

Columbia County New Building Permit Application

4

For Office Use Only Application # 55443 Date Received _____ By MG Permit # 45106
 Zoning Official _____ Date _____ Flood Zone _____ Land Use _____ Zoning _____
 FEMA Map # _____ Elevation _____ MFE _____ River _____ Plans Examiner _____ Date _____
 Comments _____

☐ NOC ☐ EH ☐ Deed or PA ☐ Site Plan ☐ State Road Info ☐ Well letter ☐ 911 Sheet ☐ Parent Parcel #
☐ Dev Permit # _____ ☐ In Floodway ☐ Letter of Auth. from Contractor ☐ F W Comp. letter
☐ Owner Builder Disclosure Statement ☐ Land Owner Affidavit ☐ Ellisville Water ☐ App Fee Paid ☐ Sub VF Form

Septic Permit No. _____ OR City Water _____ Fax _____

Applicant (Who will sign/pickup the permit) CASTAGNA CONIS. Phone 386-755-6867

Address 1459 GRANDVIEW AVE LAKE CITY FLA.

Owners Name LEON & DENICE JONES Phone 315-842-0966

911 Address W/A.

Contractors Name CASTAGNA CONIS. INC Phone 386-755-6867

Address 1459 GRANDVIEW AVE LAKE CITY FLA

Contractor Email CASTAGNA CONSTR @ BELL SOUTH. NET ***Include to get updates on this job.

Fee Simple Owner Name & Address LEON & DENICE JONES

Bonding Co. Name & Address W/A.

Architect/Engineer Name & Address NICK FEISLER 1752 N BROWN RD LC.

Mortgage Lenders Name & Address W/A.

Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Duke Energy

Property ID Number 21-45-16-03087-1B2 (14713) Estimated Construction Cost 300,000

Subdivision Name FOREST COUNTRY Lot 32 Block _____ Unit _____ Phase LYA

Driving Directions from a Major Road NORTH ON 47 TO FOREST COUNTRY SUB

TURN LEFT INTO SUB STOP SIGN TURN RIGHT GO TO ROAD TURNS

FIVE HURST DR CORNER LOT

Construction of NEW HOME Commercial OR ☒ Residential

Proposed Use/Occupancy _____ Number of Existing Dwellings on Property 0

Is the Building Fire Sprinkled? NO If Yes, blueprints included _____ Or Explain _____

Circle Proposed - Culvert Permit or Culvert Waiver or D.O.T. Permit or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 50' Side 78' Side 50' Rear 40'

Number of Stories 1 Heated Floor Area 2087 Total Floor Area 3070 Acreage 1/2

Zoning Applications applied for (Site & Development Plan, Special Exception, etc.) _____

41

CODE: Florida Building Code 2014 and the 2011 National Electrical Code.

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

TIME LIMITATIONS OF APPLICATION : An application for a permit for any proposed work shall be deemed to have been abandoned 180 days after the date of filing, unless pursued in good faith or a permit has been issued.

TIME LIMITATIONS OF PERMITS: Every permit issued shall become invalid unless the work authorized by such permit is commenced within 180 days after its issuance, or if the work authorized by such permit is suspended or abandoned for a period of 180 days after the time work is commenced. A valid permit receives an approved inspection every 180 days. Work shall be considered not suspended, abandoned or invalid when the permit has received an approved inspection within 180 days of the previous approved inspection.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment: According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO CONTRACTOR AND AGENT: YOU ARE HEREBY NOTIFIED as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

OWNERS CERTIFICATION: I CERTIFY THAT ALL THE FOREGOING INFORMATION IS ACCURATE AND THAT ALL WORK WILL BE DONE IN COMPLIANCE WITH ALL APPLICABLE LAWS REGULATING CONSTRUCTION AND ZONING.

NOTICE TO OWNER: There are some properties that may have deed restrictions recorded upon them. These restrictions may limit or prohibit the work applied for in your building permit. You must verify if your property is encumbered by any restrictions or face possible litigation and or fines.

DENISE L. JONES

Print Owners Name

DENISE L. JONES

Owners Signature

****Property owners must sign here before any permit will be issued.**

****If this is an Owner Builder Permit Application then, ONLY the owner can sign the building permit when it is issued.**

CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit including all application and permit time limitations.

[Signature]
Contractor's Signature

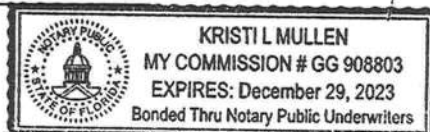
Contractor's License Number CBC047842
Columbia County
Competency Card Number _____

Affirmed under penalty of perjury to by the Contractor and subscribed before me this 10 day of May 2022

Personally known X or Produced Identification _____

[Signature]
State of Florida Notary Signature (For the Contractor)

SEAL:



SUBCONTRACTOR VERIFICATION FORM

65

APPLICATION NUMBER _____ CONTRACTOR _____ PHONE _____

THIS FORM MUST BE SUBMITTED PRIOR TO THE ISSUANCE OF A PERMIT

In Columbia County one permit will cover all trades doing work at the permitted site. It is **REQUIRED** that we have records of the subcontractors who actually did the trade specific work under the permit. Per Florida Statute 440 and Ordinance 89-6, a contractor shall require all subcontractors to provide evidence of workers' compensation or exemption, general liability insurance and a valid Certificate of Competency license in Columbia County.

Any changes, the permitted contractor is responsible for the corrected form being submitted to this office prior to the start of that subcontractor beginning any work. Violations will result in stop work orders and/or fines.

ELECTRICAL	Print Name _____ License #: _____	Signature _____ Phone #: _____
MECHANICAL/ A/C _____	Print Name _____ License #: _____	Signature _____ Phone #: _____
PLUMBING/ GAS	Print Name _____ License #: _____	Signature _____ Phone #: _____
ROOFING	Print Name _____ License #: _____	Signature _____ Phone #: _____
SHEET METAL	Print Name _____ License #: _____	Signature _____ Phone #: _____
FIRE SYSTEM/ SPRINKLER	Print Name _____ License #: _____	Signature _____ Phone #: _____
SOLAR	Print Name _____ License #: _____	Signature _____ Phone #: _____

Specialty License	License Number	Sub-Contractors Printed Name	Sub-Contractors Signature
MASON	CBC047842	CASTAGNA CONS	
CONCRETE FINISHER			
FRAMING			
INSULATION			
STUCCO			
DRYWALL			
PLASTER			
CABINET INSTALLER			
PAINTING			
ACOUSTICAL CEILING			
GLASS			
CERAMIC TILE			
FLOOR COVERING			
ALUM/VINYL SIDING			
GARAGE DOOR			
METAL BLDG ERECTOR			

F. S. 440.103 Building permits; identification of minimum premium policy.--Every employer shall, as a condition to applying for and receiving a building permit, show proof and certify to the permit issuer that it has secured compensation for its employees under this chapter as provided in ss. 440.10 and 440.38, and shall be presented each time the employer applies for a building permit.

SUBCONTRACTOR VERIFICATION

APPLICATION/PERMIT # _____

JOB NAME

Castagna

THIS FORM MUST BE SUBMITTED BEFORE A PERMIT WILL BE ISSUED

Columbia County issues combination permits. One permit will cover all trades doing work at the permitted site. It is **REQUIRED** that we have records of the subcontractors who actually did the trade specific work under the general contractors permit.

NOTE: It shall be the responsibility of the general contractor to make sure that all of the subcontractors are licensed with the Columbia County Building Department.

Use website to confirm licenses: <http://www.columbiacountyfla.com/PermitSearch/ContractorSearch.aspx>

NOTE: If this should change prior to completion of the project, it is your responsibility to have a corrected form submitted to our office, before that work has begun.

Violations will result in stop work orders and/or fines.

ELECTRICAL <input checked="" type="checkbox"/>	Print Name <u>Robert Davis</u> Signature <u>[Signature]</u>	Need: JC Lab W/C EX DE
CC# _____	Company Name: <u>Mountaineer Electric LLC</u>	
	License #: <u>EL13005826</u> Phone #: <u>904-710-3208</u>	
MECHANICAL/ A/C <input type="checkbox"/>	Print Name _____ Signature _____	Need: JC Lab W/C EX DE
CC# _____	Company Name: _____	
	License #: _____ Phone #: _____	
PLUMBING/ GAS <input type="checkbox"/>	Print Name _____ Signature _____	Need: JC Lab W/C EX DE
CC# _____	Company Name: _____	
	License #: _____ Phone #: _____	
ROOFING <input type="checkbox"/>	Print Name _____ Signature _____	Need: JC Lab W/C EX DE
CC# _____	Company Name: _____	
	License #: _____ Phone #: _____	
SHEET METAL <input type="checkbox"/>	Print Name _____ Signature _____	Need: JC Lab W/C EX DE
CC# _____	Company Name: _____	
	License #: _____ Phone #: _____	
FIRE SYSTEM/ SPRINKLER <input type="checkbox"/>	Print Name _____ Signature _____	Need: JC Lab W/C EX DE
CC# _____	Company Name: _____	
	License #: _____ Phone #: _____	
SOLAR <input type="checkbox"/>	Print Name _____ Signature _____	Need: JC Lab W/C EX DE
CC# _____	Company Name: _____	
	License #: _____ Phone #: _____	
STATE SPECIALTY <input type="checkbox"/>	Print Name _____ Signature _____	Need: JC Lab W/C EX DE
CC# _____	Company Name: _____	
	License #: _____ Phone #: _____	

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ELECTRICAL <input type="checkbox"/>	Print Name _____ Signature _____ Company Name: _____ License #: _____ Phone #: _____	Need <input type="checkbox"/> Lic <input type="checkbox"/> Liab <input type="checkbox"/> W/C <input type="checkbox"/> EX <input type="checkbox"/> DE
MECHANICAL/A/C <input checked="" type="checkbox"/>	Print Name <u>David Hall</u> Signature <u>D Hall</u> Company Name: <u>David Hall's A/C and Heating</u> License #: <u>CAC057424</u> Phone #: <u>(386) 255-9792</u>	Need <input type="checkbox"/> Lic <input type="checkbox"/> Liab <input type="checkbox"/> W/C <input type="checkbox"/> EX <input type="checkbox"/> DE
PLUMBING/GAS <input type="checkbox"/>	Print Name _____ Signature _____ Company Name: _____ License #: _____ Phone #: _____	Need <input type="checkbox"/> Lic <input type="checkbox"/> Liab <input type="checkbox"/> W/C <input type="checkbox"/> EX <input type="checkbox"/> DE
ROOFING <input type="checkbox"/>	Print Name _____ Signature _____ Company Name: _____ License #: _____ Phone #: _____	Need <input type="checkbox"/> Lic <input type="checkbox"/> Liab <input type="checkbox"/> W/C <input type="checkbox"/> EX <input type="checkbox"/> DE
SHEET METAL <input type="checkbox"/>	Print Name _____ Signature _____ Company Name: _____ License #: _____ Phone #: _____	Need <input type="checkbox"/> Lic <input type="checkbox"/> Liab <input type="checkbox"/> W/C <input type="checkbox"/> EX <input type="checkbox"/> DE
FIRE SYSTEM/SPRINKLER <input type="checkbox"/>	Print Name _____ Signature _____ Company Name: _____ License #: _____ Phone #: _____	Need <input type="checkbox"/> Lic <input type="checkbox"/> Liab <input type="checkbox"/> W/C <input type="checkbox"/> EX <input type="checkbox"/> DE
SOLAR <input type="checkbox"/>	Print Name _____ Signature _____ Company Name: _____ License #: _____ Phone #: _____	Need <input type="checkbox"/> Lic <input type="checkbox"/> Liab <input type="checkbox"/> W/C <input type="checkbox"/> EX <input type="checkbox"/> DE
STATE SPECIALTY <input type="checkbox"/>	Print Name _____ Signature _____ Company Name: _____ License #: _____ Phone #: _____	Need <input type="checkbox"/> Lic <input type="checkbox"/> Liab <input type="checkbox"/> W/C <input type="checkbox"/> EX <input type="checkbox"/> DE

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MECHANICAL/A/C <input type="checkbox"/>	Print Name _____ Signature _____ Company Name: _____ CC# _____ License #: _____ Phone #: _____	Need: <input type="checkbox"/> Lic <input type="checkbox"/> Lab <input type="checkbox"/> W/C <input type="checkbox"/> EX <input type="checkbox"/> DE
PLUMBING/GAS <input type="checkbox"/>	Print Name <i>Frank Soucinick</i> Signature <i>[Signature]</i> Company Name: <i>Dependable Plumbing</i> CC# _____ License #: <i>CFC057747</i> Phone #: <i>352-752-5218</i>	Need: <input type="checkbox"/> Lic <input type="checkbox"/> Lab <input type="checkbox"/> W/C <input type="checkbox"/> EX <input type="checkbox"/> DE
ROOFING <input type="checkbox"/>	Print Name _____ Signature _____ Company Name: _____ CC# _____ License #: _____ Phone #: _____	Need: <input type="checkbox"/> Lic <input type="checkbox"/> Lab <input type="checkbox"/> W/C <input type="checkbox"/> EX <input type="checkbox"/> DE
SHEET METAL <input type="checkbox"/>	Print Name _____ Signature _____ Company Name: _____ CC# _____ License #: _____ Phone #: _____	Need: <input type="checkbox"/> Lic <input type="checkbox"/> Lab <input type="checkbox"/> W/C <input type="checkbox"/> EX <input type="checkbox"/> DE
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MECHANICAL/ A/C <input type="checkbox"/>	Print Name _____ Signature _____ Company Name: _____ License #: _____ Phone #: _____	Need: [] Lic [] Lab [] W/C [] EX [] DE
PLUMBING/ GAS <input type="checkbox"/>	Print Name _____ Signature _____ Company Name: _____ License #: _____ Phone #: _____	Need: [] Lic [] Lab [] W/C [] EX [] DE
ROOFING <input type="checkbox"/>	Print Name <u>JERRY CASTAGNA</u> Signature <u>[Signature]</u> Company Name: <u>Castagna Const. Inc.</u> License #: <u>CBC 047842</u> Phone #: <u>386-755-6867</u>	Need: [] Lic [] Lab [] W/C [] EX [] DE
SHEET METAL <input type="checkbox"/>	Print Name _____ Signature _____ Company Name: _____ License #: _____ Phone #: _____	Need: [] Lic [] Lab [] W/C [] EX [] DE
FIRE SYSTEM/ SPRINKLER <input type="checkbox"/>	Print Name _____ Signature _____ Company Name: _____ License #: _____ Phone #: _____	Need: [] Lic [] Lab [] W/C [] EX [] DE
SOLAR <input type="checkbox"/>	Print Name _____ Signature _____ Company Name: _____ License #: _____ Phone #: _____	Need: [] Lic [] Lab [] W/C [] EX [] DE
STATE SPECIALTY <input type="checkbox"/>	Print Name _____ Signature _____ Company Name: _____ License #: _____ Phone #: _____	Need: [] Lic [] Lab [] W/C [] EX [] DE



COLUMBIA COUNTY BUILDING DEPARTMENT
135 NE Hernando Ave, Suite B-21, Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

12

LETTER OF AUTHORIZATION TO SIGN FOR PERMITS

I, JERRY CASTAGNA (license holder name), licensed qualifier
for CASTAGNA CONS INC (company name), do certify that
the below referenced person(s) listed on this form is/are contracted/hired by me, the license
holder, or is/are employed by me directly or through an employee leasing arrangement; or, is an
officer of the corporation; or, partner as defined in Florida Statutes Chapter 468, and the said
person(s) is/are under my direct supervision and control and is/are authorized to purchase
permits, call for inspections and sign on my behalf.

Printed Name of Person Authorized	Signature of Authorized Person
1. <u>JERRY CASTAGNA</u>	1. <u>[Signature]</u>
2.	2.
3.	3.
4.	4.
5.	5.

I, the license holder, realize that I am responsible for all permits purchased, and all work done
under my license and fully responsible for compliance with all Florida Statutes, Codes, and
Local Ordinances. I understand that the State and County Licensing Boards have the power and
authority to discipline a license holder for violations committed by him/her, his/her agents,
officers, or employees and that I have full responsibility for compliance with all statutes, codes
and ordinances inherent in the privilege granted by issuance of such permits.

If at any time the person(s) you have authorized is/are no longer agents, employee(s), or
officer(s), you must notify this department in writing of the changes and submit a new letter of
authorization form, which will supersede all previous lists. Failure to do so may allow
unauthorized persons to use your name and/or license number to obtain permits.

[Signature] 086047842 Feb 1 2022
License Holders Signature (Notarized) License Number Date

NOTARY INFORMATION:

STATE OF: Florida COUNTY OF: Columbia

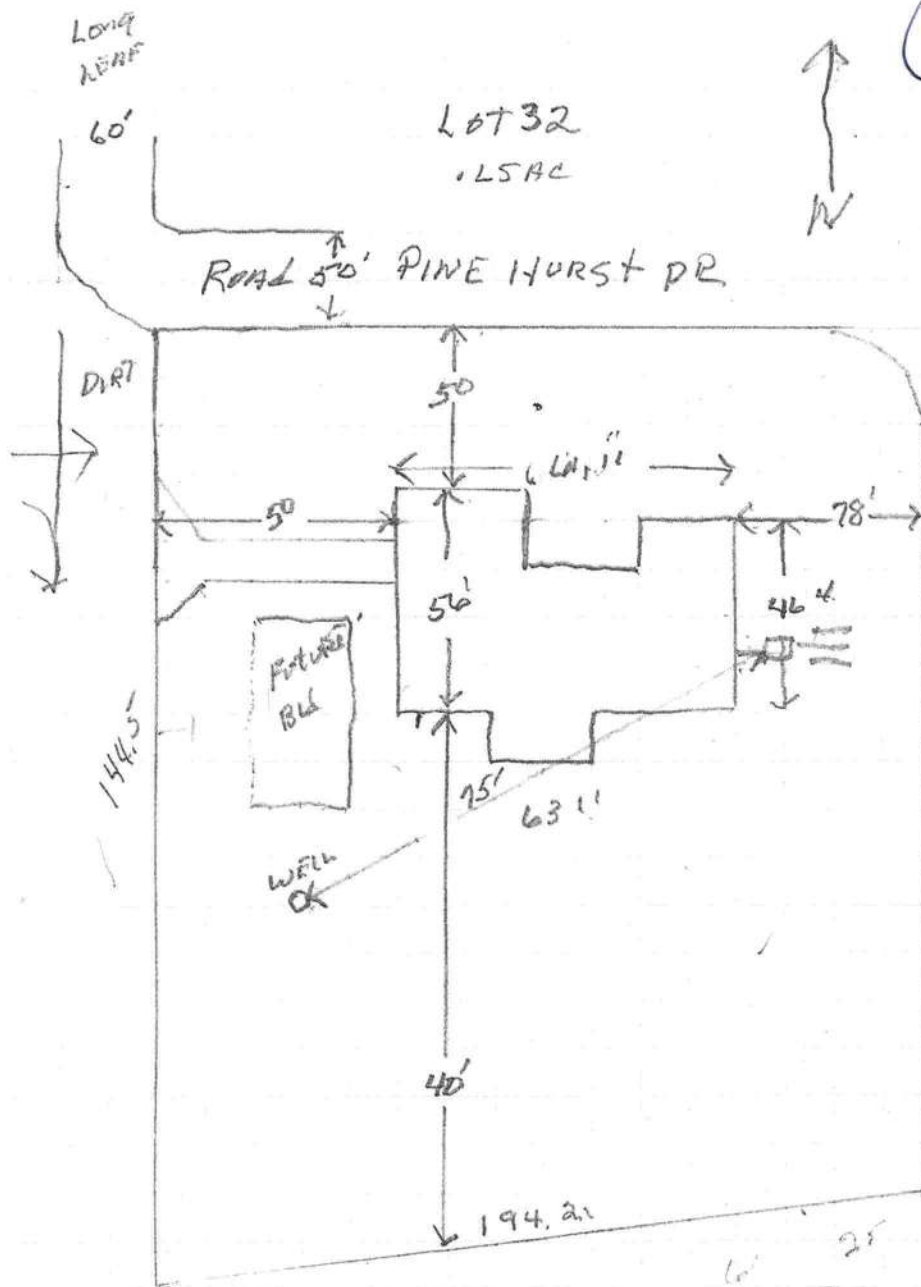
The above license holder, whose name is Jerry Castagna,
personally appeared before me and is known by me or has produced identification
(type of I.D.) _____ on this 10 day of May, 2022.

Kristi L Mullen
NOTARY'S SIGNATURE

(Seal/Stamp)



58



Columbia County Property Appraiser

Jeff Hampton

2021 Working Values

updated: 5/6/2021

Parcel: << 21-4S-16-03087-132 (14713) >>

Aerial Viewer Pictometry Google Maps

2019 2016 2013 2010 2007 2005 Sales

Owner & Property Info

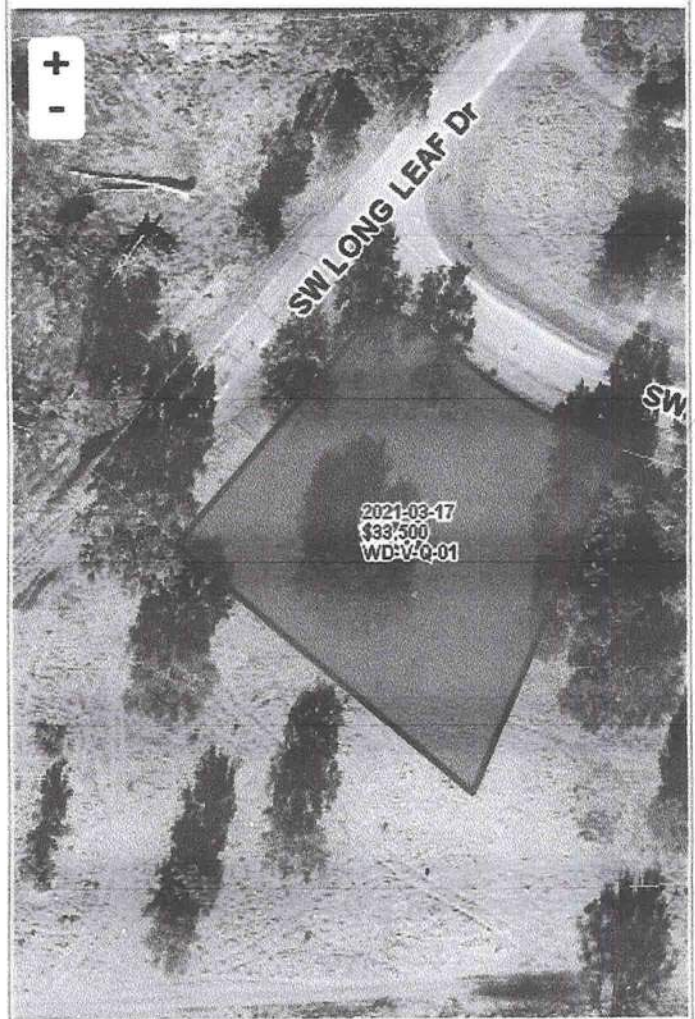
Owner	JONES LEON L III JONES DENISE L 591 NORTH RACQUETTE RIVER ROAD MASSENA, NY 13662		
Site	120 PINEHURST DR, LAKE CITY		
Description*	LOT 32 FOREST COUNTRY 6TH ADDITION. WD 1338-610, WD 1433-451,		
Area	0.69 AC	S/T/R	21-4S-16E
Use Code**	VACANT (0000)	Tax District	3

*The Description above is not to be used as the Legal Description for this parcel in any legal transaction.

**The Use Code is a FL Dept. of Revenue (DOR) code and is not maintained by the Property Appraiser's office. Please contact your city or county Planning & Zoning office for specific zoning information.

Property & Assessment Values

2020 Certified Values		2021 Working Values	
Mkt Land	\$26,200	Mkt Land	\$26,200
Ag Land	\$0	Ag Land	\$0
Building	\$0	Building	\$0
XFOB	\$0	XFOB	\$0
Just	\$26,200	Just	\$26,200
Class	\$0	Class	\$0
Appraised	\$26,200	Appraised	\$26,200
SOH Cap [?]	\$0	SOH Cap [?]	\$0
Assessed	\$26,200	Assessed	\$26,200
Exempt	\$0	Exempt	\$0
Total Taxable	county:\$26,200 city:\$26,200 other:\$26,200 school:\$26,200	Total Taxable	county:\$26,200 city:\$0 other:\$0 school:\$26,200



Sales History

Sale Date	Sale Price	Book/Page	Deed	V/I	Qualification (Codes)	RCode
3/17/2021	\$33,500	1433/0451	WD	V	Q	01
5/17/2017	\$0	1338/0610	WD	V	U	11

Building Characteristics

Bldg Sketch	Description*	Year Blt	Base SF	Actual SF	Bldg Value
NONE					

Extra Features & Out Buildings (Codes)

Code	Desc	Year Blt	Value	Units	Dims
NONE					

Land Breakdown

Code	Desc	Units	Adjustments	Eff Rate	Land Value
0000	VAC RES (MKT)	1.000 LT (0.690 AC)	1.0000/1.0000 1.0000/ /	\$26,200 /LT	\$26,200

37

June 1, 2022

To: Union County Building Department

A&B Well Drilling, Inc.

5673 NW Lake Jeffery Road
Lake City, FL 32055
Telephone: (386) 758-3409
Cell: (386) 823-3151
Fax: (386) 758-3410
Owner: Bruce Park

Description of Well to be installed for Customer ____ Lee Jones ____

Located @ Address: ____ Lot 32 Forest Country. ____

1 HP 20 GPM submersible pump, 1 1/4" drop pipe, 85 gallon captive tank, and backflow prevention.
With SRWMD permit.

____ Bruce Park ____

Sincerely,
Bruce N. Park
President

A & B Construction Inc.
546 SW Dortch St. Fort White Fl. 32038
(O) 386-497-2311
(F) 386-497-4866

ATTENTION: Jerry Castagna Date 6-16-22
THIS IS AN ESTIMATE FOR: Customer Jones

Septic: Tank Size 400 Drain Field Size 282 Fill _____ \$ 8500.00
(THIS IS AN ESTIMATE, EXACT PRICES GIVEN ONLY AFTER SOIL SAMPLE)

Septic System Permit: \$ 575.00

Lift Station: \$ _____

~~Sewer Connections:~~ ~~\$ 200.00~~

Pump & Certify: \$ _____

Driveway: _____ \$ _____

House Pad: _____ \$ _____

Build and Spre _____ \$ _____

Other: _____ \$ _____

_____ \$ _____

_____ \$ _____

_____ \$ _____

Total: \$ 9275.00

Please assign PO 9075.00

Thank you,

Kelly

Permit #
22-0557

Contract prices are only good for 30 days. Prices may increase if materials go up 10% or more from the time the contract is signed to the time the work is completed.

June 1, 2022

To: Union County Building Department

A&B Well Drilling, Inc.

5673 NW Lake Jeffery Road
Lake City, FL 32055
Telephone: (386) 758-3409
Cell: (386) 623-3151
Fax: (386) 758-3410
Owner: Bruce Park

Description of Well to be installed for Customer _____ Lee Jones _____

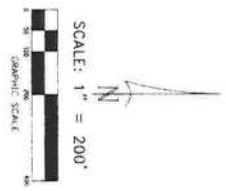
Located @ Address: _____ Lot 32 Forest Country. _____

1 HP 20 GPM submersible pump, 1 1/4" drop pipe, 85 gallon captive tank, and backflow prevention.
With SRWMD permit.

Bruce Park _____

Sincerely,
Bruce N. Park
President

PLAT BOOK 9
PAGES 67
SHEET 2 OF 2



HENRY & SON, INC. 1000 E. 11th, OMAHA, NE 68102
 (402) 733-7153 FAX (402) 733-2573
 WIRE 1361400-1 **16670**

NOTICE. ALL RENTED UTILITY FACILITIES SHALL PROVIDE THAT SUCH FACILITIES SHALL ALSO BE FOR THE EXCLUSIVE INSTALLATION, MAINTENANCE, AND OPERATION OF CABLE TELEVISION SERVICES. PROVIDER, CONTRACTOR, SIGN CONSTRUCTION, INSTALLATION, MAINTENANCE, AND OPERATION OF CABLE TELEVISION SERVICES SHALL INTERFERE WITH THE FACILITIES AND SERVICES OF ELECTRIC, TELEPHONE, GAS OR OTHER UTILITY IN THE EVENT THAT A CABLE TELEVISION COMPANY DAMAGES THE FACILITIES OF A PUBLIC UTILITY, IT SHALL BE SILENTLY RESPONSIBLE FOR THE DAMAGES.

UNLESS OTHERWISE SHOWN HEREIN ALL CUSTOMER SHOWN HEREIN ARE U.S. BIRTH PL. S. 85757

UPDATED

44

Sheet1

FLORIDA PRODUCT APPROVALS

1/5/2022

ITEM:	MANUFACTURER:	PRODCUT DESCRIPTION:	APPROVAL NUMBER:
EXTERIOR DOORS:	MASONITE	IN swing & OUT swing FIBERGLASS	FL-5507-R9
	MASONITE	IN swing & OUT swing STEEL	FL-5465-R10
	PLASTPRO	8' IN swing & OUT swing FIBERGLASS	FL-15220-R3
	PLASTPRO	IN swing & OUT swing STEEL	
		6'8" FIBERGLASS DOOR	FL-17347-R4
WINDOWS:	MI	ALUMINUM 185 SINGLE HUNG	FL-17499-R8
		ALUMINUM 185 PICTURE WINDOW	FL-15349-R15
		53"x50" SLIDER	FL-13349-R8
		VINYL 3540 SNGLE HUNG	FL-17676-R19
		VINYL 3540 PICTURE WINDOW	FL-18644-R7
	ATRIUM	150/160	
	MAGNOLIA	VINYL 400 SINGLE HUNG	
		VINYL 400 PICTURE WINDOW	
SOFFIT:	KAYCAN	VINYL/PVC & ALUMINUM SOFFIT	FL-16503-R6
		VINYL SIDING	FL-12192-R7
UNDERLAYMENT:	WOODLAND	30# FELT	FL-17206-R6
	LCI HOUSE WRAP	WRAP WITH LOGO	ESR3774
	INTERWRAP	RHINO	FL-15216-R10
ROOFING:	CERTAINTED	ASPHALT SHINGLES	FL-5444-R17
	GAF	ASPHALT SHINGLES	FL-10124-R31
	TAMKO	ASPHALT SHINGLES	FL-18355-R6
SIDING:	ALLURA OF PLYCEN	CEMENT BOARD LAP SIDING	
	JAMES HARDIE	CEMENT BOARD LAP SIDING	FL-13192-R6
		SHEET PANELS	FL-13223-R6
SIMPSON:		LSTA-MSTA, SPH4	FL-13872-R4
	GAF	TIGER PAW UNDERLAYMENT	FL-10626-R22
METAL ROOFING:		5V ROOFING	FL-9555-R5
		MASTER RIB ROOFING	FL-9557-R5
		WOOD CONNECTORS	FL-9589-R5
SIMONTON		VINYL WINDOW SH	FL 5414-R31



COLUMBIA COUNTY BUILDING DEPARTMENT RESIDENTIAL CHECK LIST

48

MINIMUM PLAN REQUIREMENTS: FLORIDA BUILDING CODE RESIDENTIAL 2020 EFFECTIVE 1 JANUARY 2021
AND THE NATIONAL ELECTRICAL 2017 EFFECTIVE 1 JANUARY 2021

ALL REQUIREMENTS ARE SUBJECT TO CHANGE

ALL BUILDING PLANS MUST INDICATE COMPLIANCE WITH THE CURRENT FLORIDA BUILDING CODES RESIDENTIAL AND THE NATIONAL ELECTRICAL CODE. ALL PLANS OR DRAWINGS SHALL PROVIDE CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS, FBC 1609.1 THRU 1609.6.

FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEEDS ARE PER FLORIDA BUILDING CODE FIGURE 1609.3(1) THROUGH 1609.3(4) ULTIMATE DESIGN WIND SPEEDS FOR RISK CATEGORY AND BUILDINGS AND OTHER STRUCTURES Revised 7/1/20

Submit Online at- <http://www.columbiacountyfla.com/BuildingandZoning.asp>

Items to Include-
Each Box shall be
Circled as
Applicable

GENERAL REQUIREMENTS:

APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

Select From Drop down

1	Two (2) complete sets of plans containing the following:	<input checked="" type="checkbox"/>			
2	All drawings must be clear, concise, drawn to scale, details that are not used shall be marked void	<input checked="" type="checkbox"/>			
3	Condition space (Sq. Ft.)	3087	Total (Sq. Ft.) under roof	3070	Yes No NA

Designers name and signature shall be on all documents and a licensed architect or engineer, signature and official embossed seal shall be affixed to the plans and documents as per the FLORIDA BUILDING CODES BUILDING 107.1.

Site Plan information including:

4	Dimensions of lot or parcel of land	- 69		
5	Dimensions of all building set backs	-		
6	Location of all other structures (include square footage of structures) on parcel, existing or proposed well and septic tank and all utility easements.	-		
7	Provide a full legal description of property.	-		

Wind-load Engineering Summary, calculations and any details are required.

GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL		Items to Include- Each Box shall be Circled as Applicable		
8	Plans or specifications must show compliance with FBCR Chapter 3	Yes	No	NA
Select From Drop down				
9	Basic wind speed (3-second gust), miles per hour	-		
10	(Wind exposure – if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated)	-		
11	Wind importance factor and nature of occupancy	-		
12	The applicable internal pressure coefficient, Components and Cladding	-		
13	The design wind pressure in terms of psf (kN/m ²), to be used for the design of exterior component, cladding materials not specifically designed by the registered design professional.	-		

Elevations Drawing including:

14	All side views of the structure	-		
15	Roof pitch	-		
16	Overhang dimensions and detail with attic ventilation	-		
17	Location, size and height above roof of chimneys	-		
18	Location and size of skylights with Florida Product Approval	-		
19	Number of stories	-		
20	Building height from the established grade to the roofs highest peak	-		

Floor Plan Including:

21	Dimensioned area plan showing rooms, attached garage, breeze ways, covered porches, deck, balconies	- ✓		
22	Raised floor surfaces located more than 30 inches above the floor or grade	- ✓		
23	All exterior and interior shear walls indicated	- ✓		
24	Shear wall opening shown (Windows, Doors and Garage doors)	- ✓		
25	Show compliance with Section FBCR 310 Emergency escape and rescue opening shown in each bedroom (net clear opening shown) and Show compliance with Section FBCR 312.2.1 where the opening of an operable window is located more than 72 inches above the finished grade or surface below, the lowest part of the clear opening of the window shall be a minimum of 24 inches above the finished floor of the room in which the window is located. Glazing between the floor and 24 inches shall be fixed or have openings through which a 4-inch-diameter sphere cannot pass.	- NA		
26	Safety glazing of glass where needed	-		
27	Fireplaces types (gas appliance) (vented or non-vented) or wood burning with Hearth (see chapter 10 and chapter 24 of FBCR)	- NA		
28	Show stairs with dimensions (width, tread and riser and total run) details of guardrails, Handrails	-		
29	Identify accessibility of bathroom (see FBCR SECTION 320)	-		

All materials placed within opening or onto/into exterior walls, soffits or roofs shall have Florida product approval number and mfg. installation information submitted with the plans (see Florida product approval form)

GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL		Items to Include- Each Box shall be Circled as Applicable
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FBCR 403: Foundation Plans

		Select From Drop down		
30	Location of all load-bearing walls footings indicated as standard, monolithic, dimensions, size and type of reinforcing.	- ✓		
31	All posts and/or column footing including size and reinforcing	- ✓		
32	Any special support required by soil analysis such as piling.	- ✓		
33	Assumed load-bearing value of soil Pound Per Square Foot	- ✓		
34	Location of horizontal and vertical steel, for foundation or walls (include # size and type) For structures with foundation which establish new electrical utility companies service connection a Concrete Encased Electrode will be required within the foundation to serve as an grounding electrode system. Per the National Electrical Code article 250.52.3	- ✓		

FBCR 506: CONCRETE SLAB ON GRADE

35	Show Vapor retarder (6mil. Polyethylene with joints overlaid 6 inches and sealed)	- ✓		
36	Show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and Supports	- ✓		

FBCR 318: PROTECTION AGAINST TERMITES

37	Indicate on the foundation plan if soil treatment is used for subterranean termite prevention or Submit other approved termite protection methods. Protection shall be provided by registered termiticides	- ✓		
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FBCR 606: Masonry Walls and Stem walls (load bearing & shear Walls)

38	Show all materials making up walls, wall height, and Block size, mortar type	- ✓		
39	Show all Lintel sizes, type, spans and tie-beam sizes and spacing of reinforcement	-		

Metal frame shear wall and roof systems shall be designed, signed and sealed by Florida Prof. Engineer or Architect

Floor Framing System: First and/or second story

40	Floor truss package shall including layout and details, signed and sealed by Florida Registered Professional Engineer	- ✓		
41	Show conventional floor joist type, size, span, spacing and attachment to load bearing walls, stem walls and/or piers	- ✓		
42	Girder type, size and spacing to load bearing walls, stem wall and/or piers	- ✓		
43	Attachment of joist to girder	- ✓		
44	Wind load requirements where applicable	- ✓		
45	Show required under-floor crawl space	- ✓		
46	Show required amount of ventilation opening for under-floor spaces	- ✓		
47	Show required covering of ventilation opening	- ✓		
48	Show the required access opening to access to under-floor spaces	- ✓		
49	Show the sub-floor structural panel sheathing type, thickness and fastener schedule on the edges & intermediate of the areas structural panel sheathing	- ✓		
50	Show Draftstopping, Fire caulking and Fire blocking	- ✓		
51	Show fireproofing requirements for garages attached to living spaces, per FBCR section 302.6	- ✓		
52	Provide live and dead load rating of floor framing systems (psf).	- ✓		

FBCR CHAPTER 6 WOOD WALL FRAMING CONSTRUCTION

GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL		Items to Include- Each Box shall be Circled as Applicable		
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Select from Drop down

53	Stud type, grade, size, wall height and oc spacing for all load bearing or shear walls	- ✓		
54	Fastener schedule for structural members per table FBC 2304.10.1 are to be shown	- ✓		
55	Show wood structural panel's sheathing attachment to studs, joist, trusses, rafters and structural members, showing fastener schedule attachment on the edges & intermediate of the areas structural panel sheathing	- ✓		
56	Show all required connectors with a max uplift rating and required number of connectors and oc spacing for continuous connection of structural walls to foundation and roof trusses or rafter systems	- ✓		
57	Show sizes, type, span lengths and required number of support jack studs, king studs for shear wall opening and girder or header per FBC 2304.3.	- ✓		
58	Indicate where pressure treated wood will be placed	- ✓		
59	Show all wall structural panel sheathing, grade, thickness and show fastener schedule for structural panel sheathing edges & intermediate areas	- ✓		
60	A detail showing gable truss bracing, wall balloon framing details or/and wall hinge bracing detail	- ✓		

FBC :ROOF SYSTEMS:

61	Truss design drawing shall meet section FBC 2303.1.1.1 Wood trusses	- ✓		
62	Include a layout and truss details, signed and sealed by Florida Professional Engineer	-		
63	Show types of connector's assemblies' and resistance uplift rating for all trusses and rafters	-		
64	Show gable ends with rake beams showing reinforcement or gable truss and wall bracing details	-		
65	Provide dead load rating of trusses	-		

FBC 2304.4:Conventional Roof Framing Layout

66	Rafter and ridge beams sizes, span, species and spacing	-		
67	Connectors to wall assemblies' include assemblies' resistance to uplift rating	-		
68	Valley framing and support details	-		
69	Provide dead load rating of rafter system	-		

FBC 2304.8 ROOF SHEATHING

70	Include all materials which will make up the roof decking, identification of structural panel sheathing, grade, thickness	- ✓		
71	Show fastener Size and schedule for structural panel sheathing on the edges & intermediate areas	- ✓		

ROOF ASSEMBLIES FRC Chapter 15

72	Include all materials which will make up the roof assemblies covering	-		
73	Submit Florida Product Approval numbers for each component of the roof assemblies covering	-		

FBC Energy Chapter 4

Residential construction shall comply with this code by using the following compliance methods in the FBC Chapter 4, Residential buildings compliance methods. **Two of the required forms are to be submitted, N1100.1.1.1 As an alternative to the computerized Compliance Method A, the Alternate Residential Point System Method hand calculation, Alternate Form 600A, may be used. All requirements specific to this calculation are located in Sub appendix C to Appendix G. Buildings complying by this alternative shall meet all mandatory requirements of this chapter. Computerized versions of the Alternate Residential Point System Method shall not be acceptable for code compliance.**

GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL		Items to Include- Each Box shall be Circled as Applicable		
Select from Drop Down				
74	Show the insulation R value for the following areas of the structure	-	✓	
75	Attic space	-	✓	
76	Exterior wall cavity	-	✓	
77	Crawl space	-		

HVAC information

78	Submit two copies of a Manual J sizing equipment or equivalent computation study	-	✓	
79	Exhaust fans shown in bathrooms Mechanical exhaust capacity of 50 cfm intermittent or 20 cfm continuous required	-	✓	
80	Show clothes dryer route and total run of exhaust duct	-		

Plumbing Fixture layout shown

81	All fixtures waste water lines shall be shown on the foundation plan	-	✓	
82	Show the location of water heater	-	✓	

Private Potable Water

83	Pump motor horse power	-	✓	
84	Reservoir pressure tank gallon capacity	-	✓	
85	Rating of cycle stop valve if used	-		

Electrical layout shown including

86	Show Switches, receptacles outlets, lighting fixtures and Ceiling fans	-	✓	
87	Show all 120-volt, single phase, 15- and 20-ampere branch circuits outlets required to be protected by Ground-Fault Circuit Interrupter (GFCI) Article 210.8 A	-	✓	
88	Show the location of smoke detectors & Carbon monoxide detectors	-	✓	
89	Show service panel, sub-panel, location(s) and total ampere ratings	-	✓	
90	On the electrical plans identify the electrical service overcurrent protection device for the main electrical service. This device shall be installed on the exterior of structures to serve as a disconnecting means for the utility company electrical service. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground. Indicate if the utility company service entrance cable will be of the overhead or underground type. For structures with foundation which establish new electrical utility companies service connection a Concrete Encased Electrode will be required within the foundation to serve as an Grounding electrode system. Per the National Electrical Code article 250.52.3	-	✓	
91	Appliances and HVAC equipment and disconnects	-	✓	
92	Show all 120-volt, single phase, 15- and 20-ampere branch circuits supplying outlets installed in dwelling unit family rooms, dining rooms, living rooms, parlors, libraries, dens, bedrooms, sunrooms, recreation rooms, closets, hallways, or similar rooms or areas shall be protected by a listed Combination arc-fault circuit interrupter, Protection device.	-	✓	

Notice Of Commencement:

A notice of commencement form **RECORDED** in the Columbia County Clerk Office is required to be filed with the Building Department **BEFORE ANY INSPECTIONS** can be performed.

GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL	Items to Include- Each Box shall be Circled as Applicable
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****ITEMS 95, 96, & 98 Are Required After APPROVAL from the ZONING DEPT.****

Select from Drop down

93	Building Permit Application A current Building Permit Application is to be completed, by following the Checklist all supporting documents must be submitted. There is a \$15.00 application fee. The completed application with attached documents and application fee can be mailed.	- /		
94	Parcel Number The parcel number (Tax ID number) from the Property Appraisers Office (386) 758-1083 is required. A copy of property deed is also required. www.columbiacountyfla.com	- /		
95	Environmental Health Permit or Sewer Tap Approval A copy of a approved Columbia County Environmental Health (386) 758-1058	- /		
96	City of Lake City A City Water and/or Sewer letter. Call 386-752-2031	- /		
97	Toilet facilities shall be provided for all construction sites	- /		
98	Town of Fort White (386) 497-2321 If the parcel in the application for building permit is within the Corporate city limits of Fort White, an approval land use development letter issued by the Town of Fort is required to be submitted with the application for a building permit.	-		
99	Flood Information: All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting a application to this office. Any project located within a flood zone where the base flood elevation (100 year flood) has been established shall meet the requirements of Section 8.5.2 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.5.3 of the Columbia County Land Development Regulations (Municode.com)	-		
100	CERTIFIED FINISHED FLOOR ELEVATIONS will be required on any project where the approved FIRM Flood Maps show the property is in a AE, Floodway, and AH flood zones. Additionally One Foot Rise letters are required for AE and AH zones. In the Floodway Flood zones a Zero Rise letter is required.	- /		
101	A Flood development permit is also required for AE, Floodway & AH. Development permit cost is \$50.00	-		
102	Driveway Connection: If the property does not have an existing access to a public road, then an application for a culvert permit (\$25.00) must be made. County Public Works Dept. determines the size and length of every culvert before instillation and completes a final inspection before permanent power is granted. If the applicant feels that a culvert is not needed, they may apply for a culvert waiver (\$50.00) Separate Check when issued. If the project is to be located on an F.D.O.T. maintained road, then an F.D.O.T. access permit is required.	- /		
103	911 Address: An application for a 911 address must be applied for and received through the Columbia County Emergency Management Office of 911 Addressing Department (386) 758-1125.	- /		

Ordinance Sec. 90-75. - Construction debris. (e) It shall be unlawful for any person to dispose of or discard solid waste, including construction or demolition debris at any place within the county other than on an authorized disposal site or at the county's solid waste facilities. The temporary storage, not to exceed seven days of solid waste (excluding construction and demolition debris) on the premises where generated or vegetative trash pending disposition as authorized by law or ordinance, shall not be deemed a violation of this section. The temporary storage of construction and demolition debris on the premises where generated or vegetative trash pending disposition as authorized by law or ordinance shall not be deemed in violation of this section; provided, however, such construction and demolition debris must be disposed of in accordance with this article prior to the county's issuance of a certificate of occupancy for the premises. The burning of lumber from a construction or demolition project or vegetative trash when done so with legal and proper permits from the authorized agencies and in accordance with such agencies' rules and regulations, shall not be deemed a violation of this section. No person shall bury, throw, place, or deposit, or cause to be buried, thrown, placed, or deposited, any solid waste, special waste, or debris of any kind into or on any of the public streets, road right-of-way, highways, bridges, alleys, lanes, thoroughfares, waters, canals, or vacant lots or lands within the county. No person shall bury any vegetative trash on any of the public streets, road right-of-way, highways, bridges, lanes, thoroughfares, waters, canals, or lots less than ten acres in size within the county.

Disclosure Statement for Owner Builders:

If you as the Applicant will be acting as your own contractor or owner/builder under section 489.103(7) Florida Statutes, you must submit the required notarized Owner Builder Disclosure Statement form.

****This form can be printed from the Columbia County Website on the Building and Zoning page under Documents. Web address is - <http://www.columbiacountyfla.com/BuildingandZoning.asp>**

Section 105 of the Florida Building Code defines the:

Time limitation of application.

An application for a permit for any proposed work shall be deemed to have been abandoned 180 days after the date of filing, unless such application has been pursued in good faith or a permit has been issued; except that the building official is authorized to grant one or more extensions of time for additional periods not exceeding 90 days each. The extension shall be requested in writing and justifiable cause demonstrated.

Single-family residential dwelling.

Section 105.3.4 A building permit for a single-family residential dwelling must be issued within 30 working days of application therefor unless unusual circumstances require a longer time for processing the application or unless the permit application fails to satisfy the Florida Building Code or the enforcing agency's laws or ordinances.

Permit intent.

Section 105.4.1: A permit issued shall be constructed to be a license to proceed with the work and not as authority to violate, cancel, alter or set aside any of the provisions of the technical codes, nor shall issuance of a permit prevent the building official from thereafter requiring a correction of errors in plans, construction or violations of this code. Every permit issued shall become invalid unless the work authorized by such permit is commenced within six months after its issuance, or if the work authorized by such permit is suspended or abandoned for a period of six months after the time the work is commenced.

If work has commenced.

Section 105.4.1.1: If work has commenced and the permit is revoked, becomes null and void, or expires because of lack of progress or abandonment, a new permit covering the proposed construction shall be obtained before proceeding with the work.

New Permit.

Section 105.4.1.2: If a new permit is not obtained within 180 days from the date the initial permit became null and void, the building official is authorized to require that any work which has been commenced or completed be removed from the building site. Alternately, a new permit may be issued on application, providing the work in place and required to complete the structure meets all applicable regulations in effect at the time the initial permit became null and void and any regulations which may have become effective between the date of expiration and the date of issuance of the new permit.

Work Shall Be:

Section 105.4.1.3: Work shall be considered to be in active progress when the permit has received an approved inspection within 180 days. This provision shall not be applicable in case of civil commotion or strike or when the building work is halted due directly to judicial injunction, order or similar process.

The Fee:

Section 105.4.1.4: The fee for renewal reissuance and extension of a permit shall be set forth by the administrative authority.

Notification:

When the application is approved for permitting the applicant will be notified by phone as to the status by the Columbia County Building & Zoning Department.

FLORIDA BUILDING CODE, ENERGY CONSERVATION
Residential Building Thermal Envelope Approach
R-Value Computation Method

FORM R402—2020

Florida Climate Zone **2A**PROJECT NAME
AND ADDRESS:*Jones Residence*BUILDER: *Castagna Construction*

PERMITTING OFFICE:

JURISDICTION NUMBER:

OWNER:

PERMIT NUMBER:

PERMIT TYPE:

NUMBER OF UNITS: *1*

WORST CASE?

CONDITIONED FLOOR AREA: *2087.4 SF*

Scope: Compliance with Section R402.1.2 of the *Florida Building Code, Energy Conservation*, shall be demonstrated by the use of Form R402 for single- and multiple-family residences of three stories or less in height, additions to existing residential buildings, alterations, renovations and building systems in existing buildings, as applicable. To comply, a building must meet or exceed all of the energy efficiency requirements and applicable mandatory requirements summarized on this form. If a building does not comply with this method, or by the UA Alternative method, it may still comply under Section R405 or R406 of the *Florida Building Code, Energy Conservation*.

General Instructions:

1. Fill in all the applicable spaces of the "INSTALLED" row in the INSULATION AND FENESTRATION REQUIREMENTS BY COMPONENT table with the information requested. All "INSTALLED" values must be equal to or more efficient than the required levels. "AVG" indicates an area weighted average is allowed; "LOWEST" indicates the lowest R-value to be installed must be entered.
2. Complete the tables for air infiltration and installed equipment.
3. Read the MANDATORY REQUIREMENTS table and check each box to indicate your intent to comply with all applicable items.
4. Read, sign and date the "Prepared By" certification statement at the bottom of this form. The owner or owner's agent must also sign and date the form.

INSULATION AND FENESTRATION REQUIREMENTS BY COMPONENT¹

REQUIREMENTS	FENESTRATION U-FACTOR ^{2,3,4}	SKYLIGHT ⁷ U-FACTOR	GLAZED FENESTRATION SHGC ^{5,6}	CEILING R-VALUE	WOOD FRAME WALL R- VALUE ⁸	MASS WALL R-VALUE ⁸	FLOOR R-VALUE	BASEMENT WALL R- VALUE	SLAB ⁹ R- VALUE & DEPTH	CRAWL SPACE WALL R- VALUE
CLIMATE ZONE 1	NR	0.75	0.25	30	13	3/4	13	0	0	0
CLIMATE ZONE 2	0.40	0.65	0.25	38	13	4/5	13	0	0	0
VALUE	AVG	AVG	AVG	LOWEST	LOWEST	LOWEST	LOWEST	LOWEST	LOWEST	LOWEST
INSTALLED:										

R-Value Calculation Method - [PASS / FAIL]

For SI: 1 foot = 304.8 mm; NR = No requirement.

- (1) R-values are minimums. U-factors and SHGC are maximums. When insulation is installed in a cavity which is less than the label or design thickness of the insulation, the installed R-value of the insulation shall not be less than the R-value specified in the table.
- (2) The fenestration U-factor column excludes skylights. The SHGC column applies to all glazed fenestration. Exception: Skylights may be excluded from glazed fenestration SHGC requirements in Climate Zones 1 through 3 where the SHGC for such skylights does not exceed 0.30.
- (3) For impact rated fenestration complying with Section R301.2.1.2 of the *Florida Building Code, Residential* or Section 1609.1.2 of the *Florida Building Code, Building*, the maximum U-factor shall be 0.65 in Climate Zone 2. An area-weighted average of U-factor and SHGC shall be accepted to meet the requirements, and up to 15 square feet of glazed fenestration area are exempted from the U-factor and SHGC requirement based on Section R402.3.1, R402.3.2 and R402.3.3.
- (4) One side-hinged opaque door assembly up to 24 square feet is exempted from this U-factor requirement based on Section R402.3.4.
- (5) R-values are for insulation material only as applied in accordance with manufacturer's installation instructions.
- (6) The second R-value applies when more than half the insulation is on the interior of the mass wall.
- (7) R-5 shall be added to the required slab edge R-values for heated slabs. Insulation depth shall be the depth of the footing or 2 feet, whichever is less in Climate Zones 1 through 3 for heated slabs.

Air Infiltration:

Blower door test is required on the building envelope to verify leakage ≤ 7 ACH50; test report must be provided to code official before CO is issued. *Florida Building Code, Energy Conservation* Section R402.4.1.2 testing exception may apply for additions, alterations, or renovations.

(continued)



APPENDIX RD — FORMS

FORM R402—continued
EQUIPMENT REQUIREMENTS AND INSTALLED VALUES

Fill in the "INSTALLED EFFICIENCY LEVEL" column with the information requested. For multiple systems of the same type, indicate the minimum efficient system. All "INSTALLED" values must be equal to or more efficient than the required level. If a listed "SYSTEM TYPE" is not to be installed, write in "N/A" for not applicable.

SYSTEM TYPE	MINIMUM EFFICIENCY LEVEL REQUIRED	INSTALLED EFFICIENCY LEVEL
Air distribution system ¹	Not allowed in attic	Location: <u>Attic</u>
Air handling unit	Factory Sealed	Factory Sealed? <u>Y/N</u>
Duct R-value	- R-8 (Ducts in unconditioned attics, Diameter > 3 in.) - R-6 (Ducts in unconditioned non attics, Diam. > 3 in.) - R-6 (Ducts in unconditioned attics, Diameter < 3 in.) - R-4.2 (Ducts in unconditioned non attics, Diam. < 3 in.) All ducts are in conditioned space (No minimum)	R-Value (In uncond. attic) = <u>R-8</u> R-Value (In uncond. non attics) = R-Value (Small ducts in attic) = R-Value (Small ducts in uncond.) = All in conditioned space? <u>Y/N</u>
Air leakage Duct test	Air handler installed: Total leakage = 4 cfm/100 s.f. Air handler not installed: Total leakage = 3 cfm/100 s.f.	Total leakage = <u>< 4</u> cfm/100 s.f. Air handler installed? <u>Y/N</u>
Duct testing	Test not required if all ducts and AHU are within the building thermal envelope and for additions or alterations where ducts extended from existing heating and cooling system through unconditioned space are < 40 linear ft	Test report required? <u>Y/N</u>
Air conditioning systems:	Minimum federal standard required by NAECA ² :	
Central system ≤ 65,000 Btu/h	SEER 14.0	SEER (Min) = <u>14.0</u>
PIAC	EER (from Table C403.2.3(3))	EER (Min) =
Other	See Tables C403.2.3(1)–(11)	Type = Effic. (min) =
Heating systems:	Minimum federal standard required by NAECA ² :	
Heat pump ≤ 65,000 Btu/h	HSPF ≥ 8.2	HSPF (Min) = <u>8.2</u>
Gas furnace, non-weatherized	AFUE ≥ 80%	AFUE (Min) =
Oil furnace, non-weatherized	AFUE ≥ 83%	AFUE (Min) =
Other:		Type = Effic. (min) =
Water heating system (storage type):	Minimum federal standard required by NAECA ² :	Capacity = <u>50 gal.</u>
Electric ^{3, 5}	UEF 40 gal. 0.923; 50 gal.: 0.921; 60 gal.: 2.051	UEF (Min) = <u>0.921</u>
Gas fired ^{4, 5}	UEF 40 gal. 0.590; 50 gal.: 0.563; 60 gal.: 0.765	UEF (Min) =
Other (describe) ^{5, 6} :		Type = Effic. (min) =

Equipment Efficiency—[PASS / FAIL]

- (1) Ducts & AHU installed "substantially leak free" per Section R403.3.2. Test required by other individuals as defined in Section 553.993(5) or (7), *Florida Statutes*, or individuals licensed as set forth in Section 489.105(3)(f), (g), or (i), *Florida Statutes*. The total leakage test is not required for ducts and air handlers located entirely within the building thermal envelope, and for additions where ducts from an existing heating and cooling system extended to the addition through unconditioned space are less than 40 linear ft.
- (2) Minimum efficiencies are those set by the *National Appliance Energy Conservation Act of 1987* for typical residential equipment and are subject to NAECA rules and regulations. For other types of equipment, see Tables C403.2.3 (1-11) of the *Commercial Provisions of the Florida Building Code, Energy Conservation*.
- (3) For electric storage volumes ≤ 55 gallons, minimum UEF = 0.9349 – (0.0001 * volume). For electric storage volumes > 55 gallons, minimum UEF = 2.2418 – (0.0011 * volume).
- (4) For natural gas storage volumes ≤ 55 gallons, minimum UEF = 0.692 – (0.0013 * volume). For natural gas storage volumes > 55 gallons, minimum UEF = 0.8072 – (0.0003 * volume).
- (5) For electric tankless, min. UEF = 0.92. For natural gas tankless, min. UEF = 0.81.
- (6) Referenced UEFs shown are for medium draw pattern value provided by manufacturer.

(continued)

FORM R402—continued

MANDATORY REQUIREMENTS

Component	Section	Summary of Requirements	Check
Air leakage	R402.4	To be caulked, gasketed, weathersripped or otherwise sealed per Table R402.4.1.1. Recessed lighting IC-rated as having ≤ 2.0 cfm tested to ASTM E283. Windows and doors: 0.3 cfm/sq.ft (swinging doors: 0.5 cfm/sf) when tested to NFRC 400 or AAMA/WDMA/CSA 101/I.S. 2/A440. Fireplaces: Tight-fitting flue dampers & outdoor combustion air.	✓
Programmable thermostat	R403.1.2	A programmable thermostat is required for the primary heating or cooling system.	✓
Air distribution system	R403.3.2 R403.3.4	Ducts shall be tested as per Section R403.3.2 by either individuals as defined in Section 553.993(5) or (7), <i>Florida Statutes</i> , or individuals licensed as set forth in Section 489.105(3) (f), (g) or (i), <i>Florida Statutes</i> . Air handling units are not allowed in attics.	✓
Water heaters	R403.5	Comply with efficiencies in Table C404.2. Hot water pipes insulated to $\geq R-3$ to kitchen outlets, other cases. Circulating systems to have an automatic or accessible manual OFF switch. Heat trap required for vertical pipe risers.	✓
Cooling/heating equipment	R403.7	Sizing calculation performed & attached. Special occasion cooling or heating capacity requires separate system or variable capacity system.	✓
Swimming pools & spas	R403.10	Spas and heated pools must have vapor-retardant covers or a liquid cover or other means proven to reduce heat loss except if 70% of heat from site-recovered energy. Off-timer switch required. Gas heaters minimum thermal efficiency is 82%. Heat pump pool heaters minimum COP is 4.0.	✓
Lighting equipment	R404.1	Not less than 90% of the lamps in permanently installed luminaires shall have an efficacy of at least 45 lumens-per-watt or shall utilize lamps with an efficacy of not less than 65 lumens-per-watt.	✓
I hereby certify that the plans and specifications covered by this form are in compliance with the <i>Florida Building Code, Energy Conservation</i> . PREPARED BY: <u>N. P. Geisler</u> Date: <u>07-24-22</u> I hereby certify that this building is in compliance with the <i>Florida Building Code, Energy Conservation</i> . OWNER/AGENT: _____ Date: _____		Review of plans and specifications covered by this form indicate compliance with the <i>Florida Building Code, Energy Conservation</i> . Before construction is complete, this building will be inspected for compliance in accordance with Section 553.908, F.S. CODE OFFICIAL: _____ Date: _____	

N. P.
GEISLER

Digitally signed by: N. P.
GEISLER
DN: CN = N. P. GEISLER
C = US O = AR0007005
OU = ARCHITECT
Date: 2022.06.08 17:25:
15 -05'00'

JONES RESIDENCE HVAC LOAD ANALYSIS

for

CASTAGNA CONSTRUCTION

LAKE CITY, FL

120 PINE HURST DR
LOT 32

Prepared By:

DAVID HALL
DAVID HALL'S INC.
PO BOX 244
LAKE CITY, FL 32056
386-755-9792

06/22/22



Project Summary

Project:	JONES RESIDENCE	Company:	DAVID HALL'S INC.
Client:	CASTAGNA CONSTRUCTION	Representative:	DAVID HALL
Address:		Address:	PO BOX 244
City:	LAKE CITY, FL	City:	LAKE CITY, FL 32056
Phone:		Phone:	386-755-9792
Fax:		Fax:	386-755-9100
		Comment:	

Design Data

Project Name:	JONES RESIDENCE
Reference City:	Jacksonville, Florida
Daily Temperature Range:	Medium
Latitude:	30 Degrees
Elevation:	26 Feet
Elevation Sensible Adj. Factor:	1.000
Elevation Total Adj. Factor:	1.000

	Outdoor Dry Bulb	Outdoor Wet Bulb	Indoor Rel.Hum.	Indoor Dry Bulb	Grains Difference
Winter:	27	N/A	N/A	72	N/A
Summer:	96	78	50%	75	51

Check Figures

Total Building Supply CFM:	1000	CFM per square foot:	0.48
Square feet of room area:	2,084	Square feet per ton:	661.655

Building Loads

Total heating required with outside air:	38,821 Btuh	38.821 MBH
Total sensible gain:	29,103 Btuh	78 %
Total latent gain:	8,319 Btuh	22 %
Total cooling required with outside air:	37,422 Btuh	3.118 Tons (based on sensible + latent)
		3.150 Tons (based on 77% sensible capacity)

Notes

Calculations are based on 7th edition of ACCA Manual J.
All computed results are estimates as building use and weather may vary.
Be sure to select a unit that meets both sensible and latent loads.

Miscellaneous Project Data

Project File Name: CASTAGNA, JONES

System Input Data

--System 1--	Outdoor Dry Bulb	Outdoor Wet Bulb	Indoor Rel.Hum.	Indoor Dry Bulb	Grains Difference
Winter:	27	N/A	N/A	72	N/A
Summer:	96	78	50%	75	51

External Overhangs

No.	Projection	Offset	No.	Projection	Offset
1	3	1	6	0	0
2	5	0	7	0	0
3	4	0.5	8	0	0
4	0	0	9	0	0
5	0	0	10	0	0

Duct Sizing Inputs

	Runouts	Main Trunk
Duct Material:	Flexible Duct	Galvanized Steel
Roughness Factor:	0.010000	0.000300
Pressure Drop:	0.1000 In.wg/100 Ft.	0.1000 In.wg/100 Ft.
Minimum Velocity:	450.0 Ft./Minute	650.0 Ft./Minute
Maximum Velocity:	750.0 Ft./Minute	900.0 Ft./Minute
Minimum Height:	0 Inches	0 Inches
Maximum Height:	0 Inches	0 Inches

Outside Air Data

	Winter	Summer
Infiltration:	0.900 AC/Hr	0.400 AC/Hr
Volume of Conditioned Space:	X 20387 Cu.Ft.	X 20387 Cu.Ft.
	18,348 Cu.Ft./Hr	8,155 Cu.Ft./Hr
	X 0.0167	X 0.0167
Total Building Infiltration:	305.805 CFM	135.9133 CFM
Total Building Ventilation:	0 CFM	0 CFM
--System 1--		
Infiltration & Ventilation Sensible Gain Multiplier:	23.10 = (1.10 X 21.00 Summer Temp. Difference)	
Infiltration & Ventilation Latent Gain Multiplier:	34.86 = (0.68 X 51.27 Grains Difference)	
Infiltration & Ventilation Sensible Loss Multiplier:	49.50 = (1.10 X 45.00 Winter Temp. Difference)	

Total Building Summary Loads

Component Description	Area Quan	Sen. Loss	Lat. Gain	Sen. Gain	Total Gain
3C Window Double Pane Clear Glass Metal Frame	155	5,056	0	6,569	6,569
10D Door Wood Solid Core	42	870	0	476	476
11C Door Metal Polystyrene Core	42	888	0	486	486
12G Wall R-13 + 3/4" ExtPoly Board(R-3.8)	1,597	4,672	0	2,553	2,553
16G Ceiling R-30 Insulation	2,084	3,095	0	3,095	3,095
22A Slab on Grade No Edge Insulation	199	7,253	0	0	0
Subtotals for structure:	4,119	21,834	0	13,179	13,179
Active People:	6	0	1,380	1,800	3,180
Inactive People:	0	0	0	0	0
Appliances:	0	0	2,200	2,200	4,400
Lighting:	0	0		6,138	
Ductwork:	0	1,850	0	2,647	2,647
Infiltration: Winter CFM: 305.8, Summer CFM: 135.9	239	15,137	4,739	3,139	7,878
Ventilation: Winter CFM: 0.0, Summer CFM: 0.0	0	0	0	0	0
Sensible Gain Total:				29,103	
Temperature Swing Multiplier:				X1.00	
Building Load Totals:		38,821	8,319	29,103	37,422

Check Figures

Total Building Supply CFM:	1000	CFM per square foot:	0.48
Square feet of room area:	2,084	Square feet per ton:	661.655

Building Loads

Total heating required with outside air:	38,821 Btuh	38.821 MBH
Total sensible gain:	29,103 Btuh	78 %
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Total cooling required with outside air:	37,422 Btuh	3.118 Tons (based on sensible + latent)
		3.150 Tons (based on 77% sensible capacity)

Notes

Calculations are based on 7th edition of ACCA Manual J.
 All computed results are estimates as building use and weather may vary.
 Be sure to select a unit that meets both sensible and latent loads.

