



Alpine, an ITW Company 6750 Forum Drive, Suite 305 Orlando, FL 32821 Phone: (800)755-6001 www.alpineitw.com

Site Information:

Customer: W. B. Howland Company, Inc.

Job Number: 20-4966

Job Description: Dale and Karla Nickelson Res

Address:

Job Engineering Criteria:				
Design Code: FBC 7th Ed. 2020 Res	IntelliVIEW Version: 20.01.01A through 20.02.01A			
	JRef #: 1X2e2150002			
Wind Standard: ASCE 7-16 Wind Speed (mph): 130	Design Loading (psf): 40.00, 55.00			
Building Type: Closed				

This package contains general notes pages, 42 truss drawing(s) and 6 detail(s).

Item	Drawing Number	Truss
1	026.21.1006.45375	A01
3	026.21.1028.22487	A01B
5	026.21.1006.45532	A03
7	026.21.1006.46094	A05
9	026.21.1006.45547	A07
11	026.21.1006.45579	B01
13	026.21.1006.46032	B03
15	026.21.1006.46171	B05
17	026.21.1006.45718	C02
19	026.21.1006.45968	C04
21	026.21.1006.46157	C06
23	026.21.1006.46188	E01
25	026.21.1006.46313	E03
27	026.21.1006.45954	F04
29	026.21.1006.46296	FT02
31	026.21.1006.46063	M02
33	026.21.1006.45735	M04
35	026.21.1006.45781	PB02
37	026.21.1006.45796	PB03
39	026.21.1006.45766	PB06
41	026.21.1006.46235	PB08
43	A14015ENC160118	
45	BRCLBSUB0119	
47	PB160160118	

Item	Drawing Number	Truss
2	026.21.1006.46126	A01A
4	026.21.1006.45703	A02
6	026.21.1006.45406	A04
8	026.21.1006.45282	A06
10	026.21.1006.45875	A08
12	026.21.1006.45485	B02
14	026.21.1006.45593	B04
16	026.21.1006.45656	C01
18	026.21.1006.45907	C03
20	026.21.1006.45329	C05
22	026.21.1006.45923	D01
24	026.21.1006.46001	E02
26	026.21.1006.45516	F03
28	026.21.1006.45359	F05
30	026.21.1006.45844	M01
32	026.21.1006.45313	M03
34	026.21.1006.46218	PB01
36	026.21.1006.46156	PB03
38	026.21.1006.45344	PB04
40	026.21.1006.45578	PB07
42	026.21.1006.45921	PB09
44	A14030ENC160118	
46	GBLLETIN0118	
48	STRBRIBR1014	

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed and detailed by the Building Designer.

Connector Plate Information:

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for of all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for of all load cases.

Max Web CSI= Maximum bending and axial Combined Stress Index for Webs for of all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment. W = Width of non-hanger bearing, in inches.

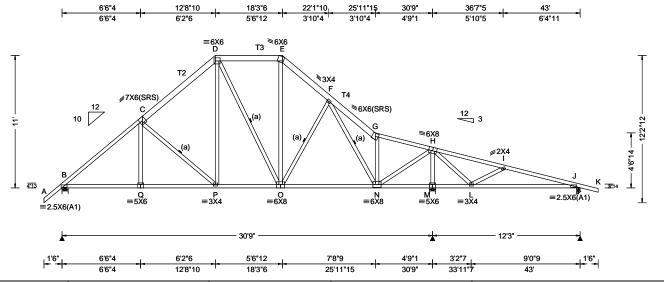
Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

References:

- 1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
- 2. ICC: International Code Council; www.iccsafe.org.
- 3. Alpine, a division of ITW Building Components Group Inc.: 514 Earth City Expressway, Suite 242, Earth City, MO 63045; www.alpineitw.com.
- 4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
- 5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcindustry.com.

Job Number: 20-4966 Ply: 1 SEQN: 335897 / T27 / COMN Cust: R215 JRef: 1X2e2150002 DrwNo: 026.21.1006.45375 Dale and Karla Nickelson Res Qty: 2 FROM: CDM Truss Label: A01 KD / WHK 01/26/2021



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria	1
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.052 P 999 240	١.
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(TL): 0.102 P 999 180	
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.025 N	
Des Ld: 40.00	EXP: C		HORZ(TL): 0.049 N	١,
NCBCLL: 10.00	Mean Height: 16.23 ft TCDL: 5.0 psf	Code / Misc Criteria	Creep Factor: 2.0	ľ
Soffit: 2.00	BCDL: 5.0 psf	Bldg Code: FBC 7th Ed. 202	OMRaesTC CSI: 0.558	
Load Duration: 1.25	MWFRS Parallel Dist: > 2h	TPI Std: 2014	Max BC CSI: 0.734	
Spacing: 24.0 "	C&C Dist a: 4.30 ft	Rep Factors Used: Yes	Max Web CSI: 0.784	
' "	Loc. from endwall: not in 13.00 ft	FT/RT:20(0)/10(0)		
	GCpi: 0.18	Plate Type(s):		1
	Wind Duration: 1.60	WAVE	VIEW Ver: 20.02.01A.1209.11	
Lumban				

▲ Maximum Reactions (lbs)						
	G	ravity		No	n-Grav	rity
Loc	R+	/ R-	/ Rh	/Rw	/ U	/ RL
В	1479	/-	/-	/878	/-	/365
М	2219	/-	/-	/1095	/-	/-
J	440	/-	/-	/221	/99	/-
Win	d reac	tions ba	sed on M	IWFRS		
В	Brg W	/idth = 6	.0	Min Red	q = 1.7	
	Brg W	/idth = 6	.0	Min Red	q = 2.6	
J	Brg W	/idth = 3	.5	Min Red	q = 1.5	
Bearings B, M, & J are a rigid surface.						
Members not listed have forces less than 375#						
Max	Maximum Top Chord Forces Per Ply (lbs)					
Cho	rds T	ens.Cor	np. C	Chords	Tens.	Comp.

Lumber

Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP #2 T2,T3, T4 2x6 SP 2400f-2.0E; Bot chord 2x4 SP #2

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Bracing

(a) Continuous lateral restraint equally spaced on member.

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Bottom chord checked for 10.00 psf non-concurrent live load

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

Refer to DWG PB160160118 for piggyback details. The overall height of this truss excluding overhang is 11-0-0.



Loc	R+	/ R-	/ Rh	/Rw	/ U	/ RL
В	1479	/-	/-	/878	/-	/365
М	2219	/-	/-	/1095	/-	/-
J	440	/-	/-	/221	/99	/-
Win	d read	tions bas	sed on M\	WFRS		
В	Brg W	/idth = 6	.0	Min Re	q = 1.7	
М	Brg V	/idth = 6	.0	Min Re	q = 2.6	
J	Brg V	/idth = 3	.5	Min Re	q = 1.5	
Bea	rings I	3, M, & J	are a rigi	d surfac	ce.	
Mer	nbers	not listed	have for	ces less	than 3	75#
Max	cimum	Top Ch	ord Forc	es Per	Ply (lbs	s)
Cho	ords T	ens.Con	np. Ch	nords	Tens.	Comp.
_						

B-C	0 - 1811	F-G	0	- 1483
C - D	0 - 1410	G - H	0	- 1119
D-E	4 - 973	I - J	166	- 391
E-F	0 - 1335			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.C	omp.	Chords	Tens. (Comp.
B-Q	1289	- 29	O - N	1073	0
Q - P	1286	- 30	N - M	57	- 792
P - O	985	0	M - L	58	- 766

Maximum Web Forces Per Ply (lbs)

vvebs	i ens.c	omp.	webs	i ens.	Comp.
C-P	130	- 410	N - H	2059	0
D - P	455	- 32	M - H	0	- 2215
0 - E	484	- 17	H-L	662	0
G - N	15	- 906	L-I	138	- 563

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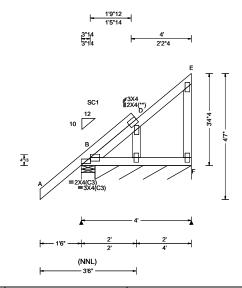
WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec. 2.

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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.001 G 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(TL): 0.001 G 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.000 E
Des Ld: 40.00	EXP: C		HORZ(TL): 0.001 E
NCBCLL: 10.00	Mean Height: 15.00 ft TCDL: 5.0 psf	Code / Misc Criteria	Creep Factor: 2.0
Soffit: 2.00	BCDL: 5.0 psf	Bldg Code: FBC 7th Ed. 202	OMReesTC CSI: 0.128
Load Duration: 1.25	MWFRS Parallel Dist: > 2h	TPI Std: 2014	Max BC CSI: 0.044
Spacing: 16.0 "	C&C Dist a: 3.00 ft	Rep Factors Used: Yes	Max Web CSI: 0.011
	Loc. from endwall: not in 9.00 ft	FT/RT:20(0)/10(0)	
	GCpi: 0.18	Plate Type(s):	
	Wind Duration: 1.60	WAVE	VIEW Ver: 20.02.01A.1209.11

▲ Maximum Reactions (Ibs), or *=PLF						
	G	avity		No	on-Gra	vity
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
В	172	/-	/-	/141	/17	/47
F*	37	/-	/-	/29	/-	/-
Wir	nd read	ctions b	ased on I	MWFRS		
В	Brg V	Vidth =	6.0	Min Re	q = 1.5	5
F	Brg V	Vidth =	42.0	Min Re	q = -	
Bearings B & B are a rigid surface.						
Members not listed have forces less than 375#						

Lumber

Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP #2 Bot chord 2x4 SP #2 Webs 2x4 SP #3

Stack Chord: SC1 2x4 SP #2;

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Plating Notes

All plates are 2X4 except as noted.

**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

In lieu of structural panels use purlins to brace TC @ 24" oc.

Wind loads based on MWFRS with additional C&C

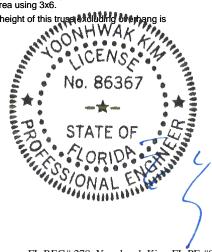
Right end vertical not exposed to wind pressure. Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

The overall height of this truss excluding overhang is



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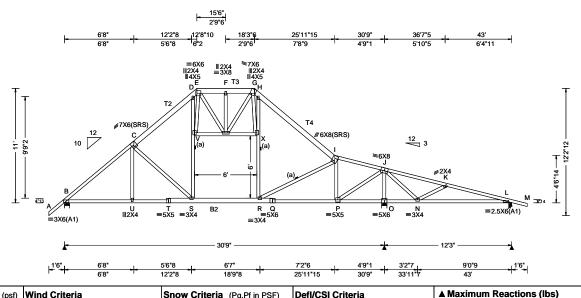
WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.085 S 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.203 S 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.093 R
Des Ld: 40.00	EXP: C Kzt: NA Mean Height: 16.23 ft		HORZ(TL): 0.224 R
NCBCLL: 10.00	TCDL: 5.0 psf	Building Code:	Creep Factor: 2.0
Soffit: 2.00	BCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.723
Load Duration: 1.25	MWFRS Parallel Dist: > 2h	TPI Std: 2014	Max BC CSI: 0.787
Spacing: 24.0 "	C&C Dist a: 4.30 ft	Rep Fac: Varies by Ld Case	Max Web CSI: 0.830
	Loc. from endwall: not in 13.00 ft	FT/RT:20(0)/10(0)	
	GCpi: 0.18	Plate Type(s):	
	Wind Duration: 1.60	WAVE	VIEW Ver: 20.01.01A.0724.11
Lumber		Wind	

v L	
Wind	

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

Refer to DWG PB160160118 for piggyback details. The overall height of this truss excluding overhang is

B - C 0 - 2172G - H - 1782 C-D 0 - 1831 0 - 1808 H - I 25 - 1553 57 - 1270 D-E I - J F-F 0 - 1205.I - K 437 - 311 F-G 0 - 1206K-L 324 - 661

Chords

Gravity

/-510

Brg Width = 6.0

Brg Width = 3.5

Chords Tens.Comp.

Chords Tens.Comp.

/Rh

/-

Wind reactions based on MWFRS Brg Width = 6.0

Bearings B, O, & L are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs)

Loc R+

2362

В 1721 /-

O

O

Non-Gravity

/RL

/365

Tens. Comp.

/Rw /U

/1181 /-

Min Req = 2.0

Min Req = 2.8

Min Rea = 1.5

Chords Tens. Comp.

/916

/221

11-0-0

B - U	1566	- 68	Q-P	1282	
U - T	1565	- 69	P - O	335	- 87
T - S	1566	-68	O - N	335	- 84
S - R	1298	0	N - L	615	- 30

Maximum Bot Chord Forces Per Ply (lbs)

0 71 45 02 1282

Maxim	um Web Forces	Per Ply	(lbs)
Webs	Tens.Comp.	Webs	Te

Webs	Tens.C	comp.	Webs	Tens.	Comp.
C-S	175	- 424	I-P	0	- 1169
S - V	768	- 14	P - J	2179	0
V - E	712	-83	O - J	0	- 2301
G - X	948	- 97	J - N	640	0
X - H	147	- 439	N - K	133	- 562
X - R	564	-3			

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Mechanical Unit Loads Supported by this Truss

Top chord: 2x4 SP #2; T2,T3, T4 2x6 SP 2400f-2.0E;

indicate 30.00 psf MAX LL.

Webs: 2x4 SP #3;

Bracing

member

Loading

Bot chord: 2x4 SP #2; B2 2x6 SP 2400f-2.0E;

(a) Continuous lateral restraint equally spaced on

Attic room loading from 12-6-0 to 18-6-0: Live Load: 30

PSF. Dead Load: 5 PSF Ceiling: 5 PSF, Kneewalls: 5

Truss designed for sleeping room only. No waterbeds permitted. Provide information to contractor, architect, and bldg owner. Trusses to be visibly stamped to

At	Truss	Unit	Unit	Supporting
X-Loc	Piece	Lbs.	Width	Trusses
15.07	BC	200.0	3.00	3
15.07	BC	200.0	3.00	3
15.07	BC	200.0	3.00	3
15.07	BC	200.0	3.00	3

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

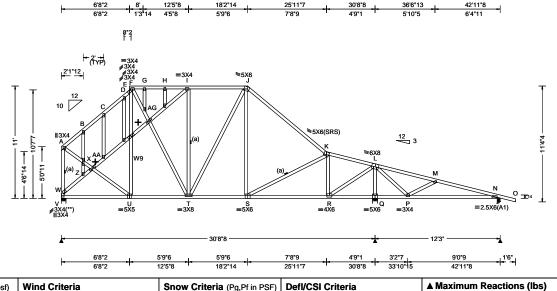
WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.052 H 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(TL): 0.100 H 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.048 C
Des Ld: 40.00	EXP: C		HORZ(TL): 0.093 C
NCBCLL: 10.00	Mean Height: 15.47 ft TCDL: 5.0 psf	Code / Misc Criteria	Creep Factor: 2.0
Soffit: 2.00	BCDL: 5.0 psf	Bldg Code: FBC 7th Ed. 202	OMReesTC CSI: 0.755
Load Duration: 1.25	MWFRS Parallel Dist: > 2h	TPI Std: 2014	Max BC CSI: 0.529
Spacing: 24.0 "	C&C Dist a: 4.30 ft	Rep Factors Used: Yes	Max Web CSI: 0.984
'	Loc. from endwall: not in 13.00 ft	FT/RT:20(0)/10(0)	
	GCpi: 0.18	Plate Type(s):	
	Wind Duration: 1.60	WAVE	VIEW Ver: 20.02.01A.1209.11
Lumber		Wind	

Wind

Wind loads based on MWFRS with additional C&C member design.

Left end vertical not exposed to wind pressure. Wind loading based on both gable and hip roof types.

+ Member to be laterally braced for horizontal wind loads. bracing system to be desiged and furnished by others.

1383 /680 /293 2147 /-/1117 /-442 /218 /101 Wind reactions based on MWFRS

Non-Gravity

/RL

/Rw /U

Brg Width = 5.5 Min Reg = 1.6Brg Width = 6.0 Min Req = 2.5 Min Rea = 1.5

/Rh

Gravity

Loc R+

Brg Width = 3.5 Bearings V, Q, & N are a rigid surface.

Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

A - B	2	- 828	H-I	89	- 742
B-C	_	- 864	11	-	- 967
C - D	84	- 820	J - K	36	- 1392
D-F	112	- 795	K-L	0	- 1022
E - G	85	- 730	M - N	135	- 402
G - H	90	- 743			

Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords V - U 384 S-R 1032 822 R-Q 63 - 778 T - S Q-P 946 63 - 754

Maximum Web Forces Per Ply (lbs)

webs	rens.comp.	webs	rens. Comp.
A - W	0 - 1036	E-F	94 - 437
A - X	804 0	E -AG	420 - 90
V - W	12 - 1328	K-R	53 - 979
W - Z	47 - 459	R-L	1951 0
X -AA	748 0	Q-L	2 - 2109
Z -AA	46 - 404	L-P	654 0
AA- U	600 -1	P - M	139 - 570

FL REG# 278, Yoonhwak Kim, FL PE #86367 01/26/2021

1/30/2013 by ALSC Bracing

(a) Continuous lateral restraint equally spaced on member.

Lumber value set "13B" uses design values approved

Plating Notes

All plates are 2X4 except as noted.

Value Set: 13B (Effective 6/1/2013)

Bot chord 2x4 SP #2 Webs 2x4 SP #3 W9 2x4 SP #2;

Top chord 2x4 SP #2

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Bottom chord checked for 10.00 psf non-concurrent live load

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Additional Notes

Refer to DWG PB160160118 for piggyback details.

The overall height of this truss excluding overhang is 11-0-0.

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

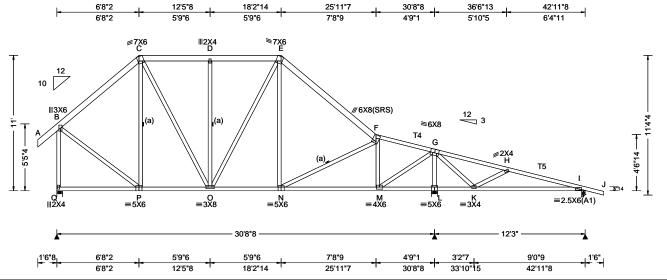
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2400 Lake Orange Dr. Suite 150 Orlando FL, 32837

Job Number: 20-4966 Ply: 1 SEQN: 335952 / T37 / SPEC Cust: R215 JRef: 1X2e2150002 DrwNo: 026.21.1006.45532 Dale and Karla Nickelson Res Qty: 11 FROM: CDM Truss Label: A03 KD / WHK 01/26/2021



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	4
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	١.
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.041 N 999 240	L
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(TL): 0.077 K 999 180	1
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.015 M	L
Des Ld: 40.00	EXP: C		HORZ(TL): 0.031 M	1
NCBCLL: 10.00	Mean Height: 15.47 ft TCDL: 5.0 psf	Code / Misc Criteria	Creep Factor: 2.0	١
Soffit: 2.00	BCDL: 5.0 psf	Bldg Code: FBC 7th Ed. 202	OMRaesTC CSI: 0.560	10
Load Duration: 1.25	MWFRS Parallel Dist: > 2h	TPI Std: 2014	Max BC CSI: 0.541	ŀ
Spacing: 24.0 "	C&C Dist a: 4.30 ft	Rep Factors Used: Yes	Max Web CSI: 0.764	'
J - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 -	Loc. from endwall: not in 13.00 ft	FT/RT:20(0)/10(0)		ľ
	GCpi: 0.18	Plate Type(s):] ,
	Wind Duration: 1.60	WAVE	VIEW Ver: 20.02.01A.1209.11	0
Lumbor			•	

▲ M	▲ Maximum Reactions (lbs)						
Gravity				Non-Gravity			
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL	
Q	1538	/-	/-	/771	/-	/341	
L	2147	/-	/-	/1098	/-	/-	
1	450	/-	/-	/226	/106	/-	
Win	d read	tions ba	sed on I	MWFRS			
Q	Brg W	/idth = 5	5.5	Min Red	q = 1.8		
L	Brg W	/idth = 6	6.0	Min Red	q = 2.5		
1	Brg W	/idth = 3	3.5	Min Red	q = 1.5		
Bearings Q, L, & I are a rigid surface.							
Members not listed have forces less than 375#							
Max	Maximum Top Chord Forces Per Ply (lbs)						
Cho	rds T	ens.Co	mp.	Chords	Tens.	Ćomp.	

