

DATE 01/16/2008

Columbia County Building Permit

PERMIT

This Permit Must Be Prominently Posted on Premises During Construction

000026624

APPLICANT PATRICK WILSON PHONE 352 206-5459
ADDRESS 6800 SOUTHPOINT PARKWAY JACKSONVILLE FL 32216
OWNER MARONDA HOMES PHONE 904 296-1490
ADDRESS 255 SW TIMBER RIDGE DR LAKE CITY FL 32216
CONTRACTOR THEODORE BROCK PHONE 407 227-3504
LOCATION OF PROPERTY 90W, TL ON 247S, TR ON 252B, TL ON TIMBER RIDGE, 7TH LOT
ON RIGHT
TYPE DEVELOPMENT SFD, UTILITY ESTIMATED COST OF CONSTRUCTION 186900.00
HEATED FLOOR AREA 2827.00 TOTAL AREA 3738.00 HEIGHT STORIES 2
FOUNDATION CONC WALLS FRAMED ROOF PITCH 5/12 FLOOR SLAB
LAND USE & ZONING RSF-2 MAX. HEIGHT 21
Minimum Set Back Requirments: STREET-FRONT 25.00 REAR 15.00 SIDE 10.00
NO. EX.D.U. 0 FLOOD ZONE X PP DEVELOPMENT PERMIT NO.

PARCEL ID 10-4S-16-02856-107 SUBDIVISION TIMBERLANDS

LOT 7 BLOCK PHASE UNIT TOTAL ACRES

000001526 CRC1326913
Culvert Permit No. Culvert Waiver Contractor's License Number Applicant/Owner/Contractor
CULVERT 07-1000 BK JH Y
Driveway Connection Septic Tank Number LU & Zoning checked by Approved for Issuance New Resident

COMMENTS: MFE: 100' PER PLAT, ELEVATION CONFIRMATION LETTER REQUIRED AT SLAB

Check # or Cash 861763

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power Foundation Monolithic
 date/app. by date/app. by date/app. by
Under slab rough-in plumbing Slab Sheathing/Nailing
 date/app. by date/app. by date/app. by
Framing Rough-in plumbing above slab and below wood floor
 date/app. by date/app. by
Electrical rough-in Heat & Air Duct Peri. beam (Lintel)
 date/app. by date/app. by date/app. by
Permanent power C.O. Final Culvert
 date/app. by date/app. by date/app. by
M/H tie downs, blocking, electricity and plumbing Pool
 date/app. by date/app. by
Reconnection Pump pole Utility Pole
 date/app. by date/app. by date/app. by
M/H Pole Travel Trailer Re-roof
 date/app. by date/app. by date/app. by

BUILDING PERMIT FEE \$ 935.00 CERTIFICATION FEE \$ 18.69 SURCHARGE FEE \$ 18.69
MISC. FEES \$ 0.00 ZONING CERT. FEE \$ 50.00 FIRE FEE \$ 0.00 WASTE FEE \$
FLOOD DEVELOPMENT FEE \$ FLOOD ZONE FEE \$ 25.00 CULVERT FEE \$ 25.00 TOTAL FEE 1072.38
INSPECTORS OFFICE CLERKS OFFICE

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED TO BE IN ACTIVE PROGRESS WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

For Office Use Only Application # 0801-06 Date Received 1/3 By JW Permit # 1526/26624
Zoning Official BLK Date 08.01.08 Flood Zone X per plot FEMA Map # N/A Zoning RSF-2
Land Use RES for Dev Elevation N/A MFE 100' per plot River N/A Plans Examiner OK JTH Date 1-7-08
Comments Elevation Confirmation Letter Required
☒ NOC ☐ EH ☐ Deed or PA ☐ Site Plan ☐ State Road Info ☐ Parent Parcel #
☐ Dev Permit # ☐ In Floodway ☐ Letter of Authorization from Contractor
☐ Unincorporated area ☐ Incorporated area ☐ Town of Fort White ☐ Town of Fort White Compliance letter

Septic Permit No. 07-1000 Fax (904)-332-6367

Name Authorized Person Signing Permit Theodore C. Brock/Melissa McKague Phone (904)-296-1490

Address 6800 Southpoint Pkwy. #300 Jacksonville, FL 32216

Owners Name Maronda Homes Inc. of Florida Phone (904)-296-1490

911 Address 255 SW Timber Ridge Dr. L.C 7C 32024

Contractors Name Theodore C. Brock Phone (407)-227-3504

Address 6800 Southpoint Pkwy. #300 Jacksonville, FL 32216

Fee Simple Owner Name & Address N/A

Bonding Co. Name & Address N/A

Architect/Engineer Name & Address Tomas Ponce 4005 Maronda Way Sanford, FL 32771

Mortgage Lenders Name & Address Bank of America 250 Park Ave. S. #400 Winter Park, FL 32789

Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progress Energy

Property ID Number 10-45-10-02850-101 Estimated Cost of Construction 96,927.00

Subdivision Name Timberlands Lot 7 Block Unit Phase 1

Driving Directions Hwy 90, Left on 247 South; Right on 252B; Left on Timber Ridge 7th lot
On Right

Number of Existing Dwellings on Property 1

Construction of Residential Single Family Dwelling Total Acreage 50 Lot Size

Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive Total Building Height 21.8

Actual Distance of Structure from Property Lines - Front 25 Side 10 Side 10 Rear 15

Number of Stories 2 Heated Floor Area 2827 Total Floor Area 3738 Roof Pitch 5/12

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment

According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE:

YOU ARE HEREBY NOTIFIED as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

OWNERS CERTIFICATION: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning. I further understand the above written responsibilities in Columbia County for obtaining this Building Permit.



Owners Signature Theodore C. Brock

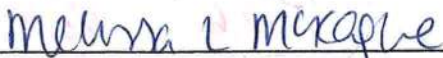
CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit.



Contractor's Signature (Permitee)
Theodore C. Brock

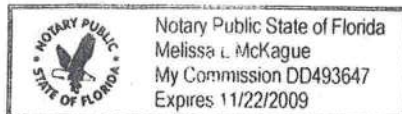
Contractor's License Number CRC 1326913
Columbia County
Competency Card Number _____

Affirmed under penalty of perjury to by the Contractor and subscribed before me this 10 day of December 2007.
Personally known XXX or Produced Identification _____



State of Florida Notary Signature (For the Contractor)
Melissa L. McKague

SEAL:





STATE OF FLORIDA
DEPARTMENT OF HEALTH

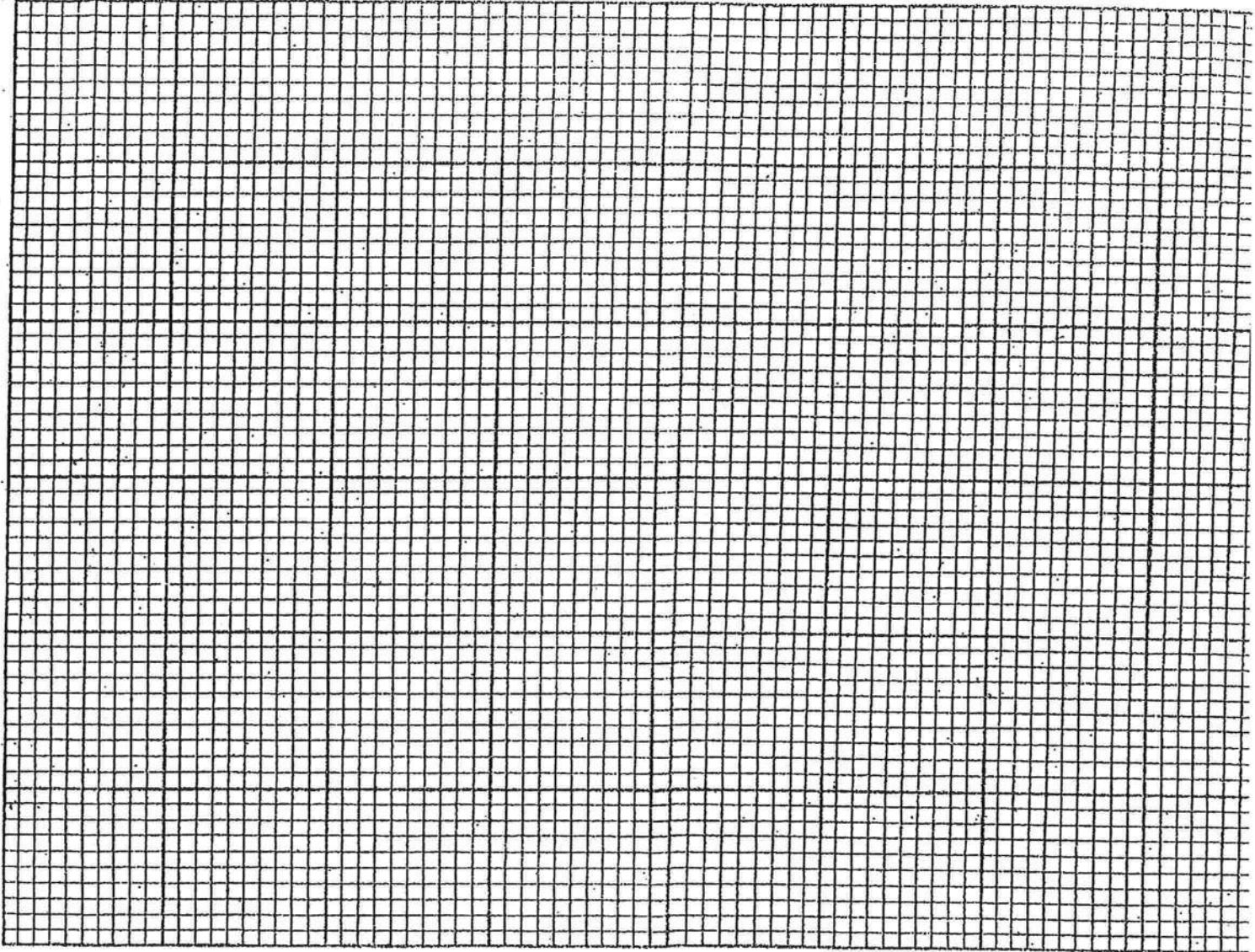
See attached.

APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number 07-01000

PART II - SITE PLAN

Scale: Each block represents 5 feet and 1 inch = 50 feet.



Notes:

Site Plan submitted by:

Jessie

Signature

Abner

Agent

Title

Plan Approved ☒

Not Approved ☐

12-31-07

Date 2007

By Mr. J. L. Linder Columbia County Health Department

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

PROPOSED BUILDING LAYOUT

IN SECTION 10, TOWNSHIP 4 SOUTH, RANGE
16 EAST, COLUMBIA COUNTY, FLORIDA

30 0
1" = 30'

LEGEND:

- = FOUND 1/2" REBAR NO IDENTIFICATION
- ⊙ = FOUND 1/2" REBAR & C. L.B. 6894
- = SET 1/2" REBAR & CAP L.B. 6894
- ⊙ = FOUND 3/4" IRON PIPE
- = FOUND 4" X 4" CONC. M NO IDENTIFICATION
- = SET 4" X 4" CONC. MON P.S.M. 5582
- ✕ = SET NAIL & DISK P.S.M.
- ✕ = FOUND NAIL & DISK
- ⊠ = FOUND 6" X 6" S.R.D. R/W MON.
- ⊠ = CATV RISER
- ⊠ = TELEPHONE PEDESTAL
- ⊠ = WOOD POWER POLE

THIS IS
CERTIFIED
NOT VALID WITH
A FLORIDA LIC
TO THIS MAP

I HEREBY CERTIFY
TRUE AND CORRECT
MY SUPERVISOR
THE MINIMUM
BOARD OF L.A.I.
STATUTES, AND

BY: JAMES E. BRIN

DATE: 10/15/07



BRIN

4607 1

PHONE:

SCALE: 1" = 30'

DATE: 10/15/07

THIS IS NOT A BOUNDARY

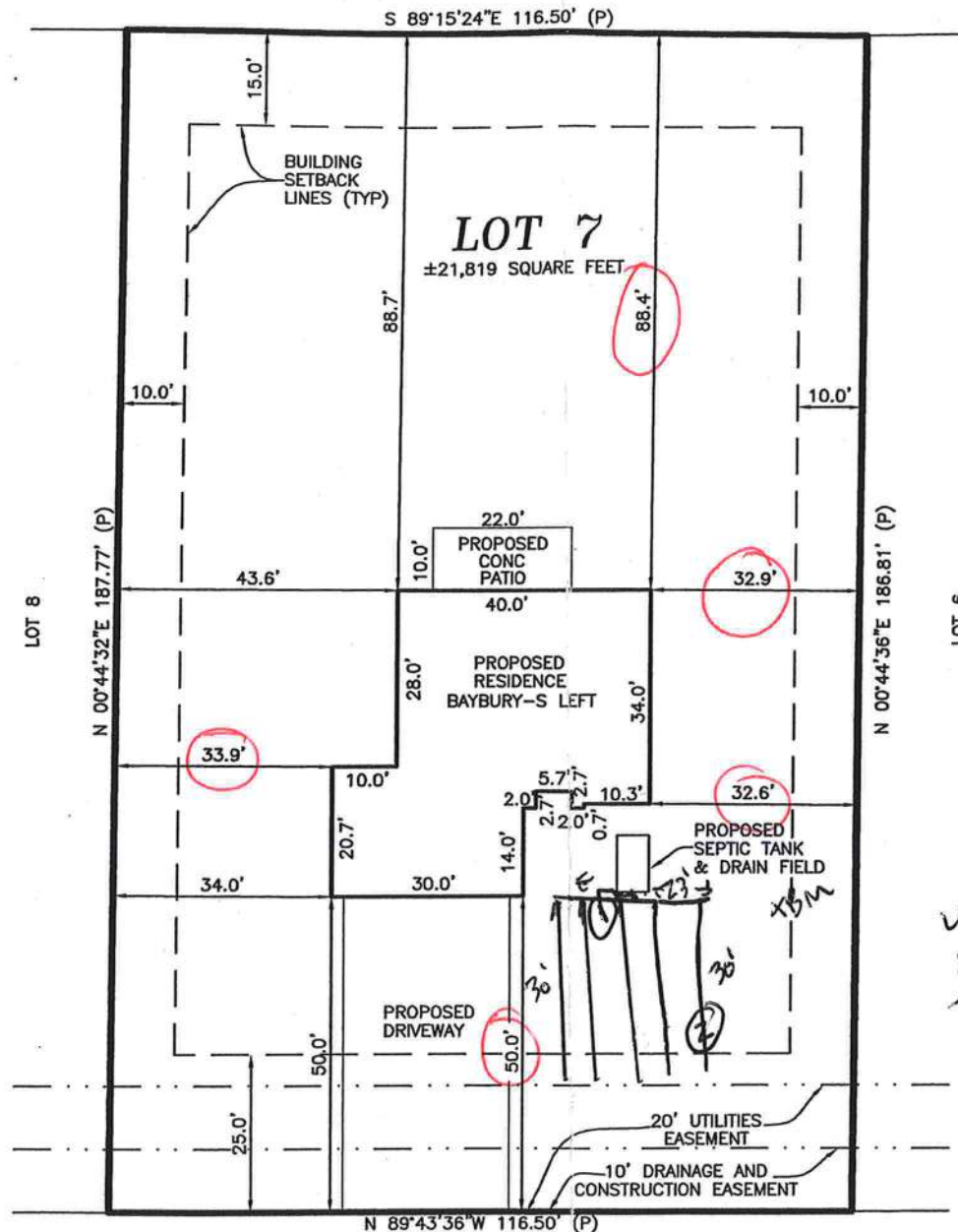
PREPARED FOR: MARI

NOTE:

INION OF THIS SURVEYOR, ACCORDING TO THE
FLOOD INSURANCE PROGRAM, FLOOD INSURANCE RATE
UNITY PANEL NO. 120070-0175-B, DATED 1-6-88,
PROPERTY IS IN FLOOD ZONE "X" WHICH IS AN AREA
D TO BE OUTSIDE 500-YEAR FLOOD PLAIN, AS
FROM SAID MAP. INFORMATION FROM THE FEDERAL
Y MANAGEMENT AGENCY (FEMA) FLOOD INSURANCE
S, SHOWN ON THIS MAP, WAS CURRENT AS OF THE
ED DATE. MAP REVISIONS AND AMENDMENTS ARE
LY MADE BY LETTER AND MAY NOT BE REFLECTED
OST CURRENT MAP.

TITLE NOTE:

THIS SURVEY IS SUBJECT TO ANY FACTS THAT MAY BE
DISCLOSED BY A FULL AND ACCURATE TITLE SEARCH. THIS
SURVEYOR HAS NOT PERFORMED A SEARCH OF THE
PUBLIC RECORDS ON THIS PARCEL FOR ANY CLAIMS OF
TITLE, EASEMENTS, OR RESTRICTIONS THAT MAY EFFECT
THIS PARCEL. THE PRESENCE OR ABSENCE OF ANY SUCH
CLAIMS ARE NOT CERTIFIED HEREON.



SW TIMBER RIDGE DRIVE

60' RIGHT-OF-WAY

This instrument prepared by:
William J. Haley, Esquire
Brannon, Brown,
Haley & Bullock, P. A.
P. O. Box 1029
Lake City, FL 32056-1029

Inst:200712024085 Date:10/26/2007 Time:4:45 PM
Doc Stamp-Deed:3276.00
28 DC,P.DeWitt Cason,Columbia County Page 1 of 2

SPECIAL WARRANTY DEED

THIS INDENTURE, made this 10th day of October, 2007, between **RML HOLDINGS, INC.**, a Florida corporation, having a mailing address of 703 NW Blackberry Circle, Lake City, Florida 32055, hereinafter referred to as Grantor, and **MARONDA HOMES, INC. OF FLORIDA**, a Florida corporation, having a mailing address of 3993 West First Street, Sanford, Florida 32771, hereinafter referred to as Grantee.

WITNESSETH: That said Grantor, for and in consideration of the sum of \$10.00 and other good and valuable considerations to said Grantor in hand paid by said Grantee, the receipt and sufficiency of which are hereby acknowledged, have granted, bargained and sold to the said Grantee, and Grantee's successors and assigns forever, the following described land, situate, lying and being in **Columbia** County, Florida, to-wit:

Lot(s) 2, 3, 4, 5, 6, 7, 8, 21, 22, 23, 25, and 26, **TIMBERLANDS, PHASE 1**, according to the plat thereof, as recorded in Plat Book 9, pages 26-27, public records of Columbia County, Florida.

PARCEL NO. Part of 10-4S-16-02856-000 and 15-4S-16-02990-000

SUBJECT TO: Taxes and special assessments for the year 2007 and subsequent years; restrictions, reservations, rights of way for public roads, easements of record, if any; and zoning and any other governmental restrictions regulating the use of the lands.

and said Grantor does hereby fully warrant the title to said land, and will defend the same against the lawful claims of all persons claiming by, through or under said Grantor.

IN WITNESS WHEREOF, Grantor has hereunto set its hand and seal the day and year first above written.

Signed, sealed and delivered
in the presence of:

RML HOLDINGS, INC., a Florida
corporation

Linda Brown
Print Name: Linda Brown

By: Margaret Lardizabal
Margaret Lardizabal
Vice President

Debbie G. Moore
Print Name: Debbie G. Moore

STATE OF FLORIDA
COUNTY OF COLUMBIA

The foregoing instrument was acknowledged before me this 10th day of October, 2007, by Margaret Lardizabal, as Vice President of RML Holdings, Inc., a Florida corporation, on behalf of said corporation, who is personally known to me.

Debbie G. Moore
Notary Public, State of Florida



Maronda Homes
BAYBURY

AMERICANA SERIES

(4 Bedroom)



Elevation R



Elevation S



Elevation T

Prices, plans, dimensions, features, specifications, materials and availability of Maronda Homes or communities are subject to change without prior notice or obligation. Maronda Homes illustrations are artist's depictions only and may differ from completed improvements. Copyright 2002 © Maronda Homes Inc. of Florida. All Rights Reserved.

www.maronda.com



Providing More Home for Less Money . . . It's the Way We Do Business!

8/04



Greater Lake City Regional Utility Authority

205 North Marion Avenue
LAKE CITY, FLORIDA 32055-3918
TELEPHONE: 386.719.5778 FAX: 386.719.5837
E-mail: customer.service@ci.lake-city.fl.us

APPLICATION FOR WATER /SEWER/GAS TAP-CAPACITY COMMITMENT

Project Name: Timberlands Date/Time: 12/10/07

Service Address: 255 SW Timber Ridge Dr.

Applicant Name: Maronda Homes Telephone Number: (904)-296-1490

Applicant's Agent: Melissa McKague Telephone Number: (904)-296-1490 ext. 117

Business Name: Maronda Homes Inc. of Telephone Number: (904)-296-1490

Parcel ID #'s: _____

Mailing Address: 6800 Southpoint Pkwy #300 Jacksonville, FL 32216

Requested Water Capacity: ☐ No

☒ Yes, in the amount of _____ gpd/gph

Requested Sewer Capacity: ☐ No

☐ Yes, in the amount of _____ gpd/gph

Requested Gas Capacity: ☐ No

☐ Yes, in the amount of _____ btu/unit

Application For:

☒ Water/Tap Size 3/4 ☐ Sewer/Tap Size _____
☐ Irrigation/Tap Size _____ ☐ Gas/Tap Size _____

Meter Size /Quantity: ☒ 3/4" ☐ 1" ☐ 1 1/2" ☐ 2" ☐ 6" ☐ Other Specify _____

Within City limits: ☐ Yes ☒ No

Fire Protection: ☒ No

☐ Yes, Diameter of new mainline _____
Quantity of new Hydrants: _____

Growth Management Zoned: _____ ☒ Residential ☐ Commercial ☐ Industrial

The above named applicant request that an inspection be made by the Greater Lake City Regional Utility Authority for verification of available services based on address, parcel numbers, lot numbers, etc. Upon confirmation of both capacity and or availability, the applicant will be notified and provided a "Cost Estimate/Tap & Impact Fees" summary.

NOTE: This is only an estimate, fees are subject to change.

Applicant: Melissa McKague

Date: 12/10/07

Service Available Date Requested

Date: ASAP

Billing will begin upon completion of tap.

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs
Residential Whole Building Performance Method A

7/1 TM

Project Name: BAYBURY GAINESVILLE	Builder: MARONDA HOMES
Address: 255 SW Timber Ridge Dr	Permitting Office: Columbia
City, State: Lake City, FL 32055	Permit Number: 26624
Owner: ELECTRIC	Jurisdiction Number: 221000
Climate Zone: North	

1. New construction or existing New ☐	12. Cooling systems
2. Single family or multi-family Single family ☐	a. Central Unit Cap: 30.6 kBtu/hr ☐
3. Number of units, if multi-family 2 ☐	SEER: 14.00 ☐
4. Number of Bedrooms 4 ☐	b. Central Unit Cap: 34.6 kBtu/hr ☐
5. Is this a worst case? Yes ☐	SEER: 14.00 ☐
6. Conditioned floor area (ft ²) 2827 ft² ☐	c. N/A ☐
7. Glass type ¹ and area: (Label reqd. by 13-104.4.5 if not default)	13. Heating systems
a. U-factor: Description Area	a. Electric Heat Pump Cap: 30.6 kBtu/hr ☐
(or Single or Double DEFAULT) 7a(Sngle Default) 266.0 ft ² ☐	HSPF: 8.70 ☐
b. SHGC:	b. Electric Heat Pump Cap: 34.6 kBtu/hr ☐
(or Clear or Tint DEFAULT) 7b. (Clear) 266.0 ft ² ☐	HSPF: 8.50 ☐
8. Floor types	c. N/A ☐
a. Slab-On-Grade Edge Insulation R=0.0, 144.0(p) ft ☐	14. Hot water systems
b. Raised Wood, Adjacent R=11.0, 480.0ft² ☐	a. Electric Resistance Cap: 50.0 gallons ☐
c. N/A ☐	EF: 0.90 ☐
9. Wall types	b. N/A ☐
a. Concrete, Int Insul, Exterior R=4.1, 802.0 ft² ☐	c. Conservation credits
b. Frame, Wood, Exterior R=11.0, 1340.0 ft² ☐	(HR-Heat recovery, Solar
c. Frame, Steel, Adjacent R=11.0, 180.0 ft² ☐	DHP-Dedicated heat pump)
d. N/A ☐	15. HVAC credits PT, ☐
e. N/A ☐	(CF-Ceiling fan, CV-Cross ventilation,
10. Ceiling types	HF-Whole house fan,
a. Under Attic R=19.0, 1620.0 ft² ☐	PT-Programmable Thermostat,
b. N/A ☐	MZ-C-Multizone cooling,
c. N/A ☐	MZ-H-Multizone heating)
11. Ducts	
a. Sup: Unc. Ret: Con. AH(Sealed):Interior Sup. R=6.0, 175.0 ft ☐	
b. Sup: Unc. Ret: Con. AH(Sealed):Interior Sup. R=6.0, 150.0 ft ☐	

Glass/Floor Area: 0.09

Total as-built points: 31934

Total base points: 35356

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: Gary CarmackDATE: 12/13/07

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: Melissa McRaeDATE: 12/13/07

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL: _____

DATE: _____



¹ Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

SUMMER CALCULATIONS**Residential Whole Building Performance Method A - Details**

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT							
GLASS TYPES											
.18 X Conditioned X BSPM = Points Floor Area				Type/SC	Overhang Ornt Len Hgt		Area X SPM X SOF = Points				
.18	2827.0	18.59	9460.0	1.Single, Clear	W	1.0	13.0	10.0	43.84	1.00	436.0
				2.Single, Clear	W	1.0	17.0	40.0	43.84	1.00	1748.0
				3.Single, Clear	W	1.0	15.0	24.0	43.84	1.00	1048.0
				4.Single, Clear	E	1.0	15.0	26.0	47.92	1.00	1241.0
				5.Single, Clear	N	1.0	14.0	30.0	21.73	1.00	649.0
				6.Single, Clear	W	1.0	6.0	16.0	43.84	0.97	679.0
				7.Single, Clear	W	1.0	6.0	16.0	43.84	0.97	679.0
				8.Single, Clear	W	1.0	6.0	30.0	43.84	0.97	1273.0
				9.Single, Clear	E	1.0	6.0	42.0	47.92	0.97	1948.0
				10.Single, Clear	E	1.0	6.0	32.0	47.92	0.97	1487.0
				As-Built Total:		266.0			11188.0		
WALL TYPES Area X BSPM = Points				Type	R-Value		Area X SPM = Points				
Adjacent	180.0	0.70	126.0	1. Concrete, Int Insul, Exterior	4.1		802.0	1.13	910.3		
Exterior	2142.0	1.70	3641.4	2. Frame, Wood, Exterior	11.0		1340.0	1.70	2278.0		
				3. Frame, Steel, Adjacent	11.0		180.0	1.00	180.0		
Base Total:				As-Built Total:		2322.0			3368.3		
DOOR TYPES Area X BSPM = Points				Type	R-Value		Area X SPM = Points				
Adjacent	18.0	2.40	43.2	1.Exterior Insulated			20.0	4.10	82.0		
Exterior	20.0	6.10	122.0	2.Adjacent Wood			18.0	2.40	43.2		
Base Total:				As-Built Total:		38.0			125.2		
CEILING TYPES Area X BSPM = Points				Type	R-Value		Area X SPM X SCM = Points				
Under Attic	1595.0	1.73	2759.4	1. Under Attic	19.0		1620.0	2.34 X 1.00	3790.8		
Base Total:				As-Built Total:		1620.0			3790.8		
FLOOR TYPES Area X BSPM = Points				Type	R-Value		Area X SPM = Points				
Slab	144.0(p)	-37.0	-5328.0	1. Slab-On-Grade Edge Insulation	0.0		144.0(p)	-41.20	-5932.8		
Raised	480.0	-3.99	-1915.2	2. Raised Wood, Adjacent	11.0		480.0	0.70	336.0		
Base Total:				As-Built Total:		624.0			-5596.8		
INFILTRATION Area X BSPM = Points				Area X SPM = Points							
2827.0 10.21 28863.7				2827.0 10.21 28863.7							

SUMMER CALCULATIONS**Residential Whole Building Performance Method A - Details**

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT						
Summer Base Points: 37772.4				Summer As-Built Points: 41739.1						
Total Summer Points	X System Multiplier	=	Cooling Points	Total Component (System - Points)	X Cap Ratio (DM x DSM x AHU)	X Duct Multiplier	X System Multiplier	X Credit Multiplier	=	Cooling Points
				(sys 1: Central Unit 30600btuh ,SEER/EFF(14.0) Ducts:Unc(S),Con(R),Int(AH),R6.0(INS) 41739 0.47 (1.08 x 1.147 x 0.86) 0.244 0.950 4863.0 (sys 2: Central Unit 34600btuh ,SEER/EFF(14.0) Ducts:Unc(S),Con(R),Int(AH),R6.0(INS) 41739 0.53 (1.08 x 1.147 x 0.86) 0.244 0.950 5498.7						
37772.4	0.3250		12276.0	41739.1	1.00	1.072	0.244	0.950		10361.7

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT							
GLASS TYPES											
.18 X Conditioned X BWPM = Points Floor Area				Type/SC	Overhang Ornt Len Hgt		Area X WPM X WOF = Points				
.18	2827.0	20.17	10264.0	1.Single, Clear	W	1.0	13.0	10.0	28.84	1.00	288.0
				2.Single, Clear	W	1.0	17.0	40.0	28.84	1.00	1154.0
				3.Single, Clear	W	1.0	15.0	24.0	28.84	1.00	692.0
				4.Single, Clear	E	1.0	15.0	26.0	26.41	1.00	689.0
				5.Single, Clear	N	1.0	14.0	30.0	33.22	1.00	996.0
				6.Single, Clear	W	1.0	6.0	16.0	28.84	1.01	465.0
				7.Single, Clear	W	1.0	6.0	16.0	28.84	1.01	465.0
				8.Single, Clear	W	1.0	6.0	30.0	28.84	1.01	872.0
				9.Single, Clear	E	1.0	6.0	42.0	26.41	1.02	1127.0
				10.Single, Clear	E	1.0	6.0	32.0	26.41	1.02	858.0
				As-Built Total:				266.0	7606.0		
WALL TYPES Area X BWPM = Points				Type	R-Value		Area X WPM = Points				
Adjacent	180.0	3.60	648.0	1. Concrete, Int Insul, Exterior	4.1		802.0	6.42		5148.8	
Exterior	2142.0	3.70	7925.4	2. Frame, Wood, Exterior	11.0		1340.0	3.70		4958.0	
				3. Frame, Steel, Adjacent	11.0		180.0	5.20		936.0	
Base Total:				As-Built Total:		2322.0		11042.8			
DOOR TYPES Area X BWPM = Points				Type	R-Value		Area X WPM = Points				
Adjacent	18.0	11.50	207.0	1.Exterior Insulated			20.0	8.40		168.0	
Exterior	20.0	12.30	246.0	2.Adjacent Wood			18.0	11.50		207.0	
Base Total:				As-Built Total:		38.0		375.0			
CEILING TYPES Area X BWPM = Points				Type	R-Value		Area X WPM X WCM = Points				
Under Attic	1595.0	2.05	3269.8	1. Under Attic	19.0		1620.0	2.70 X 1.00		4374.0	
Base Total:				As-Built Total:		1620.0		4374.0			
FLOOR TYPES Area X BWPM = Points				Type	R-Value		Area X WPM = Points				
Slab	144.0(p)	8.9	1281.6	1. Slab-On-Grade Edge Insulation	0.0		144.0(p)	18.80		2707.2	
Raised	480.0	0.96	460.8	2. Raised Wood, Adjacent	11.0		480.0	3.60		1728.0	
Base Total:				As-Built Total:		624.0		4435.2			
INFILTRATION Area X BWPM = Points								Area X WPM = Points			
2827.0 -0.59 -1667.9								2827.0 -0.59 -1667.9			

WINTER CALCULATIONS**Residential Whole Building Performance Method A - Details**

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT						
Winter Base Points:		22634.6		Winter As-Built Points:					26165.1	
Total Winter Points	X System Multiplier	=	Heating Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	=	Heating Points
				(sys 1: Electric Heat Pump 30600 btuh ,EFF(8.7) Ducts:Unc(S),Con(R),Int(AH),R6.0 26165.1 0.469 (1.060 x 1.169 x 0.88) 0.392 0.950 5005.9 (sys 2: Electric Heat Pump 34600 btuh ,EFF(8.5) Ducts:Unc(S),Con(R),Int(AH),R6.0 26165.1 0.531 (1.060 x 1.169 x 0.88) 0.401 0.950 5793.4						
22634.6	0.5540		12539.6	26165.1	1.00	1.095	0.397	0.950		10797.9

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT					
WATER HEATING				Tank	EF	Number of	X	Tank X	Multiplier X Credit = Total
Number of	X	Multiplier	= Total	Volume		Bedrooms		Ratio	Multiplier
Bedrooms									
4		2635.00	10540.0	50.0	0.90	4		1.00	2693.56 1.00 10774.2
				As-Built Total:					10774.2

CODE COMPLIANCE STATUS							
BASE				AS-BUILT			
Cooling	+	Heating	+	Cooling	+	Heating	+
Points		Points		Points		Points	
12276		12540		10362		10798	
		10540				10774	
		35356					
							31934

PASS

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum: .3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	✓
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	✓
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	✓
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	✓
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	✓
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	✓
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	✓

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	✓
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	✓
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	✓
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	✓
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	✓
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	✓

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 86.6

The higher the score, the more efficient the home.

ELECTRIC, , , ,

1. New construction or existing	New	___	12. Cooling systems	
2. Single family or multi-family	Single family	___	a. Central Unit	Cap: 30.6 kBtu/hr ___ SEER: 14.00 ___
3. Number of units, if multi-family	2	___	b. Central Unit	Cap: 34.6 kBtu/hr ___ SEER: 14.00 ___
4. Number of Bedrooms	4	___	c. N/A	___
5. Is this a worst case?	Yes	___	13. Heating systems	
6. Conditioned floor area (ft ²)	2827 ft ²	___	a. Electric Heat Pump	Cap: 30.6 kBtu/hr ___ HSPF: 8.70 ___
7. Glass type ¹ and area: (Label reqd. by 13-104.4.5 if not default)		___	b. Electric Heat Pump	Cap: 34.6 kBtu/hr ___ HSPF: 8.50 ___
a. U-factor:	Description Area	___	c. N/A	___
(or Single or Double DEFAULT)	7a(Sngle Default) 266.0 ft ²	___	14. Hot water systems	
b. SHGC:		___	a. Electric Resistance	Cap: 50.0 gallons ___ EF: 0.90 ___
(or Clear or Tint DEFAULT)	7b. (Clear) 266.0 ft ²	___	b. N/A	___
8. Floor types		___	c. Conservation credits	___
a. Slab-On-Grade Edge Insulation	R=0.0, 144.0(p) ft	___	(HR-Heat recovery, Solar	___
b. Raised Wood, Adjacent	R=11.0, 480.0ft ²	___	DHP-Dedicated heat pump)	___
c. N/A	___	___	15. HVAC credits	PT, ___
9. Wall types		___	(CF-Ceiling fan, CV-Cross ventilation,	___
a. Concrete, Int Insul, Exterior	R=4.1, 802.0 ft ²	___	HF-Whole house fan,	___
b. Frame, Wood, Exterior	R=11.0, 1340.0 ft ²	___	PT-Programmable Thermostat,	___
c. Frame, Steel, Adjacent	R=11.0, 180.0 ft ²	___	MZ-C-Multizone cooling,	___
d. N/A	___	___	MZ-H-Multizone heating)	___
e. N/A	___	___		___
10. Ceiling types		___		___
a. Under Attic	R=19.0, 1620.0 ft ²	___		___
b. N/A	___	___		___
c. N/A	___	___		___
11. Ducts		___		___
a. Sup: Unc. Ret: Con. AH(Sealed):Interior Sup. R=6.0, 175.0 ft	___	___		___
b. Sup: Unc. Ret: Con. AH(Sealed):Interior Sup. R=6.0, 150.0 ft	___	___		___

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: Melissa Meloyne

Date: 12/13/10

Address of New Home: 255 SW Timber Ridge Dr City/FL Zip: Lake City, FL 32055



*NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStarTM designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.

