50

Lumber design values are in accordance with ANSI/TPI 1-2007 section 6.3
These truss designs rely on lumber values established by others.

RE: ELIX-SISTRUNK - ROOF DESIGN INFO

MiTek USA, Inc.
6904 Parke East Blvd.
Tampa, FL 33610-4115

Site Information:
Customer Info: SISTRUNK Project Name: SISTRUNK Model:
Lot/Block: . Subdivision: .
Address: .
City: COLUMBIA COUNTY State: FLORIDA

Name Address and License # of Structural Engineer of Record, If there is one, for the building.
Name: License #:
Address:
City: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):
Design Code: FBC2010 Design Program: OnLine Plus 30.0.014
Wind Code: ASCE 7-10 Wind Speed: 120 mph Floor Load: N/A psf
Roof Load: 40.0 psf

This package includes 2 individual, dated Truss Design Drawings and 0 Additional Drawings.
With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No.	Seal#	Truss Name	Date
1	T4589408	R1	10/26/012
2	T4589409	R2GE	10/26/012

The truss drawing(s) referenced above have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by Mayo Truss Company, Inc..

Truss Design Engineer's Name: Albani, Thomas
My license renewal date for the state of Florida is February 28, 2013.

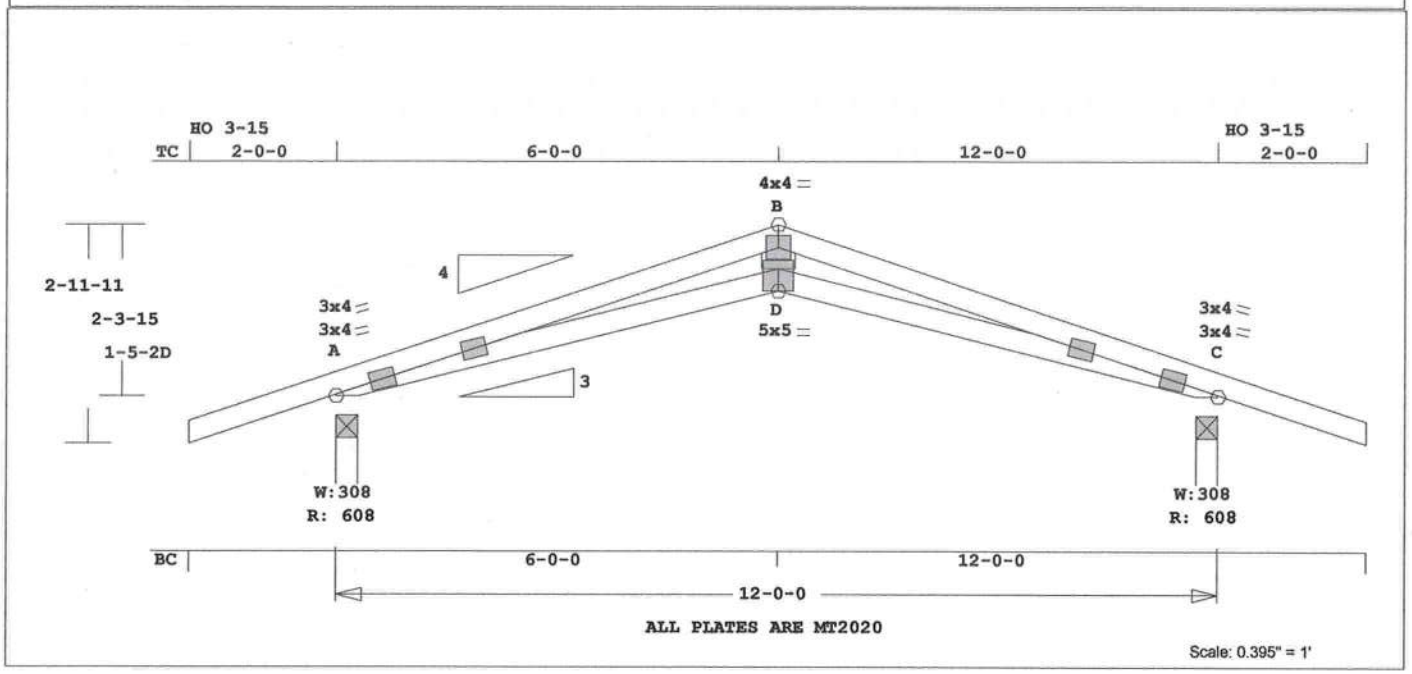
NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Sec. 2.

FL Cert. 6634

October 26,2012

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ELIX-SISTRUNK	R1	16	SCIS	120000	4	2- 0- 0	2- 0- 0	T4589408

SISTRUNK



MiTek® Online Plus™ APPROX. TRUSS WEIGHT: 56.6 LBS

Online Plus -- Version 30.0.014
RUN DATE: 26-OCT-12

Southern Pine lumber design values are those effective 06-01-12 by SPIB//ALSC UON

	CSI	-Size-	----Lumber----
TC	0.41	2x 4	SP-#2
BC	0.74	2x 4	SP-#2
WB	0.16	2x 6	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	12- 0- 0
or	36.0"	0- 0- 0	12- 0- 0
BC	Cont.	0- 0- 0	12- 0- 0
or	84.0"	0- 0- 0	12- 0- 0

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0
Total	40.0	Spacing 24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25

	Fb	Fc	Ft	Emin
TC	1.15	1.10	1.10	1.10
BC	1.10	1.10	1.10	1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	608		19 R
C	608		19 R

Jt	Brg Size	Required
A	3.5"	1.5"
C	3.5"	1.5"

Plus 21 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.41	2376 C	0.10 0.31
B -C	0.41	2376 C	0.10 0.31
--------Bottom Chords---------			
A -D	0.74	2334 T	0.49 0.25
D -C	0.74	2334 T	0.49 0.25
------------Webs-------------			
D -B	0.16	1243 T	

TL Defl -0.43" in D -C L/315
LL Defl -0.17" in D -C L/793

Hz Disp	LL	DL	TL
Jt C	0.11"	0.16"	0.27"

Shear // Grain in A -B 0.15

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	MT20	3.0x 4.0	7.4	2.5	0.53
A	MT20	3.0x 4.0-7.4-2.5			0.26
B	MT20	4.0x 4.0 Ctr	Ctr		0.72
C	MT20	3.0x 4.0-7.4	2.5		0.53
C	MT20	3.0x 4.0	7.4-2.5		0.26
D	MT20	5.0x 5.0 Ctr-1.1			0.67

REVIEWED BY:
MiTek Industries, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ONLINE PLUS GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