B - C 0 - 1100 - 1440 E - F 0 C-D 0 - 1013 F-G 0 - 1079 D-E 0 - 1013 133 - 432

Bracing

Bot chord 2x4 SP #2 Webs 2x4 SP #3

1/30/2013 by ALSC

(a) Continuous lateral restraint equally spaced on member.

Lumber value set "13B" uses design values approved

Top chord 2x6 SP 2400f-2.0E T4,T5 2x4 SP #2;

Value Set: 13B (Effective 6/1/2013)

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Bottom chord checked for 10.00 psf non-concurrent live load.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

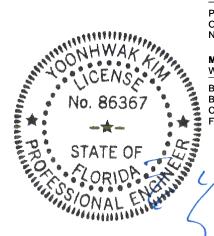
Wind loads based on MWFRS with additional C&C member design.

Left end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

Refer to DWG PB160160118 for piggyback details. The overall height of this truss excluding overhang is 11-0-0.



FL REG# 278, Yoonhwak Kim, FL PE #86367 01/26/2021

Maximum Bot Chord Forces Per Plv (lbs)

Chords	Tens.Comp.		Chords	Tens. (Comp.	
-0	746	-2	M - L	36	- 709	
) - N	994	0	L-K	37	- 684	
1 - M	1085	0	K-I	394	- 78	

Maximum Web Forces Per Ply (lbs)

0
80
0
65

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

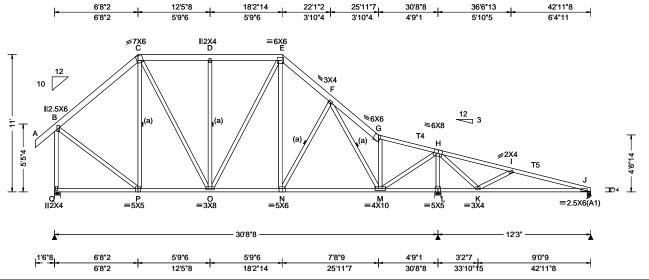
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Job Number: 20-4966 Ply: 1 SEQN: 335908 / T28 / COMN Cust: R215 JRef: 1X2e2150002 Qty: 1 DrwNo: 026.21.1006.45406 Dale and Karla Nickelson Res FROM: CDM Truss Label: A04 KD / WHK 01/26/2021



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria	
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.039 K 999 240	
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(TL): 0.079 K 999 180	
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.012 M	
Des Ld: 40.00	EXP: C		HORZ(TL): 0.025 M	
NCBCLL: 10.00	Mean Height: 15.66 ft TCDL: 5.0 psf	Code / Misc Criteria	Creep Factor: 2.0	
Soffit: 2.00	BCDL: 5.0 psf	Bldg Code: FBC 7th Ed. 202	ONReesTC CSI: 0.627	
Load Duration: 1.25	MWFRS Parallel Dist: h to 2h	TPI Std: 2014	Max BC CSI: 0.541	
Spacing: 24.0 "	C&C Dist a: 4.30 ft	Rep Factors Used: Yes	Max Web CSI: 0.666	
-	Loc. from endwall: not in 13.00 ft	FT/RT:20(0)/10(0)		
	GCpi: 0.18	Plate Type(s):		1
	Wind Duration: 1.60	WAVE	VIEW Ver: 20.02.01A.1209.11	ı

▲ M	▲ Maximum Reactions (lbs)					
	G	ravity		No	n-Gra	vity
Loc	R+	/ R-	/Rh	/ Rw	/U	/ RL
Q	1341	/-	/-	/767	/53	/332
L	2017	/-	/-	/1104	/70	/-
J	356	/-	/-	/164	/32	/-
Win	d reac	tions ba	sed on I	MWFRS		
Q	Brg W	/idth = 5	.5	Min Re	q = 1.0	6
L	Brg W	/idth = 6	6.0	Min Re	q = 2.4	4
J	Brg W	/idth = 3	.5	Min Re	q = 1.5	5
Bea	Bearings Q, L, & J are a rigid surface.					
Members not listed have forces less than 375#						
Maximum Top Chord Forces Per Ply (lbs)						
Cho	rds T	ens.Cor	np.	Chords	Tens.	Ćomp.

Lumber

Value Set: 13B (Effective 6/1/2013)

Top chord 2x6 SP 2400f-2.0E T4,T5 2x4 SP #2; Bot chord 2x4 SP #2 Webs 2x4 SP #3

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Bracing

(a) Continuous lateral restraint equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Left end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 11-0-0.



- 833 F-F 175 - 1140

G-H

- 1271

- 953

- 505

129

45

102

Maximum Bot Chord Forces Per Ply (lbs)

- 933

129

160 - 833

160

B - C

C-D

D-E

Cilolus	rens.comp.		Cilolus	i ciio. V	Jonnp.
P - O	618	- 11	M - L	77	- 691
O - N	817	0	L-K	76	- 668
N - M	916	0	K-J	467	-68

Maximum Web Forces Per Ply (lbs)

vvens	rens.comp.	webs	rens. Comp.
B-Q	80 - 1287	M - H	1748 - 44
B - P	771 -41	L-H	83 - 1985
C - O	447 - 144	H-K	677 - 5
G - M	103 -812	K - I	144 - 580

FL REG# 278, Yoonhwak Kim, FL PE #86367 01/26/2021

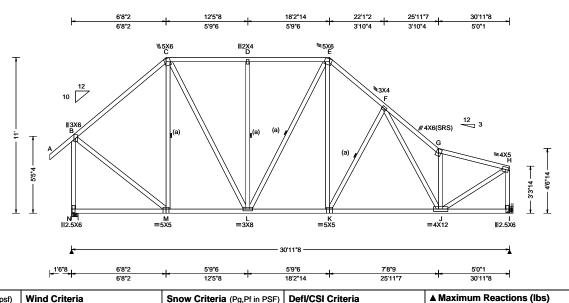
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Job Number: 20-4966 Ply: 1 SEQN: 335954 / T42 / COMN Cust: R215 JRef: 1X2e2150002 DrwNo: 026.21.1006.46094 Dale and Karla Nickelson Res Qty: 5 FROM: CDM Truss Label: A05 Page 1 of 2 KD / WHK 01/26/2021



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.070 K 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(TL): 0.126 K 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.024 C
Des Ld: 40.00	EXP: C Mean Height: 17.16 ft		HORZ(TL): 0.044 C
NCBCLL: 10.00	TCDL: 5.0 psf	Code / Misc Criteria	Creep Factor: 2.0
Soffit: 2.00	BCDL: 5.0 psf	Bldg Code: FBC 7th Ed. 202	ONReesTC CSI: 0.687
Load Duration: 1.25	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max BC CSI: 0.782
Spacing: 24.0 "	C&C Dist a: 3.10 ft	Rep Factors Used: Yes	Max Web CSI: 0.865
	Loc. from endwall: not in 4.50 ft	FT/RT:20(0)/10(0)	
	GCpi: 0.18	Plate Type(s):	
	Wind Duration: 1.60	WAVE	VIEW Ver: 20.02.01A.1209.11
Lumber		Loading	

Lumber

Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP #2 Bot chord 2x4 SP #2 Webs 2x4 SP #3

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Bracing

(a) Continuous lateral restraint equally spaced on member.

Hangers / Ties

Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

Bearing at location x=30'8"8 uses the following support conditions: 30'8"8

Bearing I (30'8"8, 10') HUS26 Supporting Member: (2)2x6 SP 2400f-2.0E (14) 0.148"x3" nails into supporting

member, (4) 0.148"x3" nails into supported member.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance. Bottom chord checked for 10.00 psf non-concurrent live load.

Purlins

 $I_{\underline{n}}$ lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MMPRS with additional member design.
End verticals not exposed to wind pressure. End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



Gravity Non-Gravity Loc R+ /R /Rh /Rw /U /RL /796 1608 /-/281 /269 1490 /-/708 /247 /-Wind reactions based on MWFRS Brg Width = 5.5Ν Min Rea = 1.9Brg Width = -Min Reg = -Bearing N is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

B - C 404 - 1548 283 - 1147 C-D 359 - 1097 F-G 460 - 2134 D-E 359 - 1097 G-H 264 - 1619

Maximum Bot Chord Forces Per Ply (lbs)						
Chords	Tens.C	omp.	Chords	Tens. 0	Comp.	
M - L L - K	782 1128	-	K-J	1323	- 173	

Maximum Web Forces Per Plv (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
B - N	308 - 1560	F-J	552 - 143
B - M	984 - 80	G - J	341 - 1221
C - L	660 - 143	J - H	1803 - 265
E-K	690 - 118	H - I	261 - 1472
K-F	214 - 414		

FL REG# 278, Yoonhwak Kim, FL PE #86367 01/26/2021

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Job Number: 20-4966 Ply: 1 SEQN: 335954 / T42 / COMN Cust: R215 JRef: 1X2e2150002 Dale and Karla Nickelson Res Qty: 5 FROM: CDM DrwNo: 026.21.1006.46094 Truss Label: A05 Page 2 of 2 KD / WHK 01/26/2021

Additional Notes

The overall height of this truss excluding overhang is 11-0-0.



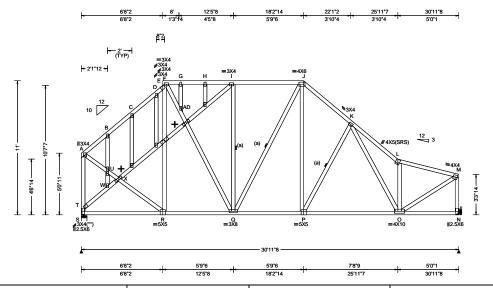
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.050 P 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(TL): 0.104 P 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.045 C
Des Ld: 40.00	EXP: C		HORZ(TL): 0.093 C
NCBCLL: 10.00	Mean Height: 17.16 ft TCDL: 5.0 psf	Code / Misc Criteria	Creep Factor: 2.0
Soffit: 2.00	BCDL: 5.0 psf	Bldg Code: FBC 7th Ed. 202	OMRæsTC CSI: 0.391
Load Duration: 1.25	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max BC CSI: 0.557
Spacing: 24.0 "	C&C Dist a: 3.10 ft	Rep Factors Used: Yes	Max Web CSI: 0.948
	Loc. from endwall: not in 4.50 ft	FT/RT:20(0)/10(0)	
	GCpi: 0.18	Plate Type(s):	
	Wind Duration: 1.60	WAVE	VIEW Ver: 20.02.01A.1209.11

Lumber

Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP #2 Bot chord 2x4 SP #2 Webs 2x4 SP #3

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 2X4 except as noted.

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is

+ Member to be laterally braced for horizontal wind loads. bracing system to be desiged and furnished

▲ Maximum Reactions (lbs) Gravity

Loc	: R+	/ R-	/ Rh	/ Rw	/ U	/ RL
s	1292	/-	/-	/704	/261	/221
N	1282	/-	/-	/713	/239	/-
Wind reactions based on MWFRS						

Non-Gravity

Brg Width = 5.5

Min Reg = 1.5Brg Width = -Min Rea = -

Bearing S is a rigid surface.

Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs)

Cilolus	rens.comp.		Ciloius	i ciio.	Comp.
A - B	132	- 752	H - I	308	- 687
B - C	241	- 787	l - J	345	- 905
C - D	291	- 743	J - K	391	- 1276
D-F	343	- 719	K-L	448	- 1793
E-G	303	- 676	L - M	254	- 1353
G - H	308	- 688			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.		Chords	Tens. (Comp.
R-Q	756		P - O	1098	- 164

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
A - T	171 - 945	E -AD	417 - 134
A - U	730 - 118	J - P	507 - 118
S - T	287 - 1231	P - K	214 - 378
T - W	175 - 451	K - O	470 - 143
U - X	674 - 71	L-0	336 - 1069
W - X	138 - 396	O - M	1497 - 254
X - R	526 - 33	M - N	253 - 1246
E-F	150 - 436		

FL REG# 278, Yoonhwak Kim, FL PE #86367 01/26/2021

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

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2400 Lake Orange Dr. Suite 150 Orlando FL, 32837

Job Number: 20-4966 Ply: 1 SEQN: 335941 / T36 / COMN Cust: R215 JRef: 1X2e2150002 DrwNo: 026.21.1006.45282 Dale and Karla Nickelson Res Qty: 1 FROM: CDM Truss Label: A06 Page 2 of 2 KD / WHK 01/26/2021

Hangers / Ties

Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

Bearing at location x=30'8"8 uses the following support conditions: 30'8"8

Bearing N (30'8"8, 10') HUS26 Supporting Member: (2)2x6 SP 2400f-2.0E (14) 0.148"x3" nails into supporting

member, (4) 0.148"x3" nails into supported member.



FL REG# 278, Yoonhwak Kim, FL PE #86367 01/26/2021

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

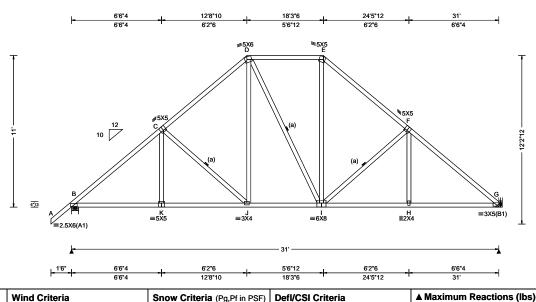
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Job Number: 20-4966 Ply: 1 SEQN: 335932 / T35 / COMN Cust: R215 JRef: 1X2e2150002 DrwNo: 026.21.1006.45547 Dale and Karla Nickelson Res Qty: 2 FROM: CDM Truss Label: A07 Page 1 of 2 KD / WHK 01/26/2021



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.059 J 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(TL): 0.120 J 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.031 H
Des Ld: 40.00	EXP: C		HORZ(TL): 0.063 H
NCBCLL: 10.00	Mean Height: 16.23 ft TCDL: 5.0 psf	Code / Misc Criteria	Creep Factor: 2.0
Soffit: 2.00	BCDL: 5.0 psf	Bldg Code: FBC 7th Ed. 202	OMRæsTCCSI: 0.468
Load Duration: 1.25	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max BC CSI: 0.481
Spacing: 24.0 "	C&C Dist a: 3.10 ft	Rep Factors Used: Yes	Max Web CSI: 0.211
'	Loc. from endwall: Any	FT/RT:20(0)/10(0)	
	GCpi: 0.18	Plate Type(s):	
	Wind Duration: 1.60	WAVE	VIEW Ver: 20.02.01A.1209.11

Lumber

Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP #2 Bot chord 2x4 SP #2 Webs 2x4 SP #3

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Bracing

(a) Continuous lateral restraint equally spaced on member.

Hangers / Ties

Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

Bearing at location x=30'9" uses the following support conditions: 30'9" Bearing G (30'9", 10') HUS26

Supporting Member: (2)2x6 SP 2400f-2.0E (14) 0.148"x3" nails into supporting member,

(4) 0.148"x3" nails into supported member.

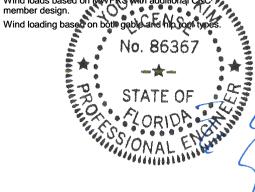
Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance. Bottom chord checked for 10.00 psf non-concurrent live load.

Purlins

 $I_{\underline{n}}$ lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind Wind loads based on WW



Gravity Non-Gravity Loc R+ /R /Rh /Rw /U /RL /904 В 1510 /-/133 /368 1388 /-/805 /105 /-Wind reactions based on MWFRS Brg Width = 6.0В Min Rea = 1.8Brg Width = -Min Reg = -

Bearing B is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs)

Chords Tens.Comp. Chords Tens. Comp. B - C 152 - 1853 206 - 1444 C - D 207 - 1460 - 1859 157 D-E 221 - 1009

Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - K 1319 I - H 1329 - 10 K-J 1318 H - G 1330 - 71 - 9 J - I 1016 0

Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. C-J 465 228 - 407 1 - F - 95

1 - F

235

- 435

D-J

484 - 94

FL REG# 278, Yoonhwak Kim, FL PE #86367 01/26/2021

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Job Number: 20-4966 Ply: 1 SEQN: 335932 / T35 / COMN Cust: R215 JRef: 1X2e2150002 Dale and Karla Nickelson Res DrwNo: 026.21.1006.45547 Qty: 2 FROM: CDM Truss Label: A07 Page 2 of 2 KD / WHK 01/26/2021

Additional Notes

Refer to DWG PB160160118 for piggyback details. The overall height of this truss excluding overhang is 11-0-0.



FL REG# 278, Yoonhwak Kim, FL PE #86367 01/26/2021

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

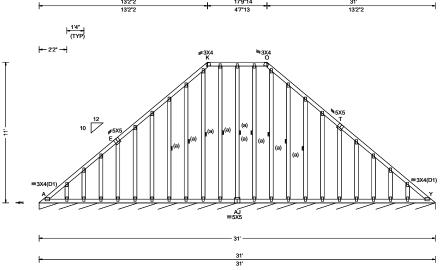
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Job Number: 20-4966 Ply: 1 SEQN: 335937 / T10 / GABL Cust: R215 JRef: 1X2e2150002 DrwNo: 026.21.1006.45875 Dale and Karla Nickelson Res Qty: 1 FROM: CDM Truss Label: A08 KD / WHK 01/26/2021



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.003 K 999 240	
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(TL): 0.004 K 999 180	
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.005 F	
Des Ld: 40.00	EXP: C		HORZ(TL): 0.007 I	
NCBCLL: 10.00	Mean Height: 15.65 ft TCDL: 5.0 psf	Code / Misc Criteria	Creep Factor: 2.0	
Soffit: 2.00	BCDL: 5.0 psf	Bldg Code: FBC 7th Ed. 202	ONReesTC CSI: 0.081	
Load Duration: 1.25	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max BC CSI: 0.055	
Spacing: 24.0 "	C&C Dist a: 3.10 ft	Rep Factors Used: Yes	Max Web CSI: 0.086	
'	Loc. from endwall: Any	FT/RT:20(0)/10(0)		
	GCpi: 0.18	Plate Type(s):		4
	Wind Duration: 1.60	WAVE	VIEW Ver: 20.02.01A.1209.11	
				_

▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ /Rw /U /RL Y* 86 /-/47 /10 Wind reactions based on MWFRS Brg Width = 371 Min Req = -Bearing A is a rigid surface. Members not listed have forces less than 375#

Lumber

Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP #2 Bot chord 2x4 SP #2 Webs 2x4 SP #3

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 2X4 except as noted.

Bottom chord checked for 10.00 psf non-concurrent live load.

In lieu of structural panels use purlins to brace TC @ 24" oc.

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

The overall height of this truss excluding overhang is



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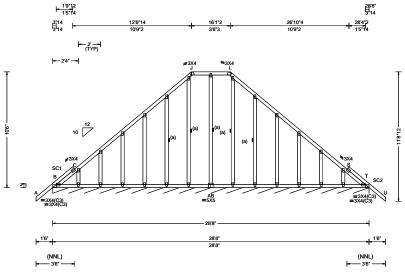
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.001 L 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(TL): 0.003 L 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.001 C
Des Ld: 40.00	EXP: C Mean Height: 15.75 ft		HORZ(TL): 0.002 C
NCBCLL: 10.00	TCDL: 5.0 psf	Code / Misc Criteria	Creep Factor: 2.0
Soffit: 2.00	BCDL: 5.0 psf	Bldg Code: FBC 7th Ed. 202	OMRæsTC CSI: 0.195
Load Duration: 1.25	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max BC CSI: 0.077
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Factors Used: Yes	Max Web CSI: 0.114
	Loc. from endwall: Any	FT/RT:20(0)/10(0)	
	GCpi: 0.18	Plate Type(s):	
	Wind Duration: 1.60	WAVE	VIEW Ver: 20.02.01A.1209.11

▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ /R /Rw /U /RL T* 93 /-/-/50 Wind reactions based on MWFRS Brg Width = 343 Min Req = -Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber

Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP #2 Bot chord 2x4 SP #2 Webs 2x4 SP #3

Stack Chord: SC1 2x4 SP #2; Stack Chord: SC2 2x4 SP #2;

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC $\,$

Bracing

(a) Continuous lateral restraint equally spaced on

Plating Notes

All plates are 2X4 except as noted.

