	4/2006			Building Pe		PERMIT
APPLICANT	EDWARD	This Per RIOPELLE	mit Expires One Y	ear From the Date o	813-672-3894	000024974
ADDRESS	12204	GLENHILL DR		— RIVERVIEW		FL 33569
OWNER		& SUE RIOPELLE		PHONE	813-672-3403	
ADDRESS	807	SW QUARRY CIF		— FORT WHITE		FL 32038
CONTRACTO		NER BUILDER		PHONE		
LOCATION C			27. L 18. R ELLIS. R OU	JARRY, FIRST ROAD MA	AKE A	
		-	FOLLOW ROAD TO LO			
TYPE DEVEL	OPMENT	SFD,UTILITY	ES	STIMATED COST OF CO	NSTRUCTION	305700.00
HEATED FLO	OOR AREA	6114.00	TOTAL AR	REA 7477.00	HEIGHT 23	.40 STORIES 2
FOUNDATIO	N CONC	CRETE WA	LLS FRAMED	ROOF PITCH $6/12$	FLC	OOR SLAB
LAND USE &	ZONING	AG-3		MAX	T. HEIGHT 35	5
Minimum Set	Back Requir	rments: STREE	Γ-FRONT 30.00) REAR	25.00	SIDE <u>25.00</u>
NO. EX.D.U.	0	_ FLOOD ZONE	<u> </u>	DEVELOPMENT PERI	MIT NO.	
PARCEL ID	02-7S-16-	04111-110	SUBDIVISIO	ON LITTLE PINE FAM	MS	
LOT <u>10</u>	BLOCK	PHASE	UNIT	тота	AL ACRES 10.0	00
Culvert Permit EXISTNG Driveway Conr	nection	06-0031-N Septic Tank Number		<u>J</u>	Applicant/Owner/C	<u>N</u>
COMMENTS:		NE FOOT ABOVE	THE ROAD			
DISCLOSURE		NE FOOT ABOVE T	THE ROAD			
		***************************************	THE ROAD		Check # or Ca	sh *** 530
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DISCLOSURE	STATEME	NT INCLUDED				
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NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

For Office Use Only Application # 660721 Date Received 7/11/06 By 67 Permit # 24974
Application Approved by - Zoning Official 614 Date 714 De Plans Examiner 714 De Date 9-14-06
Flood Zone Development Permit _ D/A _ Zoning _A-3 Land Use Plan Map Category _A-3
Comments
Applicants Name Edward & Sue Riopelle Phone 813-672-3894
Address 12204 Glenhill De. Riverview 71, 33569
Owners Name Edward & Sue Riopelle Phone 813-672-3403
911 Address 807 5. W. Quarry Circle Fort White 32038
Contractors Name Edward Sue Respelle Phone 813-672-3403
Address 12204 Glenhill De Rwerview 71, 33569
Fee Simple Owner Name & Address
Bonding Co. Name & Address
Architect/Engineer Name & Address Demco Drafting & Design 38452 6th Av. Zephynihi
Mortgage Lenders Name & Address
Circle the correct power company - FL Power & Light - Clay Elec Suwannee Valley Elec Progressive Energy
Property ID Number 62-75-16-04111-110 Estimated Cost of Construction 160,000,00
Subdivision Name Little Pine Farms Hawthornelot / D Block Unit Phase
Driving Directions 441 to 127 To 18 to requercy Rd Right turn their
first Road make a Right Follow Rd to lot 10 2nd left
Type of Construction Bus lding house SCD Number of Existing Dwellings on Property D
Total Acreage $\frac{9.74}{1}$ Lot Size Do you need a - <u>Culvert Permit</u> or <u>Culvert Waiver</u> or <u>Have an Existing Driv</u>
Actual Distance of Structure from Property Lines - Front 125 Ff Side 50 Ff Side 50 FF Rear 40 FF
Total Building Height 23'4" Number of Stories 2 Heated Floor Area 6/14 Roof Pitch 67
Total 7477
Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.
OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.
WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.
Owner Builder or Agent (Including Contractor) Contractor Signature
LAURE HODE APPRIESON LICENSE Number
STATE OF FLORIDA COUNTY OF COLUMBIA COUNTY OF COLUMBIA LAURE HOUSE, WAY COMMISSION # 109 98899 et ancy Card Number EXPIRES: June 2009 AR STAMP/SEAL Bonded Thru Notary Public Underwillers
Sworn to (or affirmed) and subscribed before the source of the subscribed before the sub
this 14 day of 9 2006. Lai H
Porconally known or Produced Identification Notary Signature

Columbia County Property Appraiser

DB Last Updated: 8/1/2006

2006 Proposed Values

Parcel: 02-7S-16-04111-110

Tax Record I

Property Card

Interactive GIS Map

Search Result: 1 of 1

Print

Owner & Property Info

Owner's Name	RIOPELLE EDWARD				
Site Address	LITTLE PINE FARMS SD				
Mailing Address	12204 GLENHILL DR RIVERVIEW, FL 33569				
Description	LOT 10 LITTLE PINE FARMS S/D. ORB 858-480,				

Use Desc. (code)	MISC RES (000700)
Neighborhood	2716.01
Tax District	3
UD Codes	MKTA02
Market Area	02
Total Land Area	9.140 ACRES

Property & Assessment Values

Mkt Land Value	cnt: (2)	\$25,767.00
Ag Land Value	cnt: (0)	\$0.00
Building Value	cnt: (0)	\$0.00
XFOB Value	cnt: (1)	\$950.00
Total Appraised Value		\$26,717.00

Just Value	\$26,717.00
Class Value	\$0.00
Assessed Value	\$26,717.00
Exempt Value	\$0.00
Total Taxable Value	\$26,717.00

Sales History

Sale Date	Book/Page	Inst. Type	Sale VImp	Sale Qual	Sale RCode	Sale Price
4/30/1998	858/480	WD	V	U	02	\$45,000.00

Building Characteristics

Bldg item	Bidg Desc	Year Bit	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value
NONE						

Extra Features & Out Buildings

Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
0040	BARN,POLE	2003	\$950.00	380.000	19 x 20 x 0	(.00)

Land Breakdown

Lnd Code	Desc	Units	Adjustments	Eff Rate	Lnd Value
000700	MISC RES (MKT)	4.000 AC	1.00/1.00/1.00/1.00	\$6,400.00	\$25,600.00
000700	MISC RES (MKT)	5.140 AC	1.00/1.00/1.00/1.00	\$32.50	\$167.00

1 of 1

Columbia County Property Appraiser

DB Last Updated: 8/1/2006

Disclaimer

This information was derived from data which was compiled by the Columbia County Property Appraiser's Office solely http://columbia.floridapa.com/GIS/D_SearchResults.asp 9/14/2006

DISCLOSURE STATEMENT

FOR OWNER/BUILDER WHEN ACTING AS THER OWN CONTRACTOR AND CLAIMING EXEMPTION OF CONTRACTOR LICENSING REQUIREMENTS IN ACCORDANCE WITH FLORIDA STATUTES, ss. 489.103(7).

State law requires construction to be done by licensed contractors. You have applied for a permit under an exemption to that law. The exemption allows you, as the owner of your property, to act as your own contractor with certain restrictions even though you do not have a license. You must provide direct, onsite supervision of the construction yourself. You may build or improve a one-family or two-family residence or a farm outbuilding. You may also build or improve a commercial building, provided your costs do not exceed \$25,000. The building or residence must be for your own use or occupancy. It may not be built or substantially improved for sale or lease. If you sell or lease a building you have built or substantially improved yourself within 1 year after the construction is complete, the law will presume that you built or substantially improved it for sale or lease, which is a violation of this exemption. You may not hire an unlicensed person to act as your contractor or to supervise people working on your building. It is your responsibility to make sure that people employed by you have licenses required by state law and by county or municipal licensing ordinances. You may not delegate the responsibility for supervising work to a licensed contractor who is not licensed to perform the work being done. Any person working on your building who is not licensed must work under your direct supervision and must be employed by you, which means that you must deduct F.I.C.A. and withholding tax and provide workers' compensation for that employee, all as prescribed by law. Your construction must comply with all applicable laws, ordinances, building codes, and zoning regulations.

TYPE OF CONSTRUCTION

() Two-Family Residence

M Single Family Dwelling

() Farm Outbuilding	() Other
() New Construction	() Addition, Alteration, Modification or other Improvement
NEW	CONSTRUCTION OR IMPROVEMENT
	, have been advised of the above disclosure statement ensing as an owner/builder. I agree to comply with all requirements .489.103(7) allowing this exception for the construction permitted by .Number Z 4 9 7 4
Edward R. Rogad Signature	Date .
	FOR BUILDING USE ONLY
I hereby certify that the above lister Florida Statutes ss 489.103(7).	ed owner/builder has been notified of the disclosure statement in
Date 9-14-06 Buildin	ng Official/Representative Sai fl

NOTICE OF COMMENCEMENT FORM **COLUMBIA COUNTY, FLORIDA**

*** THIS DOCUMENT MUST BE RECORDED AT THE COUNTY CLERKS OFFICE BEFORE YOUR FIRST INSPECTION. ***

THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement. Tax Parcel ID Number

1013.W. Wharry for	of the property and street address or 911 address) + White Fl. 32038 of Lot 15 Little Pine Acres
General description of improvement:	Wir A house
Owner Name & Address EdoSue R.	opelle 12204 Flenkis Da Rivering 33565
Name & Address of Fee Simple Owner (if o	other than owner):nom + DAcl Sohn = Dot Oempe
1141035 <u>- 1201 0 1201111 00</u>	Phone Number 8/3-294-2329 Kive(view f) 33569
Address Amount of Bond Lender Name Address	Inst:2006016456 Date:07/11/2006 Time:13:40
Persons within the State of Florida designatived as provided by section 718 13 (1)(a) 7	ated by the Owner upon whom notices or other documents may be Florida Statutes:
Name Warhouse BANK N	Phone Number Phone Number Phone Number
Address <u>P.O. Box 530554</u> f	HI ANIA GA. 30353
The Callet of	esignates
(a) 7. Phone Number of the designee	eive a copy of the Lienor's Notice as provided in Section 713.13 (1)
	ement (the expiration date is 1 (one) year from the date of recording

NOTICE AS PER CHAPTER 713, Florida Statutes:

The owner must sign the notice of commencement and no one else may be permitted to sign in his/her stead.

Sworn to (or affirmed) and subscribed before 1144 day of Tuly **GALE TEDDER**

MY BORRESON SCHOOL STANSON /S **EXPIRES: June 28, 2008**

Bonded Thru Ngfary Public Underwriters

Signature of Notary



STATE OF FLORIDA DEPARTMENT OF HEALTH

APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number PART II - SITE PLAN-Scale: Each block represents 5 feet and 1 inch = 50 feet. Site Plan submitted by: Signature Title Plan Approved **Not Approved** Date COLUMBIA **County Health Department**

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

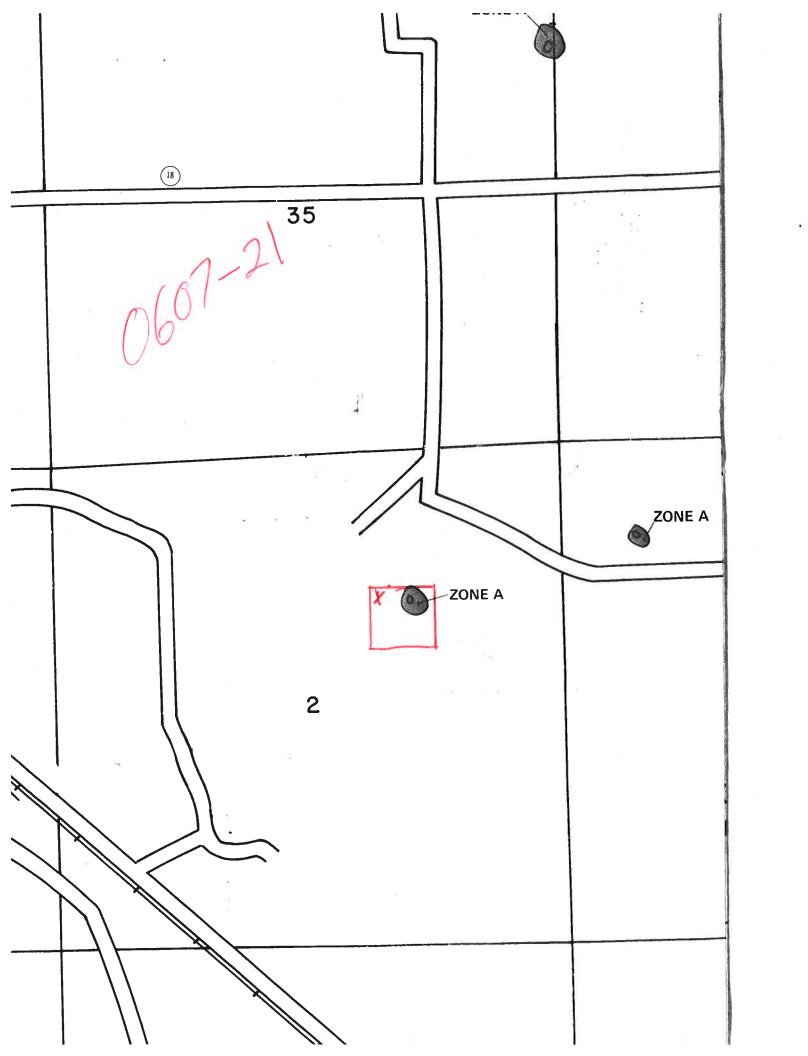
PRODUCT APPROVAL SPECIFICATION SHEET

and the second s				 ·
		•		
Location	•		Project Name	

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and the product approval number(s) on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit on or after April 1, 2004 We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. More information about statewide product approval can be obtained at www.floridabuilding.org

Category/Subcategory	Manufacturer	Product Description	Approval Number(s)
A. EXTERIOR DOORS			
1. Swinging	Thema. to corp.	Ext door	F 6
2. Silding			
3 Sectional			
4. Roll up			11 H 11
5. Automatic			
6. Other			
B. WINDOWS			
1. Single hung	NU AIT	fin and f ge	F1 21
2. Horizontal-Slider		y .	
3. Casement			
4. Double Hung			
5. Fixed	NU AIR	All shapes	712
6. Awning			
7. Pass-through			
8. Projected		1 200	
9. Mullion			
10, Wind Breaker		•	
11 Dual Action			
12. Other			
C. PANEL WALL			
1. Siding			The Asset
2. Soffits	Cameron Ashley	VIAY SOFFITS	F1 7621
3. EIFS	7	A A A A A A A A A A A A A A A A A A A	
4. Storefronts		August 1997 and 1997	The second secon
5: Curtain walls		A CONTROL OF CONTROL O	and pure from Affect Martin and Martin Company (April 1994) 1994 1995 person of COLON Parties and Colon Parties (April 1994) 1994 1995 person of COLON Parties (April 1994) 1994 1994 1995 person of COLON Parties (April 1994) 1994 1994 1994 1995 person of COLON Parties (April 1994) 1994 1994 1994 1994 1994 1994 199
6: Wall louver			* * ***********************************
47. Glass block		And the second s	ero numero e menumento comunicativa de desenta da desega habaterista del desenta del desenta de desenta de desenta de desenta de desenta de desenta de de desenta de de del del de de del del de de del de
8. Membrane			
9. Greenhouse		,	
10. Other			
The second secon			3000
D. ROOFING PRODUCTS 1. Asphalt Shingles	GAF	11 14 CONTROL STATE STATE STATE STATE OF THE	71 183
2. Underlayments	Owers Corning	30 felt	101.1
3. Roofing Fasteners	Owers		
Non-structural Metal	Rf		The second secon
5. Built-Up Roofing			
6. Modified Bitumen			
7. Single Ply Roofing Sys	2		
8. Roofing Tiles			
9. Roofing Insulation			
10. Waterproofing			have a day a sub-have the plant of the sub-state of the s
11. Wood shingles /shak	'Ac i	l.	

Website: www.tlcoermits.org Effective April 1 2011





Cal-Tech Testing, Inc.

Engineering P.O. Box 1625 • Lake City, FL 32056 4784 Rosselle Street · Jacksonville, FL 32254

Geotechnical

Environmental

LABORATORIES

2230 Greensboro Highway • Quincy, FL 32351

Tel. (386) 755-3633 · Fax (386) 752-5456 Tel. (904) 381-8901 • Fax (904) 381-8902 Tel. (850) 442-3495 · Fax (850) 442-4008

September 1, 2006

Edward Riopelle 12204 Glen Hill Drive Riverview, Florida 33569

Reference:

Proposed Residence

807 Quarry Circle Drive Columbia County, Florida Cal-Tech Project No. 06-520

Dear Mr. Riopelle,

Cal-Tech Testing, Inc. has completed the subsurface investigation and engineering evaluation of the site for a residence to be constructed at the referenced address in Columbia County, Florida. Our work was performed in conjunction with and authorized by you.

We understand the residence will be single-story and of masonry block construction with a plan area of approximately 6,000 square feet, heated and cooled. Support for the residence is to be provided by a monolithic foundation. Anticipated foundation loads were not provided; however, we assume column and wall loads will not exceed 25 kips and 2 kips per foot, respectively.

The purposes of our investigation were to evaluate the existing subgrade soils for an allowable bearing pressure of 2,500 pounds per square foot and to provide recommendations as appropriate.

Site Investigation

The building area was investigated by performing four Standard Penetration Test borings advanced to depths of 10.0 feet. Borings were performed at the approximate locations indicated on the attached drawing. Boring locations were selected by Cal-Tech Testing, Inc., and the building area was delineated on site.

The Standard Penetration Test (ASTM D-1586) is performed by driving a standard split-barrel sampler into the soil by blows of a 140-pound hammer falling 30 inches. The number of blows required to drive the sampler 1 foot, after seating 6 inches, is designated the penetration resistance, or N-value; this value is an index to soil density or consistency.

<u>Findings</u>

The soil borings generally encountered three soil strata. The first layer consists of 1.0 to 2.5 feet of loose, generally tan or grayish tan sand (SP) or sand with silt (SP/SM). The N-values of this layer are on the order of 8 to 9 blows per foot.

The second layer consists of 1.5 to 9 or more feet of very loose to loose, tan or grayish tan sand with clay (SP/SC) or clayey sand (SC) and/or stiff, grayish tan, tan and orange or gray and orange, sandy clay (CL). The N-values of this layer range from 1 to 25 blows per foot. A thin lens of limestone was encountered within this layer at boring location B-2.

The third layer consists of an undetermined thickness of loose to dense limestone for which N-values range from 8 to 42 blows per foot. This layer was encountered only at boring location B-4.

Groundwater was not encountered at the time of our investigation, and we estimate the wet season water table will occur at a depth of more than 6.0 feet below the existing surface grade. Note however that storm water will temporarily perch on clayey soils encountered near the ground surface.

For a more detailed description of the subsurface conditions encountered, please refer to the attached Boring Logs.

Discussion and Recommendations

We have performed a bearing capacity analysis for the immediate bearing soils and have used the proposed thickened edge of width 20 inches and thickness 18 inches. Embedment is assumed to be 12 inches, the minimum acceptable. For this foundation and the site soils as encountered, we obtained allowable bearing pressures of 2,500 pounds per square foot with factors of safety ranging from about 1.1 to 1.3. It is therefore our opinion the subgrade soils within the proposed building area are suitable for the proposed monolithic foundation and an allowable bearing pressure of 2,500 pounds per square foot. However, we believe factors of safety need to be improved, and we recommend thorough site preparation be performed.

Site preparation specifically should include thorough proof-rolling and proof-compaction of the stripped building limits. Proof-rolling will help to compact the bearing soils and to locate zones of especially loose or soft soils that may be present. Such zones should be excavated and replaced or otherwise treated as directed by the geotechnical engineer.

Fill materials, if required, should consist of reasonably clean, fine sand containing less than about 10% passing the No. 200 sieve. Fill should be placed in maximum 12-inch, loose lifts, and each lift should be proof-compacted to a minimum of 95% of the Modified Proctor maximum dry density.

Following proof-rolling operations, the bearing soils should be proof-compacted to a minimum of 95% of the Modified Proctor maximum dry density to a depth of at least 2 feet in foundation areas and 1 foot in floor slab areas.

Our evaluation is based upon the subsurface conditions encountered and as presented within this report. However, subsurface conditions may exist that differ from our findings. We request that we be notified if substantially different subsurface conditions are encountered.

We appreciate the opportunity to be of service on this project and look forward to a continued association. Please do not hesitate to contact us should you have questions concerning this report or if we may be of further assistance.

Respectfully submitted, Cal-Tech Testing, Inc.

Linda Creamer President / CEO John C. Dorman, Jr., Ph.D., P.E.

Geotechnical Engineer

526/2

82002 Rev 10/03

City/State/Zio_

TERMITE SERVICE REPORT 24974 Date: 11/25/2006

Customer NameEdward & Suste Riopelle	Phone # 813-872-3403 HM1
Service Address 807 Quarry Rd Fort White, FL 3	
Account Number 01-0002971 Infestation 1	Termite Complete Liquid Application TERT
1 - m	70 00 Guarantee Type
Service Covered Thru: Completion	11/25/2006 Check 11/26 [1]
Competition	Date Renewal Amount Grid #
I. Bait Activity: ☐ Yes ☐ No # of Station	s: Monitoring Bait Next Service Date:
II. Service	ent 🔲 Service Call (No Treatment) 🔲 Reinspection 📋 Bait Monitoring 🔲 Annual Bait Reinspection
III. Materials Used (Utilize Product Information Key	hat includes EPA Reg # and Active Ingredient information on back for completion of this section.)
	ilution % Product # (From Key) Amount Applied Dilution %
3)	
2)	F)
V. Conducive Conditions	
s probable that your home will experience future termite activity a any mold-like conditions. Mold is generally not a wood destroy	Moisture
	States where applicable:
	Wind Direction
	Wind Velocity
	Temperature
	Humidity
	Time on job
	Target Pest
Inspection	
) Performed on (Date):	B) Activity Found: Yes No
Retreatment Scheduled Date (if needed):	
I. Treatment Thank yo	Orkin Representat
	ture can disturb the termiticide treatment and may require additional inspection and treatment.
The location of these areas are:	
This work has been performed to my satisfaction	
	Customer Signature Date
I would like to accept the Valued Customer Savings P	rogram for Pest Control Service.
in a franche	Susied Knext 1/3/1
	EXTERMINATING COA# (If-applicable) Date
	3 WILLISTON RD. CUSTOMER COPY

GAINESVILLE FLA. 32608

COLUMBIA PERIT 24974

Dear County BUILDING, -ZONING DEFT.

My name is Suis hopelle my husband name is Elward Rights we have not finish the louse yet, because of lack of mong and no work Sus 124974.

Popult # 24974.

SHORT FURN FOR "MANUAL 1" LOAD CALCUATION FOR RESIDENTIAL HEATING AND AIR CONDITIONING

		(3)			
•	Lac	Lons	anec	1 d.t.	l neg'a
GLASS DIVIDE HEATING SO, TT	ILEA	7 000	UA		•
GLASS DOORS HEATING SO, 17.	85	xxx	40	NEAT 3400	CDOL
WINDOW REATING SO. 77.	50	exx	3 58	\$9 II	
WINDOWS AND GLASS DOORS COOLING SU. 77. N. EXP.	xxx	25	105.5		-
WINDOWS AND GLASS DOORS COOLING SQ. 77. EEW EXP.	xxx	55	, 44.		2640
WINDOWS AND GLASS DOORS COOLING SU. 17. S. EXP.	exx			1200	7959
UMER-100/13-50, 17.	85	30	147.7	xxx	443
EXTERIOR WALLS - USE NET SQ. 77. AREA	- 07	13	20	1105	1700
CONCRETE BLOCK TWERED MIN R-3.6 INSUL.					
TRAME WISHEATHING SIDING OR VENEER -MIN. R-9.7 INS	5.6	4.7	178	9985	8380
TRAME NO SHEATING SIDING OR VENEER -MIN. R-10.3 INS.	2.8	3	1521	4259	4563
OTHER WALLS -SEE MANUAL J	2.8	3			
1/00012_SGOOT	37	-			•
TLOORS-SLAD ON GRADE-LIN. 17. OUTSIDE WALL X TACTOR	40	xxxxx	264	10560	
CELLING (1) ':" GYPSUM DDTOR "U" = . 05-MIN. R-18 INS.	1.7	1	3677	1	KKKKKK
CEILING (1) 4" GYPSUM BD TOR "U" = . U8-MIN. R-10.8 INS.	2.8		13611	6251	11031
1511 1001 DECK (1) (2) FOR "U"=, 14.	4.9	XXXXX	 	ुक जात	
TLAT ROOF DECK (1) (2) TOR "U"=, 09	3.1	xxxx	 		
VENTILATION - NO. DEDROOM X TACTOR	-1	2			
PEOPLE - NO. BEDROOM X TACTOR	800	360	4	3200	1440
PPLIANCES	xxx	450	4	xxx	1860
UBTOTAL ABOVE	XXXXX.	XXXXXX	XXXXX	XXXX	200
	xxxx	rxxxx	rere	5 6660	
UBTOTAL INCLUDING DUCT LOSS AND GAIN X FACTOR	1,05	1.1			
UBTOTAL INCLUDING LATENT HEAT (COOL) X TACTOR	,,0,		eeee	62328	5 4 6 48
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HUGHES WELL DRILLING & PUMP SERVICE

12367 N US HWY 441 LAKE CITY, FLORIDA 32055 OFFICE: (386)752-1840 FAX: (386)755-2934

E-MAIL:HUGWELL1840@AOL.COM

Columbia County Building and Zoning P.O. Box 1529
Lake City, Fl. 32056-1529

Attn: Gale Tedder/Janis

Subject: Requested Info: FEd & Sue Riopelle-02-07-16-04111-110

- 1- 4" Deep Well
- 2~ 1-hp Pump-20gpm
- 3- 82 Gallon Eqv. Bladder Tank
- 4- 1-Cycle Stop Valve
- 5- 1&1/4" Drop Pipe

If you have any further questions, please feel free to phone me at the above number.