Room Load Summary Reports

System #1 Room Load Summary

No	Room Name	Area SF	Htg Sens Btuh	Htg Nom CFM	Run Duct Size	Run Duct Vel	Clg Sens Btuh	Clg Lat Btuh	Clg Nom CFM	Zone Adj Fact	Clg Adj CFM	Air Sys CFM
---Zone 1---												
1	Master Closet	112	1,618	42	1-3	631	902	0	31	1.00	41	31
2	Laundry Room	68	687	18	1-5	517	2,052	1,230	71	1.00	93	71
3	Garage Entry	48	2,269	58	1-4	426	1,082	416	37	1.00	49	37
4	Master Bath	109	1,869	48	1-4	579	1,471	40	51	1.00	67	51
5	Master Bedroom	320	5,159	133	1-6	544	3,109	1,003	107	1.00	141	107
6	Great Room	660	7,919	204	2-5	641	5,090	1,360	175	1.20	277	175
7	Kitchen	276	6,374	164	2-5	603	4,783	2,382	164	1.00	217	164
8	Pantry	76	1,315	34	1-3	553	790	0	27	1.00	36	27
9	Bedroom #2	160	7,178	185	1-8	530	5,380	1,301	185	1.35	330	185
10	Hall Bath	62	625	16	1-3	422	603	0	21	1.00	27	21
11	Hall	28	82	2	1-2	663	421	0	14	1.00	19	14
12	Bedroom#3	165	3,726	96	1-6	599	3,420	587	118	1.25	194	118
System 1 Totals		2084	38,821	1,000			29,103	8,319	1,000		1,493	1,000
Main Trunk Size: 16x12 in.												

System #1 Cooling System Summary

	Cooling Tons	Sensible/Latent Split	Sensible Btuh	Latent Btuh	Total Btuh
Net Required:	3.118	78%/22%	29,103	8,319	37,422
Recommended:	3.150	77%/23%	29,103	8,693	37,796

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Alpine, an ITW Company
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Glenview, IL 60025
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www.alpineitw.com

COA #0 278

Florida Certificate of Product Approval #FL 1999
06/13/2022



Site Information:	Page 1:
Customer: W. B. Howland Company, Inc.	Job Number: 21-6493
Job Description: Jones	
Address:	

Job Engineering Criteria:			
Design Code: FBC 7th Ed. 2020 Res.		IntelliVIEW Version: 21.02.01	
		JRef #: 1XGd2150007	
Wind Standard: ASCE 7-16	Wind Speed (mph): 130	Design Loading (psf): 40.00	
Building Type: Closed			

This package contains general notes pages, 50 truss drawing(s) and 5 detail(s).

Item	Drawing Number	Truss
1	164.22.1546.07373	A01
3	164.22.1545.50343	A03
5	164.22.1545.42240	A05
7	164.22.1545.33247	A07
9	164.22.1545.28897	A09
11	164.22.1545.23257	A11
13	164.22.1545.16240	A13
15	164.22.1545.11290	A15
17	164.22.1545.07727	A17
19	164.22.1545.00543	A19
21	164.22.1544.39763	B02
23	164.22.1543.36933	C02
25	164.22.1543.11750	C04
27	164.22.1543.00790	C06
29	164.22.1542.55300	D01
31	164.22.1542.49750	D03
33	164.22.1542.43747	D05
35	164.22.1542.29420	J01HJ
37	164.22.1541.20720	J03
39	164.22.1541.16910	V01
41	164.22.1541.13887	V03
43	164.22.1541.06850	V05
45	164.22.1540.59937	V07
47	164.22.1540.53610	V09
49	164.22.1540.42600	V11

Item	Drawing Number	Truss
2	164.22.1545.54773	A02
4	164.22.1545.44850	A04
6	164.22.1545.37857	A06
8	164.22.1545.30393	A08
10	164.22.1545.26843	A10
12	164.22.1545.19487	A12
14	164.22.1545.14813	A14
16	164.22.1545.09530	A16
18	164.22.1545.03697	A18
20	164.22.1544.45430	B01
22	164.22.1544.37923	C01
24	164.22.1543.26890	C03
26	164.22.1543.05770	C05
28	164.22.1542.58330	C07
30	164.22.1542.53793	D02
32	164.22.1542.48327	D04
34	164.22.1542.32103	J01
36	164.22.1542.21880	J02
38	164.22.1541.19047	J04
40	164.22.1541.14900	V02
42	164.22.1541.12687	V04
44	164.22.1541.05540	V06
46	164.22.1540.57997	V08
48	164.22.1540.44690	V10
50	164.22.1540.40170	V12

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COA #0 278

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06/13/2022



Alpine, an ITW Company
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025
Phone: (800)755-6001
www.alpineitw.com

Site Information:	Page 2:
Customer: W. B. Howland Company, Inc.	Job Number: 21-6493
Job Description: Jones	
Address:	

Item	Drawing Number	Truss
51	VAL180160118	
53	A14015ENC160118	
55	BRCLBSUB0119	

Item	Drawing Number	Truss
52	VALTN160118	
54	GBLLETIN0118	

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed and detailed by the Building Designer.

Connector Plate Information:

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for of all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for of all load cases.

Max Web CSI = Maximum bending and axial Combined Stress Index for Webs for of all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.

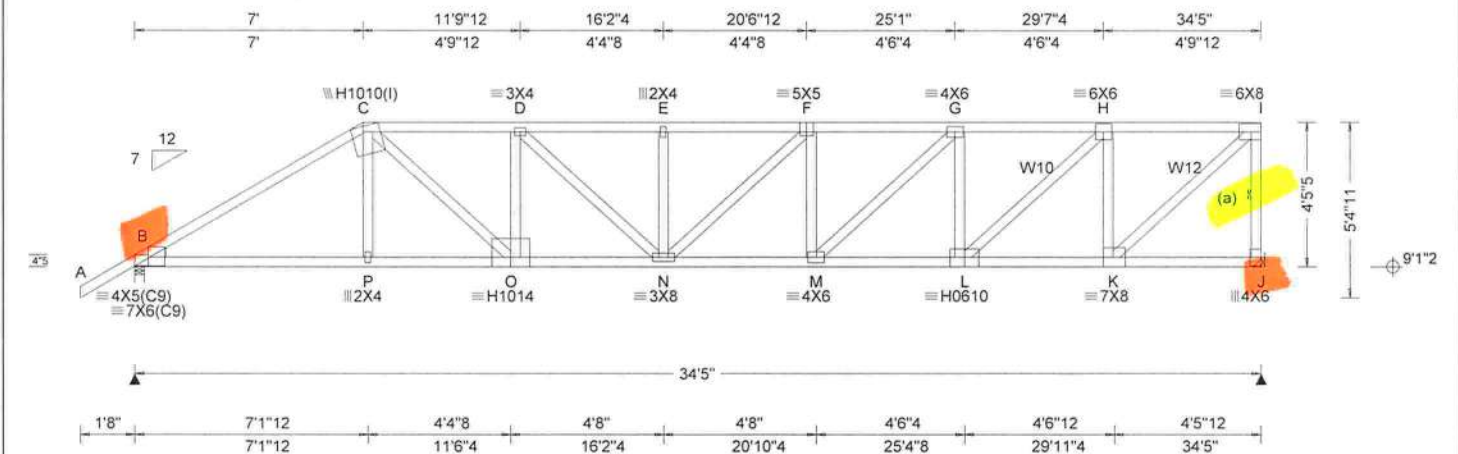
W = Width of non-hanger bearing, in inches.

Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcacomponents.com.



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity Non-Gravity
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.365 E 999 240	Loc R+ /R- /Rh /Rw /U /RL
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.734 E 560 180	B 3454 -/- -/- -/- 1784 -/-
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.101 C - -	J 3454 -/- -/- -/- 1835 -/-
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.202 C - -	
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0	Wind reactions based on MWFRS
Soffit: 0.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.743	B Brg Wid = 3.5 Min Req = 2.9 (Truss)
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.797	J Brg Wid = - Min Req = -
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Varies by Ld Case	Max Web CSI: 0.992	Bearing B is a rigid surface.
	C&C Dist a: 3.44 ft	FT/RT:20(0)/10(0)		Members not listed have forces less than 375#
	Loc. from endwall: not in 4.50 ft	Plate Type(s):		Maximum Top Chord Forces Per Ply (lbs)
	GCpi: 0.18	WAVE, HS	VIEW Ver: 21.02.01.1216.15	Chords Tens.Comp. Chords Tens. Comp.
	Wind Duration: 1.60			B - C 1370 -5975 F - G 1689 -7072

Lumber	Bracing	Maximum Bot Chord Forces Per Ply (lbs)
Top chord: 2x4 SP M-31;	(a) Continuous lateral restraint equally spaced on member.	Chords Tens.Comp. Chords Tens. Comp.
Bot chord: 2x4 SP M-31;		B - P 5049 -1143 N - M 7132 -1712
Webs: 2x4 SP #3; W10 2x4 SP #2; W12 2x4 SP M-31;		P - O 5070 -1140 M - L 5841 -1412
Lt Wedge: 2x4 SP #3;		O - N 6826 -1610 L - K 3603 -878

Special Loads	Maximum Web Forces Per Ply (lbs)
----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)	Webs Tens.Comp. Webs Tens. Comp.
TC: From 63 plf at -1.67 to 63 plf at 7.00	C - P 611 0 M - G 1725 -389
TC: From 32 plf at 7.00 to 32 plf at 34.42	C - O 2269 -594 G - L 547 -1758
BC: From 20 plf at 0.00 to 20 plf at 7.03	O - D 449 -1175 L - H 2963 -695
BC: From 10 plf at 7.03 to 10 plf at 34.42	D - N 849 -215 H - K 824 -2754
TC: 435 lb Conc. Load at 7.03	E - N 267 -534 K - I 4629 -1113
TC: 189 lb Conc. Load at 9.06,11.06,13.06,15.06	N - F 426 -74 I - J 855 -3341
17.06,19.06,21.06,23.06,25.06,27.06,29.06,31.06	
33.06	
BC: 506 lb Conc. Load at 7.03	
BC: 130 lb Conc. Load at 9.06,11.06,13.06,15.06	
17.06,19.06,21.06,23.06,25.06,27.06,29.06,31.06	
33.06	

Purlins	Wind
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.	Wind loads and reactions based on MWFRS.
	Right end vertical not exposed to wind pressure.
	Wind loading based on both gable and hip roof types.



Florida Certificate of Product Approval #FL 1999

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!

****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

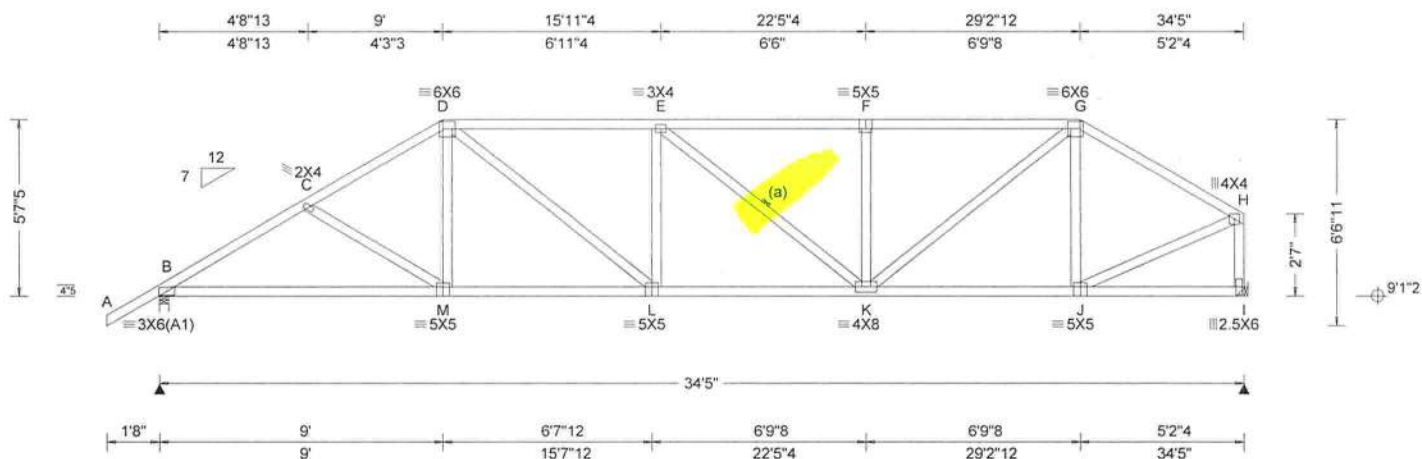
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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCE: sbcecomponents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 99944 FROM:	HIPS Qty: 1	Ply: 1 Job Number: 21-6493 Jones Truss Label: A02	Cust: R 215 JRef: 1XGd2150007 T25 DrwNo: 164.22.1545.54773 KD / WHK 06/13/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.44 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.128 E 999 240 VERT(CL): 0.263 E 999 180 HORZ(LL): 0.042 I - - HORZ(TL): 0.087 I - - Creep Factor: 2.0 Max TC CSI: 0.633 Max BC CSI: 0.785 Max Web CSI: 0.770 VIEW Ver: 21.02.01.1216.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1545 -/- /- /918 /278 /155 I 1422 -/- /- /758 /254 /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.8 (Truss) I Brg Wid = - Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 915 -2370 E - F 1054 -2136 C - D 897 -2137 F - G 1054 -2136 D - E 1119 -2311 G - H 650 -1489

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Hangers / Ties

Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

Bearing at location x=34'2" uses the following support conditions: 34'2"

Bearing I (34'2", 9'1"2) HUS26

Supporting Member: (2)2x6 SP 2400f-2.0E
(14) 0.148"x3" nails into supporting member,
(4) 0.148"x3" nails into supported member.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - M	1977 -796	L - K	2326 -1026
M - L	1797 -721	K - J	1228 -505

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
D - M	376 0	G - J	283 -401
D - L	657 -412	J - H	1326 -547
F - K	356 -415	H - I	583 -1381
K - G	1152 -568		



COA #00278

06/13/2022
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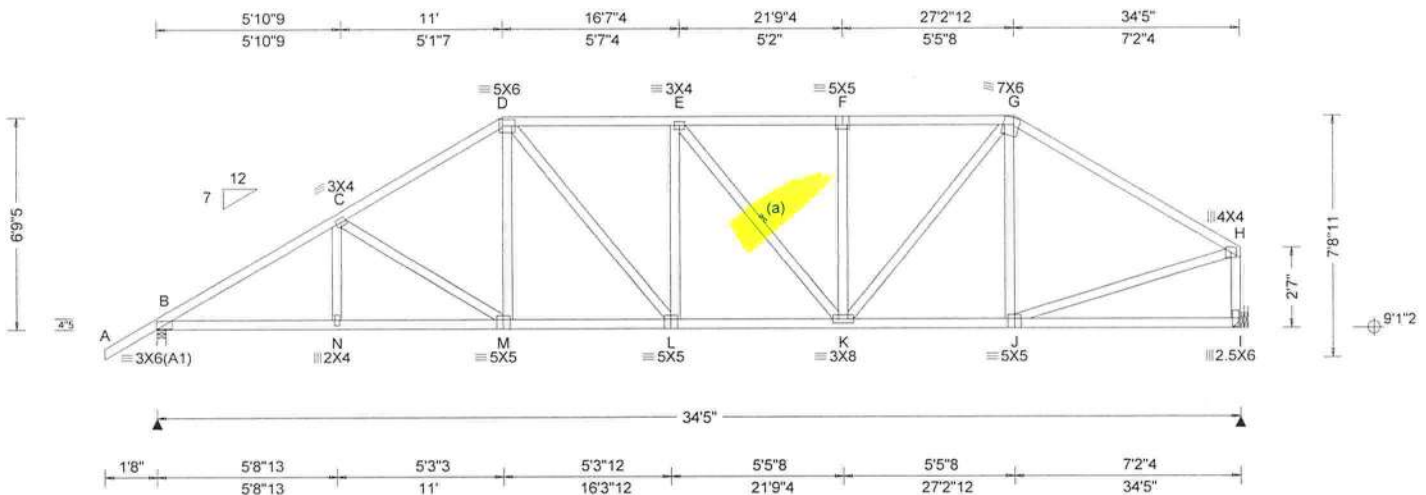
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100134 FROM:	HIPS Qty: 1	Ply: 1 Qty: 1	Job Number: 21-6493 Jones Truss Label: A03	Cust: R 215 JRef: 1XGd2150007 T12 DrwNo: 164.22.1545.50343 KD / WHK 06/13/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.44 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.101 E 999 240 VERT(CL): 0.208 E 999 180 HORZ(LL): 0.038 I - - HORZ(TL): 0.078 I - - Creep Factor: 2.0 Max TC CSI: 0.949 Max BC CSI: 0.599 Max Web CSI: 0.521 VIEW Ver: 21.02.01.1216.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1545 -/- /- /932 /275 /186 I 1422 -/- /- /774 /250 /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.8 (Truss) I Brg Wid = - Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 801 -2378 E - F 853 -1769 C - D 813 -1994 F - G 853 -1770 D - E 891 -1893 G - H 646 -1607

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Hangers / Ties

Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

Bearing at location x=34'2" uses the following support conditions: 34'2"

Bearing I (34'2", 9'1"2) HUS26

Supporting Member: (2)2x6 SP 2400f-2.0E
(14) 0.148"x3" nails into supporting member,
(4) 0.148"x3" nails into supported member.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - N	1974 -693	L - K	1901 -749
N - M	1972 -694	K - J	1299 -473
M - L	1650 -595		

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
C - M	137 -386	K - G	729 -372
D - M	380 -25	J - H	1322 -482
D - L	381 -266	H - I	535 -1358



COA #0278

06/13/2022
Florida Certificate of Product Approval #FL 1999

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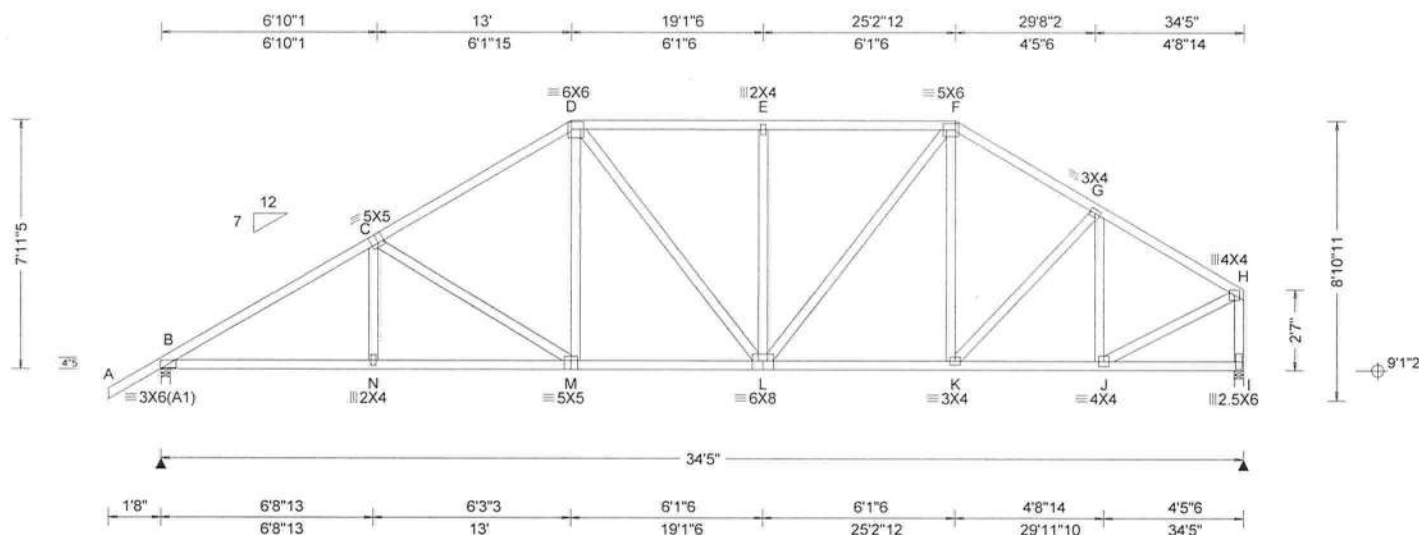
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100131 FROM:	HIPS Qty: 1	Ply: 1	Job Number: 21-6493 Jones Truss Label: A04	Cust: R 215 JRef: 1XGd2150007 T38 DrwNo: 164.22.1545.44850 KD / WHK 06/13/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.44 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.087 M 999 240 VERT(CL): 0.180 M 999 180 HORZ(LL): 0.038 I - - HORZ(TL): 0.077 I - - Creep Factor: 2.0 Max TC CSI: 0.476 Max BC CSI: 0.557 Max Web CSI: 0.543 VIEW Ver: 21.02.01.1216.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1545 -/- /- /942 /272 /217 I 1422 -/- /- /785 /247 -/ Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.8 (Truss) I Brg Wid = 3.5 Min Req = 1.7 (Truss) Bearings B & I are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 716 -2354 E - F 743 -1607 C - D 714 -1863 F - G 640 -1548 D - E 743 -1607 G - H 480 -1400

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Purlins

In lieu of structural panels use purlins to brace all flat
TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C
member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords Tens.Comp. Chords Tens. Comp.

B - N	1944	-609	L - K	1274	-397
N - M	1942	-610	K - J	1177	-370
M - L	1521	-472			

Maximum Web Forces Per Ply (lbs)

Webs Tens.Comp. Webs Tens. Comp.

C - M	171	-502	G - J	232	-491
D - M	457	-32	J - H	1283	-399
E - L	351	-401	H - I	463	-1382
L - F	532	-257			



06/13/2022
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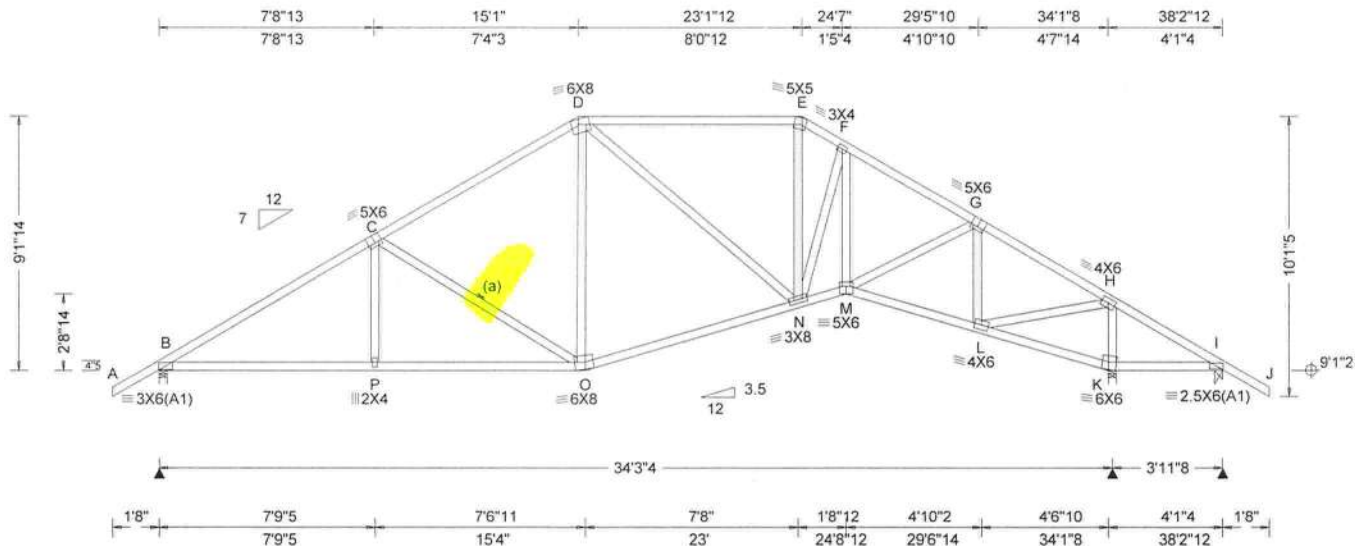
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100128 FROM:	COMN Qty: 1	Ply: 1 Jones	Job Number: 21-6493 Truss Label: A05	Cust: R 215 JRef: 1XGd2150007 T53 DrwNo: 164.22.1545 42240 KD / WHK 06/13/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.82 ft Loc. from endwall: not in 9.00 ft GCpt: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.085 O 999 240 VERT(CL): 0.175 O 999 180 HORZ(LL): 0.053 K - - HORZ(TL): 0.109 K - - Creep Factor: 2.0 Max TC CSI: 0.765 Max BC CSI: 0.679 Max Web CSI: 0.778 VIEW Ver: 21.02.01.1216.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1459 - / - / 909 / 41 / 288 K 2318 - / - / 1264 / 12 / - I - / -536 - / 78 / 296 / - Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.7 (Truss) K Brg Wid = 3.5 Min Req = 2.7 (Truss) I Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings B, K, & I are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

Negative reaction(s) of -536# MAX. from a non-wind load case requires uplift connection. See Maximum Reactions.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - P	1761 - 333	M - L	1010 - 120
P - O	1758 - 334	L - K	354 - 1138
O - N	1313 - 190	K - I	318 - 1021
N - M	1543 - 198		

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
C - O	198 - 614	M - G	614 - 99
N - E	455 - 0	G - L	272 - 945
N - F	39 - 435	L - H	2042 - 416
F - M	433 - 37	K - H	489 - 1883



COA #0208

Florida Certificate of Product Approval #FL 1999

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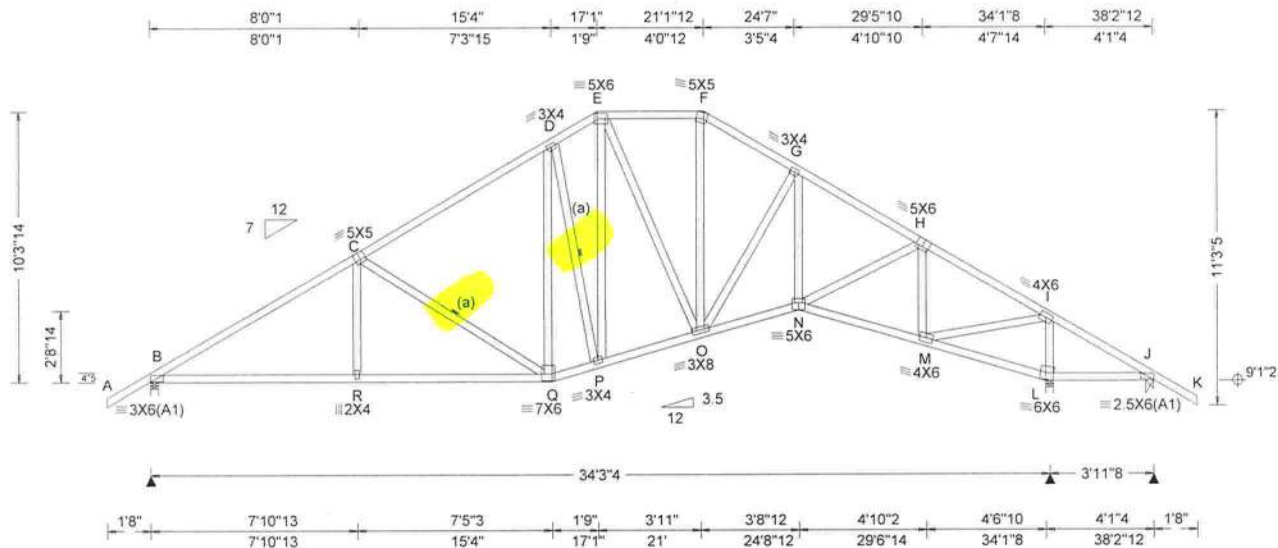
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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100119 FROM:	COMN Qty: 1	Ply: 1 Job Number: 21-6493 Jones Truss Label: A06	Cust: R 215 JRef: 1XGd2150007 T21 DrwNo: 164.22.1545.37857 KD / WHK 06/13/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.82 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.090 Q 999 240 VERT(CL): 0.184 Q 999 180 HORZ(LL): 0.052 L - - HORZ(TL): 0.106 L - - Creep Factor: 2.0 Max TC CSI: 0.660 Max BC CSI: 0.709 Max Web CSI: 0.776 VIEW Ver: 21.02.01.1216.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1455 /- /- /910 /32 /319 L 2319 /- /- /1271 /3 /- J - /-538 /- /76 /303 /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.7 (Truss) L Brg Wid = 3.5 Min Req = 2.7 (Truss) J Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings B, L, & J are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

Negative reaction(s) of -538# MAX. from a non-wind load case requires uplift connection. See Maximum Reactions.