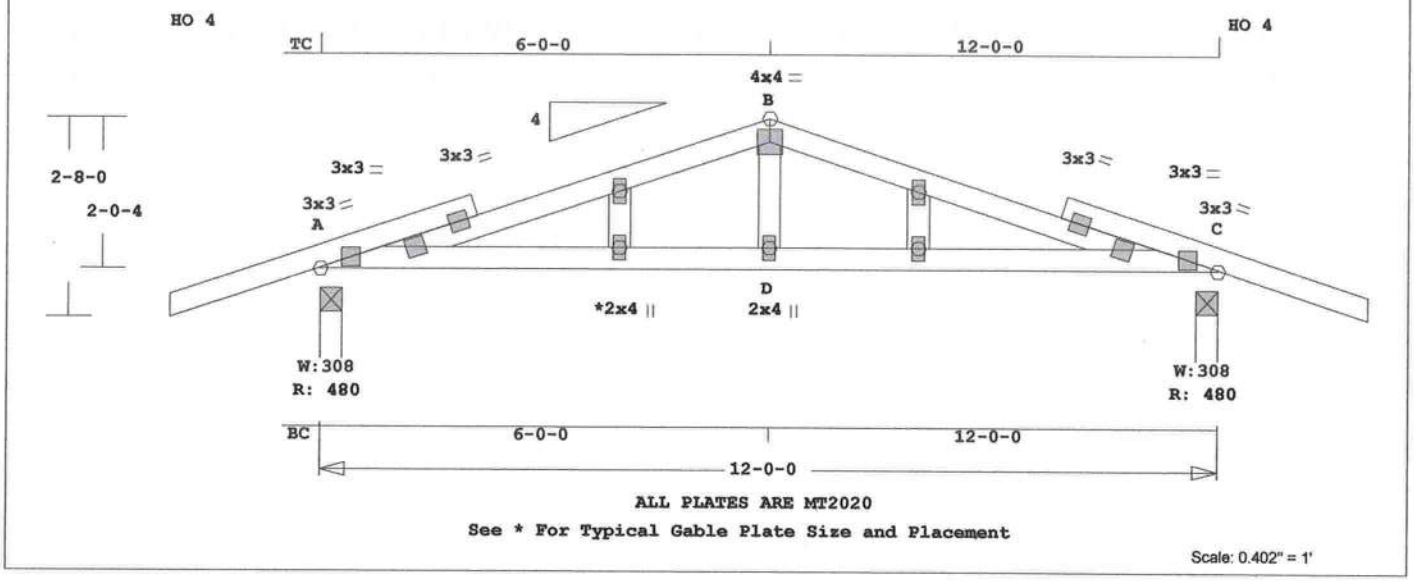
NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2010
TPI 2007
OH Loading
Soffit psf 2.0
This truss has been designed for 20.0 psf LL on the B.C. in areas where a rectangle 3- 6- 0 tall by 2- 0- 0 wide will fit between the B.C. and any other member.
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-10
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 120 mph
Risk Category : II
Mean Roof Height: 15-0
Exposure Category: B
Building Type: Enclosed
TC Dead Load: 6.0 psf
BC Dead Load: 6.0 psf
Max comp. force 2376 Lbs
Max tens. force 2334 Lbs
Connector Plate Fabrication Tolerance = 20%
This truss is designed for a creep factor of 1.5 which is used to calculate total load deflection.

FL Cert. 6634

October 26,2012

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ELIX-SISTRUNK	R2GE	2	TR	120000	4	0	0	T4589409

SISTRUNK



MiTek® Online Plus™ APPROX. TRUSS WEIGHT: 65.3 LBS

Online Plus -- Version 30.0.014
RUN DATE: 26-OCT-12

Southern Pine lumber design values are those effective 06-01-12 by SPIB//ALSC UON

	CSI	-Size-	----Lumber----
TC	0.25	2x 4	SP-#2
BC	0.39	2x 4	SP-#2
WB	0.04	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	12- 0- 0
or	48.0"	0- 0- 0	12- 0- 0
BC	Cont.	0- 0- 0	12- 0- 0
or	120.0"	0- 0- 0	12- 0- 0

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0
Total	40.0	Spacing 24.0"
Lumber Duration Factor		1.25
Plate Duration Factor		1.25

	Fb	Fc	Ft	Emin
TC	1.15	1.10	1.10	1.10
BC	1.10	1.10	1.10	1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	480		16 R
C	480		16 R

Jt	Brg Size	Required
A	3.5"	1.5"
C	3.5"	1.5"

Plus 21 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.25	937 C	0.01 0.24
B -C	0.25	937 C	0.01 0.24
--------Bottom Chords---------			
A -D	0.39	912 T	0.19 0.20
D -C	0.39	912 T	0.19 0.20
-------------Webs-------------			
D -B	0.04	232 T	

TL Defl -0.08" in A -D L/999
LL Defl -0.03" in A -D L/999
Shear // Grain in A -B 0.18

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	MT20	3.0x 3.0	Ctr	Ctr	0.61
B	MT20	4.0x 4.0	Ctr	Ctr	0.81
C	MT20	3.0x 3.0	Ctr	Ctr	0.61
D	MT20	2.0x 4.0	Ctr	Ctr	0.21

2 Gable studs to be attached with 2.0x4.0 plates each end.
REVIEWED BY:
MiTek Industries, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ONLINE PLUS GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2010
TPI 2007
WARNING Do Not Cut overframe member between outside of truss and first tie-plate to inside of heel plate.
Design checked for 10 psf non-concurrent LL on BC.
Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified Building Designer as per ANSI/TPI 1.
Wind Loads - ANSI / ASCE 7-10
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 120 mph

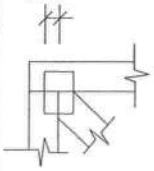
Risk Category : II
Mean Roof Height: 15-0
Exposure Category: B
Building Type: Enclosed
TC Dead Load: 6.0 psf
BC Dead Load: 6.0 psf
Max comp. force 937 Lbs
Max tens. force 912 Lbs
Connector Plate Fabrication Tolerance = 20%
This truss is designed for a creep factor of 1.5 which is used to calculate total load deflection.

FL Cert. 6634

October 26,2012

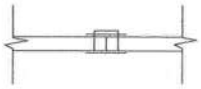
ONLINE PLUS GENERAL NOTES & SYMBOLS

PLATE LOCATION



Center plates on joints unless otherwise noted in plate list or on drawing. Dimensions are given in inches (i.e. 1 1/2" or 1.5") or IN-16ths (i.e. 108)

FLOOR TRUSS SPLICE (3X2, 4X2, 6X2)



(W) = Wide Face Plate
(N) = Narrow Face Plate

LATERAL BRACING

Designates the location for continuous lateral bracing (CLB) for support of individual truss members only. CLBs must be properly anchored or restrained to prevent simultaneous buckling of adjacent truss members.

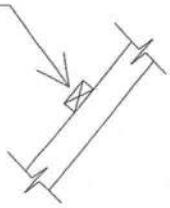
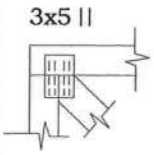


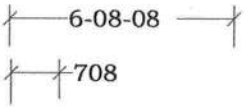
PLATE SIZE AND ORIENTATION

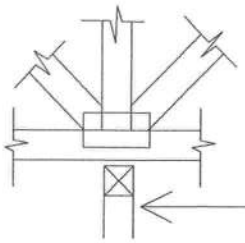


The first dimension is the width measured perpendicular to slots. The second dimension is the length measured parallel to slots. Plate orientation, shown next to plate size, indicates direction of slots in connector plates.

DIMENSIONS

All dimensions are shown in FT-IN-SX (i.e. 6'-8.5" or 6-08-08). Dimensions less than one foot are shown in IN-SX only (i.e. 708).





W = Actual Bearing Width (IN-SX)
R = Reaction (lbs.)
U = Uplift (lbs.)

BEARING

When truss is designed to bear on multiple supports, interior bearing locations should be marked on the truss. Interior support or temporary shoring must be in place before trusses are installed. If necessary, shim bearings to assure solid contact with truss.

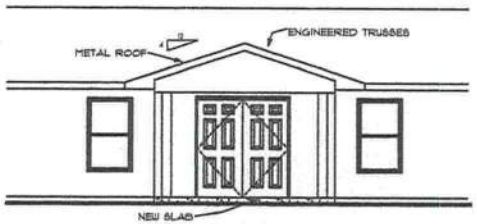
Metal connector plates shall be applied on both faces of truss at each joint. Center the plates, unless indicated otherwise. No loose knots or wane in plate contact area. Splice only where shown. Overall spans assume 4" bearing at each end, unless indicated otherwise. Cutting and fabrication shall be performed using equipment which produces snug-fitting joints and plates. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication and the attached truss designs are not applicable for use with fire retardant lumber and some preservative treatments. Nails specified on Truss Design Drawings refer to common wire nails, except as noted. The attached design drawings were prepared in accordance with " National Design Specifications for Wood Construction" (AF & PA), " National Design Standard for Metal Plate Connected Wood Truss Construction" (ANSI/TPI 1), and HUD Design Criteria for Trussed Rafters.

