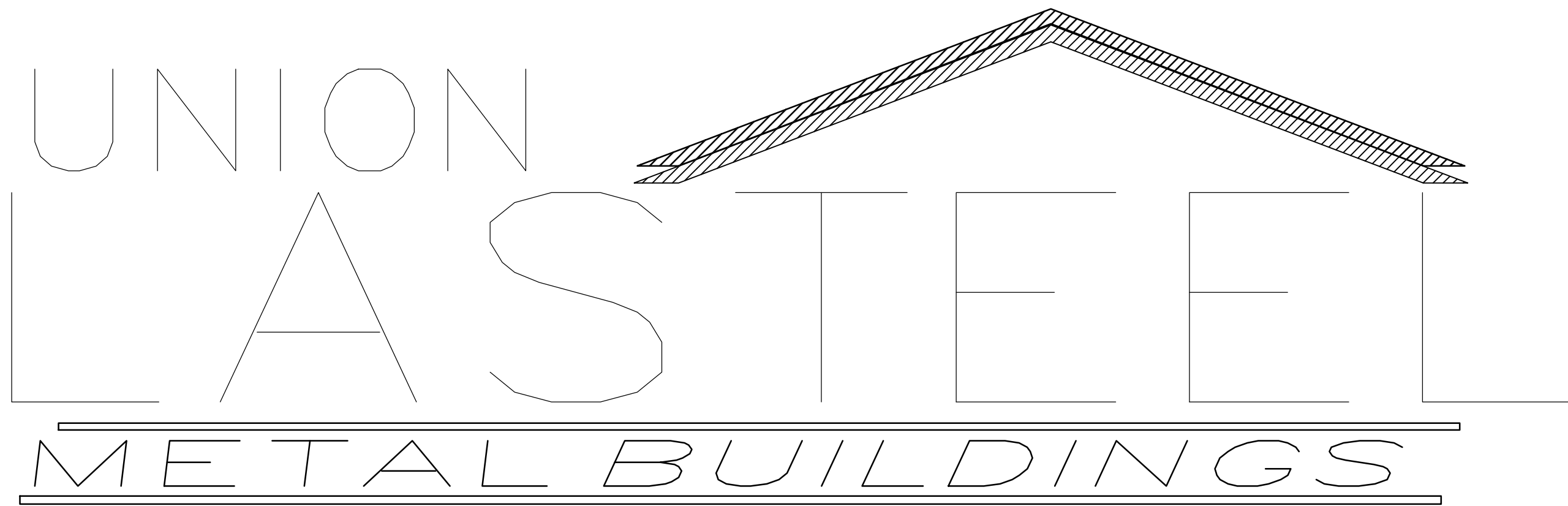




PO BOX 537 Lake Bulter, FL 32054
(386) 496-0681 (386) 496-4705 (fax)

BUILDING LOADS / DESCRIPTION:

WIDTH: 71.25 LENGTH: 60 HEIGHT: 18 /18
(BUILDING DIMENSIONS ARE NOMINAL. REFER TO PLANS).

THIS STRUCTURE IS DESIGNED UTILIZING THE LOADS INDICATED
AND APPLIED AS REQUIRED BY : FBC 23 .

THE CONTRACTOR IS TO CONFIRM THAT THESE LOADS COMPLY
WITH THE REQUIREMENTS OF THE LOCAL BUILDING DEPARTMENT.

ROOF DEAD LOAD:	2.00	PSF (ROOF PANELS & PURLINS)
COLLATERAL LOAD:	1.00	PSF
ROOF LIVE LOAD:	20.00	PSF
ROOF SNOW LOAD:	3.36	PSF
GROUND SNOW LOAD:	4	PSF
BASIC WIND SPEED:	120	MPH
SEISMIC ZONE:	B	
THERMAL FACTOR:	1.2000	
IMPORTANCE FACTORS:		

WIND LOAD	1.00	EXPANDED FORMULA	0.667*le*Sms*W/R
SNOW LOAD	1.0000	LONGITUDINAL	1.16
SEISMIC LOAD	1.00	TRANSVERSE	0.96

GENERAL NOTES:
1) MATERIALS : MINIMUM YIELD:
HOT ROLLED BAR Fy = 50.0000 ksi MIN.
STRUCTURAL STEEL SHEET Fy = 55.0000 ksi MIN.
STRUCTURAL STEEL PLATE Fy = 50.0000 ksi MIN.
COLD FORMED SHAPES Fy = 57.0000 ksi MIN.
WALL SHEETING Fy = 60.0000 ksi MIN.
ROOF SHEETING Fy = 60.0000 ksi MIN.
BOLTS A307 & A325
THE METAL BUILDING MANUFACTURER RESERVES THE RIGHT TO
SUBSTITUTE THE ABOVE MATERIALS WITH EQUAL OR BETTER MATERIAL.

2) BOLT TIGHTENING REQUIREMENTS:
ALL HIGH STRENGTH BOLTS ARE A325 UNLESS NOTED OTHERWISE.
HIGH STRENGTH BOLTS SHALL BE TIGHTENED BY THE TURN OF THE NUT METHOD
IN ACCORDANCE WITH THE LATEST EDITION AISC "SPECIFICATION FOR
STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS". A325 BOLTS SHALL BE
INSTALLED WITH OUT WASHERS WHEN TIGHTENED BY THE "TURN OF THE NUT"
METHOD. ALL BOLTED CONNECTIONS, FOR SHEAR/BEARING CONNECTION TYPE
WITH BOLT THREADS EXCLUDED FROM THE SHEAR PLANE SHALL BE SNUG TIGHT
ONLY.

3) ALL STRUCTUAL STEEL TO RECEIVE A RUST INHIBITIVE PRIMER. THIS PAINT
IS NOT INTENDED FOR LONG TERM EXPOSURE TO THE ELEMENTS.

SITE CLASS:	D
OCCUPANCY CATEGORY:	II - Normal
SEISMIC DESIGN CATEGORY:	B

Tracy
Mitchell



ROOF PANELS:

COLOR: Galvalume Plus

WALL PANELS:

COLOR:

TRIM COLORS:

CABLE: Need Sill Poly Color

CORNER: Need Sill Poly Color

EAVE: Need Sill Poly Color

FRAMED OPENINGS: Need Sill Poly Color

LINER PANELS:

COLOR: N/A

LINER TRIM:

COLOR: N/A

Digitally signed by
Tracy Mitchell
Date: 2025.03.11
09:28:56 -04'00'

DEFLECTION LIMTS:

EW COL:	180
EW RAF LIVE:	180
EW RAF WIND:	180
WALL GIRT:	90
PURL LIVE:	180
PURL WIND:	150
WALL PANEL:	60
ROOF PANEL LIVE:	60
ROOF PANEL WIND:	60
RF HORIZONTAL:	60
RF VERTICAL:	180
WIND BENT:	60
RF CRANE:	60
RF SEIS:	50
WIND BENT SEIS:	50

IMPORTANT NOTE: FINAL DETAILING, FABRICATION, AND DELIVERY DATE OF THIS PROJECT
CANNOT BE COMPLETED UNTIL THE SIGNED APPROVALS ARE RETURNED TO THE METAL
BUILDING MANUFACTURER.

BUILDER / CONTRACTOR RESPONSIBILITIES

IT IS THE RESPONSIBILITY OF THE BUILDER/CONTRACTOR TO INSURE THAT ALL PROJECT
PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE REQUIREMENTS OF ANY
GOVERNING BUILDING AUTHORITIES. THE SUPPLYING OF SEALED ENGINEERING DATA AND
DRAWINGS FOR THE METAL BUILDING SYSTEM DOES NOT IMPLY OR CONSTITUTE AN
AGREEMENT THAT THE METAL BUILDING SYSTEM MANUFACTURER OR ITS DESIGN ENGINEER IS
ACTING AS THE ENGINEER OF RECORD OR DESIGN PROFESSIONAL FOR A CONSTRUCTION
PROJECT.

THE CONTRACTOR MUST SECURE ALL REQUIRED APPROVALS AND PERMITS FROM THE
APPROPRIATE AGENCY AS REQUIRED. APPROVAL OF THE METAL BUILDING SYSTEM
MANUFACTURER'S DRAWINGS AND CALCULATIONS INDICATE THAT THE METAL BUILDING
SYSTEM MANUFACTURER CORRECTLY INTERPRETED AND APPLIED THE REQUIREMENTS OF THE
CONTRACT DRAWINGS AND SPECIFICATIONS. (SECT. 4.2.1 AISC CODE OF STANDARD
PRACTICES, 9TH ED.) WHERE DISCREPANCIES EXIST BETWEEN THE METAL BUILDING SYSTEM
MANUFACTURER'S STRUCTURAL STEEL PLANS AND THE PLANS FOR OTHER TRADES, THE
STRUCTURAL STEEL PLANS SHALL GOVERN. (SECT. 3.3 AISC CODE OF STANDARD PRACTICE
9TH ED.)

DESIGN CONSIDERATIONS OF ANY MATERIALS IN THE STRUCTURE WHICH ARE NOT
FURNISHED BY THE METAL BUILDING SYSTEM MANUFACTURER ARE THE RESPONSIBILITY OF
THE CONTRACTORS AND ENGINEERS OTHER THAN THE METAL BUILDING SYSTEM
MANUFACTURER'S ENGINEER UNLESS SPECIFICALLY INDICATED.

THE CONTRACTOR IS RESPONSIBLE FOR ALL ERECTION OF STEEL AND ASSOCIATED WORK
IN COMPLIANCE WITH THE METAL BUILDING SYSTEM MANUFACTURER "FOR CONSTRUCTION"
DRAWINGS.

ALL BRACING AS SHOWN AND PROVIDED BY THE METAL BUILDING SYSTEM MANUFACTURER
FOR THIS BUILDING IS REQUIRED AND SHALL BE INSTALLED BY THE ERECTOR AS A
PERMANENT PART OF THE STRUCTURE.

TEMPORARY SUPPORTS, SUCH AS TEMPORARY GUYS, BRACES, FALSE WORK, CRIBBING OR
OTHER ELEMENTS REQUIRED FOR THE ERECTION OPERATION WILL BE DETERMINED AND
FURNISHED AND INSTALLED BY THE ERECTOR. THESE TEMPORARY SUPPORTS WILL SECURE
THE STEEL FRAMING, OR ANY PARTLY ASSEMBLED STEEL FRAMING, AGAINST LOADS
COMPARABLE IN INTENSITY TO THOSE FOR WHICH THE STRUCTURE WAS DESIGNED, RESULTING
FROM WIND, SEISMIC FORCES AND ERECTION OPERATIONS, BUT NOT THE LOADS RESULTING
FROM THE PERFORMANCE OF WORK BY OR THE ACTS OF OTHERS, NOR SUCH
UNPREDICTABLE LOADS AS THOSE DUE TO TORNADO, EXPLOSION, OR COLLISION. (SECT. 7.9.1
AISC CODE OF STANDARD PRACTICE, 9TH ED.)

WARNING: IN NO CASE SHOULD GALVALUME STEEL PANELS BE USED IN CONJUNCTION
WITH LEAD OR COPPER. BOTH LEAD AND COPPER HAVE HARMFUL CORROSION EFFECTS ON
THE ALUMINUM ZINC ALLOY COATING WHEN THEY ARE USED IN CONTACT WITH GALVALUME
STEEL PANELS. EVEN RUN-OFF FROM COPPER FLASHING, WIRING, OR TUBING ONTO
GALVALUME SHOULD BE AVOIDED.