B - C	486 -2146	F - G	469 -1416
C - D	467 -1531	G - H	443 -1818
D - E	528 -1380	H - I	279 -1131
E - F	434 -1177	I - J	1256 -205

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - R	1755 -269	O - N	1567 -104
R - Q	1752 -271	N - M	1006 -78
Q - P	1267 -106	M - L	299 -1139
P - O	1188 -57	L - J	267 -1023

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
C - Q	217 -637	G - N	498 0
D - P	222 -417	N - H	626 -44
E - P	468 -217	H - M	222 -947
O - F	443 -85	M - I	2038 -323
O - G	123 -650	L - I	417 -1882



COA #00278

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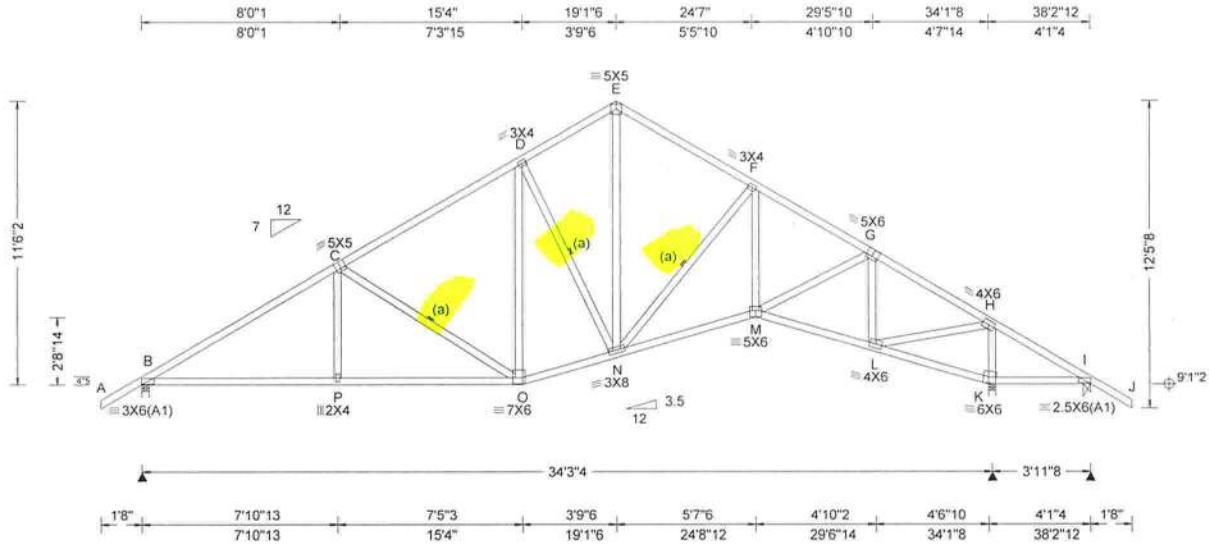
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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100116 FROM:	COMN	Ply: 1 Qty: 3	Job Number: 21-6493 Jones Truss Label: A07	Cust: R 215 JRef: 1XGd2150007 T51 DrwNo: 164.22 1545.33247 KD / WHK 06/13/2022
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Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)		Defl/CSI Criteria		▲ Maximum Reactions (lbs)	
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA	Ct: NA	CAT: NA	PP Deflection in loc L/defl L/#	VERT(LL): 0.093 O 999 240	Gravity	Loc R+ / R-	Non-Gravity / Rh / Rw / U / RL
TCCL: 10.00	Speed: 130 mph	Pf: NA	Ce: NA		VERT(CL): 0.190 O 999 180	HORZ(LL): 0.055 K - -	B 1449 /- /- /903 /24 /351		
BCLL: 0.00	Enclosure: Closed	Lu: NA	Cs: NA		HORZ(TL): 0.112 K - -	Creep Factor: 2.0	K 2380 /- /- /1302 /0 /-		
BCDL: 10.00	Risk Category: II	Snow Duration: NA			Max TC CSI: 0.655		I - /-593 /- /75 /344 /-		
Des Ld: 40.00	EXP: C Kzt: NA	Building Code:			Max BC CSI: 0.710		Wind reactions based on MWFRS		
NCBCLL: 10.00	Mean Height: 15.00 ft	FBC 7th Ed. 2020 Res.			Max Web CSI: 0.788		B Brg Wid = 3.5 Min Req = 1.7 (Truss)		
Soffit: 0.00	TCCL: 5.0 psf	TPI Std: 2014					K Brg Wid = 3.5 Min Req = 2.8 (Truss)		
Load Duration: 1.25	BCDL: 5.0 psf	Rep Fac: Yes					I Brg Wid = 3.5 Min Req = 1.5 (Truss)		
Spacing: 24.0 "	MWFRS Parallel Dist: h to 2h	FT/RT:20(0)/10(0)					Bearings B, K, & I are a rigid surface.		
	C&C Dist a: 3.82 ft	Plate Type(s):					Members not listed have forces less than 375#		
	Loc. from endwall: not in 9.00 ft	WAVE					Maximum Top Chord Forces Per Ply (lbs)		
	GCp1: 0.18						Chords Tens.Comp.	Chords Tens. Comp.	
	Wind Duration: 1.60						B - C 405 -2135	F - G 339 -1793	
							C - D 386 -1517	G - H 218 -1070	
							D - E 394 -1262	H - I 1355 -174	
							E - F 375 -1286		

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

Negative reaction(s) of -593# MAX. from a non-wind load case requires uplift connection. See Maximum Reactions.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - P	1745 -200	M - L	950 -18
P - O	1742 -201	L - K	268 -1229
O - N	1265 -74	K - I	239 -1108
N - M	1559 -18		

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
C - O	220 -639	M - G	672 0
D - N	236 -396	G - L	175 -970
E - N	935 -272	L - H	2069 -237
N - F	139 -719	K - H	353 -1915
F - M	498 0		



COA #0078

06/13/2022
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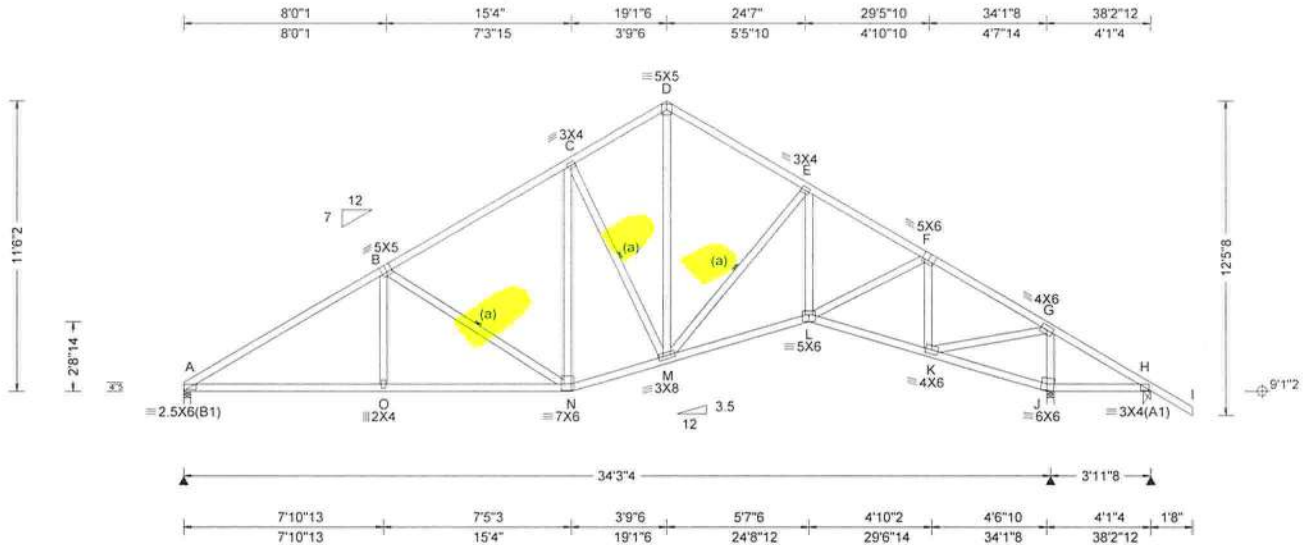
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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100113 FROM:	COMN Qty: 5	Job Number: 21-6493 Jones Truss Label: A08	Cust: R 215 JRef: 1XGd2150007 T37 DrwNo: 164.22.1545.30393 KD / WHK 06/13/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCCL: 20.00 TCDL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.82 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.091 N 999 240 VERT(CL): 0.188 N 999 180 HORZ(LL): 0.054 J - - HORZ(TL): 0.112 J - - Creep Factor: 2.0 Max TC CSI: 0.724 Max BC CSI: 0.724 Max Web CSI: 0.789 VIEW Ver: 21.02.01.1216.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 1341 - / - / - / 806 / 16 / 332 J 2380 - / - / - / 1302 / 0 / - H - / -590 / - / 75 / 341 / - Wind reactions based on MWFRS A Brg Wid = 3.5 Min Req = 1.6 (Truss) J Brg Wid = 3.5 Min Req = 2.8 (Truss) H Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings A, J, & H are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

Negative reaction(s) of -590# MAX. from a non-wind load case requires uplift connection. See Maximum Reactions.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
A - O	1768 -222	L - K	957 -25
O - N	1766 -223	K - J	265 -1225
N - M	1272 -75	J - H	237 -1103
M - L	1566 -25		

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
B - N	230 -660	L - F	673 0
C - M	237 -398	F - K	175 -971
D - M	939 -276	K - G	2071 -239
M - E	141 -722	J - G	354 -1916
E - L	499 0		



COA #0028

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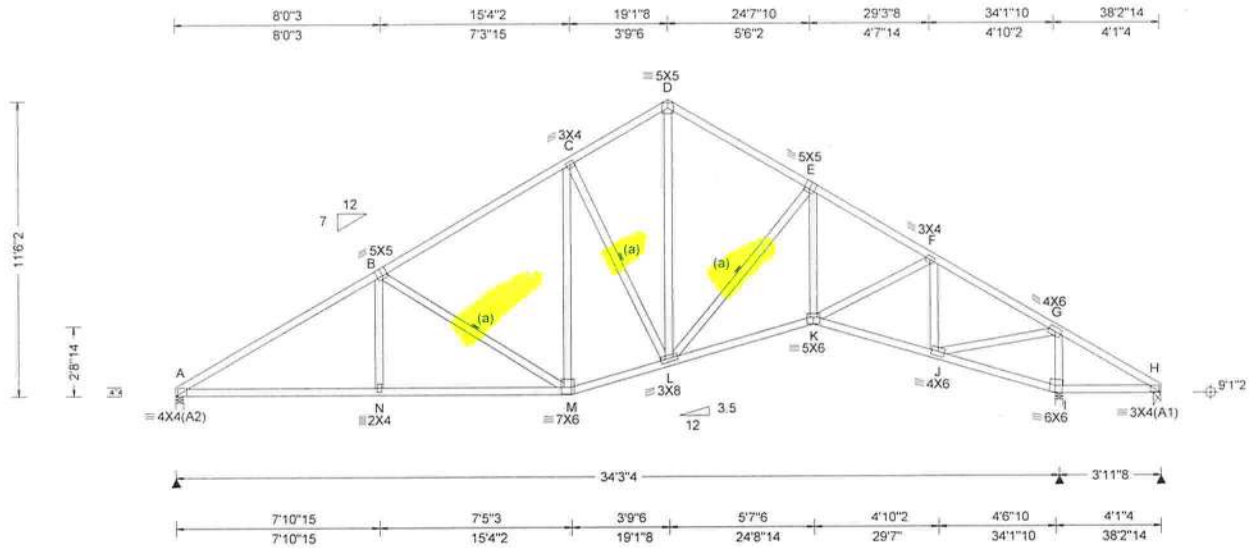
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100110	COMN	Ply: 1	Job Number: 21-6493	Cust: R 215 JRef: 1XGd2150007 T13
FROM:		Qty: 1	Jones	DrwNo: 164.22.1545.28897
			Truss Label: A09	KD / WHK 06/13/2022



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity Non-Gravity
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.094 M 999 240	Loc R+ / R- / Rh / Rw / U / RL
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.196 M 999 180	A 1336 /- /- /803 /15 /300
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.056 I - -	I 2402 /- /- /1328 /12 /-
Des Ld: 40.00	EXP: C Kzt: NA	Building Code:	HORZ(TL): 0.117 I - -	H - /-644 /- /1 /350 /-
NCBCLL: 10.00	Mean Height: 15.02 ft	FBC 7th Ed. 2020 Res.	Creep Factor: 2.0	Wind reactions based on MWFRS
Soffit: 0.00	TCDL: 5.0 psf	TPI Std: 2014	Max TC CSI: 0.854	A Brg Wid = 3.5 Min Req = 1.6 (Truss)
Load Duration: 1.25	BCDL: 5.0 psf	Rep Fac: Yes	Max BC CSI: 0.789	I Brg Wid = 3.5 Min Req = 2.8 (Truss)
Spacing: 24.0 "	MWFRS Parallel Dist: h to 2h	FT/RT: 20(0)/10(0)	Max Web CSI: 0.793	H Brg Wid = 3.5 Min Req = 1.5 (Truss)
	C&C Dist a: 3.82 ft	Plate Type(s):	VIEW Ver: 21.02.01.1216.15	Bearings A, I, & H are a rigid surface.
	Loc. from endwall: not in 9.00 ft	WAVE		Members not listed have forces less than 375#
	GCpi: 0.18			Maximum Top Chord Forces Per Ply (lbs)
	Wind Duration: 1.60			Chords Tens.Comp. Chords Tens. Comp.

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

Negative reaction(s) of -644# MAX. from a non-wind load case requires uplift connection. See Maximum Reactions.

A - B	421 -2202	E - F	362 -1816
B - C	389 -1541	F - G	228 -1094
C - D	402 -1278	G - H	1336 -194
D - E	384 -1303		

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
A - N	1813 -263	K - J	969 -65
N - M	1810 -264	J - I	223 -1232
M - L	1284 -74	I - H	195 -1110
L - K	1580 -75		

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
B - M	237 -699	K - F	673 -10
C - L	224 -401	F - J	187 -977
D - L	947 -278	J - G	2081 -260
L - E	157 -726	I - G	380 -1929
E - K	500 0		



COA #0-278

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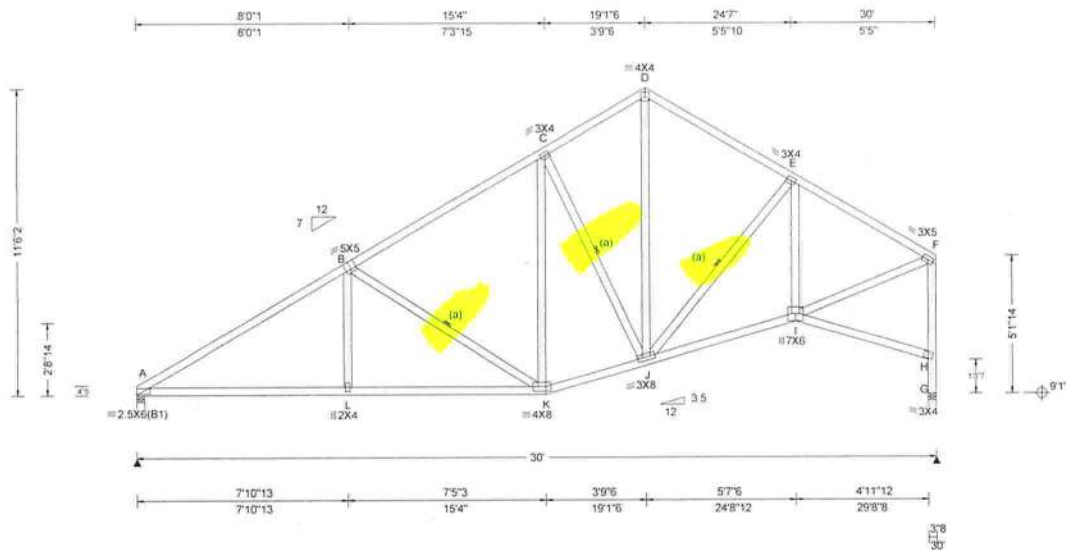
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity Non-Gravity
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.061 L 999 240	Loc R+ / R- / Rh / Rw / U / RL
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.127 L 999 180	A 1247 /- /- /750 /4 /287
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.031 H - -	G 1249 /- /- /670 /22 /-
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.064 H - -	Wind reactions based on MWFRS
NCBCLL: 10.00	Mean Height: 15.03 ft	Building Code:	Creep Factor: 2.0	A Brg Wid = 3.5 Min Req = 1.5 (Truss)
Soffit: 0.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.693	G Brg Wid = 3.5 Min Req = 1.5 (Support)
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.707	Bearings A & G are a rigid surface.
Spacing: 24.0 "	MWFRS Parallel Dist: h to 2h	Rep Fac: Yes	Max Web CSI: 0.459	Members not listed have forces less than 375#
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)		Maximum Top Chord Forces Per Ply (lbs)
	Loc. from endwall: not in 9.00 ft	Plate Type(s):		Chords Tens.Comp. Chords Tens. Comp.
	GCpi: 0.18	WAVE	VIEW Ver: 21.02.01.1216.15	A - B 347 - 1978 D - E 334 - 1090
	Wind Duration: 1.60			A - B 334 - 1249 D - E 337 - 1203

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Wind

Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

Shim all supports to solid bearing.



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▲ Maximum Reactions (lbs)

Loc	Gravity			Non-Gravity		
	R+	/R-	/Rh	/Rw	/U	/RL
A	1247	/-	/-	/750	/4	/287
G	1249	/-	/-	/670	/22	/-

Wind reactions based on MWFRS

A	Brg Wid = 3.5	Min Req = 1.5 (Truss)
G	Brg Wid = 3.5	Min Req = 1.5 (Support)

Bearings A & G are a rigid surface.

Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
A - B	347 - 1978	D - E	334 - 1090
B - C	321 - 1342	E - F	277 - 1297
C - D	351 - 1063		

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - L	1614	-375	K - J	1107	-197
L - K	1611	-376	J - I	1122	-195

Maximum Web Forces Per Ply (lbs)

Maximum Web Forces (lb)					
Webs		Tens. Comp.		Tens. Comp.	
B - K	229	-664	I - F	1184	-200
C - J	194	-444	H - G	237	-1249
D - I	746	-235	H - F	255	-1194

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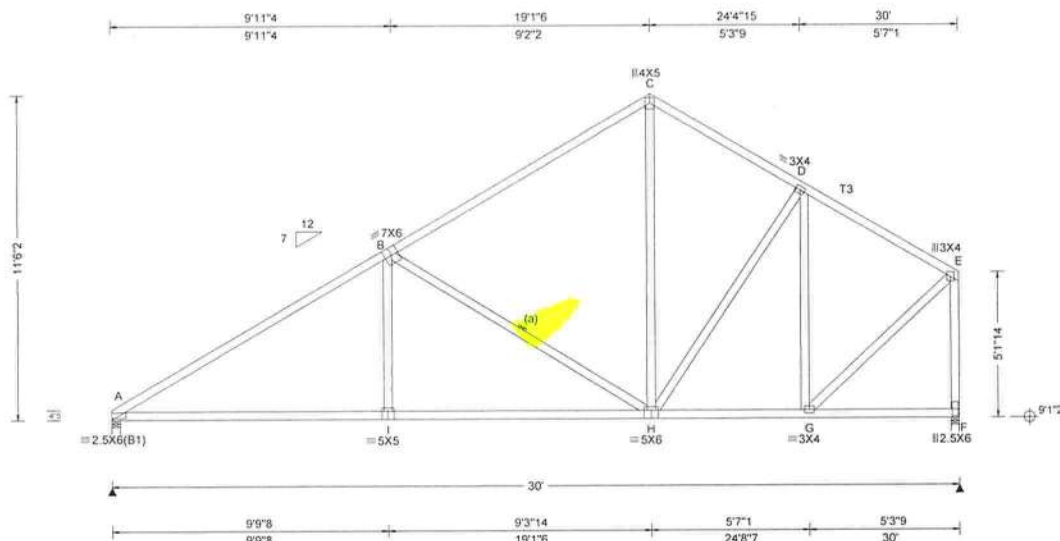
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025



Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria			▲ Maximum Reactions (lbs)								
TCLL:	20.00	Wind Std:	ASCE 7-16	Pg: NA	Ct: NA	CAT: NA	PP Deflection in	loc	L/defl	L/#	Gravity			Non-Gravity				
TCDL:	10.00	Speed:	130 mph	Pf: NA		Ce: NA	VERT(LL):	0.052	I	999	240	Loc	R+	/R-	/Rh	/Rw	/U	/RL
BCLL:	0.00	Enclosure:	Closed	Lu: NA	Cs: NA		VERT(CL):	0.108	I	999	180	A	1253	I/-	I/-	/750	/6	/287
BCDL:	10.00	Risk Category:	II	Snow Duration:	NA		HORZ(LL):	0.023	G	-	-	F	1241	I/-	I/-	/664	/24	I/-
		EXP:	C Kzt: NA				HORZ(TL):	0.049	G	-	-	Wind reactions based on MWFRS						
Des Ld:	40.00	Mean Height:	15.03 ft	Building Code:			Creep Factor:	2.0				A Brg Wid = 3.5 Min Req = 1.5 (Truss)						
NCBCLL:	10.00	TCDL:	5.0 psf	FBC 7th Ed. 2020 Res.			Max TC CSI:	0.491				F Brg Wid = 3.5 Min Req = 1.5 (Truss)						
Soffit:	0.00	BCDL:	5.0 psf	TPI Std:	2014		Max BC CSI:	0.971				Bearings A & F are a rigid surface.						
Load Duration:	1.25	MWFRS Parallel Dist:	h to 2h	Rep Fac:	Yes		Max Web CSI:	0.674				Members not listed have forces less than 375#						
Spacing:	24.0 "	C&C Dist a:	3.00 ft	FT/RT:	20(0)/10(0)							Maximum Top Chord Forces Per Ply (lbs)						
		Loc. from endwall:	not in 9.00 ft	Plate Type(s):								Chords	Tens.Comp.	Chords	Tens. Comp.			
		GCpi:	0.18	WAVE								A - B	343	- 1909	C - D	325	- 1000	
		Wind Duration:	1.60									B - C	302	- 1087	D - E	211	- 886	
												VIEW Ver: 21.02.01.1216.15						

Lumber

Top chord: 2x4 SP M-31; T3 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Wind

Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)					
Chords			Tens. Comp.		
A - I	1537	-350	H - G	717	-123
I - H	1534	-351			

Maximum Web Forces Per Ply (lbs)					
Webs	Tens.Comp.		Webs	Tens. Comp.	
I - B	416	0	D - G	161	- 533
B - H	299	- 869	G - E	952	- 161
C - H	549	- 118	E - F	260	- 1197



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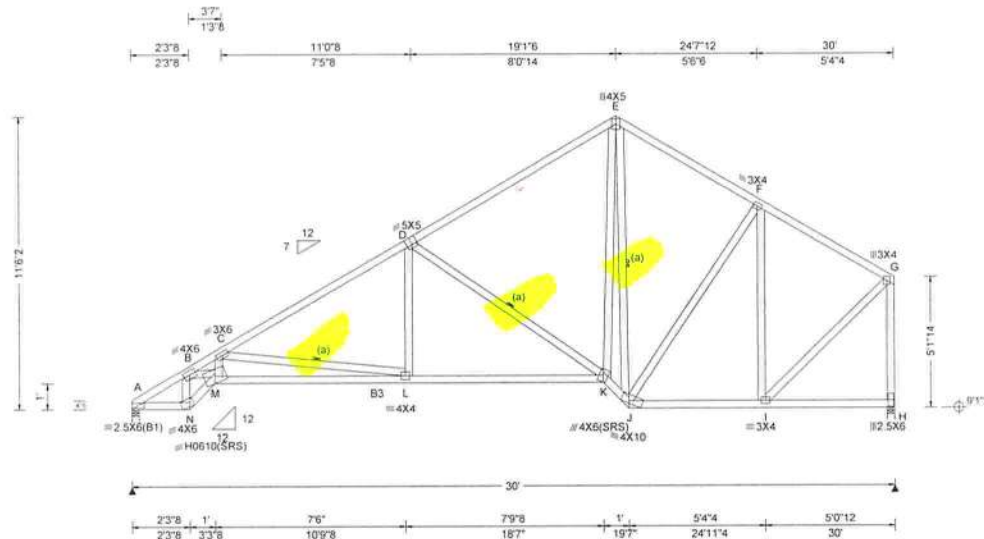
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity Non-Gravity
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.137 L 999 240	Loc R+ / R- / Rh / Rw / U / RL
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.286 L 999 180	A 1264 /- /- /761 /1 /287
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.095 l - -	H 1247 /- /- /670 /21 /-
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.199 l - -	Wind reactions based on MWFRS
NCBCLL: 10.00	Mean Height: 15.03 ft	Building Code:	Creep Factor: 2.0	A Brg Wid = 3.5 Min Req = 1.5 (Truss)
Soffit: 0.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.760	H Brg Wid = 3.5 Min Req = 1.5 (Truss)
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.625	Bearings A & H are a rigid surface.
Spacing: 24.0 "	MWFRS Parallel Dist: h to 2h	Rep Fac: Yes	Max Web CSI: 0.845	Members not listed have forces less than 375#
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)		Maximum Top Chord Forces Per Ply (lbs)
	Loc. from endwall: not in 9.00 ft	Plate Type(s):		Chords Tens.Comp. Chords Tens. Comp.
	GCpi: 0.18			A - B 348 -2108 D - E 325 -1184
	Wind Duration: 1.60	WAVE HS	VIEW Ver: 21.02.01.1216.15	

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2; B3 2x4 SP M-31;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Wind

Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

▲ Maximum Reactions (lbs)						
Loc	Gravity			Non-Gravity		
	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	1264	/-	/-	/761	/1	/287
H	1247	/-	/-	/670	/21	/-

Wind reactions based on MWFRS

A Brg Wid = 3.5 Min Req = 1.5 (Truss)

H Brg Wid = 3.5 Min Req = 1.5 (Truss)

Bearings A & H are a rigid surface.

Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords		Tens.Comp.	Chords		Tens. Comp.
A - B	348	-2108	D - E	325	-1184
B - C	906	-4313	E - F	326	-1010
C - D	400	-2060	F - G	203	-872

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
A - N	1747 -436	L - K	1671 -356
N - M	2320 -554	K - J	1115 -123
M - L	3570 -906	J - I	711 -120

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
B - N	377 - 1603	K - E	1358 - 199
B - M	2219 - 547	E - J	89 - 788
M - C	1094 - 172	F - I	175 - 565
C - L	557 - 1901	I - G	962 - 159
L - D	561 - 0	G - H	254 - 1205
D - K	301 - 944		



COA #0278
06/13/2022
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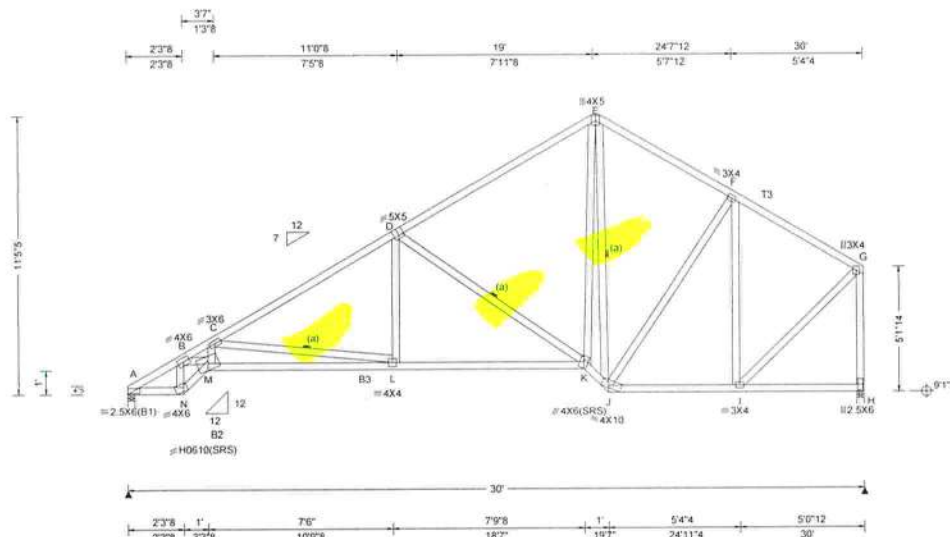
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025



Leading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity Non-Gravity
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.125 L 999 240	Loc R+ /R- /Rh /Rw /U /RL
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.261 L 999 180	A 1264 /- /- /761 /- /286
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.091 l - -	H 1247 /- /- /669 /20 /-
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.191 l - -	Wind reactions based on MWFRS
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0	A Brg Wid = 3.5 Min Req = 1.5 (Truss)
Soffit: 0.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.440	H Brg Wid = 3.5 Min Req = 1.5 (Truss)
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.485	Bearings A & H are a rigid surface.
Spacing: 24.0 "	MWFRS Parallel Dist: h to 2h	Rep Fac: Yes	Max Web CSI: 0.836	Members not listed have forces less than 375#
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)		Maximum Top Chord Forces Per Ply (lbs)
	Loc. from endwall: not in 9.00 ft	Plate Type(s):		Chords Tens.Comp. Chords Tens. Comp.
	GCpi: 0.18	WAVE HS	VIEW Ver: 21.02.01.1216.15	A - B 346 -2108 D - E 323 -1184
	Wind Duration: 1.60			A - B 999 4294 D - E 323 1034

Lumber

Top chord: 2x4 SP M-31; T3 2x4 SP #2;
Bot chord: 2x4 SP #2; B2,B3 2x4 SP M-31;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Wind

Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

▲ Maximum Reactions (lbs)						
Gravity			Non-Gravity			
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	1264	/-	/-	/761	/-	/286
H	1247	/-	/-	/669	/20	/-

Wind reactions based on MWFRS

A Brg Wid = 3.5 Min Req = 1.5 (Truss)

H Brg Wid = 3.5 Min Req = 1.5 (Truss)

Bearings A & H are a rigid surface.

Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)					
Chords			Tens. Comp.		
Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - B	346	-2108	D - E	323	-1184
B - C	892	-4291	E - F	324	-1011
C - D	398	-2059	F - G	201	-873

Maximum Bot Chord Forces Per Ply (lbs)					
Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - N	1743	-433	L - K	1673	-354
N - M	2314	-548	K - J	1113	-120
M - L	3544	-892	J - I	712	-119

Maximum Web Forces Per Ply (lbs)					
Webs	Tens.Comp.		Webs	Tens. Comp.	
B - N	371	- 1590	K - E	1359	- 197
B - M	2195	- 534	E - J	86	- 784
M - C	1094	- 170	F - I	174	- 565
C - L	544	- 1873	I - G	963	- 158
L - D	557	0	G - H	252	- 1205
D - K	302	- 948			



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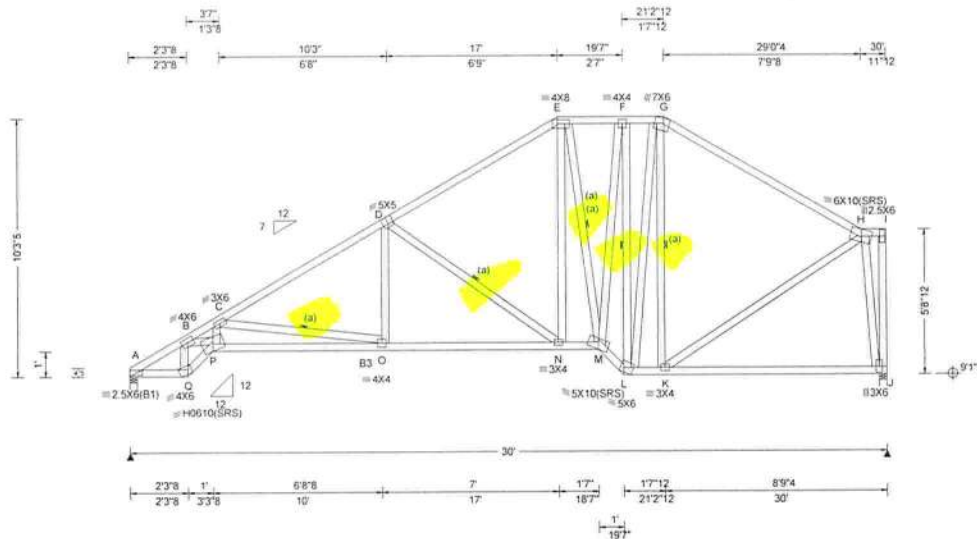
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100082	SPEC	Ply: 1	Job Number: 21-6493	Cust: R 215 JRef: 1XGd2150007 T19
FROM:		Qty: 1	Jones	DrwNo: 164.22.1545.14813
			Truss Label: A14	KD / WHK 06/13/2022



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity Non-Gravity
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.133 O 999 240	Loc R+ /R- /Rh /Rw /U /RL
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.279 O 999 180	A 1264 /- /- /767 /31 /255
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.087 J - -	J 1247 /- /- /662 /45 /-
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.182 J - -	Wind reactions based on MWFRS
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0	A Brg Wid = 3.5 Min Req = 1.5 (Truss)
Soffit: 0.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.631	J Brg Wid = 3.5 Min Req = 1.5 (Truss)
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.808	Bearings A & J are a rigid surface.
Spacing: 24.0 "	MWFRS Parallel Dist: h to 2h	Rep Fac: Yes	Max Web CSI: 0.830	Members not listed have forces less than 375#
	C&C Dist a: 3.00 ft	FT/RT: 20(0)/10(0)		Maximum Top Chord Forces Per Ply (lbs)
	Loc. from endwall: not in 9.00 ft	Plate Type(s):	VIEW Ver: 21.02.01.1216.15	Chords Tens.Comp. Chords Tens. Comp.
	GCpt: 0.18	WAVE, HS		A - B 433 -2110 E - F 418 -975
	Wind Duration: 1.60			B - C 1101 -4289 F - G 388 -854

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2; B3 2x4 SP M-31;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
A - Q	1750 -528	N - M	1041 -269
Q - P	2322 -680	M - L	1124 -291
P - O	3533 -1073	L - K	818 -204
O - N	1740 -496		

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
B - Q	470 -1604	E - N	640 -110
B - P	2179 -645	E - M	105 -393
P - C	1102 -242	M - F	1096 -278
C - O	584 -1798	F - L	249 -943
O - D	528 -25	K - H	695 -134
D - N	283 -867	H - J	521 -1409



COA #00078

Florida Certificate of Product Approval #FL 1999

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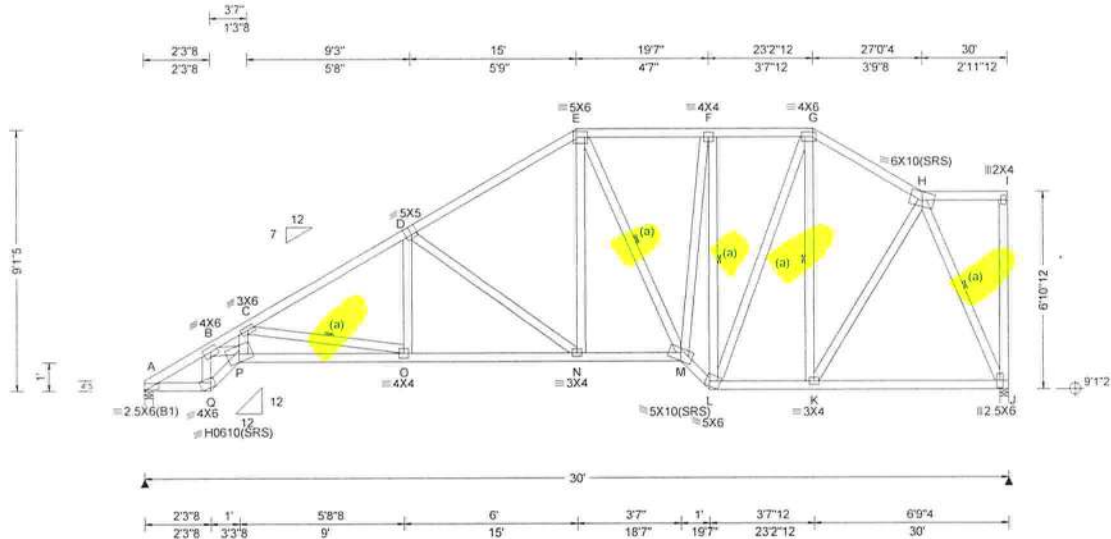
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100074 FROM:	SPEC Qty: 1	Job Number: 21-6493 Jones Truss Label: A15	Cust: R 215 JRef: 1XGd2150007 T18 DrwNo: 164.22.1545.11290 KD / WHK 06/13/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCCL: 20.00 TCCL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE, HS	PP Deflection in loc L/defl L/# VERT(LL): 0.142 O 999 240 VERT(CL): 0.297 O 999 180 HORZ(LL): 0.097 J - - HORZ(TL): 0.204 J - - Creep Factor: 2.0 Max TC CSI: 0.438 Max BC CSI: 0.957 Max Web CSI: 0.827 VIEW Ver: 21.02.01.1216.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 1264 /- /- /772 /58 /225 J 1247 /- /- /667 /135 /- Wind reactions based on MWFRS A Brg Wid = 3.5 Min Req = 1.5 (Truss) J Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings A & J are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 511 -2113 E - F 520 -1121 B - C 1304 -4261 F - G 474 -980 C - D 668 -2235 G - H 388 -937 D - E 570 -1508

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.