Mitek Industries Inc. bears no responsibility for the erection of trusses, field bracing or permanent truss bracing. Refer to "Building Component Safety Information" (BCSI 1) as published by Truss Plate Institute, 218 North Lee Street, Suite 312, Alexandria, Virginia 22314. Persons erecting trusses are cautioned to seek professional advice concerning proper erection bracing to prevent toppling and " dominoing ". Care should be taken to prevent damage during fabrication, storage, shipping and erection. Top and bottom chords shall be adequately braced in the absence of sheathing or rigid ceiling, respectively. It is the responsibility of others to ascertain that design loads utilized on these drawings meet or exceed the actual dead loads imposed by the structure and the live loads imposed by the local building code or historical climatic records. When truss hangers are specified on the Truss Design Drawing, they must be installed per manufacturer's details and specifications.

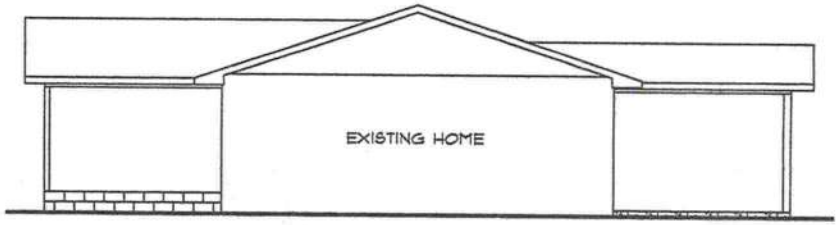
FURNISH A COPY OF THE ATTACHED TRUSS DESIGN DRAWINGS TO ERECTION CONTRACTOR. IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER TO REVIEW THESE DRAWINGS AND VERIFY THAT DATA, INCLUDING DIMENSIONS & LOADS, CONFORM TO ARCHITECTURAL PLAN / SPECS AND THE TRUSS PLACEMENT DIAGRAM FURNISHED BY THE TRUSS MANUFACTURER.

