



RE: 3458616 - NORRIS CONST. - HERITAGE

MiTek USA, Inc.

16023 Swingley Ridge Rd

Site Information:

Customer Info: JOHN NORRIS CONST. Project Name: Spec Hse Model: Custom

Chesterfield, MO 63017

Lot/Block: N/A

City: Columbia Cty

Subdivision: HERITAGE

Address: 331 SW Legacy Glen, N/A

State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name: License #:

Address:

City: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special **Loading Conditions):** 

Design Code: FBC2020/TPI2014 Design Program: MiTek 20/20 8.5

Wind Code: ASCE 7-16 Wind Speed: 130 mph Roof Load: 37.0 psf Floor Load: N/A psf

This package includes 37 individual, Truss Design Drawings and 0 Additional Drawings. With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	T30144155	CJ01	3/27/23	15	T30144169	T06	3/27/23
	T30144156	CJ03	3/27/23	16	T30144170	T07	3/27/23
3	T30144157	CJ05	3/27/23	17	T30144171	T08	3/27/23
4	T30144158	EJ01	3/27/23	18	T30144172	T08A	3/27/23
5	T30144159	EJ02	3/27/23	19	T30144173	T09	3/27/23
6	T30144160	HJ09	3/27/23	20	T30144174	T10	3/27/23
7	T30144161	HJ10	3/27/23	21	T30144175	T11	3/27/23
8	T30144162	T01	3/27/23	22	T30144176	T12	3/27/23
9	T30144163	T01G	3/27/23	23	T30144177	T13	3/27/23
10	T30144164	T02	3/27/23	24	T30144178	T14	3/27/23
11	T30144165	T02G	3/27/23	25	T30144179	T15	3/27/23
12	T30144166	T03	3/27/23	26	T30144180	T16	3/27/23
13	T30144167	T04	3/27/23	27	T30144181	†17	3/27/23
14	T30144168	T05	3/27/23	28	T30144182	T18	3/27/23
14	130144100	103	3/21/23	20	130144102	110	3/21/23



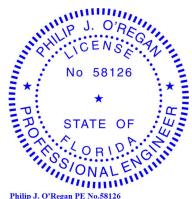
This item has been electronically signed and sealed by ORegan, Philip, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies

The truss drawing(s) referenced above have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Builders FirstSource-Lake City, FL.

Truss Design Engineer's Name: ORegan, Philip

My license renewal date for the state of Florida is February 28, 2025.

**IMPORTANT NOTE:** The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

March 27,2023



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# **Site Information:**

Customer Info: JOHN NORRIS CONST.

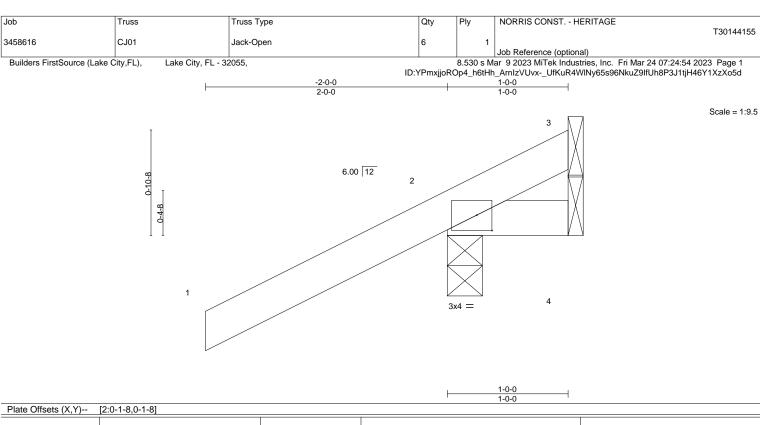
Lot/Block: N/A Address: 331 SW Legacy Glen, N/A

City: Columbia Cty

Project Name: Spec Hse Model: Custom Subdivision: HERITAGE

State: FL

No.	Seal#	Truss Name	Date
29	T30144183	T19	3/27/23
30	T30144184	T20	3/27/23
31	T30144185	T21	3/27/23
32	T30144186	<u>T22</u>	3/27/23
33	T30144187	T23	3/27/23
34	T30144188	T24	3/27/23
35	T30144189	T25	3/27/23
36	T30144190	T26 T26G	3/27/23
37	T30144191	1200	3/27/23



LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	I/defI	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.25	Vert(LL)	0.00	7	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.05	Vert(CT)	0.00	7	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	2	n/a	n/a		
BCDL	10.0	Code FBC2020/TI	PI2014	Matri	x-MP						Weight: 7 lb	FT = 20%

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

**BRACING-**

TOP CHORD **BOT CHORD**  Structural wood sheathing directly applied or 1-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 3=Mechanical, 2=0-3-8, 4=Mechanical

Max Horz 2=46(LC 12)

Max Uplift 3=-27(LC 1), 2=-102(LC 12), 4=-46(LC 1) Max Grav 3=16(LC 16), 2=254(LC 1), 4=29(LC 16)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

# NOTES-

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 27 lb uplift at joint 3, 102 lb uplift at joint 2 and 46 lb uplift at joint 4.

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Philip J. O'Regan PE No.58126 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

March 27,2023

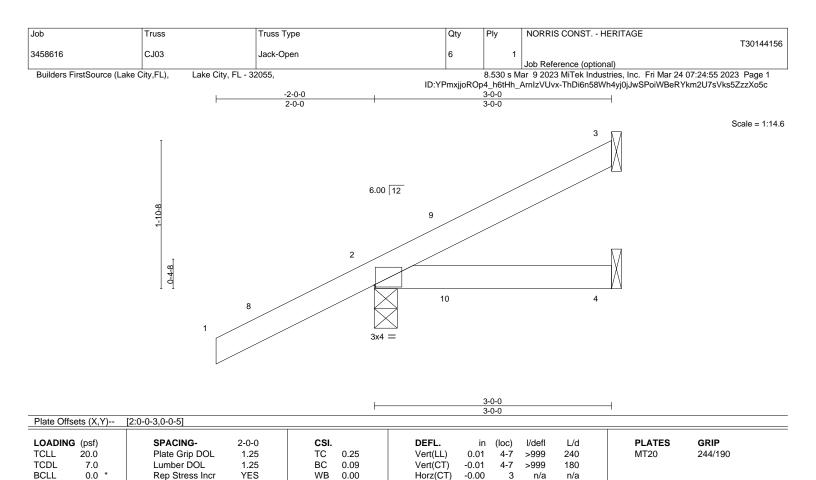
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





BCDL

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

10.0

BRACING-

TOP CHORD **BOT CHORD**  Structural wood sheathing directly applied or 3-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

Weight: 13 lb

FT = 20%

n/a

n/a

REACTIONS. (size) 3=Mechanical, 2=0-3-8, 4=Mechanical

Max Horz 2=80(LC 12)

Max Uplift 3=-31(LC 12), 2=-76(LC 12), 4=-14(LC 9) Max Grav 3=52(LC 1), 2=253(LC 1), 4=48(LC 3)

Code FBC2020/TPI2014

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

# NOTES-

1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -2-0-0 to 1-0-0, Interior(1) 1-0-0 to 2-11-4 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

Matrix-MP

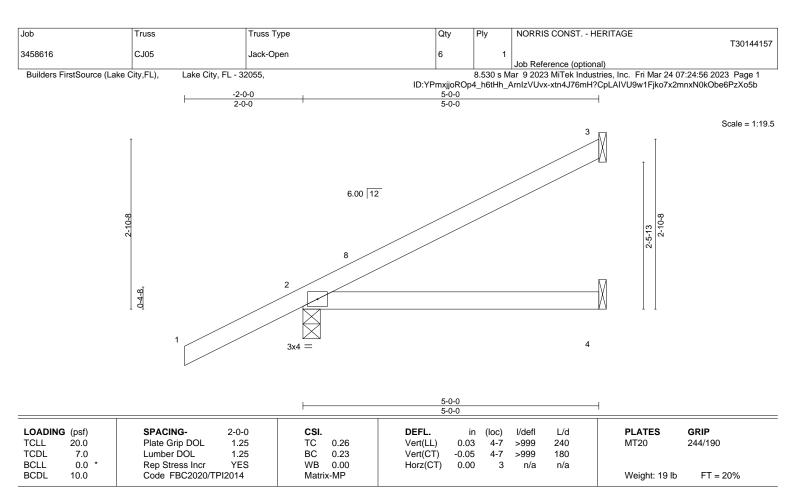
- 2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 31 lb uplift at joint 3, 76 lb uplift at joint 2 and 14 lb uplift at joint 4.

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March 27,2023





TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x4 SP No.2 BRACING-

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 3=Mechanical, 2=0-3-8, 4=Mechanical

Max Horz 2=114(LC 12)

Max Uplift 3=-64(LC 12), 2=-80(LC 12)

Max Grav 3=108(LC 1), 2=313(LC 1), 4=87(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -2-0-0 to 1-0-0, Interior(1) 1-0-0 to 4-11-4 zone; porch right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 64 lb uplift at joint 3 and 80 lb uplift at ioint 2.

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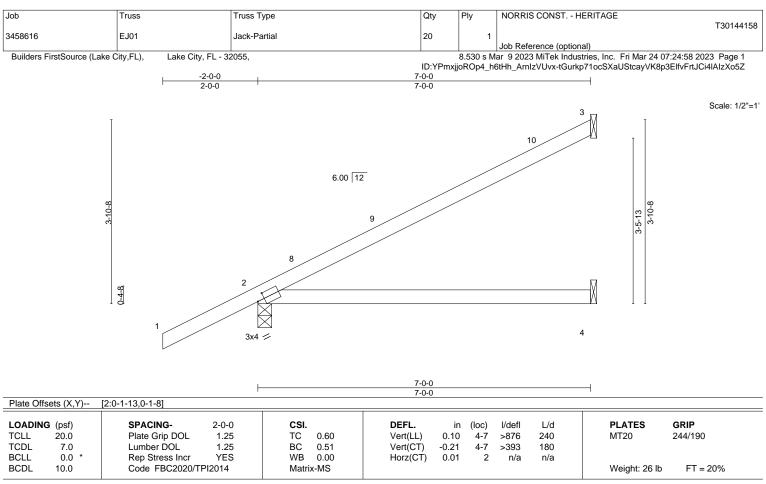


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TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 BRACING-

TOP CHORD **BOT CHORD**  Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 3=Mechanical, 2=0-3-8, 4=Mechanical

Max Horz 2=144(LC 12)

Max Uplift 3=-84(LC 12), 2=-90(LC 12)

Max Grav 3=160(LC 1), 2=380(LC 1), 4=125(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

# NOTES-

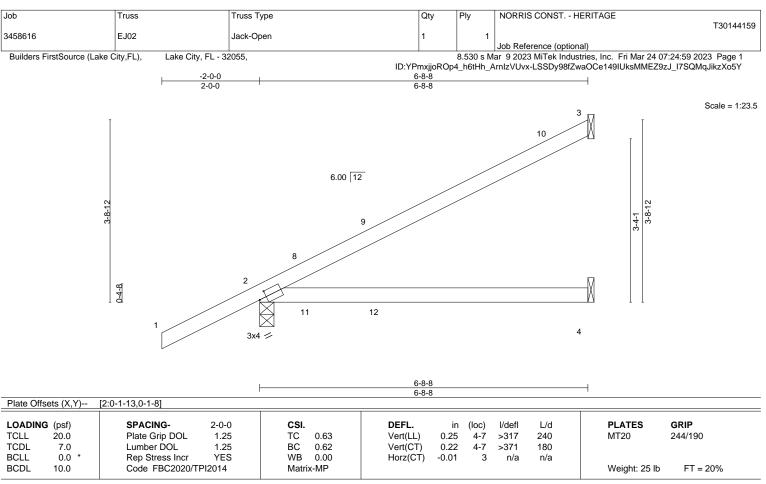
- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -2-0-0 to 1-0-0, Interior(1) 1-0-0 to 6-11-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 84 lb uplift at joint 3 and 90 lb uplift at ioint 2.

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March 27,2023





TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 BRACING-

TOP CHORD **BOT CHORD**  Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 3=Mechanical, 2=0-3-8, 4=Mechanical

Max Horz 2=140(LC 12)

Max Uplift 3=-83(LC 12), 2=-89(LC 12), 4=-38(LC 9) Max Grav 3=153(LC 1), 2=370(LC 1), 4=119(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

# NOTES-

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -2-0-0 to 1-0-0, Interior(1) 1-0-0 to 6-7-12 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 83 lb uplift at joint 3, 89 lb uplift at joint 2 and 38 lb uplift at joint 4.

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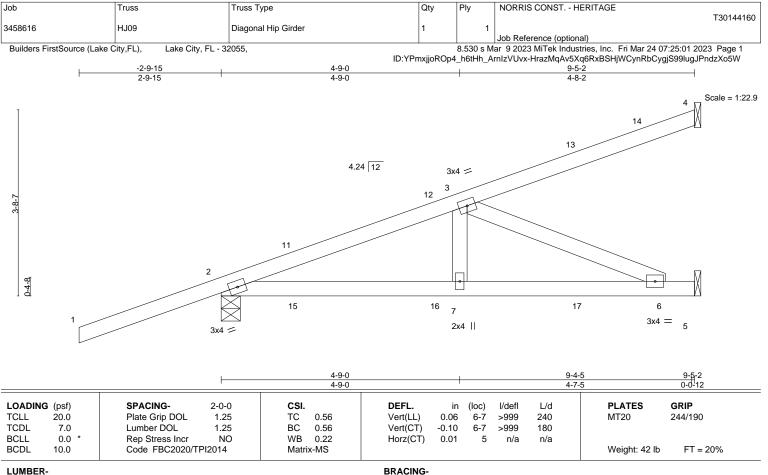
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16023 Swingley Ridge Rd Chesterfield, MO 63017



TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No.3 WFBS

(size) 4=Mechanical, 2=0-4-9, 5=Mechanical

Max Horz 2=157(LC 4)

Max Uplift 4=-78(LC 4), 2=-214(LC 4), 5=-111(LC 5) Max Grav 4=138(LC 1), 2=446(LC 1), 5=259(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-547/213

**BOT CHORD** 2-7=-252/490 6-7=-252/490

WFBS 3-6=-533/274

# NOTES-

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 78 lb uplift at joint 4, 214 lb uplift at joint 2 and 111 lb uplift at joint 5.
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 54 lb down and 103 lb up at 1-6-1, 54 lb down and 103 lb up at 1-6-1, 20 lb down and 33 lb up at 4-4-0, 20 lb down and 33 lb up at 4-4-0, and 41 lb down and 75 lb up at 7-1-15, and 41 lb down and 75 lb up at 7-1-15 on top chord, and 44 lb down and 74 lb up at 1-6-1, 44 lb down and 74 lb up at 1-6-1, 42 lb down and 21 lb up at 4-4-0, 42 lb down and 21 lb up at 4-4-0, and 65 lb down at 7-1-15, and 65 lb down at 7-1-15 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

# LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=-54, 5-8=-20

Concentrated Loads (lb)

Vert: 11=50(F=25, B=25) 13=-64(F=-32, B=-32) 15=70(F=35, B=35) 16=5(F=2, B=2) 17=-49(F=-24, B=-24)

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Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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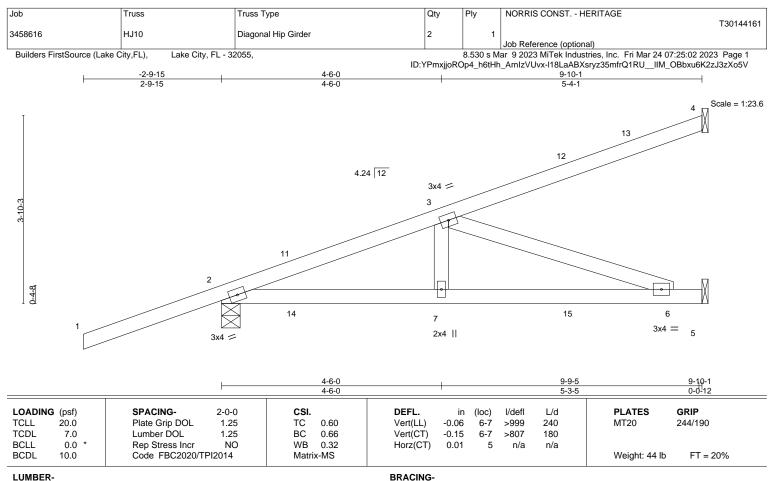
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TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WFBS

2x4 SP No.3

(size) 4=Mechanical, 2=0-4-9, 5=Mechanical Max Horz 2=160(LC 4) Max Uplift 4=-79(LC 4), 2=-200(LC 4), 5=-63(LC 8) Max Grav 4=152(LC 1), 2=463(LC 1), 5=264(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-631/216

**BOT CHORD** 2-7=-250/575 6-7=-250/575 WFBS 3-7=-4/258, 3-6=-609/265

# NOTES-

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 79 lb uplift at joint 4, 200 lb uplift at joint 2 and 63 lb uplift at joint 5.
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 54 lb down and 103 lb up at 1-6-1, 54 lb down and 103 lb up at 1-6-1, 20 lb down and 33 lb up at 4-4-0, 20 lb down and 33 lb up at 4-4-0, and 41 lb down and 75 lb up at 7-1-15, and 41 lb down and 75 lb up at 7-1-15 on top chord, and 21 lb down and 74 lb up at 1-6-1, 21 lb down and 74 lb up at 1-6-1, 19 lb down and 21 lb up at 4-4-0, 19 lb down and 21 lb up at 4-4-0, and 42 lb down at 7-1-15, and 42 lb down at 7-1-15 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

# LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-4=-54, 5-8=-20

Concentrated Loads (lb)

Vert: 7=5(F=2, B=2) 11=50(F=25, B=25) 12=-64(F=-32, B=-32) 14=70(F=35, B=35) 15=-49(F=-24, B=-24)

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Philip J. O'Regan PE No.58126 MITek Inc. DBA MITek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

March 27,2023

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

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Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

16023 Swingley Ridge Rd

Job Truss Truss Type Qty Ply NORRIS CONST. - HERITAGE T30144162 3458616 T01 Common Job Reference (optional) Builders FirstSource (Lake City,FL), Lake City, FL - 32055, 8.530 s Mar 9 2023 MiTek Industries, Inc. Fri Mar 24 07:25:04 2023 Page 1 ID:YPmxjjoROp4\_h6tHh\_ArnIzVUvx-iPG6?sCnOSDhIPv1yr3vZP38UAfVfWZBaeX3OyzXo5T 24-7-8 12-3-12 6-3-12 6-0-0 2-0-0 Scale = 1:46.0 4x6 =6.00 12 4 19 18 5x6 / 5x6 < 5 3 20 10 9 21 8 3x4 = 3x6 = 3x4 = 3x6 =3x6 =

8-4-7 Plate Offsets (X,Y)--[3:0-3-0,0-3-0], [5:0-3-0,0-3-0], [6:0-2-15,Edge]

LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.43	Vert(LL)	-0.16	8-10	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	ВС	0.75	Vert(CT)	-0.24	8-10	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.24	Horz(CT)	0.05	6	n/a	n/a		
BCDL	10.0	Code FBC2020/T	PI2014	Matri	ix-MS						Weight: 117 lb	FT = 20%

16-3-1

7-10-9

**BRACING-**

TOP CHORD

**BOT CHORD** 

LUMBER-

TOP CHORD 2x4 SP No 2 BOT CHORD 2x4 SP No 2 2x4 SP No.3 WFBS

REACTIONS. (size) 2=0-3-8, 6=0-3-8

Max Horz 2=-106(LC 13)

Max Uplift 2=-233(LC 12), 6=-233(LC 13) Max Grav 2=1084(LC 2), 6=1084(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1706/371, 3-4=-1547/356, 4-5=-1546/356, 5-6=-1705/371

**BOT CHORD** 2-10=-310/1503. 8-10=-107/985. 6-8=-254/1500

WFBS 4-8=-133/638, 5-8=-343/208, 4-10=-133/640, 3-10=-343/207

# NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -2-0-0 to 1-0-0, Interior(1) 1-0-0 to 12-3-12, Exterior(2R) 12-3-12 to 15-3-12, Interior(1) 15-3-12 to 26-7-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 233 lb uplift at joint 2 and 233 lb uplift at

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March 27,2023



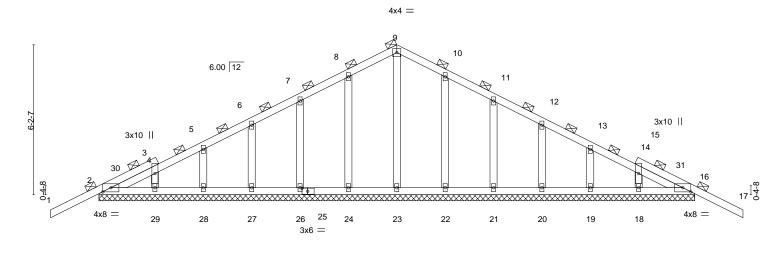
8-4-7

Structural wood sheathing directly applied or 4-2-14 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

NORRIS CONST. - HERITAGE Job Truss Truss Type Qty Ply T30144163 3458616 T01G Common Supported Gable Job Reference (optional) Builders FirstSource (Lake City,FL), Lake City, FL - 32055, 8.530 s Mar 9 2023 MiTek Industries, Inc. Fri Mar 24 07:25:06 2023 Page 1 ID:YPmxjjoROp4\_h6tHh\_ArnIzVUvx-eoNsQYE2w3TPYj3Q4G6Nfq8XozV37SgU1y0ASqzXo5R 24-7-8

Scale: 1/4"=1



24-7-8 [2:0-4-0,0-2-1], [16:0-4-0,0-2-1], [25:0-2-12,0-1-8] Plate Offsets (X,Y)--LOADING (psf) SPACING-2-0-0 CSI. DEFL. (loc) I/defI L/d **PLATES GRIP TCLL** 20.0 Plate Grip DOL 1.25 TC 0.24 Vert(LL) -0.02 17 n/r 120 MT20 244/190 TCDL вс Vert(CT) 7.0 Lumber DOL 1.25 0.04 -0.03 17 n/r 120 WB **BCLL** 0.0 Rep Stress Incr YES 0.08 Horz(CT) 0.00 16 n/a n/a BCDL 10.0 Code FBC2020/TPI2014 Matrix-S Weight: 139 lb FT = 20%

24-7-8

LUMBER-

TOP CHORD 2x4 SP No 2 BOT CHORD

2x4 SP No 2 2x4 SP No 3 OTHERS

**BRACING-**

TOP CHORD 2-0-0 oc purlins (6-0-0 max.).

**BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing.

12-3-12

REACTIONS. All bearings 24-7-8.

Max Horz 2=-102(LC 13)

Max Uplift All uplift 100 lb or less at joint(s) 2, 16, 24, 26, 27, 28, 29, 22, 21, 20, 19, 18 All reactions 250 lb or less at joint(s) 2, 16, 23, 24, 26, 27, 28, 29, 22, 21, 20, 19, 18

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

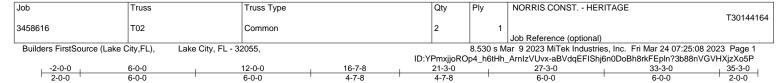
- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Corner(3E) -2-0-0 to 1-0-0, Exterior(2N) 1-0-0 to 12-3-12, Corner(3R) 12-3-12 to 15-3-12, Exterior(2N) 15-3-12 to 26-7-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 16, 24, 26, 27, 28, 29, 22, 21, 20, 19, 18.
- 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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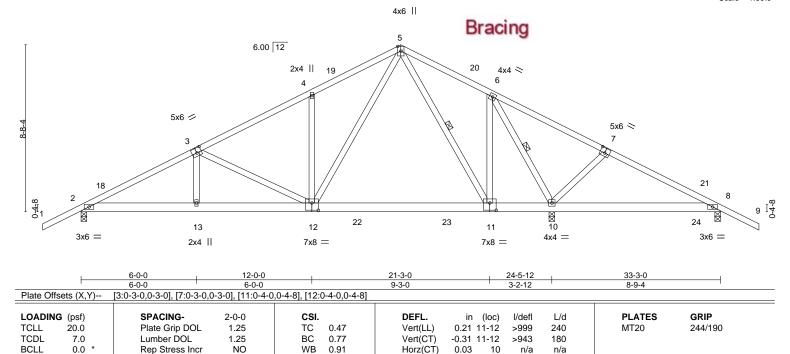
Philip J. O'Regan PE No.58126 MITek Inc. DBA MITek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

March 27,2023





Scale = 1:59.9



LUMBER-

BCDL

TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x6 SP No 2 WFBS

10.0

2x4 SP No 3

**BRACING-**TOP CHORD

**BOT CHORD** 

WFBS

Structural wood sheathing directly applied or 4-0-11 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing, Except:

Weight: 212 lb

FT = 20%

6-0-0 oc bracing: 8-10. 1 Row at midpt 5-11, 6-10

REACTIONS.