Bottom chord checked for 10.00 psf non-concurrent live load.

Purlins

In lieu of structural panels use purlins to brace TC @ 24" oc.

Wind loads based on MWFRS with additional C&C member design.

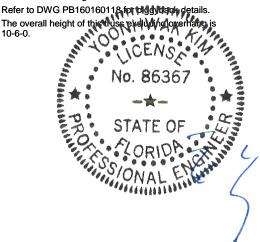
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

Refer to DWG PB160160118 for plughtlage details.



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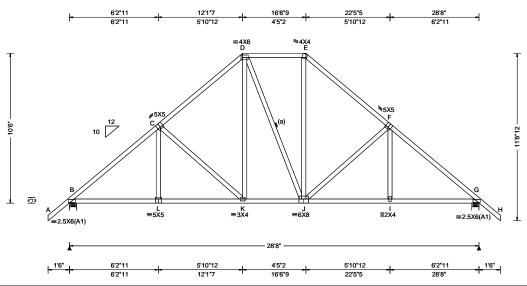
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Job Number: 20-4966 Ply: 1 SEQN: 335730 / T31 / COMN Cust: R215 JRef: 1X2e2150002 Qty: 4 DrwNo: 026.21.1006.45485 Dale and Karla Nickelson Res FROM: CDM Truss Label: B02 KD / WHK 01/26/2021



				一
Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	14
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	١.
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.051 K 999 240	!
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(TL): 0.103 K 999 180	١
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.027 I	1
Des Ld: 40.00	EXP: C		HORZ(TL): 0.055 I	١
NCBCLL: 10.00	Mean Height: 15.00 ft	Code / Misc Criteria	Creep Factor: 2.0	E
Soffit: 2.00	TCDL: 5.0 psf BCDL: 5.0 psf	Bldg Code: FBC 7th Ed. 202	OMReesTC CSI: 0.421	19
Load Duration: 1.25	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max BC CSI: 0.425	15
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Factors Used: Yes	Max Web CSI: 0.484	!
	Loc. from endwall: Any	FT/RT:20(0)/10(0)		1;
	GCpi: 0.18	Plate Type(s):] }
	Wind Duration: 1.60	WAVE	VIEW Ver: 20.02.01A.1209.11] [
				- (

▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /Rh /Rw /U /RL 1389 /-/830 /205 /372 1384 /-/830 /205 /-Wind reactions based on MWFRS Brg Width = 6.0В Min Rea = 1.6Brg Width = 6.0 Min Req = 1.6Bearings B & G are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. 266 - 1678 322 - 1287 C - D 324 - 1300 266 - 1672

Lumber

Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP #2 Bot chord 2x4 SP #2 Webs 2x4 SP #3

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Bracing

(a) Continuous lateral restraint equally spaced on member.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Bottom chord checked for 10.00 psf non-concurrent live load.

Purlins

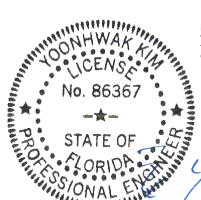
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

Refer to DWG PB160160118 for piggyback details. The overall height of this truss excluding overhang is 10-6-0.



D-E 309 - 893

Maximum Bot Chord Forces Per Ply (lbs)

Choras	rens.comp.		Choras	rens. C	omp.
B-L	1190	- 155	J - I	1184	-62
L-K	1189	- 156	I-G	1185	- 61
KI	897	- 70			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.		os Tens.Comp. Webs		Webs	Tens. Comp.		
C-K	195	- 394	J - E	412	- 78			
D-K	433	- 80	J - F	194	- 396			

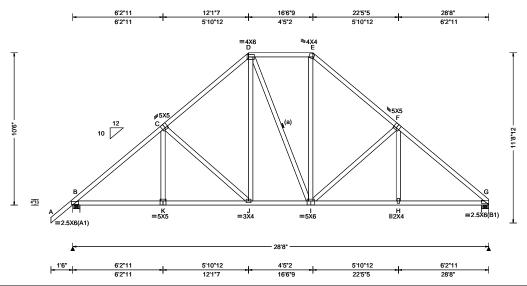
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Job Number: 20-4966 Ply: 1 SEQN: 335735 / T4 / COMN Cust: R215 JRef: 1X2e2150002 DrwNo: 026.21.1006.46032 Dale and Karla Nickelson Res Qty: 6 FROM: CDM Truss Label: B03 KD / WHK 01/26/2021



▲ Maximum Reactions (lbs)						
	Gravity		No	on-Grav	vity	
Loc R+	/ R-	/ Rh	/ Rw	/ U	/ RL	
B 1393	/-	/-	/830	/206	/348	
G 1274	/-	/-	/732	/180	/-	
Wind rea	ctions b	ased on	MWFRS			
B Brg \	Width =	6.0	Min Re	q = 1.6	;	
G Brg \	G Brg Width = 6.0			Min Req = 1.5		
Bearings	B&Ga	re a rigio	d surface.	-		
Members	not liste	ed have	forces les	s than 3	375#	
Maximur	n Top C	hord Fo	rces Per	Ply (lb	s)	
Chords	Tens.Co	mp.	Chords	Tens.	Ćomp.	
в-с	268 -	1683	E-F	325	- 1294	
C-Ď		1306	F-G	272	- 1689	
D-E	310	- 897				

Lumber

Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP #2 Bot chord 2x4 SP #2 Webs 2x4 SP #3

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Bracing

(a) Continuous lateral restraint equally spaced on member.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Bottom chord checked for 10.00 psf non-concurrent live load.

Purlins

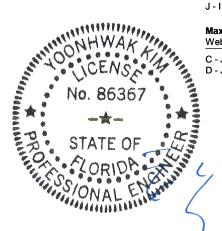
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

Refer to DWG PB160160118 for piggyback details. The overall height of this truss excluding overhang is 10-6-0.



FL REG# 278, Yoonhwak Kim, FL PE #86367 01/26/2021

Maximum Web Forces Per Ply (lbs)

1193 - 107

Chords Tens.Comp.

1194

901

B - K

K-J

Maximum Bot Chord Forces Per Ply (lbs)

Nebs	Tens.Comp.	Webs	Tens. Comp.	
C - J	194 - 394	I-E	420 -81	
D - J	434 - 80	I-F	200 - 415	

Chords

I - H

H - G

Tens. Comp.

- 104

- 104

1203

1204

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

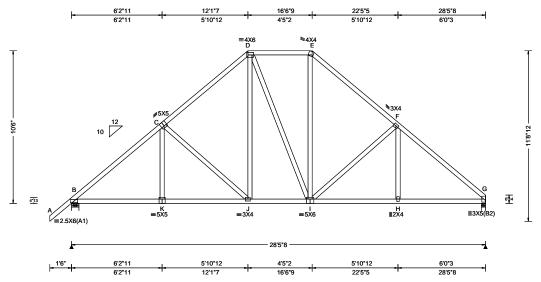
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Job Number: 20-4966 Ply: 1 SEQN: 335738 / T6 / COMN Cust: R215 JRef: 1X2e2150002 Qty: 1 DrwNo: 026.21.1006.45593 Dale and Karla Nickelson Res FROM: CDM Truss Label: B04 KD / WHK 01/26/2021



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria	1
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	١.
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.047 J 999 240	[
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(TL): 0.100 J 999 180	L
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.027 H	1
Des Ld: 40.00	EXP: C		HORZ(TL): 0.058 H	١
NCBCLL: 10.00	Mean Height: 15.75 ft	Code / Misc Criteria	Creep Factor: 2.0	E
Soffit: 2.00	TCDL: 5.0 psf BCDL: 5.0 psf	Bldg Code: FBC 7th Ed. 202	OMRaesTC CSI: 0.418	19
Load Duration: 1.25	MWFRS Parallel Dist: h to 2h	TPI Std: 2014	Max BC CSI: 0.454	15
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Factors Used: Yes	Max Web CSI: 0.489	Ľ
-pg	Loc. from endwall: not in 9.00 ft	FT/RT:20(0)/10(0)		Ľ
	GCpi: 0.18	Plate Type(s):] `
	Wind Duration: 1.60	WAVE	VIEW Ver: 20.02.01A.1209.11	ŀ
Lumber				- (

▲ M	▲ Maximum Reactions (lbs)						
	(avity		N	lon-Gra	vity	
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL	
В	1338	/-	/-	/832	/-	/348	
G	1217	/-	/-	/727	/-	/-	
Win	d rea	ctions b	ased or	n MWFRS			
В	Brg \	Nidth =	6.0	Min Re	eq = 1.6	6	
G	Brg \	Nidth =	3.5	Min Re	eq = 1.5	5	
Bea	rings	B&Ga	are a rig	jid surface			
Men	nbers	not liste	ed have	forces les	s than	375#	
Max	imur	n Top C	hord F	orces Pe	Ply (lb	s)	
Cho	rds .	Tens.Co	omp.	Chords	Tens.	Comp.	
В-0	2	14 -	1603	E-F	80	- 1213	
ا - C - ا	Ď	86 -	1219	F-G	8	- 1594	
D - I	E	93	- 836				

Maximum Bot Chord Forces Per Ply (lbs)

- 94

-6

Chords

I - H

H - G

Webs

1 - E

1 - F

Tens. Comp.

Tens. Comp.

400

131

0

0

- 31

- 393

1125

1126

Chords Tens.Comp.

1133

1132

835

Tens.Comp.

120 - 401

405 - 28

B - K

K-J

Lumber

Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP #2 Bot chord 2x4 SP #2 Webs 2x4 SP #3

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

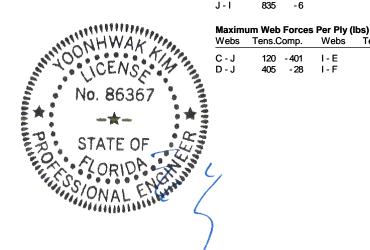
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

Refer to DWG PB160160118 for piggyback details. The overall height of this truss excluding overhang is 10-6-0.



FL REG# 278, Yoonhwak Kim, FL PE #86367 01/26/2021

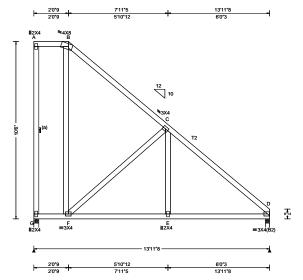
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria	4
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Mean Height: 16.39 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h	, ,	PP Deflection in loc L/defl L/# VERT(LL): 0.259 F 644 240 VERT(TL): 0.557 F 299 180 HORZ(LL): -0.213 B HORZ(TL): 0.459 B Creep Factor: 2.0	
Spacing: 24.0 "	C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft	FT/RT:20(0)/10(0)	Max Web CSI: 0.810	N
	GCpi: 0.18 Wind Duration: 1.60	Plate Type(s): WAVE	VIEW Ver: 20.02.01A.1209.11	=
	Willa Dalation. 1.00	WAVL	VIL.W Vel. 20.02.01A.1209.11]]

AN	laxim	um Rea	ctions (II	os)				
	Gravity Non-Gravity							
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL		
G	598	/-	/-	/475	/83	/259		
D	603	/-	/-	/377	/-	/-		
Wir	nd read	ctions b	ased on N	/WFRS				
G	Brg V	Vidth =	3.5	Min Re	q = 1.5	5		
D	Brg V	Vidth =	3.0	Min Re	q = 1.5	5		
Bea	arings	G&Da	are a rigid	surface.				
Mei	mbers	not list	ed have fo	orces less	s than	375#		
Maximum Top Chord Forces Per Ply (lbs)								
Cho	ords ⁻	Tens.Co	omp.			-		
 C-	D	0	- 748					

Maximum Bot Chord Forces Per Ply (lbs
Chords Tens.Comp. Chords Tens.
F-E 492 0 E-D 495

Maximum Web Forces Per Ply (lbs) Tens. Comp. Webs Tens.Comp. Webs 170 - 663 C-E 402 0

Lumber

Value Set: 13B (Effective 6/1/2013) Top chord 2x4 SP #2 T2 2x4 SP M-31; Bot chord 2x4 SP M-31 Webs 2x4 SP #3

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Bracing

(a) Continuous lateral restraint equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

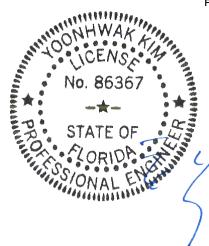
Left end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

Refer to DWG PB160160118 for piggyback details.

The overall height of this truss excluding overhang is 10-6-0.



FL REG# 278, Yoonhwak Kim, FL PE #86367 01/26/2021

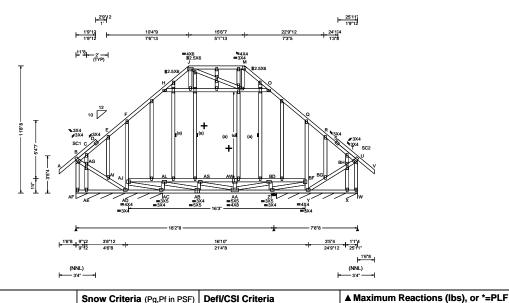
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.039 K 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(TL): 0.074 K 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.014 Q
Des Ld: 40.00	EXP: C		HORZ(TL): 0.026 Q
NCBCLL: 10.00	Mean Height: 16.67 ft TCDL: 5.0 psf	Code / Misc Criteria	Creep Factor: 2.0
Soffit: 2.00	BCDL: 5.0 psf	Bldg Code: FBC 7th Ed. 202	ONReesTC CSI: 0.611
Load Duration: 1.25	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max BC CSI: 0.332
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Factors Used: Yes	Max Web CSI: 0.406
' '	Loc. from endwall: Any	FT/RT:20(0)/10(0)	
	GCpi: 0.18	Plate Type(s):	
	Wind Duration: 1.60	WAVE	VIEW Ver: 20.02.01A.1209.11

Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP #2 Bot chord 2x4 SP #2 Webs 2x4 SP #3

Stack Chord: SC1 2x4 SP #2; Stack Chord: SC2 2x4 SP #2;

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC $\,$

Bracing

(a) Continuous lateral restraint equally spaced on

Plating Notes

All plates are 2X4 except as noted.

Bottom chord checked for 10.00 psf non-concurrent live load.

Attic room loading from 4-10-0 to 21-1-0: Live Load: 40 PSF. Dead Load: 10 PSF Ceiling: 10 PSF, Kneewalls: 10 PSF

Purlins

In lieu of structural panels use purlins to brace TC @

Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

+ Member to be laterally braced for horizontal wind loads. bracing system to be desiged and furnished by others.

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Loc R+ /Rh /Rw /U /RL AF* 296 /96 /24 1276 /-/-/299 /-W* 143 /64 Wind reactions based on MWFRS AF Brg Width = 95.5 Min Reg = Brg Width = 6.0 Min Req = 1.5 Brg Width = 89.5 Min Rea = -

Non-Gravity

Bearings AF, Z, & Z are a rigid surface. Members not listed have forces less than 375#

Gravity

Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

		-			
B - C	94	- 510	M - O	0	- 555
C - D	82	- 554	O - Q	15	- 629
D-E	1	- 539	Q-R	0	- 523
E-F	11	- 527	R-S	0	- 536
F-H	33	- 633	S-T	60	- 551
H - J	0	- 546	T - U	72	- 521
J - M	0	- 429			

Maximum Bot Chord Forces Per Ply (lbs)

Chorus	Tens.C	omp.	Chorus	Tens. C	omp.
AD-AC AB-AA		-	Z - Y	389	- 51

Maximum Web Forces Per Ply (lbs)

Webs	Tens.C	comp.	Webs	i ens.	Comp.
B-AF	2	- 743	AA-BD	1025	0
B -AG	464	- 81	AW-BD	0	- 698
AG-AI	484	- 78	BD- Z	0	- 1065
AI-AD	466	- 81	BD-BF	569	0
AJ-AC	0	- 618	Z-BF	0	- 561
AJ-AL	625	0	Y -BG	472	-62
AC-AL	0	- 1079	BG-BH	481	- 58
AL-AB	1012	0	BH- U	469	-61
AL-AS	0	- 629	U - W	0	- 739
AS-AW	0	- 698			

FL REG# 278, Yoonhwak Kim, FL PE #86367 01/26/2021

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2400 Lake Orange Dr. Suite 150 Orlando FL, 32837

Job Number: 20-4966 Ply: 1 SEQN: 335832 / T16 / GABL Cust: R215 JRef: 1X2e2150002 DrwNo: 026.21.1006.45656 Dale and Karla Nickelson Res Qty: 1 FROM: CDM Truss Label: C01 Page 2 of 2 KD / WHK 01/26/2021

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

Refer to DWG PB160160118 for piggyback details.

The overall height of this truss excluding overhang is



FL REG# 278, Yoonhwak Kim, FL PE #86367 01/26/2021

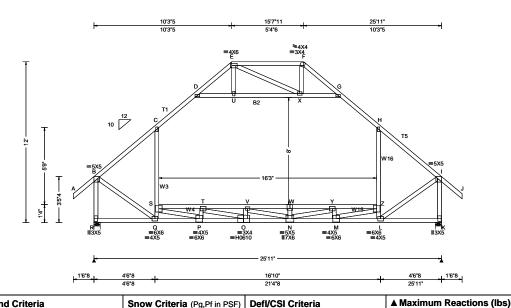
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.463 E 671 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(TL): 0.933 F 333 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.453 C
Des Ld: 40.00	EXP: C		HORZ(TL): 0.994 C
NCBCLL: 10.00	Mean Height: 16.67 ft TCDL: 5.0 psf	Code / Misc Criteria	Creep Factor: 2.0
Soffit: 2.00	BCDL: 5.0 psf	Bldg Code: FBC 7th Ed. 202	OMRæsTCCSI: 0.756
Load Duration: 1.25	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max BC CSI: 0.646
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Factors Used: Yes	Max Web CSI: 0.903
'	Loc. from endwall: Any	FT/RT:20(0)/10(0)	
	GCpi: 0.18	Plate Type(s):	
	Wind Duration: 1.60	WAVE, HS	VIEW Ver: 20.02.01A.1209.11
Lumber		Additional Notes	

Lumber

Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP #2 T1,T5 2x4 SP M-31; Bot chord 2x4 SP #3 W3,W4,W15,W16 2x4 SP #2; Webs 2x4 SP #3 W3,W4,W15,W16 2x4 SP #2;

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Plating Notes

All plates are 2X4 except as noted.

Bottom chord checked for 10.00 psf non-concurrent

Attic room loading from 4-10-0 to 21-1-0: Live Load: 40 PSF. Dead Load: 10 PSF Ceiling: 10 PSF, Kneewalls: 10 PSF

Purlins

In lieu of structural panels use purlins to brace all flat TC

Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Refer to DWG PB160160118 for piggyback details.

The overall height of this truss excluding overhang is 12-0-0

	= maximam reactions (ibc)						
	Gravity				on-Grav	√ity	
Lo	c R+	/ R-	/ Rh	/ Rw	/ U	/ RL	
	2351		/-	/721	/222	/332	
K	2351	/-	/-	/721	/222	/-	
Wi	nd read	ctions b	ased on N	/WFRS			
R	Bra V	Vidth =	5.5	Min Re	a = 1.9)	

Brg Width = 5.5 Brg Width = 5.5

Min Req = 1.9 Bearings R & K are a rigid surface.

Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

B - C	213 - 1834	F-G	167	- 435
C - D	325 - 1441	G-H	325	- 1441
D-E	170 - 448	H-I	213	- 1834

Maximum Bot Chord Forces Per Ply (lbs) Chords Tens. Comp. Chords Tens.Comp.