COA #0078

06/13/2022
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Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
A - Q	1754 -637	N - M	1216 -458
Q - P	2333 -830	M - L	1315 -525
P - O	3483 -1262	L - K	759 -287
O - N	1843 -670	K - J	504 -211

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
B - Q	581 -1616	E - N	590 -115
B - P	2127 -753	M - F	1111 -359
P - C	1123 -329	F - L	603 -1346
C - O	601 -1649	L - G	575 -279
O - D	495 -54	K - H	513 -153
D - N	268 -789	H - J	511 -1223

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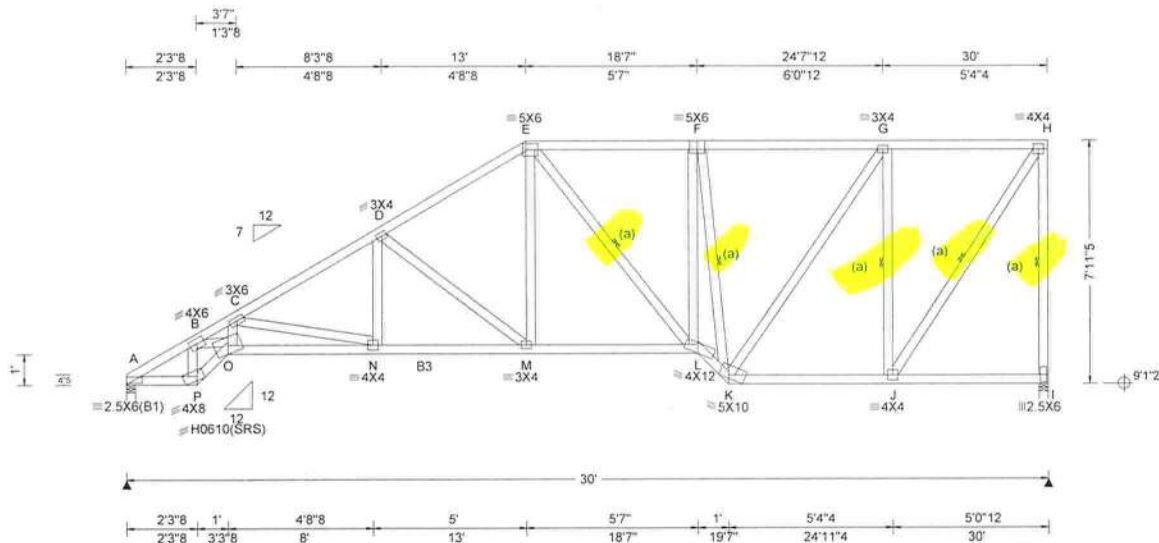
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100064 FROM:	HIPM Qty: 1	Job Number: 21-6493 Jones Truss Label: A16	Cust: R 215 JRef: 1XGd2150007 T35 DrwNo: 164.22.1545.09530 KD / WHK 06/13/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)						
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity			Non-Gravity			
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.133 M 999 240	Loc	R+	R-	/ Rh	/ Rw	/ U	/ RL
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.277 M 999 180	A	1264	/-	/-	/773	/174	/267
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.086 J - -	I	1247	/-	/-	/674	/257	/-
	EXP: C Kzt: NA		HORZ(TL): 0.179 J - -	Wind reactions based on MWFRS						
Des Ld: 40.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0	A	Brg Wid = 3.5 Min Req = 1.5 (Truss)					
NCBCLL: 10.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.528	I	Brg Wid = 3.5 Min Req = 1.5 (Truss)					
Soffit: 0.00	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.644	Bearings A & I are a rigid surface.						
Load Duration: 1.25	MWFRS Parallel Dist: h/2 to h	Rep Fac: Yes	Max Web CSI: 0.992	Members not listed have forces less than 375#						
Spacing: 24.0 "	C&C Dist a: 3.00 ft	FT/RT: 20(0)/10(0)		Maximum Top Chord Forces Per Ply (lbs)						
	Loc. from endwall: not in 9.00 ft	Plate Type(s):		Chords	Tens.Comp.		Chords	Tens. Comp.		
	GCpi: 0.18			A - B	587 - 2116		E - F	665 - 1348		
	Wind Duration: 1.60	WAVE, HS	VIEW Ver: 21.02.01.1216.15	B - C	1498 - 4229		F - G	550 - 1126		

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2; B3 2x4 SP M-31;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.		Chords	Tens. Comp.	
A - P	1757	-737	M - L	1404	-646
P - O	2327	-965	L - K	1812	-894
O - N	3446	-1445	K - J	749	-378
N - M	1959	-845			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.		Webs	Tens. Comp.	
B - P	679	-1605	F - L	1322	-530
B - O	2085	-860	F - K	881	-1704
O - C	1116	-402	K - G	682	-313
C - N	612	-1504	G - J	609	-976
N - D	462	-93	J - H	1289	-643
D - M	256	-711	H - I	655	-1205
E - M	573	-111			



COA #0078

06/13/2022
Florida Certificate of Product Approval #FL 1999

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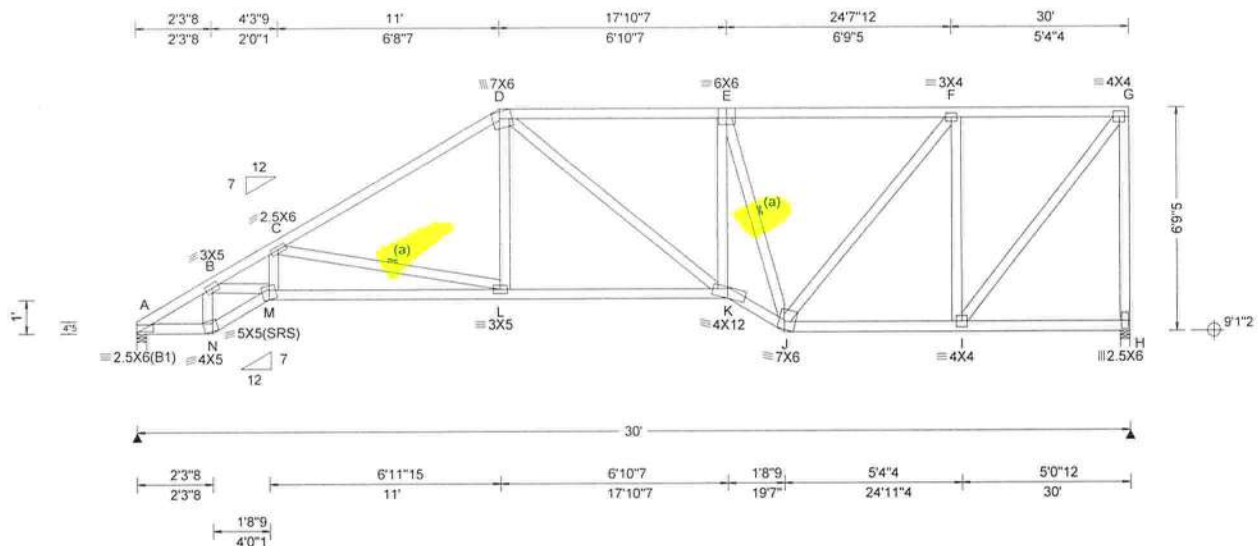
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.130 L 999 240 VERT(CL): 0.271 L 999 180 HORZ(LL): 0.078 I - - HORZ(TL): 0.162 I - - Creep Factor: 2.0 Max TC CSI: 0.534 Max BC CSI: 0.916 Max Web CSI: 0.985 VIEW Ver: 21.02.01.1216.15	Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL A 1260 /- /- /754 /187 /226 H 1245 /- /- /656 /249 /- Wind reactions based on MWFRS A Brg Wid = 3.5 Min Req = 1.5 (Truss) H Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings A & H are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 665 -2085 D - E 861 -1685 B - C 1351 -3597 E - F 658 -1307 C - D 826 -1998 F - G 432 -842

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
A - N	1735 -762	L - K	1637 -775
N - M	1959 -847	K - J	1950 -991
M - L	3010 -1316	J - I	886 -460

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
B - N	438 -995	E - J	793 -1476
B - M	1471 -625	J - F	684 -327
M - C	808 -252	F - I	621 -966
C - L	555 -1407	I - G	1368 -702
D - L	542 -49	G - H	670 -1204
E - K	1000 -368		



COA #06278

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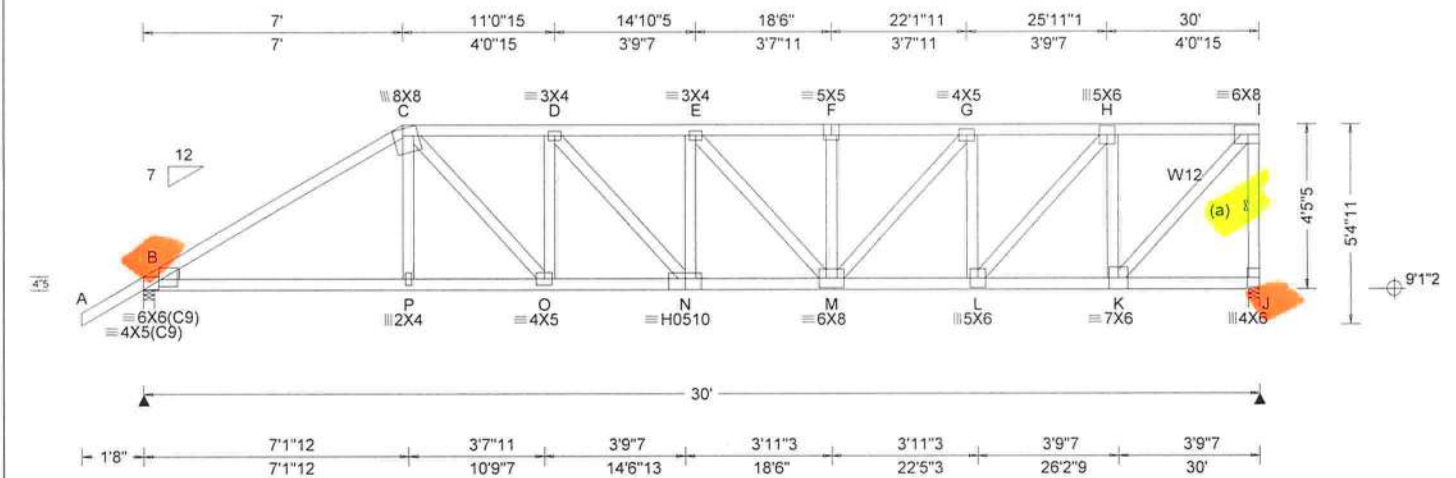


155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

Cust: R 215 JRef: 1XGd2150007 T5
DrwNo: 164.22.1545.03697
KD / WHK 06/13/2022



SEQN: 100047 FROM:	HIPM Qty: 1	Ply: 1 Jones Truss Label: A19	Job Number: 21-6493 Cust: R 215 JRef: 1XGd2150007 T45 DrwNo: 164.22.1545.00543 KD / WHK 06/13/2022
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Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria			▲ Maximum Reactions (lbs)							
TCLL: 20.00		Wind Std: ASCE 7-16		Pg: NA Ct: NA CAT: NA			PP Deflection in loc L/defl L/#			Gravity			Non-Gravity				
TCDL: 10.00		Speed: 130 mph		Pf: NA Ce: NA			VERT(LL): 0.232 E 999 240			Loc	R+	/R-	/Rh	/Rw	/U	/RL	
BCLL: 0.00		Enclosure: Closed		Lu: NA Cs: NA			VERT(CL): 0.467 E 767 180			B	3001	-	-	-	1674	-	
BCDL: 10.00		Risk Category: II		Snow Duration: NA			HORZ(LL): 0.070 K - -			J	3087	-	-	-	1744	-	
Des Ld: 40.00		EXP: C Kzt: NA					HORZ(TL): 0.140 K - -			Wind reactions based on MWFRS							
NCBCLL: 10.00		Mean Height: 15.00 ft		Building Code:			Creep Factor: 2.0			B Brg Wid = 3.5 Min Req = 2.5 (Truss)							
Soffit: 0.00		TCDL: 5.0 psf		FBC 7th Ed. 2020 Res.			Max TC CSI: 0.472			J Brg Wid = 3.5 Min Req = 2.6 (Truss)							
Load Duration: 1.25		BCDL: 5.0 psf		TPI Std: 2014			Max BC CSI: 0.635			Bearings B & J are a rigid surface.							
Spacing: 24.0 "		MWFRS Parallel Dist: 0 to h/2		Rep Fac: Varies by Ld Case			Max Web CSI: 0.931			Members not listed have forces less than 375#							
		C&C Dist a: 3.00 ft		FT/RT: 20(0)/10(0)						Maximum Top Chord Forces Per Ply (lbs)							
		Loc. from endwall: not in 4.50 ft		Plate Type(s):						Chords		Tens.Comp.		Chords		Tens. Comp.	
		GCpi: 0.18								B - C		1159 -5102		F - G		1275 -5374	
		Wind Duration: 1.60		WAVE HS			VIEW Ver: 21.02.01.1216.15			C - D		1240 -5356		G - H		1015 -4260	
										D - E		1341 -5713		H - I		601 -2514	
										E - F		1275 -5374					

Lumber

Top chord: 2x4 SP M-31;
 Bot chord: 2x4 SP M-31;
 Webs: 2x4 SP #3; W12 2x4 SP #2;
 Lt Wedge: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Special Loads

----- (Lumber Dur. Fac. = 1.25 / Plate Dur. Fac. = 1.25)
 TC: From 63 plf at -1.67 to 63 plf at 7.00
 TC: From 32 plf at 7.00 to 32 plf at 30.00
 BC: From 20 plf at 0.00 to 20 plf at 7.03
 BC: From 10 plf at 7.03 to 10 plf at 30.00
 TC: 435 lb Conc. Load at 7.03
 TC: 189 lb Conc. Load at 9.06, 11.06, 13.06, 15.06
 17.06, 19.06, 21.06, 23.06, 25.06, 27.06, 29.06
 BC: 506 lb Conc. Load at 7.03
 BC: 130 lb Conc. Load at 9.06, 11.06, 13.06, 15.06
 17.06, 19.06, 21.06, 23.06, 25.06, 27.06, 29.06

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads and reactions based on MWFRS.
 Right end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.



COA #062022

Florida Certificate of Product Approval #FL 1999

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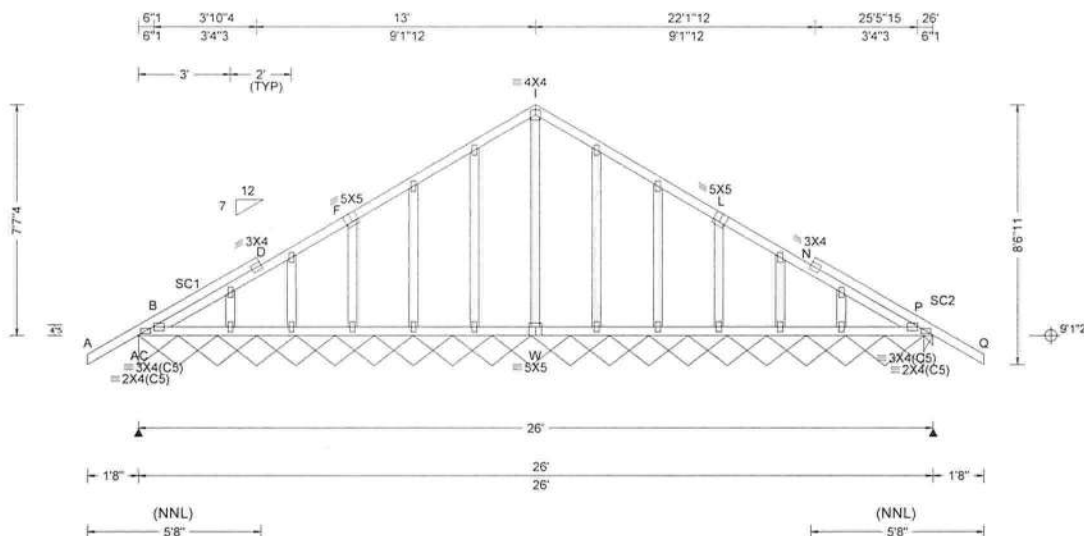
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ALPINE
 AN ITW COMPANY
 155 Harlem Ave
 North Building, 4th Floor
 Glenview, IL 60025

SEQN: 99985 FROM:	GABL Qty: 1	Ply: 1 Jones Truss Label: B01	Job Number: 21-6493 Cust: R 215 JRef: 1XGd2150007 T2 DrwNo: 164.22.1544.45430 KD / WHK 06/13/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or * = PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.002 N 999 240 VERT(CL): 0.004 N 999 180 HORZ(LL): 0.004 N - - HORZ(TL): 0.005 N - - Creep Factor: 2.0 Max TC CSI: 0.306 Max BC CSI: 0.042 Max Web CSI: 0.133 VIEW Ver: 21.02.01.1216.15	Gravity Loc R+ /R- /Rh /Rw /U /RL AC*80 /- /- /44 /14 /9 P 305 /- /- /209 /38 /- Wind reactions based on MWFRS AC Brg Wid = 308 Min Req = - P Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings AC & P are a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;

Plating Notes

All plates are 2X4 except as noted.

Purlins

In lieu of structural panels use purlins to brace TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.



COA #062022

06/13/2022
Florida Certificate of Product Approval #FL 1999

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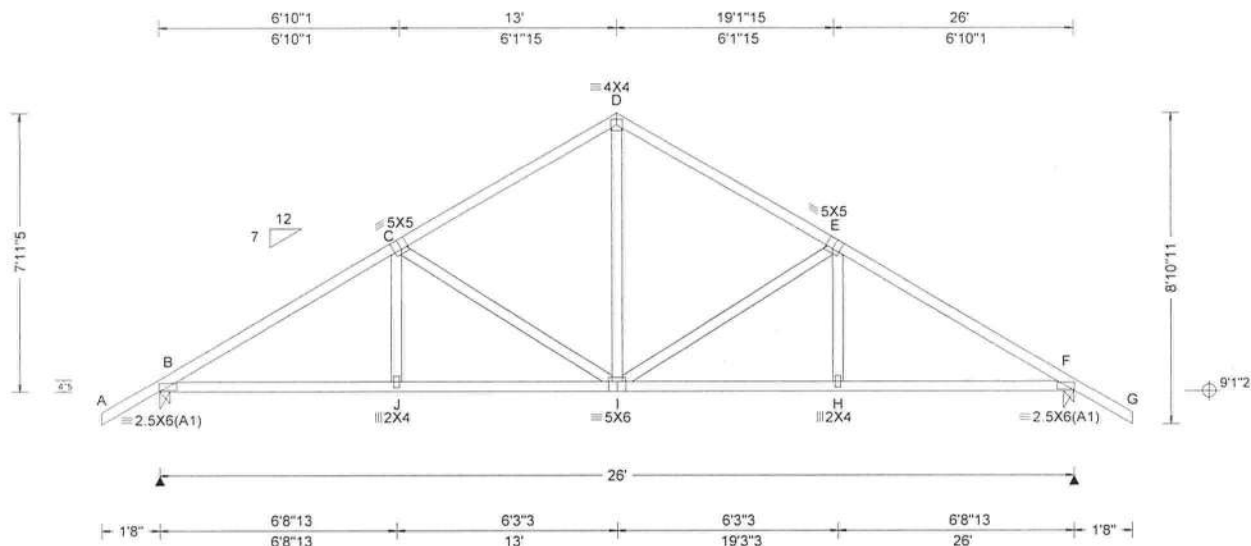
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 99928 FROM:	CONN Qty: 4	Ply: 1 Job Number: 21-6493 Jones Truss Label: B02	Cust: R 215 JRef: 1XGd2150007 T1 DrwNo: 164.22.1544.39763 KD / WHK 06/13/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.058 I 999 240 VERT(CL): 0.117 I 999 180 HORZ(LL): 0.027 F - - HORZ(TL): 0.055 F - - Creep Factor: 2.0 Max TC CSI: 0.426 Max BC CSI: 0.492 Max Web CSI: 0.542 VIEW Ver: 21.02.01.1216.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1186 /- /- /717 /206 /245 F 1186 /- /- /717 /206 /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) F Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings B & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 463 -1669 D - E 424 -1164 C - D 424 -1164 E - F 463 -1669

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - J	1356 -238	I - H	1353 -254
J - I	1353 -239	H - F	1356 -253

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
C - I	249 -518	I - E	249 -518
D - I	690 -201		



COA #0228

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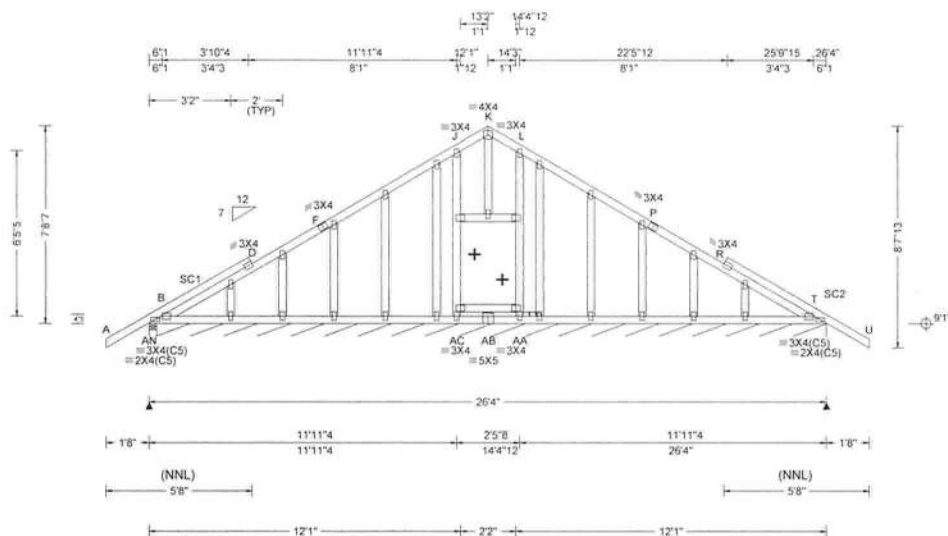
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 99982 FROM:	GABL Qty: 1	Ply: 1 Qty: 1	Job Number: 21-6493 Jones Truss Label: C01	Cust: R 215 JRef: 1XGd2150007 T7 DrwNo: 164.22.1544.37923 KD / WHK 06/13/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or * = PLF
TCCL: 20.00 TCCL: 10.00 BCCL: 0.00 BCCL: 10.00 Des Ld: 40.00 NCBCL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCCL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.004 K 999 240 VERT(CL): 0.005 D 999 180 HORZ(LL): 0.006 R - - HORZ(TL): 0.007 R - - Creep Factor: 2.0 Max TC CSI: 0.302 Max BC CSI: 0.049 Max Web CSI: 0.119 VIEW Ver: 21.02.01.1216.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL AN 319 /- /- /183 /28 /244 T* 80 /- /- /43 /15 /- Wind reactions based on MWFRS AN Brg Wid = 3.5 Min Req = 1.5 (Truss) T Brg Wid = 312 Min Req = - Bearings AN & AN are a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;
 Stack Chord: SC1 2x4 SP #2;
 Stack Chord: SC2 2x4 SP #2;

Plating Notes

All plates are 2X4 except as noted.

Wind

Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

+ Member to be laterally braced for horizontal wind loads, bracing system to be designed and furnished by others.



COA #0028

06/13/2022 Florida Certificate of Product Approval #FL 1999

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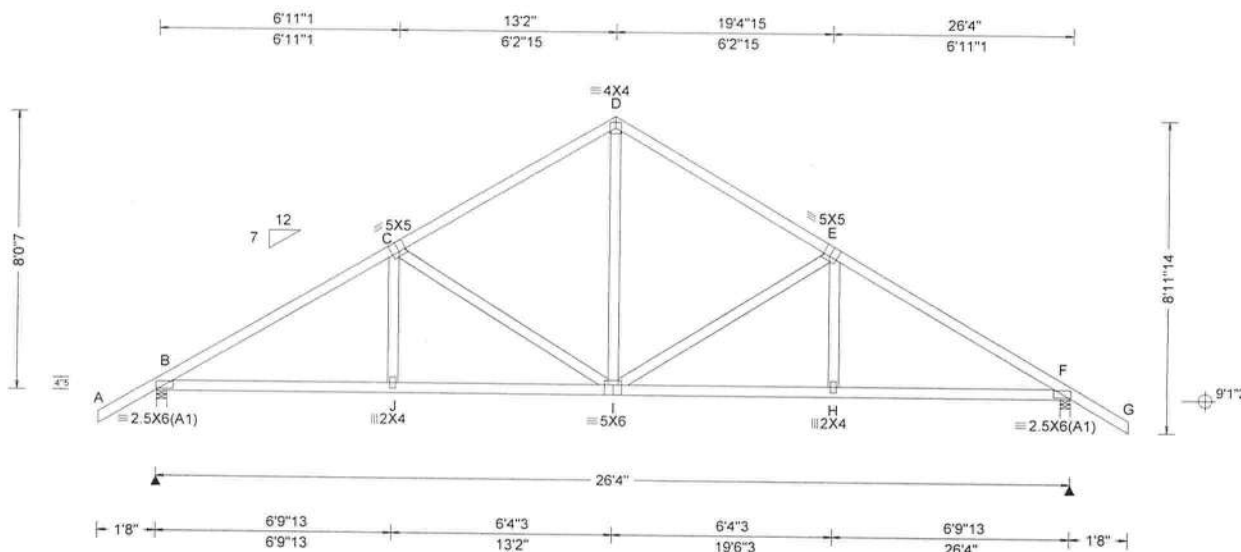
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155 Harlem Ave
 North Building, 4th Floor
 Glenview, IL 60025



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity Non-Gravity
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.059 I 999 240	Loc R+ / R- / Rh / Rw / U / RL
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.120 I 999 180	B 1200 /- /- /725 /208 /247
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.028 F - -	F 1200 /- /- /725 /208 /-
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.057 F - -	Wind reactions based on MWFRS
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0	B Brg Wid = 3.5 Min Req = 1.5 (Truss)
Soffit: 0.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.440	F Brg Wid = 3.5 Min Req = 1.5 (Truss)
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.504	Bearings B & F are a rigid surface.
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes	Max Web CSI: 0.565	Members not listed have forces less than 375#
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)		Maximum Top Chord Forces Per Ply (lbs)
	Loc. from endwall: Any	Plate Type(s):		Chords Tens.Comp. Chords Tens. Comp.
	GCpi: 0.18	WAVE	VIEW Ver: 21.02.01.1216.15	
Lumber	Wind Duration: 1.60			B - C 469 -1693 D - E 429 -1180
				C - D 429 -1180 E - F 468 -1693

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3:

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

▲ Maximum Reactions (lbs)						
Loc	Gravity			Non-Gravity		
	R+	/R-	/Rh	/Rw	/U	/RL
B	1200	/-	/-	/25	/208	/247
F	1200	/-	/-	/25	/208	/-

Wind reactions based on MWFRS

B Brg Wid = 3.5 Min Req = 1.5 (Truss)

F Brg Wid = 3.5 Min Req = 1.5 (Truss)

Bearings B & F are a rigid surface.

Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

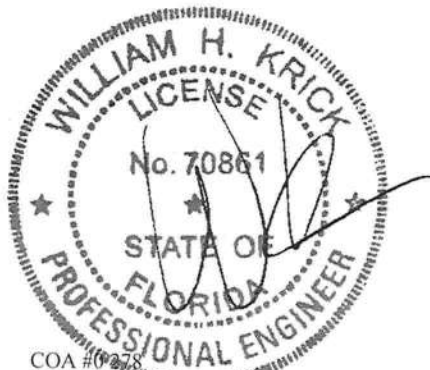
Chords	Tens.Comp.	Chords	Tens. Comp.
B - C	469 - 1693	D - E	429 - 1180
C - D	429 - 1180	E - F	468 - 1693

Maximum Bot Chord Forces Per Ply (lbs)

Maximum Net Chord Forces Per Fly (lbs)							
Chords		Tens. Comp.		Chords		Tens. Comp.	
B - J	1375	-243	I - H	1373	-258		
J - I	1373	-244	H - F	1375	-257		

Maximum Web Forces Per Ply (lbs)

Maximum Web Forces Per Ply (lbs)					
Webs	Tens.Comp.		Webs	Tens. Comp.	
C - I	253	-527	I - E	253	-527
D - I	700	-203			



COA #0278

06/13/2032
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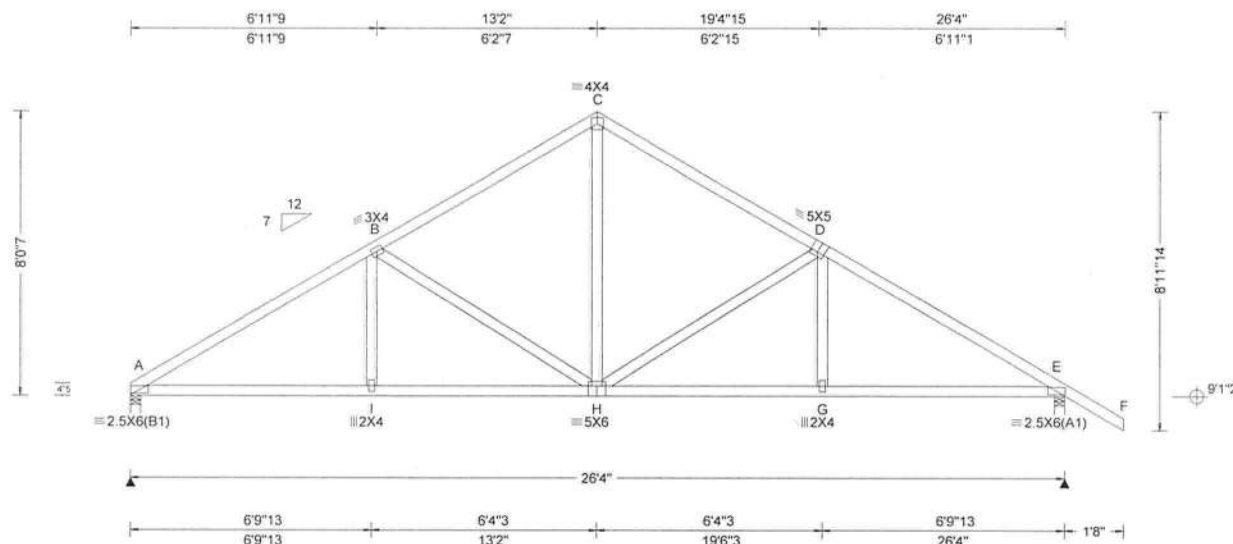
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)					
				Gravity			Non-Gravity		
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Loc	R+ / R- / Rh				
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.058 H 999 240	A	1091	/- /-	/629	/12	/229
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.119 H 999 180	E	1204	/- /-	/725	/20	/-
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.028 E - -	Wind reactions based on MWFRS					
Des Ld: 40.00	EXP: C Kzt: NA	Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	HORZ(TL): 0.057 E - -	A Brg Wid = 3.5 Min Req = 1.5 (Truss)					
NCBCLL: 10.00	Mean Height: 15.00 ft		Creep Factor: 2.0	E Brg Wid = 3.5 Min Req = 1.5 (Truss)					
Soffit: 0.00	TCDL: 5.0 psf		Max TC CSI: 0.485	Bearings A & E are a rigid surface.					
Load Duration: 1.25	BCDL: 5.0 psf		Max BC CSI: 0.518	Members not listed have forces less than 375#					
Spacing: 24.0 "	MWFRS Parallel Dist: h to 2h		Max Web CSI: 0.592	Maximum Top Chord Forces Per Ply (lbs)					
	C&C Dist a: 3.00 ft			Chords Tens.Comp.		Chords Tens. Comp.			
	Loc. from endwall: not in 9.00 ft			A - B 330 -1719		C - D 306 -1187			
	GCPi: 0.18		VIEW Ver: 21.02.01.1216.15						
	Wind Duration: 1.60								

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.