(size) 2=0-3-8, 10=0-3-8, 8=0-3-8

Max Horz 2=138(LC 12)

Max Uplift 2=-281(LC 12), 10=-394(LC 13), 8=-84(LC 8) Max Grav 2=1162(LC 2), 10=1949(LC 2), 8=308(LC 24)

Code FBC2020/TPI2014

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1922/588, 3-4=-1505/622, 4-5=-1496/712, 5-6=-622/378, 6-7=-129/475,

7-8=-97/291

BOT CHORD 2-13=-434/1760, 12-13=-432/1762, 11-12=-134/761, 10-11=-95/566

5-11=-405/111, 6-11=-337/992, 6-10=-1830/733, 7-10=-316/191, 5-12=-536/1239, **WEBS** 

4-12=-309/199, 3-12=-448/184

# NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -2-0-0 to 1-3-14, Interior(1) 1-3-14 to 16-7-8, Exterior(2R) 16-7-8 to 19-11-6, Interior(1) 19-11-6 to 35-3-0 zone; porch right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

Matrix-MS

- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8 except (jt=lb) 2=281, 10=394
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

# LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-5=-54, 5-9=-54, 2-12=-20, 11-12=-80(F=-60), 8-11=-20

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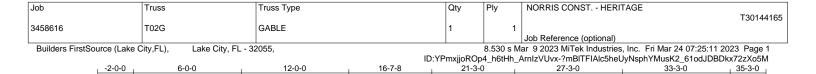
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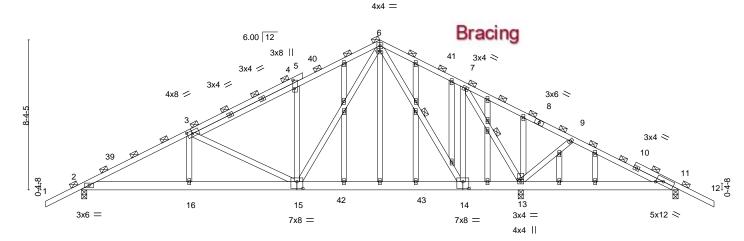
4-7-8

6-0-0

12-0-0

Scale: 3/16"=1"

6-0-0



	0-0-0	0-0-0	9-3-0 3-1-0 0-	1-12 0-9-4
Plate Offsets (X,Y)	[3:0-3-8,0-2-0], [4:0-5-10,0-1-0], [6:0-2-	0,0-0-0], [11:0-4-12,Edge]	], [13:0-0-12,0-2-0], [14:0-4-0,0-4-8], [15:0-4-0	,0-4-8]
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl	L/d PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.25	TC 0.40	Vert(LL) -0.10 14-15 >999	240 MT20 244/190
TCDL 7.0	Lumber DOL 1.25	BC 0.44	Vert(CT) -0.17 14-15 >999	180
BCLL 0.0 *	Rep Stress Incr YES	WB 0.43	Horz(CT) 0.02 13 n/a	n/a
BCDL 10.0	Code FBC2020/TPI2014	Matrix-MS		Weight: 277 lb FT = 20%

LUMBER-TOP CHORD

WFBS

2x4 SP No.2 2x6 SP No 2

**BOT CHORD** 2x4 SP No.3

**OTHERS** 2x4 SP No.3 **BRACING-**

TOP CHORD 2-0-0 oc purlins (4-4-0 max.).

**BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing, Except:

6-0-0 oc bracing: 11-13.

WFBS 1 Row at midpt 6-14, 7-13

6-0-0

REACTIONS. (size) 2=0-3-8, 11=0-3-8, 13=0-3-8

Max Horz 2=-134(LC 17)

Max Uplift 2=-238(LC 12), 11=-91(LC 13), 13=-284(LC 13) Max Grav 2=1008(LC 2), 11=314(LC 24), 13=1615(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1658/334, 3-4=-1149/245, 4-6=-1122/344, 6-7=-384/181, 7-9=-168/502,

9-11=-116/294

BOT CHORD  $2\hbox{-}16\hbox{--}341/1513,\ 15\hbox{--}16\hbox{--}343/1490,\ 14\hbox{--}15\hbox{--}27/564,\ 13\hbox{--}14\hbox{--}0/350$ 

WEBS 3-15=-509/202, 4-15=-308/194, 6-15=-272/953, 6-14=-430/148, 7-14=-55/644,

7-13=-1393/236, 9-13=-319/187

# NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -2-0-0 to 1-3-14, Interior(1) 1-3-14 to 16-7-8, Exterior(2R) 16-7-8 to 19-11-6, Interior(1) 19-11-6 to 35-3-0 zone; porch right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 11 except (jt=lb) 2=238 13=284
- 10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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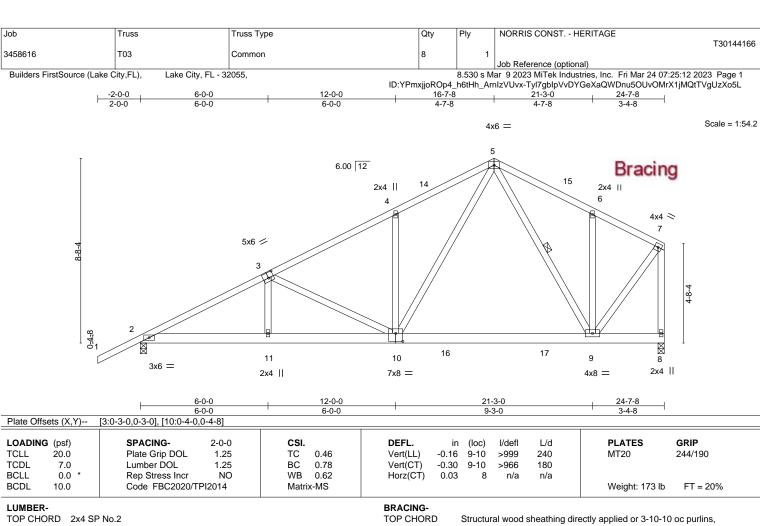
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Chesterfield, MO 63017



2x6 SP No 2

**BOT CHORD** 2x4 SP No 3 WFBS

except end verticals.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:

6-0-0 oc bracing: 8-9. WEBS 1 Row at midpt

REACTIONS. (size) 2=0-3-8, 8=0-3-8

Max Horz 2=239(LC 12)

Max Uplift 2=-290(LC 12), 8=-275(LC 12) Max Grav 2=1235(LC 2), 8=1337(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2085/453, 3-4=-1666/414, 4-5=-1655/509, 5-6=-898/313, 6-7=-874/235,

7-8=-1461/378

BOT CHORD 2-11=-540/1847, 10-11=-539/1849, 9-10=-203/849

WEBS 3-10=-450/183, 4-10=-308/198, 5-10=-364/1230, 7-9=-307/1279

# NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -2-0-0 to 0-9-14, Interior(1) 0-9-14 to 16-7-8, Exterior(2R) 16-7-8 to 19-7-8, Interior(1) 19-7-8 to 24-5-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=290. 8=275.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

# LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-5=-54, 5-7=-54, 2-10=-20, 9-10=-80(F=-60), 8-9=-20

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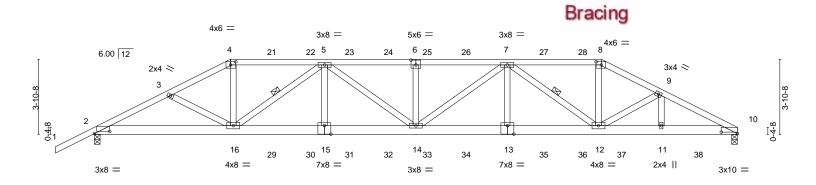
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16023 Swingley Ridge Rd Chesterfield, MO 63017

Job	Truss	Truss Type		Qty	Ply	NORRIS CONST	HERITAGE		
3458616	T04	Hip Girder		1	1				T30144167
						Job Reference (option	nal)		
Builders FirstSource (Lake C	City,FL), Lake City, F	L - 32055,			8.530 s M	ar 9 2023 MiTek Indu	stries, Inc. Fri N	Mar 24 07:25:15 2023	Page 1
				ID:YPmxjjoR0	Dp4_h6tHh	_ArnIzVUvx-tXQGIdL	hoqb775F95fmL	JWk0tHcU8kNJp6ri9l	HpzXo5I
2-0-0 3-10-1	15 7-0-0	11-10-2	16-6-8	21-2-14		26-1-0	29-2-1	33-1-0	
2-0-0 3-10-1	3-1-1	4-10-2	4-8-6	4-8-6		4-10-2	3-1-1	3-10-15	

Scale = 1:59.3



	7-0-0	11-10-2	16-6-8	21-2-14	26-1-0	29-2-1 33	3-1-0
	7-0-0	4-10-2	4-8-6	4-8-6	4-10-2	3-1-1 3-1	10-15
Plate Offsets (X,Y	[2:0-9-7,0-1-6], [4:0-3-4,0	-2-0], [6:0-3-0,0-3-0],	[8:0-3-4,0-2-0], [10:0-10	0-0,0-0-15], [13:0-4-0,0-5	-4], [15:0-4-0,0-5-4]		
LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL. in (lo	oc) I/defl L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.96	Vert(LL) -0.36	14 >999 240	MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC 0.43	Vert(CT) -0.67	14 >591 180		
BCLL 0.0	Rep Stress Incr	NO	WB 0.69	Horz(CT) 0.13	10 n/a n/a		
BCDL 10.0	Code FBC2020/T	PI2014	Matrix-MS			Weight: 201 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

WERS

Structural wood sheathing directly applied.

1 Row at midpt

Rigid ceiling directly applied or 8-2-7 oc bracing.

5-16 7-12

LUMBER-

REACTIONS.

WFBS

TOP CHORD 2x4 SP No 2 BOT CHORD 2x6 SP M 26

2x4 SP No 3

(size) 10=0-3-8, 2=0-3-8 Max Horz 2=82(LC 27)

Max Uplift 10=-772(LC 9), 2=-647(LC 8) Max Grav 10=2655(LC 1), 2=2447(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-4819/1256 3-4=-4657/1210 4-5=-4212/1122 5-6=-6240/1627 6-7=-6240/1627

7-8=-4336/1234, 8-9=-4789/1333, 9-10=-5348/1547

BOT CHORD 2-16=-1128/4269, 15-16=-1476/5786, 14-15=-1476/5785, 13-14=-1488/5833,

12-13=-1487/5833, 11-12=-1339/4756, 10-11=-1339/4756

WEBS 4-16=-347/1690, 5-16=-2001/573, 5-15=0/405, 5-14=-197/606, 6-14=-464/239, 7-14=-145/558, 7-13=0/402, 7-12=-1918/501, 8-12=-435/1809, 9-12=-578/285,

9-11=-163/391

# NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) Provide adequate drainage to prevent water ponding.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb)
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 125 lb down and 88 lb up at 7-0-0, 106 lb down and 88 lb up at 9-0-12, 106 lb down and 88 lb up at 11-0-12, 106 lb down and 88 lb up at 13-0-12, 106 lb down and 88 lb up at 15-0-12, 106 lb down and 85 lb up at 17-0-12, 106 lb down and 88 lb up at 19-0-12, 106 lb down and 88 lb up at 21-0-12, and 106 lb down and 88 lb up at 23-0-12, and 106 lb down and 88 lb up at 25-0-12 on top chord, and 293 lb down and 90 lb up at 7-0-0, 85 lb down at 9-0-12, 85 lb down at 11-0-12, 85 lb down at 13-0-12, 85 lb down at 15-0-12, 85 lb down at 17-0-12, 85 lb down at 19-0-12, 85 lb down at 21-0-12, 85 lb down at 23-0-12, 85 lb down at 25-0-12, 218 lb down and 111 lb up at 27-0-12, and 218 lb down and 111 lb up at 29-0-12, and 218 lb down and 111 lb up at 31-0-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

This item has been electronically signed and sealed by ORegan, Philip, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Philip J. O'Regan PE No.58126 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

March 27,2023

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chorembers only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

\*\*ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information\*\* available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



16023 Swingley Ridge Rd Chesterfield, MO 63017

Job	Truss	Truss Type	Qty	Ply	NORRIS CONST HERITAGE
					T30144167
3458616	T04	Hip Girder	1	1	
					Job Reference (optional)

Builders FirstSource (Lake City,FL),

Lake City, FL - 32055,

8.530 s Mar 9 2023 MiTek Industries, Inc. Fri Mar 24 07:25:15 2023 Page 2 ID:YPmxjjoROp4\_h6tHh\_ArnIzVUvx-tXQGIdLhoqb775F95fmUWk0tHcU8kNJp6ri9HpzXo5I

### LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-4=-54, 4-8=-54, 8-10=-54, 2-10=-20

Concentrated Loads (lb)

Vert: 4=-106(F) 16=-282(F) 13=-61(F) 7=-106(F) 11=-218(F) 21=-106(F) 22=-106(F) 23=-106(F) 24=-106(F) 25=-106(F) 25=-106(

Job Truss Truss Type Qty Ply NORRIS CONST. - HERITAGE T30144168 3458616 T05 diH Job Reference (optional) Builders FirstSource (Lake City,FL), Lake City, FL - 32055, 8.530 s Mar 9 2023 MiTek Industries, Inc. Fri Mar 24 07:25:17 2023 Page 1 ID:YPmxjjoROp4\_h6tHh\_ArnIzVUvx-pwY0jJMxKSrrMPPXD4oyb96liP3JCNN6Z9BFLizXo5G <del>-2-0-0</del> <del>2-0-0</del> 4-9-8 16-6-8 24-1-0 28-3-8 33-1-0 4-9-8 7-6-8 7-6-8 4-2-8 4-9-8

Scale = 1:59.1

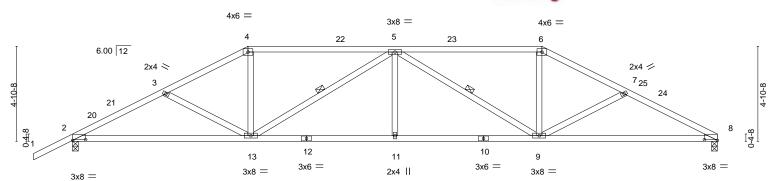
# Bracing

Structural wood sheathing directly applied or 3-3-3 oc purlins.

5-13 5-9

Rigid ceiling directly applied or 8-8-3 oc bracing.

1 Row at midpt



	9-0-0	16-6-8	24-1-0	33-1-0
	9-0-0	7-6-8	7-6-8	9-0-0
Plate Offsets (X,Y)	[2:0-8-0,0-0-8], [8:0-8-0,0-0-7]			
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl	L/d PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.25	TC 0.65	Vert(LL) -0.16 11 >999 2	240 MT20 244/190
TCDL 7.0	Lumber DOL 1.25	BC 0.83	Vert(CT) -0.34 9-16 >999	180
BCLL 0.0 *	Rep Stress Incr YES	WB 0.27	Horz(CT) 0.11 8 n/a	n/a
BCDL 10.0	Code FBC2020/TPI2014	Matrix-MS		Weight: 163 lb FT = 20%

**BRACING-**

TOP CHORD

**BOT CHORD** 

WFBS

LUMBER-

REACTIONS.

WFBS

TOP CHORD 2x4 SP No 2 2x4 SP No 2

**BOT CHORD** 2x4 SP No.3

> (size) 8=0-3-8, 2=0-3-8 Max Horz 2=97(LC 12)

Max Uplift 8=-264(LC 13), 2=-309(LC 12) Max Grav 8=1221(LC 1), 2=1335(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2283/499, 3-4=-2034/430, 4-5=-1791/418, 5-6=-1804/426, 6-7=-2049/440,

7-8=-2304/518

2-13=-459/2000, 11-13=-449/2282, 9-11=-449/2282, 8-9=-412/2035 BOT CHORD

**WEBS**  $3-13=-259/148,\ 4-13=-81/596,\ 5-13=-669/213,\ 5-11=0/268,\ 5-9=-661/209,\ 6-9=-81/604,$ 

7-9=-285/162

# NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -2-0-0 to 1-3-11, Interior(1) 1-3-11 to 9-0-0, Exterior(2R) 9-0-0 to 13-8-2, Interior(1) 13-8-2 to 24-1-0, Exterior(2R) 24-1-0 to 28-9-2, Interior(1) 28-9-2 to 33-1-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) Provide adequate drainage to prevent water ponding.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 8=264, 2=309.

This item has been electronically signed and sealed by ORegan, Philip, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Philip J. O'Regan PE No.58126 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

March 27,2023

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

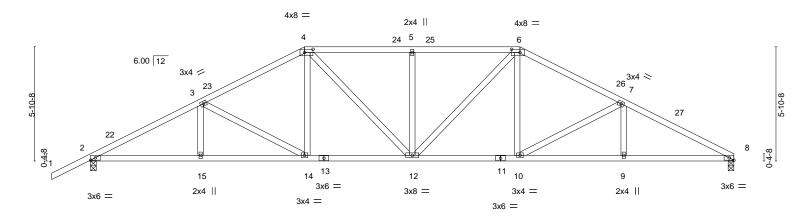
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



NORRIS CONST. - HERITAGE Job Truss Truss Type Qty Ply T30144169 3458616 T06 Hip Job Reference (optional) Builders FirstSource (Lake City,FL), Lake City, FL - 32055, 8.530 s Mar 9 2023 MiTek Industries, Inc. Fri Mar 24 07:25:19 2023 Page 1 ID:YPmxjjoROp4\_h6tHh\_ArnIzVUvx-mlgn8\_OCs35ZbiZwKUrQgaBjyDpygGcO1TgMQazXo5E <del>-2-0-0</del> <del>2-0-0</del> 11-0-0 16-6-8 22-1-0 5-6-8 5-6-8 5-7-15

Scale = 1:59.2



		3-1-13	11-0-0	10-0-0	22-1-0	21-3-1	33-1-0
		5-7-15	5-4-1	5-6-8	5-6-8	5-4-1	5-7-15
Plate Offsets	s (X,Y)	[4:0-5-4,0-2-0], [6:0-5-4	4,0-2-0], [8:0-2-1	5,Edge]			<u> </u>
			· · · · · · · · · · · · · · · · · · ·				
LOADING (	psf)	SPACING-	2-0-0	CSI.	DEFL. in (loc)	l/defl L/d	PLATES GRIP
TCLL 2	20.0	Plate Grip DOL	1.25	TC 0.34	Vert(LL) -0.13 12	>999 240	MT20 244/190
TCDL	7.0	Lumber DOL	1.25	BC 0.56	Vert(CT) -0.24 10-12	>999 180	
BCLL	0.0 *	Rep Stress Incr	YES	WB 0.35	Horz(CT) 0.10 8	n/a n/a	
BCDL 1	10.0	Code FBC2020	/TPI2014	Matrix-MS			Weight: 177 lb FT = 20%

16-6-8

LUMBER-

WFBS

TOP CHORD 2x4 SP No 2 BOT CHORD 2x4 SP No 2 2x4 SP No.3 **BRACING-**

22-1-0

TOP CHORD **BOT CHORD**  Structural wood sheathing directly applied or 3-7-7 oc purlins.

33-1-0

Rigid ceiling directly applied or 8-10-6 oc bracing.

27-5-1

REACTIONS. (size) 8=0-3-8, 2=0-3-8

Max Horz 2=111(LC 16)

5-7-15

Max Uplift 8=-262(LC 13), 2=-306(LC 12) Max Grav 8=1221(LC 1), 2=1335(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2299/473, 3-4=-1875/416, 4-5=-1840/448, 5-6=-1840/448, 6-7=-1882/421,

11\_0\_0

7-8=-2333/493 2-15=-446/2003. 14-15=-446/2003. 12-14=-287/1622. 10-12=-243/1627. 9-10=-383/2037.

8-9=-383/2037

WEBS 3-14=-444/181, 4-14=-53/376, 4-12=-131/419, 5-12=-339/166, 6-12=-129/413,

6-10=-59/379, 7-10=-477/198

BOT CHORD

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -2-0-0 to 1-3-11, Interior(1) 1-3-11 to 11-0-0, Exterior(2R) 11-0-0 to 15-8-2, Interior(1) 15-8-2 to 22-1-0, Exterior(2R) 22-1-0 to 26-9-2, Interior(1) 26-9-2 to 33-1-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) Provide adequate drainage to prevent water ponding.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 8=262, 2=306.

This item has been electronically signed and sealed by ORegan, Philip, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Philip J. O'Regan PE No.58126 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

March 27,2023



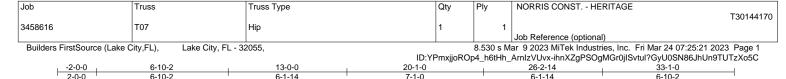
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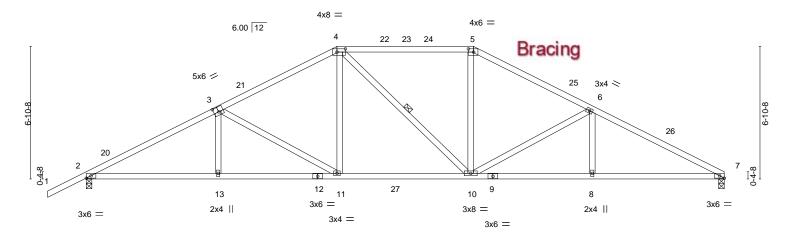
ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Chesterfield, MO 63017



Scale = 1:59.7



		0-10-2		0-1-14	7-1-0		0-1-14		0-10-2	
Plate Offsets	s (X,Y)	[3:0-3-0,0-3-0], [4:0-5-4,0	-2-0], [5:0-3-4,	0-2-0], [7:0-2-15,	Edge]					
LOADING (	(psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 2	20.0	Plate Grip DOL	1.25	TC 0.7	79 Vert(LL)	-0.18 10-11	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC 0.7	76 Vert(CT)	-0.31 10-11	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB 0.5	59 Horz(CT	0.10 7	n/a	n/a		
BCDL 1	10.0	Code FBC2020/T	PI2014	Matrix-MS	S				Weight: 169 lb	FT = 20%

**BRACING-**

TOP CHORD

**BOT CHORD** 

WFBS

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No 2 **BOT CHORD** 2x4 SP No 2 WFBS

2x4 SP No 3

(size) 7=0-3-8, 2=0-3-8 Max Horz 2=126(LC 12)

Max Uplift 7=-259(LC 13), 2=-304(LC 12) Max Grav 7=1323(LC 2), 2=1421(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2447/458. 3-4=-1894/403. 4-5=-1637/392. 5-6=-1884/405. 6-7=-2443/473 BOT CHORD 2-13=-436/2138, 11-13=-436/2141, 10-11=-242/1646, 8-10=-354/2149, 7-8=-354/2149 WFBS 3-13=0/263, 3-11=-589/223, 4-11=-66/551, 5-10=-58/537, 6-10=-607/237, 6-8=0/267

# NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -2-0-0 to 1-3-11, Interior(1) 1-3-11 to 13-0-0, Exterior(2R) 13-0-0 to 17-8-2, Interior(1) 17-8-2 to 20-1-0, Exterior(2R) 20-1-0 to 24-9-2, Interior(1) 24-9-2 to 33-1-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

13-0-0

- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) Provide adequate drainage to prevent water ponding.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 7=259, 2=304.

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33-1-0

Structural wood sheathing directly applied or 2-2-0 oc purlins.

Rigid ceiling directly applied or 8-10-6 oc bracing.

1 Row at midpt

Philip J. O'Regan PE No.58126 MITek Inc. DBA MITek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

March 27,2023



NORRIS CONST. - HERITAGE Job Truss Truss Type Qty Ply T30144171 3458616 T08 Hip Job Reference (optional) Builders FirstSource (Lake City,FL), Lake City, FL - 32055, 8.530 s Mar 9 2023 MiTek Industries, Inc. Fri Mar 24 07:25:23 2023 Page 1 ID:YPmxjjoROp4\_h6tHh\_ArnIzVUvx-e3vH\_MRiwIc\_4KshZKvMrQMKkq77c6C\_x5eaZLzXo5A

18-1-0

3-1-0

21-3-0

26-11-15

5-8-15

15-0-0

3-2-0

Scale = 1:59.0

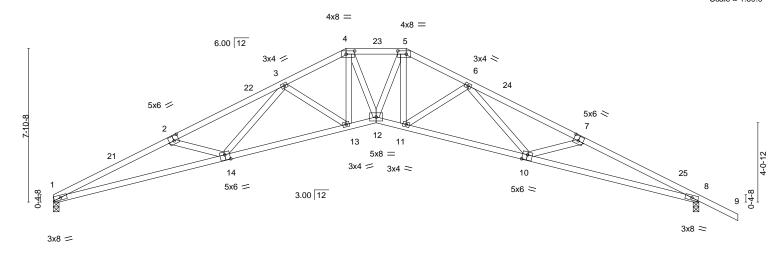
2-0-0

6-1-1

33-1-0

Structural wood sheathing directly applied or 2-2-14 oc purlins.

Rigid ceiling directly applied or 6-8-6 oc bracing.