Q-P	1363	- 482	N - M	4119	0
P - O	4115	- 320	M - L	1363	- 155
O - N	5180	0			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens.	Comp.
 В - R	232 - 2311	V - W	0	- 3963
B - Q	1500 - 61	N - Y	1350	- 224
Q-S	124 - 658	W - Y	0	- 3963
C - S	501 - 71	X - G	242	- 1140
S - P	2686 0	Y - M	110	- 733
S - T	271 - 2857	Y - Z	281	- 2862
D - U	239 - 1129	M - Z	2690	0
P - T	110 - 740	Z - H	500	- 71
T - O	1306 - 214	Z - L	125	- 660
T - V	0 - 3921	L-I	1500	-62
U - X	238 - 1118	I-K	232	- 2311

FL REG# 278, Yoonhwak Kim, FL PE #86367 01/26/2021

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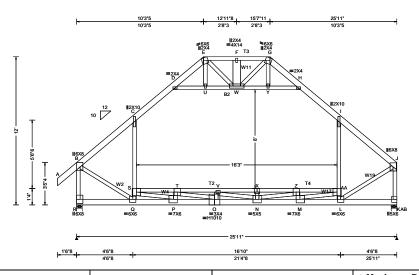
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Job Number: 20-4966 Ply: 2 SEQN: 335839 / T46 / ATIC Cust: R215 JRef: 1X2e2150002 Qty: 1 DrwNo: 026.21.1006.45907 Dale and Karla Nickelson Res FROM: CDM Truss Label: C03 Page 1 of 2 KD / WHK 01/26/2021

2 Complete Trusses Required



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.471 G 660 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(TL): 0.930 G 334 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.186 C
Des Ld: 40.00	EXP: C		HORZ(TL): 0.367 C
NCBCLL: 0.00	Mean Height: 18.19 ft TCDL: 5.0 psf	Code / Misc Criteria	Creep Factor: 2.0
Soffit: 2.00	BCDL: 5.0 psf	Bldg Code: FBC 7th Ed. 202	OMRæsTC CSI: 0.660
Load Duration: 1.25	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max BC CSI: 0.731
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Factors Used: Varies by	LMa&aNs/ebCSI: 0.976
' "	Loc. from endwall: not in 9.00 ft	FT/RT:20(0)/10(0)	
	GCpi: 0.18	Plate Type(s):	
	Wind Duration: 1.60	WAVE, HS	VIEW Ver: 20.02.01A.1209.11

Lumber

Value Set: 13B (Effective 6/1/2013)

Top chord 2x6 SP 2400f-2.0E T2,T4 2x4 SP M-31; T3 2x4 SP #2

Bot chord 2x6 SP 2400f-2.0E B2 2x4 SP #2; Webs 2x4 SP #3 W2,W4,W17,W19 2x4 SP #2;

W11 2x8 SP 2400f-2.0E;

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC $\,$

Nailnote

Nail Schedule:0.131"x3", min. nails Top Chord: 1 Row @ 7.50" o.c. Bot Chord: 1 Row @ 5.00" o.c. Webs : 1 Row @ 4" o.c.

Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special Loads

---(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
"C: From 66 plf at -1.54 to 66 plf at 25. TC: From 66 plf at 26 plf at 66 plf at 26 plf at 25.92 7.79 PLT: From 4.83 to PLT: From 20 plf at 7.79 to 20 plf at 18.12 PLT: From 26 plf at 18.12 to 26 plf at 21.08 4.83 to -1.54 to 100 plf at 5 plf at PLT: From 100 plf at 21.08 5 plf at BC: From 0.00 0.00 to 20 plf at BC: From 20 plf at 25.92 TC: 3000 lb Conc. Load at 12.96 660 lb Conc. Load at 2.15, 4.15, 6.15, 8.15 10.15,12.15,14.15,16.15,18.15,20.15,22.15,24.15

Plating Notes

All plates are 7X8 except as noted.

BC: 111 lb Conc. Load at 4.83,21.08

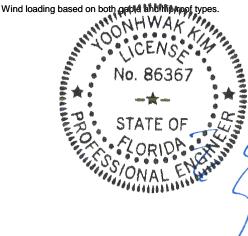
Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Wind

Wind loads and reactions based on MWFRS. End verticals not exposed to wind pressure.



▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /R /Rh /Rw /U /RL R 7757 /-/566 /55 AB 7755 /-/-/494 /-Wind reactions based on MWFRS Brg Width = 5.5Min Req = 3.2 AB Brg Width = 5.5 Min Req = 3.2 Bearings R & AB are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

B - C 244 - 3237 498 - 1419 C-D 302 - 2492 G-H 366 - 1123 D-E 367 - 1122 302 - 2490 H - I 498 - 1419 232 - 3236

Maximum Bot Chord Forces Per Ply (lbs)

Choras	rens.c	omp.	Choras	rens. (Jomp.	
Q-P	2205	- 178	N - M	6643	- 85	
P - O	6587	- 97	M - L	2219	- 174	
O - N	9029	- 65				

Webs

Tens Comp

Maximum Web Forces Per Ply (lbs)

Tens Comp

***	rens.comp.	******	10113.	oomp.
B - R	291 - 3822	F-W	352	- 1517
B - Q	2705 - 206	W - Y	0	- 1748
C-S	1028 0	W - G	1137	- 263
S - P	4268 0	N - Z	2449	0
S - T	0 -4055	X - Z	0	- 6685
D - U	0 - 1768	Y - H	0	- 1766
P - T	0 -814	Z - M	0	- 766
T - O	2517 0	Z -AA	0	- 4125
T - V	0 -6684	M -AA	4325	0
E - W	1141 - 262	AA- I	1027	0
U - W	0 - 1751	L - J	2706	- 221
V - X	0 - 6685	J - K	258	- 3765

FL REG# 278, Yoonhwak Kim, FL PE #86367 01/26/2021

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IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

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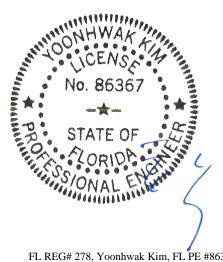


Job Number: 20-4966 Ply: 2 SEQN: 335839 / T46 / ATIC Cust: R215 JRef: 1X2e2150002 Qty: 1 DrwNo: 026.21.1006.45907 Dale and Karla Nickelson Res FROM: CDM Truss Label: C03 Page 2 of 2 KD / WHK 01/26/2021

Additional Notes

Refer to DWG PB160160118 for piggyback details. The overall height of this truss excluding overhang is 12-0-0.

IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER TO VERIFY AND APPROVE THE LOADING. THE ENGINEERING SEAL ON THIS LOADING. THE ENGINEERING SEAL ON THIS DESIGN IS ONLY VALID IF THE BUILDING DESIGNER APPROVES THAT THIS DESIGN MEETS SECTION 1601.2.2 OF THE 2001 FLORIDA BUILDING CODE: "THE STRUCTURAL SYSTEM SHALL BE ABLE TO SUSTAIN LOCAL DAMAGE OR FAILURE WITH THE OVERALL STRUCTURE REMAINING STABLE."



FL REG# 278, Yoonhwak Kim, FL PE #86367 01/26/2021

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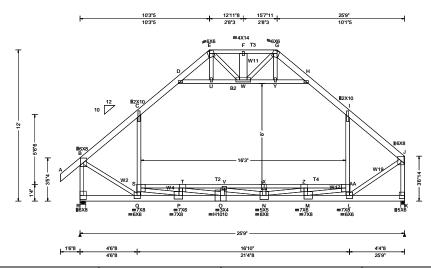
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2400 Lake Orange Dr. Suite 150 Orlando FL, 32837

Job Number: 20-4966 Ply: 2 SEQN: 335836 / T22 / ATIC Cust: R215 JRef: 1X2e2150002 DrwNo: 026.21.1006.45968 FROM: CDM Dale and Karla Nickelson Res Qty: 1 Truss Label: C04 Page 1 of 2 KD / WHK 01/26/2021

2 Complete Trusses Required



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.464 E 666 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(TL): 0.916 E 337 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.210 C
Des Ld: 40.00	EXP: C		HORZ(TL): 0.415 C
NCBCLL: 0.00	Mean Height: 18.19 ft	Code / Misc Criteria	Creep Factor: 2.0
Soffit: 2.00	TCDL: 5.0 psf BCDL: 5.0 psf	Bldg Code: FBC 7th Ed. 202	OMReesTC CSI: 0.624
Load Duration: 1.25	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max BC CSI: 0.729
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Factors Used: Varies by	Mula 2aNs/eb CSI: 0.975
- 3	Loc. from endwall: not in 9.00 ft	FT/RT:20(0)/10(0)	
	GCpi: 0.18	Plate Type(s):	
	Wind Duration: 1.60	WAVE, HS	VIEW Ver: 20.02.01A.1209.11

Lumber

Value Set: 13B (Effective 6/1/2013)

Top chord 2x6 SP 2400f-2.0E T2,T4 2x4 SP M-31; T3 2x4 SP #2

Bot chord 2x6 SP 2400f-2.0E B2 2x4 SP #2; Webs 2x4 SP #3 W2,W4,W17,W19 2x4 SP #2;

W11 2x8 SP 2400f-2.0E;

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC $\,$

Nailnote

Nail Schedule:0.131"x3", min. nails Top Chord: 1 Row @ 7.50" o.c. Bot Chord: 1 Row @ 4.75" o.c. Webs : 1 Row @ 4" o.c.

Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special Loads

----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25) TC: From 66 plf at -1.54 to 66 plf at 25. TC: From 66 plf at 26 plf at 66 plf at 26 plf at 25.75 7.79 PLT: From 4.83 to PLT: From 20 plf at 7.79 to 20 plf at 18.12 PLT: From 26 plf at 18.12 to 26 plf at 21.08 100 plf at 5 plf at 4.83 to -1.54 to PLT: From 100 plf at 21.08 5 plf at BC: From 0.00 0.00 to 20 plf at BC: From 20 plf at 25.75 TC: 3000 lb Conc. Load at 12.96 660 lb Conc. Load at 2.15, 4.15, 6.15, 8.15 10.15,12.15,14.15,16.15,18.15,20.15,22.15,24.15

Plating Notes

All plates are 2X4 except as noted.

BC: 111 lb Conc. Load at 4.83,21.08

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Wind

Wind loads and reactions based on MWFRS. End verticals not exposed to wind pressure.



▲ Maximum Reactions (lbs) Gravity

D-E

Loc	R+	/ R-	/Rh	/ Rw	/ U	/ RL
R 7	7707	/-	/-	/-	/562	/55
K	7791	/-	/-	/-	/495	/-
Wind	d read	tions bas	sed on	MWFRS		
R	Brg W	/idth = 5	.5	Min Re	q = 3.2	!
K	Brg W	/idth = 3	.5	Min Re	q = 3.2	
Bear	ings l	₹&Kare	e a rigi	d surface.		
Mem	bers	not listed	have	forces less	s than 3	375#
Max	imum	Top Ch	ord Fo	orces Per	Ply (lb	s)
Chor	rds T	ens.Con	np.	Chords	Tens.	Comp.
B - C	;	241 - 3°	194	F-G	499	- 1440
C - E)	300 - 24	458	G - H	367	- 1135

Non-Gravity

301 - 2465

226 - 3169

Maximum Bot Chord Forces Per Ply (lbs)

Cnoras	rens.C	omp.	Choras	rens. (Jomp.
Q - P P - O O - N	2200 6625 9000	- 99	N - M M - L	6493 2137	- 76 - 169
O - 14	3000	- 00			

H - I

Maximum Web Forces Per Ply (lbs)

367 - 1138

499 - 1440

Webs	Tens.Comp.	Webs	Tens.	Comp.
 В - R	288 - 3774	F-W	352	- 1514
B - Q	2663 - 203	W - Y	0	- 1695
C-S	1009 0	W - G	1147	- 264
S - P	4320 0	N - Z	2518	0
S - T	0 -4140	X - Z	0	- 6640
D - U	0 - 1698	Y - H	0	- 1713
P - T	0 - 787	Z - M	0	- 796
T - O	2451 0	Z -AA	0	- 4005
T - V	0 - 6699	M -AA	4247	0
E - W	1128 - 261	AA- I	973	0
U - W	0 - 1681	L-J	2726	- 224
V - X	0 -6640	J - K	261	- 3811

FL REG# 278, Yoonhwak Kim, FL PE #86367 01/26/2021

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Orlando FL, 32837

Job Number: 20-4966 Ply: 2 SEQN: 335836 / T22 / ATIC Cust: R215 JRef: 1X2e2150002 Qty: 1 DrwNo: 026.21.1006.45968 Dale and Karla Nickelson Res FROM: CDM Truss Label: C04 Page 2 of 2 KD / WHK 01/26/2021

Additional Notes

Refer to DWG PB160160118 for piggyback details. The overall height of this truss excluding overhang is 12-0-0.

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FL REG# 278, Yoonhwak Kim, FL PE #86367 01/26/2021

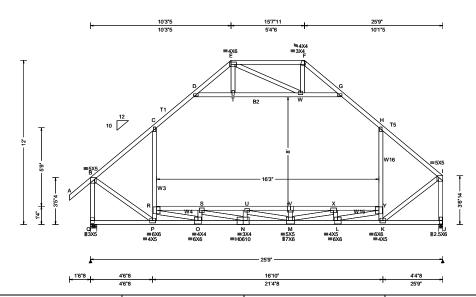
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.457 E 675 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(TL): 0.927 N 333 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.483 C
Des Ld: 40.00	EXP: C		HORZ(TL): 1.046 C
NCBCLL: 10.00	Mean Height: 16.67 ft TCDL: 5.0 psf	Code / Misc Criteria	Creep Factor: 2.0
Soffit: 2.00	BCDL: 5.0 psf	Bldg Code: FBC 7th Ed. 202	OMRænsTCCSI: 0.752
Load Duration: 1.25	MWFRS Parallel Dist: h/2 to h	TPI Std: 2014	Max BC CSI: 0.644
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Factors Used: Yes	Max Web CSI: 0.987
-	Loc. from endwall: not in 9.00 ft	FT/RT:20(0)/10(0)	
	GCpi: 0.18	Plate Type(s):	
	Wind Duration: 1.60	WAVE, HS	VIEW Ver: 20.02.01A.1209.11

Additional Notes

Lumber

Value Set: 13B (Effective 6/1/2013) Top chord 2x4 SP #2 T1,T5 2x4 SP M-31; Bot chord 2x4 SP #3 W3,W4,W15,W16 2x4 SP #2; Webs 2x4 SP #3 W3,W4,W15,W16 2x4 SP #2;

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Plating Notes

All plates are 2X4 except as noted.

Bottom chord checked for 10.00 psf non-concurrent

Attic room loading from 4-10-0 to 21-1-0: Live Load: 40 PSF. Dead Load: 10 PSF Ceiling: 10 PSF, Kneewalls: 10 PSF

Purlins

In lieu of structural panels use purlins to brace all flat TC

Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Wind loads based on MWFRS with additional C&C

End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

Refer to DWG PB160160118 for piggyback details.

The overall height of this truss excluding overhang is



▲ N	▲ Maximum Reactions (lbs)					
	(avity		N	on-Gra	vity
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
Q	2339	/-	/-	/733	/83	/307
J	2237	/-	/-	/632	/66	/-
Wi	nd rea	ctions b	ased on	MWFRS		
Q	Brg \	Nidth =	5.5	Min Re	eq = 1.9)
J	Brg \	Nidth =	3.5	Min Re	q = 1.9)
Be	arings	Q&Ja	re a rigi	d surface.	-	
Me	mbers	not list	ed have	forces les	s than :	375#
Ma	ximur	n Top (hord F	orces Per	Ply (lb	s)
Ch	ords .	Tens.Co	omp.	Chords	Tens.	Comp.
В-	С	91 -	1818	F-G	0	- 439
c-	-			G-H	166	
D-	E	0	- 455	H-I	82	- 1787

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.		Chords	Tens. Comp		
P - O	1381	- 406	M - L	4054	0	
O - N	4132	- 248	L-K	1302	- 149	
N - M	5166	0				

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. C	omp.
B-Q	89 - 2292	U - V	0	- 3945
B - P	1485 - 22	M - X	1385	- 213
P - R	103 - 668	V - X	0	- 3945
C - R	493 - 73	W - G	307	- 1119
R - O	2688 0	X - L	105	- 748
R-S	271 - 2894	X - Y	289	- 2810
D - T	302 - 1102	L - Y	2683	0
0 - S	110 - 727	Y - H	472	- 101
S - N	1275 - 215	Y - K	115	- 690
S - U	0 - 3924	K - I	1513	0
T - W	301 - 1091	I - J	76	- 2209

FL REG# 278, Yoonhwak Kim, FL PE #86367 01/26/2021

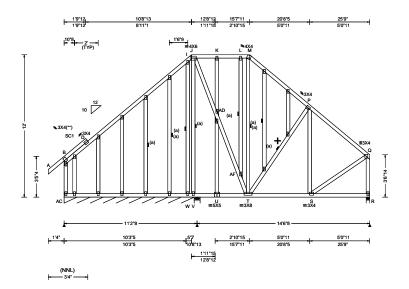
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.021 N 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(TL): 0.044 N 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.020 N
Des Ld: 40.00	EXP: C		HORZ(TL): 0.028 N
NCBCLL: 10.00	Mean Height: 18.19 ft TCDL: 5.0 psf	Code / Misc Criteria	Creep Factor: 2.0
Soffit: 2.00	BCDL: 5.0 psf	Bldg Code: FBC 7th Ed. 202	OMRænsTC CSI: 0.352
Load Duration: 1.25	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max BC CSI: 0.251
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Factors Used: Yes	Max Web CSI: 0.309
-	Loc. from endwall: not in 4.50 ft	FT/RT:20(0)/10(0)	
	GCpi: 0.18	Plate Type(s):	
	Wind Duration: 1.60	WAVE	VIEW Ver: 20.02.01A.1209.11

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

Refer to DWG PB160160119 for niggypack details
The overall height of this trus excluding overlang
12-0-0.

+ Member to be laterally braced for nonzontal wind loads, bracing system to be designed and furnished by others.

by others.

Gravity Non-Gravity Loc R+ /Rh /Rw /U /RL AC*126 /76 /25 /170 305 /-/-629 /356 Wind reactions based on MWFRS AC Brg Width = 131 Min Reg = Brg Width = 6.0 Min Req = 1.5 Brg Width = 3.5 Min Rea = 1.5Bearings AC, V, & R are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp.

▲ Maximum Reactions (lbs), or *=PLF

P - Q 39 - 486

Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp.

539 - 390

Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens Comp

J - \ J -A AD-	D 495	- 439 - 133 - 135	AF- T Q - R	478 21	- 145 - 588

Bracing

Lumber

Top chord 2x4 SP #2

Bot chord 2x4 SP #2 Webs 2x4 SP #3

1/30/2013 by ALSC

(a) Continuous lateral restraint equally spaced on member.

Lumber value set "13B" uses design values approved

Plating Notes

All plates are 2X4 except as noted.

Value Set: 13B (Effective 6/1/2013)

Stack Chord: SC1 2x4 SP #2;

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

In lieu of structural panels use purlins to brace TC @

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

FL REG# 278, Yoonhwak Kim, FL PE #86367 01/26/2021

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

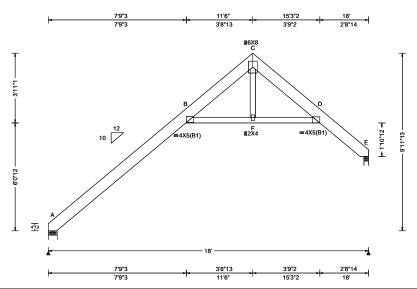
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Job Number: 20-4966 Ply: 1 SEQN: 335814 / T30 / COMN Cust: R215 JRef: 1X2e2150002 FROM: CDM DrwNo: 026.21.1006.45923 Dale and Karla Nickelson Res Qty: 11 Truss Label: D01 KD / WHK 01/26/2021



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲
Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 16.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Mean Height: 18.53 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Code / Misc Criteria Bldg Code: FBC 7th Ed. 202 TPI Std: 2014 Rep Factors Used: Yes FT/RT:20(0)/10(0) Plate Type(s):	PP Deflection in loc L/defl L/# VERT(LL): 0.441 F 470 240 VERT(TL): 0.819 F 253 180 HORZ(LL): 0.377 F HORZ(TL): 0.701 F Creep Factor: 2.0 OMResTC CSI: 0.379 Max BC CSI: 0.193 Max Web CSI: 0.026	A E W A E Be M
	Wind Duration: 1.60	WAVE	VIEW Ver: 20.02.01A.1209.11	В

▲ Maximum Reactions (lbs)							
	Gravity Non-Gravity						
Loc R-	+ / R-	/ Rh	/ Rw	/ U	/ RL		
A 437	· /-	/-	/231	/45	/183		
E 455	/-	/-	/262	/45	/-		
Wind re	actions b	ased or	MWFRS				
A Bro	Width =	6.0	Min Re	eq = 1.5	5		
E Bro	Width =	3.0	Min Re	q = 1.5	5		
Bearing	s A & E a	are a rig	id surface.	-			
Membe	rs not list	ed have	forces les	s than 3	375#		
Maximum Top Chord Forces Per Ply (lbs)							
			Chords				
в-с	45	- 551	C-D	105	- 703		

Maximum Bot Chord Forces Per Ply (lbs)

Chords Tens. Comp.