COA #0028

06/13/2022
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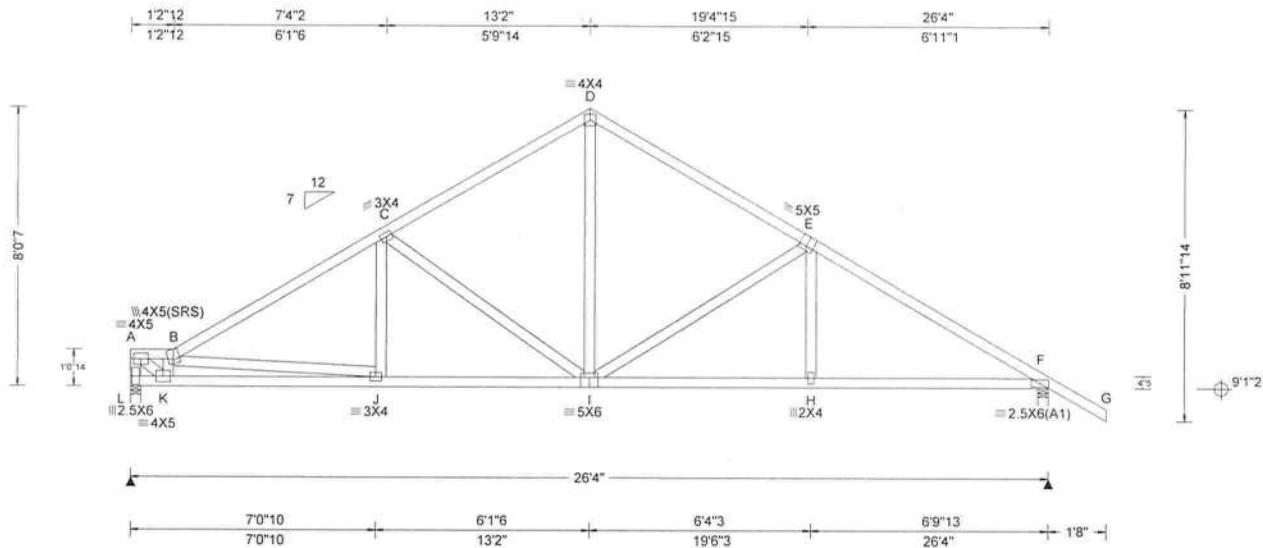
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100030 FROM:	COMN Ply: 1 Qty: 1	Job Number: 21-6493 Jones Truss Label: C04	Cust: R 215 JRef: 1XGd2150007 T15 DrwNo: 164.22.1543.11750 KD / WHK 06/13/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.062 I 999 240 VERT(CL): 0.127 I 999 180 HORZ(LL): 0.029 F - - HORZ(TL): 0.060 F - - Creep Factor: 2.0 Max TC CSI: 0.444 Max BC CSI: 0.541 Max Web CSI: 0.648 VIEW Ver: 21.02.01.1216.15	Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL L 1085 /- /- /607 /14 /218 F 1210 /- /- /728 /20 /- Wind reactions based on MWFRS L Brg Wid = 3.5 Min Req = 1.5 (Truss) F Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings L & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 260 - 1313 D - E 309 - 1198 B - C 333 - 1703 E - F 324 - 1713 C - D 312 - 1192

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.
Left end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
K - J	1710 - 309	I - H	1391 - 155
J - I	1390 - 154	H - F	1393 - 154

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
A - L	201 - 1057	C - I	191 - 550
A - K	1702 - 336	D - I	734 - 151
K - B	315 - 1125	I - E	185 - 531



COA #0928

Florida Certificate of Product Approval #FL 1999

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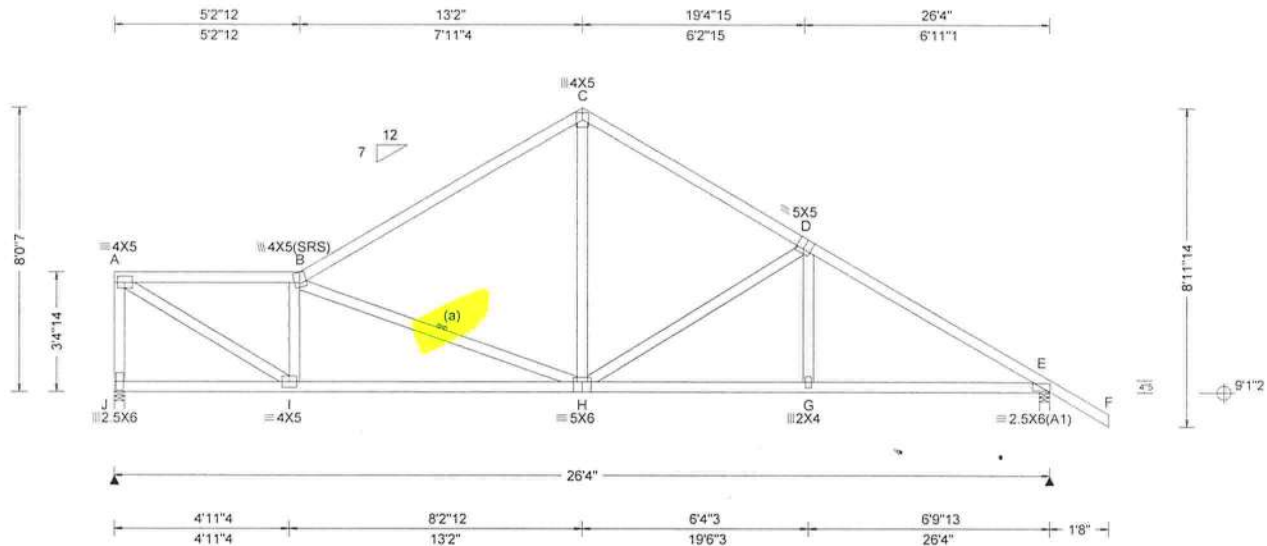
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100037 FROM:	CONN Qty: 1	Ply: 1 Jones	Job Number: 21-6493 Truss Label: C06	Cust: R 215 JRef: 1XGd2150007 T36 DrwNo: 164.22.1543.00790 KD / WHK 06/13/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defll/CSI Criteria	▲ Maximum Reactions (lbs)						
				Gravity			Non-Gravity			
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Loc	R+	/R-	/Rh	/Rw	/U	/RL
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.064 H 999 240	J	1085	/-	/-	/563	/58	/216
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.130 H 999 180	E	1210	/-	/-	/736	/21	/-
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.024 E - -	Wind reactions based on MWFRS						
	EXP: C Kzt: NA		HORZ(TL): 0.050 E - -	J Brg Wid = 3.5 Min Req = 1.5 (Truss)						
Des Ld: 40.00	Mean Height: 15.00 ft		Creep Factor: 2.0	E Brg Wid = 3.5 Min Req = 1.5 (Truss)						
NCBCLL: 10.00	TCDL: 5.0 psf	Building Code:	Max TC CSI: 0.720	Bearings J & E are a rigid surface.						
Soffit: 0.00	BCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max BC CSI: 0.675	Members not listed have forces less than 375#						
Load Duration: 1.25	MWFRS Parallel Dist: h to 2h	TPI Std: 2014	Max Web CSI: 0.665	Maximum Top Chord Forces Per Ply (lbs)						
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Fac: Yes		Chords	Tens.	Comp.	Chords	Tens.	Comp.	
	Loc. from endwall: not in 9.00 ft	FT/RT:20(0)/10(0)								
	GCpi: 0.18	Plate Type(s):								
	Wind Duration: 1.60	WAVE	VIEW Ver: 21.02.01.1216.15	A - B	417	-1484	C - D	332	-1210	

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Left end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
I - H	1557	-300	G - E	1385	-171
H - G	1383	-172			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
A - J	390	-1049	B - H	296	-637
A - I	1745	-484	C - H	686	-129
I - B	338	-793	H - D	176	-499



COA #0238

Florida Certificate of Product Approval #FL 1999

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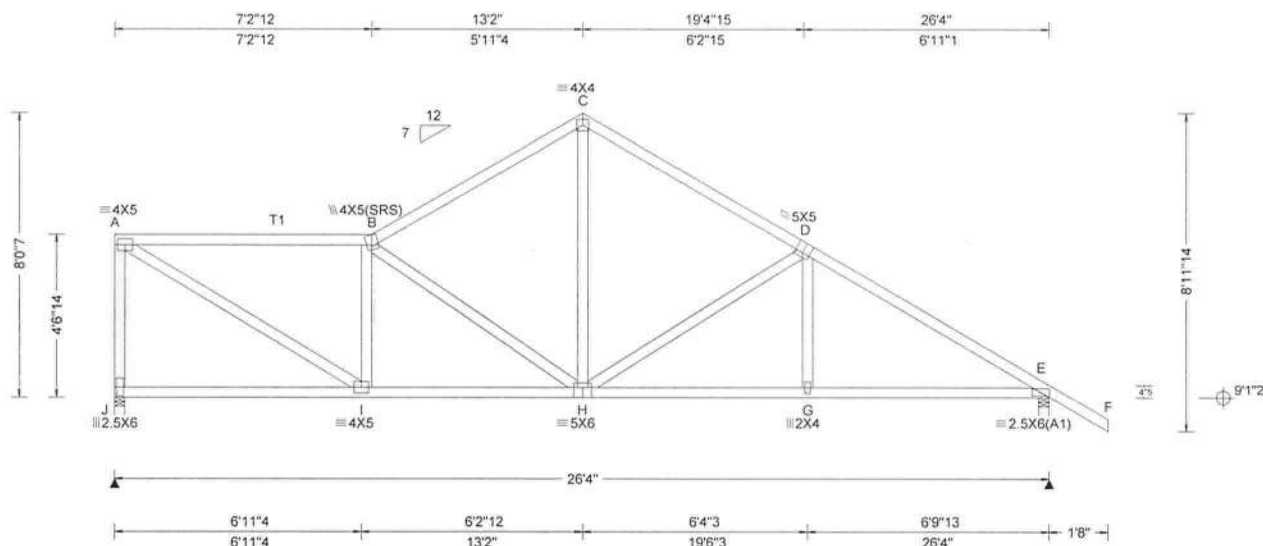
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100040 FROM:	COMN Ply: 1 Qty: 1	Job Number: 21-6493 Jones Truss Label: C07	Cust: R 215 JRef: 1XGd2150007 T40 DrwNo: 164 22.1542.58330 KD / WHK 06/13/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.062 H 999 240 VERT(CL): 0.128 H 999 180 HORZ(LL): 0.021 E - - HORZ(TL): 0.043 E - - Creep Factor: 2.0 Max TC CSI: 0.464 Max BC CSI: 0.577 Max Web CSI: 0.642 VIEW Ver: 21.02.01.1216.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL J 1085 /- /- /572 /97 /216 E 1210 /- /- /743 /30 /- Wind reactions based on MWFRS J Brg Wid = 3.5 Min Req = 1.5 (Truss) E Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings J & E are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 439 -1351 C - D 354 -1198 B - C 363 -1193 D - E 368 -1713

Lumber

Top chord: 2x4 SP #2; T1 2x4 SP M-31;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Purlins

In lieu of structural panels use purlins to brace all flat
 TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C
 member design.
 Left end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
I - H	1397 -272	G - E	1393 -192
H - G	1391 -193		

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
A - J	467 -1027	B - H	290 -558
A - I	1587 -516	C - H	736 -213
I - B	349 -688	H - D	187 -532



COA #0278

Florida Certificate of Product Approval #FL 1999

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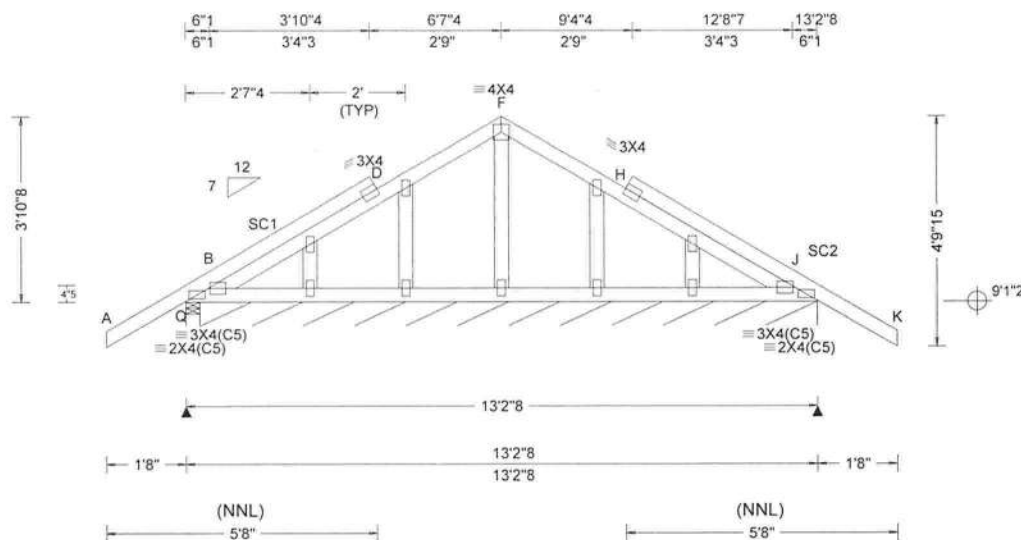
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155 Harlem Ave
 North Building, 4th Floor
 Glenview, IL 60025



Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria				▲ Maximum Reactions (lbs), or *=PLF							
TCLL:	20.00	Wind Std:	ASCE 7-16	Pg: NA	Ct: NA	CAT: NA	PP Deflection in loc L/defl L/#				Gravity			Non-Gravity				
TCDL:	10.00	Speed:	130 mph	Pf: NA		Ce: NA	VERT(LL):	0.001	H	999	240	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
BCLL:	0.00	Enclosure:	Closed	Lu: NA	Cs: NA		VERT(CL):	0.002	H	999	180	Q	298	/-	/-	/198	/69	/145
BCDL:	10.00	Risk Category:	II	Snow Duration: NA			HORZ(LL):	0.001	H	-	-	Q*	78	/-	/-	/45	/12	/-
		EXP:	C Kzt: NA				HORZ(TL):	0.002	H	-	-	Wind reactions based on MWFRS						
Des Ld:	40.00	Mean Height:	15.00 ft				Creep Factor:	2.0	Q Brg Wid = 3.5 Min Req = 1.5 (Truss)									
NCBCLL:	10.00	TCDL:	5.0 psf	Building Code:			Max TC CSI:	0.338	Q Brg Wid = 155 Min Req = -									
Softit:	0.00	BCDL:	5.0 psf	FBC 7th Ed. 2020 Res.			Max BC CSI:	0.032	Bearings Q & Q are a rigid surface.									
Load Duration:	1.25	MWFRS Parallel Dist:	0 to h/2	TPI Std: 2014			Max Web CSI:	0.037	Members not listed have forces less than 375#									
Spacing:	24.0 "	C&C Dist a:	3.00 ft	Rep Fac: Yes														
		Loc. from endwall:	Any	FT/RT:20(0)/10(0)														
		GCpi:	0.18	Plate Type(s):														
		Wind Duration:	1.60	WAVE			VIEW Ver: 21.02.01.1216.15											

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2.

Plating Notes

All plates are 2X4 except as noted.

Purlins

In lieu of structural panels use purlins to brace TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Additional Notes

See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.



COA #0278

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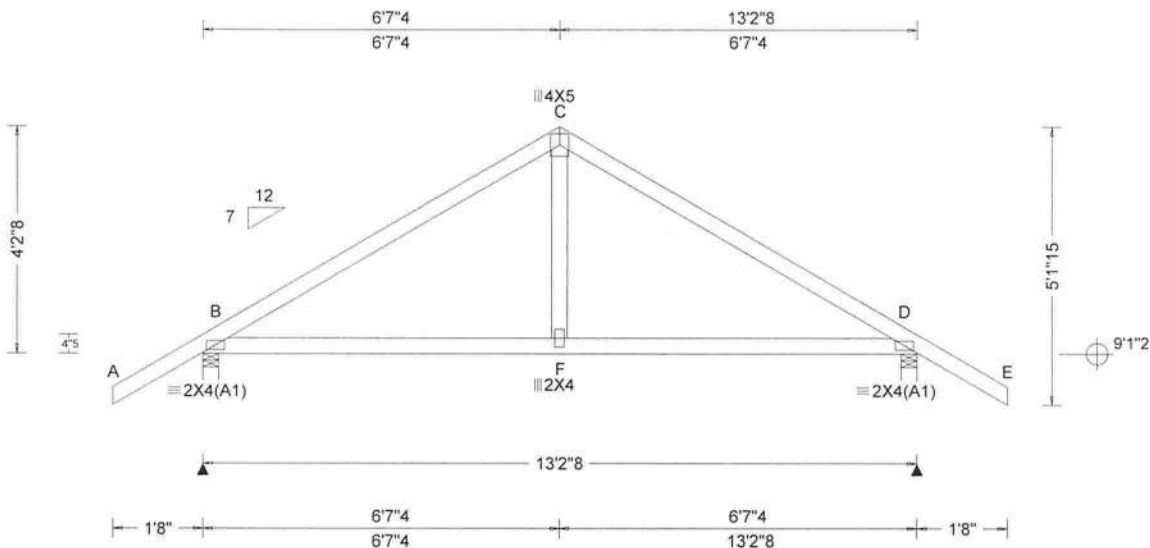
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North Building, 4th Floor
Glenview, IL 60025



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.009 F 999 240 VERT(CL): 0.017 F 999 180 HORZ(LL): 0.005 D - - HORZ(TL): 0.009 D - - Creep Factor: 2.0 Max TC CSI: 0.413 Max BC CSI: 0.417 Max Web CSI: 0.111 VIEW Ver: 21.02.01.1216.15	Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL B 654 /- /- /415 /117 /148 D 654 /- /- /321 /117 /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) D Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings B & D are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 320 -669 C - D 320 -669

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - F	498 -67	F - D	498 -67



COA #0-278

Florida Certificate of Product Approval #FL 1999

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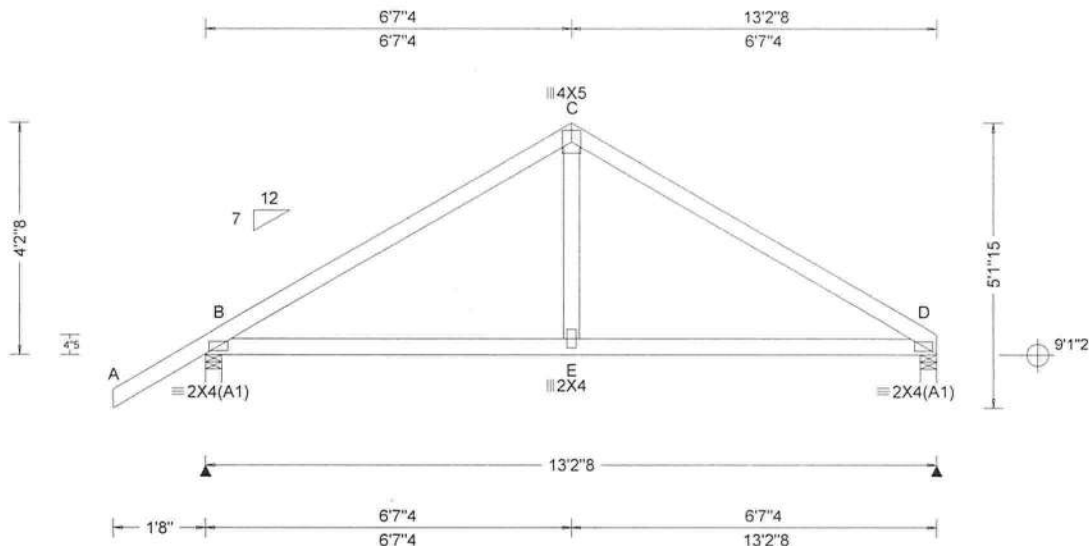
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 99946 FROM:	COMN Qty: 1	Ply: 1	Job Number: 21-6493 Jones Truss Label: D03	Cust: R 215 JRef: 1XGd2150007 T26 DrwNo: 164.22.1542.49750 KD / WHK 06/13/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.009 D 999 240 VERT(CL): 0.018 D 999 180 HORZ(LL): 0.005 D - - HORZ(TL): 0.009 D - - Creep Factor: 2.0 Max TC CSI: 0.450 Max BC CSI: 0.427 Max Web CSI: 0.113 VIEW Ver: 21.02.01.1216.15	Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL B 662 -/- /- /415 /120 /130 D 541 -/- /- /315 /86 /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) D Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings B & D are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 206 -690 C - D 203 -686

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - E	517 -75	E - D	517 -75



COA #09258

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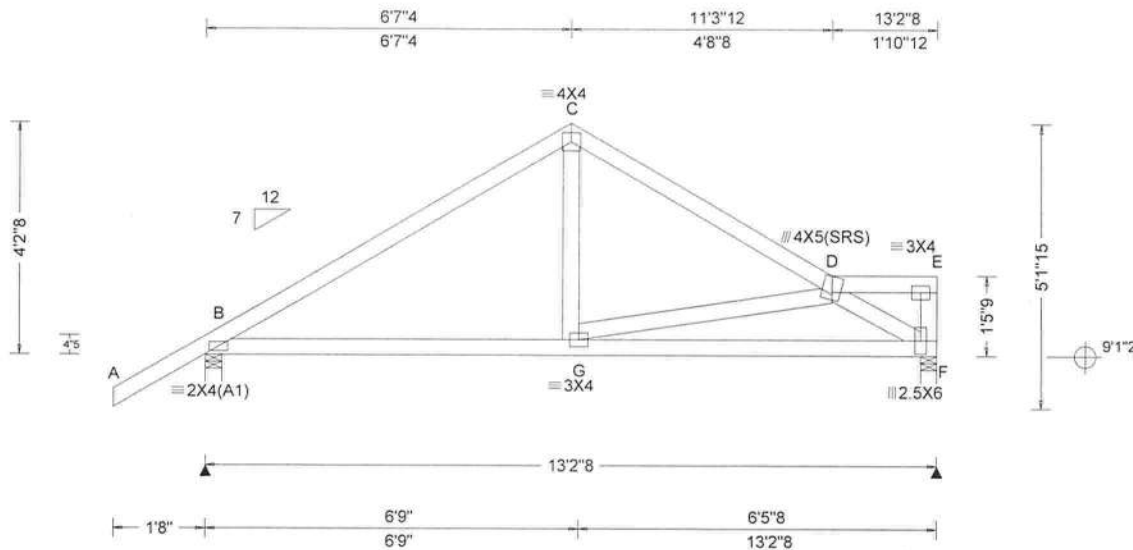
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025



Loading Criteria (psf)		Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)					
TCLL: 20.00		Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity		Non-Gravity			
TCDL: 10.00		Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.012 G 999 240	Loc	R+	/ R-	/ Rh	/ Rw	/ U / RL
BCLL: 0.00		Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.023 G 999 180	B	668	-/-	-/-	/420	/119 /117
BCDL: 10.00		Risk Category: II	Snow Duration: NA	HORZ(LL): 0.006 F - -	F	535	-/-	-/-	/286	/90 -/-
Des Ld: 40.00		EXP: C Kzt: NA		HORZ(TL): 0.011 F - -	Wind reactions based on MWFRS					
NCBCLL: 10.00		Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0	B Brg Wid = 3.5 Min Req = 1.5 (Truss)					
Soffit: 0.00		TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.391	F Brg Wid = 3.5 Min Req = 1.5 (Truss)					
Load Duration: 1.25		BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.464	Bearings B & F are a rigid surface.					
Spacing: 24.0"		MWFRS Parallel Dist: h/2 to h	Rep Fac: Yes	Max Web CSI: 0.160	Members not listed have forces less than 375#					
		C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)		Maximum Top Chord Forces Per Ply (lbs)					
		Loc. from endwall: not in 9.00 ft	Plate Type(s):		Chords		Tens.Comp.		Chords	
		GCpi: 0.18	WAVE	VIEW Ver: 21.02.01.1216.15	B - C		204 -685		C - D	
		Wind Duration: 1.60							210 -654	

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - G	510 -122	G - F	701 -302

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.
D - F	359 -810



COA #0028

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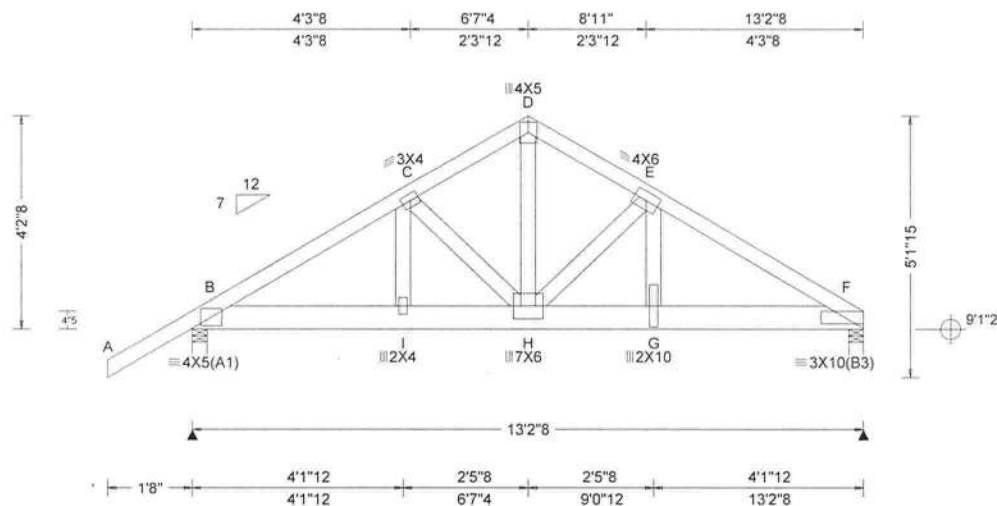
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100020	SPEC	Ply: 2	Job Number: 21-6493	Cust: R 215 JRef: 1XGd2150007 T48
FROM:		Qty: 1	Jones	DrwNo: 164.22.1542.43747
			Truss Label: D05	KD / WHK 06/13/2022

2 Complete Trusses Required



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity Non-Gravity
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.046 G 999 240	Loc R+ / R- / Rh / Rw / U / RL
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.092 G 999 180	B 2915 /- /- /- /646 /-
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.013 C - -	F 4546 /- /- /- /936 /-
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.025 C - -	Wind reactions based on MWFRS
NCBCLL: 0.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0	B Brg Wid = 3.5 Min Req = 1.5 (Truss)
Soffit: 0.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.169	F Brg Wid = 3.5 Min Req = 1.9 (Truss)
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.416	Bearings B & F are a rigid surface.
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: No	Max Web CSI: 0.923	Members not listed have forces less than 375#
	C&C Dist a: 3.00 ft	FT/RT: 20(0)/10(0)		Maximum Top Chord Forces Per Ply (lbs)
	Loc. from endwall: not in 9.00 ft	Plate Type(s):	VIEW Ver: 21.02.01.1216.15	Chords Tens.Comp. Chords Tens. Comp.
	GCpi: 0.18	WAVE		B - C 546 -2497 D - E 563 -2540
	Wind Duration: 1.60			C - D 563 -2538 E - F 778 -3643

Lumber

Top chord: 2x4 SP M-31;
Bot chord: 2x6 SP 2400F-2.0E;
Webs: 2x4 SP #3;

Nailnote

Nail Schedule: 0.131"x3", min. nails
Top Chord: 1 Row @ 12.00" o.c.
Bot Chord: 1 Row @ 3.25" o.c.
Webs: 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special Loads

----- (Lumber Dur. Fac. = 1.25 / Plate Dur. Fac. = 1.25)
TC: From 63 plf at -1.67 to 63 plf at 13.21
BC: From 20 plf at 0.00 to 20 plf at 7.06
BC: From 10 plf at 7.06 to 10 plf at 11.06
BC: From 20 plf at 11.06 to 20 plf at 13.21
BC: 3454 lb Conc. Load at 7.06
BC: 1422 lb Conc. Load at 9.06, 11.06

Wind

Wind loads and reactions based on MWFRS.
Wind loading based on both gable and hip roof types.



COA #09278

06/13/2022 Florida Certificate of Product Approval #FL 1999

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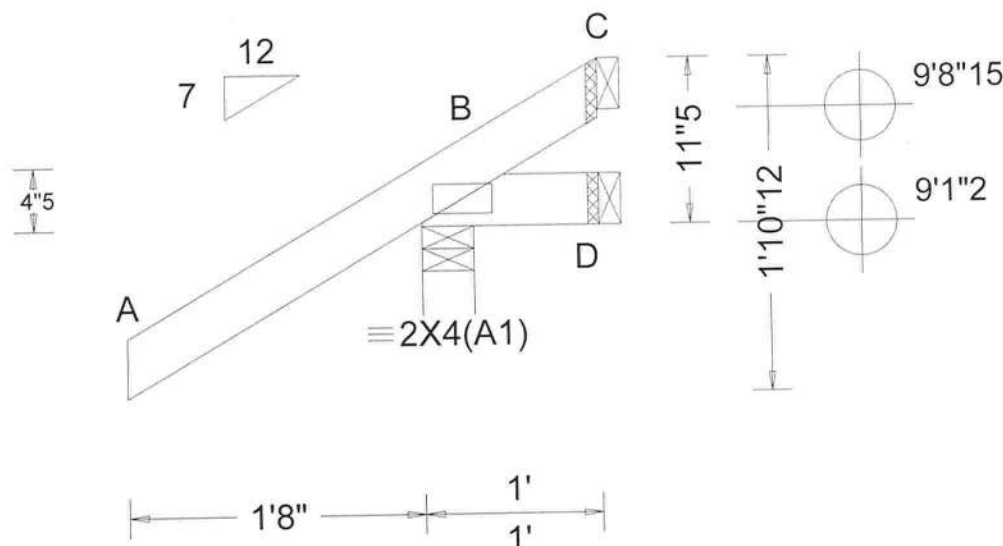
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North Building, 4th Floor
Glenview, IL 60025



Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg.Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): -0.000 B - - HORZ(TL): 0.001 B - - Creep Factor: 2.0 Max TC CSI: 0.308 Max BC CSI: 0.048 Max Web CSI: 0.000 VIEW Ver: 21.02.01.1216.15	▲ Maximum Reactions (lbs)						
				Gravity			Non-Gravity			
				Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
				B	275	/-	/-	/238	/75	/47
		D	4	/-21	/-	/17	/20	/-		
C	-	/-66	/-	/42	/70	/-				
Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#										

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Wind loads based on MWFRS with additional C&C member design.



COA #0278

Florida Certificate of Product Approval #FL 1999

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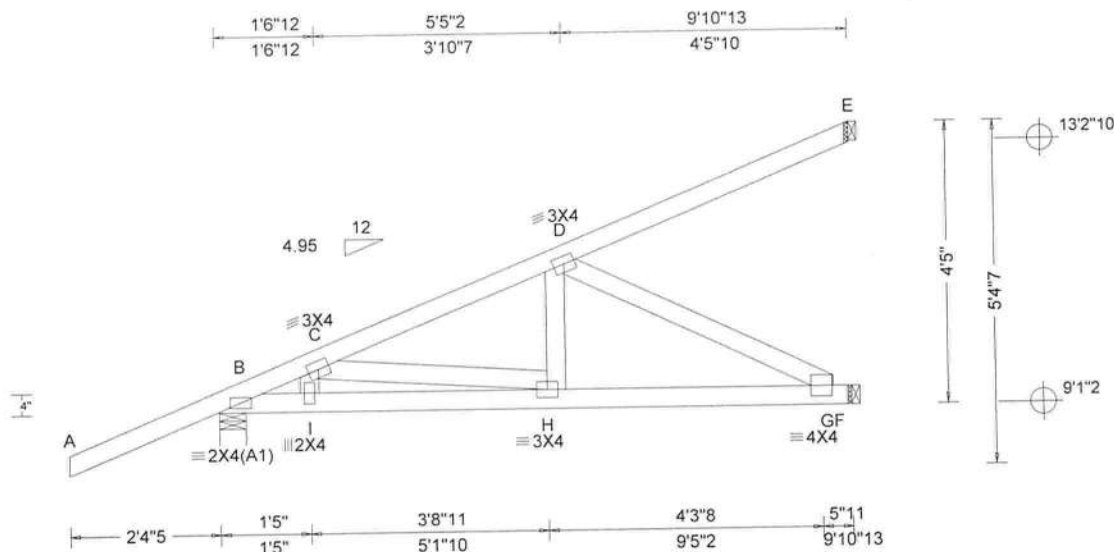
SEQN: 100002
FROM:

HIP

Ply: 1
Qty: 2

Job Number: 21-6493
Jones
Truss Label: J01HJ

Cust: R 215 JRef: 1XGd2150007 T43
DrwNo: 164 22 1542.29420
KD / WHK 06/13/2022



Loading Criteria (psf)

TCLL: 20.00
TCDL: 10.00
BCLL: 0.00
BCDL: 10.00
Des Ld: 40.00
NCBCLL: 0.00
Soffit: 0.00
Load Duration: 1.25
Spacing: 24.0"

Wind Criteria

Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: NA
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)

Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in loc L/defl L/#
VERT(LL): 0.014 H 999 240
VERT(CL): 0.028 H 999 180
HORZ(LL): -0.003 E - -
HORZ(TL): 0.006 E - -
Creep Factor: 2.0
Max TC CSI: 0.586
Max BC CSI: 0.201
Max Web CSI: 0.326

VIEW Ver: 21.02.01.1216.15

▲ Maximum Reactions (lbs)

Loc	R+	Gravity			Non-Gravity		
		/R-	/Rh	/Rw	/U	/RL	
B	491	/-	/-	/-	/100	/-	
F	376	/-	/-	/-	/9	/-	
E	246	/-	/-	/-	/92	/-	

Wind reactions based on MWFRS

B Brg Wid = 4.9 Min Req = 1.5 (Truss)

F Brg Wid = 1.5 Min Req = -

E Brg Wid = 1.5 Min Req = -

Bearing B is a rigid surface.

Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords Tens.Comp.

C - D 103 -671

Maximum Bot Chord Forces Per Ply (lbs)

Chords Tens.Comp.

