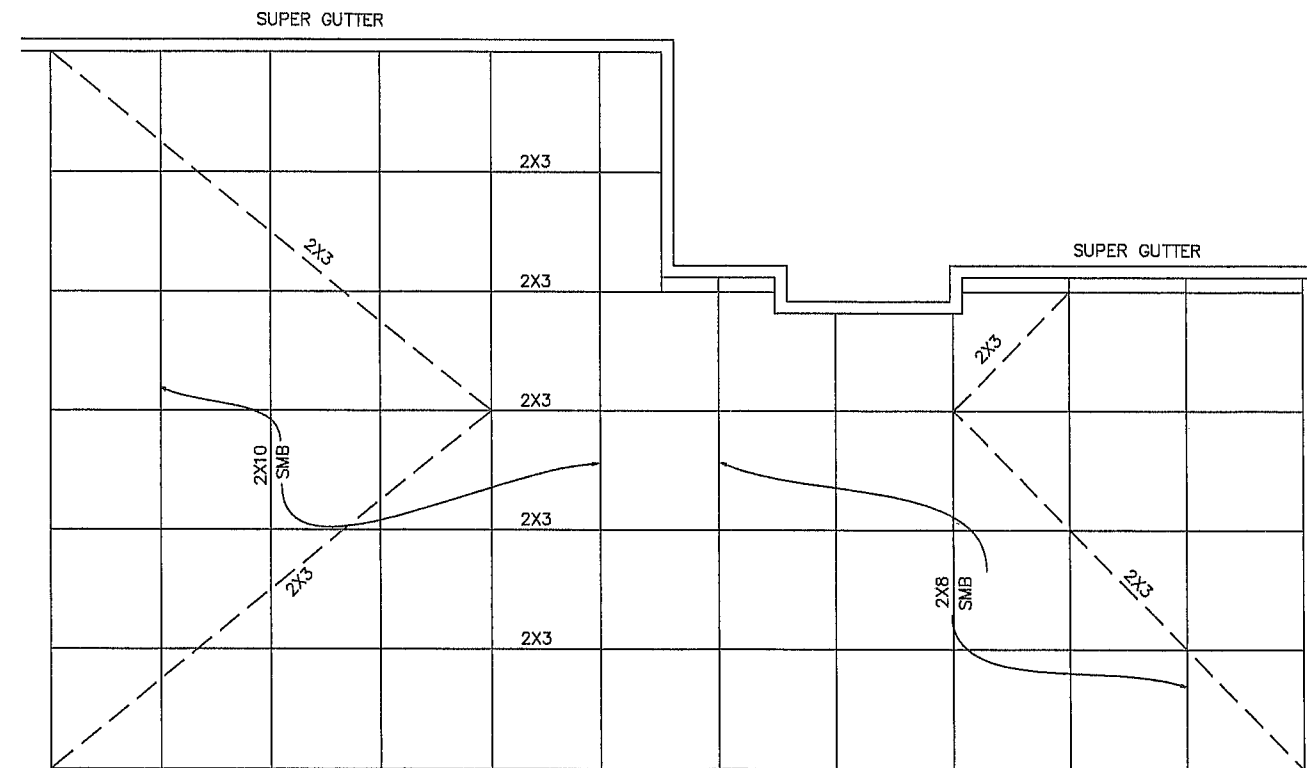
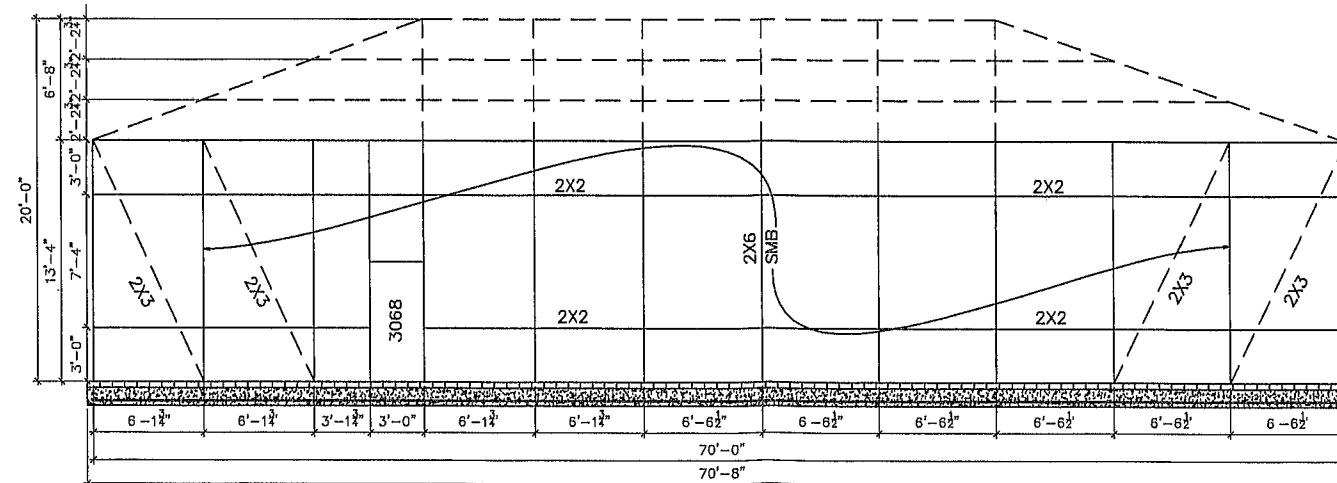


CARRIER BEAMS IN SCREEN ENCLOSURES:

- 1) Carrier Beams (Refer to schematic diagrams for limitations) must be horizontal and may not exceed a single span of 16 ft.
 - 2) At least one end of carrier beam must be supported by the host structure.
 - 4) Connect 2x8 SMB post/column at foundation using Details provided
- Concrete Cover: For foundations, minimum concrete over reinforcing bars shall be 3 inches in foundations where the concrete is cast against and permanently in contact with the earth. For No. 5 and smaller bars 1½" where concrete is formed and will be exposed to the earth or weather. Where concrete is not exposed to weather, the minimum concrete cover for reinforcing shall be 1½ inches regardless of bar size.