691

Chords Tens.Comp.

691

Lumber

Value Set: 13B (Effective 6/1/2013) Top chord 2x8 SP 2400f-2.0E Bot chord 2x4 SP #2 Webs 2x4 SP #3

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Loading

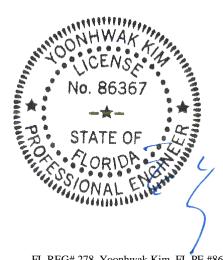
Bottom chord checked for 10.00 psf non-concurrent live load.

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is



FL REG# 278, Yoonhwak Kim, FL PE #86367 01/26/2021

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

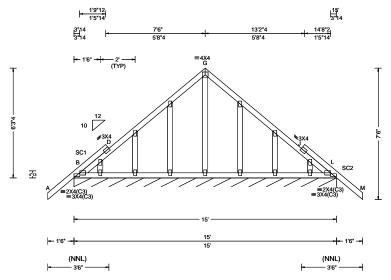
IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

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Job Number: 20-4966 Ply: 1 SEQN: 335712 / T2 / GABL Cust: R215 JRef: 1X2e2150002 DrwNo: 026.21.1006.46188 Dale and Karla Nickelson Res Qty: 1 FROM: CDM Truss Label: E01 KD / WHK 01/26/2021



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.001 N 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(TL): 0.002 N 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.001 T
Des Ld: 40.00	EXP: C Mean Height: 15.00 ft		HORZ(TL): 0.001 T
NCBCLL: 10.00	TCDL: 5.0 psf	Code / Misc Criteria	Creep Factor: 2.0
Soffit: 2.00	BCDL: 5.0 psf	Bldg Code: FBC 7th Ed. 202	ONReesTC CSI: 0.197
Load Duration: 1.25	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max BC CSI: 0.066
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Factors Used: Yes	Max Web CSI: 0.087
	Loc. from endwall: Any	FT/RT:20(0)/10(0)	
	GCpi: 0.18	Plate Type(s):	
	Wind Duration: 1.60	WAVE	VIEW Ver: 20.02.01A.1209.11

▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ /Rw /U /RL L* 100 /-/-Wind reactions based on MWFRS Brg Width = 180 Min Req = -Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber

Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP #2 Bot chord 2x4 SP #2 Webs 2x4 SP #3

Stack Chord: SC1 2x4 SP #2; Stack Chord: SC2 2x4 SP #2;

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC $\,$

Plating Notes

All plates are 2X4 except as noted.

Loading

Bottom chord checked for 10.00 psf non-concurrent live load

In lieu of structural panels use purlins to brace TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

The overall height of this truss excluding byerhang is



FL REG# 278, Yoonhwak Kim, FL PE #86367 01/26/2021

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

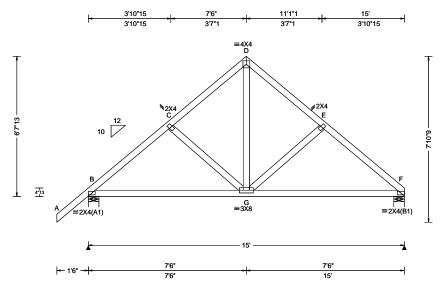
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Job Number: 20-4966 Ply: 1 SEQN: 335714 / T1 / COMN Cust: R215 JRef: 1X2e2150002 DrwNo: 026.21.1006.46001 Dale and Karla Nickelson Res Qty: 6 FROM: CDM Truss Label: E02 KD / WHK 01/26/2021



▲ Maximum Reactions (lbs)							
	Gra	avity		N	on-Grav	vity	
Loc F	₹+	/ R-	/ Rh	/ Rw	/ U	/ RL	
B 75	59	/-	/-	/483	/116	/232	
F 63	39	/-	/-	/382	/87	/-	
Wind	reacti	ons b	oased or	MWFRS			
в в	rg Wi	dth =	6.0	Min Re	eq = 1.5	;	
F B	rg Wi	dth =	6.0	Min Re	eq = 1.5	;	
Bearin	ngs B	&Fa	are a rigi	d surface.	•		
Memb	ers n	ot list	ed have	forces les	s than 3	375#	
Maxin	num '	Top (Chord F	orces Per	Ply (lb	s)	
Chord	s Te	ns.C	omp.	Chords	Tens.	Ćomp.	
в-с		143	- 758	D-E	155	- 582	
C-D		154	- 579	E-F	146	- 764	

Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP #2 Bot chord 2x4 SP #2 Webs 2x4 SP #3

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Loading

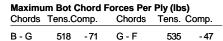
Bottom chord checked for 10.00 psf non-concurrent live load.

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is



Maximum Web Forces Per Ply (lbs)

Webs Tens.Comp. D - G 439



FL REG# 278, Yoonhwak Kim, FL PE #86367 01/26/2021

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

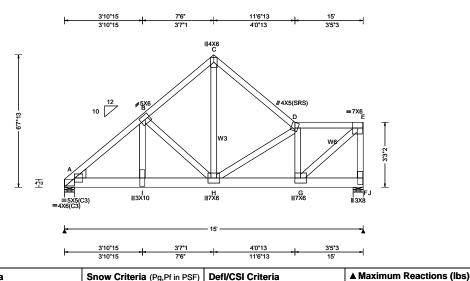
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Job Number: 20-4966 Ply: 2 SEQN: 354450 / T39 / COMN Cust: R215 JRef: 1X2e2150002 Qty: 1 DrwNo: 026.21.1006.46313 Dale and Karla Nickelson Res FROM: CDM Truss Label: E03 KD / WHK 01/26/2021

2 Complete Trusses Required



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.071 H 999 240	
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(TL): 0.141 H 999 180	
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.027 B	
Des Ld: 40.00	EXP: C		HORZ(TL): 0.054 B	
NCBCLL: 0.00	Mean Height: 15.00 ft TCDL: 5.0 psf	Code / Misc Criteria	Creep Factor: 2.0	
Soffit: 2.00	BCDL: 5.0 psf	Bldg Code: FBC 7th Ed. 202	ONRaesTC CSI: 0.349	
Load Duration: 1.25	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max BC CSI: 0.428	
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Factors Used: No	Max Web CSI: 0.835	
- -	Loc. from endwall: not in 9.00 ft	FT/RT:20(0)/10(0)		
	GCpi: 0.18	Plate Type(s):		1
	Wind Duration: 1.60	WAVE	VIEW Ver: 20.01.01A.0724.11	
				_

Gravity Non-Gravity Loc R+ /Rh /Rw /U /RL 5891 /-/828 5292 /-/855 /-Wind reactions based on MWFRS Brg Width = 6.0Α Min Rea = 2.4Brg Width = 6.0 Min Req = 2.2 Bearings A & J are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. 426 - 2640 580 - 3923 425 - 2640 D-E 442 - 2732

Lumber

Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP #2 Bot chord 2x6 SP 2400f-2.0E Webs 2x4 SP #3 W3,W6 2x4 SP #2; Lt Wedge: 2x6 SP 2400f-2.0E;

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Nailnote

Nail Schedule:0.131"x3", min. nails Top Chord: 1 Row @12.00" o.c. Bot Chord: 2 Rows @ 4.50" o.c. (Each Row) Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special Loads

----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25) TC: From 66 plf at 0.00 to 66 plf at 15.00 BC: From BC: From 10 plf at 20 plf at 0.00 to 10 plf at 12.56 12.56 to 20 plf at 15.00 BC: 1388 lb Conc. Load at 2.06, 4.06 BC: 1282 lb Conc. Load at 4.56 BC: 1490 lb Conc. Load at 6.56, 8.56,10.56,12.56

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Additional Notes

The overall height of this truss excluding overhang is

Wind

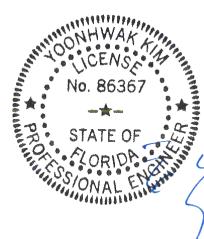
Wind loads and reactions based on MWFRS. Right end vertical not exposed to wind pressure. Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Tens.Comp. Chords		
A - I	2979 - 434	H-G	2852 - 4	62
I - H	2953 - 432			

Maximum Web Forces Per Ply (lbs)

webs	rens.Comp.	vvebs	rens. Comp.
I - B	1602 - 169	D-G	206 - 1194
B - H	155 - 1287	G-E	3700 - 598
C - H	3172 - 480	E-F	420 - 2549
H-D	176 - 1030		



FL REG# 278, Yoonhwak Kim, FL PE #86367 01/26/2021

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

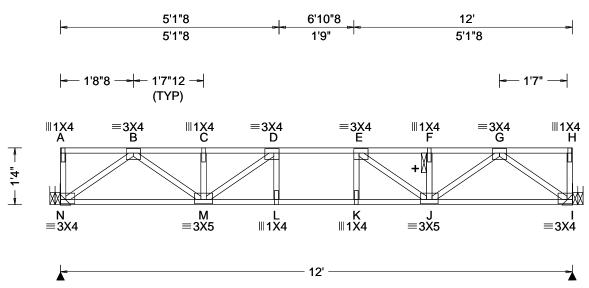
IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

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Job Number: 20-4966 Ply: 1 SEQN: 335824 / T20 / SY42 Cust: R215 JRef: 1X2e2150002 DrwNo: 026.21.1006.45516 Dale and Karla Nickelson Res Qty: 12 FROM: CDM Truss Label: F03 KD / WHK 01/26/2021



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria
TCLL: 40.00	Wind Std: NA	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: NA mph	Pf: NA Ce: NA	VERT(LL): 0.084 L 999 480
BCLL: 0.00	Enclosure: NA	Lu: NA Cs: NA	VERT(TL): 0.127 L 999 360
BCDL: 5.00	Category: NA	Snow Duration: NA	HORZ(LL): 0.017 I
Des Ld: 55.00	EXP: NA		HORZ(TL): 0.026 B
NCBCLL: 10.00	Mean Height: NA ft TCDL: NA psf	Code / Misc Criteria	Creep Factor: 2.0
Soffit: 2.00	BCDL: NA psf	Bldg Code: FBC 7th Ed. 202	ONReesTC CSI: 0.424
Load Duration: 1.00	MWFRS Parallel Dist: NA	TPI Std: 2014	Max BC CSI: 0.579
Spacing: 24.0 "	C&C Dist a: NA ft	Rep Factors Used: Yes	Max Web CSI: 0.308
' '	Loc. from endwall: NA	FT/RT:12(0)/10(0)	
	I: NA GCpi: NA	Plate Type(s):	
	Wind Duration: NA	WAVE	VIEW Ver: 20.02.01A.1209.11

Lumber

Value Set: 13B (Effective 6/1/2013)

Top chord 4x2 SP #2 Bot chord 4x2 SP #2 Webs 4x2 SP #3

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Hangers / Ties

Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

Bearing at location x=0' support conditions: 0'

uses the following

Support contains. V Bearing N (0', 10') HUS46 Supporting Member: (2)2x6 SP 2400f-2.0E (4) 0.148"x3" nails into supporting

member,

(4) 0.148"x3" nails into supported

member.
Bearing I (11'9", 10') HUS46
Supporting Member: (2)2x6 SP 2400f-2.0E

(4) 0.148"x3" nails into supporting

member,
(4) 0.148"x3" nails into supported member

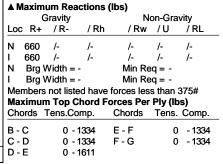
Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

Additional Notes

+ 2x6 continuous strongback. See detail STRBRIBR1014 for bracing and bridging recommendations.

Truss must be installed as shown with top chord up.

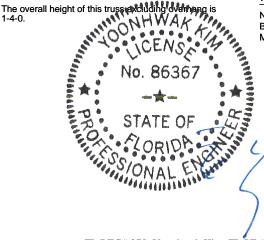


Maximum Bot Chord F	orces Per	Ply (lbs	s)
Chords Tens.Comp.	Chords	Tens.	Со

Choras	rens.comp.		Choras	rens. Comp.	
N - M	812	0	K - J	1610	0
M - L	1610	0	J - I	812	0
L-K	1611	0			

Maximum Web Forces Per Ply (lbs)

webs	Tens.Comp.		Webs	Tens. Comp.	
N - B	0	- 994	E-J	0	- 473
B - M	648	0	J - G	648	0
M - D	0	- 473	G-I	0	- 994



FL REG# 278, Yoonhwak Kim, FL PE #86367 01/26/2021

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

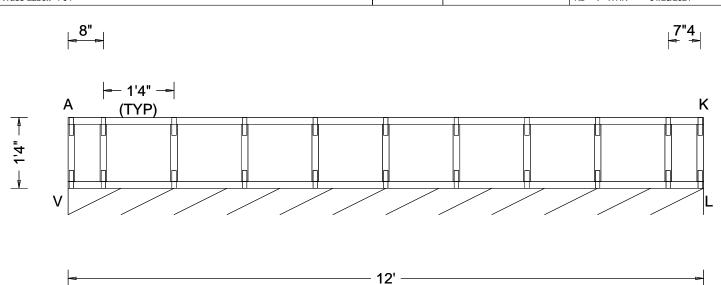
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2400 Lake Orange Dr. Suite 150 Orlando FL, 32837

Job Number: 20-4966 Ply: 1 SEQN: 335826 / T40 / SY42 Cust: R215 JRef: 1X2e2150002 DrwNo: 026.21.1006.45954 Dale and Karla Nickelson Res Qty: 1 FROM: CDM Truss Label: F04 KD / WHK 01/26/2021



Pg: NA Ct: NA CAT: NA PP Deflection in loc L/defl L/# VERT(LL): 0.000 l 999 480 VERT(TL): 0.000 l 999 480 VERT(TL): 0.000 l 999 360 HORZ(LL): -0.000 V HORZ(TL): 0.000 V Creep Factor: 2.0 Bldg Code: FBC 7th Ed. 202 ONResTC CSI: 0.079 Max BC CSI: 0.019 Max Web CSI: 0.032 FT/RT:20(0)/10(0) Plate Type(s):	/find Std: NA peed: NA mph nclosure: NA attegory: NA XP: NA ean Height: NA ft CDL: NA psf CDL: NA psf WFRS Parallel Dist: NA &C Dist a: NA ft peed: NA ft Code / Misc Bldg Code: FTPI Std: 201- Rep Factors Rep Factors FT/RT:20(0)/ Plate Type(s)	Loc R+ /R- /Rh /Rw /U /R L* 110 /- /- /- /- /- Bearing V is a rigid surface. Members not listed have forces less than 375#

Value Set: 13B (Effective 6/1/2013)

Top chord 4x2 SP #2 Bot chord 4x2 SP #2 Webs 4x2 SP #3

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Bracing

Sheathing is required for any longitudinal(drag) forces. All connections to be designed by the building designer.

Fasten rated sheathing to one face of this frame.

Plating Notes

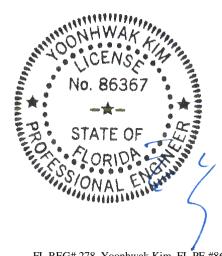
All plates are 1X4 except as noted.

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

Additional Notes

Truss must be installed as shown with top chord up. The overall height of this truss excluding overhang is



FL REG# 278, Yoonhwak Kim, FL PE #86367 01/26/2021

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

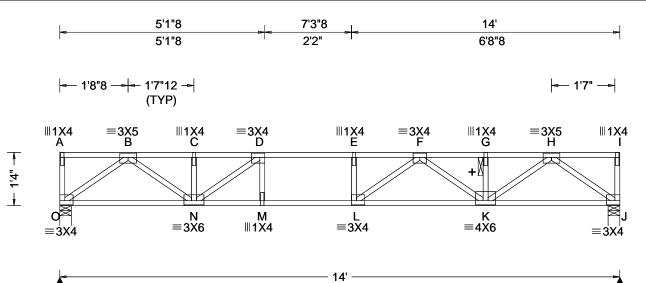
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Job Number: 20-4966 Ply: 1 SEQN: 335774 / T24 / SY42 Cust: R215 JRef: 1X2e2150002 DrwNo: 026.21.1006.45359 Dale and Karla Nickelson Res Qty: 6 FROM: CDM Truss Label: F05 KD / WHK 01/26/2021



Loading Criteria (psf) TCLL: 40.00 TCDL: 10.00 BCLL: 0.00 BCDL: 5.00 Des Ld: 55.00	Wind Criteria Wind Std: NA Speed: NA mph Enclosure: NA Category: NA EXP: NA Mean Height: NA ft	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA	PP Deflection in loc L/defl L/# VERT(LL): 0.178 E 945 480 VERT(TL): 0.266 E 632 360 HORZ(LL): 0.026 J HORZ(TL): 0.036 J	П
TCDL: 10.00 BCLL: 0.00 BCDL: 5.00 Des Ld: 55.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.00 Spacing: 24.0 "	Enclosure: NA Category: NA	Pf: NA Ce: NA Lu: NA Cs: NA	VERT(LL): 0.178 E 945 480 VERT(TL): 0.266 E 632 360 HORZ(LL): 0.026 J HORZ(TL): 0.036 J Creep Factor: 2.0	- 1
Lumber				

	▲ Maxi	mum Re	actions (lbs)		
		Gravity		No	on-Gra	vity
	Loc R	+ /R-	/ Rh	/ Rw	/ U	/ RL
	0 770) /-	/-	/-	/-	/-
	J 770) /-	/-	/-	/-	/-
	O Bro	g Width =	3.5	Min Re	q = 1.5	5
	J Bro	g Width =	3.5	Min Re	q = 1.5	5
	Bearing	gs O & Ja	are a rigio	d surface.		
	Membe	rs not list	ed have	forces les	s than :	375#
	Maxim	um Top (Chord Fo	orces Per	Ply (lb	s)
	Chords	Tens.C	omp.	Chords	Tens.	Comp.
	B-C	0 -	1621	E-F	0	- 2159
	C-D	0 -	1621	F-G	0	- 1645
_	D-E	0 -	2165	G - H	0	- 1645

Value Set: 13B (Effective 6/1/2013)

Top chord 4x2 SP M-31 Bot chord 4x2 SP #2 Webs 4x2 SP #3

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

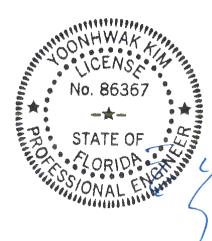
Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

Additional Notes

+ 2x6 continuous strongback. See detail STRBRIBR1014 for bracing and bridging recommendations

Truss must be installed as shown with top chord up. The overall height of this truss excluding overhang is



Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Co	mp.	Chords	Tens. Co	omp.
O - N	965	0	L-K	2050	0
N - M	2161	0	K-J	970	0
M - L	2165	0			

Maximum Web Forces Per Plv (lbs)

Webs	Tens.Comp.	Webs	Tens.	Comp.
0-В	0 - 1182	. F-K	0	- 503
B - N	813 () K-H	838	0
N - D	0 - 788	H-J	0	- 1188
	204 7/	i e		

FL REG# 278, Yoonhwak Kim, FL PE #86367 01/26/2021

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

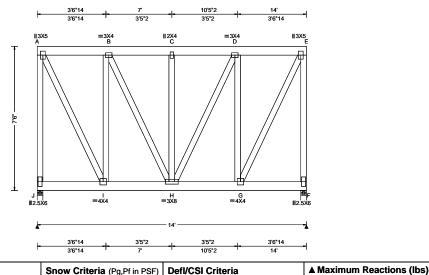
IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions.



