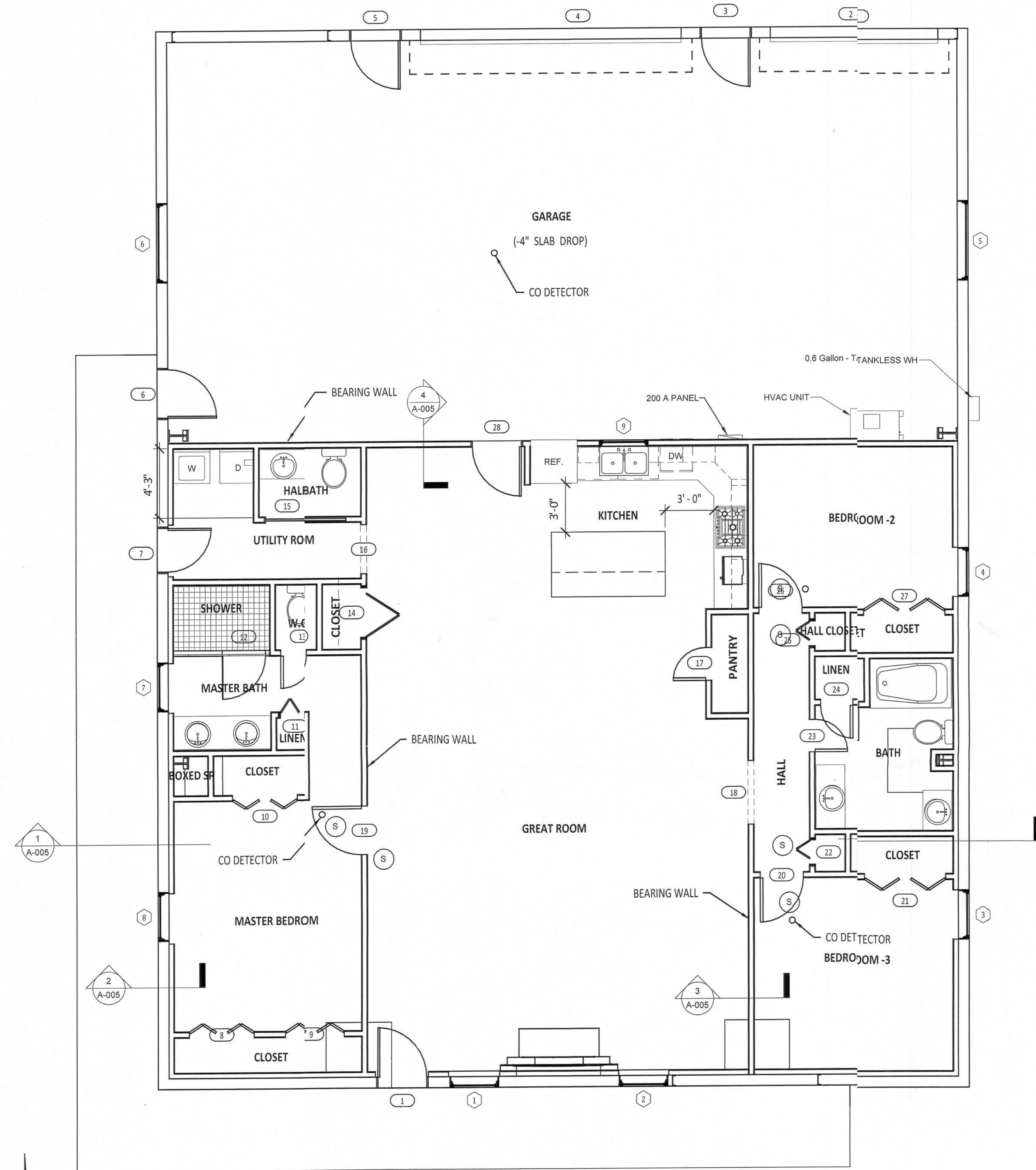




Mark	Width	Height	Finish	Description
1	3'-0"	6'-8"		ENTRY
2	10'-0"	8'-0"		ROLLUP DOOR
3	3'-0"	6'-8"		ENTRY
4	16'-0"	8'-0"		ROLLUP DOOR
5	3'-0"	6'-8"		ENTRY
6	3'-0"	6'-8"		ENTRY
7	2'-8"	6'-8"		ENTRY
8	4'-0"	6'-8"		BIFOLD DOOR
9	4'-0"	6'-8"		BIFOLD DOOR
10	4'-0"	6'-8"		BIFOLD DOOR
11	1'-6"	6'-8"		BIFOLD DOOR
12	2'-8"	6'-8"		SHOWER DOOR
13	2'-0"	6'-8"		INTERIOR
14	3'-0"	6'-7"		BIFOLD DOOR
15	2'-8"	6'-8"		POCKET DOOR
16	0'-0"	0'-0"		CASED OPENING
17	2'-0"	6'-8"		INTERIOR
18	0'-0"	0'-0"		CASED OPENING
19	3'-0"	6'-8"		INTERIOR
20	2'-8"	6'-8"		INTERIOR
21	5'-0"	6'-8"		BIFOLD DOOR
22	1'-6"	6'-8"		BIFOLD DOOR
23	2'-0"	6'-8"		INTERIOR
24	2'-0"	6'-8"		INTERIOR
25	1'-6"	6'-8"		BIFOLD DOOR
26	2'-8"	6'-8"		INTERIOR
27	5'-0"	6'-8"		BIFOLD DOOR
28	3'-0"	6'-8"		ENTRY

ROOM NAME	AREA	WALL FINISH	WALL FINISH CLASS	BASE FINISH	FLOOR FINISH	CEILING FINISH	CEILING FINISH CLASS
GREAT ROOM	666 SF	GWB	B	PER OWNER	CARPET	GWB	B
CLOSET	25 SF	GWB	B	PER OWNER	CARPET	GWB	B
MASTER BEDROOM	198 SF	GWB	B	PER OWNER	CARPET	GWB	B
CLOSET	14 SF	GWB	B	PER OWNER	CARPET	GWB	B
MASTER BATH	45 SF	GWB	B	PER OWNER	TILE	GWB	B
CLOSET	9 SF	GWB	B	PER OWNER	CARPET	GWB	B
UTILITY ROOM	64 SF	GWB	B	PER OWNER	TILE	GWB	B
KITCHEN	210 SF	GWB	B	PER OWNER	TILE	GWB	B
CLOSET	13 SF	GWB	B	PER OWNER	CARPET	GWB	B
BEDROOM-3	140 SF	GWB	B	PER OWNER	CARPET	GWB	B
CLOSET	13 SF	GWB	B	PER OWNER	CARPET	GWB	B
BEDROOM-2	119 SF	GWB	B	PER OWNER	CARPET	GWB	B
GARAGE	1207 SF	NONE	NONE	NONE	CONCRETE	NONE	NONE
HALF BATH	28 SF	GWB	B	PER OWNER	TILE	GWB	B
SHOWER	23 SF	TILE		PER OWNER	TILE	GWB	B
W.C.	11 SF	GWB	B	PER OWNER	TILE	GWB	B
LINEN	4 SF	GWB	B	PER OWNER	CARPET	GWB	B
BOXED SP	5 SF	NONE	NONE	NONE	CONCRETE	NONE	B
PANTRY	13 SF	GWB	B	PER OWNER	TILE	GWB	B
BATH	75 SF	GWB	B	PER OWNER	TILE	GWB	B
LINEN	7 SF	GWB	B	PER OWNER	CARPET	GWB	B
HALL CLOSET	4 SF	GWB	B	PER OWNER	CARPET	GWB	B
HALL	54 SF	GWB	B	PER OWNER	CARPET	GWB	B

WINDOW SCHEDULE			
MARK	ROUGH WIDTH	ROUGH HEIGHT	DESCRIPTION
1	3'-0 3/4"	5'-0 3/4"	Double Hung - Fixed Unit
2	3'-0 3/4"	5'-0 3/4"	Double Hung - Fixed Unit
3	3'-0 3/4"	5'-0 3/4"	Double Hung - Fixed Unit
4	3'-0 3/4"	5'-0 3/4"	Double Hung - Fixed Unit
5	4'-11 3/4"	5'-0 3/4"	Double Hung - Fixed Unit
6	4'-11 3/4"	5'-0 3/4"	Double Hung - Fixed Unit
7	3'-0 3/4"	1'-5 3/4"	Double Hung - Fixed Unit
8	3'-0 3/4"	5'-0 3/4"	Double Hung - Fixed Unit
9	3'-0 3/4"	3'-0 3/4"	Double Hung - Fixed Unit

TOTAL BUILDING AREA = 3,250 S.F.
HEATED AREA = 1,975 S.F.

ENGINEERING SCOPE IS LIMITED TO WIND LOAD
CALCULATION & STRUCTURAL ELEMENTS AND
COMPONENTS

1 FLOOR PLAN
1/4" = 1'-0"

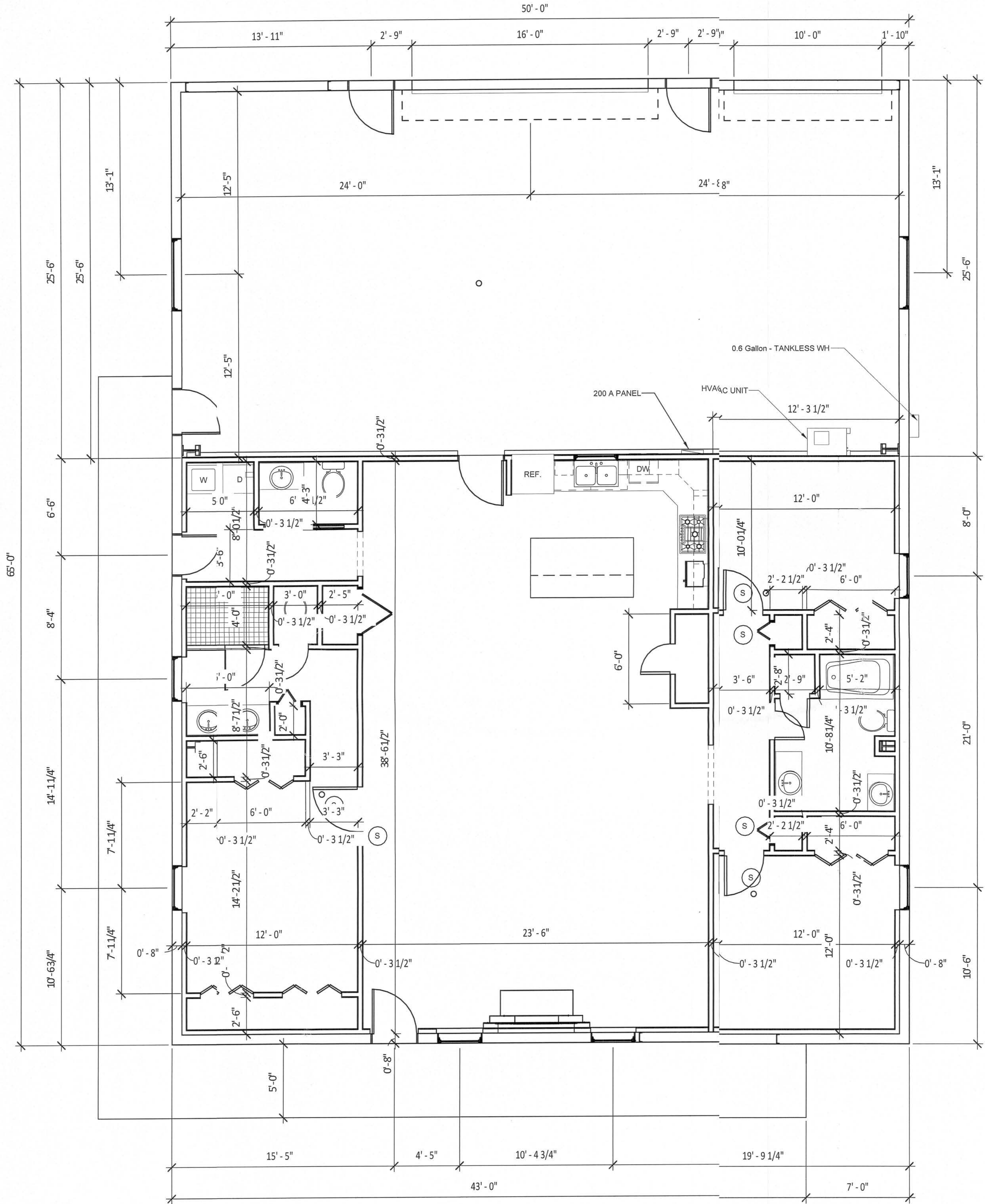


1/22/2020 3:19:36 PM
email: ggill@gillengineering.com



GRAHAM RESIDENCE COLUMBIA COUNTY, FL	
COLUMBIA CONSTRUCTION & MAINTENANCE, INC.	GILL ENGINEERING SERVICES, INC. AUTH # 30824 GARY GILL PE #51942 426 SW COMMERCIAL DR 130-M LAKE CITY, FL 32025 386-590-1242
DRAWN BY: GG	
CHKD BY: GG	
APPRD BY: GG	
FLOOR PLAN	
1945-063	DWG #: A-001
REV #:	

Revision Schedule		
Revision Number	Revision Description	Revision Date



TOTAL BUILDING AREA = 3,250 S.F.
HEATED AREA = 1,975 S.F.

ENGINEERING SCOPE IS LIMITED TO WIND LOAD
CALCULATION & STRUCTURAL ELEMENTS AND
COMPONENTS

① DIMENSION PLAN
1/4" = 1'-0"

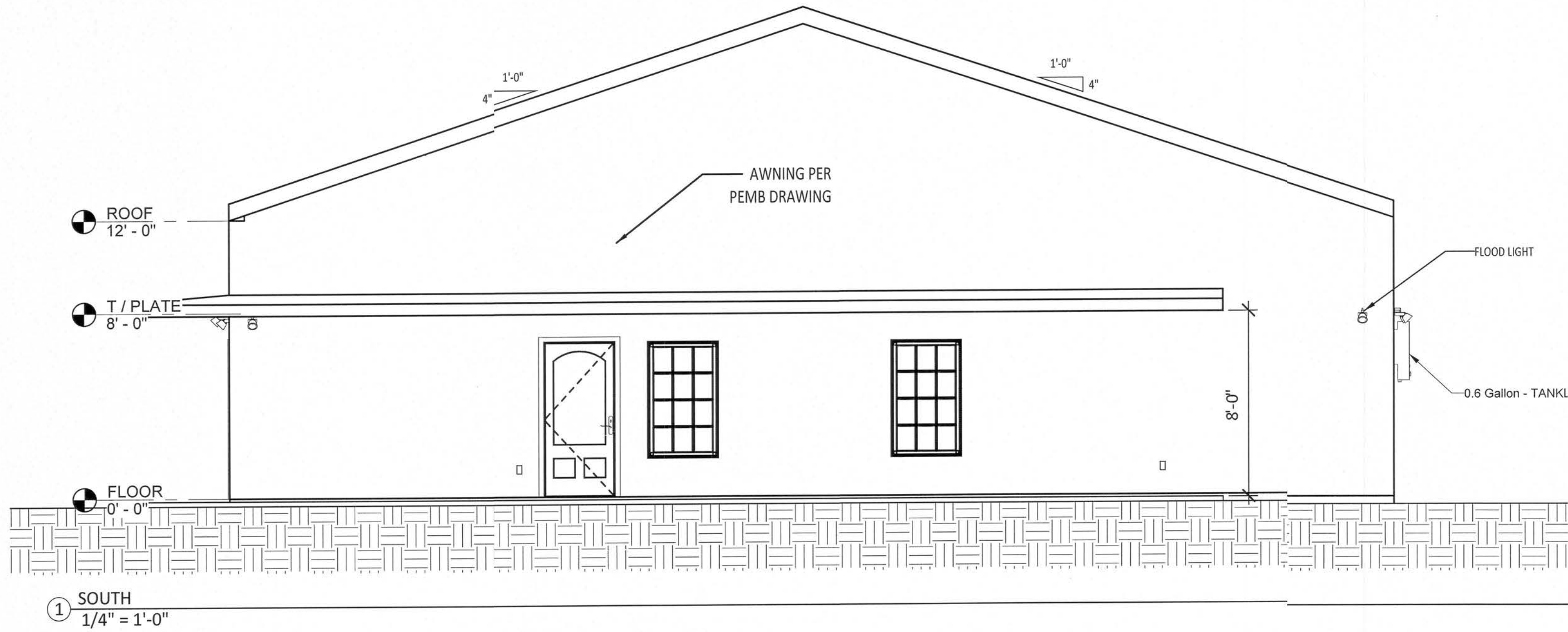
Gd
1/23/20



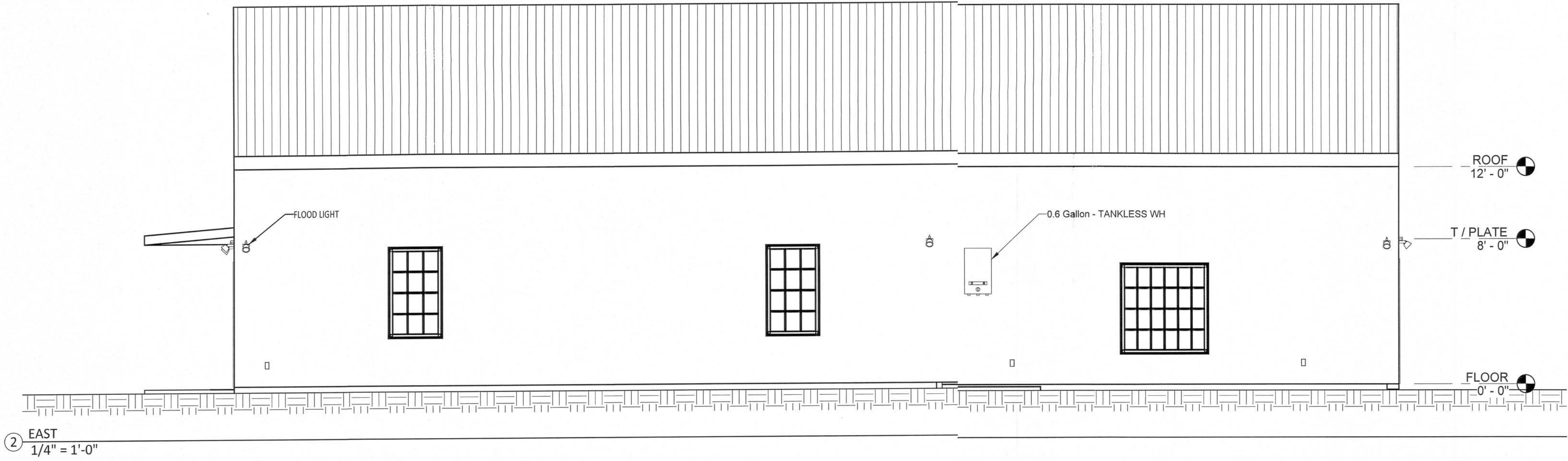
1/22/2020 3:19:37 PM
email: ggill@gillengineeringservices.com

GRAHAM RESIDENCE		
COLUMBIA COUNTY, FL		
COLUMBIA CONSTRUCTION & MAINTENANCE, INC		GILL ENGINEERING SERVICES, INC AUTH # 30824 GARY GILL PE #5142 426 SW COMMERIE DR 130-M LAKE CITY, FL 32025 386-590-1242
DRAWN BY:	Author	
CHKD BY:	Checker	
APPRD BY:	Approver	
DIMENSION PLAN		
1945-063	DWG #:	A-002
		REV #:

Revision Schedule		
Revision Number	Revision Description	Revision Date



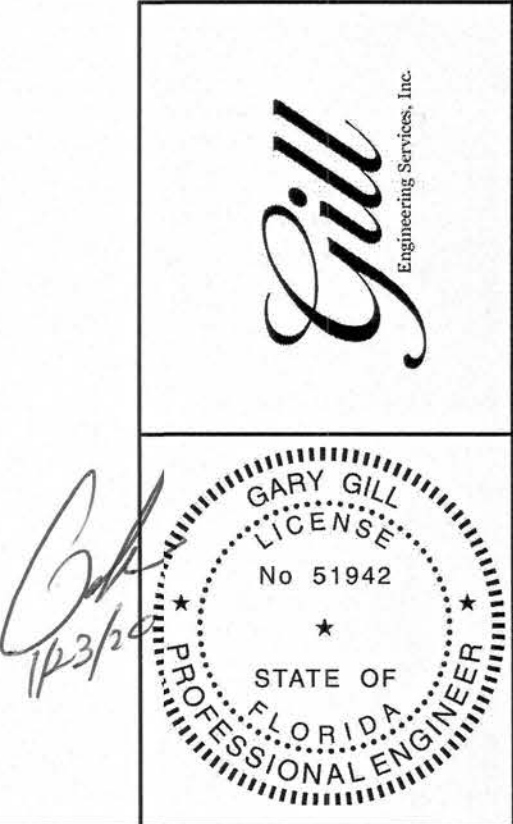
① SOUTH
1/4" = 1'-0"



② EAST
1/4" = 1'-0"

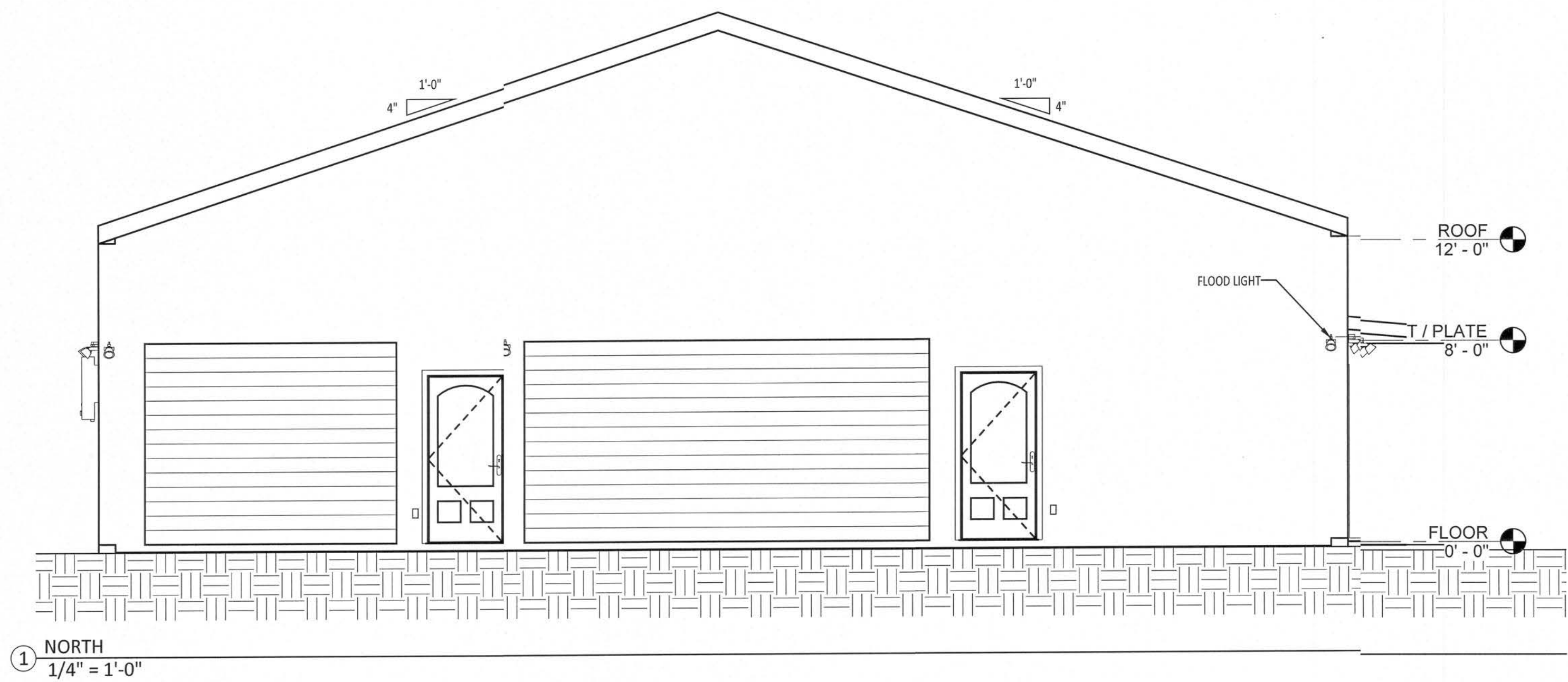
ENGINEERING SCOPE IS LIMITED TO WIND LOAD
CALCULATION & STRUCTURAL ELEMENTS AND
COMPONENTS

GRAHAM RESIDENCE				
COLUMBIA COUNTY, FL				
COLUMBIA CONSTRUCTION & MAINTENANCE, INC			GILL ENGINEERING SERVICES, INC AUTH # 30824 GARY GILL PE #51942 426 SW COMMERCE DR130-M LAKE CITY, FL 32025 385-590-1242	
DRAWN BY:	GG			
CHKD BY:	GG			
APPRD BY:	GG			
ELEVATIONS				
1945-063	DWG #:		A-003	REV #:



1/22/2020 3:19:38 PM
email: ggill@gillengineering.com

Revision Schedule		
Revision Number	Revision Description	Revision Date



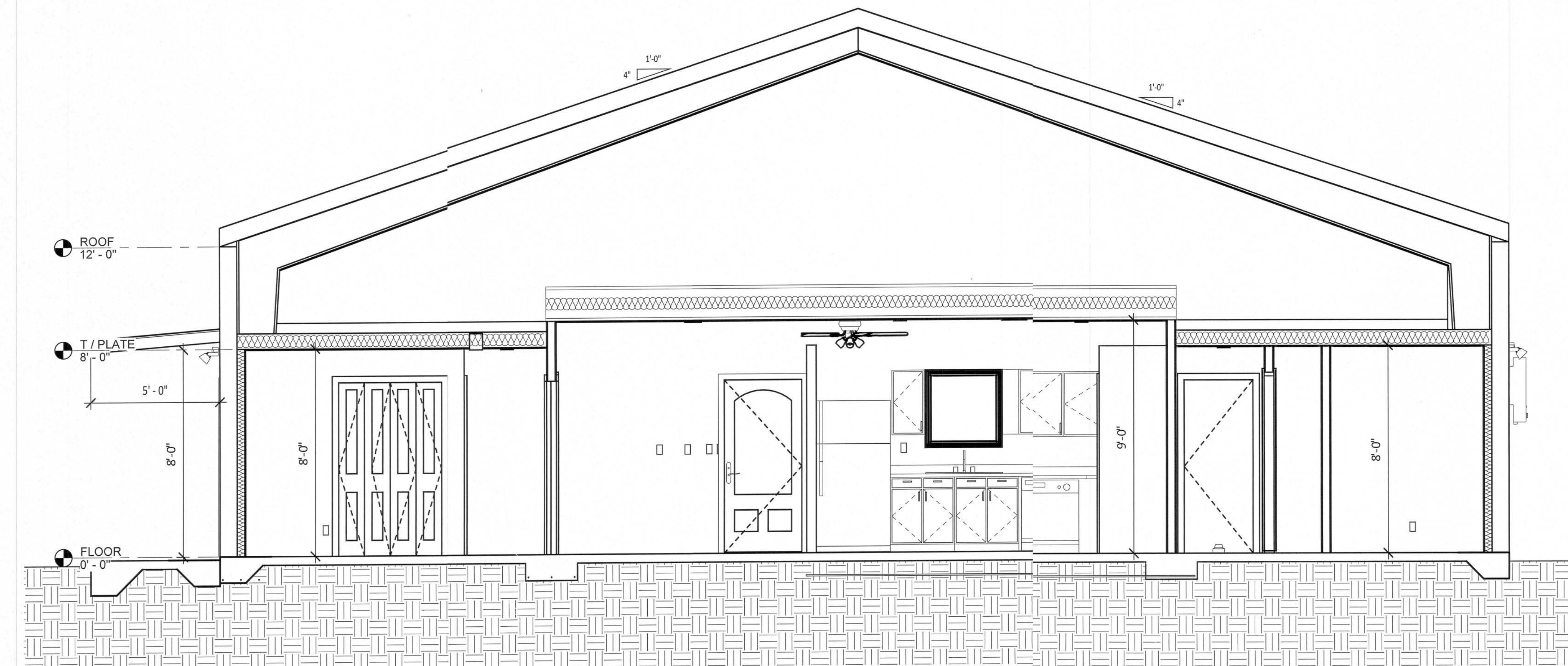
ENGINEERING SCOPE IS LIMITED TO WIND LOAD
CALCULATION & STRUCTURAL ELEMENTS AND
COMPONENTS