Sincerely,

Ronnie Hughes

WE DRILL THE GEST AND SERVICE THE REST

Windows and Doors

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	FL5340		FL5339		FL5338		FL5337		FL5336					FL5334			FL5333			FL5331				FL5330						FL5722	Approval #
	Exterior Doors		Windows		Windows		Windows		Windows					Exterior Doors			Windows			Windows				Windows						Windows	Category
	Sliding		Fixed		Horizontal Slider		Single Hung		Awning					Sliding			Fixed			Horizontal Slider				Single Hung						Mullions	Sub-category
5340.1		5339.1		5338.1		5337.1		5336.1		5334.4	5334.3	5334.2	5334.1		5333.2	5333.1		5331.2	5331.1		5330.3	5330.2	5330.1		5722.5	5722.4	5722.3	5722.2	5722.1		Sub-numbers
9000		9000		9000		9000		A02/A03		975	500	200 Corner	200		975	900		975	900		975	950	900		Standard Tube	823	814	813	812		NuAir Series #
bypass and pocket		all shapes		XO, OX and XOX		fin and flange				bypass and pocket	bypass and pocket	bypass and pocket	bypass and pocket		all shapes	all shapes			XO, OX and XOX		fin and flange	fin and flange	fin and flange		2x2,3,4,5,6	1x2 tube awning	1x4 tube	1x3 tube	1x2 tube		Description
Impact		Impact		Impact		Impact		Non-Impact		Non-Impact	Non-Impact	Non-Impact	Non-Impact		Non-Impact	Non-Impact		Non-impact	Non-Impact		Non-Impact	Non-Impact	Non-Impact		impact	Non-Impact	impact	Impact	Non-Impact		Use

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Non-Impact, 900 Series Fixed Window Installation Details for Masonry Applications (for use in Wind Zones of 150 MPH or less per Florida Building Code 2001)*

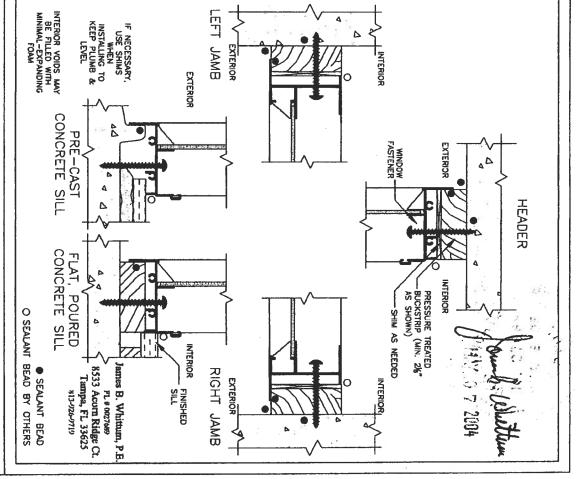
Buckstrips must be set in a bead of sealant. The exterior joint between the buckstrip and masonry must also be sealed. Buckstrips shall run the entire length of the rough opening. A buckstrip is not necessary at the sill if it is pre-cast. Buckstrips should be pressure-treated yellow pine, spruce or comparable lumber.

If using under 1-1/2" thick buckstrips:

Installation fasteners should be minimum 3/16" x 2-1/4" masonry screws, installed through the window and buckstrip, into the masonry. Window may be shimmed, as needed, provided a minimum screw embedment of 1-1/4" is maintained in the masonry. Fasteners should be located at a maximum of 4" from each corner and a maximum of 18" on center. The actual size of the buckstrips should be no less than 1/2" x 2-1/8".

If using 1-1/2" thick or greater buckstrips (not detailed):

Installation fasteners should be minimum #10 x 1-1/2" wood screws, installed through the window into the buckstrip. Window may be shimmed, as needed, provided a minimum screw embedment of 1-1/4" is maintained in the buckstrip. Fasteners should be located at a maximum of 4" from each corner and a maximum of 18" on center. The actual size of the buckstrip should be no less than 1-1/2" x 2-1/8" and should be installed using minimum 3/16" x 2-3/4" masonry screws to maintain the 1-1/4" embedment and a maximum of 18" on center.



*The product depicted above is intended for use on typical construction. The use of additional flashing, vapor barriers, fasteners, etc may be specified by the project's design professional. Basic wind speed alone is not sufficient to determine the required design pressure (DP). The DP must be calculated in accordance with ASCE 7-98 by a design professional that is familiar with the project design and location, as specified in the 2001 FBC. DP comparative analysis charts and AAMA-101 compliant test reports are available for all NuAir manufactured products. Please consult your local building codes for exact requirements.





Non-Impact, 900 Series Fixed Window Installation Details for Wood Frame Applications (for use in Wind Zones of 150 MPH or less per Florida Building Code 2001)*

11

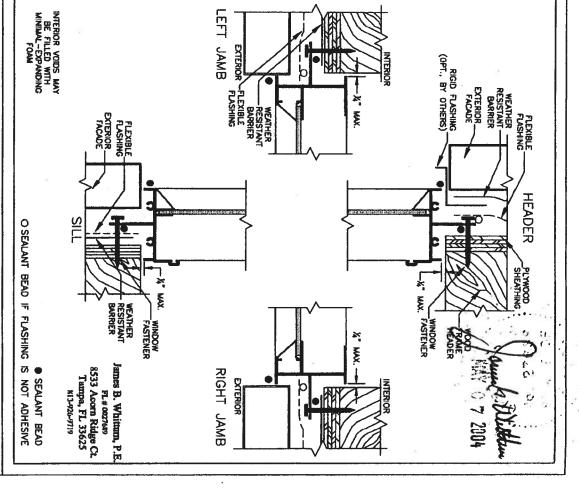
Window Installation:

Use the chart below as a guideline in selecting the appropriate window fastener. Note that fasteners should be located no more than 4" from the corner of the window. It is recommended that all screws or nails be sealed to prevent infiltration of air and water.

14" 8"	+/- 45 psf +/- 80 psf	2-1/2"	8d Nail
28" 18"	+/- 45 psf +/- 80 psf	2"	# 6 Screw
24" 14"	+/- 45 psf +/- 80 psf	1-1/2"	# 6 Screw
Center-to-Center Fastener Distance	Maximum Design Pressure	Fustener Length	Fastener Diameter
Wood/Drywall/Decking Screws or Common Nail	cking Screw	ywall/De	Wood/D ₁

Weatherproofing:

Flexible flashing should be installed in a weatherboard fashion. The top layer should overlay any layer underneath it. The weather resistant barrier should then cover the flexible flashing at the header. The application of the weather resistant barrier will vary based on when it is installed. If installed after the window, it should be tucked under the sill flashing and overlap the jambs and head flashing.



*The product depicted above is intended for use on typical construction. The use of additional flashing, vapor harriers, fasteners, etc may be specified by the project's design professional. Basic wind speed alone is not sufficient to determine the required design pressure (DP). The DP must be calculated in accordance with ASCE 7-98 by a design professional that is familiar with the project design and location, as specified in the 2001 FBC. DP comparative analysis charts and AAMA-101 compliant test reports are available for all NuAir manufactured products. Please consult your local building codes for exact requirements.





Non-Impact, 900 & 950 Series Single Hung Window Installation
Details for Masonry Applications (for use in Wind Zones of 150
MPH or less per Florida Building Code 2001)*

Buckstrips must be set in a bead of sealant. The exterior joint between the buckstrip and masonry must also be sealed. Buckstrips shall run the entire length of the rough opening. A buckstrip is not necessary at the sill if it is pre-cast. Buckstrips should be pressure-treated yellow pine, spruce or comparable lumber. Window fasteners may be installed on either the interior or exterior side.

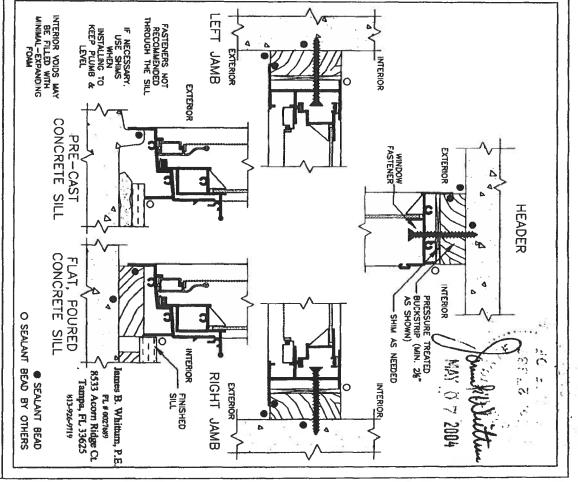
If using under 1-1/2" thick buckstrips:

Installation fasteners should be minimum 3/16" x 2-1/4" masonry screws, installed through the window and buckstrip, into the masonry. Window may be shimmed, as needed, provided a minimum screw embedment of 1-1/4" is maintained in the masonry. Fasteners should be located at a maximum of 4" from each corner and a maximum of 18" on center or use the manufacturer's pre-punched installation holes. It is recommended that fasteners not be installed through the sill of the window. The actual size of the buckstrips should be no less than 1/2" x 2-1/8".

If using 1-1/2" thick or greater buckstrips (not detailed):

Installation fasteners should be minimum #10 x 1-1/2" wood screws, installed through the window into the buckstrip. Window may be shimmed, as needed, provided a minimum screw embedment of 1-1/4" is maintained in the buckstrip. Fasteners should be located at a maximum of 4" from each corner and a maximum of 18" on center or use the manufacturer's pre-punched installation holes. It is recommended that fasteners not be installed through the sill of the window. The actual size of the buckstrip should be no less than 1-1/2" x 2-1/8" and should be installed using minimum 3/16" x 2-3/4" masonry screws to maintain the 1-1/4" embedment and a maximum of 18" on center.

Note: For minimum masonry anchoring based on design pressure, see "Alternate Masonry Installation Fastening Methods for Single Hung Windows" drawing.



sufficient to determine the required design pressure (DP). The DP must be calculated in accordance with ASCE 7-98 by a design professional that is familiar with the project design and location, as specified in the 2001 FBC. DP comparative analysis charts and AAMA-101 compliant test reports are available for all NuAir manufactured products. Please consult your local building codes for exact requirements. The product depicted above is intended for use on typical construction. The use of additional flashing, vapor barriers, fasteners, etc may be specified by the project's design professional. Basic wind speed alone is not





Non-Impact, 900 & 950 Series Single Hung Window Installation
Details for Wood Frame Applications (for use in Wind Zones of 150
MPH or less per Florida Building Code 2001)*

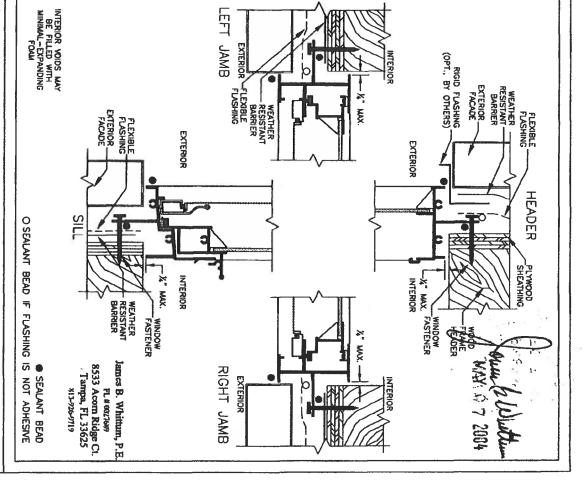
Window Installation:

Use the chart below as a guideline in selecting the appropriate window fastener. Note that fasteners should be located no more than 4" from the corner of the window. It is recommended that all screws or nails be sealed to prevent infiltration of air and water.

14" 8"	+/- 45 psf +/- 80 psf	2-1/2"	8d Nail
28" 18"	+/- 45 psf +/- 80 psf	2"	# 6 Screw
24" 14"	+/- 45 psf +/- 80 psf	1-1/2"	# 6 Screw
Center-to-Center Fastener Distance	Maximum Design Pressure	Fastener Length	Fastener Diameter
Wood/Drywall/Decking Screws or Common Nail	cking Screw	ywall/De	Wood/Dr

Weatherproofing:

Flexible flashing should be installed in a weatherboard fashion. The top layer should overlay any layer underneath it. The weather resistant barrier should then cover the flexible flashing at the header. The application of the weather resistant barrier will vary based on when it is installed. If installed after the window, it should be tucked under the sill flashing and overlap the jambs and head flashing.



*The product depicted above is intended for use on typical construction. The use of additional flashing, vapor barriers, fasteners, etc may be specified by the project's design professional. Basic wind speed alone is not sufficient to determine the required design pressure (DP). The DP must be calculated in accordance with ASCE 7-98 by a design professional that is familiar with the project design and location, as specified in the 2001 FBC. DP comparative analysis charts and AAMA-101 compliant test reports are available for all NuAir manufactured products. Please consult your local building codes for exact requirements.







From: The Columbia County Building & Zoning Department

Plan Review

135 NE Hernando Av.

P.O. Box 1529

Lake City Florida 32056-1529

Reference to a building permit application Number: 0607-21

Contractor: Black Ball Construction Owners Edward & Sue Riopelle 02-7s-16-04111-110

On the date of July 12, 2006 application 0607-21 and plans for construction of a single family dwelling were reviewed and the following information or alteration to the plans will be required to continue processing this application. If you should have any question please contact the above address, or contact phone number (386) 758-1163 or fax any information to (386) 754-7088.

Please include application number 0607-21 and when making reference to this application.

This is a plan review for compliance with the Florida Residential Code 2004 only and doesn't make any consideration toward the land use and zoning requirements.

To help ensure compliance with the Florida Residential Code 2004 the comments below need to be addressed on the plans.

1. Please have Mr. David W. Smith PE provide the following information per the Florida Residential Code.

Wind-load Engineering Summary, calculations

Basic wind speed (3-second gust), miles per hour (km/hr)

Wind importance factor, lw, and building classification from Table 1604.5 or Table 6-1, ASCE 7 and building classification in Table 1-1, ASCE 7.

Wind exposure, if more than one wind exposure is utilized, the wind exposure and applicable wind direction shall be indicated.

The applicable enclosure classifications and, if designed with ASCE 7, internal pressure coefficient components and cladding.

- 2. As required by Florida statute 553.842 and Florida Administrative code 9B-72, please provide the product approval number(s) on the components listed on the attached form.
- 3. Bathroom number two has a window which may be defined Hazardous locations: FRC-2004 section R308.4 Glazing in doors and enclosures for hot tubs, whirlpools, saunas, steam rooms, bathtubs and showers. Glazing in any part of a building wall enclosing these compartments where the bottom exposed edge of the glazing is less than 60 inches (1524 mm) measured vertically above any standing or walking surface. Each pane of glazing installed in hazardous locations as defined in Section R308.4 shall be provided with a manufacturer's or installer's label, designating the type and thickness of glass and the safety glazing standard with which it complies, which is visible in the final installation. The label shall be acid etched, sandblasted, ceramic-fired, embossed mark, or shall be of a type which once applied cannot be removed without being destroyed. Please verify this window will comply with section R308.4

- 4. Please verify that at least one window in each bedroom will meet the requirements of the FBC-2004 Section R310.1.1 Minimum opening area: All emergency escape and rescue openings shall have a minimum net clear opening of 5.7 square feet (0.530 m2). Exception: Grade floor openings shall have a minimum net clear opening of 5 square feet (0.465 m2): R310.1.2 Minimum opening height. The minimum net clear opening height shall be 24 inches (610 mm): R310.1.3 Minimum opening width.
 The minimum net clear opening width shall be 20 inches (508 mm).
- 5. The stairwell to the second floor must be designed to meet the requirement of section R311.5 Stairways. R311.5.1 Width. Stairways shall not be less than 36 inches (914 mm) in clear width at all points above the permitted handrail height and below the required headroom height. Handrails shall not project more than 4.5 inches (114 mm) on either side of the stairway and the minimum clear width of the stairway at and below the handrail height, including treads and landings, shall not be less than 31.5 inches (787 mm) where a handrail is installed on one side and 27 inches (698 mm) where handrails are provided on both sides. Exception: The width of spiral stairways shall be in accordance with Section R311.5.8. R311.5.2 Headroom. The minimum headroom in all parts of the stairway shall not be less than 6 feet 8 inches (2036 mm) measured vertically from the sloped plane adjoining the tread nosing or from the floor surface of the landing or platform. R311.5.3 Stair treads and risers. R311.5.3.1 Riser height. The maximum riser height shall be 7% inches

(196 mm). The riser shall be measured vertically between leading edges of the adjacent treads. The greatest riser height within any flight of stairs shall not exceed the smallest by more than 3/8 inch (9.5 mm). R311.5.3.2 Tread depth. The minimum tread depth, exclusive of nosing, shall be not less than 9 inches (229 mm). Treads and risers of stairs shall be permitted to be so proportioned that the sum of two risers and a tread, exclusive of projection of nosing, is not less than 24 inches (610 mm) nor more than 25 inches (635 mm). The tread depth shall be measured horizontally between the vertical planes of the foremost projection of adjacent treads and at a right angle to the tread's leading edge. The greatest tread depth within any flight of stairs shall not exceed the smallest by more than 3/8 inch (9.5 mm). Winder treads shall have a minimum tread depth of 10 inches (254 mm) measured as above at a point 12 inches (305) mm from the side where the treads are narrower. Winder treads shall have a minimum tread depth of 6 inches (152 mm) at any point. Within any flight of stairs, the greatest winder tread depth at the 12 inch (305 mm) walk line shall not exceed the smallest by more than 3/8 inch (9.5 mm). Please show a drawing which will indicate the above requirements and include the total rise and run of the stair.

9-11-06

6. The general structural notes (#18) assume that the soil load bearing capacity is 2,500 pound per square foot. The building department only assumes for all soils in Columbia County to have a load bearing capacity is 1,000 pound per square foot. Therefore one of the two prescribed

methods must be preformed to insure the proper load bearing soils to support the structure foundation. Method one: Have the structural designer Mr. David W. Smith redesign the foundation to be so designed to support the structure using a load bearing capacity equal to 1,000 pound per square foot. Method two: Have the follow prescribed testing methods done to reveal the soil load bearing capacities. Please have a registered professional conduct subsurface explorations at the project site upon which foundations are to be constructed, a sufficient number (not less than four, one boring on each corner of the building foundation) borings shall be made to a depth of not less than 10 feet (3048 mm) below the level of the foundations to provide assurance of the soundness of the foundation bed and its load-bearing capacity.

- 7. Two sets of truss plans were submitted with the application, neither had the truss designer razed engineered seal which must be affixed to these truss plans. Please submit two sets of the roof truss and two sets floor truss plans with a razed engineered seal on each set.
- **8.** On the wall section drawing please identify the size rim joist and the attachment method which will connect the 16" floor truss to the foundation and second floor structural framing.
- **9.** On the electrical plan show the location of the electrical panel and include the total amperage rating of the electrical service panel also show the overcurrent protection device which shall be installed on the exterior of structures to serve as a disconnecting means. Conductors used from the

exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground.

- Efficiency Code for Building Construction, 13-100.2 Intent. The provisions of this code shall regulate (1) the design of building envelopes for adequate thermal resistance and low air leakage and (2) the design and selection of mechanical, electrical, and illumination systems and equipment which will enable the effective use of energy in new building construction, additions, alterations or any change in building configuration. Forms are available from the local jurisdiction permitting offices or may be obtained from the Department of Community Affairs, Codes and Standards Section, 2555 Shumard Oak Blvd., Tallahassee, Florida 32399-2100. Copies of Subchapter 6 forms may be found in Appendix 13-D of this chapter or online at www.floridabuilding.org. Also submit a Manual J sizing equipment or equivalent computation for the HVAC system to be installed within the dwelling.
- 11. The building permit application lists Edward & Sue Riopelle as the applicants. The listed building contractor is Black Ball Construction Inc. In order for the applicants to obtain a building permit a notarized statement will need to be submitted which authorizes you as the applicant to sign on the behalf of the contractor.

- **12.** Please submit a letter form the potable water well contractor which will describe the equipment to be used to supply potable water to this dwelling. Include the size of pump motor, size of pressure tank and cycle stop valve if used.
- **13.** On the building permit application form please establish a estimated cost of construction.

Joe Haltiwanger

Plan Examiner

Columbia County Building Department

PRODUCT APPROVAL SPECIFICATION SHEET

	ODUUI AIII	TOVAL OF LOW TON OFFICE	
Location:		Project Name:	
product approval number(s) on which you are applying for a l	the building compo building permit on he product approva	A Administrative Code 9B-72, please provide the onents listed below if they will be utilized on the or after April 1, 2004. We recommend you coll number for any of the applicable listed product at www.floridabuilding.org	construction project for intact your local product
Category/Subcategory	Manufacturer	Product Description	Approval Number(s)
A. EXTERIOR DOORS			
1. Swinging			
2. Sliding			
3. Sectional			
4. Roll up			
5. Automatic			
6. Other			
B. WINDOWS			
1. Single hung			
2. Horizontal Slider			
3. Casement			
4. Double Hung			
5. Fixed			
6. Awning			
7. Pass-through			
8. Projected			· · · · · · · · · · · · · · · · · · ·
9. Mullion			
10. Wind Breaker			
11 Dual Action			
12. Other			
C. PANEL WALL			
1. Siding			
2. Soffits			
3. EIFS			
4. Storefronts	,		
5. Curtain walls			
6. Wall louver			
7. Glass block			
8. Membrane			
9. Greenhouse		*	
10. Other			
D. ROOFING PRODUCTS			
Asphalt Shingles			
2. Underlayments			
Roofing Fasteners			
4. Non-structural Metal Rf			
5. Built-Up Roofing			
6. Modified Bitumen		II	
7. Single Ply Roofing Sys			
8. Roofing Tiles			
9. Roofing Insulation			
10. Waterproofing			
11. Wood shingles /shakes	3		
12. Roofing Slate			

02/02/04 - 1 of 2 Website: www.tlcpermits.org Effective April 1, 2004

Category/Subcategory (cont.)	Manufacturer	Product Description	Approval Number(s)
13. Liquid Applied Roof Sys			
 Cements-Adhesives – Coatings 			
15. Roof Tile Adhesive			
16. Spray Applied			
Polyurethane Roof			
17. Other			
E. SHUTTERS			
1. Accordion			
2. Bahama			
3. Storm Panels			
4. Colonial			
5. Roll-up			
6. Equipment			
7. Others			
F. SKYLIGHTS			
1. Skylight			
2. Other			
G. STRUCTURAL			
COMPONENTS			
Wood connector/anchor			
2. Truss plates			
3. Engineered lumber			
4. Railing			
5. Coolers-freezers			
6. Concrete Admixtures			
7. Material			
8. Insulation Forms			
9. Plastics 10. Deck-Roof			
11. Wall			
12. Sheds			
13. Other			
			<u> </u>
H. NEW EXTERIOR			
ENVELOPE PRODUCTS 1.			
2.			_
The products listed below di		te product approval at plan review. I underwing information must be available to the	
jobsite; 1) copy of the produ	ct approval, 2) th	ne performance characteristics which the poplicable manufacturers installation require	product was tested
I understand these products	may have to be	removed if approval cannot be demonstra	ited during inspection
·	•	. • •	<u> </u>
FC			
Contractor or Contractor's Authorize	d Agent Signature	Print Name	Date
Location		Permit # (FOR STAFF USE	ONLY)

02/02/04 - 2 of 2 Website: www.tlcpermits.org Effective April 1, 2004

From: The Columbia County Building & Zoning Department

Plan Review

135 NE Hernando Av.

P.O. Box 1529

Lake City Florida 32056-1529



Phone Number 386-758-1163 Fax Number 386-754-7088

FAX TRANSMITTAL FORM

To: Edward Riopelle From: Joe Haltiwanger Name: Date Sent: 07/13/06

CC: Building permit application 0607-21

Phone: Number of Pages: Ten pages including the cover page

Fax: 813-672-3894

Message: Reference to building permit application Number: 0607-21 Contactor: Black Ball Construction Owners: Edward and Sue Riopelle

The review of the party to whom it is addressed. It may contain proprietary and/or privileged information protected by law. If you are not the intended recipient, you may not use, copy or distribute this facsimile message or its attachments. If you have received this transmission in error, please immediately telephone the sender above to arrange for its return.