MiTek USA, Inc.
6904 Parke East Blvd.
Tampa, FL 33610-4115

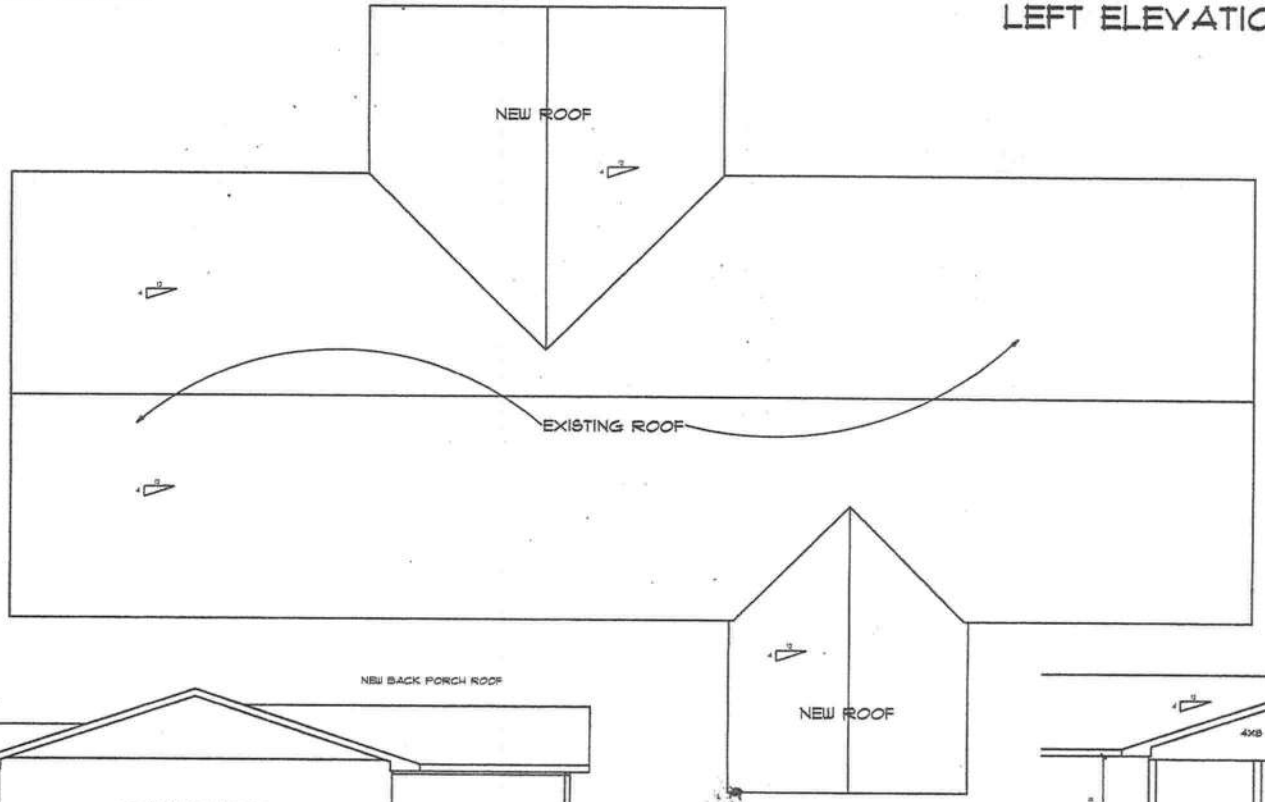
Tel: 813-972-1135
Fax: 813-971-6117

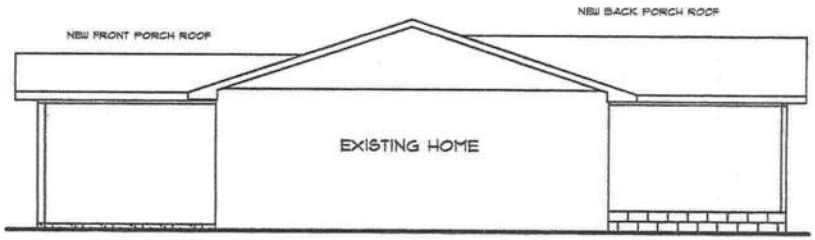


FRONT ELEVATION

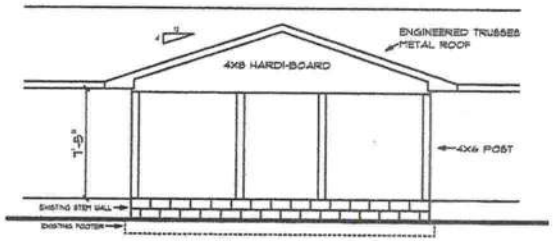


LEFT ELEVATION





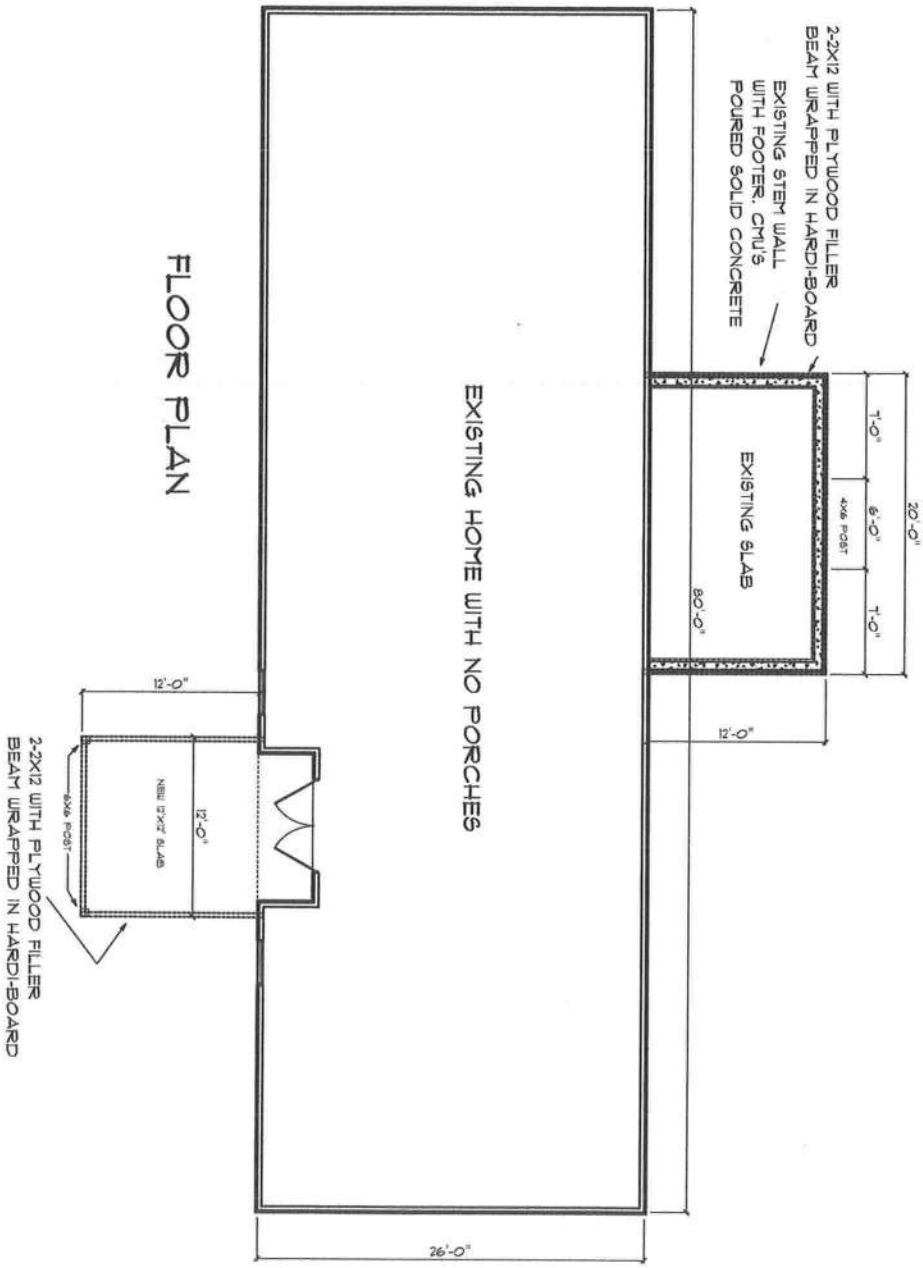
RIGHT ELEVATION



BACK ELEVATION

SISTRUNK PORCH ADDITION

DRAWN BY:
JASON ELIXSON CONSTRUCTION, LLC
4853 WEST SR. 238
LAKE BUTLER, FL. 32054
(386) 623-1741



		SISTRUNK PORCH ADDITION	DRAWN BY:
			JASON ELIXSON CONSTRUCTION, LLC 4853 WEST SR. 238 LAKE BUTLER, FL. 32054 (386) 623-1741

Schafer Engineering, LLC

14705 Main St. Alachua FL 32615

Prepared for:

JASON ELIXSON CONSTRUCTION
SISTRUNK PORCH ADDITION
COLUMBIA COUNTY, FLORIDA

By:

Schafer Engineering, LLC

386-462-1340 / 352-375-6329

NO COPIES ARE TO BE PERMITTED

SCHAFER ENGINEERING, LLC

7104 NW 42ND LANE \ GAINESVILLE FL. 32606
PHONE: 386-462-1340 \ 352-375-6329

Trusses: Pre-engineered, pre-fabricated with the manufacturer's required bracing system installed.

Roof Sheathing: Type: OSB Size: 7/16" Fastener type nails: 8d / .113 Ring Shank
Interior zone spacing: Interior: 6" Periphery: 4"
Edge and end zone spacing: Interior: 6" Periphery: 4"

Double Top Plate: Type: Spruce Grade: #1 #2 Size: 2 x 4 Nail Spacing: 8" o.c.

Stud Type: Spruce Grade: #1 #2 Size: 2 x 4
Interior stud spacing: 16" End stud spacing: 16"

Shear Wall Siding: Type: OSB Thickness: 7/16"
6 ft Trans: Fastener 8d/131 Spacing: Int: 8 Edge: 4"
6 ft Trans: Fastener 8d/131 Spacing: Int: 8 Edge: 4"

Allowable Unit Shear on Shear Walls: 314 pounds per linear foot
Unit Shear Transferred from Diaphragm: Trans: 254 Long: 207

Wall Tension Transferred by: Siding Nails: 8d/131 @ 4" O.C. Edges

Foundation Anchor Bolts: Concrete Strength: 3000 psi Size: 1/2"
Washer: 2" Embedment: 7" Location of first anchor bolt from corner: 8"

Anchor Bolts @ 48" o.c. Model: A307 Loc. from corner: 8"
Type of Foundation: (1) - #5 rebar continuous required in bond beam.
Floor Slab: 4" Cmu size: 8" x 16" Height: 24" Rein.: #5 at 72" o.c.

Monolithic Footing: Depth: 20" Bottom Width: 12 Rein.: 2 #5 rebars

Stemwall Footing: Width: 20 Depth: 10 Rein.: 2 #5 rebar
Interior Footings 16" Wide X 10" Deep with 2-#5 rebar continuous

Porch Columns: 6 x 6 x 8' syp pt @ 148" o.c. max spacing Column Fasteners: Simpson CB66 \ (2) LCE4 or equal

Special Comments: Install ceiling diaphragm on open porch using same size nail, same nail spacing and same grade material as roof sheathing.

Notes:

1. Balloon frame all gable ends unless accompanied by gable end detail
2. All trusses must bear on exterior walls and porch beams.
3. All walls to be nailed with same nailing pattern as the shear walls.
4. This wind load is not valid without a raised, embossed seal. (NO COPIES).
5. 1500 psf soil bearing pressure minimum.
6. Fiber mesh or WWM may be used in concrete slab. All steel must be grade 40 min.
7. Trusses must be installed and anchored in accordance to the truss engineering.
8. All headers spanning over 12' must be pre-engineered.
9. The foundatioin is for minimum design use, and may be increased.
10. Wind load is for one use only \ FBC-2010 \ No copies permitted