¹ Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.
EnergyGauge® (Version: FLRCSB v4.5)



Project Summary Entire House MARONDA HOMES

Job: BAYBURY 1ST FLOOR
Date:
By: G. CARMACK

4005 MARONDA WAY, SANFORD, FL 32771 Phone: (407) 321-0064

Project Information

For: BAYBURY 1ST FLOOR

Notes:

Design Information

Weather: Gainesville, FL, US

Winter Design Conditions

Outside db	33 °F
Inside db	70 °F
Design TD	37 °F

Summer Design Conditions

Outside db	92 °F
Inside db	75 °F
Design TD	17 °F
Daily range	M
Relative humidity	50 %
Moisture difference	52 gr/lb

Heating Summary

Structure	21333 Btuh
Ducts	2428 Btuh
Central vent (50 cfm)	2024 Btuh
Humidification	0 Btuh
Piping	0 Btuh
Equipment load	25785 Btuh

Sensible Cooling Equipment Load Sizing

Structure	15628 Btuh
Ducts	5274 Btuh
Central vent (50 cfm)	930 Btuh
Blower	0 Btuh

Infiltration

Method	Simplified
Construction quality	Average
Fireplaces	0

	Heating	Cooling
Area (ft²)	1232	1232
Volume (ft³)	9856	9856
Air changes/hour	0.45	0.23
Equiv. AVF (cfm)	74	38

Latent Cooling Equipment Load Sizing

Structure	3328 Btuh
Ducts	1181 Btuh
Central vent (50 cfm)	1758 Btuh
Equipment latent load	6267 Btuh

Equipment total load	27444 Btuh
Req. total capacity at 0.74 SHR	2.4 ton

Heating Equipment Summary

Make TEMPSTAR
Trade HEATPUMP
Model N4H330AKA

Efficiency	8.7 HSPF
Heating input	
Heating output	0 Btuh @ 47°F
Temperature rise	0 °F
Actual air flow	0 cfm
Air flow factor	0.000 cfm/Btuh
Static pressure	0.60 in H2O
Space thermostat	

Cooling Equipment Summary

Make TEMPSTAR
Trade HEATPUMP
Cond N4H330AKA
Coil FEM4X3600

Efficiency	14 SEER
Sensible cooling	22644 Btuh
Latent cooling	7956 Btuh
Total cooling	30600 Btuh
Actual air flow	935 cfm
Air flow factor	0.045 cfm/Btuh
Static pressure	0.60 in H2O
Load sensible heat ratio	0.78

Bold/italic values have been manually overridden

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Page 1

Project Information

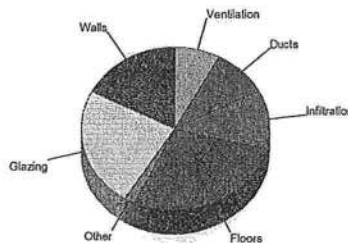
For: BAYBURY 1ST FLOOR

Design Conditions

Location:		Indoor:		Heating	Cooling
Gainesville, FL, US		Indoor temperature (°F)		70	75
Elevation: 151 ft		Design TD (°F)		37	17
Latitude: 30°N		Relative humidity (%)		30	50
		Moisture difference (gr/lb)		10.6	52.0
Outdoor:		Heating	Cooling		
Dry bulb (°F)	33	92			
Daily range (°F)	-	19 (M)			
Wet bulb (°F)	-	77			
Wind speed (mph)	15.0	7.5			
		Infiltration:			
		Method	Simplified		
		Construction quality	Average		
		Fireplaces	0		

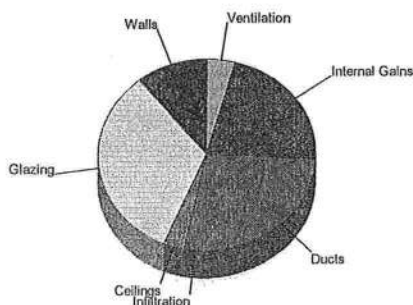
Heating

Component	Btuh/ft²	Btuh	% of load
Walls	4.4	4520	17.5
Glazing	47.0	6109	23.7
Doors	0.0	0	0.0
Ceilings	1.8	477	1.8
Floors	14.7	7235	28.1
Infiltration	2.6	2992	11.6
Ducts		2428	9.4
Piping		0	0.0
Humidification		0	0.0
Ventilation		2024	7.8
Adjustments		0	
Total		25785	100.0



Cooling

Component	Btuh/ft²	Btuh	% of load
Walls	2.3	2363	10.8
Glazing	55.3	7184	32.9
Doors	0.0	0	0.0
Ceilings	2.6	679	3.1
Floors	0.0	0	0.0
Infiltration	0.6	703	3.2
Ducts		5274	24.2
Ventilation		930	4.3
Internal gains		4700	21.5
Blower		0	0.0
Adjustments		0	
Total		21832	100.0



Overall U-value = 0.260 Btuh/ft²-°F

Data entries checked.



Project Summary Entire House MARONDA HOMES

Job: BAYBURY 2ND FLOOR
Date:
By: G. CARMACK

4005 MARONDA WAY, SANFORD, FL 32771 Phone: (407) 321-0064

Project Information

For: BAYBURY 2ND FLOOR

Notes:

Design Information

Weather: Gainesville, FL, US

Winter Design Conditions

Outside db	33 °F
Inside db	70 °F
Design TD	37 °F

Summer Design Conditions

Outside db	92 °F
Inside db	75 °F
Design TD	17 °F
Daily range	M
Relative humidity	50 %
Moisture difference	52 gr/lb

Heating Summary

Structure	19209 Btuh
Ducts	2699 Btuh
Central vent (0 cfm)	0 Btuh
Humidification	0 Btuh
Piping	0 Btuh
Equipment load	21908 Btuh

Sensible Cooling Equipment Load Sizing

Structure	20223 Btuh
Ducts	5842 Btuh
Central vent (0 cfm)	0 Btuh
Blower	0 Btuh
Use manufacturer's data	n
Rate/swing multiplier	0.97
Equipment sensible load	25284 Btuh

Infiltration

Method	Simplified
Construction quality	Average
Fireplaces	0

Latent Cooling Equipment Load Sizing

Structure	3295 Btuh
Ducts	1314 Btuh
Central vent (0 cfm)	0 Btuh
Equipment latent load	4609 Btuh

	Heating	Cooling
Area (ft ²)	1595	1595
Volume (ft ³)	12760	12760
Air changes/hour	0.38	0.20
Equiv. AVF (cfm)	81	43

Equipment total load	29893 Btuh
Req. total capacity at 0.78 SHR	2.7 ton

Heating Equipment Summary

Make TEMPSTAR
Trade HEATPUMP
Model N4H336AKA

Efficiency	8.5 HSPF
Heating input	
Heating output	0 Btuh @ 47°F
Temperature rise	0 °F
Actual air flow	0 cfm
Air flow factor	0.000 cfm/Btuh
Static pressure	0.60 in H2O
Space thermostat	

Cooling Equipment Summary

Make TEMPSTAR
Trade HEATPUMP
Cond N4H336AKA
Coil FEM4X3600

Efficiency	14 SEER
Sensible cooling	26988 Btuh
Latent cooling	7612 Btuh
Total cooling	34600 Btuh
Actual air flow	935 cfm
Air flow factor	0.036 cfm/Btuh
Static pressure	0.60 in H2O
Load sensible heat ratio	0.85

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Page 1

Project Information

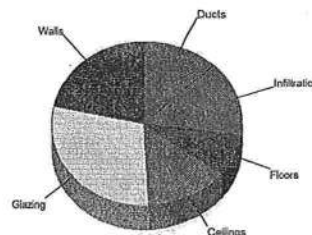
For: BAYBURY 2ND FLOOR

Design Conditions

Location:		Indoor:		Heating	Cooling
Gainesville, FL, US		Indoor temperature (°F)		70	75
Elevation: 151 ft		Design TD (°F)		37	17
Latitude: 30°N		Relative humidity (%)		30	50
		Moisture difference (gr/lb)		10.6	52.0
Outdoor:		Heating	Cooling		
Dry bulb (°F)		33	92		
Daily range (°F)		-	19 (M)		
Wet bulb (°F)		-	77		
Wind speed (mph)		15.0	7.5		
		Infiltration:			
		Method		Simplified	
		Construction quality		Average	
		Fireplaces		0	

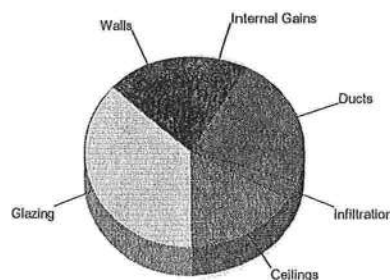
Heating

Component	Btuh/ft²	Btuh	% of load
Walls	3.6	4737	21.6
Glazing	47.0	6391	29.2
Doors	0.0	0	0.0
Ceilings	1.8	3001	13.7
Floors	2.9	1809	8.3
Infiltration	2.2	3271	14.9
Ducts		2699	12.3
Piping		0	0.0
Humidification		0	0.0
Ventilation		0	0.0
Adjustments		0	0.0
Total		21908	100.0



Cooling

Component	Btuh/ft²	Btuh	% of load
Walls	2.6	3425	13.1
Glazing	71.1	9668	37.1
Doors	0.0	0	0.0
Ceilings	2.6	4270	16.4
Floors	0.0	0	0.0
Infiltration	0.5	791	3.0
Ducts		5842	22.4
Ventilation		0	0.0
Internal gains		2070	7.9
Blower		0	0.0
Adjustments		0	0.0
Total		26066	100.0



Overall U-value = 0.115 Btuh/ft²-°F

Data entries checked.

Duct System Summary
Entire House
MARONDA HOMES

Job: BAYBURY 1ST FLOOR
 Date:
 By: G. CARMACK

4005 MARONDA WAY, SANFORD, FL 32771 Phone: (407) 321-0064

Project Information

For: BAYBURY 1ST FLOOR

	Heating	Cooling
External static pressure	0.60 in H2O	0.60 in H2O
Pressure losses	0.00 in H2O	0.00 in H2O
Available static pressure	0.60 in H2O	0.60 in H2O
Supply / return available pressure	0.46 / 0.14 in H2O	0.46 / 0.14 in H2O
Lowest friction rate	2.308 in/100ft	2.308 in/100ft
Actual air flow	935 cfm	935 cfm
Total effective length (TEL)	26 ft	

Supply Branch Detail Table

Name	Design (Btuh)	Htg (cfm)	Clg (cfm)	Design FR	Diam (in)	Rect Size (in)	Duct Matl	Actual Ln (ft)	Ftg.Eqv Ln (ft)	Trunk
KITCHEN	h 5247	200	200	4.615	7	x0	VIFx	10.0	0.0	st1
FAMILY	h 4430	200	200	4.615	7	x0	VIFx	10.0	0.0	st1
DINING	h 4708	200	200	2.308	7	x0	VIFx	20.0	0.0	st6
UTILITY	c 1472	65	65	9.231	4	x0	VIFx	5.0	0.0	st3
BATH #1	h 1142	65	65	4.615	4	x0	VIFx	10.0	0.0	st4
LIVING	c 6106	145	145	3.077	6	x0	VIFx	15.0	0.0	st2
FOYER	c 1445	65	65	4.615	4	x0	VIFx	10.0	0.0	st5

Supply Trunk Detail Table

Name	Trunk Type	Htg (cfm)	Clg (cfm)	Design FR	Veloc (fpm)	Diam (in)	Rect Duct Size (in)	Duct Material	Trunk
st1	Peak AVF	400	400	4.615	585	10	0 x 0	VinIFlx	
st2	Peak AVF	145	145	3.077	608	6	0 x 0	VinIFlx	
st3	Peak AVF	65	65	9.231	475	4	0 x 0	VinIFlx	
st4	Peak AVF	60	60	4.615	621	4	0 x 0	VinIFlx	
st5	Peak AVF	65	65	4.615	466	4	0 x 0	VinIFlx	
st6	Peak AVF	200	200	2.308	640	7	0 x 0	VinIFlx	

Bold/italic values have been manually overridden

Return Branch Detail Table

Name	Grill Size (in)	Htg (cfm)	Clg (cfm)	TEL (ft)	Design FR	Veloc (fpm)	Diam (in)	RectSize (in)	Stud/Joist Opening (in)	Duct Matl	Trunk
rb1	0x0	935	935	6.0	2.308	670	16	x0 0		VIFx	

Duct System Summary

Entire House

MARONDA HOMES

Job: BAYBURY 2ND FLOOR
 Date:
 By: G. CARMACK

4005 MARONDA WAY, SANFORD, FL 32771 Phone: (407) 321-0064

Project Information

For: BAYBURY 2ND FLOOR

	Heating	Cooling
External static pressure	0.60 in H2O	0.60 in H2O
Pressure losses	0.00 in H2O	0.00 in H2O
Available static pressure	0.60 in H2O	0.60 in H2O
Supply / return available pressure	0.43 / 0.17 in H2O	0.43 / 0.17 in H2O
Lowest friction rate	2.857 in/100ft	2.857 in/100ft
Actual air flow	935 cfm	935 cfm
Total effective length (TEL)	21 ft	

Supply Branch Detail Table

Name	Design (Btuh)	Htg (cfm)	Clg (cfm)	Design FR	Diam (in)	Rect Size (in)	Duct Matl	Actual Ln (ft)	Ftg.Eqv Ln (ft)	Trunk
BDR #4	h 261 5	165	165	4.286	7	x00	VIFx	10.0	0.0	st2
BDR #3	c 274 7	100	100	4.286	6	x00	VIFx	10.0	0.0	st1
MASTER TOILET	c 397	25	25	2.857	4	x00	VIFx	15.0	0.0	st1
BDR #2	c 475 2	150	150	2.857	7	x00	VIFx	15.0	0.0	st1
HALLWAY	c 391	35	35	4.286	5	x00	VIFx	10.0	0.0	st2
LOFT	h 537 3	165	165	4.286	7	x00	VIFx	10.0	0.0	st2
BATH #2	c 451	25	25	4.286	4	x00	VIFx	10.0	0.0	st1
MASTER BATH	h 749	30	30	8.571	5	x00	VIFx	5.0	0.0	st3
MASTER BDR-A	c 398 5	120	120	4.286	7	x00	VIFx	10.0	0.0	st3
MASTER BDR	c 398 5	120	120	4.286	7	x00	VIFx	10.0	0.0	st3

Supply Trunk Detail Table

Name	Trunk Type	Htg (cfm)	Clg (cfm)	Design FR	Veloc (fpm)	Diam (in)	Rect Duct Size (in)	Duct Material	Trunk
st1	Peak AVF	570	570	2.857	556	12	0 x 0	VinIFlx	st1
st2	Peak AVF	365	365	4.286	520	10	0 x 0	VinIFlx	
st3	Peak AVF	270	270	4.286	686	10	0 x 0	VinIFlx	

Bold/italic values have been manually overridden

Return Branch Detail Table

Name	Grill Size (in)	Htg (cfm)	Clg (cfm)	TEL (ft)	Design FR	Veloc (fpm)	Diam (in)	RectSize (in)	Stud/Joist Opening (in)	Duct Matl	Trunk
rb1	0x 0	935	935	6.0	2.857	670	16	x0 0		VIFx	

**Columbia County Building Department
Culvert Permit**

**Culvert Permit No.
000001526**

DATE 01/16/2008 PARCEL ID # 10-4S-16-02856-107
APPLICANT PATRICK WILSON PHONE 352 206-5459
ADDRESS 6800 SOUTHPOINT PARKWAY JACKSONVILLE FL 32216
OWNER MARONDA HOMES PHONE 904 296-1490
ADDRESS 255 SW TIMBER RIDGE DR LAKE CITY FL 32024
CONTRACTOR THEODORE BROCK PHONE 321-436-0393
LOCATION OF PROPERTY 90W, TL ON 247S, TR ON 252B, TL ON TIMBER RIDGE,
7TH LOT ON RIGHT

SUBDIVISION/LOT/BLOCK/PHASE/UNIT TIMBERLANDS 7

SIGNATURE

INSTALLATION REQUIREMENTS



Culvert size will be 18 inches in diameter with a total length of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4 : 1 slope and poured with a 4 inch thick reinforced concrete slab.

INSTALLATION NOTE: Turnouts will be required as follows:

- a) a majority of the current and existing driveway turnouts are paved, or;
 - b) the driveway to be served will be paved or formed with concrete.
- Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concreted turnouts.



Culvert installation shall conform to the approved site plan standards.



Department of Transportation Permit installation approved standards.



Other _____

**ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED
DURING THE INSTALATION OF THE CULVERT.**

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00



NOTICE OF COMMENCEMENT

266 24

County Clerk's Office Stamp or Seal

Tax Parcel Identification Number _____

THE UNDERSIGNED hereby gives notice that improvements will be made to certain real property, and in accordance with Section 713.13 of the Florida Statutes, the following information is provided in this NOTICE OF COMMENCEMENT.

1. Description of property (legal description): 711 Timberlands
a) Street (job) Address: 255 SW Timber Ridge Dr.
2. General description of improvements: Construction of a single family dwelling
3. Owner Information
a) Name and address: Maronda Homes Inc. of FL 6800 Southpoint Pkwy #300 Jax. FL 32216
b) Name and address of fee simple titleholder (if other than owner) _____
c) Interest in property _____
4. Contractor Information
a) Name and address: Theodore C. Brock 6800 Southpoint Pkwy #300 Jax. FL 32216
b) Telephone No.: (407)-227-3504 Fax No. (Opt.) _____
5. Surety Information
a) Name and address: _____
b) Amount of Bond: _____
c) Telephone No.: _____ Fax No. (Opt.) _____
6. Lender
a) Name and address: Bank of America 250 Park Ave S. #400 Winter Park, FL 32789
b) Phone No. _____
7. Identity of person within the State of Florida designated by owner upon whom notices or other documents may be served:
a) Name and address: Kampf Title and Guaranty Corp. P.O. Box 1359 200 W. 1st St.
b) Telephone No.: _____ Fax No. (Opt.) Winter Park, FL 32771
8. In addition to himself, owner designates the following person to receive a copy of the Lienor's Notice as provided in Section 713.13(l)(b), Florida Statutes:
a) Name and address: Staff at Maronda Homes Inc. of FL
b) Telephone No.: (904)-296-1490 Fax No. (Opt.) _____
9. Expiration date of Notice of Commencement (the expiration date is one year from the date of recording unless a different date is specified): _____

WARNING TO OWNER: ANY PAYMENTS MADE BY THE OWNER AFTER THE EXPIRATION OF THE NOTICE OF COMMENCEMENT ARE CONSIDERED IMPROPER PAYMENTS UNDER CHAPTER 713, PART I, SECTION 713.13, FLORIDA STATUTES, AND CAN RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY; A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT YOUR LENDER OR AN ATTORNEY BEFORE COMMENCING WORK OR RECORDING YOUR NOTICE OF COMMENCEMENT.

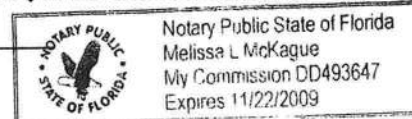
STATE OF FLORIDA
COUNTY OF COLUMBIA

10. [Signature]
Signature of Owner or Owner's Authorized Officer/Director/Partner/Manager
Theodore C. Brock
Print Name

The foregoing instrument was acknowledged before me, a Florida Notary, this 3 day of January, 2008, by:
Theodore C. Brock as V.P. of Construction (type of authority, e.g. officer, trustee, attorney fact) for Maronda Homes Inc. of FL (name of party on behalf of whom instrument was executed).

Personally Known XX OR Produced Identification _____ Type _____

Notary Signature Melissa L McKague Notary Stamp or Seal:



11. Verification pursuant to Section 92.525, Florida Statutes. Under penalties of perjury, I declare that I have read the foregoing and that the facts stated in it are true to the best of my knowledge and belief

[Signature]
Signature of Natural Person Signing (if line #10 above.)

2603 NW 74TH PLACE • GAINESVILLE, FLORIDA 32653 • PHONE: (352) 372-1274 • FAX: (352) 372-2721

REPORT OF SOIL DENSITY BY NUCLEAR METHODS

Client:..... Maronda Homes
 Address:..... 7478 SW 60th Avenue, Ste B
 City, State, Zip Code:..... Ocala, FL 34476
 Date of Test:..... February 1, 2008
 Test Location:..... Timber Lands, Lot 7 – 255 S.W. Timber Ridge Drive
 Area of Test:..... Building Pad & Footing

Test Methods: In Place Compaction Test per ASTM D-2922 Nuclear Method
 MAXIMUM DRY DENSITY PER AASHTO T-180 (MODIFIED PROCTOR)

TEST LOCATION	LAB DENSITY lb/ft ³	PERCENT OPTIMUM MOISTURE	DRY DENSITY lb/ft ³	PERCENT FIELD MOISTURE	PERCENT DENSITY
Center of Pad final grade	103.7	12.0	101.8	8.6	98.2
Center of N. Footing final grade	103.7	12.0	98.8	11.3	95.3
Center of S. Footing final grade	103.7	12.0	99.2	10.9	95.7

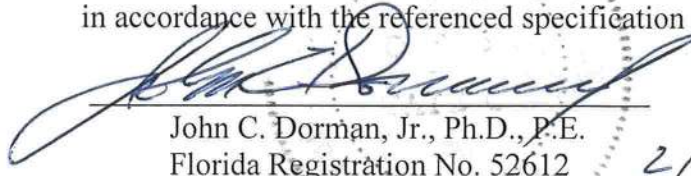
GTI is not responsible for determining thickness of fill soils. Above mentioned tests represent that location only. No other warranties are expressed or implied.

Lift = 12" Thickness

Field Technician: WC

Remarks: Permit No. 22624

The above tests were performed and reported
 in accordance with the referenced specification


 John C. Dorman, Jr., Ph.D., P.E.
 Florida Registration No. 52612

2/11/08

COLUMBIA COUNTY FLORIDA

OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 10-4S-16-02856-107

Building permit No. 000026624

Use Classification SFD, UTILITY

Fire: 32.10

Permit Holder THEODORE BROCK

Waste: 83.75

Owner of Building MARONDA HOMES

Total: 115.85

Location: 255 SW TIMBER RIDGE DR, LAKE CITY, FL

Date: 05/19/2008

Wayne P. Lee

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)



Maronda Systems

Page 1 of 3

Maronda Systems 4005 Maronda Way Sanford FL 32771 (407) 321-0064
 Engineer of Record: Tomas Ponce, P.E. 367 Medallion PL. Chuluota, FL. 32766
 Design Criteria: TPI Design: Matrix Analysis TEE-LOK software

Fax (407) 321-3913
 FL PE # 50068,

PLAN JOB #	LOT	ADDRESS	DIV/SUB	MODEL	BAYBURY S 4
1TM00701	7-1		ORA-1TM	BBRS4 LEFT	

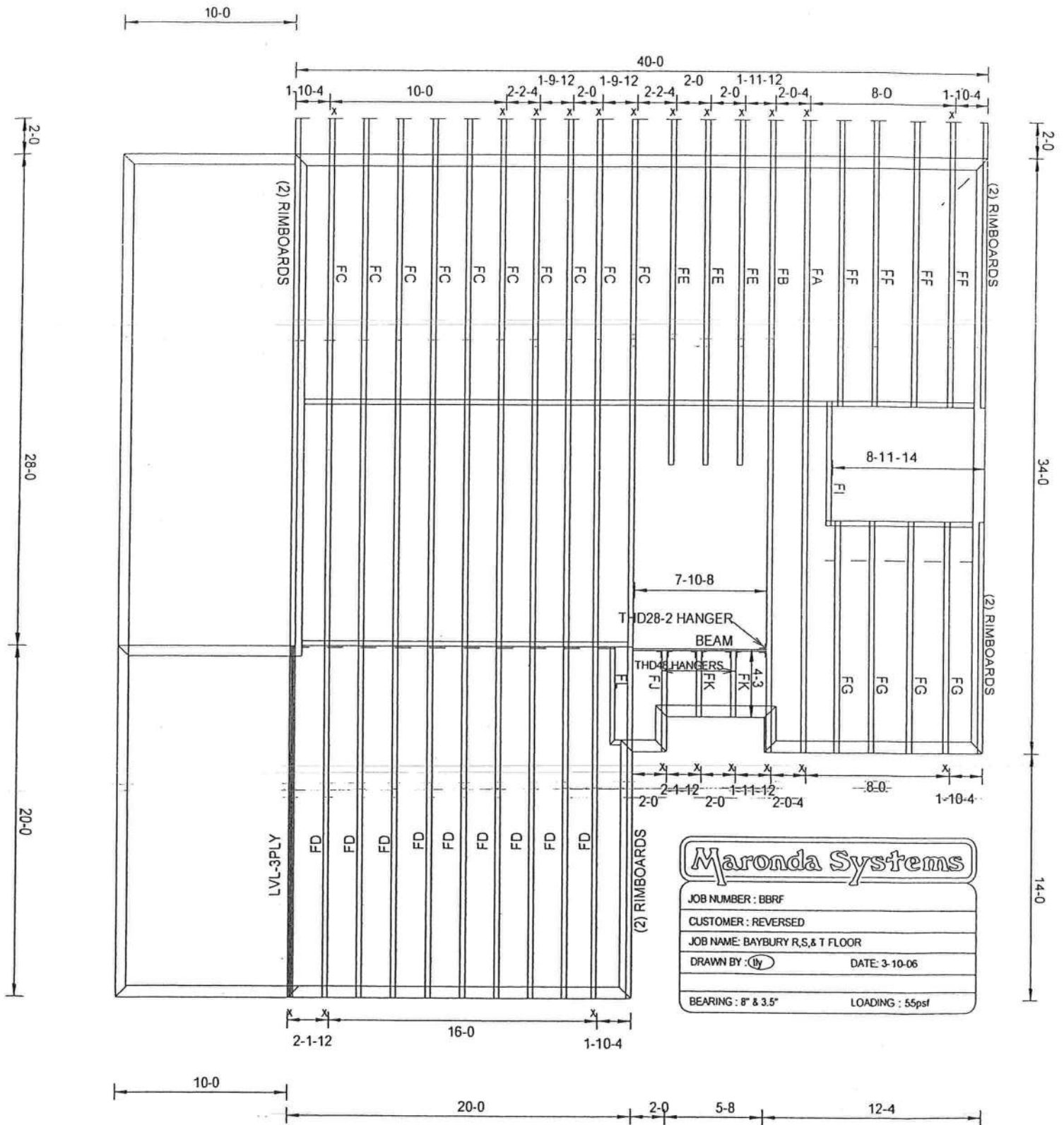
This structure was designed in accordance with, and meets the requirements of TPI standards and the FLORIDA 2004 BUILDING CODE for 125 M.P.H. Wind Zone.

Truss loading is in accordance with ASCE 7-02. These trusses are designed for an enclosed building.

Truss ID	Run Date	Drawing Reviewed	Truss ID	Run Date	Drawing Reviewed	No. of Eng. Dwgs: 74
Truss Layout	7-31-06	✓	PJGRD	7-29-06	✓	Roof Loads-
V	7-27-05		PMGRD1	7-29-06		TC Live: 16.0 psf
HIP	11-2-06		PMH1	7-29-06		TC Dead: 7.0 psf
C1	7-29-06		PMH2	7-29-06		BC Live: 10.0 psf
C1-1 Showing field assembly.			RG1	7-29-06		BC Dead: 10.0 psf
H1	7-29-06		S1	7-29-06		Total 43.0 psf
H2	7-29-06		S1A	7-29-06		DurFac- Lbr: 1.25
H3	7-29-06		S1B	7-29-06		DurFac- Plt: 1.25
H4	7-29-06		V1	7-29-06		O.C. Spacing: 24.0"
H5	7-29-06		V10	7-29-06		
HGRD1	7-29-06		V2	7-29-06		Floor Loads-
HGRD2	7-29-06		V3	7-29-06		TC Live: 40.0 psf
HS1	7-29-06		V4	7-29-06		TC Dead: 10.0 psf
HSGRD1	7-29-06		V5	7-29-06		BC Live: 0.0 psf
J	7-29-06		V8	3-16-07		BC Dead: 5.0 psf
J1	8-28-07		V9	3-16-07		Total 55.0 psf
J1A	8-28-07		Floor Layout	3-10-06	✓	DurFac- Lbr: 1.00
J2	8-28-07		FA	5-10-07		DurFac- Plt: 1.00
J2A	8-28-07		FB	5-10-07		O.C. Spacing: 24.0"
J3	8-28-07		FC	5-10-07		
J3A	8-28-07		FD	5-10-07		
J4	7-29-06		FE	5-10-07		
J5	8-28-07		FF	5-10-07		
J6	8-28-07		FG	5-10-07		
J7	8-28-07		FI	5-10-07		
JA	8-28-07		FJ	5-10-07		
JB	7-29-06		FK	5-10-07		
JGRD1	7-29-06		FL	5-14-07		
JGRD2	7-29-06					
JS	7-29-06					
JS1	7-29-06					
JS2	7-29-06					
JS3	7-29-06					
JS4	7-29-06		INV #	DESC	QNTY	
JSGRD1	7-29-06		18331	THD26	1	
MGRD1	7-29-06		18336	THD28-2	1	
MH1	7-29-06		18363	JUS26	3	
MHGRD2	7-29-06		18370	THJ26	6	
P1	8-28-07		18329	THD48	3	
P2	8-28-07		FLOOR SEAT PLATES		52	
P3	8-28-07		ROOF SEAT PLATES		45	



DATE: OCT 12 2007



OCT 12 2007

WARNING! BCSI-B1 SUMMARY SHEET - GUIDE FOR HANDLING TRUSSES

GENERAL NOTES

Trusses are not marked in any way to identify the frequency or location of temporary lateral restraint and diagonal bracing. Follow the recommendations for handling, installing and temporary restraining and bracing of trusses. Refer to *BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses* for more detailed information.

Truss Design Drawings may specify locations of permanent lateral restraint or reinforcement for individual truss members. Refer to the *BCSI-B3 Summary Sheet - Permanent Restraint/Bracing of Chords & Web Members* for more information. All other permanent bracing design is the responsibility of the Building Designer.

NOTAS GENERALES

Los trusses no están marcados de ningún modo que identifique la frecuencia o localización de restricción lateral y arrioste diagonal temporales. Use las recomendaciones de manejo, instalación, restricción y arrioste temporal de los trusses. Vea el folleto *BCSI Guía de Buena Práctica para el Manejo, Instalación, Restricción y Arrioste de los Trusses de Madera Conectados con Placas de Metal* para información más detallada.

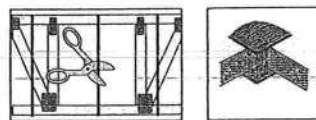
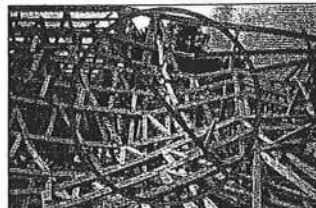
Los dibujos de diseño de los trusses pueden especificar las localizaciones de restricción lateral permanente o refuerzo en los miembros individuales del truss. Vea la hoja resumen *BCSI-B3 - Restricción/Arrioste Permanente de Cuerdas y Miembros Secundarios* para más información. El resto de los diseños de arriostres permanentes son la responsabilidad del Diseñador del Edificio.

⚠ The consequences of improper handling, erecting, installing, restraining and bracing can result in a collapse of the structure, or worse, serious personal injury or death.

El resultado de un manejo, levantamiento, instalación, restricción y arrioste incorrecto puede ser la caída de la estructura o aún peor, heridos o muertos.

⚠ Banding and truss plates have sharp edges. Wear gloves when handling and safety glasses when cutting banding.

Empaques y placas de metal tienen bordes afilados. Use guantes y lentes protectores cuando corte los empaques.



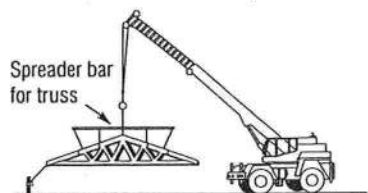
HANDLING — MANEJO

⚠ Avoid lateral bending. — Evite la flexión lateral.



⚠ Use special care in windy weather or near power lines and airports.

Utilice cuidado especial en días ventosos o cerca de cables eléctricos o de aeropuertos.

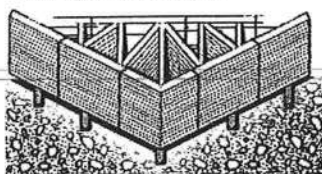


✓ Use proper rigging and hoisting equipment.

Use equipo apropiado para levantar e improvisar.

⚠ The contractor is responsible for properly receiving, unloading and storing the trusses at the jobsite.

El contratista tiene la responsabilidad de recibir, descargar y almacenar adecuadamente los trusses en la obra.



✓ If trusses are to be stored horizontally, place blocking of sufficient height beneath the stack of trusses at 8' to 10' on center.

For trusses stored for more than one week, cover bundles to prevent moisture gain but allow for ventilation.

Refer to *BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses* for more detailed information pertaining to handling and jobsite storage of trusses.

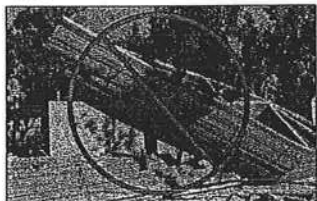
Si los trusses estarán guardados horizontalmente, ponga bloqueando de altura suficiente detrás de la pila de los trusses.

Para trusses guardados por más de una semana, cubra los paquetes para prevenir aumento de humedad pero permita ventilación.

Vea el folleto *BCSI Guía de Buena Práctica para el Manejo, Instalación, Restricción y Arriostres de los Trusses de Madera Conectados con Placas de Metal* para información más detallada sobre el manejo y almacenamiento de los trusses en área de trabajo.

⚠ Do not store unbraced bundles upright.

No almacene verticalmente los trusses sueltos.



⚠ Do not store on uneven ground.

No almacene en tierra desigual.



