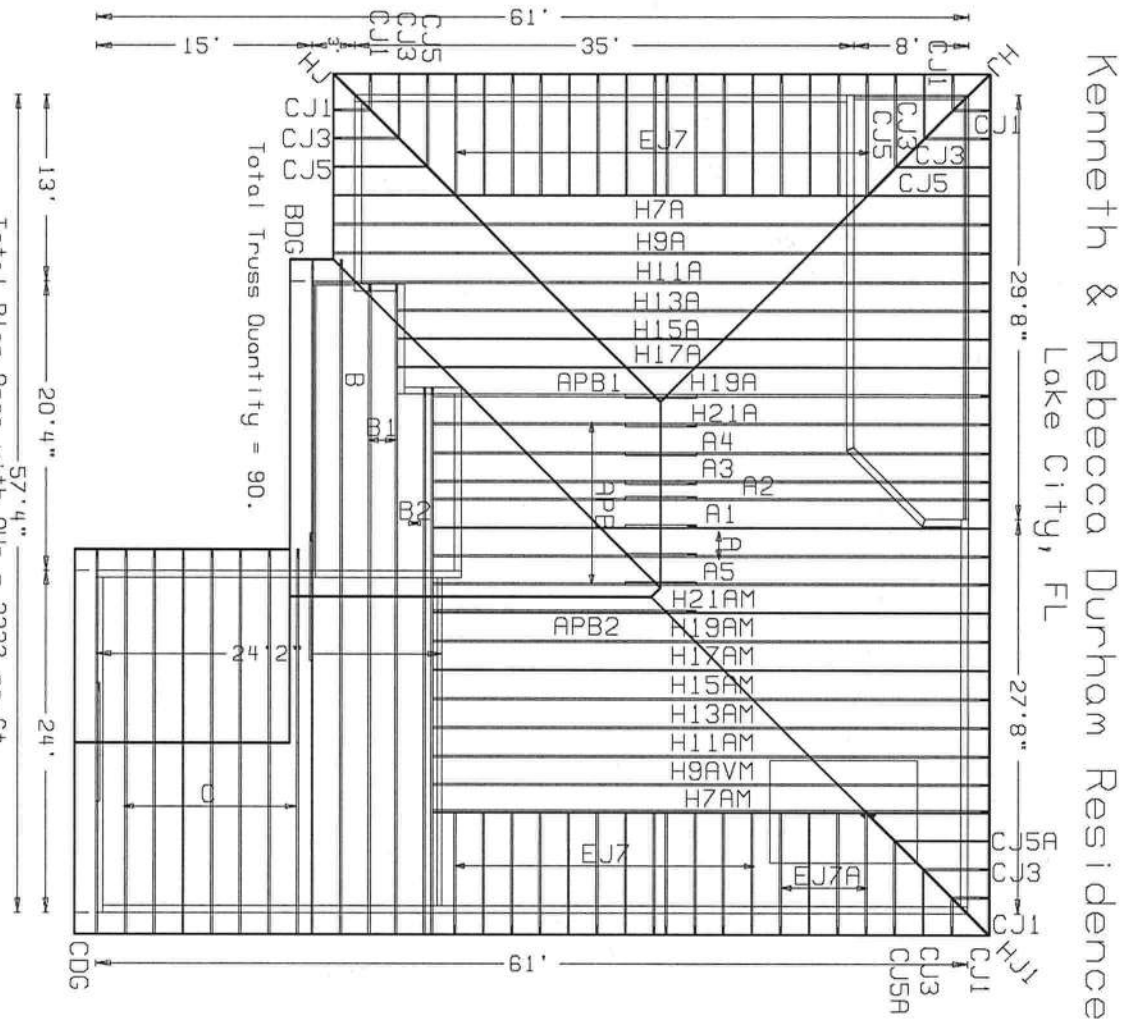


Kenneth & Rebecca Durham Residence

Lake City, FL

Total Truss Quantity = 90.

Total Plan Area with OHs = 3322 sq. ft

Roof Plane Sheathing Area = 3714 sq. ft

PAGE NO: 1 OF 1	JOB NO: 13-029	ADDRESS: JOB DESCRIPTION:: Doug Edgley /: Durham Res II JOB #: 13-029 DESIGNER: ColeMan Burlingame SALESMAN: Curt V Burlingame	: <Not Found>

ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 0 278
Florida Certificate of Product Approval # FL1999
 Document ID:1UT7487-Z0225161556

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
01/25/2013

Truss Fabricator:	Anderson Truss Company
Job Identification:	13-029--Doug Edgley Durham Res II -- Lake City, FL
Truss Count:	39
Model Code:	Florida Building Code 2010
Truss Criteria:	FBC2010Res/TPI-2007(STD)
Engineering Software:	Alpine Software,Version 10.03.
Structural Engineer of Record:	The identity of the structural EOR did not exist as of
Address:	the seal date per section 61G15-31.003(5a) of the FAC
Minimum Design Loads:	Roof - 37.0 PSF @ 1.25 Duration
	Floor - N/A
	Wind - 120 MPH ASCE 7-10 -Closed

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR487

Douglas Fleming
-Truss Design Engineer-

1950 Marley Drive
Haines City, FL 33844

Details: PB16010-BRCLBSUB-12015EC1-GBLLETIN-GABRST10-CNNAILSP-

#	Ref Description	Drawing#	Date
1	13425-A 37'6" Stepdown	13025023	01/25/13
2	13426-A1 37'6" Stepdow	13025024	01/25/13
3	13427-A2 37'6" Stepdow	13025025	01/25/13
4	13428-A3 37'6" Stepdow	13025026	01/25/13
5	13429-A4 37'6" Stepdow	13025027	01/25/13
6	13430-A5 37'6" Stepdow	13025028	01/25/13
7	13431-APB 4'11"14 Comm	13025029	01/25/13
8	13432-APB1 4'11"14 Ste	13025030	01/25/13
9	13433-APB2 18'5"15 Mon	13025031	01/25/13
10	13434--B 44'4" Common	13025032	01/25/13
11	13435--B1 44'2" Common	13025033	01/25/13
12	13436-B2 36'10" Common	13025034	01/25/13
13	13437--BDG 44'4" Gable	13025035	01/25/13
14	13438--C 24' Common	13025036	01/25/13
15	13439--CDG 24' Gable	13025037	01/25/13
16	13440--CJ1 1' Jack	13025038	01/25/13
17	13441--CJ3 3' Jack	13025039	01/25/13
18	13442--CJ5 5' Jack	13025040	01/25/13
19	13443--CJ5A 5' Jack	13025041	01/25/13
20	13444--EJ7 7' End Jack	13025042	01/25/13
21	13445-EJ7A 7' End Jack	13025043	01/25/13
22	13446-H11A 43' Stepdow	13025044	01/25/13
23	13447-H11AM 37'6" Step	13025045	01/25/13
24	13448-H13A 43' Stepdow	13025046	01/25/13
25	13449-H13AM 37'6" Step	13025047	01/25/13
26	13450-H15A 40' Stepdow	13025048	01/25/13
27	13451-H15AM 37'6" Step	13025049	01/25/13
28	13452-H17A 40' Stepdow	13025050	01/25/13
29	13453-H17AM 37'6" Step	13025051	01/25/13
30	13454-H19A 40' Stepdow	13025052	01/25/13
31	13455-H19AM 37'6" Step	13025053	01/25/13
32	13456-H21A 37'6" Stepd	13025054	01/25/13
33	13457-H21AM 37'6" Mono	13025055	01/25/13
34	13458-H7A 43' Stepdown	13025056	01/25/13
35	13459-H7AM 37'6" Stepd	13025057	01/25/13
36	13460-H9A 43' Stepdown	13025058	01/25/13
37	13461-H9AVM 37'6" Step	13025059	01/25/13
38	13462-HJ 9'10"13 Hip J	13025060	01/25/13
39	13463-HJ1 9'10"13 Hip	13025061	01/25/13

Revised these were replaced

COLUMBIA COUNTY BUILDING DEPARTMENT Received for FILE COPY Code Compliance PLANS EXAMINER

(13-029--Doug Edgley Durham Res II -- Lake City, FL - A 37'6" Stepdown Hip)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Right cantilever is exposed to wind

(b) 2x6 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

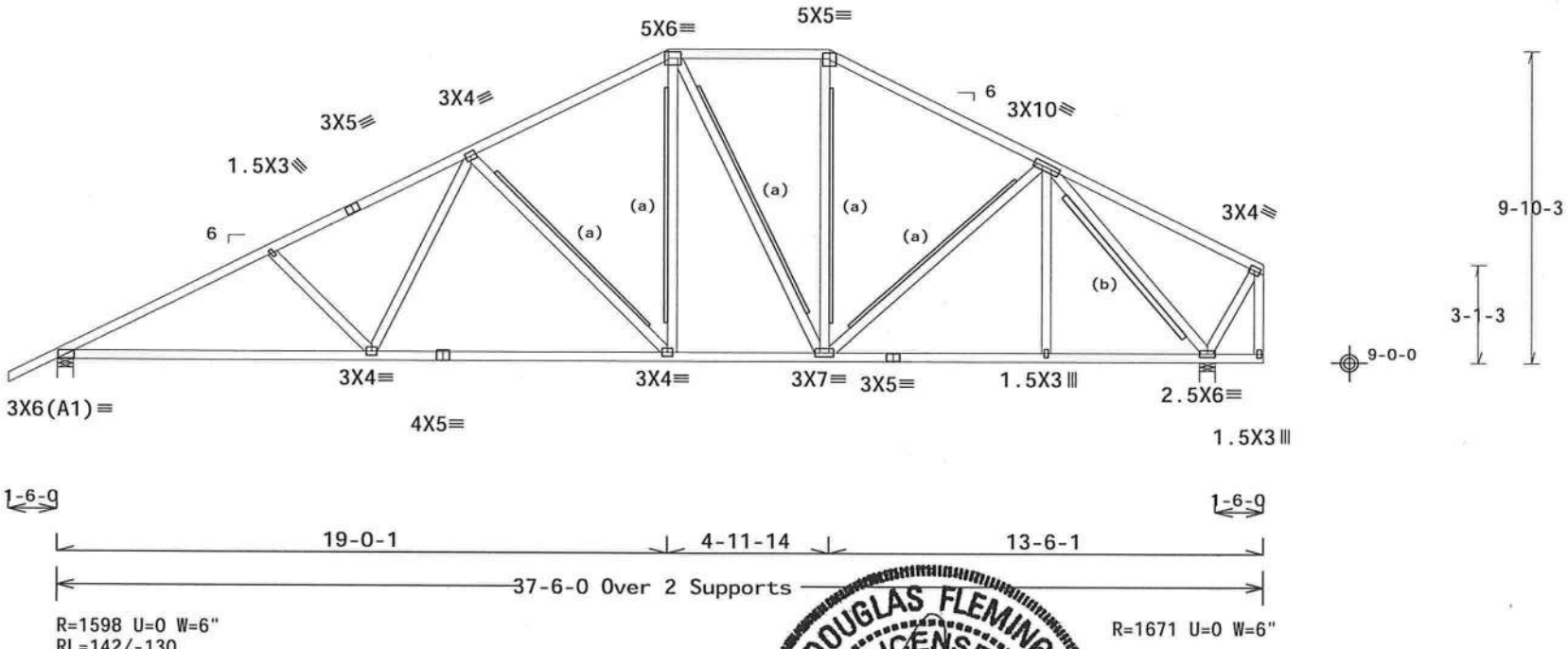
Right end vertical not exposed to wind pressure.

(a) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.



DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER 01/25/2013

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

10.03.11.0209.19 QTY: 4

PLT TYP. Wave

FL/-/4/-/-/R/-

Scale =.1875"/Ft.

TC LL	20.0 PSF	REF	R487-- 13425
TC DL	7.0 PSF	DATE	01/25/13
BC DL	10.0 PSF	DRW	HCUSR487 13025023
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	37.0 PSF	SEQN-	48801
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UT7487_Z02

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and WTCA) for general practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of web shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.

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(13-029--Doug Edgley Durham Res II -- Lake City, FL - A1 37'6" Stepdown Hip)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Right cantilever is exposed to wind

(b) 2x4 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

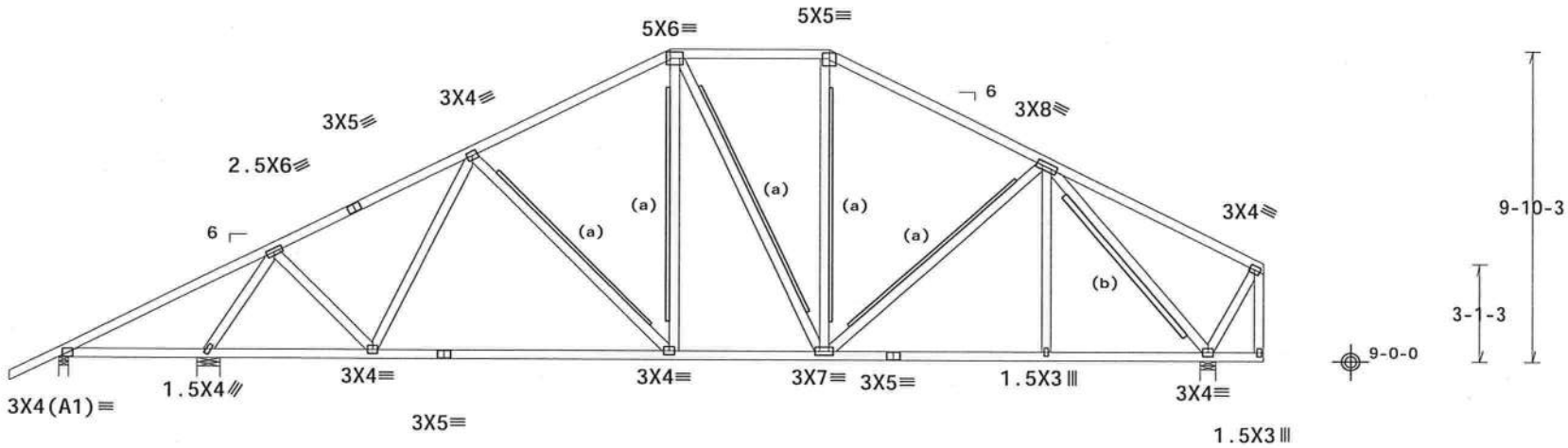
Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

(a) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

10.03.11.0209.19

QTY:

PLT TYP. Wave

FL/-/4/-/-/R/-

Scale =.1875"/Ft.

TC LL	20.0 PSF	REF	R487-- 13426
TC DL	7.0 PSF	DATE	01/25/13
BC DL	10.0 PSF	DRW	HCUSR487 13025024
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	37.0 PSF	SEQN-	48803
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UT7487_Z02

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

01/25/2013

(13-029--Doug Edgley Durham Res II -- Lake City, FL - A2 37'6" Stepdown Hip)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Right cantilever is exposed to wind

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

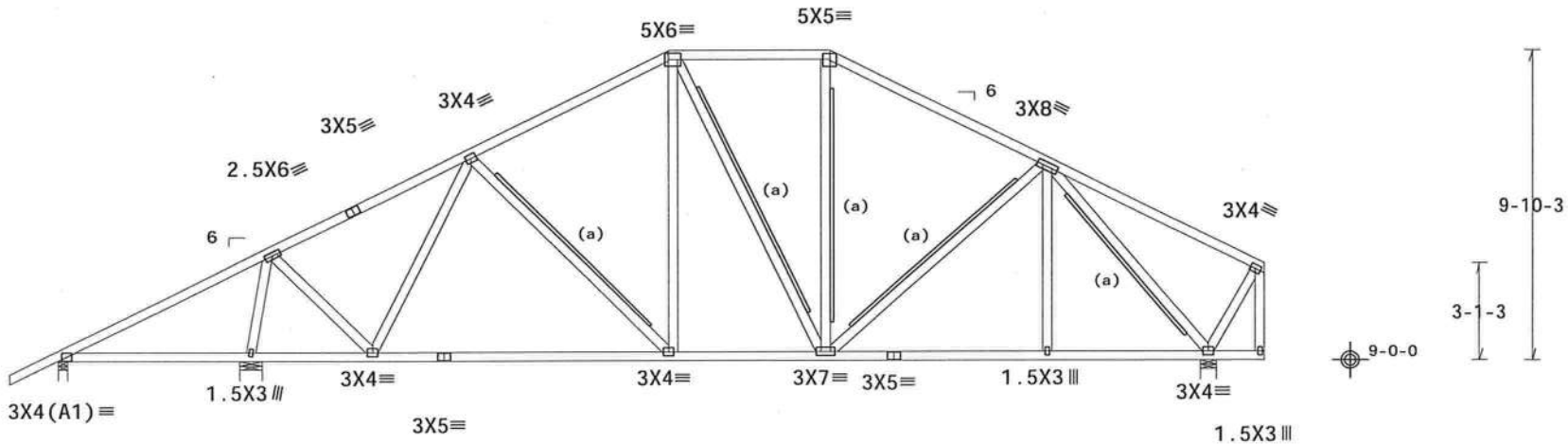
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

(a) 1x4 #3SRB SPF-S or better "T" brace, 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

10.03.11.0209.19

QTY:

FL/-/4/-/-/R/-

Scale =.1875"/Ft.

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER 01/25/2013

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ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R487-- 13427
TC DL	7.0 PSF	DATE	01/25/13
BC DL	10.0 PSF	DRW	HCUSR487 13025025
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	37.0 PSF	SEQN-	48791
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UT7487_Z02

(13-029--Doug Edgley Durham Res II -- Lake City, FL - A3 37'6" Stepdown Hip)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Right cantilever is exposed to wind

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

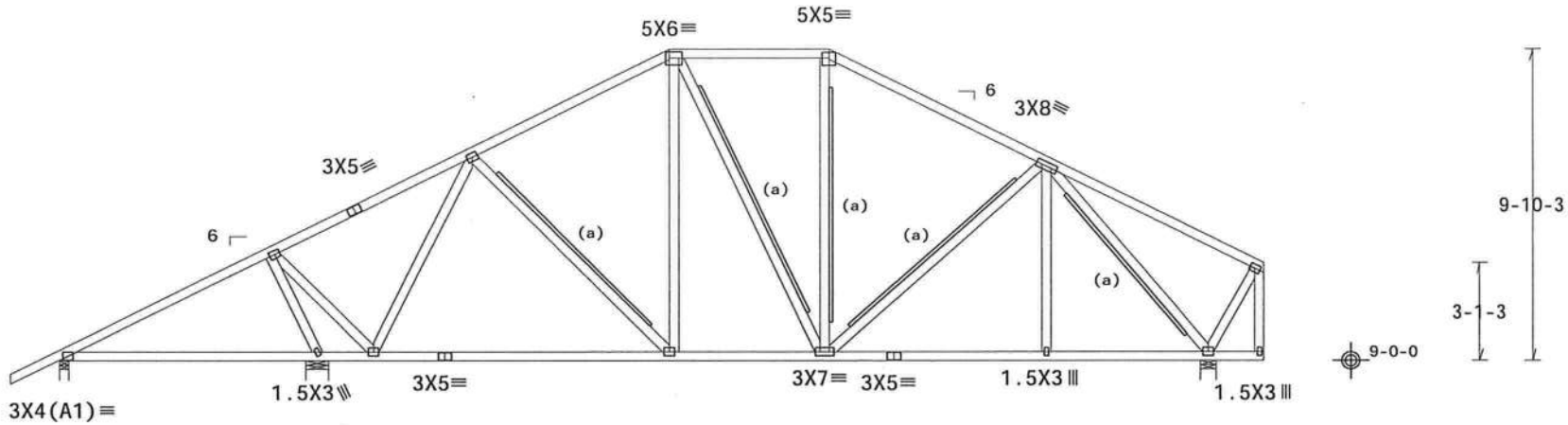
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

(a) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Note: All Plates Are 3X4 Except As Shown.