H - G 597 -100

Maximum Web Forces Per Ply (lbs)

Webs Tens.Comp.

D - G 113 -676

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP M-31;
Webs: 2x4 SP #3;

Loading

Hipjack supports 7-0-0 setback jacks with no webs.

Wind

Wind loads and reactions based on MWFRS.

Wind loading based on both gable and hip roof types.



COA #62018

Florida Certificate of Product Approval #FL 1999

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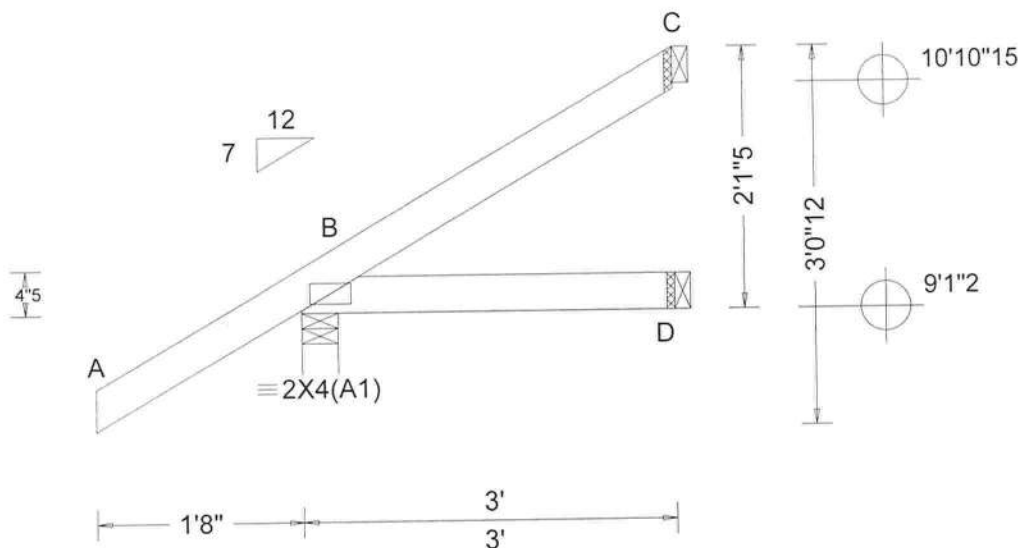
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North Building, 4th Floor
Glenview, IL 60025



Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 0.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: > 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg.Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.001 B - - HORZ(TL): 0.001 B - - Creep Factor: 2.0 Max TC CSI: 0.238 Max BC CSI: 0.065 Max Web CSI: 0.000	▲ Maximum Reactions (lbs) <table><tr><th rowspan="2"></th><th colspan="3">Gravity</th><th colspan="3">Non-Gravity</th></tr><tr><th>Loc</th><th>R+ / R-</th><th>/ Rh</th><th>/ Rw</th><th>/ U</th><th>/ RL</th></tr><tr><td>B</td><td>272</td><td>/-</td><td>/-</td><td>/208</td><td>/41</td><td>/88</td></tr><tr><td>D</td><td>50</td><td>/-</td><td>/-</td><td>/33</td><td>/41</td><td>/-</td></tr><tr><td>C</td><td>61</td><td>/-</td><td>/-</td><td>/37</td><td>/37</td><td>/-</td></tr></table> Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#		Gravity			Non-Gravity			Loc	R+ / R-	/ Rh	/ Rw	/ U	/ RL	B	272	/-	/-	/208	/41	/88	D	50	/-	/-	/33	/41	/-	C	61	/-	/-	/37	/37	/-
			Gravity			Non-Gravity																																
			Loc		R+ / R-	/ Rh	/ Rw	/ U	/ RL																													
B	272	/-	/-	/208	/41	/88																																
D	50	/-	/-	/33	/41	/-																																
C	61	/-	/-	/37	/37	/-																																
Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	VIEW Ver: 21.02.01.1216.15																																					

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Wind loads based on MWFRS with additional C&C member design.



COA #0278

Florida Certificate of Product Approval #FL 1999

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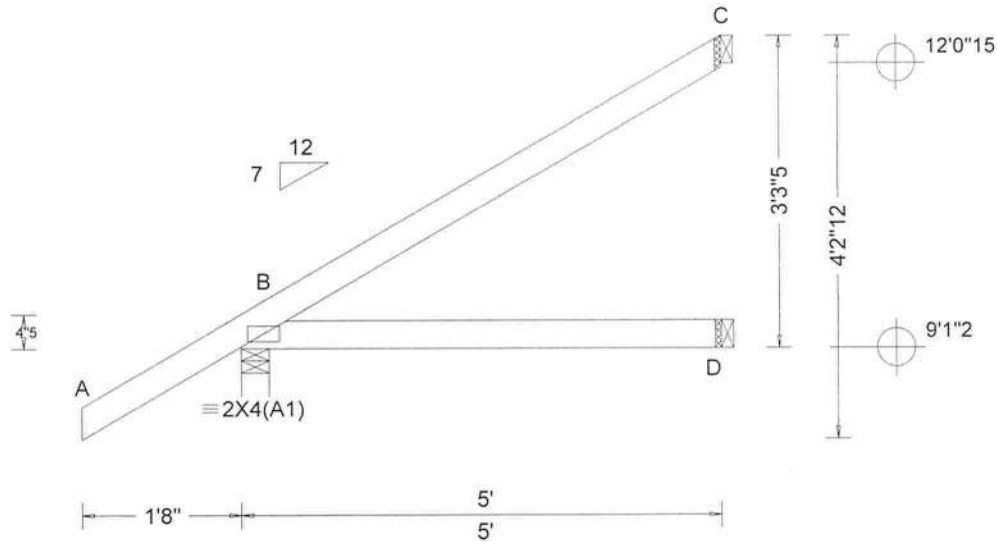
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 99994	JACK	Ply: 1	Job Number: 21-6493	Cust: R 215 JRef: 1XGd2150007 T11
FROM:		Qty: 4	Jones	DrwNo: 164 22 1541.20720
			Truss Label: J03	KD / WHK 06/13/2022



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity Non-Gravity
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): NA	Loc R+ / R- / Rh / Rw / U / RL
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): NA	B 341 /- /- /245 /38 /130
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.004 B - -	D 90 /- /- /52 /- /-
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.007 B - -	C 128 /- /- /81 /71 /-
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0	Wind reactions based on MWFRS
Soffit: 0.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.318	B Brg Wid = 3.5 Min Req = 1.5 (Truss)
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.237	D Brg Wid = 1.5 Min Req = -
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes	Max Web CSI: 0.000	C Brg Wid = 1.5 Min Req = -
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)		Bearing B is a rigid surface.
	Loc. from endwall: not in 4.50 ft	Plate Type(s):		Members not listed have forces less than 375#
	GCpi: 0.18	WAVE	VIEW Ver: 21.02.01.1216.15	
	Wind Duration: 1.60			

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.



COA #0-278

Florida Certificate of Product Approval #FL 1999

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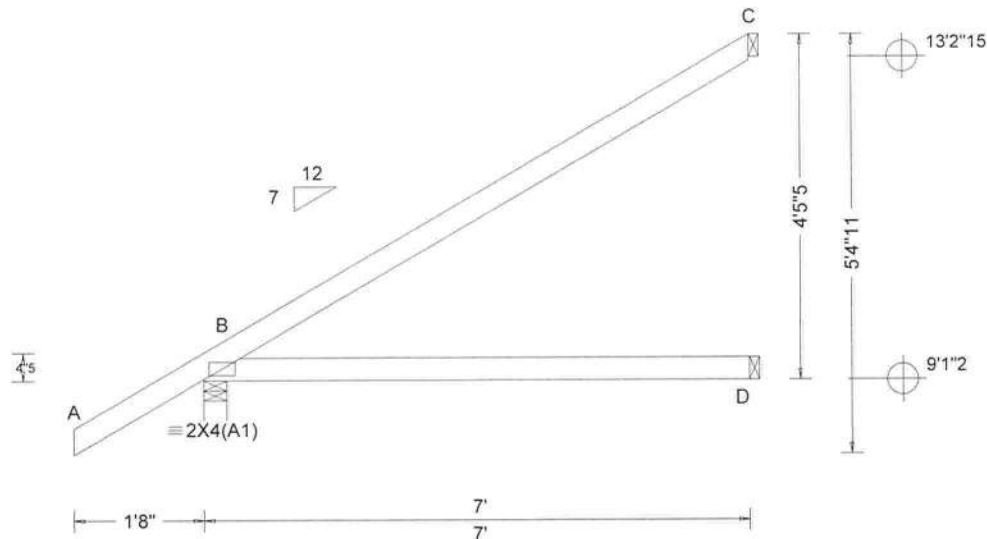
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 99997 FROM:	EJAC	Ply: 1 Qty: 26	Job Number: 21-6493 Jones Truss Label: J04	Cust: R 215 JRef: 1XGd2150007 T4 DrwNo: 164.22.1541.19047 KD / WHK 06/13/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.013 B - - HORZ(TL): 0.027 B - - Creep Factor: 2.0 Max TC CSI: 0.723 Max BC CSI: 0.519 Max Web CSI: 0.000 VIEW Ver: 21.02.01.1216.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 418 /- /- /291 /37 /171 D 130 /- /- /73 /- /- C 189 /- /- /123 /102 /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.



COA #0278

06/13/2022
Florida Certificate of Product Approval #FL 1999

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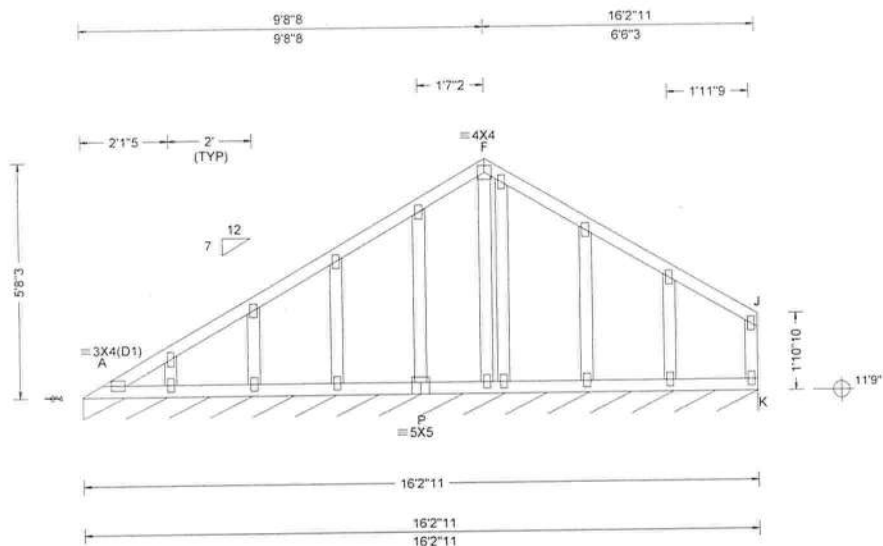
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155 Harlem Ave.
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100143 FROM:	GABL Qty: 1	Ply: 1 Job Number: 21-6493 Jones Truss Label: V01	Cust: R 215 JRef: 1XGd2150007 T33 DrwNo: 164.22.1541.16910 KD / WHK 06/13/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or * =PLF							
				Gravity				Non-Gravity			
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Loc	R+	/R-	/Rh	/Rw	/U	/RL	
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.001 A 999 240	K*	83	/-	/-	/45	/13	/8	
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.002 A 999 180	Wind reactions based on MWFRS							
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.002 J - -	K Brg Wid = 194 Min Req = -							
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.004 J - -	Bearing A is a rigid surface.							
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0	Members not listed have forces less than 375#							
Soffit: 0.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.048								
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.033								
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes	Max Web CSI: 0.052								
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)									
	Loc. from endwall: not in 4.50 ft	Plate Type(s):									
	GCpi: 0.18	WAVE									
	Wind Duration: 1.60		VIEW Ver: 21.02.01.1216.15								

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.



COA #0278

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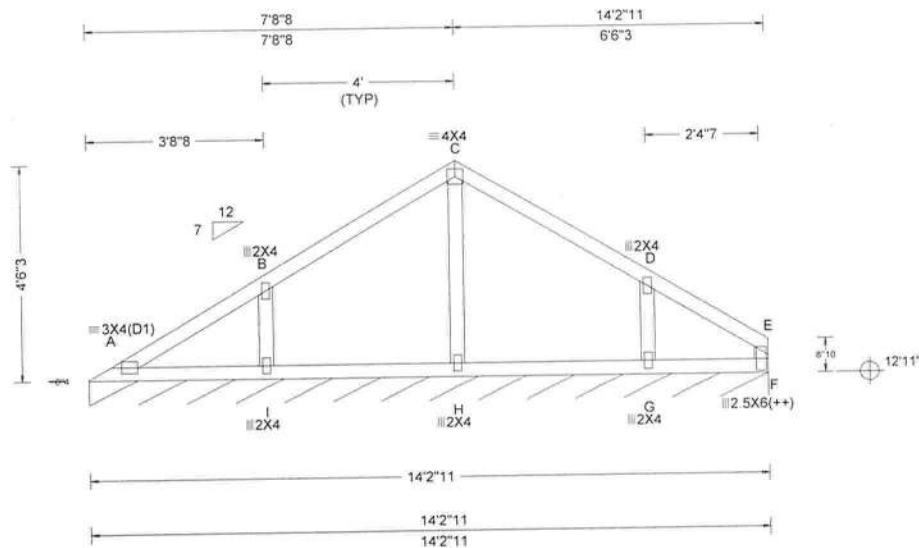
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100139	VAL	Ply: 1	Job Number: 21-6493	Cust: R 215 JRef: 1XGd2150007 T23
FROM:		Qty: 1	Jones	DrwNo: 164.22.1541.14900
			Truss Label: V02	KD / WHK 06/13/2022



Loading Criteria (psf)	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.33 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.004 A 999 240 VERT(CL): 0.008 A 999 180 HORZ(LL): 0.002 D - - HORZ(TL): 0.003 D - - Creep Factor: 2.0 Max TC CSI: 0.267 Max BC CSI: 0.130 Max Web CSI: 0.079 VIEW Ver: 21.02.01.1216.15	▲ Maximum Reactions (lbs), or *=PLF																					
				<table><tr><th colspan="3">Gravity</th><th colspan="3">Non-Gravity</th></tr><tr><th>Loc</th><th>R+</th><th>/R-</th><th>/Rh</th><th>/Rw</th><th>/U</th><th>/RL</th></tr><tr><td>F*</td><td>83</td><td>/-</td><td>/-</td><td>/43</td><td>/13</td><td>/8</td></tr></table> <p>Wind reactions based on MWFRS F Brg Wid = 170 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#</p>						Gravity			Non-Gravity			Loc	R+	/R-	/Rh	/Rw	/U	/RL	F*	83	/-
Gravity			Non-Gravity																						
Loc	R+	/R-	/Rh	/Rw	/U	/RL																			
F*	83	/-	/-	/43	/13	/8																			
TCLL: 20.00																									
TCDL: 10.00																									
BCLL: 0.00																									
BCDL: 10.00																									
Des Ld: 40.00																									
NCBCLL: 10.00																									
Soffit: 0.00																									
Load Duration: 1.25																									
Spacing: 24.0 "																									

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

(++) - This plate works for both joints covered.

Wind

Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN160118 and VAL180160118 for valley details.



COA #0278

Florida Certificate of Product Approval #FL 1999

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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

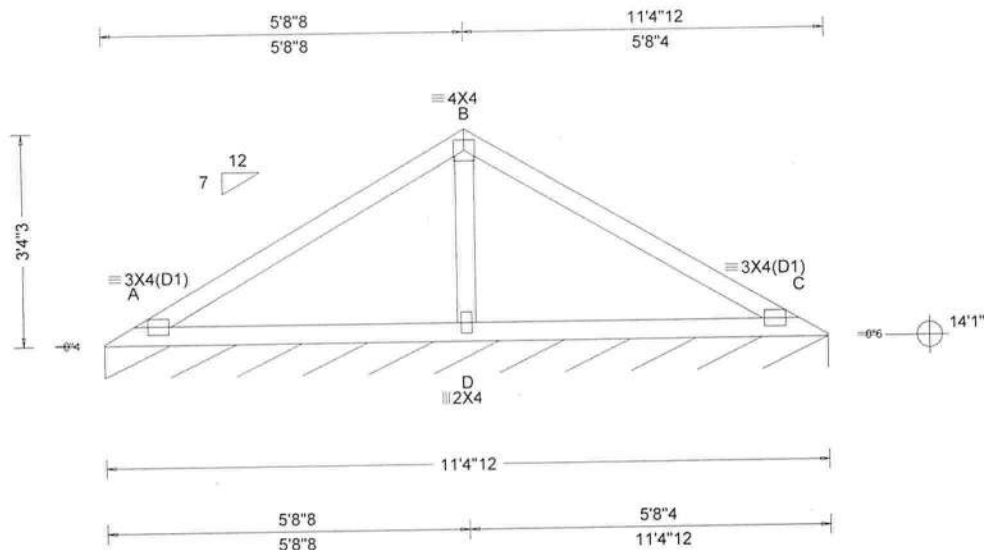
SEQN: 99958
FROM:

VAL

Ply: 1
Qty: 1

Job Number: 21-6493
Jones
Truss Label: V03

Cust: R 215 JRef: 1XGd2150007 T41
DrwNo: 164.22.1541.13887
KD / WHK 06/13/2022



Loading Criteria (psf)
TCLL: 20.00
TCDL: 10.00
BCLL: 0.00
BCDL: 10.00
Des Ld: 40.00
NCBCLL: 10.00
Soffit: 0.00
Load Duration: 1.25
Spacing: 24.0"

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.91 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h/2 to h
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg.Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT: 20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria
PP Deflection in loc L/defl L/#
VERT(LL): 0.020 A 999 240
VERT(CL): 0.041 A 999 180
HORZ(LL): -0.009 C - -
HORZ(TL): 0.018 C - -
Creep Factor: 2.0
Max TC CSI: 0.456
Max BC CSI: 0.381
Max Web CSI: 0.157

VIEW Ver: 21.02.01.1216.15

▲ Maximum Reactions (lbs), or *=PLF
Gravity Non-Gravity
Loc R+ /R- /Rh /Rw /U /RL
C* 83 /- /- /42 /11 /7
Wind reactions based on MWFRS
C Brg Wid = 136 Min Req = -
Bearing A is a rigid surface.
Members not listed have forces less than 375#
Maximum Top Chord Forces Per Ply (lbs)
Chords Tens.Comp. Chords Tens. Comp.
A - B 440 - 189 B - C 440 - 189

Maximum Web Forces Per Ply (lbs)
Webs Tens.Comp.
B - D 343 - 655

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN160118 and VAL180160118 for valley details.



COA #06218

06/13/2022
Florida Certificate of Product Approval #FL 1999

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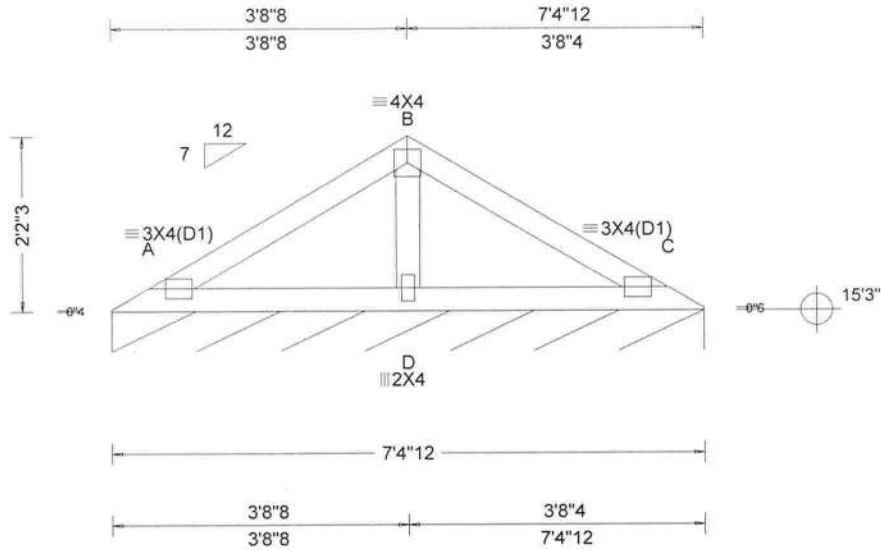
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 99937 FROM:	VAL	Ply: 1 Qty: 1	Job Number: 21-6493 Jones Truss Label: V04	Cust: R 215 JRef: 1XGd2150007 T39 DrwNo: 164.22.1541.12687 KD / WHK 06/13/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCDL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 16.50 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.005 A 999 240 VERT(CL): 0.011 A 999 180 HORZ(LL): -0.002 C - - HORZ(TL): 0.005 C - - Creep Factor: 2.0 Max TC CSI: 0.163 Max BC CSI: 0.145 Max Web CSI: 0.066 VIEW Ver: 21.02.01.1216.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL C* 83 /- /- /41 /10 /6 Wind reactions based on MWFRS C Brg Wid = 88.8 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN160118 and VAL180160118 for valley details.



COA #092028

Florida Certificate of Product Approval #FL 1999

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!

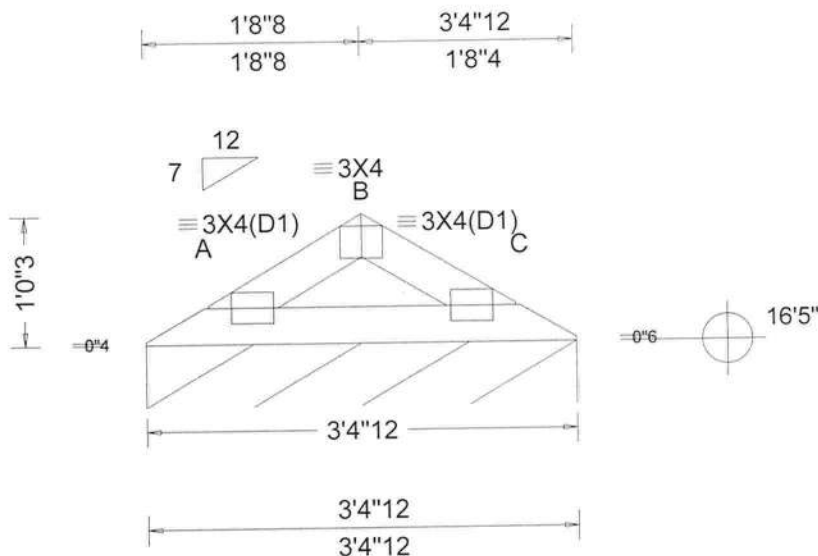
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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025



Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 17.08 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.002 A 999 240 VERT(CL): 0.004 A 999 180 HORZ(LL): -0.001 A - - HORZ(TL): 0.002 A - - Creep Factor: 2.0 Max TC CSI: 0.053 Max BC CSI: 0.073 Max Web CSI: 0.000	▲ Maximum Reactions (lbs), or * = PLF <table><tr><th colspan="3">Gravity</th><th colspan="3">Non-Gravity</th></tr><tr><th>Loc</th><th>R+ / R-</th><th>/ Rh</th><th>/ Rw</th><th>/ U</th><th>/ RL</th></tr><tr><td>C* 82</td><td>/-</td><td>/-</td><td>/37</td><td>/5</td><td>/5</td></tr></table> Wind reactions based on MWFRS C Brg Wid = 40.8 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#	Gravity			Non-Gravity			Loc	R+ / R-	/ Rh	/ Rw	/ U	/ RL	C* 82	/-	/-	/37	/5	/5
		Gravity			Non-Gravity																	
		Loc	R+ / R-	/ Rh	/ Rw	/ U	/ RL															
C* 82	/-	/-	/37	/5	/5																	
Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	VIEW Ver: 21.02.01.1216.15																					

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN160118 and VAL180160118 for valley details.



COA #0278

Florida Certificate of Product Approval #FL 1999

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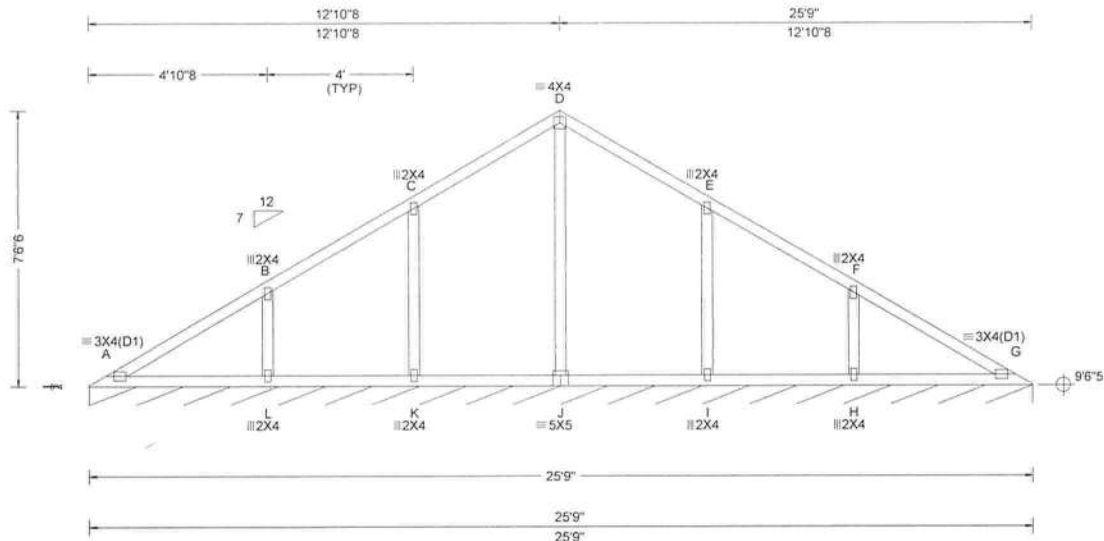
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 99934	VAL	Ply: 1	Job Number: 21-6493	Cust: R 215 JRef: 1XGd2150007 T28
FROM:		Qty: 1	Jones	DrwNo: 164.22.1541.05540
			Truss Label: V06	KD / WHK 06/13/2022



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity Non-Gravity
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.011 A 999 240	Loc R+ / R- / Rh / Rw / U / RL
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.023 A 999 180	G* 83 /- /- /43 /13 /7
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.004 A - -	Wind reactions based on MWFRS
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.009 A - -	G Brg Wid = 308 Min Req = -
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0	Bearing A is a rigid surface.
Soffit: 0.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.280	Members not listed have forces less than 375#
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.196	
Spacing: 24.0 "	MWFRS Parallel Dist: h/2 to h	Rep Fac: Yes	Max Web CSI: 0.290	
	C&C Dist a: 3.00 ft	FT/RT: 20(0)/10(0)		
	Loc. from endwall: not in 9.00 ft	Plate Type(s):		
	GCpi: 0.18			
	Wind Duration: 1.60	WAVE	VIEW Ver: 21.02.01.1216.15	

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN160118 and VAL180160118 for valley details.



COA #0678

06/13/2022 Florida Certificate of Product Approval #FL 1999

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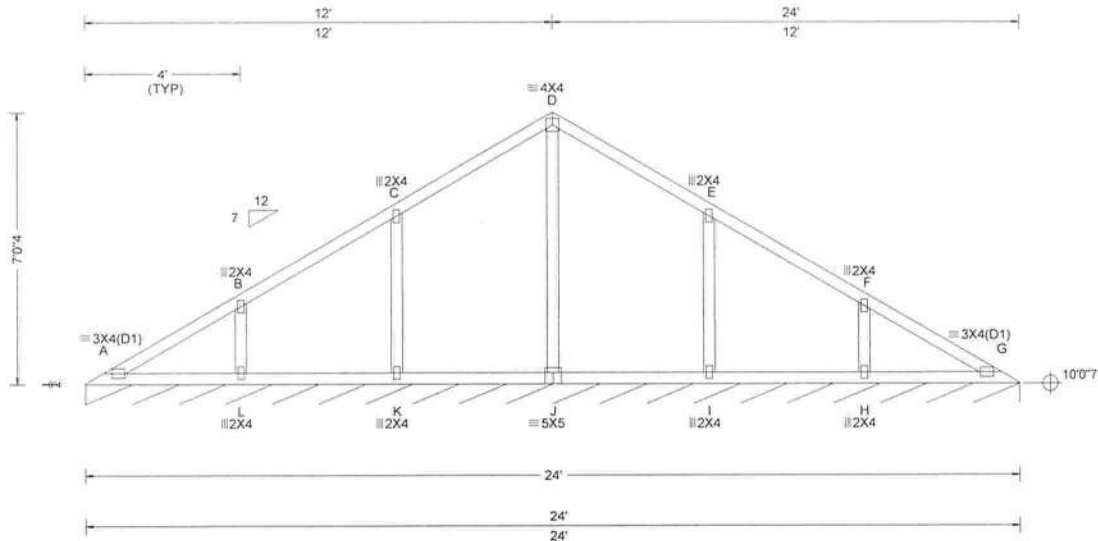
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155 Harlem Ave.
North Building, 4th Floor
Glenview, IL 60025

SEQN: 99956 FROM:	VAL	Ply: 1 Qty: 1	Job Number: 21-6493 Jones Truss Label: V07	Cust: R 215 JRef: 1XGd2150007 T31 DrwNo: 164.22.1540.59937 KD / WHK 06/13/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or * = PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.006 A 999 240 VERT(CL): 0.012 A 999 180 HORZ(LL): 0.002 A - - HORZ(TL): 0.004 A - - Creep Factor: 2.0 Max TC CSI: 0.206 Max BC CSI: 0.146 Max Web CSI: 0.209 VIEW Ver: 21.02.01.1216.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL G* 83 /- /- /43 /13 /7 Wind reactions based on MWFRS G Brg Wid = 287 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN160118 and VAL180160118 for valley details.



COA #0028

Florida Certificate of Product Approval #FL 1999

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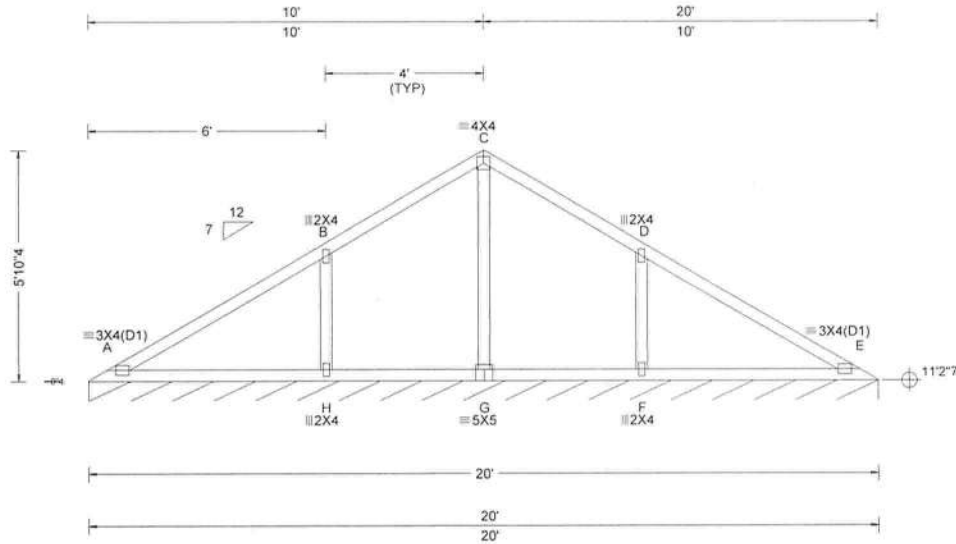
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 99905 FROM:	VAL	Ply: 1 Qty: 1	Job Number: 21-6493 Jones Truss Label: V08	Cust: R 215 JRef: 1XGd2150007 T10 DrwNo: 164.22.1540.57997 KD / WHK 06/13/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.020 E 999 240 VERT(CL): 0.041 E 999 180 HORZ(LL): -0.008 E - - HORZ(TL): 0.016 E - - Creep Factor: 2.0 Max TC CSI: 0.487 Max BC CSI: 0.275 Max Web CSI: 0.231 VIEW Ver: 21.02.01.1216.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL E* 83 /- /- /43 /12 /7 Wind reactions based on MWFRS E Brg Wid = 239 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375# Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. C - G 15 -387

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN160118 and VAL180160118 for valley details.