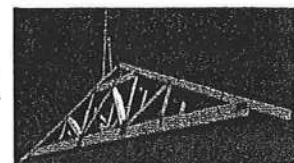
HOISTING RECOMMENDATIONS FOR TRUSS BUNDLES RECOMENDACIONES PARA LEVANTAR PAQUETES DE TRUSSES.

⚠ Warning! Don't overload the crane.
¡Advertencia! ¡No sobrecarga la grúa!

⚠ Never use banding alone to lift a bundle.
Do not lift a group of individually banded bundles.
Nunca use sólo los empaques para levantar un paquete.
No levante un grupo de empaques individuales.

✓ A single lift point may be used for bundles with trusses up to 45'.
Two lift points may be used for bundles with trusses up to 60'.
Use at least 3 lift points for bundles with trusses greater than 60'.

Puede usar un solo lugar de levantar para paquetes de trusses hasta 45 pies.
Puede usar dos puntos de levantar para paquetes más de 60 pies.
Use por lo menos tres puntos de levantar para paquetes más de 60 pies.



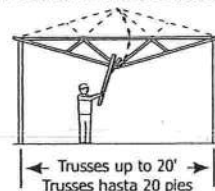
⚠ Warning! Do not over load supporting structure with truss bundle.
¡Advertencia! No sobrecargue la estructura apoyada con el paquete de trusses.

✓ Place truss bundles in stable position.
Puse paquetes de trusses en una posición estable.

INSTALLATION OF SINGLE TRUSSES BY HAND INSTALACIÓN POR LA MANO DE TRUSSES INDIVIDUALES

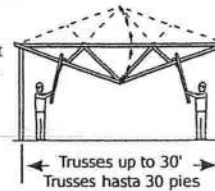
✓ Trusses 20' or less, support at peak.

Levante del pico los trusses de 20 pies o menos.



✓ Trusses 30' or less, support at quarter points.

Levante de los cuartos de tramo los trusses de 30 pies o menos.



HOISTING OF SINGLE TRUSSES — LEVANTAMIENTO DE TRUSSES INDIVIDUAL

✓ Hold each truss in position with the erection equipment until top chord temporary lateral restraint is installed and the truss is fastened to the bearing points.

Sostenga cada truss en posición con equipo de grúa hasta que la restricción lateral temporal de la cuerda superior esté instalado y el truss está asegurado en los soportes.

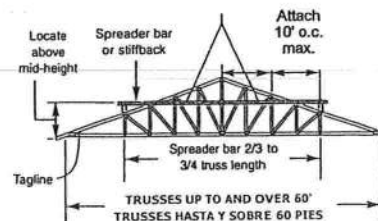
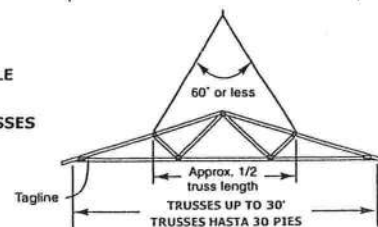
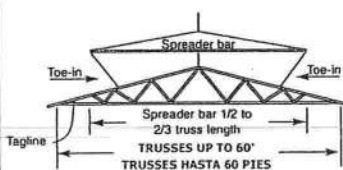
⚠ Warning! Using a single pick-point at the peak can damage the truss.

¡Advertencia! El uso de un solo lugar para levantar en el pico puede hacer daño al truss.



HOISTING RECOMMENDATIONS FOR SINGLE TRUSSES

RECOMENDACIONES PARA LEVANTAR TRUSSES INDIVIDUALES



TEMPORARY RESTRAINT & BRACING RESTRICCIÓN Y ARRIOSTRE TEMPORAL

⚠ Refer to *BCSI-B2 Summary Sheet - Truss Installation & Temporary Restraint/Bracing* for more information.

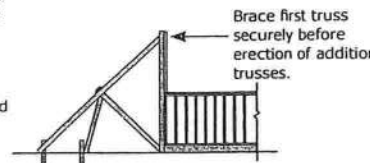
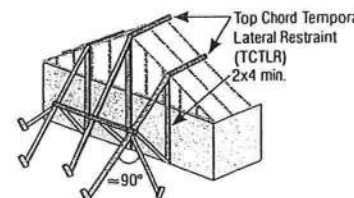
Vea el resumen *BCSI B2 - Restricción/Arrioste Temporal y Instalación de los Trusses* para más información.

✓ Locate ground braces for first truss directly in line with all rows of top chord temporary lateral restraint (see table in the next column).

Coloque los arriostres de tierra para el primer truss directamente en línea con cada una de las filas de restricción lateral temporal de la cuerda superior (vea la tabla en la próxima columna).



⚠ Do not walk on unbraced trusses.
No camine en trusses sueltos.



TEPS TO SETTING TRUSSES

AS MEDIDAS DE LA INSTALACIÓN DE LOS TRUSSES

- 1) Install ground bracing. 2) Set first truss and attach securely to ground bracing. 3) Set next 4 trusses with short member temporary lateral restraint (see below). 4) Install top chord diagonal bracing (see below). 5) Install web member plane diagonal bracing to stabilize the first five trusses (see below). 6) Install bottom chord temporary lateral restraint and diagonal bracing (see below). 7) Repeat process on groups of four trusses until all trusses are set.

- 1) Instale los arriostres de tierra. 2) Instale el primero truss y ate seguramente al arriostre de tierra. 3) Instale los próximos cuatro trusses con restricción lateral temporal de miembro corto (vea abajo). 4) Instale el arriostre diagonal de la cuerda superior (vea abajo). 5) Instale arriostre diagonal para los planos de los miembros secundarios para estable los primeros cinco trusses (vea abajo). 6) Instale la restricción lateral temporal y arriostre diagonal para la cuerda inferior (vea abajo). 7) Repita este procedimiento en grupos de cuatro trusses hasta que todos los trusses estén instalados.

- Refer to BCSI-B2 Summary Sheet - Truss Installation & Temporary Restraint/Bracing for more information.

Vea el resumen BCSI-B2 - Instalación de Trusses y Arriostre Temporal para mayor información.

ESTRAINT/BRACING FOR ALL PLANES OF TRUSSES

L RESTRICCIÓN/ARRIOSTRE EN TODOS PLANOS DE TRUSSES.

- This restraint & bracing method is for all trusses except 3x2 and 4x2 parallel chord trusses.

Este método de restricción y arriostre es para todo trusses excepto trusses de cuerdas paralelas 3x2 y 4x2.

TOP CHORD — CUERDA SUPERIOR

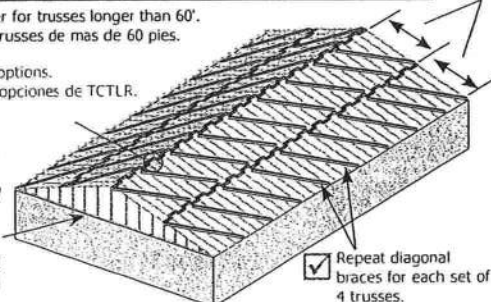
Truss Span Longitud de Tramo	Top Chord Temporary Lateral Restraint (TCLTR) Spacing Espaciamiento del Arriostre Temporal de la Cuerda Superior
Up to 30' Hasta 30 pies	10' o.c. max. 10 pies máximo
30' to 45' 30 a 45 pies	8' o.c. max. 8 pies máximo
45' to 60' 45 a 60 pies	6' o.c. max. 6 pies máximo
60' to 80'* 60 a 80 pies*	4' o.c. max. 4 pies máximo

*Consult a Professional Engineer for trusses longer than 60'.
*Consulte a un ingeniero para trusses de mas de 60 pies.

- See BCSI-B2 for TCLTR options.
Vea el BCSI-B2 para las opciones de TCLTR.

- Refer to BCSI-B3 Summary Sheet - Permanent Restraint/Bracing of Chords & Web Members for Gable End Frame restraint/bracing/reinforcement information.

Para información sobre restricción/arriostre/refuerzo para armazón de hastial vea el resumen BCSI-B3 - Restricción/Arriostre Permanente de Cuerdas y Miembros Secundarios.

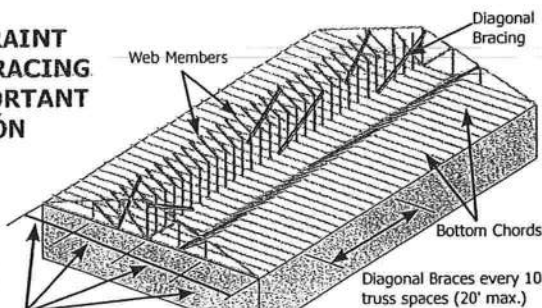


Ground bracing not shown for clarity.

- Repeat diagonal braces for each set of 4 trusses.
Repita los arriostres diagonales para cada grupo de 4 trusses.

WEB MEMBER PLANE — PLANO DE LOS MIEMBROS SECUNDARIOS

LATERAL RESTRAINT & DIAGONAL BRACING ARE VERY IMPORTANT LA RESTRICCIÓN LATERAL Y EL ARRIOSTRE DIAGONAL SON MUY IMPORTANTES!



10'-15' max. Same spacing as bottom chord Lateral Restraint

Some chord and web members not shown for clarity.

BOTTOM CHORD — CUERDA INFERIOR

Lateral Restraints - 2x4x12' or greater lapped over two trusses.

Bottom chords

10'-15' max.

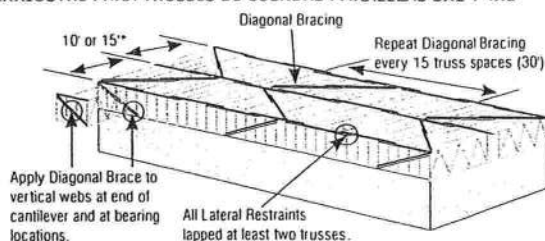
Some chord and web members not shown for clarity.

RESTRAINT & BRACING FOR 3x2 AND 4x2 PARALLEL CHORD TRUSSES

LA RESTRICCIÓN Y EL ARRIOSTRE PARA TRUSSES DE CUERDAS PARALELAS 3X2 Y 4X2

- Refer to BCSI-B7 Summary Sheet - Temporary & Permanent Restraint/Bracing for Parallel Chord Trusses for more information.

Vea el resumen BCSI-B7 - Restricción y Arriostre Temporal y Permanente de Trusses de Cuerdas Paralelas para más información.

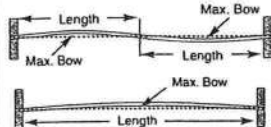


Apply Diagonal Brace to vertical webs at end of cantilever and at bearing locations.

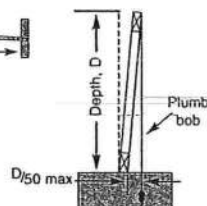
*Top chord Temporary Lateral Restraint spacing shall be 10' o.c. max. for 3x2 chords and 15' o.c. for 4x2 chords.

INSTALLING — INSTALACION

- Tolerances for Out-of-Plane.
Tolerancias para Fuera-de-Plano.



- Tolerances for Out-of-Plumb.
Tolerancias para Fuera-de-Plomada.



D/50	D (ft.)
1/4"	1'
1/2"	2'
3/4"	3'
1"	4'
1-1/4"	5'
1-1/2"	6'
1-3/4"	7'
2"	≥8'

Out of Plane	Max. Bow	Truss Length
1/4"	3/4"	12.5'
1/2"	7/8"	14.6'
3/4"	1"	16.7'
1"	1-1/8"	18.8'
1-1/4"	1-1/4"	20.8'
1-1/2"	1-3/8"	22.9'
1-3/4"	1-1/2"	25.0'
2"	1-3/4"	29.2'
2"	2"	≥33.3'

CONSTRUCTION LOADING — CARGA DE CONSTRUCCION

- Do not proceed with construction until all lateral restraint and bracing is securely and properly in place.

No proceda con la construcción hasta que todas las restricciones laterales y los arriostres estén colocados en forma apropiada y segura.

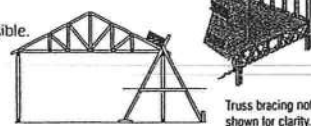
- Do not exceed maximum stack heights. Refer to BCSI-B4 Summary Sheet - Construction Loading for more information.

No exceda las máximas alturas recomendadas. Vea el resumen BCSI-B4 Carga de Construcción para mayor información.

Material	Height
Gypsum Board	12"
Plywood or OSB	16"
Asphalt Shingles	2 bundles
Concrete Block	8"
Clay Tile	3-4 tiles high



- Do not overload small groups or single trusses.
No sobrecargue pequeños grupos o trusses individuales.
- Never stack materials near a peak.
Nunca amontone material cerca del pico.
- Place loads over as many trusses as possible.
Coloque las cargas sobre tantos trusses como sea posible.
- Position loads over load bearing walls.
Coloque las cargas sobre las paredes soportantes.

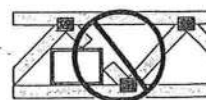


ALTERATIONS — ALTERACIONES

- Refer to BCSI-B5 Summary Sheet - Truss Damage, Jobsite Modifications & Installation Errors.

Vea el resumen BCSI-B5 Daños de trusses, Modificaciones en la Obra y Errores de Instalación.

- Do not cut, alter, or drill any structural member of a truss unless specifically permitted by the Truss Design Drawing.
No corte, altere o perforo ningún miembro estructural de los trusses, a menos que esté específicamente permitido en el dibujo del diseño del truss.



- Trusses that have been overloaded during construction or altered without the Truss Manufacturer's prior approval may render the Truss Manufacturer's limited warranty null and void.

Trusses que se han sobrecargado durante la construcción o han sido alterados sin una autorización previa del Fabricante de Trusses, pueden reducir o eliminar la garantía del Fabricante de Trusses.

NOTE: The Truss Manufacturer and Truss Designer rely on the presumption that the Contractor and crane operator (if applicable) are professionals with the capability to undertake the work they have agreed to do on any given project. If the Contractor believes it needs assistance in some aspect of the construction project, it should seek assistance from a competent party. The methods and procedures outlined in this document are intended to ensure that the overall construction techniques employed will put the trusses into place SAFELY. These recommendations for handling, installing, restraining and bracing trusses are based upon the collective experience of leading personnel involved with truss design, manufacture and installation, but must, due to the nature of responsibilities involved, be presented only as a GUIDE for use by a qualified Building Designer or Contractor. It is not intended that these recommendations be interpreted as superior to the Building Designer's design specification for handling, installing, restraining and bracing trusses and it does not preclude the use of other equivalent methods for restraining/bracing and providing stability for the walls, columns, floors, roofs and all the interrelated structural building components as determined by the Contractor. Thus, WTCA and TPI expressly disclaim any responsibility for damages arising from the use, application, or reliance on the recommendations and information contained herein.



6300 Enterprise Lane • Madison, WI 53719
608/274-4849 • www.sbcindustry.com



TRUSS PLATE INSTITUTE
218 N. Lee St., Ste. 312 • Alexandria, VA 22314
703/683-1010 • www.tpinst.org

Maronda Systems

MARONDA SYSTEMS

4005 Maronda Way

Sanford, FL 32771

(407) 321-0064

Fax (407) 321-3913

Date: November 1, 2006

To: Building Department

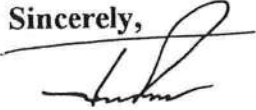
**From: Maronda Systems
Tomas Ponce
Professional Engineer
State of Florida #0050068**

Subject: Valley Trusses

All valley trusses labeled V-1 through 100 are covered under the general valley sheet provided in the truss package signed and sealed by the engineer of record. The connections are noted on the structural info sheet of the plans. All criteria of the valley trusses are noted on the general sheet.

If you have any questions please feel free to call at 407-321-0064.

Sincerely,



Tomas Ponce, P.E.

Date: 11/1/06

DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the trust designed by the client and the correctness or accuracy of this information as it may apply to a specific project and does not assume any responsibility for design or construction errors, fabrication, shipping and installation of trusses. This truss has been designed in accordance with ANSI/TPI 1-1995 and NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the building designer, the design loadings shown must be checked to be sure that the data shown is in agreement with the local building codes, local climatic records for wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rafter, they should be braced in a similar manner. (See Figure 10-16) (10/95) This truss was manufactured from 200 grade, hot dipped galvanized steel in accordance with ASTM A36, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing familiarity with specific verification procedures. Fabricators are to be put in notice before cutting or fabricating. Plates shall be cut in advance of cutting, bolts or disks to wood grain. Members shall be cut for tight fitting, both to wood and steel. All plates shall be cut to size, both for the joints with nails, plates welded and shall be cut to match the joint unless otherwise shown. A 5/4 shall be cut to 4" long. A 6/8 plate is 6" wide, 3" long. Steel plates shall be cut to the plate length specified. Double cut web members shall meet at the middle of the web with no overlap. Connectors plate ends are minimum 2 inches based on the forces shown and may need to be increased for certain handling and/or erection stresses. This must be not be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSII/TPI-1-1995.

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be loaded with particular care during banding and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary erection bracing requires such temporary bracing during installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until after all fastening and bracing is completed.

Job: BAYBURY S 4	Customer: 10# UPLIFT D.L.	WO: BERS4	TI: C1	Qty: 4
------------------	---------------------------	-----------	--------	--------

TC: 2x 4 SP #2
BC: 2x 4 SP #2
WB: 2x 6 SP #2
2x 4 SP #2
5-70-0
1-52-4,3-4
0-0-0

0.0.0

MULTIPLE LOADCASES -- This design is the composite result of multiple loadcases. All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise. Windload per ASCE 7-98, C&C, V = 15mph, H = 22.0 ft, I = 1.00, Exp. Cat. B, Kzt = 1.0 Bld Type = encl I = 36.0 ft W = 4.8 ft Truss in END zone, TCDF = 4.2 psf, BCDL = 6.0 psf Impact Resistant Covering and/or Glazing Req FORCES (lb) - Max Compression/Max Tension TOP CHORD 1-2 = 363/835, 2-3 = 68/32, BOT CHORD 7-6 = -14/4, 6-5 = 361/1072, 5-4 = 805/230, 5-2 = 487/215, Webs 7-1 = -221/651, 7-5 = 1893/630, 1-5 = -513/127, 2-4 = -348/1047, 4-3 = 55/145, -----GLOBAL MAX DEFLECTIONS-----

	LL	TL
I. Span	in./Ratio 0.02/xxx	in./Ratio 0.01/xxx
Horiz.	-0.12	-0.23
	Max DL Deflection L/xxx = -0.01	
	Long term deflection factor = 1.50	

Job: BAYBURY S 4	Customer: 10# UPLIFT D.L.	WO: BERS4	TI: C1	Qty: 4
------------------	---------------------------	-----------	--------	--------

[] or {}: Plate(s) OFFSET from joint center. The Joint Detail Report must be included with any submittal, inspection, and/or fabrication documentation. This truss is designed to bear on multiple supports. Interior bearing locations should be marked on truss. Shim or wedge if necessary to achieve full bearing.

Bracing Schedule:

Web	Brace Pt	Joint to Joint
	1/2	1'-7.5'-7

Bracing shown is for visual purposes only.

Continuous lateral bracing attached to either edge of web(s) shown OR a T-brace, nailed flat to edge of web with 3"x 0.120" nails spaced 8" o.c. T-brace must extend at least 90% of web length and be same size, species, and grade as web.

2x6 T-brace required on any web over 14'-0".

WEB.	FORCE.	CSI.	Cr	WEB.	FORCE.	CSI.
6-5	1072	0.23	B	1-5	-53.0	0.
5-2	-487	0.04	C	2-4	1047	0.
7-1	651	0.72	C	4-3	145	0.
7-5	-1899	0.60	C			

Job: BAYBURY S 4	Customer: 10# UPLIFT D.L.	WO: BERS4	TI: C1	Qty: 4
------------------	---------------------------	-----------	--------	--------

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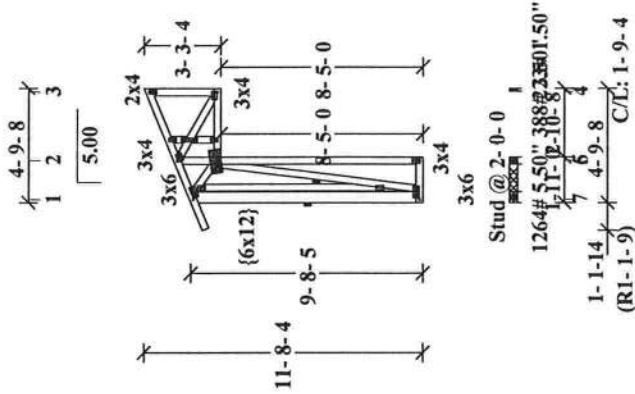
Joint Locations=
1) 0 -1-12      4) 4 -7-12      7) 0 -1-12
2) 1 -9 -4      5) 1 -9 -4
3) 4 -7-12      6) 1 -9 -4

L. and R. End verticals designed for wind
This truss has been designed for wind
in the plane of the truss only!!!
In-Plant Quality Assurance with Cq= 1
In-Plant Quality Assurance, per
sec. 3.2.4 of TPI-1-02, for joint(s):
4, 5

---MAX. REACTIONS PER BEARING LOCATION---
X-Loc      Bset Vert Horiz Uplift Y Type
0 -2-12    1 1264      681      -412 B Pin
1 -9 -4    1 388      0      -1103 B H Roll
4 -8-12    1 223      0      -695 B H Roll

PROVIDE UPLIFT CONNECTION PER SCHEDULE
PROVIDE HORIZONTAL CONNECTION PER SCHEDULE
..FC. ..FORCE..CSI.. Cr ..BC..FORCE..CSI.
1-2      835 0.24 A 7- 6      -14 0.08
2- 3      -68 0.16 A 5- 4      -805 0.09

```



COSMETIC PLATES (12) 2X4

EXCEPT AS SHOWN PLATES ARE MITAK MT20

Over 3 Supports Scale = 0.1736

Maronda Systems

WARNING:
**READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO
ERECTING CONTRACTOR. BRACING WARNING:**
Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members and roof purlins. It is not to be used for erection bracing at peak and eave locations. (See RIB-91 locations determined by the building designer. Additional bracing of the overall trusswork. (See RIB-91 of TPI). For specific truss bracing requirements, contact building designer (Truss Plate Institute, TPI is located at 383 O'nefro Drive, Madison, Wisconsin 53719).
Component Engineering by Truss Engineering Co., P.A., 818 Soundside Rd, Edenton, NC 27932.

4005 MARONDA WAY
Sanford, FL 32771
(407) 321-0064 Fax (407) 321-3913
TOMAS PONCE P.E. LICENSE #0050068
367 Medallion Pl. Chuluota, FL 32766

Eng Job:	WO: BBR54
Dwg:	TI: C1
Dsgnr: TLY	7/29/2006
TC Live 16.0 psf	Lbr DF: 1.25
TC Dead 7.0 psf	Plt DF: 1.25
BC Live 10.0 psf	O.C.: 2- 0- 0
BC Dead 10.0 psf	TP1-02/FBC-04
	Code: FLA
TOTAL 43.0 psf	v4.7.32-58646

Design: Matrix Analysis

Profile Path: C:\TEE-LOK\Work\Jobs\BBS4S\BAYBUP~1\C1.prx

	Joint Locations										
1)	0	0	0	6)	28	-5	-7	11)	23	0	0
2)	6	-1	-3	7)	33	-10	-13	12)	17	-0	0
3)	11	-6	-9	8)	40	-0	0	13)	8	-9	-14
4)	17	-0	0	9)	40	-0	0	14)	0	0	0
5)	23	-0	0	10)	31	-2	-2				

n-Plant Quality Assurance with Cq= 1
 n-Plant Quality Assurance, per

	TC	FORCE	CSI	Cr	BC	FORCE	CSI
1-	2	-3678	0.48	A 14-13		3352	1.00
2-	3	-3476	0.38	A 13-12		2869	0.90
3-	4	-2606	0.35	A 12-11		2359	0.71
4-	5	-2362	0.32	A 11-10		2869	0.85
5-	6	-2598	0.35	A 10-9		3353	0.92
6-	7	-3477	0.38	A			
7- 8		-3678	0.47	A			

Long term deflection factor = 1.50

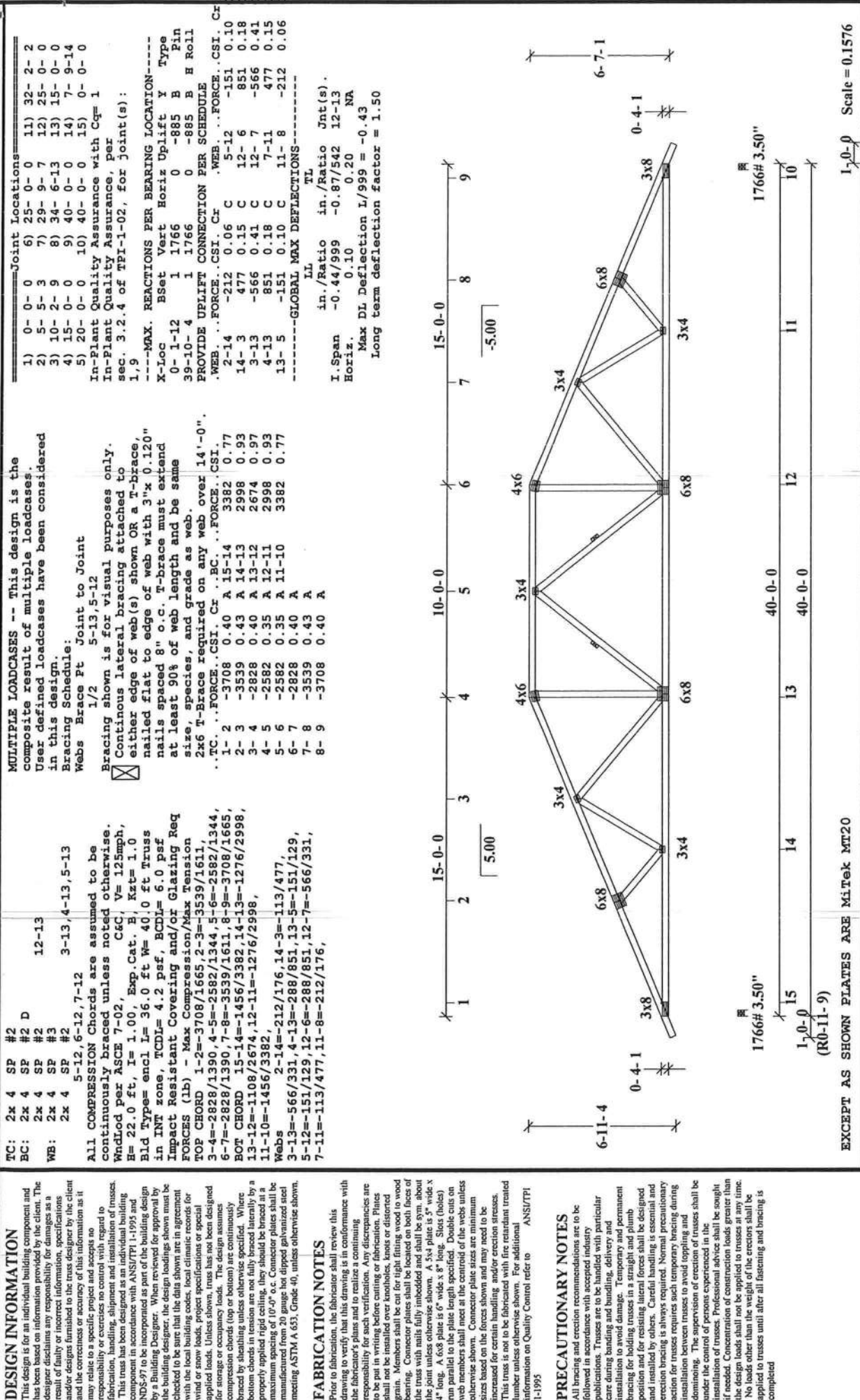


1 - 0. Scale = 0.1548

4005 MARONDA WAY
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(407) 321-0064 Fax (407) 321-3913
OMAS PONCE P.E. LICENSE #0050068
987 Medallion Pl, Chuluota, FL 32766

Eng Job:	WO: BERS4(R0-11-9)		
Dwg:	TT: H1		
Dsgnr: TLY	Chk:	7/29/2006	
TTC Live	16.0	psf	Lbr DF: 1.25
BC Dead	7.0	psf	Plt DF: 1.25
TTC Live	10.0	psf	O.C.: 2 - 0 - 0
BC Dead	10.0	psf	TPI-02/FBC-04
SBC Live	10.0	psf	Code: FLA
TOTAL	43.0	psf	v4.7 32-58647

Profile Path: C:\TEE-LOK\Work\Jobs\BBSRS4S\BAYBUP~1\H1.PRX



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Sanford, FL 32771
(407) 321-0064 Fax (407) 321-3913
TOMAS PONCE P.E. LICENSE #0050068
387 Medallion Pl, Chuluota, FL 32766

Design: Matrix Analysis

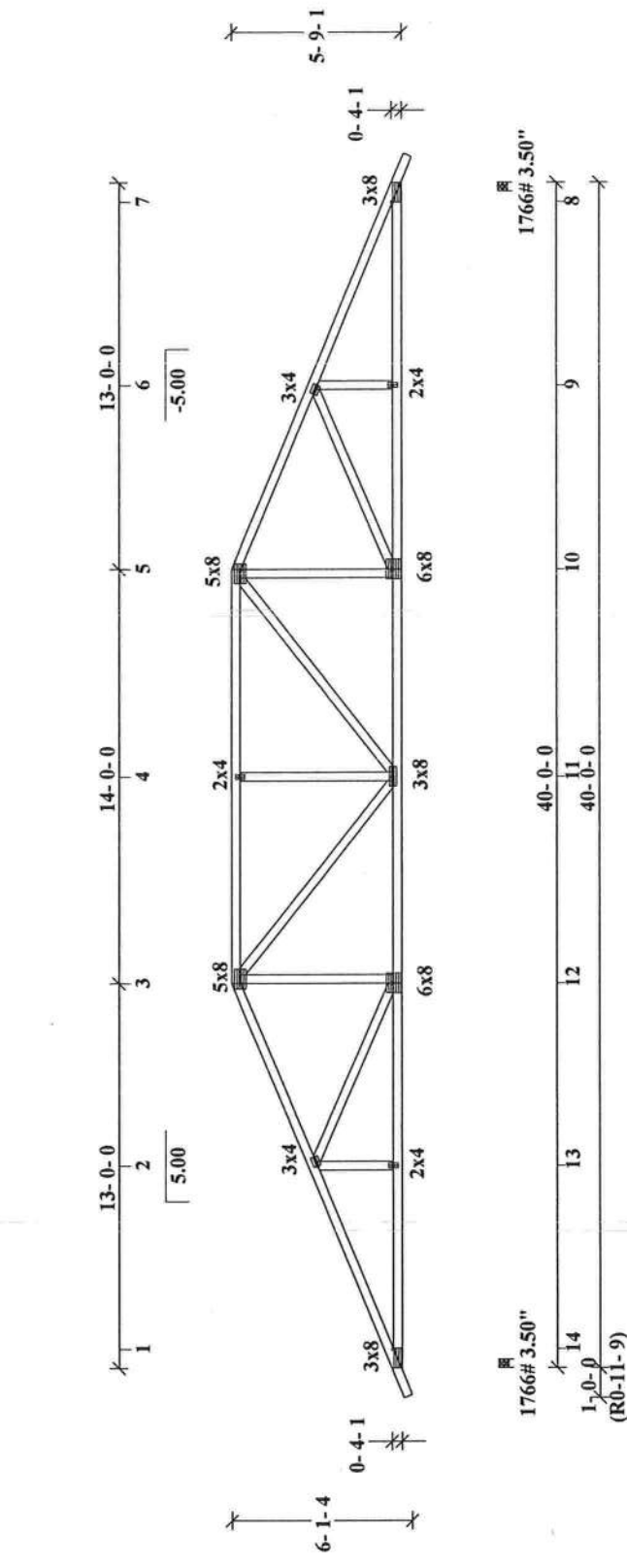
EXCEPT AS SHOWN PLATES ARE Mitek MT20

WARNING: READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING: Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HIB-91 of TPI) For specific truss bracing requirements, contact building designer (Truss Plate Institute, TPI is located at 583 D'Oro Drive, Madison, Wisconsin 53719). Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd, Edenon, NC 27932

Eng Job:	WO: BBSR4
Dwg:	TI: H2
Dsgnr: TLY	7/29/2006
Chk:	
TC Live	16.0 psf
TC Dead	7.0 psf
BC Live	10.0 psf
BC Dead	10.0 psf
TOTAL	43.0 psf

Lbr DF: 1.25
Plt DF: 1.25
O.C.: 2- 0- 0
TPI-02/FBC-04
Code: FLA
v4.7.32-58648

5
5
0
04
19



DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The building designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the trust designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accepts no responsibility for a specific project not to control with regard to construction, handling, shipment and installation of the materials, equipment, and components used in the project. The design is in accordance with ANSI/PTI-11995 and component is incorporated as part of the building design by a building designer. When reviewed for approval by the building designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind or snow loads, project specifications or special applied loads. Or, otherwise, trust has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by slatting unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a property appraiser of 10'-0" or less. Connector plates shall be manufactured from 26 gauge but dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to verify a continuing responsibility for the design. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over livebolts, knots or distorted wood. Connector plates shall be fitted, both faces of the joint with nails fully imbedded and shall be sym. about the centering. Connector plates shall be fabricated to wood dimensions as shown. A 5x4 plate is 5" wide x 4" thick. A 6x8 plate is 6" wide x 8" long. Steel (holser) members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or reaction stresses. This must is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI-1995.

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during banding and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary erection bracing for trusses requires such temporary bracing during installation between trusses to avoid toppling and domination. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than design loads shall not be applied to trusses at any time. No design loads shall be applied to trusses until they are applied to trusses until after all fastening and bracing is completed.

MULTIPLE LOADCASES -- This design is the composite result of multiple loadcases. User defined loadcases have been considered in this design.

In-Plant Quality Assurance with Cq= 1
In-Plant Quality Assurance, per sec. 3.2.4 of TPI-1-02, for joint(s):

1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50	51	52	53	54	55	56	57	58	59	60	61	62	63	64	65	66	67	68	69	70	71	72	73	74	75	76	77	78	79	80	81	82	83	84	85	86	87	88	89	90	91	92	93	94	95	96	97	98	99	100
H= 22.0 ft, I= 1.00, Exp. Cat. B, Kxt= 1.0 Weld Type= encl I= 40.0 ft W= 40.0 ft Truss n INT zone, TCDL= 4.2 psf, BCDL= 6.0 psf Impact Resistant Covering and/or Glazing Req TC...FORCE...CSI, Cr. BC...FORCE, CSI 1- 2 -3745 0.48 A 16-15 3411 0.77 2- 3 -3244 0.45 A 15-14 3404 0.78 3- 4 -3612 0.47 A 14-13 2943 0.70 4- 5 -3625 0.45 A 13-12 3626 0.82 5- 6 -3225 0.47 A 12-11 2944 0.71 6- 7 -3245 0.45 A 11-10 3405 0.78 7- 8 -3745 0.49 A 10- 9 -3411 0.77 -----GLOBAL MAX DEFLECTIONS-----																																																																																																			

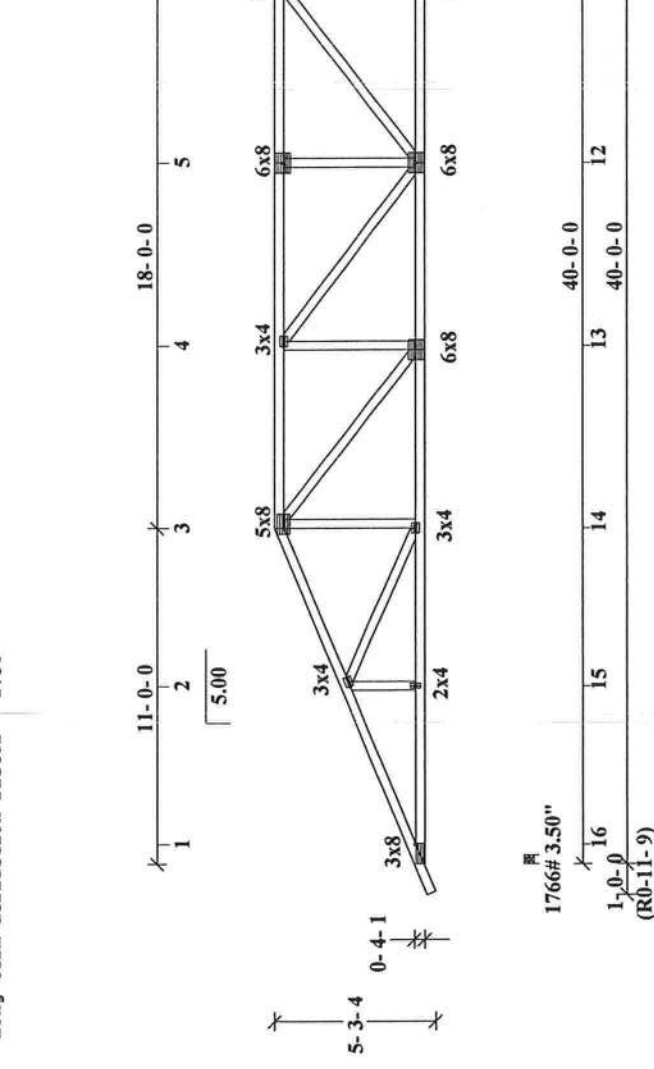
	LL	TL
in./Ratio	in./Ratio	in./Ratio
I. Span	-0.37/999	-0.74/640
Horiz.	0.12	0.23
		NA
	Max DL Deflection I/999 = -0.36	
	Long term deflection factor = 1.50	

MULTIPLE LOADCASES -- This design is the composite result of multiple loadcases. User defined loadcases have been considered in this design.

In-Plant Quality Assurance with Cq= 1
In-Plant Quality Assurance, per sec. 3.2.4 of TPI-1-02, for Joint(s):

1.8
FORCES (lb) - Max Compression/Max Tension
TOP CHORD 1-2=-3745/1712, 2-5=-3244/1535
3-4=-3612/1723, 4-5=-3625/1730, 5-6=-3625/1730, 6-7=-3245/1535, 7-8=-3745/1712,
BOT CHORD 16-15=-1500/3411, 15-14=-1501/3404, 14-13=-1222/2943, 13-12=-1483/3626, 12-11=-1222/2944, 11-10=-1501/3405, 10-9=-1500/3411,
Web3 15-2=0/178, 2-14=-506/309, 14-3=-124/461, 3-13=-317/842, 13-4=-271/217, 4-12=-4/8, 15-5=-266/271, 12-6=-324/851, 6-11=-125/464, 11-7=-506/309, 7-10=0/177

	LL	TL
in./Ratio	in./Ratio	in./Ratio
I. Span	-0.37/999	-0.74/640
Horiz.	0.12	0.23
		NA
	Max DL Deflection I/999 = -0.36	
	Long term deflection factor = 1.50	



EXCEPT AS SHOWN PLATES ARE MITek MT20

Maronda Systems

4005 MARONDA WAY
Sanford, FL 32771
(407) 321-0064 Fax (407) 321-3913
OMAS PONCE P.E. LICENSE #0050068
67 Medallion Pl. Chuluota, FL 32766

Design: Matrix Analysis

Profile Path: C:\TEE-LOK\Work\Jobs\BBS4S\BAYBUR~1\B4.prk

WARNING:
READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO
AFFECTING CONTRACTOR. BRACING WARNING:
Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of
the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss
members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified
members determined by the building designer. Additional bracing of the overall structure may be required. (SEE HUB-6)
OF TYPICAL BRACING CONNECTIONS. CONTRACT BUILDING DESIGNER (Truss Plate Institute, TPI) is located at 563
O'Connor Drive, Madison, Wisconsin 53719)
Consulting Engineer for: Truss Engineering Co., P.O. 818 Sundside Rd. Edenburg, NC 27932

Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd. Edenton, NC 27932

Eng Job:

Page 10

Dwg: Dsgnr: TLY Chk:

REC 11-10-68

Live 16.0 psi

TC Dead 7.0 psf

BC Live 10.0 psf

BC Dead 10.0 psf

TOTAL	43.0	psf
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WO: BBR54

ET. WA

7/29/2006

THE DE. 1 25

TWT DE: 1.25
 P1+ DE: 1.25

FAC DEF : 1-20
O C : 2-0-0

TPT-02/FRC-04

Code: FT.A

v4.7.32-58650

$$(\overrightarrow{R0-11-9})Scale = 0.1576$$

This design is for an individual building component and has been based on information provided by the client. The trustee disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the trustee designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accepts no liability for reliance on or excessive profit made by the contractor, handling, shipment and installation of trusses.

This trust has been designed in accordance with ANSHP-1995 standards and is intended to be incorporated as part of the building design by a Building Designer. When reviewed for approval by a Building Designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for windward or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes cladding sheathings (top or bottom) are continuously attached by steeling bolts otherwise specified. Where bottom chords in tension are not fully braced laterally by a permanent supporting system, the top chord shall be manufactured from 100x76 C-Sections. Connector plates shall be manufactured from A36, Grade 40, unless otherwise sheeting, ASTM A 653, Grade 40, unless otherwise sheeting.

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the applicable codes and standards and to assume the responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabricating. Plates shall be cut to size before cutting, known or suspected members shall be cut for tight fitting wood to wood joints. Corner plates shall be beveled both faces to wood joints usually 1/4" thick bevel and shall be 5/16" wide x 4' long. A 6x8 is 6" wide, 8" long. Double studs must parallel to plate and plate specified sizes are minimum unless otherwise noted. Precut plate plate sizes are minimum unless based on the forces shown and may need to be increased for certain handling and/or erection stresses. This mass is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1995.

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be braced with particular care during banding and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary erection bracing for trusses requires such temporary bracing and installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until after all fastening and bracing is completed.

Maronda Systems

Design: Matrix Analysis

Profile Path: C:\TEE-LOK\Work\Jobs\BBSR4S\BAYBUR~1\HSL.prx

WARNING:
READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO
ERECTING CONTRACTOR. BRACING WARNING:
Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of
the building design and which must be installed by the building designer. Bracing shown is for lateral support of truss
members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified
locations determined by the building designer. Additional bracing of the overall structure may be required. (See HB-9)
For more information, contact building designer (Truss Plate Institute, TPI is located at 583
O'Donnell Drive, Madison, Wisconsin 53719).
Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd, Edinburg, NC 27931.

Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd. Edenton, NC 27932

...

1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50	51	52	53	54	55	56	57	58	59	60	61	62	63	64	65	66	67	68	69	70	71	72	73	74	75	76	77	78	79	80	81	82	83	84	85	86	87	88	89	90	91	92	93	94	95	96	97	98	99	100	101	102	103	104	105	106	107	108	109	110	111	112	113	114	115	116	117	118	119	120	121	122	123	124	125	126	127	128	129	130	131	132	133	134	135	136	137	138	139	140	141	142	143	144	145	146	147	148	149	150	151	152	153	154	155	156	157	158	159	160	161	162	163	164	165	166	167	168	169	170	171	172	173	174	175	176	177	178	179	180	181	182	183	184	185	186	187	188	189	190	191	192	193	194	195	196	197	198	199	200	201	202	203	204	205	206	207	208	209	210	211	212	213	214	215	216	217	218	219	220	221	222	223	224	225	226	227	228	229	230	231	232	233	234	235	236	237	238	239	240	241	242	243	244	245	246	247	248	249	250	251	252	253	254	255	256	257	258	259	260	261	262	263	264	265	266	267	268	269	270	271	272	273	274	275	276	277	278	279	280	281	282	283	284	285	286	287	288	289	290	291	292	293	294	295	296	297	298	299	300	301	302	303	304	305	306	307	308	309	310	311	312	313	314	315	316	317	318	319	320	321	322	323	324	325	326	327	328	329	330	331	332	333	334	335	336	337	338	339	340	341	342	343	344	345	346	347	348	349	350	351	352	353	354	355	356	357	358	359	360	361	362	363	364	365	366	367	368	369	370	371	372	373	374	375	376	377	378	379	380	381	382	383	384	385	386	387	388	389	390	391	392	393	394	395	396	397	398	399	400	401	402	403	404	405	406	407	408	409	410	411	412	413	414	415	416	417	418	419	420	421	422	423	424	425	426	427	428	429	430	431	432	433	434	435	436	437	438	439	440	441	442	443	444	445	446	447	448	449	450	451	452	453	454	455	456	457	458	459	460	461	462	463	464	465	466
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TC Dead 7.0

BC Dead 10.0

TOTAL. 430

ERROR

TO
FROM

THE DE. 1

PTC DE: 1. 0

Code: ET A

7 32-58

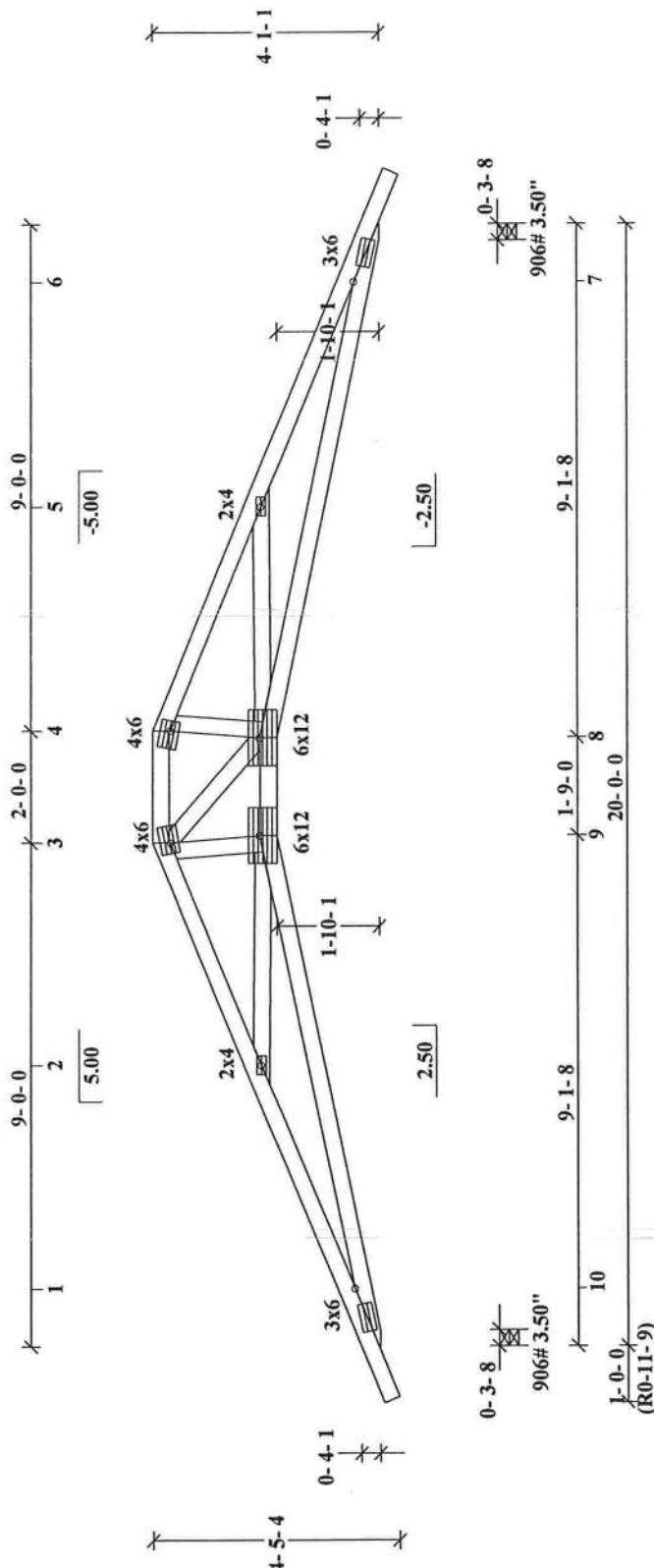
2017.05.14

Qty:1