	0-9-0	0-3-0	-8 1-0-8 0-3-0		0-9-0
Plate Offsets (X,Y)-	[2:0-3-0,0-3-0], [4:0-5-4,0-2-0], [5:0-5-4,	0-2-0], [7:0-3-0,0-3-0], [10	:0-3-0,0-3-4], [14:0-3-0,0-3-4]		
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc)	l/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.25	TC 0.68	Vert(LL) -0.43 12	>930 240	MT20 244/190
TCDL 7.0	Lumber DOL 1.25	BC 0.80	Vert(CT) -0.79 10-11	>501 180	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.25	Horz(CT) 0.55 8	n/a n/a	
BCDL 10.0	Code FBC2020/TPI2014	Matrix-MS			Weight: 164 lb FT = 20%

**BRACING-**

TOP CHORD

**BOT CHORD** 

16-6-8 18-1-0

LUMBER-

TOP CHORD 2x4 SP No 2

2x4 SP No.2 \*Except\* **BOT CHORD** 

1-14,8-10: 2x4 SP No.1

WFBS 2x4 SP No.3

REACTIONS. (size) 1=0-3-8, 8=0-3-8

Max Horz 1=-141(LC 13)

Max Uplift 1=-256(LC 12), 8=-300(LC 13)

8-9-0

Max Grav 1=1221(LC 1), 8=1335(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-4154/925, 2-3=-3791/784, 3-4=-2922/547, 4-5=-2842/526, 5-6=-2922/540,

6-7=-3756/679. 7-8=-4105/770

BOT CHORD 1-14=-889/3779, 13-14=-569/3125, 12-13=-360/2649, 11-12=-303/2647, 10-11=-434/3116, 8-10=-631/3724

11-10-0

5-8-15

2-14=-355/237, 3-14=-141/550, 3-13=-556/250, 4-13=-164/567, 4-12=-91/645, 5-12=-158/649, 5-11=-161/566, 6-11=-546/247, 6-10=-132/542, 7-10=-336/232

# NOTES-

**WEBS** 

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-0-0 to 3-3-11, Interior(1) 3-3-11 to 15-0-0, Exterior(2E) 15-0-0 to 18-1-0, Exterior(2R) 18-1-0 to 22-9-2, Interior(1) 22-9-2 to 35-1-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

15-0-0

- 4) Provide adequate drainage to prevent water ponding.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Bearing at joint(s) 1, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=256, 8=300.

This item has been electronically signed and sealed by ORegan, Philip, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Philip J. O'Regan PE No.58126 MITek Inc. DBA MITek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

March 27,2023

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

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ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Chesterfield, MO 63017

Job Truss Truss Type Qty Ply NORRIS CONST. - HERITAGE T30144172 3458616 T08A 2 Scissor Job Reference (optional) Builders FirstSource (Lake City,FL), Lake City, FL - 32055, 8.530 s Mar 9 2023 MiTek Industries, Inc. Fri Mar 24 07:25:25 2023 Page 1 ID:YPmxjjoROp4\_h6tHh\_ArnIzVUvx-aS12P2SyRvsiKd04hlxqwrRgrem64seHPP7hdEzXo58 11-10-0 16-6-8 21-3-0 26-11-15 33-1-0 2-0-0 5-8-15 4-8-8 6-1-1 Scale = 1:59.2 4x4 || 1 6.00 12 20 3x4 / 19 3x4 < 5 3 5x6 / 5x6 > 10 5x8 =9 5x8 = 5x8 = 3.00 12 3x8 = 3x8 = 8-9-0 16-6-8 8-9-0 7-9-8 8-9-0 7-9-8 Plate Offsets (X,Y)--[2:0-3-0,0-3-0], [6:0-3-0,0-3-0], [9:0-4-0,0-3-0], [11:0-4-0,0-3-0]

DEFL.

Vert(LL)

Vert(CT)

Horz(CT)

**BRACING-**

TOP CHORD

**BOT CHORD** 

in (loc)

9-10

9-10

-0.45

-0.89

0.57

I/defI

>881

>447

n/a

L/d

240

180

n/a

Rigid ceiling directly applied or 6-7-11 oc bracing.

Structural wood sheathing directly applied or 2-2-4 oc purlins.

**PLATES** 

Weight: 153 lb

MT20

GRIP

244/190

FT = 20%

LUMBER-

LOADING (psf)

**TCLL** 

TCDL

**BCLL** 

BCDL

TOP CHORD 2x4 SP No.2

**BOT CHORD** 2x4 SP No.2 \*Except\* 1-11.7-9: 2x4 SP No.1

WFBS 2x4 SP No.3

20.0

7.0

0.0

10.0

REACTIONS.

(size) 1=0-3-8, 7=0-3-8 Max Horz 1=-152(LC 13)

Max Uplift 1=-253(LC 12), 7=-298(LC 13) Max Grav 1=1221(LC 1), 7=1335(LC 1)

SPACING-

Plate Grip DOL

Rep Stress Incr

Code FBC2020/TPI2014

Lumber DOL

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-4167/926, 2-3=-3802/779, 3-4=-2731/485, 4-5=-2731/494, 5-6=-3769/652,

2-0-0

1.25

1.25

YES

CSI.

TC

вс

WB

Matrix-MS

0.70

0.90

0.83

6-7=-4119/760

BOT CHORD  $1\hbox{-}11\hbox{=-}901/3793,\ 10\hbox{-}11\hbox{=-}590/3126,\ 9\hbox{-}10\hbox{=-}421/3116,\ 7\hbox{-}9\hbox{=-}607/3742$ WEBS

4-10=-356/2170, 5-10=-687/307, 5-9=-117/594, 6-9=-335/233, 3-10=-698/309,

3-11=-125/602, 2-11=-355/239

# NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-0-0 to 3-3-11, Interior(1) 3-3-11 to 16-6-8, Exterior(2R) 16-6-8 to 19-10-3, Interior(1) 19-10-3 to 35-1-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Bearing at joint(s) 1, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=253, 7=298,

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Philip J. O'Regan PE No.58126 MITek Inc. DBA MITek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

March 27,2023



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Chesterfield, MO 63017

Job Truss Truss Type Qty Ply NORRIS CONST. - HERITAGE T30144173 3458616 T09 Roof Special Girder Job Reference (optional) Builders FirstSource (Lake City,FL), Lake City, FL - 32055, 8.530 s Mar 9 2023 MiTek Industries, Inc. Fri Mar 24 07:25:27 2023 Page 1 ID:YPmxjjoROp4\_h6tHh\_ArnIzVUvx-Xr9oqjUDzW6QZxASoA\_I?GWxeRRSYIgasjcni6zXo56

Scale = 1:76.6

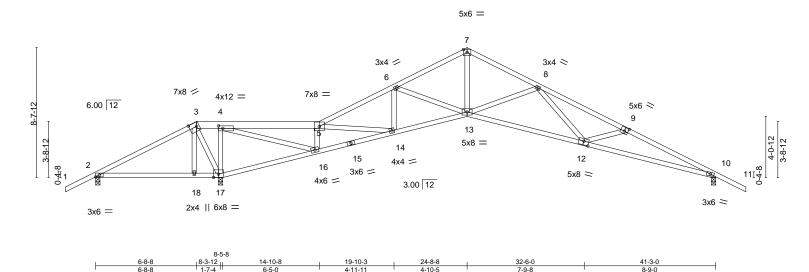


Plate Offsets (X,Y)--[3:0-5-12,0-3-4], [4:0-3-8,0-2-0], [9:0-3-0,0-3-0], [12:0-4-0,0-3-0], [17:0-4-0,0-2-10]LOADING (psf) SPACING-2-0-0 CSI. DEFL. (loc) I/defI L/d **PLATES** GRIP **TCLL** 20.0 Plate Grip DOL 1.25 TC 0.94 Vert(LL) -0.26 12-13 >999 240 MT20 244/190 TCDL 7.0 Lumber DOL 1.25 BC 0.97 Vert(CT) -0.54 12-13 >736 180 WB **BCLL** 0.0 Rep Stress Incr NO 0.86 Horz(CT) 0.22 10 n/a n/a BCDL 10.0 Code FBC2020/TPI2014 Matrix-MS Weight: 213 lb FT = 20%

**BRACING-**

TOP CHORD

**BOT CHORD** 

LUMBER-

2x4 SP No.2 \*Except\* TOP CHORD 3-5: 2x6 SP No.2

BOT CHORD 2x4 SP No.2 2x4 SP No.3 **WEBS** 

REACTIONS.

(size) 2=0-3-8, 17=0-3-8 (req. 0-3-11), 10=0-3-8

Max Horz 2=138(LC 12)

Max Uplift 2=-617(LC 16), 17=-776(LC 8), 10=-276(LC 28) Max Grav 2=124(LC 27), 17=3149(LC 1), 10=1071(LC 1)

0-1-12

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-501/2061, 3-4=-421/2469, 4-5=-23/410, 5-6=-1409/302, 6-7=-1582/263,

7-8=-1583/279, 8-9=-2681/528, 9-10=-3053/672 BOT CHORD 2-18=-1771/442, 17-18=-1788/432, 16-17=-2519/434, 14-16=-391/188, 13-14=-226/1262,

12-13=-219/2075, 10-12=-525/2768

 $3-18 = -222/498,\ 3-17 = -1750/490,\ 4-17 = -886/253,\ 4-16 = -462/2249,\ 5-16 = -1026/279,$ 5-14=-166/1405, 6-14=-420/88, 6-13=-65/301, 7-13=-158/1133, 8-13=-703/308,

8-12=-121/636, 9-12=-356/236

**WEBS** 

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) Provide adequate drainage to prevent water ponding.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) WARNING: Required bearing size at joint(s) 17 greater than input bearing size.
- 8) Bearing at joint(s) 17, 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=617, 17=776, 10=276,
- 10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 139 lb down and 87 lb up at 6-8-8 on top chord, and 282 lb down and 203 lb up at 6-8-8 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 11) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

This item has been electronically signed and sealed by ORegan, Philip, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Philip J. O'Regan PE No.58126 MITek Inc. DBA MITek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

March 27,2023

# LOAD CASE(S) Standard

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFUKE USE.

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ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



8-9-0

Structural wood sheathing directly applied or 3-1-11 oc purlins.

Rigid ceiling directly applied or 3-6-4 oc bracing.

16023 Swingley Ridge Rd Chesterfield, MO 63017

Job	Truss	Truss Type	Qty	Ply	NORRIS CONST HERITAGE
					T30144173
3458616	T09	Roof Special Girder	1	1	
					Job Reference (optional)

Builders FirstSource (Lake City,FL),

Lake City, FL - 32055,

8.530 s Mar 9 2023 MiTek Industries, Inc. Fri Mar 24 07:25:27 2023 Page 2 ID:YPmxjjoROp4\_h6tHh\_ArnIzVUvx-Xr9oqjUDzW6QZxASoA\_I?GWxeRRSYIgasjcni6zXo56

### LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-3=-54, 3-5=-54, 5-7=-54, 7-11=-54, 17-19=-20, 13-17=-20, 13-22=-20

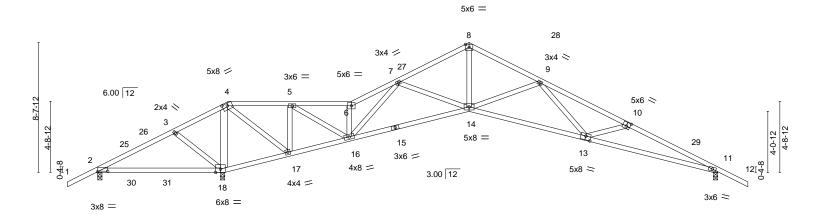
Concentrated Loads (lb) Vert: 3=-99(F) 18=-278(F)



Job Truss Truss Type Qty Ply NORRIS CONST. - HERITAGE T30144174 3458616 T10 Roof Special Job Reference (optional) Builders FirstSource (Lake City,FL), Lake City, FL - 32055, 8.530 s Mar 9 2023 MiTek Industries, Inc. Fri Mar 24 07:25:29 2023 Page 1

ID:YPmxjjoROp4\_h6tHh\_ArnIzVUvx-TDGZEPVTV8M8oFKrwb0m4hbl6F9Z0jTtK15um?zXo54 8-8-8 12-9-8 16-10-8 20-0-0 24-8-8 29-5-0 35-1-15 41-3-0 4-1-0 4-1-0 3-1-8 4-8-8 5-8-15 6-1-1 2-0-0

Scale = 1:76.6



		8-3-12 0-1	12 4-4-0	4-1-0	7-10-0	7-9-8	8-9-0
Plate Offse	ets (X,Y)	[2:0-8-0,0-0-7], [4:0-4-0,0	)-1-15], [10:0-3-	0,0-3-0], [13:0-4	-0,0-3-0], [18:0-4-0,0-2-10]		
LOADING	(psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc) I/defl L/d	PLATES GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC 0.	88 Vert(LL)	0.24 18-21 >411 240	MT20 244/190
TCDL	7.0	Lumber DOL	1.25	BC 0.	86 Vert(CT)	-0.50 13-14 >796 180	
BCLL	0.0 *	Rep Stress Incr	YES	WB 0.	58 Horz(CT)	0.21 11 n/a n/a	
BCDL	10.0	Code FBC2020/T	PI2014	Matrix-M	S		Weight: 213 lb FT = 20%

**BRACING-**

TOP CHORD

**BOT CHORD** 

24-8-8

32-6-0

41-3-0

Structural wood sheathing directly applied or 3-2-5 oc purlins.

Rigid ceiling directly applied or 3-5-10 oc bracing.

LUMBER-TOP CHORD **BOT CHORD** 

2x4 SP No 2 2x4 SP No 2

2x4 SP No.3 \*Except\* WFBS 4-18: 2x6 SP No.2

REACTIONS.

(size) 2=0-3-8, 18=0-3-8, 11=0-3-8

Max Horz 2=138(LC 16)

8-3-12

Max Uplift 2=-707(LC 1), 18=-607(LC 12), 11=-270(LC 13) Max Grav 2=92(LC 12), 18=2919(LC 1), 11=1056(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-393/2259, 3-4=-435/2463, 4-5=-82/888, 5-6=-540/153, 6-7=-659/204,

7-8=-1517/304, 8-9=-1523/294, 9-10=-2618/505, 10-11=-2992/650 BOT CHORD 2-18=-1947/359, 17-18=-2025/429, 16-17=-919/294, 14-16=-192/1132, 13-14=-265/2019,

12-9-8

16-10-8

11-13=-504/2713

**WEBS** 5-17=-1052/251, 5-16=-248/1527, 6-16=-431/151, 7-16=-990/142, 7-14=0/371,

8-14=-156/1074, 9-14=-701/309, 9-13=-123/628, 10-13=-358/236, 4-18=-2116/461,

3-18=-294/244, 4-17=-289/1366

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -2-0-0 to 2-1-8, Interior(1) 2-1-8 to 8-7-8, Exterior(2R) 8-7-8 to 12-9-8, Interior(1) 12-9-8 to 24-8-8, Exterior(2R) 24-8-8 to 28-10-0, Interior(1) 28-10-0 to 43-3-0 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) Provide adequate drainage to prevent water ponding.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Bearing at joint(s) 18, 11 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=707. 18=607. 11=270.

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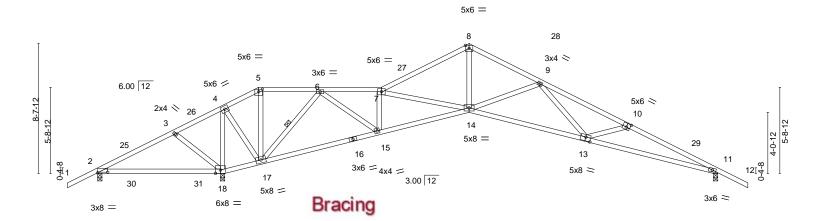
ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



16023 Swingley Ridge Rd Chesterfield, MO 63017

Job Truss Truss Type Qty Ply NORRIS CONST. - HERITAGE T30144175 3458616 T11 Roof Special Job Reference (optional) Builders FirstSource (Lake City,FL), Lake City, FL - 32055, 8.530 s Mar 9 2023 MiTek Industries, Inc. Fri Mar 24 07:25:31 2023 Page 1 ID:YPmxjjoROp4\_h6tHh\_ArnIzVUvx-PcOJf5Xj1lcs2YUD102E96heg2qnUay9nLa?ruzXo52

Scale = 1:76.6



	0-3-12 0-1	-010-0-0	10-10-0		24-0-0		32-0-0		41-3-0	
	8-3-12 0-1	122-3-0	8-2-0		5-10-0		7-9-8		8-9-0	
Plate Offsets (X,Y)	[2:0-8-0,0-0-7], [5:0-3-0,0	0-2-0], [10:0-3-	0,0-3-0], [13:0	0-4-0,0-3-0]	, [18:0-4-0,0-2-10]					
LOADING (psf)	SPACING-	2-0-0	CSI.		DEFL.	in (loc)	I/defI	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC	0.87	Vert(LL)	0.21 18-21	>461	240	MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC	0.88	Vert(CT)	-0.52 13-14	>761	180		
BCLL 0.0 *	Rep Stress Incr	YES	WB	0.77	Horz(CT)	0.23 11	n/a	n/a		
BCDL 10.0	Code FBC2020/1	PI2014	Matrix	x-MS					Weight: 215 lb	FT = 20%
			1		1				1	

LUMBER-TOP CHORD

WFBS

2x4 SP No 2 **BOT CHORD** 2x4 SP No 2

2x4 SP No.3

**BRACING-**TOP CHORD

**BOT CHORD** WFBS

Structural wood sheathing directly applied or 3-1-10 oc purlins.

Rigid ceiling directly applied or 3-6-10 oc bracing. 1 Row at midpt 6-17

32-6-0

REACTIONS.

(size) 2=0-3-8, 18=0-3-8, 11=0-3-8

Max Horz 2=138(LC 16)

Max Uplift 2=-688(LC 1), 18=-593(LC 12), 11=-271(LC 13) Max Grav 2=81(LC 12), 18=2895(LC 1), 11=1061(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-370/2217, 3-4=-411/2421, 4-5=-146/1222, 5-6=-111/1083, 6-7=-955/266,

9.5.910.9.9

8-5-8 10-8-8 3-3-8 2-3-0

7-8=-1548/316, 8-9=-1543/313, 9-10=-2639/508, 10-11=-3014/654

BOT CHORD 2-18=-1909/352, 17-18=-2141/454, 15-17=-346/226, 14-15=-191/1049, 13-14=-278/2033,

11-13=-508/2733

WEBS 3-18=-313/212, 4-18=-2084/433, 4-17=-304/1766, 5-17=-701/144, 6-17=-1395/298,  $6-15 = -216/1431, \ 7-15 = -996/215, \ 7-14 = -6/458, \ 8-14 = -152/1046, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703/305, \ 9-14 = -703$ 

9-13=-122/638, 10-13=-358/237

# NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -2-0-0 to 2-1-8, Interior(1) 2-1-8 to 10-8-8, Exterior(2R) 10-8-8 to 14-9-8, Interior(1) 14-9-8 to 24-8-8, Exterior(2R) 24-8-8 to 28-10-0, Interior(1) 28-10-0 to 43-3-0 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) Provide adequate drainage to prevent water ponding.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Bearing at joint(s) 18, 11 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=688, 18=593, 11=271.

This item has been electronically signed and sealed by ORegan, Philip, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Philip J. O'Regan PE No.58126 MITek Inc. DBA MITek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

March 27,2023

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

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\*\*ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information\*\* available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



NORRIS CONST. - HERITAGE Job Truss Truss Type Qty Ply T30144176 3458616 T12 Roof Special Job Reference (optional) Builders FirstSource (Lake City,FL), Lake City, FL - 32055, 8.530 s Mar 9 2023 MiTek Industries, Inc. Fri Mar 24 07:25:33 2023 Page 1 ID:YPmxjjoROp4\_h6tHh\_ArnIzVUvx-L?W34nZ\_ZMsZHsdc9R5iFXm\_\_sWAyVASFf36vmzXo50

24-8-8 3-10-0

29-5-0

4-8-8

35-1-15

5-8-15

41-3-0

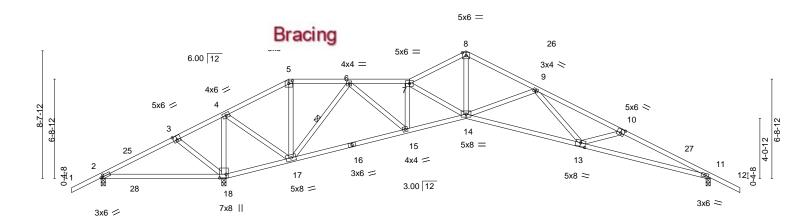
6-1-1

20-10-8

4-1-0

Scale = 1:78.0

2-0-0



	8-3-12 0-1 <sup>!</sup> -12 4-3-0	8-2-0	3-10-0	7-9-8	8-9-0
Plate Offsets (X,Y)	[2:0-1-15,0-1-8], [3:0-3-0,0-3-4], [5:0-3-0	),0-2-0], [10:0-3-0,0-3-0], [	13:0-4-0,0-3-0], [18:0-3-0,0-5-	4]	
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc	c) I/defl L/d	PLATES GRIP
u /			(	,	
TCLL 20.0	Plate Grip DOL 1.25	TC 0.88	Vert(LL) 0.25 18-2		MT20 244/190
TCDL 7.0	Lumber DOL 1.25	BC 0.89	Vert(CT) -0.52 13-1	4 >762 180	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.73	Horz(CT) 0.22 1	1 n/a n/a	
BCDL 10.0	Code FBC2020/TPI2014	Matrix-MS			Weight: 216 lb FT = 20%

20-10-8

LUMBER-

WFBS

TOP CHORD 2x4 SP No 2 2x4 SP No 2

**BOT CHORD** 2x4 SP No 3 **BRACING-**

TOP CHORD BOT CHORD WFBS

24-8-8

Structural wood sheathing directly applied or 3-2-3 oc purlins.

41-3-0

Rigid ceiling directly applied or 3-4-13 oc bracing. 1 Row at midpt 6-17

32-6-0

REACTIONS.

(size) 2=0-3-8, 18=0-3-8, 11=0-3-8

8-3-12

Max Horz 2=138(LC 16)

Max Uplift 2=-597(LC 1), 18=-568(LC 12), 11=-270(LC 13) Max Grav 2=65(LC 13), 18=2807(LC 1), 11=1059(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 $2\text{-}3\text{--}322/2049,\ 3\text{-}4\text{--}363/2257,\ 4\text{-}5\text{--}31/633,\ 5\text{-}6\text{--}1/532,\ 6\text{-}7\text{--}1162/308,}$ TOP CHORD

7-8=-1501/333, 8-9=-1526/322, 9-10=-2630/512, 10-11=-3003/649 BOT CHORD 2-18=-1758/343, 17-18=-2139/444, 15-17=-64/384, 14-15=-176/1245, 13-14=-290/2025,

8-5-8 12-8-8

11-13=-503/2723

WEBS 3-18=-301/200, 4-18=-1978/428, 4-17=-304/1875, 5-17=-475/110, 6-17=-1292/272,

 $6-15 = -169/1219, \ 7-15 = -906/179, \ 7-14 = -104/268, \ 8-14 = -185/1085, \ 9-14 = -705/308,$ 

12-8-8

4-3-0

16-9-8

4-1-0

9-13=-121/641, 10-13=-358/237

# NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -2-0-0 to 2-1-8, Interior(1) 2-1-8 to 12-8-8, Exterior(2R) 12-8-8 to 16-9-8, Interior(1) 16-9-8 to 24-8-8, Exterior(2R) 24-8-8 to 28-10-0, Interior(1) 28-10-0 to 43-3-0 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) Provide adequate drainage to prevent water ponding.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Bearing at joint(s) 11 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=597, 18=568, 11=270.

This item has been electronically signed and sealed by ORegan, Philip, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Philip J. O'Regan PE No.58126 MITek Inc. DBA MITek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

March 27,2023

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

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\*\*ABSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information\*\* available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



NORRIS CONST. - HERITAGE Job Truss Truss Type Qty Ply T30144177 3458616 T13 Hip Job Reference (optional) Builders FirstSource (Lake City,FL), Lake City, FL - 32055, 8.530 s Mar 9 2023 MiTek Industries, Inc. Fri Mar 24 07:25:36 2023 Page 1

2-10-8

5-8-15

Structural wood sheathing directly applied or 2-11-9 oc purlins.

Rigid ceiling directly applied or 3-11-8 oc bracing.

6-1-1

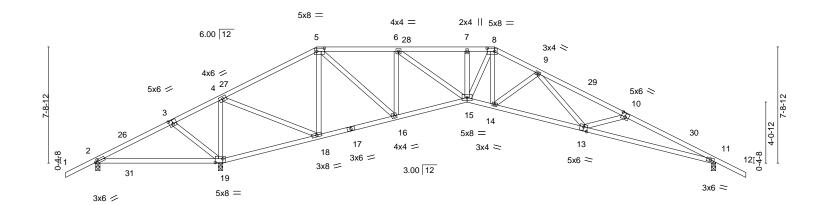
ID:YPmxjjoROp4\_h6tHh\_ArnIzVUvx-mZBCiobssHF88KMBqZePs9OWQ3Xb9sWvxcHmV5zXo4z 14-8-8 20-0-0 24-8-8 26-6-8 29-5-0 35-1-15 41-3-0

5-3-8

6-3-0

Scale = 1:76.6

2-0-0



1	8-3-12 8-	·5-8 14-8-8	20-0-0	24-8-8	<sub>1</sub> 26-6-8 <sub>1</sub>	32-6-0	41-3-0	1
	8-3-12 0-1	1 <sup>!</sup> -12 6-3-0	5-3-8	4-8-8	1-10-0	5-11-8	8-9-0	1
Plate Offsets (X,Y)	[2:0-1-15,0-1-8], [3:0-3-0	0,0-3-0], [5:0-6-0	,0-2-8], [8:0-6-0,0-2-8], [10	0:0-3-0,0-3-0], [13	3:0-3-0,0-3-4],	[19:0-6-0,0-3-0]		
LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	I/defl L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.81	Vert(LL)	0.26 19-22	>394 240	MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC 0.84	Vert(CT)	-0.46 13-14	>861 180		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.75	Horz(CT)	0.23 11	n/a n/a		
BCDL 10.0	Code FBC2020/	TPI2014	Matrix-MS				Weight: 222 lb	FT = 20%
	1						1	

**BRACING-**

TOP CHORD

**BOT CHORD** 

LUMBER-

REACTIONS.

