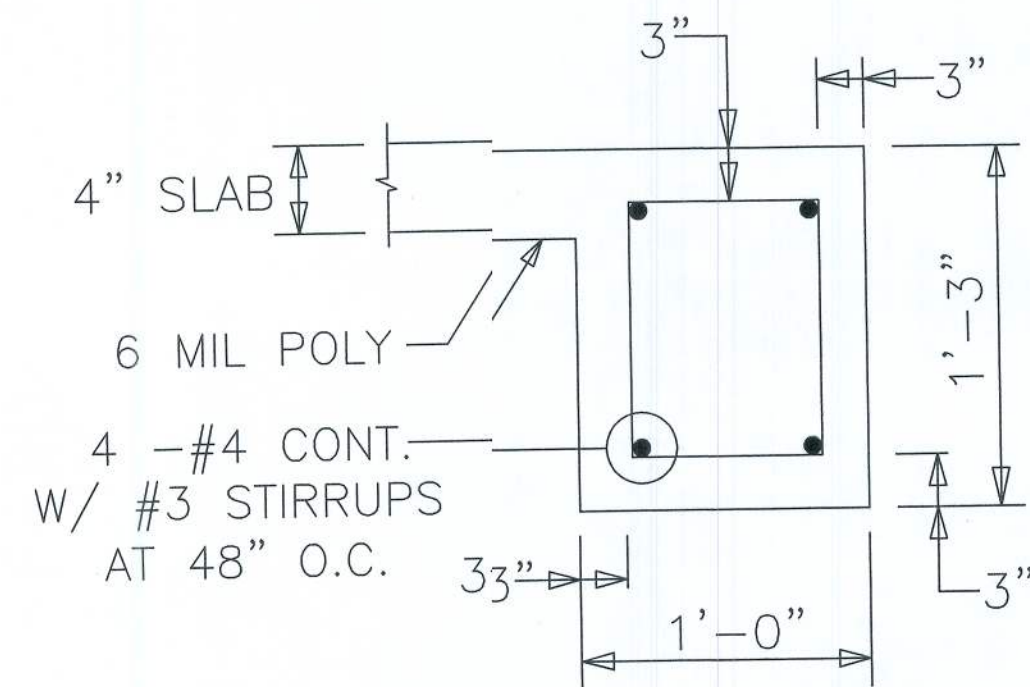


ANCHOR BOLTS SHALL BE ASTM F1554 GR36

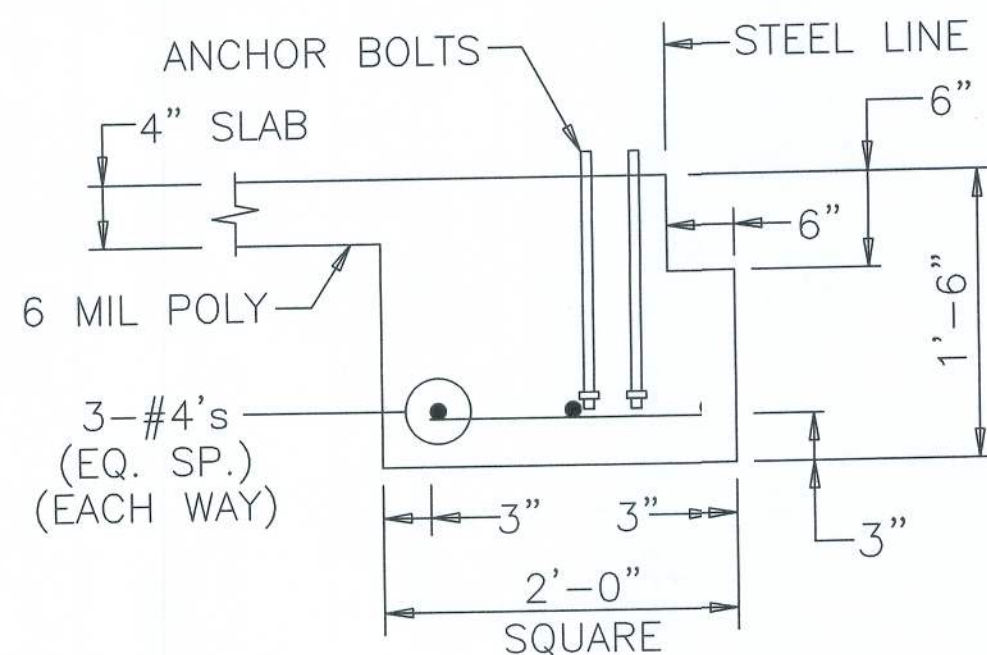
ANCHOR BOLTS
DOOR JAMB ANCHOR BOLTS BY OTHERS



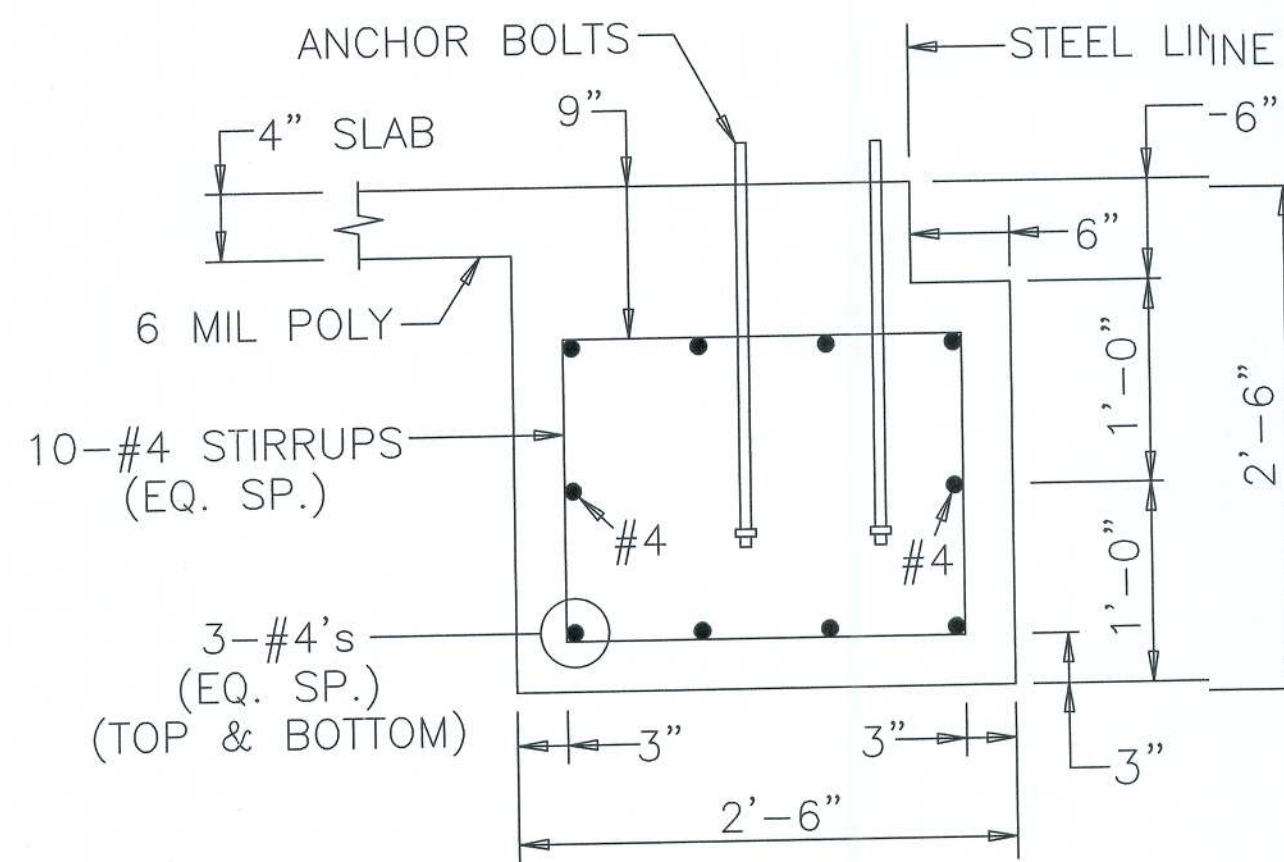
SLAB DETAIL



SECTION A - A



SECTION E - B



SECTION C - C

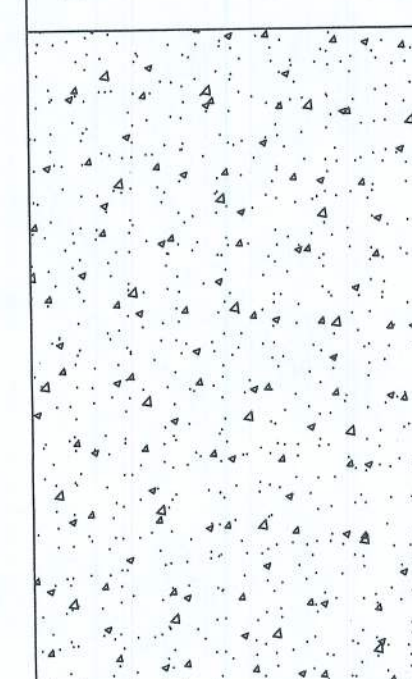
NO.	REVISION	DATE



FOUNDATION PLANS
SHEPS TRUCKING
LIKE CITY, FL.
JOB #: GSB-1968

GREGORY S. BARFIELD, PE

2149 NELL PURVIS ROAD
ADEL, GEORGIA 31620



GREGORY S. BARFIELD
LICENSE
No 54419
STATE OF
FLORIDA
PROFESSIONAL ENGINEER
10-20-21
SHEET NO.
2 of 3

1. All foundation work and materials shall be in accordance with the FBC 20.

2. Prior to construction all vegetation, stumps, roots, foreign material, and surficial topsoil shall be removed from the area under the foundation and to a minimum distance of 5 feet beyond the limits of the proposed building. After this stripping and clearing has been completed the exposed natural soils shall be compacted to 95% of Modified Proctor in accordance with ASTM D 1557. At a distance of 50 feet or more from an existing structure this compaction shall be achieved by use of a vibratory drum roller in which the drum weighs a minimum of 5 tons with a minimum drum diameter of 3 feet. At a distance of with 50 feet from an existing structure this compaction shall be achieved by use of a light weight vibratory drum or sled compactor having a maximum weight of 300 pounds.

3. Groundwater levels shall be controlled to a minimum of 2 feet below the construction level. Groundwater elevations may fluctuate during construction therefore temporary dewatering may be necessary to control the groundwater levels.

4. All fill material shall be placed in lifts not to exceed 8 inches and shall be compacted to 95% of Modified Proctor in accordance with ASTM D 1557.