```

MULTIPLE LOADCASES -- This design is the
composite result of multiple loadcases.
User defined loadcases have been considered
in this design.
In-Plant Quality Assurance with Cq= 1
-----MAX. REACTIONS PER BEARING LOCATION-----
X-Loc      Bset      Vert      Horiz      Uplift      Y      Type
10-10-4    1      906      137      -466      B      P
19-10-4    1      906      0      -466      B      R
PROVIDE UPLIFT CONNECTION PER SCHEDULE
---WEB...FORCE...CSI...CT...WEB...FORCE...CSI...
2-9      -464      0.17      C      8-4      750      0
3-9      750      0.24      C      8-5      -456      0
3-8      118      0.04      C

```

[illegible]

Scale = 0.2947

(B0-11-9)

Scale = 0.2128

DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accept no responsibility or excessive costs to control with regard to fabrication, handling, shipment and installation of trusses. This truss is designed in accordance with ANS/PTI-1995 and AISC-1989 and is to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the Building Designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind and snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by shalloot walls otherwise specified. Where a bottom chord in tension are not fully braced laterally by a portion of applied load, they should be braced laterally by a portion of applied load of 10 kN^2 or 100 lb^2 . The truss shall be manufactured from 20 gauge hot dipped galvanized steel, meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to verify that the fabricator is responsible for fabricating and erecting the members shall not be installed before cutting or fabricating. Plates shall not be installed over heptalones, knots or distorted members. Members shall be cut for tight fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 5/4 plate is 5" wide x 1/4" long. A 6/8 plate is 6" wide x 8" long. Stubs (holes) cut parallel to the plate length specified. Double cuts on members shall meet at the centroid of the web unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI-1-1995.

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during bracing and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary erection for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominoing. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until after all fastening and bracing is completed.

Analysis based on Simplified Analog Model.

ALL COMPRESSION Chords are assumed to be continuously braced unless noted otherwise. User defined loadcases have been considered in this design.

In-Plant Quality Assurance with $C_q = 1$

-----MAX. REACTIONS PER BEARING LOCATION-----

X-Loc	Bset	Vert	Horiz	Uplift	Y Type
0- 1-12	1	381	81	-486	pin

0-1-12	1	201	81	Pin
6-11-4	1	113	292	Pin

6-11-4	1	154	0	-184	B	H	Roll
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PROVIDE UPLIFT CONNECTION PER SCHEDULE

PROVIDE HORIZONTAL CONNECTION PER SCHEDULE

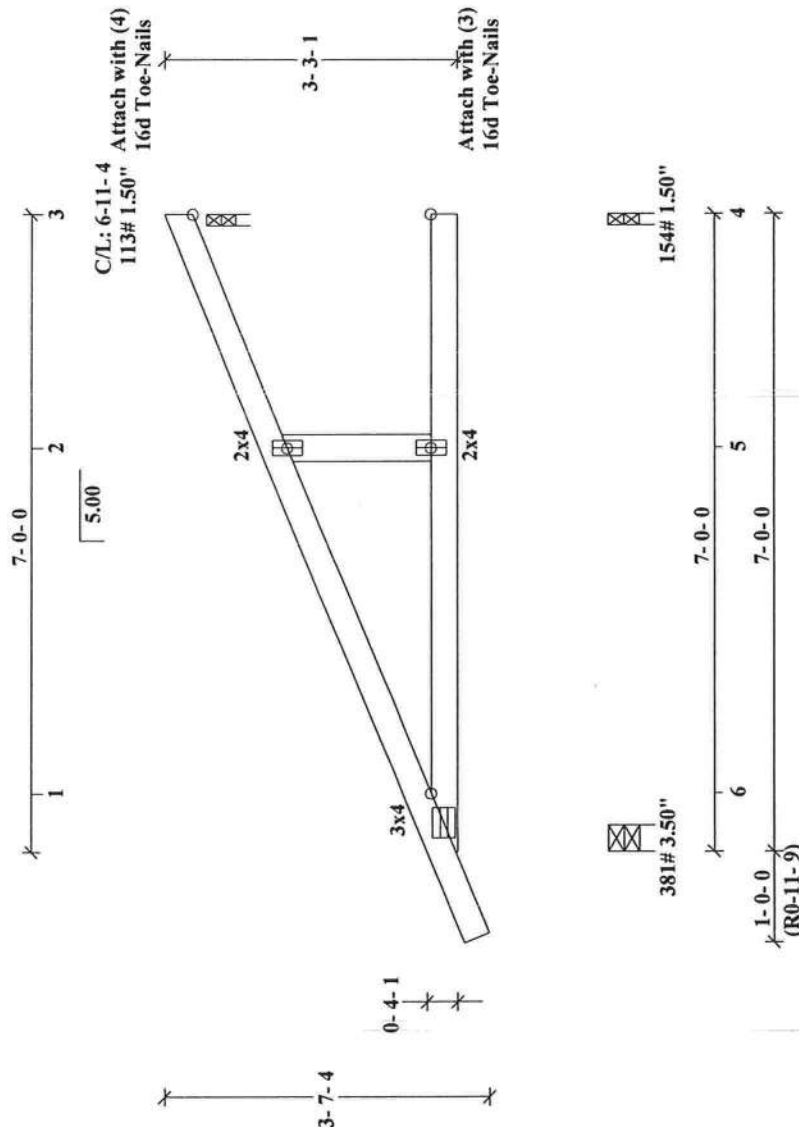
7-0-0

—

1122

5.00

— 333 —



EXCEPT AS SHOWN PLATES ARE MITEK MT20

Over 3 Supports Scale = 0.4705

Maronda Systems

4005 MARONDA WAY
Sanford, FL. 32771
(407) 321-0064 Fax (407) 321-3913
OMAS PONCE P.E. LICENSE #0050068
667 Medallion Pl. Chuluota, FL 32766

WARNING:
READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO
THE SELECTING CONTRACTOR. BRACING WARNING:
Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of
the structure. It is intended to provide temporary support for the structure during erection. Erection bracing must be
members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified
locations specified by the building designer. Additional bracing of the overall structure may be required. (See HIB-91
TPI). For specific truss bracing requirements, contact building designer (Truss Plate Institute, TPI is located at 383
O'Nofrio Drive, Madison, Wisconsin 53719).
Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd. Edenton, NC 27932

Eng Job:	
Dwg:	
Dsgnr:TL	
TC Live	
TC Dead	
BC Live	
BC Dead	
TOTAL	

WO: BBR54
TI: J
7/29/2006
Lbr DF: 1.
Plt DF: 1.
O.C.: 2-
TPI-02/FBO
Code: FLA
v4.7.32-58

Customer:10# UPLIFT D.L.

WO:BBRS4

TI:JLA

Qty:1

TC: 2x 4 SP #2

BC: 2x 4 SP #2

Unbalanced load case(s) have been checked.

[] or []: Plate(s) OFFSET from joint center.

The Joint Detail Report must be included with any submittal, inspection, and/or fabrication documentation.

User defined loadcases have been considered in this design.

This truss is designed in accordance with the 2004 Florida Building Code section 2306.1 and referenced standards, U.N.O.

Analysis based on Simplified Analog Model.

MULTIPLE LOADCASES -- This design is the composite result of multiple loadcases.

All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise.

WindLoad per ASCE 7-02, C6C, V= 125mph, W= 22.0 ft, I= 1.00, Exp.Cat. B, Kzt= 1.0

Bld Type= encl L= 5.0 ft W= 5.0 ft Truss

In END zone, TCCL= 4.2 psf, BCCL= 6.0 psf

Impact Resistant Covering and/or Glazing Req

-----GLOBAL MAX DEFLECTIONS-----

LL TL Jnt(s)

I. Span -0.04/999 -0.08/656 3-1

Horiz. 0.00 0.00 NA

Long term deflection factor = 1.50

1) 0- 7-12 3) 5- 0- 0

2) 5- 0- 0 4) 0- 7-12

This truss is designed to bear on multiple supports. Interior bearing locations should be marked on truss. Shim or wedge if necessary to achieve full bearing.

In-Plant Quality Assurance with Cq = 1.00

-----MAX. REACTIONS PER BEARING LOCATION-----

X-Loc BSet Vert Horiz Uplift Y Type

0- 1-12 1 215 165 -308 B Pin

4-11- 4 1 113 127 -282 T Pin

4-11- 4 1 103 0 -42 B H Roll

PROVIDE UPLIFT CONNECTION PER SCHEDULE

2

5-0-0

1

2

5-1

1

3

5-0-0

1

4

5-0-0

1

215# 3.50"

103# 1.50"

113# 1.50"

Attach with (3)

16d Toe-Nails

Attach with (2)

16d Toe-Nails

2

5-0-0

1

2

5-1

1

3

5-0-0

1

4

5-0-0

1

215# 3.50"

103# 1.50"

113# 1.50"

Attach with (3)

16d Toe-Nails

Attach with (2)

16d Toe-Nails

2

5-0-0

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2

5-1

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3

5-0-0

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4

5-0-0

1

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Attach with (2)

16d Toe-Nails

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5-0-0

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4

5-0-0

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4

5-0-0

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113# 1.50"

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Attach with (2)

16d Toe-Nails

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1

215# 3.50"

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113# 1.50"

Attach with (3)

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Attach with (2)

16d Toe-Nails

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4

5-0-0

1

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113# 1.50"

Attach with (3)

16d Toe-Nails

Attach with (2)

16d Toe-Nails

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113# 1.50"

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Attach with (2)

16d Toe-Nails

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215# 3.50"

103# 1.50"

113# 1.50"

Attach with (3)

16d Toe-Nails

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16d Toe-Nails

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215# 3.50"

103# 1.50"

113# 1.50"

Attach with (3)

16d Toe-Nails

Attach with (2)

16d Toe-Nails

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215# 3.50"

103# 1.50"

113# 1.50"

Attach with (3)

16d Toe-Nails

Attach with (2)

16d Toe-Nails

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215# 3.50"

103# 1.50"

113# 1.50"

Attach with (3)

16d Toe-Nails

Attach with (2)

16d Toe-Nails

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215# 3.50"

103# 1.50"

113# 1.50"

Attach with (3)

16d Toe-Nails

Attach with (2)

16d Toe-Nails

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215# 3.50"

103# 1.50"

113# 1.50"

Attach with (3)

16d Toe-Nails

Attach with (2)

16d Toe-Nails

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215# 3.50"

103# 1.50"

113# 1.50"

Attach with (3)

16d Toe-Nails

Attach with (2)

16d Toe-Nails

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215# 3.50"

103# 1.50"

113# 1.50"

Attach with (3)

16d Toe-Nails

Attach with (2)

16d Toe-Nails

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215# 3.50"

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113# 1.50"

Attach with (3)

16d Toe-Nails

Attach with (2)

16d Toe-Nails

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113# 1.50"

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Attach with (2)

16d Toe-Nails

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215# 3.50"

103# 1.50"

113# 1.50"

Attach with (3)

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Attach with (2)

16d Toe-Nails

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215# 3.50"

103# 1.50"

113# 1.50"

Attach with (3)

16d Toe-Nails

Attach with (2)

16d Toe-Nails

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215# 3.50"

103# 1.50"

113# 1.50"

Attach with (3)

16d Toe-Nails

Attach with (2)

16d Toe-Nails

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215# 3.50"

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113# 1.50"

Attach with (3)

16d Toe-Nails

Attach with (2)

16d Toe-Nails

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16d Toe-Nails

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113# 1.50"

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Attach with (2)

16d Toe-Nails

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5-0-0

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215# 3.50"

103# 1.50"

113# 1.50"

Attach with (3)

16d Toe-Nails

Attach with (2)

16d Toe-Nails

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103# 1.50

This design is for an individual building component and is based on information provided by the client. The building designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the trust designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accept no responsibility for any errors or omissions with regard to design, shipment and installation of building accessories. This trust has been created for individual building components in accordance with ANSI/P1-1995 and P1-2001 and is to be incorporated as part of the building design by a building designer. When reviewed for approval by the building designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind or snow loads, project specifications or special supplied loads. Unless shown, trust has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced against slenderness effects. Where a property agency in tension are not fully braced laterally by a permanent application of 100% c/c. Connector joints shall be manufactured from 20 gauge hot dipped galvanized steel in accordance with ASTM A 653, grade 40, unless otherwise shown.

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knuckles, knolls or distorted areas. Members shall be cut for tight fitting weld to wood bearing. Connector plates shall be located on both faces of the truss with nail fully imbedded and shall be sym. about the joint unless otherwise shown. A 5/16" plate is 5" wide x 1' long. A 6/16" plate is 6" wide x 8" long. Double (nails) on parallel to the plate length specified. Double cut web members shall meet at the centroid of the web unless otherwise shown. Connector plate sizes are minimum increases based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995.

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during banding and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary erection for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until after all bracing and bracing is completed.

TC: 2x 4 SP #2 C&C, V= 125mph,
 [C] or [{}]: Plate(s) OFFSET from joint center. WndLoad per ASCE 7-02, BldType= encl I= 40.0 ft W= 3.0 ft Truss
 [C] or [{}]: The Joint Detail Report must be included in END zone, TCIDL= 0.0 psf, BCIDL= 0.0 psf
 with any submittal, inspection, and/or Impact Resistant Covering and/or Glazing Reg
 fabrication documentation.

Analysis based on Simplified Analog Model.
MULTIPLE LOADCASES -- This design is the
composite result of multiple loadcases.
All COMPRESSION Chords are assumed to be
continuously braced unless noted otherwise.

This truss is designed in accordance with
the 2004 Florida Building Code section
2306.1 and referenced standards, U.N.O.
In-Plant Quality Assurance with Cg = 1.00
-----GLOBAL MAX DEFLECTIONS-----

	LL		TL	
	in./Ratio	in./Ratio	in./Ratio	Jnt(s)
I.I.Span	-0.01/999	-0.01/999	2-1	
Horiz.	0.00	0.00	NA	
Long term deflection factor				150

Unbalanced load case(s) have been checked.

Joint Locations					
1)	0-	7-13	3)	3-	0-0
2)	3-	0-0	4)	0-	7-13

This truss is designed to bear on multiple supports. Interior bearing locations should be marked on truss. Shim or wedge if necessary to achieve full bearing.

-----MAX. REACTIONS PER BEARING LOCATION-----

X-Loc	BSet	Vert	Horiz	Uplift	Y	Type
0-1-12	1	129	99	-216	B	Pi
2-11-4	1	68	77	-183	T	Pi
2-11-4	1	62	0	-42	B	H Rol

PROVIDE UPLIFT CONNECTION PER SCHEDULE

Unbalanced load case(s) have been checked.

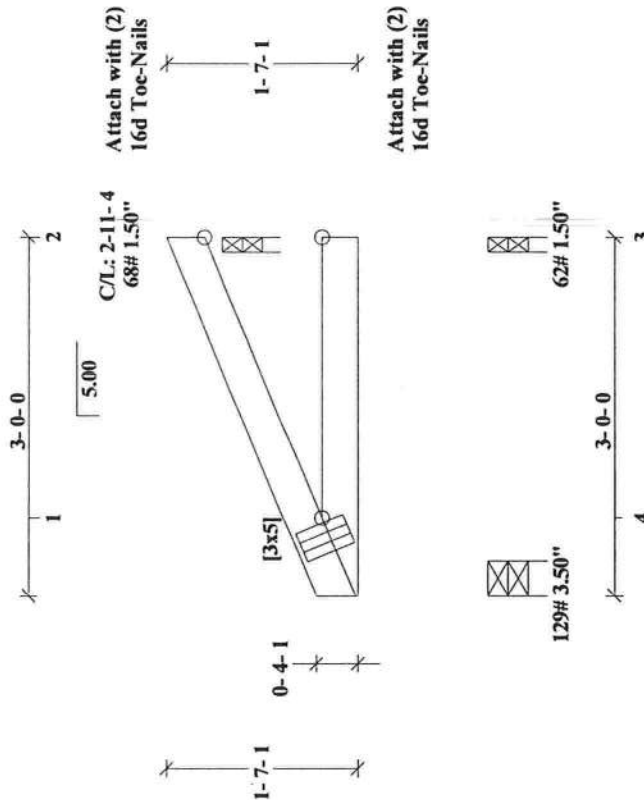
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2-11-4	1	62	0	-42	B	H Rol

PROVIDE UPLIFT CONNECTION PER SCHEDULE



Over 3 Supports Scale = 0.6190

WARNING:
READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO THE SELECTING CONTRACTOR. BRACING WARNING:
Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HB-91 for details.)
For more information, contact building designer (Truss Plate Institute, TPI is located at 583 O'Connell Drive, Madison, Wisconsin 53716).
Competent Engineer/bracing by: Truss Engineering Co., P.O. 818, Saukville, Rd. Eldorado, NC 27631.

Eng Job:		
Dwg:		
Dsgnr:	TLY	Chk:
TC Live	16.0	
TC Dead	7.0	
BC Live	10.0	
BC Dead	10.0	
TOTAL:	43.0	

Profile Path: T:\TEE-IOK\Work\Jobs\BARS4S\BARS4S\J2A.prx

DESIGN INFORMATION

This design is for an individual building component and has been based on information furnished to the designer. The designer assumes no responsibility for design or construction of the building or its components, or for the safety of the building or its occupants, or for the accuracy of the information furnished to the designer. The designer assumes no responsibility for the design or construction of the building or its components, or for the safety of the building or its occupants, or for the accuracy of the information furnished to the designer.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knots, knots or distorted grain. Members shall be cut for tight fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nailing fully imbedded and shall be sym. about the joint unless otherwise shown. A 3x4 plate is 5" wide x 4" long. A 6x8 plate is 6" wide x 8" long. Slots (holes) run parallel to the plate length specified. Double cuts on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANS/TPI 1-1995

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during banding and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until after all fastening and bracing is completed

NAME: RAYBURY S 4

Customer: 10# UPLIFT D.L.

WO: BBR54

TT: J3

Qty: 5

TC: 2x 4 SP #2

BC: 2x 4 SP #2

Unbalanced load case(s) have been checked.

[] or []: Plate(s) OFFSET from joint center.

The Joint Detail Report must be included with any submittal, inspection, and/or fabrication documentation.

User defined loadcases have been considered in this design.

This truss is designed in accordance with the 2004 Florida Building Code section 2306.1 and referenced standards, U.N.O.

Analysis based on Simplified Analog Model.

MULTIPLE LOADCASES -- This design is the composite result of multiple loadcases.

All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise.

WindLoad per ASCE 7-02, C6C, V= 125mph, B= 22.0 ft, I= 1.00, Exp.Cat. B, Kzt= 1.0

Bld Type= encl L= 40.0 ft W= 1.0 ft Truss in END zone, TCDF= 4.2 psf, BCDF= 6.0 psf

Impact Resistant Covering and/or Glazing Req

-----GLOBAL MAX DEFLECTIONS-----

LL TL in./Ratio in./Ratio Jnt(s)

I. Span 0.00/999 0.00/999 2-1

Horiz. 0.00 0.00 NA

Long term deflection factor = 1.50

Joint Locations

1) 0- 7-12 3) 1- 0- 0

2) 1- 0- 0 4) 0- 7-12

This truss is designed to bear on multiple supports. Interior bearing locations should be marked on truss. Shim or wedge if necessary to achieve full bearing.

In-Plant Quality Assurance with Cq = 1.00

-----MAX. REACTIONS PER BEARING LOCATION-----

X-Loc BSet Vert Horiz Uplift Y Type

0- 1-12 1 89 33 -211 B Pin

0-11- 4 1 23 27 -59 T Pin

0-11- 4 1 20 0 -6 B H Roll

PROVIDE UPLIFT CONNECTION PER SCHEDULE

Attach with (2)

16d Toe-Nails

0- 9- 1

Attach with (2)

16d Toe-Nails

1- 0- 0

1 2

5.00

C/L: 0-11-4

23# 1.50"

3x5

0- 4- 1

1- 1- 4

89# 3.50"

20# 1.50"

1- 0- 0

4 3

1- 0- 0

1- 0- 0

(R0-11-9)

Over 3 Supports

Scale = 0.6788

Eng Job:

Dwg:

Dsgnr: TLY

Chk:

TC Live 16.0 psf

TC Dead 7.0 psf

BC Live 10.0 psf

BC Dead 10.0 psf

TOTAL 43.0 psf

WO: BBR54

TI: J3

8/28/2007

Lbr DF: 1.25

Plt DF: 1.25

O.C.: 2- 0- 0

FBC-04/TPI-02/NDS-01

Code: FLA

v5.5.6-9851

WARNING:

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Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HDS 9.1 and TPI) For specific truss bracing requirements, contact building designer (Truss Plate Institute, TPI is located at 581 D'Ondra Drive, Madison, Wisconsin 53719).

Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd, Edenon, NC 27932

EXCEPT AS SHOWN PLATES ARE MiTek MT20

Maronda Systems

4005 MARONDA WAY

Sanford, FL 32771

(407) 321-0064 Fax (407) 321-3913

TOMAS PONCE P.E. LICENSE #0050068

387 Medallion Pl, Chuluota, FL 32766

Desirm. Matrix Analysis

Profile Path: P:\WORK\TAP\Work\BR54S\BR54S\J3.DWG

DESIGN INFORMATION

This design is for an individual building component and this has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accept no responsibility for the effects of such information on the fabrication, handling, shipment and installation of trusses. This truss has been designed in accordance with BS5426:1993 and BS5951:1995 and has been designed in accordance with ANS/PRI-1:1995 and ENDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by a Building Designer, the design loadings shown must be checked to be sure that the data shown is in agreement with the local building codes, local climatic records for wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rafter, they should be braced laterally by a properly applied rafter. (10-47) (10-48) (10-49) (10-50) (10-51) (10-52) (10-53) (10-54) (10-55) (10-56) (10-57) (10-58) (10-59) (10-60) (10-61) (10-62) (10-63) (10-64) (10-65) (10-66) (10-67) (10-68) (10-69) (10-70) (10-71) (10-72) (10-73) (10-74) (10-75) (10-76) (10-77) (10-78) (10-79) (10-80) (10-81) (10-82) (10-83) (10-84) (10-85) (10-86) (10-87) (10-88) (10-89) (10-90) (10-91) (10-92) (10-93) (10-94) (10-95) (10-96) (10-97) (10-98) (10-99) (10-100) (10-101) (10-102) (10-103) (10-104) (10-105) (10-106) (10-107) (10-108) (10-109) (10-110) (10-111) (10-112) (10-113) (10-114) (10-115) (10-116) (10-117) (10-118) (10-119) (10-120) (10-121) (10-122) (10-123) (10-124) (10-125) (10-126) (10-127) (10-128) (10-129) (10-130) (10-131) (10-132) (10-133) (10-134) (10-135) (10-136) (10-137) (10-138) (10-139) (10-140) (10-141) (10-142) (10-143) (10-144) (10-145) (10-146) (10-147) (10-148) (10-149) (10-150) (10-151) (10-152) (10-153) (10-154) (10-155) (10-156) (10-157) (10-158) (10-159) (10-160) (10-161) (10-162) (10-163) (10-164) (10-165) (10-166) (10-167) (10-168) (10-169) (10-170) (10-171) (10-172) (10-173) (10-174) (10-175) (10-176) (10-177) (10-178) (10-179) (10-180) (10-181) (10-182) (10-183) (10-184) (10-185) (10-186) (10-187) (10-188) (10-189) (10-190) (10-191) (10-192) (10-193) (10-194) (10-195) (10-196) (10-197) (10-198) (10-199) (10-200) (10-201) (10-202) (10-203) (10-204) (10-205) (10-206) (10-207) (10-208) (10-209) (10-210) (10-211) (10-212) (10-213) (10-214) (10-215) (10-216) (10-217) (10-218) (10-219) (10-220) (10-221) (10-222) (10-223) (10-224) (10-225) (10-226) (10-227) (10-228) (10-229) (10-230) (10-231) (10-232) (10-233) (10-234) (10-235) (10-236) (10-237) (10-238) (10-239) (10-240) (10-241) (10-242) (10-243) (10-244) (10-245) (10-246) (10-247) (10-248) (10-249) (10-250) (10-251) (10-252) (10-253) (10-254) (10-255) (10-256) (10-257) (10-258) (10-259) (10-260) (10-261) (10-262) (10-263) (10-264) (10-265) (10-266) (10-267) (10-268) (10-269) (10-270) (10-271) (10-272) (10-273) (10-274) (10-275) (10-276) (10-277) (10-278) (10-279) (10-280) (10-281) (10-282) (10-283) (10-284) (10-285) (10-286) (10-287) (10-288) (10-289) (10-290) (10-291) (10-292) (10-293) (10-294) (10-295) (10-296) (10-297) (10-298) (10-299) (10-300) (10-301) (10-302) (10-303) (10-304) (10-305) (10-306) (10-307) (10-308) (10-309) (10-310) (10-311) (10-312) (10-313) (10-314) (10-315) (10-316) (10-317) (10-318) (10-319) (10-320) (10-321) (10-322) (10-323) (10-324) (10-325) (10-326) (10-327) (10-328) (10-329) (10-330) (10-331) (10-332) (10-333) (10-334) (10-335) (10-336) (10-337) (10-338) (10-339) (10-340) (10-341) (10-342) (10-343) (10-344) (10-345) (10-346) (10-347) (10-348) (10-349) (10-350) (10-351) (10-352) (10-353) (10-354) (10-355) (10-356) (10-357) (10-358) (10-359) (10-360) (10-361) (10-362) (10-363) (10-364) (10-365) (10-366) (10-367) (10-368) (10-369) (10-370) (10-371) (10-372) (10-373) (10-374) (10-375) (10-376) (10-377) (10-378) (10-379) (10-380) (10-381) (10-382) (10-383) (10-384) (10-385) (10-386) (10-387) (10-388) (10-389) (10-390) (10-391) (10-392) (10-393) (10-394) (10-395) (10-396) (10-397) (10-398) (10-399) (10-400) (10-401) (10-402) (10-403) (10-404) (10-405) (10-406) (10-407) (10-408) (10-409) (10-410) (10-411) (10-412) (10-413) (10-414) (10-415) (10-416) (10-417) (10-418) (10-419) (10-420) (10-421) (10-422) (10-423) (10-424) (10-425) (10-426) (10-427) (10-428) (10-429) (10-430) (10-431) (10-432) (10-433) (10-434) (10-435) (10-436) (10-437) (10-438) (10-439) (10-440) (10-441) (10-442) (10-443) (10-444) (10-445) (10-446) (10-447) (10-448) (10-449) (10-450) (10-451) (10-452) (10-453) (10-454) (10-455) (10-456) (10-457) (10-458) (10-459) (10-460) (10-461) (10-462) (10-463) (10-464) (10-465) (10-466) (10-467) (10-468) (10-469) (10-470) (10-471) (10-472) (10-473) (10-474) (10-475) (10-476) (10-477) (10-478) (10-479) (10-480) (10-481) (10-482) (10-483) (10-484) (10-485) (10-486) (10-487) (10-488) (10-489) (10-490) (10-491) (10-492) (10-493) (10-494) (10-495) (10-496) (10-497) (10-498) (10-499) (10-500) (10-501) (10-502) (10-503) (10-504) (10-505) (10-506) (10-507) (10-508) (10-509) (10-510) (10-511) (10-512) (10-513) (10-514) (10-515) (10-516) (10-517) (10-518) (10-519) (10-520) (10-521) (10-522) (10-523) (10-524) (10-525) (10-526) (10-527) (10-528) (10-529) (10-530)

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knuckles, knots or distorted areas. Members shall be cut for tight fitting, wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 5/4 plate is 5" wide x 4" long. A 6/8 plate is 6" wide x 8" long. Double cuts on nail parallel to the plate length specified. Slost cuts on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with the restraint treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995.

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during banding and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary erection trusses requires such temporary bracing during installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until after all fastening and bracing is completed.

TC: 2x 4 SP #2
BC: 2x 4 SP #2
WB: 2x 4 SP #3

This truss is designed to bear on multiple supports. Interior bearing locations should be marked on truss. Shim or wedge if necessary to achieve full bearing.

..TC..	FORCE..CSI..CF..	BC..	FORCE..CSI..
1-2	-128	A 1-5	-9 0.81
2-3	191	A 5-4	0 0.81

Analysis based on Simplified Analog Model.
All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise.
User defined loadcases have been considered in this design.

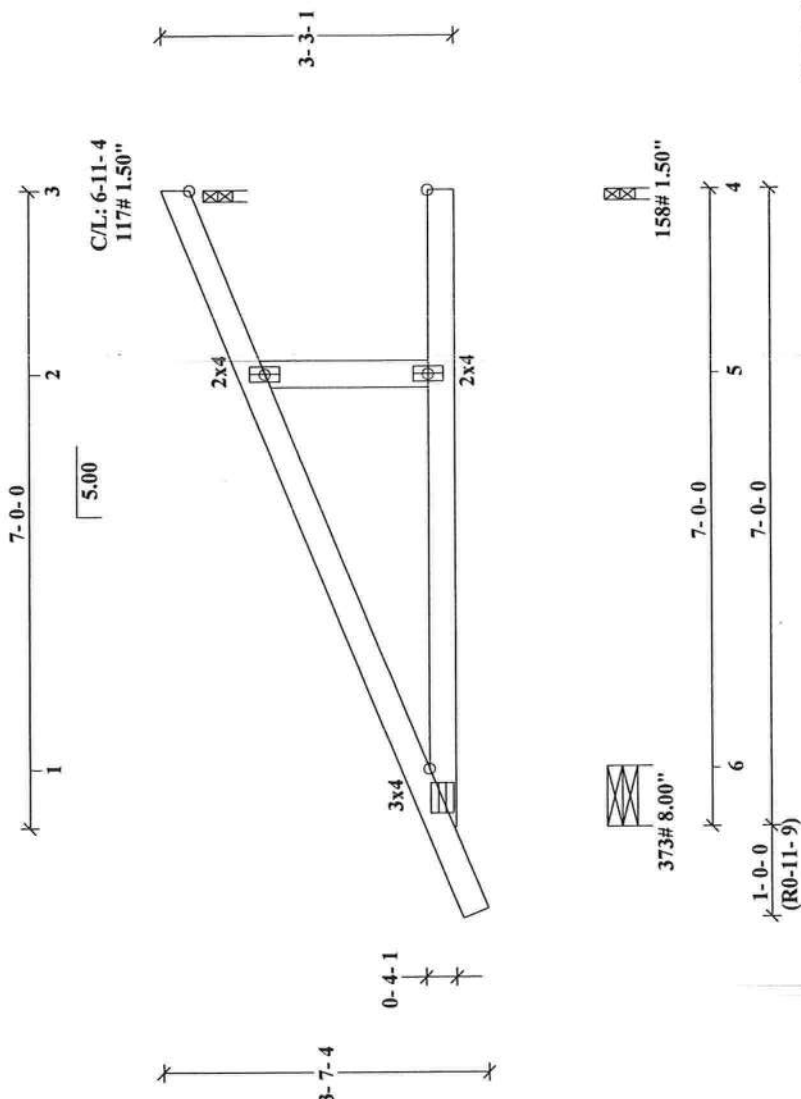
In-Plant Quality Assurance with Cq= 1			
-----MAX. REACTIONS PER BEARING LOCATION-----			
X-Loc	BSet	Vert	Horiz Uplift Y
0- 4- 0	1	373	93 -473 B

PROVIDE UPLIFT CONNECTION PER SCHEDULE				H	R	P
				B	T	
6-11-	4	1	117	262	-132	
6-11-	4	1	158	0	-223	

Joint Locations

1)	0- 7-12	3)	7- 0- 0	5)	4-11-14
2)	4-11-14	4)	7- 0- 0	6)	0- 7-12

C&C, V= 125mph,
WindLoad per ASCE 7-02,
H= 22.0 ft, I=1.00, Exp.Cat. B, Kzt= 1.0



EXCEPT AS SHOWN PLATES ARE MITek MT20

Over 3 Supports Scale = 0.4705

WARNING:

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO THE ERECTING CONTRACTOR. BRACING WARNING: Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and supports. (See AISC 360, Part 10, and AISC 360, Part 11 for specific truss bracing requirements. Contact building designer (Truss Plate Institute, TPI) is located at 583 D'Oroville Drive, Madison, Wisconsin 53719).

Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd, Edenburg, NC 27932

Eng Job:

Dwg:

Dsgnr: TLY Chk:

ATC Live	16.0	psf
ATC Dead	7.0	psf
STC Live	10.0	psf
STC Dead	10.0	psf
TOTAL	43.0	psf

WO: BBR54

FI: J4

7/29/2006

Lbr DF: 1.25
 Plt DF: 1.25
 O.C.: 2- 0- 0
 TPI-02/FBC-04
 Code: FLA
 v4.7.32-58656

Maronda Systems

4005 MARONDA WAY
Sanford, FL 32771
(407) 321-0064 Fax (407) 321-3913
OMAS PONCE P.E. LICENSE #0050066
167 Medallion Pl. Chuluota, FL 32766

Design: Matrix Analysis

Profile Path: C:\TEE-LOK\Work\Jobs\BRS4S\BAYBUR~1\J4.PRX

DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accept no responsibility or exercises no control with regard to fabrication, handling, shipping, shipment and installation of trusses. This truss has been designed as an individual building component in accordance with ANSI/TPI 1-1995 and NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the building designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knotholes, knots or distorted grain. Members shall be cut for tight fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 3x4 plate is 5" wide x 4" long. A 6x8 plate is 6" wide x 8" long. Slots (holes) run parallel to the plate length specified. Double cuts on web members shall meet at the centroid of the web unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during handling and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the erection of trusses. Erection stresses shall be sought if needed. Connections of construction loads greater than the design loads shall not be applied to trusses after they are erected. No loads other than the weight of the erection shall be applied to trusses until after all fastening and bracing is completed.

Analysis based on Simplified Analog Model.

MULTIPLE LOADCASES -- This design is the composite result of multiple loadcases. All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise. WindLoad per ASCE 7-02, C6C, V= 125mph, H= 22.0 ft, I= 1.00, Exp.Cat. B, Kzt= 1.0 Bid type= encl L= 40.0 ft W= 5.0 ft Truss in END zone, TCDF= 4.2 psf, BCDF= 6.0 psf -----GLOBAL MAX DEFLECTIONS-----

LL	TL	in./Ratio	in./Ratio	Jnt(s)
I. Span	-0.04/999	-0.08/605	3-1	
Horiz.	0.00	0.00	NA	

Long term deflection factor = 1.50

1) 0- 7-12 3) 5- 0- 0
2) 5- 0- 0 4) 0- 7-12

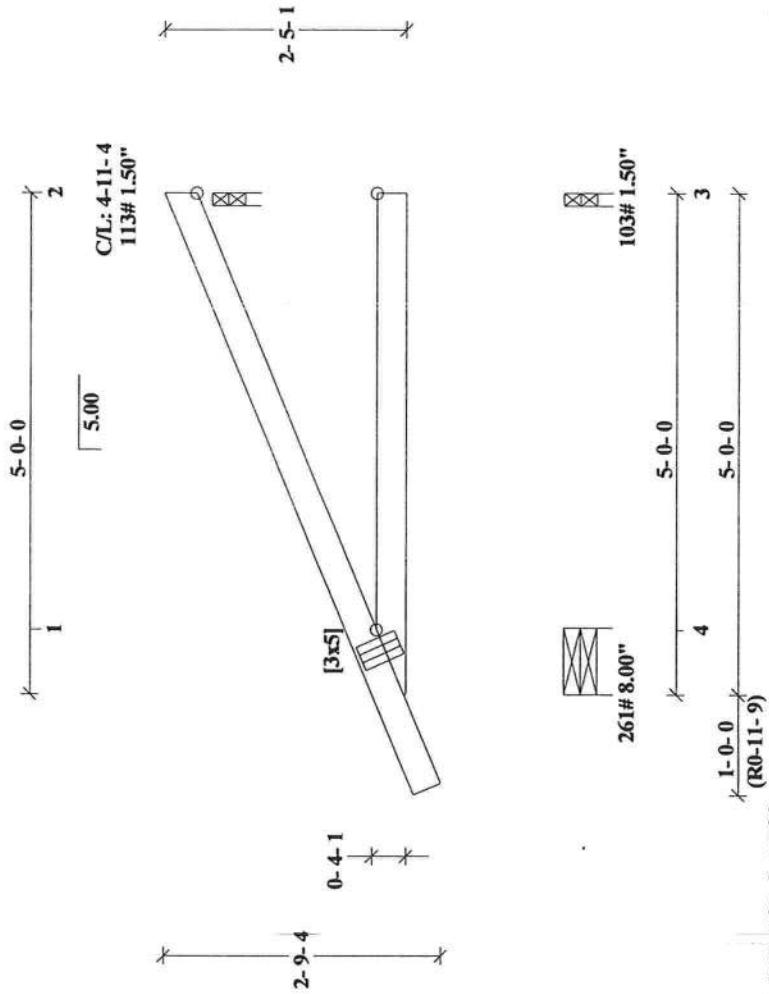
This truss is designed to bear on multiple supports. Interior bearing locations should be marked on truss. Shim or wedge if necessary to achieve full bearing.

In-Plant Quality Assurance with Cq = 1.00
-----MAX. REACTIONS PER BEARING LOCATION-----

X-Loc	BSet	Vert	Horiz	Uplift	Type
0- 4- 0	1	261	161	-397	B
4-11- 4	1	113	124	-274	T
4-11- 4	1	103	0	-41	B

Pin
Pin
H Roll

PROVIDE UPLIFT CONNECTION PER SCHEDULE



Maronda Systems

4005 MARONDA WAY
Sanford, FL 32771
(407) 321-0064 Fax (407) 321-3913
TOMAS PONCE P.E. LICENSE #0050068
367 Medallion Pl. Chuluota, FL 32766

Design: Matrix Analysis

WARNING:

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING:

Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified intermediate points. Erection bracing required for erection of truss shall be designed by others. (See HIB-91 and TPI 1-1995 for details.) For more information, contact building designer (Truss Plate Institute, TPI is located at 381 P.O. Box 100, Madison, WI 53719).

Component Engineering by: Truss Engineering Co., P.A., 118 Soundside Rd., Edenton, NC 27932

Eng Job:
Dwg:
Dsgnr: TLY
Chk:

WO: BBR54
TI: J5
8/28/2007

TC Live	16.0	psf
TC Dead	7.0	psf
BC Live	10.0	psf
BC Dead	10.0	psf
TOTAL	43.0	psf

EXCEPT AS SHOWN PLATES ARE MITEK MT20

Over 3 Supports Scale = 0.5171

TC: 2x 4 SP #2

BC: 2x 4 SP #2

DESIGN INFORMATION

This design is for an individual building component and is based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accept no responsibility of errors or omissions with regard to the design of the truss, which is to be used in the building of the truss. This truss has been designed in accordance with the component in accordance with ANSI/TPI 1-1995 and NDS-47 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the building designer, the design shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies shall be put in writing before cutting or fabrication. Plates shall not be installed over knotholes, knots or distorted grain. Members shall be cut for tight fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 5x4 plate is 5" wide x 4" long. A 6x8 plate is 6" wide x 8" long. Slots (holes) run parallel to the plate length specified. Double cuts on web members shall meet at the centroid of the web unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during handling and bundling. Delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominoing. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until after all fastening and bracing is completed

Customer: 10# UPLIFT D.L.

WO: BBSR4

TI: J6

Qty: 2

Analysis based on Simplified Analog Model. MULTIPLE LOADCASES -- This design is the composite result of multiple loadcases. ALL COMPRESSION Chords are assumed to be continuously braced unless noted otherwise. WindLoad per ASCE 7-02, C6C, V= 125mph, H= 22.0 ft, I= 1.00, Exp.Cat. B, Kzt= 1.0 Bid type= encl I= 40.0 ft W= 3.0 ft Truss in END zone, TCDF= 4.2 psf, BCDF= 6.0 psf -----GLOBAL MAX DEFLECTIONS-----

LL

in./Ratio

in./Ratio

Jnt(s)

I. Span

-0.01/999

-0.01/999

2-1

Horiz.

0.00

0.00

NA

Long term deflection factor = 1.50

1) 0- 7-12

2) 3- 0- 0

3) 3- 0- 0

4) 0- 7-12

This truss is designed to bear on multiple supports. Interior bearing locations should be marked on truss. Shim or wedge if necessary to achieve full bearing.

In-Plant Quality Assurance with Cq = 1.00

---MAX. REACTIONS PER BEARING LOCATION---

X-Loc

BSet

Vert

Horiz

Uplift

Y

Type

0- 4- 0

1

175

99

-282

B

Pin

2-11- 4

1

68

77

-170

T

Pin

2-11- 4

1

62

0

-24

B

H Roll

PROVIDE UPLIFT CONNECTION PER SCHEDULE

1

2

3

4

3-0-0

5.00

2-11-4

68# 1.50"

1-1-4

0-4-1

175# 8.00"

62# 1.50"

1-0-0

3-0-0

3-0-0

(R0-11-9)

Eng Job:

Dwg:

Dsgnr: TLY

Chk:

TC Live

16.0

psf

TC Dead

7.0

psf

BC Live

10.0

psf

BC Dead

10.0

psf

TOTAL

43.0

psf

WO: BBSR4

TI: J6

8/28/2007

Lbr DF: 1.25

Plt DF: 1.25

O.C.: 2- 0- 0

FBC-04/TPI-02/NDS-01

Code: FLA

v5.5.6-9854

EXCEPT AS SHOWN PLATES ARE MITEK MT20

Over 3 Supports Scale = 0.5747

WARNING:

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING:

Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HIB-91 of TPI) For specific truss bracing requirements, consult building designer (Truss Plate Institute, TPI is located at 581 D'Onofrio Drive, Madison, Wisconsin 53719).

Component Engineering by: Truss Engineering Co., P.A., 819 Soundside Rd. Edenton, NC 27932

Maronda Systems

4005 MARONDA WAY

Sanford, FL 32271

(407) 321-0064 Fax (407) 321-3913

TOMAS PONCE P.E. LICENSE #0050068

367 Medallion Pl. Chuluota, FL 32766

Design: Matrix Analysis

Profile Path: T:\PFR-10K\Work\10k\BBSR4S\BBSR4S\1.76 PRX

DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the trust designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accepts no responsibility or exercises no control with regard to fabrication, handling, shipment and installation of building materials. This trust has been designed in accordance with the provisions of the NSR/PD 1985 and the NDS-917 to be incorporated as part of the building design by a building designer. When reviewed for approval by the building designer, the design loadings shown must be checked to be sure that the data shown is in agreement with the local building codes, local climatic records for wind or snow loads, project specifications or special applied loads. Unless shown, trust has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid tie, they should be braced at a maximum spacing of 10' (3048 mm) (10'0").

Manufactured from 230 grade, box dipped polyvinyl sheet membrane ASTM A 633, grade 40, unless otherwise shown.

FABRICATION NOTES

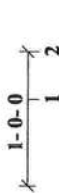
Prior to fabrication, the fabricator shall review this drawing fully to ensure that all work is in conformance with the manufacturer's specifications and tolerances. The fabricator must verify that all dimensions are acceptable for each verification. Any discrepancies are to be put in writing before cutting or fabricating. Plates shall not be installed over bolts, fasteners, rivets, or other hardware. Members shall be cut for tight fitting and smooth bearing. Connector plates shall be located both faces of the member with nail, fully embedded and shall be in firm contact with the joint unless otherwise shown. A 5x4 plate shall be used. A 6x8 plate is 6" wide, 8" long. A 6x8 plate is 6" wide, 8" long. (Note: Holes may be drilled to the plate length specified.) Double end web members shall meet at the centroid of the web; unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This must be noted by the contractor. All steel reinforcement is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANS/TPP-1095.

PRECAUTIONARY NOTES

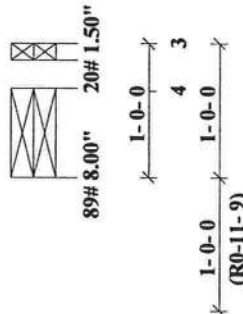
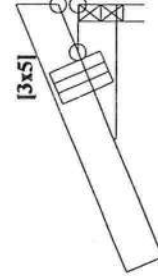
All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during bundling and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary erection bracing for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until after all fastening and bracing is completed.

Analysis based on Simplified Analog Model.
MULTIPLE LOADCASES -- This design is the
composite result of multiple loadcases.
All COMPRESSION Chords are assumed to be
continuously braced unless noted otherwise
Windload per ASCE 7-02.
C5C. V= 12.5mph

H= 22.0 ft, I= 1.00, Exp.Cat. B, Kzt= 1.0
Bld Type= encl L= 40.0 ft W= 1.0 ft Truss
in END zone, TCdL= 4.2 psf, BCdL= 6.0 psf
-----GLOBAL MAX DEFLECTIONS-----



3.00
C/L: 0-11-4
23# 1.50"



0-9-1

```
TC: 2x 4 SP #2
BC: 2x 4 SP #2
[[ or {}] Plate(s) OFFSET from joint center.
The Joint Detail Report must be included
with any submittal, inspection, and/or
fabrication documentation.
User defined loadcases have been considered
in this design.
```

This truss is designed in accordance with the 2004 Florida Building Code section 2306.1 and referenced standards, U.N.O.

```

=====Joint Locations=====
1) 0- 7-12 3) 1- 0- 0
2) 1- 0- 0 4) 0- 7-12

This truss is designed to bear on multiple
supports. Interior bearing locations should
be marked on truss. Shim or wedge if
necessary to achieve full bearing.

In-Plant Quality Assurance with Cq = 1.00

-----MAX. REACTIONS PER BEARING LOCATION-----
X-Loc      BSet Vert Horiz Uplift Y Type
0- 4- 0    1 89 33 -158 B Pin
0-11- 4    1 23 27 -59 T Pin
0-11- 4    1 20 0 -6 B H Roll