WFBS

TOP CHORD 2x4 SP No 2 BOT CHORD 2x4 SP No 2 2x4 SP No.3

(size) 2=0-3-8, 19=0-3-8, 11=0-3-8 Max Horz 2=-124(LC 13)

Max Uplift 2=-692(LC 24), 19=-460(LC 12), 11=-293(LC 13) Max Grav 2=93(LC 13), 19=2662(LC 1), 11=1088(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-253/1807, 3-4=-257/1991, 4-5=-269/115, 5-6=-921/267, 6-7=-1790/368, TOP CHORD

7-8=-1790/368, 8-9=-1894/390, 9-10=-2736/598, 10-11=-3118/747

BOT CHORD 2-19=-1582/387, 18-19=-1881/452, 15-16=-149/956, 14-15=-131/1709, 13-14=-292/2146,

11-13=-594/2829

WEBS 3-19=-254/180, 4-19=-1899/376, 4-18=-250/1978, 5-18=-1074/213, 5-16=-216/1224,  $6\text{-}16\text{=-}926/209,\ 6\text{-}15\text{=-}185/1070,\ 8\text{-}15\text{=-}132/388,\ 8\text{-}14\text{=-}177/535,\ 9\text{-}14\text{=-}540/242,}$ 

9-13=-126/571, 10-13=-360/240

# NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -2-0-0 to 2-1-8, Interior(1) 2-1-8 to 14-8-8, Exterior(2R) 14-8-8 to 20-6-8, Interior(1) 20-6-8 to 26-6-8, Exterior(2R) 26-6-8 to 32-4-8, Interior(1) 32-4-8 to 43-3-0 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) Provide adequate drainage to prevent water ponding.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Bearing at joint(s) 11 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=692, 19=460, 11=293,

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Philip J. O'Regan PE No.58126 MITek Inc. DBA MITek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

March 27,2023

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

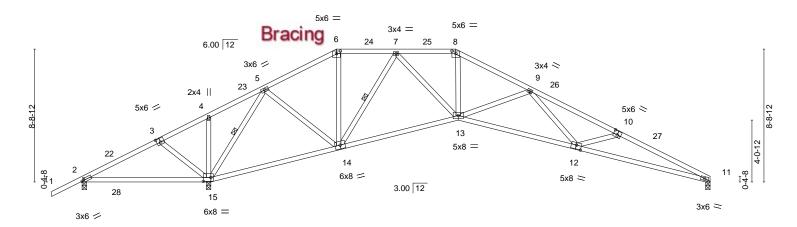
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ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



NORRIS CONST. - HERITAGE Job Truss Truss Type Qty Ply T30144178 3458616 T14 diH Job Reference (optional) Builders FirstSource (Lake City,FL), Lake City, FL - 32055, 8.530 s Mar 9 2023 MiTek Industries, Inc. Fri Mar 24 07:25:38 2023 Page 1 ID:YPmxjjoROp4\_h6tHh\_ArnIzVUvx-iyJy7Uc6OvVsNdWZy\_gtyaTsTtC5dn?BOwmta\_zXo4x 12-0-0 16-8-8 20-7-8 <u>29</u>-5-0 35-1-15 41-3-0 24-6-8 3-11-0 3-6-8 4-8-8 3-11-0 4-10-8 5-8-15 6-1-1

Scale = 1:75.7



	1	0-3-12 0-4	1-0 IU	1-0-0	24-0-0		32-0-0	41-3-0	1
		8-3-12 0-1 <sup>-</sup>	-12 8-	-3-0	8-0-0		7-9-8	8-9-0	
Plate Offse	ets (X,Y)	[2:0-1-15,0-1-8], [3:0-3-0,	,0-3-0], [6:0-3-0	,0-2-0], [8:0-3-0,	,0-2-0], [10:0-3-0,0-3-0], [1	2:0-4-0,0-3-0],	[15:0-6-0,0-3-0]		
LOADING	(psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	I/defl L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC 0.7	77 Vert(LL)	0.26 15-18	>388 240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC 0.9	90 Vert(CT)	-0.50 12-13	>790 180		
BCLL	0.0 *	Rep Stress Incr	YES	WB 0.6	69 Horz(CT)	0.23 11	n/a n/a		
BCDL	10.0	Code FBC2020/T	PI2014	Matrix-MS	S			Weight: 217 lb	FT = 20%

**BRACING-**

TOP CHORD

**BOT CHORD** 

WFBS

24-8-8

32-6-0

1 Row at midpt

41<sub>-</sub>3<sub>-</sub>0

Structural wood sheathing directly applied or 2-11-12 oc purlins.

5-15 7-14

Rigid ceiling directly applied or 4-2-10 oc bracing.

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No 2 **BOT CHORD** 2x4 SP No 2

2x4 SP No.3 WFBS

(size) 2=0-3-8, 15=0-3-8, 11=0-3-8

Max Horz 2=154(LC 16)

8-3-12

Max Uplift 2=-630(LC 24), 15=-474(LC 12), 11=-243(LC 13) Max Grav 2=102(LC 13), 15=2615(LC 1), 11=987(LC 1)

8-5-8

16-8-8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-271/1709, 3-4=-282/1915, 4-5=-250/1945, 5-6=-529/176, 6-7=-418/184,

7-8=-1446/343, 8-9=-1710/356, 9-10=-2842/626, 10-11=-3223/773

BOT CHORD  $2 - 15 = -1461/387,\ 14 - 15 = -631/292,\ 13 - 14 = -108/904,\ 12 - 13 = -348/2210,\ 11 - 12 = -651/2930$ 3-15=-295/221, 5-15=-2306/408, 5-14=-121/1151, 7-14=-992/209, 7-13=-144/856, **WEBS** 

8-13=-72/530, 9-13=-715/316, 9-12=-133/634, 10-12=-363/242

# NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -2-0-0 to 2-1-8, Interior(1) 2-1-8 to 16-8-8, Exterior(2R) 16-8-8 to 22-6-8, Interior(1) 22-6-8 to 24-6-8, Exterior(2R) 24-6-8 to 30-4-8, Interior(1) 30-4-8 to 41-3-0 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) Provide adequate drainage to prevent water ponding.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Bearing at joint(s) 11 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=630, 15=474, 11=243,

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Philip J. O'Regan PE No.58126 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

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ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Chesterfield, MO 63017

NORRIS CONST. - HERITAGE Job Truss Truss Type Qty Ply T30144179 3458616 T15 diH Job Reference (optional) Builders FirstSource (Lake City,FL), Lake City, FL - 32055, 8.530 s Mar 9 2023 MiTek Industries, Inc. Fri Mar 24 07:25:40 2023 Page 1 ID:YPmxjjoROp4\_h6tHh\_ArnIzVUvx-eLRjYAeNvWladxgy3PjL1?ZCohuU5coUsEFzeszXo4v

3-10-0

24-8-8

29-5-0

35-1-15

5-8-15

18-8-8

3-8-8

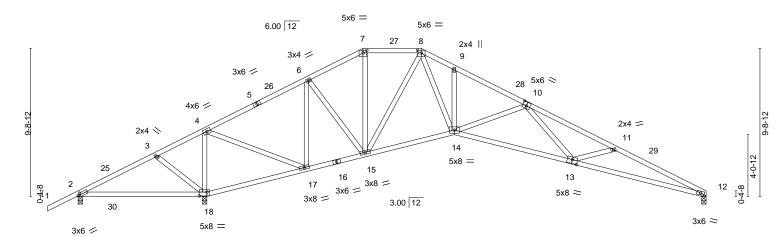
18-8-8

15-0-0

6-6-8

Scale = 1:75.7

6-1-1



	'	8-3-12 0-1	-12 6-6-8	3-	-8-8	6-0-0		7-9-8	'	8-9-0	<u>'</u>
Plate Off	sets (X,Y)	[2:0-1-15,0-1-8], [7:0-3-0	,0-2-0], [8:0-3-0	0,0-2-0], [10:0-3-0	0,0-3-0], [	13:0-4-0,0-3-0], [1	8:0-6-0,0-3-0]				
LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in (loc)	I/defI	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC 0.	78	Vert(LL)	0.26 18-21	>396	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC 0.9	91	Vert(CT)	-0.52 13-14	>755	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB 0.9	99	Horz(CT)	0.23 12	n/a	n/a		
BCDL	10.0	Code FBC2020/T	PI2014	Matrix-M	S	` '				Weight: 227 lb	FT = 20%

LUMBER-TOP CHORD BOT CHORD

WFBS

2x4 SP No 2 2x4 SP No 2 2x4 SP No.3 **BRACING-**

24-8-8

TOP CHORD **BOT CHORD**  Structural wood sheathing directly applied or 3-0-5 oc purlins.

41-3-0

Rigid ceiling directly applied or 4-2-6 oc bracing.

32-6-0

REACTIONS. (size) 2=0-3-8, 18=0-3-8, 12=0-3-8

Max Horz 2=169(LC 16)

8-3-12

Max Uplift 2=-595(LC 24), 18=-475(LC 12), 12=-241(LC 13) Max Grav 2=93(LC 13), 18=2600(LC 1), 12=990(LC 1)

8-5-8

15-0-0

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-253/1686, 3-4=-265/1879, 4-6=-364/113, 6-7=-662/235, 7-8=-538/226, 8-9=-1692/411, 9-10=-1728/351, 10-11=-2852/613, 11-12=-3247/774

BOT CHORD 2-18=-1439/386, 17-18=-1777/454, 15-17=-45/270, 14-15=-28/987, 13-14=-334/2236,

12-13=-653/2954

WEBS 3-18=-255/192, 4-18=-1863/393, 4-17=-271/1959, 6-17=-1064/225, 6-15=-75/711, 8-15=-863/173, 8-14=-276/1446, 10-14=-718/312, 10-13=-123/624, 11-13=-375/251

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -2-0-0 to 2-1-8, Interior(1) 2-1-8 to 18-8-8, Exterior(2E) 18-8-8 to 22-6-8, Exterior(2R) 22-6-8 to 28-4-8, Interior(1) 28-4-8 to 41-3-0 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) Provide adequate drainage to prevent water ponding.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Bearing at joint(s) 12 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=595, 18=475, 12=241.

This item has been electronically signed and sealed by ORegan, Philip, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Philip J. O'Regan PE No.58126 MITek Inc. DBA MITek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

March 27,2023





4-1-0

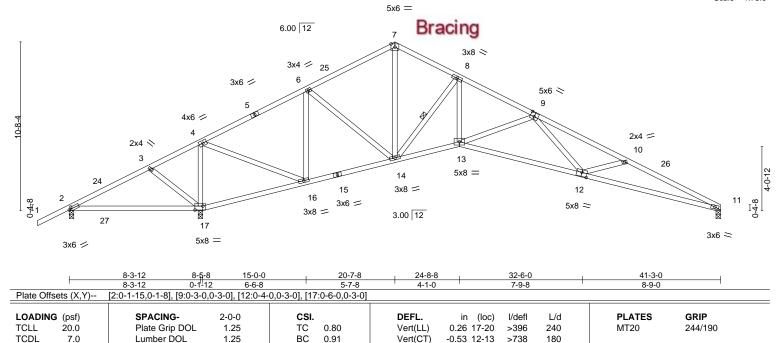
4-8-8

5-8-15

5-7-8

Scale = 1:73.0

6-1-1



LUMBER-

**BCLL** 

BCDL

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No 2 WFBS

0.0

10.0

2x4 SP No 3

**BRACING-**

Horz(CT)

0.23

11

n/a

TOP CHORD **BOT CHORD** WFBS

Structural wood sheathing directly applied or 3-0-2 oc purlins.

Weight: 220 lb

FT = 20%

Rigid ceiling directly applied or 4-0-10 oc bracing. 1 Row at midpt 8-14

n/a

REACTIONS. (size) 2=0-3-8, 17=0-3-8, 11=0-3-8

Max Horz 2=183(LC 12)

Rep Stress Incr

Code FBC2020/TPI2014

Max Uplift 2=-588(LC 24), 17=-483(LC 12), 11=-238(LC 13) Max Grav 2=89(LC 13), 17=2632(LC 1), 11=984(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-253/1740, 3-4=-287/1935, 4-6=-334/108, 6-7=-789/245, 7-8=-760/255, TOP CHORD

8-9=-1703/347, 9-10=-2826/599, 10-11=-3224/762

 $2-17 = -1486/392,\ 16-17 = -1831/461,\ 14-16 = -91/250,\ 13-14 = -104/1520,\ 12-13 = -315/2205,$ BOT CHORD

YES

6-6-8

11-12=-642/2933

WEBS 3-17=-262/198, 4-17=-1879/397, 4-16=-278/1995, 6-16=-1062/231, 6-14=-75/696, 7-14=-109/416, 8-14=-1360/285, 8-13=-158/1263, 9-13=-707/305, 9-12=-124/631,

10-12=-383/253

# NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -2-0-0 to 2-1-8, Interior(1) 2-1-8 to 20-7-8, Exterior(2R) 20-7-8 to 24-8-8, Interior(1) 24-8-8 to 41-3-0 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

WB

Matrix-MS

0.76

- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Bearing at joint(s) 11 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=588, 17=483, 11=238.

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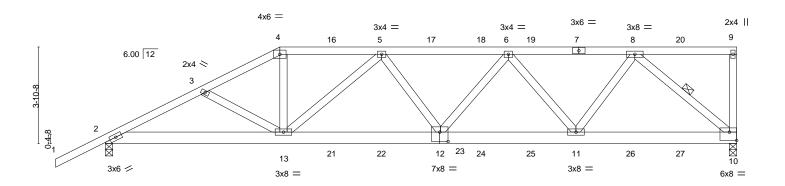
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Job	Truss	Truss Type	Qty	Ply	NORRIS CONST HERITAG	GE .	
							T30144181
3458616	T17	Half Hip Girder	1	1			
					Job Reference (optional)		
Builders FirstSource (Lake C	City,FL), Lake City, FL - 3	2055,		8.530 s M	far 9 2023 MiTek Industries, In-	c. Fri Mar 24 07:25:44 2023	Page 1
			ID:YPmxjjo	ROp4_h6tHl	h_ArnIzVUvx-X6gEOXhtzIF?5Y	zjlEnHBrjvmllm1Wc4msDB	ndzXo4r
-2-0-0	3-11-15 7-0-0	11-1-1	16-2-1	1	21-3-0	25-4-0	ı
2-0-0	3-11-15 3-0-1	1-1-1	5-1-0		5-0-15	/ <sub>-1-0</sub>	

Scale = 1:46.2



	<u> </u>	7-0-0 7-0-0			13-5-6 6-5-6		18-10- 5-5-3		-	25-4-0 6-5-6	
Plate Offs	sets (X,Y)	[10:Edge,0-4-0], [12:0-4-0	,0-4-8]		0-5-6		<u> </u>	1		0-5-0	
		00.400.0				5551	: (1 )	./	1./1	DI 4750	anın
LOADING	(pst)	SPACING-	2-0-0	CSI.		DEFL.	in (loc)	I/defI	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.66	Vert(LL)	-0.14 12-13	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	ВС	0.76	Vert(CT)	-0.29 12-13	>999	180		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.55	Horz(CT)	0.08 10	n/a	n/a		
BCDL	10.0	Code FBC2020/TF	PI2014	Matrix	c-MS					Weight: 155 lb	FT = 20%

BRACING-

TOP CHORD

**BOT CHORD** 

**WEBS** 

LUMBER-TOP CHORD

2x4 SP No 2 2x6 SP No 2

BOT CHORD 2x4 SP No.3 WFBS

REACTIONS. (size) 10=0-3-8, 2=0-3-8

Max Horz 2=151(LC 27)

Max Uplift 10=-565(LC 5), 2=-482(LC 8) Max Grav 10=2020(LC 1), 2=1807(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-3423/890, 3-4=-3250/841, 4-5=-2926/783, 5-6=-3530/890, 6-8=-2662/659,

9-10=-253/128

2-13=-870/3025, 12-13=-960/3521, 11-12=-885/3311, 10-11=-524/1887 BOT CHORD

**WEBS** 4-13=-210/1115, 5-13=-844/297, 6-12=-41/419, 6-11=-1060/380, 8-11=-252/1385,

8-10=-2488/695

# NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) Provide adequate drainage to prevent water ponding.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb)
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 125 lb down and 88 lb up at 7-0-0, 106 lb down and 88 lb up at 9-0-12, 106 lb down and 88 lb up at 11-0-12, 106 lb down and 88 lb up at 13-0-12, 106 lb down and 88 lb up at 15-0-12, 106 lb down and 88 lb up at 17-0-12, 106 lb down and 88 lb up at 19-0-12, 106 lb down and 88 lb up at 21-0-12, and 106 lb down and 88 lb up at 23-0-12, and 130 lb down and 86 lb up at 25-2-4 on top chord, and 293 lb down and 90 lb up at 7-0-0, 85 lb down at 9-0-12, 85 lb down at 11-0-12, 85 lb down at 13-0-12, 85 lb down at 15-0-12, 85 lb down at 17-0-12, 85 lb down at 15-0-12, 85 lb down lb down at 19-0-12, and 85 lb down at 21-0-12, and 85 lb down at 23-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

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Philip J. O'Regan PE No.58126 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

March 27,2023

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Structural wood sheathing directly applied or 2-8-2 oc purlins,

8-10

Rigid ceiling directly applied or 7-6-14 oc bracing.

except end verticals.

1 Row at midpt

16023 Swingley Ridge Rd Chesterfield, MO 63017

Job	Truss	Truss Type	Qty	Ply	NORRIS CONST HERITAGE
					T30144181
3458616	T17	Half Hip Girder	1	1	
					Job Reference (optional)

Builders FirstSource (Lake City,FL),

Lake City, FL - 32055,

8.530 s Mar 9 2023 MiTek Industries, Inc. Fri Mar 24 07:25:44 2023 Page 2 ID:YPmxjjoROp4\_h6tHh\_ArnIzVUvx-X6gEOXhtzIF?5YzjIEnHBrjvmIIm1Wc4msDBndzXo4r

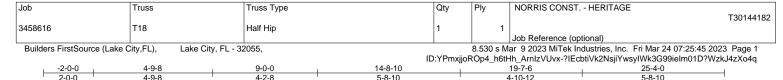
### LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-4=-54, 4-9=-54, 2-10=-20

Concentrated Loads (lb)

Vert: 4=-106(B) 7=-106(B) 9=-130(B) 13=-282(B) 5=-106(B) 11=-61(B) 8=-106(B) 16=-106(B) 17=-106(B) 18=-106(B) 19=-106(B) 20=-106(B) 21=-61(B) 22=-61(B) 23=-61(B) 25=-61(B) 25=-



Scale = 1:46.2

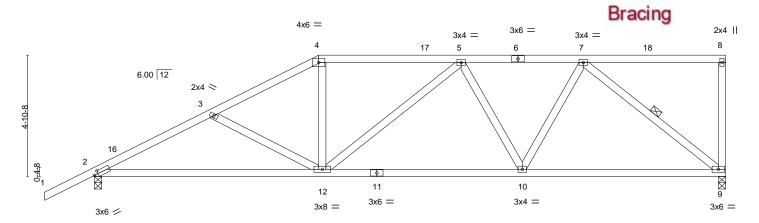


Plate Offse	oto (V V)	9-0-0 [2:0-1-15,0-1-8]		'		8-2-1				8-1-15	'
Flate Offse	515 (7,1)	[2.0-1-13,0-1-6]									
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in (loc)	I/defI	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.36	Vert(LL)	-0.14 12-15	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.74	Vert(CT)	-0.29 12-15	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.35	Horz(CT)	0.05 9	n/a	n/a		
BCDL	10.0	Code FBC2020/T	PI2014	Matri	x-MS					Weight: 136 lb	FT = 20%

17-2-1

**BRACING-**

TOP CHORD

**BOT CHORD** 

WEBS

LUMBER-

TOP CHORD 2x4 SP No 2 **BOT CHORD** 2x4 SP No 2

2x4 SP No.3 WFBS

REACTIONS. (size) 9=0-3-8, 2=0-3-8

Max Horz 2=185(LC 12)

Max Uplift 9=-239(LC 9), 2=-262(LC 12) Max Grav 9=928(LC 1), 2=1044(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

9-0-0

TOP CHORD 2-3=-1645/400, 3-4=-1392/325, 4-5=-1206/321, 5-7=-1123/266

**BOT CHORD** 2-12=-460/1442, 10-12=-321/1259, 9-10=-239/897

 $3-12=-280/157,\ 4-12=-24/399,\ 5-10=-288/164,\ 7-10=-105/484,\ 7-9=-1135/307$ WFBS

# NOTES-

9=239, 2=262.

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -2-0-0 to 1-0-0, Interior(1) 1-0-0 to 9-0-0, Exterior(2R) 9-0-0 to 13-2-15, Interior(1) 13-2-15 to 25-2-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) Provide adequate drainage to prevent water ponding.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members. 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb)

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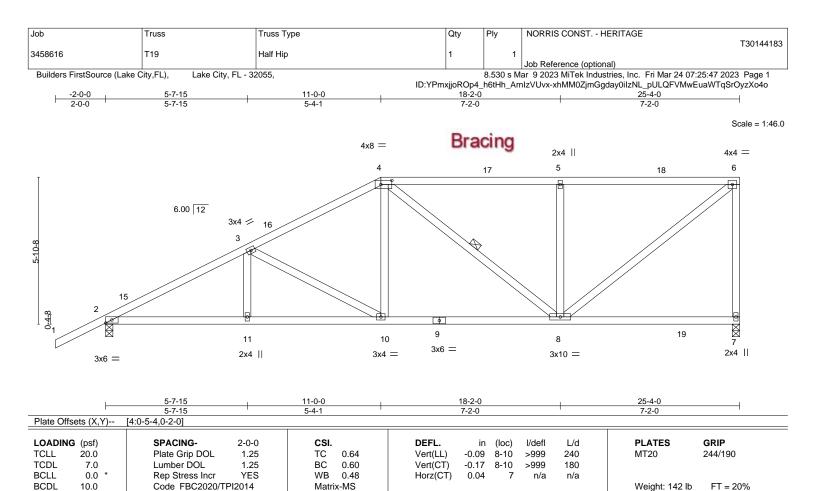
25-4-0

Structural wood sheathing directly applied or 4-5-11 oc purlins,

Rigid ceiling directly applied or 8-8-3 oc bracing.

except end verticals.

1 Row at midpt



**BRACING-**

TOP CHORD

**BOT CHORD** 

WEBS

LUMBER-

TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x4 SP No 2

2x4 SP No 3 WFBS

REACTIONS. (size) 7=0-3-8, 2=0-3-8

Max Horz 2=219(LC 12)

Max Uplift 7=-232(LC 9), 2=-261(LC 12) Max Grav 7=1039(LC 2), 2=1105(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 2-3=-1811/373, 3-4=-1352/302, 4-5=-997/223, 5-6=-997/223, 6-7=-906/249 TOP CHORD

BOT CHORD 2-11=-464/1578, 10-11=-464/1578, 8-10=-306/1171

WFBS 3-10=-485/180, 4-10=-48/474, 5-8=-448/217, 6-8=-279/1248

# NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -2-0-0 to 1-0-0, Interior(1) 1-0-0 to 11-0-0, Exterior(2R) 11-0-0 to 15-2-15, Interior(1) 15-2-15 to 25-2-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) Provide adequate drainage to prevent water ponding.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 7=232, 2=261.

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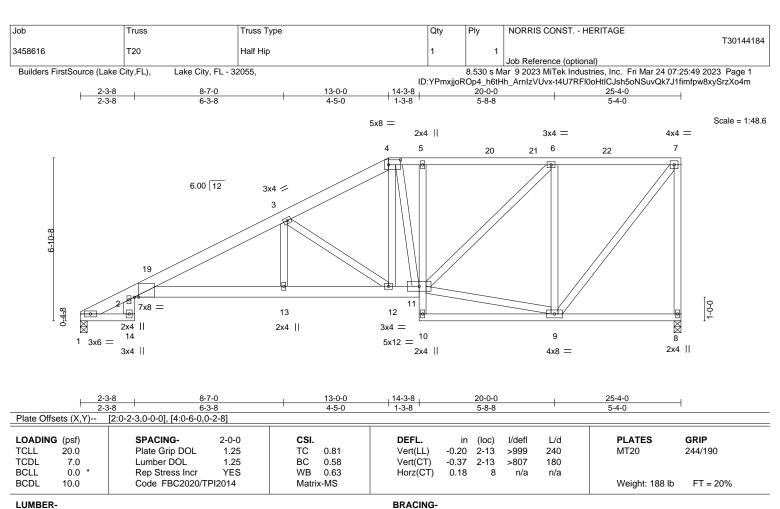


Structural wood sheathing directly applied or 4-2-9 oc purlins,

Rigid ceiling directly applied or 8-8-3 oc bracing.

except end verticals.