APPROVAL NOTES

THE FOLLOWING CONDITIONS APPLY IN THE EVENT THAT THESE DRAWINGS ARE USED AS
APPROVAL DRAWINGS: IT IS IMPERATIVE THAT ANY CHANGES TO THESE DRAWINGS BE MADE
IN CONTRASTING INK (PREFERABLY RED INK), HAVE ALL INSTANCES OF CHANGE CLEARLY
INDICATED, AND BE LEGIBLE AND UNAMBIGUOUS. A SIGNATURE AND DATE IS REQUIRED ON
ALL PAGES. MANUFACTURER RESERVES THE RIGHT TO RE-SUBMIT DRAWINGS WITH EXTENSIVE
OR COMPLEX CHANGES REQUIRED TO AVOID MISFABRICATION. THIS MAY IMPACT THE
DELIVERY SCHEDULE. APPROVAL OF THESE DRAWINGS INDICATES CONCLUSIVELY THAT THE
METAL BUILDING SYSTEM MANUFAACTURER HAS CORRECTLY INTERPRETED THE CONTRACT
REQUIREMENTS, AND FURTHER CONSTITUTES AGREEMENT THAT THE BUILDING AS DRAWN WITH
INDICATED CHANGES REPRESENTS THE TOTAL OF THE MATERIALS TO BE SUPPLIED BY
MANUFACTURER. ANY CHANGES NOTED ON THHE DRAWINGS NOT IN COMFORMANCE WITH THE
TERMS AND REQUIREMENTS OF THE CONTRACT BETWEEN MANUFACTURER AND ITS CUSTOMER
ARE NOT BINDING ON MANUFACTURER UNLESS SUBSEQUENTLY SPECIFICALLY ACKNOWLEDGED
AND AGREED TO IN WRITING BY CHANGE ORDER OR SEPARATE DOCUMENTATION.
MANUFACTURER RECONGNIZES THAT RUBBER STAMPS ARE ROUTINELY USED FOR INDICATING
APPROVAL, DISAPPROVAL, REJECTION, OR MERE REVIEW OF THE DRAWINGS SUBMITTED.
HOWEVER, MANUFACTURER DOES NOT ACCEPT CHANGES OR ADDITIONS TO CONTRACTURAL
TERMS AND CONDITIONS THAT MAY APPEAR WITH USE OF A STAMP OR SIMILIAR INDICATIOIN
OF APPROVAL, DISAPPROVAL, ETC. SUCH LANGUAGE APPLIED TO MANUFACTURER'S DRAWINGS
BY THE CUSTOMER, ARCHITECT, ENGINEER, OR ANY OTHER PARTY WILL BE CONSIDERED AS
UNACCEPTABLE ALTERNATIONS TO THESE DRAWING NOTES, AND WILL NOT ALTER THE
CONTRACTUAL RIGHTS AND OBLIGATIONS EXISTING BETWEEN MANUFACTURER AND ITS
CUSTOMER.

SHEET 1 OF 14

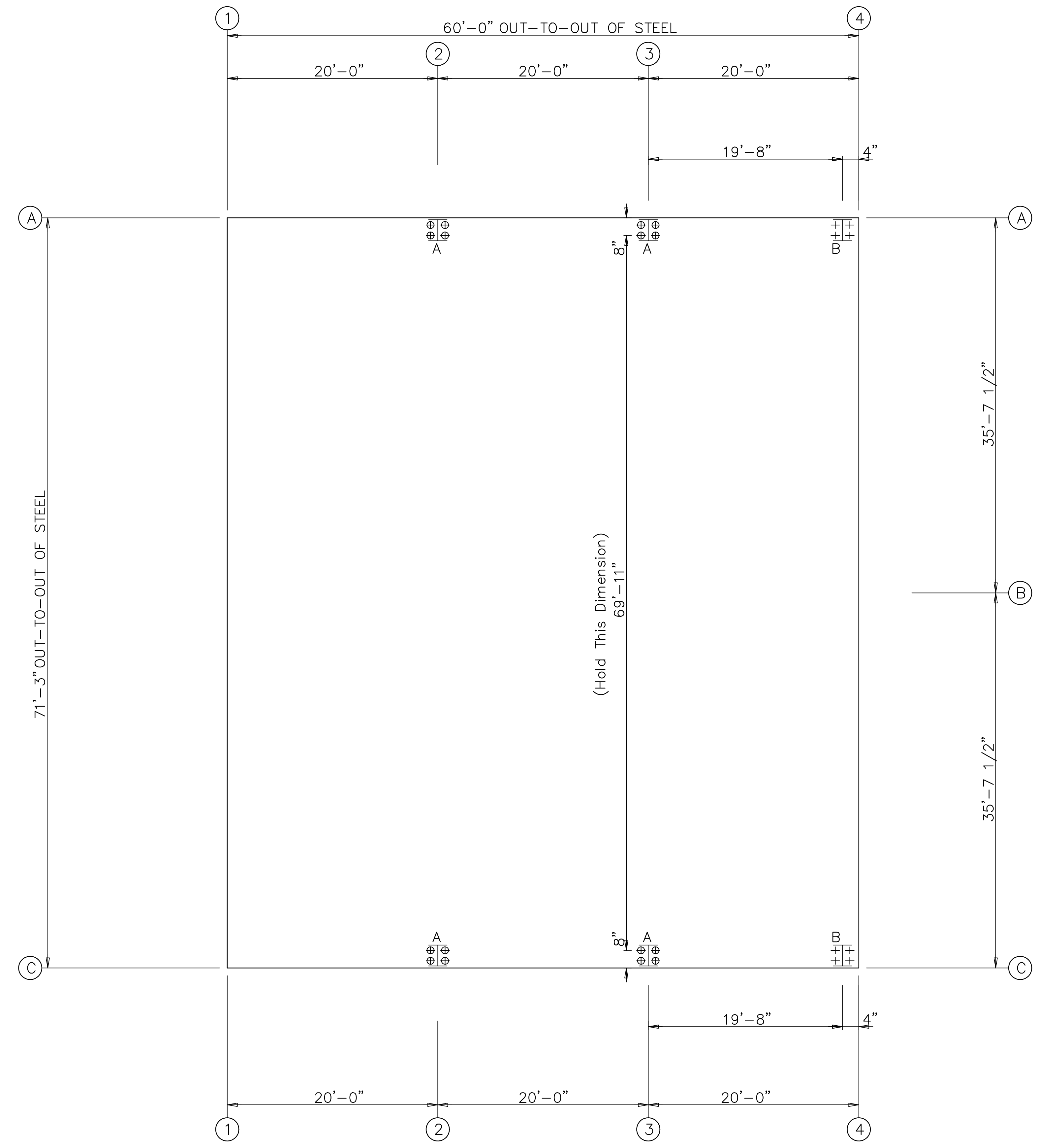
1291 SE Baya Drive
Lake City, FI 32025

△		
△		
△		
△		
△	02/27/25	FOR PERMIT
REV.	DATE	REVISION

PURCHASER: Wilson Containers

PROJECT: Building Addition

JOB NUMBER: WilsonContainers71x60Addition



⊕ Dia= 3/4"
+ Dia=1"

ANCHOR BOLT PLAN
NOTE: All Base Plates @ 100'-0" (U.N.)



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Union LaSteel				
PROJECT	Building Addition	ANCHOR BOLT PLAN		
ID	WilsonContainers71x60Addition	DESIGN:	DRAFT:	CHECK:
PROJECT ADDRESS	1291 SE Baya Drive Lake City, FL 32025	DATE: 2/24/25	SHEET 2 OF 14	

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	-----Dead-----	-----Collateral-----	-----Live-----	-----Snow-----	-----Wind_Left1-----	-----Wind_Right1-----
4	A	Horz 1.0	Vert 2.4	Horz 0.2	Vert 0.4	Horz 2.5	Vert 4.3
4	C	Horz -1.0	Vert 2.4	Horz -0.2	Vert 0.4	Horz -2.5	Vert 4.3
4	A	-----Wind_Left2-----	-----Wind_Right2-----	-----Wind_Long1-----	-----Wind_Long2-----	-----Seismic_Left-----	-----Seismic_Right-----
4	A	Horz -3.1	Vert -6.5	Horz -3.4	Vert -4.9	Horz -4.6	Vert -92.1
4	C	Horz 3.4	Vert -4.9	Horz 3.1	Vert -6.5	Horz 4.6	Vert -92.1
4	A	-----Seismic_Long-----	-----MIN_SNOW-----	F1UNB_SL_L-----	F1UNB_SL_R-----		
4	A	Horz 0.0	Vert -25.7	Horz 0.8	Vert 1.4	Horz 0.5	Vert 1.1
4	C	Horz 0.0	Vert -25.7	Horz -0.8	Vert 1.4	Horz -0.5	Vert 0.7
2*	A	-----Dead-----	-----Collateral-----	-----Live-----	-----Snow-----	-----Wind_Left1-----	-----Wind_Right1-----
2*	A	Horz 1.4	Vert 3.1	Horz 0.4	Vert 0.7	Horz 4.9	Vert 8.5
2*	C	Horz -1.4	Vert 3.1	Horz -0.4	Vert 0.7	Horz -4.9	Vert 8.6
2*	A	-----Wind_Left2-----	-----Wind_Right2-----	-----Wind_Long1-----	-----Wind_Long2-----	-----Seismic_Left-----	-----Seismic_Right-----
2*	A	Horz -6.1	Vert -12.8	Horz -6.7	Vert -9.7	Horz -7.9	Vert -13.8
2*	C	Horz 6.7	Vert -9.7	Horz 6.1	Vert -12.8	Horz 7.9	Vert -13.8
2*	A	-----MIN_SNOW-----	F2UNB_SL_L-----	F2UNB_SL_R-----			
2*	A	Horz 1.6	Vert 2.8	Horz 1.1	Vert 2.2	Horz 1.1	Vert 1.3
2*	C	Horz -1.6	Vert 2.8	Horz -1.1	Vert 1.3	Horz -1.1	Vert 2.2
2*	Frame lines:		2	3			

NOTES FOR REACTIONS

Building reactions are based on the following building data:

Width (ft) = 71.3

Length (ft) = 60.0

Eave Height (ft) = 18.0

Roof Slope (rise/12) = 1.00/ 1.00

Roof Dead Load (psf) = 2.0

Wall Dead Load

Left Endwall (psf) = 2.0

Right Endwall (psf) = 2.0

Front Sidewall (psf) = 2.0

Back Sidewall (psf) = 2.0

Roof Live Load (psf) = 20.0

Frame Live Load (psf) = 12.0

Collateral Load (psf) = 1.0

Snow Load (psf) = 3.4

Minimum Snow (psf) = 4.0

Wind Speed (mph) = 120.0

Wind Code = FBC 23 (IBC 24)