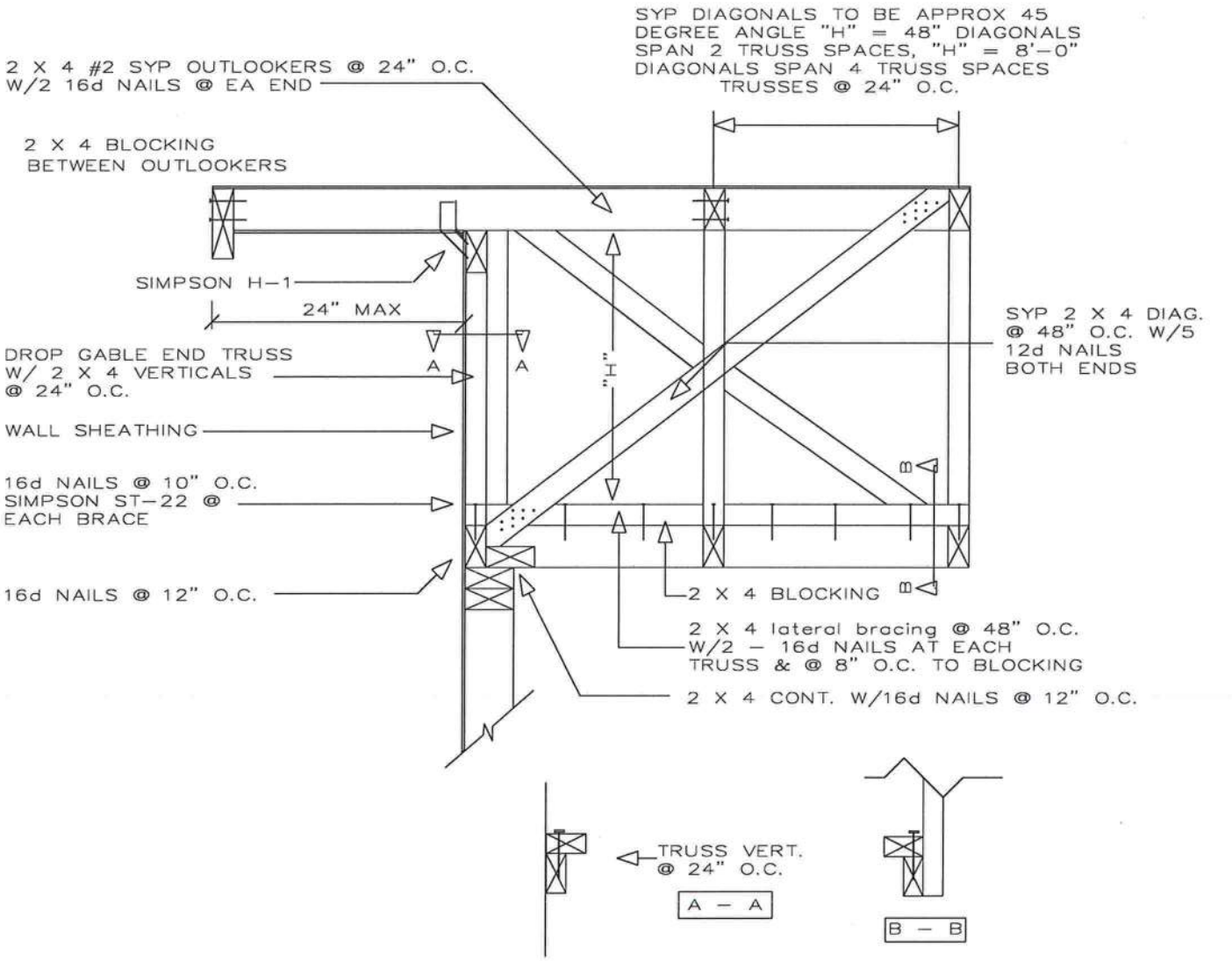
10-24-12

Bruce Schafer, P. E. #48984
7104 NW 42ND LN
GAINESVILLE, FL. 32606

SCHAFER ENGINEERING, LLC

7104 NW 42ND LANE \ GAINESVILLE FL. 32606

PHONE: 386-462-1340 \ 352-375-6329



TYPICAL GABLE END BRACING

10-24-12

Bruce Schafer, P. E. #48984
7104 NW 42ND LN
GAINESVILLE, FL. 32606

SCHAFER ENGINEERING, LLC

7104 NW 42ND LANE \ GAINESVILLE FL. 32606

PHONE: 386-462-1340 \ 352-375-6329

TIE-DOWN TABLES

HEADER STRAPPING				
Uplift Lbs	Top Connector	Rating Lbs	Bottom Connector	Rating Lbs
to 455	LSTA19	635	H3	320
to 910	LSTA12	795	2-H3	640
to 1265	LSTA18	1110	LTT19	1305
to 1750	2-LSTA12	1810	LTT20	1750
to 2530	2-LSTA18	2530	HD2A-2.5	2165
to 2865	3-LSTA18	3255	HD2A-3.5	2865
to 3700	3-LSTA24	3880	HD5A-3	3130

Total the uplift for each truss sitting on the header and divide by 2 to determine the uplift on the header. Use proper bolt anchors sufficient to support required uplift loads.

TRUSSES \ GIRDERS			
Uplift Lbs	Top Connector	Bottom Connector	Rating Lbs
to 535	H2.5A	NA	
to 1015	H10A	NA	
to 1215	TS22	LTT19	1305
to 1750	2-TS22	LTT20	1750
to 2570	2-TS22	HD2A	2775
to 3665	3-TS22	HD5A	4010
to 5420	2-MST37	HTT22	5250
to 9660	2-MST60	HD10A	9540

Two 12d common toenials are required per truss for each bearing point into top plate.
It is the contractors responsibility to provide a continuous load path from truss to foundation.

	TOP CONNECTOR	RATING LBS	BOTTOM CONNECTOR	RATING LBS
BEAM SEATS	LSTA18	1110	LTT19	1305
POSTS	2-LSTA18	2220	ABU44	2300

1. Simpson or equivlent hardware may be used. For nailing into spruce members, multiply table values by .86
2. See truss engineering for anchor uplift values.
3. This schedule is not meant to be a replacement to the specified values of any manufactures values.

Bruce Schafer, P. E. #48984
7104 NW 42ND LN
GAINESVILLE, FL. 32606

Wind Load Design per ASCE 7-10

User Input Data		
Structure Type	Building	
Basic Wind Speed (V)	135	mph
Structural Category	II	
Exposure	B	
Struc Nat Frequency (n1)	1	Hz
Slope of Roof (Theta)	18.4	Deg
Type of Roof	Gabled	
Eave Height (Eht)	8.00	ft
Ridge Height (RHt)	10.54	ft
Mean Roof Height (Ht)	9.52	ft
Width Perp. to Wind (B)	12.00	ft
Width Parallel to Wind (L)	12.00	ft
Damping Ratio (beta)	0.01	

Red values should be changed only through "Main Menu"

Calculated Parameters		
Type of Structure		
Height/Least Horizontal Dim	0.79	
Flexible Structure	No	

Calculated Parameters		
Importance Factor	1	
Non-Hurricane, Hurricane (v=85-100 mph) & Alaska		
Table C6-4 Values		
Alpha =	7.000	
zg =	1200.000	
At =	0.143	
Bt =	0.840	
Am =	0.250	
Bm =	0.450	
Cc =	0.300	
l =	320.00	ft
Epsilon =	0.333	
Zmin =	30.00	ft

Gust Factor Category I: Rigid Structures - Simplified Method			
Gust1	For rigid structures (Nat Freq > 1 Hz) use 0.85	0.85	
Gust Factor Category II: Rigid Structures - Complete Analysis			
Zm	Zmin	30.00	ft
Izm	Cc * (33/z)^0.167	0.3048	
Lzm	l*(zm/33)^Epsilon	309.99	ft
Q	(1/(1+0.63*((B+Ht)/Lzm)^0.63))^0.5	0.9460	
Gust2	0.925*((1+1.7*Izm*3.4*Q)/(1+1.7*3.4*Izm))	0.8932	
Gust Factor Category III: Flexible or Dynamically Sensitive Structures			
Vhref	V*(5280/3600)	198.00	ft/s
Vzm	bm*(zm/33)^Am*Vhref	87.00	ft/s
NF1	NatFreq*Lzm/Vzm	3.56	Hz
Rn	(7.47*NF1)/(1+10.302*NF1)^1.667	0.0627	
Nh	4.6*NatFreq*Ht/Vzm	0.50	
Nb	4.6*NatFreq*B/Vzm	0.63	
Nd	15.4*NatFreq*Depth/Vzm	2.12	
Rh	1/Nh-(1/(2*Nh^2)*(1-Exp(-2*Nh)))	0.7344	
Rb	1/Nb-(1/(2*Nb^2)*(1-Exp(-2*Nb)))	0.6832	
Rd	1/Nd-(1/(2*Nd^2)*(1-Exp(-2*Nd)))	0.3616	
RR	((1/Beta)*Rn*Rh*Rb*(0.53+0.47*Rd))^0.5	1.4843	
gg	+(2*LN(3600*n1))^0.5+0.577/(2*LN(3600*n1))^0.5	4.19	
Gust3	0.925*((1+1.7*Izm*(3.4^2*Q^2+GG^2*RR^2)^0.5)/(1+1.7*3.4*Izm))	1.55	

Gust Factor Summary			
Main Wind-force resisting system:		**Components and Cladding:**	
Gust Factor Category:	I	Gust Factor Category:	I
Gust Factor (G)	0.89	Gust Factor (G)	0.89