Cal-Tech Testing, Inc.

- Engineering
- Geotechnical
- Environmental

P.O. Box 1825 - Lake City, FL 32006 4784 Rossella Street - Jacksonvilla, FL 32254

2230 Greensbore Highway - Cuincy, FL 32351

Tel (866) 755-3633 • Fax (266) 752-5456
Tel (804) 381-8901 • Fax (804) 381-8902
Tel (850) 442-3486 • Fax (850) 442-4068

September 1, 2006

Edward Riopelle 12204 Glen Hill Drive Riverview, Florida 33569

Reference: Proposed Residence

807 Quarry Circle Drive Columbia County, Florida Cal-Tech Project No. 06-520

Dear Mr. Riopelle,

Cal-Tech Testing, Inc. has completed the subsurface investigation and engineering evaluation of the site for a residence to be constructed at the referenced address in Columbia County, Florida. Our work was performed in conjunction with and authorized by you.

We understand the residence will be single-story and of masonry block construction with a plan area of approximately 6,000 square feet, heated and cooled. Support for the residence is to be provided by a monolithic foundation. Anticipated foundation loads were not provided; however, we assume column and wall loads will not exceed 25 kips and 2 kips per foot, respectively.

The purposes of our investigation were to evaluate the existing subgrade soils for an allowable bearing pressure of 2,500 pounds per square foot and to provide recommendations as appropriate.

Site Investigation

The building area was investigated by performing four Standard Penetration Test borings advanced to depths of 10.0 feet. Borings were performed at the approximate locations indicated on the attached drawing. Boring locations were selected by CalTech Testing, Inc., and the building area was delineated on site.

The Standard Penetration Test (ASTM *D-1586) is performed by driving a standard split-barrel sampler into the soil by blows of a 140-pound hammer falling 30 inches. The number of blows required to drive the sampler 1 foot, after seating 6 inches, is designated the penetration resistance, or N-value, this value is an index to soil density or consistency.

Findings

The soil borings generally encountered three soil strata. The first layer consists of 1.0 to 2.5 feet of loose, generally tan or grayish tan sand (SP) or sand with silt (SP/SM). The N-values of this layer are on the order of 8 to 9 blows per foot.

The second layer consists of 1.5 to 9 or more feet of very loose to loose, tan or grayish tan sand with clay (SP/SC) or clayey sand (SC) and/or stiff, grayish tan, tan and orange or gray and orange, sandy clay (CL). The N-values of this layer range from 1 to 25 blows per foot. A thin lens of limestone was encountered within this layer at boring location B-2.

The third layer consists of an undetermined thickness of loose to dense limestone for which N-values range from 8 to 42 blows per foot. This layer was encountered only at boring location B-4.

Groundwater was not encountered at the time of our investigation, and we estimate the wet season water table will occur at a depth of more than 6.0 feet below the existing surface grade. Note however that storm water will temporarily perch on clayey soils encountered near the ground surface.

For a more detailed description of the subsurface conditions encountered, please refer to the attached Boring Logs.

Discussion and Recommendations

We have performed a bearing capacity analysis for the immediate bearing soils and have used the proposed thickened edge of width 20 inches and thickness 18 inches. Embedment is assumed to be 12 inches, the minimum acceptable. For this foundation and the site soils as encountered, we obtained allowable bearing pressures of 2,500 pounds per square foot with factors of safety ranging from about 1.1 to 1.3. It is therefore our opinion the subgrade soils within the proposed building area are suitable for the proposed monolithic foundation and an allowable bearing pressure of 2,500 pounds per square foot. However, we believe factors of safety need to be improved, and we recommend thorough site preparation be performed.

Site preparation specifically should include thorough proof-rolling and proof-compaction of the stripped building limits. Proof-rolling will help to compact the bearing soils and to locate zones of especially loose or soft soils that may be present. Such zones should be excavated and replaced or otherwise treated as directed by the geotechnical engineer.

Fill materials, if required, should consist of reasonably clean, fine sand containing less than about 10% passing the No. 200 sieve. Fill should be placed in maximum 12-inch, loose lifts, and each lift should be proof-compacted to a minimum of 95% of the Modified Proctor maximum dry density.

Following proof-rolling operations, the bearing soils should be proof-compacted to a minimum of 95% of the Modified Proctor maximum dry density to a depth of at least 2 feet in foundation areas and 1 foot in floor slab areas.

Our evaluation is based upon the subsurface conditions encountered and as presented within this report. However, subsurface conditions may exist that differ from our findings. We request that we be notified if substantially different subsurface conditions are encountered.

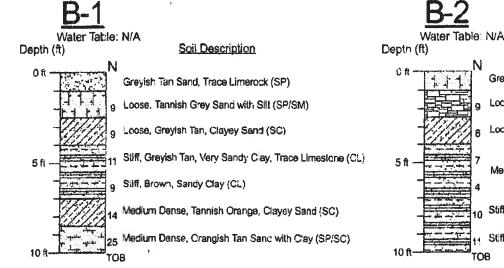
We appreciate the opportunity to be of service on this project and look forward to a continued association. Please do not hesitate to contact us should you have questions concerning this report or if we may be of further assistance.

Respectfully submitted, Cal-Tech Testing, Inc.

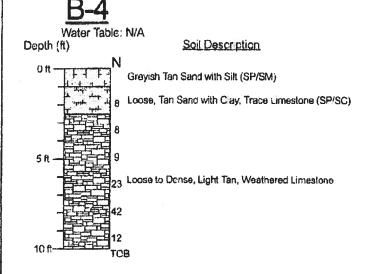
Linda Creamer President / CEO John C. Dorman, Jr., Ph.D./P.E.

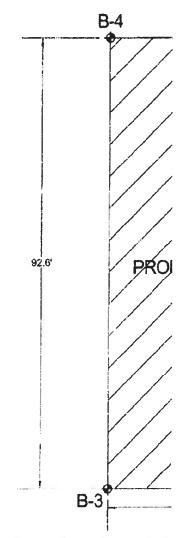
Geotechnical Engineer

526/2

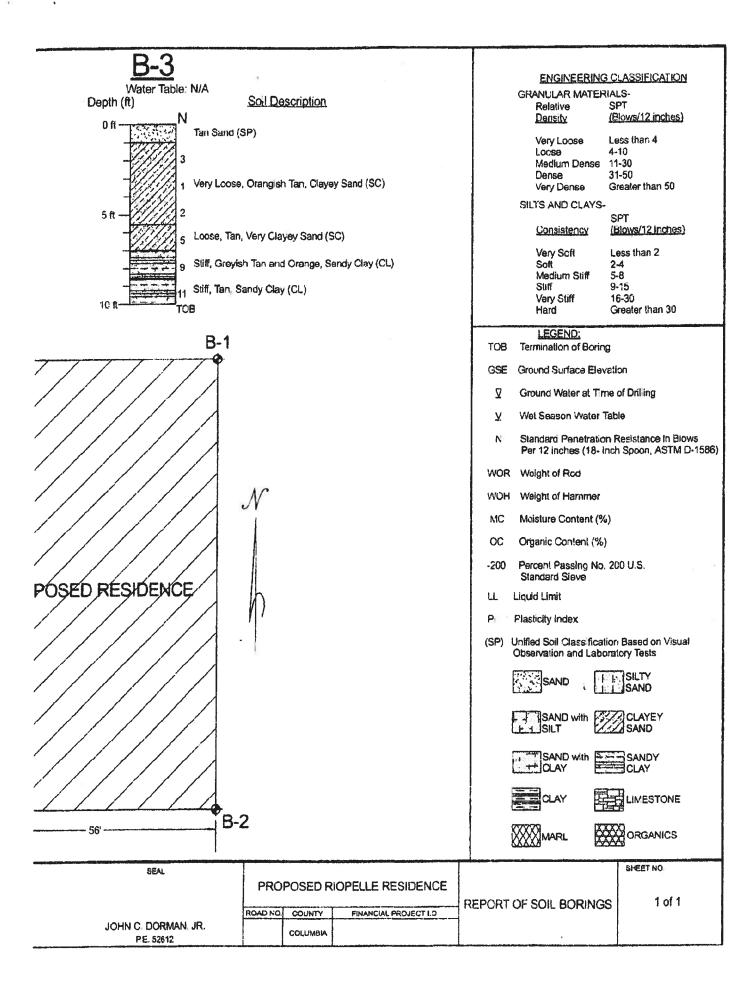


Water Table: N/A Depth (ft) Soil Description Oft Greyish Tan Sand with Silt (SP/SM) Locse, Light Tan Limercok Locse, Orengish Tan, Clayey Sand (SC) Medium Stiff, Greyish Tan, Sandy Clay (CL) Stiff, Tan, Grey and Orange, Sandy Clay (CL) Stiff, Grey, Tan and Orange, Sandy Clay (CL) TOB





		REVI	SIONS	}			NAMES	DATE	ENGINEER OF RECORD CAL- TECH TESTING, INC.
DATE	BY	DESCRIPTION	DATE	BY	DESCRIPTION	DRAWNBY	S.C. Young	9/4/06	CAL- IECH IESTING, INC.
						CHECKED BY	J.C. Dorman		P.O. BOK 1625
						CHECKED BY			LAKE C/TY, FL. 32056
						APPROVED BY			PHONE NO. (388) 755-3633
						CAL-TECH JO	OB NO.	06-520	FAX NO. (385) 752-5456





CONFIDENTIAL

Fax Cover Sheet

From: Gulf Steel, Inc.

Richard E. Gavaletz, Sr., President P.O. Box 1710, Pensacola, FL 32591

Phone: 1-850-497-0301 (local)

Fax: 1-850-497-0302

THIS FACSIMILE IS INTENDED FOR USE BY THE RECIPIENT ONLY PLEASE DIRECT THE CONTENTS ACCORDINGLY

Recipient: John Kerce / Building Official

Of (company): Columbia County Building & Zoning Department

Phone: 386-758-1008

Fax: 386-758-2160

Regarding: Permit 000024882

Comments:

Letter from:

Richard E. Gavaletz, Sr.

Gulf Steel, Inc.

Number of pages including cover sheet: 3

Gulf Steel, Inc. PO Box 1710 Pensacola, FL 32591 800-289-7944 fax 850-497-0302

September 7, 2006

Page 1 of 2

Mr. John Kerce / Building Official Columbia County Building & Zoning Department 135 NE Hernando Ave., Suite B-21 Lake City, FL 32055

Re:

Columbia County Building Permit # 000024882

Donny Williams / Contractor Raymond Logan / Owner

Gulf Steel, Inc. Engineered plans SS-001, SS-002, SS-003, SS-004, SS-006 & SS-007

Dear Mr. Kerce:

The above referenced Gulf Steel jobs are for a project that Gulf Steel, Inc. is under contract to supply pre-engineered buildings for with Mr. Raymond Logan of Mini Storage of Columbia County in Lake City, Florida.

I met with your building inspector yesterday, Mr. Harry Dicks, to discuss certain aspects of this project and to make him aware of certain legal issues that have developed recently regarding this project. Upon your review and as pointed out to Mr. Dicks you will note that the cover page for this project contains a note directed to the appropriate building official that reads:

Note to building Official:
THESE PLANS ARE SUBMITTED BY THE CONTRACTOR FOR
THE PURPOSE OF OBTAINING A BUILDING PERMIT. THIS
BUILDING PACKAGE MUST BE SUPPLIED BY GULF STEEL
IN ORDER TO ASSURE THE SUPPLYING OF REQUIRED
COMPONENTS SIZES, SHAPES, GAUGE AND GRADE OF
STEEL NEEDED TO MEET DESIGN LOAD REQUIREMENTS

Please be aware that the owner of this project, Raymond Logan of Mini Storage of Columbia County, has chosen to acquire a building permit, through Donny Williams, his contractor, to build this project but has chosen to initiate the cancellation of the contractual agreement with Gulf Steel, Inc.

As the president of Gulf Steel, Inc., a Florida Corporation, it is my position that the plans submitted to the building department for this project be immediately withdrawn for use on this project which includes but is not limited to Columbia County Building Permit #000024882, for

Page 2 of 2

the construction of a concrete slab, Building-1 as well as any other permits that may have been issued utilizing the subject sealed plans provided by Gulf Steel, Inc.

This action is being taken due to the fact that the buildings could be constructed using inferior materials that do not meet minimum specifications in order to meet the appropriate design loads as outlined in the Florida Building Code. Furthermore, Gulf Steel, Inc. is under contract to supply building packages and not simply just the plans. As you may or may not be aware, this exposes Gulf Steel, Inc. and Gulf Steel's engineer of record, Mr. Chander Nangia, to a liability we are not willing to accept. This includes all concrete work, such as that under your permit number 000024882 as it is part of the project drawings and therefore is inclusive in this decision. Your acknowledgment of taking the appropriate actions on this matter is greatly appreciated

Sincerely

Richard E. Gavaletz, Sr.

President Gulf Steel, Inc.

Ce: Chander Nangia, PE

Raymond Logan Terry McDavid, PA

COLUMBIA COUNTY BUILDING DEPARTMENT Application 0607-21

RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2004 and FLORIDA RESIDENTIAL CODE 2004 WITH AMENDMENTS ONE (1) AND TWO (2) FAMILY DWELLINGS

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ------ 100 MPH

ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE ------110 MPH X

Square footage of different areas shall be shown on plans. X

Wind-load Engineering Summary, calculations and any details required <u>none</u> <u>provider see note in letter</u>

Basic wind speed (3-second gust), miles per hour (km/hr)

Wind importance factor, Iw, and building classification from Table 1604.5 or Table 6-1, ASCE 7 and building classification in Table 1-1, ASCE 7.

Wind exposure, if more than one wind exposure is utilized, the wind exposure and applicable wind direction shall be indicated.

The applicable enclosure classifications and, if designed with ASCE 7, internal pressure coefficient components and cladding.

The design wind pressures in terms of psf (kN/m²) to be used for the design of exterior component and cladding materials not specially designed by the registered design professional

Elevations including:

All sides **OK**

Roof pitch OK

Overhang dimensions and detail with attic ventilation OK

Location, size and height above roof of chimneys OK

Location and size of skylights NONE

Building height 23'1"

Number of stories TWO

Floor Plan including:

Rooms labeled and dimensioned. YES

Shear walls identified. ON FOUNDATION PLAN

Show product approval specification as required by Fla. Statute 553.842 and Fla. Administrative

Code 9B-72 (see attach forms). No see note in letter

Show safety glazing of glass, where required by code. Bath room #2 noncompliance

Identify egress windows in bedrooms, and size. Not shown on plans

Fireplace (gas vented), (gas non-vented) or wood burning with hearth **OK**

Stairs with dimensions (width, tread and riser) and details of guardrails and handrails **Not Shown**

Must show and identify accessibility requirements (accessible bathroom) Bathroom #2

Foundation Plan including:

Location of all load-bearing walls with required footings indicated as standard or monolithic and dimensions and reinforcing. **ER-6**

All posts and/or column footing including size and reinforcing ER-6

Any special support required by soil analysis such as piling See note for 2,500 Psf

Location of any vertical steel **None**

Roof System:

Truss package including: No seal No floor truss plans

Truss layout and truss details signed and sealed by Fl. Pro. Eng.

Roof assembly (FBC 106.1.1.2) Roofing system, materials, manufacturer, fastening

requirements and product evaluation with wind resistance rating)

Floor truss layout and truss details signed and sealed by Fl. Pro Eng No seal No floor truss plans

Roof assembly (FBC 106.1.1.2) Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating) Yes

Conventional Framing Layout including:

Rafter size, species and spacing

Attachment to wall and uplift

Ridge beam sized and valley framing and support details

Roof assembly (FBC 106.1.1.2)Roofing systems, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

Wall Sections including:

Masonry wall first floor

All materials making up wall Yes

Block size and mortar type with size and spacing of reinforcement Yes

Lintel, tie-beam sizes and reinforcement Yes

Gable ends with rake beams showing reinforcement or gable truss and wall bracing details See

Wood Detail Second Floor

All required connectors with uplift rating and required number and size of fasteners for continuous tie from roof to foundation shall be designed by a Windload engineer using the engineered roof truss plans.

Roof assembly shown here or on roof system detail (FBC 106.1.1.2) Roofing system, materials, manufacturer, fastening requirements and product evaluation with resistance rating)

Fire resistant construction (if required)

Fireproofing requirements

Show type of termite treatment (termiticide or alternative method) Foundation Notes on plans Slab on grade Vapor retarder (6mil. Polyethylene with joints lapped 6 inches and sealed) Foundation Notes on plans

Slab on grade

Must show control joints, synthetic fiber reinforcement or

Welded fire fabric reinforcement and supports YES

Indicate where pressure treated wood will be placed Yes

Provide insulation R value for the following:

Attic space

Exterior wall cavity Yes

Crawl space (if applicable)

Wood frame wall

All materials making up wall Second Floor

Size and species of studs

Sheathing size, type and nailing schedule

Headers sized

Gable end showing balloon framing detail or gable truss and wall hinge bracing detail

All required fasteners for continuous tie from roof to foundation (truss anchors, straps, anchor
bolts and washers) shall be designed by a Windload engineer using the engineered roof truss
plans.

Roof assembly shown here or on roof system detail (FBC 106.1.1.2) Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

Fire resistant construction (if applicable)

Fireproofing requirements

Show type of termite treatment (termiticide or alternative method)

Slab on grade

Vapor retarder (6Mil. Polyethylene with joints lapped 6 inches and sealed

Must show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and

supports

Indicate where pressure treated wood will be placed

Provide insulation R value for the following:

Attic space

Exterior wall cavity

Crawl space (if applicable)

Metal frame wall and roof (designed, signed and sealed by Florida Pro .Engineer or Architect)

Floor Framing System:

Floor truss package including layout and details, signed and sealed by Florida Registered

Professional Engineer

Floor joist size and spacing

Girder size and spacing See note 8

Attachment of joist to girder

Wind load requirements where applicable

Electrical layout including:

Switches, outlets/receptacles, lighting and all required GFCI outlets identified

Ceiling fans

Smoke detectors

Service panel and sub-panel size and location(s) and overcurrent disconnect exterior of dwelling

Meter location with type of service entrance (overhead or underground) See note 9

Appliances and HVAC equipment

Arc Fault Circuits (AFCI) in bedrooms

Exhaust fans in bathroom

HVAC information

Energy Calculations (dimensions shall match plans) See note 10

Manual J sizing equipment or equivalent computation

Gas System Type (LP or Natural) Location and BTU demand of equipment

Disclosure Statement for Owner Builders

Notice of Commencement Required Before Any Inspections Will Be Done

Private Potable Water

Size of pump motor See note 12

Size of pressure tank

Cycle stop valve if used

PERMIT 24974 Dear county? can get another fur menths were so dose to being done and that yer Ed a Sue Rupelle

Columbia county Building permit

Edward Susic Riopelle 386-497-2732 permit number 000024974

WE are Building A House and we have finsh it yet we need plittle more time please.

Ed & Sue Riopelle 807 SW QUARRY CIR Fortwhite fl 32038



OCCUPANCY

COLUMBIA COUNTY, FLORIDA

partment of Building and Zoning nspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 02-7S-16-04111-110

Building permit No. 000024974

Use Classification SFD,UTILITY

Fire: 48.84

Permit Holder OWNER BUILDER

Date: 06/23/2008

Location: 807 SW QUARRY CIRCLE, FT. WHITE, FL

Waste: 67.00

Owner of Building EDWARD & SUE RIOPELLE Total:

otal: 115.84

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)



OGGUPANGY

COLUMBIA COUNTY, FLORIDA

tment of Building and Zonin

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 21-4S-16-03084-014

Building permit No. 000024975

Use Classification MOTHER-IN-LAW SUITE

Waste:

Fire:

0.00

Total:

0.00

Permit Holder J.L. DUPREE, JR.

Owner of Building DENNIS & TAMMY MORSE

AND CALLED

Location: 259 SW MELON COURT, LAKE CITY, FL

Date: 12/14/2006

by Building Inspector

POST IN A CONSPICUOUS PLACE (Business Places Only)

FORM 600A-04	FLORIDA ENERGY Residentia		ODE FOR BUILD Performance Met		JCTION	CENTRA	L456
PROJECT NAME: AND ADDRESS:	Susie Riopelle 807 SW QUARRY Fort white 32038	BUILDER: PERMITTING OFFICE:	Inck Ball	cont,	CLIMATE ZONE: 4	5 6	
OWNER:	ColosuE Rispelle	PERMIT NO.:	- L - E - K	24 (0)	JURISDICTION I	NO.:	
 Single-family of the processing of	-grade (R-value + perimeter) raised (R-value + sq. ft.) e, raised (R-value) area and insulation: 1. Concrete block (Insulation R-value)	if not default)	6.15	8b. 8c. 9a-1	Please T	sq. ft. ft. Area sq. ft. sq. ft. 26 1783 sq. ft.	СК
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compliance with the PREPARED BY:	e plans and specifications covered by the calc brida Energy Code.	E: <u>4 - 5 - 0</u>		code. Before cons cordance with Se	truction is complete ction 553.908, F.S.	ation indicates complia	

Predominant glass type. For actual glass type and areas, see summer and winter glass output on Pages 2 and 4.

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SUMMER POINT MULTIPLIERS (SPM)

CLIMATE ZONES 456

8 INCH

EXT 1.0

.8

6A-1 SUMMER OVERHANG FACTORS (SOF	FOR SINGLE AND DOUBLE-PANE GLASS

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7.7	North	1.00	0.992	0.971	0.931	0.891	0.848	0.811	0.776	0.748	0.695	0.651	0.611
	Northeast	1.00	0.995	0.966	0.908	0.846	0.777	0.719	0.665	0.623	0.549	0.491	0.445
. ≽l	East	1.00	0.993	0.964	0.903	0.835	0:7 65	0.687	0.622	0.571	0.482	0.414	0.463
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ᆲ	South	1.00	0.988	0.935	0.849	0.776	0.708	0.659	0.618	0.588	0.539	0.503	0.475
S	Southwest	1.00	0.997	0.956	0.874	0.793	0.709	0.645	0.588	0.547	0.479	0.431	0.396
Ban of	West	1.00	0.994	0.964	0.902	0.834	0.757	0.691	0.630	0.582	0.500	0.438	0.391
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6A-2 WALL SU	MMER POINT	MULTIPL	IERS	(SPM)
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0-6.9	6.4	2.2	8.9	2.9	0-2.9	2.5	9	2.5	11-18.9	.4	7-9.9	.4	0-2.9	1.7	l
7-10.9	2.3	.8	4.1	1.3	3-4.9	1.4	.7	.7	19-25.9	.2	10 & UP	.2	3-6.9	1.1	l
11-12.9	1.9	.7	3.0	1.0	5-6.9	1.0	.6	.3	26 & UP	.1			7 & UP	.8	l
13-18.9	1.7	.6	2.8	0.9	7-10.9	.8	.4	.1							
19-25.9	1.0	.3	2.4	0.8	11-18.9	.4	.3	0	1 2						
10 2010			4.0	0.4	10.25.0	2	2		7						

NOTE: SEE SECTION 2.0 OF APPENDIX C FOR MULTIPLIERS OF ENVELOPE COMPONENTS NOT ON THIS FORM.