Exposure = B

Closure = Open

Risk Category = II - Normal

Importance - Wind = 1.00

Importance - Seismic = 1.00

Seismic Design Category = B

Seismic Coeff (Sms) = 0.17

ID

Description

1

Dead+0.6Wind_Long2R

2

Dead+Collateral+0.75Live+0.45Wind_Long2L

3

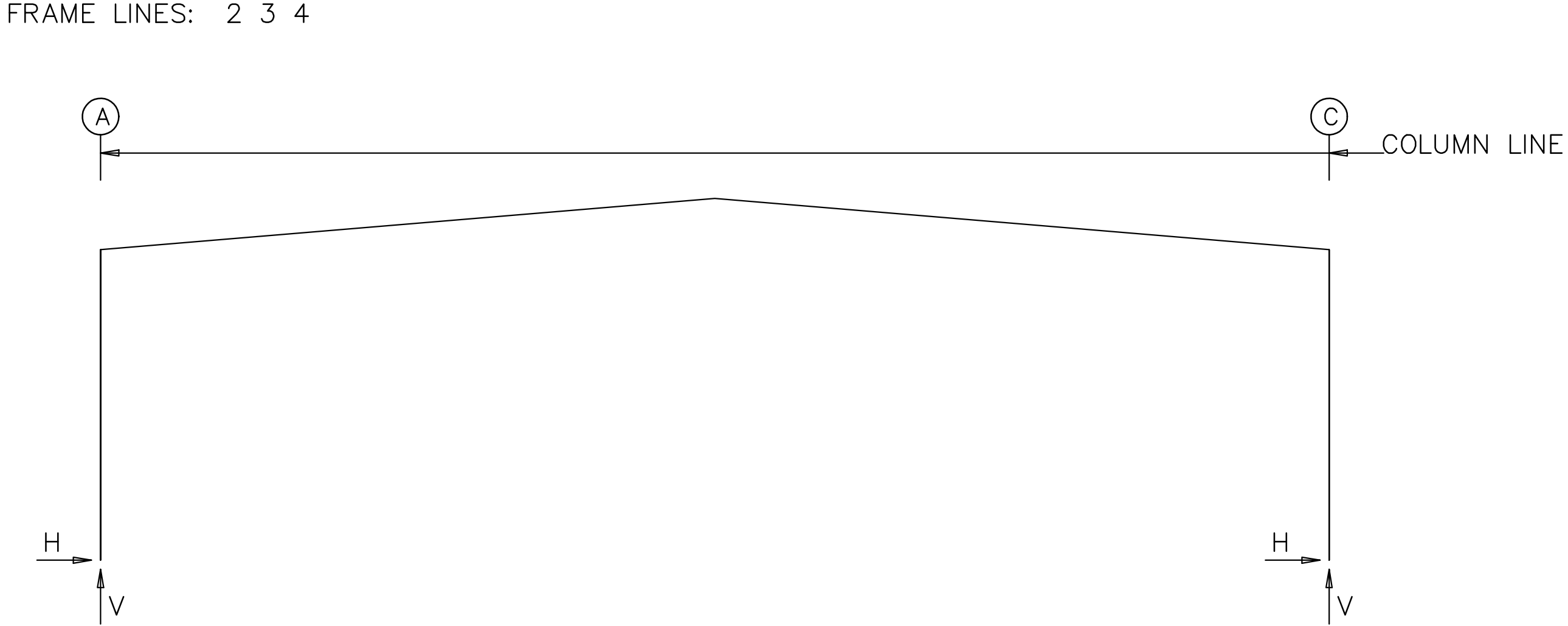
Dead+Collateral+0.75Live+0.45Wind_Long2R

4

0.6Dead+0.6Wind_Long1L

5

0.6Dead+0.6Wind_Long1R



RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k)						Bolt(in)		Base_Plate(in)			Grout (in)
		Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin			Width	Length	Thick	
2*	A	3	7.2	13.2	5	-3.9	-6.5	4	0.750	8.000	14.00	0.375	0.0
2*	C	5	3.9	-6.5	3	-7.2	13.2	4	0.750	8.000	14.00	0.375	0.0
2*		3	-7.2	13.2	5	3.9	-6.5						
2*	Frame lines:		2	3									

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k)						Bolt(in)		Base_Plate(in)			Grout (in)
		Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin			Width	Length	Thick	
4	A	2	4.0	-30.3	4	-2.1	-53.8	4	1.000	8.000	14.00	1.000	0.0
4	C	1	2.2	54.8									
4		4	2.1	-53.8	2	-4.0	-30.3	4	1.000	8.000	14.00	1.000	0.0
4		1	-2.2	54.8	4	2.1	-53.8						

ANCHOR BOLT SUMMARY

Qty	Locate	Dia (in)	Type	Proj (in)
+ 8	Frame	1"	A307	3.00
⊕ 16	Frame	3/4"	A307	3.00

BUILDING BRACING REACTIONS

---	Wall Loc	Col Line	± Reactions(k)				Panel_Shear (lb/ft)		Note
			Wind Horz	Wind Vert	Seismic Horz	Seismic Vert	Wind	Seis	
L_EW	1	Bracing	Not	Used			0	0	
F_SW	C						94	29	
R_EW	4								(h)
B_SW	A						94	29	

(h)Rigid frame at endwall

Reactions for seismic represent shear force, Eh
Reaction values shown are unfactored

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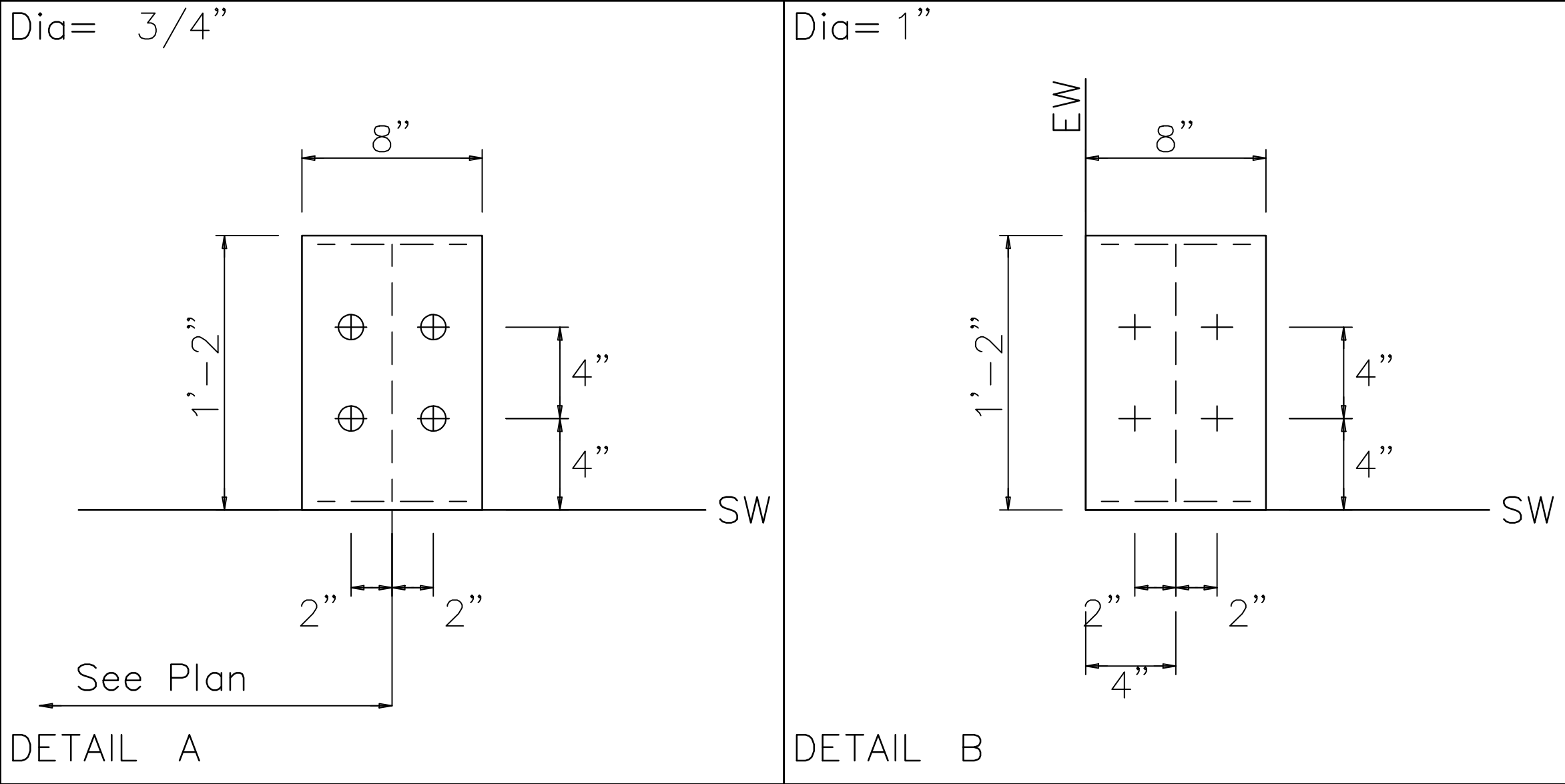
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PROJECT	Building Addition	ANCHOR BOLT REACTIONS		
ID	WilsonContainers71x60Addition	DESIGN:	DRAFT:	CHECK:
PROJECT	1291 SE Baya Drive	DATE: 2/24/25	SHEET 3 OF 14	
ADDRESS	Lake City, FI 32025			





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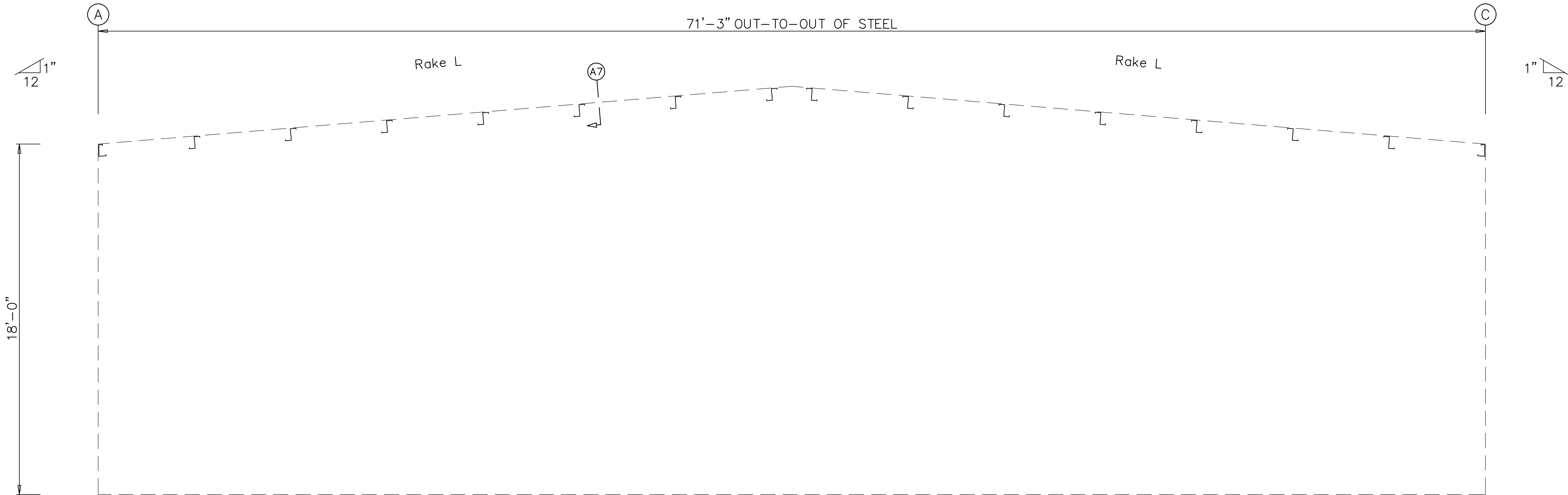
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PROJECT	Building Addition		ANCHOR BOLT DETAILS		
ID	WilsonContainers71x60Addition		DESIGN:	DRAFT:	CHECK:
PROJECT	1291 SE Baya Drive		DATE: 2/24/25		SHEET 4 OF 14
ADDRESS	Lake City, FI 32025				