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 10.03.11.0200.19 QTY 1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER 01/25/2013

ALPINE
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Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R487-- 13428
TC DL	7.0 PSF	DATE	01/25/13
BC DL	10.0 PSF	DRW	HCUSR487 13025026
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	37.0 PSF	SEQN-	48789
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UT7487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(13-029--Doug Edgley Durham Res II -- Lake City, FL - A4 37'6" Stepdown Hip)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Right cantilever is exposed to wind

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

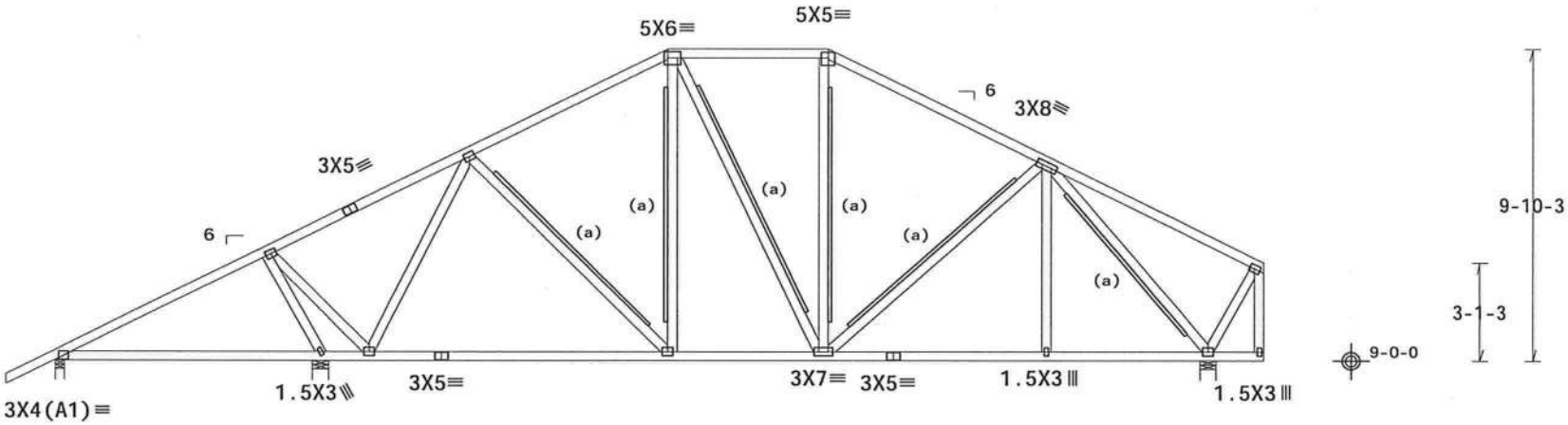
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

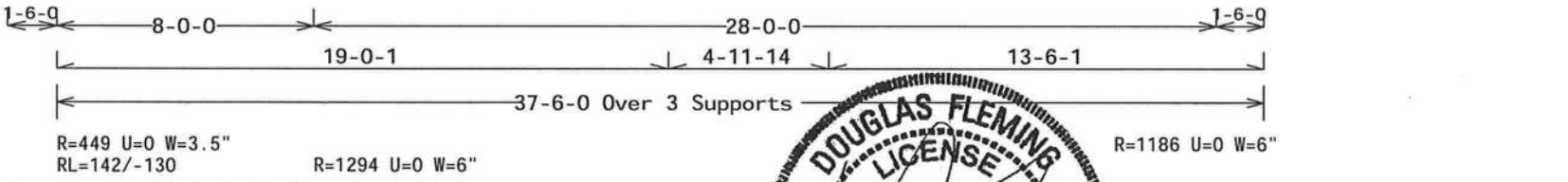
Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

(a) 1x4 #3SRB SPF-S or better "T" brace, 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





Note: All Plates Are 3X4 Except As Shown.

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 10.03.11.0209.19 QTY 1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER 01/25/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R487-- 13429
TC DL	7.0 PSF	DATE	01/25/13
BC DL	10.0 PSF	DRW	HCUSR487 13025027
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	37.0 PSF	SEQN-	48787
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UT7487_Z02

(13-029--Doug Edgley Durham Res II -- Lake City, FL - A5 37'6" Stepdown Hip)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Right cantilever is exposed to wind

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

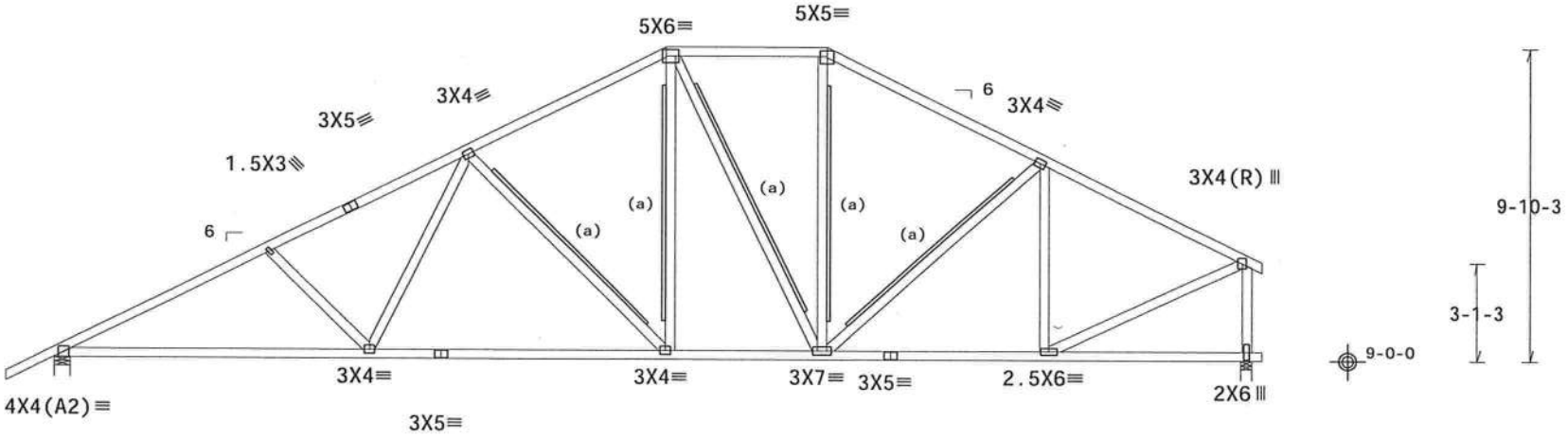
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

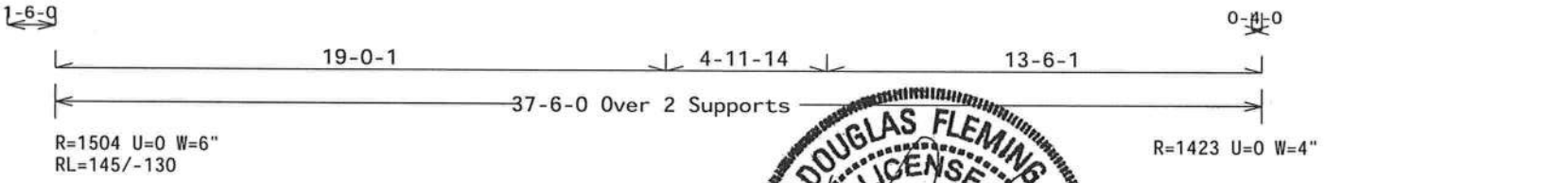
Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

(a) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

10.03.11.0209.19

QTY:

PLT TYP. Wave

FL/-/4/-/-/R/-

Scale =.1875"/Ft.

DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER 01/25/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R487-- 13430
TC DL	7.0 PSF	DATE	01/25/13
BC DL	10.0 PSF	DRW	HCUSR487 13025028
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	37.0 PSF	SEQN-	48822
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UT7487_Z02

(13-029--Doug Edgley Durham Res II -- Lake City, FL - APB 4'11"14 Common)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Wind loads and reactions based on MWFRS with additional C&C member design.

In lieu of rigid ceiling use purlins to brace BC @ 24" OC.

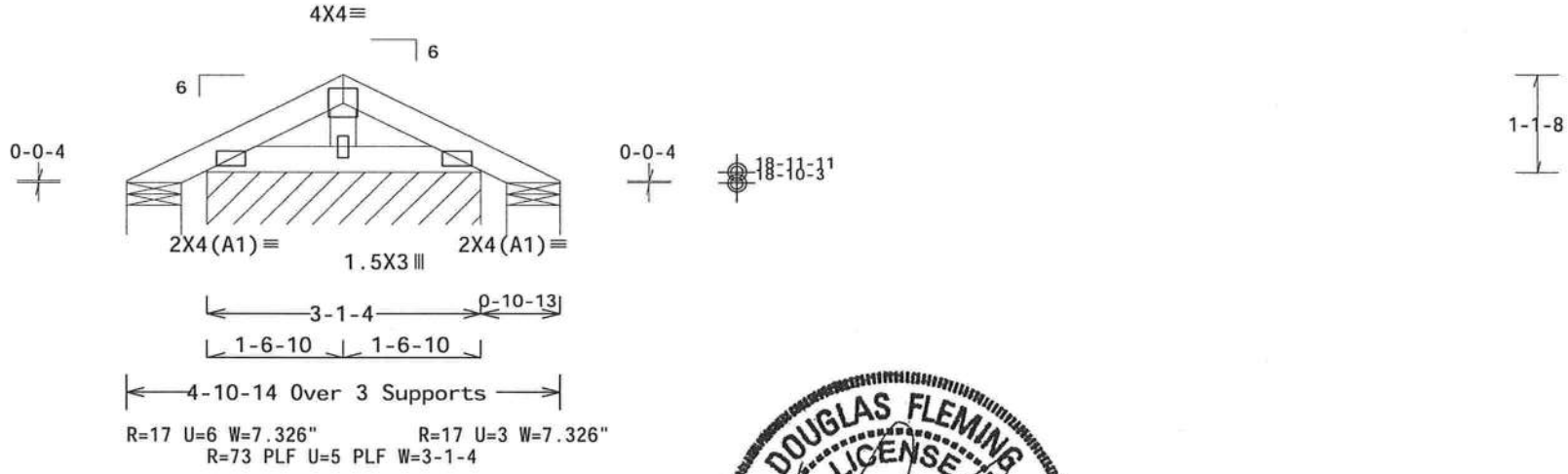
MWFRS loads based on trusses located at least 19.48 ft. from roof edge.

Refer to DWG PB160100212 for piggyback details.

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 56 plf at -0.90 to 56 plf at 1.55
TC- From 56 plf at 1.55 to 56 plf at 4.01
BC- From 4 plf at -0.90 to 4 plf at 4.01

120 mph wind, 19.48 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=2.0 psf.
GCpi(+/-)=0.18

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

10.03.11.0209.19

QTY:

FL/-/4/-/-/R/-

Scale =.5"/Ft.

DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
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TC LL	20.0 PSF	REF	R487-- 13431
TC DL	7.0 PSF	DATE	01/25/13
BC DL	10.0 PSF	DRW	HCUSR487 13025029
BC LL	0.0 PSF		HC-ENG DF/DF
TOT.LD.	37.0 PSF	SEQN-	48805
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UT7487_Z02

(13-029--Doug Edgley Durham Res II -- Lake City, FL - APB1 4'11"14 Stepdown Hip)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

120 mph wind, 19.36 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=2.0 psf.
GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

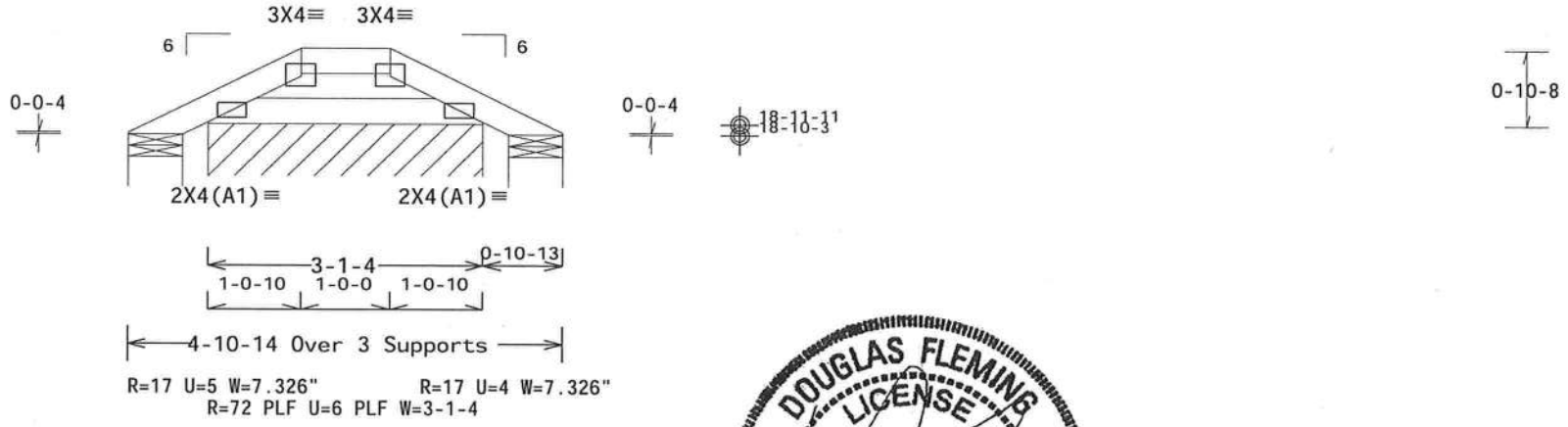
MWFRS loads based on trusses located at least 19.36 ft. from roof edge.

Refer to DWG PB160100212 for piggyback details.

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 56 plf at -0.90 to 56 plf at 1.05
TC- From 56 plf at 1.05 to 56 plf at 2.05
TC- From 56 plf at 2.05 to 56 plf at 4.01
BC- From 4 plf at -0.90 to 4 plf at 4.01

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" OC, all BC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

10.03.11.0209.19

QTY:1

FL/-/4/-/-/R/-

Scale =.5"/Ft.

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TC LL	20.0 PSF	REF	R487-- 13432
TC DL	7.0 PSF	DATE	01/25/13
BC DL	10.0 PSF	DRW	HCUSR487 13025030
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	37.0 PSF	SEQN-	48804
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UT7487_Z02

(13-029--Doug Edgley Durham Res II -- Lake City, FL - APB2 18'5"15 Mono Hip)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Wind loads and reactions based on MWFRS with additional C&C member design.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

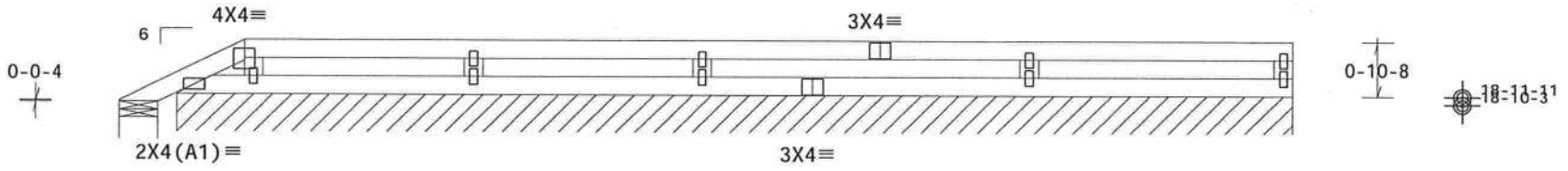
MWFRS loads based on trusses located at least 19.36 ft. from roof edge.

Refer to DWG PB160100212 for piggyback details.

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 56 plf at -0.90 to 56 plf at 1.05
TC- From 56 plf at 1.05 to 56 plf at 17.55
BC- From 4 plf at -0.90 to 4 plf at 17.55

120 mph wind, 19.36 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=2.0 psf. GCpi(+/-)=0.18

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



R=20 U=0 W=7.326"
R=61 PLF U=13 PLF W=17-6-10

Note: All Plates Are 1.5X3 Except As Shown.

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

10.03.11.0209.19

QTY: 1

FL/-/4/-/-/R/-

Scale =.375"/Ft.

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TC LL	20.0 PSF	REF	R487-- 13433
TC DL	7.0 PSF	DATE	01/25/13
BC DL	10.0 PSF	DRW	HCUSR487 13025031
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	37.0 PSF	SEQN-	48839
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UT7487_Z02

(13-029--Doug Edgley Durham Res II -- Lake City, FL - B 44'4" Common)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP M-30 :B3, B4 2x4 SP_#1_12A:
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

(a) Continuous lateral bracing equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Refer to DWG PB160100212 for piggyback details.

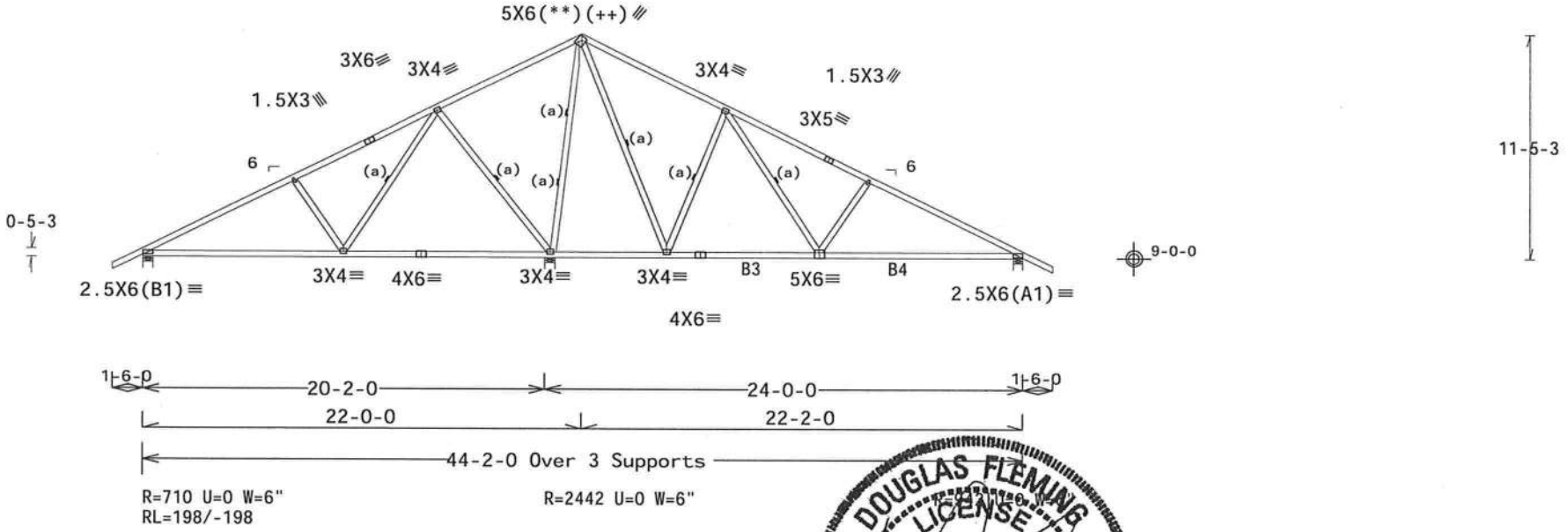
(++) - This plate works for both joints covered.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 13.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

10.03.11.0209.19

QTY:1

FL/-/4/-/-/R/-

Scale =.125"/Ft.

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TC LL	20.0 PSF	REF	R487-- 13434
TC DL	7.0 PSF	DATE	01/25/13
BC DL	10.0 PSF	DRW	HCUSR487 13025032
BC LL	0.0 PSF		HC-ENG DF/DF
TOT.LD.	37.0 PSF	SEQN-	47588
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UT7487_Z02

(13-029--Doug Edgley Durham Res II -- Lake City, FL - B1 44'2" Common)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP M-30 :B3, B4 2x4 SP_#1_12A:
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

(a) Continuous lateral bracing equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Refer to DWG PB160100212 for piggyback details.

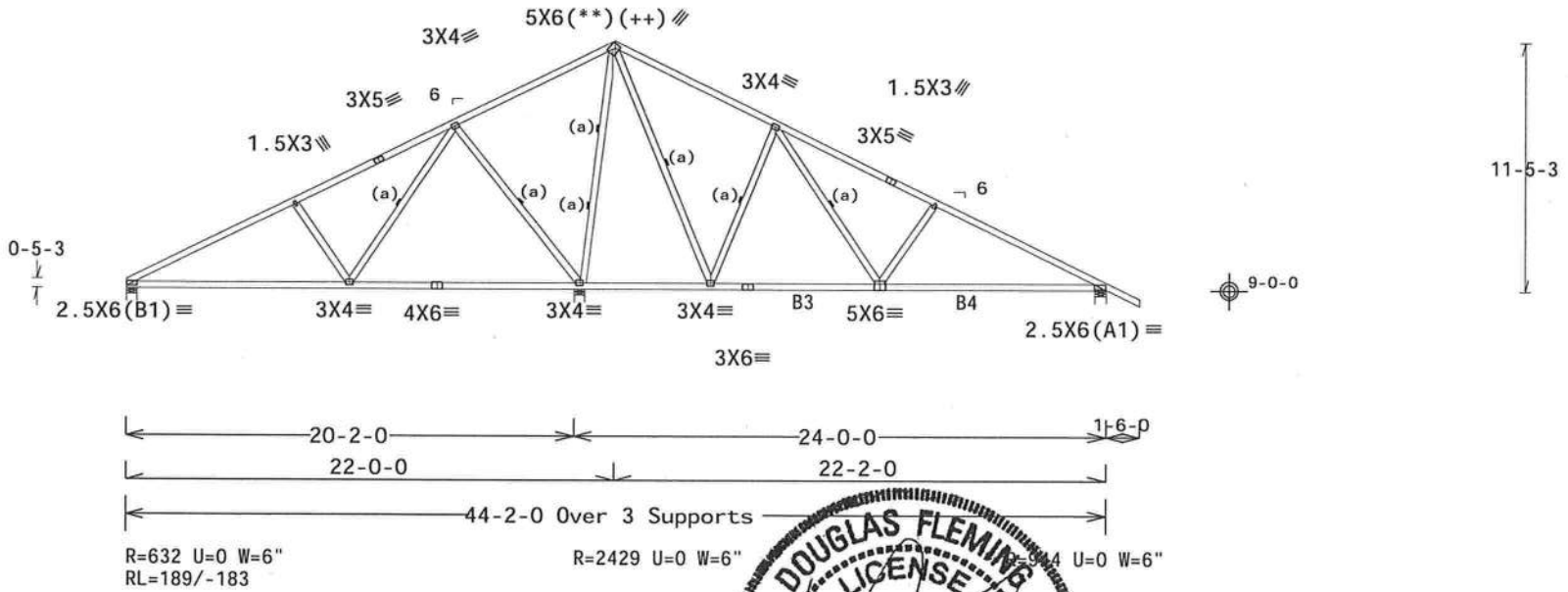
(++) - This plate works for both joints covered.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 13.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf, GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.



Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

10.03.11.0209.19 / CTY:2

FL/-/4/-/-/R/-

Scale =.125"/Ft.

PLT TYP. Wave

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TC LL	20.0 PSF	REF	R487-- 13435
TC DL	7.0 PSF	DATE	01/25/13
BC DL	10.0 PSF	DRW	HCUSR487 13025033
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	37.0 PSF	SEQN-	47576
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UT7487_Z02

(13-029--Doug Edgley Durham Res II -- Lake City, FL - B2 36'10" Common)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Left end vertical not exposed to wind pressure.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Refer to DWG PB160100212 for piggyback details.

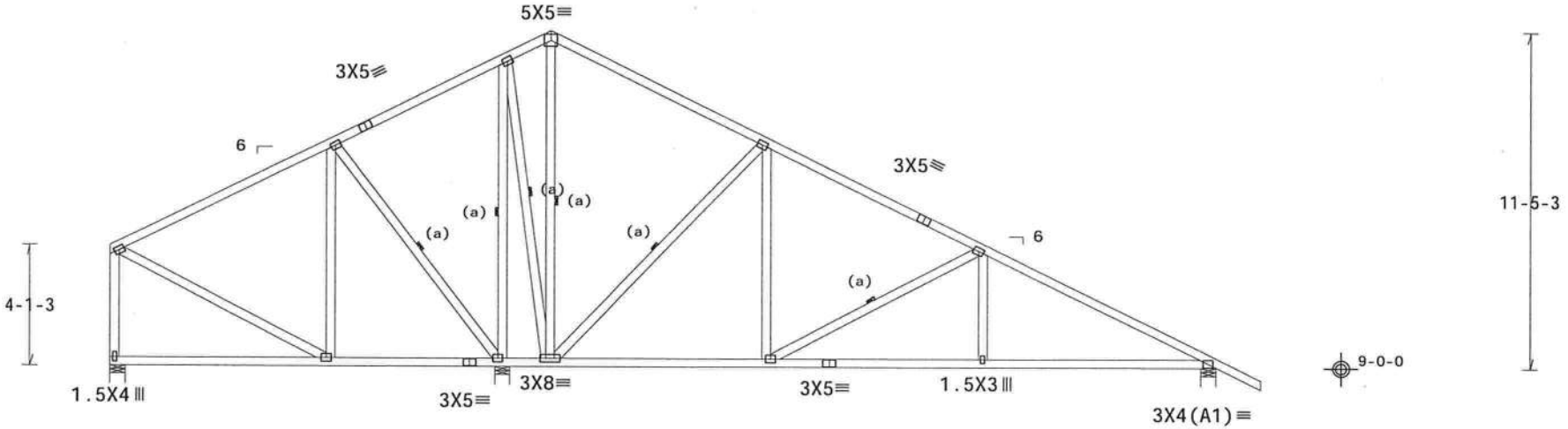
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

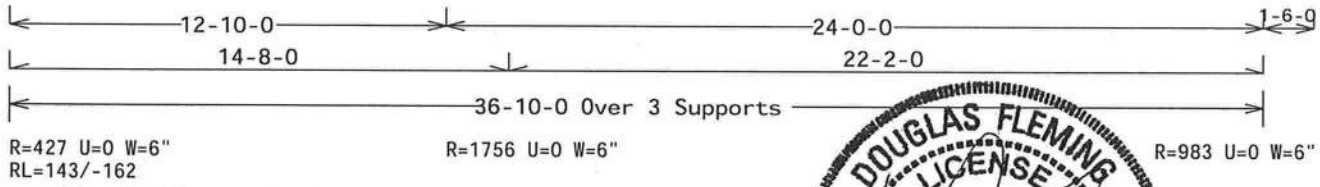
Wind loads and reactions based on MWFRS with additional C&C member design.

(a) Continuous lateral bracing equally spaced on member.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.





Note: All Plates Are 3X4 Except As Shown.

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

10.03.11.0209.19 / CTY:2

FL/-/4/-/-/R/-

Scale =.1875"/Ft.

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TC LL	20.0 PSF	REF	R487-- 13436
TC DL	7.0 PSF	DATE	01/25/13
BC DL	10.0 PSF	DRW	HCUSR487 13025034
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	37.0 PSF	SEQN-	296366
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UT7487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(13-029--Doug Edgley Durham Res II -- Lake City, FL - BDG 44'4" Gable)

Top chord 2x4 SP M-30
Bot chord 2x4 SP M-30
Webs 2x4 SP_#3__12A
:W2, W6, M1, M19, M20 2x4 SP_#1_12A:
:Stack Chord SC1 2x4 SP M-30::Stack Chord SC2 2x4 SP M-30:

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

Bottom chord checked for 10.00 psf non-concurrent live load.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

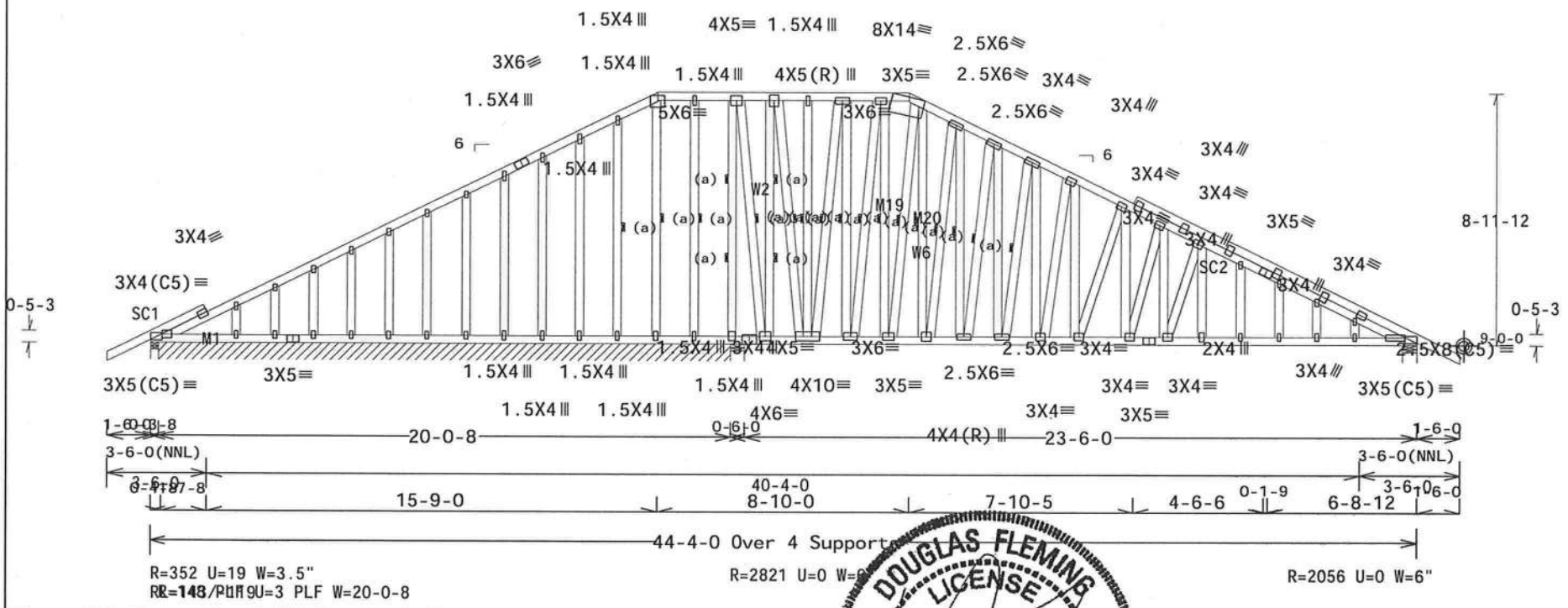
Gable end supports 8" max rake overhang.

See DWGS A12015ENC100212, GBLLETIN0212, & GABRST100212 for more requirements.

(a) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Note: All Plates Are 1.5X3 Except As Shown.

PLT TYP. Wave

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

10.03.11.0209.19 /0 CY 1

FL/-/4/-/-/R/-

Scale =.1875"/Ft.

TC LL	20.0 PSF	REF	R487-- 13437
TC DL	7.0 PSF	DATE	01/25/13
BC DL	10.0 PSF	DRW	HCUSR487 13025035
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	37.0 PSF	SEQN-	47692
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UT7487_Z02

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER 01/25/2013

(13-029--Doug Edgley Durham Res II -- Lake City, FL - C 24' Common)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

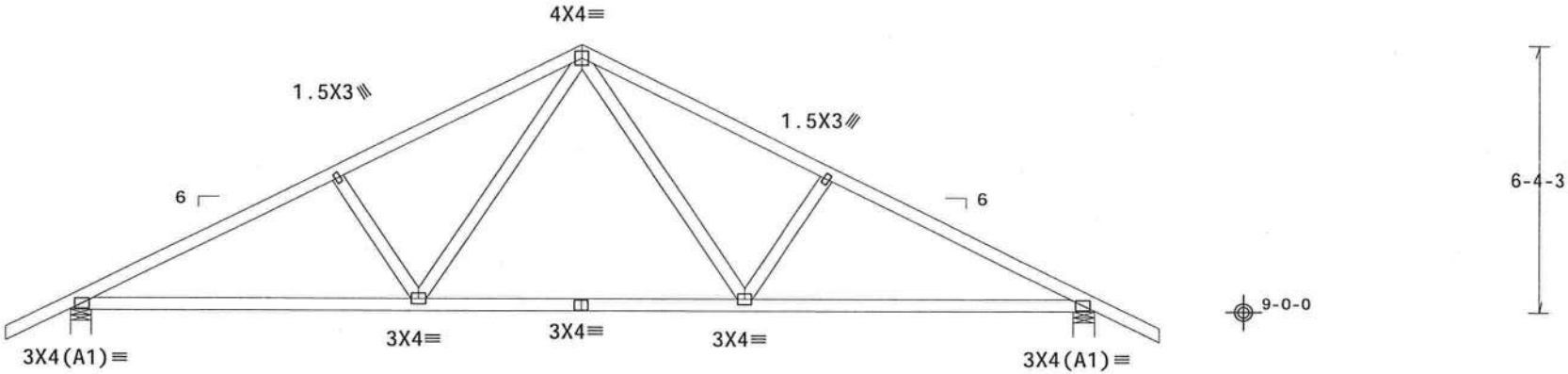
Bottom chord checked for 10.00 psf non-concurrent live load.

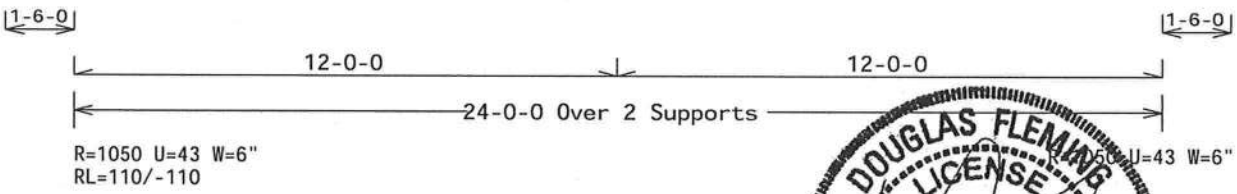
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf.
GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.





Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

10.03.11.0209.19

CTY: 7

FL/-/4/-/-/R/-

Scale =.25"/Ft.

PLT TYP. Wave

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
01/25/2013

TC LL	20.0 PSF	REF	R487-- 13438
TC DL	7.0 PSF	DATE	01/25/13
BC DL	10.0 PSF	DRW	HCUSR487 13025036
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	37.0 PSF	SEQN-	296321
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UT7487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(13-029--Doug Edgley Durham Res II -- Lake City, FL - CDG 24' Gable)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A
:Stack Chord SC1 2x4 SP_#1_12A::Stack Chord SC2 2x4 SP_#1_12A:

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf.
GCpi(+/-)=0.18

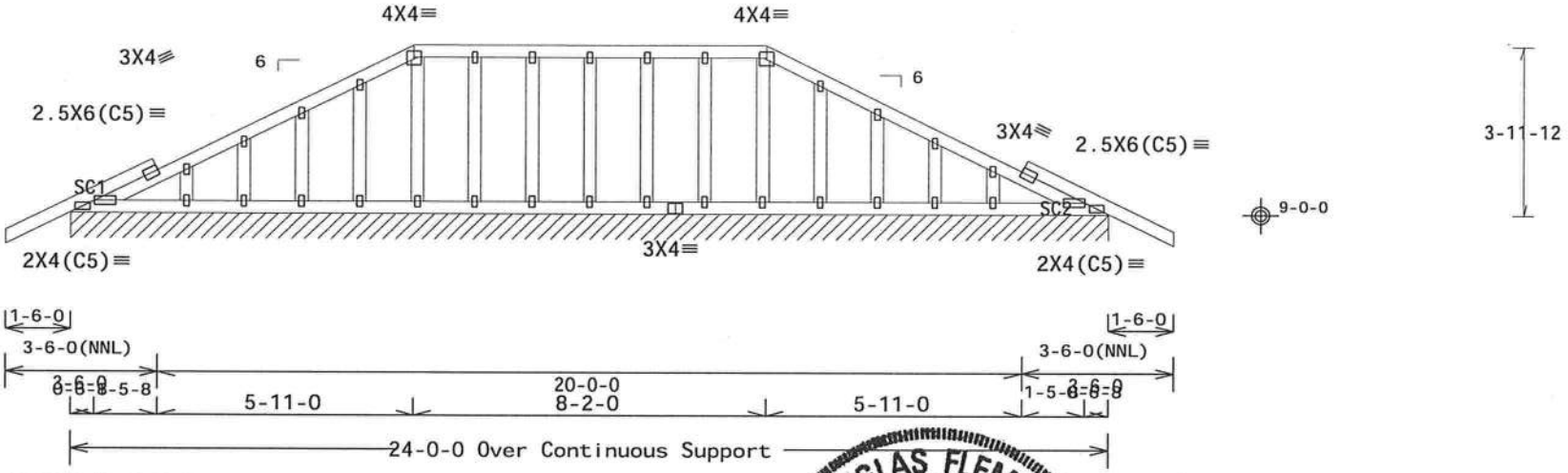
Wind loads and reactions based on MWFRS with additional C&C member design.

See DWGS A12015ENC100212, GBLLETIN0212, & GABRST100212 for more requirements.

In lieu of structural panels use purlins to brace TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



R=164 PLF U=4 PLF W=24-0-0
RL=3/-3 PLF

Note: All Plates Are 1.5X3 Except As Shown.

PLT TYP. Wave

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

10.03.11.0209.19

QTY:

FL/-/4/-/-/R/-

Scale =.25"/Ft.

DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
01/25/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R487-- 13439
TC DL	7.0 PSF	DATE	01/25/13
BC DL	10.0 PSF	DRW	HCUSR487 13025037
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	37.0 PSF	SEQN-	47638
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UT7487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(13-029--Doug Edgley Durham Res II -- Lake City, FL - CJ1 1' Jack)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

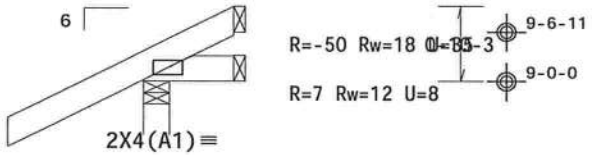
Bottom chord checked for 10.00 psf non-concurrent live load.

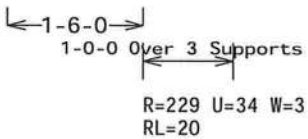
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf.
GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





Design Crit: FBC2010Res/TPI-2007(STD FT/RT=10%(0%)/0(0)

10.03.11.0209.19 QTY:

PLT TYP. Wave

FL/-/4/-/-/R/- Scale =.5"/Ft.

DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER 01/25/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R487-- 13440
TC DL	7.0 PSF	DATE	01/25/13
BC DL	10.0 PSF	DRW	HCUSR487 13025038
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	37.0 PSF	SEQN-	296095
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UT7487_Z02

(13-029--Doug Edgley Durham Res II -- Lake City, FL - CJ3 3' Jack)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

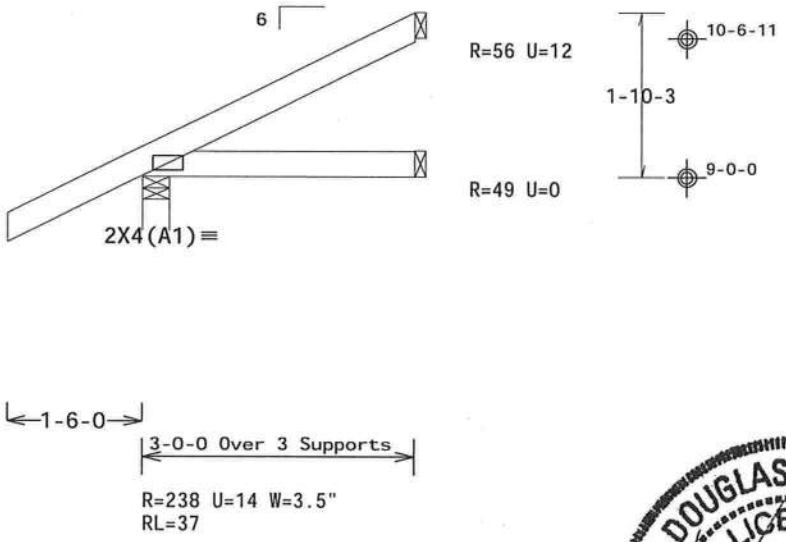
Bottom chord checked for 10.00 psf non-concurrent live load.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf.
GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

10.03.71.0209.19

QTY:

PLT TYP. Wave

FL/-/4/-/-/R/-

Scale =.5"/Ft.

DOUGLAS FLEMING LICENSE No 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
01/25/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF R487-- 13441
TC DL	7.0 PSF	DATE 01/25/13
BC DL	10.0 PSF	DRW HCUSR487 13025039
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	37.0 PSF	SEQN- 296096
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UT7487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(13-029--Doug Edgley Durham Res II -- Lake City, FL - CJ5 5' Jack)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

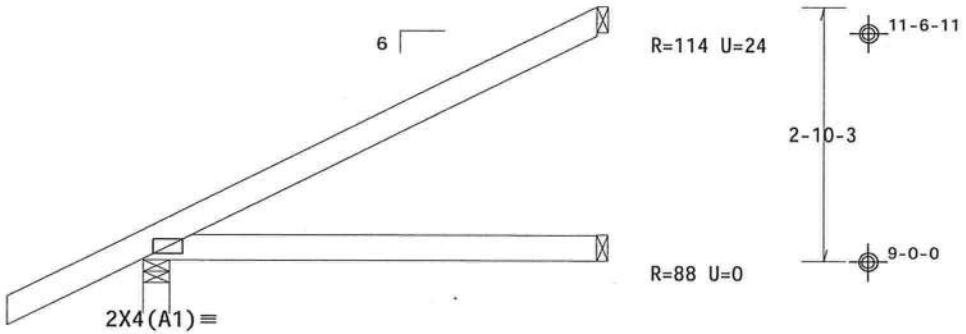
Bottom chord checked for 10.00 psf non-concurrent live load.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4.50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 10.03.11.0209.19 QTY 4 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
01/25/2013

TC LL	20.0 PSF	REF	R487-- 13442
TC DL	7.0 PSF	DATE	01/25/13
BC DL	10.0 PSF	DRW	HCUSR487 13025040
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	37.0 PSF	SEQN-	296097
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UT7487_Z02

(13-029--Doug Edgley Durham Res II -- Lake City, FL - CJ5A 5' Jack)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

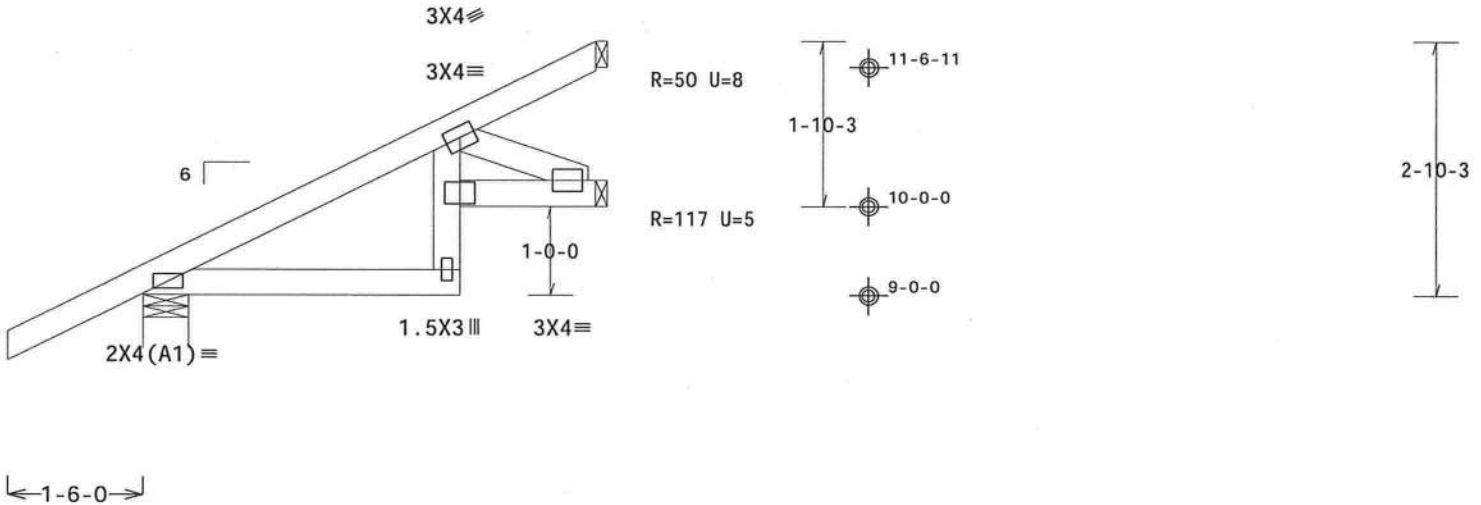
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4.50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.