1/22/2020 3:19:39 PM
email: ggill@gillengineeringservices.com

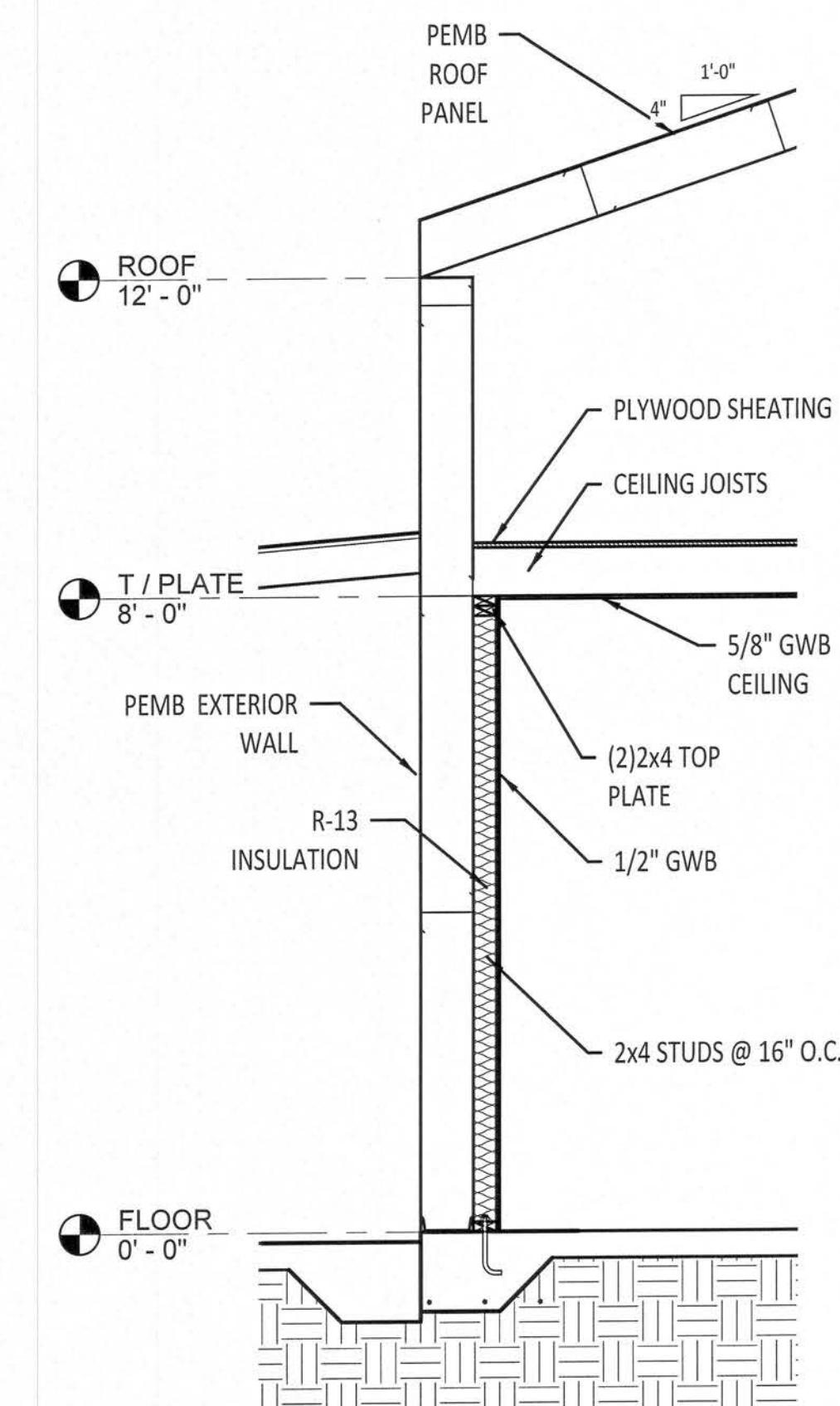


GRAHAM RESIDENCE			
COLUMBIA COUNTY, FL			
COLUMBIA CONSTRUCTION & MAINTENANCE, INC			GILL ENGINEERING SERVICES, INC AUTH # 30824 GARY GILL PE #51942 426 SW COMMERCE DR 130-M LAKE CITY, FL 32025 386-590-1242
DRAWN BY:	GG		
CHKD BY:	GG		
APPRD BY:	GG		
ELEVATIONS			
1945-063	DWG #:	A-004	REV #:

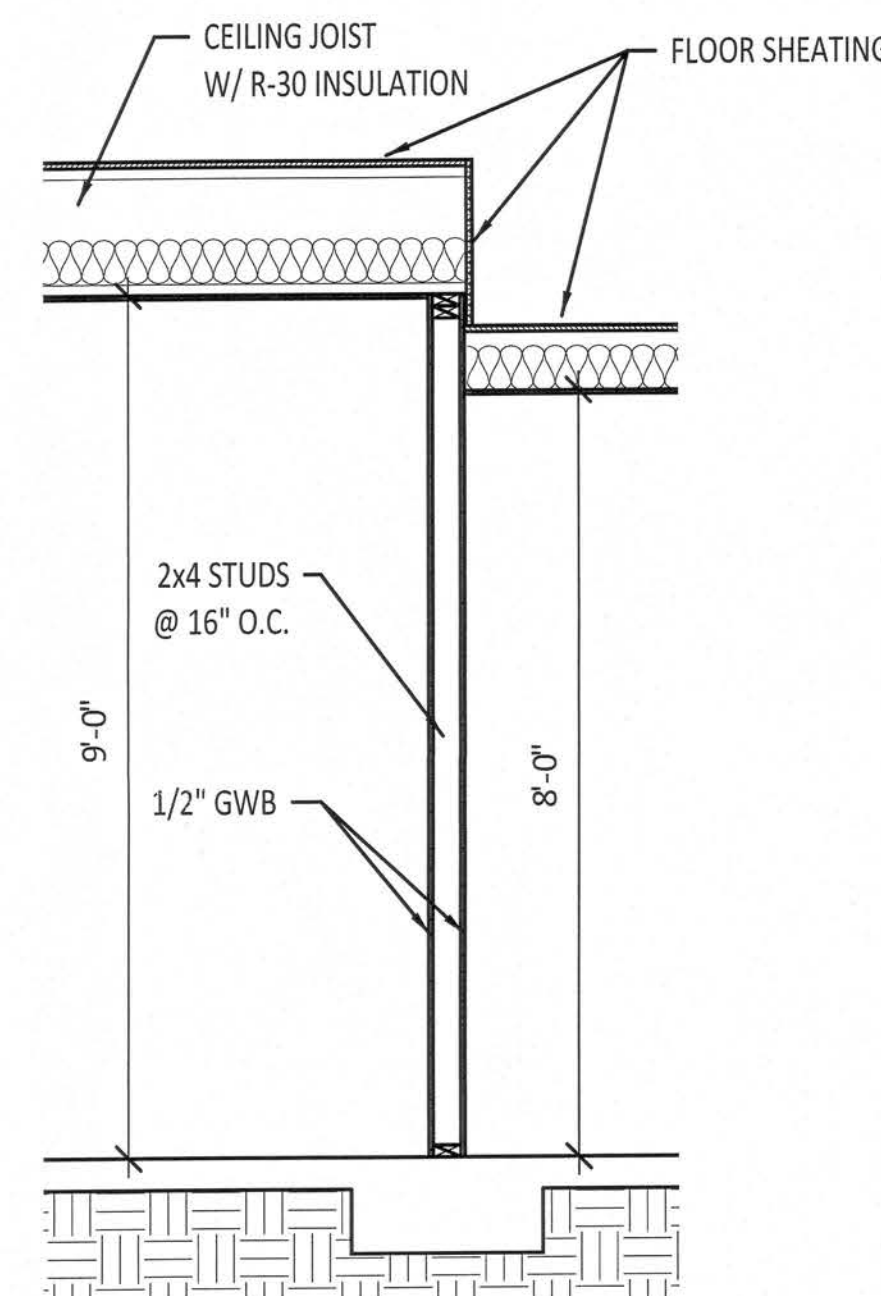
Revision Schedule		
Revision Number	Revision Description	Revision Date



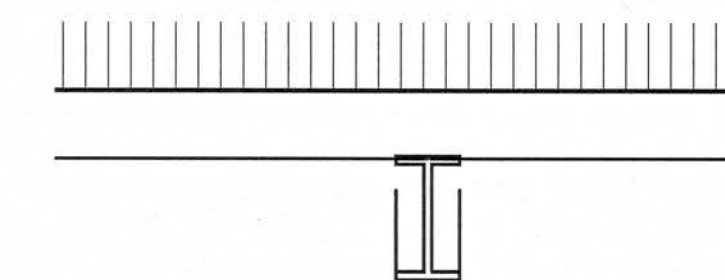
1 ARCHITECTURAL SECTION
3/8" = 1'-0"



2 WALL SECTION
1/2" = 1'-0"



3 INTERIOR WALL SECTION
1/2" = 1'-0"



4 GARAGE WALL SECTION
1/2" = 1'-0"

ENGINEERING SCOPE IS LIMITED TO WIND LOAD
CALCULATION & STRUCTURAL ELEMENTS AND
COMPONENTS

GRAHAM RESIDENCE

COLUMBIA COUNTY, FL

COLUMBIA CONSTRUCTION &
MAINTENANCE, INC.

DRAWN BY: GG
CHKD BY: GG
APPRD BY: GG

GILL ENGINEERING SERVICES, INC.
AUTH # 30824
GARY GILL PE #51942
426 SW COMMERCE DR 110-M
LAKE CITY, FL 32025 386590-1242

SECTIONS

1945-063

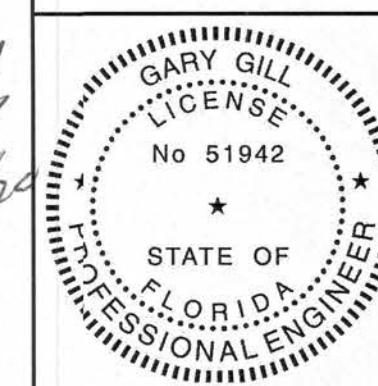
DWG #: A-005

REV #:

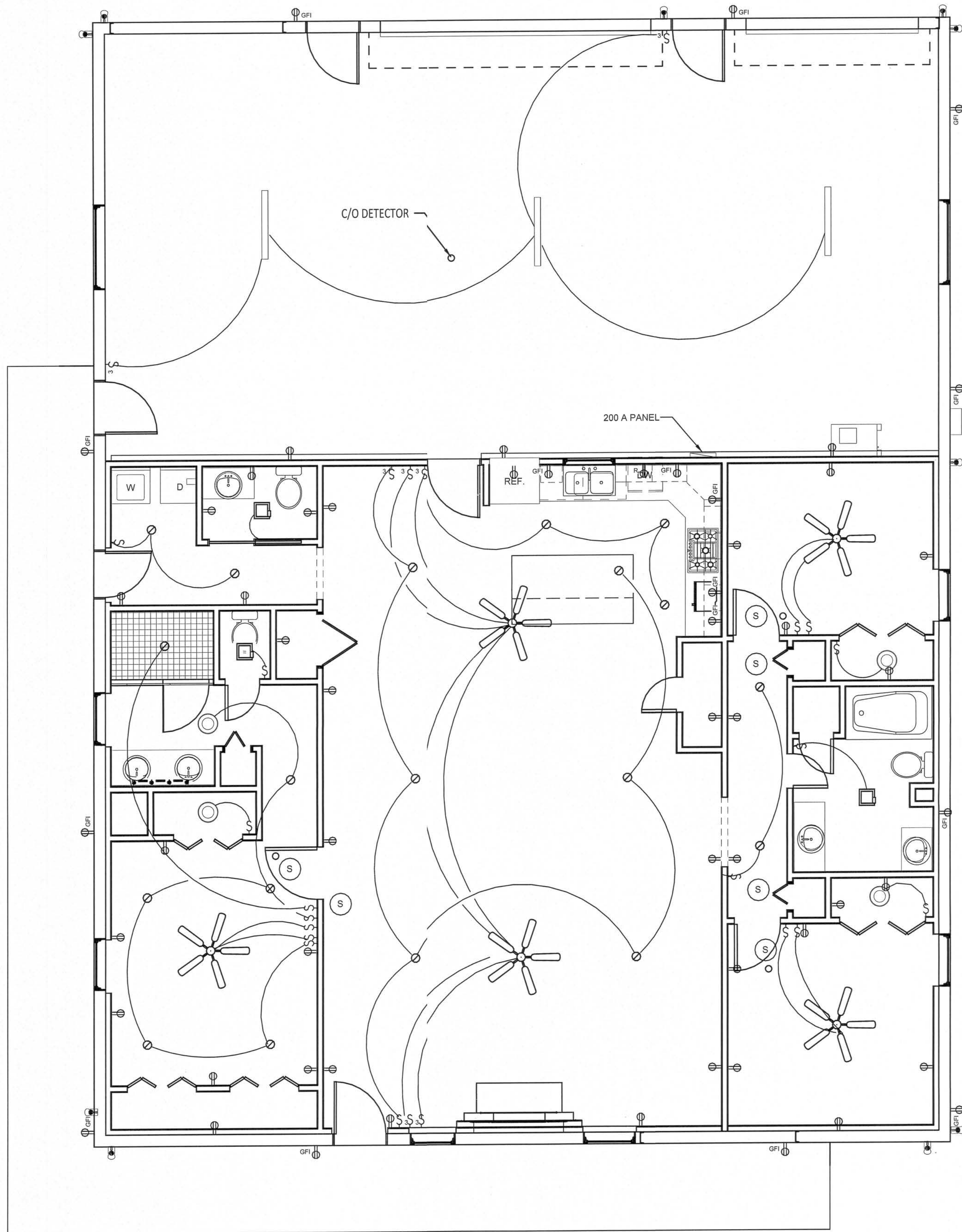
1/22/2020 3:19:40 PM

email: ggill@gillengineeringservices.com

Gill
Engineering Services, Inc.



Revision Schedule		
Revision Number	Revision Description	Revision Date



ALL ELECTRICAL SYSTEMS , COMPONENTS, AND DEVICES TO BE DESIGNED BY A CERTIFIED ELECTRICAL CONTRACTOR. ALL INFORMATION ON ELECTRICAL PLANS ARE CONCEPTUAL ONLY AND NOT INTENDED TO SERVE AS DESIGN DOCUMENTS

- ELECTRICAL PLAN NOTES:
1. WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUFACTURER'S SPECIFICATIONS.
 2. CONSULT THE OWNER FOR THE NUMBER OF SEPARATE TELEPHONE LINES TO BE INSTALLED.
 3. INSTALLATION SHALL BE PER NATIONAL ELECTRIC CODE.
 4. ALL SMOKE DETECTORS SHALL BE 120V WITH BATTERY BACKUP OF THE PHOTOELECTRIC TYPE AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.
 5. TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER OWNER'S DIRECTIONS, & IN ACCORDANCE WITH APPLICABLE SECTIONS OF NEC - LATEST ADDITION.
 6. ELECTRICAL CONTRACTOR SHALL PREPARE "AS-BUILT" SHOP DRAWINGS INDICATING ALL ELECTRICAL WORK, INCLUDING ANY CHANGES TO THE ELECTRICAL PLAN, ADDITIONS TO THE ELECTRICAL PLAN, RISER DIAGRAM, AS-BUILT PANEL SCHEDULE WITH ALL CIRCUITS IDENTIFIED WITH CIRCUIT NUMBER, DESCRIPTION, AND BREAKER SERVICE ENT. AND ALL UNDERGROUND WIRE LOCATIONS/ROUTING/DEPTH. RISER DIAGRAM SHALL INCLUDE WIRE SIZES/TYPE AND EQUIPMENT TYPE WITH RATINGS AND LOADS. CONTRACTOR SHALL PROVIDE 1 COPY OF "AS-BUILT" DRAWINGS TO OWNER AND 1 COPY TO PERMITTING AUTHORITY.
 7. ALL BEDROOM RECEPTACLES SHALL BE ON AFCI PROTECTED CIRCUITS.
 8. ALL BATHROOM RECEPTACLES SHALL BE GFCI.

2 ELECTRICAL NOTES
1/4" = 1'-0"

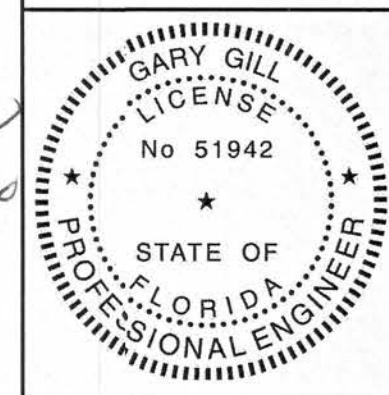
- Legend:
- S SINGLE SWITCH
 - S THREE WAY SWITCH
 - OUTLET DUPLEX
 - GFI OUTLET DUPLEX
 - FAN LIGHT COMBO
 - CEILING FIXTURE
 - 6" RECESS CAN
 - CEILING FAN
 - HIGH BAY LIGHTS
 - EXT. FLOOD LIGHTS
 - 200 AMP PANEL
 - SMOKE DETECTOR
 - CARBON MONOXIDE DETECTOR

1 ELECTRICAL
1/4" = 1'-0"

1 ELECTRICAL PLAN
1/4" = 1'-0"

1/22/2020 3:19:42 PM
email: ggill@gillengineering.com

Gill
Engineering Services, Inc.



GRAHAM RESIDENCE			
COLUMBIA COUNTY, FL			
COLUMBIA CONSTRUCTION & MAINTENANCE, INC		GILL ENGINEERING SERVICES, NC	
DRAWN BY:	Author	AUTH # 30824	
CHKD BY:	Checker	GARY GILL PE #51942	
APPRD BY:	Approver	426 SW COMMERCE DR 130-N LAKE CITY, FL 32025 386-5901242	
ELECTRICAL			
1945-063	DWG #:	E-001	REV #:

GENERAL STRUCTURAL NOTES

GENERAL NOTES:

1.

ALL CONSTRUCTION AND DESIGN SHALL CONFORM TO THE 2017 FBC (6TH ED)
2.

THE STRUCTURAL DRAWINGS SHALL BE UTILIZED IN CONJUNCTION WITH OTHER CONSULTANTS' DRAWINGS.
3.

THE STRUCTURAL DRAWINGS ARE INTENDED FOR THE STRUCTURE TO ACT AS WHOLE ONE CONSTRUCTION IS COMPLETE. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO ENSURE SAFETY AND STABILITY (I.E, TEMPORARY BRACING IF REQUIRED) DURING CONSTRUCTION AS A RESULT OF CONSTRUCTIONS METHODS AND SEQUENCES.
4.

THE CONTRACTOR SHALL FIELD VERIFY ALL EXISTING STRUCTURES. THE ENGINEER SHALL BE NOTIFIED ON ANY DISCREPANCY BETWEEN THE EXISTING CONDITIONS AND CONSTRUCTION DOCUMENTS.
5.

DESIGN CRITERIA

A. CODE: 2017 FBC (6TH ED)

B. LOADS AND DESIGN CRITERIA: THE FOLLOWING LOADS AND CRITERIA WERE USED IN ADDITION TO THE DEAD LOAD OF THE STRUCTURE.

LIVE LOADS:

ROOF

20 PSF

CEILING

50 PSF

SOIL CRITERIA:

ALLOWABLE SOIL BEARING

2000 PSF

PASSIVE PRESSURE

150 PCF

FRICTION COEFFICIENT

0.35

WIND CRITERIA:

WIND SPEED:

130 MPH (3-SECOND GUST)

CATEGORY:

II

EXPOSURE

B

INTERNAL PRESSURES:

=/- 0.18

CLADDING AND COMPONENTS

ZONE 1

21.3 / -34.15 PSF

ZONE 2

21.5 / -59.45 PSF

ZONE 3

21.5 / -69.75 PSF

ZONE 4

37.32 / -40.48 PSF

ZONE 5

37.32 / 49.96 PSF

CONCRETE AND REINFORCING STEEL:

1.

ALL CONCRETE DESIGNED PER CURRENT EDITION OF ACI 318
2.

CONCRETE SHALL HAVE THE FOLLOWING MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS:

A. FOUNDATION WALLS, PIERS, AND FOOTINGS

3000 PSI

B. SLAB ON GARE:

3000 PSI

C. ALL OTHER CONCRETE

3000 PSI
3.

ALL CONCRETE SHALL BE NORMAL WEIGHT CONCRETE WITH A NORMAL AIR DENSITY OF 145 PSF.
4.

PROVIDE CONSTRUCTION JOINTS WHERE SHOWN, OMIT NONE AND ADD NONE WITHOUT WRITTEN APPROVAL FROM THE ARCHITECT / ENGINEER. SUBMIT DRAWINGS SHOWING ALL PROPOSED CONSTRUCTION JOINT LOCATIONS FOR APPROVAL PRIOR TO PREPARATIONS OF AFFECTED REINFORCEMENT SHOP DRAWINGS
5.

MINIMUM ELAPSED TIME BETWEEN ADJACENT CONCRETE PLACEMENTS SHALL BE 48 HOURS
6.

CONCRETE MIX DESIGN FOR EACH TYPE AND STRENGTH OF CONCRETE SPECIFIED SHALL BE SUBMITTED FOR ARCHITECT / ENGINEER REVIEW 30 DAYS PRIOR TO PLACEMENT OF CONCRETE
7.

ALL REINFORCING STEEL ASTM A615 GRADE 60, ALL WELDED WIRE FABRIC ASTM A185

REINFORCING STEEL:

1.

ALL BAR REINFORCEMENT SHALL BE CONFORM TO ASTM 615 GRADE 60.
2.

WELD WIRE FABRIC REINFORCEMENT SHALL CONFORM TO ASTM A185
3.

CLEARANCE OF MAIN REINFORCEMENT FROM ADJACENT SHALL BE CONFORM TO THE FOLLOWING (UNLESS OTHERWISE SHOWN IN DETAIL).

A. UNFORMED SURFACES IN CONTACT WITH GROUND (FOOTING OR WALL BOTTOM).....3"

B. SLAB ON GRADE2 1/2"

C. FORMED SURFACE IN CONTACT WITH GROUND OR EXPOSED TO WEATHER (WALLS, PIES).....2"

D. IN ALL CASES, CLEARANCE NOT LESS THAN DIAMETER OF BARS.

NOTE: MAXIMUM DEVIATION FROM THESE REQUIREMENTS SHALL BE + 1/4" FOR SECTIONS 10" OR LESS AND +1/2" FOR SECTIONS OVER 10" THICK.
4.

REINFORCEMENT SHALL BE CONTINUOUS THROUGH ALL CONSTRUCTION JOINTS UNLESS OTHERWISE INDICATED ON DRAWS
5.

WHERE REINFORCEMENT IS NOT SHOWN ON DRAWINGS, PROVIDE REINFORCEMENT IN ACCORDANCE WITH APPLICABLE TYPICAL DETAILS OR SIMILAR TO THAT SHOWN FOR MOST NEARLY SIMILAR SITUATION, AS DETERMINED BY THE ARCHITECT / ENGINEER. IN NO CASE SHALL REINFORCEMENT BE LESS THAN MINIMUM PERMITTED BY APPLICABLE CODES.
6.

ALL WORKMANSHIP AND MATERIAL SHALL BE CONFORMED TO THE MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES" (ACI-315)
7.

ALL REINFORCEMENT SHALL BE INSPECTED AND APPROVED BY THE ARCHITECT/ENGINEER OR OWNER TESTING AGENCY BEFORE CONCRETE IS PLACED.
8.

WHERE CONTINUOUS BARS ARE CALLED FOR THEY SHALL BE CONTINUOUSLY AROUND CORNERS, LAPPED AT NECESSARY SPLICES AND HOOKED AT CONTINUOUS ENDS.
9.

WELDED WIRE FABRIC SHALL BE LAPPED ONE FULL MESH PANEL OR 6" MIN.
10.

ALL REINFORCING SPLICES SHALL CONFORM TO THE TABLE(S) PROVIDED IN THE GENERAL NOTES FOR STRENGTH OF CONCRETE BUT IN NO CASE LESS THAN THE REQUIREMENTS OF THE LATEST EDITION OF ACI 318
11.

SLABS AND WALLS SHALL NOT BE SLEEVED OR BOXED OUT OR HAVE THEIR REINFORCEMENT INTERRUPTED EXCEPT SPECIFICALLY NOTED ON THE DRAWINGS. PROVIDE ADDITIONAL REINFORCEMENT AROUND OPENINGS AS SHOWN IN THE DETAILS.
12.

SUBMIT CHECKED SHOP DRAWINGS TO THE ARCHITECT / ENGINEER FOR REVIEW PRIOR TO FABRICATION OF REINFORCEMENT.
13.

BAR SUPPORTS SHALL BE GALVANIZED OR STAINLESS STEEL. BAR SUPPORTS IN CONTACT WITH EXPOSED SURFACE SHALL BE GALVANIZE AND PLASTIC TIPPED.

ADDITION CONCRETE ITEMS:

SLAB AND WALL REINFORCING LAP SPLICE LENGTHS			
LAP SPLICE LENGTHS FOR REINFORCING IN 4000 PSI CONCRETE AS FOLLOWS			
BAR SIZE	TENSION SPLICE		DEVELOPMENT LENGTH
	TOP	OTHER	
#3	21"	15"	13"
#4	29"	20"	17"
#5	35"	25"	21"
#6	43"	31"	25"
#7	54"	39"	32"
#8	71"	51"	42"

LAP SPLICE LENGTHS FOR REINFORCING IN 3000 PSI CONCRETE AS FOLLOWS			
BAR SIZE	TENSION SPLICE		DEVELOPMENT LENGTH
	TOP	OTHER	
#3	21"	15"	13"
#4	29"	20"	17"
#5	35"	25"	21"
#6	46"	33"	27"
#7	63"	45"	37"
#8	83"	59"	49"

- NOTES:
1.

LAPPED SPLICE LENGTHS BASED ON ASTM A-615, GRADE 60, REBAR
2.

REINFORCING BARS CLASSIFIED AS TOP BARS WHEN MORE THAN 12" ON CONCRETE IS CAST BENEATH RESPECTIVE REINFORCING BAR.
3.

COMPRESSION SPLICES SHALL BE PERMISSIBLE ONLY WHERE SPECIFICALLY NOTED ON THE DRAWINGS
4.

TENSION SPLICES SHALL BE USED IN ALL BEAMS, SLABS, AND WALLS UNLESS OTHERWISE NOTED.
5.

WHEN LAPPING LARGER BARS WITH SMALLER BARS, LAP LENGTH FOR SMALLER BAR SHALL GOVERN RESPECTIVE SPLICE.
6.

SPLICE CONTINUOUS TOP REINFORCING BARS AT CENTER OF CLEAR SPAN WITH COMPRESSION SPLICES
7.

SPLICE CONTINUOUS REINFORCING BARS AT CENTER OF SUPPORTING ELEMENT WITH COMPRESSION SPLICES.

FLOOR SLABS:

1.