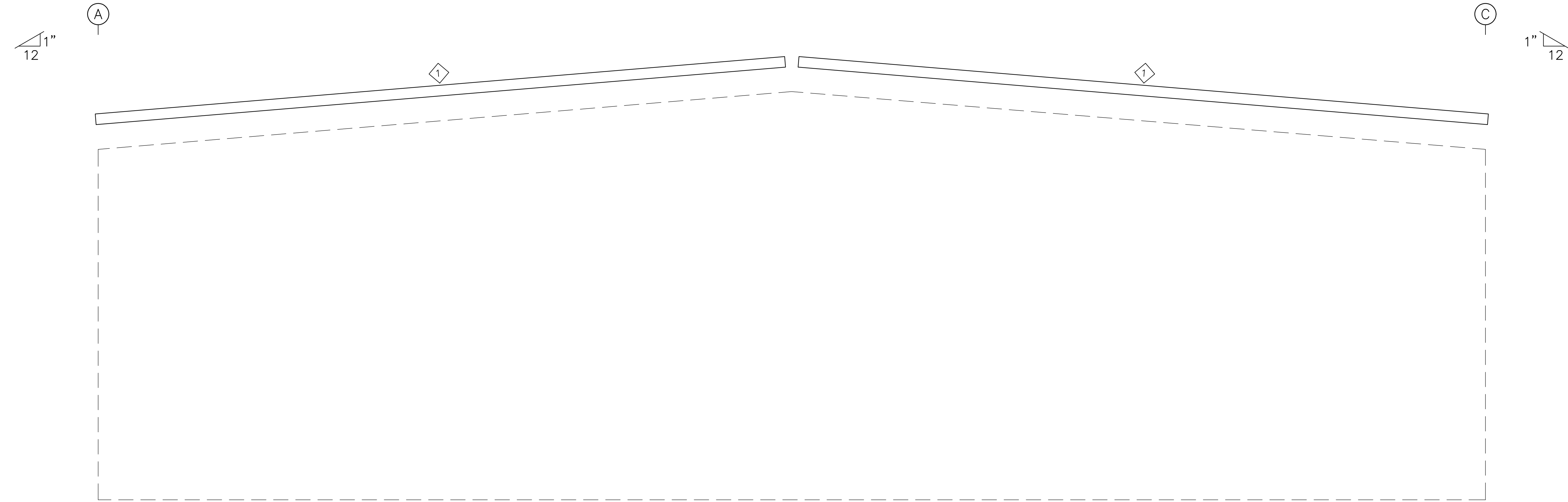
END OF BUILDING GABLE RAKE ANGLE #12 S. D. SCREW ROOF PURLIN ENDWALL RAFTER WELDED PLATE BOLT, 0.5" BY 1"(TYP) A7 SECTION THRU ENDWALL RAFTER	OUT OF ROOF STEEL RAKE ANGLE (~RA) TOP OF PURLIN @ SHEETED ENDWALL BTM. OF PURLIN @ OPEN ENDWALL ROOF PURLIN RIGID FRAME RAFTER 1/2"Ø x 1 1/4" A307 BOLTS (2 TYP.) FLANGE BRACE - IF REQ'D. (SEE CROSS SECTION OR ENDWALL FRAMING ELEV.) A10 ROOF PURLIN TO EXPANDABLE ENDWALL RIGID FRAME	(SEE ROOF PLAN) LAP LAP 1/2"Ø x 1 1/4" A307 HEX HD. (TYP.) 2 bolts std. ROOF PURLIN varies varies RIGID FRAME RAFTER RAFTER FLANGE BRACE (1 or 2 req, see RF Drawing) G2 TYPICAL SECTION THROUGH PURLIN LAP	EAVE STRUT FRAME COLUMN BOLTS, 0.5" x 1.5" J1 EAVE STRUT TO RIGID FRAME	EAVE STRUT END OF BUILDING BOLTS, .5"x1.0" J23 EAVE STRUT TO RIGID FRAME	SHOP CLAMP ON CABLE CABLE EYEBOLT COLUMN OR RAFTER WEB HILLSIDE WASHER FLAT WASHER NUT SLOT IN WEB TO INSERT HILLSIDE WASHER AND EYEBOLT Q2 DIAGONAL CABLE, EYEBOLT END
SIDEWALL COLUMN ANCHOR BOLTS BASE PLATE R2 ANCHOR BOLTS AT SIDEWALL COLUMN	12" 12" 12" 2" 8" 2" 8" 2" 8" 2" 8" 2" 8" 2" 8" PANEL FASTENER LOCATIONS, DETAIL "A" (EAVE MEMBER, PEAK MEMBER AND PANEL END LAPS) PANEL FASTENER LOCATIONS, DETAIL "A" (ALL MEMBERS EXCEPT AS NOTED) MEMBER SCREWS CONTINUOUS TAPE SEAL ROOF PANEL PURLIN STITCH SCREW AT 20" O.C. TAPE SEAL (FOR ROOF ONLY) 3/8 X 3/32 THICK (STD) EXTENDED LEG (ON "PBR" PANEL ONLY) DETAIL "A" NOTE: SCREW PATTERNS SHOWN SATISFY U.L. 90 REQUIREMENTS FOR ROOF FASTENER LOCATION - "PBR" PANEL AT ROOF DRAWING NO. SCREW_4	5" Gutter Field Tie Downspout Strap Finished Floor 1'-1 1/2" 2 1/2" Lap 3 1/2" 1 3/4" 1 3/4" 1 11/16" 2" 1 11/16" Interior Downspout Exterior Fastener Downspout Always measure the required length starting at the swaged end. After cutting to length, insert the swaged end of the cut downspout into the top of the lower downspout. Secure with pop rivets. 3 1/2" x 5 3/8" Roll Form Downspout Elevation DRAWING NO. TRIM_175 1. A Downspout Strap is required at all Downspout Splices.	FASTENER FASTENER TRIM FASTENER FASTENER ROOF PANEL SEALANT SHEETING ANGLE CLOSURE FASTENER SEALANT WALL PANEL PURLIN GABLE TRIM_229	RIBBED ROOF PANEL STRAP EAVE TRIM EAVE STRUT WALL PANEL GUTTER GUTTER FLASHING TRIM_317	STITCH FASTENERS RIDGE ROLL PURLIN RIDGE ROLL TRIM_321
TOP BOLTS, SEE DRAWING FOR SIZE. INTERMEDIATE BOLTS, THE NUMBER OF TOP OR BOTTOM BOLTS GREATER THAN FOUR. BOTTOM BOLTS, SEE DRAWING FOR SIZE. EXTENSION BEYOND FLANGE IS OPTIONAL, AT TOP & BOTTOM. U2 BOLTED END PLATE CONNECTION AT BUILDING PEAK	BOLTED END PLATE TOP FLANGE WEB BOTTOM FLANGE TOP BOLTS, SEE DRAWING FOR SIZE. INTERMEDIATE BOLTS, THE NUMBER OF TOP OR BOTTOM BOLTS GREATER THAN FOUR. BOTTOM BOLTS, SEE DRAWING FOR SIZE. U3 BOLTS FOR RAFTER TO COLUMN CONNECTION				



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PROJECT	Building Addition	STRUCTURAL DETAILS		
ID	WilsonContainers71x60Addition	DESIGN:	DRAFT:	CHECK:
PROJECT ADDRESS	1291 SE Baya Drive Lake City, FL 32025	DATE: 2/24/25	SHEET 5 OF 14	