Design Crit: FBC2010Res/TPI-2007(STD) FT/RT=10%(0%)/0(0) 10.03.11.0209.19 QTY:

PLT TYP. Wave

FL/-/4/-/-/R/-

Scale =.5"/Ft.

DOUGLAS FLEMING LICENSE No 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER 01/25/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R487-- 13443
TC DL	7.0 PSF	DATE	01/25/13
BC DL	10.0 PSF	DRW	HCUSR487 13025041
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	37.0 PSF	SEQN-	47946
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UT7487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(13-029--Doug Edgley Durham Res II -- Lake City, FL - EJ7 7' End Jack)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Bottom chord checked for 10.00 psf non-concurrent live load.

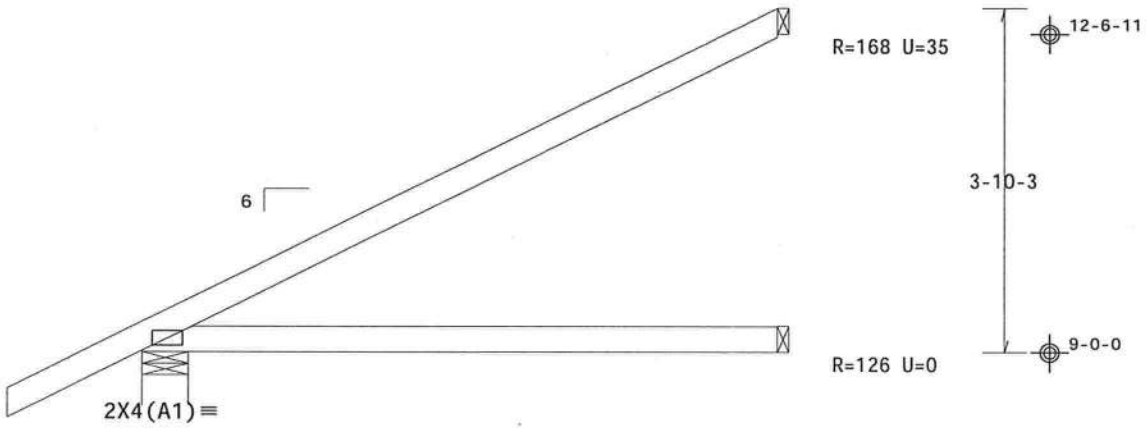
MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4.50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 10.03.11.0209.19 QTY:28 FL/-/4/-/-/R/- Scale =.5"/Ft.

DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER 01/25/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R487-- 13444
TC DL	7.0 PSF	DATE	01/25/13
BC DL	10.0 PSF	DRW	HCUSR487 13025042
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	37.0 PSF	SEQN-	296112
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UT7487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(13-029--Doug Edgley Durham Res II -- Lake City, FL - EJ7A 7' End Jack)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#1_12A :W2 2x4 SP_#3__12A:

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

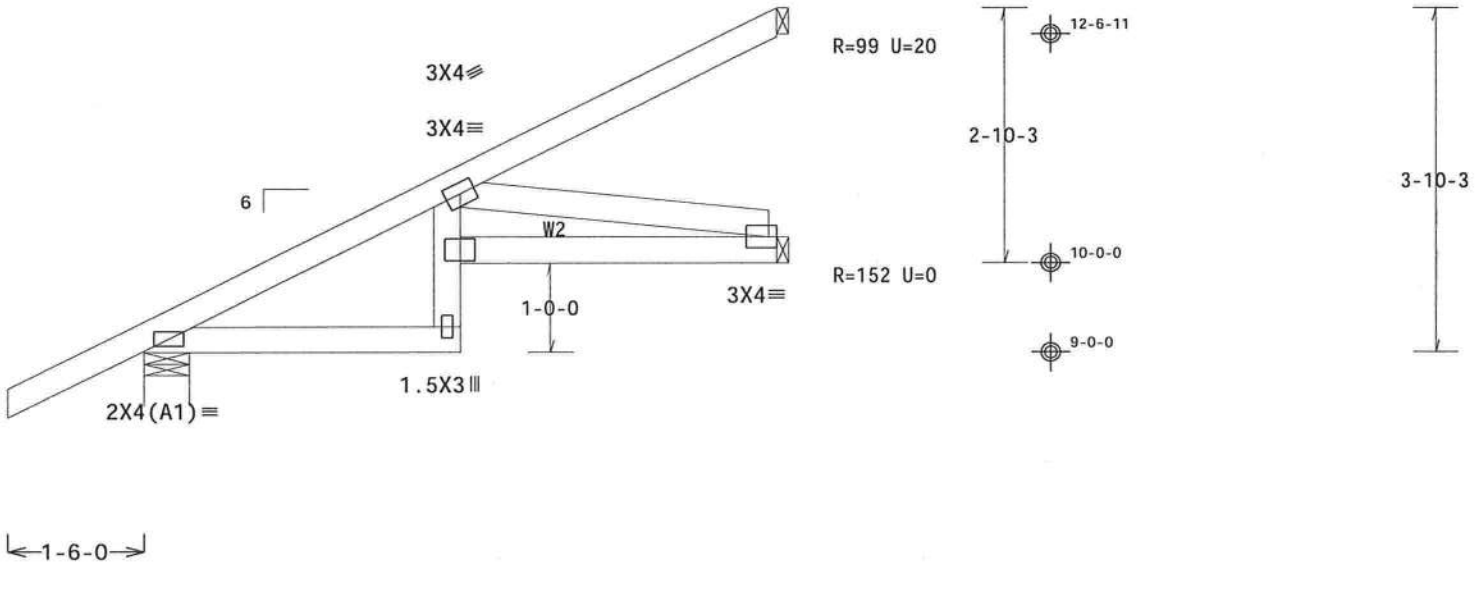
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4.50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

(T1) = (J) Tie not calculated Unavailable

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD) FT/RT=10%(0%)/0(0) 10.03.71.0209.19 QTY:

PLT TYP. Wave

FL/-/4/-/-/R/- Scale =.5"/Ft.

TC LL	20.0 PSF	REF	R487-- 13445
TC DL	7.0 PSF	DATE	01/25/13
BC DL	10.0 PSF	DRW	HCUSR487 13025043
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	37.0 PSF	SEQN-	47950
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UT7487_Z02

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
01/25/2013

(13-029--Doug Edgley Durham Res II -- Lake City, FL - H11A 43' Stepdown Hip)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

(a) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

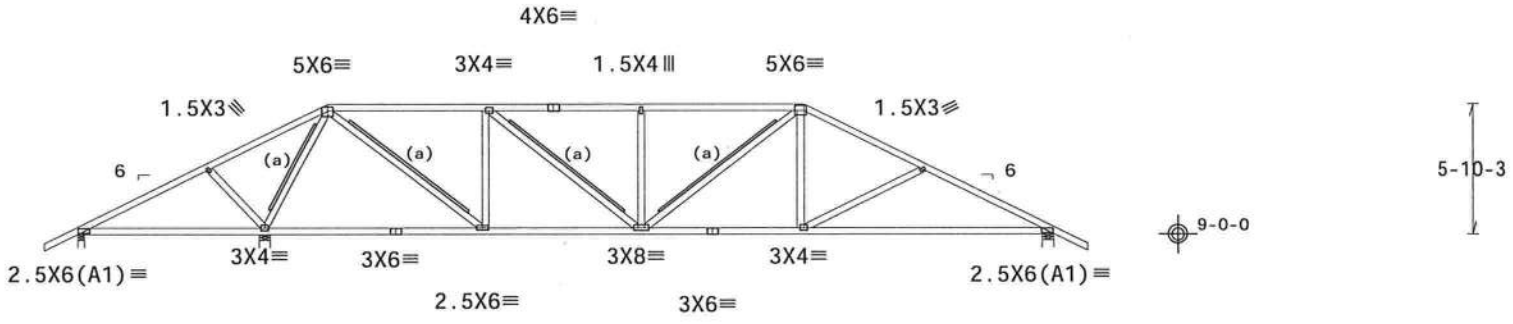
Negative reaction(s) of -193# MAX. (See below) from a non-wind load case requires uplift connection.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 6.50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave | FT/RT=10%(0%)/0(0) | 10.03.11.0209.19 | QTY:1 | FL/-/4/-/-/R/- | Scale =.125"/Ft.

DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER 01/25/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R487-- 13446
TC DL	7.0 PSF	DATE	01/25/13
BC DL	10.0 PSF	DRW	HCUSR487 13025044
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	37.0 PSF	SEQN-	296092
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UT7487_Z02

(13-029--Doug Edgley Durham Res II -- Lake City, FL - H11AM 37'6" Stepdown Hip)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Right cantilever is exposed to wind

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

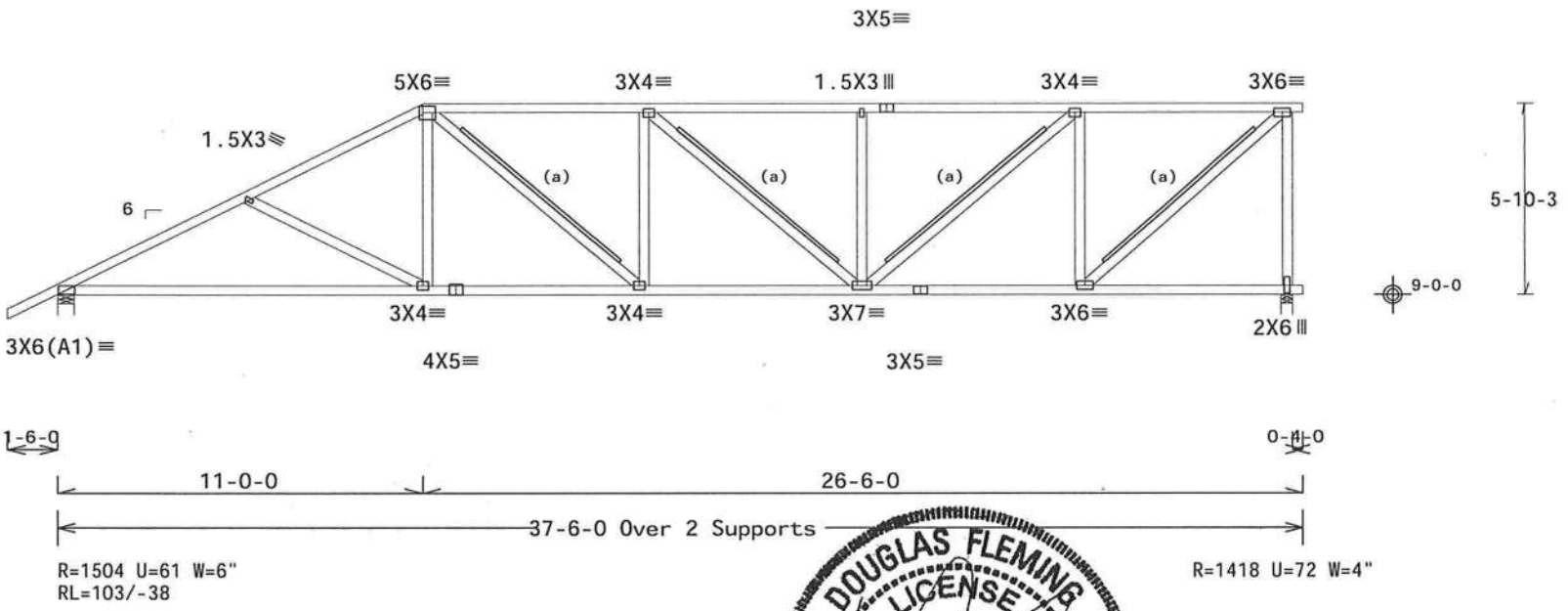
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

(a) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

10.03.11.0209.19

QTY:

FL/-/4/-/-/R/-

Scale =.1875"/Ft.

DOUGLAS FLEMING LICENSE No 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER 01/25/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R487-- 13447
TC DL	7.0 PSF	DATE	01/25/13
BC DL	10.0 PSF	DRW	HCUSR487 13025045
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	37.0 PSF	SEQN-	47978
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UT7487_Z02

(13-029--Doug Edgley Durham Res II -- Lake City, FL - H13A 43' Stepdown Hip)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

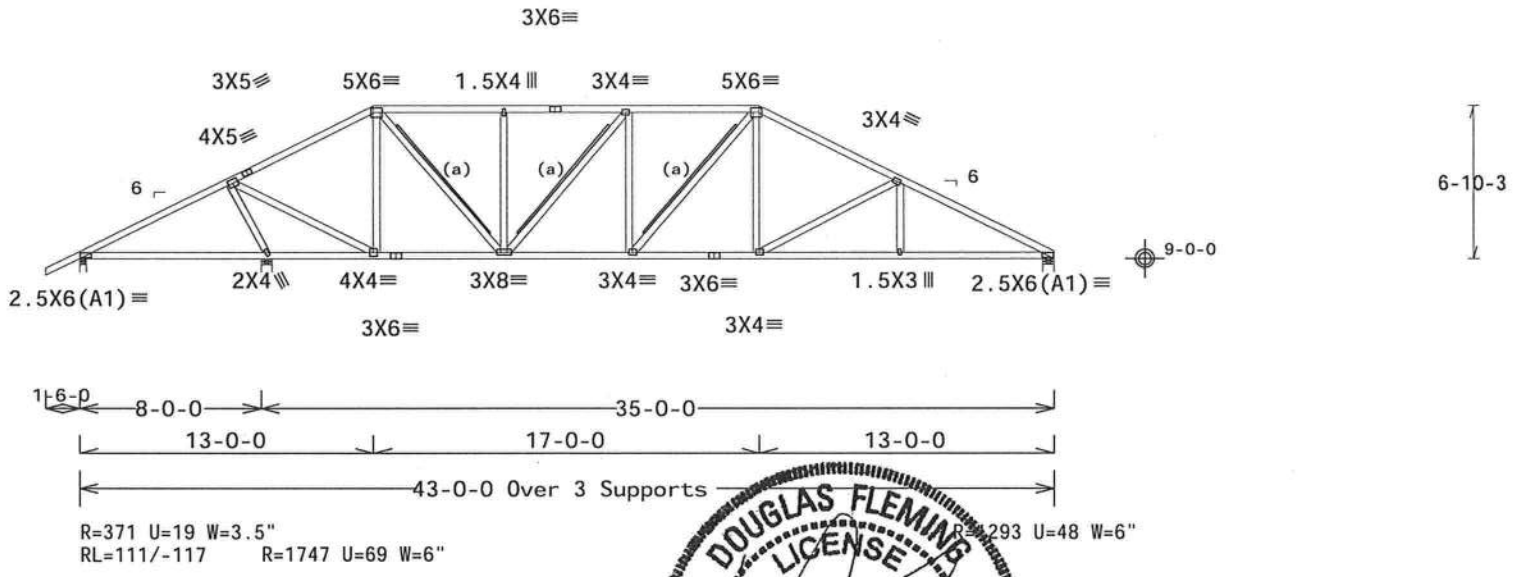
MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 13.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

(a) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

10.03.11.0209.19

QTY:1

FL/-/4/-/-/R/-

Scale =.125"/Ft.

DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER 01/25/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R487-- 13448
TC DL	7.0 PSF	DATE	01/25/13
BC DL	10.0 PSF	DRW	HCUSR487 13025046
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	37.0 PSF	SEQN-	296118
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UT7487_Z02

(13-029--Doug Edgley Durham Res II -- Lake City, FL - H13AM 37'6" Stepdown Hip)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Right cantilever is exposed to wind

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

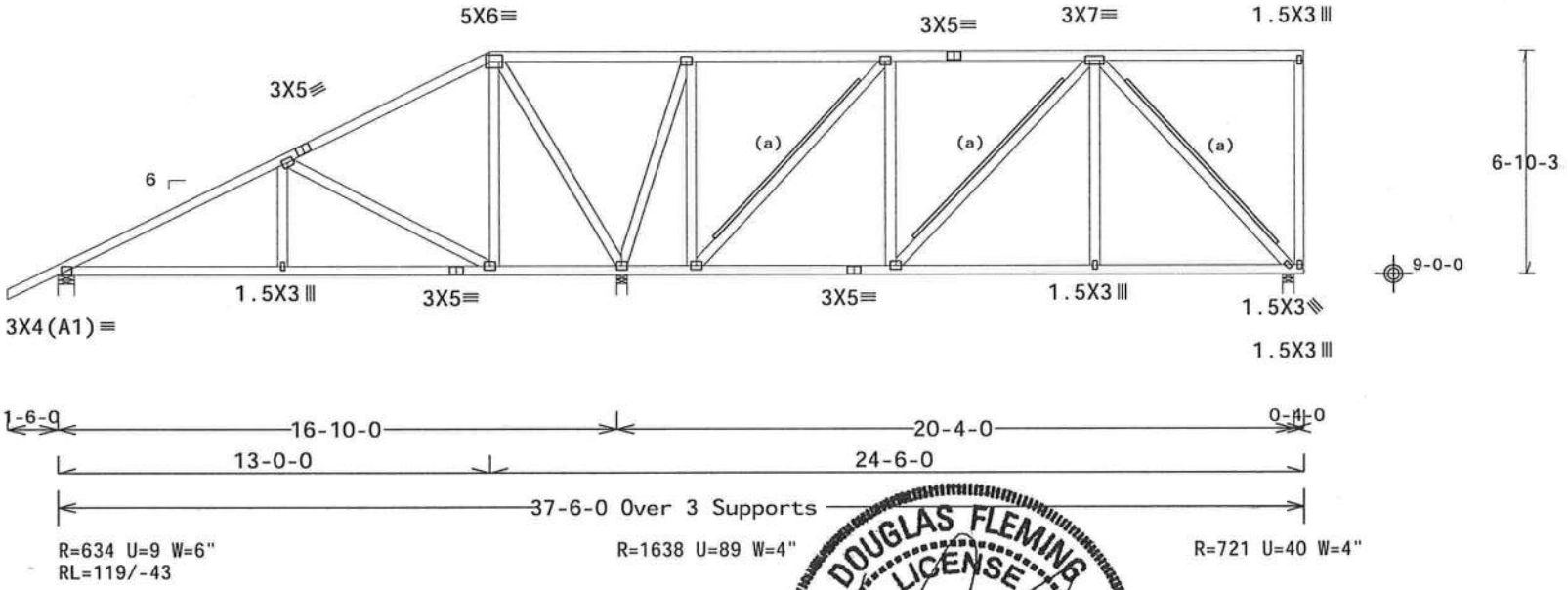
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

(a) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Note: All Plates Are 3X4 Except As Shown.

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

10.03.11.0209.19

QTY:

FL/-/4/-/-/R/-

Scale =.1875"/Ft.

DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER 01/25/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R487-- 13449
TC DL	7.0 PSF	DATE	01/25/13
BC DL	10.0 PSF	DRW	HCUSR487 13025047
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	37.0 PSF	SEQN-	47838
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UT7487_Z02

(13-029--Doug Edgley Durham Res II -- Lake City, FL - H15A 40' Stepdown Hip)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

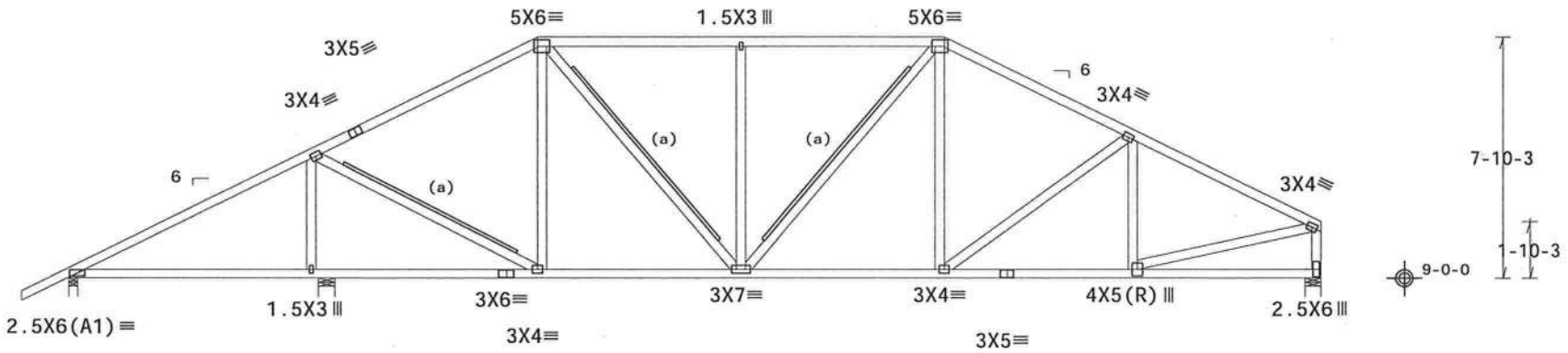
MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

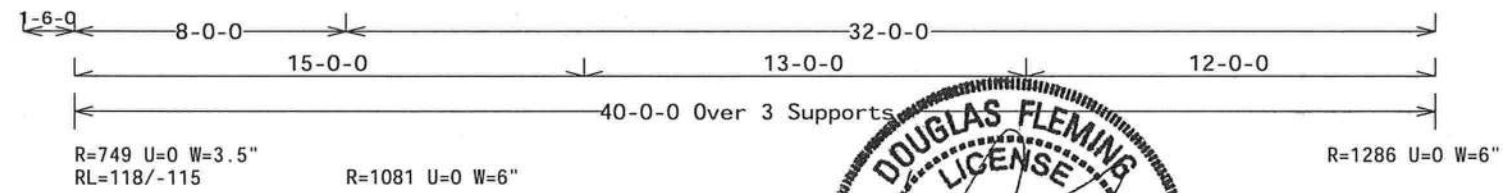
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 13.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

(a) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave | FT/RT=10%(0%)/0(0) | 10.03.11.0208.19 | 01Y11 | FL/-/4/-/-/R/- | Scale =.1875"/Ft.

DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
01/25/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
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TC LL	20.0 PSF	REF	R487-- 13450
TC DL	7.0 PSF	DATE	01/25/13
BC DL	10.0 PSF	DRW	HCUSR487 13025048
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	37.0 PSF	SEQN-	296090
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UT7487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(13-029--Doug Edgley Durham Res II -- Lake City, FL - H15AM 37'6" Stepdown Hip)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Right cantilever is exposed to wind

(b) 2x6 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

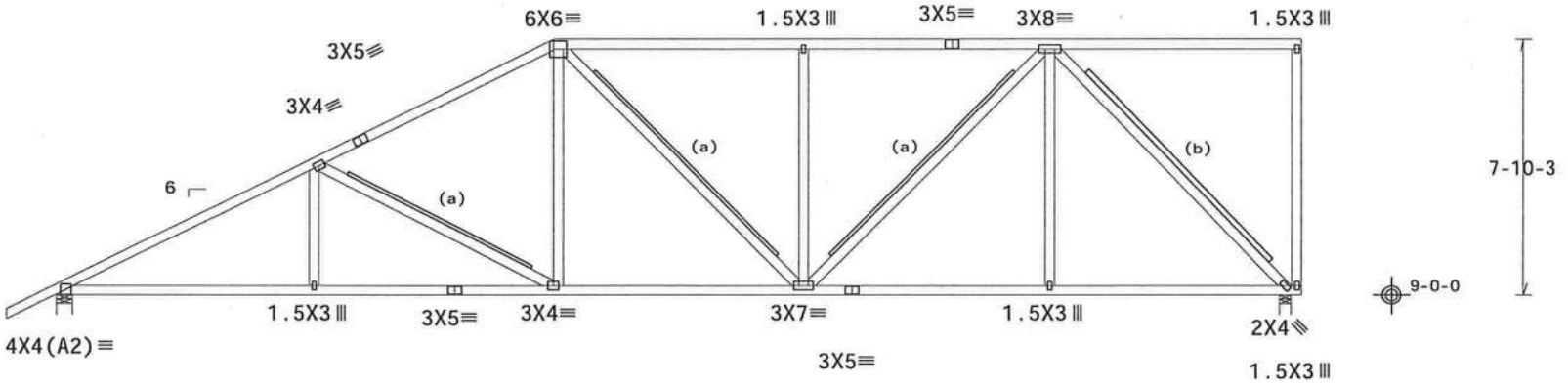
Wind loads and reactions based on MWFRS with additional C&C member design.

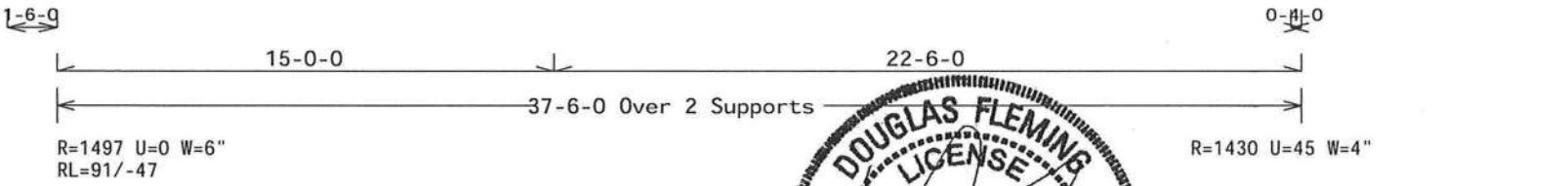
Right end vertical not exposed to wind pressure.

(a) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 10.03.11.0209.19 QTY:1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER 01/25/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

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TC LL	20.0 PSF	REF	R487-- 13451
TC DL	7.0 PSF	DATE	01/25/13
BC DL	10.0 PSF	DRW	HCUSR487 13025049
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	37.0 PSF	SEQN-	47846
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UT7487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(13-029--Doug Edgley Durham Res II -- Lake City, FL - H17A 40' Stepdown Hip)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

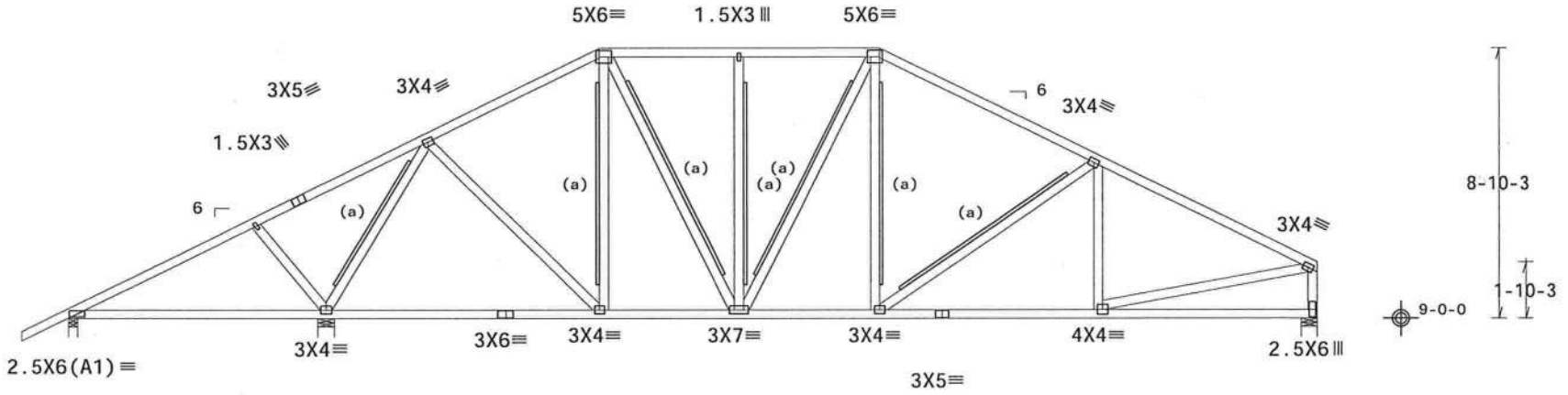
MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

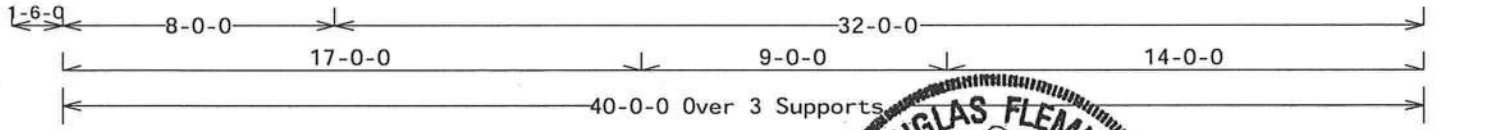
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 13.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

(a) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





R=351 U=0 W=3.5"
RL=134/-130

R=1650 U=0 W=6"

R=1173 U=0 W=6"

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave | FT/RT=10%(0%)/0(0) | 10.03.11.0209.19 | QTY 1 | FL/-/4/-/-/R/- | Scale =.1875"/Ft.

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ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R487-- 13452
TC DL	7.0 PSF	DATE	01/25/13
BC DL	10.0 PSF	DRW	HCUSR487 13025050
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	37.0 PSF	SEQN-	296088
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UT7487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(13-029--Doug Edgley Durham Res II -- Lake City, FL - H17AM 37'6" Stepdown Hip)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Right cantilever is exposed to wind

(b) 2x6 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

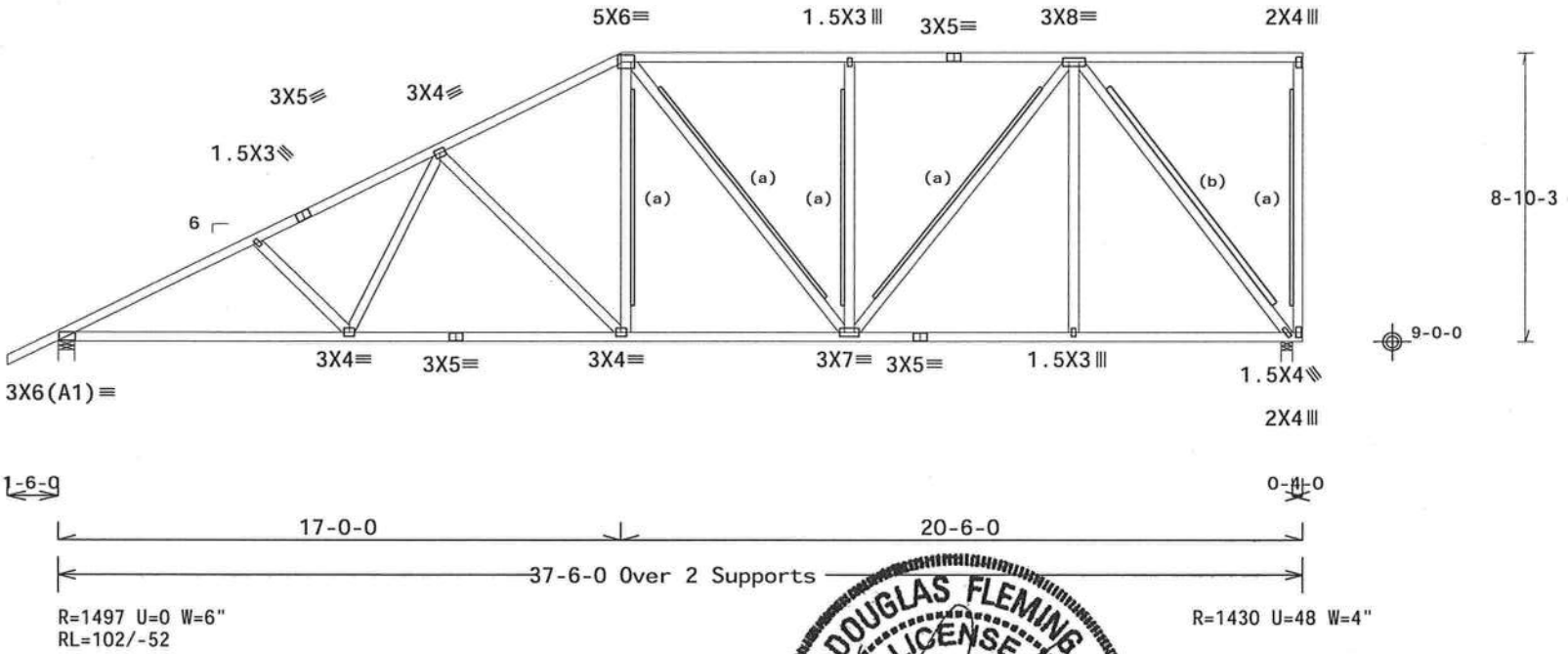
Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

(a) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

10.03.11.0209.19

QTY:

FL/-/4/-/-/R/-

Scale =.1875"/Ft.

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TC LL	20.0 PSF	REF	R487-- 13453
TC DL	7.0 PSF	DATE	01/25/13
BC DL	10.0 PSF	DRW	HCUSR487 13025051
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	37.0 PSF	SEQN-	47844
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UT7487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(13-029--Doug Edgley Durham Res II -- Lake City, FL - H19A 40' Stepdown Hip)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

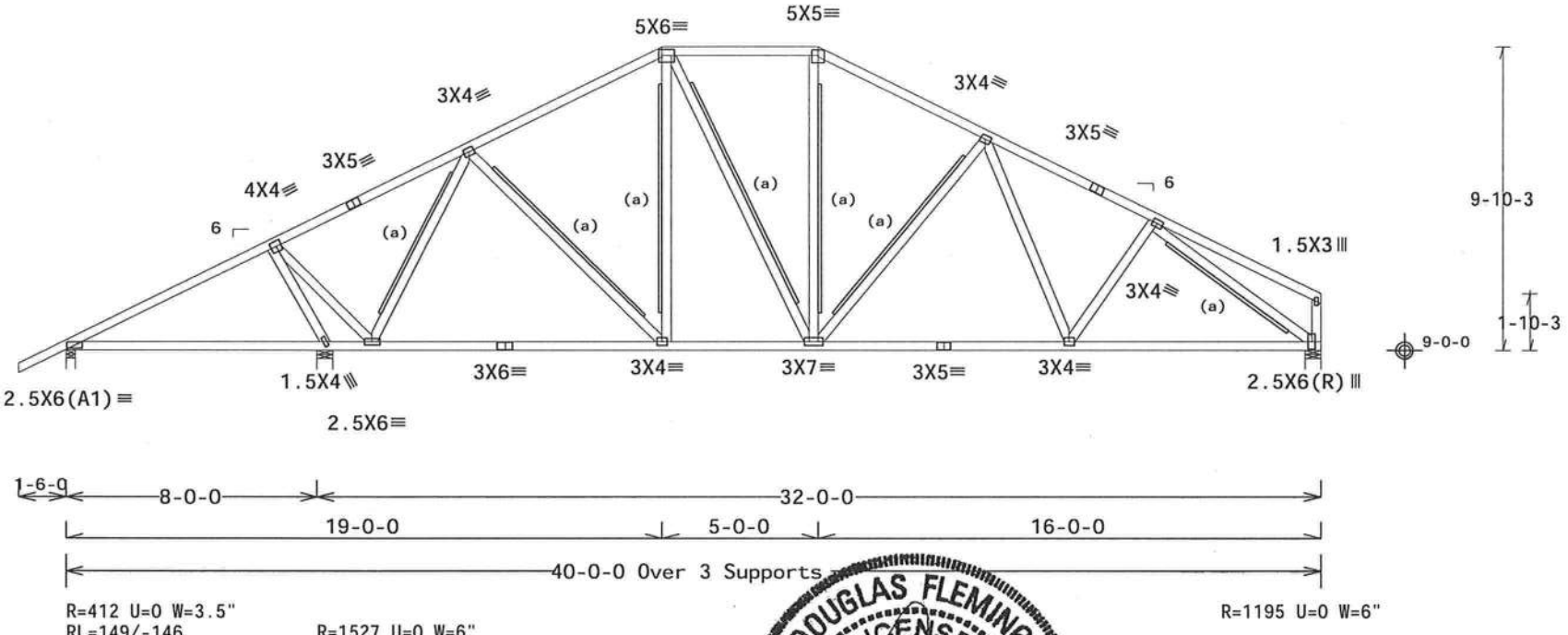
MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 13.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

(a) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER 01/25/2013

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0) 10.03.11.0209.19 QTY:

FL/-/4/-/-/R/- Scale =.1875"/Ft.

ALPINE
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Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R487-- 13454
TC DL	7.0 PSF	DATE	01/25/13
BC DL	10.0 PSF	DRW	HCUSR487 13025052
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	37.0 PSF	SEQN-	296086
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UT7487_Z02

(13-029--Doug Edgley Durham Res II -- Lake City, FL - H19AM 37'6" Stepdown Hip)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Right cantilever is exposed to wind

(b) 2x4 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

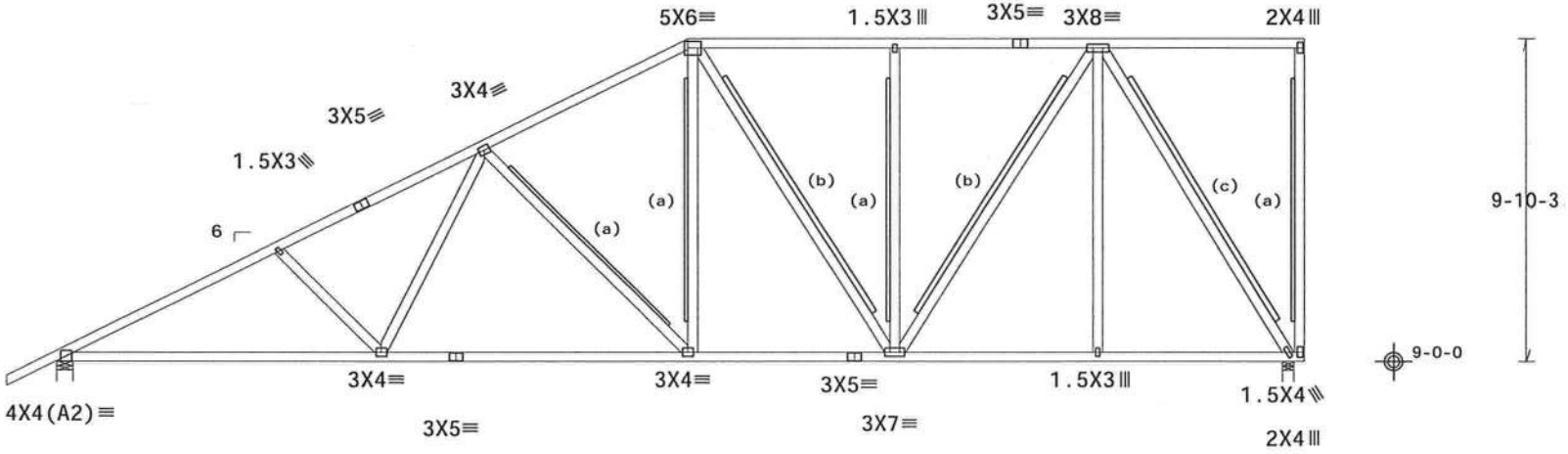
Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

(a) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

(c) 2x6 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave | FT/RT=10%(0%)/0(0) | 10.03.11.0209.19 | 07Y11 | FL/-/4/-/-/R/- | Scale =.1875"/Ft.

DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER 01/25/2013

ALPINE
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TC LL	20.0 PSF	REF	R487-- 13455
TC DL	7.0 PSF	DATE	01/25/13
BC DL	10.0 PSF	DRW	HCUSR487 13025053
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	37.0 PSF	SEQN-	47842
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UT7487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(13-029--Doug Edgley Durham Res II -- Lake City, FL - H21A 37'6" Stepdown Hip)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Right cantilever is exposed to wind

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

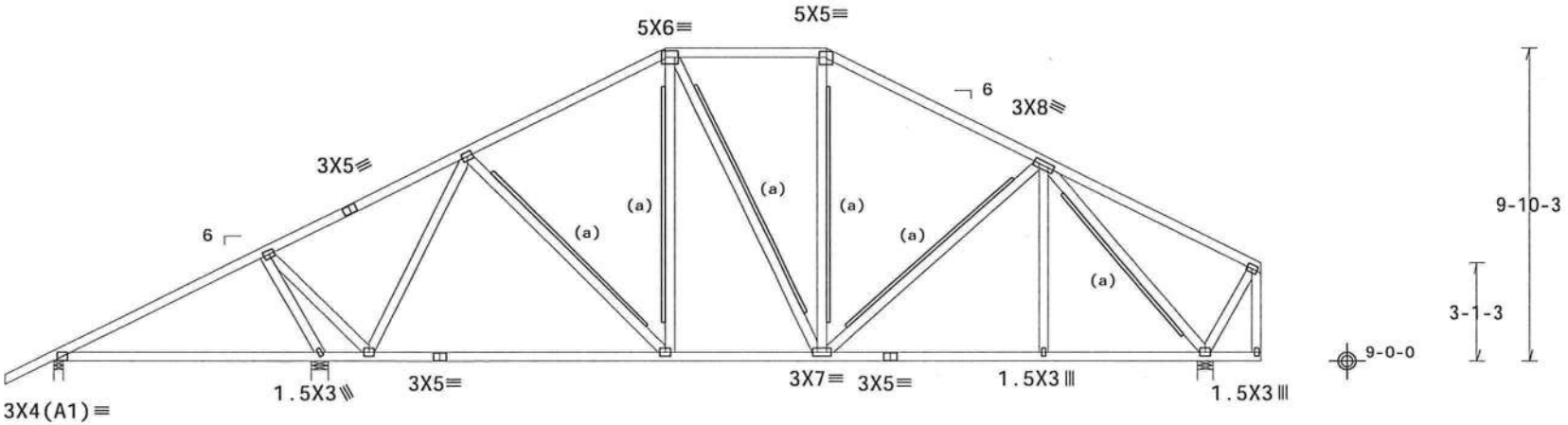
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

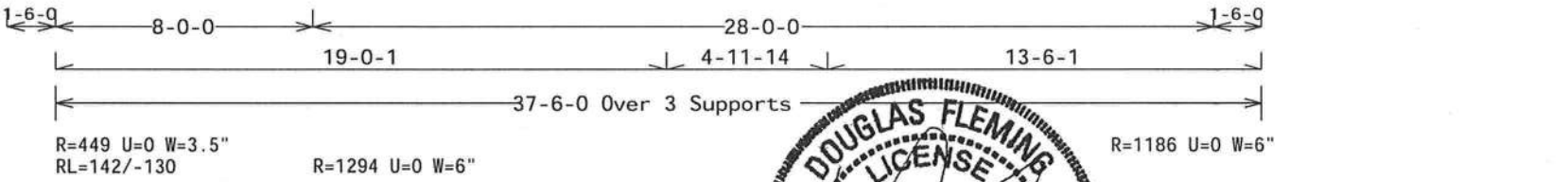
Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

(a) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





Note: All Plates Are 3X4 Except As Shown.

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 10.03.11.0209.19 QTY: 1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

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ALPINE
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TC LL	20.0 PSF	REF	R487-- 13456
TC DL	7.0 PSF	DATE	01/25/13
BC DL	10.0 PSF	DRW	HCUSR487 13025054
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	37.0 PSF	SEQN-	48785
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UT7487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(13-029--Doug Edgley Durham Res II -- Lake City, FL - H21AM 37'6" Mono Hip)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Right cantilever is exposed to wind

(b) 2x4 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

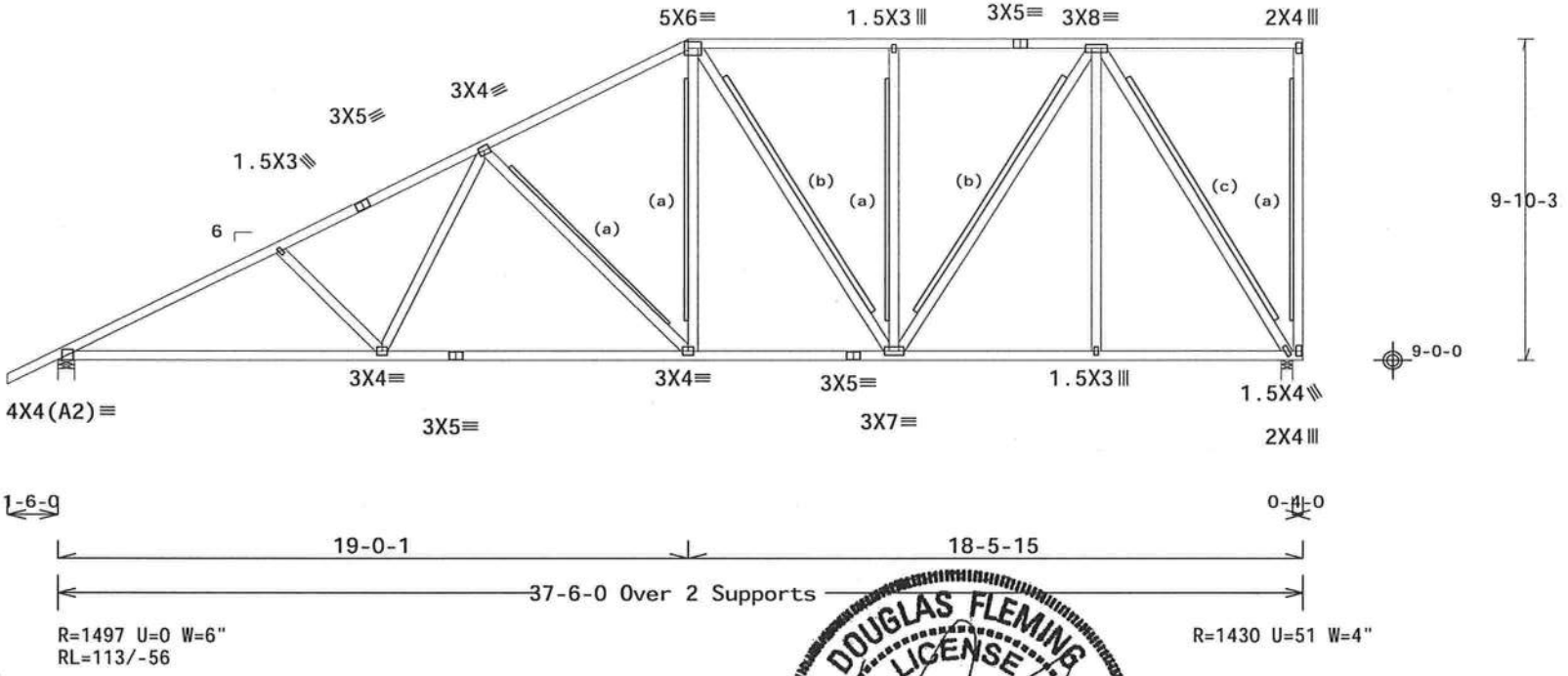
Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

(a) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

(c) 2x6 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

10.03.11.0209.19

QTY:

FL/-/4/-/-/R/-

Scale =.1875"/Ft.

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TC LL	20.0 PSF	REF	R487-- 13457
TC DL	7.0 PSF	DATE	01/25/13
BC DL	10.0 PSF	DRW	HCUSR487 13025055
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	37.0 PSF	SEQN-	48797
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UT7487_Z02

(13-029--Doug Edgley Durham Res II -- Lake City, FL - H7A 43' Stepdown Hip Girder)

Top chord 2x4 SP M-30 :T2 2x6 SP_SS_12A:
:T3 2x6 SP_#1_Dense_12A: :T4 2x4 SP_#1_12A:
Bot chord 2x6 SP_#1_Dense_12A :B3 2x6 SP_#2_12A:
Webs 2x4 SP_#3__12A :W5, W7 2x4 SP_#1_12A:

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Brg blocks:0.131"x3", min. nails

brg	x-loc	#blocks	length/blk	#nails/blk	wall plate
2	8.000'	1	12"	5	Rigid Surface

Brg block to be same size and species as chord.
Refer to drawing CNNAILSP0109 for more information.

Negative reaction(s) of -1592# MAX. (See below) from a non-wind load case requires uplift connection.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

(b) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

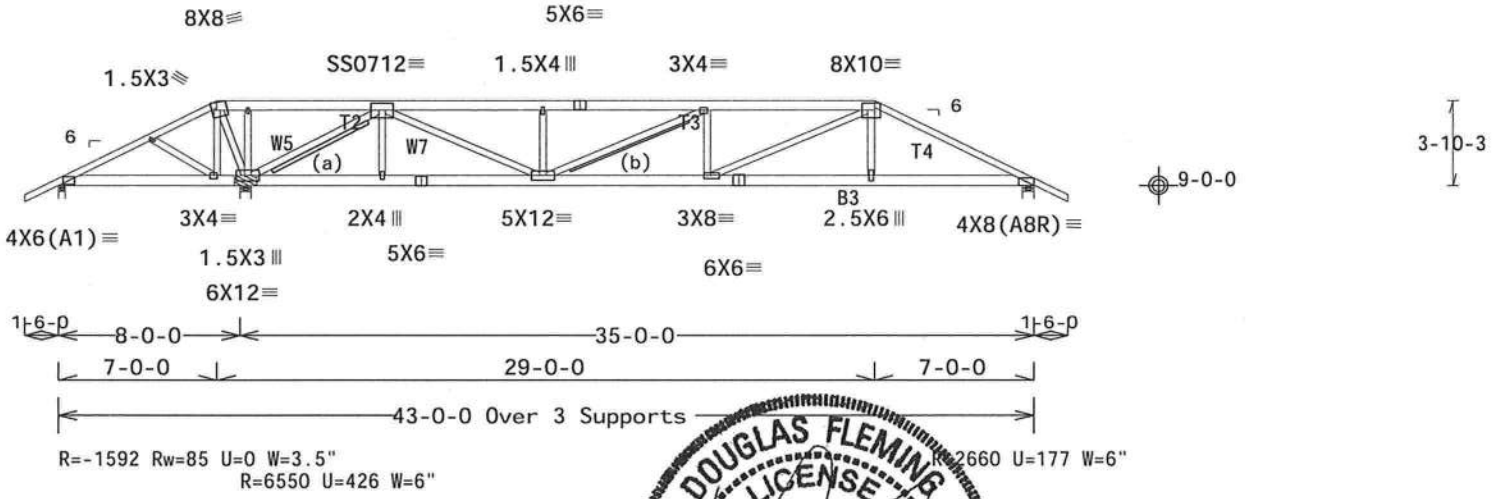
Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 56 plf at -1.50 to 56 plf at 7.00
TC- From 28 plf at 7.00 to 28 plf at 36.00
TC- From 56 plf at 36.00 to 56 plf at 44.50
BC- From 4 plf at -1.50 to 4 plf at 0.00
BC- From 20 plf at 0.00 to 20 plf at 7.03
BC- From 10 plf at 7.03 to 10 plf at 35.97
BC- From 20 plf at 35.97 to 20 plf at 43.00
BC- From 4 plf at 43.00 to 4 plf at 44.50
TC- 242.01 lb Conc. Load at 7.03,35.97
TC- 167.95 lb Conc. Load at 9.06,11.06,13.06,15.06 17.06,19.06,21.06,21.94,23.94,25.94,27.94,29.94,31.94 33.94
BC- 443.53 lb Conc. Load at 7.03,35.97
BC- 126.35 lb Conc. Load at 9.06,11.06,13.06,15.06 17.06,19.06,21.06,21.94,23.94,25.94,27.94,29.94,31.94 33.94

Wind loads and reactions based on MWFRS.

(a) 2x8 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. 18 Gauge HS,Wave

FT/RT=10%(0%)/0(0)

10.03.11.0209.19

QTY:

FL/-/4/-/-/R/-

Scale =.125"/Ft.

DOUGLAS FLEMING LICENSE No 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER 01/25/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R487-- 13458
TC DL	7.0 PSF	DATE	01/25/13
BC DL	10.0 PSF	DRW	HCUSR487 13025056
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	37.0 PSF	SEQN-	47621
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UT7487_Z02

(13-029--Doug Edgley Durham Res II -- Lake City, FL - H7AM 37'6" Stepdown Hip Girder)

Top chord 2x4 SP_#1_12A :T2 2x6 SP_#1_Dense_12A:
Bot chord 2x4 SP_#1_12A :B2 2x4 SP M-30:
:B3 2x6 SP_SS_12A:
Webs 2x4 SP_#3__12A
:W2, W3, W4, W6, W7, W8 2x4 SP_#1_12A:

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 56 plf at -1.50 to 56 plf at 7.00
TC- From 56 plf at 7.00 to 56 plf at 15.06
TC- From 28 plf at 15.06 to 28 plf at 23.00
TC- From 28 plf at 23.00 to 28 plf at 37.50
BC- From 4 plf at -1.50 to 4 plf at 0.00
BC- From 20 plf at 0.00 to 20 plf at 2.50
BC- From 28 plf at 2.50 to 28 plf at 3.50
BC- From 20 plf at 3.50 to 20 plf at 13.83
BC- From 10 plf at 13.83 to 10 plf at 25.83
BC- From 10 plf at 25.83 to 10 plf at 37.50
TC- 159.35 lb Conc. Load at 7.04
TC- 98.58 lb Conc. Load at 9.06,11.06,13.06
TC- 167.95 lb Conc. Load at 15.06,17.06,19.06,21.06 21.94,23.94,25.94,27.94,29.94,31.94,33.94,35.97
BC- 436.27 lb Conc. Load at 7.03
BC- 151.90 lb Conc. Load at 9.06,11.06,13.06
BC- 126.35 lb Conc. Load at 15.06,17.06,19.06,21.06 21.94,23.94,25.94,27.94,29.94,31.94,33.94,35.97

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

2 COMPLETE TRUSSES REQUIRED

Nail Schedule:0.131"x3", min. nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

(++) - This plate works for both joints covered.

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS.

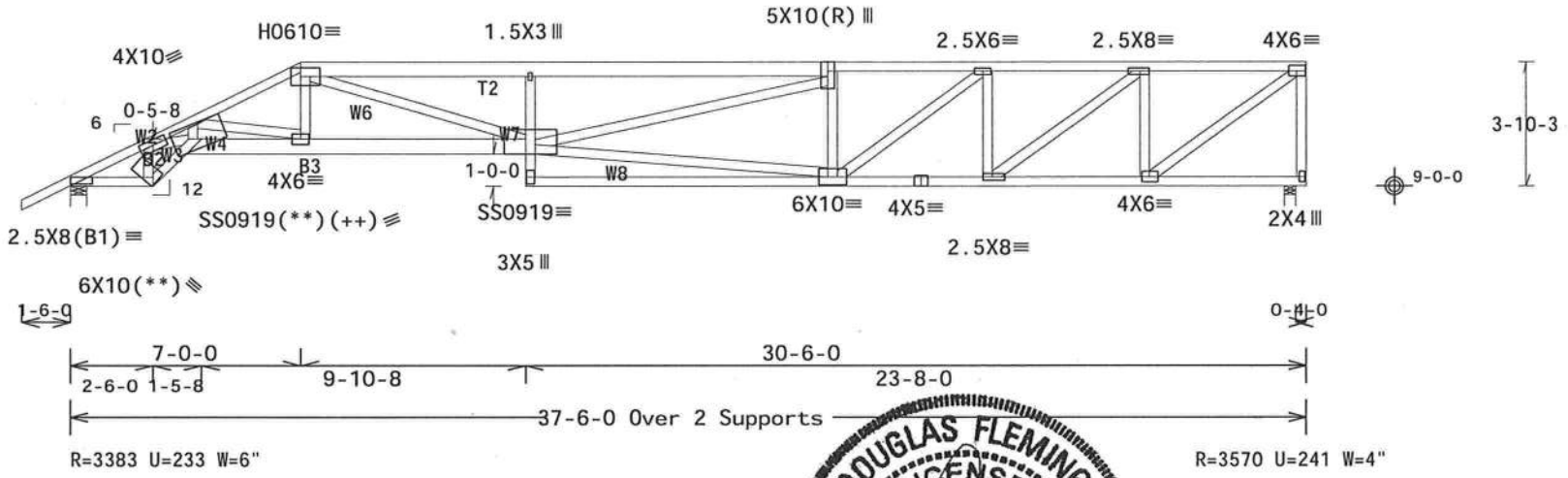
Right end vertical not exposed to wind pressure.

Right cantilever is exposed to wind

Calculated horizontal deflection is 0.19" due to live load and 0.25" due to dead load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Calculated vertical deflection is 0.58" due to live load and 0.75" due to dead load at X = 13-11-12.



PLT TYP. 20 Gauge HS,18 Gauge HS, Wave

Design Crit: FBC2010Res/TPI-2007(STD) FT/RT=10%(0%)/0(0)

10.03.11.0209.19 QTY:

FL/-/4/-/-/R/-

Scale =.1875"/Ft.

DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER 01/25/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R487-- 13459
TC DL	7.0 PSF	DATE	01/25/13
BC DL	10.0 PSF	DRW	HCUSR487 13025057
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	37.0 PSF	SEQN-	48951
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UT7487_Z02

(13-029--Doug Edgley Durham Res II -- Lake City, FL - H9A 43' Stepdown Hip)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

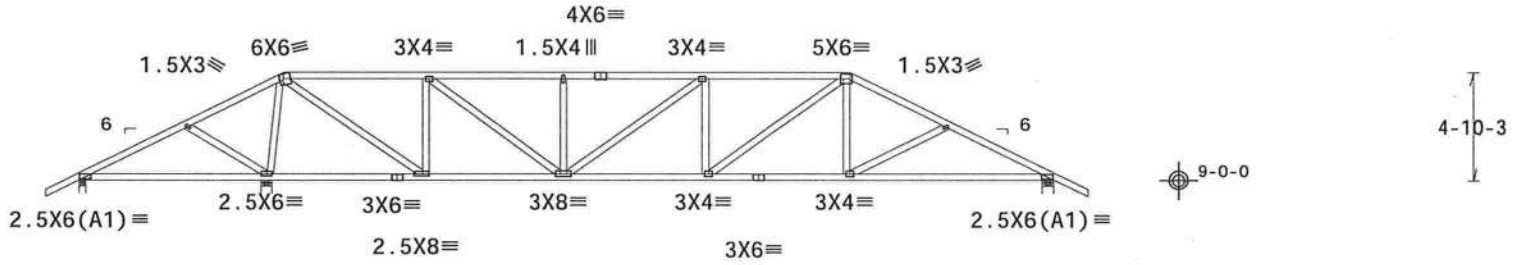
MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

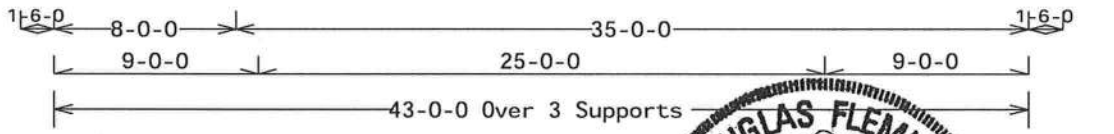
Negative reaction(s) of -372# MAX. (See below) from a non-wind load case requires uplift connection.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 6.50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





R=133/-372 U=109 W=3.5"
RL=94/-94 R=2316 U=101 W=6"
R=882 U=59 W=6"

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0) 10.03.11.0209.19 QTY 1 FL/-/4/-/-/R/- Scale =.125"/Ft.

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ALPINE
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FL COA #0 278

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TC LL	20.0 PSF	REF	R487-- 13460
TC DL	7.0 PSF	DATE	01/25/13
BC DL	10.0 PSF	DRW	HCUSR487 13025058
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	37.0 PSF	SEQN-	296094
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UT7487_Z02

(13-029--Doug Edgley Durham Res II -- Lake City, FL - H9AVM 37'6" Stepdown Hip)

Top chord 2x4 SP_#1_12A :T1 2x4 SP M-30:
Bot chord 2x4 SP M-30 :B4, B5 2x4 SP_#1_12A:
Webs 2x4 SP_#3__12A :W2, W8 2x4 SP_#1_12A:

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Right cantilever is exposed to wind

Calculated horizontal deflection is 0.14" due to live load and 0.19" due to dead load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

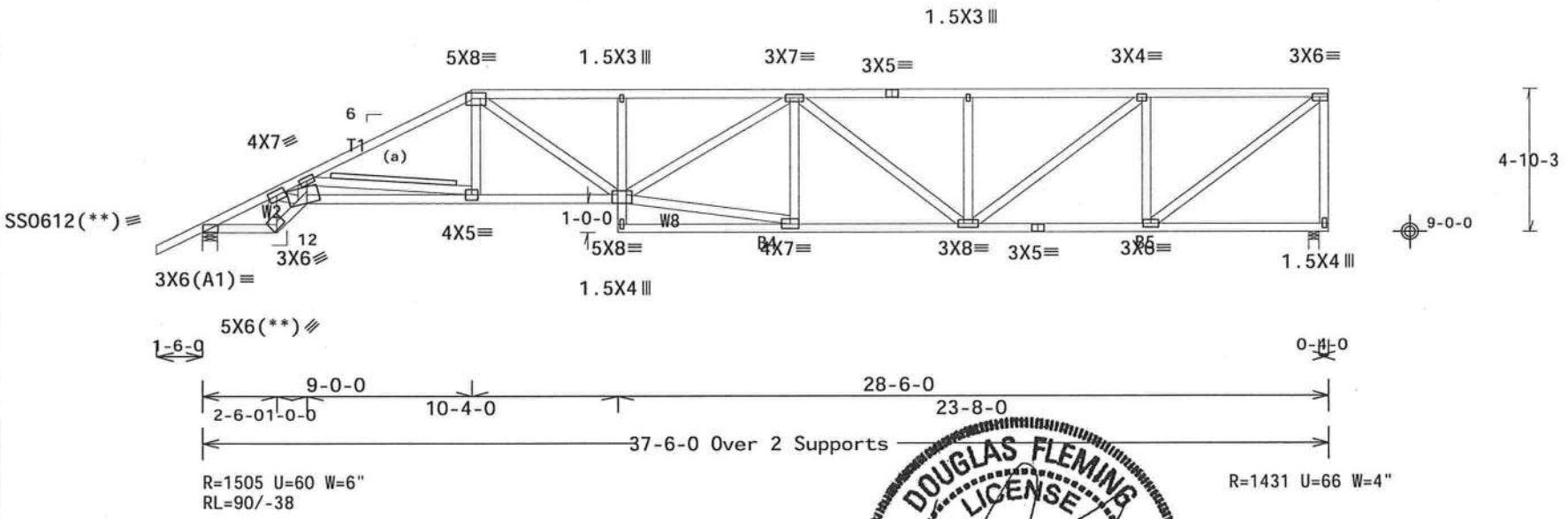
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

(a) 2x4 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. 18 Gauge HS,Wave | FT/RT=10%(0%)/0(0) | 10.03.11.0209.19 | QTY: 1 | FL/-/4/-/-/R/- | Scale =.1875"/Ft.