PROVIDE UPLIFT CONNECTION PER SCHEDULE

```

EXCEPT AS SHOWN PLATES ARE MITAK MT20

Over 3 Simports Scale = 0.6030

Maronda Systems

4005 MARONDA WAY
Sanford, FL 32771
(407) 321-0064 Fax (407) 321-3913
TOMAS PONCE P.E. LICENSE #0050068
367 Medallion Pl. Chuluota, FL 32766

WARNING:
**READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO
ERECTING CONTRACTOR. BRACING WARNING:**
Bracing shown on this drawing is not erection bracing, weld bracing, portal bracing or similar bracing which is a part of the building design and must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HDB-91 for bracing requirements.) For more information, contact building designer (Truss Plate Institute, TPI is located at 583 D'Oroville Drive, Madison, WI 53719).
Component Fabricator by: Truss Engineering Co., P.A. 818 Sunnyside Blvd. Edinboro, NC 27929.

Eng Job:

Dwa:

Decm r: 11 v chb:

WO: BBR54

TT: 17

9/28/2007

Lbr DF: 1.25

Plt DF: 1.25

O.C.: 2-0-0

Code: FT.A

v5.5.6-985

1

Design: Matrix Analysis

Profile Path: T:\TEE-LOK\Work\Jobs\BHR4S\BHR4S\J7.PRX

DESIGN INFORMATION

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or forming. Plates shall be installed over toeboards, knots or distorted members. Connector plates shall be located on both faces of the joint unless otherwise shown. A 5/4 plate is 5" wide x 4" long. A 6/8 plate is 6" wide x 8" long. Double cut x web members shall meet at the center of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSII/TPI-1 information.

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during banding and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight shaft and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominoing. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Contamination of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until after all fastening and bracing is completed.

Maronda Systems

Design: Matrix Analysis

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO THE ERECTING CONTRACTOR. BRACING WARNING:

Bracing shown on this drawing is not considered by the building designer, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be erected by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified dimensions determined by the building designer. Additional bracing of the overall structure may be required. (See HB-91 for TPI) For specific truss bracing requirements, contact Building Research Institute, 1731 N. Lincoln Ave., Chicago, IL 60614. (See HB-91 for TPI) For specific truss bracing requirements, contact Building Research Institute, 1731 N. Lincoln Ave., Chicago, IL 60614. (See HB-91 for TPI)

Component Erecting Inc., P. A., 818 Soundside Rd., Edenton, NC 27932

Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd. Edenton, NC 27932

Profile Path: C:\TEE-LOK\Work\Jobs\BBSRS4S\BAYBUR~1\JS1.prx

Dwg: Dsqnr: TLY Chk:

UTC Live	16.0	psf
UTC Dead	7.0	psf
SC Live	10.0	psf
SC Dead	10.0	psf
TOTAL	43.0	psf

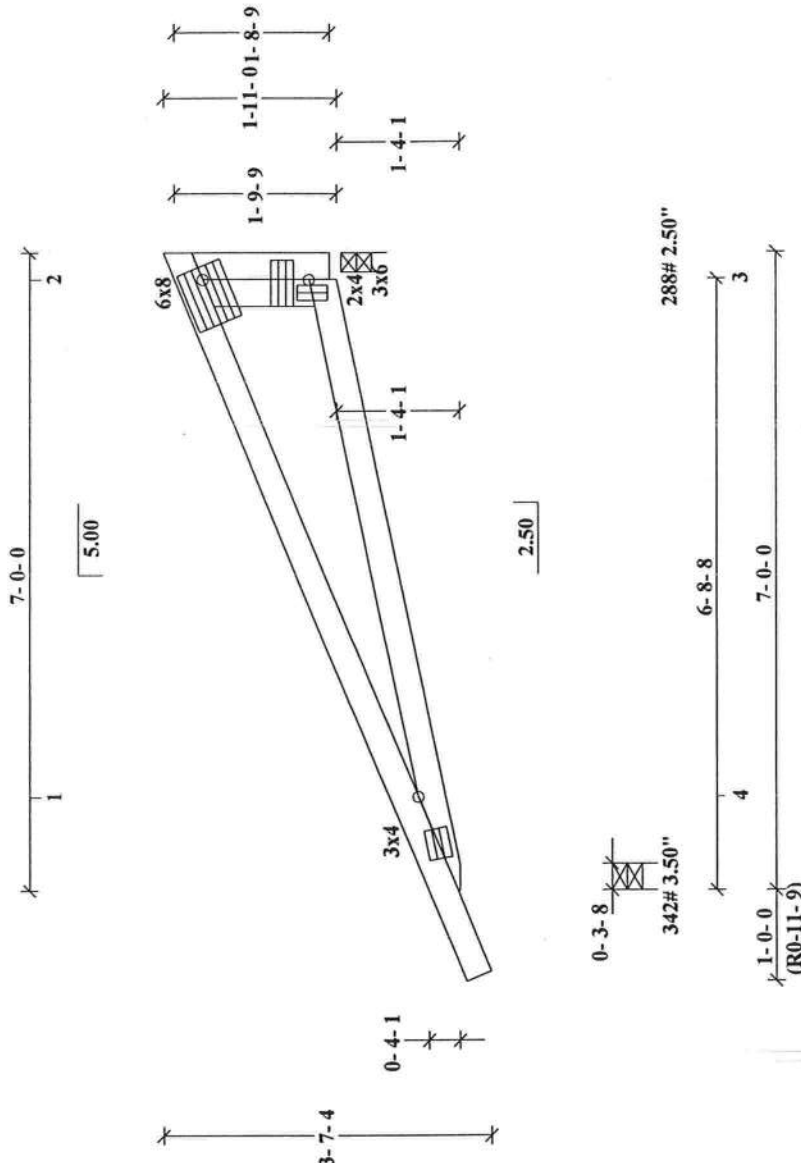
TI: JS1
7/29/2006

Lbr DF: 1.25
Plt DF: 1.25
O.C.: 2-0-0
TPI-02/FBC-04
Code: FLA
v4.7 32-58674

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Qty: 2

	LL	TL
I.Span	in./Ratio -0.11/125	in./Ratio -0.22/349
Horiz.	0.00	-0.01
		NA
	Max DL Deflection L/167 = -0.11	
	Long term deflection factor = 1.50	



Scale = 0.4705

This design is for an individual building component and this has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the trust designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accept no responsibility for any errors or omissions.

The design is for the building component and does not include fabrication, handling, shipment and installation of trusses. This truss has been designed in accordance with BS5426:1997 and BS5427:1995 and is to be incorporated as part of the building design by a Building Designer. When reviewed for approval by a Building Designer, the design loadings shown must be checked to be sure that the data shown is in agreement with the local building codes, local climatic records for wind and snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a sheathing applied rigidly to the bottom chord, the bottom chord is to be designed in accordance with AISC 360-96. Connector plates shall be manufactured from 20 gauge 48, unpainted galvanized steel in accordance with ASTM A 633, Grade 40, unless otherwise shown.

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for quality and safety. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knuckles, knots or distorted bearing. Members shall be cut for tight fitting wood to wood joints. Connector plates shall be located on both faces of the joint with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 5/4 plate is 5" wide x 4" long. A 6/4 plate is 6" wide x 3" long. Double ends on top parallel to the plate length specified. Scribe ends on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSII/TPI 1-1995.

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during banding and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary erection bracing for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until after all fastening and bracing is completed.

TC: 2x 4 SP #2
SBC: 2x 4 SP #2
MULTIPLE LOADCASES
composite result of
This truss is design
supports. Interior
be marked on truss.
necessary to achieve
FORCES (lb) - Max C
1-2=-45/
TOP CHORD
1-3=-20/
BOT CHORD
-1000000
-1000000

LL	
in./Ratio	
0.15/xxx	
0.00	
Max DL Deflection	
Long term deflection	
Horiz.	
I. Span	

TC: 2x 4 SP #2
BC: 2x 4 SP #2

Global Test Results			
	LL	IL	
I Span	in/Ratio	in/Ratio	Jnt(s)
	0.15/xxx	0.04/xxx	3-1
Horiz.	0.00	0.00	NA
Max DL Deflection L/xxx = -0.11			
Long term deflection factor = 1.50			

Analysis based on
ALL COMPRESSION
continuously bra
Shim or wedge if
bearing.
Welded per ASCE
E = 22.0 ft, I = 1
Bld Type= encl L
in INT zone, TCD
Impact Resistant
.TC..FORCE.C
-45

C6C, V= 125mph,
Cat. B, Krt= 1.0
t W= 5.0 ft Truss
psf, BCDL= 6.0 psf
and/or Glaring Req
BC. FORCE. CSI.
1- 3 20 0 40

5-0-0

Joint Load	
0- 6	3) 5-
0- 0	4) 1-
defined loadcases	
design.	
Quality Assurance	
REACTIONS PER	
BSet	Vert E
1	261
1	113
1	103

[illegible]

consider	Cq= 1
	LOCATION-
	ft Y
	93 B
	75 T
	41 B
	REQUIRE

ENTER & WIN!

Over 3 Supports Scale = 0.5492

WARNING:
READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO
ERECTING CONTRACTOR. BRACING WARNING:
Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of
the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss
members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified
locations determined by the building designer. Additional bracing of the overall structure may be required. (See HB-91
for details.) For more information, contact building designer. (Truss Plate Number, TPI is located at 38)
Columbo Engineering, Wisconsin Engineering Co., P.A., 818 Southside Rd., Eden, NC 27932
C/Oncor Drives, Madison, Wisconsin 53719.

25
25
- 0

4005 MARONDA WAY
Sanford, FL 32771
(407) 321-0064 Fax (407) 321-3913
OMAS PONCE P.E. LICENSE #0050066
67 Medallion Pl Chuluota, FL 32766

profile Path: C:\TEE-LOK\Work\Jobs\BBS4S\BAYBUR-1\JS2.prx

Job: BAYBURY S 4

Customer: 10# UPLIFT D.L.

WO: BBR4

TI: JS4

Qty: 4

TC: 2x 4 SP #2

BC: 2x 4 SP #2

Bottom chord live loads based on uninhabitable areas without storage loaded concurrently per IRC-00 R301.4. MULTIPLE LOADCASES -- This design is the composite result of multiple loadcases. This truss is designed to bear on multiple supports. Interior bearing locations should be marked on truss. Shim or wedge if necessary to achieve full bearing.

..TC..FORCE..CSI..Cr..BC..FORCE..CSI..1-2 -10 0.02 A 1-3 -3 0.01

Analysis based on Simplified Analog Model. All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise. Shim or wedge if necessary to achieve full bearing.

In-Plant Quality Assurance with Cq= 1

---MAX. REACTIONS PER BEARING LOCATION---

X-Loc Bset Vert Horiz Uplift Y Type

0- 1-12 1 89 35 -175 B Pin

0-11- 4 1 23 29 -63 T Pin

0-11- 4 1 20 0 -12 B H Roll

PROVIDE UPLIFT CONNECTION PER SCHEDULE

-----GLOBAL MAX DEFLECTIONS-----

LL TL

I-Span in./Ratio in./Ratio Jnt(s).

Horiz. 0.00/999 0.00/999 2-1

Max DL Deflection L/999 = 0.00

Long term deflection factor = 1.50

Unbalanced load case(s) have been checked.

1) 0-10-14 3) 1-0-0

2) 1-0-0 4) 0-10-14

WindLoad per ASCE 7-02, C&C, V= 125mph, E= 15.0 ft, I= 1.00, Exp.Cat. B, Kzt= 1.0

Bld Type= encl L= 1.0 ft W= 1.0 ft Truss in END zone, TCDL= 0.0 psf, BCDL= 0.0 psf

Impact Resistant Covering and/or Glazing Req FORCES (lb) - Max Compression/Max Tension

TOP CHORD 1-2=-10/0,

BOT CHORD 1-3=-3/3,

DESIGN INFORMATION

This design is for an individual building component and is not to be used for any other purpose. The designer assumes no responsibility for damage as a result of faulty or incorrect information, specifications and/or design furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accepts no responsibility or exercises no control with regard to fabrication, handling, shipment and installation of trusses. This truss has been designed as an individual building component in accordance with ANS/TPI 1-1995 and NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the Building Designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climate records for wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knotholes, knots or distorted grain. Members shall be cut for tight fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 5x4 plate is 5" wide x 4" long. A 6x8 plate is 6" wide x 8" long. Slots (holes) run parallel to the plate length specified. Double cuts on web members shall meet at the centroid of the web unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for corner loading and end connections. Trusses are to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANS/TPI 1-1995

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during banding and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection tracing is always required. Normal precautionary action for trusses requires erecting crews to erect and install trusses in accordance with the erection and bracing drawings. The erection of erection of trusses shall be under the control of person experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until after all fastening and bracing is completed

EXCEPT AS SHOWN PLATES ARE MITEK MT20

Maronda Systems

4005 MARONDA WAY

Sanford, FL 32771

(407) 321-0064 Fax (407) 321-3913

TOMAS PONCE P.E. LICENSE #00300668

367 Medallion Pl. Chuluota, FL 32766

Design: Matrix Analysis

WARNING:

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING:

Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HIB-91 of TPI) For specific truss bracing requirements, contact building designer. (Truss Plate Institute, TPI is located at 583 D'Oonofrio Drive, Madison, Wisconsin 53719).

Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd. Edenton, NC 27932

Eng Job:

WO: BBR4

Dwg:

TI: JS4

Dsgnr: TLY

Chk:

7/29/2006

TC Live 16.0 psf

TC Dead 7.0 psf

BC Live 10.0 psf

BC Dead 10.0 psf

TOTAL 43.0 psf

Libr DF: 1.25

Plt DF: 1.25

O.C.: 2- 0- 0

TPI-02/FBC-04

Code: FLA

v4.7.32-58672

Over 3 Supports Scale = 0.6060

Profile Path: C:\TEE-LOK\Work\Jobs\BBR4S\BAYBUR-1\JS4.prn

Job: BAYBURY S 4

Customer: 10# UPLIFT D.L.

WO: BBSR4

TI: MGRD1

Qty: 1

DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The design is not to be used for any other building component or for any other building component without the approval of the designer. The designer is not responsible for the accuracy or completeness of the information provided by the client. The designer is not responsible for the accuracy or completeness of the information provided by the client. The designer is not responsible for the accuracy or completeness of the information provided by the client.

TC: 2x 4 SP #2

BC: 2x10 SP #2

WB: 2x 4 SP #3

All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise.

This 1-PLY truss designed to carry

20' 0" span framed to BC one face

2' 0" span split-framed to opposite face

---MAX. REACTIONS PER BEARING LOCATION---

X-Loc Bset Vert Horiz Uplift Y Type

0-1-12 1 1634 170 -236 B Pin

6-10-4 1 1634 0 -290 B H Roll

PROVIDE UPLIFT CONNECTION PER SCHEDULE

---GLOBAL MAX DEFLECTIONS---

LL

in./Ratio in./Ratio Jnt(s).

I. Span -0.19/399 -0.38/201 2-1

Horiz. 0.08 0.16 NA

Long term deflection factor = 1.50

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knots, knots or distorted grain. Members shall be cut for tight fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 5x4 plate is 5" wide x 4" long. A 6x8 plate is 6" wide x 8" long. Slots (holes) run parallel to the plate length specified. Double cuts on web members shall meet at the centroid of the web unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANS/TP1 1-1995

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during handling and bundling. Delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be the responsibility of a competent person. The designer is not responsible for the accuracy or completeness of the information provided by the client. The designer is not responsible for the accuracy or completeness of the information provided by the client. The designer is not responsible for the accuracy or completeness of the information provided by the client.

MULTIPLE LOADCASES -- This design is the composite result of multiple loadcases. Right End vertical designed for wind. User defined loadcases have been considered in this design.

WindLoad per ASCE 7-02, C&C, V= 125mph, H= 22.0 ft, I= 1.00, Exp.Cat. B, Kzt= 1.0

Bld Type= encl I= 40.0 ft W= 7.0 ft Truss in INT zone, TCDF= 4.2 psf, BCDL= 6.0 psf

Impact Resistant Covering and/or Glazing Req

Supported span based on int. wind zone.

..TC..FORCE..CSI..Cr..BC..FORCE..CSI..

1-2 -76 0.73 C 4-3 67 0.97

..WEB..FORCE..CSI..Cr..

3-2 -186 0.17 C

Joint Locations

1) 0-0-0 3) 7-0-0

2) 7-0-0 4) 0-0-0

----- TOTAL DESIGN LOADS -----

Uniform PLF From PLF To

TC Vert L+D -46 0-0-0 -46 7-0-0

BC Vert L+D -420 0-0-0 -420 7-0-0

In-Plant Quality Assurance with Cq= 1

FORCES (lb) - Max Compression/Max Tension

TOP CHORD 1-2=-76/69,

BOT CHORD 4-3=0/67,

Welds 3-2=-186/179,

..WEB..FORCE..CSI..Cr..

3-2 -186 0.17 C

Diagram

3-3-1

7-0-0

5.00

2x4

3x8

3x8

3x6

3-3-1

0-4-1

1634# 3.50"

7-0-0

1634# 3.50"

3

EXCEPT AS SHOWN PLATES ARE MITEK MT20

Scale = 0.4962

WARNING:

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING:

Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HIB-91 of TPI) For specific truss bracing requirements, contact building designer, Truss Plate Institute, TPI is located in 583 D'Oro Drive, Madison, Wisconsin 53719.

Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd, Edenton, NC 27932

Maronda Systems

4005 MARONDA WAY

Sanford, FL 32771

(407) 321-0064 Fax (407) 321-3913

TOMAS PONCE P.E. LICENSE #0030068

367 Medallion Pl. Chuluota, FL 32766

Design: Matrix Analysis

Profile Path: C:\TEE-LOK\Work\Jobs\BBSR4S\BAYBUR-1\MGRD1.prx

Eng Job:

WO: BBSR4

TI: MGRD1

Dgnr: TLY

Chk:

7/29/2006

TC Live 16.0 psf

TC Dead 7.0 psf

BC Live 10.0 psf

BC Dead 10.0 psf

TOTAL 43.0 psf

Lbr DF: 1.25

Plt DF: 1.25

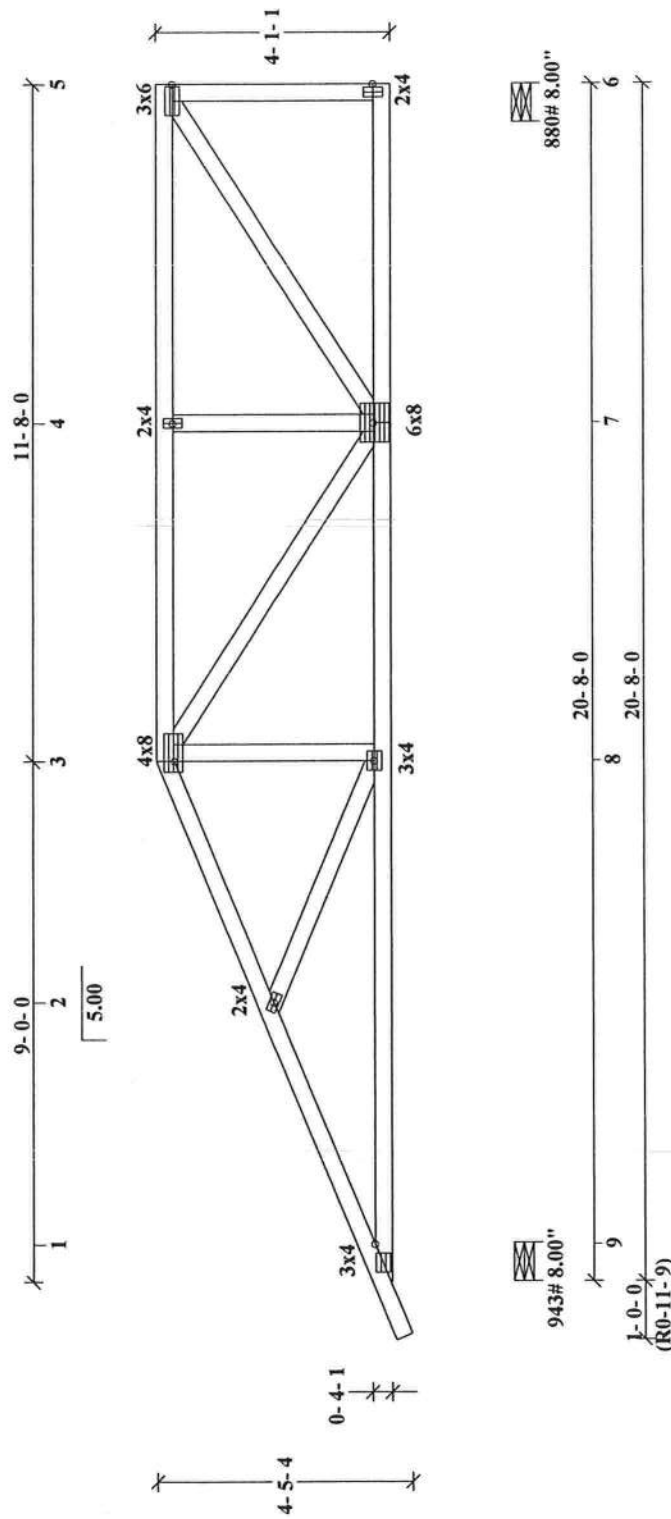
O.C.: 2-0-0

TPI-02/FBC-04

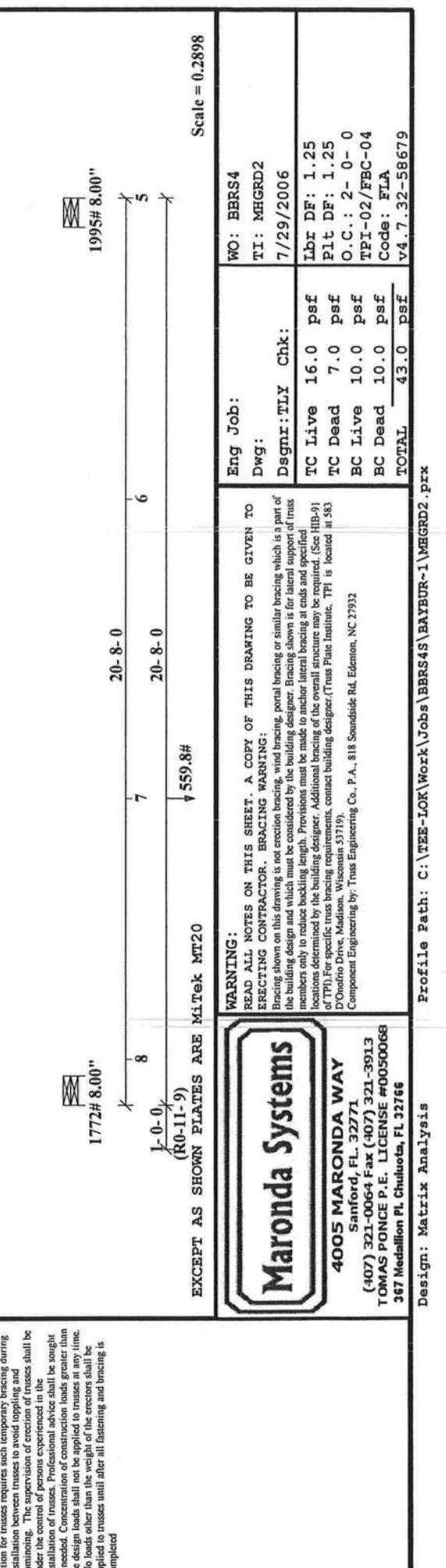
Code: FLA

v4.7.32-58677

Profile Path: C:\TEE-LOK\Work\Jobs\BBSRS4S\BAYBUR~1\MH1.prx



Scale = 0.2989



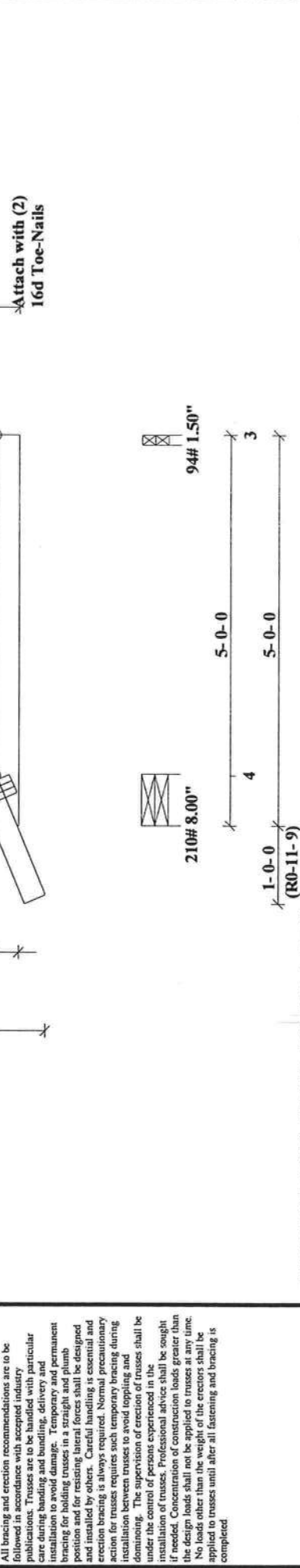
DESIGN INFORMATION
This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accept no responsibility or exemption no control with regard to fabrication, handling, shipping and installation of trusses. This truss has been designed as an individual building component in accordance with ANSI/TPI 1-1995 and NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the building designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-4" o.c. Connector plates shall be manufactured from 1/2" gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES
Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knots, knots or distorted grain. Members shall be cut for right fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 3x4 plate is 5" wide x 4" long. A 6x6 plate is 6" wide x 8" long. Slots (holes) run parallel to the plate length specified. Double cuts on web members shall meet at the centroid of the web unless otherwise shown. Connector plates are minimum 1/2" thick. The fabricator shall increase the size of the truss based on the foregoing and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995

PRECAUTIONARY NOTES
All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during handling and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and trussing is always required. Normal precautionary action for trusses is always required during erection and installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the trusses shall be applied to trusses until after all fastening and bracing is completed.

TC: 2x 4 SP #2
BC: 2x 4 SP #2
Unbalanced load case(s) have been checked.
[] or [] Plates OFFSET from joint center.
The Joint Detail Report must be included with any submittal, inspection, and/or fabrication documentation.
User defined loadcases have been considered in this design.
This truss is designed in accordance with the 2004 Florida Building Code section 2306.1 and referenced standards, U.N.O.

Analysis based on Simplified Analog Model.
MULTIPLE LOADCASES -- This design is the composite result of multiple loadcases.
ALL COMPRESSION Chords are assumed to be continuously braced unless noted otherwise.
Windload per ASCE 7-02, $C_e C_s V = 125 \text{ mph}$, $H = 22.0 \text{ ft}$, $I = 1.00$, Exp. Cat. B , $K_{zt} = 1.0$
Bid type= part I= 50.0 ft W= 5.0 ft Truss in INT zone, $\text{TCOL} = 4.2 \text{ psf}$, $\text{BCOL} = 6.0 \text{ psf}$
-----GLOBAL MAX DEFLECTIONS-----
LL TL
in./Ratio in./Ratio Jnt(s).
I. Span -0.03/999 -0.08/607 2-1
Horiz. 0.00 0.00 NA
Long term deflection factor = 1.50



EXCEPT AS SHOWN PLATES ARE MITEK MT20

Maronda Systems
4005 MARONDA WAY
Sanford, FL 32771
(407) 321-0064 Fax (407) 321-3913
TOMAS PONCE P.E. LICENSE #0050068
367 Medallion Pl, Chuluota, FL 32766

WARNING:
READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING:
Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified intermediate points by building designer. Additional bracing of the overall structure may be required. (See HIB-91 of TPI) For specific details, contact building designer (Truss Plate Institute, TPI is located at 381 D'Ossorio Drive, Madison, Wisconsin 53719).
Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd, Edenon, NC 27932

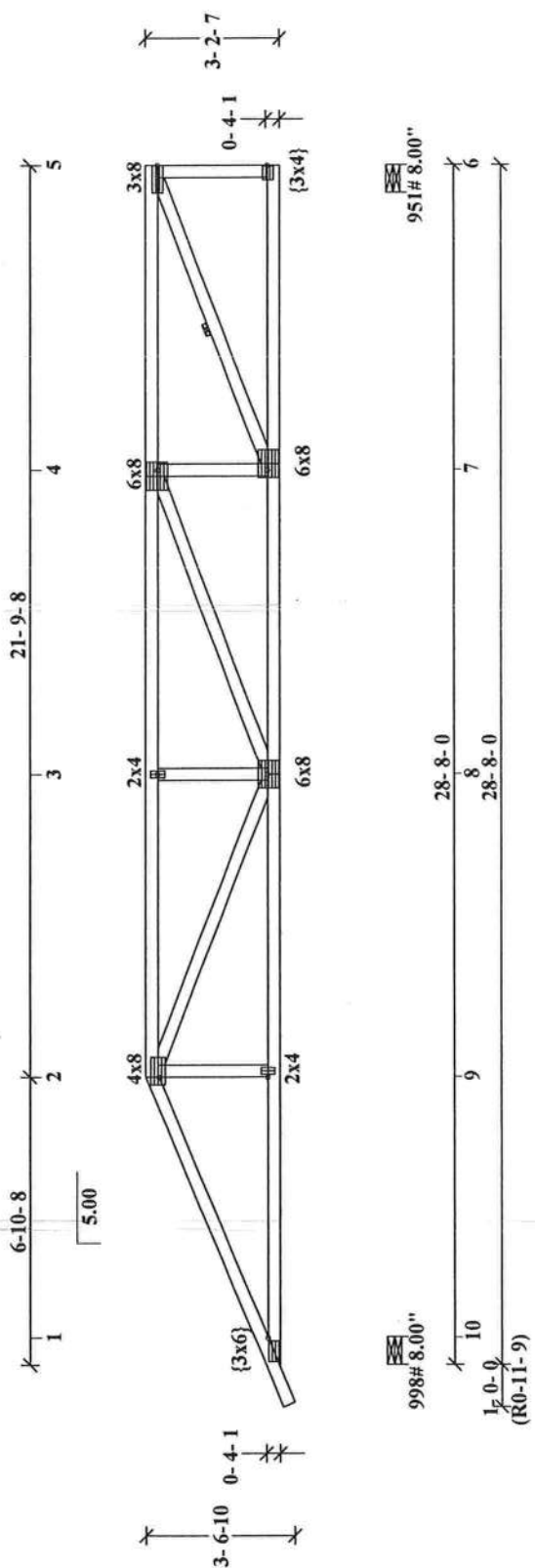
Eng Job:
Dwg:
Dsgnr: TLX Chk:
TC Live 16.0 psf
TC Dead 7.0 psf
BC Live 0.0 psf
BC Dead 10.0 psf
TOTAL 33.0 psf

WO: BBSR4
TI: P1
8/28/2007
Lbr DF: 1.25
Plt DF: 1.25
O.C.: 2-0-0
FBC-04/TPI-02/NDS-01
Code: FLA
v5.5.6-9857

Over 3 Supports Scale = 0.5171

Profile Path: T:\TEE-LOK\Work\Jobs\BBSR4S\BBSR4S\PI.prx

(R0-11-9)



This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accepts no responsibility or exercises no control with regard to fabrication, handling, shipment and installation of these

Prior to fabrication, the fabricator shall review this drawing to verify that this fabrication is in conformance with the manufacturer's plans and realize that connecting members shall be fabricated in accordance with the design. Connecting members shall be fabricated in accordance with the design to be put in during bore cutting of fabrication. Plates shall not be installed over kneebolts, bolts or distortions. Connector plates shall be cut out for tight fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 5/4 plate is 5" wide x 4" long. A 6/4 plate is 6" wide x 8" long. Double cut on nail parallel to the plate length specified. Double cut on nail members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. Truss members shall be fabricated with fire resistant lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during banding and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of connection loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erections shall be applied to trusses until after all fastening and bracing is completed.

Maronda Systems

4005 MARONDA WAY
Sanford, FL 32771
(407) 321-0064 Fax (407) 321-3913
TOMAS PONCE P.E. LICENSE #0050068
367 Medallion Pl. Chuluota, FL 32766

Profile Path: C:\TEE-LOK\Work\Jobs\BBS4S\BAYBUR-1\RGL.prx

WARNING:
READ ALL NOTES ON THIS SHEET, A COPY OF THIS DRAWING TO BE GIVEN TO
ERECTING CONTRACTOR. BRACING WARNING:
Bracing shown on this drawing is not considered bracing, wind bracing, portal bracing or similar bracing which is a part of
the building design and which must be erected by the building designer. Bracing shown is for lateral support of truss
members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified
locations determined by the building designer. Additional bracing of the overall structure may be required. (See HB-91
for details.) For more information, contact building designer (Truss Plate Institute, TPI is located at 583
D'Oroville Drive, Madison, Wisconsin 53719).
Consulting Engineer: Truss Engineering Co., P.A., 818 Somerside Rd, Edenburg, NC 27932

Eng Job:	
Dwg:	
Dsgnr: TLY	Chk:
TC Live	16.0
TC Dead	7.0
BC Live	10.0
BC Dead	2.0
TOTAL	35.0

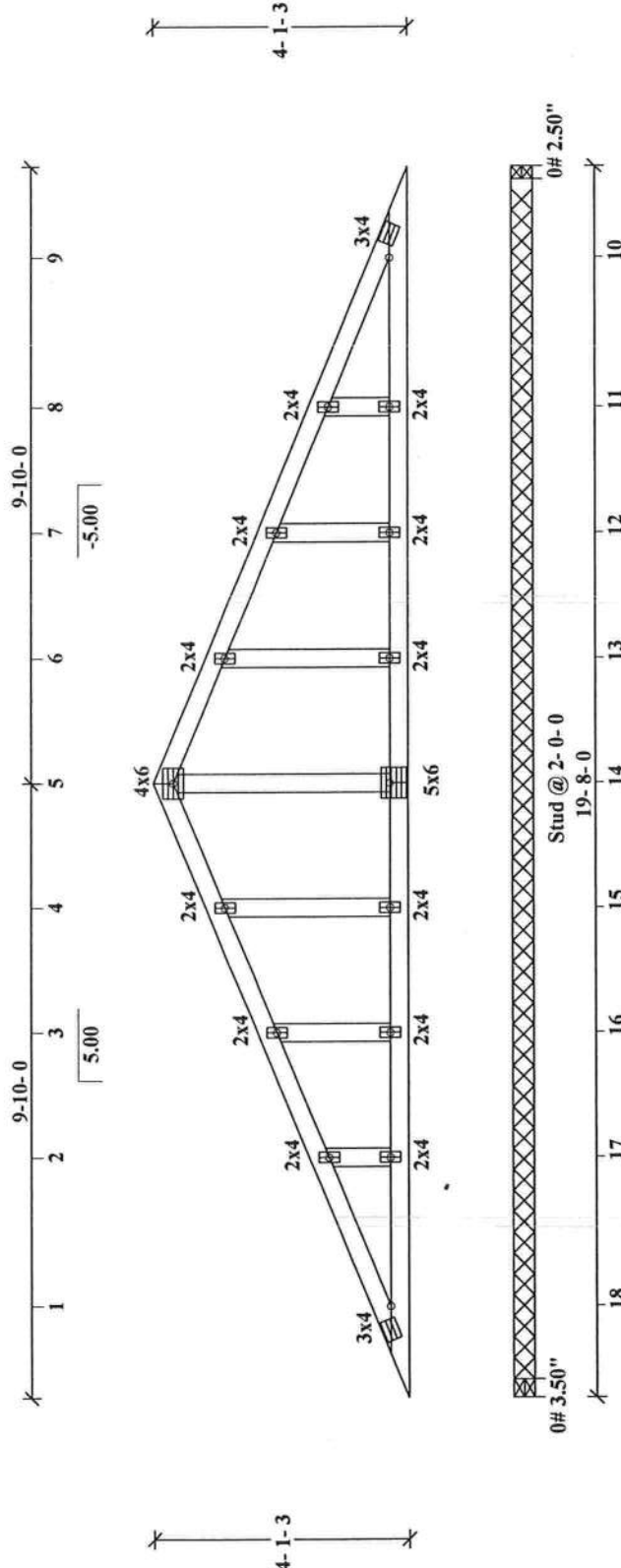
WO: BBR54
TI: RG1
7/29/2006
Lbr DF: 1
Plt DF: 1
O.C.: 2-
TPI-02/FB
Code: FLA
v4.7.325

Analysis based on Simplified Analog Model.
All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise
W/ndload per ASCE 7-02, C6C, V=125mph
W= 22.0 ft, I= 1.00, Exp.Cat. B, Kzt= 1.0
Bld Type= encl I= 36.0 ft W= 19.7 ft Truss
In END zone, TCDL= 4.2 psf, BCDL= 1.2 psf
In-Plant Quality Assurance with Cq= 1
FORCES (lb) - Max Compression/Max Tension
TOP CHORD

TC: 2x 4 SP #2
BC: 2x 4 SP #2
WB: 2x 4 SP #3
GBL: 2x 4 SP #3
***** CUTTING ONLY *****

MULTIPLE LOADCASES -- This design is the composite result of multiple loadcases.
User defined loadcases have been considered in this design.
Wind loads have been checked perpendicular to the face of the truss.

Joint		Locations		Type	
X-loc	Bset	Vert	Horiz	Uplift	Pin
1)	0-0-0	7)	13-10-0	13)	11-10-0
2)	3-10-0	8)	15-10-0	14)	9-10-0
3)	5-10-0	9)	19-8-0	15)	7-10-0
4)	7-10-0	10)	19-8-0	16)	5-10-0
5)	9-10-0	11)	15-10-0	17)	3-10-0
6)	11-10-0	12)	13-10-0	18)	0-0-0
---MAX. REACTIONS PER BEARING LOCATION---					
19-6-12	1	0	0	0	B
19-6-12	1	0	0	0	B
CONNECTION PER SCHEDULE					



Job: BAYBURY S 4

Customer: 10# UPLIFT D.L.

WO: BBR54

Qty: 3

TC: 2x 4 SP #2

BC: 2x 4 SP #2

WB: 2x 4 SP #3

DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accepts no part of the responsibility for the design of this information.

This truss has been designed as an individual building component to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the building designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knotholes, knots or distorted grain. Members shall be cut for tight fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 5x4 plate is 5" wide x 4" long. A 6x8 plate is 6" wide x 8" long. Slot (holes) run parallel to the plate length specified. Double cuts on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during handling and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominoing. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erection shall be applied to trusses until all bracing and bracing is completed

MULTIPLE LOADCASES -- This design is the composite result of multiple loadcases. User defined loadcases have been considered in this design.

In-Plant Quality Assurance with Cq= 1

FORCES (lb) - Max Compression/Max Tension

TOP CHORD 1-2=-2782/1187, 2-3=-2224/847, 3-4=-2224/847, 4-5=-2782/1187, 7-8=-1049/2634, BOT CHORD 8-7=-1049/2634, 7-6=-1049/2634, Webs 2-7=-527/391, 7-3=-425/1477, 7-4=-527/391.

IMPACT RESISTANT COVERING AND/OR GLAZING REQ. WEB. . . FORCE. . CSI. Cr 7-4 -527 0.22 C 7-4 -527 0.22 C 7-3 1477 0.47 C

GLOBAL MAX DEFLECTIONS-----

TL

I Span in./Ratio in./Ratio Jnt(s).

Horiz. 0.12 0.24 NA

Max DL Deflection L/774 = -0.30

Long term deflection factor = 1.50

Joint Locations

1) 0-0-0 4) 14-5-13 7) 10-0-0

2) 5-6-3 5) 20-0-0 8) 0-0-0

3) 10-0-0 6) 20-0-0

---MAX. REACTIONS PER BEARING LOCATION---

X-Loc BSet Vert Horiz Uplift Y Type

0-1-12 1 906 0 -452 B H Roll

19-10-4 1 906 0 -452 B H Roll

PROVIDE UPLIFT CONNECTION PER SCHEDULE

..TC. . . FORCE. . CSI. Cr ..BC. . . FORCE. . CSI.

1-2 -2782 0.42 A 8-7 2634 0.85

2-3 -2224 0.22 A 7-6 2634 0.85

3-4 -2224 0.22 A

4-5 -2782 0.42 A

Diagram showing a truss structure with dimensions and loadings. Key dimensions include 10'-0" spans, 4'-10" height, and various member sizes (2x4, 3x6, 4x6, 6x8). Loadings include 16.0 psf live, 7.0 psf dead, and 10.0 psf wind. Deflection values are shown as -5.00, -2.50, and -2.50. A scale of 0.2886 is indicated.

EXCEPT AS SHOWN PLATES ARE MITEK MT20

Scale = 0.2886

WARNING:

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING:

Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HIB-91 of TPI) For specific truss bracing requirements, contact building designer (Truss Plate Institute, TPI is located at 583 D'Oroff Drive, Madison, Wisconsin 53719).

Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd, Edenton, NC 27932

Maronda Systems

4005 MARONDA WAY

Sanford, FL 32771

(407) 321-0064 Fax (407) 321-3913

TOMAS PONCE P.E. LICENSE #0050069

387 Medallion Pl, Chuluota, FL 32766

Design: Matrix Analysis

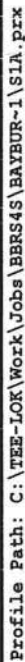
Profile Path: C:\TEE-LOK\Work\Jobs\BBR54S\BAYBUR-1\SI.prx

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the trust designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accepts no responsibility or specific project control with regard to fabrication, handling, shipment and installation of trusses. The trusses are designed in accordance with the building component in accordance with ANSI/PD-1:1995 and NDS-97 to be incorporated as part of the building design by the building designer. When reviewed for approval by the building designer, the design loadings shown must be checked to be sure that the data shown is in agreement with the local building codes, local climatic records for wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid roof deck, they should be braced to the roof deck. The trusses are designed in accordance with the manufacturer's design manual, ASTM A 633, grade 40, unless otherwise shown.

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for its verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over kneebuckles, knots or distorted grain. Members shall be cut for tight fitting wood to wood bearing. Connector plates shall be located on both faces of the joint unless otherwise shown. A 5/4 plate is 5" wide x 4" long. A 6/8 plate is 6" wide x 8" long. Double end joints shall be parallel to the plate length specified. Double cut joints shall meet at the center of the web unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This must be noted to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1985.

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during banding and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral force shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until after all fastening and bracing is completed.

Qty:1



DESIGN INFORMATION

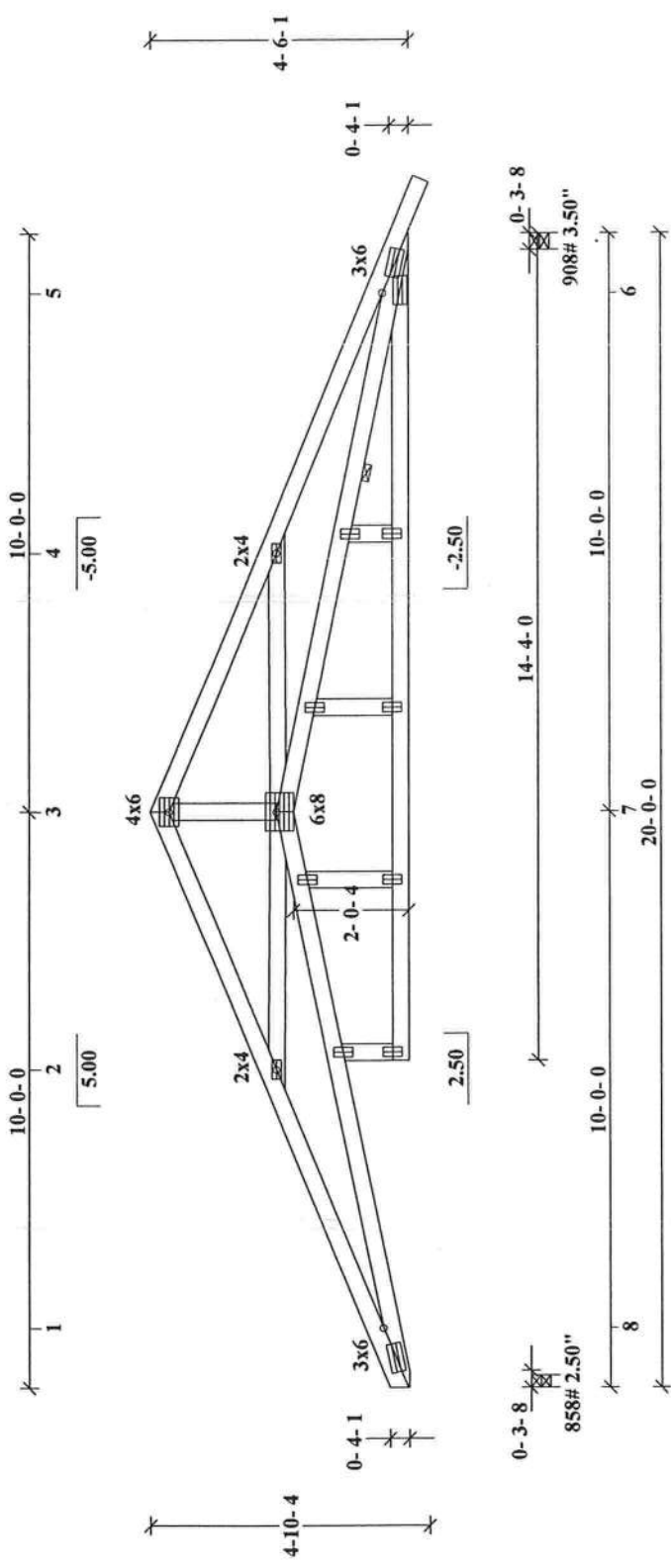
This design is for an individual building component and has been based on information provided by the client. The Designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the trust designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accepts no responsibility for any damages or losses not covered or referred to in this document. The Designer is not to be held responsible for conditions, conditions and/or conditions of trusses. This truss has been designed as an individual building component in accordance with ANSI/TPI 1-1995 and NDS-97 to be incorporated as part of the building design by a building Designer. When reviewed for approval by a building Designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind and snow loads, project specifications or special unapproved loads. Unless shown, truss has not been designed for storage of loads (top or bottom) and are continuously supported by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a project engineer applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be designed in accordance with the provisions of the American Institute of Steel Construction, Inc. (AISC) 360-10, except that the design shall be in accordance with ASTM A 36, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for each verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knuckles, knots or distorted members. Connector plates shall be cut to tight fitting wood to wood bearing. Connector plates shall be located on both faces of the joint with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 5x4 plate is 5" wide x 4" long. A 6x8 plate is 6" wide x 8" long. Double cut on joint parallel to the plate length specified. Sizable cut on webs of members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling, and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSII/TPI 1-1995

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry applications. Trusses are to be installed with particular care during building and bundling, delivery and installation to avoid damage. Temporary bracing for holding trusses in straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary bracing for trusses requires such temporary bracing during installation between trusses to avoid toppling and buckling. The supervision of erection is to be under the control of a competent person capable of installing the trusses. Professional advice shall be sought if needed. Connection of trusses should be made greater than design loads shall not be applied to trusses any time. No loads other than the weight of the trusses shall be applied to trusses until all all dunnage and bracing is supplied.



COSMETIC PLATES (2) 3X6 (16) 2X4

EXCEPT AS SHOWN PLATES ARE MiTek MT20

Maronda Systems

4005 MARONDA WAY
Sanford, FL 32771
(407) 321-0064 Fax (407) 321-3913
TOMAS PONCE P.E. LICENSE #0050006
367 Medallion Pl. Chuluota, FL 32766

Design: Matrix Analysis

WARNING:

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO EACH ERECTING CONTRACTOR. BRACING WARNING:

Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and supports as determined by the building designer. Additional bracing of the overall structure may be required. (See HIB-9) (TYP). For more information contact building designer (Truss Plate Institute, TPI is located at 583 O'Donnell Drive, Madison, Wisconsin 53719).

Compass Engineering, Truss Engineering Co., P.A., 818 Soundside Rd, Edenburg, NC 27932

Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd, Edenlon, NC 27932

Profile Path: C:\TEE-LOK\Work\Jobs\BRS4S\BAYBUR~1\S1B.prx

Eng Job:

Dwg:	TI: SLB
Dsgnr: TLY	Chk: 7/29/2006
TC Live	16.0 psf
TC Dead	7.0 psf
BC Live	10.0 psf
BC Dead	10.0 psf
TOTAL	43.0 psf

WO: BBR54

TI: S1B	Libr DF: 1.0
7/29/2006	Plt DF: 1.0
	O.C.: 2-0
	TPI-02/FBC
	Code: FLA
	v4.7.32-58

Scale = 0.3007

UPLIFT D.L. WO:BR84

Analysis based on Simplified Analog Model.

All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise.

Windrod per ASCE 7-02, C&C, V= 125mph, W= 22.0 ft, I= 1.00, Exp.Cat. B, Kzt= 1.0

Blld Type= encl I= 36.0 ft W= 18.0 ft Truss

in END zone, TCDL= 4.2 psf, BCDI= 1.2 psf

FORCES (lb) - Max Compression/Max Tension

TOP CHORD