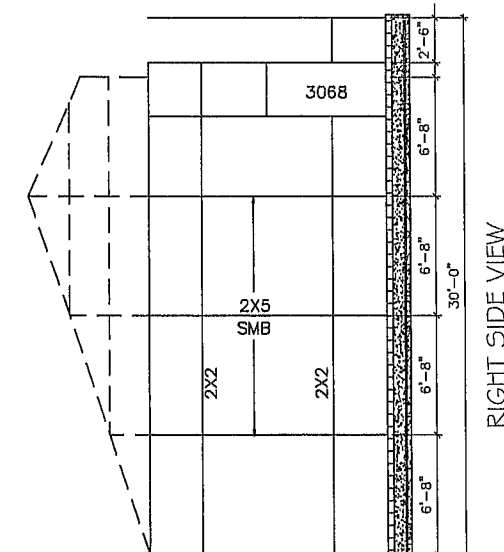
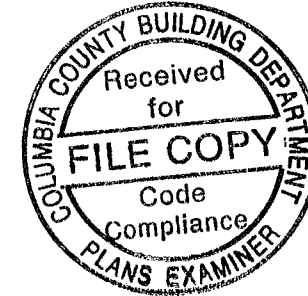
PLAN VIEW



FRONT VIEW

2X3 MEMBERS ARE .050
HOLLOW 6005T5 OR 6061T6
ALUMINUM

BRICK PAVERS OVER 12" X 12"
CONCRETE FOOTER WITH (2) #5
REBARS CONTINUOUS TED INTO
EXISTING FOOTERS/FOUNDAITON





RIGHT SIDE VIEW

REVISIONS	BY
2/21/2013	JFK

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The Pearce Residence
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Lake, City Florida 32055

	John F. Kennedy III, P.E. Reg. No. 322684	Date: 11/18/94
	DRAWN BY JFK MODEL SCREEN MASTER NO. MA-ALUM-1 SCALE N.T.S. FILE DATE 1-20-2012 DATE DRAWN 05-15-2013 SHEET	
<div style="text-align: center;">  </div>		
OF	6	SHEETS
FILE NO MA-65-2013		

GENERAL & STRUCTURAL NOTES

Miscellaneous Notes:

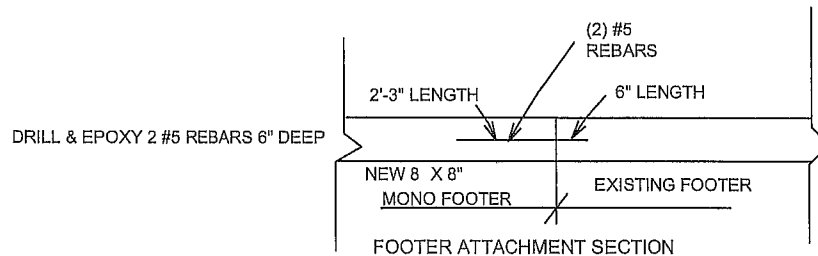
1. The contractor shall verify all conditions and dimensions at the job site prior to commencing work. The contractor shall report all discrepancies between the drawings and existing conditions to the designer prior to commencing work.
2. The contractor shall supply, locate and build into the work all inserts, anchors, angles, plates, openings, sleeves, hangers, slab depressions and pitches as may be required to attach and accommodate other work.
3. All details and sections shown on the drawings are intended to be typical and shall be constructed to apply to any similar situation elsewhere in the work except where a different detail is shown.
4. Subsurface soil condition information is not available. Foundations are designed for a soil bearing capacity of 2,000 PSF. The contractor shall report any differing conditions to the designer prior to commencing work.
5. Structural drawings shall be used in conjunction with Job Specification and Architectural, Mechanical, Electrical, Plumbing, and Site Drawings. Consult these Drawings for sleeves, depressions, and other details not shown on Structural Drawings.
6. All bolts, nuts, washers, straps, and fasteners including nails, shall be Hot-dip Galvanized or Stainless Steel. Continuous anchorage shall be provided between all trusses, wall sections, beams, posts, and footings with the use of straps and connectors as specified herein, except for interior framing.
7. All specified fasteners may be substituted with equivalent fasteners. The installation of the fasteners shall be in accordance with the manufacturer's specifications.
8. Wind bracing (1st and 2nd floors) with 18 gauge X 1-1/4" galvanized straps 10'-0" long placed diagonally, all bearing walls at corners or with 4'-0" X 10'-0" or 8'-0" X 5/8" exterior grade plywood siding.
9. The structure is designed to be self-supporting and stable after the building is complete. It is the contractor's sole responsibility to determine erection procedures and sequence to ensure safety of the building and its components during erection. This includes the addition of necessary shoring, sheeting, temporary bracing, guys or tie downs.