6A-3 DOOR SUM	MER POINT MU	LTIPLIERS (SPM
DOOR TYPE	EXTERIOR	ADJACENT
WOOD	7.2	2.4
	4.0	1.6

6A-4 CEILING SUMMER POINT M	IULTIPLIERS	(SPM)
444000 4000	00101	- 400

UNDER	ATTIC	SINGLE AS	SSEMBLY	CONCRETE DECK ROOF			
A-VALUE	SPM	R-VALUE	SPM	l	CEILIN	G TYPE	
19-21.9	2.82	10-10.9	10.27	#-VALUE	EXPOSED	DROPPED	
22-25.9	2.55	11-12.9	9.73	10-13.9	11.13	10.40	
26-29.9	2.28	13-18.9	8.72	14-20.9	8.42	7.99	
30-37.9	2.13	19-25.9	6.90	21 & UP	5.99	5.76	
38 & UP	1.84	26-29.9	5.82		E. A. Person		
RBS Credit	0.700	30 & Up	5.40				
RCC Credit	0.864				18		

ELOOR SUMMER	POINT MULTIPLIERS	I SECRET W	hite Roof Credit	0.550			
- Jali		S1			RAIS	ED WOOD	
SLAB-ON- EDGE INSI		RAIS		# 14(S	POST OR PIER CONSTRUCTION	STEM WALL W/UNDER FLOOR INSULATION	ADJACENT
R-VALUE	SPM	A-VALUE	SPM	A-VALUE	SPM	SPM	SPM
0-2.9	-31.9	0.0-2.9	-1.0	0-6.9	4.50	5.8	5.3
3-4.9	-31.8	3-4.9	-1.7	7-10.9	2.28	-2.8	2.1
5-6.9	-31.7	5-6.9	-1.7	11-18.9	1.83	-2.2	1.8
78110	-316	7.8 UP	-1.7	19 & UP	1.36	-1.8	1.0

6A-6 INFILTRATION & INTERNAL GAINS (SF	PM)	ŀ
---	-----	---

Air intitration	3.17
Internal Gains	49.14
Infiltration/Internal Gains (Compined)	14.31
BA-7 AIR HANDLER MULTIPLIERS (SPM)	
Located in garage	1.00
Located in conditioned area	0.90
Located on exterior of building	1.02
Lecated in attic	1 10

6A-8 DUCT MULTIPLIERS (DM	See Table 13-610.1.ABC.2.1 for Code minimums.
---------------------------	---

	DUAT		RET	URN DUCTS	In:	
SUPPLY DUCTS IN:	A-VALUE	Unconditioned space	Attic/ RBS	Attic/ IRCC	Attic/ White roof	Conditioned space
	4.2	1.113	1.107	1.108	1.107	1.103
Unconditioned Space	6.0	1.087	1.081	1.083	1.081	1.079
	8.0	1.069	1.064	1.065	1.064	1.062
	4.2	1.072	1.066	- A	_	1:061
Attic/Radiant Barrier (RBS)	6.0	1.056	1.051	× -		1.047
411	8.0	1.045	1.041	111		1.038
1000	4.2	1.098	_	1.092	_	1.084
Attic/Interior Radiation Control	6.0	1.076		1.071		1.065
Coatings (IRCC)	8.0	1.060	- I	1.057		1.052
7 9 =	4.2	1.069	_	_	1.063	1.058
Attic/White Roof	6.0	1.052			1.047	1.044
	8.0	1.041	_	45-	1.037	1.034
The second	4.2	1.006	1.005	1.007	1.003	1.000
Conditioned Space	6.0	1.005	1.004	1.005	1.002	1.000
	8.0	1.004	1.003	1.004	1.002	1.000

6A-9 COOLING SYSTEM MULTIPLIERS (CSM)

SYSTEM TYPE See Table 13-807.1.A	BC.3.2 A,B,D for code minimume		COOLING SYSTEM MULTIPLIERS (CSM)								N.	
	Rating	4 Å ¹ 4	7.5-7.9	8.0-8.4	8.5-8.8	8.9-9.4	9.5-9.9	10.0-10.4	10.5-10.9	11.0-11.4	11.5-11.9	12.0-12.4
Central Units (SEER)	CSM	41 100	.45	.43	.40	.38	.36	.34	.32	31 /	.30	.28
i: =	Rating	12.5-12.9	13.0-13.4	13.5-13.9	14.0-14.4	14.5-14.9	15.0-15.4	15.5-15.9	16.0-16.4	16.5-16.9	17.0-17.4	17.5 & UP
PTAC & Room Units (EER)	CSM	.27	.26	.25	.24	.24	.23	.22	.21	.21	.20	.19

NIN.	TER C	ALCUI	ATION	IS			200	-11				ONES 4 5 6
- T					OVERHANG	GLASS	SINGLE-PA	NE WINTER		PANE WINTER MULTIPLIER	WINTER Y OH FACTOR	AS-BUILT
- 1				ORIENTATIO	ON LENGTH OH (FEET)	AREA (SQ. FT.)	CLEAR	TINT	CLEAR	TINT*	(from 6A-10	
				N	1	26.95	15.07	15.38	11.00	11.29	1992	1327
				NE NE	-6	30.7	14.70	15.07	10.70	11.04		
				E	6	81.7	12.37	13.04	8.82	9.46	1.104	1178
			•	SE	-	-	10.59	11.49	7.31	8.18	,	
1			1	s	6	97.7	9.90	10.88	6.74	7.70	1.862	1660
		11 I		sw			11.59	12.36	8.12	8.86		
		Ų, Į		w	1	63	13.25	13.80	9.55	10.07	, 929	869
Ì		F-L-4	н	NW	1		14.97	15.30	10.91	11.21		
		ſ	ī	'Н			14.78	15.61	10.20	11.01		
ļ		Ъ	<u>+</u>	11	1	1816		15.38			: 998	786
SS				-	1	50.0		10.88			1994	541
GLASS				>		30.0						
9		^										
		//>										
	//:	_										
	4 :		<i>>></i>			<u> </u>						
		П				1	-	+				
		1			+	1	 					-
						11						
						-	-	-		_		
						-		+		_		
						11	 		+			
						 - - - - - - -		+			-	
						-	,					
			10									
		74.5			÷2		1					AS-BUILT
ဟ္က	.18 X	COND.	EA X	WEIGHTE	D GLASS PLIER	BASE GLASS SUBTOTAL	ļ				GL	ASS SUBTOTAL
GLASS	 	FLOOR AF	IEA T			6449	1					5861
	.18	611		5.			J					V
				DAG	E WINTER POINT	BASE WINTER	COM	PONENT		WINTER PO	INT MULT.	AS-BUILT
	COMPONE		ARE	A BAS	MULT.	POINTS		CRIPTION	AREA)	(6A-11 THR		INTER POINTS
	DESCRIPT		190	2	2.0	3566			1783	2.	1	5171
ا با		RIOR			1.8	200	1					
WALL	ADJA	CENT	16 1	7		3042	1		1521	210)	3042
>			153	7	20	200	1		7 7 7			
L						_						
Γ.,	EXT	ERIOR	20		5.1	102			20	5.1		102
DOORS		ACENT	20	- :	4.0							
١ğ	ALLOY	CENT					1					
						-						V
[47	LINDER	ATTIC OR	36	דקת	0.64	2353		-10-6	3657	v 63		1353
CEILING	SI	NGLE	30			-	PBS/IB	CC/white roof ³		_ x_		
	ASS	EMBLY	l						A FOLIAL S AC		SOLIABE ECOT	AGE
Lo		BASE C	EILING ARE	A EQUALS F	OOR AREA DIRE	CTLY UNDER CEIL	ING, AS-BUIL	CEILING ARE	A ECOALS AC	A OAL CEILING	JOGORNE FOOT	AGE.
	1		0.7	U	-10	- 502	7		264	a.	5	660
5	_	(PERIMETER)	76	7	-1.9	100	1		1			
FLOOR	RAIS	ED (AREA)	1		2	L ROUND CONDITION	NED EL COR 4	OD DAIGED E	LOORS LISE A	REA OVER LIN	CONDITIONED	SPACE.
		FOR SLAI	3-ON-GRADE	USE PERIM	EIER LENGTH AF	TOUND CONDITION	NEU FLOOR.	ON FINISED P	LOUIS USE P			V
	9		111			17/2			12114	1 .0	.28	-1712
	INFILTRAT		61	4	-0.28	- 1712		A OF CONDITI	ONED SPACE	1 -0		
	INTERNAL	GAINS						A OF CONDITI	UNED SPACE.			15.107
aete						13298	11 -	OTAL COMPO	MENT AC DITT	T WINTER PO	INTS	15477
	то	TAL COMP	ONENT BAS	E WINTER PO	JINTS		1 📗 <u>."</u>	CIAL COMPU	TENT AS BUIL			
					▼-		, <u> </u>		T		- II II	1 AC DUNT
			Heating	Ĵ	Total Base	BASE	TOTA AS-BU		As-Built X DSM X		Built As Built ISM X HCM	
н	EATING		lystem ultiplier	^	Winter Points	HEATING POINTS	WIN. P				A-18) (6A-21)	
	YSTEM	- W		1,2					4:16-or	192 .1	116 0-	6708
			.63	13	298	8378	154	771078	1.0	170 16	16 .95	10108
=	T	т									N. W. Z. LIGT	TOTAL
	BASE C		BASE		BASE HOT	TOTAL BASE	AS COOL	S-BUILT NG POINTS +	AS-BUILT HEATING PO	I AS-E INTS + WATE	BUILT HOT ER POINTS ==	AS-BUILT
TOTAL	POII (From		HEATIN POINT	lG +Fy S I	VATER POINTS = (From P. 2)	POINTS (Enter on P. 1)		rom P.2)			rom P. 2)	POINTS (Enter on P. 1)
1 2								023	67A	2 0	304	43035
L	523	15	837		1840	10531			670		100	73001
		UTAL C: 12	1010/1:01:	r FOR	GLASS WITH KN	OWN SHGC, SEE	SECTION 2.	1.1 APPENDI	K C. TINT	1 3 _{MU}	ST MEET CRITE	RIA OF S.607.1A.
'H	= HORIZOI	NIAL GLASS	S (SKYLIGHT	S) MULTI	PLIERS MAY BE	USED FOR GLAS	S WITH SOL	AR SCREENS	, FILM, OR T	NT.		
_												Page 4

WINTER POINT MULTIPLIERS (WPM)

CLIMATE ZONES 4 5 6

	OH Ratio	.0011	.1217	.1826	.2735	.3646	.4757	.5870	.7183	.84-1.18	1.19-1.72	1.73-2.73	2.74 & up
	North	1.00	0.998	0.996	0.995	0.995	0.994	0.993	0.992	0.990	0.988	1.986	0.984
	Northeast	1.00	1.000	1.001	1.001	1.001	1.001	1.001	1.001	1.001	1.001	1.001	1.000
	East	1.00	1.005	1.010	1.020	1.034	1.055	1.078	1.106	1.133	1.198	1.264	1.320
ا <u>.</u>	Southeast	1.00	1.010	1.025	1.058	1.102	1.167	1.238	1.324	1.407	1.596	1.783	1.939
SELE OR	South	1.00	0.994	1.011	1.062	1.040	1.262	1.400	1.562	1.709	1.992	2.192	2.291
1	Southwest	1.00	1.002	1.013	1.038	1.071	1.118	1.168	1.225	1.278	1.388	1.490	1.573
	West	1.00	0.999	1.003	1.013	1.025	1.040	1.053	1.067	1.077	1.095	1.107	1.116
	Northwest	1.00	0.999	0.998	0.997	0.997	0.996	0.995	0.994	0.993	0.992	0.990	0.989
	OH Length	0.0'	1.0'	1.5'	2.0'	3.0'	3.5'	4.5'	5.5'	6.5'	9.5'	14.0'	20.0

6A-11 WALL WINTER POINT MULTIPLIERS (WPM)

					CONCE	RETE BLO	CK (NORM	AL WT)		FACE	BRICK		ļ	LOG	
		FRAME			I ſ	INTE	RIOR	EXT.	A-VALUE	WOOD FR	A-VALUE	BLOCK	= 11	LOG	
	WC	OOD	ST	EEL	1	INSUL	ATION	INSUL.	0-6.9	7.0	0-2.9	3.7	====	6 INCH	8 INCH
A-VALUE	EXT	ADJ	EXT	ADJ	R-VALUE	EXT	ADJ	EXT	7-10.9	2.1	3-6.9	2.6	A-VALUE	EXT	EXT
0-6.9	6.8	5.3	9.4	6.7	0-2.9	6.0	3.1	6.0	11-18.9	1.7	7-9.9	1.8	0-2.9	2.2	1.2
7-10.9	2.5	2.1	4.4	3.3	3-4.9	3.8	2.3	2.8	19-25.9	1.0	10 & UP	1.3	3-6.9	1.2	.9
11-12.9	2.0	1.8	3.3	2.6	5-6.9	2.9	1.9	2.0	26 & UP	.6			7 & UP	.9	.7
13-18.9	1.8	1.6	3.0	2.4	7-10.9	2.3	1.5	1.5						100000	
19-25.9	1.1	1.0	2.6	2.2	11-18.9	1.5	1,1	.8	20						
	-	-	4.4	4.0	40.000	0	7								

.5 ENVELOPE COMPONENTS NO

6A-12 DOOR WIN	ITER POINT MU	LTIPLIERS (WPN
DOOR TYPE	EXTERIOR	ADJACENT
WOOD	7.6	5.9
INSULATED	5.1	4.0

UNDER	ATTIC	SINGLE AS	SEMBLY	CONCRETE DECK ROOF					
A-VALUE	WPM	R-VALUE	WPM	1	CEILIN	G TYPE			
19-21,9	.87	10-10.9	1.02	A-VALUE	EXPOSED	DROPPED			
22-25.9	.78	11-12.9	.96	10-13.9	1.16	1.05			
26-29.9	.69	13-18.9	.84	14-20.9	.83	.76			
30-37,9	.64	19-25.9	.62	21 & UP	.54	.50			
38 & UP	.55	26-29.9	.50	1.0					
BS Credit	0.850	30 & UP	.46						
RCC Credit	0.905								

6A-14 FLOOR WINTER POINT MULTIPLIERS (WPM)

		B	1050	語	RAIS	ED WOOD		
	N-GRADE SULATION		ISED CRETE		POST OR PIER CONSTRUCTION	STEM WALL W/UNDER FLOOR INSULATION	ADJACENT	
R-VALUE	WPM	#-VALUE	WPM	#-VALUE	WPM	WPM	WPM	
0-2.9	2.5	0-2.9	4.0	0-6.9	2.49	1.8	5.3	
3-4.9	-1.7	3-4.9	1.8	7-10.9	0.78	.7	2.1	
5-6.9	-2.4	5-6.9	1.1	11-18.9	0.47	.5	1.8	
7 & UP	-2.7	7 & UP	.8	19 & UP	0.14	ii .3 N	1.0	

Air Inflitration	0.87
Internal Gains	-1.15
Infiltration/Internal Gains (Combined)	-0.28
BA-16 AIR HANDLER MULTIPLIERS (WPM)	12 4 14 EU
Located in garage	1.00
Located in conditioned area	0.92
Located on exterior of building	1.09
Located in attic	1.11

	DUCT		RET	URN DUCTS	In:	
SUPPLY DUCTS IN:	R-VALUE	Unconditioned space	Attic/ RBS	Attic/ IRCC	Attic/ White roof	Conditioned space
	4.2	1.107	1.098	1.100	1.102	1.092
Unconditioned Space	6.0	1.078	1.072	1.074	1.075	1.068
	8.0	1.061	1.056	1.057	1.058	1.052
	4.2	1.076	1.067		1000	1.059
Attic/Radiant Barrier (RBS)	6.0	1.058	1.051	7 7 7	T	1.045
	8.0	1.046	1.041		_	1.036
	4.2	1.097	JI	1.088		1.077
Attic/Interior Radiation Control Coatings (IRCC)	6.0	1.073		1.066	. –	1.057
Coalings (InCC)	8.0	1.057		1.052	LBIT	1.045
25 7 7	4.2	1.120		4	1.110	1.095
Attic/White Roof	6.0	1.088	_		1.081	1.070
	8.0	1.068		_	1.063	1.054
	4.2	1.009	1.008	1.010	1.009	1.000
Conditioned Space	6.0	1.007	1.006	1.007	1.007	1.000
	8.0	1.005	1.005	1.006	1.005	1.000

6A-18 HEATING SYSTEM	MULTIPLIERS (HSI	M)	10 10 10		8.0	1.005	1.005	1.006 1.008	1.000					
SYSTEM TYPE See Table 13-61 12-608.1 ABC-3.2 EF for Code minims			HEATING SYSTEM MULTIPLIERS (HSM)											
	HSPF	6.40-6.79	6.80-6.89	6.90-7.39	7.40-7.89	7.90-8.39	8.40-8.89	8.9-9.39	9.4-9.89					
	HSM	.53	.50	.49	.46	.43	.41	.38	.36					
Central Heat Pump Linits	HSPF	9.90-10.39	10.40-10.89	10.90-11.39	11.40-11.89	11.90-12.39	12.40 & UP	7						
Α.	HSM	.34	.33	.31	.30	.29	.28		= 1					
PT 10	COP	2.50-2.69	2.70-2.89	2.90-3.09	3.10-3.29	3.30-3.49	3.50-3.69	3.70-3.89	3.90-4.19					
PTHP	HSM	.40	.37	.34	.32	.30	.29	.27	.26					

ADDITIONAL TABLES

SYSTEM TYPE	Cooling credit multipliers (CCM)
Ceiling Fans	.95*
Cross Ventilation	.95*
Whole House Fan	.95*
Multizone	.95
Programmable Thermostat	.95

⁶A-20 AIR DISTRIBUTION SYSTEM CREDIT MULTIPLIERS

TYPE CREDIT	Prescriptive requirements	Multiplier
Air-tight Duct Credit	610.1.A.1	1.00
Factory-sealed AHU Credit ²	610.2.A.2.1	0.95

CLIMATE ZONES 456

6A-21 HEATING CREDIT MULTIPLIERS (HCM)

SYSTEM TYPE		HEATING CREDIT MULTIPLIERS (HCM)							
Programmable Thermostat	нсм	.95							
Multizone	нсм	.95							
	AFUE	.6872	.73-,77	.7882	.8387	.8892	.93 & Up		
Natural Gas	нсм	.61	.56	.53	.50	.47	.44		
LP-Gas	нсм	.77	.72	.67	.63	.60	.57		

64-22 HOT WATER MILLTIPLIERS (HWM)

SYSTEM TYPE See Table 13-612 1 ABC.3.2 for code minimums		HOT WATER MULTIPLIERS (HWM)										
	EF	1			.8081	.8283	.8485	.8687	.8890	.9193	.9496	.97 &Up
Electric Resistance	HWM				2820	2752	2685	2624	2564	2479	2400	2326
	EF	.4347	.4849	.5051	.5253	.5455	.5657	.5859	.6061	.6263	.6465	.66 &Up
Natural Gas	HWM	2162	1936	1859	1787	1721	1660	1602	1549	1499	1452	1408
LP-Gas	HWM	2645	2368	2274	2186	2106	2031	1960	1895	1834	1776	1722
	EF	1.0-1.49	1.5-1.99	2.0-2.49	2.5-2.99	3.0-3.49	3.5-3.99	4.0-4.49	4.5-4.99	5.0-Up		
Ded. HP or Solar System with Tank	HWM	2256	1504	1128	902	752	645	564	501	451		

6A-23 HOT WATER CREDIT MULTIPLIERS (HWCM)

SYSTEM TYPE	HOT WATER CREDIT MULTIPLIERS (HWCM)						
	With	Air Con	ditioner	Heat Pump			
Heat Recovery Unit	нисм	.8.	4			.78	
Add-on Dedicated Heat Pump (without	EF	2.0-2.49	2.5-2.99	3.0-3.49		3.5 & Up	
tank)	HWCM	.44	.35	.29		.25	
	EF	1.0-1.9	2.0-2.9	3.0-3.9	4.0-4.9	5.0 & Up	
Add-on Solar Water Heater (without tank)	HWCM	.84	.42	.28	.21	.17	

NOTE: An HWM must be used in conjunction with all HWCM. See Table 6A-22. EF Means Energy Factor.

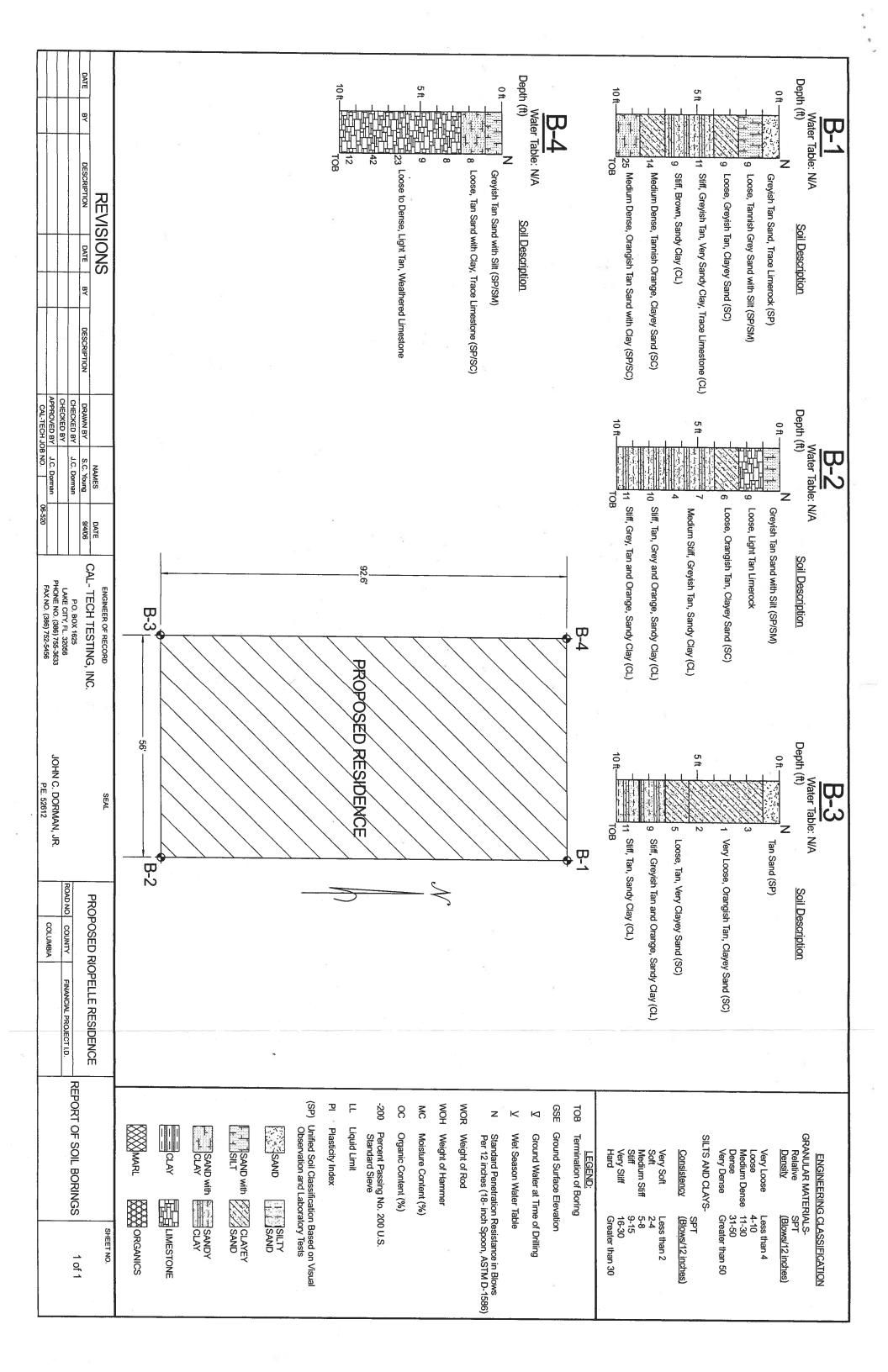
COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Max: 3 cfm/sq. ft. window area; .5cfm/sq. ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at comers; utility penetrations; between wall panels & top/bottom plates; between walls & floor. EXCEPTION: Frame walls where a continous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings > 1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Seal: Between walls & ceilings: penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with <2.0 cfm from conditioned space, tested.	
Multistory Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration regts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-25 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required for vertical pipe risers.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower Heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 minimum insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

¹Duct Sealing Multiplier (DSM) shall be 1.15 (summer) or 1.16 (winter) unless Air-tight Duct Credit is demonstrated by test report.

²Multiply Factory-sealed AHU Credit by summer (Table 6A-7) or winter (Table 6A-16) AHU multiplier. Insert total in the "As Built AHU" box on page 2 or 4.



Architectural Services and Engineering, Inc 24710 SR 54, Lutz, FI 33559 Phone: 813-948-2812 EBO 7882

Engineering Index Sheet

Truss Fabricator: Builders First Source Tampa, FI 33619

Permit Number:	Lot Number:
Customer Info: The Information in this box is for administrative	Address:e purpsoes only and not part of the engineering review.

Software: MiTek Industries, Inc. Refer to sheets for version number. Loading: Refer to sheets Typical Floor 55 psf, Typical Roof 37 psf

Job Number

RIOPE

Specification Quantity

30

A Professional Engineer's seal on to this Index Sheet indicates the acceptance of Professional Engineering responsibilities for individual truss components fabricated in accordance with the listed and attached Truss Specification Sheets. Determination as to the suitability of these individual truss components for any structure is the responsibility of the Building Designer, as defined in ANSI/TPI 1-2002, Section 2.2. Permanent files of the original Truss Specification Sheets are maintained by Architectural Services and Engineering, Inc. Questions regarding this Index Sheet and/or the attached Specification Sheets may be directed to the truss fabricator listed above .