5. If "pumping" of the near surface soils or fill material occurs during construction which results in strength loss of the subsequent soil, work shall be terminated in these areas and the disturbed soils removed. After removal of these soils, fill material which has a water content of not more than 10% shall be replaced and compacted. In lieu of removing the disturbed soils the excess moisture may be allowed to dissipate and the soil re-compacted.

6. When the fill material has been placed and properly compacted a "smoothed" bucket backhoe may be used to excavate to the planned foundation levels. After this excavation all soils at the bearing level shall be compacted to 95% of Modified Proctor in accordance with ASTM D 1557 to a depth of at least 1 foot. If backfilling is required in the footing excavations the fill shall be placed in lifts not to exceed 6 inches.

7. All fill material shall consist of soil with no more than 10% of the particles passing a #200 sieve and shall be free of vegetation, organic material, construction debris, large rocks, and all foreign material.

8. All footings have been designed for the following assumed soil properties:

Bearing Capacity = 2,000 psf
Angle of Internal Friction (ϕ angle) = 28 degrees
Coefficient of Friction = 0.45
Soil Weight = 110 lbs per cubic ft.

9. If it is determined after the soils survey that the actual soil properties are different than these assumed values, the contractor shall follow the recommendations of the geotechnical engineer.

9. The outlying perimeter areas of the proposed building shall be graded in such a way as to provide positive drainage away from the proposed building.
10. A vapor retarder shall be installed underneath the slab consisting of 6 mil minimum polyethylene with joints lapped not less than 6 inches and sealed.
11. All concrete shall have a minimum compressive strength of 3,000 psi at 28 days.
12. All concrete shall contain 2.5% to 6% entrained air to enhance frost resistance.