Concrete Notes:

1. Concrete shall have (3/4" Maximum Diameter Aggregate) a minimum compressive strength of 3,000 PSI at 28 days, unless noted otherwise. Placement shall be in accordance with ACI 318.
2. All concrete slabs on grade shall be the thickness indicated on the drawings, over minimum 6 Mil Polyethylene Vapor Barrier. Such slabs shall be reinforced with 6 X 6-W14 X W1.4 Welded Wire Fabric lapped 8" at edges and ends.
3. Fill under concrete slabs shall be clean sand or rock and free of debris and other deleterious material. Fill shall be compacted to a density of at least 95% of Standard Proctor Maximum Dry Density (ASTM D1557).
4. Footings shall bear upon undisturbed treated soil or upon soil compacted to at least 95% of Standard Proctor Maximum Dry Density (ASTM D1557) for a depth of at least three (3) feet below the bottom of the footing.
5. Reinforcing steel shall be ASTM A-615 Grade 60 Deformed Steel. All continuous vertical and horizontal reinforcing steel in footings, beams, and columns shall be lap spliced a minimum of 30 Bar Diameter or 12", whichever is greater.
6. All 90 degree hooks with side cover (perpendicular to plane of hook) not less than 2.5 inches, and cover on bar extension beyond hook (also called end cover) not less than 2 inches; shall be embedded in support member a minimum of 15.5 Bar Diameters and the hook shall be extended at least 12 Bar Diameters at the free end of the bar within the support member.
*All 90 degree hooks with side cover (perpendicular to plane of hook) less than 2.5 inches, and cover on bar extension beyond hook (also called end cover) less than 2 inches; shall be embedded in support member a minimum of 22 Bar Diameters, and the hook shall be extended at least 12 Bar Diameters at the free end of the bar within the support member.
7. Concrete cover and spacing of reinforcing steel shall be as follows:
 - A. Footings: 3" Bottoms and Sides, 2" Top
 - B. Beams: 1-1/2" Bottom, Sides and Top
 - C. Columns: 1-1/2"
 - D. Slabs on Grade: 2" Bottom, 1" Top
 - E. Rebar C to C: 1 Bar Diameter or 1", whichever is greater
(Center to Center spacing of rebar within concrete)
 - F. Others: Per ACI
8. Welded wire fabric shall conform to ASTM A-185, free from oil, scale, and rust and placed in accordance with the typical place details of ACI Standards and Specifications. Minimum lap shall be one space plus two (2) inches.

Concrete Mono Footer Notes:

1. These notes and specifications apply to all concrete mono footers unless noted otherwise.
2. See Concrete Notes for the concrete, and steel strengths and specifications.
3. All reinforcement shall have a minimum of 3" cover on all sides.
4. All horizontal reinforcement shall be continuous.
5. All horizontal reinforcement bars shall be evenly spaced horizontally within the footer minus the 3" cover on each side.
6. All horizontal reinforcement bars shall have a minimum horizontal spacing within the footer of (1) horizontal bar diameter or 1" whichever is greater.
7. All horizontal reinforcement bars shall be located at the depth specified in plan.
8. All vertical reinforcement that terminates within the footer shall have (1) 90-degree hook of equal bar size as the vertical from above for each vertical reinforcing bar.
9. All 90-degree hooks shall either be bent into the end of the vertical steel or lap spliced to the vertical steel.
10. All lap splices shall be a minimum of 30 (18.75" for #5 bar) bar diameters in concrete and 48 (30" for #5 bar) bar diameters in masonry or 12" whichever is greater.
11. All 90 degree hooks shall have a minimum of a 12 bar diameters (8" for #5 bar) extension.
12. All hook extensions shall be located in the horizontal plane at 3" plus 1/2" bar diameter from the center of the extension to the bottom edge of the concrete footer to the centerline of the reinforcement.
13. All hook extensions shall be run parallel to the horizontal steel in the footer.
14. When specified by footing detail, slabs under vertical reinforced masonry walls shall have a #4 bar with a 90 degree hook, extending 30" into slab @ a depth of 2" below slab surface & extending 6" vertically down the mono footer @ 3" clear distance from exterior edge of mono footer or @ the center line of an interior footing. Provide these hooks @ every reinforced vertical cell extending up to the tie beam (except where noted otherwise) and within wall opening @ 48" o.c. from vertical at each end of wall opening.
15. When specified by footing detail slabs under bearing frame walls shall have a #4 bar with a 90 degree hook extending 30" into slab @ a depth of 2" below slab surface, extending 6" vertically down the mono footer @ 3" clear distance from exterior edge of mono footer or @ the center line of an interior footing. Provide these hooks @ 48" o.c. under frame, @ every post location, and within wall opening @ 48" o.c. from post hook at each end of wall opening.



DRILL & EPOXY NEW FOOTERS INTO EXISTING FOOTERS W (2) #5 REBARS 6" DEEP INTO EXISTING FOOTER AND 2'-3" LENGTH AT NEW FOOTER FOR SPLICING. 2'-9" TOTAL REBAR LENGTH EPOXY REBARS USING SIMPSON STRONG TIE EPOXY FOR ALL NEW CONNECTIONS TYPICAL. TYPICAL ALL NEW CONC MONO FOOTER TO EXISTING MONO CONC FOOTER CONNECTION. ATTCH NEW 4" CONC SLAB TO EXISTING WITH (1) #2 OR #3 REBAR EPOXY 6" DEEP X 12" LONG @ 24" ON CENTERS.