FLOOR SLABS SHALL BE ON AT LEAST 4" OF RELATIVELY CLEAN GRANULAR MATERIAL SUCH AS SAND, SAND AND GRAVEL, OR CRUSHED STONE. GRANULAR MATERIAL SHALL HAVE 100% PASSING THE 1 1/2" SIEVE AND A MAXIMUM OF 10% PASSING THE NO. 200 SIEVE.
2.

STRUCTURAL FILL SHALL BE PLACED IN THIN LOOSE LIFTS NOT EXCEEDING 12" IN THICKNESS AND COMPACTED WITH A HEAVY ROLLER. EACH LIFT SHALL BE THOROUGHLY COMPACTED WITH THE LABORATORY ROLLER TO PROVIDE DENSITIES TO AT LEAST 95% OF THE PROCTOR MAXIMUM DRY DENSITY. STRUCTURAL FILL SHALL CONSIST OF AN INORGANIC NON-PLASTIC, GRANULAR SOIL CONTAINING LESS THAN 10% MATERIAL PASSING THE 200 MESH SIEVE.

POST-INSTALLED REBAR:

1.

POST-INSTALLED REINFORCING BAR CONNECTIONS SHALL BE DESIGNED PER THE ACI BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE (ACI 318). POST-INSTALLED REINFORCING BAR CONNECTIONS SHALL BE CONSIST OF HILTI EPOXY SYSTEMS OR EQUAL
2.

THE DESIGN OF STRAIGHT POST-INSTALLED REINFORCING BARS SHALL BE PERFORMED PER THE DEVELOPMENT AND SPLICE REQUIREMENTS OF THE ACI 318. THE POST-INSTALLED REINFORCING BAR SYSTEM IS AN ALTERNATIVE TO CAST-IN-PLACE REINFORCING BARS GOVERNED BY ACI 318 AND IBC CHAPTER 19.
3.

THE EPOXY SYSTEM SHALL BE TESTED IN ACCORDANCE WITH THE ICC-ES ACCEPTANCE CRITERIA FOR POST-INSTALLED EPOXY ANCHORS IN CONCRETE ELEMENTS (ACI 308), TABLE 3.8 TECHNICAL DATA SHALL BE PUBLISHED IN AN ICC-ES EVALUATION SERVICE REPORT SHOWING COMPLIANCE WITH IBC.
4.

POST-INSTALLED REINFORCING BAR INSTALLATION SHALL BE PERFORMED BY PERSONNEL TRAINED TO INSTALL THE SYSTEM PER THE MANUFACTURED PRINTED INSTALLATION INSTRUCTION (MP1), AS INCLUDED IN THE ANCHOR PACKAGING.

COMPACTION REQUIREMENTS

1.

SUBGRADE SOILS AND STRUCTURAL FILL MATERIALS SHALL BE COMPACTED TO THE FOLLOWING PERCENTAGES OF THE ASTM D1557 MAXIMUM DRY DENSITY AT +/- 2% OPTIMUM MOISTURE CONTENT:

MATERIAL	MINIMUM PERCENT COMPACTION
STRUCTURAL FILL IN THE BUILDING AREA	95
SUBBASE FOR SLAB SUPPORT	95
SUBGRADE BELOW STRUCTURAL FILL	95
MISCELLANEOUS BACKFILL	90

PRE-ENGINEERED METAL BUILDING FRAME:

1.

THE COMPLETE DESIGN OF METAL BUILDING INCLUDING ALL COMPONENTS SHOWN OR NOTE SHOWN ON THE DRAWINGS SHALL BE ACCOMPLISHED BY THE BUILDING MANUFACTURER. SEE THE ARCHITECTURAL DRAWINGS FOR EAVE HEIGHTS AND WALL CONDITIONS.
2.

THE DESIGN SHALL BE MADE BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE WHERE PROJECT IS CONSTRUCTED.

THE BUILDING AND ALL OF ITS COMPONENTS SHALL BE DESIGNED FOR THE FOLLOWING DEAD AND LIVE LOADS

A.

ACTUAL WEIGHT OF STEEL STRUCTURE

B.

5 PSF COLLATERAL LOAD IN ADDITION TO ACTUAL WEIGHT

C.

20 PSF ROOF LIVE LOAD

D.

ANY ADDITIONAL LOADS AND REACTIONS THAT ARE SHOWN OR NOTED ON THE DRAWINGS, SEE PLAN NOTES.

E.

WIND LOADS AS REQUIRED BY CURRENT ADDITION OF THE FLORIDA BUILDING CODE
3.

RIGID FRAME COLUMN BASES SHALL BE DESIGNED FOR A PIN CONNECTION,
4.

THE DEFLECTION OF GIRTS AND PURLINS SHALL BE LIMITED TO 1/180 OF THE SPAN. DEFLECTION OF RIGID FRAMES SHALL BE LIMITED TO 1/360 OF THE SPAN. DEFLECTIONS SHALL BE BASED ON TOTAL LOAD (DEAD LOAD PLUS LIVE LOAD).
5.

THE PREFABRICATED METAL BUILDING SHALL BE DESIGNED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE ABOVE NOTES. THE PLANS AND DETAILS, SPECIFICATIONS TO BE IN ACCORDANCE WITH THE RECOMMENDED DESIGN PRACTICES MANUAL OF THE METAL BUILDING MANUFACTURES ASSOCIATION (MBMA) AND IN ACCORDANCE WITH MBMA SPECIFICATIONS

Revision Schedule		
Revision Number	Revision Description	Revision Date

1/22/2020 3:19:43 PM
email: ggill@gillengineeringervices.com



GRAHAM RESIDENCE
COLUMBIA COUNTY, FL

COLUMBIA CONSTRUCTION & MAINTENANCE, INC.			GILL ENGINEERING SERVICES, INC AUTH # 30824 GARY GILL PE #51942 426 SW COMMERCE DR 130-M LAKE CITY, FL 32025 386-590-1242
DRAWN BY:	GG		
CHKD BY:	GG		
APPRD BY:	GG		

STRUCTURAL NOTES

1945-063	OWG #:	S-001	REV #:
----------	--------	-------	--------

GENERAL STRUCTURAL NOTES CONTINUED

GENERAL WOOD NOTES:

- DIMENSIONAL LUMBER
- DIMENSIONAL LUMBER USED AS STRUCTURAL FRAMING (i.e. JOISTS, RAFTERS, HEADERS) SHALL BE SOUTHERN YELLOW PINE NO.2 OR EQUAL.
 - DIMENSIONAL LUMBER USED FOR STUDS WALLS SHALL BE STUD GRADE UNLESS NOTED OTHERWISE. STUDS SHALL HAVE BE SPACES AT 16" MIN WITH A DOUBLE TOP PLATE. SPLICES IN THE DOUBLE TOP WALLS SHALL BE ALTERNATE TOP AND BOTTOM. IN NO CASE SHALL 2x4 BEARING WALLS SUPPORT MORE THAN TWO FLOORS OF FRAMING IN ADDITION TO ROOF AND CEILING
 - ROUGH CUT TIMBER USED AS STRUCTURAL FRAMING SHALL BE AS SPECIFIED IN THE CONSTRUCTION DOCUMENTS
 - ALL LUMBER IN CONTACT WITH THE GROUND, CONCRETE SHALL BE PRESSURED-TREATED. CONTRACTOR MAY SUBMIT FOR APPROVAL A MOISTURE BARRIER IN- LIEU OF THE PRESSURE TREATED WOOD.
 - FASTENERS FOR PRESERVATIVE-TREATED AND FIRE-RETARDANT TREATED WOOD SHALL BE OF HOT-DIPED ZINC COATED GALVANIZED STEEL OR STAINLESS STEEL AND SHALL FOLLOW CURRENT SIMPSON GUIDELINES BASED ON WEATHER EXPOSURE WHERE STAINLESS STEEL CONNECTORS OR HOT DIPPED CONNECTORS ARE SPECIFIED IN THE DRAWINGS, STAINLESS STEEL OR HOT DIPPED GALVANIZED FASTENERS SHALL BEISED TO MATCH THE CONNECTORS TYPE.
 - ALL NAILS FOR STRUCTURAL WORK SHALL BE COMMON WIRE NAILS UNLESS NOTED OR DETAILED OTHERWISE MEETING ASTM F1667. HOLES SHALL BE PRE-DRILLED WHERE NECESSARY TO PREVENT SPLITTING. NAIL SHALL HAVE THE MINIMUM PROPERTIES SPECIFIED IN THE TABLE BELOW:

NAIL TYPE	SHANK DIAMETER- INCHES	MINIMUM PENETRATION - INCHES
6d	0.113	1.13
8d	0.131	1.31
10d	0.148	1.48
12d	0.148	1.48
16d	0.162	1.63
20d	0.192	1.92

NAILING NOT SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE PER THE NAILING SCHEDULE BELOW:

- | | |
|---|--------------------------------------|
| A. JOIST SITTING ON SILL OR GIRDER | (3) 8d TOENAILS, EA. SIDE |
| B. BLOCKING BETWEEN JOIST/RAFTERS | (2) 10d TOENAILS EA. SIDE, EA. END |
| RIM BLOCKING BETWEEN JOIST/RAFTERS | (3) 10d TOENAILS EA. END |
| C. TOP PLATE TO STUD | (2) 16d END NAILS |
| D. STUD TO SILL PLATE | (2) 16d END NAILS OR (4) 8d TOENAILS |
| E. DOUBLE STUDS | (2) 10d @ 12" O.C. |
| F. DOUBLE TOP STUDS - BETWEEN SPLICE NAILING | 16d @ 16" O.C. FACE NAILS |
| G. DOUBLE TOP STUDS - EACH SIDE OF SPLICE PLATE | (8) 16d |
| H. BLOCKING TO TOP PLATE | (2) 10d TOENAILS EACH SIDE |
| I. BLOCKING TO FLOOR/ROOF SHEATHING | (4) 10d NAILS |
| J. RIM JOIST OR BLK TO TOP PLATE OR SILL PLATE | 8d TOENAILS @ 6" O.C. |
| K. CONTINUOUS (2) AND (3) PIECE HEADERS | 16d @ 16" O.C. ALONG EACH EDGE |
| L. CEILING JOIST LAPS OVER PARTITIONS | (3) 16d FACE NAILS, MINIMUM |
| M. RAFTER TO TOP PLATE OR SILL PLATE | (3) 8d TOENAILS EACH SIDE |
| N. BUILT-UP CORNER STUDS | 16d @ 24" O.C. |
| O. TONGUE AND GROOVE DECKING | (2) 16d AT EACH BEARING |
| P. CROSS BRIDGING | (2) 10d EACH END |
| Q. HORIZONTAL BLOCKING BETWEEN WALL STUDS | (2) 10d TOENAILS EACH END |
| R. I-JOISTS SITTING ON TOP PLATE OR BEAM | (2) 10d NAILS THROUGH JOIST FLANGE |

NAILING SCHEDULE NOTES:

- ALL OTHER NAILING REQUIREMENTS NOTE SHOWN ON DRAWINGS OR IN SCHEDULE ABOVE SHALL BE IN ACCORDANCE WITH 2012 FBC.
- POWER DRIVEN OR PNEUMATIC NAILS OTHER THAN COMMON NAILS MAY BE USED IF DATA IS SUBMITTED TO THE ENGINEER FOR APPROVAL PRIOR TO USE.
- MINIMUM NAIL LENGTHS SHALL BE SUFFICIENT TO ACHIEVE MINIMUM PENETRATION INTO MAIN MEMBER AS NOTED IN SCHEDULE ON NOTE ABOVE.

ENGINEERED LUMBER / COMPOSITE WOODS

- GLU-LAMINATED BEAMS FOR SIMPLE SPANS SHALL BE 24F-V4 SP/SP. GLU-LAMINATED BEAMS FOR CONTINUOUS SPANS AND CANTILEVERS SHALL BE 24F-V8 SP/SP. DO NOT INSTALL GLU-LAMINATED BEAMS UPSIDE DOWN.
- LAMINATED VENEER LUMBER AND THE LIKE SHALL BE INSTALLED PER MANUFACTURER'S RECOMMENDATIONS AND SPECIFICATIONS.
- I-JOISTS SHALL BE TJI OR EQUIVALENT AND SHALL BE INSTALLED PER MANUFACTURER'S RECOMMENDATIONS AND SPECIFICATIONS.
- ENGINEERED LUMBER SHALL NOT BE USED IN EXTERIOR APPLICATIONS.
- COMPOSITE WOODS SHALL BE MANUFACTURED BY TRUSS JOIST OR APPROVED EQUAL AND BE OF THE TYPE AND SIZE SHOWN ON THE DRAWINGS. THE MATERIAL TYPE AND GRADE SHALL BE AS SHOWN BELOW:

BEAMS	LVL	E = 1,900,000 PSI	Fb = 2,600 PSI
	PSL	E = 2,000,000 PSI	Fb = 2,900 PSI
	POST PSL	E = 1,800,000 PSI	Fb = 2,600 PSI
- STRUCTURAL COMPOSITE LUMBER SHALL BE MANUFACTURED IN ACCORDANCE WITH ASTM D5456.

WOOD STRUCTURAL PANELS

- STRUCTURAL WOOD PANELS SHALL CONFORM TO THE REQUIREMENTS ON ONE OF THE FOLLOWING STANDARDS AND PUBLICATIONS:

A.	U.S. PRODUCT STANDARD PS-1 FOR CONSTRUCTION AND INDUSTRIAL PLYWOOD
B.	U.S. PRODUCT STANDARD PS-2 PERFORMANCE STANDARD FOR WOOD BASED STRUCTURAL USE PANELS
C.	APA PRP-108 PERFORMANCE STANDARDS
- ROOF AND WALL PANELS SHALL BE APA RATED, EXPOSURE 1, 1/2" OR 5/8" (AS NOTED ON DRAWINGS), 5 PLY PLYWOOD WITH A MIN. 32/16 SPAN RATING UNLESS NOTE OTHERWISE ON THE DRAWINGS. SHEATHING SHALL BE EXTERIOR GRADE WHERE EITHER SIDE OF SHEATHING IS PERMANENTLY EXPOSED TO WEATHER.
- FLOOR SHEATHING SHALL BE TONGUE AND GROOVE APA RATED 5-PLY 3/4" PLYWOOD OR OSB SHEATHING (MIN APA RATED 48/24 SPAN RATING) PROVIDE A-C GRADE PLYWOOD AT ALL DECK SHEATHING LOCATIONS.
- ALL FLOOR AND ROOF SHEATHING SHALL BE INSTALLED WITH THE FACE GRAIN PERPENDICULAR TO THE SUPPORTS AND A 1/8" GAP AT ALL PANEL EDGES UNLESS RECOMMENDED OTHERWISE BY THE PANEL MANUFACTURER.
- ALL SHEATHING PANELS SHALL BE INSTALLED WITH END JOINTS STAGGERED UNLESS NOTED OTHERWISE ON THE DRAWINGS.
- WHERE BLOCKING IS NOT SPECIFICALLY REQUIRED FOR THE ROOF SHEATHING, PLY CLIPS ON OR TONGUE AND GROOVE PLYWOOD SHALL BE USED.
- SUB-FLOORING SHEATHING SHALL BE UNBLOCKED UNLESS NOTED OTHERWISE ON THE STRUCTURAL DRAWINGS, SUB-FLOOR SHEATHING SHALL BE GLUED DOWN TO THE SUPPORTING MEMBERS AND GLUED AT THE TONGUE AND GROOVE JOINTS.
- ALL NAILS SHALL BE COMMON NAILS. ROOF SHEATHING SHALL UTILIZE RING SHANK NAILS. STAINLESS STEEL (TYPE 316) NAILS SHALL BE USED AT PERMANENTLY EXPOSED EXTERIOR AREAS. ALL NAILS THAT ARE NOT EXPOSED TO THE ELEMENTS BUT IN CONTACT WITH PRESERVATIVE TREATED LUMBER SHALL BE MINIMUM HOT DIPPED GALVANIZED MEETING ASTM A153.

Revision Schedule		
Revision Number	Revision Description	Revision Date

1/22/2020 3:19:43 PM
email: ggill@gillengineering.com



GRAHAM RESIDENCE

COLUMBIA COUNTY, FL

COLUMBIA CONSTRUCTION & MAINTENANCE, INC

DRAWN BY:	GG	
CHKD BY:	GG	
APPRD BY:	GG	

GILL ENGINEERING SERVICES, INC
AUTH # 30824
GARY GILL PE #51942
426 SW COMMERCE DR 130M
LAKE CITY, FL 32025 386-550-1242

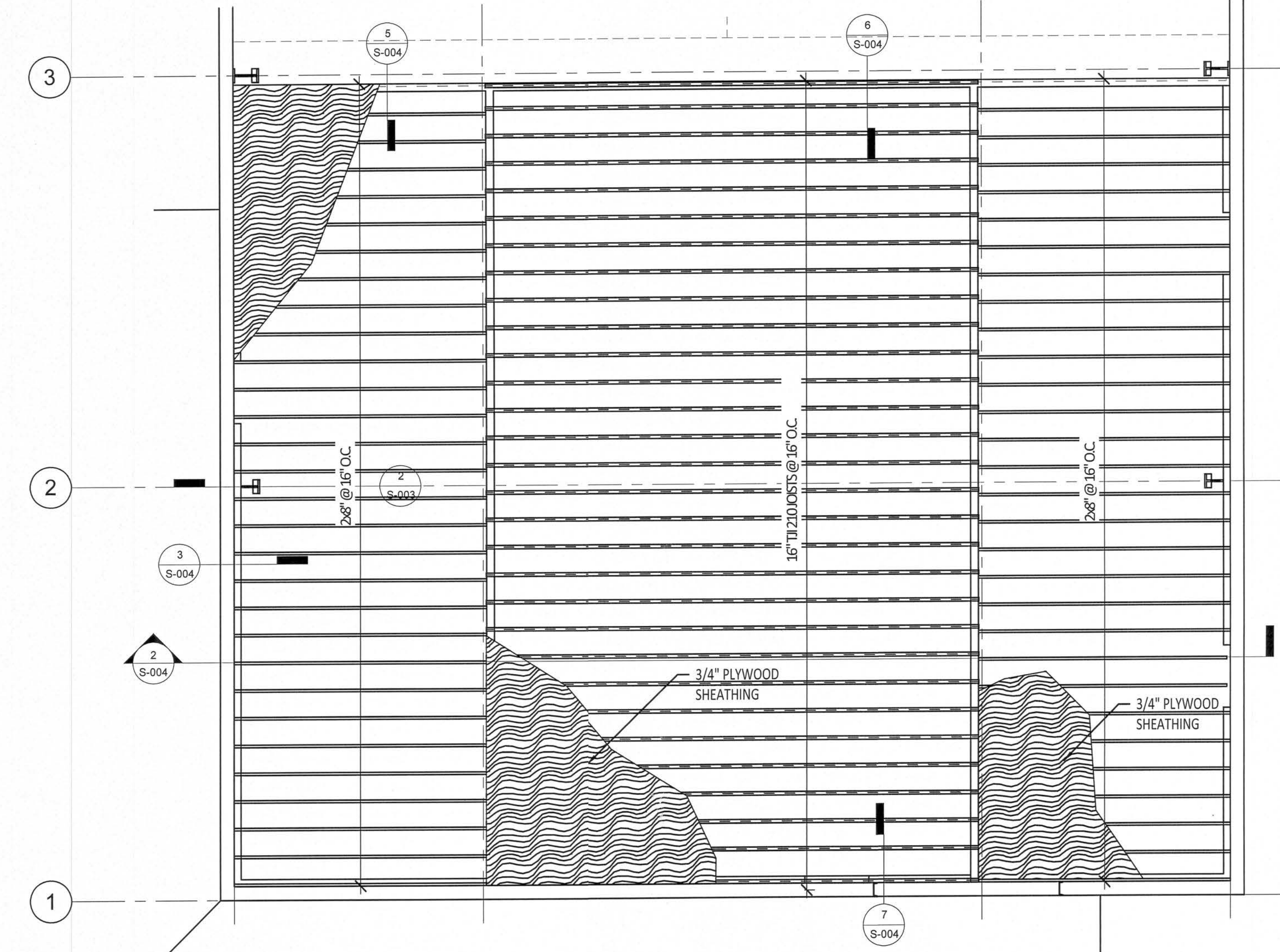
STRUCTURAL NOTES

1945-063

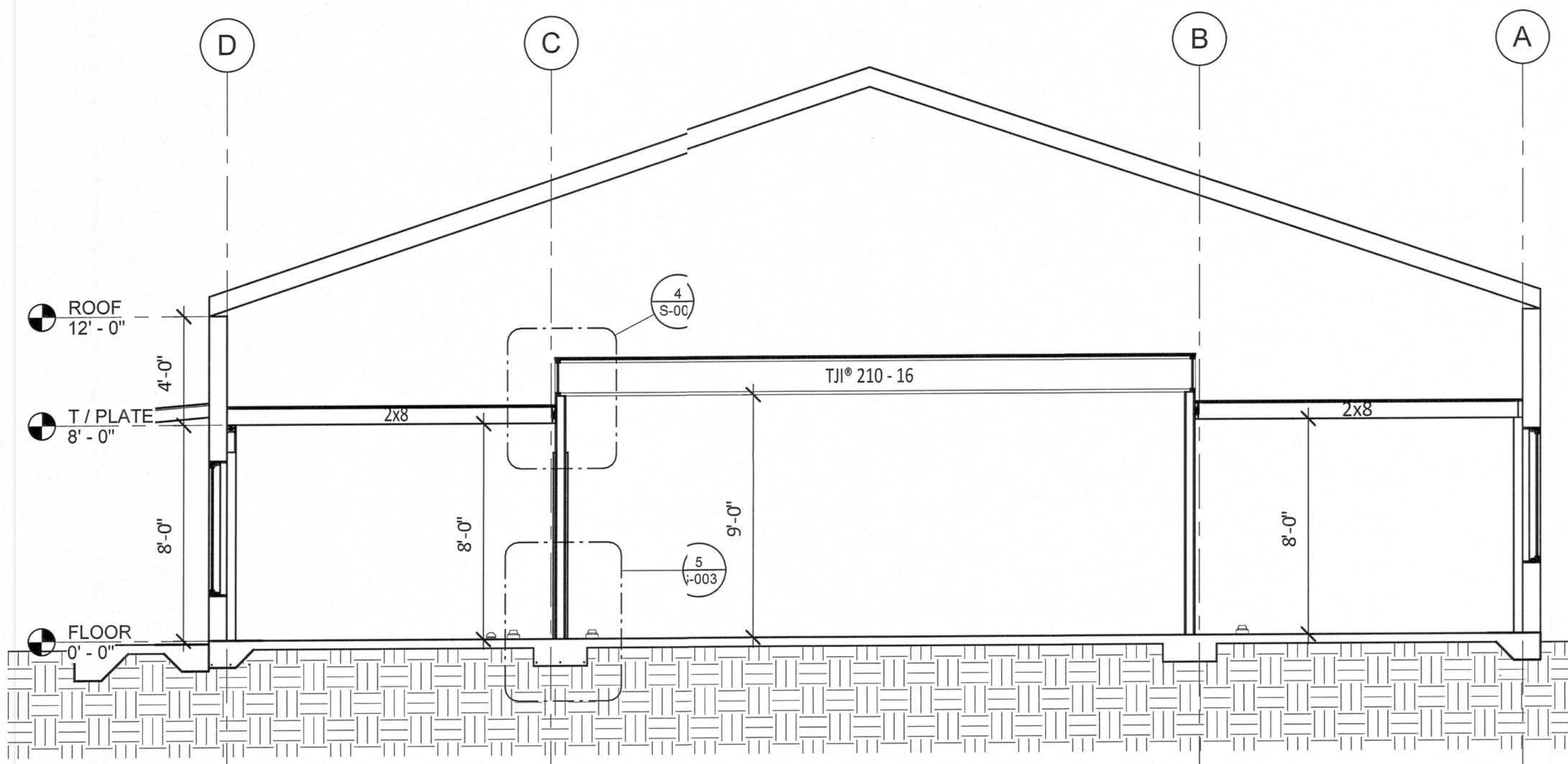
DWG #:

S-002

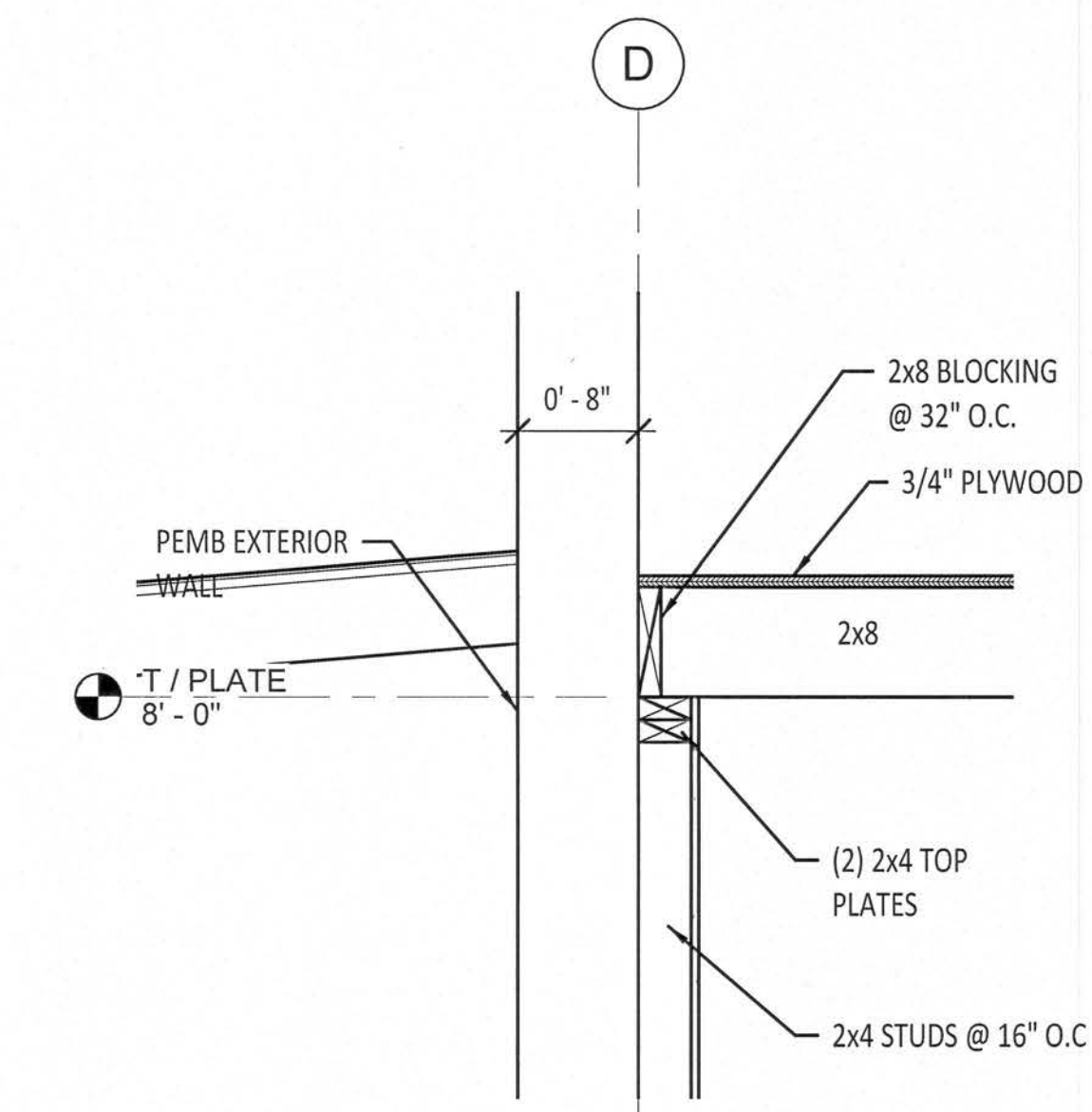
REV #:



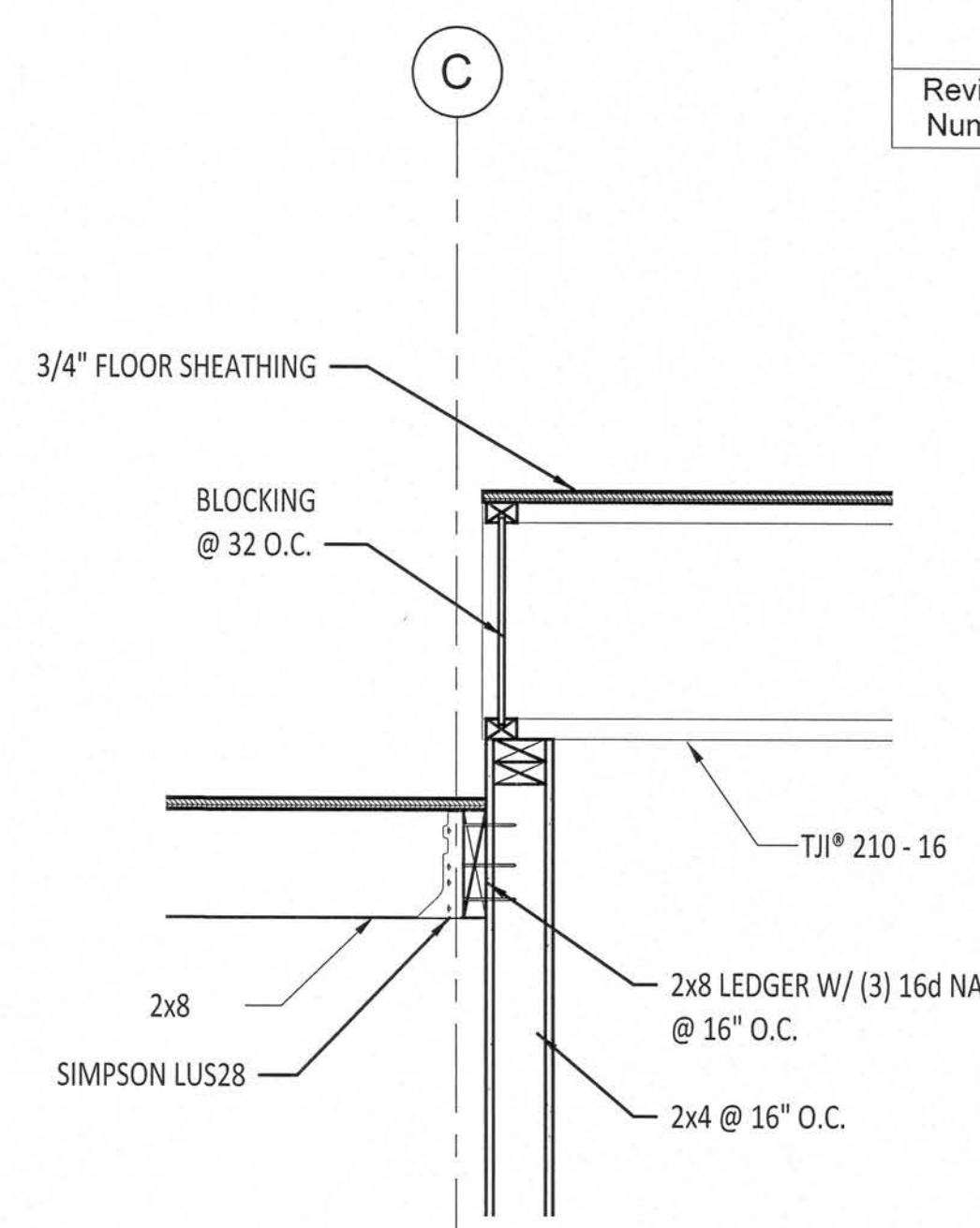
1 CEILING FRAMING PLAN
1/4" = 1'-0"



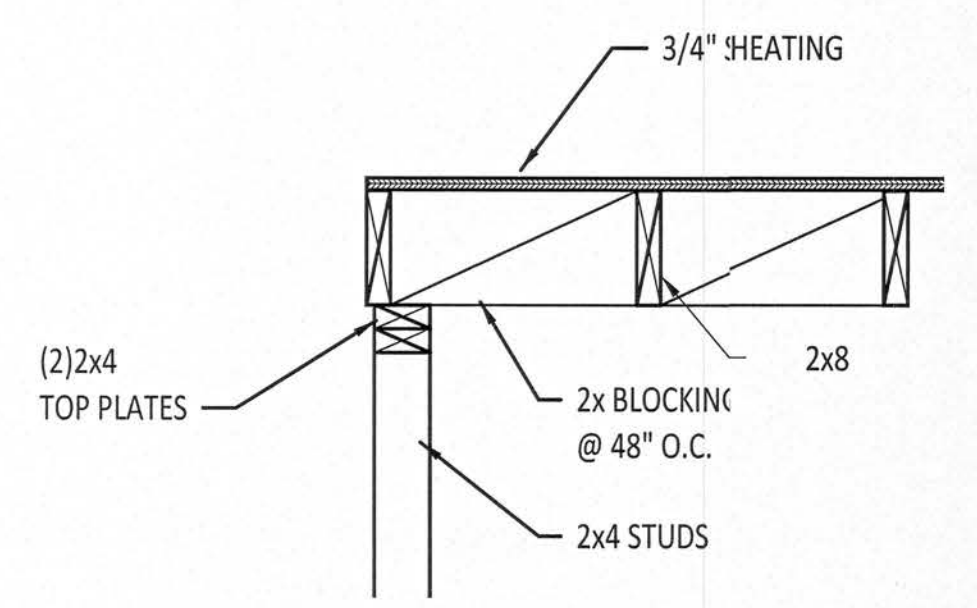
2 SECTION @ CEILING JOISTS
1/4" = 1'-0"



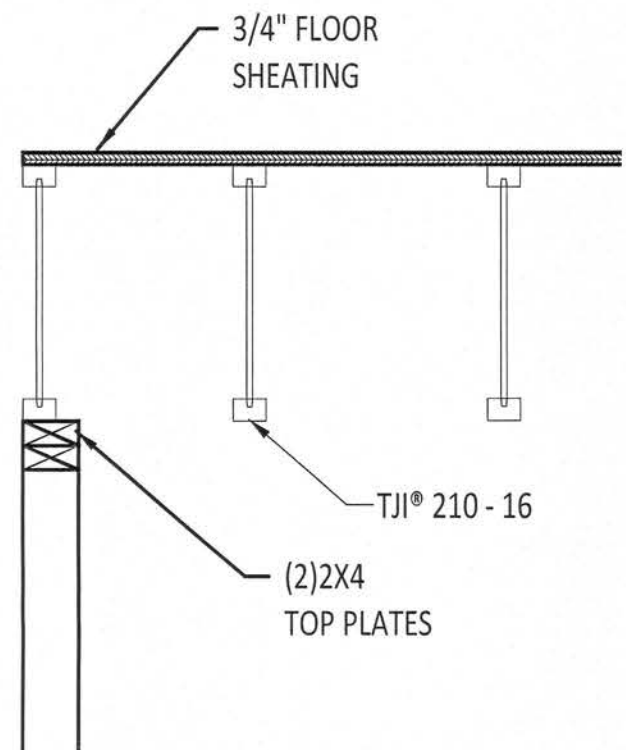
3 DETAIL - CEILING CONNECTION 1/5
1" = 1'-0"



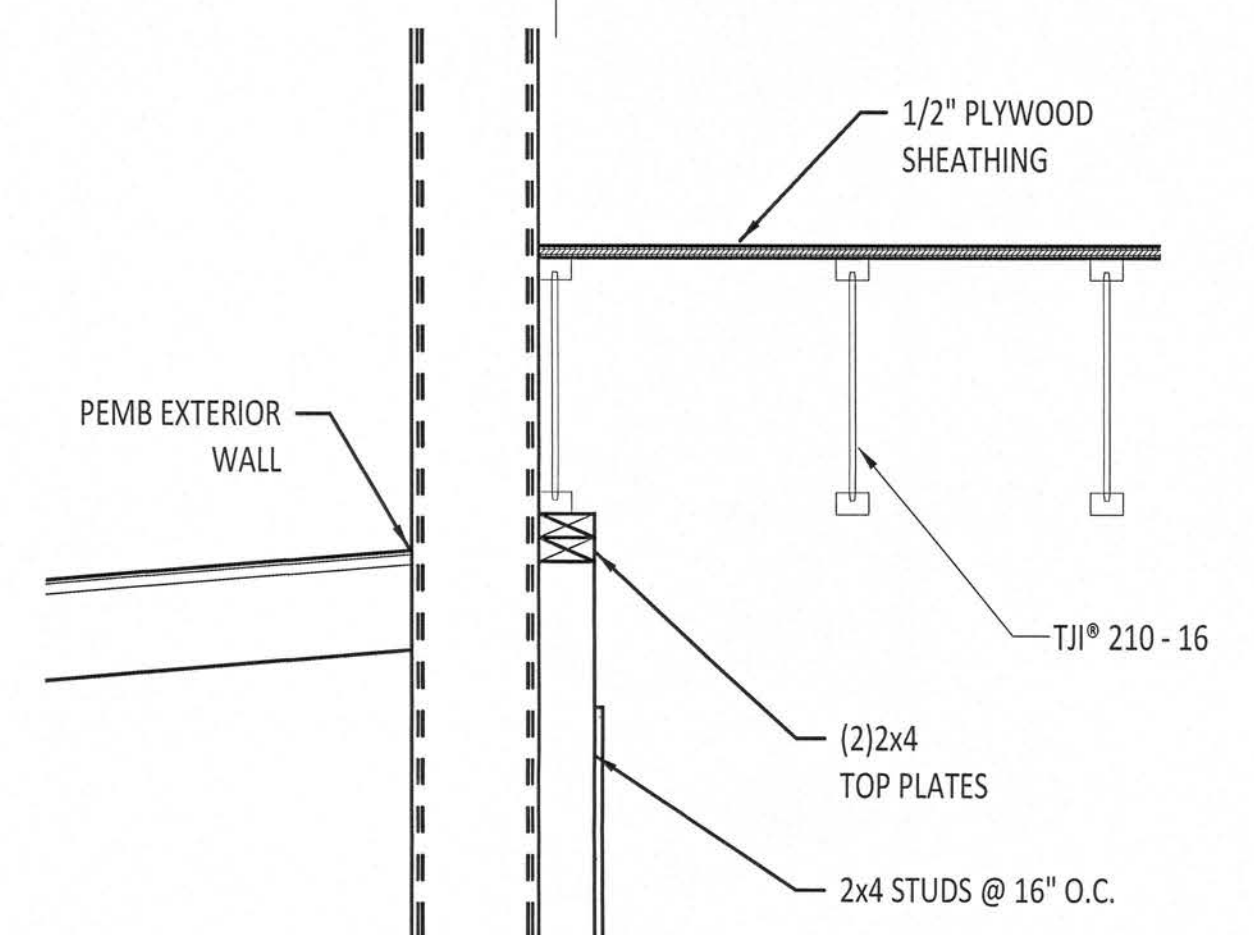
4 DETAIL - CEILING CONNECTION 2/5
1" = 1'-0"



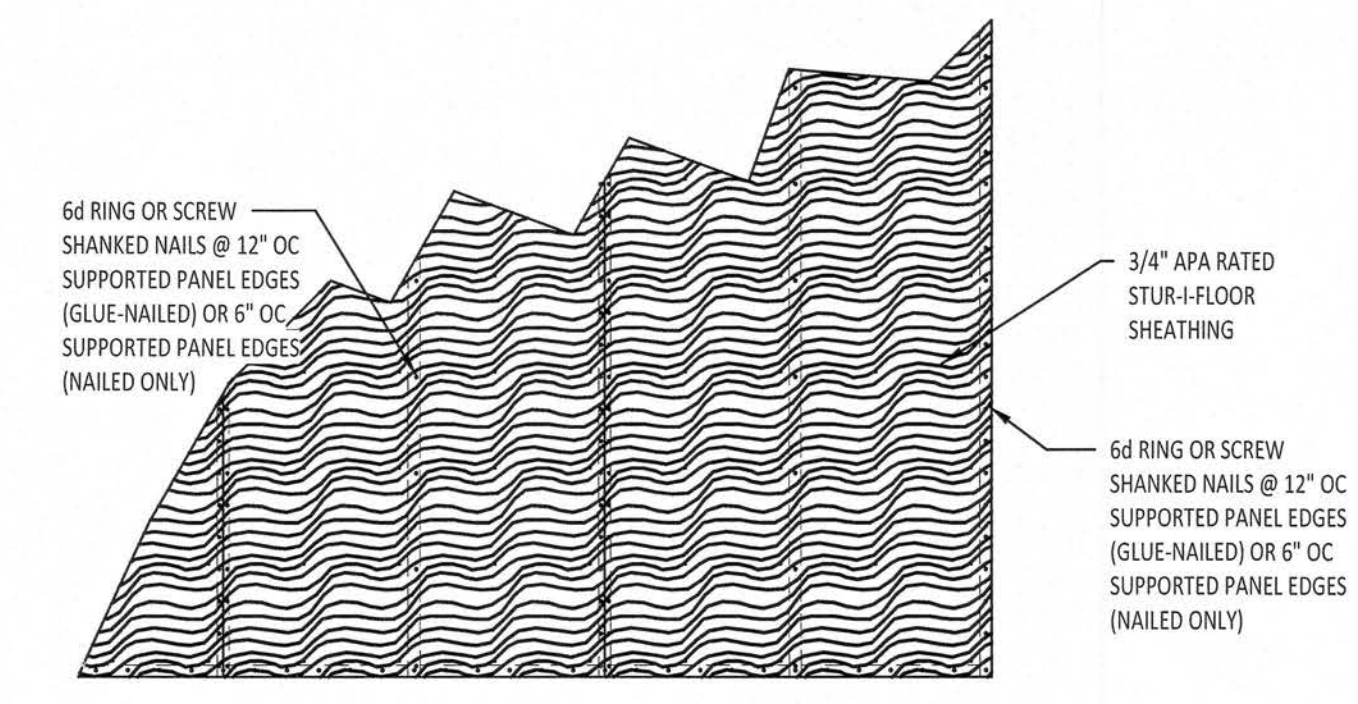
5 DETAIL - CEILING CONNECTION 3/5
1" = 1'-0"



6 DETAIL - CEILING CONNECTION 4/5
1" = 1'-0"



7 DETAIL - CEILING CONNECTION 5/5
1" = 1'-0"



8 FLOOR SHEATHING
1/2" = 1'-0"

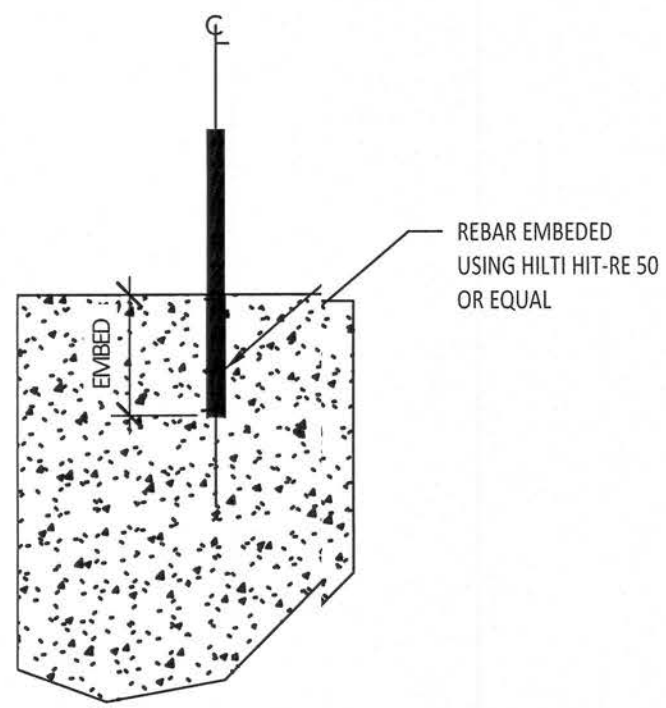
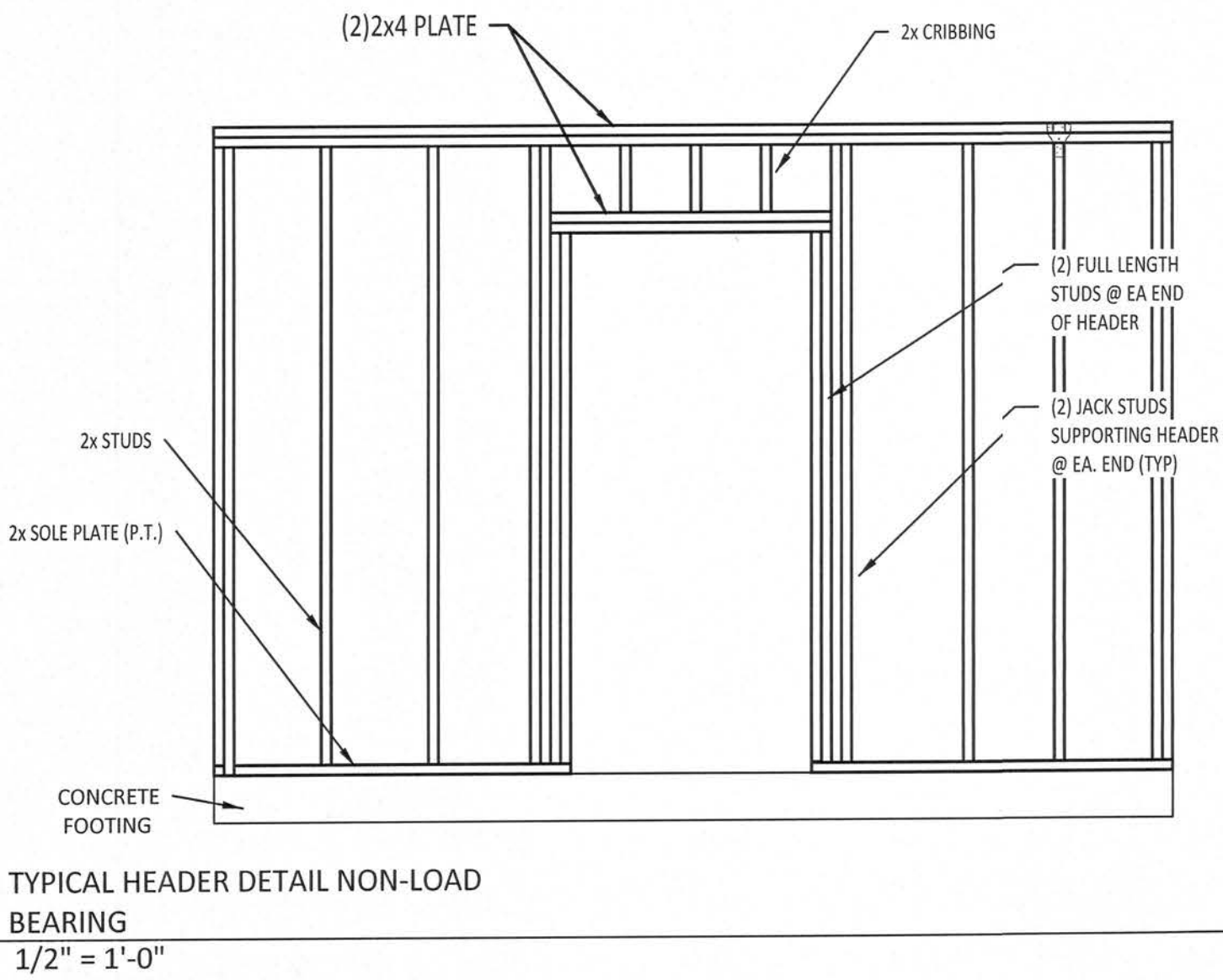
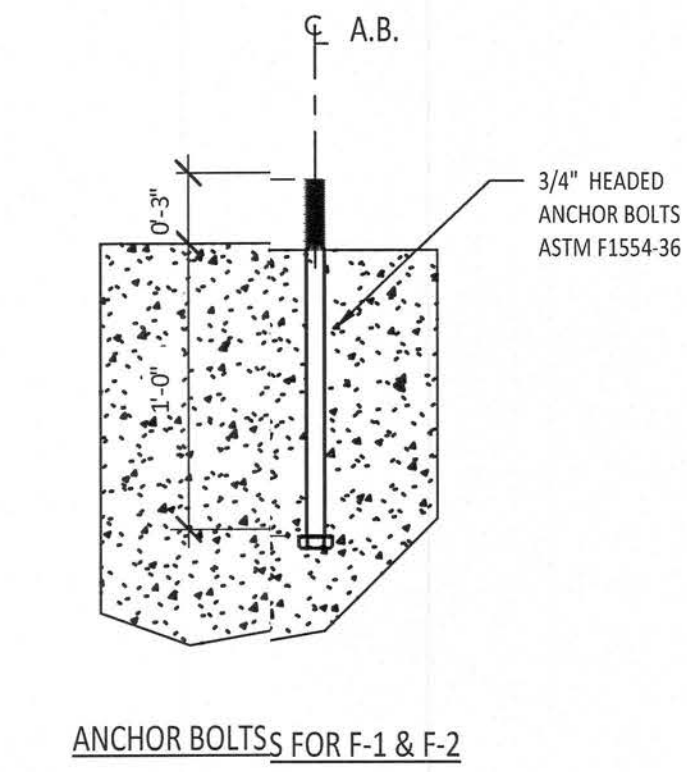
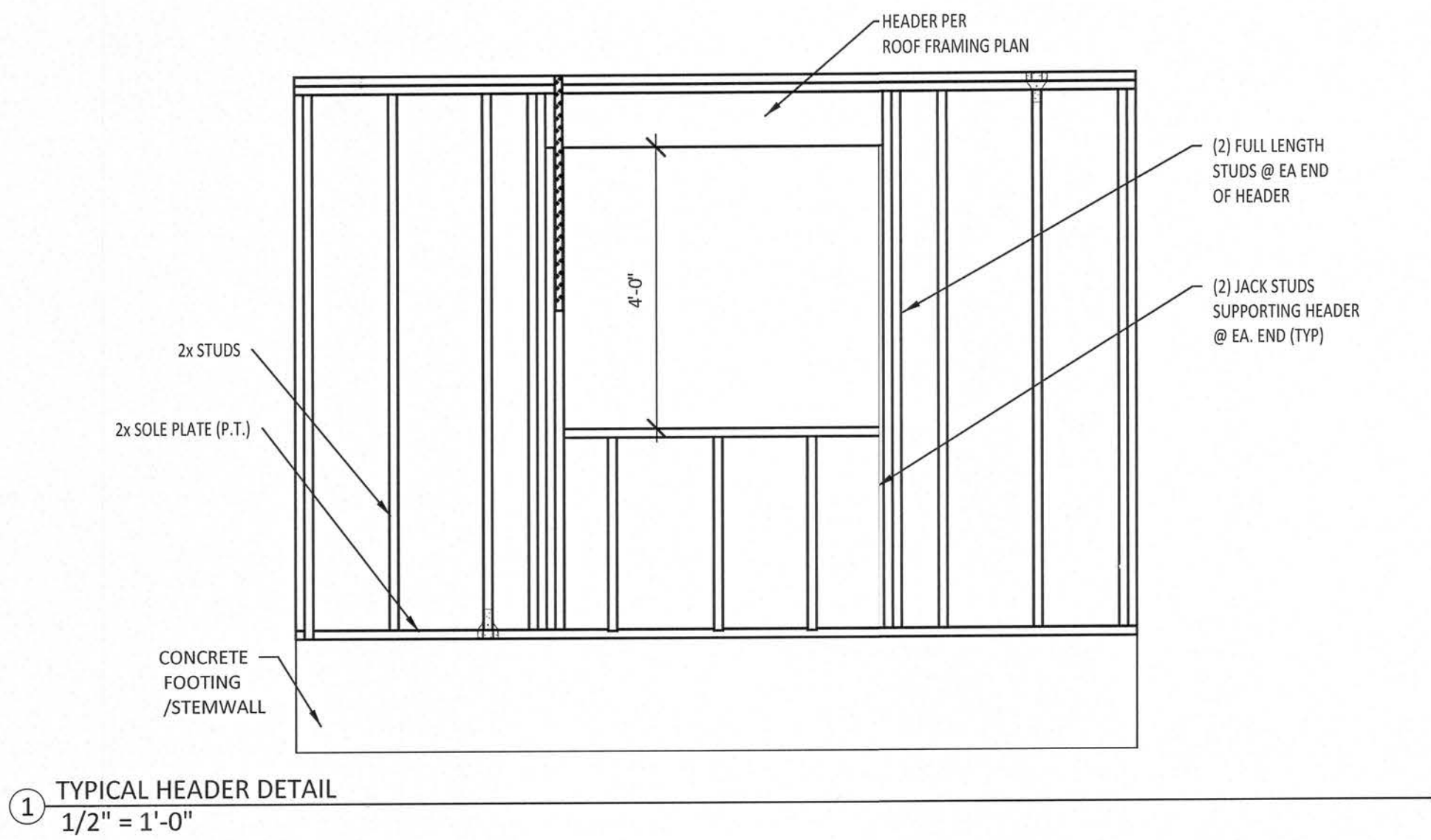
Revision Schedule		
Revision Number	Revision Description	Revision Date

1/22/2020 3:19:46 PM
email: ggill@gillengineering.com

GARY GILL
LICENSE
No 51942
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

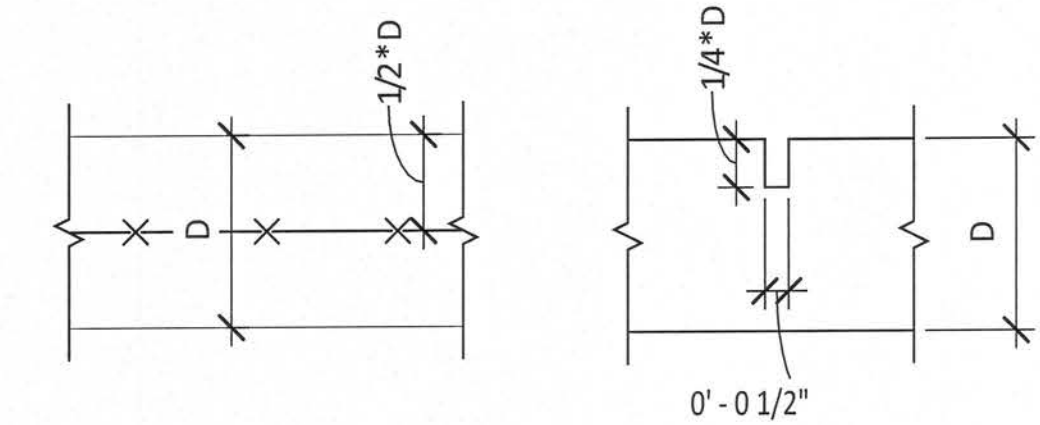
GRAHAM RESIDENCE COLUMBIA COUNTY, FL			GILL ENGINEERING SERVICES, INC AUTH # 30824 GARY GILL PE #51942 426 SW COMMERCE DR 13J-M LAKE CITY, FL 32025 386-90-1242	
COLUMBIA CONSTRUCTION & MAINTENANCE, INC			CEILING FRAMING PLAN	
DRAWN BY:	GG			
CHKD BY:	GG			
APPRD BY:	GG		1945-063 DWG #: S-004 REV #:	