13. The maximum water-cement ration of the concrete shall be 0.55.
14. The slump limits of all concrete shall be 2 - 4 inches.
15. All concrete shall be mixed until there is a uniform distribution of materials in accordance with ACI 318.
16. All reinforcing bars that do not require welding shall conform to ASTM-615, Grade 60. All reinforcing bars that are to be welded shall conform to ASTM A706, Grade 60. Welded wire fabric shall conform to ASTM A-185.
17. The #4 rebar in the slab turn down shall be continuous for the entire perimeter of the foundation and shall be lap spliced a minimum of 2 feet at terminal points in order to maintain continuity.
18. The slab reinforcing including the welded wire fabric and hairpins shall not be cut during or anytime after construction since this reinforcement provides structural stability for the building.
19. Fiber reinforced concrete is an acceptable alternative to welded-wire fabric. Synthetic fibers shall be fibermesh 650 as manufactured by Propex Concrete Systems applied at the rate of 3 pounds per cubic yard of concrete (or another equal system).
20. Since the passive resistance of the soil is an integral part of the ability of the foundation to resist the horizontal forces that will be present when the design loads are applied to the building system, it is expressly forbidden for any future excavation to take place within 50 feet of the building without the consent of the Engineer of Record.
21. Control joints shall be installed in the foundation at intervals not to exceed 15 feet.
22. Maintain 3 inches minimum clearance for all rebar and anchor bolts, unless otherwise noted.

GREGORY S. BARFIELD, PE		FOUNDATION PLANS	
		SHEPS TRUCKING	
		LIKE CITY, FL.	
		JOB #: GSB-1968	
2149 NELL PURVIS ROAD ADEL, GEORGIA 31620			
10-20-21			
SHEET NO.			
3 of 3			
		NO.	
		REVISION	
		DATE	