Truss ID AH11 AH13 AH15 AH17 AH19	Truss ID CJ1 CJ3 CJ5 EJ5 EJ7	Truss ID	Truss ID	Truss ID	Truss ID	<u>Truss ID</u>
AH21 AH7 AH9 B BH11 BH13 BH15 BH17 BH19 BH21 BH21 BH9	HJ5 HJ7 MGR MH5 MT PB PB1 PB2					

ENGINEER OF RECORD Not Specified

Robert W. Wall P.E. F. Reg. 46021

photo blala.

Architectural Services and Engineering, Inc.

Florida

24710 State Road 54

Lutz, Florida 33559

1-813-948-2812 FAX: 1-813-949-2016

Florida engineering license CA 7882

RIOPE

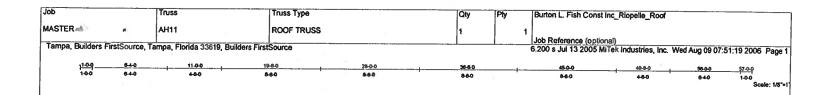
Texas
3000 Sage Road, Suite 1374
Houston, Texas 77056
1-713-963-8840 FAX: 1-713-963-9840
Texas engineering license 95105

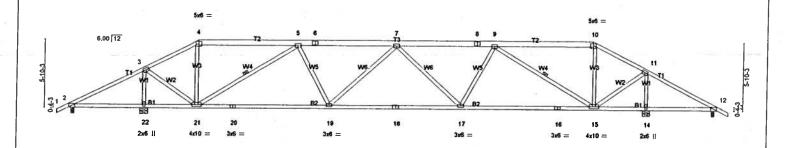
E-Mail: office@asande.com
Designers and engineers since 1965

TRUSS REVIEW COVER SHEET

Job Number Date Checked By Date Checked Received 8-9.06 Rya 8/9/06

ě	Hold (date)
	Number of Trusses30
	Number of Raised Sealed Copies
8	
	Number of Flat Sealed Copies
٠	Cover Sheet
*	(3)
	BFS-TAMPA Mailed daily. <u>UPS GROUND</u> only.
	Ivialied daily. Of a dicourd only.





0- <u>2</u> -			240	33-8-0	4500	49-5-0	55-9-8	56,00
0-2-6			11-4-0	11-40	11-4-0	480	6-1-8	0-2-8
te Offsets (X,Y): [4:0-3-	-0,0-2-7], [10:0-3-0,0-2-7]						
ADING (psf)		SPACING 2-0-0	CSI	DEFL in	(loc) Vdefi L/d	PLATES	CDID	
L 20.0		Plates Increase 1.25	TC 0.48	Vert(LL) -0.32			GRIP 244/190	
DL 7.0	1	Lumber Increase 1.25	BC 0.91	Vert(TL) -0.86		MIZU	244/190	
LL 10.0	•	Rep Stress Incr YES	WB 0.89	Horz(TL) 0,10	14 n/a n/a			
DL 10.0		Code FBC2004/TPI2002	(Matrix)	1.0.2(.2)		Weight: 312 lb		

LUMBER

TOP CHORD 2 X 6 SYP No.2 "Except"

T1 2 X 4 SYP No.2, T1 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3 BRACING

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 5-1-2 oc purlins. Rigid ceiling directly applied or 4-7-12 oc bracing.

WEBS

1 Row at midpt 5-21, 9-15

REACTIONS (lb/size) 22=2450/0-8-0, 14=2450/0-8-0, 2=-326/0-3-0, 12=-326/0-3-0

Max Horz 2=-113(load case 7)
Max Uplift22=-1810(load case 5), 14=-1782(load case 4), 2=-480(load case 11), 12=-480(load case 10)
Max Grav22=2450(load case 1), 14=2450(load case 1), 2=262(load case 5), 12=268(load case 5)

FORCES (lb) - First Load Case Only

TOP CHORD BOT CHORD

- First Load Case Only 1-2=23, 2-3=1346, 3-4=-656, 4-5=-537, 5-6=-2271, 6-7=-2271, 7-8=-2271, 8-9=-2271, 9-10=-537, 10-11=-656, 11-12=1346, 12-13=23 2-22=-1122, 21-22=-1122, 20-21=2035, 19-20=2035, 18-19=2501, 17-18=2501, 16-17=2035, 15-16=2035, 14-15=-1122, 12-14=-1122 3-22=-2334, 3-21=2031, 4-21=-58, 5-21=-1773, 5-19=514, 7-19=-320, 7-17=-320, 9-17=514, 9-15=-1773, 10-15=-58, 11-15=2031, 11-14=-2334

WEBS

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; partially; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and

for MWRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

4) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

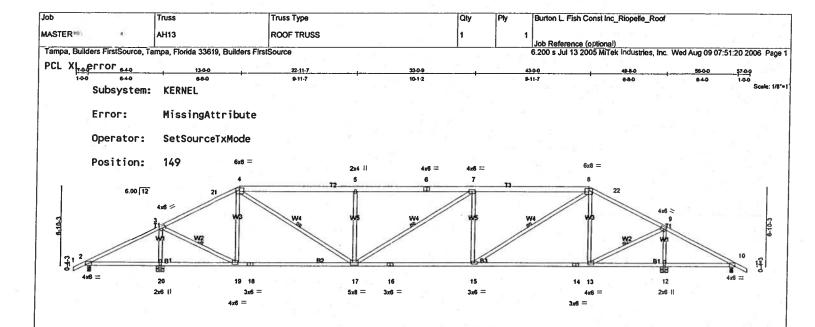
5) All plates are 4x6 MT20 unless otherwise indicated.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1810 lb uplift at joint 22, 1782 lb uplift at joint 14, 480 lb uplift at joint 2 and 480 lb uplift at joint 12.

LOAD CASE(S) Standard



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- 11	40 3-0	9-11-7	33-0-9	43-0-0 9-11-7	680	6-1-0	0-2-8
Dista Officeto /V V)+ (4							
-iate Oliseis (A,1). [4.	0-4-0,0-1-15], [8:0-4-0,0-1-15]						
.OADING (psf)	SPACING 2-0-0	CSI	DEFL is	n (loc) l/defl L/d	PLATES	GRIP	
CLL 20.Ó	Plates Increase 1.25	TC 0.51	Vert(LL) 0.24	15-17 >999 240	MT20	244/190	
CDL 7.0	Lumber Increase 1.25	BC 0.76	Vert(TL) -0.56	5 13-15 >925 180			
BCLL 10.0 pr	Rep Stress Incr YES	WB 0.74	Horz(TL) 0.06	5 12 n/a n/a			
3CDL 10.0	Code FBC2004/TPI2002	(Matrix)			Weight: 318 II		

LUMBER

TOP CHORD 2 X 4 SYP No.2 "Except" T2 2 X 6 SYP No.2, T3 2 X 6 SYP No.2

BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3

BRACING TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 4-10-9 oc purins. Rigid ceiling directly applied or 5-0-11 oc bracing.

1 Row at midpt 3-19, 4-17, 7-17, 8-15, 9-13

WEBS

REACTIONS (lb/size) 20=2188/0-8-0, 12=2195/0-8-0, 2=-65/0-3-0, 10=-71/0-3-0

Max Horz 2=-115(load case 7)

Max Uplift20=-1564(load case 5), 12=-1526(load case 4), 2=-212(load case 11), 10=-217(load case 10)

Max Grav20=2188(load case 1), 12=2195(load case 1), 2=63(load case 5), 10=56(load case 5)

FORCES (b) - First Load Case Only
TOP CHORD
BOT CHORD
WEBS

To Residual Case Only
TOP CHORD
BOT CHORD
TOP CHORD
TOP CHORD
BOT CHORD
TOP CHORD
BOT CHORD
TOP CHORD
TOP

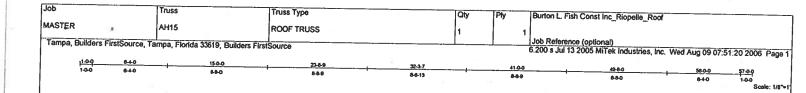
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; partially; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

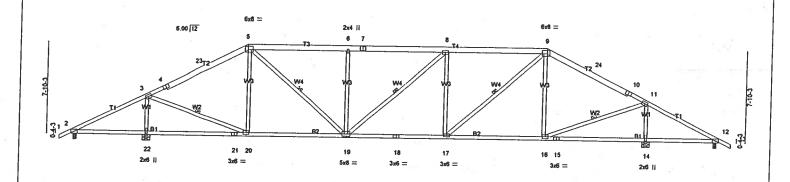
3) Provide adequate drainage to prevent water ponding.
4) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1564 lb uplift at joint 20, 1526 lb uplift at joint 12, 212 lb uplift at joint 2 and 217 lb uplift at joint 10.

LOAD CASE(S) Standard



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	. [10:0-3-0,Edge]			8-8-0	6-1-8 0-2-8	
SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr YES Code FBC2004TPI2002	CSI TC 0.57 BC 0.59 WB 0.56	DEFL Vert(LL) Vert(TL) Horz(TL)	in (loc) Vdefi L/d 0.20 19 >999 240 -0.35 17-19 >999 180 0.06 14 n/a n/a	PLATES MT20	GRIP 244/190	
_	Plates Increase 1.25 Lumber Increase 1.25	Plates Increase 1.25 TC 0.57 Lumber Increase 1.25 BC 0.59 Rep Stress Incr YES WB 0.56	Plates Increase 1.25 TC 0.57 Vert(LL) Lumber Increase 1.25 BC 0.59 Vert(TL) Rep Stress Incr YES WB 0.56 Horz(TL)	Plates Increase 1.25 TC 0.57 Verl(IL) 0.20 19 >999 240	Plates Increase 1.25 TC 0.57 Vert(LL) 0.20 19 >999 240 MT20	Plates Increase 1.25 TC 0.57 Vert(LL) 0.20 19 >999 240 MT20 244/190 Rep Stress Incr YES WB 0.56 Horz(TL) 0.06 14 n/a n/a

TOP CHORD 2 X 6 SYP No.2 *Except*

T1 2 X 4 SYP No.2, T1 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3 **BRACING**

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 4-8-14 oc purlins. Rigid ceiling directly applied or 5-4-15 oc bracing.

1 Row at midpt 3-20, 5-19, 8-19, 9-17, 11-16

WEBS

REACTIONS (lb/size) 22=2013/0-8-0, 14=2012/0-8-0, 2=111/0-3-0, 12=111/0-3-0

Max Horz 2=-123(load case 4)
Max Uplift22=-1382(load case 5), 14=-1335(load case 4), 2=-224(load case 6), 12=-266(load case 7) Max Grav22=2013(load case 1), 14=2012(load case 1), 2=119(load case 10), 12=119(load case 11)

FORCES (Ib) - First Load Case Only TOP CHORD 1-2=23, 2-3=361, 3-4 BOT CHORD 2-22=-243, 21-22=-24

FIRST LOBG CASE O'NY
1-2=23, 2-3=361, 3-4=-1669, 4-23=-1570, 5-23=-1552, 5-6=-2137, 6-7=-2137, 7-8=-2137, 8-9=-2137, 9-24=-1552, 10-24=-1570, 10-11=-1669, 11-12=361, 12-13=23
2-22=-243, 21-22=-243, 20-21=-243, 19-20=1402, 18-19=2137, 17-18=2137, 16-17=1402, 15-16=-243, 14-15=-243, 12-14=-243
3-22=-1829, 3-20=1759, 5-20=-430, 5-19=968, 6-19=-458, 8-19=-0, 8-17=-458, 9-17=968, 9-16=-430, 11-16=1759, 11-14=-1828

WEBS NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02: 120mph (3-second gust): h=25ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; partially; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and

cantilever left and right exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for G-C for members and lorces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

4) 'This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

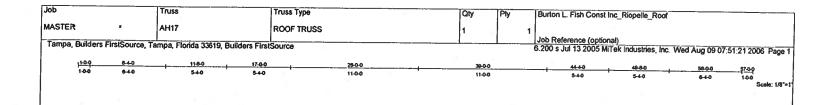
5) All plates are 4x6 MT20 unless otherwise indicated.

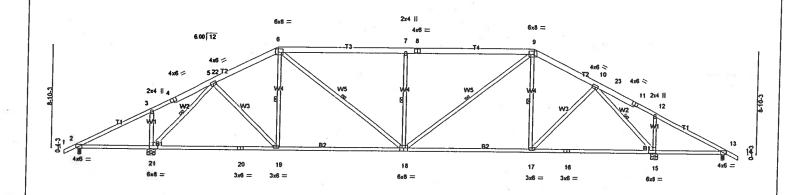
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1382 ib uplift at joint 22, 1335 lb uplift at joint 14, 224 lb uplift at joint 2 and 266 lb uplift at loint 12

LOAD CASE(S) Standard



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028	640	17-8-6	28-0-0	39-0-0		49.8-0	55-9-8	56-0-0
1	6-1-8	10-8-0	11-0-0	11-0-0		10-8-0	6-1-8	0-2-0
Plate Offsets (X,Y	<u>): [4:0-3-0</u>	,Edge], [6:0-5-4,0-3-8], [9:0-5-4,0-3-8], [11:0-3-0,Edge], [15:0-3-8,0-3-0	[, [18:0-4-0,Edge], [21:0-3-8,0-3-0]				
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 10.0 BCDL 10.0	.	SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr YES Code FBC2004/TPI2002	CSI TC 0.52 BC 0.73 WB 0.62 (Matrix)	DEFL in (loc) I/def Vert(LL) -0.20 19-21 >999 Vert(TL) -0.51 19-21 >999 Horz(TL) 0.09 15 r/e	240 180	PLATES MT20	GRIP 244/190	
LUMBER			()	PRACING		Weight: 341 I	0	

TOP CHORD 2 X 6 SYP No.2 *Except* T1 2 X 4 SYP No.2, T1 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3

TOP CHORD **BOT CHORD** WEBS

Structural wood sheathing directly applied or 5-2-11 oc purlins.

Rigid ceiling directly applied or 6-0-0 oc bracing.

1 Row at midpt 5-21, 6-19, 6-18, 7-18, 9-18, 9-17, 10-15

REACTIONS (lb/size) 21=2048/0-8-0, 15=2048/0-8-0, 2=76/0-3-0, 13=76/0-3-0

Max Horz 2=-141((load case 4)
Max Uplift21=-1360(load case 6), 15=-1330(load case 7), 2=-197(load case 6), 13=-227(load case 7)
Max Grav21=2048(load case 1), 15=2048(load case 1), 2=87(load case 10), 13=87(load case 11)

FORCES (lb) - First Load Case Only
TOP CHORD
TOP CHORD
BOT CHORD
BOT CHORD
WEBS
1-2=23, 2-3=431, 3-4=274, 4-22=375, 5-22=377, 5-6=-1628, 6-7=-1981, 7-8=-1980, 8-9=-1981, 9-10=-1628, 10-23=377, 11-23=375, 11-12=274, 12-13=431, 13-14=23
2-21=-308, 20-21=1043, 19-20=1043, 18-19=1435, 17-18=1435, 16-17=1043, 15-16=1043, 13-15=-308
3-21=-376, 5-21=-1993, 5-19=574, 6-19=-198, 6-18=688, 7-18=-623, 9-18=688, 9-17=-197, 10-17=574, 10-15=-1993, 12-15=-376

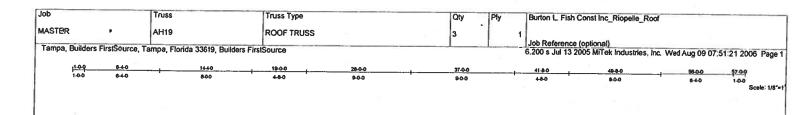
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; partially; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

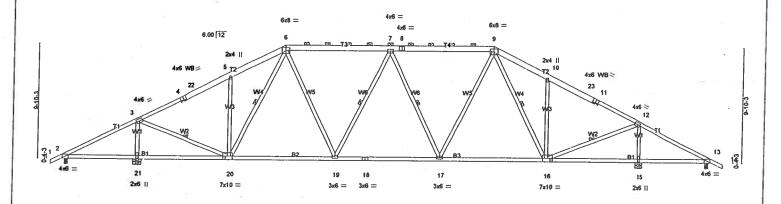
3) Provide adequate drainage to prevent water ponding.
4) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1360 lb uplift at joint 21, 1330 lb uplift at joint 15, 197 lb uplift at joint 2 and

LOAD CASE(S) Standard



Robert W. Wall, PE 46021 24710 State Road 54 Lutz, FI 33559





028	6-1-8	8-0-0	91-5	915		9-1-5	800	55-9-8 6-1-8	56,00 0-2-8
Plate Offsets (X,Y)	: [3:0-0-0,0-0-	O], [4:0-3-0,Edge], [5:	:0-0-0,0-0-0], [6:0-0-0,0-0-0],	. [7:0-0-0,0-0-0], [8:0-0-0,0-0-0	. [9:0-0-0,0-0-0]. [10	0:0-0-0.0-0	0], [11:0-3-0,Edge], [12:0-0-0,0-0	0], [16:0-2-8,E	dge], <u>[20:0-2-8</u>
OADING (psf) CLL 20.0 CDL 7.0 CLL 10.0 CDL 10.0		Plates Increase 1		0.60 Vert(L 0.59 Vert(7 0.63 Horz(L) 0.15 19-20 L) 0.12 13-15	Vdefi L >999 24 >621 18 n/a n	0 MT20	GRIP 244/190	

TOP CHORD 2 X 6 SYP No.2 *Except*

T1 2 X 4 SYP No.2, T1 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3 BRACING TOP CHORD

WEBS

Structural wood sheathing directly applied or 4-6-11 oc purlins, except 2-0-0 oc purlins (5-10-15 max.): 6-9.

BOT CHORD

Rigid ceiling directly applied or 6-0-0 oc bracing.

1 Row at midpt 3-20, 6-20, 7-19, 7-17, 9-16, 12-16

REACTIONS (lb/size) 21=1981/0-8-0, 15=1981/0-8-0, 2=143/0-3-0, 13=143/0-3-0

Max Horz2=158(load case 5)
Max Uplift21=-1323(load case 6), 15=-1289(load case 7), 2=-234(load case 6), 13=-268(load case 7)
Max Grav21=1981(load case 1), 15=1981(load case 1), 2=156(load case 10), 13=156(load case 11)

FORCES (lb) - First Load Case Only TOP CHORD 1-2=23, 2-3=294, 3-4

1-223, 2-3=294, 3-4=-1635, 4-2=-1529, 5-22=-1518, 5-6=-1597, 6-7=-1644, 7-8=-1644, 8-9=-1644, 9-10=-1597, 10-23=-1518, 11-23=-1529, 11-12=-1635, 12-13=294, 13-14=23 2-21=-181, 20-21=-181, 19-20=1435, 18-19=1769, 17-18=1769, 16-17=1435, 15-16=-181, 13-15=-181 3-21=-181, 20-21=-181, 5-20=-346, 6-20=-125, 6-19=491, 7-19=-288, 7-17=-288, 9-17=491, 9-16=-125, 10-16=-347, 12-16=1681, 12-15=-1812 BOT CHORD

WEBS

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust): h=25ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; partially; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.
4) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

10 This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

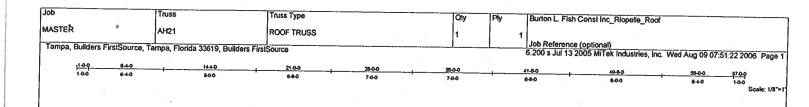
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1323 lb uplift at joint 21, 1289 lb uplift at joint 15, 234 lb uplift at joint 2 and

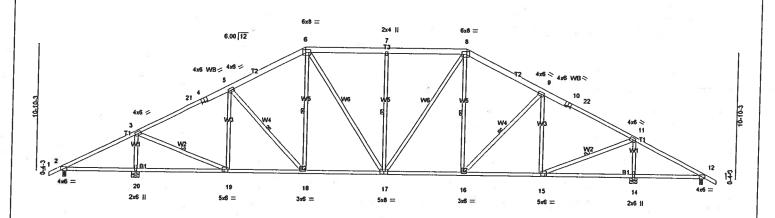
6) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard



Robert W. Wall, PE 46021 24710 State Road 54 Lutz, FI 33559





028	8-4-0	1440		21-0-0	28-0-0	36		41-8-0		49.60	55-9-8	56,0-0
		800	,9957	6-8-0	7-0-0		0-0	8-8-0		800	2.2	
Flate Offsets (X,1): [3:0-0	D-0,0-0-0], [4:0-3-0,Edge], [5:0-0-0,0-0-0],	[6:0-5-4,0-3-0]	<u>, [7:0-0-0,0-0-0]</u>	. [8:0-5-4,0-3-0], [9],[0-0-0,0-0-0]	10:0-3-0,Edge	.[11:0-0-0,0	-0-0], [15:0-2-0,0-3-0]. [17:0-4-0,0	3-0], [19:0-2-0.0-3-0
TCLL 20.0 TCDL 7.0 BCLL 10.0		SPACING Plates increase Lumber increase Rep Stress incr	2-0-0 1.25 1.25 YES	CSI TC BC WB	0.60 0.58 0.86	DEFL Vert(LL) Vert(TL) Horz(TL)	in (loc) 0.14 17 0.12 12-14 0.06 14	l/defl L/d >999 240 >611 180 n/a n/a		PLATES MT20	GRIP 244/190	
BCDL 10.0		Code FBC2004/TF	12002	(Mati	rix)					Weight: 369 lb		

WEBS 2 X 4 SYP No.3 BRACING TOP CHORD BOT CHORD

WEBS

Structural wood sheathing directly applied or 4-7-11 oc purlins.
Rigid ceiling directly applied or 6-0-0 oc bracing.
1 Row at midpt 3-19, 5-18, 6-18, 7-17, 8-16, 9-16, 11-15

REACTIONS (lb/size) 20=1968/0-8-0, 14=1968/0-8-0, 2=156/0-3-0, 12=156/0-3-0
Max Horz 2=175(load case 5)
Max Uplif20=-1306(load case 6), 14=-1272(load case 7), 2=-251(load case 6), 12=-285(load case 7)
Max Grav20=1968(load case 1), 14=1968(load case 1), 2=171(load case 10), 12=171(load case 11)

FORCES (lb) - First Load Case Only

TOP CHORD BOT CHORD 1-2-23, 2-3=263, 3-21=-1650, 4-21=-1532, 4-5=-1481, 5-6=-1685, 6-7=-1603, 7-8=-1603, 8-9=-1685, 9-10=-1481, 10-22=-1532, 11-22=-1650, 11-12=263, 12-13=23 2-20=-154, 19-20=-154, 18-19=1397, 17-18=1431, 16-17=1431, 15-16=1397, 14-15=-154, 12-14=-154 3-20=-1791, 3-19=1671, 5-19=-480, 5-18=47, 6-18=102, 6-17=308, 7-17=-375, 8-17=308, 8-16=102, 9-16=47, 9-15=-480, 11-15=1671, 11-14=-1791

NOTES

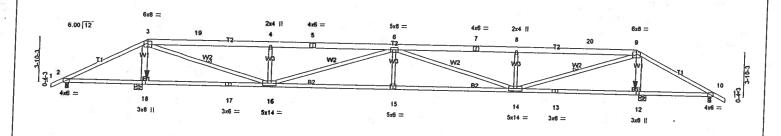
Unbalanced roof live loads have been considered for this design.
 Wind: ASCE 7-02; 120mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; partially; MWFRS gable end zone and C-C Exterior(2) zone; for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.
4) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1306 lb uplift at joint 20, 1272 lb uplift at joint 14, 251 lb uplift at joint 1 2.

LOAD CASE(S) Standard



Job Truss Truss Type Qty Burton L. Fish Const Inc_Riopelle_Roof MASTER AH7 ROOF TRUSS 2 Tampa, Builders FirstSource, Tampa, Florida 33619, Builders FirstSource Job Reference (optional) 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Aug 09 07:51:23 2006 Page 1 38-6-14 10-5-2



028 600 700 1752 028 598 100 1752	28-0-0	39.6-14	
Plate Offsets (X,Y): [3:0-4-0,0-1-15], [9:0-4-0,0-1-15], [15:0-3	0,0-3-0]	106-14	10-5-2 1-0-0 58-9-8 56-0-0 10-5-2 1-0-0 5-9-6 0-2-8
LOADING (psf) SPACING 1-0-0	CSI TC 0.77 BC 0.69 WB 0.96 (Matrix)	DEFL in (loc) l/defi L/d Vert(LL) 0.58 15 >882 240 Vert(TL) -0.74 15-16 >690 180 Horz(TL) -0.02 18 n/a n/a	PLATES GRIP MT20 244/190 Weight: 602 ib
TOP CHORD 2 X 6 SYP No.2 "Except" T1 2 X 4 SYP No.2, T1 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No. 10 "Except"		WEBS Rigid Celling directly appli	ng directly applied or 6-0-0 oc purlins. illed or 6-0-0 oc bracing.