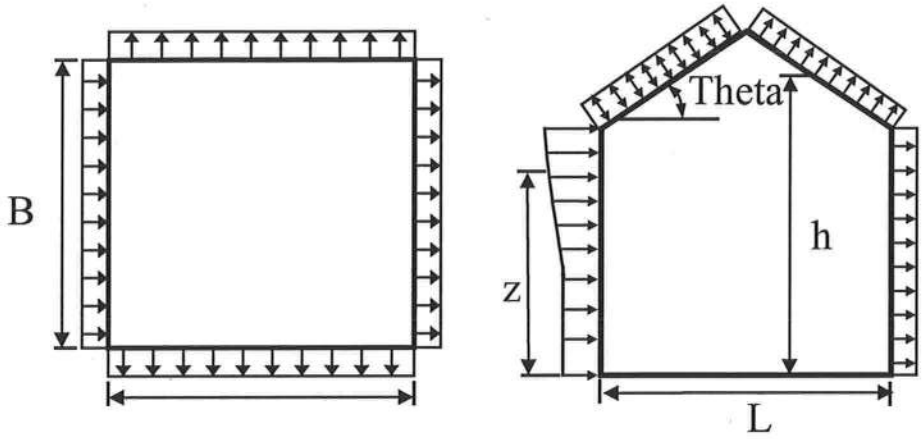
Wind Load Design per ASCE 7-10

6.5.12.2.1 Design Wind Pressure - Buildings of All Heights (Non-flexible)

Elev.	Kz	Kzt	Kd	qz	Pressure (lb/ft^2)	
					Windward Wall*	
ft			1.00	lb/ft^2	+GCpi	-GCpi
15	0.70	1.00	1.00	32.69	23.36	23.36

Figure 6-3 - External Pressure Coefficients, Cp

Loads on Main Wind-Force Resisting Systems



Variable	Formula	Value	Units
Kh	2.01*(15/zg)^(2/Alpha)	0.57	
Kht	Topographic factor (Fig 6-2)	1.00	
Qh	.00256*(V)^2*ImpFac*Kh*Kht*Kd	26.81	psf

Wall Pressure Coefficients, Cp	
Surface	Cp
Windward Wall (See Figure 6.5.12.2.1 for Pressures)	0.80

Roof Pressure Coefficients, Cp	
Roof Area (sq. ft.)	-
Reduction Factor	1.00

Description	Cp	Pressure (psf)	
		+GCpi	-GCpi
Leeward Walls (Wind Dir Parallel to 12 ft wall)	-0.50	-11.97	-11.97
Leeward Walls (Wind Dir Parallel to 12 ft wall)	-0.50	-11.97	-11.97
Side Walls	-0.70	-16.76	-16.76
Roof - Normal to Ridge (Theta>=10)			
Windward - Max Negative	-0.67	-16.09	-16.09
Windward - Max Positive	0.00	0.00	0.00
Leeward Normal to Ridge	-0.59	-14.05	-14.05
Overhang Top	-0.67	-16.09	-16.09
Overhang Bottom	0.80	0.71	0.71
Roof - Parallel to Ridge (All Theta)			
Dist from Windward Edge: 0 ft to 4.76 ft	-1.13	-27.17	-27.17
Dist from Windward Edge: 4.76 ft to 9.52 ft	-0.78	-18.74	-18.74
Dist from Windward Edge: 9.52 ft to 19.04 ft	-0.62	-14.78	-14.78
	0.00	0.00	0.00

Wind Load Design per ASCE 7-10

* Horizontal distance from windward edge

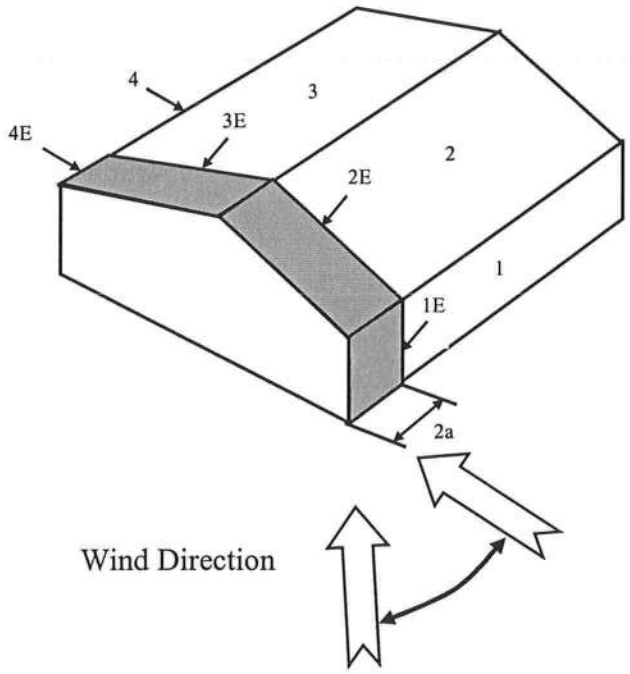
Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht <= 60 ft

Kh =	2.01*(15/zg)^(2/Alpha)	=	0.57
Kht =	Topographic factor (Fig 6-2)	=	1.00
Qh =	0.00256*(V)^2*ImpFac*Kh*Kht*Kd	=	26.81

Case A						
Surface	**GCpf**	**+GCpi**	**-GCpi**	**qh (psf)**	**Min P (psf)**	**Max P (psf)**
1	0.52	0	0	32.69	16.87	16.87
2	-0.69	0	0	32.69	-22.55	-22.55
3	-0.47	0	0	32.69	-15.31	-15.31
4	-0.42	0	0	32.69	-13.57	-13.57
5	0.00	0	0	32.69	0.00	0.00
6	0.00	0	0	32.69	0.00	0.00
1E	0.78	0	0	32.69	25.49	25.49
2E	-1.07	0	0	32.69	-34.97	-34.97
3E	-0.67	0	0	32.69	-22.00	-22.00
4E	-0.62	0	0	32.69	-20.19	-20.19
5E	0.00	0	0	32.69	0.00	0.00
6E	0.00	0	0	32.69	0.00	0.00

* p = qh * (GCpf - GCpi)



Wind Load Design per ASCE 7-10

Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht <= 60 ft

Kh =	2.01*(15/zg)^(2/Alpha)	=	0.57
Kht =	Topographic factor (Fig 6-2)	=	1.00
Qh =	0.00256*(V)^2*ImpFac*Kh*Kht*Kd	=	26.81

Case B						
Surface	**GCpf**	**+GCpi**	**-GCpi**	**qh (psf)**	**Min P (psf)**	**Max P (psf)**
1	-0.45	0	0	32.69	-14.71	-14.71
2	-0.69	0	0	32.69	-22.55	-22.55
3	-0.37	0	0	32.69	-12.09	-12.09
4	-0.45	0	0	32.69	-14.71	-14.71
5	0.40	0	0	32.69	13.07	13.07
6	-0.29	0	0	32.69	-9.48	-9.48
1E	-0.48	0	0	32.69	-15.69	-15.69
2E	-1.07	0	0	32.69	-34.97	-34.97
3E	-0.53	0	0	32.69	-17.32	-17.32
4E	-0.48	0	0	32.69	-15.69	-15.69
5E	0.61	0	0	32.69	19.94	19.94
6E	-0.43	0	0	32.69	-14.06	-14.06

* p = qh * (GCpf - GCpi)