1 Row at midpt



TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x6 SP M 26 \*Except\* 4-7: 2x4 SP No.2

**BOT CHORD** 2x4 SP No.2 \*Except\*

2-14: 2x6 SP No.2, 2-11: 2x6 SP M 26, 5-10: 2x4 SP No.3

**WEBS** 2x4 SP No.3

REACTIONS. (size) 1=0-3-8, 8=0-3-8

Max Horz 1=225(LC 12)

Max Uplift 1=-210(LC 12), 8=-225(LC 9) Max Grav 1=926(LC 1), 8=926(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD  $2\text{-}18\text{=-}665/9, 2\text{-}3\text{=-}2055/525, 3\text{-}4\text{=-}1246/333, 4\text{-}5\text{=-}1040/316, 5\text{-}6\text{=-}1040/317,}$ 

6-7=-601/149, 7-8=-883/236

BOT CHORD 2-14=-124/356, 2-13=-619/1840, 12-13=-619/1840, 11-12=-316/1033 **WEBS** 3-13=-70/441, 3-12=-1001/374, 4-12=-142/502, 9-11=-147/536, 6-11=-231/602,

6-9=-749/259, 7-9=-243/970

# NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-1-12 to 3-1-12, Interior(1) 3-1-12 to 13-0-0, Exterior(2R) 13-0-0 to 17-2-15, Interior(1) 17-2-15 to 25-2-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) Provide adequate drainage to prevent water ponding.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=210, 8=225,

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Structural wood sheathing directly applied or 5-6-6 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

Philip J. O'Regan PE No.58126 MITek Inc. DBA MITek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

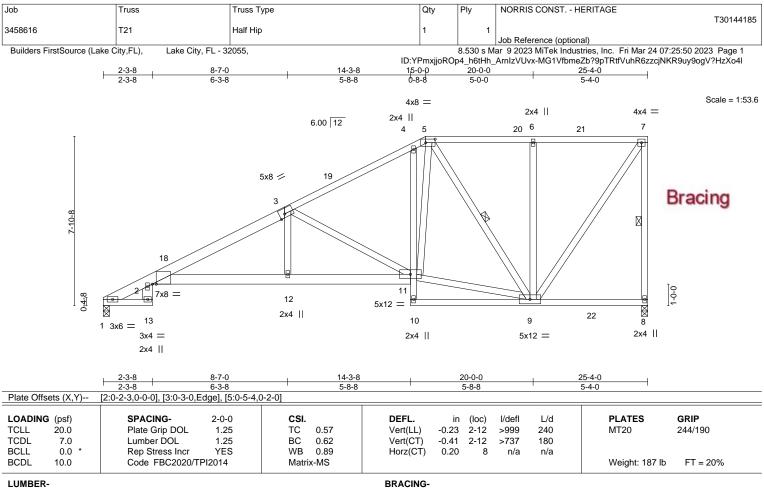
March 27,2023

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TOP CHORD

**BOT CHORD** 

**WEBS** 

LUMBER-

TOP CHORD 2x4 SP No.2 \*Except\* 1-3: 2x6 SP M 26

**BOT CHORD** 2x4 SP No.2 \*Except\*

2-13: 2x6 SP No.2, 2-11: 2x6 SP M 26, 4-10: 2x4 SP No.3

**WEBS** 2x4 SP No.3

REACTIONS. (size) 8=0-3-8, 1=0-3-8

Max Horz 1=259(LC 12)

Max Uplift 8=-221(LC 12), 1=-203(LC 12) Max Grav 8=1025(LC 2), 1=991(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-17=-708/0, 2-3=-2104/497, 3-4=-1169/287, 4-5=-1069/338, 5-6=-578/147,

6-7=-581/149, 7-8=-919/233

BOT CHORD 2-13=-129/387, 2-12=-624/1885, 11-12=-626/1908

**WEBS** 3-12=-55/506, 3-11=-1051/366, 9-11=-240/789, 5-11=-293/829, 6-9=-318/155,

7-9=-259/1002, 5-9=-592/231

# NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-1-12 to 3-1-12, Interior(1) 3-1-12 to 15-0-0, Exterior(2R) 15-0-0 to 19-2-15, Interior(1) 19-2-15 to 25-2-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) Provide adequate drainage to prevent water ponding.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 8=221, 1=203,

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Structural wood sheathing directly applied or 4-11-9 oc purlins,

7-8, 5-9

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

1 Row at midpt

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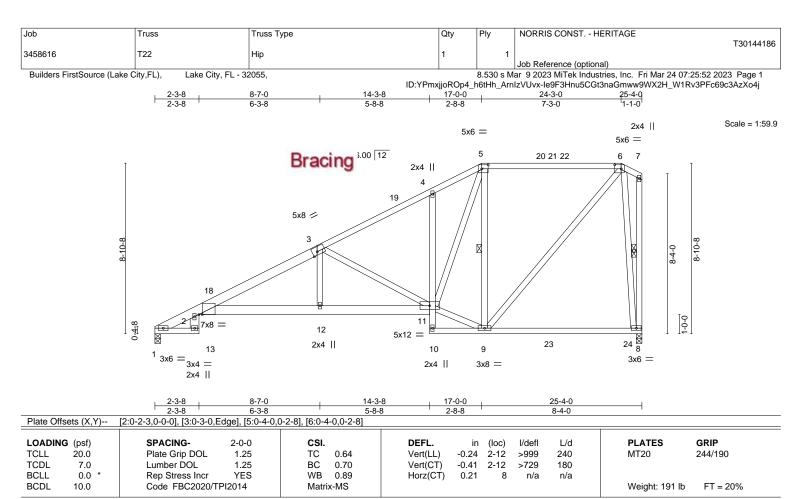
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**BRACING-**

TOP CHORD

BOT CHORD

WEBS

LUMBER-

TOP CHORD 2x4 SP No.2 \*Except\* 1-3: 2x6 SP M 26

BOT CHORD 2x4 SP No.2 \*Except\*

2-13: 2x6 SP No.2, 2-11: 2x6 SP M 26, 4-10: 2x4 SP No.3

**WEBS** 2x4 SP No.3

REACTIONS. (size) 1=0-3-8, 8=0-3-8

Max Horz 1=284(LC 12)

Max Uplift 1=-197(LC 12), 8=-237(LC 12) Max Grav 1=996(LC 2), 8=1044(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-17=-711/0, 2-3=-2117/489, 3-4=-1183/275, 4-5=-1117/339, 5-6=-680/201

BOT CHORD 2-13=-132/389, 2-12=-642/1897, 11-12=-645/1920

WEBS 3-12=-58/505, 3-11=-1049/371, 9-11=-154/690, 5-11=-356/910, 5-9=-664/302,

6-9=-251/886, 6-8=-957/303

### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-1-12 to 3-1-12, Interior(1) 3-1-12 to 17-0-0, Exterior(2R) 17-0-0 to 21-2-15, Interior(1) 21-2-15 to 24-3-0, Exterior(2E) 24-3-0 to 25-2-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) Provide adequate drainage to prevent water ponding.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=197, 8=237.

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Structural wood sheathing directly applied or 4-11-5 oc purlins,

5-9, 7-8, 6-8

Rigid ceiling directly applied or 10-0-0 oc bracing, Except:

except end verticals.

1 Row at midpt

6-0-0 oc bracing: 10-11,9-10.

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March 27,2023

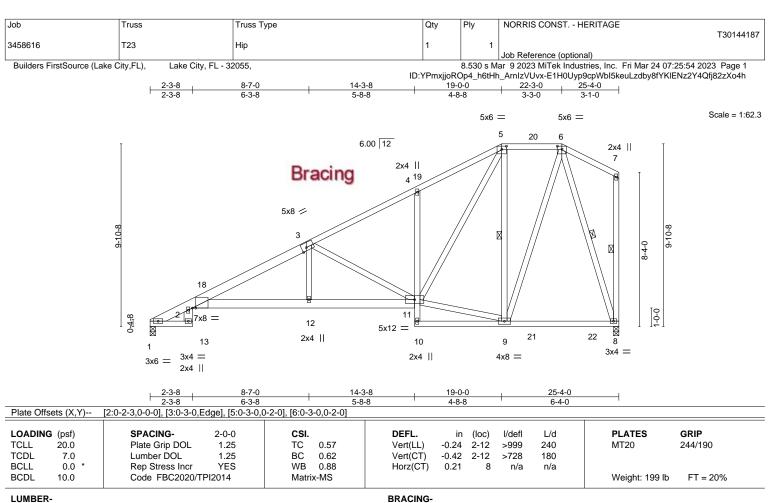


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TOP CHORD

**BOT CHORD** 

**WEBS** 

LUMBER-

TOP CHORD 2x4 SP No.2 \*Except\* 1-3: 2x6 SP M 26

**BOT CHORD** 2x4 SP No.2 \*Except\*

2-13: 2x6 SP No.2, 2-11: 2x6 SP M 26, 4-10: 2x4 SP No.3

**WEBS** 2x4 SP No.3

REACTIONS. (size) 1=0-3-8, 8=0-3-8

Max Horz 1=299(LC 12)

Max Uplift 1=-192(LC 12), 8=-242(LC 12) Max Grav 1=995(LC 2), 8=1032(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-17=-711/0, 2-3=-2112/477, 3-4=-1183/266, 4-5=-1161/364, 5-6=-497/166 BOT CHORD 2-13=-134/389, 2-12=-646/1891, 11-12=-648/1914, 4-11=-281/186, 8-9=-72/269

3-12=-59/508, 3-11=-1038/365, 9-11=-109/432, 5-11=-382/1017, 5-9=-598/268, 6-9=-214/767, 6-8=-895/246

### NOTES-

WEBS

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-1-12 to 3-1-12, Interior(1) 3-1-12 to 19-0-0, Exterior(2E) 19-0-0 to 25-2-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) Provide adequate drainage to prevent water ponding.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=192, 8=242.

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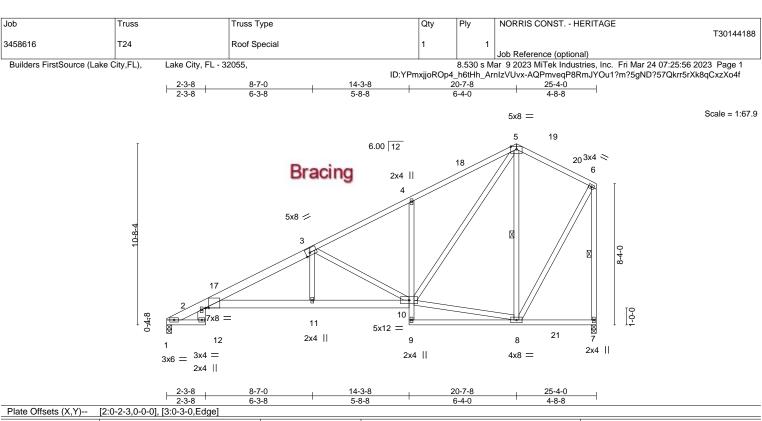
Structural wood sheathing directly applied or 4-11-14 oc purlins,

5-9, 7-8, 6-8

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

1 Row at midpt



**BRACING-**

TOP CHORD

**BOT CHORD** 

**WEBS** 

LOADING (psf) SPACING-CSI. DEFL. in (loc) I/defI L/d **TCLL** 20.0 1.25 TC 0.57 Vert(LL)

Plate Grip DOL TCDL Vert(CT) 7.0 Lumber DOL 1.25 BC 0.62 -0.42 2-11 WB **BCLL** 0.0 Rep Stress Incr YES 0.97 Horz(CT) 0.20 BCDL 10.0 Code FBC2020/TPI2014 Matrix-MS

**PLATES** -0.24 2-11 >999 240 MT20 244/190 >728 180 n/a n/a Weight: 189 lb

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

1 Row at midpt

Structural wood sheathing directly applied or 4-11-3 oc purlins,

5-8, 6-7

LUMBER-TOP CHORD 2x4 SP No.2 \*Except\* 1-3: 2x6 SP M 26

2x4 SP No.2 \*Except\*

2-12: 2x6 SP No.2, 2-10: 2x6 SP M 26, 4-9: 2x4 SP No.3

**WEBS** 2x4 SP No.3

BOT CHORD

REACTIONS. (size) 1=0-3-8, 7=0-3-8

Max Horz 1=311(LC 12)

Max Uplift 1=-187(LC 12), 7=-247(LC 12) Max Grav 1=991(LC 2), 7=1022(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD  $2\text{-}16\text{=-}707/0,\ 2\text{-}3\text{=-}2097/465,\ 3\text{-}4\text{=-}1173/258,\ 4\text{-}5\text{=-}1173/380,\ 5\text{-}6\text{=-}457/145,}$ 

6-7=-942/266

BOT CHORD 2-12=-134/387, 2-11=-646/1875, 10-11=-649/1899, 4-10=-340/221 **WEBS** 

3-11=-61/510, 3-10=-1027/357, 8-10=-77/258, 5-10=-420/1106, 5-8=-531/230,

### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-1-12 to 3-1-12, Interior(1) 3-1-12 to 20-7-8, Exterior(2R) 20-7-8 to 23-7-8, Interior(1) 23-7-8 to 25-2-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=187, 7=247.

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GRIP

FT = 20%

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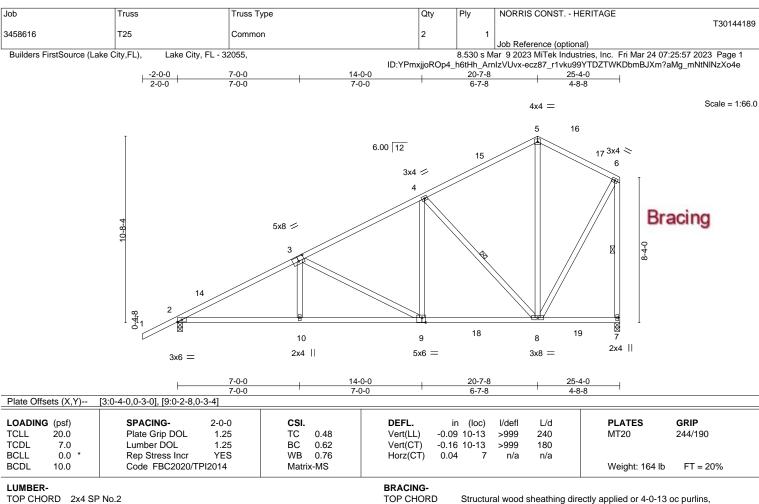
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**BOT CHORD** 

WEBS

except end verticals.

1 Row at midpt

Rigid ceiling directly applied or 8-2-4 oc bracing.

4-8, 6-7

2x4 SP No.2 **BOT CHORD** 2x4 SP No 2 2x4 SP No 3 WFBS

REACTIONS. (size) 2=0-3-8, 7=0-3-8

Max Horz 2=340(LC 12)

Max Uplift 2=-233(LC 12), 7=-245(LC 12) Max Grav 2=1106(LC 2), 7=1053(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 2-3=-1760/302, 3-4=-1139/204, 4-5=-511/144, 5-6=-477/147, 6-7=-970/266 TOP CHORD

BOT CHORD 2-10=-510/1526. 9-10=-509/1534. 8-9=-302/957

WFBS 3-10=0/290, 3-9=-644/230, 4-9=-58/563, 4-8=-830/295, 6-8=-204/787

### NOTES-

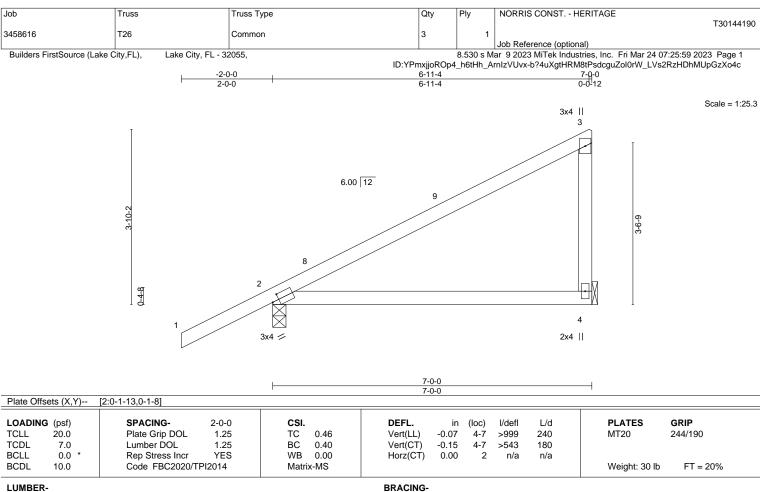
- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -2-0-0 to 1-0-0, Interior(1) 1-0-0 to 20-7-8, Exterior(2R) 20-7-8 to 23-7-8, Interior(1) 23-7-8 to 25-2-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=233, 7=245.

This item has been electronically signed and sealed by ORegan, Philip, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

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March 27,2023





TOP CHORD

BOT CHORD

LUMBER-TOP CHORD BOT CHORD

REACTIONS.

2x4 SP No.2 2x4 SP No 2

2x4 SP No 3 WFBS

(size) 2=0-3-8, 4=Mechanical Max Horz 2=147(LC 12)

Max Uplift 2=-88(LC 12), 4=-91(LC 12) Max Grav 2=377(LC 1), 4=238(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

### NOTES-

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -2-0-0 to 1-0-0, Interior(1) 1-0-0 to 6-10-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.

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Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

Philip J. O'Regan PE No.58126 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

March 27,2023

16023 Swingley Ridge Rd Chesterfield, MO 63017

NORRIS CONST. - HERITAGE Job Truss Truss Type Qty Ply T30144191 3458616 T26G Common Supported Gable Job Reference (optional) Builders FirstSource (Lake City,FL), Lake City, FL - 32055, 8.530 s Mar 9 2023 MiTek Industries, Inc. Fri Mar 24 07:26:00 2023 Page 1 ID:YPmxjjoROp4\_h6tHh\_ArnIzVUvx-3BeHl0twCfGk00BoEc41rDOjOlw7nuQRSL61LizXo4b 7-0-0 0-0-12 6-11-4 2-0-0 Scale = 1:22.9 2x4 || 6.00 12 2x4 || 4 3x4 / 0-4-8 9 8 4x8 / 2x4 || 2x4 || Plate Offsets (X,Y)-- [2:0-4-0,0-1-15]

LOADING	G (psf)	SPACING- 2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL 1.25	TC 0.	36	Vert(LL)	0.01	1	n/r	120	MT20	244/190
TCDL	7.0	Lumber DOL 1.25	BC 0.0	08	Vert(CT)	0.00	1	n/r	120		
BCLL	0.0 *	Rep Stress Incr YES	WB 0.0	05	Horz(CT)	-0.00	7	n/a	n/a		
BCDL	10.0	Code FBC2020/TPI2014	Matrix-S							Weight: 38 lb	FT = 20%

**BRACING-**

TOP CHORD

**BOT CHORD** 

2-0-0 oc purlins (6-0-0 max.), except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

LUMBER-

TOP CHORD 2x4 SP No.2 2x4 SP No.2

BOT CHORD 2x4 SP No.3 WFBS **OTHERS** 2x4 SP No.3

REACTIONS. All bearings 7-0-0.

(lb) -Max Horz 2=137(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 2, 7, 9, 8

Max Grav All reactions 250 lb or less at joint(s) 7, 9, 8 except 2=256(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

### NOTES-

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Corner(3E) -2-0-0 to 1-0-0, Exterior(2N) 1-0-0 to 6-10-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) Gable requires continuous bottom chord bearing.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 7, 9, 8.
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

This item has been electronically signed and sealed by ORegan, Philip, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Philip J. O'Regan PE No.58126 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

March 27,2023

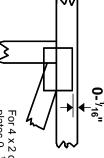


### Symbols

## PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated.
Dimensions are in ft-in-sixteenths.
Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- <sup>1</sup>/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

\* Plate location details available in MiTek 20/20 software or upon request.

### PLATE SIZE

4 × 4

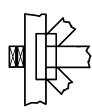
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

## LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

### **BEARING**



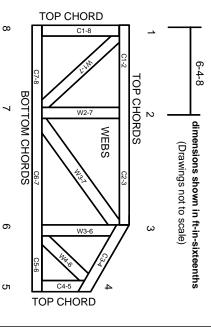
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

### Industry Standards:

National Design Specification for Metal Plate Connected Wood Truss Construction. Design Standard for Bracing.
Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

ANSI/TPI1: DSB-89:

## **Numbering System**



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

## PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988 ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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MiTek Engineering Reference Sheet: MII-7473 rev. 5/19/2020

# **General Safety Notes**

# Failure to Follow Could Cause Property Damage or Personal Injury

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.

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- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- Cut members to bear tightly against each other.

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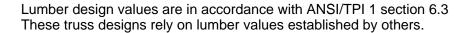
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.

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Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber

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- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- 15. Connections not shown are the responsibility of others
- Do not cut or alter truss member or plate without prior approval of an engineer.
- 17. Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.
- 21.The design does not take into account any dynamic or other loads other than those expressly stated.





RE: 2718981 - DETAILS

MiTek USA, Inc. 6904 Parke East Blvd. Tampa, FL 33610-4115

Site Information:

Customer Info: DETAILS Project Name: N/A Model: N/A Lot/Block: N/A Subdivision: N/A

Address: N/A, N/A

City: N/A State: N/A

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name: License #:

Address:

City: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2020/TPI2014 Design Program: MiTek 20/20 8.4

Wind Code: ASCE 7-16 Wind Speed: 130 mph Roof Load: 37.0 psf Floor Load: N/A psf

This package includes 20 individual, General Truss Details and 0 Additional Drawings. With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No.	Seal#	Detail Name	Date	No.	Seal#	Detail Name	Date
1	T23399806	MII-REP10	4/2/21	15	T23399820	MII-VALLEY HIGH WIND2	., _,
2	T23399807 T23399808	MII-T-BRACE 2 MII-SCAB-BRACE	4/2/21 4/2/21	16 17	T23399821 T23399822	MII-VALLEY SP MII-VALLEY SP	4/2/21 4/2/21
4	T23399809	MII-REP05	4/2/21	18	T23399823	MII-VALLE 1 31 MII-GE146-001	4/2/21
5	T23399810	MII-GE130-D-SP	4/2/21	19	T23399824	MII-REP13B	4/2/21
6	T23399811	MII-GE130-SP	4/2/21	20	T23399825	MII-STRGBCK	4/2/21
8	T23399812 T23399813	MII-GE140-001 MII-GE170-D-SP	4/2/21 4/2/21				
9	T23399814	MII-GE180-D-SP	4/2/21				
10	T23399815	MII-GE180-D-SP	4/2/21				
11	T23399816	MII-PIGGY-ALT-7-16 MII-REP01A1	4/2/21				
12 13	T23399817 T23399818	MII-REPUTAT MII-TOENAIL SP	4/2/21 4/2/21				
14	T23399819	MII-VALLEY HIGH WIND1					

This item has been electronically signed and sealed by ORegan, Philip, PE using a Digital Signature.

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The truss drawing(s) referenced above have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Builders FirstSource-Jacksonville.

Truss Design Engineer's Name: ORegan, Philip

My license renewal date for the state of Florida is February 28, 2023.

**IMPORTANT NOTE:** The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:



RE: \$JOBNAME - \$JOBDESC

MiTek USA, Inc. 6904 Parke East Blvd. Tampa, FL 33610-4115

### **Site Information:**

Customer Info: \$SI\_CUSTOMER Project Name: \$SI\_JOBNAME Model: \$SI\_MODEL Lot/Block: \$SI\_LOTNUM Subdivision: \$SI\_SUBDIV Address: \$SI\_SITEADDR

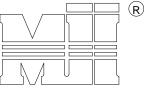
City: \$SI\_SITECITY State: \$SI\_SITESTATE OCTOBER 28, 2016

### STANDARD REPAIR FOR ADDING A FALSE BOTTOM CHORD

MII-REP10 T23399806

MiTek USA, Inc.

Page 1 of 1



MAIN TRUSS MANUFACTURED WITHOUT FALSE BOTTOM CHORD.

MAIN TRUSS (SPACING = 24" O.C.)

MiTek USA, Inc. REFER TO THE BOTTOM CHORD BRACING SECTION OF THE INDIVIDUAL TRUSS DESIGN FOR MAXIMUM SPACING OF CONTINUOUS LATERAL BRACING WHENEVER RIGID CEILING MATERIAL IS NOT DIRECTLY ATTACHED TO THE VERTICAL STUDS @ 48" O.C.. ATTACHED BOTTOM CHORD. WITH (3) - 10d (0.131" X 3") NAILS AT EACH END OF VERTICAL (TYP.). VERTICAL STUDS TO BE 2 x 4 STUD GRADE (OR BETTER) SPF, HF, DF OR SP. (BOARD SIZÉ SPECIFIED IS MINIMUM, LARGER SIZE MAY BE USED) 2 x 4 NO. 2 (OR BETTER) SPF, HF DF OR SP FALSE BOTTOM CHORD (BOARD SIZE SPECIFIED IS MINIMUM, LARGER SIZE MAY BE USED) **FALSE BOTTOM** TRUSS SPAN

### NOTES:

- 1. LOADING: TOP CHORD: (REFER TO THE MAIN TRUSS DESIGN FOR TOP CHORD LOADING). BOTTOM CHORD: LL = 0 PSF, DL = 10 PSF.
- 2. REFER TO THE MAIN TRUSS DESIGN FOR LUMBER AND PLATING REQUIREMENTS.
- 3. MAXIMUM BOTTOM CHORD PITCH = 6/12.
- 4. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID SPLITTING OF THE WOOD.
- 5. FALSE BOTTOM CHORD ONLY DESIGNED TO CARRY VERTICAL LOAD. NO LATERAL (SHEAR) LOAD ALLOWED.
- 6. FILLER MAY EXTEND FOR FULL LENGTH OF TRUSS.