ENDWALL FRAMING: FRAME LINE 1



ENDWALL SHEETING & TRIM: FRAME LINE 1

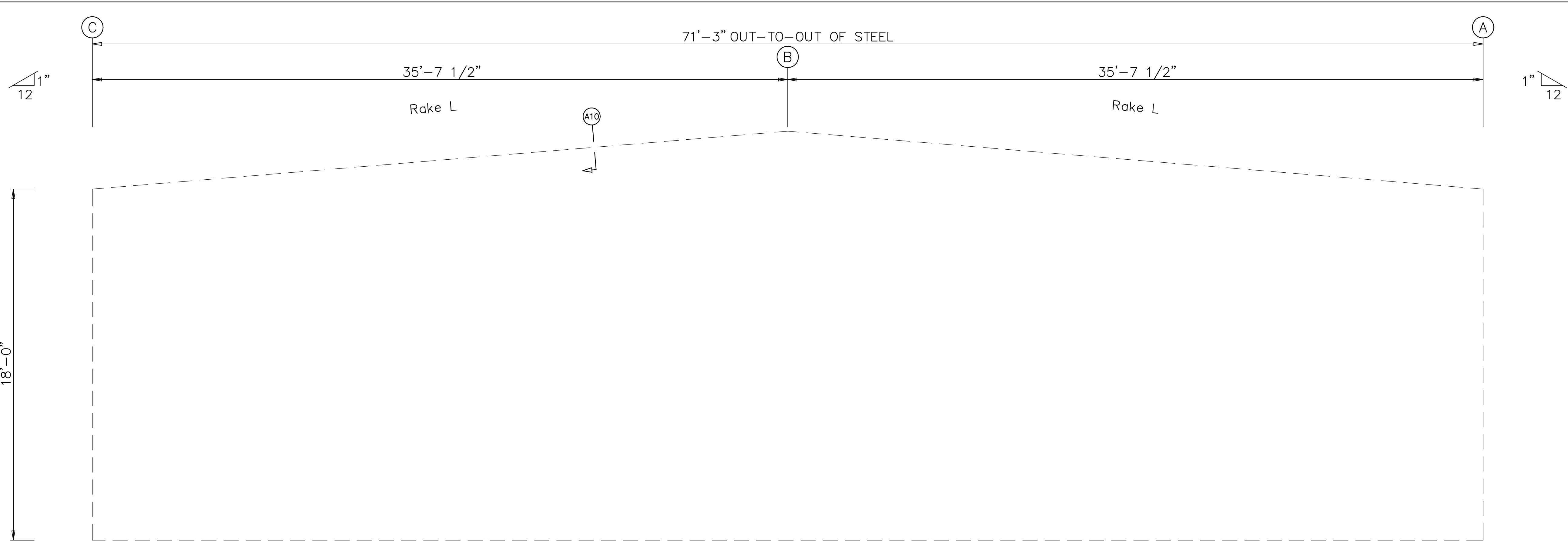
TRIM TABLE			
FRAME LINE 1			
◇ID	PART	LENGTH	DETAIL
1	RT-101	20'-3"	TRIM_229

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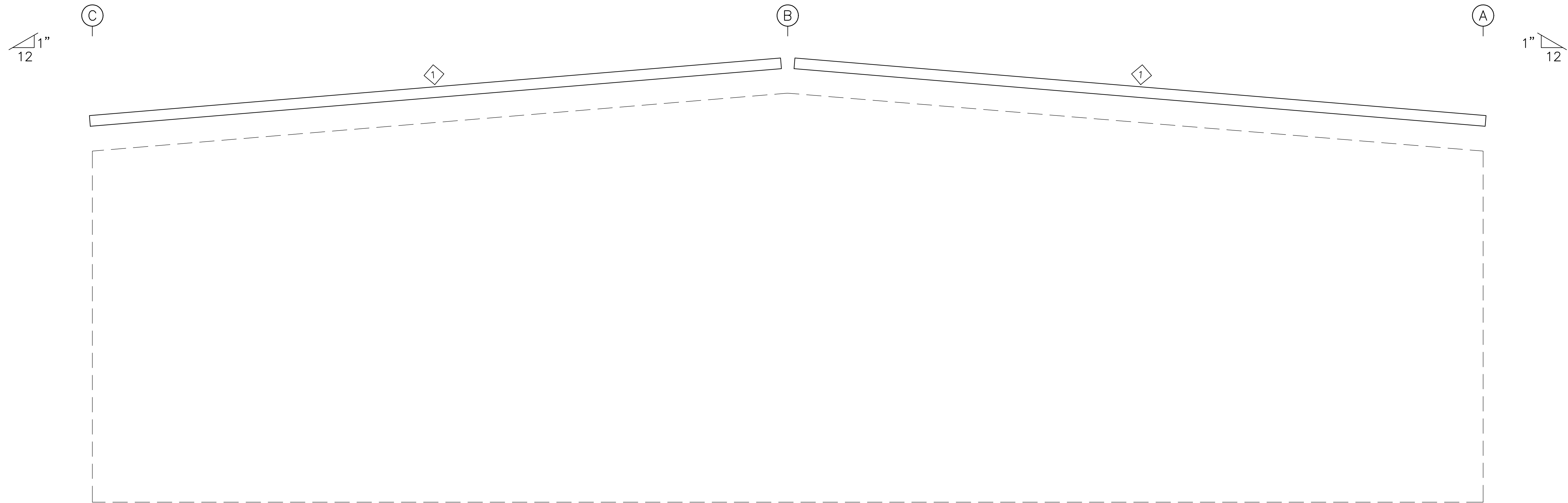
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ID	WilsonContainers71x60Addition		DESIGN:	DRAFT:	CHECK:
PROJECT	1291 SE Baya Drive		DATE: 2/24/25		SHEET 6 OF 14
ADDRESS	Lake City, FL 32025				





ENDWALL FRAMING: FRAME LINE 4



ENDWALL SHEETING & TRIM: FRAME LINE 4

TRIM TABLE			
FRAME LINE 4			
◇ID	PART	LENGTH	DETAIL
1	RT-101	20'-3"	TRIM_229



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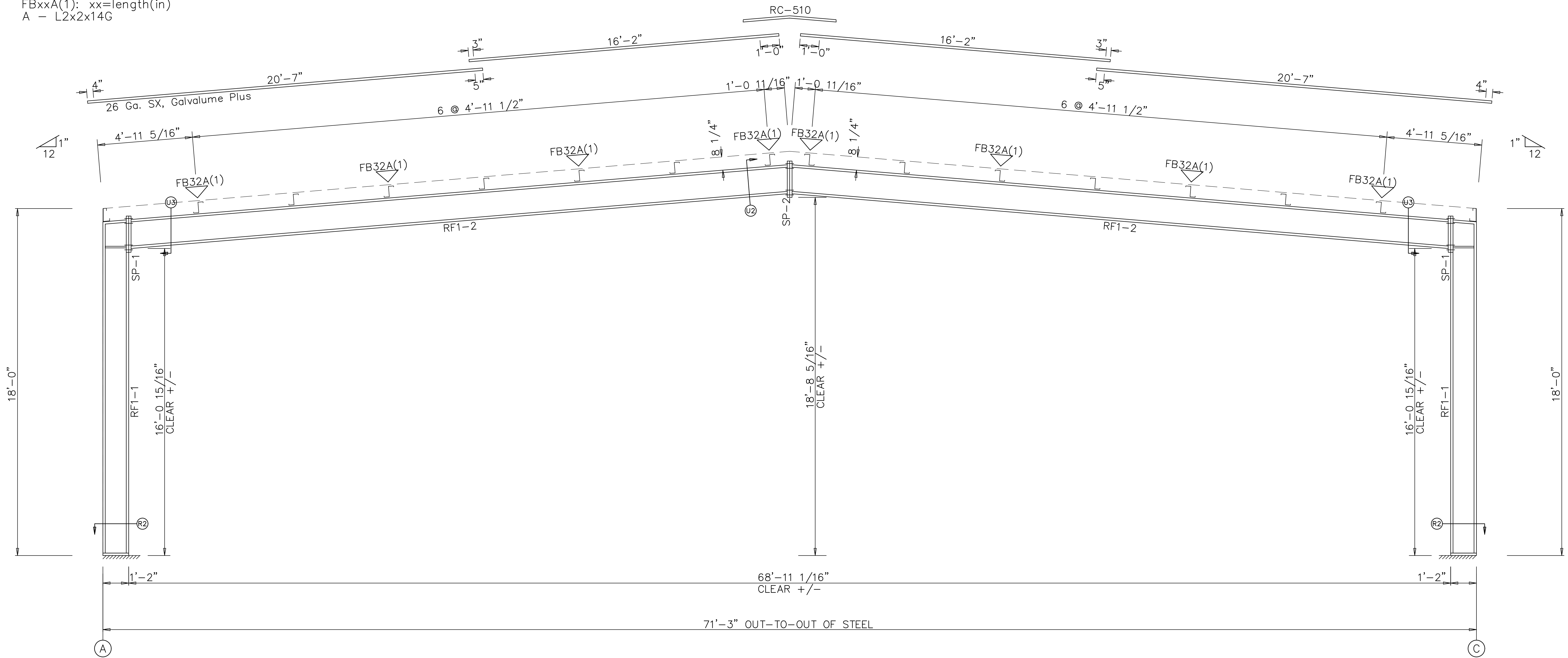
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PROJECT	Building Addition		ENDWALL FRAMING		
ID	WilsonContainers71x60Addition		DESIGN:	DRAFT:	CHECK:
PROJECT	1291 SE Baya Drive		DATE: 2/24/25		SHEET 7 OF 14
ADDRESS	Lake City, FI 32025				

SPLICE BOLT TABLE						
Mark	Qty		Int	Type	Dia	Length
	Top	Bot				
SP-1	4	4	0	A325	0.750	2.00
SP-2	4	4	0	A325	0.750	1.75

MEMBER SIZE TABLE		
MARK	MEMBER	LENGTH
RF1-1	W14X34	17'-3 3/4"
RF1-2	W16X31	34'-6 15/16"

▽FLANGE BRACES: Both Sides(U.N.)
FBxxA(1): xx=length(in)
A - L2x2x14G



RIGID FRAME ELEVATION: FRAME LINE 4



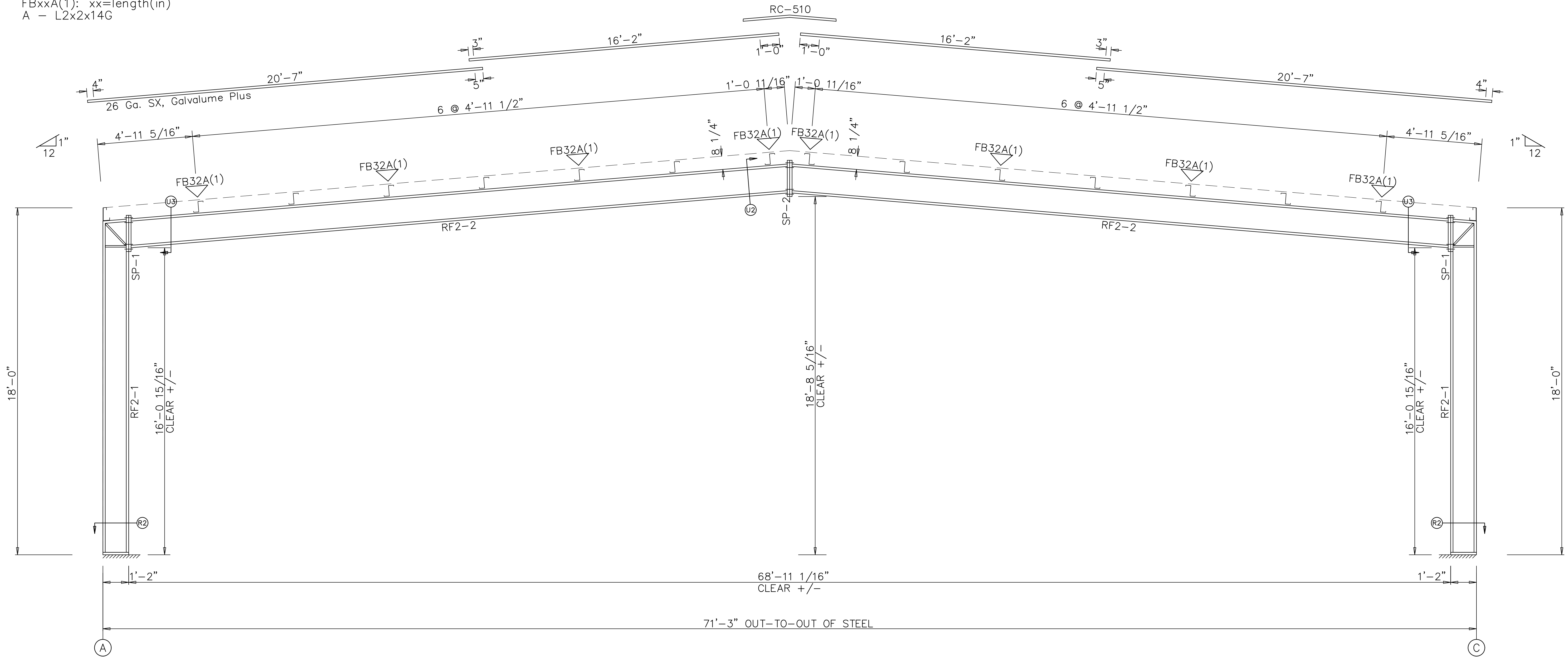
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ID	WilsonContainers71x60Addition	DESIGN:	DRAFT:	CHECK:
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ADDRESS	Lake City, FL 32025			

SPLICE BOLT TABLE						
Mark	Qty		Int	Type	Dia	Length
	Top	Bot				
SP-1	4	4	0	A325	1.000	2.50
SP-2	4	4	0	A325	0.750	2.00

MEMBER SIZE TABLE		
MARK	MEMBER	LENGTH
RF2-1	W14X34	17'-3 3/4"
RF2-2	W16X31	34'-6 13/16"