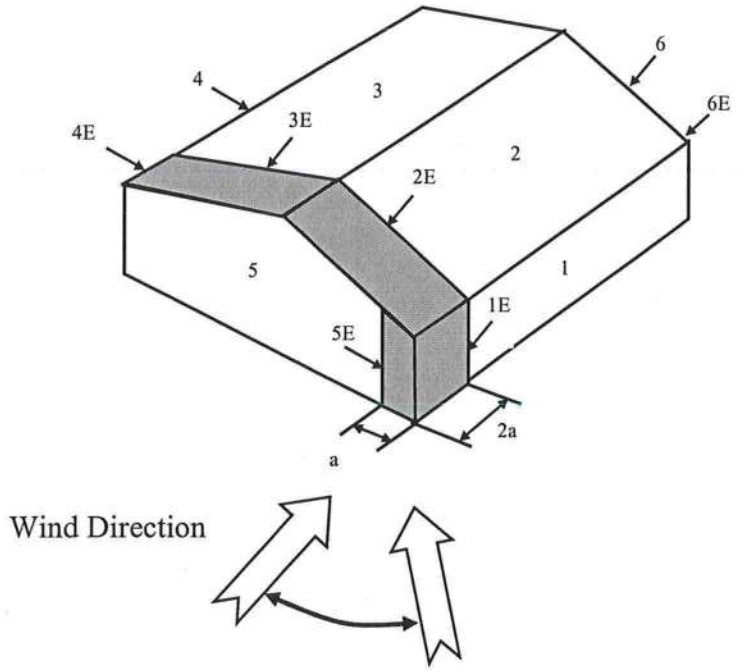
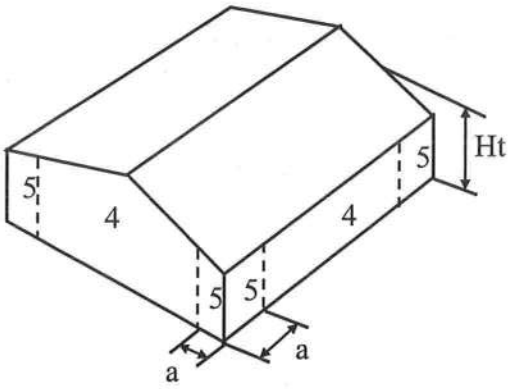
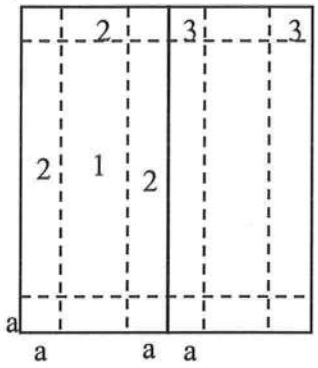


Figure 6-5 - External Pressure Coefficients, GCp

Loads on Components and Cladding for Buildings w/ Ht <= 60 ft

Wind Load Design per ASCE 7-10





Gabled Roof

10 < Theta <= 45

a = 1.2 ==> 3.00 ft

Component	Width (ft)	Length (ft)	Area (ft^2)	Zone	GCp		Wind Press (lb/ft^2)	
					Max	Min	Max	Min
	16	7	112.00	5	0.81	-1.03	21.85	-27.60
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
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	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					

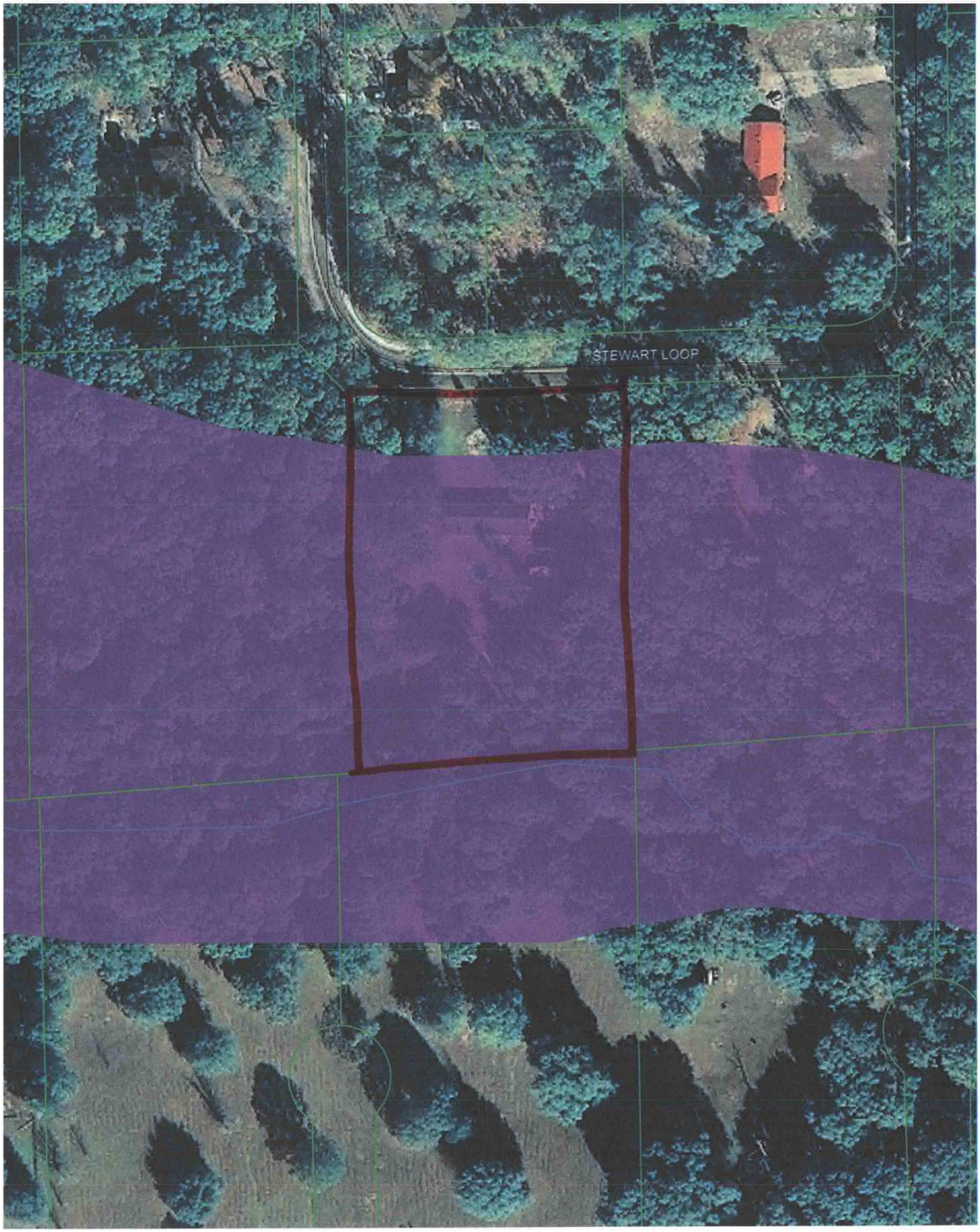
Note: * Enter Zone 1 through 5, or 1H through 3H for overhangs.

Table 6-7 Internal Pressure Coefficients for Buildings, Gcpi

Condition	Gcpi	
	Max +	Max -
Open Buildings	0.00	0.00

Wind Load Design per ASCE 7-10

Partially Enclosed Buildings	0.55	-0.55
Enclosed Buildings	0.18	-0.18
Open Buildings	**0.00**	**0.00**



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Columbia County
BUILDING DEPARTMENT

Inspection Affidavit **RE: Permit Number:** 30690

I Jason Elixsun ,licensed as a(n) Contractor* /Engineer/Architect,
(please print name and circle Lic. Type) FS 468 Building Inspector*

License #; CCC1325779

On or about 12-30-2012, I did personally inspect the
(Date & time)

☐ roof deck attachment ☒ secondary water barrier ☐ roof to wall connection

work at 359 SW Stewart Loop Lake City Fl. 32024
(Job Site Address)

Based upon that examination I have determined the installation was done according to the Hurricane Mitigation Retrofit Manual (Based on 553.844 F.S.)