Revision Schedule		
Revision Number	Revision Description	Revision Date

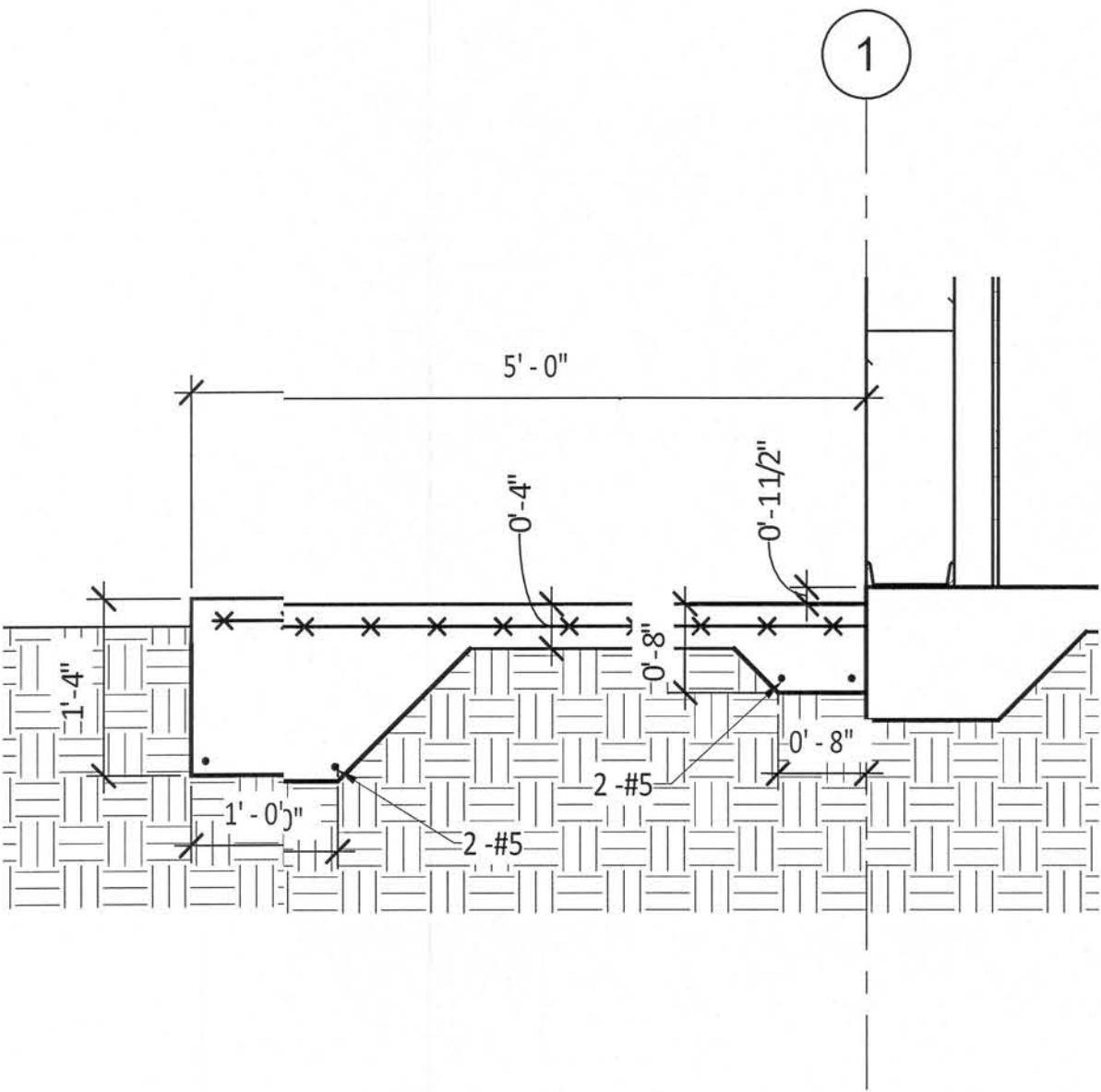


Tension Loads and Development Lengths for Rebar Dowels in Normal-Weight Concrete

Rebar Size No. (mm)	Drill Bit Dia. in.	Embed. Depth in. (mm)	f'c ≥ 2000 psi (13.8 MPa) Concrete			f'c ≥ 4000 psi (27.6 MPa) Concrete			ASTM A615 Grade 60 Rebar	
			Ultimate Bond Strength lbs. (kN)	Embed. To Develop Yield Strength in. (mm)	Embed. To Develop Tensile Strength in. (mm)	Ultimate Bond Strength lbs. (kN)	Embed. To Develop Yield Strength in. (mm)	Embed. To Develop Tensile Strength in. (mm)	Yield Strength lbs. (kN)	Tensile Strength lbs. (kN)
#4 (12.7)	3/8	4 1/4 (108)	16,400 (73.3)	4 1/4 (108)	5 1/4 (133)	18,320 (81.5)	4 1/4 (108)	4 1/4 (108)	12,000 (53.4)	18,000 (80.1)
#5 (15.9)	3/4	5 (127)	24,600 (109)	5 (127)	5 1/4 (146)	26,040 (116)	5 (127)	5 1/2 (140)	18,600 (82.7)	27,900 (124)
#6 (19.1)	7/8	6 1/4 (171)	38,380 (171)	6 1/4 (171)	7 (178)	40,500 (180)	6 1/4 (171)	6 1/4 (171)	26,400 (117)	39,600 (176)
#7 (22.2)	1	7 1/4 (197)	47,760 (212)	7 1/4 (197)	8 1/4 (222)	47,760 (212)	7 1/4 (197)	8 1/4 (222)	36,000 (160)	54,000 (240)



SLAB THICKNESS, IN.	LESS THAN 3/4-INCH AGGREGATE; SPACING, FT	LARGER THAN 3/4-INCH AGGREGATE; SPACING, FT	SLUMP LESS THAN 4 INCHES; SPACING, FT
4	8	10	2
5	10	13	5
6	12	15	8



1/22/2020 3:19:47 PM
email: ggill@gillengineering.com

Gill
Engineering Services, Inc.



GRAHAM RESIDENCE			COLUMBIA COUNTY, FL		
COLUMBIA CONSTRUCTION & MAINTENANCE, INC					
DRAWN BY:	Author		GILL ENGINEERING SERVICES, INC		
CHKD BY:	Checker		AUTH # 30824		
APPRD BY:	Approver		GARY GILL PE #51942		
			426 SW COMMERCIAL DR 130-M		
			LAKE CITY, FL 32025 386-590-1242		
MISC DETAILS					
1945-063		DWG #:		S-005	
				REV #:	

GENERAL NOTES

FABRICATION SHALL BE IN ACCORDANCE WITH ELITE STRUCTURES STANDARD PRACTICES AND IN COMPLIANCE WITH THE APPLICABLE SECTIONS, RELATING TO DESIGN REQUIREMENT AND ALLOWABLE STRESSES OF THE LATEST EDITION OF THE AWS STRUCTURAL WELDING CODE D 1.1 AND D1.3

MATERIALS	ASTM DESIGNATION	MIN. YIELD STRENGTH
HOT ROLLED STEEL SHAPES	A572	FY = 50 KSI
STEEL PIPES	A500	FY = 42 KSI
STRUCTURAL TUBING	A500	FY = 45 KSI
STRUCTURAL STEEL WEB PLATE	A572/A1011	FY = 50 KSI
STRUCTURAL STEEL FLANGE PLATES/BARS	A589/A572	FY = 50 KSI
COLD FORM LIGHT GAUGE	A653/A1011	FY = 50, 55 KSI
ROOF AND WALL SHEETS	A792/A603	FY = 50, 60 KSI
CABLE BRACE	A475-TYPE1	EXTRA HIGH STRENGTH
ROD BRACE	A36	FY = 36 KSI
HULL SECTIONS	A36	FY = 36 KSI
		MIN. TENSILE STRENGTH
		FU = 60 KSI
MACHINE BOLTS & NUTS	A307	FU = 120 KSI
HIGH STRENGTH BOLTS (1" DIA. & LESS)	A325 - TYPE 1	FU = 105 KSI
HIGH STRENGTH BOLTS (1" TO 1-1/2")	A325 - TYPE 1	FU = 105 KSI
ANCHOR BOLTS (IF SUPPLIED)	F1554 - GR55	

CLOSURE STRIPS ARE FURNISHED ONLY IF NOTED ON SHIPPING DOCUMENTS
INSIDE - UNDER ROOF PANELS AT EAVE
OUTSIDE - BETWEEN END WALL PANELS AND RAKE TRIM
UNDER CONTINUOUS RIDGE VENT SKIRTS

ERECTION NOTE:
ALL BRACING, STRAPPING, & BRIDGING SHOWN AND PROVIDED BY ELITE STRUCTURES FOR THIS BUILDING IS REQUIRED AND SHALL BE INSTALLED BY THE ERECTOR AS A PERMANENT PART OF THE STRUCTURE. IF ADDITIONAL BRACING IS REQUIRED FOR STABILITY DURING ERECTION, IT SHALL BE THE ERECTOR'S RESPONSIBILITY TO DETERMINE THE AMOUNT OF SUCH BRACING AND TO PROVIDE AND INSTALL AS NEEDED.

ERECTION AND UNLOADING NOT BY ELITE STRUCTURES

SHORTAGES

ANY CLAIMS OR SHORTAGES BY BUYERS MUST BE MADE TO ELITE STRUCTURES AT TIME OF DELIVERY, OR SUCH CLAIMS SHALL BE CONSIDERED WAIVED BY THE CUSTOMER AND DISALLOWED IF MATERIAL IS SHOWN DELIVERED

CORRECTIONS OF ERRORS AND REPAIRS (MBMA 6.10)

CLAIMS FOR CORRECTIONS OF ALLEGED MISFITS WILL BE DISALLOWED UNLESS ELITE STRUCTURES SHALL HAVE RECEIVED PRIOR NOTICE THEREOF AND ALLOWED REASONABLE INSPECTION OF SUCH MISFITS. THE CORRECTION OF MINOR MISFITS BY THE USE OF DRIFT PINS TO DRY THE COMPONENTS INTO LINE, MODERATE AMOUNTS OF REAMING, CHIPPING AND CUTTING, AND THE REPLACEMENT OF MINOR SHORTAGES OF MATERIAL ARE A NORMAL PART OF ERECTION AND ARE NOT SUBJECT TO CLAIM.

A-325 BOLT TIGHTENING REQUIREMENTS

ALL BOLTED JOINTS WITH A325 TYPE 1 BOLTS ARE SPECIFIED AS UNOBTAINED JOINTS IN ACCORDANCE WITH THE SPECIFICATION FOR STRUCTURAL JOINTS USING A325 OR A490 BOLTS, LINE 36.2004, PRETENSIONING METHOD, INCLUDING TURN-OF-NUT CALIBRATED WRENCH/TWIST OFF TYPE TENSION CONTROL BOLTS OR DIRECT TENSION INDICATOR ARE NOT REQUIRED. INSTALLATION INSPECTION REQUIREMENTS FOR SINGLE TIGHT SPECIFICATION FOR STRUCTURAL JOINTS 5.13 IS SUGGESTED.

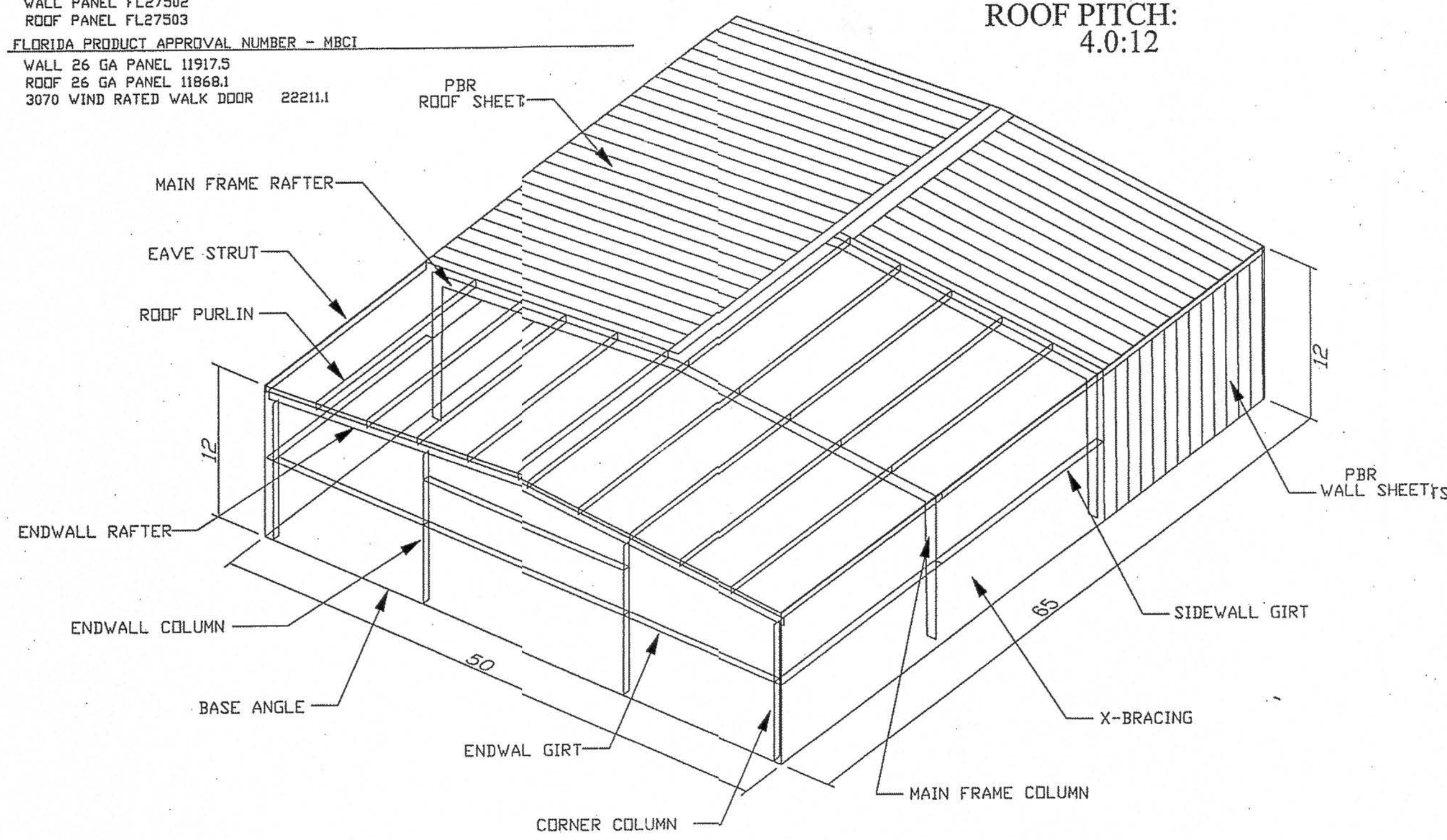
REFERENCE: STEEL CONSTRUCTION MANUAL
AMERICAN INSTITUTE OF STEEL CONSTRUCTION INC.
THIRTEENTH EDITION (AISC)

FLORIDA PRODUCT APPROVAL NUMBER - ELITE STRUCTURES 26 GAUGE PBR PANEL

WALL PANEL FL27502
ROOF PANEL FL27503

FLORIDA PRODUCT APPROVAL NUMBER - MBCT

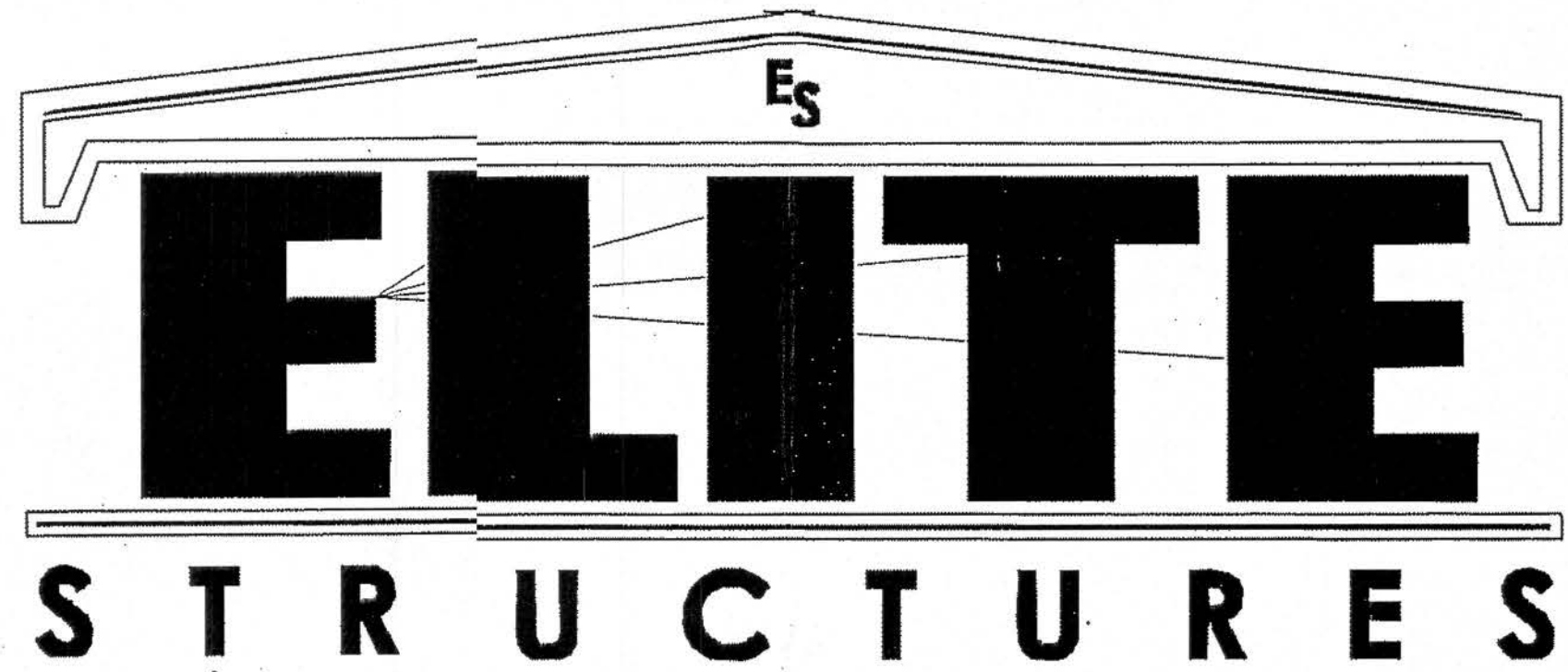
WALL 26 GA PANEL 11917.5
ROOF 26 GA PANEL 11868.1
3070 WIND RATED WALK DOOR 2221.1



BUILDING DRAWING FOR ILLUSTRATION ONLY



401 OLD QUITMAN RD.
ADEL, GA. 31620
229-896-7569



ELITE-STRUCTURES.COM

CONSTRUCTION MANUAL AVAILABLE ONLINE
WWW.ELITE-STRUCTURES.COM

PROJECT:
JODY DUPREE
REF:SON

PROJECT MANAGER
LEE WHITEHURST

LOCATION:
LAKE CITY, FL.

DATE: 1/6/2020

BUYER/CUSTOMER RESPONSIBILITIES

APPROVAL OF ELITE STRUCTURES DRAWINGS AND CALCULATIONS INDICATES THAT ELITE STRUCTURES HAS CORRECTLY INTERPRETED AND APPLIED THE CONTRACT DOCUMENTS. THIS APPROVAL CONSTITUTES THE CONTRACTOR/OWNER ACCEPTANCE OF THE ELITE STRUCTURES DESIGN CONCEPTS, ASSUMPTIONS, AND LOADS.

ONCE THE BUYER/END USER CUSTOMER HAS SIGNED ELITE STRUCTURES APPROVAL PACKAGE AND THE PROJECT IS RELEASED FOR FABRICATION, CHANGES SHALL BE BILLED TO THE BUYER/END USER CUSTOMER INCLUDING MATERIAL, ENGINEERING AND OTHER COSTS. AN ADDITIONAL FEE MAY BE CHARGED IF THE PROJECT MUST BE MOVED FROM THE FABRICATION AND SHIPPING SCHEDULE.

THE BUYER/END USER CUSTOMER IS RESPONSIBLE FOR OVERALL PROJECT COORDINATION, ALL INTERFACE, COMPATIBILITY AND DESIGN CONSIDERATIONS CONCERNING ANY MATERIAL NOT FURNISHED BY ELITE STRUCTURES ARE TO BE CONSIDERED AND COORDINATED BY THE BUYER/END USER CUSTOMER. SPECIFIC DESIGN CRITERIA CONCERNING THIS INTERFACE BETWEEN MATERIALS MUST BE FURNISHED BEFORE RELEASE FOR FABRICATION OR ELITE STRUCTURES ASSUMPTIONS WILL GOVERN SECTION 4 AND COMMENTARY, AISC CODE OF STANDARD 9TH EDITION.

IT IS THE RESPONSIBILITY OF THE BUYER/END CUSTOMER TO INSURE THAT ELITE STRUCTURES PLANS COMPLIES WITH THE APPLICABLE REQUIREMENTS OF ANY GOVERNING BUILDING AUTHORITIES. THE SUPPLYING OF SEALED ENGINEERING DATA AND DRAWINGS FOR THE METAL BUILDING SYSTEM DOES NOT IMPLY OR CONSTITUTE AN AGREEMENT THAT ELITE STRUCTURES OR ITS DESIGN ENGINEERS ARE ACTING AS THE ENGINEER OF RECORD OR DESIGN PROFESSIONAL FOR A CONSTRUCTION PROJECT. THESE DRAWINGS ARE SEALED ONLY TO CERTIFY THE DESIGN OF THE STRUCTURAL COMPONENTS FURNISHED BY ELITE STRUCTURES.

THE BUYER/END CUSTOMER IS RESPONSIBLE FOR SETTING OF ANCHOR BOLTS AND ERECTION OF THE STEEL IN ACCORDANCE WITH ELITE STRUCTURES CONSTRUCTION DRAWINGS ONLY. TEMPORARY SUPPORTS SUCH AS GUYS, BRACES, FALSEWORK, CRIBBING OR OTHER ELEMENTS REQUIRED FOR ERECTION OPERATION SHALL BE DETERMINED, FURNISHED AND INSTALLED BY ERECTOR. NO ITEMS SHOULD BE PURCHASED FROM A PRELIMINARY SET OF DRAWINGS, INCLUDING ANCHOR BOLTS, USE ONLY FINAL CONSTRUCTION DRAWINGS FOR THE USE.

ELITE STRUCTURES IS RESPONSIBLE FOR THE DESIGN OF THE ANCHOR BOLTS TO PERMIT THE TRANSFER OF FORCES BETWEEN THE BASE PLATE AND ANCHOR BOLT IN SHEAR, BEARING AND TENSION, BUT IS NOT RESPONSIBLE FOR THE TRANSFER OF ANCHOR BOLT FORCES TO THE CONCRETE OR THE ADEQUACY OF THE ANCHOR BOLT IN RELATION TO THE CONCRETE.

UNLESS OTHERWISE PROVIDED IN THE ORDER DOCUMENT, ELITE STRUCTURES DOES NOT DESIGN AND IS NOT RESPONSIBLE FOR THE DESIGN, MATERIAL AND CONSTRUCTION OF THE FOUNDATION OR FOUNDATION EMBELEMMENT. THE END USER CUSTOMER SHOULD ASSURE HIMSELF THAT ADEQUATE PROVISIONS ARE MADE IN THE FOUNDATION DESIGN FOR LOADS IMPOSED BY COLUMN REACTIONS OF THE BUILDING, OTHER IMPOSED LOADS, AND BEARING CAPACITY OF THE SOIL AND OTHER CONDITIONS OF THE BUILDING SITE.

NORMAL ERECTION OPERATIONS INCLUDE THE CORRECTIONS OF MINOR MISFITS BY MODERATE AMOUNTS OF REAMING, CHIPPING, WELDING OR CUTTING, AND THE DRAWING OF ELEMENTS INTO LINE THROUGH THE USE OF DRIFT PINS. ERRORS WHICH CANNOT BE CORRECTED BY FOREGOING MEANS OR WHICH REQUIRE MAJOR CHANGES IN MEMBERS CONFIGURATION ARE TO REPORT IMMEDIATELY TO ELITE STRUCTURES BY THE BUYER/END USER CUSTOMER. TO ENSURE WHOMEVER IS RESPONSIBLE EITHER TO CORRECT THE ERROR OR TO APPROVE THE MOST EFFICIENT AND ECONOMIC METHOD OF CORRECTION TO BE USED BY OTHERS.

SECTION 7 AISC CODE OF STANDARD 9TH EDITION

BUILDING DESCRIPTION:
50 x 65 x 12

LOCAL BUILDING CODE:
ENCLOSURE TYPE:
BUILDING RISK CATAGORY
LOAD REDUCTION USED:
LIVE LOAD TO FRAMES:
LIVE LOAD TO PURLINS:
DEAD LOAD:
COLLATERAL LOAD:
SNOW LOAD (GROUND):
SNOW LOAD (ROOF):
WIND SPEED:
WIND EXPOSURE:
SEISMIC ZONE:
SEISMIC IMPORTANCE:
SPECTRAL RESPONSE Ss
SPECTRAL RESPONSE S1
RAIN INTENSITY: 5 yr.
25 yr.