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2/21/2013	JFK

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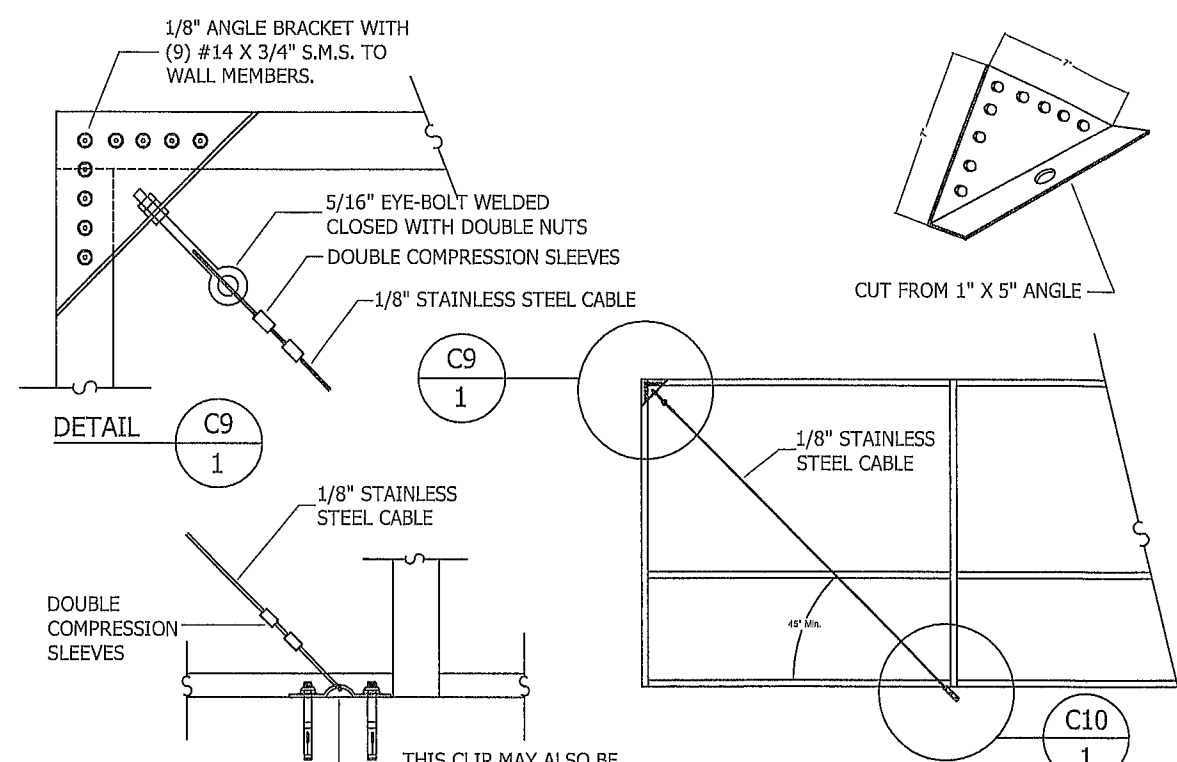
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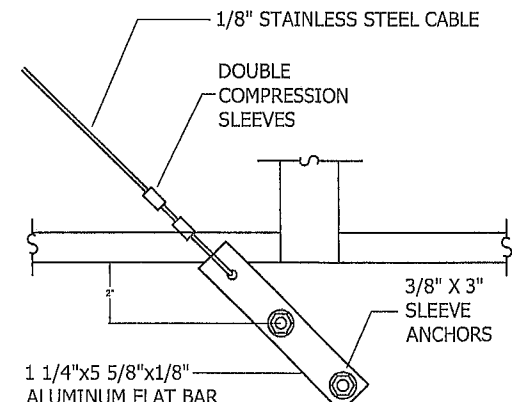
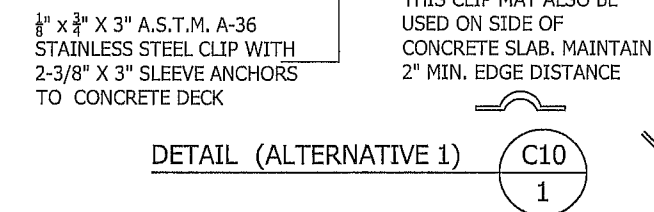
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Reg. No. 52684
Date: 1/18/14

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DATE DRAWN	05-15-2013
SHEET	6