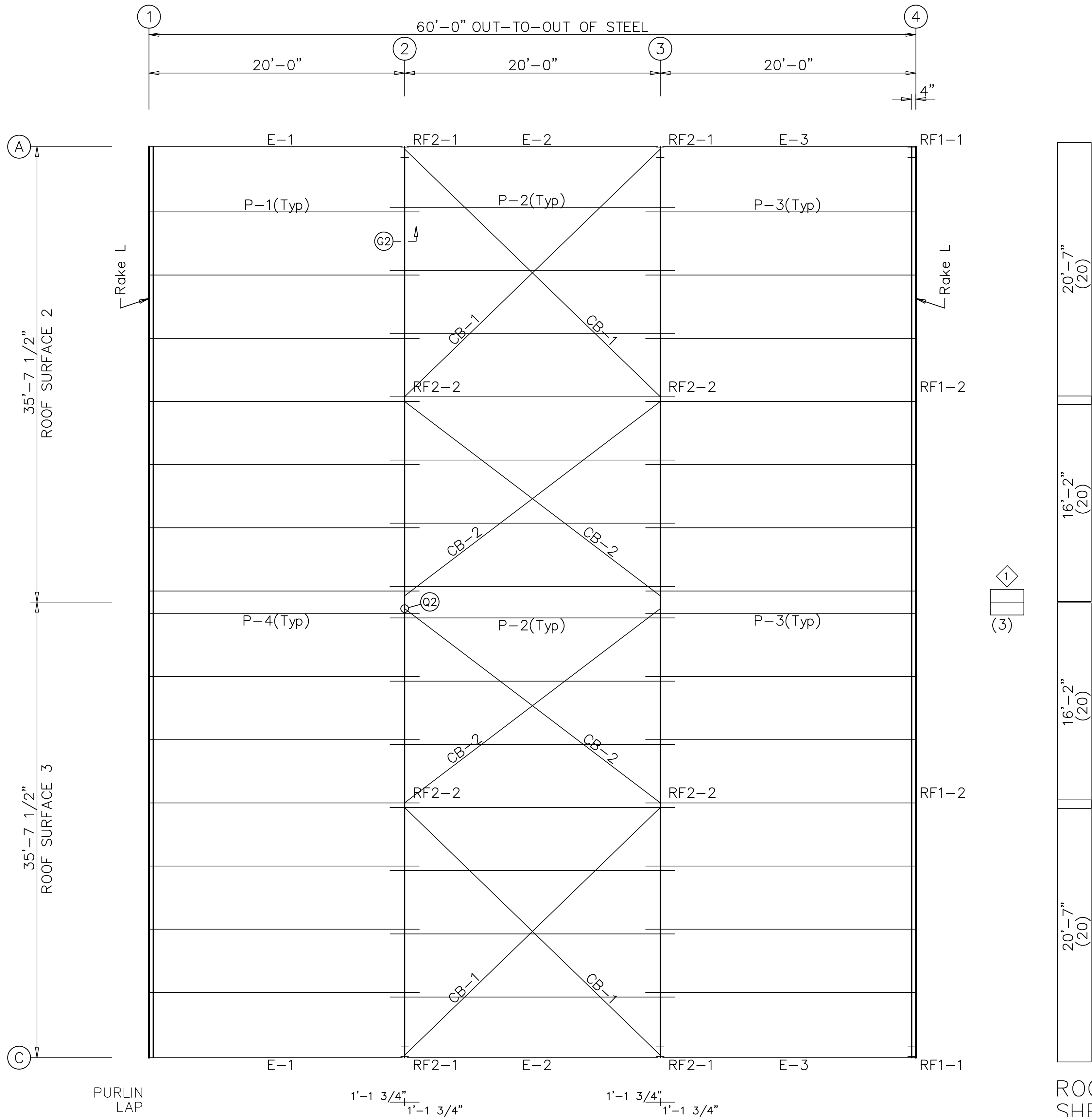
▽FLANGE BRACES: Both Sides(U.N.)
FBxxA(1): xx=length(in)
A - L2x2x14G



RIGID FRAME ELEVATION: FRAME LINE 2 3



Union LaSteel				
PROJECT	Building Addition	RIGID FRAME ELEVATION		
ID	WilsonContainers71x60Addition	DESIGN:	DRAFT:	CHECK:
PROJECT ADDRESS	1291 SE Baya Drive Lake City, FL 32025	DATE: 2/24/25	SHEET 9 OF 14	



ROOF FRAMING PLAN

ROOF SHEETING
PANELS: 26 Ga. SX
Galvalume Plus

TRIM TABLE		
ROOF PLAN		
ID	PART	LENGTH
1	RC-510	20'-3"
MEMBER TABLE		
ROOF PLAN		
MARK	PART	LENGTH
P-1	8X25Z16	21'-1 1/2"
P-2	8X25Z16	22'-3 1/2"
P-3	8X25Z16	21'-1 1/2"
P-4	8X25Z16	21'-1 1/2"
E-1	E085341L	19'-11 1/2"
E-2	E085341L	19'-11 1/2"
E-3	E085341L	19'-11 1/2"
CB-1	0.25_CBL	26'-1 3/4"
CB-2	0.25_CBL	23'-7"



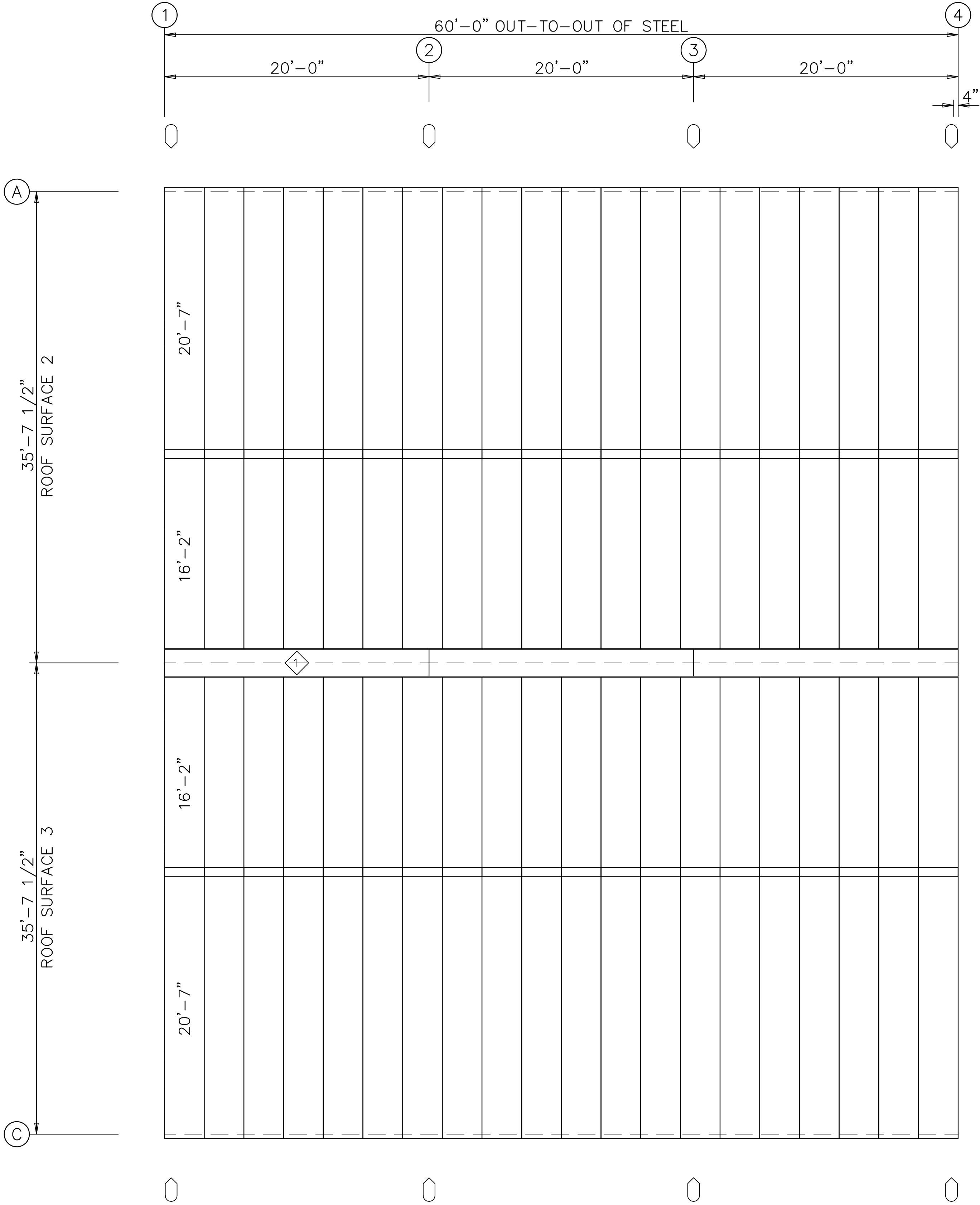
ASG LLC
AMERICAN STRUCTURAL GROUP
CONSULTING STRUCTURAL ENGINEERS

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PH 513-402-7600
WWW.ASG-ENG.COM
"Value Engineering the First Time"

Union LaSteel				
PROJECT	Building Addition	ROOF FRAMING		
ID	WilsonContainers71x60Addition	DESIGN:	DRAFT:	CHECK:
PROJECT ADDRESS	1291 SE Baya Drive Lake City, FL 32025	DATE: 2/24/25	SHEET 10 OF 14	