1 Row at midpt

BOT CHORD 2 X 4 SYP No.2, 11 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.10

Except*

W1 2 X 6 SYP No.10 *Except*

W1 2 X 6 SYP No.2, W3 2 X 4 SYP No.3, W3 2 X 4 SYP No.3, W3 2 X 4 SYP No.3

REACTIONS (lb/size) 18=5023/0-8-0, 12=5023/0-8-0, 2=-1636/0-3-0, 10=-1636/0-3-0

(Ib/size) 18=5023/0-8-0, 12=5023/0-8-0, 2=-1030/0-3-0, 10=-1030/0-3-0 Max Horz2=40(load case 5) Max Uplift18=-4734(load case 4), 12=-4726(load case 3), 2=-1721(load case 10), 10=-1721(load case 9) Max Uplift18=5026(load case 9), 12=5026(load case 10), 2=1549(load case 3), 10=1557(load case 4)

FORCES (Ib) -

- First Load Case Unity
1-2=11, 2-3=3796, 3-19=-3009, 4-19=-3011, 4-5=-3009, 5-6=-3009, 6-7=-3009, 7-8=-3009, 8-20=-3011, 9-20=-3009, 9-10=3796, 10-11=11
2-18=-3322, 17-18=-3042, 16-17=-3042, 15-16=4942, 14-15=4942, 13-14=-3042, 12-13=-3042, 10-12=-3322
3-18=-4303, 3-16=6384, 4-16=-1035, 6-16=-2035, 6-15=349, 6-14=-2035, 8-14=-1035, 9-14=6384, 9-12=-4303

BOT CHORD WEBS

NOTES

1) 2-ply truss to be connected together with 16d Common(.162"x3.5") Nails as follows:

Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc. 2 X 6 - 2 rows at 0-9-0 oc. Bottom chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc. 2 X 6 - 2 rows at 0-9-0 oc. Webs connected as follows: 2 X 6 - 2 rows at 0-9-0 oc. 2 X 4 - 1 row at 0-9-0 oc.

Webs connected as follows: 2 X 6 - 2 rows at 0-9-0 oc, 2 X 4 - 1 row at 0-9-0 oc.

2) All loads are considered equally applied to all piles, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

3) Unbalanced roof live loads have been considered for this design.

4) Wind: ASCE 7-02; 120mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; partially; MWFRS gable end zone; cantilever left and right exposed; purchase to prevent water condition.

exposed; porchient and right exposed; Lumber DOL=1.00 pare gnp DOL=1.00.

5) Provide adequate drainage to prevent water ponding.

6) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 4734 ib uplift at joint 18, 4726 lb uplift at joint 12, 1721 lb uplift at joint 2 8) Girder carries hip end with 7-0-0 end setback.
9) Uplift for first LC exceeds limits

10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 475 lb down and 505 lb up at 49-0-0, and 475 lb down and 505 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

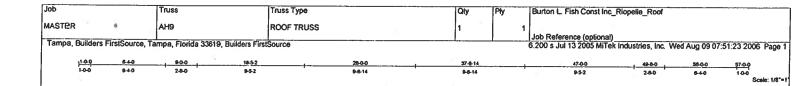
Vert: 1-3=-27, 3-9=-91(F=-64), 9-11=-27, 2-18=-10, 12-18=-34(F=-24), 10-12=-10

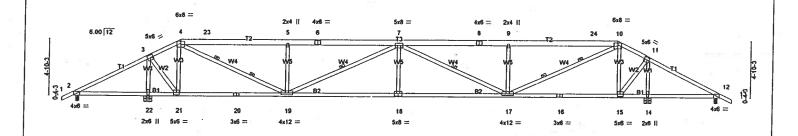
Vert: 18=-475(F) 12=-475(F)

CORID.

Robert W. Wall, PE 46021

24710 State Road 54 Lutz, Fl 33559





	028	640	900	18-5-2	2600	37-	&14	47-0-0	, 49-8-0	55-9-8	56,0-0
- '	0-2-8	6-1-8	2-8-0	9-5-2	9-6-14	94	6-14	9-5-2	2-8-0	6-1-8	0-2-8
Plate Offse	ets (X,Y	: [4:0-4	-0,0-1-15], [10:0-4-0,0-1-1	5], [18:0-4-0,0-	3-0]						
	20.ó		SPACING Plates Increase	2-0-0 1.25	CSI TC 0.51	DEFL Vert(LL) 0.	in (loc) I/deft .44 18 >999	L/d 240	PLATES MT20	GRIP 244/190	
	7.0 10.0	.	Lumber Increase Rep Stress Incr	1.25 YES	BC 0.79 WB 0.82		.65 18-19 >794 .06 14 n/a	180 n/a		1	
BCDL 1	10.0	1	Code FBC2004/TP	12002	(Matrix)				Weight: 317	lb	

TOP CHORD 2 X 6 SYP No.2 "Except"

T1 2 X 4 SYP No.2, T1 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3 BRACING TOP CHORD BOT CHORD

WEBS

Structural wood sheathing directly applied or 5-0-8 oc purlins. Rigid celling directly applied or 4-3-15 oc bracing. 1 Row at midpt 7-19, 7-17

2 Rows at 1/3 pts 4-19, 10-17

REACTIONS (lb/size) 22=2622/0-8-0, 14=2622/0-8-0, 2=-498/0-3-0, 12=-498/0-3-0

Max Horz 2=97(load case 6)
Max Upliff22=-1986(load case 5), 14=-1964(load case 4), 2=-656(load case 11), 12=-656(load case 10)

Max Grav22=2622(load case 1), 14=2622(load case 1), 2=391(load case 5), 12=403(load case 5)

FORCES (ib) - First Load Case Only TOP CHORD 1-2=23, 2-3=1711, 3-

1-2=23, 2-3=1711, 3-4=245, 4-23=-2155, 5-23=-2156, 5-6=-2155, 6-7=-2155, 7-8=-2155, 8-9=-2155, 9-24=-2156, 10-24=-2155, 10-11=245, 11-12=1711, 12-13=23 2-22=-1448, 21-22=-1448, 20-21=-145, 19-20=-145, 18-19=2885, 17-18=2885, 16-17=-145, 15-16=-145, 14-15=-1448, 12-14=-1448 3-22=-2502, 3-21=2029, 4-21=-1481, 4-19=2550, 5-19=-561, 7-19=-805, 7-18=187, 7-17=-805, 9-17=-561, 10-17=2550, 10-15=-1481, 11-15=2029, 11-14=-2502 BOT CHORD WEBS

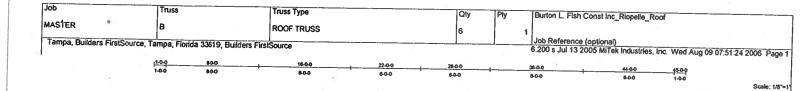
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; partially; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; porch left and right exposed; burnber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1986 lb uplift at joint 22, 1964 lb uplift at joint 14, 656 lb uplift at joint 2 and 656 lb uplift at joint 12.

LOAD CASE(S) Standard



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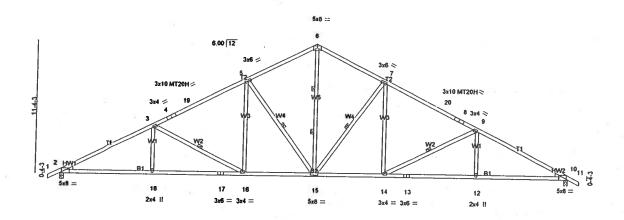


Plate Offsets (X,Y): [2:0-1	-11,Edge], [10:0-1-11,Edge]	16-0-0	22-0-0 6-0-0	28-0-6 6-0-0	36-0-0 8-0-0	44-00 8-00	
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 10.0 • BCDL 10.0	SPACING 2-0-0 Piates increase 1.25 Lumber increase 1.25 Rep Stress incr YES Code FBC2004/TPI2002	CSI TC 0.85 BC 0.79 WB 1.00 (Matrix)		Vert(LL) 0.6	in (loc) V/defi L/d 67 16-18 >777 240 56 16-18 >942 180 25 10 n/a n/a	PLATES GRIP MT20 244/190 MT20H 187/143 Weight: 249 lb	
LUMBER TOP CHORD 2 X 4 SYP N T1 2 X 4 SYP N BOT CHORD 2 X 4 SYP N WEBS 2 X 4 SYP N	P No.1D, T1 2 X 4 SYP No.1D			BRACING TOP CHORD BOT CHORD WEBS	Structural wood sheathing dire Rigid ceiling directly applied or 1 Row at midpt 3-16.	ctly applied or 3-4-8 oc purlins. 2-3-9 oc bracing. 5-15, 7-15, 9-14	- <u> </u>

2 Rows at 1/3 pts

6-15

WEDGE

Left: 2 X 4 SYP No.3, Right: 2 X 4 SYP No.3

REACTIONS (lb/size) 2=1679/0-4-0, 10=1679/0-4-0

Max Horz 2=-185(load case 4)
Max Uplift2=-1762(load case 6), 10=-1762(load case 7)

FORCES (lb) - First Load Case Only TOP CHORD 1-2=23, 2-3=-3093, 3 BOT CHORD 2-18=2675, 17-18=26 1-2=23, 2-3=-3093, 3-4=-2406, 4-19=-2329, 5-19=-2303, 5-6=-1881, 6-7=-1881, 7-20=-2303, 8-20=-2329, 8-9=-2406, 9-10=-3093, 10-11=23 2-18=2675, 17-18=2675, 16-17=2675, 15-16=2060, 14-15=2060, 13-14=2675, 12-13=2675, 10-12=2675 3-18=189, 3-16=-688, 5-16=447, 5-15=-749, 6-15=1313, 7-15=-749, 7-14=447, 9-14=-688, 9-12=189

WEBS

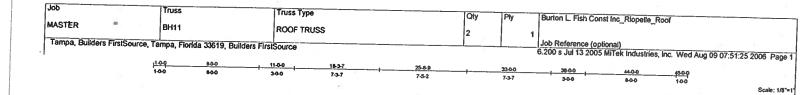
NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; partially; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and

3) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All plates are MT20 plates unless otherwise indicated.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1762 lb uplift at joint 2 and 1762 lb uplift at joint 10.

LOAD CASE(S) Standard





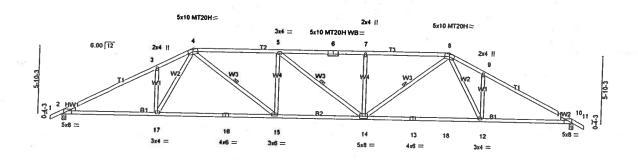


Plate Offsets (X,Y): [2:0	800 804 -1-11.Edge], [6:0-5-0,Edge], [10:0-1-11,Edge]	18-3-7 10-3-7	258 9 7-5-2	10-3-7	4400 800	
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 10.0 BCDL 10.0	SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr YES Code FBC2004/TPI2002	CSI TC 0.92 BC 0.95 WB 0.76 (Matrix)	Vert(LL) 1.09	(loc) I/defl L/d 12-14 >481 240 12-14 >588 180 10 n/a n/a	PLATES GRIP MT20 244/190 MT20H 187/143 Weight: 224 lb	2
LUMBER TOP CHORD 2 X 4 SYP T2 2 X 4 S BOT CHORD 2 X 4 SYP WEBS 2 X 4 SYP	YP No.2, T3 2 X 4 SYP No.2 No.2		BOT CHOKE	reigid ceiling directly applied.	ectly applied or 2-2-0 oc purling	n (1=

1 Row at midpt

4-15, 5-14, 8-14

BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3 2 X 4 SYP No.3 Left: 2 X 4 SYP No.3, Right: 2 X 4 SYP No.3

REACTIONS (lb/size) 2=1679/0-4-0, 10=1679/0-4-0 Max Horz 2=-114(load case 7) Max Uplift2=-1759(load case 6), 10=-1759(load case 7)

FORCES (lb) - First Load Case Only TOP CHORD 1-2=23, 2-3=-3087, 3

1-2=23, 2-3=-3087, 3-4=-3023, 4-5=-3143, 5-6=-3142, 6-7=-3142, 7-8=-3143, 8-9=-3023, 9-10=-3087, 10-11=23 2-17=2661, 16-17=2385, 15-16=2385, 14-15=3143, 13-14=2385, 13-18=2385, 12-18=2385, 10-12=2661 3-17=-286, 4-17=580, 4-15=953, 5-15=-406, 5-14=-1, 7-14=-406, 8-14=952, 8-12=580, 9-12=-286 **BOT CHORD** WEBS

NOTES

NOTES

1) Unbelanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 120mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; partially; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Tor MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

4) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

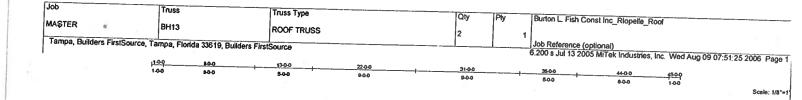
5) All plates are MT20 plates unless otherwise indicated.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1759 lb uplift at joint 2 and 1759 lb uplift at joint 10.

LOAD CASE(S) Standard



Robert W. Wall, PE 46021 24710 State Road 54 Lutz, Fl 33559



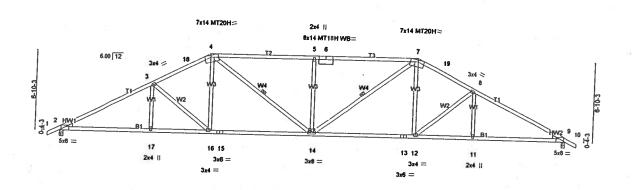


Plate Offsets (X,Y): [2:0-		300 2200 500 900 9), [9:0-1-11,Edge]	31-0	38-0-0	800	
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 10.0 BCDL 10.0	SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr YES Code FBC2004/TPI2002	CSI TC 0.85 BC 0.86 WB 0.61 (Matrix)	DEFL in (loc) I/det Vert(LL) 0.84 12-14 >624 Vert(TL) 0.69 12-14 >762 Horz(TL) -0.24 9 n/e	5 240 2 180	PLATES GRIP MT20 244/190 MT20H 187/143 MT18H 244/190 Weight: 230 ib	A
LUMBER TOP CHORD 2 X 4 SYP 1 T3 2 X 4 SYP 1 BOT CHORD 2 X 4 SYP 1 WEBS 2 X 4 SYP 1	/P No.2 No.2		BRACING TOP CHORD BOT CHORD WEBS Structural we Rigid ceiling 1 Row at mid	uirecuy applied or	city applied or 2-11-12 oc purlins. 2-2-0 oc bracing.	

T3 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3 WEDGE

Left: 2 X 4 SYP No.3, Right: 2 X 4 SYP No.3

REACTIONS (lb/size) 2=1679/0-4-0, 9=1679/0-4-0 Max Horz 2=116(load case 6) Max Uplift2=-1762(load case 6), 9=-1762(load case 7)

FORCES (lb) - First Load Case Only TOP CHORD 1-2=23, 2-3=-3062, 3 BOT CHORD 2-17=2640, 16-17=26 - First Load Case Uniy
1-2=23, 2-3=-3062, 3-18=-2599, 4-18=-2490, 4-5=-2772, 5-6=-2771, 6-7=-2772, 7-19=-2490, 8-19=-2599, 8-9=-3062, 9-10=23
2-17=2640, 16-17=2640, 15-16=2280, 14-15=2280, 13-14=2281, 12-13=2281, 11-12=2640, 9-11=2640
3-17=158, 3-16=-470, 4-16=418, 4-14=608, 5-14=-522, 7-14=607, 7-12=417, 8-12=-468, 8-11=158

WEBS

NOTES

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 120mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp 8; partially; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and

for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

4) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All plates are MT20 plates unless otherwise indicated.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1762 lb uplift at joint 2 and 1762 lb uplift at joint 9.

LOAD CASE(S) Standard



Robert W. Wall, PE 46021 24710 State Road 54 Lutz, Fl 33559

Job		Truss		Truss Type		Qty	In.			_		
MASTER		BH15		ROOF TRUSS		2	Pty	Burton L. Fish (Const Inc_Riopell	e_Roof		
Tampa, B	uilders FirstSou	rce, Tampa, Flori	da 33619, Builders Firs	tSource			'	Job Reference	(optional)		Wast to 11	
		1100	800	15-0-0		29-0-0			005 MiTek Industr	ies, Inc. Wed A	Aug 09 07:51:26 2006 Page	e 1
		1-0-0	80-0	7-0-0	7-0-0	7-0-0	-	7-0-0	800	100		
											Scale; 1/8	1-1

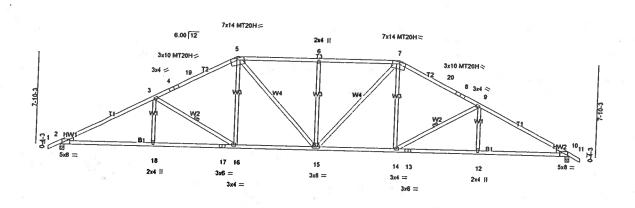


	Plate Offsets (X,Y): [2:0-1-11,Edge], [5:0-6-3,Edge], [7:0-6-3,Edge]	740 740 740 780 740 740 750	7-00 38-00 7-00 7-00	800
	Color	CSI TC 0.80 BC 0.81 WB 0.90 (Matrix)	DEFL in (loc) I/defl L/d Vert(LL) 0.64 14-15 >813 240 Vert(TL) 0.53 14-15 >988 180 Horz(TL) -0.25 10 n/a n/a	PLATES GRIP MT20 244/190 MT20H 187/143 Weight: 238 ib
- 1	LUMBER TOP CHORD 2 X 4 SYP No.2 "Except"		ACCOUNT RIGID Celling directly applied o	potty applied as 2 4 4

REACTIONS (lib/size) 2=1679/0-4-0, 10=1679/0-4-0 Max Horz2=-125(load case 4) Max Uplift2=-1762(load case 6), 10=-1762(load case 7)

Left: 2 X 4 SYP No.3, Right: 2 X 4 SYP No.3

FORCES (lb) - First Load Case Only
TOP CHORD
BOT CHORD
BOT CHORD
WEBS
1-2=23, 2-3=-3080, 3-4=-2462, 4-19=-2365, 5-19=-2310, 5-6=-2369, 6-7=-2369, 7-20=-2310, 8-20=-2365, 8-9=-2462, 9-10=-3080, 10-11=23
1-2=23, 2-3=-3080, 3-4=-2462, 4-19=-2365, 5-19=-2310, 5-6=-2369, 6-7=-2369, 7-20=-2310, 8-20=-2365, 8-9=-2462, 9-10=-3080, 10-11=23
1-2=23, 2-3=-3080, 3-4=-2462, 4-19=-2365, 5-19=-2310, 12-12=660, 12-12=660, 12-12=660, 12-12=660
1-2=23, 2-3=-3080, 3-4=-2462, 4-19=-2365, 5-19=-2310, 12-12=660, 12-12=660, 12-12=660, 12-12=660
1-2=23, 2-3=-3080, 3-4=-2462, 4-19=-2365, 5-19=-2310, 12-12=660, 1

NOTES

1) Unbalanced roof live loads have been considered for this design.

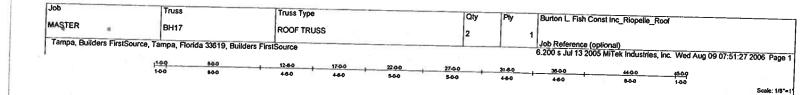
2) Wind: ASCE 7-02; 120mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; partially; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and 3) Provide adequate drainage to prevent water ponding.

4) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All plates are MT20 plates unless otherwise indicated.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1762 lb uplift at joint 2 and 1762 lb uplift at joint 10.





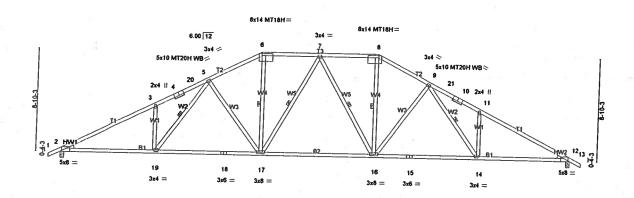


Plate Offsets (X,Y): [2:0-	\$00 \$00 1-11,Edge], [4:0-5-0,Edge], [6:0-11-0,0-2	17-00 9-00 -0), [8:0-11-0.0-2-0], [10:0-5-0	27-00 10-00 Edgel (12-0.1, 11 Edgel	900	800
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 10.0 BCDL 10.0	SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr YES Code FBC2004/TPI2002	CSI TC 0.88 BC 0.83 WB 0.53 (Matrix)	DEFL Vert(LL) 0.8	in (loc) I/defl 1/d 14 16-17 >626 240 9 16-17 >760 180	PLATES GRIP MT20 244/190 MT20H 187/143 MT18H 244/190 Weight: 251 ib
LUMBER TOP CHORD 2 X 4 SYP	/P No.1D, T1 2 X 4 SYP No.1D No.2		BRACING TOP CHORD BOT CHORD WEBS	rigid ceiling directly applied of	cth applied or 3.3.12 so guiding

T1 2 X 4 SYP No.1D, T1 2 X 4 SYP No.1D BOT CHORD 2 X 4 SYP No.2 WERS 2 X 4 SYP No.3 WEDGE

Left: 2 X 4 SYP No.3, Right: 2 X 4 SYP No.3

REACTIONS (Ib/size) 2=1679/0-4-0, 12=1679/0-4-0

Max Horz 2=-142(load case 4)

Max Uplift2=-1762(load case 6), 12=-1762(load case 7)

FORCES (Ib) - First Load Case Only TOP CHORD 1-2=23, 2-3=-3078, 3 BOT CHORD 2-19=2655, 18-19=22 - First Load Case Unity 1-2=23, 2-3=-3078, 3-4=-3043, 4-20=-2996, 5-20=-2965, 5-6=-2255, 6-7=-1987, 7-8=-1987, 8-9=-2255, 9-21=-2965, 10-21=-2996, 10-11=-3042, 11-12=-3078, 12-13=23 2-19=2655, 18-19=2264, 17-18=2264, 16-17=2058, 15-16=2264, 14-15=2264, 12-14=2655 3-19=-349, 5-19=673, 5-17=-487, 6-17=707, 7-17=-145, 7-16=-145, 8-16=707, 9-16=-487, 9-14=673, 11-14=-349 WEBS

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; partially; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and

Tor MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

4) 'This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All plates are MT20 plates unless otherwise indicated.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1762 lb uplift at joint 2 and 1762 lb uplift at joint 12.

LOAD CASE(S) Standard



t-t						
Job	Truss	Truss Type		Qty	Pty	Burton L. Fish Const inc_Riopelle_Roof
MASTER 1		ROOF TRUSS		2		
Tampa, Builders FirstSource, Ta	mpa, Florida 33619, Builders Firsl	Source				Job Reference (optional)
	00 800	15-0-0 8-0-0	1900 2500 300 600	28-0-0 3-0-0	36	6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Aug 09 07:51:27 2006 Page 1
				3-0-0	8-	800 100 Scale: 1/8"s13

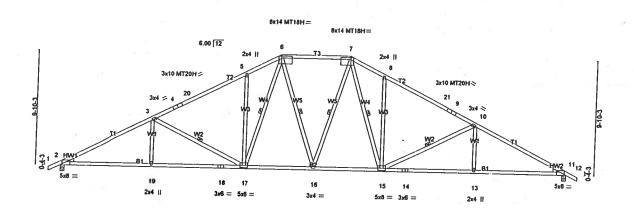


Plate Offsets (X,Y): [2:0-1-11,Edge], [6:0-11-0,0-2-0], [7:0	19-00 22-00 8-00 8-00 11-0,0-2-0], [11:0-1-11,Edge]	28-00 36-00 8-00 8-00	#00 800	
LOADING (psf) SPACING 2-0-0	CSI TC 0.92 BC 0.78 WB 0.66 (Matrix)	DEFL in (loc) I/defl L/d Vert(LL) 0.64 13-15 >818 240 Vert(TL) 0.53 13-15 >993 180 Horz(TL) -0.24 11 n/a n/a	PLATES GRIP MT20 244/190 MT20H 187/143 MT18H 244/190 Weight: 261 lb	*
LUMBER TOP CHORD 2 X 4 SYP No.2 "Except"	100 (8 10 K 10	rigid ceiling directly applied	rectly applied or 2-2-0 on purious	*1

REACTIONS (lb/size) 2=1679/0-4-0, 11=1679/0-4-0 Max Horz2=159(load case 5)

Max Upliff2=-1762(load case 6), 11=-1762(load case 7)

FORCES (lb) - First Load Case Only
TOP CHORD
BOT CHORD
BOT CHORD
WEBS
1-2=23, 2-3=-3095, 3-4=-2404, 4-20=-2326, 5-20=-2298, 5-6=-2347, 6-7=-1836, 7-8=-2347, 8-21=-2298, 9-21=-2326, 9-10=-2404, 10-11=-3095, 11-12=23
1-2=23, 2-3=-3095, 3-4=-2404, 4-20=-2326, 5-6=-2347, 6-7=-1836, 7-8=-2347, 8-21=-2298, 9-21=-2326, 9-10=-2404, 10-11=-3095, 11-12=23
1-3=188, 3-17=-697, 5-17=-304, 6-17=791, 6-16=59, 7-16=59, 7-15=791, 8-15=-304, 10-15=-697, 10-13=188

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust): h=25ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp 8; partially; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and

To reactions specified.