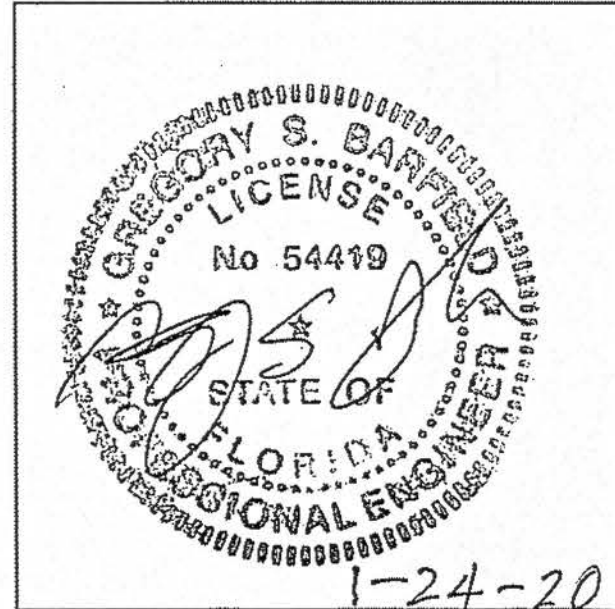
FBC 17
Closed
II - Normal
Yes
12
20.00
2.500
0
0
0
130
B
B
1.00
0.10
0.05
10.0000
11.0000



Gregory S. Barfield, P.E.
2149 Neil Purvis Road
Adel, GA 31620
P.E. # 54419

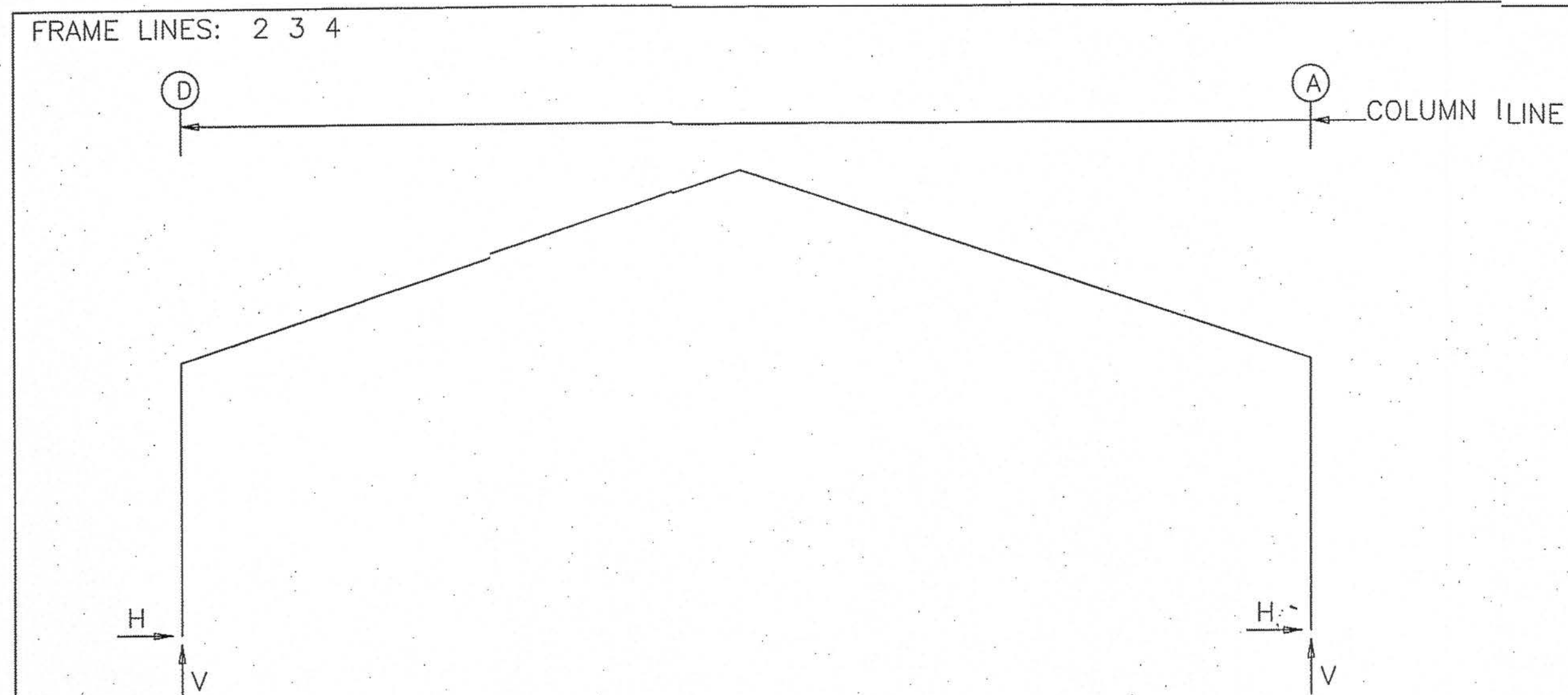
ENGINEERING SEAL

THIS CERTIFICATION COVERS PARTS MANUFACTURED AND DELIVERED BY ELITE STRUCTURES ONLY



SEALING OF THESE DRAWINGS DOES NOT IMPLY OR CONSTITUTE THAT ELITE STRUCTURES ENGINEER IS THE ENGINEER OF RECORD OR THE DESIGN PROFESSIONAL FOR THE THIS PROJECT. ONLY THE DESIGN OF THE METAL BUILDING SYSTEM AS FURNISHED BY ELITE STRUCTURES IS INCLUDED. FOUNDATIONS ANALYSIS, ELECTRICAL, AND MECHANICAL SYSTEMS, AND/OR OTHER PARTS SUPPLIED BY ANYONE OTHER THAN ELITE STRUCTURES ARE SPECIFICALLY EXCLUDED, NO INSPECTION OR SUPERVISION IS IMPLIED.

DESIGN SPECIFICATIONS
FOR
JOB ID: 11456



RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k)						Bolt(in) Qty Dia		Base_Plate(in)			Growt (in)
		Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin			Width	Length	Thick	
2*	D	1	4.0	11.3	2	-3.5	-8.1	4	0.750	6.000	11.00	0.500	0.0
2*	A	3	3.8	-6.0	1	-4.0	8.7	4	0.750	6.000	11.00	0.500	0.0
		1	-4.0	8.7	4	2.6	-8.5						
2*	Frame lines: 2 3 4												

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	Dead		Live		Wind_Left1		Wind_Right1		Wind_Left2		Wind_Right2	
		Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
2*	D	0.9	2.5	3.1	8.9	-6.8	-16.0	-1.3	-10.8	-5.9	-10.8	-0.5	-5.6
2*	A	-0.9	2.1	-3.1	6.5	1.2	-9.3	7.3	-12.1	0.4	-4.1	6.4	-6.9
Frame Line	Column Line	Wind_Long1		Wind_Long2		Seismic_Left		Seismic_Right		Seismic_Long			
		Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert		
2*	D	-5.3	-12.7	1.0	-7.7	-0.1	-0.1	0.1	0.1	-0.4	-0.2		
2*	A	-1.0	-11.3	5.3	-16.3	-0.1	0.1	0.1	-0.1	-0.4	-0.1		
2*	Frame lines: 2 3 4												

ENDWALL COLUMN: BASIC COLUMN REACTIONS (k)

Frm Line	Col Line	Dead		Live		Wind Left1	Wind Right1	Wind Left2	Wind Right2	Wind_Press		Wind_Suct		Wind Long1
		Horz	Vert	Horz	Vert	Vert	Vert	Vert	Vert	Horz	Vert	Horz	Vert	Vert
1	D	-0.1	0.4	-0.1	2.1	-0.9	-0.7	-0.2	0.0	-0.3	1.2	0.8	-3.2	-0.9
1	C	-0.1	1.3	-0.1	6.8	-6.3	-4.7	-4.7	-3.1	-3.7	1.9	4.7	-5.0	-6.2
1	B	0.0	1.2	-0.1	6.3	-4.7	-6.3	-3.1	-4.7	-3.6	1.4	4.5	-3.9	-4.1
1	A	0.0	0.2	0.1	0.6	-0.7	-0.9	0.0	-0.2	0.0	0.0	0.0	0.0	-0.6
Frm Line	Col Line	Wind Long2	Seis Left	Seis Right										
		Vert	Vert	Vert										
1	D	-0.6	0.0	0.2										
1	C	-4.1	-0.1	-0.1										
1	B	-6.2	-0.1	-0.1										
1	A	-0.9	0.2	0.0										

ENDWALL COLUMN: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k)						Bolt(in) Qty	Dia	Base_Plate(in)			Grosut (in)
		Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin			Width	Length	Thick	
1	D	5 1	0.4 -0.4	-2.2 2.5	1 5	-0.4 0.4	2.5 -2.2	4	0.750	6.000	7.875	0.375	0.0
1	C	5 1	2.8 -0.5	-6.0 8.2	6 5	-2.3 2.8	-1.3 -6.0	4	0.750	6.000	7.875	0.375	0.0
1	B	7 1	2.6 -0.4	-5.4 7.6	8 7	-2.2 2.6	-1.6 -5.4	4	0.750	6.000	7.875	0.375	0.0
1	A	4 1	0.0 0.0	-0.4 0.8	4	0.0	-0.4	4	0.750	6.000	7.875	0.375	0.0

BUILDING BRACING REACTIONS

Wall Loc	Col Line	Col Line	± Reactions(k)				Panel Shear (lb/ft)		Note
			Wind Horz	Wind Vert	Seismic Horz	Seismic Vert	Wind	Seis	
L_EW	1						42	11	
F_SW	A	3,4	4.2	3.6	0.3	0.3			(b)
R_EW	4								(h)
B_SW	D	Torsional Bracing Used							

(b) Wind bent in bay, base above finish floor
(h) Rigid frame at endwall

NOTES FOR REACTIONS

Building reactions are based on the following building data:

Width (ft)	=	50.0
Length (ft)	=	65.0
Eave Height (ft)	=	12.0/ 12.0
Roof Slope (rise/12)	=	4.0/ 4.0
Dead Load (psf)	=	2.5
Collateral Load (psf)	=	0.0
Roof Live Load (psf)	=	20.0
Frame Live Load (psf)	=	12.0
Wind Speed (mph)	=	130.0
Wind Code	=	FBC 17 (IBC 15)
Exposure	=	B
Closed/Open	=	C
Importance Wind	=	1.00
Importance Seismic	=	1.00
Seismic Zone	=	B
Seismic Coeff (Fa*Ss)	=	0.16

ID Description

- 1 Dead+Collateral+Live
- 2 0.6Dead+0.6Wind_Left1
- 3 0.6Dead+0.6Wind_Right1
- 4 0.6Dead+0.6Wind_Long2L
- 5 0.6Dead+0.6Wind_Left1+0.6Wind_Suction
- 6 Dead+0.6Wind_Pressure+0.6Wind_Long1L
- 7 0.6Dead+0.6Wind_Right1+0.6Wind_Suction
- 8 Dead+0.6Wind_Pressure+0.6Wind_Long2L

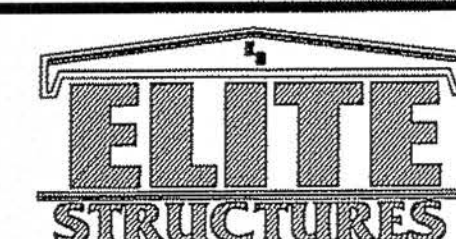
ANCHOR BOLT SUMMARY

Qty	Locate	Dia (in)	Type	Proj (in)
○ 60	Jamb	1/2"	GR36	1.50
⊗ 16	Endwall	3/4"	GR36	2.50
⊗ 24	Frame	3/4"	GR36	2.50

Gregory S. Barfield, P.E.
2149 Nell Purvis Road
Adel, GA 31620
P.E. # 54419

DESIGN CRITERIA

BUILDING CODE:	FBC 17
LIVE LOAD TO FRAMES:	12 PSF
LIVE LOAD TO PURLINS:	20 PSF
COLLATERAL LOAD:	0 PSF
SNOW LOAD (GROUND):	0 PSF
WIND LOAD (ULTIMATE):	130 MPH
WIND IMPORTANCE FACTOR:	1
BUILDING RISK CATEGORY:	II
ENCLOSURE CLASSIFICATION:	ENCLOSED



PH 229-896-7569
FAX 229-896-7560
P.O. BOX 207
ADEL, GA 31620

SCALE: N.T.S.	DATE: 1/6/2020	REVISION
DRAWN BY: B.BROWN	SALES: L.WHITEHURST	DATE: 1/13/2020
BUILDING SIZE: 50' X 65' X 12'		REVISED

TITLE/LOCATION

JODY DUPREE
REF: SON
LAKE CITY, FL

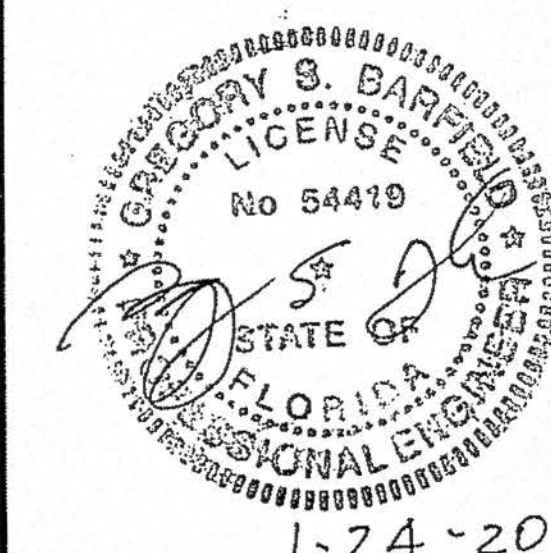
PLAN:

COLUMN REACTIONS

JOB #

11456-CR

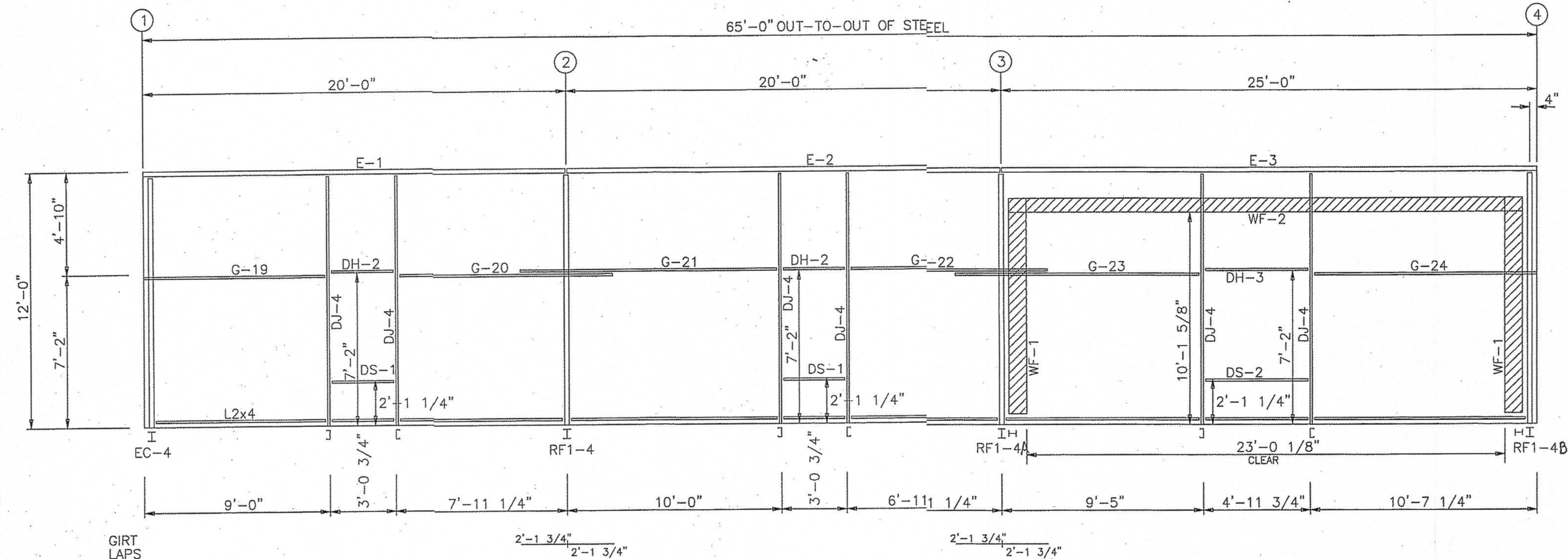
SHEET 2 OF 11



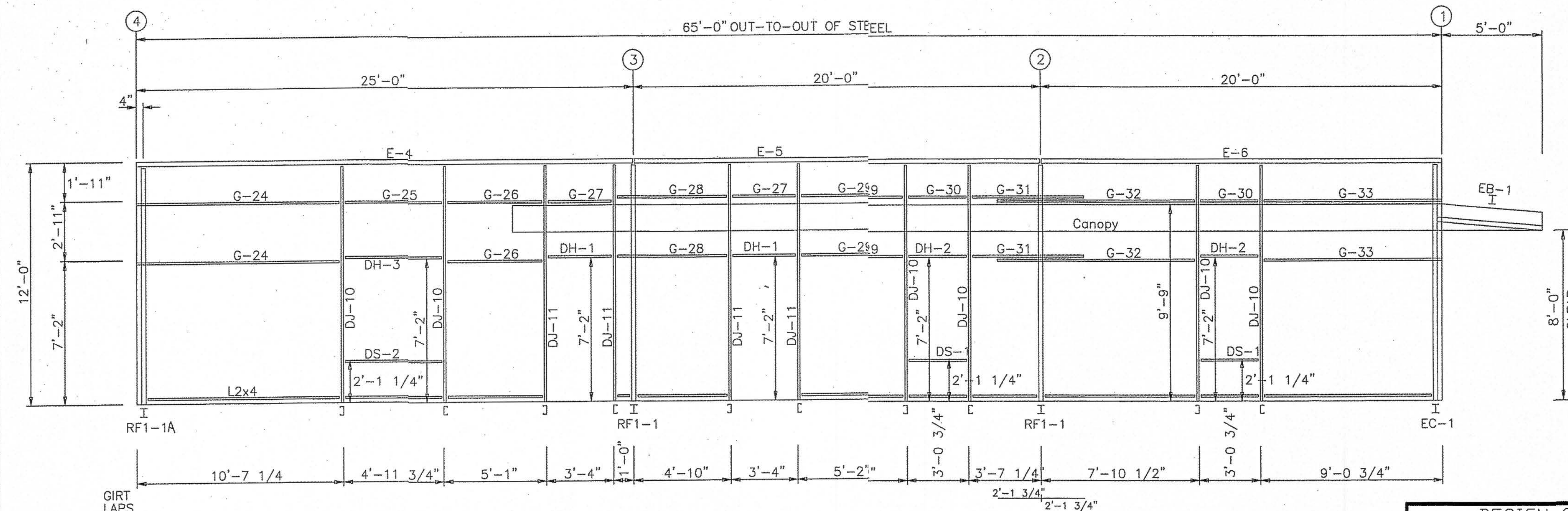
1-24-20

BOLT TABLE FRAME LINE A & D				
LOCATION	QUAN	TYPE	DIA	LENGTH
WF-1 - WF-2	8	A325	3/4"	3"
WF-1 - RF1-4	10	A325	3/4"	3"

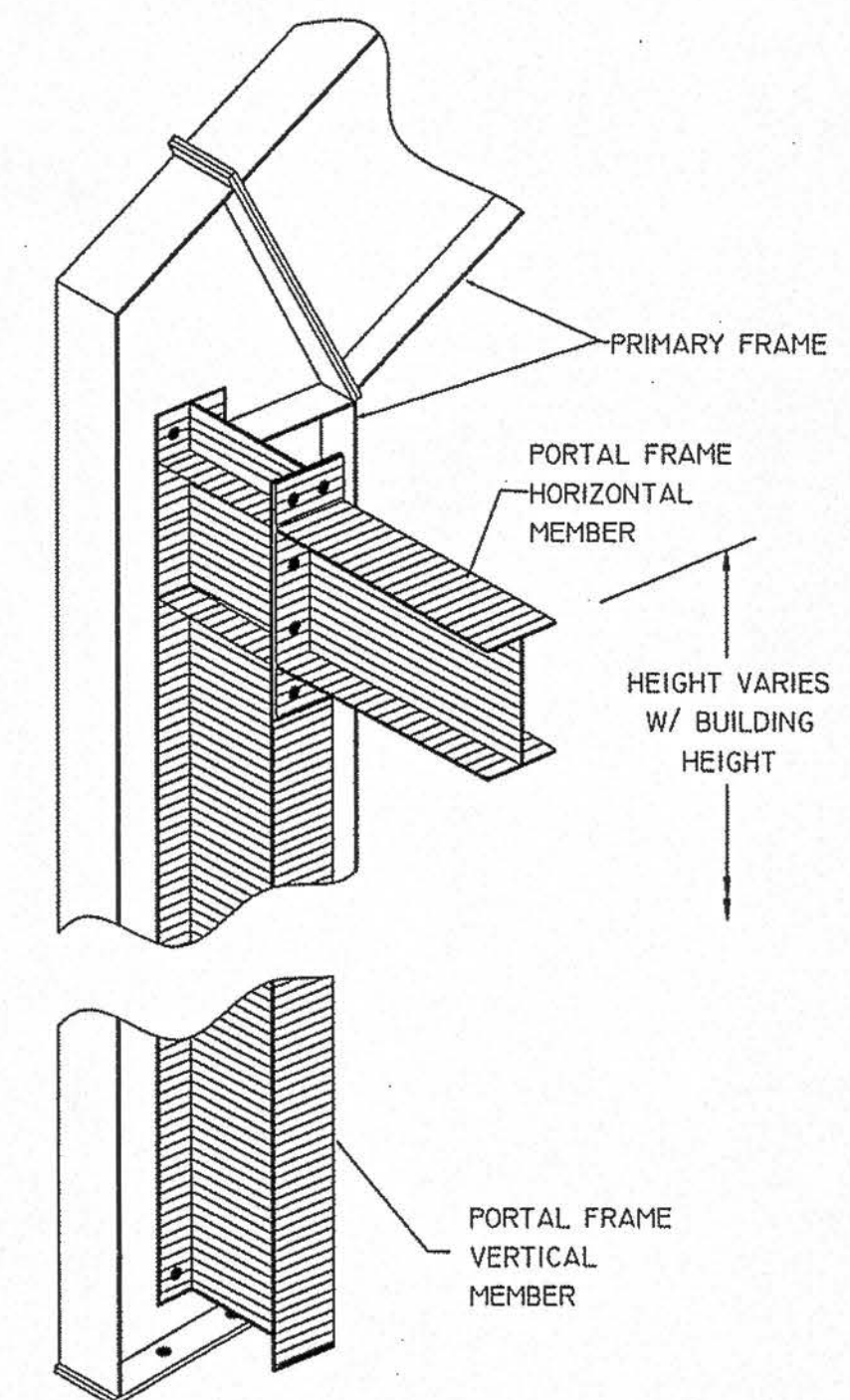
MEMBER TABLE FRAME LINE A & D		
MARK	PART	LENGTH
WF-1	W10X12	10'-3 9/16"
WF-2	W8X24	22'-11 1/8"
EB-1	W8X10	5'-7 7/8"
DJ-4	8X25C14	11'-6 1/4"
DJ-10	8X25C14	11'-6 1/4"
DJ-11	8X25C14	11'-6 1/4"
DH-1	8X25C14	3'-3 1/2"
DH-2	8X25C14	3'-0 1/4"
DH-3	8X25C14	4'-11 1/4"
DS-1	8X25C14	3'-0 1/4"
DS-2	8X25C14	4'-11 1/4"
E-1	8E214L4	19'-11 1/2"
E-2	8E214L4	19'-11 1/2"
E-3	8E214L4	24'-11 1/2"
E-4	8E214L4	24'-11 1/2"
E-5	8E214L4	19'-11 1/2"
E-6	8E214L4	19'-11 1/2"
G-19	8X25Z16	8'-9"
G-20	8X25Z16	9'-10 1/4"
G-21	8X25Z16	11'-11"
G-22	8X25Z16	8'-10 1/4"
G-23	8X25Z16	11'-4"
G-24	8X25Z16	10'-4 1/4"
G-25	8X25Z16	4'-11 1/4"
G-26	8X25Z16	4'-7 1/2"
G-27	8X25Z16	3'-3 1/2"
G-28	8X25Z16	5'-4 1/2"
G-29	8X25Z16	4'-8 1/2"
G-30	8X25Z16	3'-0 1/4"
G-31	8X25Z16	5'-6 1/4"
G-32	8X25Z16	9'-9 1/2"
G-33	8X25Z16	8'-9 3/4"



SIDEWALL FRAMING: FRAME LINE A



SIDEWALL FRAMING: FRAME LINE D

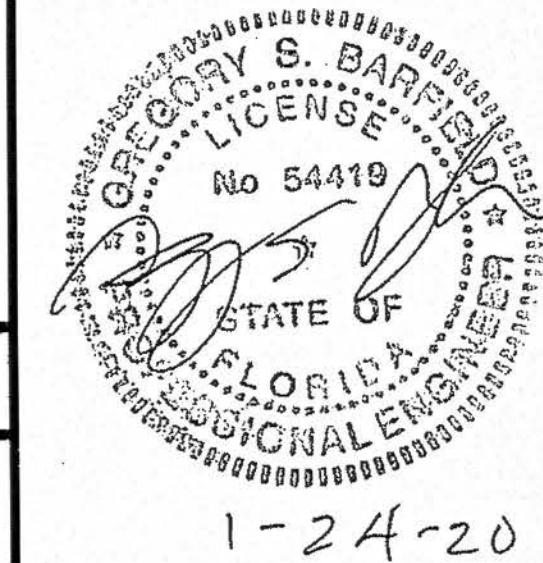


PORTAL FRAME CONNECTION DETAIL

DESIGN CRITERIA

BUILDING CODE:	FBC 17
LIVE LOAD TO FRAMES:	12 PSF
LIVE LOAD TO PURLINS:	20 PSF
COLLATERAL LOAD:	0 PSF
SNOW LOAD (GROUND):	0 PSF
WIND LOAD (ULTIMATE):	130 MPH
WIND IMPORTANCE FACTOR:	I B
BUILDING RISK CATEGORY:	II
ENCLOSURE CLASSIFICATION:	ENCLOSED

Gregory S. Barfield, P.E.
2149 Nell Purvis Road
Adel, GA 31620
P.E. # 54419



PH 229-896-7569
FAX 229-896-7560
P.O. BOX 207
ADEL, GA 31620

SCALE:	N.T.S.	DATE:	1/6/2020
DRAWN BY:	B.BROWN	SALES:	L.WHITEHURST
BUILDING SIZE:	50' X 65' X 12'	DATE:	1/13/2020

REVISION

DESCRIPTION	DATE
REVISED	1/13/2020

TITLE/LOCATION

JODY DUPREE
REF: SON
LAKE CITY, FL

PLAN:

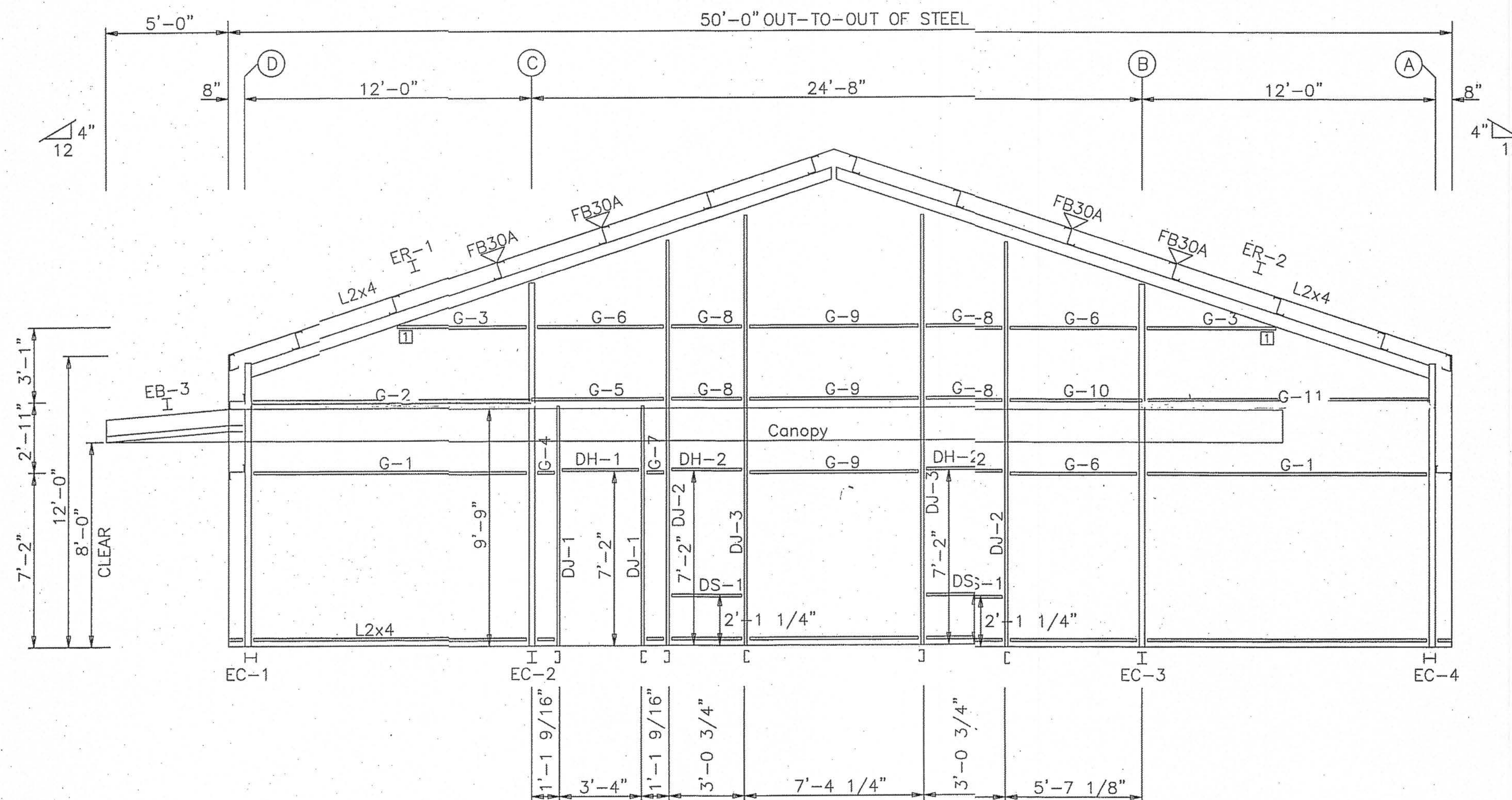
SIDEWALL FRAMING

JOB #

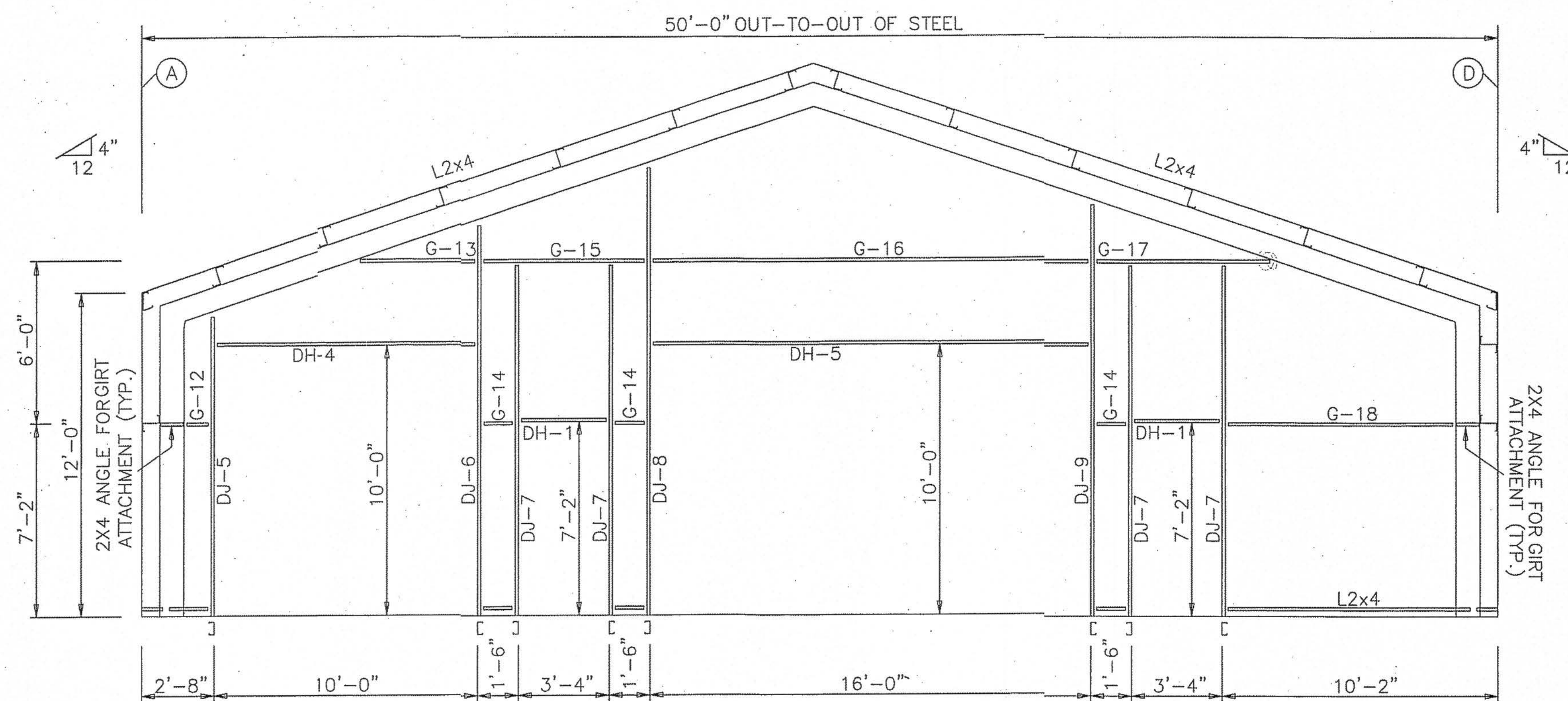
11456-SF

SHEET 3 OF 11

1-24-20



ENDWALL FRAMING: FRAME LINE 1



ENDWALL FRAMING: FRAME LINE 4

BOLT TABLE FRAME LINE 1 & 4				
LOCATION	QUAN	TYPE	DIA	LENGTH
Cor_Column/Raf	4	A325	3/4"	3"
ER-1/ER-2	8	A325	3/4"	3"
Int_Column/Raf	4	A325	1/2"	1 1/2"
Jamb	2	A325	1/2"	1 1/2"

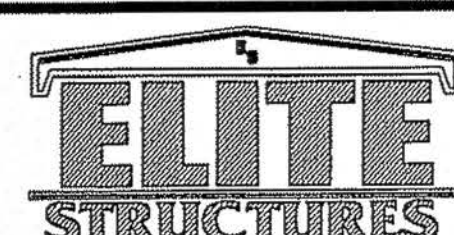
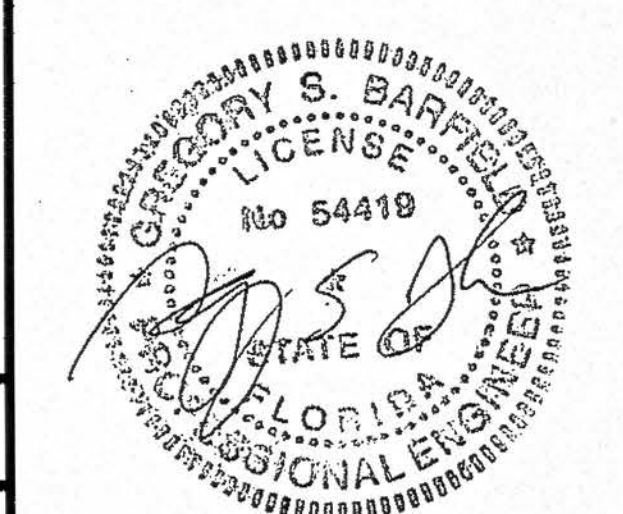
MEMBER TABLE FRAME LINE 1 & 4		
MARK	PART	LENGTH
EB-3	W8X10	5'-0 7/8"
EC-1	W8X10	11'-8 7/8"
EC-2	W8X10	14'-10 9/16"
EC-3	W8X10	14'-10 9/16"
EC-4	W8X10	11'-8 7/8"
ER-1	W8X10	25'-2 1/8"
ER-2	W8X10	25'-2 1/8"
DJ-1	8X25C14	10'-1"
DJ-2	8X25C14	16'-7 1/4"
DJ-3	8X25C14	17'-7 1/2"
DJ-5	8X35C14	11'-0 1/2"
DJ-6	8X35C14	14'-4 1/2"
DJ-7	8X25C14	13'-2"
DJ-8	8X35C14	16'-5 3/4"
DJ-9	8X35C14	15'-1 3/4"
DH-1	8X25C14	3'-3 1/2"
DH-2	8X25C14	3'-0 1/4"
DH-4	8X35C14	9'-11 1/2"
DH-5	8X35C14	15'-11 1/2"
DS-1	8X25C14	3'-0 1/4"
G-1	8X25Z16	11'-1 5/8"
G-2	8X25Z16	11'-1 1/2"
G-3	8X25Z16	4'-9 1/2"
G-4	8X25Z16	8 9/16"
G-5	8X25Z16	5'-2 1/8"
G-6	8X25Z16	5'-2 1/8"
G-7	8X25Z16	8 1/16"
G-8	8X25Z16	3'-0 1/4"
G-9	8X25Z16	6'-10 3/4"
G-10	8X25Z16	5'-2 1/8"
G-11	8X25Z16	11'-1 5/8"
G-12	8X25Z16	9 7/16"
G-13	8X25Z16	4'-3 7/8"
G-14	8X25Z16	11 1/2"
G-15	8X25Z16	5'-8 1/2"
G-16	8X25Z16	15'-11 1/2"
G-17	8X25Z16	6'-4 3/8"
G-18	8X25Z16	8'-4 7/16"

FLANGE BRACE TABLE FRAME LINE 1 & 4		
VID	MARK	LENGTH
1	FB30A	2'-6"

CONNECTION PLATES FRAME LINE 1 & 4	
ID	MARK/PART
1	b1

Gregory S. Barfield, P.E.
2149 Nell Purvis Road
Adel, GA 31620
P.E. # 54419

DESIGN CRITERIA	
BUILDING CODE:	FBC 17
LIVE LOAD TO FRAMES:	12 PSF
LIVE LOAD TO PURLINS:	20 PSF
COLLATERAL LOAD:	0 PSF
SNOW LOAD (GROUND):	0 PSF
WIND LOAD (ULTIMATE):	130 MPH
WIND IMPORTANCE FACTOR:	1
WIND EXPOSURE:	B
BUILDING RISK CATEGORY:	II
ENCLOSURE CLASSIFICATION:	ENCLOSED



PH 229-896-7569
FAX 229-896-7560
P.O. BOX 207
ADEL, GA 31620

SCALE: N.T.S.	DATE: 1/6/2020
DRAWN BY: B. BROWN	SALES: L. WHITEHURST
BUILDING SIZE: 50' X 65' X 12'	

REVISION	
DATE	DESCRIPTION
1/13/2020	REVISED

TITLE/LOCATION
JODY DUPREE
REF: SON
LAKE CITY, FL

PLAN: ENDWALL FRAMING

JOB # 11456-EF
SHEET 4 OF 11

1-24-20



MEMBER TABLE		
ROOF PLAN		
MARK	PART	LENGTH
EB-1	W8X10	5'-0 7/8"
EB-2	W8X10	5'-0 7/8"
EB-3	W8X10	5'-0 7/8"
P-1	8X35Z16	23'-1 1/2"
P-2	8X35Z16	26'-3 1/2"
P-3	8X35Z14	28'-1 1/2"
P-4	8X35Z16	23'-1 1/2"
P-6	8X25Z16	16'-1 1/4"
P-7	8X25Z16	33'-5 3/4"
P-8	8X25Z16	29'-1 1/2"
P-9	8X25Z16	23'-7 3/4"
E-1	8E214L4	19'-11 1/2"
E-2	8E214L4	19'-11 1/2"
E-3	8E214L4	24'-11 1/2"
E-4	8E214L4	24'-11 1/2"
E-5	8E214L4	19'-11 1/2"
E-6	8E214L4	19'-11 1/2"
E-7	8E214L1	17'-7 1/2"
E-8	8E212L1	30'-3 1/2"
E-9	8E214L1	25'-11 1/2"
E-10	8E214L1	24'-11 1/2"
CB-1	1/4_CBL	35'-8"
CB-2	5/16_CBL	35'-8"

A circular professional engineer seal for Gregory S. Barfield, License No. 54419, State of Florida. The seal features the text "GREGORY S. BARFIELD", "LICENSE", "No 54419", "STATE OF", "FLORIDA", and "PROFESSIONAL ENGINEER" around the perimeter. A signature is written across the center of the seal.

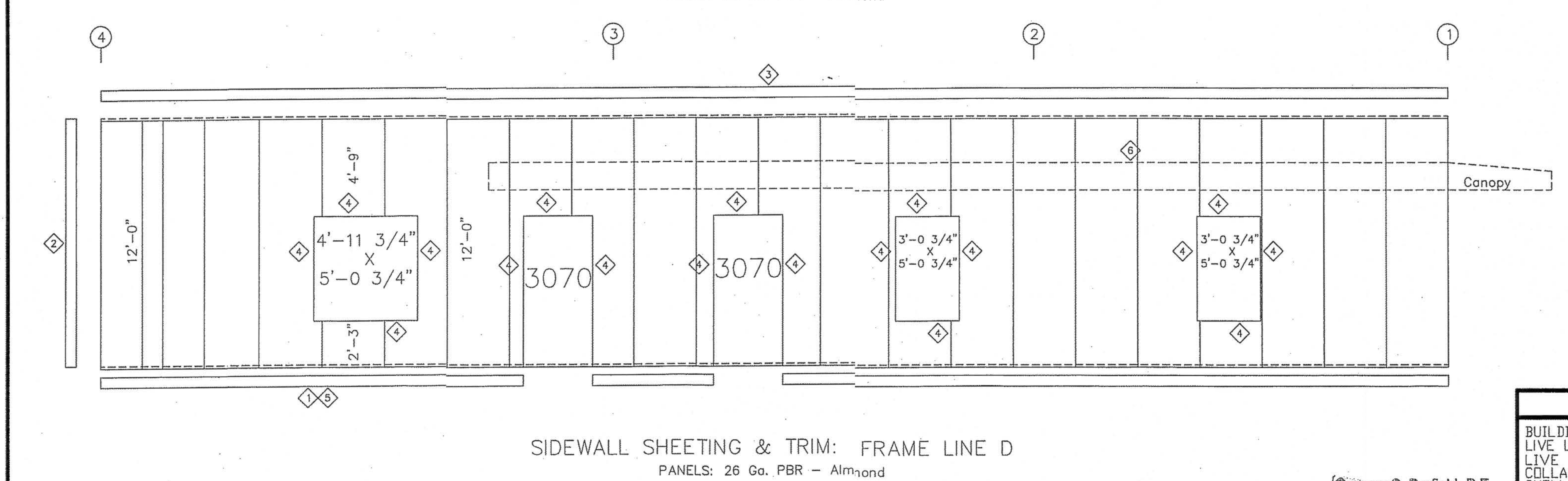
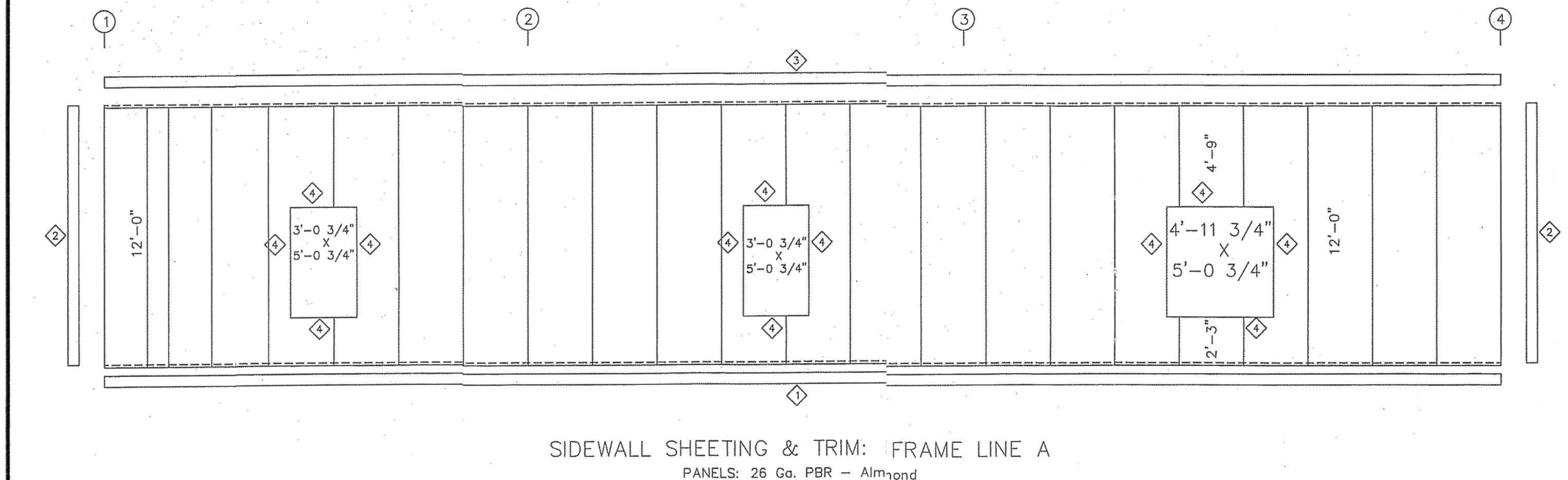
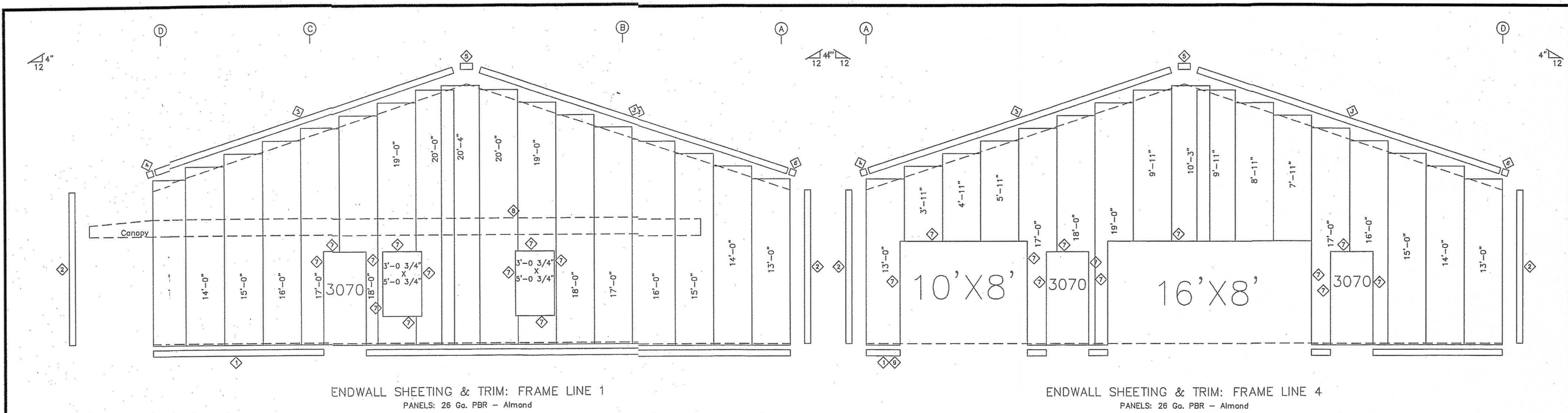
SCALE: N.T.S.	DATE: 1/6/2020
DRAWN BY: B. BROWN	SALES: L. WHITE
BUILDING SIZE: 50' X 65' X 12'	

REVISION	
DATE:	DESCRIPTION
1/13/2020	REVISED

JODY DUPREE
REF: SON
LAKE CITY, FL

JOB # 11456-RF

SHEET 5 OF 11

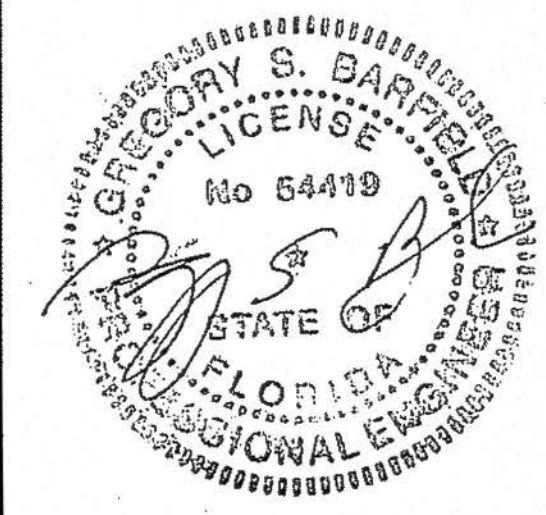


TRIM TABLE FRAME LINE 1 & 4		
ID	PART	DETAIL
1	E-108	TRIM_43
2	FL-830	TRIM_47
3	E-101	TRIM_16
4	E-145	TRIM_7
5	E-146	TRIM_23
6	E-105	TRIM_7
7	FL-26	TRIM_49
8	FL-874	TRIM_15
9	E-108	TRIM_43

TRIM TABLE FRAME LINE A & D		
ID	PART	DETAIL
1	E-108	TRIM_43
2	FL-830	TRIM_47
3	FL-19	TRIM_5
4	FL-26	TRIM_49
5	E-108	TRIM_43
6	FL-874	TRIM_15

DESIGN CRITERIA	
BUILDING CODE:	FBC 17
LIVE LOAD TO FRAMES:	12 PSF
LIVE LOAD TO PURLINS:	20 PSF
COLLATERAL LOAD:	0 PSF
SNOW LOAD (GROUND):	0 PSF
WIND LOAD (ULTIMATE):	130 MPH
WIND IMPORTANCE FACTOR:	1
WIND EXPOSURE:	B
BUILDING RISK CATEGORY:	II
ENCLOSURE CLASSIFICATION:	ENCLOSED

Gregory S. Barfield, P.E.
2149 Nell Purvis Road
Adel, GA 31620
P.E. # 54419



PH 229-896-7569
FAX 229-896-7560
P.O. BOX 207
ADEL, GA 31620

SCALE: N.T.S.	DATE: 1/6/2020
DRAWN BY: B. BROWN	SALES: L. WHITEHURST
BUILDING SIZE:	50' X 65' X 12'

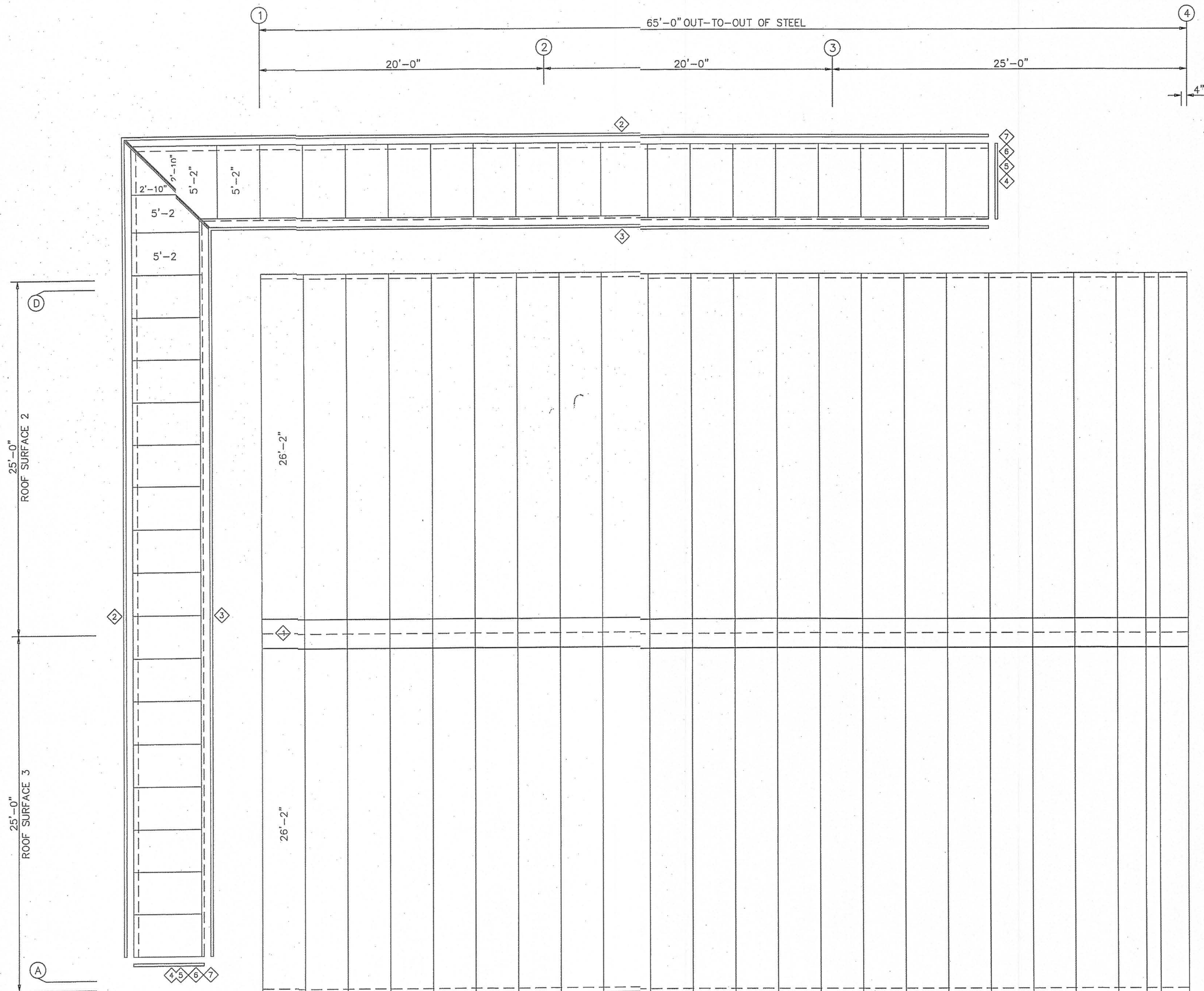
REVISION	
DESCRIPTION	DATE
REVISED	1/13/2020

TITLE/LOCATION
JODY DUPREE
REF: SON
LAKE CITY, FL

PLAN:
WALL SHEETING

JOB #
11456-WS
SHEET 6 OF 11

1-24-20

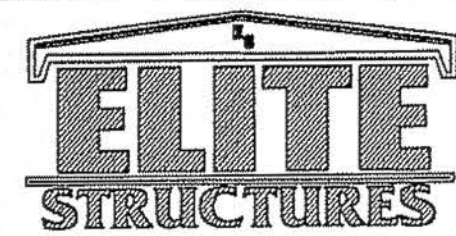


TRIM TABLE ROOF PLAN		
◇ ID	PART	DETAIL
1	FL-51	
2	FL-19	TRIM_5
3	FL-874	TRIM_15
4	E-101	TRIM_16
5	E-101	TRIM_16
6	E-141	TRIM_27
7	E-141	TRIM_27

ROOF SHEETING PLAN
PANELS: 26 Ga. PBR - Charcoal Gray

DESIGN CRITERIA	
BUILDING CODE:	FBC 17
LIVE LOAD TO FRAMES:	12 PSF
LIVE LOAD TO PURLINS:	20 PSF
COLLATERAL LOAD:	0 PSF
SNOW LOAD (GROUND):	0 PSF
WIND LOAD (ULTIMATE):	130 MPH
WIND IMPORTANCE FACTOR:	I
WIND EXPOSURE:	B
BUILDING RISK CATEGORY:	II
ENCLOSURE CLASSIFICATION:	ENCLOSED

Gregory S. Barfield, P.E.
2149 Nell Purvis Road
Adel, GA 31620
P.E. # 54419



PH 229-896-7569
FAX 229-896-7560
P.O. BOX 207
ADEL, GA 31620

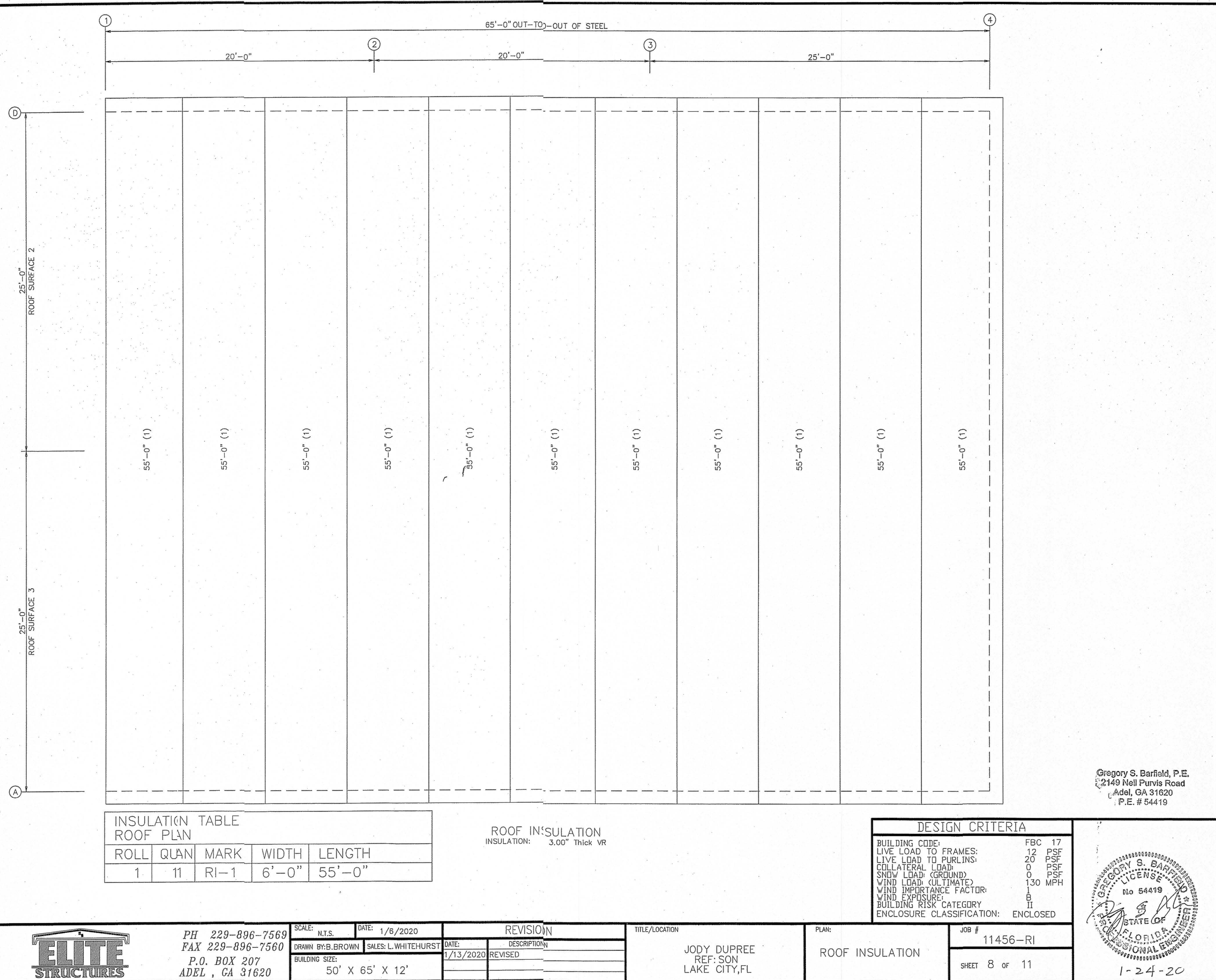
SCALE: N.T.S.	DATE: 1/6/2020	REVISION	
DRAWN BY: B. BROWN	SALES: L. WHITEHURST	DATE: 1/13/2020	DESCRIPTION
BUILDING SIZE: 50' X 65' X 12'			REVIS