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER 01/25/2013

ALPINE
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Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R487-- 13461
TC DL	7.0 PSF	DATE	01/25/13
BC DL	10.0 PSF	DRW	HCUSR487 13025059
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	37.0 PSF	SEQN-	48913
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UT7487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(13-029--Doug Edgley Durham Res II -- Lake City, FL - HJ 9'10"13 Hip Jack Girder)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

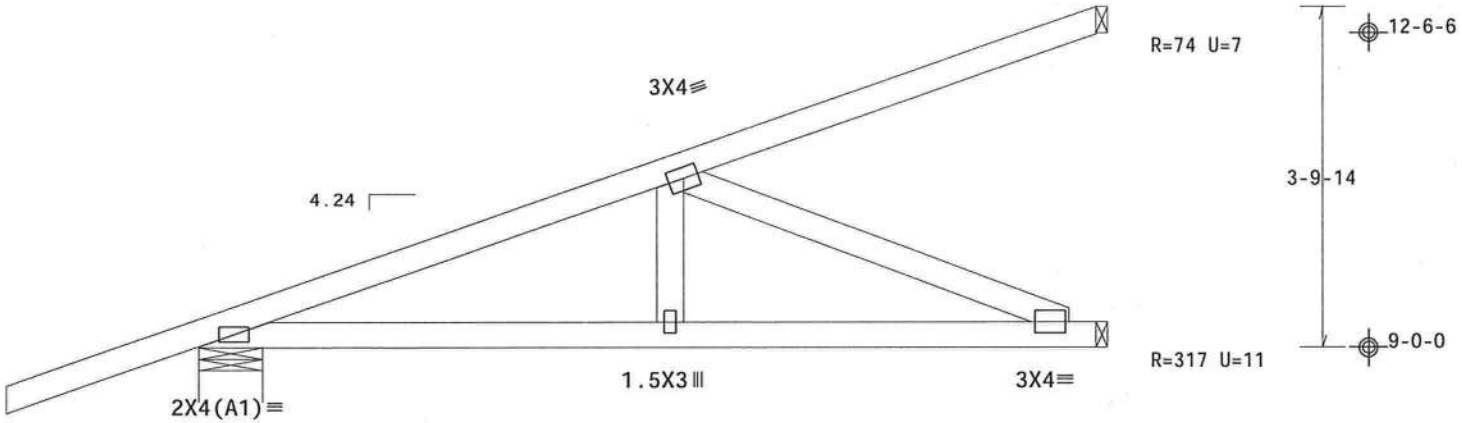
Wind loads and reactions based on MWFRS.

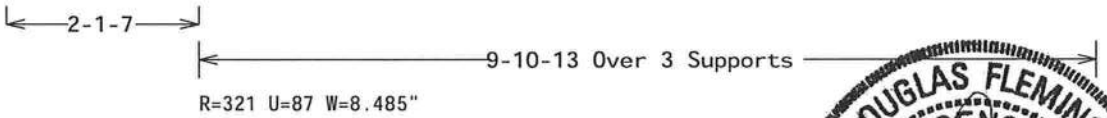
Bottom chord checked for 10.00 psf non-concurrent live load.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (3) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 0 plf at -2.12 to 55 plf at 0.00
TC- From 2 plf at 0.00 to 2 plf at 9.90
BC- From 0 plf at -2.12 to 4 plf at 0.00
BC- From 2 plf at 0.00 to 2 plf at 9.90
TC- -31.38 lb Conc. Load at 1.48
TC- 111.08 lb Conc. Load at 4.31
TC- 228.42 lb Conc. Load at 7.13
BC- 14.23 lb Conc. Load at 1.48
BC- 98.68 lb Conc. Load at 4.31
BC- 176.50 lb Conc. Load at 7.13

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





Design Crit: FBC2010Res/TPI-2007(STD) FT/RT=10%(0%)/0(0) 10.03.11.0209.19 QTY: 1

PLT TYP. Wave

FL/-/4/-/-/R/-

Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
01/25/2013

TC LL	20.0 PSF	REF R487-- 13462
TC DL	7.0 PSF	DATE 01/25/13
BC DL	10.0 PSF	DRW HCUSR487 13025060
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	37.0 PSF	SEQN- 296134
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UT7487_ZO2

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(13-029--Doug Edgley Durham Res II -- Lake City, FL - HJ1 9'10"13 Hip Jack Girder)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#1_12A :W2 2x4 SP_#3__12A:

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS.

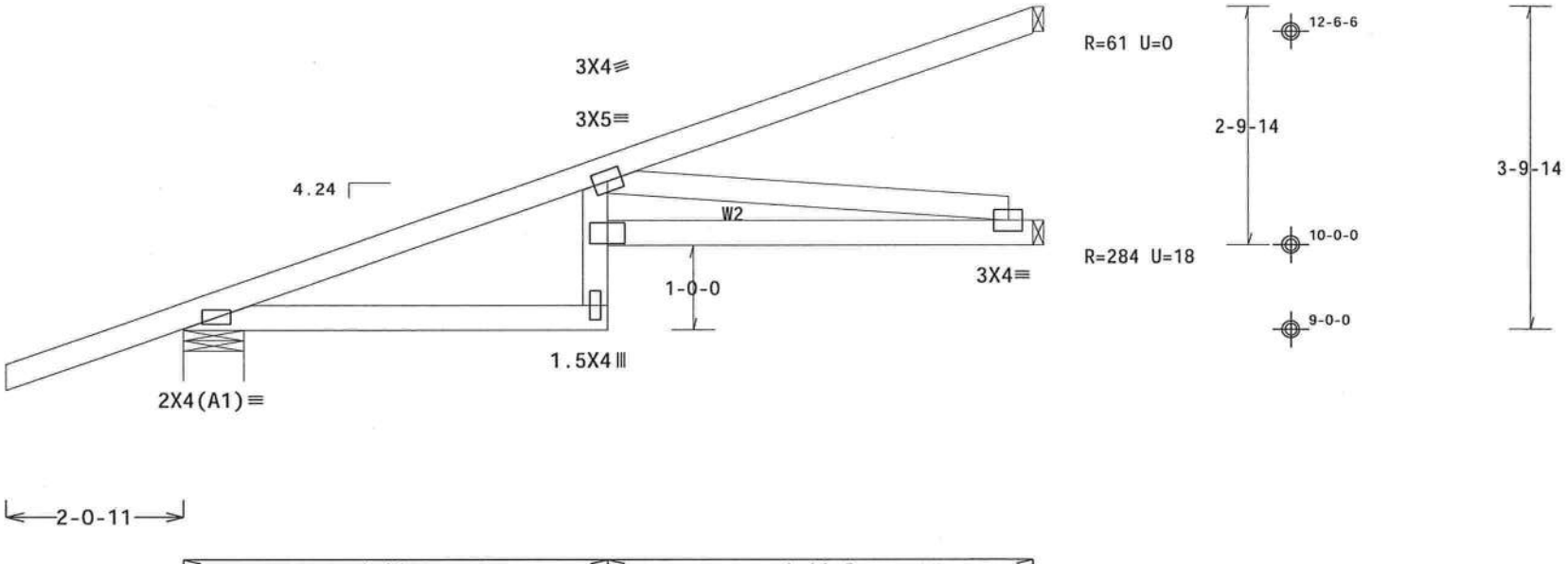
(T1) = (J) Tie not calculated Unavailable

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 2 plf at -2.06 to 2 plf at 9.90
BC- From 2 plf at 0.00 to 2 plf at 4.95
BC- From 2 plf at 4.95 to 2 plf at 9.90
TC- -31.38 lb Conc. Load at 1.48
TC- 111.08 lb Conc. Load at 4.31
TC- 99.02 lb Conc. Load at 7.13
BC- 14.23 lb Conc. Load at 1.48
BC- 98.68 lb Conc. Load at 4.31
BC- 234.61 lb Conc. Load at 7.13

Bottom chord checked for 10.00 psf non-concurrent live load.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave | FT/RT=10%(0%)/0(0) | 10.03.11.0209.19 | QTY: 1 | FL/-/4/-/-/R/- | Scale =.5"/Ft.

DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
01/25/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and WTCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of web shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.

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TC LL	20.0 PSF	REF	R487-- 13463
TC DL	7.0 PSF	DATE	01/25/13
BC DL	10.0 PSF	DRW	HCUSR487 13025061
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	37.0 PSF	SEQN-	48783
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UT7487_Z02

Piggyback Detail - ASCE 7-10: 160 mph, 30' Mean Height, Enclosed, Exposure C, Kzt=1.00

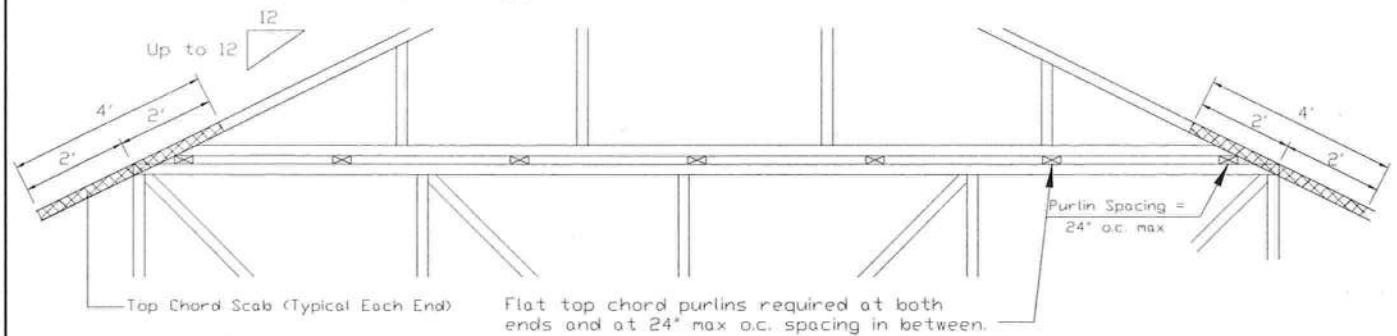
160 mph Wind, 30.00 ft Mean Hgt, ASCE 7-10, Enclosed Bldg. located anywhere in roof, Exp C, Wind DL= 5.0 psf (min), Kzt=1.0.
Or 140 mph wind, 30.00 ft Mean Hgt, ASCE 7-10, Enclosed Bldg. located anywhere in roof, Exp D, wind DL= 5.0 psf (min), Kzt=1.0.

Note: Top chords of trusses supporting piggyback cap trusses must be adequately braced by sheathing or purlins. The building Engineer of Record shall provide diagonal bracing or any other suitable anchorage to permanently restrain purlins, and lateral bracing for out of plane loads over gable ends.

Maximun truss spacing is 24" o.c. detail is not applicable if cap supports additional loads such as cupola, steeple, chimney or drag strut loads.

** Refer to Engineer's sealed truss design drawing for piggyback and base truss specifications.

Detail A : Purlin Spacing = 24" o.c. or less



Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

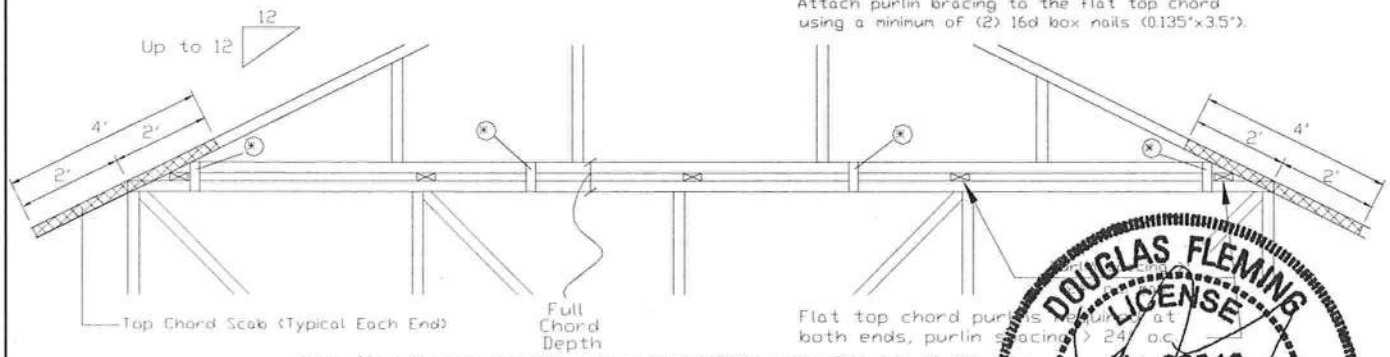
Attach purlin bracing to the flat top chord using (2) 16d box nails (0.135"x3.5").

The top chord #3 grade 2x4 scab may be replaced with either of the following: (1) 3X8 Trulox plate attached with (8) 0.120"x1.375" nails, (4) into cap TC & (4) into base truss TC or (1) 28PB wave piggyback plate plated to the piggyback truss TC and attached to the base truss TC with (4) 0.120"x1.375" nails. Note: Nailing thru holes of wave plate is acceptable.

Detail B : Purlin Spacing > 24" o.c.

Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

Attach purlin bracing to the flat top chord using a minimum of (2) 16d box nails (0.135"x3.5").



Note: If purlins or sheathing are not specified on the flat top of the base truss, purlins must be installed at 24" o.c. max. and use Detail A.

* In addition, provide connection with one of the following methods:

Trulox
Use 3X8 Trulox plates for 2x4 chord member, and 3X10 Trulox plates for 2x6 and larger chord members. Attach to each face @ 8' o.c. with (4) 0.120"x1.375" nails into cap bottom chord and (4) in base truss top chord. Trulox plates may be staggered 4' o.c. front to back faces.

APA Rated Gusset
8"x8"x7/16" (min) APA rated sheathing gussets (each face). Attach @ 8' o.c. with (8) 6d common (0.113"x2") nails per gusset, (4) in cap bottom chord and (4) in base truss top chord. Gussets may be staggered 4' o.c. front to back faces.

2x4 Vertical Scabs
2x4 SPF #2, full chord depth scabs (each face). Attach @ 8' o.c. with (6) 10d box nails (0.128"x3") per scab, (3) in cap bottom chord and (3) in base truss top chord. Scabs may be staggered 4' o.c. front to back faces.

28PB Wave Piggyback Plate
One 28PB wave piggyback plate to each face @ 8' o.c. Attach teeth to piggyback at time of fabrication. Attach to supporting truss with (4) 0.120"x1.375" nails per face per ply. Piggyback plates may be staggered 4' o.c. front to back faces.

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
01/25/2013

Jan 25 '13

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Earth City, MO 63045

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ITWBCG: www.itwbcg.com; TPI: www.tpinst.org; WTCA: www.sbcindustry.org; ICC: www.iccsafe.or

REF	PIGGYBACK
DATE	2/14/12
DRWG	PB160100212
SPACING	24.0"

CLB WEB BRACE SUBSTITUTION

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON A TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

NOTES:

THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB BRACING.

ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE. FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE BRACING.

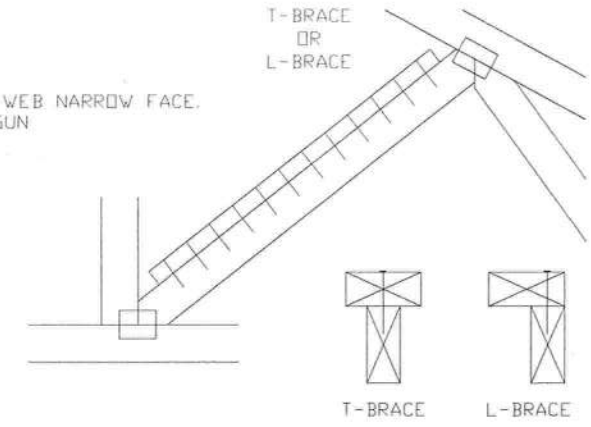
WEB MEMBER SIZE	SPECIFIED CLB BRACING	ALTERNATIVE BRACING T OR L-BRACE	ALTERNATIVE BRACING SCAB BRACE
2X3 OR 2X4	1 ROW	2X4	1-2X4
2X3 OR 2X4	2 ROWS	2X6	2-2X4
2X6	1 ROW	2X4	1-2X6
2X6	2 ROWS	2X6	2-2X4(*)
2X8	1 ROW	2X6	1-2X8
2X8	2 ROWS	2X6	2-2X6(*)

T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

(*) CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH FACE OF WEB.

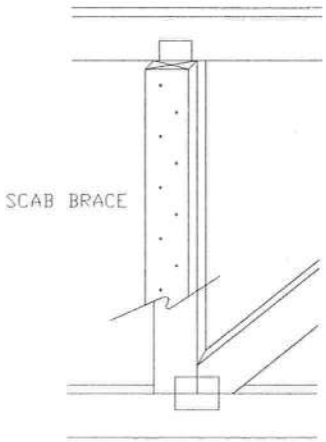
T-BRACING OR L-BRACING:

APPLY TO EITHER SIDE OF WEB NARROW FACE.
ATTACH WITH 10d BOX OR GUN
(0.128"x 3",MIN) NAILS.
AT 6" O.C.
BRACE IS A
MINIMUM 80% OF WEB
MEMBER LENGTH



SCAB BRACING:

APPLY SCAB(S) TO WIDE FACE OF WEB.
NO MORE THAN (1) SCAB PER FACE.
ATTACH WITH 10d BOX OR GUN
(0.128"x 3",MIN) NAILS.
AT 6" O.C.
BRACE IS A MINIMUM
80% OF WEB MEMBER LENGTH



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01/25/2013

Jan 25 '13

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Earth City, MO 63045

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Follow BCSI (Building Component Safety Information, by TPI and WTCA) for safety practices pr
performing these functions. Installers shall provide temporary bracing per BCSI. Unless not
otherwise, top chord shall have properly attached structural panels and bottom chord shall
properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall
bracing installed per BCSI sections B3 & B7. See this job's general notes page for more infor

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design, any failure to build the truss in conformance with TPI, or fabricating, handling, shipping, installing & bracing of trusses. ITWBCG connector plates are made of 20/18/16GA (W,H/S/K) ASTM A653 grade 37/40/60 (K/W/H,S) galv. steel. Apply plates to each face of truss, positioned as shown above and on Joint Details.
A seal on this drawing or cover page indicates acceptance and professional engineering responsibility solely for the truss component design shown. The suitability and use of this component for any building is the responsibility of the Building Designer per ANSI/TPI 1 Sec. 2.
ITW-BCG: www.itwbcg.com; TPI: www.tpinst.com; WTCA: www.sbcindustry.com; ICC: www.iccsafe.org

TC LL	PSF	REF CLB SUBST.
TC DL	PSF	DATE 1/1/09
BC DL	PSF	DRWG BRCLBSUB0109
BC LL	PSF	
TOT. LD.	PSF	
DUR. FAC.		
SPACING		

Gable Stud Reinforcement Detail

ASCE 7-10: 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or: 100 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00
Or: 100 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00

Max Gable Vertical Length

2x4 Gable Vertical Spacing	Species	Brace Grade	No Braces	(1) 1x4 "L" Brace *		(1) 2x4 "L" Brace *		(2) 2x4 "L" Brace **		(1) 2x6 "L" Brace *		(2) 2x6 "L" Brace **	
				Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
24" o.c.	SPF HF	#1 / #2	4' 10"	8' 2"	8' 6"	9' 8"	10' 1"	11' 6"	12' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 7"	7' 9"	8' 3"	9' 7"	9' 11"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 7"	8' 1"	8' 4"	9' 7"	9' 11"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 7"	8' 1"	8' 4"	9' 7"	9' 11"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	4' 11"	8' 3"	8' 7"	9' 9"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 10"	8' 2"	8' 6"	9' 8"	10' 1"	11' 6"	12' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 7"	6' 11"	7' 4"	9' 3"	9' 10"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 7"	6' 11"	7' 4"	9' 3"	9' 10"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 7"	6' 0"	6' 4"	8' 0"	8' 6"	10' 10"	11' 7"	12' 6"	13' 5"	14' 0"	14' 0"
16" o.c.	SPF HF	#1 / #2	5' 6"	9' 5"	9' 9"	11' 1"	11' 6"	13' 2"	13' 9"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 3"	9' 3"	9' 9"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 3"	9' 3"	9' 7"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 3"	9' 3"	9' 7"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5' 8"	9' 5"	9' 10"	11' 2"	11' 7"	13' 3"	13' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	5' 6"	9' 5"	9' 9"	11' 1"	11' 6"	13' 2"	13' 9"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 3"	8' 6"	9' 0"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 3"	8' 6"	9' 0"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 3"	7' 4"	7' 10"	9' 9"	10' 5"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
12" o.c.	SPF HF	#1 / #2	6' 1"	10' 4"	10' 8"	12' 2"	12' 8"	13' 2"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 9"	10' 2"	10' 7"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 9"	10' 2"	10' 7"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 9"	10' 2"	10' 7"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	6' 2"	10' 5"	10' 9"	12' 3"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	6' 1"	10' 4"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 9"	9' 9"	10' 5"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 9"	9' 9"	10' 5"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 9"	8' 6"	9' 0"	11' 3"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"

Bracing Group Species and Grades:

Group A:

Spruce-Pine-Fir	
#1 / #2	Standard
#3	Stud

Hem-Fir	
#2	Stud
#3	Standard

Douglas Fir-Larch
#3
Stud
Standard

Southern Pine***
#3
Stud
Standard

Group B:

Hem-Fir
#1 & Btr
#1

Douglas Fir-Larch
#1
#2

Southern Pine***
#1
#2

1x4 Braces shall be SRB (Stress-Rated Board).