Job Number: 20-4966 Ply: 2 SEQN: 354452 / T8 / FLAT Cust: R215 JRef: 1X2e2150002 Qty: 1 DrwNo: 026.21.1006.46296 FROM: CDM Dale and Karla Nickelson Res Truss Label: FT02 KD / WHK 01/26/2021

2 Complete Trusses Required



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.028 C 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(TL): 0.057 C 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.002 A
Des Ld: 40.00	EXP: C		HORZ(TL): 0.005 A
NCBCLL: 0.00	Mean Height: 17.50 ft TCDL: 5.0 psf	Code / Misc Criteria	Creep Factor: 2.0
Soffit: 2.00	BCDL: 5.0 psf	Bldg Code: FBC 7th Ed. 202	OMRaesTC CSI: 0.108
Load Duration: 1.25	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max BC CSI: 0.103
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Factors Used: No	Max Web CSI: 0.595
-	Loc. from endwall: not in 12.50 ft	FT/RT:20(0)/10(0)	
	GCpi: 0.18	Plate Type(s):	
	Wind Duration: 1.60	WAVE	VIEW Ver: 20.01.01A.0724.11
		A I Prince I Markey	

Lumber

Top chord 2x6 SP 2400f-2.0E Bot chord 2x6 SP 2400f-2.0E Webs 2x4 SP #3

Value Set: 13B (Effective 6/1/2013)

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Nailnote

Nail Schedule:0.131"x3", min. nails Top Chord: 1 Row @ 4.25" o.c. Bot Chord: 1 Row @12.00" o.c. : 1 Row @ 4" o.c.

Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special Loads

---(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25) TC: From 30 plf at 0.00 to 30 plf at 14.00 BC: From 20 plf at 0.00 to 20 plf at 14.00 455 lb Conc. Load at 0.44, 1.77, 3.10, 4.44 5.77, 7.10, 8.44, 9.77, 11.10, 12.44, 13.65

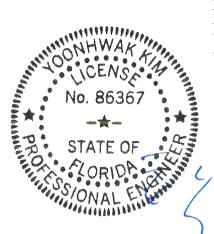
The TC of this truss shall be braced with attached spans at 24" oc in lieu of structural sheathing.

Wind loads and reactions based on MWFRS. End verticals not exposed to wind pressure.

Additional Notes

Truss must be installed as shown with top chord up. The overall height of this truss excluding overhang is 7-6-0

WIND LOAD CASE MODIFIED!



Loc R+ /Rh /Rw /U /RL 2821 /-/274 /-2887 /-/-/-/281 Wind reactions based on MWFRS Brg Width = 3.5Min Rea = 1.5Brg Width = 3.5 Min Req = 1.5Bearings J & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Non-Gravity

50

553

-519

- 54

Gravity

50 - 518 -690

B - C D-E Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

H-G

Maximum Web Forces Per Ply (lbs)

552

- 690

Webs	Tens.Comp.	Webs	Tens. Comp.
A - J	126 - 1208	D-G	101 -815
A - I	1184 - 114	G-E	1187 - 115
I - B	101 -813	E-F	128 - 1232
C - H	62 - 460		

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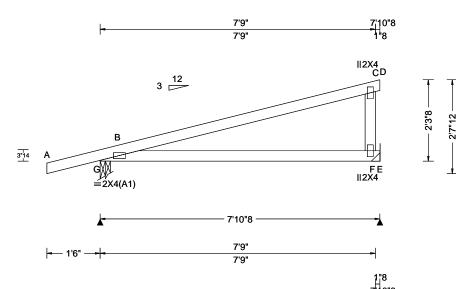
WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

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Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec. 2.

Job Number: 20-4966 Ply: 1 SEQN: 335722 / T25 / MONO Cust: R215 JRef: 1X2e2150002 DrwNo: 026.21.1006.45844 Dale and Karla Nickelson Res Qty: 17 FROM: CDM Truss Label: M01 KD / WHK 01/26/2021



			/1
Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): NA
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(TL): NA
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.019 F
Des Ld: 40.00	EXP: C		HORZ(TL): 0.036 F
NCBCLL: 10.00	Mean Height: 15.00 ft TCDL: 5.0 psf	Code / Misc Criteria	Creep Factor: 2.0
Soffit: 2.00	BCDL: 5.0 psf	Bldg Code: FBC 7th Ed. 202	OMRæsTC CSI: 0.748
Load Duration: 1.25	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max BC CSI: 0.545
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Factors Used: Yes	Max Web CSI: 0.280
'	Loc. from endwall: not in 4.50 ft	FT/RT:20(0)/10(0)	
	GCpi: 0.18	Plate Type(s):	
	Wind Duration: 1.60	WAVE	VIEW Ver: 20.02.01A.1209.11

▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /Rh /Rw /U /RL G 427 /232 305 /-/161 /-/65 Wind reactions based on MWFRS Brg Width = 3.5 G Min Rea = 1.5Brg Width = -Min Req = -Bearing G is a rigid surface. Members not listed have forces less than 375#

Lumber

Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP #2 Bot chord 2x4 SP #2 Webs 2x4 SP #3

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Loading

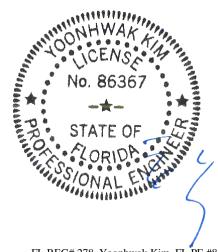
Bottom chord checked for 10.00 psf non-concurrent live load.

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

The overall height of this truss excluding overhang is



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WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

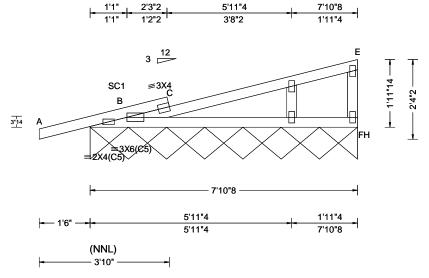
IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

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Job Number: 20-4966 Ply: 1 SEQN: 335724 / T15 / GABL Cust: R215 JRef: 1X2e2150002 DrwNo: 026.21.1006.46063 Dale and Karla Nickelson Res Qty: 2 FROM: CDM Truss Label: M02 KD / WHK 01/26/2021



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Mean Height: 15.00 ft	, ,	PP Deflection in loc L/defl L/# VERT(LL): 0.030 C 999 240 VERT(TL): 0.058 C 999 180 HORZ(LL): -0.004 E - HORZ(TL): 0.008 E - Creep Factor: 2.0
NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Bldg Code: FBC 7th Ed. 202 TPI Std: 2014 Rep Factors Used: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	· •

▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ /Rw /U /RL H* 93 /-/-/49 Wind reactions based on MWFRS H Brg Width = 94.5 Min Req = -Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber

Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP #2 Bot chord 2x4 SP #2 Webs 2x4 SP #3

Stack Chord: SC1 2x4 SP #2;

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Plating Notes

All plates are 2X4 except as noted.

Bottom chord checked for 10.00 psf non-concurrent live load.

Purlins

In lieu of structural panels use purlins to brace TC @

Wind

Wind loads based on MWFRS with additional C&C member design.

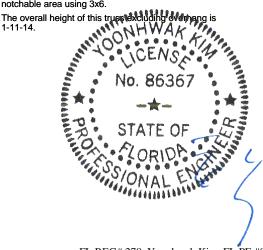
Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.



FL REG# 278, Yoonhwak Kim, FL PE #86367 01/26/2021

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

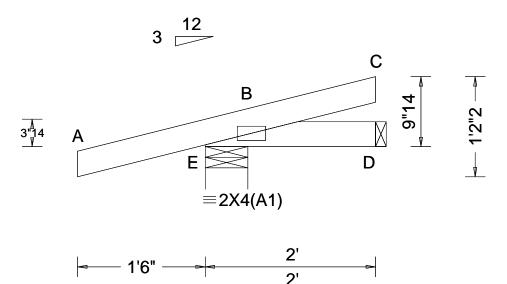
IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

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Job Number: 20-4966 Ply: 1 SEQN: 335878 / T17 / MONO Cust: R215 JRef: 1X2e2150002 Dale and Karla Nickelson Res Qty: 7 FROM: CDM DrwNo: 026.21.1006.45313 Truss Label: M03 KD / WHK 01/26/2021



TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " Speed: Enclose Risk Ca EXP: C Mean H TCDL: 9 BCDL: MWFR: C&C Di Loc. fro	rtd: ASCE 7-16 130 mph ure: Closed ategory: II deight: 15.00 ft 5.0 psf 5.0 psf 5 Parallel Dist: 0 to h/2 sist a: 3.00 ft FT/ Fr/ Fr/ Fr/ Fr/	NA Ct: NA NA NA Cs: NA ow Duration: NA de / Misc Criter g Code: FBC 7t Std: 2014 o Factors Used: RT:20(0)/10(0) te Type(s):	CAT: NA Ce: NA ia h Ed. 202	Defl/CSI Criteria PP Deflection in VERT(LL): 0.005 VERT(TL): 0.008 HORZ(LL): 0.001 HORZ(TL): 0.002 Creep Factor: 2.0 0MResTC CSI: 0.1 Max BC CSI: 0.0 Max Web CSI: 0.0	loc L/defl C 999 C 999 C - C - 148 036	240 180 - -	Loc E D Win E D Bea	G R+ 231 38 d read Brg V Brg V ring E	ravity /R- /- /- ttions t Vidth = is a rig not list
---	---	--	--------------------------------------	--	--	----------------------	---------------------------------------	--	---

eactions (lbs) Non-Gravity /Rh /Rw /U /RL /135 /-/30 /-/16 based on MWFRS = 6.0 Min Req = 1.5 = 1.5 Min Req = rigid surface. isted have forces less than 375#

Lumber

Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP #2 Bot chord 2x4 SP #2

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Loading

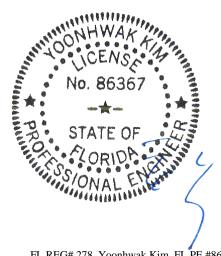
Bottom chord checked for 10.00 psf non-concurrent live load.

Wind loads based on MWFRS with additional C&C

Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is



FL REG# 278, Yoonhwak Kim, FL PE #86367 01/26/2021

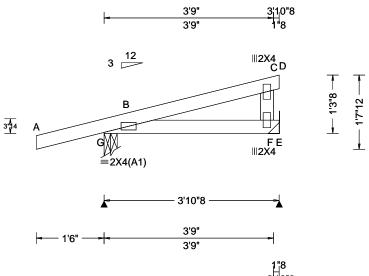
WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

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Job Number: 20-4966 Ply: 1 SEQN: 335960 / T18 / MONO Cust: R215 JRef: 1X2e2150002 Dale and Karla Nickelson Res Qty: 8 DrwNo: 026.21.1006.45735 FROM: CDM Truss Label: M04 KD / WHK 01/26/2021



Coading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 BCDL	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(TL): NA HORZ(LL): 0.001 F HORZ(TL): 0.003 F
Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Code / Misc Criteria Bldg Code: FBC 7th Ed. 202 TPI Std: 2014 Rep Factors Used: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Creep Factor: 2.0

▲ M	laxim	um Rea	ctions (I	bs)		
	G	avity		No	on-Gra	vity
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
G	278	/-	/-	/157	/87	/47
Е	131	/-	/-	/75	/9	/-
Win	d read	ctions b	ased on I	MWFRS		
G	Brg V	Vidth =	3.5	Min Reg = 1.5		
Е	Brg V	Vidth =	-	Min Re	q = -	
Bea	ıring G	is a ric	aid surfac		•	
Mer	nbers	not list	, ed have f	orces les	s than	375#

Lumber

Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP #2 Bot chord 2x4 SP #2 Webs 2x4 SP #3

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

The overall height of this truss excluding overhang is



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WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

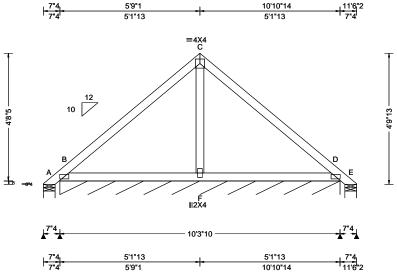
IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

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2400 Lake Orange Dr. Suite 150 Orlando FL, 32837

Job Number: 20-4966 Ply: 1 SEQN: 335972 / T7 / COMN Cust: R215 JRef: 1X2e2150002 FROM: CDM DrwNo: 026.21.1006.46218 Dale and Karla Nickelson Res Qty: 17 Truss Label: PB01 KD / WHK 01/26/2021



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.001 F 999 240	
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(TL): 0.002 F 999 180	
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.001 F	
Des Ld: 40.00	EXP: C		HORZ(TL): 0.003 F	
NCBCLL: 10.00	Mean Height: 23.42 ft	Code / Misc Criteria	Creep Factor: 2.0	
Soffit: 2.00	TCDL: 5.0 psf BCDL: 2.0 psf	Bldg Code: FBC 7th Ed. 202	ONReesTC CSI: 0.256	
Load Duration: 1.25	MWFRS Parallel Dist: h to 2h	TPI Std: 2014	Max BC CSI: 0.085	
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Factors Used: Yes	Max Web CSI: 0.009	
-F	Loc. from endwall: not in 13.00 ft	FT/RT:20(0)/10(0)		
	GCpi: 0.18	Plate Type(s):		
	Wind Duration: 1.60	WAVE	VIEW Ver: 20.02.01A.1209.11	Ī
Lumber				_

	1102						
	▲ N	laximu	m Reac	tions (lb	s), or *=	PLF	
		Gı	ravity		No	n-Grav	rity
	Loc	: R+	/ R-	/ Rh	/Rw	/ U	/ RL
	Α	4	/-1	/-	/-	/16	/-
	В*	48	/-	/-	/42	/-	/-
	Е	-	/-193	/-	/-	/94	/-
	Wir	nd reac	tions bas	sed on M	WFRS		
	Α	Brg W	/idth = 5	.2	Min Red	q = 1.5	
			/idth = 1	23	Min Red	q = -	
	Е	Brg W	/idth = 5	.2	Min Red	q = 1.5	
	Bearings A, B, & E are a rigid surface.						
	Members not listed have forces less than 375#						
_							
	ı						

Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP #2 Bot chord 2x4 SP #2 Webs 2x4 SP #3

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Plating Notes

All plates are 2X4(A1) except as noted.

Bottom chord checked for 10.00 psf non-concurrent live load.

Wind

Wind loads based on MWFRS with additional C&C

Wind loading based on both gable and hip roof types.

Additional Notes

Negative reaction(s) of -193# MAX. from a non-wind load case requires uplift connection. See Maximum

Refer to DWG PB160160118 for piggyback details.

The overall height of this truss excluding overhang is



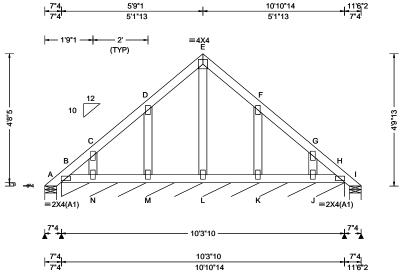
FL REG# 278, Yoonhwak Kim, FL PE #86367 01/26/2021

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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Mean Height: 23.42 ft TCDL: 5.0 psf BCDL: 2.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft	Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Code / Misc Criteria Bldg Code: FBC 7th Ed. 202 TPI Std: 2014 Rep Factors Used: Yes FT/RT:20(0)/10(0)	PP Deflection in loc L/defl L/# VERT(LL): 0.000 F 999 240 VERT(TL): 0.000 F 999 180 HORZ(LL):-0.000 F HORZ(TL): 0.000 F Creep Factor: 2.0	- 1
Spacing: 24.0		'	VIEW Ver: 20.02.01A.1209.11	

110	_					
▲ N	laxim	um Rea	ctions (I	bs), or *=	PLF	<u> </u>
	(avity	-	No	on-Gra	vity
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
Α	1	/-	/-	/5	/-	/-
В*	28	/-	/-	/30	/-	/-
1	17	/-	/-	/11	/-	/-
Wir	nd rea	ctions b	ased on I	MWFRS		
Α	Brg V	Vidth =	5.2	Min Re	q = 1.3	5
В	Brg V	Vidth =	123	Min Re	q = -	
I Brg Width = 5.2 Min Reg = 1.5						
Bearings A, B, & I are a rigid surface.						
Mei	mbers	not liste	ed have fo	orces les	s than	375#
1						

Lumber

Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP #2 Bot chord 2x4 SP #2 Webs 2x4 SP #3

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Plating Notes

All plates are 2X4 except as noted.

Bottom chord checked for 10.00 psf non-concurrent live load.

Wind

Wind loads based on MWFRS with additional C&C

Wind loading based on both gable and hip roof types.

Additional Notes

Refer to DWG PB160160118 for piggyback details. The overall height of this truss excluding overhang is



FL REG# 278, Yoonhwak Kim, FL PE #86367 01/26/2021

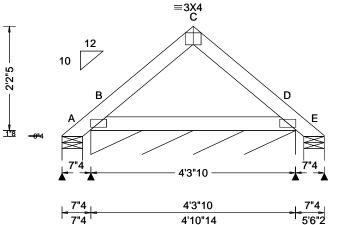
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria	۱,
Loading Criteria (psf)	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Mean Height: 16.23 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: > 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 13.00 ft GCpi: 0.18	, ,	PP Deflection in loc L/defl L/# VERT(LL): 0.000 C 999 240 VERT(TL): 0.001 C 999 180 HORZ(LL): -0.000 HORZ(TL): 0.001 Creep Factor: 2.0	ի և
	Wind Duration: 1.60	WAVE	VIEW Ver: 20.02.01A.1209.11	
Lumbor				

▲ N			ctions (I			
	G	avity		No	on-Gra	vity
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
Α	1	/-1	/-	/53	/60	/67
В*	83	/-	/-	/72	/6	/-
Е	1	/-1	/-	/11	/18	/-
Wir	nd read	ctions b	ased on I	MWFRS		
Α	Brg V	Vidth =	5.2	Min Re	q = 1.5	5
В	Brg V	Vidth =	51.6	Min Re	q = -	
E Brg Width = 5.2 Min Req = 1.5						
Bearings A, B, & E are a rigid surface.						
Mei	mbers	not list	ed have fo	orces les	s than	375#

Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP #2 Bot chord 2x4 SP #2

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Plating Notes

All plates are 2X4(A1) except as noted.

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

Refer to DWG PB160160118 for piggyback details. The overall height of this truss excluding overhang is 2-3-13.



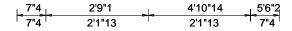
FL REG# 278, Yoonhwak Kim, FL PE #86367 01/26/2021

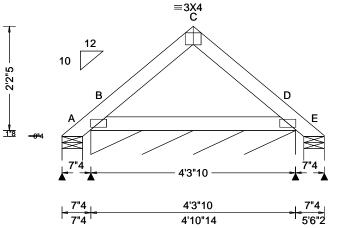
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	
Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Mean Height: 16.23 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft	, ,	PP Deflection in loc L/defl L/# VERT(LL): 0.000 C 999 240 VERT(TL): 0.001 C 999 180 HORZ(LL):-0.000 HORZ(TL): 0.001 Creep Factor: 2.0	
opasing. 2 iii	Loc. from endwall: Any GCpi: 0.18	FT/RT:20(0)/10(0) Plate Type(s):		ľ
	Wind Duration: 1.60	WAVE	VIEW Ver: 20.02.01A.1209.11	
Lumbor				

▲ Maximum Reactions (lbs), or *=PLF							
	G	avity		No	on-Gra	vity	
Loc	: R+	/ R-	/Rh	/ Rw	/ U	/ RL	
Α	1	/-1	/-	/52	/58	/65	
В*	83	/-	/-	/71	/26	/-	
Е	1	/-1	/-	/11	/18	/-	
Wir	nd read	ctions b	ased on I	MWFRS			
Α	Brg V	Vidth =	5.2	Min Re	q = 1.5	5	
В	Brg V	Vidth =	51.6	Min Re	q = -		
E	Brg V	Vidth =	5.2	Min Re	$\dot{q} = 1.5$	5	
Bearings A, B, & E are a rigid surface.							
Ме	mbers	not list	ed have f	orces les	s than	375#	

Lumber

Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP #2 Bot chord 2x4 SP #2

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Plating Notes

All plates are 2X4(A1) except as noted.