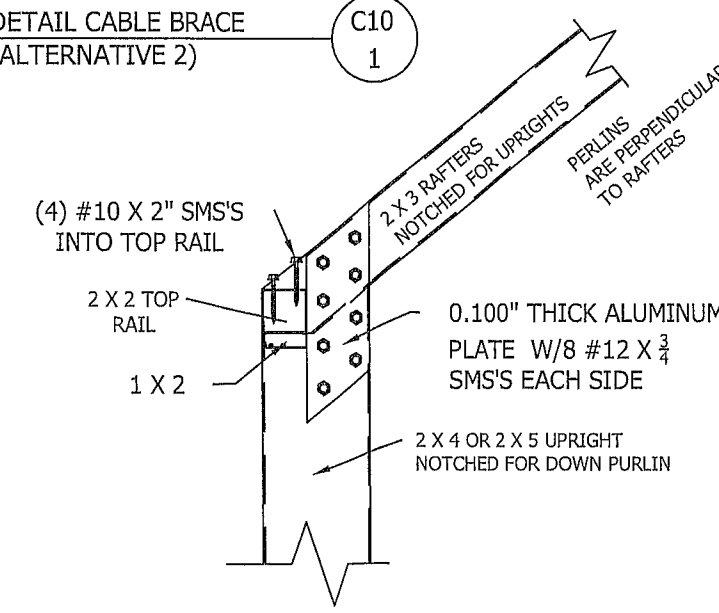
52
OF 6 SHEETS
FILE NO. MA-65-2013



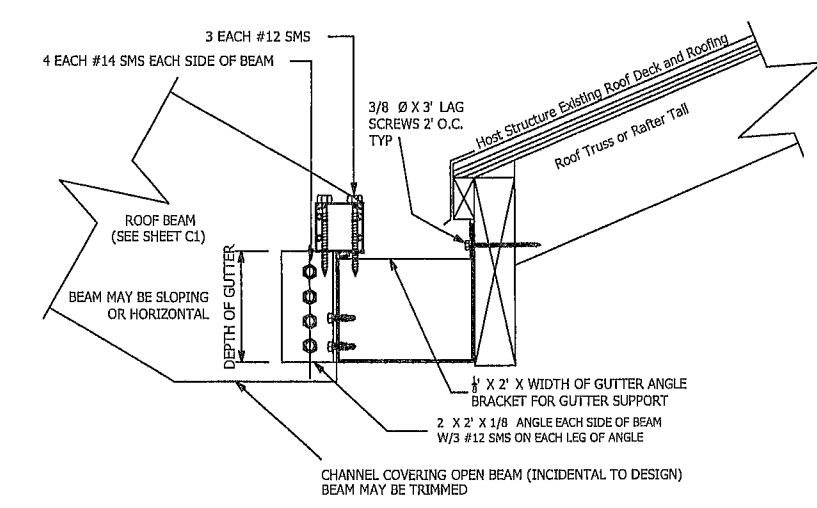
TYPICAL WALL ELEVATION



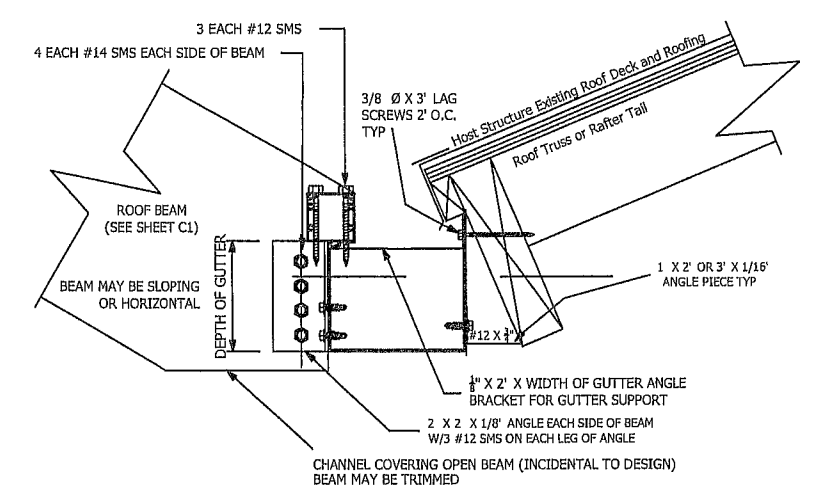
DETAIL CABLE BRACE (ALTERNATIVE 2)



TYPICAL RAFTER TO WALL CONNECTION



TYPICAL BEAM TO GUTTER TO FASCIA CONNECTION



TYPICAL BEAM TO GUTTER TO FASCIA CONNECTION
ANGLED FASCIA

TYPICAL CABLE BRACE DETAIL, USE THIS DETAIL OR
TYPICAL WALL BRACING CONNECTION DETAIL ON D4

**EACH WALL NOT LATERALLY SUPPORTED
BY HOST STRUCTURE MUST BE CABLED -
TABULAR VALUES REPRESENT WALLS
PERPENDICULAR TO CABLED WALL FOR
NUMBER OF CABLES REFER TO TABLE 12

TABLE 12 BEAM ALLOWABLE AREAS OF WALL (WIND) EXPOSURE BY ZONE FOR WALL BRACING							
CABLE PAIRS	PRESSURE = >	15.9 PSF	19.2 PSF	22.8 PSF	26.8 PSF	31.1 PSF	35.7 PSF
	WIND ZONE = >	100	110	120	130	140	150
1	TABULAR VALUES IN SQ. FT.	181	150	128	107	92	81
2		337	279	235	200	172	150
3		448	371	312	266	229	199
36" LONG SOLID PANEL (PAIRS)							
1		377	313	263	224	193	168
2		698	578	487	414	357	311

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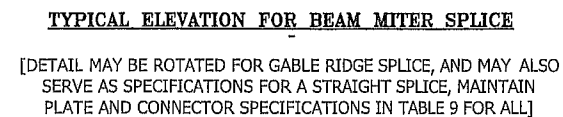
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MA-1-2012