3) Provide adequate drainage to prevent water ponding.

4) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All plates are MT20 plates unless otherwise indicated.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1762 ib uplift at joint 2 and 1762 ib uplift at joint 11.

LOAD CASE(S) Standard



Robert W. Wall, PE 46021 24710 State Road 54 Lutz, Fl 33559

	Job		Truss	Te							
	1		11055	Truss Type			Qty	Ply	Burton L. Fish Const Inc_Riopelle Roof		
		_	1				1	1. 3	Sector E. Fish Constitut_Riopetie_Roof		
	MASTER	-	BH21	ROOF TRUSS			10				
			1				-	1 1			
	Tampa Builders Fire	SSOurce To	mna Florida 22510 Buildage Firet					L .	Job Reference (optional)		
Tampa, Builders FirstSource, Tampa, Florida 33619, Builders FirstSource								5.200 s Jul 13 2005 MiTek Industries, Inc. Wed Aug 09 07:51 28 2006 Page 1			
								·	1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2		
		.15	00 800	1600	21-0-0	***					
			0-0 800	800			28-0-0		00 44-0-0 45-0-0		
			•••	8-0-0	5-0-0	200	5-0-0	84	800 100		
									Scale: 1/8"mt*		

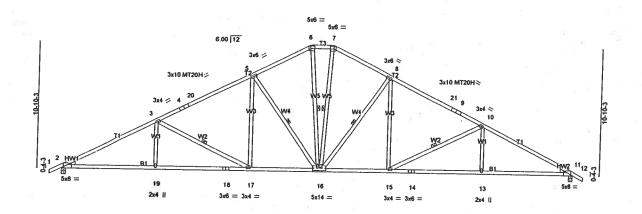


Plate Offsets (X,Y): [2:0-1-11,Edge], [6:0-4-0,0-2-8], [7:0-4-0,0-2	1600 800 -8], [11:0-1-11,Edge]	22-0-0 6-0-0	8-0-0	36-0-0 8-0-0	800	3
LOADING (psf) SPACING 2-0-0	CSI TC 0.88 BC 0.79 WB 0.96 (Matrix)		DEFL in (k Vert(LL) 0.66 17- Vert(TL) 0.54 17- Horz(TL) -0.25	19 >797 240	PLATES GRIP MT20 244/190 MT20H 187/143 Weight: 263 lb	
LUMBER TOP CHORD 2 X 4 SYP No.2 *Except*		13 13 200 Miles	BOT CHOKD KIG	id celling directly applied or	ctly applied or 3.4.9 oc pudine	

REACTIONS (lb/size) 2=1679/0-4-0, 11=1679/0-4-0 Max Horz2=177(load case 5)

Left: 2 X 4 SYP No.3, Right: 2 X 4 SYP No.3

Max Uplift2=-1762(load case 6), 11=-1762(load case 7)

FORCES (lb) - First Load Case Only
TOP CHORD
1-2=23, 2-3=-3093, 3-4=-2405, 4-20=-2328, 5-20=-2302, 5-6=-1881, 6-7=-1677, 7-8=-1881, 8-21=-2302, 9-21=-2328, 9-10=-2405, 10-11=-3093, 11-12=23
BOT CHORD
WEBS
2-19=2675, 18-19=2675, 17-18=2675, 16-17=2059, 18-16=2059, 18-15=2675, 13-14=2675, 11-13=2675
3-19=189, 3-17=-690, 5-17=447, 5-16=-744, 6-16=656, 8-15=447, 10-15=-690, 10-13=189, 8-16=-744

NOTES

Unbalanced roof live loads have been considered for this design.
 Wind: ASCE 7-02; 120mph (3-second gust): h=25ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; partially; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and

To inverted for reacuous specimed.

3) Provide adequate drainage to prevent water ponding.

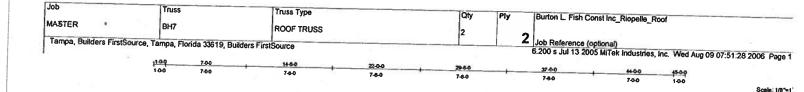
4) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All plates are MT20 plates unless otherwise indicated.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1762 lb uplift at joint 2 and 1762 lb uplift at joint 11.

LOAD CASE(S) Standard





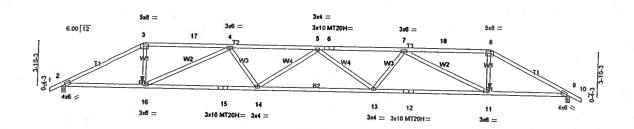


Plate Offsets (X,Y): [2:0-	7-00 7-00 3-14,0-1-12], [3:0-5-0,0-2-0], [8:0-5-0,0-2-	17-00 10-00 0], [9:0-3-14,0-1-12]	27-0-0	37-0-0 10-0-0	700	
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 10.0 * BCDL 10.0	SPACING 1-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr NO Code FBC2004/TPI2002	CSI TC 0.98 BC 1.00 WB 0.85 (Matrix)	DEFL in Vert(LL) 1.09 t Vert(TL) -1.15 t Horz(TL) -0.27		PLATES GRIP MT20 244/190 MT20H 187/143 Weight: 401 ib	-
TOP CHORD 2 X 4 SYP POT CHARLES TO THE CHARLES TO T	lo.2	NI VERN	BRACING TOP CHORD S BOT CHORD R	tructural wood sheathing directly applied.	ety applied or 4-1-11 oc purlins.	

REACTIONS (lb/size) 2=2629/0-4-0, 9=2629/0-4-0

(IDSIZE) 2-2023/04-0, 3-2023/04-0 Max Horz2=-40(load case 6) Max Uplifi2=-2716(load case 5), 9=-2716(load case 6)

FORCES (Ib) - First Load Case Only
TOP CHORD 1-2=12, 2-3=-5509, 3-17=-4951, 4-5=-8297, 5-6=-8297, 6-7=-8297, 7-18=-4951, 8-18=-4951, 8-9=-5509, 9-10=12

BOT CHORD 2-16=4865, 15-16=7819, 14-15=7819, 13-14=8768, 12-13=7819, 11-12=7819, 9-11=4865

WEBS 3-16=2077, 4-16=-3186, 4-14=832, 5-14=-578, 5-13=-578, 7-13=832, 7-11=-3186, 8-11=2077

1) 2-ply truss to be connected together with 16d Common(.162"x3.5") Nails as follows: Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

- Top chords connected as follows: 2 X 4 1 row at 0-9-0 oc.
 Bottom chords connected as follows: 2 X 4 1 row at 0-9-0 oc.
 Bottom chords connected as follows: 2 X 4 1 row at 0-9-0 oc.
 Webs connected as follows: 2 X 4 1 row at 0-9-0 oc.

 2) All loads are considered equally applied to all piles, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

 3) Unbalanced roof live loads have been considered for this design.

 4) Wind: ASCE 7-02; 120mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; partially; MWFRS gable end zone; cantilever left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.

 5) Provide adequate drainage to prevent water ponding.

5) Provide adequate drainage to prevent water ponding.
6) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) All plates are MT20 plates unless otherwise indicated.

7) Air plates are will 20 plates unless otherwise indicated.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2716 lb uplift at joint 2 and 2716 lb uplift at joint 9.
9) Girder carries hip end with 7-0-0 end setback.
10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 475 lb down and 505 lb up at 37-0-0, and 475 lb down and 505 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-27, 3-8=-91(F=-64), 8-10=-27, 2-16=-10, 11-16=-34(F=-24), 9-11=-10 Concentrated Loads (ib) Vert: 16=-475(F) 11=-475(F)



Robert W. Wall, PE 46021 24710 State Road 54 Lutz, FI 33559

Job	-					
	Truss	Truss Type	50	Qty	Ply	Burton L Fish Const Inc_Riopelle_Roof
	вн9	ROOF TRUSS		2	1 1	
Tampa, Builders FirstSource, Ta		Source				Job Reference (optional) 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Aug 09 07:51:29 2006 Page 1
1 <u>1-</u> 1-4		1789	26-3-7 8-6-13		35-0-0 8-8-9	300 4400 4500 300 600 100

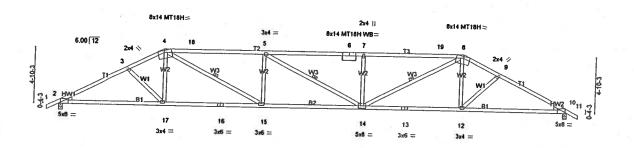


Plate Offsets (X,Y): [2:0-1	+	e), [10:0-1-11,Edge]	26-3-7 8-6-13	35.0.0 8-8-9	900	75 6
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 10.0 BCDL 10.0	SPACING 2-0-0 Plates increase 1.25 Lumber increase 1.25 Rep Stress incr YES Code FBC2004/TPI2002	CSI TC 0.97 BC 0.79 WB 0.86 (Matrix)	DEFL in Vert(LL) 1.15 1 Vert(TL) 0.94 1 Horz(TL) -0.27		PLATES GRIP MT20 244/190 MT18H 244/190 Weight: 219 lb	
LUMBER	In 0 ar		BRACING		**************************************	

TOP CHORD BOT CHORD

1 Row at midpt

WEBS

Structural wood sheathing directly applied or 2-2-0 oc purlins. Rigid ceiling directly applied or 2-0-2 oc bracing.

4-15, 5-14, 8-14

TOP CHORD 2 X 4 SYP No.2 "Except"

T2 2 X 4 SYP No.1D, T3 2 X 4 SYP No.1D

BOT CHORD 2 X 4 SYP No.2 "Except"

B2 2 X 4 SYP No.1D

WEBS 2 X 4 SYP No.3

WEDGE

Left: 2 X 4 SYP No.3, Right: 2 X 4 SYP No.3

REACTIONS (lib/size) 2=1679/0-4-0, 10=1679/0-4-0 Max Horz 2=97(load case 6) Max Uplift2=-1764(load case 6), 10=-1764(load case 7)

FORCES (lb) - First Load Case Only
TOP CHORD
1-2=23, 2-3=-3074, 3-4=-2897, 4-18=-3811, 5-6=-3810, 6-7=-3810, 7-19=-3810, 8-19=-3810, 8-9=-2898, 9-10=-3074, 10-11=23
BOT CHORD
WEBS
2-17=2679, 16-17=2585, 15-16=2585, 14-15=3811, 13-14=2586, 12-13=2586, 10-12=2679
3-17=-139, 4-17=291, 4-15=1382, 5-15=-473, 5-14=-1, 7-14=-473, 8-14=1381, 8-12=291, 9-12=-139

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 120mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; partially; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and 3) Provide adequate drainage to prevent water ponding.

4) 'This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

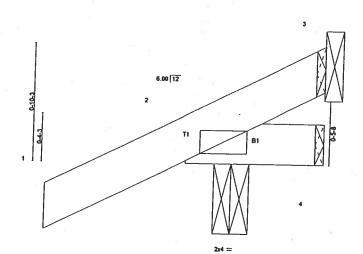
5) All plates are MT20 plates unless otherwise indicated.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1764 lb uplift at joint 2 and 1764 lb uplift at joint 10.

LOAD CASE(S) Standard



IJ	ob		Truss	Total Tree			
1				Truss Type	Qty	Ply	Burton L. Fish Const Inc. Riopelle Roof
A	MASTER	*	CJ1	DOOG TOUGO		l .	The state of the s
1			301	ROOF TRUSS	16	1 1	6
Г	Tampa, Builders Fi	rstSource Tai	mpa, Florida 33619, Builders First	C-1111	L		Job Reference (optional)
Г	, , ,		mpa, monda 33019, Builders First	Source			6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Aug 09 07:51:29 2006 Page 1
•							Page 1
				-1-0-0	1-0-0	1	



0-2-8 LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 10.0 **SPACING** CSI TC 0.08 BC 0.04 WB 0.00 DEFL (loc) 4 4 3 Vdefl L/d 240 180 **PLATES** GRIP 1.25 1.25 Plates Increase Vert(LL) Vert(TL) Horz(TL) 0.00 >999 Lumber Increase MT20 244/190 -0.00 >999 Rep Stress Incr YES
Code FBC2004/TPI2002 -0.00 BCDL 10.0 Weight: 5 lb

LUMBER TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

BRACING TOP CHORD **BOT CHORD**

Structural wood sheathing directly applied or 1-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=120/0-3-0, 3=6/Mechanical Max Horz2=73(load case 6) Max Uplift2=-133(load case 6), 3=-12(load case 9) Max Grav2=120(load case 1), 3=19(load case 2)

FORCES (ib) - First Load Case Only TOP CHORD 1-2=23, 2-3=-22 BOT CHORD 2-4=0

NOTES

NOTES

1) Wind: ASCE 7-02; 120mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; partially; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; porch left and right exposed; porch left and forces, and for MWFRS for reactions specified.

2) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) Refer to girder(s) for truss to truss connections.

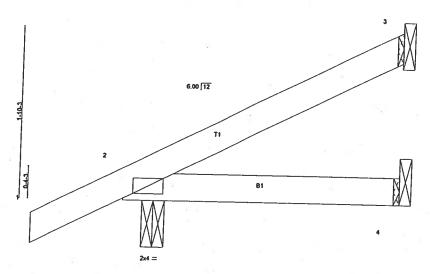
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 133 lb uplift at joint 2 and 12 lb uplift at joint 3.

LOAD CASE(S) Standard



Robert W. Wall, PE 46021 24710 State Road 54 Lutz, FI 33559

Job Truss Truss Type Qty Burton L Fish Const Inc_Riopelle_Roof MASTER CJ3 **ROOF TRUSS** 16 Tampa, Builders FirstSource, Tampa, Florida 33619, Builders FirstSource Job Reference (optional) 6,200 s Jul 13 2005 MiTek Industries, Inc. Wed Aug 09 07:51:29 2006 Page 1



0-2-8 LOADING (psf) TCLL 20.0 TCDL 7.0 SPACING TCLL TCDL BCLL CSI DEFL Plates Increase in l/deft TC 0.22 BC 0.10 WB 0.00 1.25 1.25 **PLATES** GRIP Vert(LL) Vert(TL) Horz(TL) 2-4 2-4 0.01 >999 240 180 Lumber Increase MT20 244/190 10.0 0.01 Rep Stress Incr YES Code FBC2004/TPI2002 BCDL 10.0 (Matrix) Weight: 11 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

BRACING TOP CHORD **BOT CHORD**

Structural wood sheathing directly applied or 3-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (Ib/size) 3=64/Mechanical, 4=28/Mechanical, 2=177/0-3-0
Max Horz 2=143(load case 6)
Max Uplift3=-99(load case 6), 4=-34(load case 4), 2=-188(load case 6)
Max Grav3=64(load case 1), 4=56(load case 2), 2=177(load case 1)

FORCES (lb) - First Load Case Only TOP CHORD 1-2=23, 2-3=23 BOT CHORD 2-4=0

NOTES

NOTES

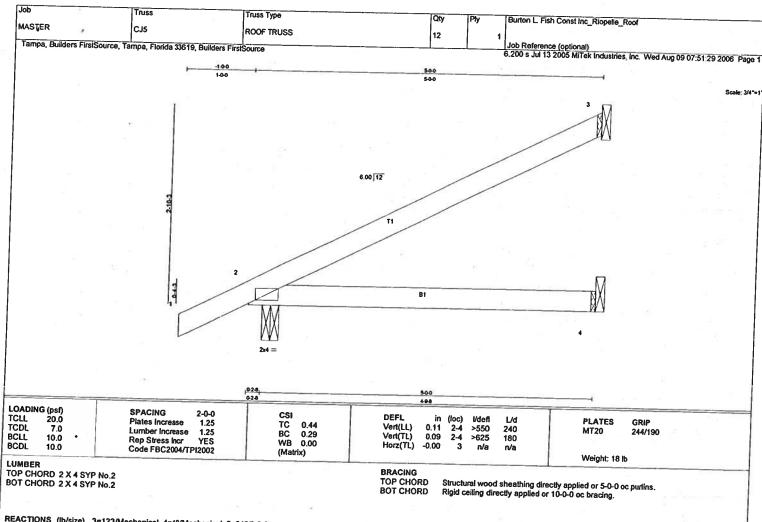
1) Wind: ASCE 7-02; 120mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; partially; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; porch left exposed

LOAD CASE(S) Standard



08/09/2006

Robert W. Wall, PE 46021 24710 State Road 54 Lutz, FI 33559



REACTIONS (lb/size) 3=123/Mechanical, 4=48/Mechanical, 2=246/0-3-0

Max Horz2=214(load case 6), 4=57(load case 4), 2=-250(load case 6) Max Grav3=123(load case 1), 4=96(load case 2), 2=246(load case 1)

FORCES (lb) - First Load Case Only TOP CHORD 1-2=23, 2-3=45 BOT CHORD 2-4=0

NOTES

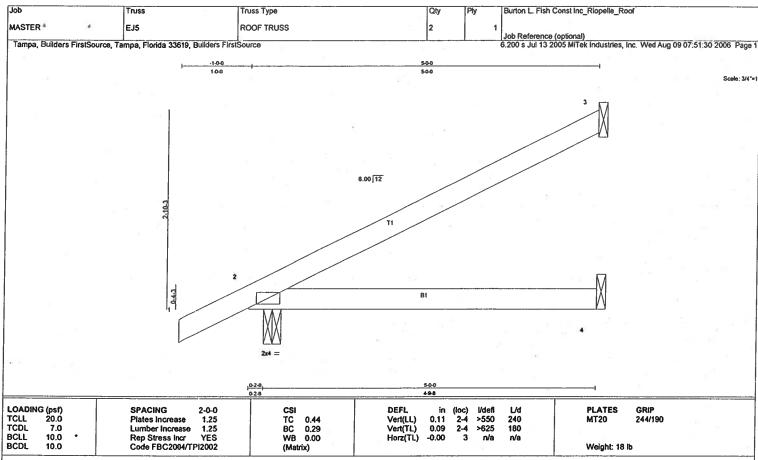
NOTES

1) Wind: ASCE 7-02; 120mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; partially; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; porch left and right exposed; porch left and right exposed; porch left and for MWFRS for reactions specified.
2) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 187 lb uplift at joint 3, 57 lb uplift at joint 4 and 250 lb uplift at joint 2.

LOAD CASE(S) Standard



Robert W. Wall, PE 46021 24710 State Road 54 Lutz, FI 33559



LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 5-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (Ib/size) 3=123/Mechanical, 4=48/Mechanical, 2=246/0-3-0 Max Horz 2=214(load case 6)

Max Uplift3=-187(load case 6), 4=-57(load case 4), 2=-250(load case 6)

Max Grav3=123(load case 1), 4=96(load case 2), 2=246(load case 1)

FORCES (lb) - First Load Case Only TOP CHORD 1-2=23, 2-3=45 BOT CHORD 2-4=0

1) Wind: ASCE 7-02; 120mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; partially; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads."

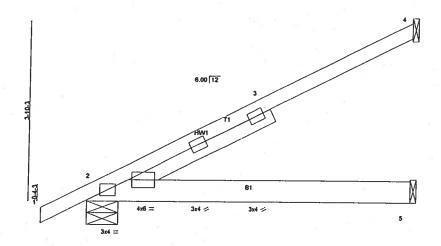
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 187 lb uplift at joint 3, 57 lb uplift at joint 4 and 250 lb uplift at joint 2.

LOAD CASE(S) Standard



Robert W. Wall, PE 46021 24710 State Road 54 Lutz, Fl 33559

Job Truss Truss Type Burton L. Fish Const Inc_Riopelle_Roof MASTER EJ7 ROOF TRUSS 54 Job Reference (optional) 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Aug 09 07:51:30 2006 Page 1 Tampa, Builders FirstSource, Tampa, Florida 33619, Builders FirstSource Scale: 1/2"=1



7-0-0 7-0-0

Plate Offsets (X,Y): [2:0-0-15,0-2-0] LOADING (psf) SPACING DEFL l/defl PLATES GRIP TCLL 20.ó Plates Increase 1.25 1.25 TC BC 0.83 0.27 Vert(LL) 0.09 >836 240 MT20 244/190 7.0 Lumber Increase Vert(TL) 2-5 >952 180 BCLL 10.0 Rep Stress Incr YES WB 0.00 Horz(TL) -0.01 n/a n/a 10.0 Code FBC2004/TPI2002

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 6 SYP No.2

SLIDER Left 2 X 4 SYP No.3 3-5-1

BRACING TOP CHORD BOT CHORD Structural wood sheathing directly applied or 4-5-10 oc purtins. Rigid ceiling directly applied or 10-0-0 oc bracing.

Weight: 35 lb

REACTIONS (lb/size) 4=171/Mechanical, 2=324/0-8-0, 5=66/Mechanical

(ID/SIZE) 4-17 (medicalism, 2-25/70-0-7, 5-2

FORCES (lb) - First Load Case Only TOP CHORD 1-2=27, 2-3=-71, 3-4=60 BOT CHORD 2-5=0

NOTES

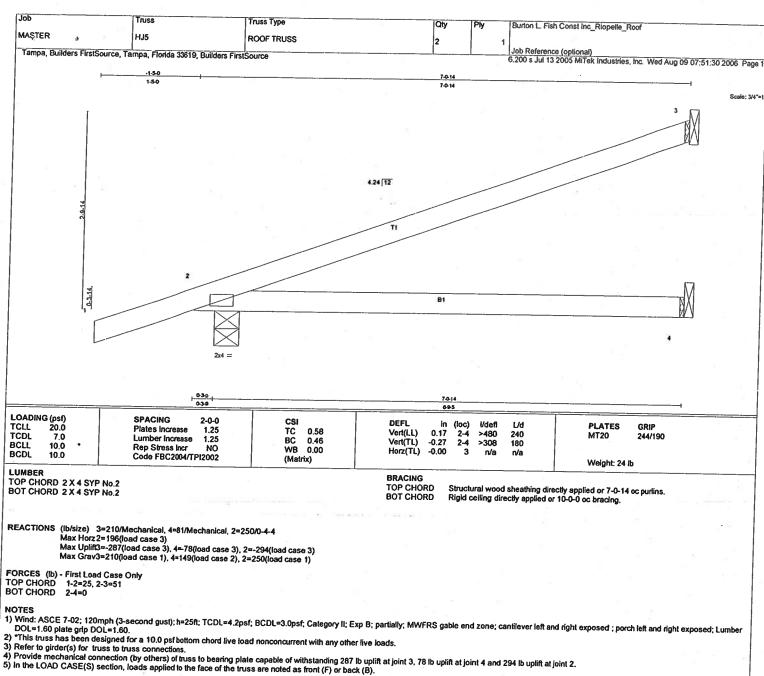
1) Wind: ASCE 7-02; 120mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; partially; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and cantilever len and right exposed; porchien and right exposed, cantiles 505 - 1.05 plants of the form of the process of the pro

(Matrix)

LOAD CASE(S) Standard



Robert W. Wall, PE 46021 24710 State Road 54 Lutz, FI 33559



LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf) Vert: 1-2=-54

Trapezoidal Loads (plf)

Vert: 2=-2(F=26, B=26)-to-3=-95(F=-21, B=-21), 2=0(F=10, B=10)-to-4=-35(F=-8, B=-8)



Robert W. Wall, PE 46021 24710 State Road 54 Lutz, FI 33559

Job Truss Truss Type Burton L. Fish Const Inc_Riopelle_Roof Qty MASTER 4 НЈ7 ROOF TRUSS 6 Job Reference (optional) 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Aug 09 07:51.31 2006 Page 1 Tampa, Builders FirstSource, Tampa, Florida 33619, Builders FirstSource 9-10-13

> 4.24 12 W2 BI

	039	\$-0 \$-07	\$-10-13 4-6-13
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 10.0 BCDL 10.0	SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr NO Code FBC2004/TPI2002	CSI TC 0.46 BC 0.39 WB 0.27 (Matrix)	DEFL in (loc) I/defl L/d PLATES GRIP Vert(LL) 0.04 5-6 >999 240 MT20 244/190 Vert(TL) -0.05 5-6 >999 180 Horz(TL) -0.01 5 n/a n/a Weight: 42 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD BOT CHORD

2x4 [

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 6-11-8 oc bracing.

3x8 >

REACTIONS (lb/size) 4=219/Mechanical, 5=369/Mechanical, 2=392/0-4-4 Mex Horz 2=354(load case 3)

Max Uplift4=-311(load case 3), 5=-390(load case 3), 2=-408(load case 3)

FORCES (lb) - First Load Case Only TOP CHORD 1-2=25, 2-3=-714, 3-4=53 BOT CHORD 2-6=656, 5-6=656 WEBS 3-6=133, 3-5=-716

NOTES

- 1) Wind: ASCE 7-02; 120mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; partially; MWFRS gable end zone; cantilever left and right exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.