COA #0278

06/13/2022
Florida Certificate of Product Approval #FL 1999

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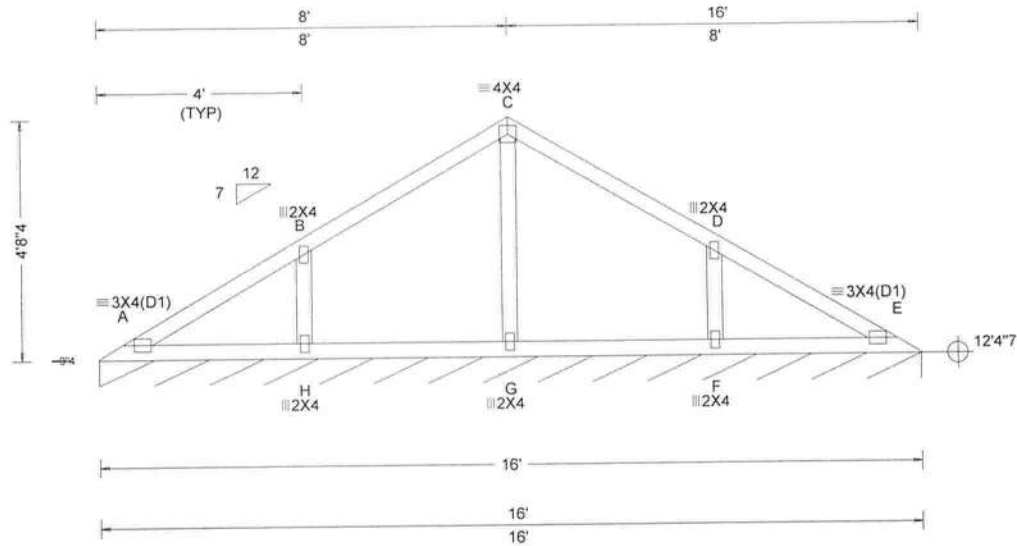
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or * = PLF					
				Gravity			Non-Gravity		
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Loc	R+	/ R-	/ Rh	/ Rw	/ U / RL
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.005 E 999 240	E*	83	/-	/-	/43	/0 /7
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.010 E 999 180	Wind reactions based on MWFRS					
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.002 E - -	E Brg Wid = 191 Min Req = -					
Des Ld: 40.00	EXP: C Kzt: NA	Building Code:	HORZ(TL): 0.004 E - -	Bearing A is a rigid surface.					
NCBCLL: 10.00	Mean Height: 15.00 ft	FBC 7th Ed. 2020 Res.	Creep Factor: 2.0	Members not listed have forces less than 375#					
Soffit: 0.00	TCDL: 5.0 psf	TPI Std: 2014	Max TC CSI: 0.297						
Load Duration: 1.25	BCDL: 5.0 psf	Rep Fac: Yes	Max BC CSI: 0.142						
Spacing: 24.0 "	MWFRS Parallel Dist: h to 2h	FT/RT:20(0)/10(0)	Max Web CSI: 0.098						
	C&C Dist a: 3.00 ft	Plate Type(s):	VIEW Ver: 21.02.01.1216.15						
	Loc. from endwall: not in 9.00 ft	WAVE							
	GCpi: 0.18								
	Wind Duration: 1.60								

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN160118 and VAL180160118 for valley details.



COA #0228

Florida Certificate of Product Approval #FL 1999

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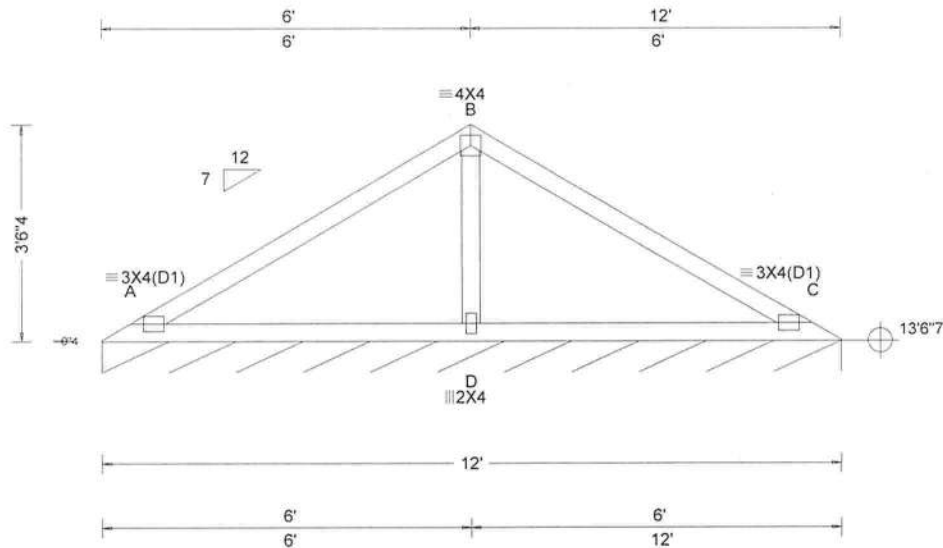
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 99961	VAL	Ply: 1	Job Number: 21-6493	Cust: R 215 JRef: 1XGd2150007 T32
FROM:		Qty: 1	Jones	DrwNo: 164.22.1540.44690
			Truss Label: V10	KD / WHK 06/13/2022



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity Non-Gravity
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.023 A 999 240	Loc R+ / R- / Rh / Rw / U / RL
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.047 A 999 180	C* 83 /- /- /42 /0 /7
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.010 C - -	Wind reactions based on MWFRS
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.022 C - -	C Brg Wid = 143 Min Req = -
NCBCLL: 10.00	Mean Height: 15.45 ft	Building Code:	Creep Factor: 2.0	Bearing A is a rigid surface.
Soffit: 0.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.513	Members not listed have forces less than 375#
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.424	Maximum Top Chord Forces Per Ply (lbs)
Spacing: 24.0"	MWFRS Parallel Dist: h to 2h	Rep Fac: Yes	Max Web CSI: 0.182	Chords Tens.Comp. Chords Tens. Comp.
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)	VIEW Ver: 21.02.01.1216.15	A - B 494 -206 B - C 494 -206
	Loc. from endwall: not in 9.00 ft	Plate Type(s):		Maximum Web Forces Per Ply (lbs)
	GCpi: 0.18	WAVE		Webs Tens.Comp.
	Wind Duration: 1.60			B - D 359 -721

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN160118 and VAL180160118 for valley details.



COA #09238

Florida Certificate of Product Approval #FL 1999

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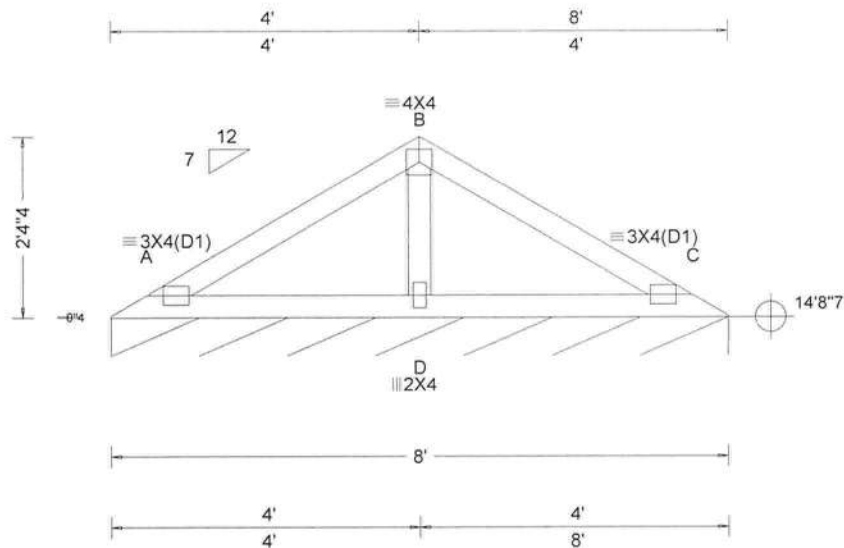
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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCEA: sbcecomponents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 99911	VAL	Ply: 1	Job Number: 21-6493	Cust: R 215 JRef: 1XGd2150007 T24
FROM:		Qty: 1	Jones	DrwNo: 164.22.1540.42600
			Truss Label: V11	KD / WHK 06/13/2022



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or * = PLF
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity Non-Gravity
TCCL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.007 A 999 240	Loc R+ / R- / Rh / Rw / U / RL
BCCL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.014 A 999 180	C* 83 /- /- /41 /- /7
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.003 C - -	Wind reactions based on MWFRS
Des Ld: 40.00	EXP: C Kzt: NA	Building Code:	HORZ(TL): 0.006 C - -	C Brg Wid = 96.0 Min Req = -
NCBCLL: 10.00	Mean Height: 16.03 ft	FBC 7th Ed. 2020 Res.	Creep Factor: 2.0	Bearing A is a rigid surface.
Soffit: 0.00	TCCL: 5.0 psf	TPI Std: 2014	Max TC CSI: 0.198	Members not listed have forces less than 375#
Load Duration: 1.25	BCDL: 5.0 psf	Rep Fac: Yes	Max BC CSI: 0.173	
Spacing: 24.0 "	MWFRS Parallel Dist: h to 2h	FT/RT: 20(0)/10(0)	Max Web CSI: 0.073	
	C&C Dist a: 3.00 ft	Plate Type(s):	VIEW Ver: 21.02.01.1216.15	
	Loc. from endwall: not in 9.00 ft	WAVE		
	GCpi: 0.18			
	Wind Duration: 1.60			

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN160118 and VAL180160118 for valley details.



COA #0028

Florida Certificate of Product Approval #FL 1999

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!

****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

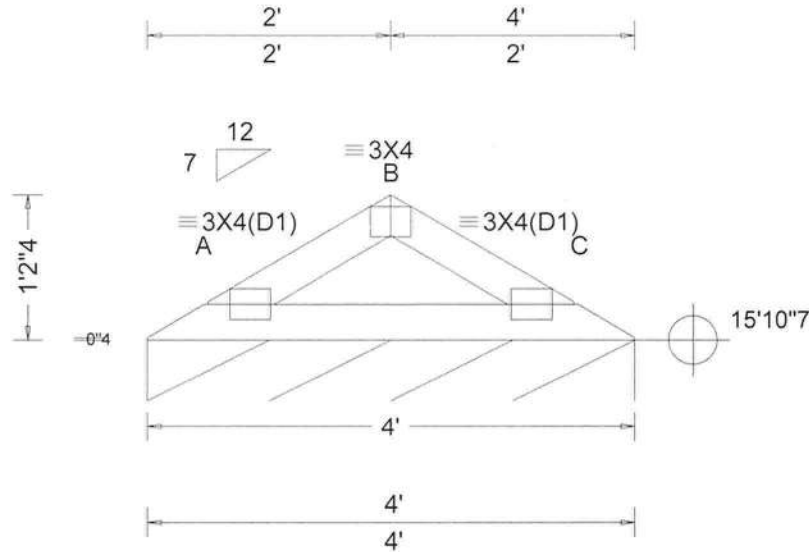
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbacompnents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 99945 FROM:	VAL	Ply: 1 Qty: 1	Job Number: 21-6493 Jones Truss Label: V12	Cust: R 215 JRef: 1XGd2150007 T30 DrwNo: 164.22.1540.40170 KD / WHK 06/13/2022
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Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)		Defl/CSI Criteria		▲ Maximum Reactions (lbs), or *=PLF	
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA	Ct: NA	CAT: NA	PP Deflection in loc L/defl L/#	VERT(LL): 0.003 C 999 240	Gravity	Loc R+	/ R-
TCDL: 10.00	Speed: 130 mph	Pf: NA	Ce: NA		VERT(CL): 0.007 C 999 180	HORZ(LL): -0.001 A - -	Non-Gravity	/ Rh	/ Rw
BCLL: 0.00	Enclosure: Closed	Lu: NA	Cs: NA		HORZ(TL): 0.003 A - -	Creep Factor: 2.0		/ U	/ RL
BCDL: 10.00	Risk Category: II	Snow Duration: NA			Max TC CSI: 0.075				
Des Ld: 40.00	EXP: C Kzt: NA	Building Code:			Max BC CSI: 0.098				
NCBCLL: 10.00	Mean Height: 16.62 ft	FBC 7th Ed. 2020 Res.			Max Web CSI: 0.000				
Soffit: 0.00	TCDL: 5.0 psf	TPI Std: 2014			VIEW Ver: 21.02.01.1216.15				
Load Duration: 1.25	BCDL: 5.0 psf	Rep Fac: Yes							
Spacing: 24.0 "	MWFRS Parallel Dist: h to 2h	FT/RT:20(0)/10(0)							
	C&C Dist a: 3.00 ft	Plate Type(s):							
	Loc. from endwall: not in 9.00 ft	WAVE							
	GCpi: 0.18								
	Wind Duration: 1.60								

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN160118 and VAL180160118 for valley details.



COA #06278

06/13/2022 Florida Certificate of Product Approval #FL 1999

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!
****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbacompcomponents.com; ICC: iccsafe.org; AWC: awc.org



155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

Valley Detail - ASCE 7-16: 180 mph, 30' Mean Height, Partially Enclosed, Exp. C, Kzt=1.00

Top Chord 2x4 SP #2N, SPF #1/#2, DF-L #2 or better,
Bot Chord 2x4 SP #2N or SPF #1/#2 or better,
Webs 2x4 SP #3, SPF #1/#2, DF-L #2 or better,

**** Attach each valley to every supporting truss with:
535# connection or with (1) Simpson H2.5A or
equivalent connector for**

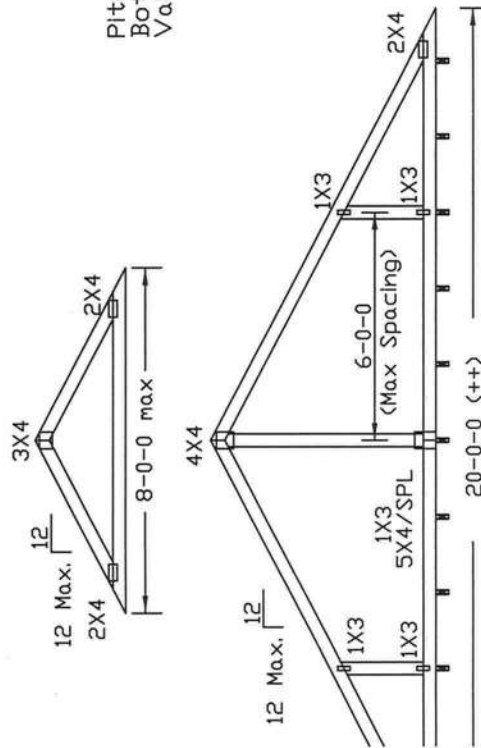
ASCE 7-16 180 mph. 30' Mean Height, Part. Enc. Building, Exp. C, Wind TC DL=5 psf, Kzt = 1.00

Dr ASCE 7-16 160 mph. 30' Mean Height, Part. Enc.
Building, Exp. D, Wind TC DL=5 psf, Kzt = 1.00

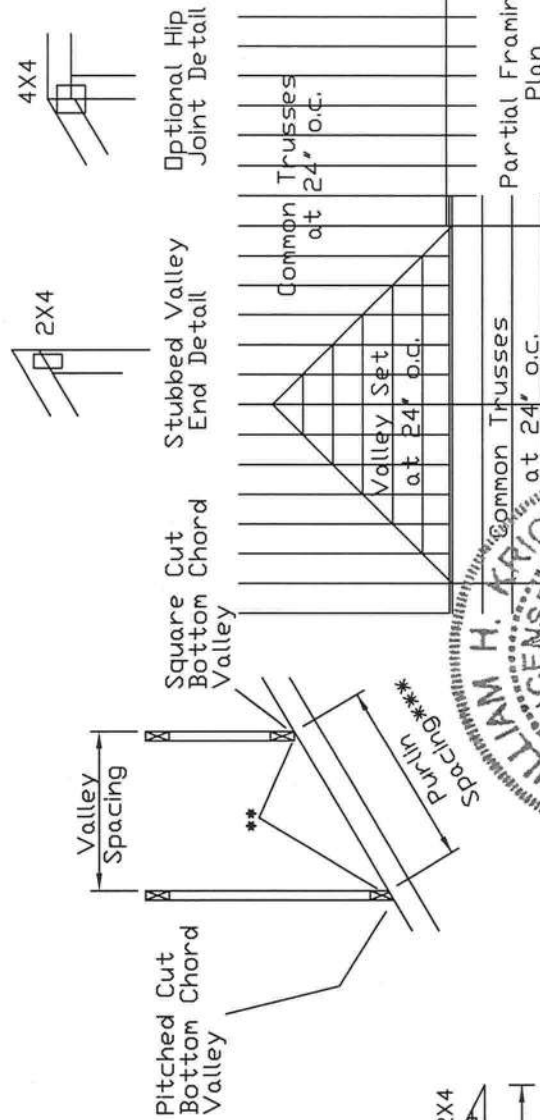
Bottom chord may be square or pitched cut as shown.

Valleys short enough to be cut as solid triangular members from a single 2x6, or larger as required, shall be permitted in lieu of fabricating from separate 2x4 members.

All plates shown are Alpine Wave Plates.



Supporting trusses at 24' o.c. maximum spacing.



Unless specified otherwise on engineer's sealed design, for vertical valley webs taller than 7'-9" apply 2x4 'T' reinforcement, 80% length of web, same species and grade or better, attached with 10d box (0.128" x 3.0") nails at 6" o.c. In lieu of 'T' reinforcement, 2x4 Continuous Lateral Restraint applied at mid-length of web is permitted with diagonal bracing as shown in DRWG BRCLBANC1014.

Top chord of truss beneath valley set must be braced with properly attached, rated sheathing applied prior to valley truss installation.

By valley trusses used in lieu of purlin spacing as specified on Engineer's sealed design.

*** Note that the purlin spacing for bracing the top chord of the truss beneath the valley is measured along the slope of the top chord.

++ Larger spans may be built as long as the vertical height does not exceed 14'-0".

IG-LL	30	30	40PSF	REF	VALLEY DETAIL
TC DL	20	15	7 PSF	DATE	01/26/2018
BC DL	10	10	10 PSF	DRWG	VAL180160118
BC LL	0	0	0 PSF		
TOT. L.D.	60	55	57PSF		
DUR.FAC.	1.25/1.33	1.15	1.15		
SPACING	24.0"	24.0"	24.0"		

IMPORTANT! READ AND FOLLOW ALL NOTES ON THIS DRAWING.
IMPORTANT! FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the latest edition of BECI (Building Component Safety) by TPI and SBCA for safety practices prior to performing these functions. Installers shall provide temporary bracing pending the erection of permanent bracing. Trusses shall be braced in accordance with the following instructions. Trusses shall have bracing installed per BECI sections B3, B7 or B10, as applicable. Apply plates to each BECI brace truss and position as shown above and on the Joint Details, unless noted otherwise.

Alpine, a division of ITV Building Components Group Inc. shall not be responsible for any deviation from the drawings or specifications of the Truss in conformance with ANSI/TPI 1, or for handling, shipping, installation or bracing of trusses.

A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see this job's general notes page and these web sites:
ALPINE: www.alpinecorp.com/TPI www.tips4steel.org www.sbsccomponents.com www.alpsccorp.org



155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

Gable Stud Reinforcement Detail

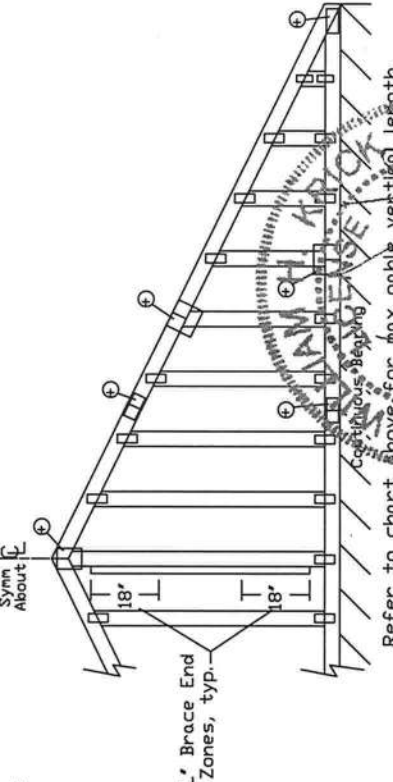
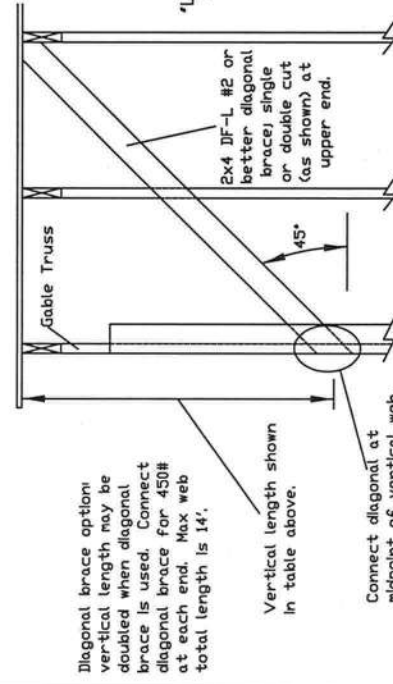
ASCE 7-16: 140 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or 120 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00

Or 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00

Or 100 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

Max Gable Vertical Length	Gable Vertical Spacing	2x4 Species	Brace Grade	No Braces	(1) 1x4 'L' Brace				(2) 2x4 'L' Brace				(1) 2x6 'L' Brace				(2) 2x6 'L' Brace			
					Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
12" o.c.	SPF	#1 / #2	Standard	4' 3"	7' 3"	7' 7"	8' 7"	8' 11"	10' 3"	10' 8"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	HF	#3	Standard	4' 1"	6' 7"	7' 1"	8' 6"	8' 10"	10' 1"	10' 6"	13' 4"	13' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	SP	#1	Standard	4' 1"	5' 8"	6' 0"	7' 7"	8' 1"	10' 1"	10' 6"	11' 10"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	DFL	#2	Standard	4' 3"	7' 3"	7' 7"	8' 7"	8' 11"	10' 3"	10' 8"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
16" o.c.	SPF	#1 / #2	Standard	4' 2"	6' 0"	6' 4"	7' 11"	8' 6"	10' 2"	10' 7"	12' 5"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	HF	#3	Standard	4' 11"	8' 4"	8' 8"	9' 10"	10' 3"	11' 8"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	SP	#1	Standard	4' 8"	8' 1"	8' 5"	9' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	DFL	#2	Standard	5' 1"	8' 5"	8' 9"	9' 11"	10' 4"	11' 10"	12' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
12" o.c.	SPF	#1 / #2	Standard	5' 1"	9' 0"	9' 4"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	HF	#3	Standard	5' 8"	9' 3"	9' 7"	10' 11"	11' 4"	13' 0"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	SP	#1	Standard	5' 3"	9' 2"	9' 6"	10' 10"	11' 3"	12' 11"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	DFL	#2	Standard	5' 3"	8' 5"	8' 9"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"



155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

For more information see this job's general notes page and these web sites:
ALPINE: www.alpineitw.com TPI: www.tpiinc.org SBCA: www.sbcacomponents.com ICC: www.iccsafe.org

IMPORTANT: READ AND FOLLOW ALL NOTES ON THIS DRAWING

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the following notes and details for proper installation and bracing. Trusses shall be installed in accordance with the manufacturer's instructions and the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see this job's general notes page and these web sites:
ALPINE: www.alpineitw.com TPI: www.tpiinc.org SBCA: www.sbcacomponents.com ICC: www.iccsafe.org

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0'

Bracing Group Species and Grades:			
Group A:		Group B:	
Spruce-Pine-Fir	#1 / #2	Douglas Fir-Larch	#3
	Standard		Standard
Douglas Fir-Larch	#3	Southern Pine	#3
	Standard		Standard

1x4 Braces shall be SRB (Stress-Rated Board).
For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:
Wind Load deflection criterion is L/240.
Provide uplift connections for 55 plf over continuous bearing (5 psf 1C Dead Load).
Gable end supports load from 4' 0" outliners with 2' 0" overhang, or 12' plywood overhang.

Attach 'L' braces with 10d (0.128"x3.0" min) nails.
For (1) 'L' brace: space nails at 2' o.c. in 18" end zones and 4' o.c. between zones.
For (2) 'L' braces: space nails at 3' o.c. in 18" end zones and 6' o.c. between zones.
'L' bracing must be a minimum of 80% of web member length.

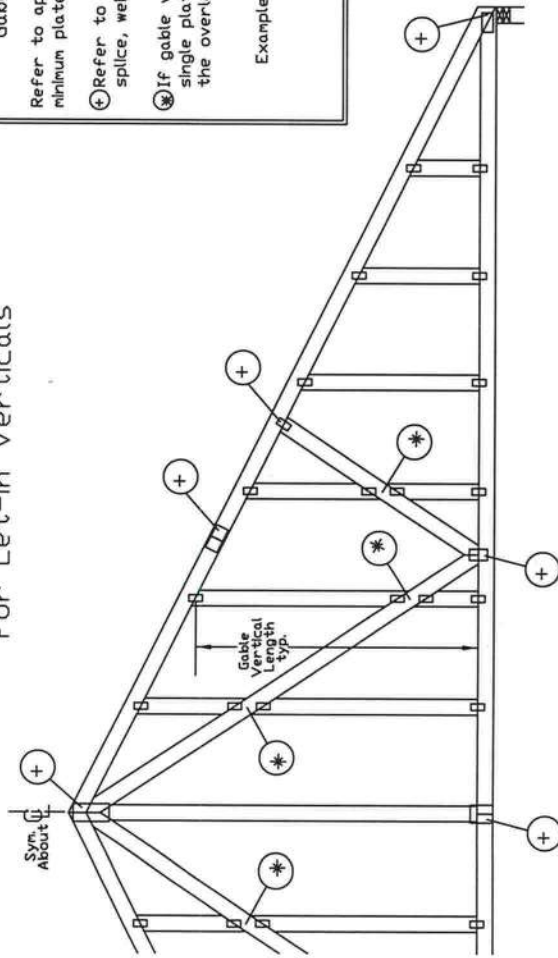
Gable Vertical Plate Sizes	
Vertical Length	No Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4' 0"	3X4

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.

REF	ASCE7-16-GABI4015
DATE	01/26/2018
DRWG	A14015ENC160118

Gable Detail For Let-In Verticals



Gable Truss Plate Sizes

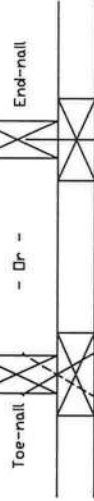
Refer to appropriate Alpine gable detail for minimum plate sizes for vertical studs.

⊕ Refer to Engineered truss design for peak, splice, web, and heel plates.

⊗ If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.



'T' Reinforcement Attachment Detail



To convert from 'L' to 'T' reinforcing members, multiply 'T' increase by length (based on appropriate Alpine gable detail).

Maximum allowable 'T' reinforced gable vertical length is 14' from top to bottom chord.

'T' reinforcing member material must match size, species, and grade of the 'L' reinforcing member.

'T' Reinf. Mbr. Size	'T' Increase
2x4	30 %
2x6	20 %

Web Length Increase w/ 'T' Brace

Example:
ASCE 7-10 Wind Speed = 120 mph
Mean Roof Height = 30 ft, Kzt = 1.00
Gable Vertical = 24' o.c. SP #3
'T' Reinforcing Member Size = 2x4
'T' Brace Increase (From Above) = 30% = 1.30
(1) 2x4 'L' Brace Length = 8' 7"
Maximum 'T' Reinforced Gable Vertical Length
1.30 x 8' 7" = 11' 2"

Provide connections for uplift specified on the engineered truss design.

Attach each 'T' reinforcing member with

End Driven Nails:

10d Common (0.148"x 3", min) Nails at 4' o.c. plus

(4) nails in the top and bottom chords.

Toenailed Nails:

10d Common (0.148"x 3", min) Toenails at 4' o.c. plus

(4) toenails in the top and bottom chords.

This detail to be used with the appropriate Alpine gable detail for ASCE

wind load.

ASCE 7-05 Gable Detail Drawings

A13015051014, A12015051014, A1015051014, A10015051014, A14015051014,

A13030051014, A12030051014, A1030051014, A10030051014, A14030051014

ASCE 7-10 & ASCE 7-16 Gable Detail Drawings

A11515ENC100118, A12015ENC100118, A14015ENC100118, A16015ENC100118,

A18015ENC100118, A20015ENC100118, A20015PEDI00118, A20015PEDI00118,

A11530ENC100118, A12030ENC100118, A14030ENC100118, A16030ENC100118,

A18030ENC100118, A20030ENC100118, A20030PEDI00118, A20030PEDI00118,

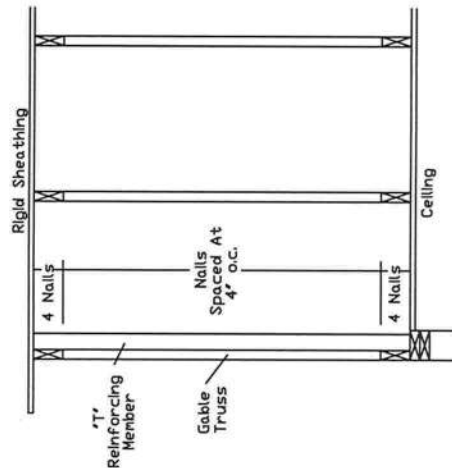
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S18015ENC100118, S20015ENC100118, S20015PEDI00118, S20015PEDI00118,

S11530ENC100118, S12030ENC100118, S14030ENC100118, S16030ENC100118,

S18030ENC100118, S20030ENC100118, S20030PEDI00118, S20030PEDI00118

See appropriate Alpine gable detail for maximum gable vertical length.

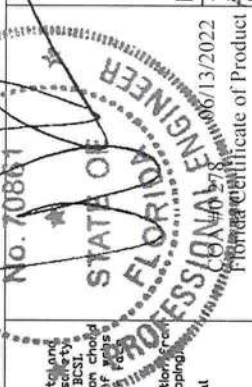


IMPORTANT: READ AND FOLLOW ALL NOTES ON THIS DRAWING

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the latest edition of ECSI Building Component Safety Information, by TPI and SBCA for safety practices prior to performing these functions. Installers shall provide temporary bracing per ECSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have bracing installed per ECSI. All trusses shall be braced in accordance with ECSI. Refer to drawings 160A-Z for standard plate positions.

Alpine, a division of ITV Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.

A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineer. For more information see this job's technical notes page and these web sites: ALPINE: www.alpineitv.com TPI: www.tpi.org SBCA: www.sbcacomponents.com ICD: www.icdse.org



REF	LET-IN VERT
DATE	01/02/2018
DRWG	GBLLETIN0118

MAX. TOT. LD.	60 PSF
DUR. FAC.	ANY
Approval #	1999
MAX. SPACING	24.0'

CLR Reinforcing

Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforcement or scab reinforcement.

Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

Use scabs instead of L- or T- reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

Web Member Size	Specified CLR Restraint	Alternative Reinforcement T- or L- Reinf.	Scab Reinf.
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4(*)
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6(*)

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

(*) Center scab on wide face of web. Apply (1) scab to each face of web.

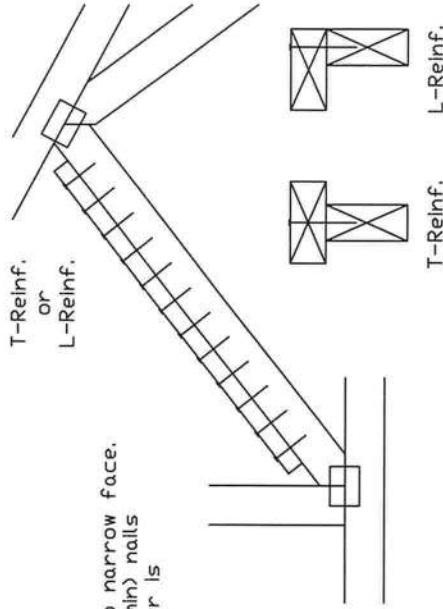
T-Reinforcement

or

L-Reinforcement:

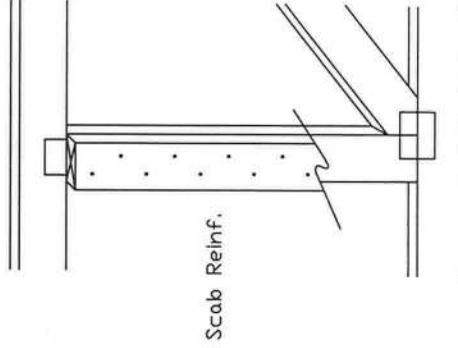
Apply to either side of web narrow face.


Attach with 10d (0.128"x3.0", min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



Scab Reinforcement:

Apply scab(s) to wide face of web. No more than (1) scab per face. Attach with 10d (0.128"x3.0", min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.





155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

IMPORTANT: READ AND FOLLOW ALL NOTES ON THIS DRAWING

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI Building Component Safety Information, by TPI and SBCA for safety instructions and details. Truss designers shall provide temporary bracing per BCSI Building Component Safety Information. Truss installers shall provide permanent bracing per BCSI Building Component Safety Information. Truss designers shall provide locations for permanent lateral restraint of webs. Truss installers shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installing or bracing of trusses.

A seal on this drawing indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see this job's general notes page and these web sites:
ALPINE: www.alpineitw.com TPI: www.tpihst.org SBCA: www.sbcacomponents.com ICD: www.icdcsafe.org

WILLIAM H. KRICK

Professional Engineer

FLORIDA

NO. 70861

06/13/2022

Florida Certificate of Product Approval

06/13/2022

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BC LL

TOT. LD.

DUR. FAC.

REF CLR Subst.

DATE 01/02/19

DRWG BRCLBSUB0119

JOB #: 21-6493