```

Job: BAYBURY S 4
***** CUTTING ONLY *****
TC: 2x 4 SP #2
BC: 2x 4 SP #2
WB: 2x 4 SP #3
GGBL: 2x 4 SP #3

***** THIS DESIGN IS THE
***** COMPOSITE RESULT OF MULTIPLE LOADCASES.
***** USER DEFINED LOADCASES HAVE BEEN CONSIDERED
***** IN THIS DESIGN.
***** WIND LOADS HAVE BEEN CHECKED PERPENDICULAR
***** TO THE FACE OF THE TRUSS
Custom

```

[illegible]

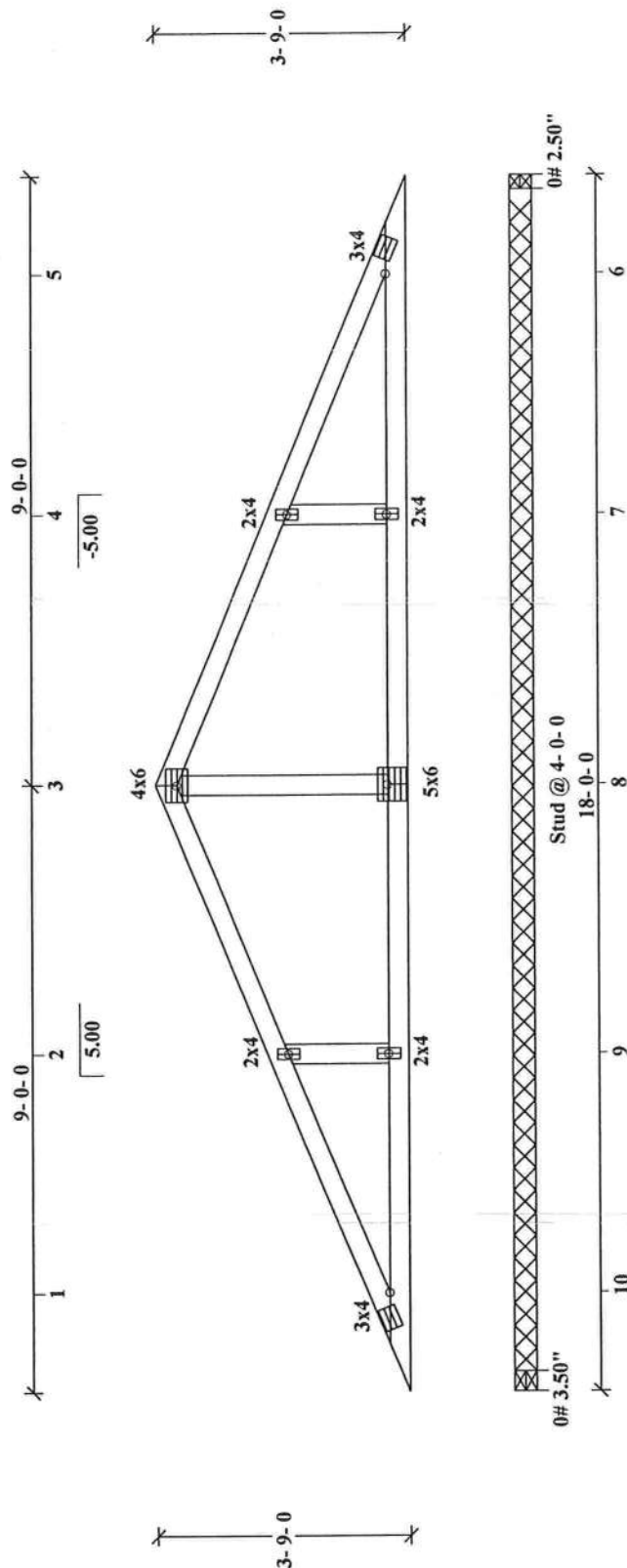
by a Building Designer. When reviewed for approval by the Building Department, the design loadings shown must be checked to be sure that the data shown is in agreement with the local building codes, local climatic records for wind and snow loads, project specifications or special approved loads. Unless shown, loads must not be designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a shear wall or other bracing, the design assumes that the maximum applied rigid roof ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be designed in accordance with the provisions of the American Institute of Steel Construction, Inc. (AISC) Specification ASTM A 653, Grade 40, unless otherwise stated.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that the drawing is in conformance with the applicable code and that the drawing is continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall be installed over topologies, knots or distorted grain. Members shall be cut for tight fitting wood to wood joints. Members shall be located on both sides of the truss with male fully imbedded and shall be sym. about the joint unless otherwise shown. A 5/4 plate is 5" wide x 1' long. A 6/4 plate is 6" wide x 1' long. Double cuts on truss parallel to the plate length specified. Double cuts on truss members shall meet at the centroid of the webs unless otherwise specified. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSIT/PPI 1995.

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during banding and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary erection bracing for trusses requires such temporary bracing and installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until after all fastening and bracing is completed.



Scale = 0.3567

EXCEPT AS SHOWN PLATES ARE MiTek MT20

WARBNTNC:

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO THE ERECTING CONTRACTOR. BRACING WARNING:
Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design. Bracing shown is for lateral support of trusses and members only to reduce buckling length. Provision must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HB19-9 of TPI) For specific bracing requirements, consult building designer (Truss Institute, TPI is located at 583 Oconomowoc Drive, Madison, Wisconsin 53719).
Component Engineer by: Truss Engineering Co., P.A., 818 Soundside Rd., Edenport, NC 27932

Maronda Systems

4005 MARONDA WAY
Sanford, FL 32771
(407) 321-0064 Fax (407) 321-3913
TOMAS PONCE P.E. LICENSE #0050068
367 Medallion Pl, Chuluota, FL 32766

Design: Matrix Analysis

Profile Path: C:\TEE-LOK\Work\Jobs\BBRS4S\BAYBUR-1\VL0.prx

Eng Job:	WO: BERS4
Dwg:	TI: V10
Dsgnr: TLY	7/29/2006
Chk:	Lbr DF: 1
TC Live	16.0 psf
TC Dead	7.0 psf
BC Live	10.0 psf
BC Dead	2.0 psf
TOTAL	35.0 psf

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accepts no responsibility or exercises no control with regard to fabrication, handling, shipment and installation of trusses. This document has been prepared for an individual building

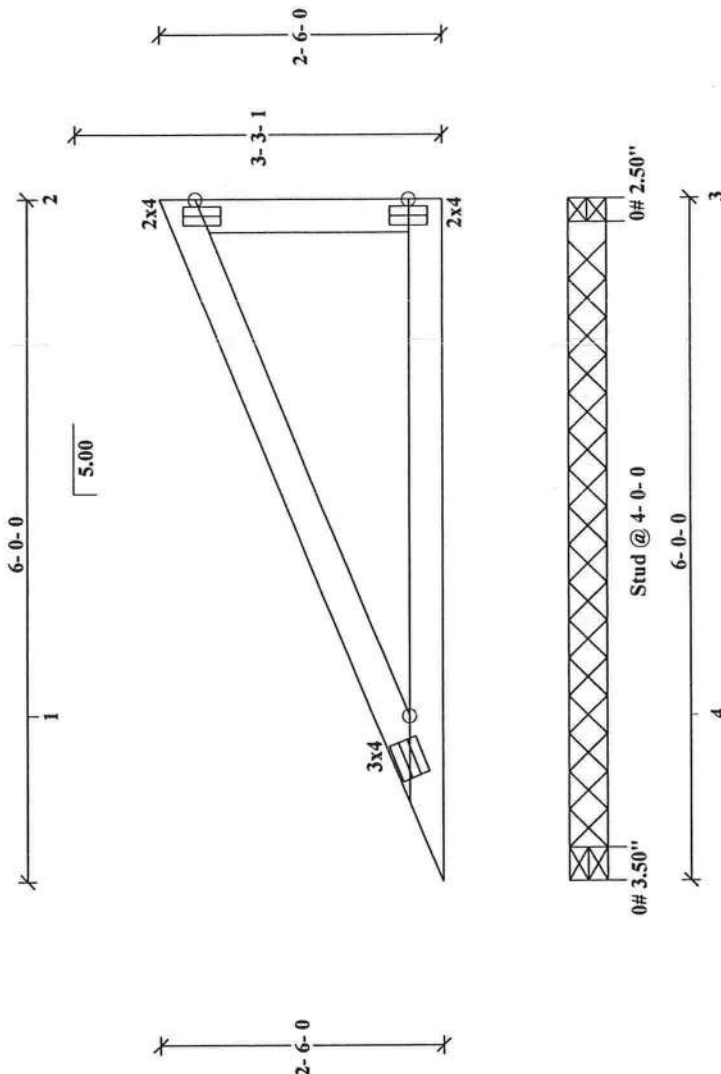
Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for each modification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over laplothes, knots or distorted members. Members shall be cut for tight fitting wood to wood bearing. Connector plates shall be fitted on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 5x4 plate is 5" wide x 4" long. A 6x8 plate is 6" wide x 8" long. Double cut on truss parallel to the plate length specified. Slope cuts on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during banding and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary erection for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until after all fastening and bracing is completed.

Design: Matrix Analysis

v4.7.32-58692

Scale = 0.5875




```

=====Joint Locations=====
1) 0-0-0 3) 6-0-0 5) 3-0-0
2) 3-0-0 4) 6-0-0 6) 0-0-0

Wind loads have been checked perpendicular
to the face of the truss.