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### T-BRACE / I-BRACE DETAIL WITH 2X BRACE ONLY

### MII-T-BRACE 2 T23399807

MiTek USA, Inc.

Page 1 of 1



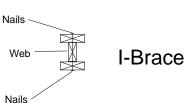
Note: T-Bracing / I-Bracing to be used when continuous lateral bracing is impractical. T-Brace / I-Brace must cover 90% of web length.

Note: This detail NOT to be used to convert T-Brace / I-Brace webs to continuous lateral braced webs.

Nailing Pattern							
T-Brace size Nail Size Nail Spacing							
2x4 or 2x6 or 2x8	10d (0.131" X 3")	6" o.c.					

Note: Nail along entire length of T-Brace / I-Brace (On Two-Ply's Nail to Both Plies)

	Nails
WEB	SPACING
	T-BRACE
Nails	Section Detail  T-Brace  Web



	Brace Size for One-Ply Truss				
	Specified Continuous Rows of Lateral Bracing				
Web Size	1	2			
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace			
2x6	2x6 T-Brace	2x6 I-Brace			
2x8	2x8 T-Brace 2x8 I-Brace				

	Brace Size for Two-Ply Truss			
	Specified Continuous Rows of Lateral Bracing			
Web Size	1	2		
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace		
2x6	2x6 T-Brace 2x6 I-Brace			
2x8	2x8 T-Brace	2x8 I-Brace		

T-Brace / I-Brace must be same species and grade (or better) as web member.

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Philip J. O'Regan PE No.58126 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

April 2,2021

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



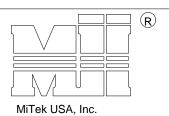
6904 Parke East Blvd. Tampa, FL 36610

### SCAB-BRACE DETAIL

### MII-SCAB-BRACE T23399808

MiTek USA, Inc.

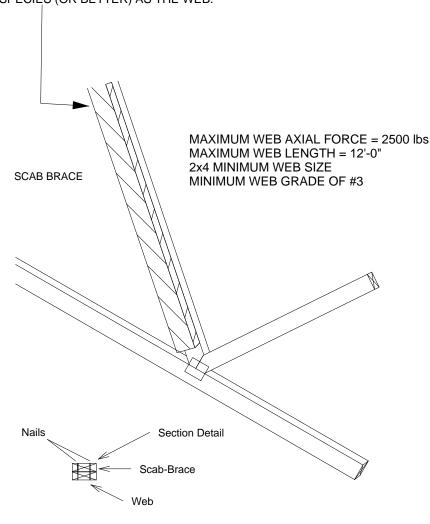
Page 1 of 1



Note: Scab-Bracing to be used when continuous lateral bracing at midpoint (or T-Brace) is impractical. Scab must cover full length of web +/- 6".

\*\*\* THIS DETAIL IS NOT APLICABLE WHEN BRACING IS \*\*\* REQUIRED AT 1/3 POINTS OR I-BRACE IS SPECIFIED.

SCAB TO ONE FACE OF WEB WITH APPLY 2x 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 6" O.C. SCAB MUST BE THE SAME GRADE, SIZE AND SPECIES (OR BETTER) AS THE WEB.



Scab-Brace must be same species grade (or better) as web member.

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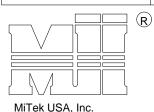
Philip J. O'Regan PE No.58126 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610



### STANDARD REPAIR TO REMOVE END **VERTICAL (RIBBON NOTCH VERTICAL)**

### MII-REP05 T23399809

MiTek USA, Inc. Page 1 of 1

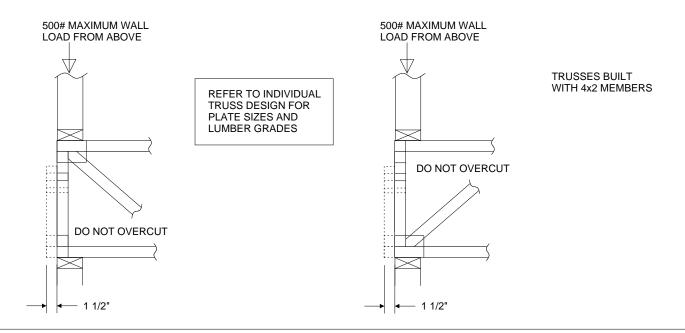


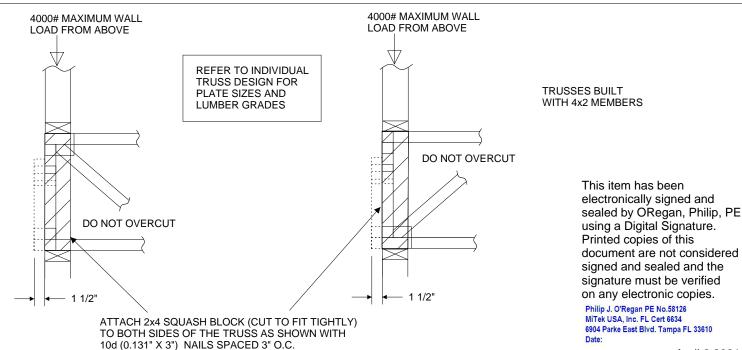
- 1. THIS IS A SPECIFIC REPAIR DETAIL TO BE USED ONLY FOR ITS ORIGINAL INTENTION. THIS REPAIR DOES NOT IMPLY THAT THE REMAINING PORTION OF THE TRUSS IS UNDAMAGED. THE ENTIRE TRUSS SHALL BE INSPECTED TO VERIFY THAT NO FURTHER REPAIRS ARE REQUIRED. WHEN THE REQUIRED REPAIRS ARE PROPERLY APPLIED, THE TRUSS WILL BE CAPABLE OF SUPPORTING THE LOADS INDICATED.
- 2. ALL MEMBERS MUST BE RETURNED TO THEIR ORIGINAL POSITIONS BEFORE
- APPLYING REPAIR AND HELD IN PLACE DURING APPLICATION OF REPAIR.

  3. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE
- SUCH AS TO AVOID SPLITTING OF THE WOOD.

  4. LUMBER MUST BE CUT CLEANLY AND ACCURATELY AND THE REMAINING WOOD MUST BE UNDAMAGED.

  5. THIS REPAIR IS TO BE USED FOR SINGLE PLY TRUSSES IN THE 4X\_ ORIENTATION ONLY.
- 6. CONNECTOR PLATES MUST BE FULLY IMBEDDED AND UNDISTURBED.





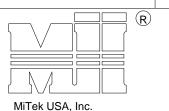


### Standard Gable End Detail

### MII-GE130-D-SP T23399810

Page 1 of 2

MiTek USA. Inc.



Typical \_x4 L-Brace Nailed To 2x\_ Verticals W/10d Nails spaced 6" o.c. Vertical Stud SECTION B-B

DIAGONAL BRACE 4'-0" O.C. MAX TRUSS GEOMETRY AND CONDITIONS SHOWN ARE FOR ILLUSTRATION ONLY. 12 Varies to Common Truss SEE INDIVIDUAL MITEK ENGINEERING DRAWINGS FOR DESIGN CRITERIA \*\* 3x4 = - Diagonal Bracing - L-Bracing Refer \*\* Refer to Section A-A to Section B-B

Vertical Stud DIAGONAL (4) - 16d Nails 16d Nails Spaced 6" o.c. (2) - 10d Nails into 2x6 2x6 Stud or 2x4 No.2 of better Typical Horizontal Brace Nailed To 2x\_ Verticals w/(4)-10d Nails **SECTION A-A** 2x4 Stud

> PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SPF BLOCK

### Roof Sheathing

24" Max

if needed

**End Wall** 

### 1'-3" (2) - 10d/ Max. **NAILS** (2) - 10d NAILS

2x6 DIAGONAL BRACE SPACED 48" O.C. Diag. Brace ATTACHED TO VERTICAL WITH (4) -16d at 1/3 points NAILS AND ATTACHED

TO BLOCKING WITH (5) - 10d NAILS.

HORIZONTAL BRACE (SEE SECTION A-A)

### NOTE:

- 1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS. 2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND
- WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
- 3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
- 4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 1x4 SRB OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C. 5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF
- DIAPHRAM AT 4'-0" O.C.
- 6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
- 7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
- 8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES
- DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.
- 10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC
- NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Minimum Stud Size Species	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
and Grade			Maximur	n Stud Ler	ngth	
2x4 SP No. 3 / Stud	12" O.C.	3-9-13	4-1-1	5-9-6	7-1-3	11-5-7
2x4 SP No. 3 / Stud	16" O.C.	3-5-4	3-6-8	5-0-2	6-10-8	10-3-13
2x4 SP No. 3 / Stud	24" O.C.	2-9-11	2-10-11	4-1-1	5-7-6	8-5-1

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING EXPOSURE D

ASCE 7-10, ASCE 7-10 130 MPH

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING.

DURANION OF COAD INCREASE LIFE AS OF READ NOT CONNECTION OF BRACTING IS BASED ON MY FIRST. 519/2020 BEFORE USE.

Design valid for use only with MITEK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

\*\*ANSITP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information\*\* available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



6904 Parke East Blvd

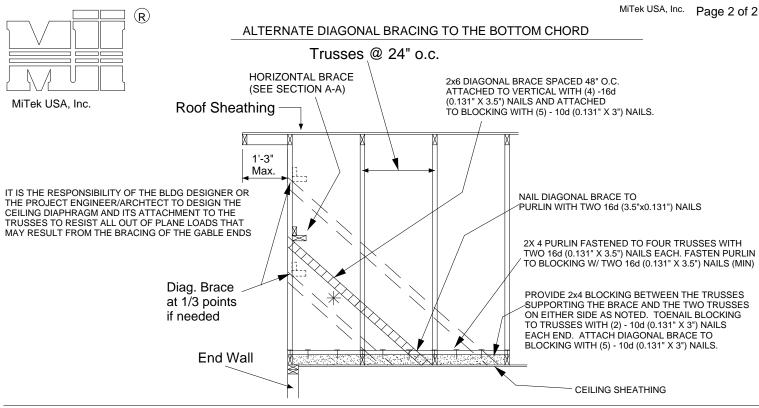
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Trusses @ 24" o.c.

Philip J. O'Regan PE No.58126 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

### Standard Gable End Detail

### MII-SHEET 2 T23399810



### BRACING REQUIREMENTS FOR STRUCTURAL GABLE TRUSSES

STRUCTURAL GABLE TRUSSES MAY BE BRACED AS NOTED:

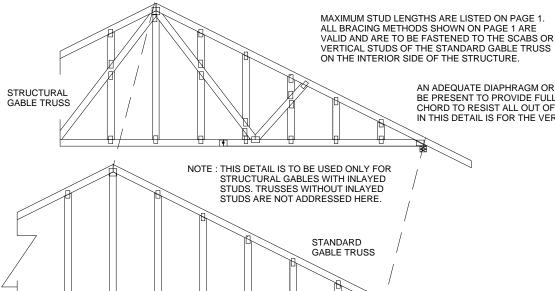
METHOD 1: ATTACH A MATCHING GABLE TRUSS TO THE INSIDE FACE OF THE STRUCTURAL GABLE AND FASTEN PER THE

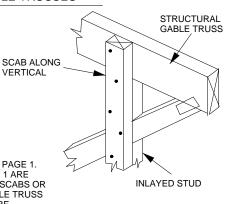
FOLLOWING NAILING SCHEDULE.

METHOD 2: ATTACH 2X\_SCABS TO THE FACE OF EACH VERTICAL
MEMBER ON THE STRUCTURAL GABLE PER THE FOLLOWING
NAILING SCHEDULE. SCABS ARE TO BE OF THE SAME SIZE, GRADE AND SPECIES AS THE TRUSS VERTICALS

NAILING SCHEDULE:

EING SCHEDEL:
- FOR WIND SPEEDS 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7-10, 16) OR LESS, NAIL ALL
- MEMBERS WITH ONE ROW OF 10d (0.131" X 3") NAILS SPACED 6" O.C.
- FOR WIND SPEEDS 120-150 MPH (ASCE 7-98, 02, 05), 150-190 MPH (ASCE 7-10, 16) NAIL ALL
- MEMBERS WITH TWO ROWS OF 10d (0.131" X 3") NAILS SPACED 6" O.C. (2X 4 STUDS MINIMUM)





AN ADEQUATE DIAPHRAGM OR OTHER METHOD OF BRACING MUST BE PRESENT TO PROVIDE FULL LATERAL SUPPORT OF THE BOTTOM CHORD TO RESIST ALL OUT OF PLANE LOADS. THE BRACING SHOWN IN THIS DETAIL IS FOR THE VERTICAL/STUDS ONLY.

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Philip J. O'Regan PE No.58126 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

April 2,2021

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ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

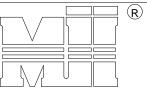


### Standard Gable End Detail

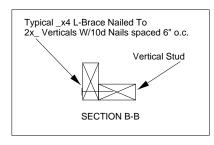
### MII-GE130-SP T23399811

Page 1 of 2

MiTek USA, Inc.



MiTek USA, Inc.



DIAGONAL BRACE TRUSS GEOMETRY AND CONDITIONS 4'-0" O.C. MAX SHOWN ARE FOR ILLUSTRATION ONLY. 12 Varies to Common Truss SEE INDIVIDUAL MITEK ENGINEERING DRAWINGS FOR DESIGN CRITERIA 3x4 =- Diagonal Bracing - L-Bracing Refer

to Section B-B

Vertical Stud DIAGONAL (4) - 16d Nails 16d Nails Spaced 6" o.c. (2) - 10d Nails into 2x6 2x6 Stud or 2x4 No.2 of better Typical Horizontal Brace Nailed To 2x\_ Verticals w/(4)-10d Nails **SECTION A-A** 2x4 Stud

> PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

(4) - 8d (0.131" X2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SPF BLOCK

### Roof Sheathing

24" Max



Trusses @ 24" o.c.

2x6 DIAGONAL BRACE SPACED 48" O.C. Diag. Brace ATTACHED TO VERTICAL WITH (4) -16d at 1/3 points NAILS AND ATTACHED

TO BLOCKING WITH (5) - 10d NAILS. if needed

HORIZONTAL BRACE **End Wall** (SEE SECTION A-A)

### NOTE:

Refer to Section A-A

- 1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS. 2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND
- WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT. 3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG.
- ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
- 4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 1x4 SRB
- OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C. 5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.
- 6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
- GABLE STUD DEFLECTIÓN MEETS OR EXCEEDS L/240.
- 8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
  9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.
- 10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.
- NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Minimum Stud Size Species	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
and Grade			Maximur	n Stud Ler	ngth	
2x4 SP No. 3 / Stud	12" O.C.	4-0-7	4-5-6	6-3-8	8-0-15	12-1-6
2x4 SP No. 3 / Stud	16" O.C.	3-8-0	3-10-4	5-5-6	7-4-1	11-0-1
2x4 SP No. 3 / Stud	24" O.C.	3-0-10	3-1-12	4-5-6	6-1-5	9-1-15

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING

ASCE 7-98, ASCE 7-02, ASCE 7-05 130 MPH

ASCE 7-36, ASCE 7-20 ISO MPH

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING.

DURANION OF GABUNGREASE 1914.60 READ NOT CONNECTION OF BRACTINGRS BASED ON MWFRS. 5/19/2020 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

\*\*ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information\*\* available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



6904 Parke East Blvd

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signature must be verified

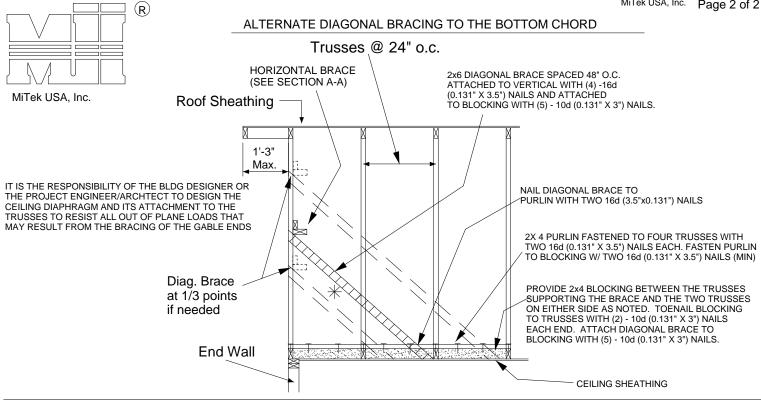
April 2,2021

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Philip J. O'Regan PE No.58126 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

EXPOSURE B or C

MiTek USA, Inc. Page 2 of 2



### BRACING REQUIREMENTS FOR STRUCTURAL GABLE TRUSSES

STRUCTURAL GABLE TRUSSES MAY BE BRACED AS NOTED: METHOD 1 : ATTACH A MATCHING GABLE TRUSS TO THE INSIDE FACE OF THE STRUCTURAL GABLE AND FASTEN PER THE

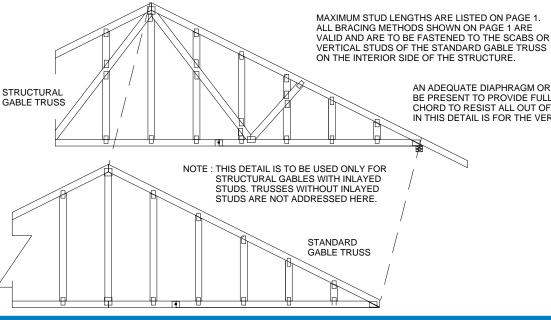
FOLLOWING NAILING SCHEDULE.

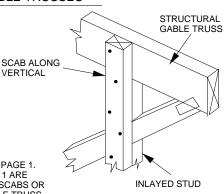
METHOD 2: ATTACH 2X \_ SCABS TO THE FACE OF EACH VERTICAL MEMBER ON THE STRUCTURAL GABLE PER THE FOLLOWING NAILING SCHEDULE. SCABS ARE TO BE OF THE SAME SIZE, GRADE AND SPECIES AS THE TRUSS VERTICALS

### NAILING SCHEDULE:

FOR WIND SPEEDS 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7-10, 16) OR LESS, NAIL ALL MEMBERS WITH ONE ROW OF 10d (0.131" X 3") NAILS SPACED 6" O.C.

- FOR WIND SPEEDS 120-150 MPH (ASCE 7-98, Ó2, 05), 150-190 MPH (ASCE 7-10, 16) NAIL ALL MEMBERS WITH TWO ROWS OF 10d (0.131" X 3") NAILS SPACED 6" O.C. (2X 4 STUDS MINIMUM)





AN ADEQUATE DIAPHRAGM OR OTHER METHOD OF BRACING MUST BE PRESENT TO PROVIDE FULL LATERAL SUPPORT OF THE BOTTOM CHORD TO RESIST ALL OUT OF PLANE LOADS. THE BRACING SHOWN IN THIS DETAIL IS FOR THE VERTICAL/STUDS ONLY.

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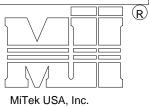
### **JANUARY 6, 2017**

### Standard Gable End Detail

### MII-GE140-001 T23399812

Page 1 of 2

MiTek USA, Inc.



DIAGONAL BRACE

Typical \_x4 L-Brace Nailed To 2x\_ Verticals W/10d Nails spaced 6" o.c. Vertical Stud SECTION B-B

Vertical Stud DIAGONAL (4) - 16d Nails 16d Nails Spaced 6" o.c. (2) - 10d Nails into 2x6 2x6 Stud or 2x4 No.2 of better TRUSS GEOMETRY AND CONDITIONS SHOWN ARE FOR ILLUSTRATION ONLY. Typical Horizontal Brace Nailed To 2x\_ Verticals w/(4)-10d Nails **SECTION A-A** 2x4 Stud

4'-0" O.C. MAX Varies to Common Truss SEE INDIVIDUAL MITEK ENGINEERING DRAWINGS FOR DESIGN CRITERIA 3x4 =Diagonal Bracing \*\* - L-Bracing Refer Refer to Section A-A to Section B-B

TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS. (4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD DF/SPF BLOCK

PROVIDE 2x4 BLOCKING BETWEEN THE FIRST

Roof Sheathing -

- 1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS. 2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND
- WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.

  3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
- "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 1x4 SRB OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
- 5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C. 6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A
- 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
- GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
- THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.
- 10. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

	1 tool Ollowill	9			/	
0.4" 1.4		.,	<u> </u>		×	
24" Max	M					//
	1'-3" Max.	(2) - NAII	10d LS			(2) - 10d NAILS
			*/	<b>/</b> /	russes	s @ 24" o.c.
at 1	g. Brace /3 points eeded		ATTAC NAILS	CHED TO V AND ATTA	/ERTICAL	ACED 48" O.C. WITH (4) -16d Od NAILS.
	End Wall	\		_	ZONTAL E SECTION	-
	_	1 1				

Minimum Stud Size Species	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
and Grade			Maximur	n Stud Ler	ngth	
2x4 DF/SPF Std/Stud	12" O.C.	3-10-1	3-11-7	5-7-2	7-8-2	11-6-4
2x4 DF/SPF Std/Stud	16" O.C.	3-3-14	3-5-1	4-10-2	6-7-13	9-11-11
2x4 DF/SPF Std/Stud	24" O.C.	2-8-9	2-9-8	3-11-7	5-5-2	8-1-12

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of web with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAXIMUM WIND SPEED = 140 MPH MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING EXPOSURE B or C

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April 2,2021

ASCE 7-98, ASCE 7-02, ASCE 7-05

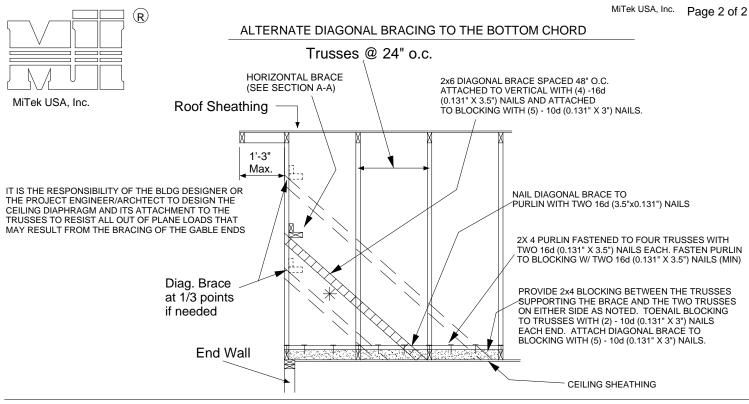
DURANON-OFIGOAD/INGREASE::pt.6:0 REAP GONNECTION IOFISBASED ON COMPONENTS AND CLADDING.

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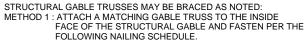
\*\*ANSITP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information\*\*

available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





### BRACING REQUIREMENTS FOR STRUCTURAL GABLE TRUSSES



METHOD 2 : ATTACH 2X \_ SCABS TO THE FACE OF EACH VERTICAL

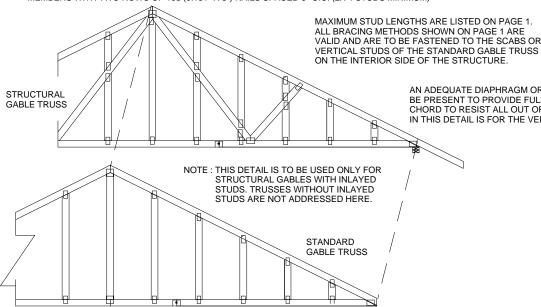
MEMBER ON THE STRUCTURAL GABLE PER THE FOLLOWING

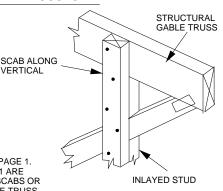
NAILING SCHEDULE. SCABS ARE TO BE OF THE SAME SIZE, GRADE AND SPECIES AS THE TRUSS VERTICALS

### NAILING SCHEDULE:

FOR WIND SPEEDS 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7-10, 16) OR LESS, NAIL ALL MEMBERS WITH ONE ROW OF 10d (0.131" X 3") NAILS SPACED 6" O.C.
FOR WIND SPEEDS 120-150 MPH (ASCE 7-98, 02, 05), 150-190 MPH (ASCE 7-10, 16) NAIL ALL

MEMBERS WITH TWO ROWS OF 10d (0.131" X 3") NAÎLS SPACED 6" O.C. (2X 4 STÚDS MINIMUM)





AN ADEQUATE DIAPHRAGM OR OTHER METHOD OF BRACING MUST BE PRESENT TO PROVIDE FULL LATERAL SUPPORT OF THE BOTTOM CHORD TO RESIST ALL OUT OF PLANE LOADS. THE BRACING SHOWN IN THIS DETAIL IS FOR THE VERTICAL/STUDS ONLY.

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April 2,2021



### Standard Gable End Detail

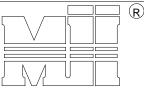
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### MII-GE170-D-SP T23399813

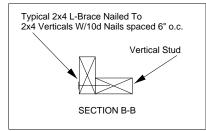




Page 1 of 2



MiTek USA, Inc.



TRUSS GEOMETRY AND CONDITIONS SHOWN ARE FOR ILLUSTRATION ONLY.