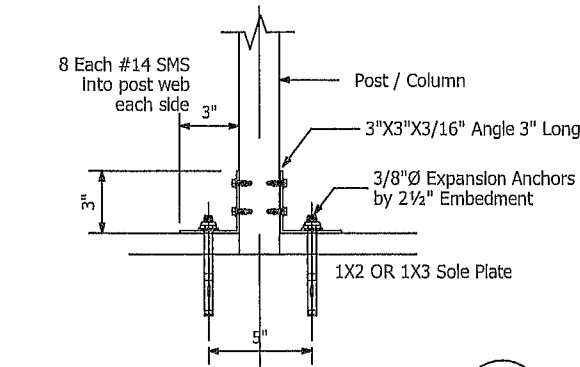
Table 9 Notes:
1 6061T6 beams use same plates as 6005T5 beams
2 5052H32 is a sheet alloy, 6061T6 is a plate alloy
3 Ma (allowable moments for beams in in-kips)



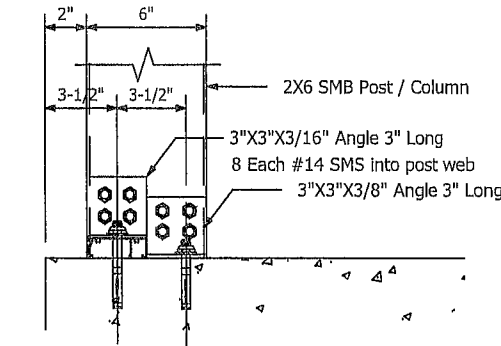
SUBSTITUTE ANCHORS

TAPCON ULTIMATE HOLDING VALUES IN BLOCK BLUE AND WHITE CLIMASEAL TAPCON®					
ANCHOR DIA. (IN)	EMBEDMENT DEPTH (IN)	LIGHT WEIGHT BLOCK		MEDIUM WEIGHT BLOCK	
		TENSION (LBS)	SHEAR (LBS)	TENSION (LBS)	SHEAR (LBS)
3/16	1	220	400	340	730
1/4	1	250	620	500	1000

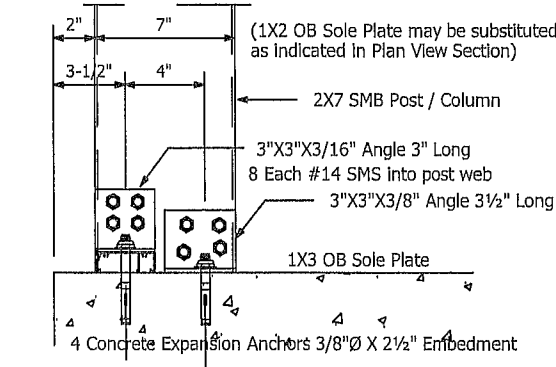
TAPCON ULTIMATE HOLDING VALUES IN NORMAL WEIGHT CONCRETE BLUE AND WHITE CLIMASEAL TAPCON®					
ANCHOR DIA. (IN)	EMBEDMENT DEPTH (IN)	2000 PSI CONCRETE		4000 PSI CONCRETE	
		TENSION (LBS)	SHEAR (LBS)	TENSION (LBS)	SHEAR (LBS)
3/16	1	600	720	650	720
	1-1/2	1090	860	1090	860
	1-3/4	1450	870	1460	890
1/4	1	750	900	800	1360
	1-1/2	1380	1200	1820	1380
	1-3/4	2020	1670	2380	1670



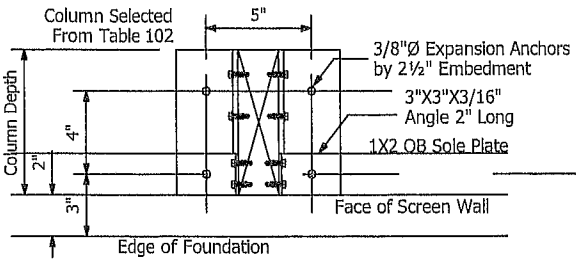
Wall Face Partial Elevation
Typical C2 Post/Column at Foundation
Minimum Column Size for Detail C2
(support of K-Bracing) 2X6 SMB



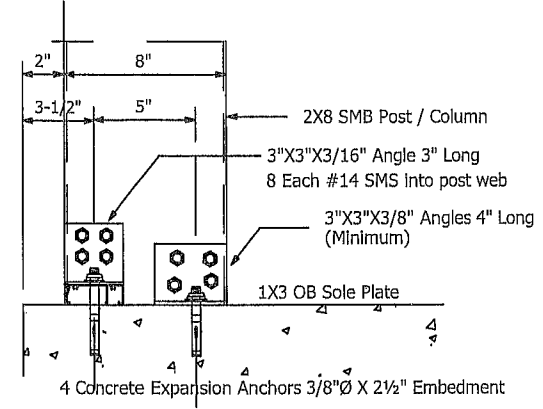
Section / Side Elevation / 2X6 SMB Post
(Locations supporting K-Bracing)



Section / Side Elevation / 2X7 SMB Post
(Locations supporting K-Bracing)



Plan View Section (w/ 1X2 Sole Plate)
Typical C2 Post/Column at Foundation
Minimum Column Size for Detail C2
(support of K-Bracing) 2X6 SMB



Section / Side Elevation / 2X8 SMB Post
(Locations supporting K-Bracing)
2X8 SMB Connection Detail C2 also applies to 2X9 and 2X10
(maintain required component sizes and fastener spacings)