In-Plant Quality Assurance with Cq= 1
-----MAX. REACTIONS PER BEARING LOCATION-----
X-K-Loc Bset Vert Horiz Uplift Y Type Pin
0-1-12 1 0 0 0 0 B Pin
5-10-12 1 0 0 0 0 B Pin
PENDING UPLIFT CONNECTION PER SCHEDULE

```

Profile Path: C:\TEE-LOK\Work\Jobs\BBRS4S\BAYBUR~1\V5.prx

Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd. Edenton, NC 27932

TOTAL	35.0	psf
-------	------	-----

v4.7.32-58697

Scale = 0.8158

Job: BAYBURY S 4

Customer: 10# UPLIFT D.L.

WO: BBS4

TI: V6

Qty: 1

DESIGN INFORMATION

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FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knotholes, knots or distorted grain. Members shall be cut for tight fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nuts fully imbedded and shall be sym. about the joint unless otherwise shown. A 3x4 plate is 5" wide x 4" long. A 6x8 plate is 6" wide x 8" long. Slots (holes) run parallel to the plate length specified. Double cuts on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and erection stresses. This truss is not to be fabricated with reentrant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during banding and handling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid topping and dominating. The supervision of erection of trusses shall be under the control of persons experienced and trained in the erection of trusses. Proper erection advice shall be sought if needed. Connections of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erections shall be applied to trusses until after all fastening and bracing is completed.

Analysis based on Simplified Analog Model.
All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise.
In-Plant Quality Assurance with Cq= 1
FORCES (lb) - Max Compression/Max Tension
TOP CHORD

1) 0-0-0 3) 10-0-0 5) 5-0-0
2) 5-0-0 4) 10-0-0 6) 0-0-0

-----MAX. REACTIONS PER BEARING LOCATION-----
X-Loc BSet Vert Horiz Uplift Y Type
0-1-12 1 0 0 0 0 B Pin
9-10-12 1 0 0 0 0 B Pin
PROVIDE UPLIFT CONNECTION PER SCHEDULE

5-0-0 5-0-0 3

2

4x6

3x4

2x4

5.00

-5.00

2-1-0

2-1-0

0# 3.50"

Stud @ 4-0-0

10-0-0 5

6

4

0# 2.50"

EXCEPT AS SHOWN PLATES ARE MITEK MT20

Scale = 0.6163

Maronda Systems

4005 MARONDA WAY
Sanford, FL 32771
(407) 321-0064 Fax (407) 321-3913
TOMAS PONCE P.E. LICENSE #0050068
367 Medallion Pl. Chuluota, FL 32766

Design: Matrix Analysis

Profile Path: C:\TEE-LOK\Work\Jobs\BBS4S\BAYBUR-1\V6.prx

WARNING:
READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO
ERECTING CONTRACTOR. BRACING WARNING:
Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HB-91 of TPI) For specific truss bracing requirements, contact building designer (Truss Plate Institute, TPI is located at 583 D'Oro Drive, Madison, Wisconsin 53719).
Component Engineering by: Truss Engineering Co., P.A., 418 Soundside Rd, Edenton, NC 27932

Eng Job:

WO: BBS4

TT: V6

7/29/2006

Dsgnr: TLY

Chk:

TC Live 16.0 psf

TC Dead 7.0 psf

BC Live 10.0 psf

BC Dead 2.0 psf

TOTAL 35.0 psf

W: BBS4

TT: V6

7/29/2006

Lbr DF: 1.25

Plt DF: 1.25

O.C.: 2-0-0

TPI-02/FBC-04

Code: FLA

v4.7.32-36626

Job: BAYBURY S 4

Customer: 10# UPLIFT D.L.

WO: BBR4

TI: V7

Qty: 1

DESIGN INFORMATION

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FABRICATION NOTES

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PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during banding and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erection shall be applied to trusses until after all flattening and bracing is completed.

Analysis based on Simplified Analog Model.

All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise.

In-Plant Quality Assurance with Cq= 1

---MAX. REACTIONS PER BEARING LOCATION---

X-Loc BSet Vert Horiz Uplift Y Type

0- 1-12 1 0 0 0 0 B Pin

13-10-12 1 0 0 0 0 B Pin

PROVIDE UPLIFT CONNECTION PER SCHEDULE

TC: 2x 4 SP #2

BC: 2x 4 SP #2

WB: 2x 4 SP #3

GSL: 2x 4 SP #3

***** CUTTING ONLY *****

MULTIPLE LOADS -- This design is the composite result of multiple loads.

WindLoad per ASCE 7-02, C&C, V= 125mph, H= 22.0 ft, I= 1.00, Exp. Cat. B, Kzt= 1.0

Bld Type= encl L= 36.0 ft W= 14.0 ft Truss in END zone, TCDL= 4.2 psf, BCDL= 1.2 psf

Joint Locations

1) 0- 0- 0 5) 14- 0- 0 9) 3- 0- 0

2) 3- 0- 0 6) 14- 0- 0 10) 0- 0- 0

3) 7- 0- 0 7) 11- 0- 0

4) 11- 0- 0 8) 7- 0- 0

FORCES (lb) - Max Compression/Max Tension

TOP CHORD

Diagram

Scale = 0.4518

EXCEPT AS SHOWN PLATES ARE MITEK MT20

Maronda Systems

4005 MARONDA WAY
Sanford, FL 32771
(407) 321-0064 Fax (407) 321-3913
TOMAS PONCE P.E. LICENSE #0050068
367 Medallion Pl Chuluota, FL 32766

Design: Matrix Analysis

WARNING:

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING:

Bracing shown on this drawing is not erection bracing, wind bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HIB-91 of TPI) For specific truss bracing requirements, contact building designer. (Truss Plate Institute, TPI is located at 583 D'Osoff Drive, Madison, Wisconsin 53719).

Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd, Edenton, NC 27932

Eng Job:

WO: BBR4

Dwg:

TI: V7

Dsgnr: TLY

Chk:

7/29/2006

TC Live 16.0 psf

TC Dead 7.0 psf

BC Live 10.0 psf

BC Dead 2.0 psf

TOTAL 35.0 psf

Lbr DF: 1.25

Plt DF: 1.25

O.C.: 2- 0- 0

TPI-02/FBC-04

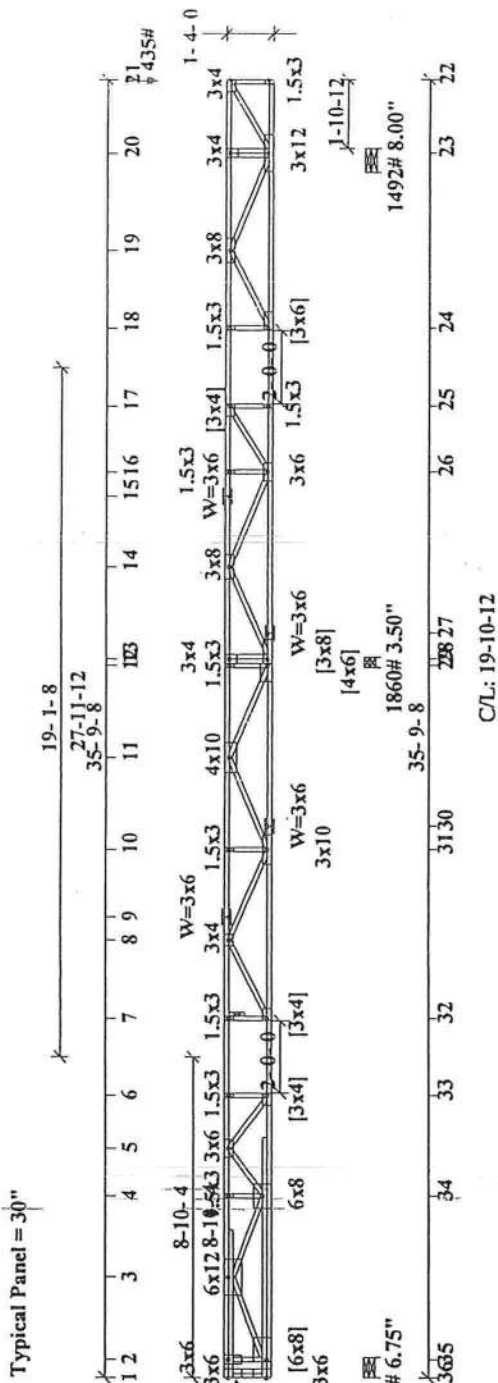
Code: FLA

v4.7.32-36627

Profile Path: C:\TEE-LOK\Work\Jobs\BBR4S\BAYBURY-1\77.prx

1000

CANT: 1-10-12
Over 3 Supports Scale = 0.1842



DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or design furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accept no responsibility for errors or omissions in the design of trusses. This truss has been designed as an individual building component in accordance with ANSI/TPI 1-1995 and NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the building designer, the design loading shown must be checked to be sure that the data shown are in agreement with the local building codes, local climate records for wind and snow loads, project specifications or special applied loads. Unless shown, trusses have not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be manufactured from 30 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

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PRECAUTIONARY NOTES

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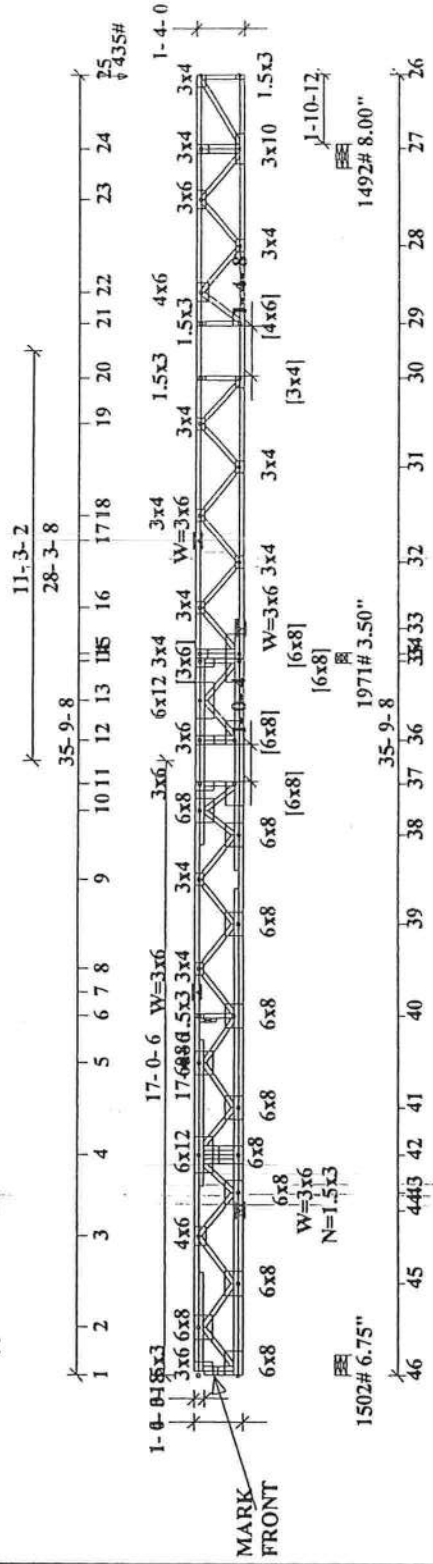
TC:	4x 2	SP #2	7-17
BC:	4x 2	SP #2 D	
WB:	4x 2	SP #1 D	33-44
	4x 2	SP #3	4-42
	4x 2	SP #3	12-36, 15-34, 24-27
	4x 2	SP #1 D	0-0, 0-0
	4x 2	SP #2 D	0-0
	4x 2	SP #2	0-0, 0-0

Unbalanced load case(s) have been checked.
[] or []: Plate(s) OFFSET from joint center.
The Joint Detail Report must be included with any submittal, inspection, and/or fabrication documentation.
ALL COMPRESSION Chords are assumed to be continuously braced unless noted otherwise.
*** CUT TOP CHORD OVER INTERIOR SUPPORT ***
This truss is designed in accordance with the 2004 Florida Building Code section 2306.1 and referenced standards, U.N.O.
---GLOBAL MAX DEFLECTIONS---
TL
in./Ratio in./Ratio Jnt(s)
I. Span -0.41/555 -0.83/276 7-6
R. Cant. -0.01/999 -0.04/603 26
Horiz. 0.10 0.15 NA
Long term deflection factor = 1.50

2x6 continuous strongback. Attach to each truss with 3-3.5"x 0.135" nails. Splice strongback on truss vertical.
MULTIPLE LOADCASES -- This design is the composite result of multiple loadcases.
Partial Double Chord to be same size and grade as chord to which it is attached.
This truss is designed to bear on multiple supports. Interior bearing locations should be marked on truss. Shim or wedge if necessary to achieve full bearing.
In-Plant Quality Assurance with Cq = 1.11
In-Plant Quality Assurance, per sec. 3.2.4 of TPI-1-02, for joint(s):
2,3,9,10,11,13,16,23,25,27,28,32,36,37,45
---MAX. REACTIONS PER BEARING LOCATION---
X-Loc Bset Vert Horiz Uplift Y Type
0- 3- 6 1 1502 0 0 B Pin
19-10-12 1 1971 0 0 B H Roll
33- 6-12 1 1492 0 0 B H Roll
PROVIDE UPLIFT CONNECTION PER SCHEDULE

	Joint Locations				
1)	0- 0- 0	17)	23- 1- 8	33)	20- 8- 8
2)	1- 3-12	18)	23- 9- 8	34)	20- 0- 4
3)	3- 9-12	19)	26- 3- 8	35)	19- 9-12
4)	6- 0- 8	20)	27- 6- 8	36)	17- 7- 4
5)	8- 7- 4	21)	29- 0- 8	37)	16- 4-12
6)	9-11- 0	22)	29-10-12	38)	14-11-12
7)	10- 7- 0	23)	32- 4-12	39)	12- 5-12
8)	11- 2-12	24)	33- 9- 4	40)	9-11- 0
9)	13- 8-12	25)	35- 9- 8	41)	7- 4- 4
10)	15- 7-14	26)	35- 9- 8	42)	6- 0- 8
11)	16- 4-12	27)	33- 9- 4	43)	5- 0- 2
12)	17- 7- 4	28)	31- 1-12	44)	4- 6- 2
13)	18- 8- 8	29)	29- 0- 8	45)	2- 6- 0
14)	19- 9-12	30)	27- 6- 8	46)	0- 0- 0
15)	20- 0- 4	31)	25- 1- 4		
16)	21- 3- 8	32)	22- 6- 8		
----- TOTAL DESIGN LOADS -----					
Uniform	PLF	From	PLF	To	
TC Vert L+D	-100	0- 0- 0	-100	35- 9- 8	
BC Vert L+D	-10	0- 0- 0	-10	35- 9- 8	
Concentrated		LBS	Location		

Typical Panel = 30"



EXCEPT AS SHOWN PLATES ARE MiTek MT20
CANT: 1-10-12 Scale = 0.1842
Over 3 Supports

Maronda Systems

4005 MARONDA WAY
Sanford, FL 32771
(407) 321-0064 Fax (407) 321-3913
TOMAS PONCE P.E. LICENSE #00050066
367 Medallion Pl. Chuluota, FL 32766

WARNING:
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Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See MIB-91 of TPI). For specific truss bracing requirements, contact building designer. (Truss Plate Institute, TPI is located at 581 D'Oroff Drive, Madison, Wisconsin 53719).
Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd., Eden, NC 27032

Eng Job: BBRFT	WO: BBRFT
Dwg:	TI: FB
Dgnr: TLY	Chk: 5/10/2007
TC Live 40.0 psf	Lbr DF: 1.00
TC Dead 10.0 psf	Plt DF: 1.00
BC Live 0.0 psf	O.C.: 2- 0- 0
BC Dead 5.0 psf	FBC-04/TPI-02/NDS-01
TOTAL 55.0 psf	Code:
	V5.4.18-6776

Design: Matrix Analysis

Profile Path: T:\TZE-LOK\Work\Jobs\BBRFT\BAYBURY RT FLOOR\FB.prx

	Point Locations		
1)	0-0-0	14)	18-1-4
2)	2-3-0	15)	20-2-0
3)	4-3-12	16)	20-10-0
4)	5-2-3	17)	21-6-4
5)	6-11-9	18)	23-0-4
6)	9-0-5	19)	25-7-0
7)	10-2-1	20)	27-9-0
8)	11-1-0	21)	29-9-4
9)	12-2-8	22)	29-9-4
10)	13-9-12	23)	27-9-0
11)	13-11-3	24)	23-0-4
12)	15-5-4	25)	21-6-4
13)	16-0-0	26)	18-1-4
14)	13-11-12	27)	16-0-0
15)	14-5-0	28)	13-11-12
16)	13-9-12	29)	14-5-0
17)	12-2-8	30)	13-9-12
18)	11-1-0	31)	12-2-8
19)	10-2-1	32)	11-1-0
20)	9-0-5	33)	10-2-1
21)	8-11-9	34)	6-11-9
22)	5-2-3	35)	5-2-3
23)	4-3-12	36)	4-3-12
24)	3-0-0	37)	0-0-0

Typical Panel = 24"

The diagram shows a horizontal line representing a panel. Below the line, there are labels: '1' at the left end, '2' at the first tick mark, '3' at the second tick mark, '4' at the third tick mark, and '5' at the right end. Above the line, there are labels: '6-0-14' at the first tick mark, '6-0-14' at the second tick mark, and '6-0-14' at the third tick mark. There are also labels '0-3-8' and '1.5x3' on the left side, and '[3x4]' on the right side.

Technical drawing of a window frame. The drawing shows a cross-section of the frame with various dimensions and material specifications. The top horizontal member is labeled "1.5x3" and "3x4". The vertical member on the right is labeled "3x6". The bottom horizontal member is labeled "3x6". The left vertical member is labeled "1.5x3". The drawing also shows a "MARK FRONT" label with a dimension of "1'-4 0\"". A dimension of "1'-7-14\" is shown for the width of the frame. A dimension of "762# 1.50\" is shown for the height of the frame. A dimension of "34" is shown for the width of the frame. A dimension of "36 35" is shown for the height of the frame. A dimension of "37" is shown for the width of the frame.

EXCEPT AS SHOWN PLATES ARE MITEK MT20

WARNING:
READ ALL NOTES

Maronda Systems

4005 MARONDA WAY
Sanford, FL 32771
(407) 321-0064 Fax (407) 321-3913

TOMAS PONCE P.E. LICENSE #00030068
367 Medallion Pl. Chuluota, FL 32766

Design: Matrix Analysis

DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of failure or incompletion of information, specifications and/or designs furnished to the information designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accept no responsibility or assumes no control with regard to fabrication, handling, shipment and installation of this component in accordance with ANSIRP 1-1993 and NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the building designer, the design loadings shown must be compared with the design loadings are in agreement with the local building code requirements for special applied loads. Unless shown, must be designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" e.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for each verification. Any discrepancies are to be pointed out to the fabricator before fabrication. Plates are to be welded to the web of the truss members. Members shall be cut for light fitting wood framing. Connector plates shall be located both ends of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 5/4 plate is 5" wide x 4" long. A 6/8 plate is 6" wide x 8" long. Slots (holes) run parallel to the plate length specified. Double cuts on web members shall meet at the centroid of the web unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant (treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSIRP 1-1993

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during handling and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary bracing for trusses requires not temporary bracing during installation. Trusses are to be braced and supported during installation. The supervision of erection of trusses shall be under the direct or indirect supervision of the fabricator. Installation of trusses shall be in accordance with the design of the truss. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erection shall be applied to trusses until after all fastening and bracing is completed.

TC: 4x 2 SP #2
BC: 4x 2 SP #2
WB: (2) 4x 2 SP #1 D
4x 2 SP #3
4x 2 SP #3

11-16
1-17, 2-17, 2-15
3-15, 4-15, 4-14, 4-13, 5-13, 6-13, 6-12
7-12, 9-12, 9-11, 10-11, 10-0

() or () Plate(s) OFFSET from joint center.
The Joint Detail Report must be included with any submittal, inspection and/or fabrication documentation.

This truss is designed in accordance with the 2004 Florida Building Code section 2306.1 and referenced standards, U.N.O.

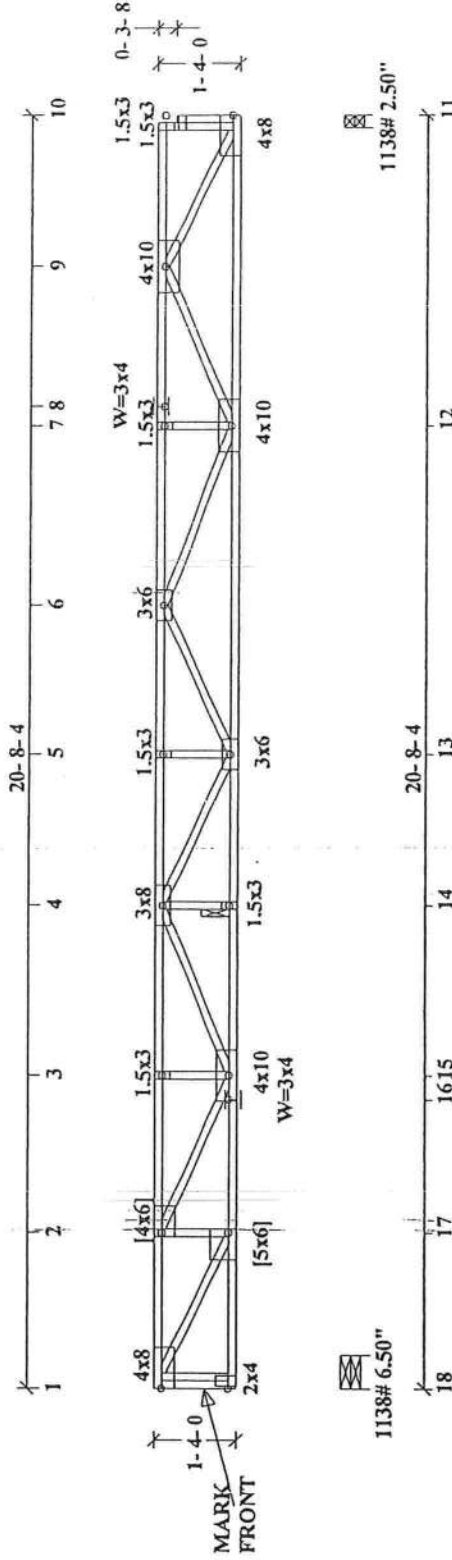
2x6 continuous strongback. Attach to each truss with 3-3.5"x 0.135" nails. Splice strongback on truss vertical.

ALL COMPRESSION Chords are assumed to be continuously braced unless noted otherwise. In-Plant Quality Assurance with Cq = 1.11 In-Plant Quality Assurance, per sec. 3.2.4 of TPI-1-02, for joint(s): 1, 9, 11, 12, 17

-----GLOBAL MAX DEFLECTIONS-----
I.L. TL

I.Span in./Ratio in./Ratio Jnt(s).
Horiz. -0.40/601 -0.62/384 5-4
Long term deflection factor = 1.50

Typical Panel = 30"



EXCEPT AS SHOWN PLATES ARE MITEK MT20

Scale = 0.3125

Maronda Systems

4005 MARONDA WAY

Sanford, FL 32771

(407) 321-0064 Fax (407) 321-3913

TOMAS PONCE P.E. LICENSE #0030068

367 Medallion Pl, Chuluota, FL 32766

Design: Matrix Analysis

WARNING:

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING:

Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HIB-91 of TPI) For specific truss bracing requirements, contact building designer (Truss Plate Institute, TPI is located at 583 O'Neale Drive, Madison, Wisconsin 53719).

Component Engineering by: Truss Engineering Co., P.A., 418 Soundside Rd. Edison, NC 27932

Eng Job: BBRFT
Dwg:

Dsgnr: TLY Chk:

TC Live 40.0 psf
TC Dead 10.0 psf
BC Live 0.0 psf
BC Dead 5.0 psf
TOTAL 55.0 psf

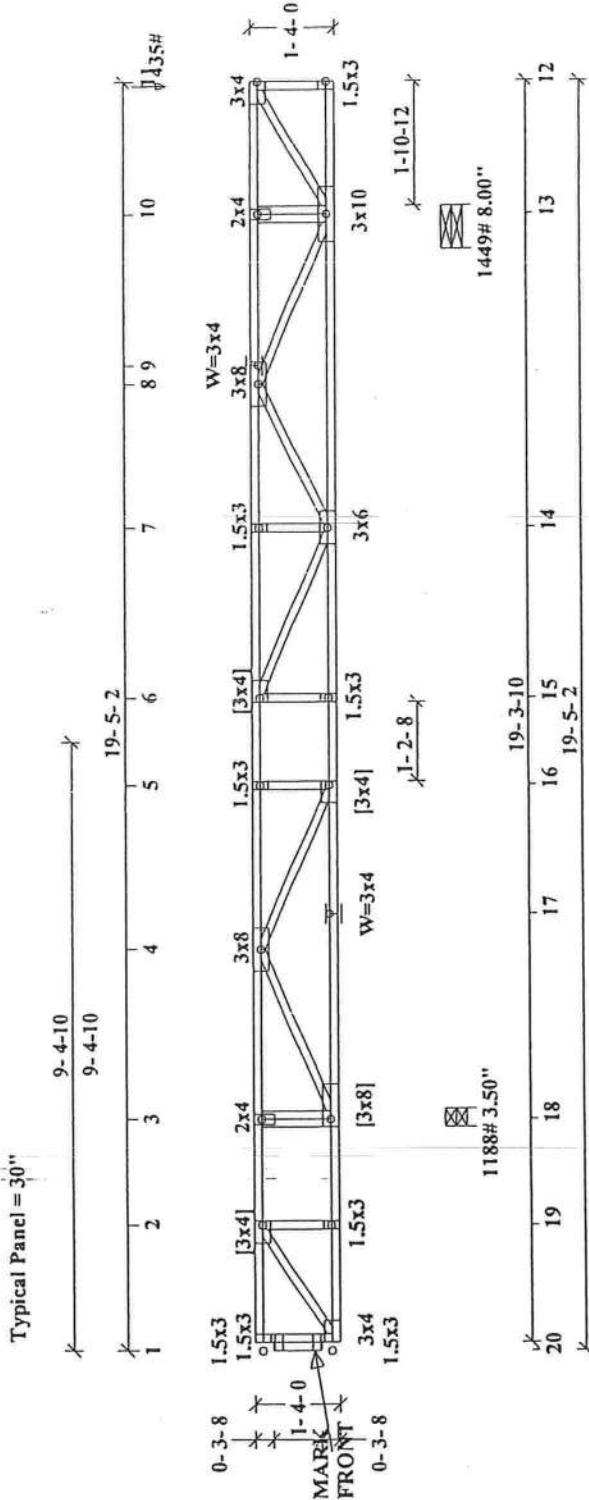
WO: BBRFT
TI: FD

S/10/2007

Lbr DF: 1.00
Plt DF: 1.00
O.C.: 2-0-0
FBC-04/TPI-02/NDS-01
Code:
v5.4.18-6716

Profile Path: T:\TEE-LOK\Work\Jobs\BBRFT\BAYBURY RT FLOOR.FD.prx

LF	To
000	19-5-3
140	19-5-3
5	



CANT: 1-10-12

Scale = 0.3318

00 0

0 00/00 01

10-SDN/70-

Profile Path: T:\TEE-LOK\Work\Jobs\BBRFT\HAYBURY RT FLOOR\FE.prx

Profile Path: T:\TEE-LOK\Work\Jobs\BBRFT\HAYBURY RT FLOOR\FE.prx

DESIGN INFORMATION