 DOWNSPOUT LOCATIONS

TRIM TABLE			
ROOF PLAN			
ID	PART	LENGTH	DETAIL
1	RC-510	20'-3"	TRIM_321



ROOF SHEETING PLAN
 PANELS: 26 Ga. SX – Galvalume Plus

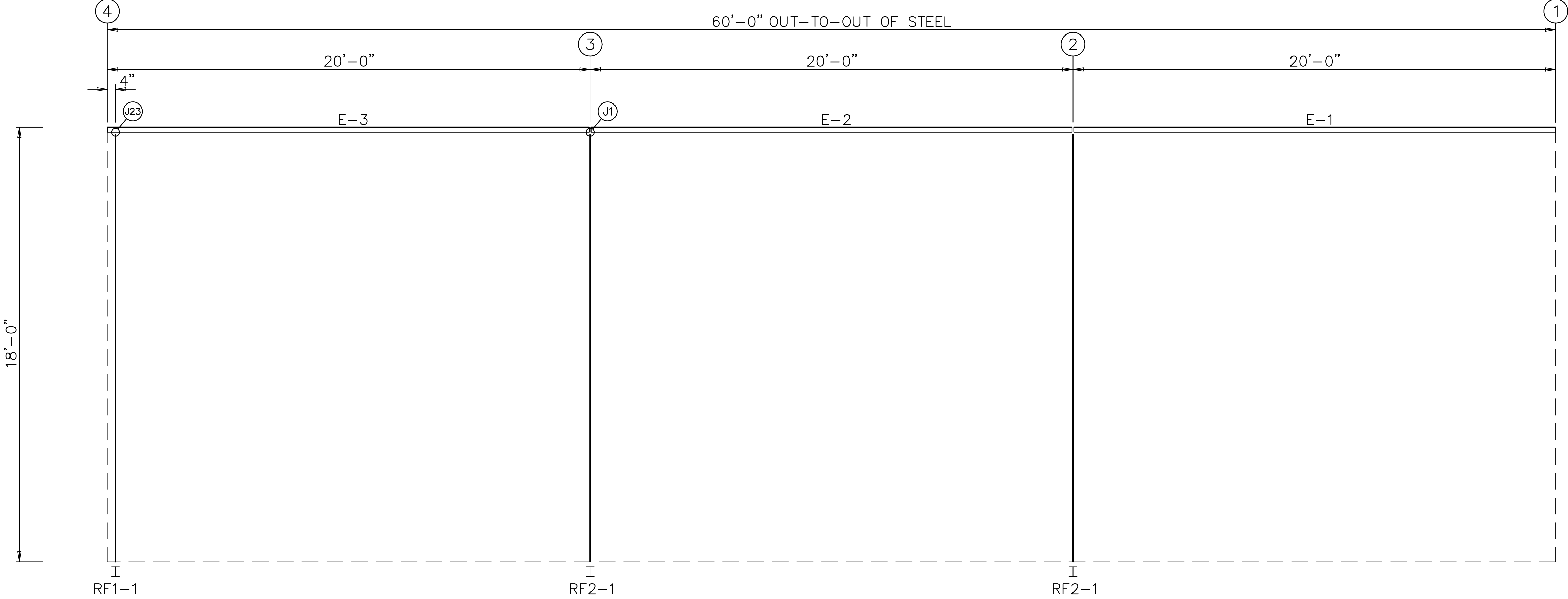




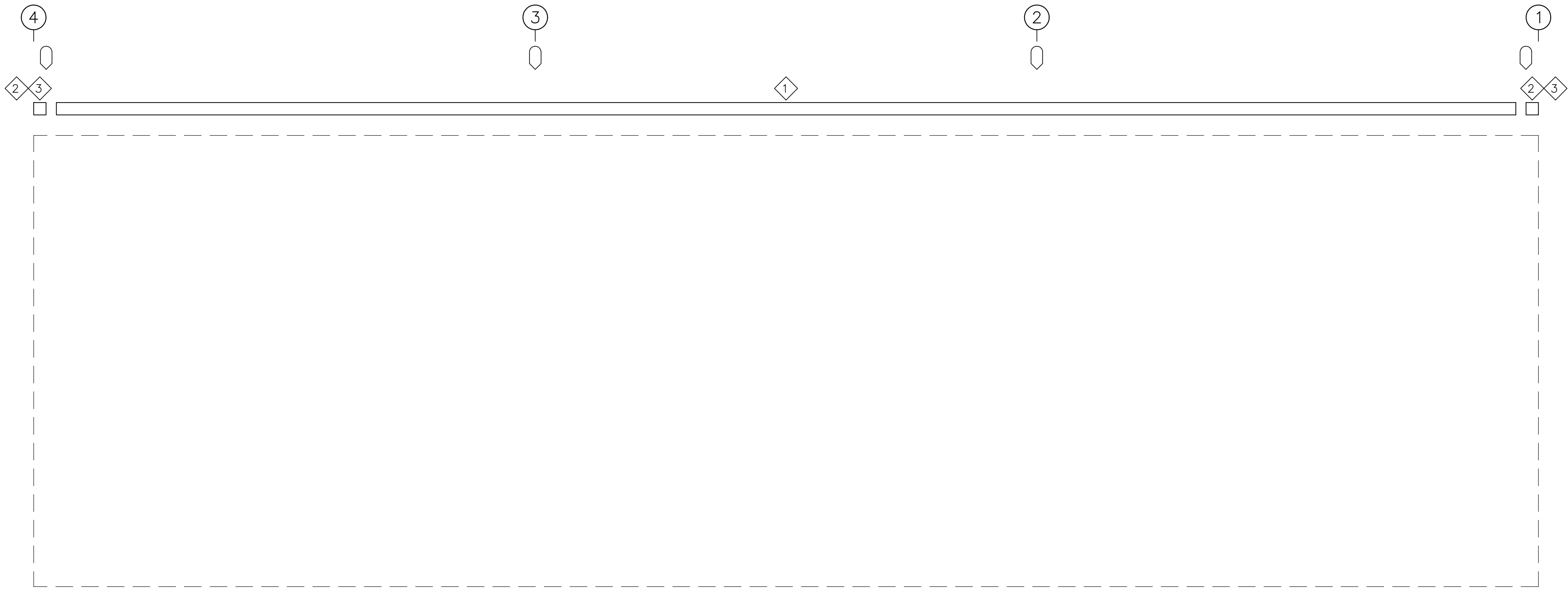
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Union LaSteel					
PROJECT	Building Addition		ROOF SHEETING		
ID	WilsonContainers71x60Addition		DESIGN:	DRAFT:	CHECK:
PROJECT ADDRESS	1291 SE Baya Drive Lake City, FL 32025		DATE: 2/24/25		SHEET 11 OF 14

⬇️ DOWNSPOUT LOCATIONS



SIDEWALL FRAMING: FRAME LINE A



SIDEWALL SHEETING & TRIM: FRAME LINE A

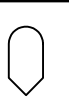
MEMBER TABLE		
FRAME LINE A		
MARK	PART	LENGTH
E-1	E085341L	19'-11 1/2"
E-2	E085341L	19'-11 1/2"
E-3	E085341L	19'-11 1/2"

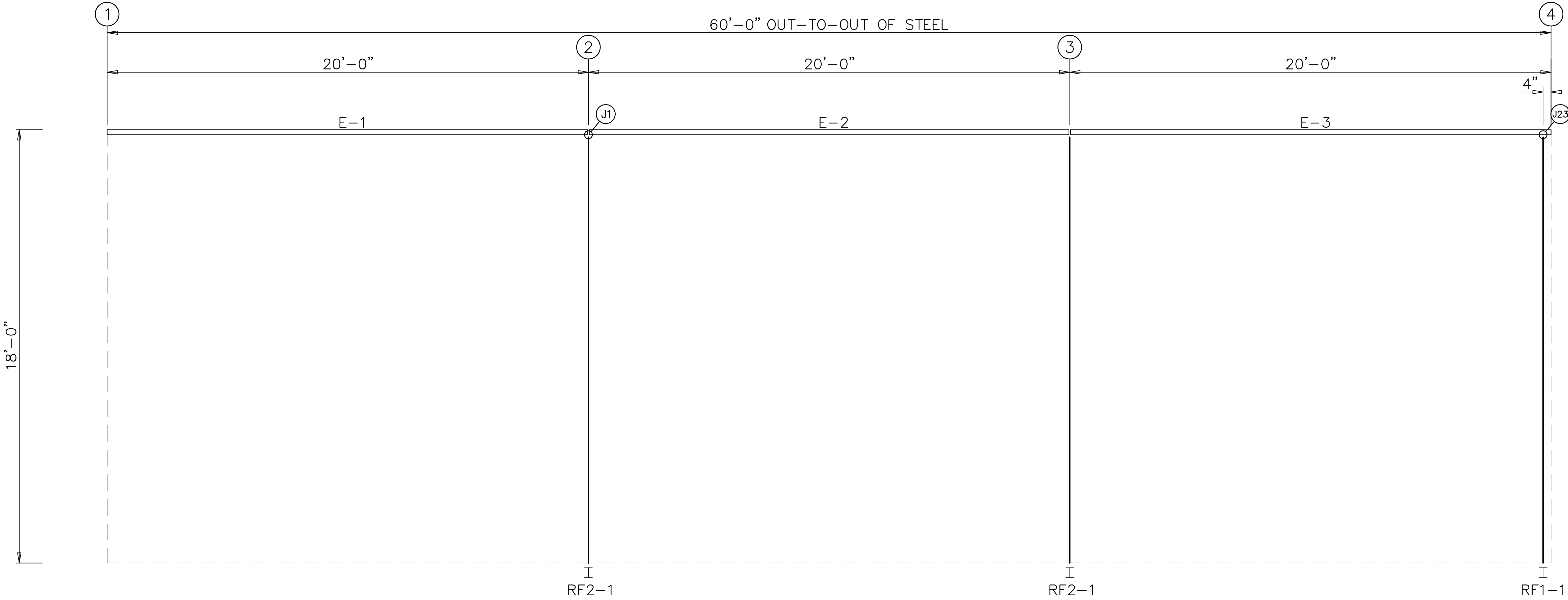
TRIM TABLE		
FRAME LINE A		
◇ID	PART	LENGTH
1	GU-121	20'-3"
2	EC-121	6"
3	SCB	6"



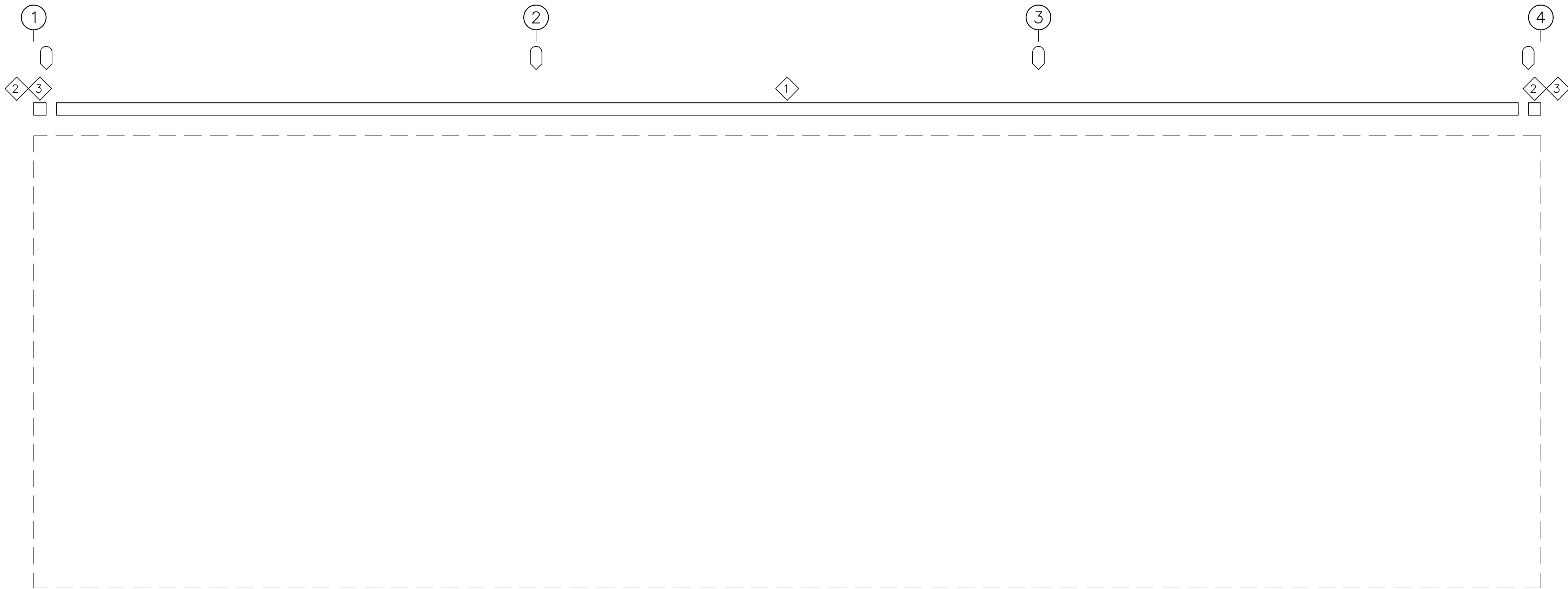
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Union LaSteel					
PROJECT	Building Addition		SIDEWALL FRAMING		
ID	WilsonContainers71x60Addition		DESIGN:	DRAFT:	CHECK:
PROJECT	1291 SE Baya Drive		DATE: 2/24/25		SHEET 12 OF 14
ADDRESS	Lake City, FL 32025				

 DOWNSPOUT LOCATIONS



SIDEWALL FRAMING: FRAME LINE C



SIDEWALL SHEETING & TRIM: FRAME LINE C

MEMBER TABLE FRAME LINE C		
MARK	PART	LENGTH
E-1	E085341L	19'-11 1/2"
E-2	E085341L	19'-11 1/2"
E-3	E085341L	19'-11 1/2"