Signature

STATE OF FLORIDA
COUNTY OF
Sworn to and subscribed before me this 1 day of May. 2013

By La. H. Notary Public, State of Florida

(Print, type or stamp name)

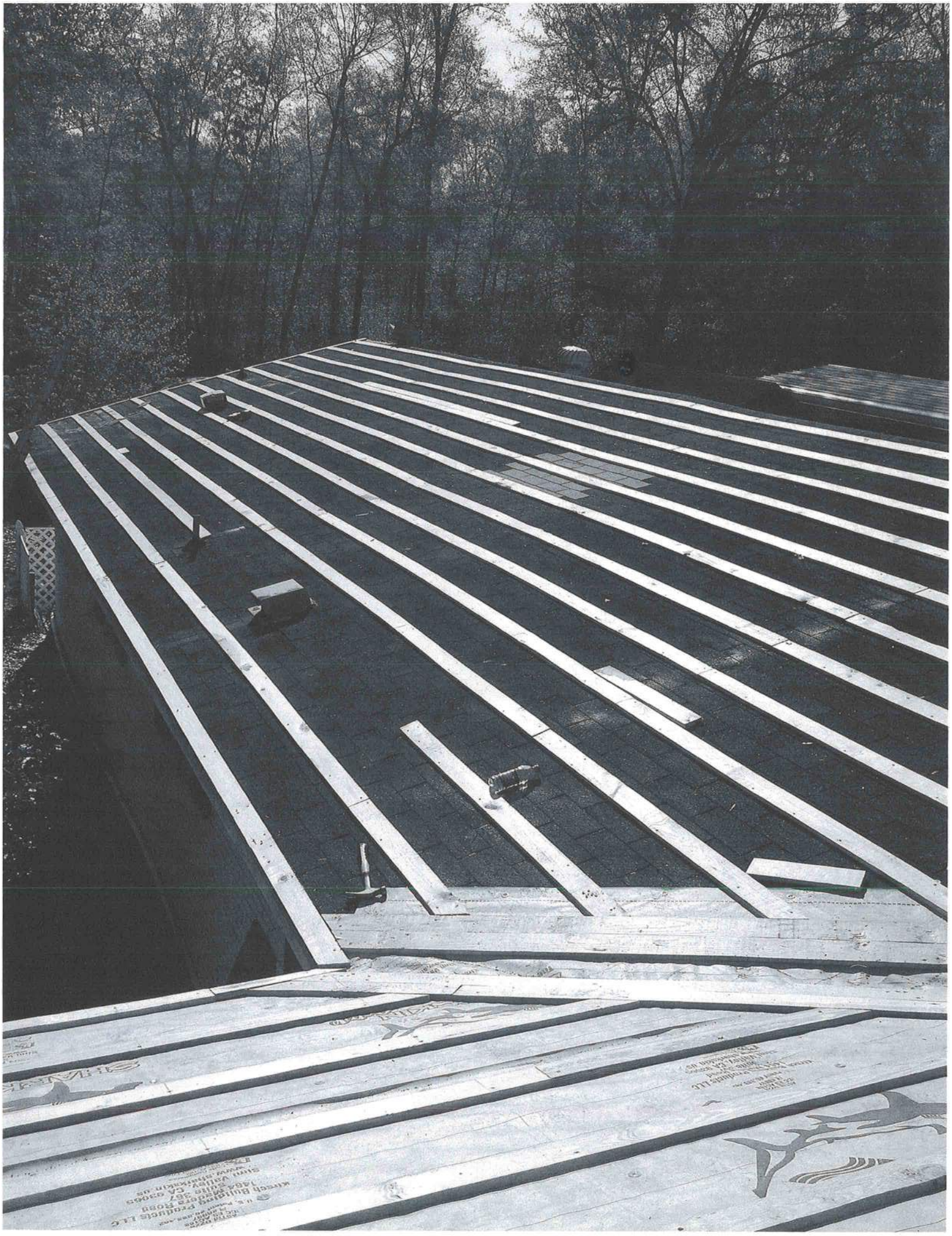
LAURIE HODSON
MY COMMISSION # EE 214728
EXPIRES: July 14, 2016
Bonded Thru Notary Public Underwriters

Personally known ✓ or
Produced Identification____ Type of identification produced.____

*** Include photographs of each plane of the roof with the permit number clearly shown marked on the deck for each inspection. Place a tape measure next to the nailing pattern to show distance between nails.**

*** Photographs must clearly show all work and have the permit number indicated on the roof.**

*** Affidavit and Photographs must be provided when final inspection is requested.**



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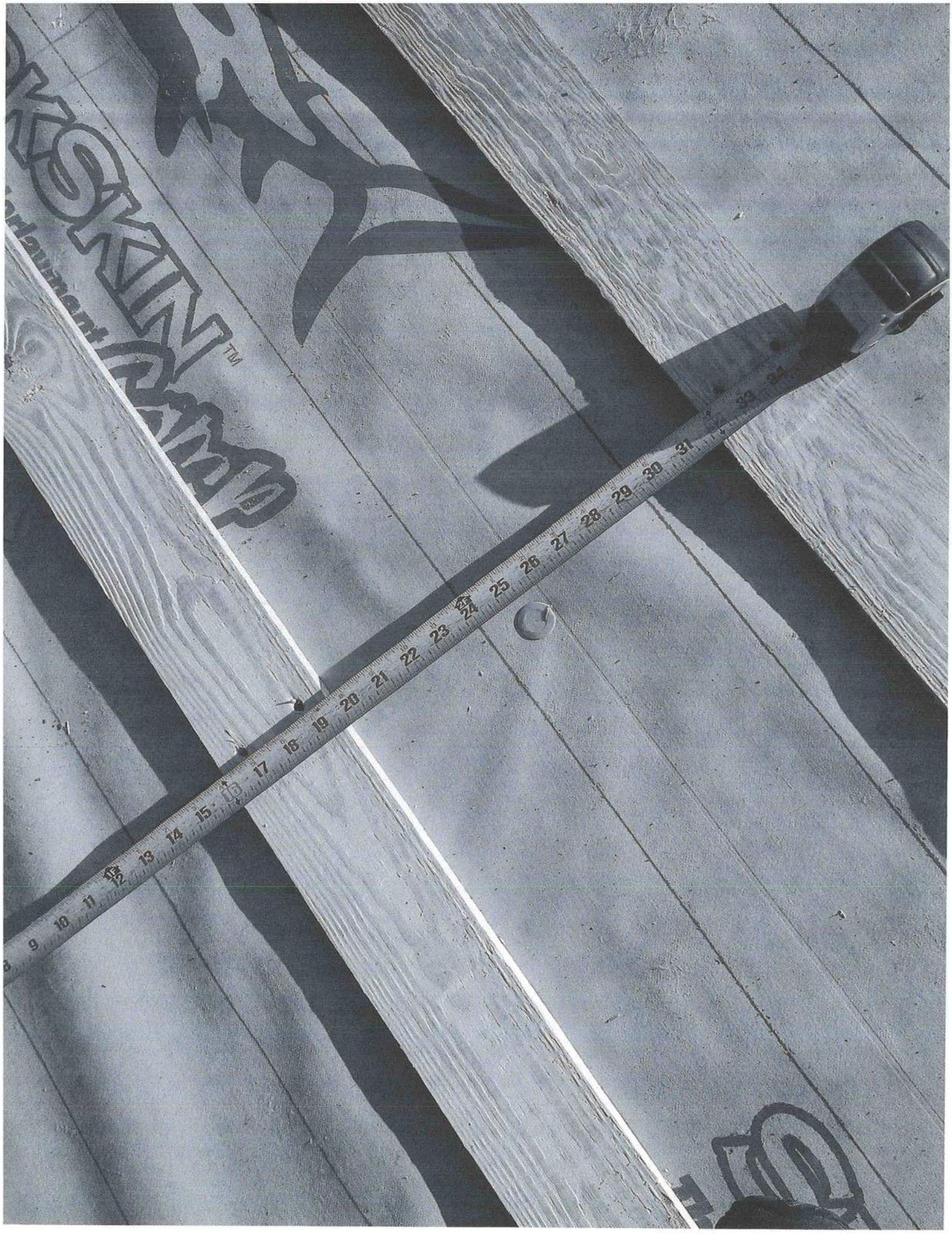
ESR Report 17
BOCA
SBCCI
Florida Building Co
ASTM D226
Texas Department of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