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[Signature]
John F. Vilduff III, P.E.
Reg. No. 11181M
Date: 1/18/14

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TABLE 6 K-BRACE LEGEND	
KEY	EXTRUSION
A	2 X 2 X 0.046 HOL
B	2 X 3 X 0.050
C	2 X 2 X 0.090
D	2 X 3 X 0.072
E	3 X 3 X 0.072
F	3 X 3 X 0.090

TABLE 7 MINIMUM LENGTH OF K-BRACING AREA FOR ONE END OF LONGITUDINAL WALL																
ENCLOSURE PROTECTION	MAXIMUM TOTAL HEIGHT OF ENCLOSURE BY WIND ZONE															
	110				120				130				140			
20	12	15	18	12	15	18	12	15	18	12	15	18	12	15	18	12
25	5	6	7	5	6	7	5	6	7	5	6	7	5	6	7	5
30	5	6	7	5	6	7	5	6	7	5	6	7	5	6	7	5
35	5	6	7	5	6	7	5	6	7	5	6	7	5	6	7	5
40	5	6	7	5	6	7	5	6	7	5	6	7	5	6	7	5

K-BRACING REQUIREMENTS ON LONGITUDINAL WALL(S):

- 1) QUALITY K-BRACING SHALL BE PROVIDED AT BOTH ENDS OF THE LONGITUDINAL WALL UNLESS BRACED BY HOST STRUCTURE AS PROVIDED IN SCHEMATICS
- 2) LOOKUP ENCLOSURE HEIGHT ENCLOSURE PROJECTION AND LONGITUDINAL PANEL WIDTH IN TABLE 8 TO FIND DIAGONAL EXTRUSION TYPE AND END FASTENING (EXTRUSION KEY TABLE 106) ALL SCREWS #14 SMS EACH SIDE EACH END.
- 3) TABLE 7 INDICATES MINIMUM LENGTH OF K-BRACING AREA FOR ONE END
- 4) TABLE 7 DOES NOT DICTATE POST SPACING, I.E. LENGTH OF K-BRACING PROVIDED BY DESIGN MAY EXCEED TABULAR VALUE BUT MAY NOT BE LESS THAN TABULAR VALUE.
- 5) ORDER OF PROFILES IN TABLE 8 INDICATE RELATIVE STRENGTH FROM WEAKEST (A) TO STRONGEST (F) A STRONGER MEMBER THAN REQUIRED MAY BE SUBSTITUTED.
- 6) INTEGER AFTER SLANT SIGN IN TABLE 108 INDICATES NUMBER OF #14 SMS PER SIDE PER END OF DIAGONAL.
- 7) LONGITUDINAL GABLE ENDS REQUIRE ADDITIONAL BRACING, A MINIMUM OF 2 BAYS EACH AND BAY NEARER CENTER (HIGHEST) MUST BE BRACED TO TOP OF COLUMN (REFER OR ISOMETRIC SCHEMATICS AT BEGINNING OF CHAPTER)

K-BRACING REQUIREMENTS FOR SIDE WALLS:

- 1) WHEN SIDE WALL IS 3 BAYS PROVIDE K-BRACING IN CORNER (1st) BAY
- 2) FOR 1 OR 2 BAY SIDE WALLS K-BRACING OPTIONAL.
- 3) FOR SIDE WALLS OF 4 BAYS OR MORE, PROVIDE ONE BAY K-BRACING IN 2nd BAY

TABLE 8 REQUIRED K-BRACE DIAGONAL EXTRUSION 120 mph ZONE						
AND NUMBER OF #14 SMS / SIDE / END						
PANEL WIDTH	ENCLOSURE HEIGHT	ENCLOSURE PROJECTION FROM HOST				
		20	25	30	35	40
5'	UP TO 12'	A/3	B/3	B/3	C/2	D/3
6'	UP TO 12'	B/2	B/3	C/2	D/3	E/3
7'	UP TO 12'	B/2	C/2	C/2	E/3	E/3
8'	UP TO 12'	B/2	C/2	E/2	E/3	E/3
5'	12' AND OVER	E/3	E/3	E/4	E/4	E/4
6'	12' AND OVER	E/3	E/3	E/3	E/4	E/4
7'	12' AND OVER	E/2	E/3	E/3	E/4	F/4
8'	12' AND OVER	E/2	E/3	E/3	E/3	F/4