Varies to Common Truss SEE INDIVIDUAL MITEK ENGINEERING DRAWINGS FOR DESIGN CRITERIA

3x4 =

- Diagonal Bracing - L-Bracing Refer \*\* Refer to Section A-A to Section B-B

DIAGONAL BRACE

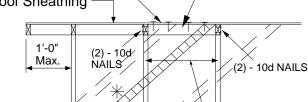
4'-0" O.C. MAX

### Vertical Stud 2X6 SP OR SPF No. 2 DIAGONAL BRACE (4) - 16d Nails 16d Nails Spaced 6" o.c. (2) - 10d Nails into 2x6 2X6 SP OR SPF No. 2 Typical Horizontal Brace Nailed To 2x4 Verticals w/(4)-10d Nails SECTION A-A 2X4 SP OR SPF No. 2

PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SPF BLOCK

### Roof Sheathing 24" Max



Diag. Brace at 1/3 points if needed

**End Wall** 

2x6 DIAGONAL BRACE SPACED 48" O.C. ATTACHED TO VERTICAL WITH (4) -16d NAILS, AND ATTACHED TO BLOCKING WITH (5) -10d NAILS.

∕Trusses @ 24" o.c.

HORIZONTAL BRACE (SEE SECTION A-A)

### NOTE:

- 1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS. 2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND
- WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
- 3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
- "L" BRACES SPECIFIED ARE TO BE FULL LENGTH, SPF or SP No.3 OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
  5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF
- DIAPHRAM AT 4'-0" O.C.
- 6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 AND A 2x4 AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST GABLE STUD. ATTACH TO VERTICAL GABLE STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
- 7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
- THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
- DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.
- 10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.
- 11. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Minimum Stud Size Species	d Size Spacing		2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
and Grade			Maximum St	ud Length	
2x4 SP No. 3 / Stud	12" O.C.	3-9-7	5-8-8	6-11-1	11-4-4
2x4 SP No. 3 / Stud	16" O.C.	3-4-12	4-11-15	6-9-8	10-2-3
2x4 SP No. 3 / Stud	24" O.C.	2-9-4	4-0-7	5-6-8	8-3-13
2x4 SP No. 2	12" O.C.	3-11-13	5-8-8	6-11-1	11-11-7
2x4 SP No. 2	16" O.C.	3-7-7	4-11-5	6-11-1	10-10-5
2x4 SP No. 2	24" O.C.	3-1-15	4-0-7	6-3-14	9-5-14

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 l-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 6" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length. T or I braces must be 2x4 SPF No. 2 or SP No. 2.

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April 2,2021

MAX MEAN ROOF HEIGHT = 30 FEET EXPOSURE D

DURATION OF LOAD INCREASE: 1.60
WARNING - Verify design parameters and READ NOTE CONNECTION OF BRACING IS BASED ON MAYERS: 5/19/2020 BEFORE USE

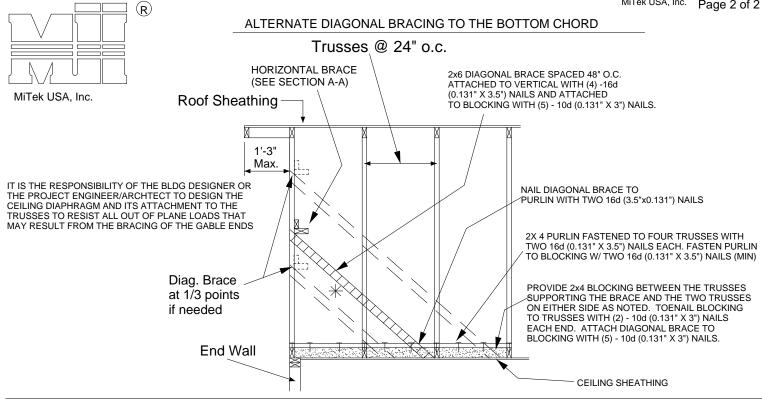
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TUD DESIGN IS BASED ON COMPONENTS



MiTek USA, Inc. Page 2 of 2



### BRACING REQUIREMENTS FOR STRUCTURAL GABLE TRUSSES

STRUCTURAL GABLE TRUSSES MAY BE BRACED AS NOTED:

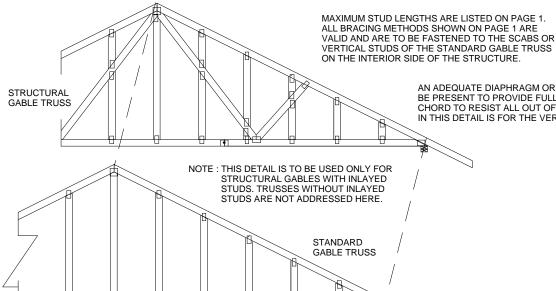
METHOD 1: ATTACH A MATCHING GABLE TRUSS TO THE INSIDE FACE OF THE STRUCTURAL GABLE AND FASTEN PER THE

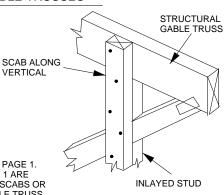
FOLLOWING NAILING SCHEDULE.

METHOD 2: ATTACH 2X\_SCABS TO THE FACE OF EACH VERTICAL
MEMBER ON THE STRUCTURAL GABLE PER THE FOLLOWING
NAILING SCHEDULE. SCABS ARE TO BE OF THE SAME SIZE, GRADE AND SPECIES AS THE TRUSS VERTICALS

NAILING SCHEDULE:

EING SCHEDEL:
- FOR WIND SPEEDS 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7-10, 16) OR LESS, NAIL ALL
- MEMBERS WITH ONE ROW OF 10d (0.131" X 3") NAILS SPACED 6" O.C.
- FOR WIND SPEEDS 120-150 MPH (ASCE 7-98, 02, 05), 150-190 MPH (ASCE 7-10, 16) NAIL ALL
- MEMBERS WITH TWO ROWS OF 10d (0.131" X 3") NAILS SPACED 6" O.C. (2X 4 STUDS MINIMUM)





AN ADEQUATE DIAPHRAGM OR OTHER METHOD OF BRACING MUST BE PRESENT TO PROVIDE FULL LATERAL SUPPORT OF THE BOTTOM CHORD TO RESIST ALL OUT OF PLANE LOADS. THE BRACING SHOWN IN THIS DETAIL IS FOR THE VERTICAL/STUDS ONLY.

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Philip J. O'Regan PE No.58126 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

April 2,2021

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ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

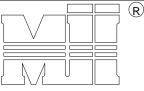


### Standard Gable End Detail

### MII-GE180-D-SP T23399814



Page 1 of 2



DIAGONAL BRACE

4'-0" O.C. MAX

MiTek USA, Inc.

Typical 2x4 L-Brace Nailed To 2x4 Verticals W/10d Nails spaced 6" o.c.

Vertical Stud

SECTION B-B

TRUSS GEOMETRY AND CONDITIONS SHOWN ARE FOR ILLUSTRATION ONLY.

Varies to Common Truss

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\*\*

SEE INDIVIDUAL MITEK ENGINEERING

24" Max

End Wall

DRAWINGS FOR DESIGN CRITERIA

3x4 =

Vertical Stud

(4) - 16d Nails

2X6 SP OR SPF No. 2
DIAGONAL BRACE

16d Nails
Spaced 6" o.c.

2X6 SP OR SPF No. 2
Typical Horizontal Brace
Nailed To 2x4 Verticals
w/(4)-10d Nails

SECTION A-A

2X4 SP OR SPF No. 2

PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

 ★ - Diagonal Bracing Refer to Section A-A

★★ - L-Bracing Refer to Section B-B

### NOTE:

- 1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
- 2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
- 3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY, CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
- "L" BRACES SPECIFIED ARE TO BE FULL LENGTH, SPF or SP No.3 OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
   DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF
- 5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.
- 6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 AND A 2x4 AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST GABLE STUD. ATTACH TO VERTICAL GABLE STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
- 7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
- 8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
- 9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.
- 10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.
- 11. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

NAILS DESIGNATED 16d ARE (0.131" X 3.5")								
Minimum Stud Size Species	Stud Spacing	Without Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS			
and Grade			Maximum St	ud Length				
2x4 SP No. 3 / Stud	12" O.C.	3-7-12	5-4-11	6-2-1	10-11-3			
2x4 SP No. 3 / Stud	16" O.C.	3-2-8	4-8-1	6-2-1	9-7-7			
2x4 SP No. 3 / Stud	24" O.C.	2-7-7	3-9-12	5-2-13	7-10-4			
2x4 SP No. 2	12" O.C.	3-10-0	5-4-11	6-2-1	11-6-1			
2x4 SP No. 2	16" O.C.	3-5-13	4-8-1	6-2-1	10-5-7			
2x4 SP No. 2	24" O.C.	3-0-8	3-9-12	6-1-1	9-1-9			

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 6in o.c., with 3in minimum end distance. Brace must cover 90% of diagonal length. T or I braces must be 2x4 SPF No. 2 or SP No. 2.

Roof Sheathing

1'-0"

Max.

(2) - 10d

NAILS

(2) - 10d NAILS

(2) - 10d NAILS

(2) - 10d NAILS

2x6 DIAGONAL BRACE SPACED
48" O.C. ATTACHED TO VERTICAL WITH
(4) -16d NAILS, AND ATTACHED TO
BLOCKING WITH (5) -10d NAILS.

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD

SHEATHING TO 2x4 STD SPF BLOCK

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HORIZONTAL BRACE

(SEE SECTION A-A)

Philip J. O'Regan PE No.58126 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

April 2,2021

MAX MEAN ROOF HEIGHT = 30 FEET EXPOSURE D

ASCE 7-10, ASCE 7-16 180 MPH

DURATION ARIJE CAPY INCREASE: 13 AR READ NOTES CONNECTION OF BRACING IS BASED ON INVISES. 5/19/2020 BEFORE USE

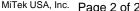
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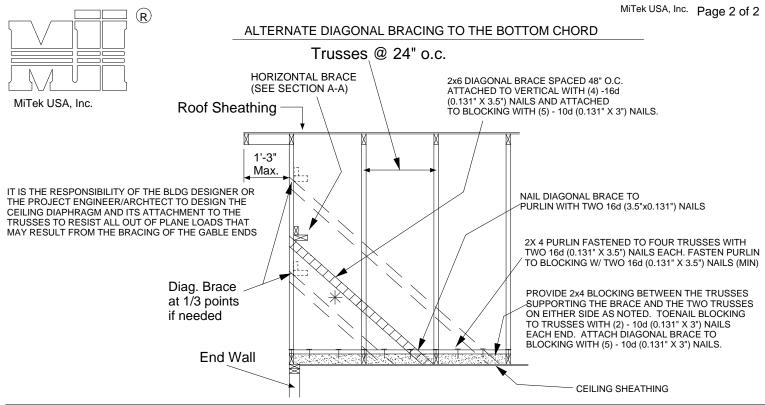


6904 Parke East Blvd Tampa, FL 36610

### Standard Gable End Detail

MII-SHEET 2 T23399814





### BRACING REQUIREMENTS FOR STRUCTURAL GABLE TRUSSES

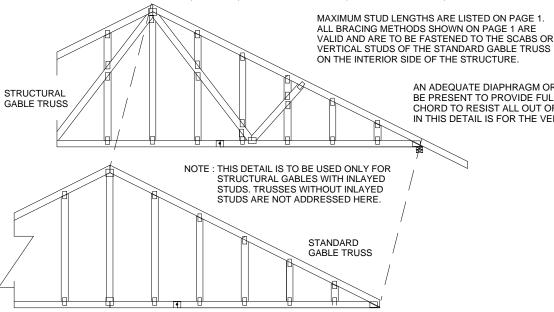
STRUCTURAL GABLE TRUSSES MAY BE BRACED AS NOTED: METHOD 1 : ATTACH A MATCHING GABLE TRUSS TO THE INSIDE FACE OF THE STRUCTURAL GABLE AND FASTEN PER THE FOLLOWING NAILING SCHEDULE.

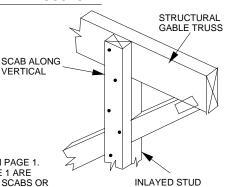
METHOD 2: ATTACH 2X \_ SCABS TO THE FACE OF EACH VERTICAL MEMBER ON THE STRUCTURAL GABLE PER THE FOLLOWING NAILING SCHEDULE. SCABS ARE TO BE OF THE SAME SIZE, GRADE

AND SPECIES AS THE TRUSS VERTICALS NAILING SCHEDULE:

FOR WIND SPEEDS 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7-10, 16) OR LESS, NAIL ALL MEMBERS WITH ONE ROW OF 10d (0.131" X 3") NAILS SPACED 6" O.C. FOR WIND SPEEDS 120-150 MPH (ASCE 7-98, 02, 05), 150-190 MPH (ASCE 7-10, 16) NAIL ALL

MEMBERS WITH TWO ROWS OF 10d (0.131" X 3") NAILS SPACED 6" O.C. (2X 4 STUDS MINIMUM)





AN ADEQUATE DIAPHRAGM OR OTHER METHOD OF BRACING MUST BE PRESENT TO PROVIDE FULL LATERAL SUPPORT OF THE BOTTOM CHORD TO RESIST ALL OUT OF PLANE LOADS. THE BRACING SHOWN IN THIS DETAIL IS FOR THE VERTICAL/STUDS ONLY.

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April 2,2021

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MiTek USA, Inc.

### STANDARD PIGGYBACK TRUSS CONNECTION DETAIL

### MII-PIGGY-7-16 T23399815

MiTek USA, Inc. Page 1 of 1

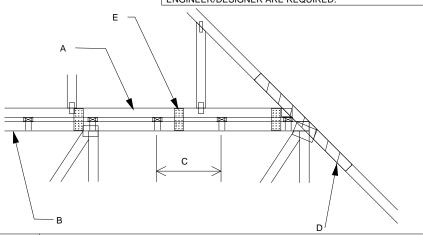
(R)

MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E MAX MEAN ROOF HEIGHT = 30 FEET MAX TRUSS SPACING = 24 " O.C. CATEGORY II BUILDING EXPOSURE B or C **ENCLOSED BUILDING** LOADING = 5 PSF TCDL ASCE 7-10, ASCE 7-16 **DURATION OF LOAD INCREASE: 1.60** 

DETAIL IS NOT APPLICABLE FOR TRUSSES TRANSFERING DRAG LOADS (SHEAR TRUSSES). ADDITIONAL CONSIDERATIONS BY BUILDING ENGINEER/DESIGNER ARE REQUIRED.

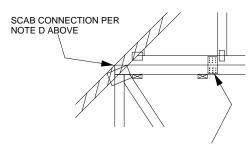
- A PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
- A PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
  SHALL BE CONNECTED TO EACH PURLIN
  WITH (2) (0.131" X 3.5") TOE-NAILED.
  B BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
  C PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24" O.C.
  UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING.
- UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING. CONNECT TO BASE TRUSS WITH (2) (0.131" X 3.5") NAILS EACH.

   2 X \_\_ X 4'-0" SCAB, SIZE TO MATCH TOP CHORD OF PIGGYBACK TRUSS, MIN GRADE #2, ATTACHED TO ONE FACE, CENTERED ON INTERSECTION, WITH (2) ROWS OF (0.131" X 3") NAILS @ 4" O.C. SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH DIRECTIONS AND:
- 1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR 2. WIND SPEED OF 116 MPH TO 180 MPH WITH A MAXIMUM
- PIGGYBACK SPAN OF 12 ft.
   FOR WIND SPEEDS BETWEEN 116 AND 180 MPH, ATTACH
  MITEK NP37 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 72" O.C. W/ (4) (0.131" X 1.5") NAILS PER MEMBER. STAGGER NAILS FROM OPPOSING FACES. ENSURE 0.5" NAIL EDGE DISTANCE. (MIN. 2 PAIRS OF PLATES REQ. REGARDLESS OF SPAN)

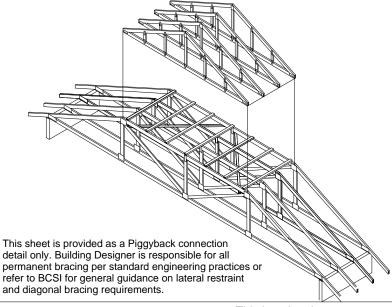


### WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

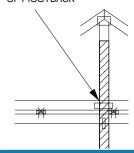
REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH Nail-On PLATES AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.



FOR ALL WIND SPEEDS, ATTACH MITEK NP37 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 48" O.C. W/ (4) (0.131" X 1.5") PER MEMBER. STAGGER NAILS FROM OPPOSING FACES ENSURE 0.5" NAIL EDGE DISTANCE.



VERTICAL WEB TO EXTEND THROUGH BOTTOM CHORD OF PIGGYBACK



### FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

- VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL
- x 4'-0" SCAB TO EACH FACE OF TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 4" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.) (MINIMUM 2X4)
- THIS CONNECTION IS ONLY VALID FOR A MAXIMUM CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS GREATER THAN 4000 LBS.
- 4) FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS,
  NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS.
  5) CONCENTRATED LOAD MUST BE APPLIED TO BOTH

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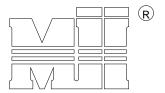


### JANUARY 8, 2019

### STANDARD PIGGYBACK TRUSS CONNECTION DETAIL

### MII-PIGGY-ALT-7-16 T23399816

Page 1 of 1 MiTek USA, Inc.



MiTek USA, Inc.

MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E MAX MEAN ROOF HEIGHT = 30 FEET MAX TRUSS SPACING = 24 " O.C. CATEGORY II BUILDING EXPOSURE B or C ENCLOSED BUILDING LOADING = 5 PSF TCDL MINIMUM

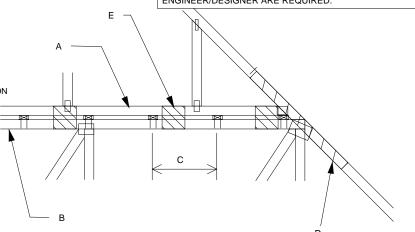
ASCE 7-10, ASCE 7-16 **DURATION OF LOAD INCREASE: 1.60** 

DETAIL IS NOT APPLICABLE FOR TRUSSES TRANSFERING DRAG LOADS (SHEAR TRUSSES). ADDITIONAL CONSIDERATIONS BY BUILDING ENGINEER/DESIGNER ARE REQUIRED.

- A PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING. SHALL BE CONNECTED TO EACH PURLIN
  WITH (2) 0(0.131" X 3.5") TOE-NAILED.

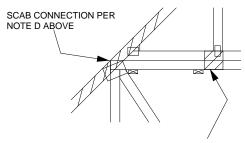
  BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
  - PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24" O.C.
  UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING.

- UNLESS SPECIFIED CLOSER OIN MITEX TRUSS DESIGN DRAWING.
  CONNECT TO BASE TRUSS WITH (2) (0.131" X 3.5") NAILS EACH.
   2 X \_ X 4'-0" SCAB, SIZE TO MATCH TOP CHORD OF
  PIGGYBACK TRUSS, MIN GRADE #2, ATTACHED TO ONE FACE, CENTERED ON
  INTERSECTION, WITH (2) ROWS OF (0.131" X 3") NAILS @ 4" O.C.
  SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH DIRECTIONS AND:
- 1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR 2. WIND SPEED OF 116 MPH TO 180 MPH WITH A MAXIMUM
- PIGGYBACK SPAN OF 12 ft.
  E FOR WIND SPEED IN THE RANGE 116 MPH 180 MPH ADD 9" x 9" x 1/2" PLYWOOD (or 7/16" OSB) GUSSET EACH SIDE AT 48" O.C. OR LESS. ATTACH WITH 3 - 6d (0.113" X 2") NAILS INTO EACH CHORD FROM EACH SIDE (TOTAL - 12 NAILS)

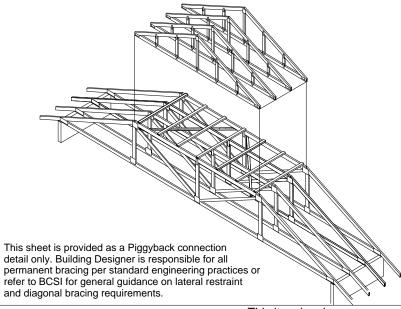


WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

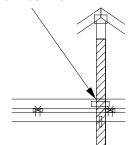
REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH PLYWOOD GUSSETS AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.



7" x 7" x 1/2" PLYWOOD (or 7/16" OSB) GUSSET EACH SIDE AT 24" O.C. ATTACH WITH 3 - 6d (0.113" X 2") NAILS INTO EACH CHORD FROM EACH SIDE (TOTAL - 12 NAILS)



VERTICAL WEB TO EXTEND THROUGH **BOTTOM CHORD** OF PIGGYBACK



### FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

- 1) VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL
- ATTACH 2 x \_\_\_ x 4-0" SCAB TO EACH FACE OF TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 4" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH ATTACH 2 x VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.) (MINIMUM 2X4)
- THIS CONNECTION IS ONLY VALID FOR A MAXIMUM CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS
- GREATER THAN 4000 LBS.
  4) FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS, NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS.
- 5) CONCENTRATED LOAD MUST BE APPLIED TO BOTH

IE PIGGYBACK AND THE BASE TRUSS DESIGN

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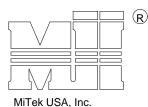


### STANDARD REPAIR DETAIL FOR BROKEN CHORDS, WEBS AND DAMAGED OR MISSING CHORD SPLICE PLATES

### MII-REP01A1 T23399817

MiTek USA, Inc.

Page 1 of 1

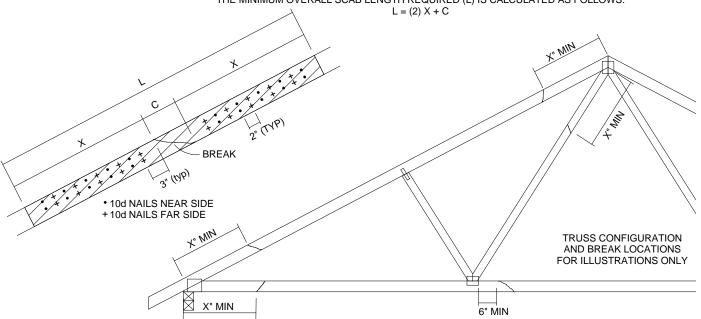


TOTAL NUMBER OF NAILS EACH SIDE OF BREAK *		X INCHES	MAXIMUM FORCE (lbs) 15% LOAD DURATION							
			SP		DF		SPF		HF	
2x4	2x6		2x4	2x6	2x4	2x6	2x4	2x6	2x4	2x6
20	30	24"	1706	2559	1561	2342	1320	1980	1352	2028
26	39	30"	2194	3291	2007	3011	1697	2546	1738	2608
32	48	36"	2681	4022	2454	3681	2074	3111	2125	3187
38	57	42"	3169	4754	2900	4350	2451	3677	2511	3767
44	66	48"	3657	5485	3346	5019	2829	4243	2898	4347

<sup>\*</sup> DIVIDE EQUALLY FRONT AND BACK

ATTACH 2x\_ SCAB OF THE SAME SIZE AND GRADE AS THE BROKEN MEMBER TO EACH FACE OF THE TRUSS (CENTER ON BREAK OR SPLICE) WITH 10d (0.131" X 3") NAILS (TWO ROWS FOR 2x4, THREE ROWS FOR 2x6) SPACED 4" O.C. AS SHOWN. STAGGER NAIL SPACING FROM FRONT FACE AND BACK FACE FOR A NET 0-2-0 O.C. SPACING IN THE MAIN MEMBER. USE A MIN. 0-3-0 MEMBER END DISTANCE.

THE LENGTH OF THE BREAK (C) SHALL NOT EXCEED 12". (C=PLATE LENGTH FOR SPLICE REPAIRS) THE MINIMUM OVERALL SCAB LENGTH REQUIRED (L) IS CALCULATED AS FOLLOWS:



THE LOCATION OF THE BREAK MUST BE GREATER THAN OR EQUAL TO THE REQUIRED X DIMENSION FROM ANY PERIMETER BREAK OR HEEL JOINT AND A MINIMUM OF 6" FROM ANY INTERIOR JOINT (SEE SKETCH ABOVE)

### DO NOT USE REPAIR FOR JOINT SPLICES

### NOTES

- THIS REPAIR DETAIL IS TO BE USED ONLY FOR THE APPLICATION SHOWN. THIS REPAIR DOES
  NOT IMPLY THAT THE REMAINING PORTION OF THE TRUSS IS UNDAMAGED. THE ENTIRE TRUSS
  SHALL BE INSPECTED TO VERIFY THAT NO FURTHER REPAIRS ARE REQUIRED. WHEN THE REQUIRED
  REPAIRS ARE PROPERLY APPLIED, THE TRUSS WILL BE CAPABLE OF SUPPORTING THE LOADS INDICATED.
- 2. ALL MEMBERS MUST BE RETURNED TO THEIR ORIGINAL POSITIONS BEFORE APPLING REPAIR AND HELD IN PLACE DURING APPLICATION OF REPAIR
- THE END DISTANCE, EDGE DISTANCE AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD. WHEN NAILING THE SCABS, THE USE OF A BACKUP WEIGHT IS RECOMMENDED TO AVOID
- LOOSENING OF THE CONNECTOR PLATES AT THE JOINTS OR SPLICES.
  THIS REPAIR IS TO BE USED FOR SINGLE PLY TRUSSES IN THE 2x\_ ORIENTATION ONLY.
- 6. THIS REPAIR IS LIMITED TO TRUSSES WITH NO MORE THAN THREE BROKEN MEMBERS.