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

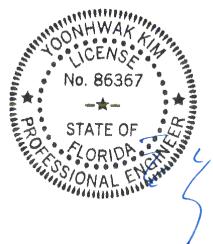
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

Refer to DWG PB160160118 for piggyback details. The overall height of this truss excluding overhang is 2-3-13.



FL REG# 278, Yoonhwak Kim, FL PE #86367 01/26/2021

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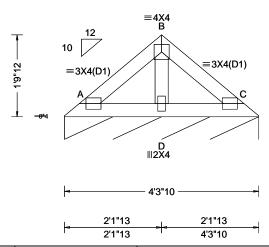
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Job Number: 20-4966 Ply: 1 SEQN: 335947 / T14 / COMN Cust: R215 JRef: 1X2e2150002 DrwNo: 026.21.1006.45344 Dale and Karla Nickelson Res Qty: 1 FROM: CDM Truss Label: PB04 KD / WHK 01/26/2021





Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.000 D 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(TL): 0.000 D 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.000 D
Des Ld: 40.00	EXP: C		HORZ(TL): 0.000 D
NCBCLL: 10.00	Mean Height: 22.17 ft TCDL: 5.0 psf	Code / Misc Criteria	Creep Factor: 2.0
Soffit: 2.00	BCDL: 2.0 psf	Bldg Code: FBC 7th Ed. 202	OMReesTC CSI: 0.006
Load Duration: 1.25	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max BC CSI: 0.012
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Factors Used: Yes	Max Web CSI: 0.002
' "	Loc. from endwall: Any	FT/RT:20(0)/10(0)	
	GCpi: 0.18	Plate Type(s):	
	Wind Duration: 1.60	WAVE	VIEW Ver: 20.02.01A.1209.11
Lumban			

▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ /R /Rh /Rw /U /RL A* 4 /-/-/20 Wind reactions based on MWFRS A Brg Width = 51.6 Min Req = -Bearing A is a rigid surface. Members not listed have forces less than 375#

Lumber

Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP #2 Bot chord 2x4 SP #2 Webs 2x4 SP #3

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

Refer to DWG PB160160118 for piggyback details. The overall height of this truss excluding overhang is



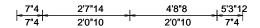
FL REG# 278, Yoonhwak Kim, FL PE #86367 01/26/2021

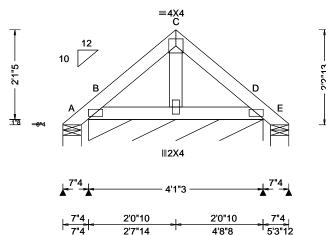
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Wind Criteria (psf) TCLL: 20.00 Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Mean Height: 18.19 ft TCDL: 5.0 psf BCDL: 5.	Snow Criteria (Pg,Pf in PSF) Pg: NA
--	---------------------------------------

▲ M	aximı	um Rea	ctions (I	bs), or *=		
	G	ravity		No	on-Gra	vity
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
Α	9	/-15	/-	/53	/50	/63
В*	86	/-	/-	/68	/33	/-
Е	-	/-4	/-	/12	/8	/-
Win	d read	ctions b	ased on I	MWFRS		
Α	Brg V	Vidth =	5.2	Min Re	q = 1.5	5
В	Brg V	Vidth =	49.2	Min Re	q = -	
E Brg Width = 5.2 Min Reg = 1.5						
Bearings A, B, & E are a rigid surface.						
Mer	nbers	not liste	ed have f	orces les	s than	375#

Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP #2 Bot chord 2x4 SP #2 Webs 2x4 SP #3

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Plating Notes

All plates are 2X4(A1) except as noted.

Bottom chord checked for 10.00 psf non-concurrent

Wind

Wind loads based on MWFRS with additional C&C

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Refer to DWG PB160160118 for piggyback details.

The overall height of this truss excluding overhang is



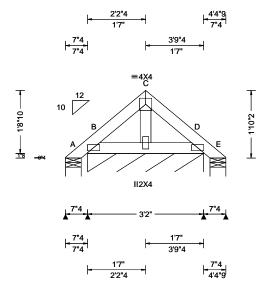
FL REG# 278, Yoonhwak Kim, FL PE #86367 01/26/2021

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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Mean Height: 15.75 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Code / Misc Criteria Bldg Code: FBC 7th Ed. 202 TPI Std: 2014 Rep Factors Used: Yes FT/RT:20(0)/10(0) Plate Type(s):	PP Deflection in loc L/defl L/# VERT(LL): 0.000 F 999 240 VERT(TL): 0.000 F 999 180 HORZ(LL): 0.000 F HORZ(TL): 0.000 F Creep Factor: 2.0
	1	` ' ` ' '	VIEW Ver: 20.02.01A.1209.11
Lumber			

▲ Maximum Reactions (lbs), or *=PLF							
	G	avity		No	on-Gra	vity	
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL	
Α	8	/-	/-	/42	/33	/50	
В*	83	/-	/-	/67	/24	/-	
Е	8	/-	/-	/8	/-	/-	
Wir	nd read	ctions b	ased on I	MWFRS			
Α	Brg V	Vidth =	5.2	Min Re	q = 1.5	5	
В	Brg V	Vidth =	38.0	Min Re	q = -		
E	Brg V	Vidth =	5.2	Min Re	$\dot{q} = 1.5$	5	
Bea	arings	A, B, &	E are a r	igid surfa	ce.		
Mei	mbers	not liste	ed have f	orces les	s than	375#	

Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP #2 Bot chord 2x4 SP #2 Webs 2x4 SP #3

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Plating Notes

All plates are 2X4(A1) except as noted.

Bottom chord checked for 10.00 psf non-concurrent

Wind

Wind loads based on MWFRS with additional C&C

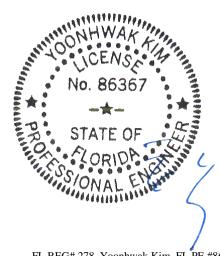
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Refer to DWG PB160160118 for piggyback details.

The overall height of this truss excluding overhang is



FL REG# 278, Yoonhwak Kim, FL PE #86367 01/26/2021

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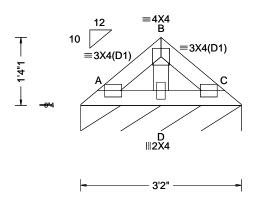
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Job Number: 20-4966 Ply: 1 SEQN: 335748 / T38 / SPEC Cust: R215 JRef: 1X2e2150002 DrwNo: 026.21.1006.46235 Dale and Karla Nickelson Res Qty: 1 FROM: CDM Truss Label: PB08 KD / WHK 01/26/2021





Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria	Γ
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.000 D 999 240	
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(TL): 0.000 D 999 180	
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.000 D	
Des Ld: 40.00	EXP: C		HORZ(TL): 0.000 D	I
NCBCLL: 10.00	Mean Height: 21.43 ft TCDL: 5.0 psf	Code / Misc Criteria	Creep Factor: 2.0	
Soffit: 2.00	BCDL: 2.0 psf	Bldg Code: FBC 7th Ed. 202	OMRaesTCCSI: 0.004	
Load Duration: 1.25	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max BC CSI: 0.005	
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Factors Used: Yes	Max Web CSI: 0.001	
' -	Loc. from endwall: Any	FT/RT:20(0)/10(0)		
	GCpi: 0.18	Plate Type(s):		1

WAVE

▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ /Rw /U /RL A* 4 /-/-/20 Wind reactions based on MWFRS A Brg Width = 38.0 Min Req = -Bearing A is a rigid surface. Members not listed have forces less than 375#

Lumber

Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP #2 Bot chord 2x4 SP #2 Webs 2x4 SP #3

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Wind Duration: 1.60

Loading

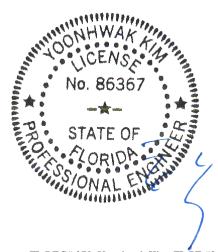
Bottom chord checked for 10.00 psf non-concurrent live load.

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

Refer to DWG PB160160118 for piggyback details. The overall height of this truss excluding overhang is



VIEW Ver: 20.02.01A.1209.11

FL REG# 278, Yoonhwak Kim, FL PE #86367 01/26/2021

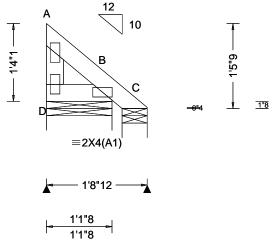
WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

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Job Number: 20-4966 Ply: 1 SEQN: 335750 / T19 / GABL Cust: R215 JRef: 1X2e2150002 DrwNo: 026.21.1006.45921 Dale and Karla Nickelson Res Qty: 3 FROM: CDM Truss Label: PB09 KD / WHK 01/26/2021



		10	_	
Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	T
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	١.
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.000 D 999 240	<u> </u>
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(TL): 0.000 D 999 180	1
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.000 A	
Des Ld: 40.00	EXP: C		HORZ(TL): 0.000 D	١
NCBCLL: 10.00	Mean Height: 16.39 ft	Code / Misc Criteria	Creep Factor: 2.0	1
Soffit: 2.00	TCDL: 5.0 psf BCDL: 5.0 psf	Bldg Code: FBC 7th Ed. 202	OMRæsTC CSI: 0.011	19
Load Duration: 1.25	MWFRS Parallel Dist: h to 2h	TPI Std: 2014	Max BC CSI: 0.005	[
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Factors Used: Yes	Max Web CSI: 0.004	'
-F	Loc. from endwall: not in 9.00 ft	FT/RT:20(0)/10(0)		
	GCpi: 0.18	Plate Type(s):		
	Wind Duration: 1.60	WAVE	VIEW Ver: 20.02.01A.1209.11	

▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /Rh /Rw /U /RL D 93 /85 /33 14 /-/13 /-Wind reactions based on MWFRS Min Req = 1.5 Brg Width = 13.5 D Brg Width = 5.2 Min Req = 1.5 Bearings D & C are a rigid surface. Members not listed have forces less than 375#

Lumber

Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP #2 Bot chord 2x4 SP #2 Webs 2x4 SP #3

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Plating Notes

All plates are 2X4 except as noted.

Bottom chord checked for 10.00 psf non-concurrent

Wind

Wind loads based on MWFRS with additional C&C

Left end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Refer to DWG PB160160118 for piggyback details.

The overall height of this truss excluding overhang is



FL REG# 278, Yoonhwak Kim, FL PE #86367 01/26/2021

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Gable Stud Reinforcement Detail

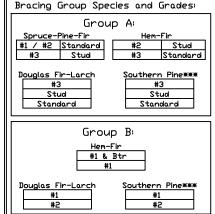
ASCE 7-16: 140 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Dr: 120 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00

Dr: 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00

Dr. 100 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure D. Kzt = 1.00

		2x4 Vertica	Brace	No	(1) 1×4 *L	Brace *	(1) 2×4 *L	" Brace *	(2) 2×4 L	" Brace **	(1) 2×6 'L	" Brace *	(2) 2x6 *L	Brace **
ا ے اا	Spacing	Species	Grade		Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
 仁		CDE	#1 / #2	4′ 3″	7′ 3″	7′ 7″	8′ 7 ″	8′ 11″	10′ 3″	10′ 8″	13′ 6″	14′ 0″	14′ 0″	14′ 0″
	1.7	SPF	#3	4′ 1 ″	6′ 7 ″	7′ 1″	8′ 6 ″	8′ 10 ″	10′ 1″	10′ 6 ″	13′ 4″	13′ 10 ″	14′ 0″	14′ 0″
21	Ų	HF	Stud	4′ 1 ″	6′ 7 ″	7′ 0 ″	8′ 6 ″	8′ 10 ′	10′ 1″	10′ 6″	13′ 4″	13′ 10 ″	14′ 0″	14′ 0 ″
>	Ō	1 11	Standard	4′ 1″	5′ 8 ″	6′ 0 ″	7′ 7″	8′ 1 ″	10′ 1″	10′ 6″	11′ 10″	12′ 8″	14′ 0″	14′ 0″
ا به اا			#1	4′ 6″	7′ 4″	7′ 8 ″	8′ 8 ″	9′ 0″	10′ 4″	10′ 9″	13′ 8″	14′ 0″	14′ 0″	14′ 0″
-	*	SP	#2	4′ 3″	7′ 3″	7′ 7″	8′ 7 ″	8′ 11 ″	10′ 3″	10′ 8″	13′ 6″	14′ 0″	14′ 0″	14′ 0″
	4	l	#3	4′ 2 ″	6′ 0 ″	6′ 4″	7′ 11″	8′ 6″	10′ 2″	10′ 7″	12′ 5 ″	13′ 4″	14′ 0″	14′ 0″
	Ω	IDFLI	Stud	4′ 2 ″	6′ 0 ″	6′ 4″	7′ 11″	8′ 6″	10′ 2″	10′ 7″	12′ 5 ″	13′ 4″	14′ 0″	14′ 0″
설			Standard	4′ 0″	5′ 3″	5′ 7 ″	7′ 0 ″	7′ 6″	9′ 6″	10′ 2″	11′ 0″	11′ 10″	14′ 0″	14′ 0″
II <u>.</u>		SPF	#1 / #2	4′ 11″	8′ 4″	8′ 8 ″	9′ 10″	10′ 3″	11′ 8″	12′ 2″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
🚎		7 L L	#3	4′ 8″	8′ 1 ″	8′ 8″	9′ 8″	10′ 1″	11′ 7″	12′ 1″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
	\cup	HF	Stud	4′ 8″	8′ 1″	8′ 6 ″	9′ 8″	10′ 1″	11′ 7″	12′ 1″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
امَا	Ō	1 11	Standard	4′ 8 ″	6′ 11″	7′ 5 ′	9′ 3 ″	9′ 11″	11′ 7″	12′ 1″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
\parallel $\stackrel{\sim}{\smile}$ \parallel			#1	5′ 1 ″	8′ 5″	8′ 9″	9′ 11″	10′ 4″	11′ 10″	12′ 4″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
/		SP	#2	4′ 11″	8′ 4″	8′ 8 ″	9′ 10″	10′ 3″	11′ 8″	12′ 2 ″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
	è	L	#3	4′ 9″	7′ 4″	7′ 9″	9′ 9″	10′ 2″	11′ 8″	12′ 1″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
IJωl	Ţ	IDFLI	Stud	4′ 9 ″	7′ 4″	7′ 9″	9′ 9″	10′ 2″	11′ 8″	12′ 1″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
			Standard	4′ 8″	6′ 5 ″	6′ 10 ″	8′ 7 ″	9′ 2″	11′ 7″	12′ 1″	13′ 6 ″	14′ 0″	14′ 0″	14′ 0″
abl		SPF	#1 / #2	5′ 5 ″	9′ 2″	9′ 6″	10′ 10″	11′ 3″	11′ 8″	13′ 5 ″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
0		766	#3	5′ 1 ″	9′ 0″	9′ 4″	10′ 8″	11′ 1″	12′ 9″	13′ 3″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
0	Ū	HF	Stud	5′ 1 ″	9′ 0″	9′ 4″	10′ 8″	11′ 1″	12′ 9″	13′ 3″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
	Ō	1 11	Standard	5′ 1 ″	8′ 0 ″	8′ 6″	10′ 8″	11′ 1″	12′ 9″	13′ 3″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
×	0		#1	5′ 8 ″	9′ 3″	9′ 8″	10′ 11″	11′ 4″	13′ 0″	13′ 6″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
ਰ	*	SP	#2	5′ 5″	9′ 2″	9′ 6″	10′ 10″	11′ 3″	12′ 11″	13′ 5 ′	14′ 0″	14′ 0″	14′ 0″	14′ 0″
IJĔIJ	ù		#3	5′ 3″	8′ 5″	9′ 0″	10′ 9″	11′ 2″	12′ 10″	13′ 4″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
	Ţ	DFL	Stud	5′ 3 ″	8′ 5 ″	9′ 0″	10′ 9″	11′ 2″	12′ 10 ″	13′ 4″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
			Standard	5′ 1 ″	7′ 5″	7′ 11″	9′ 11″	10′ 7″	12′ 9″	13′ 3″	14′ 0″	14′ 0″	14′ 0″	14′ 0″



1x4 Braces shall be SRB (Stress-Rated Board) **For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards, Group B values may be used with these grades.

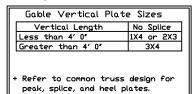
Gable Truss Detail Notes: Wind Load deflection criterion is 1/240.

Provide uplift connections for 55 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

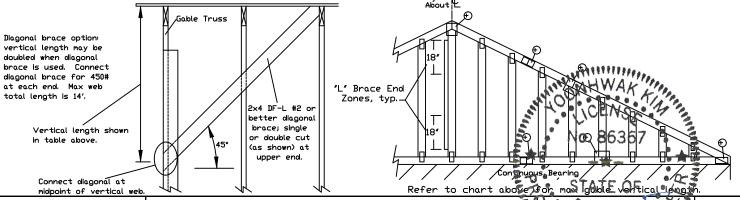
Attach "L" braces with 10d (0.128"x3.0" min) nails. * For (1) "L" brace: space nails at 2" o.c. in 18" end zones and 4" o.c. between zones. ₩¥For (2) "L" braces: space nails at 3" o.c. in 18" end zones and 6" o.c. between zones.

"L" bracing must be a minimum of 80% of web member length.



Refer to the Building Designer for conditions not addressed by this detail.