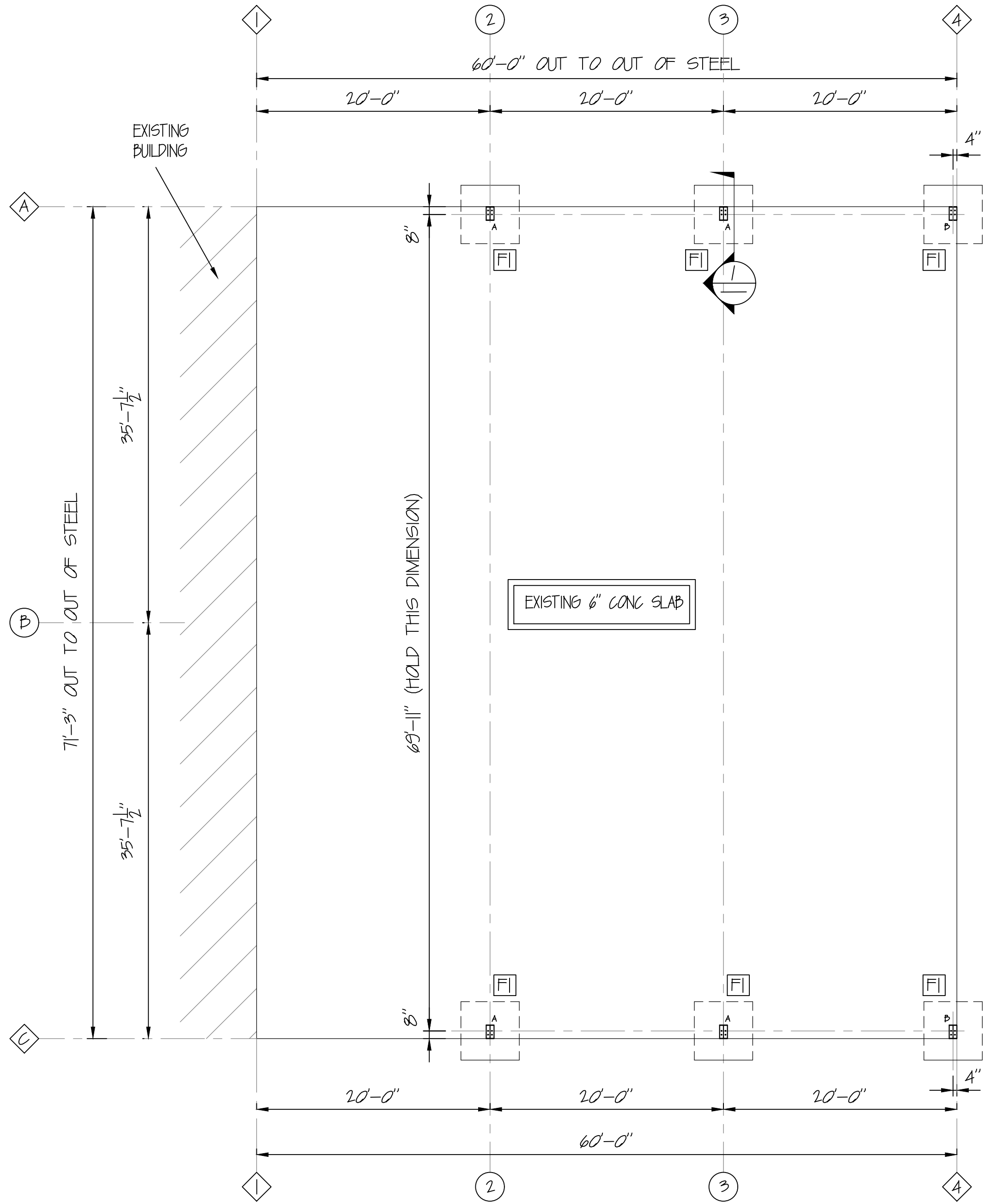
TRIM TABLE FRAME LINE C		
◇ID	PART	LENGTH
1	GU-121	20'-3"
2	EC-121	6"
3	SCB	6"





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Union LaSteel					
PROJECT	Building Addition		SIDEWALL FRAMING		
ID	WilsonContainers71x60Addition		DESIGN:	DRAFT:	CHECK:
PROJECT ADDRESS	1291 SE Baya Drive Lake City, FL 32025		DATE: 2/24/25	SHEET 13 OF 14	



FOUNDATION PLAN
NOTE: All Base Plates @ 100'-0" (U.N.)

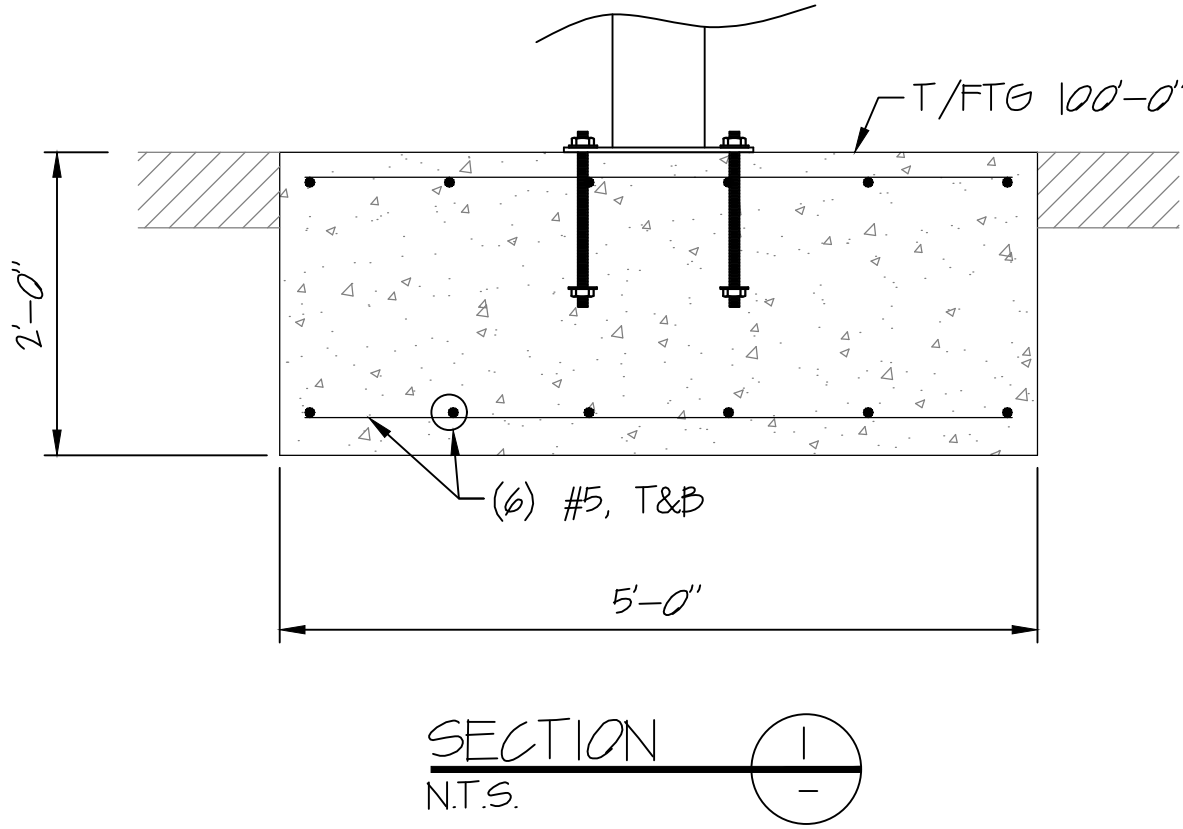
CONCRETE

- ALL CONCRETE WORK SHALL CONFORM TO ACI 301-18, "SPECIFICATIONS FOR BUILDINGS", EXCEPT AS NOTED BELOW. REFER TO CHAPTER 16 OF ACI 301 FOR REPORTS FROM TESTS THAT SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER, ARCHITECT, OWNER, ANY APPLICABLE CONTRACTORS, CONCRETE SUPPLIER, AND BUILDING OFFICIAL.
- CONCRETE WORK IN COLD WEATHER SHALL CONFORM TO ALL REQUIREMENTS OF ACI 308R-16 "STANDARD SPECIFICATION FOR COLD WEATHER CONCRETING" AND ACI 308R-16 "COLD WEATHER CONCRETING".
- CONCRETE WORK IN HOT WEATHER SHALL CONFORM TO ALL REQUIREMENTS OF ACI 308R-10 "HOT WEATHER CONCRETING". THE AIR TEMPERATURE, RELATIVE HUMIDITY, CONCRETE TEMPERATURE, AND WIND VELOCITY SHALL BE ENTERED INTO NOMOGRAPH FIG. 2.15 TO DETERMINE IF PRECAUTIONS AGAINST PLASTIC SHRINKAGE ARE REQUIRED.
- REBAR FABRICATION, PLACEMENT, SPlicing, ETC. SHALL BE IN ACCORDANCE WITH THE LATEST EDITIONS OF ACI 318, THE CONCRETE REINFORCING STEEL INSTITUTE (CRSI) HANDBOOK AND ANY OTHER APPLICABLE PUBLICATIONS.
- A SLUMP TEST SHALL BE PERFORMED BEFORE ANY HRWR IS ADDED.
- CONCRETE MATERIAL SPECIFICATION: 3000 PSF f'c BASED ON 28 DAYS STRENGTH.
- UNLESS OTHERWISE NOTED, REINFORCEMENT BARS TO HAVE MINIMUM CONCRETE COVER AS SHOWN BELOW:

CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH:	3 INCHES
CONCRETE EXPOSED TO EARTH OR WEATHER:	
#5 BARS AND SMALLER	1 1/2 INCHES
OTHER	2 INCHES
CONCRETE NOT EXPOSED TO EARTH OR WEATHER:	
BEAM AND COLUMN BARS INCLUDING TIES, STIRRUPS AND SPIRALS	1 1/2 INCHES
SLABS, WALLS AND JOISTS:	
#11 BARS AND SMALLER	3/4 INCHES
OTHERS	1 1/2 INCHES
- ALL EXPOSED EDGES ARE TO BE CHAMFERED 3/4 INCH, UNO.
- LAP SPLICE LENGTHS TO BE OF CLASS "B" PER ACI.
- CONCRETE REINFORCEMENT SHALL CONFORM TO ASTM A615 OR A616, GRADE 60.
- ISOLATED COLUMN FOOTINGS HAVE BEEN DESIGNED WITH AN ASSUMED ALLOWABLE SOIL BEARING PRESSURE OF 1500 PSF, UPON ENCOUNTERING ANY ADVERSE SOIL CONDITIONS CONTACT THE STRUCTURAL ENGINEER.
- DO NOT CAST CONCRETE AGAINST WATER OR FROZEN WATER.

FOOTING SCHEDULE			
FND MARK	FND SIZE (L x B x D)	REINFORCING	
		TOP, E.W.	BOTT, E.W.
F1	5'-0"x5'-0"x2'-0"	(6)~#5's	(6)~#5's

THE SOIL BEARING PRESSURE IS ASSUMED TO BE 1500 PSF MAXIMUM. UPON ENCOUNTERING SOILS LESS THAN THIS CONTACT THE ENGINEER.



Union LaSteel				
PROJECT	Building Addition	FOUNDATION PLAN		
ID	WilsonContainers71x60Addition	DESIGN: TSM	DRAFT: GSM	CHECK: TSM
PROJECT ADDRESS	1291 SW Baya Drive Lake City, Fl. 32025	DATE: 09/21/23	SHEET 14 OF 14	