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### LATERAL TOE-NAIL DETAIL

MII-TOENAIL SP T23399818

MiTek USA. Inc.

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### NOTES:

- TOE-NAILS SHALL BE DRIVEN AT AN ANGLE OF 45 DEGREES WITH THE MEMBER AND MUST HAVE FULL WOOD SUPPORT. (NAIL MUST BE DRIVEN THROUGH AND EXIT AT THE BACK CORNER OF THE MEMBER END AS SHOWN.
- 2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
- 3. ALLOWABLE VALUE SHALL BE THE LESSER VALUE OF THE TWO SPECIES FOR MEMBERS OF DIFFERENT SPECIES.

THIS DETAIL APPLICABLE TO THE THREE END DETAILS SHOWN BELOW

> VIEWS SHOWN ARE FOR ILLUSTRATION PURPOSES ONLY

> > SIDE VIEW (2x3) 2 NAILS

> > > NEAR SIDE NEAR SIDE

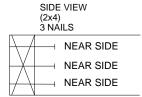
	TOE-NAIL SINGLE SHEAR VALUES PER NDS 2018 (lb/nail)							
	DIAM.	SP	DF	HF	SPF	SPF-S		
3.5" LONG	.131	88.0	80.6	69.9	68.4	59.7		
	.135	93.5	85.6	74.2	72.6	63.4		
	.162 108.8		99.6	86.4	84.5	73.8		
LONG	.128	74.2	67.9	58.9	57.6	50.3		
	.131	75.9	69.5	60.3	59.0	51.1		
3.25"	.148	81.4	74.5	64.6	63.2	52.5		
3.2			•	•				

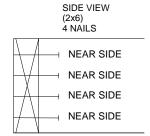
VALUES SHOWN ARE CAPACITY PER TOE-NAIL APPLICABLE DURATION OF LOAD INCREASES MAY BE APPLIED.

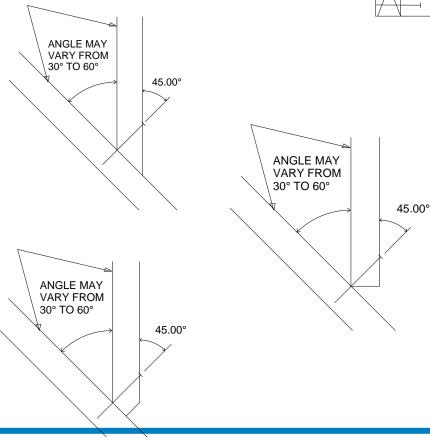
(3) - 16d (0.162" X 3.5") NAILS WITH SPF SPECIES BOTTOM CHORD

For load duration increase of 1.15:

3 (nails) X 84.5 (lb/nail) X 1.15 (DOL) = 291.5 lb Maximum Capacity







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April 2,2021

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\*\*ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information\*\* available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

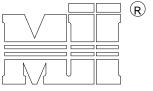


### TRUSSED VALLEY SET DETAIL

### MII-VALLEY HIGH WIND1 T23399819

MiTek USA, Inc.

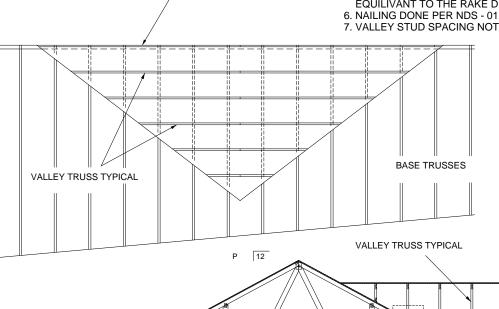
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MiTek USA, Inc.

### **GENERAL SPECIFICATIONS**

- 1. NAIL SIZE 10d (0.131" X 3")
- 2. WOOD SCREW = 3" WS3 USP OR EQUIVALENT DO NOT USE DRYWALL OR DECKING TYPE SCREW
- 3. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND SECURE PER DETAIL A
- 4. BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS.
- 5. BASE TRUSS SHALL BE DESIGNED WITH A PURLIN SPACING EQUILIVANT TO THE RAKE DIMENSION OF THE VALLEY TRUSS SPACING.
- 7. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.



GABLE END, COMMON TRUSS

OR GIRDER TRUSS

GABLE END, COMMON TRUSS OR GIRDER TRUSS

SEE DETAIL A BELOW (TYP.)

SECURE VALLEY TRUSS W/ ONE ROW OF 10d NAILS 6" O.C. ATTACH 2x4 CONTINUOUS NO.2 SP TO THE ROOF W/ TWO USP WS3 (1/4" X 3") WOOD SCREWS INTO EACH BASE TRUSS. **DETAIL A** (NO SHEATHING) N.T.S.

WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH WIND DESIGN PER ASCE 7-10, ASCE 7-16 160 MPH MAX MEAN ROOF HEIGHT = 30 FEET ROOF PITCH = MINIMUM 3/12 MAXIMUM 6/12 CATEGORY II BUILDING **EXPOSURE C** WIND DURATION OF LOAD INCREASE: 1.60 MAX TOP CHORD TOTAL LOAD = 50 PSF MAX SPACING = 24" O.C. (BASE AND VALLEY)
MINIMUM REDUCED DEAD LOAD OF 6 PSF

ON THE TRUSSES

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\*\*ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information\*\* available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



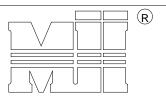
MiTek USA, Inc.

### TRUSSED VALLEY SET DETAIL

### MII-VALLEY HIGH WIND2 T23399820

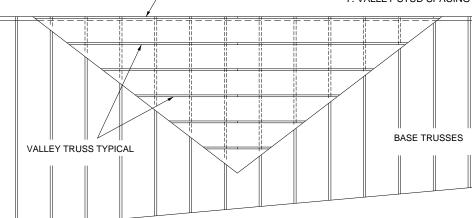
MiTek USA, Inc.

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### **GENERAL SPECIFICATIONS**

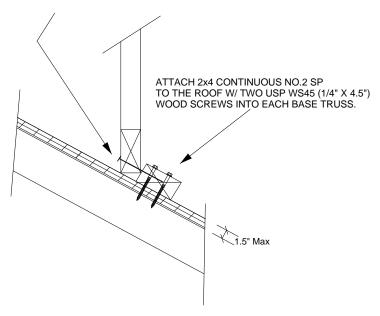
- 1. NAIL SIZE 10d (0.131" X 3")
- 2. WOOD SCREW = 4.5" WS45 USP OR EQUILIVANT
- 3. INSTALL SHEATHING TO TOP CHORD OF BASE TRUSSES.
- INSTALL SHEATING TO TOP CHORD OF BASE TRUS
   INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND
   SECURE TO BASE TRUSSES AS PER DETAIL A
   BRACE VALLEY WEBS IN ACCORDANCE WITH THE
- INDIVIDUAL DESIGN DRAWINGS.
- 6. NAILING DONE PER NDS-01
- 7. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.



GABLE END, COMMON TRUSS OR GIRDER TRUSS

> GABLE END, COMMON TRUSS VALLEY TRUSS TYPICAL OR GIRDER TRUSS 12 SEE DETAIL A BELOW (TYP.)

SECURE VALLEY TRUSS W/ ONE ROW OF 10d NAILS 6" O.C.



WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH WIND DESIGN PER ASCE 7-10, ASCE 7-16 160 MPH MAX MEAN ROOF HEIGHT = 30 FEET ROOF PITCH = MINIMUM 3/12 MAXIMUM 6/12 CATEGORY II BUILDING EXPOSURE C WIND DURATION OF LOAD INCREASE: 1.60 MAX TOP CHORD TOTAL LOAD = 50 PSF MAX SPACING = 24" O.C. (BASE AND VALLEY)
MINIMUM REDUCED DEAD LOAD OF 6 PSF ON THE TRUSSES

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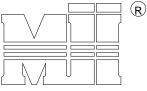


### TRUSSED VALLEY SET DETAIL

### MII-VALLEY SP T23399821

MiTek USA, Inc.

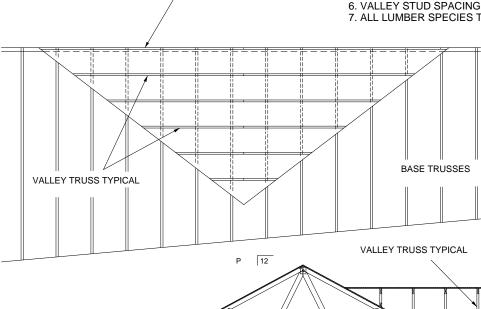
Page 1 of 1



MiTek USA, Inc.

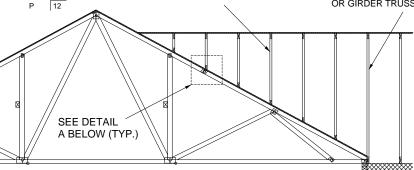
### **GENERAL SPECIFICATIONS**

- 1. NAIL SIZE 16d (0.131" X 3.5")
- 2. INSTALL VALLÈY TRUSSES (24" O.C. MAXIMUM) AND SECURE PER DETAIL A
- 3. BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS.
- 4. BASE TRUSS SHALL BE DESIGNED WITH A PURLIN SPACING EQUILIVANT TO THE RAKE DIMENSION OF THE VALLEY TRUSS SPACING.
- 5. NAILING DONE PER NDS 01
- 6. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.
- 7. ALL LUMBER SPECIES TO BE SP.



GABLE END, COMMON TRUSS OR GIRDER TRUSS

GABLE END, COMMON TRUSS OR GIRDER TRUSS



SECURE VALLEY TRUSS W/ ONE ROW OF 16d NAILS 6" O.C. ATTACH 2x4 CONTINUOUS NO.2 SP TO THE ROOF W/ TWO 16d NAILS INTO EACH BASE TRUSS. THE REAL PROPERTY. **DETAIL A** (MAXIMUM 1" SHEATHING) N.T.S.

WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 120 MPH WIND DESIGN PER ASCE 7-10, ASCE 7-16 150 MPH MAX MEAN ROOF HEIGHT = 30 FEET ROOF PITCH = MINIMUM 3/12 MAXIMUM 10/12 CATEGORY II BUILDING EXPOSURE C OR B
WIND DURATION OF LOAD INCREASE: 1.60
MAX TOP CHORD TOTAL LOAD = 60 PSF
MAX SPACING = 24" O.C. (BASE AND VALLEY)
MINIMUM REDUCED DEAD LOAD OF 4.2 PSF ON THE TRUSSES

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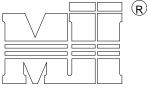


### TRUSSED VALLEY SET DETAIL

### MII-VALLEY SP T23399822

MiTek USA, Inc.

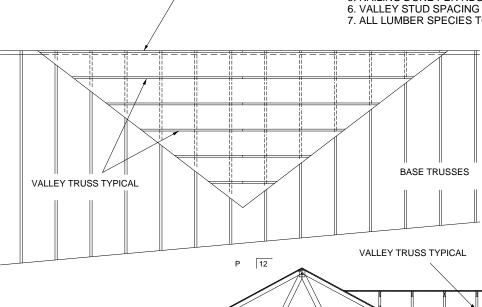
Page 1 of 1



MiTek USA, Inc.

### **GENERAL SPECIFICATIONS**

- 1. NAIL SIZE 16d (0.131" X 3.5")
- 2. INSTALL VALLÈY TRUSSES (24" O.C. MAXIMUM) AND SECURE PER DETAIL A
- 3. BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS.
- 4. BASE TRUSS SHALL BE DESIGNED WITH A PURLIN SPACING EQUILIVANT TO THE RAKE DIMENSION OF THE VALLEY TRUSS SPACING.
- 5. NAILING DONE PER NDS 01
- 6. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.
- 7. ALL LUMBER SPECIES TO BE SP.



GABLE END, COMMON TRUSS OR GIRDER TRUSS

GABLE END, COMMON TRUSS OR GIRDER TRUSS

SEE DETAIL A BELOW (TYP.) SECURE VALLEY TRUSS

W/ ONE ROW OF 16d NAILS 6" O.C. ATTACH 2x4 CONTINUOUS NO.2 SP TO THE ROOF W/ TWO 16d NAILS INTO EACH BASE TRUSS. THE REAL PROPERTY. **DETAIL A** (MAXIMUM 1" SHEATHING) N.T.S.

WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 120 MPH WIND DESIGN PER ASCE 7-10, ASCE 7-16 150 MPH MAX MEAN ROOF HEIGHT = 30 FEET ROOF PITCH = MINIMUM 3/12 MAXIMUM 10/12 CATEGORY II BUILDING EXPOSURE C OR B WIND DURATION OF LOAD INCREASE: 1.60 MAX TOP CHORD TOTAL LOAD = 60 PSF MAX SPACING = 24" O.C. (BASE AND VALLEY) MINIMUM REDUCED DEAD LOAD OF 4.2 PSF ON THE TRUSSES

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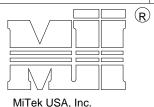
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### Standard Gable End Detail

### MII-GE146-001 T23399823



Page 1 of 2



Typical \_x4 L-Brace Nailed To 2x\_ Verticals W/10d Nails spaced 6" o.c. Vertical Stud SECTION B-B

DIAGONAL BRACE TRUSS GEOMETRY AND CONDITIONS 4'-0" O.C. MAX SHOWN ARE FOR ILLUSTRATION ONLY. Varies to Common Truss SEE INDIVIDUAL MITEK ENGINEERING DRAWINGS FOR DESIGN CRITERIA 3x4 =Ŕ

- L-Bracing Refer

to Section B-B

Vertical Stud DIAGONAL (4) - 16d Nails 16d Nails Spaced 6" o.c. (2) - 10d Nails into 2x6 2x6 Stud or 2x4 No.2 of better Typical Horizontal Brace Nailed To 2x\_ Verticals w/(4)-10d Nails **SECTION A-A** 2x4 Stud

> PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SP BLOCK

### NOTE:

Diagonal Bracing

Refer to Section A-A

- 1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
- 2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
- 3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
- 4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 2x4 No 3/STUD SP OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
- 5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.
- 6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
- 7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
- THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES
- DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES
- 10. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Minimum Stud Size Species	Stud Spacing	Without Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS	
and Grade		Maxir	num Stud Le	ength		
2x4 SP No 3/Stud	12" O.C.	3-11-3	6-8-0	7-2-14	11-9-10	
2x4 SP No 3/Stud	16" O.C.	3-6-14	5-9-5	7-1-13	10-8-11	
2x4 SP No 3/Stud	24" O.C.	3-1-8	4-8-9	6-2-15	9-4-7	

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of web with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

Roof Sheathing 24" Max 1'-3' (2) - 10d/ Max. **NÁILS** (2) - 10d NAILS Trusses @ 24" o.c. 2x6 DIAGONAL BRACE SPACED 48" O.C. Diag. Brace ATTACHED TO VERTICAL WITH (4) -16d at 1/3 points NAILS AND ATTACHED TO BLOCKING WITH (5) - 10d NAILS. if needed HORIZONTAL BRACE **End Wall** (SEE SECTION A-A)

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April 2,2021

MAXIMUM WIND SPEED = 146 MPH MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING EXPOSURE B or C

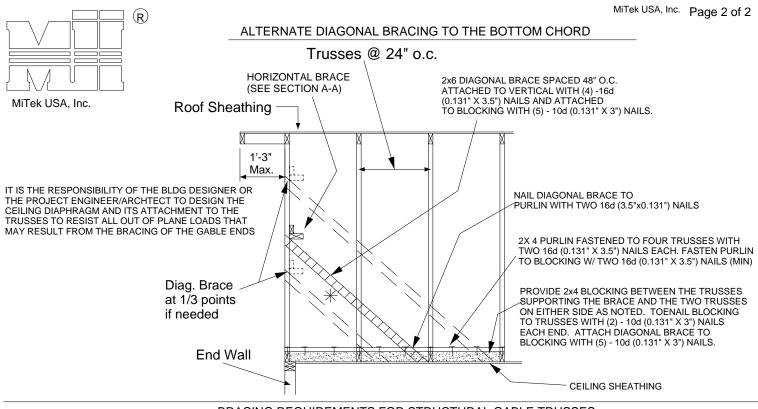
ASCE 7-98, ASCE 7-02, ASCE 7-05
STUD DESIGN IS BASED ON COMPONENTS AND CLADDING.
DURANOMONONO ON COMPONENTS AND CLADDING.
DURANOMONO ON COMPONENTS AND CLADDING.
DURANOMONO ON COMPONENTS AND CLADDING.

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### Standard Gable End Detail

MII-SHEET 2 T23399823



### BRACING REQUIREMENTS FOR STRUCTURAL GABLE TRUSSES

STRUCTURAL GABLE TRUSSES MAY BE BRACED AS NOTED: METHOD 1: ATTACH A MATCHING GABLE TRUSS TO THE INSIDE FACE OF THE STRUCTURAL GABLE AND FASTEN PER THE

FOLLOWING NAILING SCHEDULE.

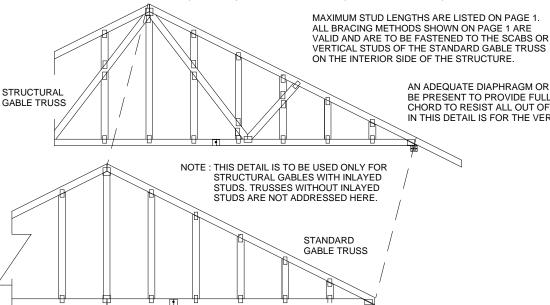
METHOD 2: ATTACH 2X SCABS TO THE FACE OF EACH VERTICAL

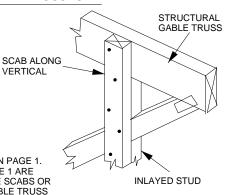
MEMBER ON THE STRUCTURAL GABLE PER THE FOLLOWING

NAILING SCHEDULE. SCABS ARE TO BE OF THE SAME SIZE, GRADE AND SPECIES AS THE TRUSS VERTICALS

FOR WIND SPEEDS 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7-10, 16) OR LESS, NAIL ALL MEMBERS WITH ONE ROW OF 10d (0.131" X 3") NAILS SPACED 6" O.C. - FOR WIND SPEEDS 120-150 MPH (ASCE 7-98, 02, 05), 150-190 MPH (ASCE 7-10, 16) NAIL ALL

MEMBERS WITH TWO ROWS OF 10d (0.131" X 3") NAILS SPACED 6" O.C. (2X 4 STUDS MINIMUM)





AN ADEQUATE DIAPHRAGM OR OTHER METHOD OF BRACING MUST BE PRESENT TO PROVIDE FULL LATERAL SUPPORT OF THE BOTTOM CHORD TO RESIST ALL OUT OF PLANE LOADS. THE BRACING SHOWN IN THIS DETAIL IS FOR THE VERTICAL/STUDS ONLY.

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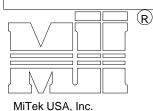
### **OCTOBER 5, 2016**

### REPLACE BROKEN OVERHANG

MII-REP13B T23399824

MiTek USA. Inc.

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TRUSS CRITERIA:

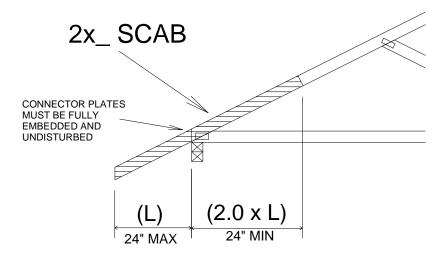
LOADING: 40-10-0-10 DURATION FACTOR: 1.15 SPACING: 24" O.C. TOP CHORD: 2x4 OR 2x6 PITCH: 4/12 - 12/12

HEEL HEIGHT: STANDARD HEEL UP TO 12" ENERGY HEEL

END BEARING CONDITION

### NOTES:

- 1. ATTACH 2x\_ SCAB (MINIMUM NO.2 GRADE SPF, HF, SP, DF) TO ONE FACE OF TRUSS WITH TWO ROWS OF 10d (0.131" X 3") SPACED 6" O.C.
- 2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
- 3. WHEN NAILING THE SCABS, THE USE OF A BACKUP WEIGHT IS RECOMMENDED TO AVOID LOOSENING OF THE CONNECTOR PLATES AT THE JOINTS OR SPLICES.



### **IMPORTANT**

This detail to be used only with trusses (spans less than 40') spaced 24" o.c. maximum and having pitches between 4/12 and 12/12 and total top chord loads not exceeding 50 psf.

Trusses not fitting these criteria should be examined individually.

REFER TO INDIVIDUAL TRUSS DESIGN FOR PLATE SIZES AND LUMBER GRADES This item has been electronically signed and sealed by ORegan, Philip, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

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Philip J. O'Regan PE No.58126 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

April 2,2021



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information

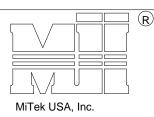
available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



6904 Parke East Blvd. Tampa, FL 36610

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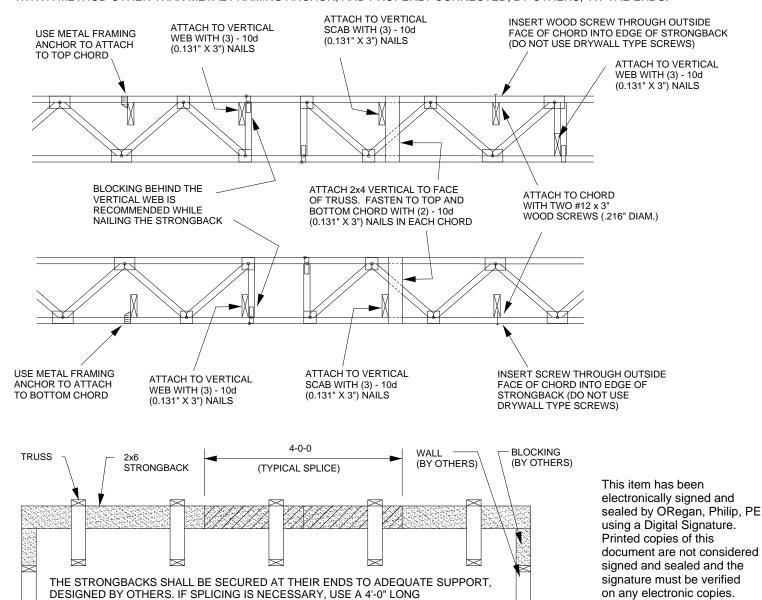
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TO MINIMIZE VIBRATION COMMON TO ALL SHALLOW FRAMING SYSTEMS, 2x6 "STRONGBACK" IS RECOMMENDED, LOCATED EVERY 8 TO 10 FEET ALONG A FLOOR TRUSS.

NOTE 1: 2X6 STRONGBACK ORIENTED VERTICALLY MAY BE POSITIONED DIRECTLY UNDER THE TOP CHORD OR DIRECTLY ABOVE THE BOTTOM CHORD. SECURELY FASTENED TO THE TRUSS USING ANY OF THE METHODS ILLUSTRATED BELOW.

NOTE 2: STRONGBACK BRACING ALSO SATISFIES THE LATERAL BRACING REQUIREMENTS FOR THE BOTTOM CHORD OF THE TRUSS WHEN IT IS PLACED ON TOP OF THE BOTTOM CHORD, IS CONTINUOUS FROM END TO END, CONNECTED WITH A METHOD OTHER THAN METAL FRAMING ANCHOR, AND PROPERLY CONNECTED, BY OTHERS, AT THE ENDS.



(0.131" X 3") NAILS EQUALLY SPACED.
ALTERNATE METHOD OF SPLICING:

OVERLAP STRONGBACK MEMBERS A MINIMUM OF 4'-0" AND FASTEN WITH (12) - 10d (0.131" X 3") NAILS STAGGERED AND EQUALLY SPACED.

(TO BE USED ONLY WHEN STRONGBACK IS NOT ALIGNED WITH A VERTICAL)

SCAB CENTERED ON THE SPLICE AND JOINED WITH (12) - 10d

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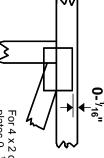


### Symbols

## PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated.
Dimensions are in ft-in-sixteenths.
Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- <sup>1</sup>/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

\* Plate location details available in MiTek 20/20 software or upon request.

### PLATE SIZE

4 × 4

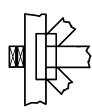
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

## LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

### **BEARING**



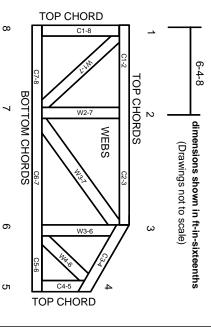
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

### Industry Standards:

National Design Specification for Metal Plate Connected Wood Truss Construction. Design Standard for Bracing.
Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

ANSI/TPI1: DSB-89:

## **Numbering System**



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

## PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988 ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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MiTek Engineering Reference Sheet: MII-7473 rev. 5/19/2020

# **General Safety Notes**

# Failure to Follow Could Cause Property Damage or Personal Injury

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.

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- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- Cut members to bear tightly against each other.

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- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.

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- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- 15. Connections not shown are the responsibility of others.
- Do not cut or alter truss member or plate without prior approval of an engineer.
- 17. Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.
- 21. The design does not take into account any dynamic or other loads other than those expressly stated.