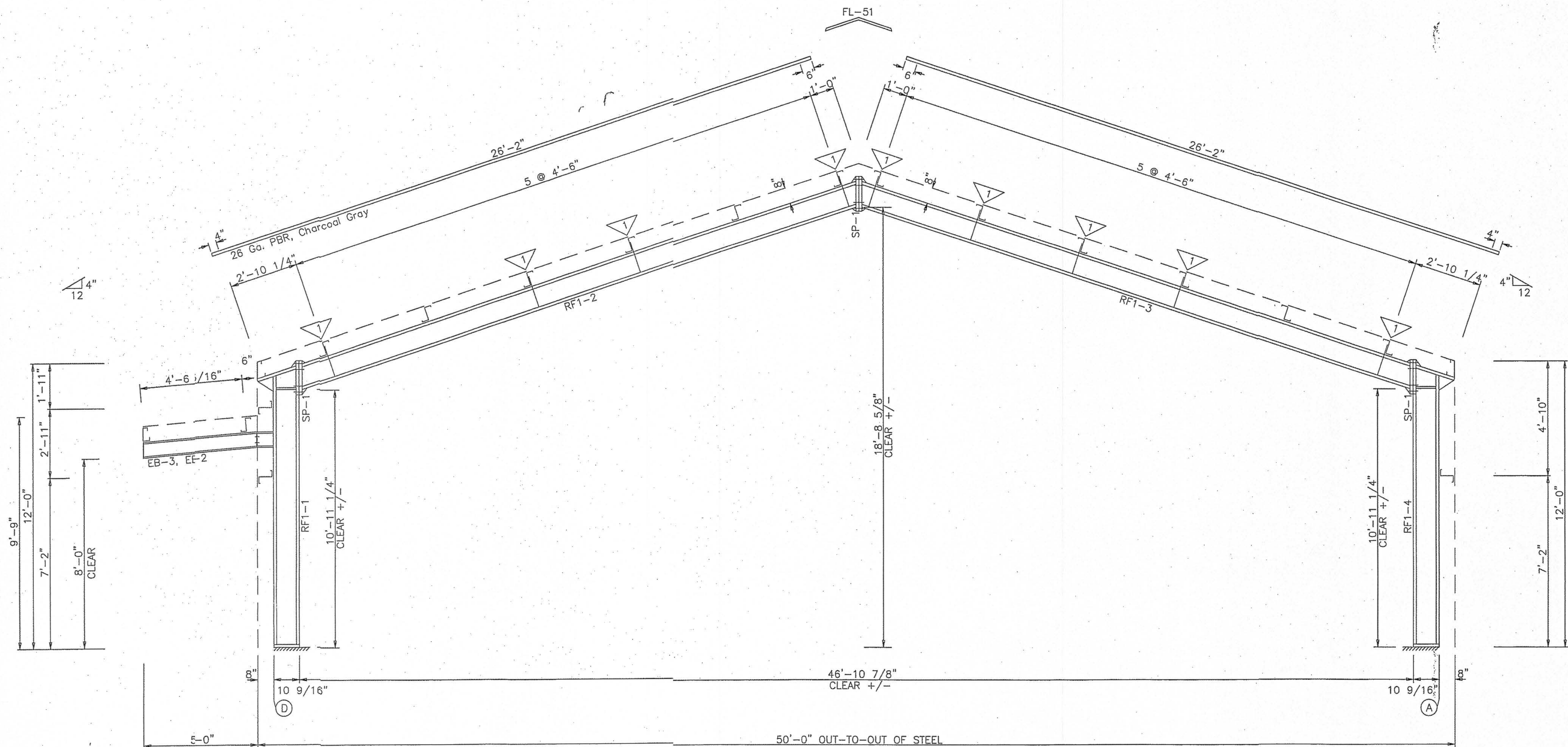
TITLE/LOCATION
JODY DUPREE
REF: SON
LAKE CITY, FL

PLAN:
ROOF SHEETING

JOB #
11456-RS
SHEET 7 OF 11

1-24-20





BUILDING CROSS SECTION: FRAME LINE 2 3 4

NOTE:
NO ADDITIONAL LOADS APPLIED
ANY FUTURE ADDITIONS SHALL
BE SELF-SUPPORTED.

MEMBER TABLE						
Mark	Weight	Web Depth Start/End	Web Plate Thick Length	Outside Flange W x Thk x Length	Inside Flange W x Thk x Length	
RF1-1	268	10.0/10.0	0.188 12.0	6 x 1/4" x 137.3	6 x 5/16" x 127.2	
		10.0/10.0	0.188 115.2	6 x 3/8" x 19.1		
		10.0/10.0	0.375 13.5			
RF1-2	524	W10X19			6 x 5/16" x 127.2	
RF1-3	529	W10X19				
RF1-4	254	10.0/10.0	0.375 13.5	6 x 3/8" x 19.1		
		10.0/10.0	0.188 115.2	6 x 1/4" x 137.3		
		10.0/10.0	0.188 12.0			

FLANGE BRACE TABLE						
FRAME LINE 2 3 4						
▽ ID	SIDES	MARK	LENGTH	OFFSET	DETAIL	CLIP
1	1	FB31A	2'-7"	2'-4"		

SPlice BOLT TABLE						
Mark	Qty	Top	Bot	Int	Type	Dia Length
SP-1	4	4	4	0	A325	0.750 3.00

GENERAL NOTES:

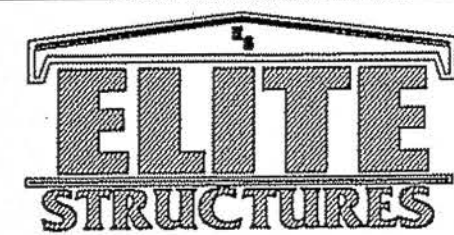
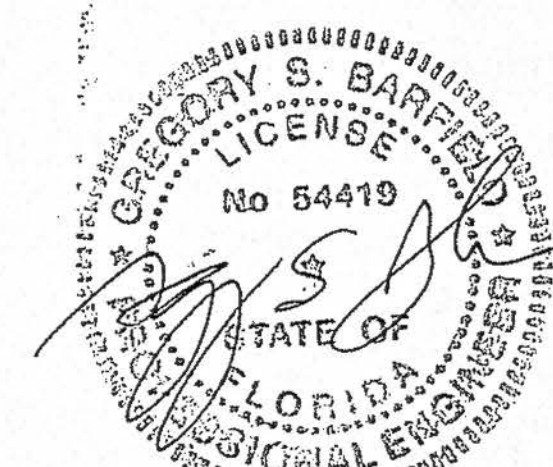
BOLT TIGHTENING - ALL BOLTED JOINTS WITH A325-09 TYPE 1 BOLTS ARE SPECIFIED AS SNUG-TIGHTENED JOINTS IN ACCORDANCE WITH THE SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 DLTS, JUNE 30, 2004. PRETENSIONING METHODS, INCLUDING TURN-OF-NUT, CALIBRATED WRENCH, TWIST OFF TYPE TENSION CONTROL BOLTS OR DIRECT TENSION INDICATORS ARE NOT REQUIRED. INSTALLATION INSPECTION REQUIREMENTS FOR SNUG TIGHT (SPECIFICATION FOR STRUCTURAL JOINTS 9.1) IS SUGGESTED.

REFERENCE: STEEL CONSTRUCTION MANUAL
AMERICAN INSTITUTE OF STEEL CONSTRUCTION INC.
THIRTEENTH EDITION (AIS)

DESIGN CRITERIA

BUILDING CODE:	FBC 17
LIVE LOAD TO FRAMES:	12 PSF
LIVE LOAD TO PURLINS:	20 PSF
COLLATERAL LOAD:	0 PSF
SNOW LOAD (GROUND):	0 PSF
WIND LOAD (ULTIMATE):	130 MPH
WIND IMPORTANCE FACTOR:	I
WIND EXPOSURE:	B
BUILDING RISK CATEGORY:	II
ENCLOSURE CLASSIFICATION:	ENCLOSED

Gregory S. Barfield, P.E.
2149 Nell Purvis Road
Adel, GA 31620
P.E. # 54419



PH 229-896-7569
FAX 229-896-7560
P.O. BOX 207
ADEL, GA 31620

SCALE: N.T.S. DATE: 1/6/2020
DRAWN BY: B. BROWN SALES: L. WHITEHURST
BUILDING SIZE: 50' X 65' X 12'

REVISION

DATE: 1/13/2020
DESCRIPTION: REVISED

TITLE/LOCATION

JODY DUPREE
REF: SON
LAKE CITY, FL

PLAN:

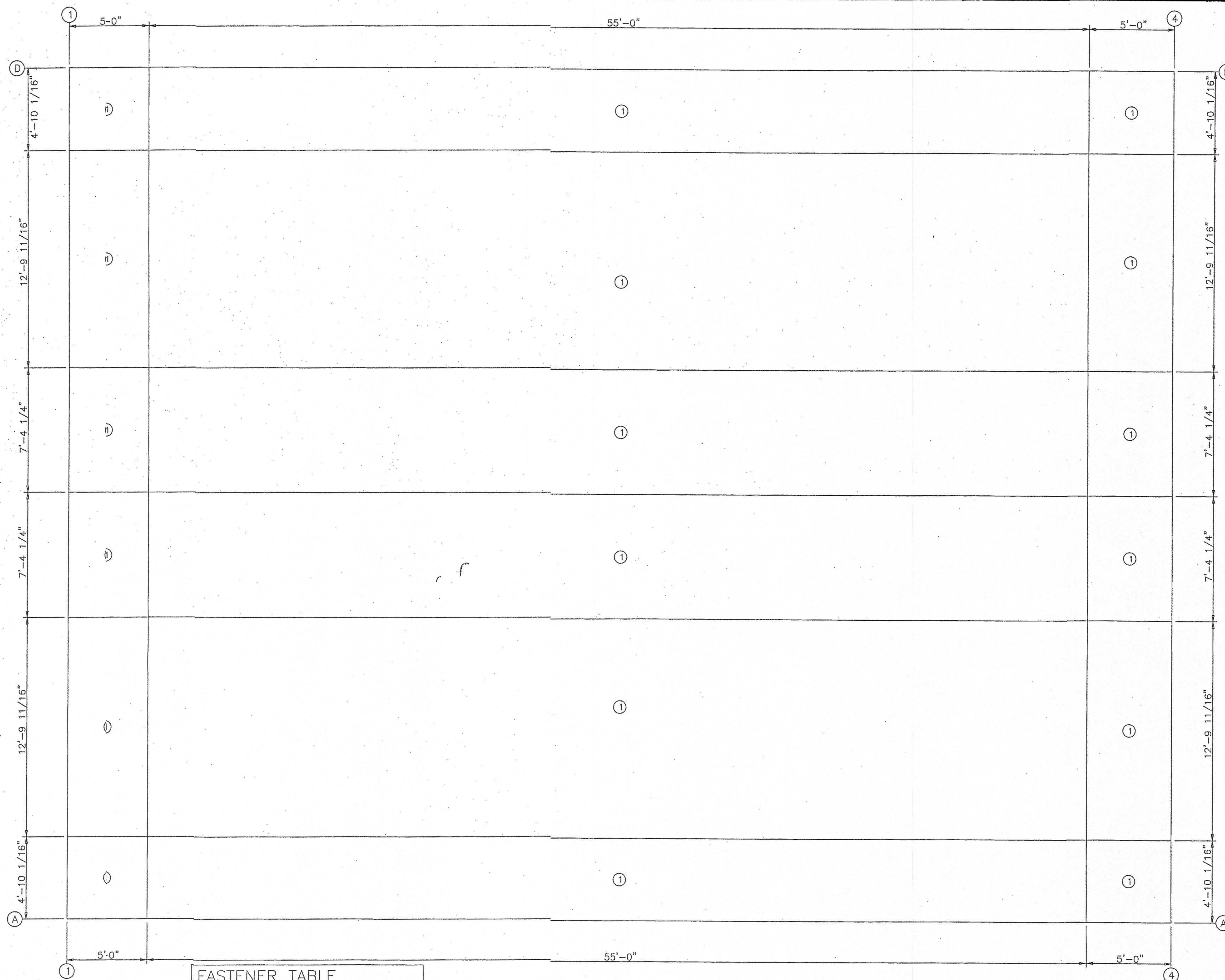
CROSS SECTION

JOB #

11456-XS

SHEET 9 OF 11

1-24-20

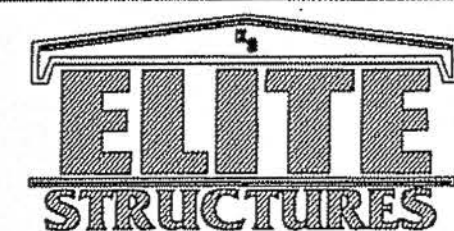


FASTENER TABLE		
○ ID	SCREW PART	SPACE (in)
1	12x1.25	12.0

PANEL ZONE LAYOUT
(Minimum Fastener Spacing)

DESIGN CRITERIA	
BUILDING CODE:	FBC 17
LIVE LOAD TO FRAMES:	12 PSF
LIVE LOAD TO PURLINS:	20 PSF
COLLATERAL LOAD:	0 PSF
SNOW LOAD (GROUND):	0 PSF
WIND LOAD (ULTIMATE):	130 MPH
WIND IMPORTANCE FACTOR:	1
BUILDING RISK CATEGORY:	II
ENCLOSURE CLASSIFICATION:	ENCLOSED

Gregory S. Barfield, P.E.
2149 Nell Purvis Road
Adel, GA 31620
P.E. # 54419



PH 229-896-7569
FAX 229-896-7560
P.O. BOX 207
ADEL, GA 31620

SCALE: N.T.S. DATE: 1/6/2020
DRAWN BY: B. BROWN SALES: L. WHITEHURST
BUILDING SIZE: 50' X 65' X 12'

REVISION

DATE	DESCRIPTION
1/13/2020	REVISED

TITLE/LOCATION

JODY DUPREE
REF: SON
LAKE CITY, FL

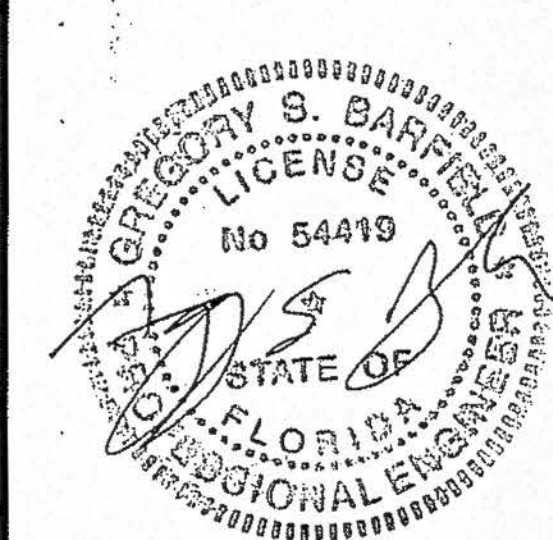
PLAN:

ZONE LAYOUT

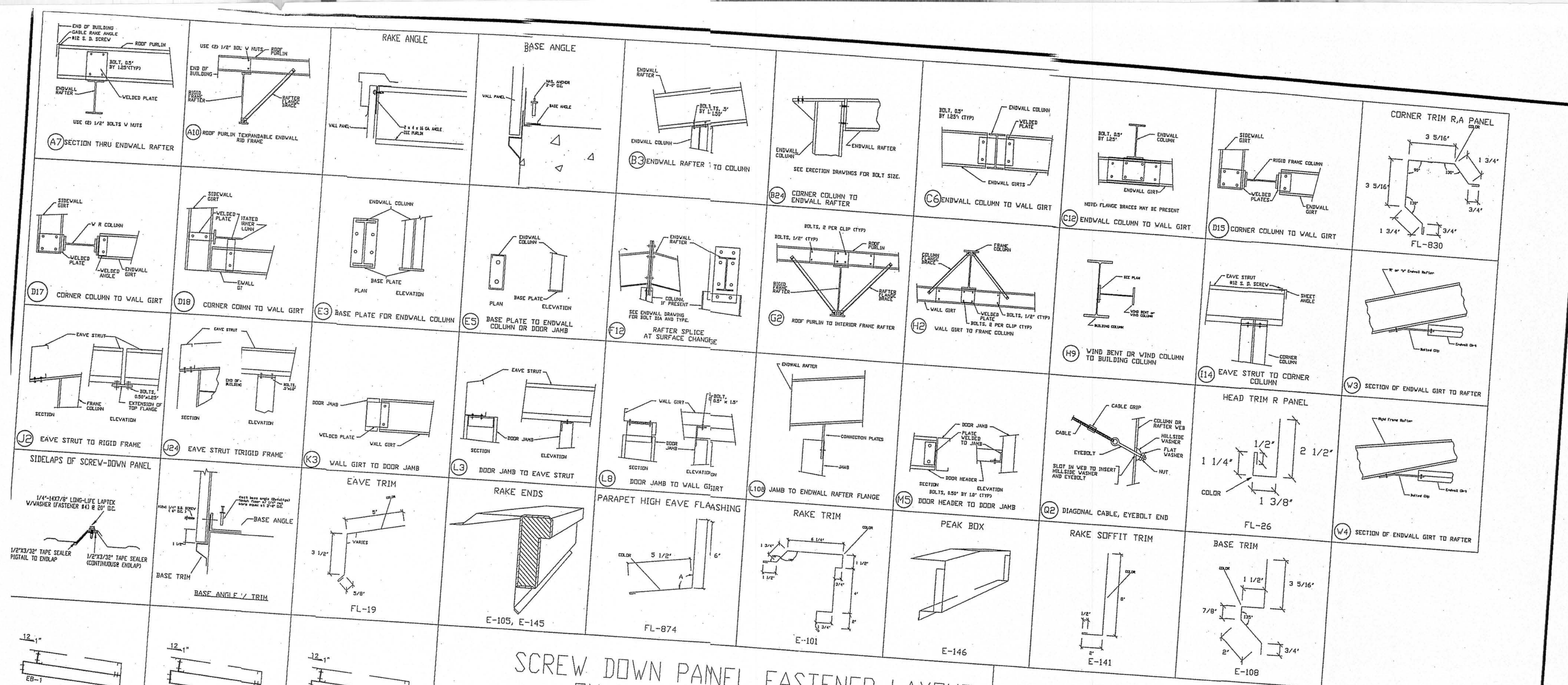
JOB #

11456-ZL

SHEET 10 OF 11

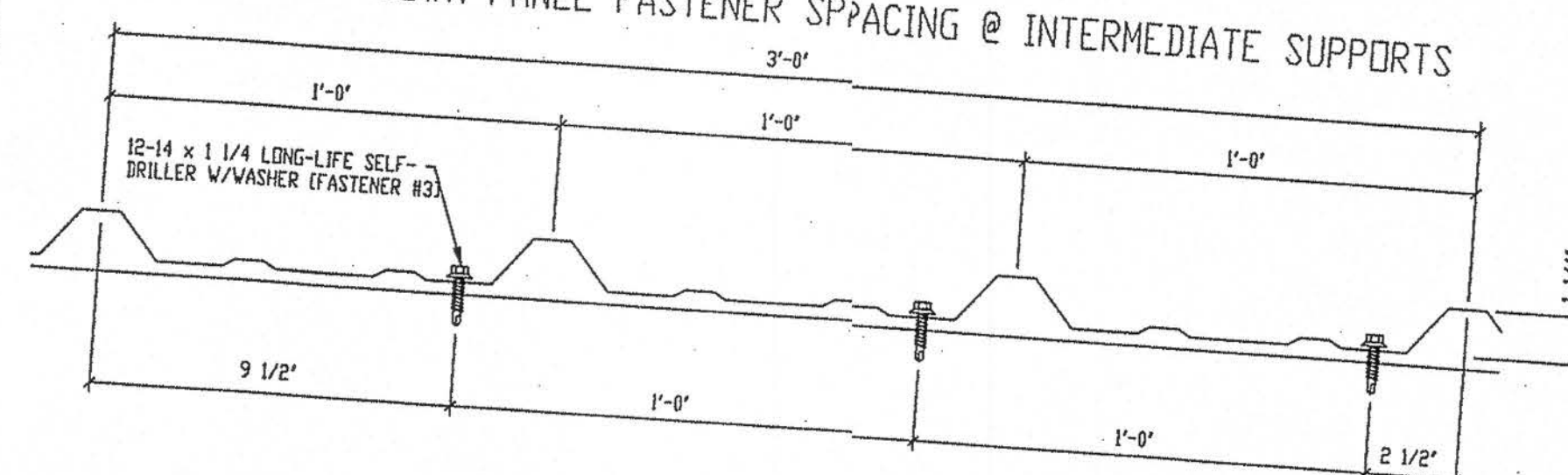


1-24-20

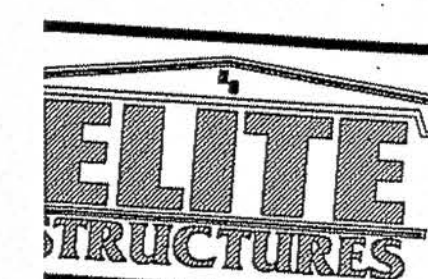
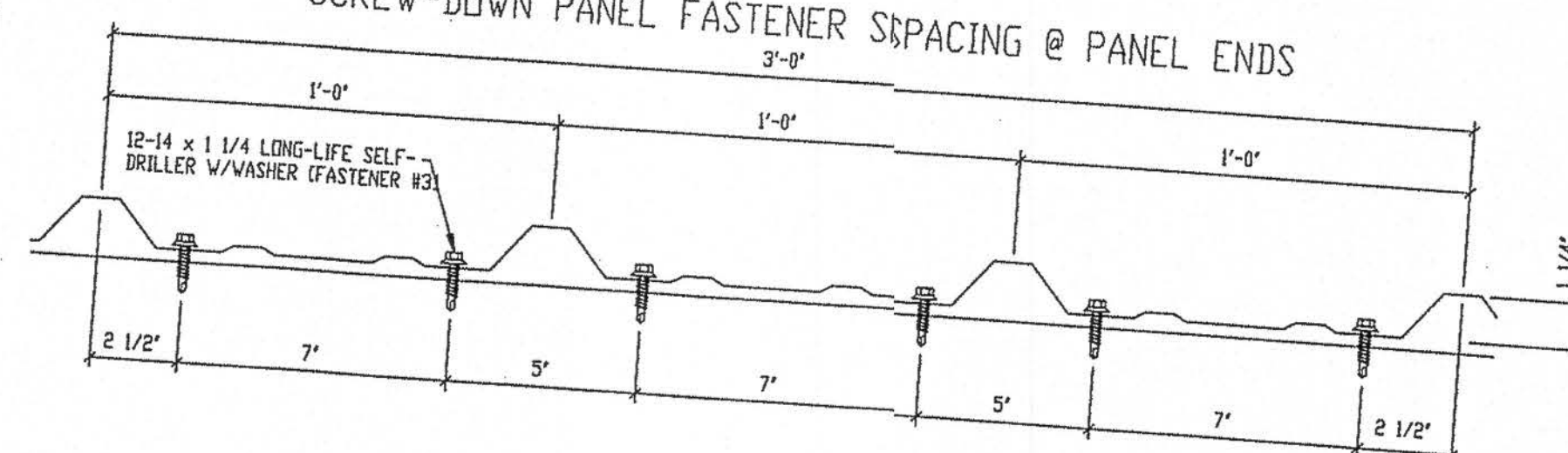


SCREW DOWN PANEL FASTENER LAYOUT ON EXTERIOR OF BUILDING

SCREW-DOWN PANEL FASTENER SPACING @ INTERMEDIATE SUPPORTS



SCREW-DOWN PANEL FASTENER SPACING @ PANEL ENDS



PH 29-896-7569
FAX 29-896-7560
P.O. BOX 207
ADEL, GA 31620

SCALE: N.T.S.

DATE: 1/6/2020

DRAWN BY: B.BROWN SALES: L.WHITEHURST

BUILDING SIZE: 50' X 65' X 12'

REVISION

DATE: 1/13/2020
DESCRIPTION: REVISED

TITLE/LOCATION

JODY DUPREE
REF: SON
LAKE CITY, FL

PLAN:

DETAIL PAGE

DESIGN CRITERIA

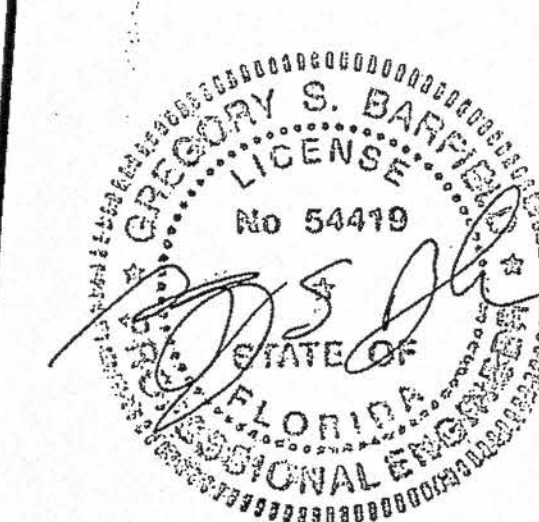
BUILDING CODE:	FBC 17
LIVE LOAD TO FRAMES:	12 PSF
LIVE LOAD TO PURLINS:	20 PSF
COLLATERAL LOAD:	0 PSF
SNOW LOAD (GROUND):	0 PSF
WIND LOAD (ULTIMATE):	130 MPH
WIND IMPORTANCE FACTOR:	1
BUILDING EXPOSURE:	B
BUILDING RISK CATEGORY:	II
ENCLOSURE CLASSIFICATION:	ENCLOSED

JOB #

11456-DP

SHEET 11 OF 11

Gregory S. Barfield, P.E.
2149 Nell Purvis Road
Adel, GA 31620
P.E. # 54419



1-24-20