ASCE7-16-GAB14015



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MAX, TOT, LD, 60 PSF

MAX. SPACING 24.0"

514 Earth City Expressway Suite 242 Earth City, MO 63045

Gable Stud Reinforcement Detail

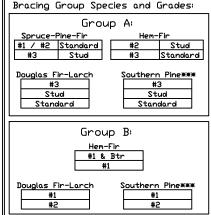
ASCE 7-16: 140 mph Wind Speed, 30' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Dr: 120 mph Wind Speed, 30' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00

Dr: 120 mph Wind Speed, 30' Mean Height, Enclosed, Exposure D, Kzt = 1.00

Or: 100 mph wind speed, 30' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

		2×4	Brace	No	(1) 1×4 *L	Brace *	(1) 2×4 *L	* Brace *	(2) 2×4 L	Brace **	(1) 2×6 L	" Brace *	(2) 2x6 *L	Brace **
_	Spacing	Vertico Species	Grade		Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
구			#1 / #2	4′ 1″	6′ 11″	7′ 2″	8′ 2 ″	8′ 6″	9′ 9″	10′ 2″	12′ 10″	13′ 4″	14′ 0″	14′ 0″
	l . . .	SPF	#3	3′ 10″	6′ 2 ″	6′ 7″	8′ 1″	8′ 5 ″	9′ 8″	10′ 0″	12′ 8″	13′ 2″	14′ 0″	14′ 0″
ō	O U	HF	Stud	3′ 10″	6′ 2″	6′ 6″	8′ 1″	8′ 5 ″	9′ 8″	10′ 0″	12′ 8″	13′ 2″	14′ 0″	14′ 0″
Č	ΙO		Standard	3′ 10″	5′ 3 ″	5′ 7″	7′ 0″	7′ 6″	9′ 6″	10′ 0″	11' 0"	11′ 10″	14′ 0″	14′ 0″
ן מי	_		#1	4′ 2″	7′ 0″	7′ 3″	8′ 3″	8′ 7″	9′ 10″	10′ 3″	13′ 0″	13′ 6″	14′ 0″	14′ 0″
		SP	#2	4′ 1″	6′ 11″	7′ 2″	8′ 2″	8′ 6″	9′ 9″	10′ 2″	12′ 10″	13′ 4″	14′ 0″	14′ 0″
	4		#3	4′ 0″	5′ 7 ″	5′ 11 ″	7′ 5″	7′ 11″	9′ 8″	10′ 1″	11′ 7″	12′ 5 ″	14′ 0″	14′ 0″
	l a	IDFL	Stud	4′ 0″	5′ 7″	5′ 11 ″	7′ 5″	7′ 11″	9′ 8″	10′ 1″	11′ 7″	12′ 5″	14′ 0″	14′ 0″
<u> </u>	_ ` -		Standard	3′ 9″	4′ 11″	5′ 13 ″	6′ 6″	7′ 0 ″	8′ 10 ″	9′ 6″	10′ 3″	11′ 0″	13′ 11″	14′ 0″
\square		CDL	#1 / #2	4′ 8″	7′ 11″	8′ 3″	9′ 4″	9′ 9″	11′ 2″	11′ 7″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
;	-	SPF	#3	4′ 5″	7′ 6″	8′ 3″	9′ 3″	9′ 7″	11′ 0″	11′ 6″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
	l o	HF	Stud	4′ 5 ″	7′ 6″	8′ 0 ″	9′ 3″	9′ 7″	11′ 0″	11′ 6″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
à	l	1 11	Standard	4′ 5 ″	6′ 5 ″	6′ 10 ″	8′ 7 ″	9′ 2″	11′ 0″	11′ 6″	13′ 6″	14′ 0″	14′ 0″	14′ 0″
\parallel			#1	4′ 10″	8′ 0″	8′ 4″	9′ 6″	9′ 10″	11′ 3″	11′ 9″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
>		SP	#2	4′ 8″	7′ 11″	8′ 3 ″	9′ 4″	9′ 9″	11′ 2″	11′ 7″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
	9		#3	4′ 7″	6′ 10 ″	7′ 3″	9′ 1″	9′ 8″	11′ 1″	11′ 6″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
IJω	16	IDFL	Stud	4′ 7″	6′ 10 ″	7′ 3″	9′ 1″	9′ 8″	11′ 1″	11′ 6″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
	_ ` '		Standard	4′ 5″	6′ 0 ″	6′ 5 ″	8′ 0 ″	8′ 7″	10′ 10″	11′ 6″	12′ 7″	13′ 15″	14′ 0″	14′ 0″
l d a		CDE	#1 / #2	5′ 2″	8′ 9″	9′ 1″	10′ 4″	10′ 9 ″	11′ 2″	12′ 9 ″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
	-	SPF	#3	4′ 10″	8′ 7″	8′ 11 ″	10′ 2″	10′ 7″	12′ 2″	12′ 8″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
0	Ų	HF	Stud	4′ 10″	8′ 7 ″	8′ 11 ″	10′ 2 ″	10′ 7″	12′ 2″	12′ 8″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
	Ιō	1 11	Standard	4′ 10″	7′ 5 ″	7′ 11″	9′ 11″	10′ 7″	12′ 2″	12′ 8 ″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
×			#1	5′ 4″	8′ 10 ″	9′ 2″	10′ 5 ″	10′ 10″	12′ 5 ″	12′ 11 ″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
11 2		SP	#2	5′ 2″	8′ 9 ″	9′ 1″	10′ 4″	10′ 9″	12′ 3″	12′ 9 ″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
ΜQ	ù		#3	5′ 0 ″	7′ 10″	8′ 4″	10′ 3″	10′ 8″	12′ 2″	12′ 8 ″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
_	10	lDF L	Stud	5′ 0 ″	7′ 10″	8′ 4″	10′ 3″	10′ 8″	12′ 2″	12′ 8 ″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
			Standard	4′ 10″	6′ 11″	7′ 4″	9′ 3″	9′ 10″	12′ 2″	12′ 8″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
	Symm C													



1x4 Braces shall be SRB (Stress-Rated Board) **For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards, Group B values may be used with these grades.

Gable Truss Detail Notes: Wind Load deflection criterion is L/240.

Provide uplift connections for 100 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

Attach "L" braces with 10d (0.128"x3.0" min) nails. * For (1) "L" brace: space nails at 2" o.c. in 18" end zones and 4" o.c. between zones. ₩¥For (2) "L" braces: space nails at 3" o.c. in 18" end zones and 6" o.c. between zones.

"L" bracing must be a minimum of 80% of web member length.

Gable Vertical Plate	Sizes				
Vertical Length	No Splice				
Less than 4' 0"	2X4				
Greater than 4' 0", but less than 11' 6"	3X4				
Greater than 11' 6"	4X4				
+ Refer to common truss design for peak, splice, and heel plates.					

Refer to the Building Designer for conditions not addressed by this detail.

> |DATE 01/26/2018

ASCE7-16-GAB14030

About E Gable Truss Diagonal brace option: vertical length may be doubled when diagonal brace is used. Connect diagonal brace for 525# at each end. Max web "L" Brace End total length is 14'. Zones, typ. 2x6 DF-L #2 or better diagonal brace; single Vertical length shown or double cut in table above. (as shown) at upper end. Constituous Bearing Connect diagonal at Refer to chart above son midpoint of vertical web.

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For more information see this Job's general notes page and these web sites /25/2021 278 Yoonhwak Kim, FL PE #86367 ALPINE: www.alpineitw.com, TPI www.tpinst.org, SBCA: www.sbcindustry.org, ICC: www.ccsafe.org, #278 Yoonhwak Kim, FL PE #86367

MAX, TOT, LD, 60 PSF 24.0"

514 Earth City Expressway Suite 242 Earth City, MO 63045

MAX. SPACING

CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforecement or scab reinforcement.

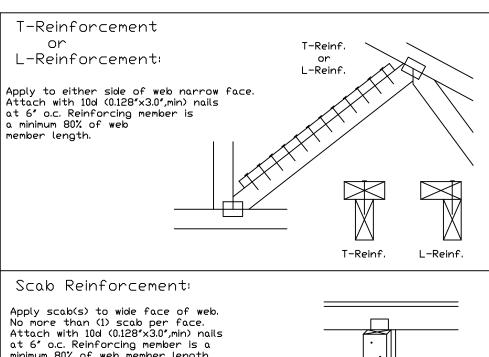
Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

Use scabs instead of L- or T- reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

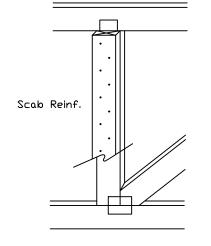
Web Member	Specified CLR	Alternative Reir	
Size	Restraint	T- or L- Reinf.	
2x3 or 2x4	1 row	2×4	1-2×4
2x3 or 2x4	2 rows	2×6	2-2×4
2×6	1 row	2×4	1-2×6
2×6	2 rows	2×6	2-2×4(米)
5×8	1 row	2×6	1-2×8
5×8	2 rows		2-2×6(*/)

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

Center scab on wide face of web. Apply (1) scab to each face of web.



minimum 80% of web member length.



VARNINGI READ AND FOLLOW ALL NOTES ON THIS DRAWING ***IMPORTANT*** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

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Refer to drawings 160A-Z for standard plate positions.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any fallure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation 8 bracing of trusses.

A seal on this drawing or cover page listing this drawing, indicates acceptance of professional

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For more information see this job's general notes page and these web sites: ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcindustry.org; ICC: www.assafeard 1_{72}

IREF CLR Subst. ום אַד DATE 01/02/19 BC DL DRWG BRCLBSUB0119 **PSF** RC II **7**□T. LD. PSF DUR. FAC. SPACING



514 Earth City Expressway Suite 242 Earth City, MO 63045

Gable Detail For Let-in Verticals Gable Truss Plate Sizes Refer to appropriate Alpine gable detail for minimum plate sizes for vertical studs. (+) Refer to Engineered truss design for peak, splice, web, and heel plates. ₩If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web. Gable Example Length typ. (*)

Provide connections for uplift specified on the engineered truss design.

Attach each "T" reinforcing member with

End Driven Nails:

10d Common (0.148"x 3.", min) Nails at 4" o.c. plus

(4) nails in the top and bottom chords.

10d Common (0.148"x3".min) Toenails at 4" o.c. plus

(4) toenails in the top and bottom chords.

This detail to be used with the appropriate Alpine gable detail for ASCE wind load.

ASCE 7-05 Gable Detail Drawings

A13015051014, A12015051014, A11015051014, A10015051014, A14015051014, A13030051014, A12030051014, A11030051014, A10030051014, A14030051014

ASCE 7-10 & ASCE 7-16 Gable Detail Drawings A11515ENC100118, A12015ENC100118, A14015ENC100118, A14015ENC100118,

A18015ENC100118, A12015ENC100118, A12015ENC100118, A12015ENC100118, A120015ENC100118, A120015ENC100118, A120015ENC100118, A120015ENC100118, A12003ENC100118, A12003ENC100118, A120030ENC100118, A120030ENC100118,

\$18015ENC100118, \$20015ENC100118, \$20015END100118, \$20015PED100118 \$11530ENC100118, \$12030ENC100118, \$14030ENC100118, \$12030ENC100118)

\$18030ENC100118, \$20030ENC100118, \$20030END100118, \$20030PED100118

See appropriate Alpine gable detail for maximum unreinforced gable vertical

"T" Reinforcement Attachment Detail



To convert from "L" to "T" reinforcing members, multiply "T" increase by length (based on appropriate Alpine gable detail).

Maximum allowable "T" reinforced gable vertical length is 14' from top to bottom chord.

"T" reinforcing member material must match size, specie, and grade of the "L" reinforcing member.

Web Length Increase w/ "T" Brace

"T" Reinf.	"T"
Mbr. Size	Increase
2×4	30 %
2x6	20 %

Example:

ASCE 7-10 Wind Speed = 120 mph Mean Roof Height = 30 ft, Kzt = 1.00 Gable Vertical = 24°o.c. SP #3

"T" Reinforcing Member Size = 2x4

"T" Brace Increase (From Above) = 30% = 1.30 (1) 2x4 "L" Brace Length = 8' 7"

Maximum 'T' Reinforced Gable Vertical Length $1.30 \times 8' \ 7'' = 11' \ 2''$

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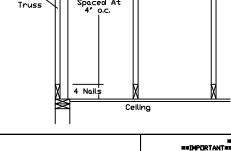
For more information see this Job's general notes page and these web stight/26/202178 Yoonhwak Kim, FL PE #86367 ALPINE: www.alpineitw.com, TPI: www.tpinstorg, SBCA: www.sbcindustry.org, ICC: www.lcessfer.org, 278 Yoonhwak Kim, FL PE #86367

REF LET-IN VERT DATE 01/02/2018 DRWG GBLLETIN0118

MAX. TOT. LD. 60 PSF

24.0"

DUR. FAC. ANY MAX. SPACING



4 Nails

Nails

Spaced At

Reinforcing Member

Gable

Rigid Sheathing

514 Earth City Expressway Suite 242 Earth City, MO 63045

Piggyback Detail - ASCE 7-16: 160 mph, 30' Mean Height, Enclosed, Exposure C, Kzt=1.00

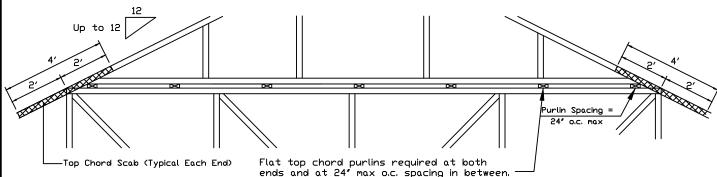
160 mph Wind, 30.00 ft Mean Hgt, ASCE 7-16, Enclosed Bldg. located anywhere in roof, Exp C, Wind DL= 5.0 psf (min), Kzt=1.0. Dr 140 mph wind, 30.00 ft Mean Hgt, ASCE 7-16, Enclosed Bldg. located anywhere in roof, Exp D, wind DL= 5.0 psf (min), Kzt=1.0.

Note: Top chords of trusses supporting piggyback cap trusses must be adequately braced by sheathing or purlins. The building Engineer of Record shall provide diagonal bracing or any other suitable anchorage to permanently restrain purlins, and lateral bracing for out of plane loads over gable ends.

Maximum truss spacing is 24' o.c. detail is not applicable if cap supports additional loads such as cupola, steeple, chimney or drag strut loads.

** Refer to Engineer's sealed truss design drawing for piggyback and base truss specifications.

Detail A: Purlin Spacing = 24" o.c. or less



Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4" o.c.

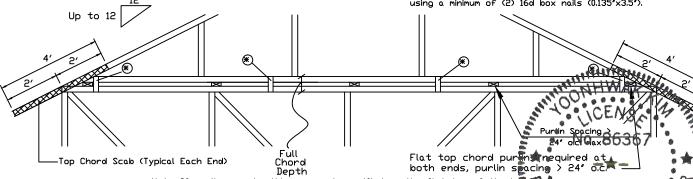
Attach purlin bracing to the flat top chord using (2) 16d box nails (0.135"x3.5").

The top chord #3 grade 2x4 scab may be replaced with either of the following: (1) 3X8 Trulox plate attached with (8) 0.120"x1.375" nalls, (4) into cap TC & (4) into base truss TC or (1) 28PB wave piggyback plate plated to the piggyback truss TC and attached to the base truss TC with (4) 0.120"x1.375" nails. Note: Nailing thru holes of wave plate is acceptable.

Detail B: Purlin Spacing > 24" o.c.

Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4" o.c.

Attach purlin bracing to the flat top chord using a minimum of (2) 16d box nails (0.135"x3.5").



Note: If purlins or sheathing are not specified on the flat top of the bose \mathfrak{I} truss, purlins must be installed at 24" o.c. max. and use Detail A.

* In addition, provide connection with one of the following methods:

Use 3X8 Trulox plates for 2x4 chord member, and 3X10 Trulox plates for 2x6 and larger chord members. Attach to each face @ 8' o.c. with (4) 0.120"x1.375" nails into cap bottom chord and (4) in base truss top chord. Trulox plates may be staggered 4' o.c. front to back faces.

APA Rated Gusset

8'x8'x7'16' (min) APA rated sheathing gussets (each face). Attach @ 8' o.c. with (8) 6d common (0.13'x2') nalls per gusset, (4) in cap bottom chord and (4) in base truss top chord. Gussets may be staggered 4' o.c. front to back faces.

2x4 Vertical Scabs

2x4 SPF #2, full chord depth scabs (each face). Attach @ 8' o.c. with (6) 10d box nails (0.128"x3") per scab, (3) in cap bottom chord and (3) in base truss top chord. Scabs may be staggered 4' o.c. front to back faces.

28PB Wave Piggyback Plate

Dine 28PB wave piggyback plate to each face 8 8' o.c. Attach teeth to piggyback at time of fabrication. Attach to supporting truss with (4) 0.120'x1.375' nails per face per ply.
Piggyback plates may be staggered 4' o.c. front to back faces.

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JANAS ONAL

PIGGYBACK DATE 01/02/2018

DRWG PB160160118

ISPACING 24.0"

13723 Riverport Drive Suite 200 Maryland Heights, MO 63043

AN ITW COMPANY

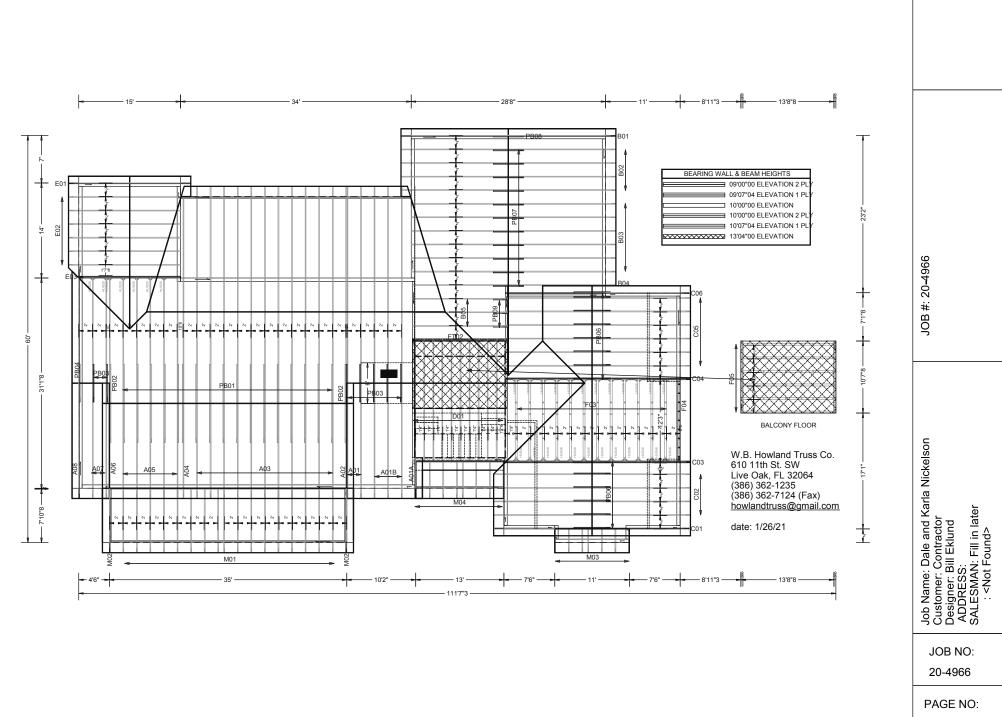
For more information see this Job's general notes page and these web sittle /26/2021. Yoonhwak Kim, FL PE #86367 ALPINE: www.alpineitw.com; TPI: www.tpinstorg; SBCA: www.sbcindustry.org; ICC: www.lccsafe.org #278.

STRONGBACK BRIDGING RECOMMENDATIONS ► All scab-on blocks shall be a minimum 2x4 BUTT STRONGBACK "stress graded lumber." BRIDGING TOGETHER ► All strongback bridging and bracing shall be a minimum 2x6 "stress graded lumber." The purpose of strongback bridging is to develop load sharing between individual trusses, ATTACH SCAB WITH 10d BOX/GUN (0.128"x3.0") NAILS IN 2 ROWS AT resulting in an overall increase in the 4' -0" 2×6 stiffness of the floor system. 2x6 NOTE: IN LIEU OF SPLICING AS SHOWN. LAP STRONGBACK BRIDGING MEMBERS strongback bridging, positioned as shown in FOR AT LEAST ONE TRUSS SPACING details, is recommended at 10' -0" o.c. (max.) STRONGBACK BRIDGING SPLICE DETAIL NDTE: Details 1 and 2 are the preferred attachment methods ▶ The terms "bridging" and "bracing" are sometimes mistakenly used interchangeably. "Bracing" is an ATTACH STRONGBACK TO WEB W/ (3) 10d COMMON important structural requirement of any floor (0.148"x3") NAILS DR or roof system. Refer to the Truss Design (4) 10d BDX/GUN (0.128"x3.0") NAILS Drawing (TDD) for the bracing requirements for each individual truss component. "Bridging," particularly "strongback bridging" is a recommendation for a truss system to help control vibration. In addition to aiding in the distribution of point loads between adjacent truss, strongback bridging serves to reduce "bounce" or residual vibration resulting from STRONGBACK BRIDGING moving point loads, such as footsteps. SPACING REQUIREMENTS Up to 10' None required 10' to 20'1 row (at center of span) The performance of all floor systems are 20' to 30'2 rows (1 at each 1/3 points) (2) 10d COMMON (0.148"x3.0") OR enhanced by the installation of strongback BDX/GUN (0.128"x3.0") NAILS AT 2x6 (MINIMUM) STRONGBACK. 30' to 40'3 rows (1 at each 1/4 points) bridging and therefore is strongly recommended TOP AND BOTTOM OF 2x4 RESTRAINED AT EACH END. Over 40' Space rows at 10' o.c. points) SCAB-ON BLOCK, ATTACH by Alpine. STRONGBACK TO BLOCK W/ (3) 10d COMMON (0.148"x3.0") NAILS For additional information regarding strongback DR (4)10d BDX/GUN (0.128*x3.0*) 3 bridging, refer to BCSI (Building Component Safety Information). ATTACH STREETERACK TO BOTTO CHORD WITH (2) #10 -31 SCBEWS STRONGBACK BRIDGING ATTACHMENT ALTERNATIVES ***VARNING*** READ AND FOLLOW ALL NOTES ON THIS DRAWING ***IMPORTANT*** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS. **PSF IREF** STRONGBACK Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Bullaing Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and botton chord shall have a properly attached rigid celling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. TC/ DL **PSF** DATE 10/01/14 ONAL ... BC DL DRWG STRBRIBR1014 BC LL **PSF** Alpine, a division of ITV Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, indicates acceptance of professional engineering responsibility of use of this drawing for any structure is the responsibility of the Building Besigner per ANSI/TPI 1 Sec.2. TOT. LD. **PSF** 514 Earth City Expressway DUR. FAC. 1.00 Suite 242

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SPACING

Earth City, MO 63045



1 OF 1