***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:

Wind Load deflection criterion is L/240.

Provide uplift connections for 35 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

So. Pine lumber design values based on the ALSC January, 2012 rulin

Attach "L" braces with 10d (0.128"x3.0" min) nails.

* For (1) "L" brace: space nails at 2" o.c. in 18" end zones and 4" o.c. between zones.

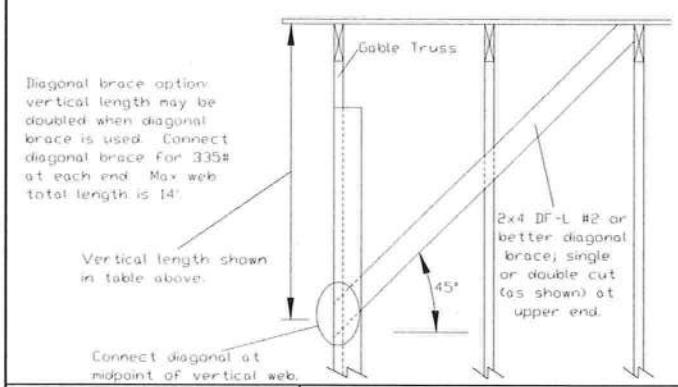
** For (2) "L" braces: space nails at 3" o.c. in 18" end zones and 6" o.c. between zones.

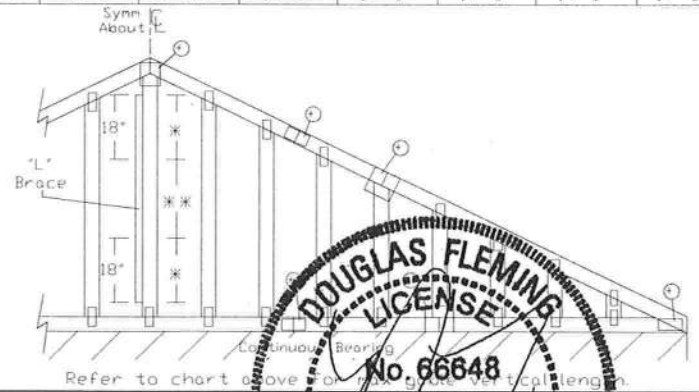
"L" bracing must be a minimum of 80% of web member length.

Gable Vertical Plate Sizes	
Vertical Length	No Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4' 0", but less than 11' 6"	2X4
Greather than 11' 6"	2.5X4

+ Refer to cannon truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.





Refer to chart above for max gable vertical length.

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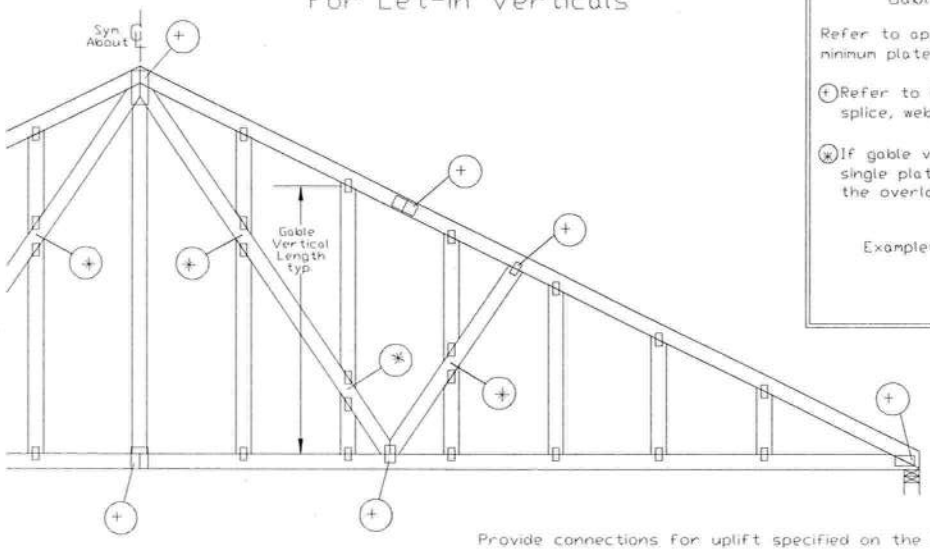
MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF	ASCE7-10-GAB12015
DATE	2/14/12
DRWG	A12015ENC100212

Gable Detail
For Let-in Verticals



Gable Truss Plate Sizes

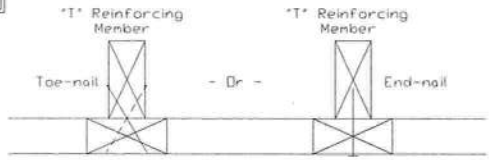
Refer to appropriate ITW gable detail for minimum plate sizes for vertical studs.

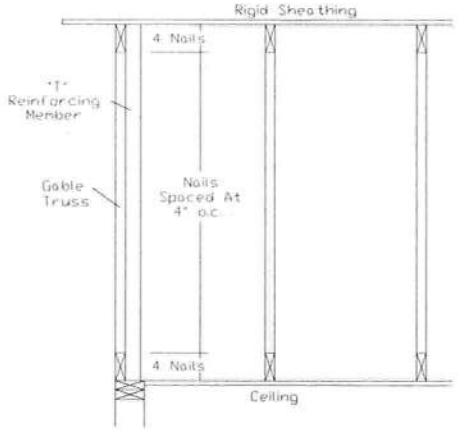
(+) Refer to Engineered truss design for peak, splice, web, and heel plates.

(*) If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.

Example:

"T" Reinforcement Attachment Detail





Provide connections for uplift specified on the engineered truss design.

Attach each "T" reinforcing member with

End Driven Nails:
- 10d Common (0.148"x 3.",min) Nails at 4" o.c. plus (4) nails in the top and bottom chords.

Toenailed Nails:
- 10d Common (0.148"x3",min) Toenails at 4" o.c. plus (4) toenails in the top and bottom chords.

This detail to be used with the appropriate ITW gable detail for ASCE wind load.

ASCE 7-98 Gable Detail Drawings
A13015980109, A12015980109, A11015980109, A10015980109, A13030980109, A12030980109, A11030980109, A10030980109

ASCE 7-02 Gable Detail Drawings
A13015020109, A12015020109, A11015020109, A10015020109, A14015020109, A13030020109, A12030020109, A11030020109, A10030020109, A14030020109

ASCE 7-05 Gable Detail Drawings
A13015050109, A12015050109, A11015050109, A10015050109, A14015050109, A13030050109, A12030050109, A11030050109, A10030050109, A14030050109

ASCE 7-10 Gable Detail Drawings
A11515ENC100212, A12015ENC100212, A14015ENC100212, A10015ENC100212, A18015ENC100212, A20015ENC100212, A20015ENC100212, A10015PEDI00212, A11530ENC100212, A12030ENC100212, A14030ENC100212, A10030ENC100212, A18030ENC100212, A20030ENC100212, A20030ENC100212, A10030PEDI00212

See appropriate ITW gable detail for maximum unbraced gable vertical length.

To convert from "L" to "T" reinforcing members, multiply "T" increase by length (based on appropriate ITW gable detail).

Maximum allowable "T" reinforced gable vertical length is 14' from top to bottom chord.

"T" reinforcing member material must match size, specie, and grade of the "L" reinforcing member.

Web Length Increase w/ "T" Brace

"T" Reinf. Mbr. Size	"T" Increase
2x4	30 %
2x6	20 %

Example:
ASCE 7-10 Wind Speed = 120 mph
Mean Roof Height = 30 ft, Kzt = 1.00
Gable Vertical = 24"o.c. SP #3
"T" Reinforcing Member Size = 2x4
"T" Brace Increase (From Above) = 30% = 1.30
(1) 2x4 "L" Brace Length = 8' 7"
Maximum "T" Reinforced Gable Vertical Length
1.30 x 8' 7" = 11' 2"

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
01/25/2013
Jan 25 '13

TW
Building Components Group Inc.

Earth City, MO 63045

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!
IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and WTCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions.

ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see this job's general notes page and these web sites:
ITWBCG: www.itwbcg.com; TPI: www.tpinst.org; WTCA: www.sbcindustry.org; ICC: www.iccsafe.or

MAX. TOT. LD. 60 PSF
DUR. FAC. ANY
MAX. SPACING 24.0"

REF LET-IN VERT
DATE 2/16/12
DRWG GBLLETIN0212

ASCE 7-10: 120 mph, 30' Mean Height, Closed, Exposure C
Common Residential Gable End Wind Bracing Requirements - Stiffeners

120 mph, 30ft. Mean Hgt, ASCE 7-10, Enclosed, Exp C, or
100 mph, 30ft. Mean Hgt, ASCE 7-10, Enclosed, Exp D, or
100 mph, 30ft. Mean Hgt, ASCE 7-10, Part. Enclosed, Exp C,
Kzt = 1.00, Wind TC DL=5.0 psf, Wind BC DL=5.0 psf.

Lateral chord bracing requirements
Top: Continuous roof sheathing
Bot: Continuous ceiling diaphragm

See Engineer's sealed design referencing this detail for lumber, plates, and other information not shown on this detail.

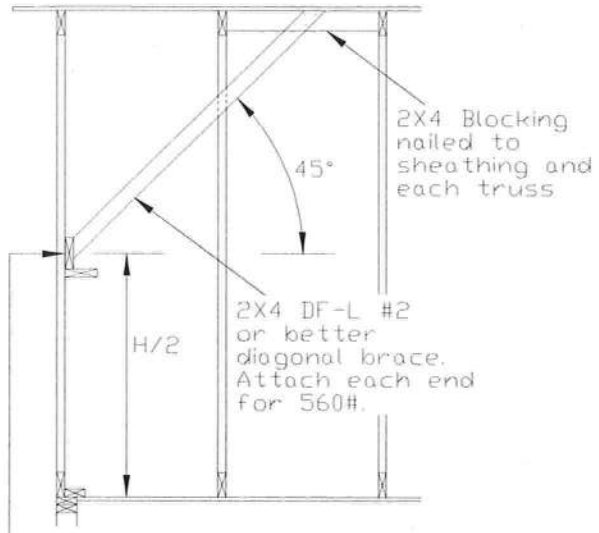
Nails: 10d box or gun (0.128"x3",min) nails.

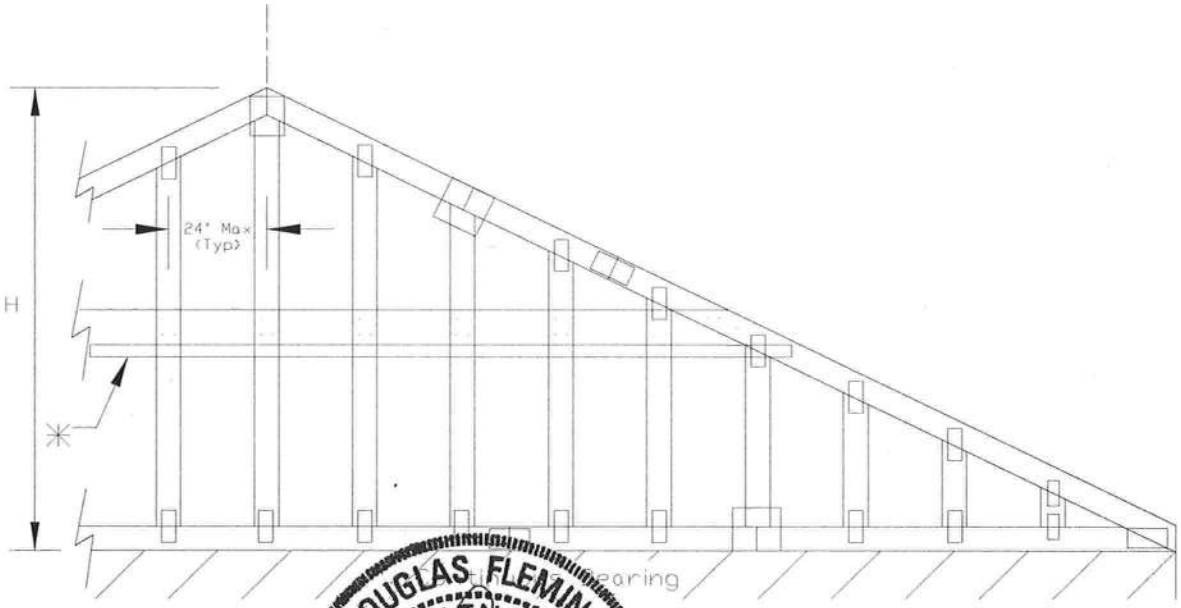
H Less than 4'6" - no stud bracing required

H Greater than 4'6" to 7'6" in length
provide a 2x6 stiffback at mid-height and brace stiffback to roof diaphragm every 6'0" (see detail below or refer to DRWG A12030ENC10).

H Greater than 7'6" to 12'0" max:
provide a 2x6 stiffback at mid-height and brace to roof diaphragm every 4'0" (see detail below or refer to DRWG A12030ENC10).

✳ Optional 2x L-reinforcement attached to stiffback with 10d box or gun (0.128" x 3", min.) nails @ 6" o.c.





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MAX. TOT. LD. 60 PSF

MAX. SPACING

REF GE WHALER
DATE 2/14/12
DRWG GABRST100212

NAIL SPACING DETAIL

MINIMUM SPACING FOR SINGLE BLOCK IS SHOWN. DOUBLE NAIL SPACINGS AND STAGGER NAILING FOR TWO BLOCKS. GREATER SPACING MAY BE REQUIRED TO AVOID SPLITTING.

BLOCK LOCATION, SIZE, LENGTH,GRADE AND TOTAL NUMBER AND TYPE OF NAILS ARE TO BE SPECIFIED ON SEALED DESIGN REFERENCING THIS DETAIL.

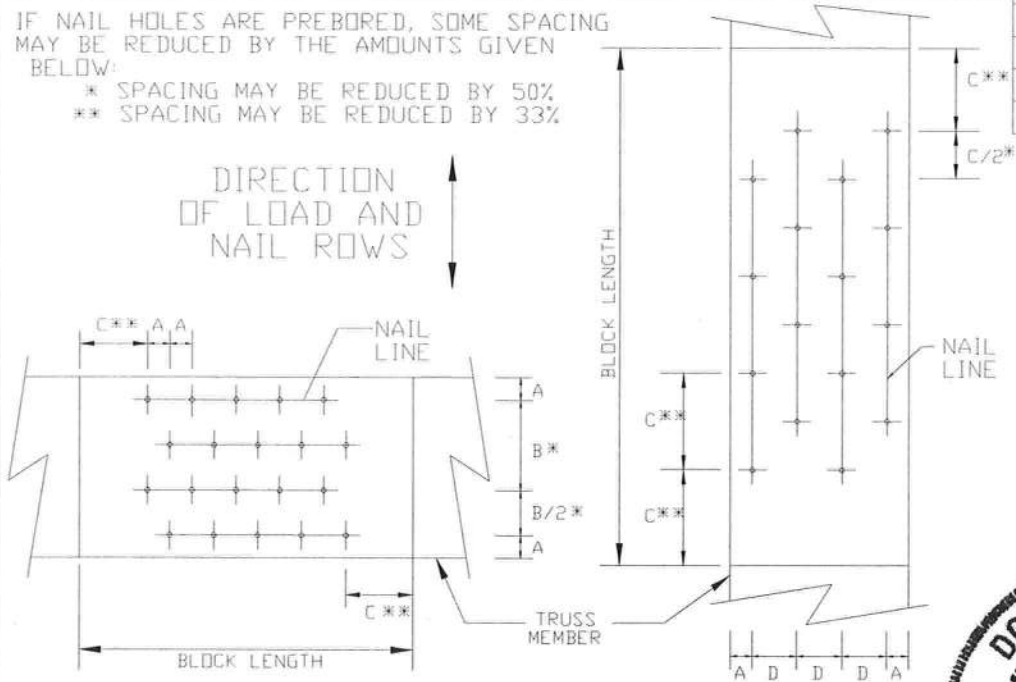
LOAD PERPENDICULAR TO GRAIN
A - EDGE DISTANCE AND SPACING BETWEEN STAGGERED ROWS OF NAILS (6 NAIL DIAMETERS)
B - SPACING OF NAILS IN A ROW (12 NAIL DIAMETERS)
C - END DISTANCE (15 NAIL DIAMETERS)

LOAD PARALLEL TO GRAIN
A - EDGE DISTANCE (6 NAIL DIAMETERS)
C - SPACING OF NAILS IN A ROW AND END DISTANCE (15 NAIL DIAMETERS)
D - SPACING BETWEEN STAGGERED ROWS OF NAILS (7 1/2 NAIL DIAMETERS)

IF NAIL HOLES ARE PREBORED, SOME SPACING MAY BE REDUCED BY THE AMOUNTS GIVEN BELOW:
* SPACING MAY BE REDUCED BY 50%
** SPACING MAY BE REDUCED BY 33%

MINIMUM NAIL SPACING DISTANCES

NAIL TYPE	DISTANCES A	B*	C**	D
8d BOX (0.113"X 2.5",MIN)	3/4"	1 3/8"	1 3/4"	7/8"
10d BOX (0.128"X 3.",MIN)	7/8"	1 5/8"	2"	1"
12d BOX (0.128"X 3.25",MIN)	7/8"	1 5/8"	2"	1"
16d BOX (0.135"X 3.5",MIN)	7/8"	1 5/8"	2 1/8"	1 1/8"
20d BOX (0.148"X 4.",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
8d COMMON (0.131"X 2.5",MIN)	7/8"	1 5/8"	2"	1"
10d COMMON (0.148"X 3.",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
12d COMMON (0.148"X 3.25",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
16d COMMON (0.162"X 3.5",MIN)	1'	2"	2 1/2"	1 1/4"
GUN (0.120"X 2.5",MIN)	3/4"	1 1/2"	1 7/8"	1"
GUN (0.131"X 2.5",MIN)	7/8"	1 5/8"	2"	1"
GUN (0.120"X 3.",MIN)	3/4"	1 1/2"	1 7/8"	1"
GUN (0.131"X 3.",MIN)	7/8"	1 5/8"	2"	1"



LOAD APPLIED PERPENDICULAR TO GRAIN LOAD APPLIED PARALLEL TO GRAIN

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
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IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design, any failure to build the truss in conformance with TPI, or fabricating, handling, shipping, installing & bracing of trusses. ITWBCG connector plates are made of 20/18/16GA (W,H/S/K) ASTM A653 grade 37/40/60 (K/W/H,S) galv. steel. Apply plates to each face of truss, positioned as shown above and on Joint Details.
A seal on this drawing or cover page indicates acceptance and professional engineering responsibility solely for the truss component design shown. The suitability and use of this component for any building is the responsibility of the Building Designer per ANSI/TPI 1 Sec. 2.
ITW-BCG: www.itwbcg.com; TPI: www.tpinst.com; WTCA: www.sbcindustry.com; ICC: www.iccsafe.org

Jan 25 '13

REF	NAIL SPACE
DATE	1/1/09
DRWG	CNNAILSP0109