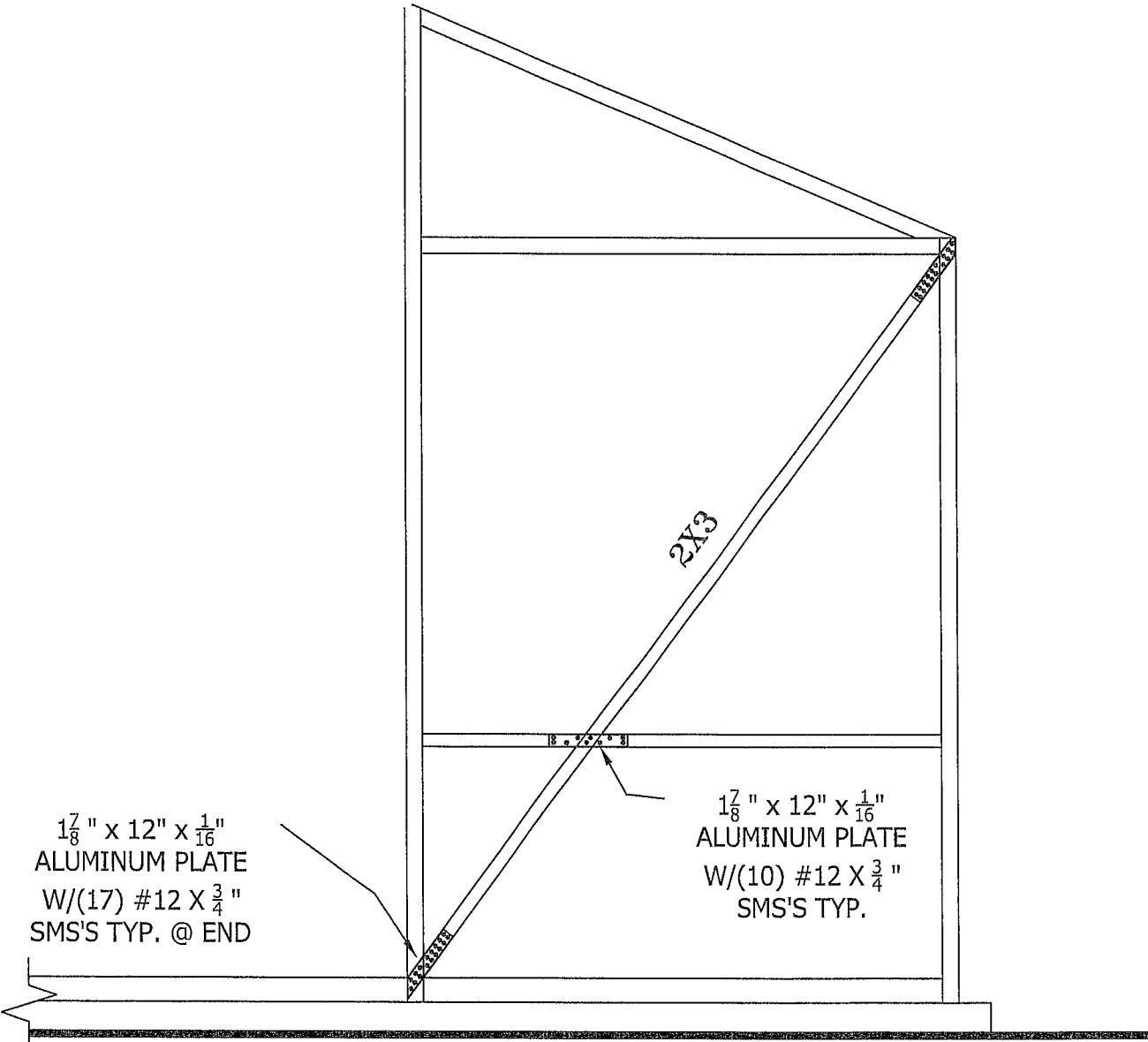
EXTRUSION KEY IN TABLE 6 BY LETTER, X=SITE SPECIFIC

TABLE 8 REQUIRED K-BRACE DIAGONAL EXTRUSION 130 mph ZONE						
AND NUMBER OF #14 SMS / SIDE / END						
PANEL WIDTH	ENCLOSURE HEIGHT	ENCLOSURE PROJECTION FROM HOST				
		20	25	30	35	40
5'	UP TO 12'	E/3	B/3	C/2	D/3	E/4
6'	UP TO 12'	B/3	C/2	D/3	E/3	E/4
7'	UP TO 12'	C/2	C/2	E/3	E/3	E/3
8'	UP TO 12'	C/2	E/2	E/3	E/3	E/3
5'	12' AND OVER	E/3	E/4	E/4	F/4	F/X
6'	12' AND OVER	E/3	E/3	E/4	F/4	F/4
7'	12' AND OVER	E/3	E/3	E/4	F/3	F/4
8'	12' AND OVER	E/3	E/3	E/3	F/3	F/X

EXTRUSION KEY IN TABLE 6 BY LETTER, X=SITE SPECIFIC

TABLE 8 REQUIRED K-BRACE DIAGONAL EXTRUSION 140 mph ZONE						
AND NUMBER OF #14 SMS / SIDE / END						
PANEL WIDTH	ENCLOSURE HEIGHT	ENCLOSURE PROJECTION FROM HOST				
		20	25	30	35	40
5'	UP TO 12'	B/3	C/2	D/3	E/4	E/4
6'	UP TO 12'	C/2	D/3	E/3	E/4	E/4
7'	UP TO 12'	C/2	E/3	E/3	E/3	E/4
8'	UP TO 12'	E/2	E/3	E/3	E/3	E/4
5'	12' AND OVER	E/4	E/4	F/4	F/X	F/X
6'	12' AND OVER	E/3	E/4	F/4	F/4	F/X
7'	12' AND OVER	E/3	E/4	F/3	F/X	F/X
8'	12' AND OVER	E/3	E/3	F/3	F/X	F/X

EXTRUSION KEY IN TABLE 6 BY LETTER, X=SITE SPECIFIC



TYPICAL WALL BRACING CONNECTION DETAIL
USE THIS DETAIL AND/OR CABLE BRACE DETAIL ON D1

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STRUCTURAL DETAILS
FLORIDA BUILDING CODE 2010

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