This design is for an individual building component and is based on information provided by the client. The designer disclaims any responsibility for damages as a result of failure or incorrect information, specifications and/or design furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accepts no responsibility or exercises no control with regard to fabrication, handling, shipment and installation of trusses. This truss has been designed as an independent building component in accordance with ANSI/TPI 1-1993 and NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the building designer, the design loadings shown must be with the local building codes, local climatic records for wind or snow loads, specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knotholes, knots or distorted grain. Members shall be cut for lightening webs to avoid the joint unless otherwise shown. A 5/8" plate is 1/2" wide x 4" long. A 6/8" plate is 1/2" wide x 6" long. Slots shall run parallel to the plate length specified. Double end on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1993.

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during bending and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral bracing is essential and shall be installed by others. Certified building inspectors and/or engineers are to inspect and certify such temporary bracing during installation. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erection shall be applied to trusses until after all fastening and bracing is completed.

- TC: 4x 2 SP #1 D
BC: 4x 2 SP #1 D
WB: (2) 4x 2 SP #3
2-15, 2-14, 3-14
4-14, 4-13, 4-12, 5-12, 6-11, 7-11, 7-10
7-9, 8-9, 0-0
4x 2 SP #1 D 0-0

ALL COMPRESSION Chords are assumed to be continuously braced unless noted otherwise. Partial Double Chord to be same size and grade as chord to which it is attached. This truss is designed in accordance with the 2004 Florida Building Code section 2306.1 and referenced standards, U.N.O.

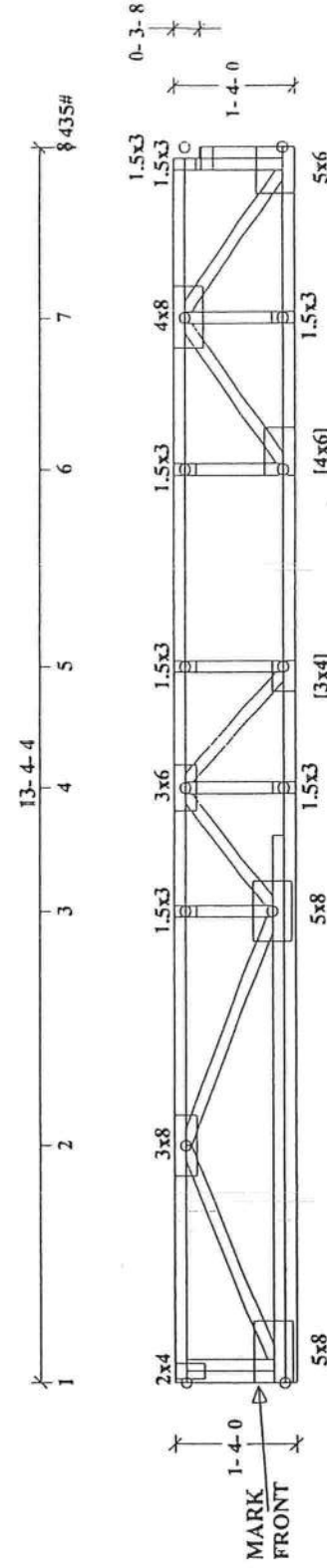
[] or [()] : Plate(s) OFFSET from joint center. The Joint Detail Report must be included with any submittal, inspection, and/or fabrication documentation. This hip jack truss has been designed to carry open-ended jacks. In-Plant Quality Assurance with Cq = 1.11. In-Plant Quality Assurance, per sec. 3.2.4 of TPI-1-02, for joint(s): 2, 7, 9, 11

---MAX. REACTIONS PER BEARING LOCATION---
X-Loc Bset Vert Horiz Uplift Y Type
0- 3- 6 1 678 0 0 B Pin
13- 2- 8 1 1808 0 0 B H Roll
PROVIDE UPLIFT CONNECTION PER SCHEDULE

- Joint Locations
1) 0- 0- 0 6) 9-11- 0 11) 9-11- 0
2) 2- 6-12 7) 11- 6- 8 12) 7- 9- 8
3) 5- 1- 8 8) 13- 4- 4 13) 6- 5-14
4) 6- 5-14 9) 13- 4- 4 14) 5- 1- 8
5) 7- 9- 8 10) 11- 6- 8 15) 0- 0- 0

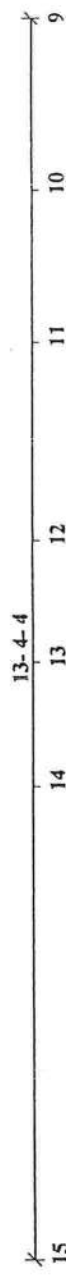
---TOTAL DESIGN LOADS---
Uniform PLF From PLF To
TC Vert L+D 0 0- 0- 0 -279 13- 4- 4
BC Vert L+D 0 0- 0- 0 -27 13- 4- 4
Concentrated LBS Location
TC Vert L+D -435 13- 4- 4
---GLOBAL MAX DEFLECTIONS---
LL TL
in./Ratio in./Ratio Jnt(s)
I. Span -0.19/790 -0.30/505 12-13
Horiz. 0.03 0.05 NA
Long term deflection factor = 1.50

Typical Panel = 30"



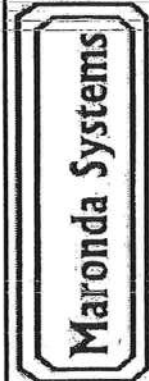
678# 6.75"

1808# 3.50"



EXCEPT AS SHOWN PLATES ARE MiTek MT20

Scale = 0.4720



4005 MARONDA WAY
Sanford, FL 32771
(407) 321-0064 Fax (407) 321-3913
TOMAS PONCE P.E. LICENSE #0050068
367 Medallion Pl, Chuluota, FL 32766

WARNING:
READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO
ERECTING CONTRACTOR. BRACING WARNING:
Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to maintain buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See TPI-91 of TPI) For specific mass bracing requirements, contact building designer. (Truss Plate Institute, TPI is located at 583 D'Oroville Drive, Madison, Wisconsin 53719).
Component Engineering by: Truss Engineering Co., P.A., 418 Sandside Rd, Eden, NC 27932

Eng Job: BBRFT
Dwg: TI: FG
Dsgnr: TLY Chk: 5/10/2007

TC Live 40.0 psf
TC Dead 10.0 psf
BC Live 0.0 psf
BC Dead 5.0 psf
TOTAL 55.0 psf

WO: BBRFT
TI: FG
5/10/2007

Lbr DF: 1.00
Plt DF: 1.00
O.C.: 2- 0- 0
FBC-04/TPI-02/NDS-01
Code:
v5.4.18-6725

Design: Matrix Analysis

Profile Path: T:\TEE-LOK\Work\Jobs\BBRFT\BAYBURY RT FLOOR\FG.prx

DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages to a building or its contents resulting from the use of this design and/or design furnished to the trust designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accepts no responsibility or exercises no control with regard to fabrication, handling, shipment and installation of trusses.

This truss has been designed as an individual building component in accordance with ANSI/TPI 1-1995 and NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the building designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chord in tension is not fully braced laterally by a properly applied rigid ceiling, they should be braced to a maximum spacing of 10 ft. Compression chords shall be maximum spacing of 10 ft. unless otherwise specified. Steel meeting ASTM A 433, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knotholes, knots or distorted grain. Members shall be cut for tight fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 3x4 plate is 3" wide x 4" long. A 6x4 plate is 6" wide x 4" long. Steel holes run parallel to the plate length specified. Double cuts on web members shall meet at the centroid of the truss unless otherwise shown. Connector plates shall be cut to fit the truss and the fasteners shall be installed in the correct position and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995.

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during bending and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid tipping and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Trusses shall not be subjected to loads greater than the design loads and shall not be applied to trusses at any time. No loads other than the weight of the trusses shall be applied to trusses until after all fastening and bracing is completed.

TC: 4x 2 SP #2

BC: 4x 2 SP #2

WB: 4x 2 SP #3

ALL COMPRESSION CHORDS ARE ASSUMED TO BE CONTINUOUSLY BRACED UNLESS NOTED OTHERWISE.

-----GLOBAL MAX DEFLECTION-----

LL

in./Ratio

in./Ratio

Jnt(s)

2-1

NA

0.01

0.01

0.01

0.01

0.01

0.01

0.01

0.01

I. Span

in./Ratio

in./Ratio

Jnt(s)

2-1

NA

0.01

0.01

0.01

0.01

0.01

0.01

0.01

0.01

0.01

0.01

0.01

0.01

0.01

Unbalanced load case(s) have been checked.

MULTIPLE LOADCASES -- This design is the composite result of multiple loadcases.

In-Plant Quality Assurance with Cq= 1.11

---MAX. REACTIONS PER BEARING LOCATION---

X-Loc

BSet

Vart

Horiz

Uplift

Type

0- 1- 0

1 346

0 0 B

6-10- 4

1 346

0 0 B

H Roll

PROVIDE UPLIFT CONNECTION PER SCHEDULE

Typical Panel = 30"

3- 5-10

3- 5-10

3- 5-10

3- 5-10

3- 5-10

3- 5-10

3- 5-10

3- 5-10

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3- 5-10

Unbalanced load case(s) have been checked.

MULTIPLE LOADCASES -- This design is the composite result of multiple loadcases.

In-Plant Quality Assurance with Cq= 1.11

---MAX. REACTIONS PER BEARING LOCATION---

X-Loc

BSet

Vart

Horiz

Uplift

Type

0- 1- 0

1 346

0 0 B

6-10- 4

1 346

0 0 B

H Roll

PROVIDE UPLIFT CONNECTION PER SCHEDULE

Typical Panel = 30"

3- 5-10

3- 5-10

3- 5-10

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3- 5-10

Joint Locations

1) 0- 7- 0

2) 2-11-12

3) 3-11- 8

4) 6- 7-12

5) 6- 7-12

6) 3-11- 8

7) 2-11-12

8) 0- 7- 0

9) 0- 7- 0

10) 0- 7- 0

11) 0- 7- 0

12) 0- 7- 0

13) 0- 7- 0

14) 0- 7- 0

15) 0- 7- 0

16) 0- 7- 0

17) 0- 7- 0

18) 0- 7- 0

19) 0- 7- 0

20) 0- 7- 0

21) 0- 7- 0

22) 0- 7- 0

23) 0- 7- 0

24) 0- 7- 0

25) 0- 7- 0

26) 0- 7- 0

27) 0- 7- 0

28) 0- 7- 0

29) 0- 7- 0

30) 0- 7- 0

31) 0- 7- 0

32) 0- 7- 0

33) 0- 7- 0

34) 0- 7- 0

35) 0- 7- 0

36) 0- 7- 0

37) 0- 7- 0

38) 0- 7- 0

39) 0- 7- 0

40) 0- 7- 0

41) 0- 7- 0

42) 0- 7- 0

43) 0- 7- 0

44) 0- 7- 0

45) 0- 7- 0

46) 0- 7- 0

47) 0- 7- 0

48) 0- 7- 0

49) 0- 7- 0

50) 0- 7- 0

51) 0- 7- 0

52) 0- 7- 0

53) 0- 7- 0

54) 0- 7- 0

55) 0- 7- 0

56) 0- 7- 0

57) 0- 7- 0

58) 0- 7- 0

59) 0- 7- 0

60) 0- 7- 0

61) 0- 7- 0

62) 0- 7- 0

63) 0- 7- 0

64) 0- 7- 0

65) 0- 7- 0

66) 0- 7- 0

67) 0- 7- 0

68) 0- 7- 0

Joint Locations

1) 0- 7- 0

2) 2-11-12

3) 3-11- 8

4) 6- 7-12

5) 6- 7-12

6) 3-11- 8

7) 2-11-12

8) 0- 7- 0

9) 0- 7- 0

10) 0- 7- 0

11) 0- 7- 0

12) 0- 7- 0

13) 0- 7- 0

14) 0- 7- 0

15) 0- 7- 0

16) 0- 7- 0

17) 0- 7- 0

18) 0- 7- 0

19) 0- 7- 0

20) 0- 7- 0

21) 0- 7- 0

22) 0- 7- 0

23) 0- 7- 0

24) 0- 7- 0

25) 0- 7- 0

26) 0- 7- 0

27) 0- 7- 0

28) 0- 7- 0

29) 0- 7- 0

30) 0- 7- 0

31) 0- 7- 0

32) 0- 7- 0

33) 0- 7- 0

34) 0- 7- 0

35) 0- 7- 0

36) 0- 7- 0

37) 0- 7- 0

38) 0- 7- 0

39) 0- 7- 0

40) 0- 7- 0

41) 0- 7- 0

42) 0- 7- 0

43) 0- 7- 0

44) 0- 7- 0

45) 0- 7- 0

46) 0- 7- 0

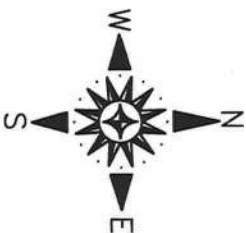
47) 0- 7- 0

48) 0- 7- 0

49) 0- 7- 0

50) 0- 7- 0

LEGAL DESCRIPTION:
LOT SEVEN (7) OF "TIMBERLANDS" AS PER PLAT THEREOF, AS RECORDED IN PLAT BOOK "D", PAGE 2 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.



BOUNDARY SURVEY
IN SECTION 10, TOWNSHIP 4 SOUTH, RANGE 16 EAST, COLUMBIA COUNTY, FLORIDA



26624

CERTIFIED TO:

1) MARONDA HOMES

BEARING NOTE:

BEARINGS SHOWN HEREON ARE REFERRED TO AN ASSUMED VALUE OF N. 89°43'36" W., FOR THE NORTH RIGHT-OF-WAY LINE OF SW TIMBER RIDGE DRIVE.

BUILDING SETBACK NOTE:

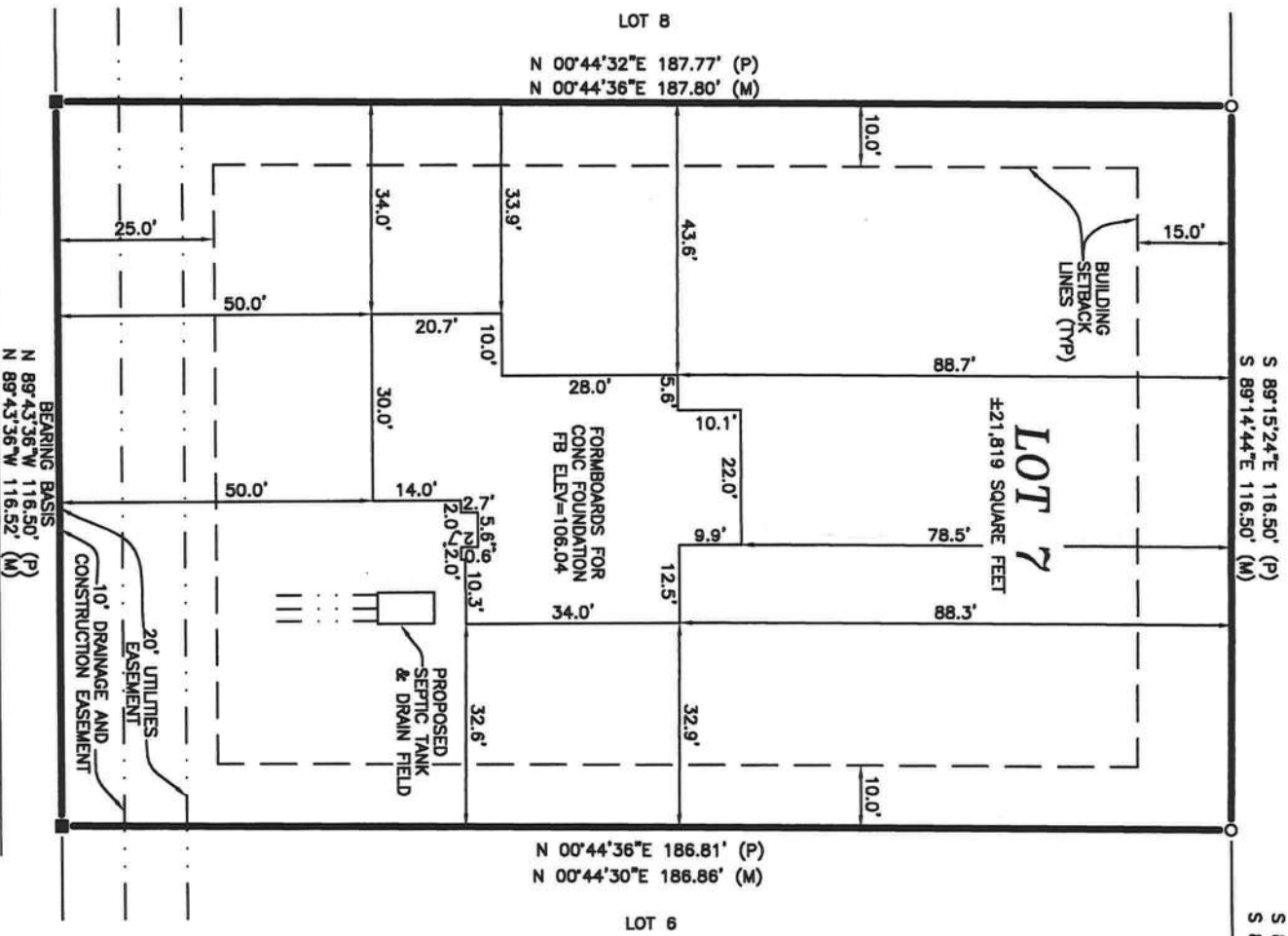
BUILDING SETBACK INFORMATION FOR "TIMBERLANDS" IS AS FOLLOWS: FRONT 25', REAR 15', SIDE 10'

FLOOD NOTE:

IN THE OPINION OF THIS SURVEYOR, ACCORDING TO THE NATIONAL FLOOD INSURANCE PROGRAM, FLOOD INSURANCE RATE MAP COMMUNITY PANEL NO. 120070-0175-B, DATED 1-8-88, THIS PROPERTY IS IN FLOOD ZONE "X" WHICH IS AN AREA DETERMINED TO BE OUTSIDE 500-YEAR FLOOD PLAIN, AS SCALED FROM SAID MAP. INFORMATION FROM THE FEDERAL EMERGENCY MANAGEMENT AGENCY (FEMA) FLOOD INSURANCE RATE MAPS, SHOWN ON THIS MAP, WAS CURRENT AS OF THE REFERENCED DATE. MAP REVISIONS AND AMENDMENTS ARE PERIODICALLY MADE BY LETTER AND MAY NOT BE REFLECTED ON THE MOST CURRENT MAP.

SURVEYOR NOTES:

- 1) TO THE BEST OF MY KNOWLEDGE, THERE ARE NO ENCROACHMENTS, BOUNDARY LINE DISPUTES, EASEMENTS, OR CLAIMS OF EASEMENTS, OTHER THAN ARE DEPICTED ON THIS DRAWING.
- 2) ALL UTILITIES AND OR IMPROVEMENTS, IF ANY, MAY NOT BE SHOWN ON THIS DRAWING.
- 3) IN THE OPINION OF THIS SURVEYOR THE BOUNDARY SHOWN HEREON BEST REPRESENTS THE LOCATION OF THE SUBJECT PROPERTY IN RELATION TO THE DESCRIPTION AND THOSE PROPERTY CORNERS FOUND TO BE ACCEPTABLE TO THIS SURVEYOR.
- 4) BUILDING SETBACK LINES DEPICTED HEREON ARE SHOWN AS PER THE RECORD PLAT, BUT ARE SUBJECT TO CHANGE. PRIOR TO ANY NEW CONSTRUCTION, THE APPROPRIATE GOVERNING AUTHORITY SHOULD BE CONTACTED FOR THE CURRENT SETBACK REQUIREMENTS.
- 5) THIS MAP OF SURVEY REFLECTS CONDITIONS LOCATED AS OF THE DATE OF FIELD WORK COMPLETION (SEE TITLE BLOCK).
- 6) AREAS OF ENVIRONMENTAL CONCERN HAVE NOT BEEN LOCATED BY THIS SURVEYOR, UNLESS OTHERWISE DEPICTED HEREON.



SW TIMBER RIDGE DRIVE

60' RIGHT-OF-WAY
±20' ASPHALT

EDGE OF PAVEMENT

TITLE NOTE:

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LEGEND:

- = FOUND 1/2" REBAR & CAP
- = FOUND 4" X 4" CONC. MON.
- NO IDENTIFICATION
- X = FOUND NAIL & DISK

ABBREVIATIONS:

- A = ARC LENGTH
- A/C = AIR CONDITIONER
- ALUM = ALUMINUM
- ASPH = ASPHALT
- AVE = AVENUE
- BLK = BLOCK
- BLVD = BOULEVARD
- BM = BENCHMARK
- C = CALCULATED FROM MEASURED
- CATV = CABLE TELEVISION
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- CMP = CORRUGATED METAL PIPE
- CONC = CONCRETE
- DOT = DEPARTMENT OF TRANSPORTATION
- ELEC = ELECTRIC
- ELEV = ELEVATION
- FND = FOUND
- FNC = FENCE
- (I) = PER LEGAL DESCRIPTION
- LB = LICENSED SURVEYOR BUSINESS
- (M) = FIELD MEASURED
- MH = MANHOLE
- P = PLAT
- PB = PLAT BOOK
- P.U.E. = PUBLIC UTILITIES EASEMENT
- R/W = RIGHT OF WAY

CERTIFICATE OF SURVEYOR:

NOT VALID WITHOUT THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER. ADDITIONS OR DELETIONS TO THIS MAP BY ANYONE OTHER THAN THIS SURVEYOR IS PROHIBITED.

I HEREBY CERTIFY THAT THE SURVEY DATA SHOWN HEREON, IS A TRUE AND CORRECT REPRESENTATION OF A SURVEY PERFORMED UNDER MY SUPERVISION OF THE HEREON DESCRIBED PROPERTY, AND IT MEETS THE MINIMUM TECHNICAL STANDARDS AS SET FORTH BY THE FLORIDA BOARD OF LAND SURVEYORS, PURSUANT TO SECTION 472.027, FLORIDA STATUTES, AND CHAPTER 61G17-6, FLORIDA ADMINISTRATIVE CODE.

BY: JAMES E. BRINKMAN, F.S.M. - F.L.A. CERT# 5582
DATE: 2/5/08



BRINKMAN SURVEYING & MAPPING INC.

4607 NW 6th STREET SUITE C, GAINESVILLE, FL 32609
PHONE: (352) 374-7707 FAX: (352) 374-8757

SCALE: 1" = 30'

DATE: 2/4/08

"THE BENCHMARK IN QUALITY SERVICE"

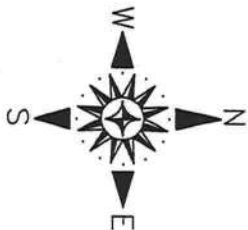
FIELD WORK COMPLETED 1/31/08

FIELDBOOK 95, PAGE 48

PREPARED FOR: MARONDA HOMES

DRAWN BY: ZL
CHECKED BY: J.B.
DRAWING NUMBER 303-07

LEGAL DESCRIPTION:
LOT SEVEN (7) OF "TIMBERLANDS" AS PER PLAT THEREOF, AS RECORDED IN PLAT BOOK "D", PAGE 2 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.



PROPOSED BUILDING LAYOUT

IN SECTION 10, TOWNSHIP 4 SOUTH, RANGE 16 EAST, COLUMBIA COUNTY, FLORIDA



CERTIFIED TO:

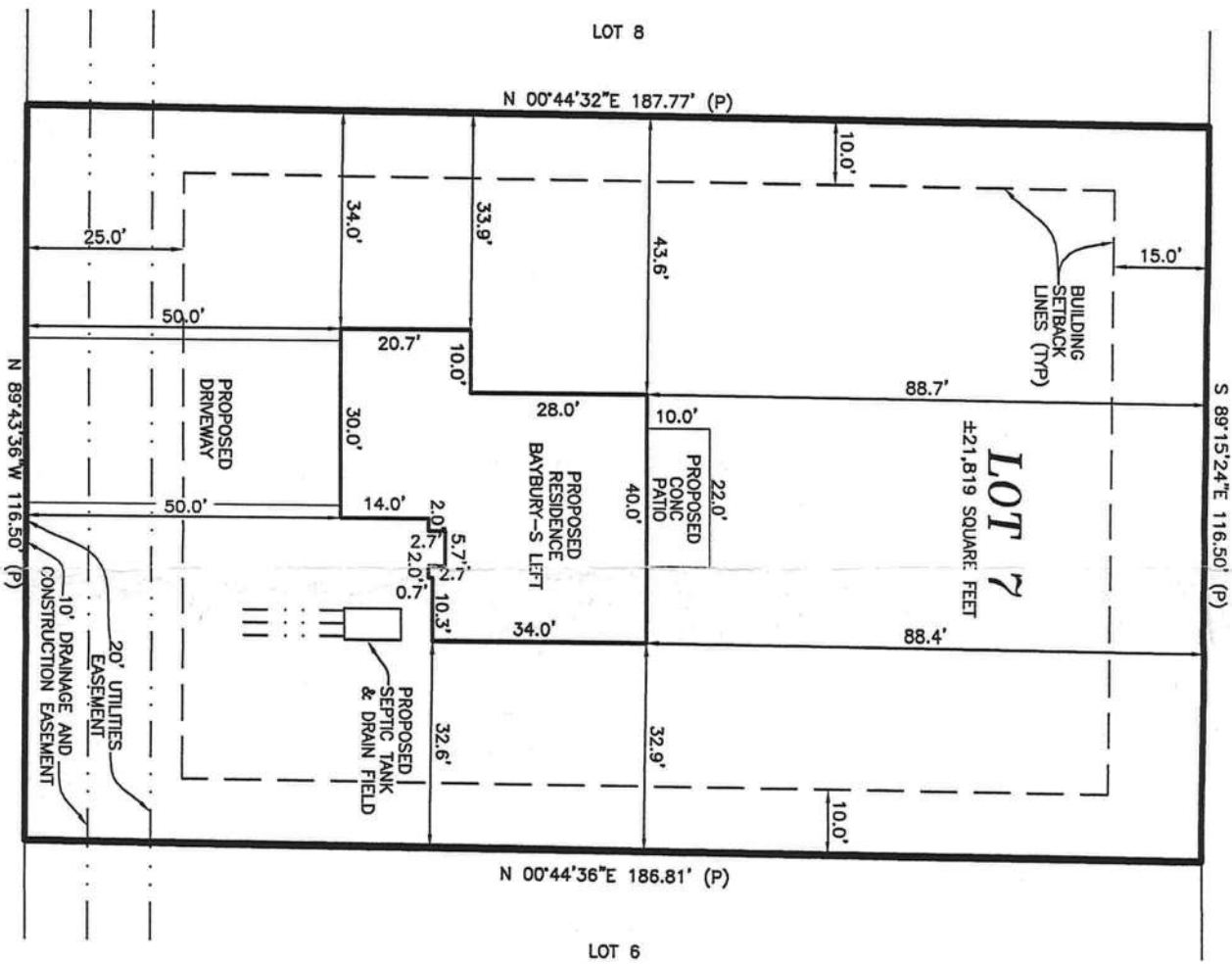
- 1) MARONDA HOMES

BUILDING SETBACK NOTE:

BUILDING SETBACK INFORMATION FOR "TIMBERLANDS" IS AS FOLLOWS: FRONT 25', REAR 15', SIDE 10'

SURVEYOR NOTES:

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FLOOD NOTE:

IN THE OPINION OF THIS SURVEYOR, ACCORDING TO THE NATIONAL FLOOD INSURANCE PROGRAM, FLOOD INSURANCE RATE MAP COMMUNITY PANEL NO. 120070-0175-B, DATED 1-6-88, THIS PROPERTY IS IN FLOOD ZONE "X" WHICH IS AN AREA DETERMINED TO BE OUTSIDE 500-YEAR FLOOD PLAIN, AS SCALED FROM SAID MAP. INFORMATION FROM THE FEDERAL EMERGENCY MANAGEMENT AGENCY (FEMA) FLOOD INSURANCE RATE MAPS, SHOWN ON THIS MAP, WAS CURRENT AS OF THE REFERENCED DATE. MAP REVISIONS AND AMENDMENTS ARE PERIODICALLY MADE BY LETTER AND MAY NOT BE REFLECTED ON THE MOST CURRENT MAP.

SW TIMBER RIDGE DRIVE

60' RIGHT-OF-WAY

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LEGEND:

- = FOUND 1/2" REBAR NO IDENTIFICATION
- = FOUND 1/2" REBAR & CAP L.B. 6894
- = SET 1/2" REBAR & CAP L.B. 6894
- = FOUND 3/4" IRON PIPE
- = FOUND 4" X 4" CONC. MON. NO IDENTIFICATION
- = SET 4" X 4" CONC. MON. P.S.M. 5582
- ✕ = SET NAIL & DISK P.S.M. 5582
- ✕ = FOUND NAIL & DISK
- ⊠ = FOUND 6" X 6" S.R.D. R/W MON.
- ⊠ = CATV RISER
- ⊠ = TELEPHONE PEDESTAL
- ⊠ = WOOD POWER POLE

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JAMES E. BRINKMAN, PSM - FLA. CERT# 5582

DATE: 10/31/07



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4607 NW 6th STREET SUITE C, GAINESVILLE, FL. 32609

PHONE: (352) 374-7707 FAX: (352) 374-8757

SCALE: 1" = 30'

DATE: 10/15/07

"THE BENCHMARK IN QUALITY SERVICE"

DRAWN BY: ZL

CHECKED BY: J.B.

THIS IS NOT A BOUNDARY SURVEY

PREPARED FOR: MARONDA HOMES

DRAWING NUMBER 303-07

**HOMETEAM****PEST DEFENSE®**

26624

TREATMENT WORKORDER☐ Termite Baiting System w/Tubes-under-the slab☐ Treat Only☒ Tubes-under-the slab and Treat☐ Bora-Care

DATE CALLED IN:	2-4	DATE OF SCHEDULE:	2-5
TIME CALLED IN:		TIME SCHEDULE:	11-11:30

JOB NAME:	Maeanda	SUBDIVISION:	Timberland
JOB ADDRESS:	255 S.W. Timber Ridge Dr Lake City, FL		
BILLING NAME:		BILLING PHONE:	
BILLING ADDRESS:			
CALLER IN BY:	PHONE:	PERMIT NUMBER:	26624

LOT & MODEL NUMBER: Lot 7

DATE & TIME COMPLETED: 2-5-08

SQUARE FOOT: 2363 LINEAR FOOT: BLOCKVOIDS:

SLAB TYPE: Mono TYPE OF FILL: Sand

APPROX. DEPTH OF FOOTING: Outside: Inside:

☐ Addition ☒ Spot Treat ☐ Pool Addition ☐ Driveway☐ Final/Completion ☐ Other

PESTICIDE USED: Imaxx Pro TOTAL APPLIED: 120 gal

PERCENT (%) USED: .1% STICKER POSTED: yes

PRICE PER SQ. FT. =	TOTAL FOR P.T.	
	ADDITIONAL	
	TAX:	
/ /	TOTAL AMOUNT	\$

X TECHNICAL: CHAD

I hereby acknowledge the satisfactory completion of the above described work.