DCI=1.60 parter grip DCI=1.60.
2) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 311 lb uplift at joint 4, 390 lb uplift at joint 5 and 408 lb uplift at joint 2.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf) Vert: 1-2=-54

Trapezoidal Loads (plf)

Vert: 2=-2(F=26, B=26)-to-4=-134(F=-40, B=-40), 2=0(F=10, B=10)-to-5=-49(F=-15, B=-15)

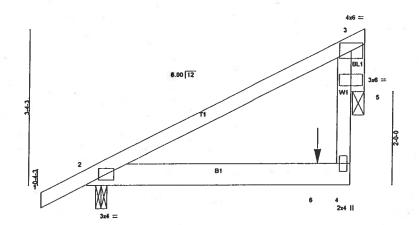


Robert W. Wall, PE 46021 24710 State Road 54 Lutz, Fl 33559

Qty Burton L. Fish Const Inc_Riopelle_Roof Job Truss Truss Type ROOF TRUSS MASTER4 MGR 1 Job Reference (optional) 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Aug 09 07:51:31 2006 Page 1 Tampa, Builders FirstSource, Tampa, Florida 33619, Builders FirstSource

-1-0-0

Scale: 1/2"#1



6-0-0 5-9-8 LOADING (psf) DEFL (loc) 2-4 2-4 **PLATES** SPACING 1-0-0 in 0.04 TCLL 20.0 7.0 1.25 1.25 TC BC 0.24 0.20 Vert(LL) Vert(TL) Plates Increa >999 240 MT20 244/190 0.03 180 >999 Lumber Increase WB 0.14 -0.00 Rep Stress Incr Code FBC2004/TPI2002 Weight: 30 lb BCDI 10.0 (Matrix)

LUMBER
TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 6 SYP No.2 WEBS 2 X 4 SYP No.3 OTHERS 2 X 4 SYP No.2 BRACING

TOP CHORD **BOT CHORD**

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 5=424/0-3-0, 2=171/0-3-0 Max Horz2=121(load case 5)

Max Uplift5=-521(load case 5), 2=-179(load case 5)

FORCES (lb) - First Load Case Only TOP CHORD 1-2=13, 2-3=30 BOT CHORD 2-6=-4, 4-6=-4

WEBS 4-5=350, 3-5=-74

NOTES

- NOTES

 1) Wind: ASCE 7-02; 120mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; partially; MWFRS gable end zone; cantilever left and right exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.

 2) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

 3) Bearing at joint(s) 5 considers parallel to grain value using ANSI/TP1 1 angle to grain formula. Building designer should verify capacity of bearing surface.

 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 521 lb uplift at joint 5 and 179 lb uplift at joint 2.

 5) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 360 lb down and 421 lb up at 5-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

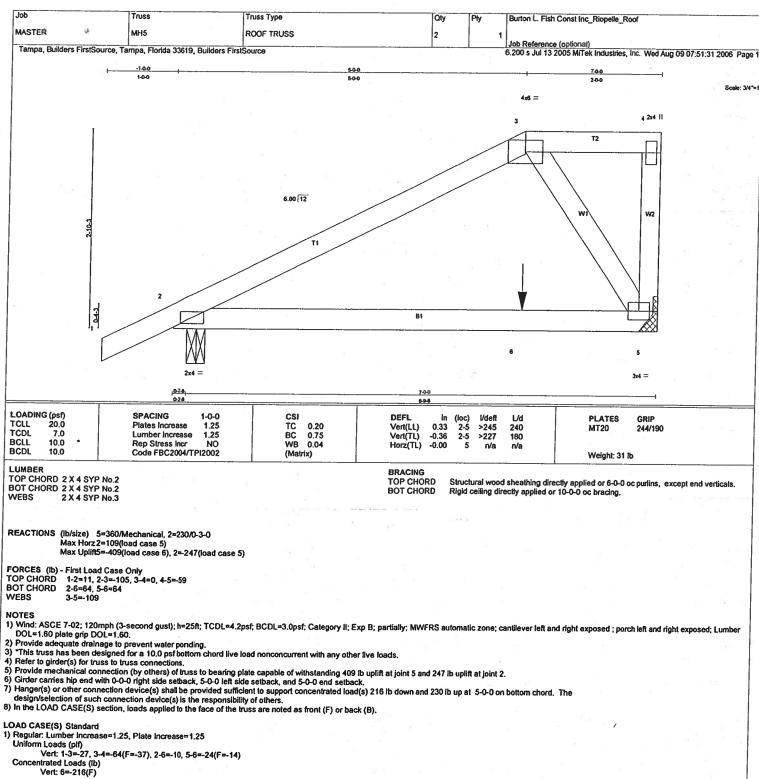
LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf) Vert: 1-3=-27, 2-4=-10

Concentrated Loads (lb) Vert: 6=-360(F)



Robert W. Wall, PE 46021 24710 State Road 54 Lutz, Fl 33559



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Robert W. Wall, PE 46021 24710 State Road 54 Lutz, Fl 33559

ì	Job	Truss	Truss Type	Qty	Ply	Burton L. Fish Const Inc_Riopelle_Roof
	MASTER	MT	ROOF TRUSS	76		Job Reference (optional)
	Tampa, Builders FirstSource, Tai	mpa, Florida 33619, Builders First	Source		71	6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Aug 09 07:51:31 2006 Page 1

6.00 12 Wi W

600 CSI TC 0.60 BC 0.64 WB 0.09 2-0-0 1.25 1.25 L/d 240 180 LOADING (psf) TCLL 20.0 SPACING DEFL in 0.17 (loc) 2-4 **PLATES** GRIP Vert(LL) >382 MT20 244/190 Plates Increase TCDL 2-4 Vert(TL) 0.15 >435 Lumber Increase Horz(TL) 10.0 Rep Stress Incr YES Code FBC2004/TPI2002 0.00 Weight: 26 lb 10.0 (Matrix)

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3 **OTHERS** 2 X 4 SYP No.3 BRACING

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 6-0-0 oc bracing.

2x4 ||

REACTIONS (lb/size) 5=203/0-3-0, 2=268/0-3-0

Max Horz 2=241(load case 6) Max Uplift5=-290(load case 6), 2=-267(load case 6)

FORCES (lb) - First Load Case Only TOP CHORD 1-2=23, 2-3=61 BOT CHORD 2-4=-7 WEBS 4-5=54, 3-5=-148

NOTES

- 1) Wind: ASCE 7-02; 120mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp 8; partially; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 Bearing at joint(s) 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 290 lb uplift at joint 5 and 267 lb uplift at joint 2.

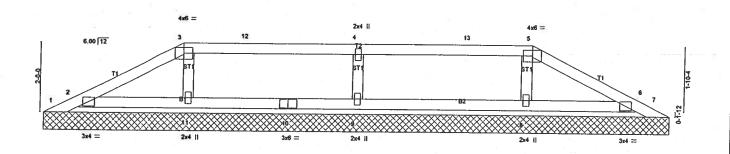
1-0-0

LOAD CASE(S) Standard



Robert W. Wall, PE 46021 24710 State Road 54 Lutz, Fl 33559

Job	Truss	Truss Type		Qty	Pty	Burton L. Fish Const Inc_Riopelle_Roof
MASTER .	РВ	HIP PIGGYBACK		1	1 1	
Tampa, Builders FirstSc	urce, Tampa, Florida 33619, B	kuilders FirstSource				Job Reference (optional)
	, ,					6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Aug 09 07:51:32 2006 Page 1
100	400		1400			18-00 19-0-0
1-0-0	400		10-0-0			4-00 1-00 Scale: 3/8*n1



18-0-0 LOADING (psf) TCLL 20.0 TCDL 7.0 SPACING CSI TC BC WB 2-0-0 1.25 in **V**deft (loc) L/d PLATES GRIP Plates Increase 0.17 0.12 Vert(LL) Vert(TL) 999 999 n/a n/a MT20 244/190 Lumber Increase Rep Stress Incr 1.25 n/a n/a BCLL YES 0.09 Horz(TL) 0.00 n/a BCDL 10.0 Code FBC2004/TPI2002 (Matrix) Weight: 58 lb LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 OTHERS 2 X 4 SYP No.3

BRACING

TOP CHORD **BOT CHORD**

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid celling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=-8/18-0-0, 7=-8/18-0-0, 2=153/18-0-0, 8=303/18-0-0, 6=153/18-0-0, 9=389/18-0-0, 11=303/18-0-0

Max Horz 1=32(load case 5)
Max Uplifit=-31(load case 4), 7=-17(load case 5), 2=-147(load case 6), 8=-221(load case 7), 6=-140(load case 7), 9=-306(load case 5), 11=-228(load case 6)
Max Grav1=32(load case 6), 7=21(load case 10), 2=154(load case 10), 8=303(load case 1), 6=154(load case 11), 9=393(load case 10), 11=303(load case 1)

FORCES (Ib) - First Load Case Only TOP CHORD 1-2=23, 2-3=19, 3-12 BOT CHORD 2-11=25, 10-11=5, 9-1-2=23, 2-3=19, 3-12=-5, 4-12=-5, 4-13=-5, 5-13=-5, 5-6=-56, 6-7=23 2-11=25, 10-11=5, 9-10=5, 8-9=5, 6-8=25

WEBS

5-8=-218, 4-9=-287, 3-11=-218

NOTES

1) Unbalanced roof live loads have been considered for this design.

1) Unbalanced roof live loads have been considered for this design.
2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) Gable requires continuous bottom chord bearing.

(a) Gable requires continuous bottom criota bearing.

6) Gable studies spaced at 4-0-0 oc.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 31 lb uplift at joint 1, 17 lb uplift at joint 7, 147 lb uplift at joint 2, 221 lb uplift at joint 8, 140 lb uplift at joint 6, 306 lb uplift at joint 9 and 228 lb uplift at joint 11.

8) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2, 8, 6, 9, 11.

9) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.

LOAD CASE(S) Standard



Robert W. Wall, PE 46021 24710 State Road 54 Lutz, FI 33559

Job	Truss	Truss Type		Qty	Ply	Burton L. Fish Const Inc_Riopelle_Roof	
MASTER (P)	PB1	HIP PIGGYBACK		1	.	1	
Tampa, Builders FirstSource, Ta	mpa, Florida 33619. Builders First	Source		- 11		Job Reference (optional)	
	, , , , , , , , , , , , , , , , , , , ,	334100				6.200 s Jul 13 2005 MiTek Industries, Inc	. Wed Aug 09 07:51:32 2006 Page 1
1-00	600		12-0-0			18-0-0	
100	60-0		600			6-0-0	1-0-0
							Scale: 3/8"=

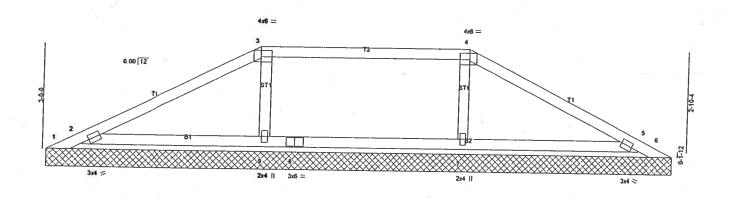


Plate Offsets (X,Y): [2:0-	2-10,0-1-8], [3:0-3-8,0-2-4], [4:0-3-8,0-2-4], [5:0-2-10,0-1-8)	18-0-0		
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 10.0 * BCDL 10.0	SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr YES Code FBC2004/TPl2002	CSI TC 0.25 BC 0.14 WB 0.09 (Matrix)	DEFL in (Vert(LL) n/a Vert(TL) n/a Horz(TL) 0.00	loc) I/defi L/d - n/a 999 - n/a 999 5 n/a n/a	PLATES GRIP MT20 244/190 Weight: 59 lb
LUMBER	3 3		BRACING		

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 OTHERS 2 X 4 SYP No.3

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=-138/18-0-0, 6=-138/18-0-0, 2=372/18-0-0, 7=409/18-0-0, 9=409/18-0-0, 5=372/18-0-0

FORCES (Ib) - First Load Case Only TOP CHORD 1-2=76, 2-3=32, 3-4=-15, 4-5=-91, 5-6=76 BOT CHORD 2-9=32, 8-9=15, 7-8=15, 5-7=32 WEBS 4-7=-295, 3-9=-295

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
3) Provide adequate drainage to prevent water ponding.
4) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) Cable requires continuous bottom chord.

5) Gable requires continuous bottom chord bearing.

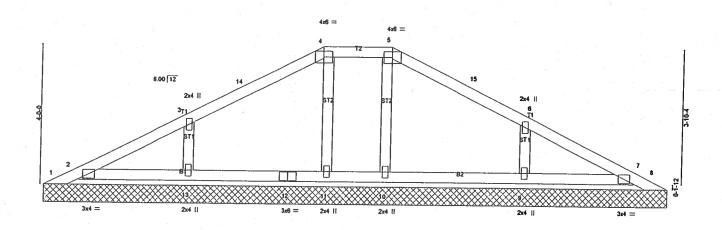
5) Gable requires continuous bottom chord bearing.
6) Gable studs spaced at 6-0-0 oc.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 138 ib uplift at joint 1, 138 ib uplift at joint 6, 361 ib uplift at joint 2, 298 ib uplift at joint 1 at joint 9 and 351 ib uplift at joint 5.
8) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2, 7, 9, 5.
9) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.

LOAD CASE(S) Standard



Robert W. Wall, PE 46021 24710 State Road 54 Lutz, Fl 33559

Job	Truss	Truss Type		Qty	Ply	Burton L Fish Const Inc_Riopelle_Roof	
MASTER P	P82	HIP PIGGYBACK		1	1	1	
Tampa Buildore CimtCo.	Tomas Fladd - 00040 5					Job Reference (optional)	
rampa, builders rissisot	irce, Tampa, Florida 33619, E	Suliders FirstSource				6.200 s Jul 13 2005 MiTek Industries, Inc.	Wed Aug 09 07:51:33 2006 Page 1
1-00		800	1000			18-0-0	
1-0-0		8-0-0	2-0-0	& (W.		800	1900
							Scale: 3/8"=1



			18-0 18-0		4
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 10.0 BCDL 10.0	SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr YES Code FBC2004/TPI2002	CSI TC 0.13 BC 0.10 WB 0.09 (Matrix)	DEFL in (loc) l/defl L/d Vert(LL) n/a - n/a 999 Vert(TL) n/a - n/a 999 Horz(TL) 0.00 7 n/a n/a	PLATES GRIP MT20 244/190 Weight: 67 lb	
LUMBER TOP CHORD 2 X 4 SYP BOT CHORD 2 X 4 SYP OTHERS 2 X 4 SYP	No.2		BRACING TOP CHORD Structural wood sheathing BOT CHORD Rigid ceiling directly applie	directly applied or 6-0-0 oc purtins.	

REACTIONS (lb/size) 1=-26/18-0-0, 8=-26/18-0-0, 2=183/18-0-0, 10=199/18-0-0, 11=199/18-0-0, 7=183/18-0-0, 13=287/18-0-0, 9=287/18-0-0

Max Horz1=-67(load case 4)

Max Uplifit=-27(load case 10), 8=-27(load case 11), 2=-148(load case 6), 10=-101(load case 6), 11=-108(load case 6), 7=-138(load case 7), 13=-288(load case 6), 9=-288(load case 7)

Max Grav1=51(load case 6), 8=31(load case 7), 2=183(load case 10), 10=221(load case 11), 11=221(load case 10), 7=183(load case 11), 13=287(load case 1), 9=287(load case 1)

FORCES (Ib) - First Load Case Only
TOP CHORD 1-2=31, 2-3=25, 3-14=-68, 4-14=26, 4-5=-11, 5-15=26, 6-15=-68, 6-7=-53, 7-8=31
BOT CHORD 2-13=17, 12-13=17, 11-12=17, 10-11=11, 9-10=17, 7-9=17

WEBS 5-10=-145, 4-11=-145, 3-13=-206, 6-9=-206

NOTES

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) Gable requires continuous bottom chord bearing.

6) Gable studs spaced at 2-0-0 oc.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 27 lb uplift at joint 1, 27 lb uplift at joint 8, 148 lb uplift at joint 2, 101 lb uplift at joint 10, 108 lb uplift at joint 1, 138 lb uplift at joint 1, 288 lb uplift at joint 13 and 288 lb uplift at joint 9.

8) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2, 10, 11, 7, 13, 9.

9) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.

LOAD CASE(S) Standard



Architectural Services and Engineering, Inc

24710 SR 54, Lutz, FI 33559 Phone: 813-948-2812 EBO 7882

Engineering Index Sheet

Truss Fabricator: Builders First Source Tampa, FI 33619

Permit Number:	Lot Number:
Customer Info:	Address:is for administrative purpsoes only and not part of the engineering review.

Software: MiTek Industries, Inc. Refer to sheets for version number. Loading: Refer to sheets Typical Floor 55 psf, Typical Roof 37 psf

Job Number

RIOPE

Specification Quantity

9

A Professional Engineer's seal on to this Index Sheet indicates the acceptance of Professional Engineering responsibilities for individual truss components fabricated in accordance with the listed and attached Truss Specification Sheets. Determination as to the suitability of these individual truss components for any structure is the responsibility of the Building Designer, as defined in ANSI/TPI 1-2002, Section 2.2. Permanent files of the original Truss Specification Sheets are maintained by Architectural Services and Engineering, Inc. Questions regarding this Index Sheet and/or the attached Specification Sheets may be directed to the truss fabricator listed above.

Truss ID FT FT1	Truss ID					
FT2 FT3						
FT4 FT4A FT5 FT6						
FT7						

ENGINEER OF RECORD

Not Specified

Robert W. Wall P.E. F. Reg. 46021

Phose In Blook

Architectural Services and Engineering, Inc.

Florida 24710 State Road 54 Lutz, Florida 33559 1-813-948-2812 FAX: 1-813-949-2016 Florida engineering license CA 7882

Texas 3000 Sage Road, Suite 1374 Houston, Texas 77056 1-713-963-8840 FAX: 1-713-963-9840 Texas engineering license 95105

E-Mail: office@asande.com

Designers and engineers since 1965 TRUSS REVIEW COVER SHEET Date Checked Checked By Date Job Number Received RIOPE FL 8.9.06 Hold (date) Number of Trusses ___ Number of Raised Sealed Copies Number of Flat Sealed Copies Cover Sheet

BFS-TAMPA Mailed daily. UPS GROUND only.

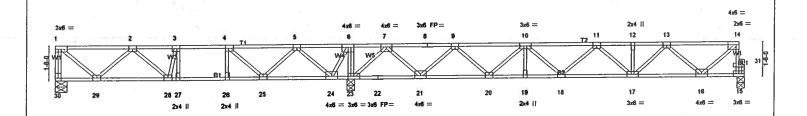
Jop	Truss	Truss Type	Qty	Ply	Burton L. Fish Const. Inc_Riopelle_FI
MASTER	FT	FLOOR	12		Job Reference (optional)

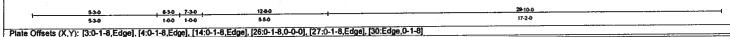
Tampa, Builders FirstSource, Tampa, Florida 33619, Builders FirstSource

1-6-0

2-00

080 | 150





LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING 2-0-0 Plates increase 1.00 Lumber increase 1.00 Rep Stress incr YES Code FBC2004/TPI2002	CSI TC 0.63 BC 0.87 WB 0.45 (Matrix)	Vert(LL) -0.12 18	l/defl L/d >999 360 >999 240 n/a n/a	PLATES GRIP MT20 244/190 Weight: 160 lb
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LUMBER
TOP CHORD 4 X 2 SYP No.2
BOT CHORD 4 X 2 SYP No.2
WFBS 4 X 2 SYP No.3

BRACING

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 30=506/0-6-0, 15=786/0-4-0, 23=1943/0-4-0 Max Grav30=594(load case 2), 15=823(load case 4), 23=1943(load case 1)

FORCES (b) - First Load Case Only

TOP CHORD

1-30=-500, 15-31=-779, 1-2=-443, 2-3=-818, 3-4=-726, 4-5=-128, 5-6=-1203, 6-7=-1641, 7-8=-289, 8-9=-289, 9-10=-1537, 10-11=-2058, 11-12=-1849, 12-13=-1849, 13-14=-814

BOT CHORD

WEBS

2-30=-0, 28-29=827, 27-28=742, 26-27=726, 25-26=726, 24-25=-399, 23-24=-1641, 22-23=-535, 21-22=-535, 20-21=-1083, 19-20=-1978, 18-19=-1978, 17-18=-2105, 16-17=-1499, 15-16=-71

3-72=-339, 4-26=185, 6-23=-930, 1-29=-521, 2-28=-13, 3-28=302, 4-25=-797, 5-25=714, 5-24=-1091, 6-24=877, 14-16=990, 13-16=-930, 13-17=466, 12-17=-51, 11-17=-341, 11-18=-64

10-18=106, 10-19=6, 10-20=-588, 9-20=615, 9-21=-1077, 7-21=1118, 7-23=-1482

NOTES

1) Unbalanced floor live loads have been considered for this design.

2) All plates are 3x4 MT20 unless otherwise indicated.

3) The following joint(s) require plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection: 22 and 8.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or

restrained by other means.
5) CAUTION, Do not erect truss backwards.

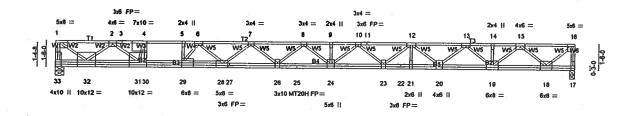
LOAD CASE(S) Standard



Job . ,	Truss	Truss Type	Qty	Ply	Burton L Fish Const. Inc_Riopelle_FI
MASTER	FT1	FLOOR	7	1	x = =
Tampa, Builders FirstSource, Ta	mpa, Florida 33619, Builders First	Source	L		Job Reference (optional) 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Aug 09 07:50:02 2006 Page 1

948 200 0109 1-6-0

Scale: 3/16"=



530 530

Flate Offsets (A,1). [1.6	:age,0-1-8[, [4:0-1-8,Eage], [5:0-1-8,0-0-0],	28:0-2-8,Edge], [29:0-1-8,Edg	gej, [33:Edge,0-1-8]	
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING 1-4-0 Plates Increase 1.00 Lumber Increase 1.00 Rep Stress Incr YES Code FBC2004/TPI2002	CSI TC 0.83 BC 0.97 WB 0.78 (Matrix)	DEFL in (loc) Vdefl L/d Verl(LL) -0.79 24-26 >452 360 Verl(TL) -1.23 24-26 >289 240 Horz(TL) 0.09 17 n/a n/a	PLATES GRIP MT20 244/190 MT20H 187/143 Weight: 212 lb

LUMBER
TOP CHORD 4 X 2 SYP No.2 "Except"
T2 4 X 2 SYP No.1D
BOT CHORD 4 X 2 SYP No.1D "Except"

B5 4 X 2 SYP No.2, B4 4 X 2 SYP No.2

WEBS 4 X 2 SYP No.3 BRACING **TOP CHORD BOT CHORD**

Structural wood sheathing directly applied or 3-9-12 oc purlins, except end verticals. Rigid celling directly applied or 10-0-0 oc bracing, Except: 2-2-0 oc bracing: 30-31,29-30.

REACTIONS (lb/size) 33=1085/0-6-0, 17=1085/0-4-0

FORCES (lb) - First Load Case Only
TOP CHORD
1-33=-1075, 16-17=-1078, 1-2=-1275, 2-3=-3484, 3-4=-3484, 4-5=-4236, 5-6=-4236, 6-7=-5436, 7-8=-5888, 8-9=-6085, 9-10=-6085, 10-11=-5638, 11-12=-5638, 12-13=-4687, 13-14=-3247, 14-15=-3247, 15-16=-1229 TOP CHORD

32-33=-0, 31-32=2438, 30-31=4139, 29-30=4236, 28-29=4891, 27-28=5768, 26-27=5802, 25-26=6090, 24-25=6090, 23-24=5962, 22-23=5306, 21-22=5306, 20-21=5306, 19-20=4088, 18-19=2347, 19-20=4088, 18-19=2347, 19-20=4088, 18-19=2347, 19-20=4088, 18-19=2347, 19-20=4088, 18-19=2347, 19-20=4088, 18-19=2347, 19-20=4088, 18-19=2347, 19-20=4088, 18-19=2347, 19-20=4088, 18-19=2347, 19-20=4088, 18-19=2347, 19-20=4088, 18-19=2347, 19-20=4088, 18-19=2347, 19-20=4088, 18-20=408 **BOT CHORD WEBS**

4-30=1879, 5-29=316, 1-32=1608, 2-32=-1515, 2-31=1358, 4-31=-2338, 16-18=1580, 15-18=-1485, 15-19=1176, 14-19=-27, 13-19=-1098, 13-20=797, 12-20=-808, 12-21=28, 12-23=433, 10-23=-431, 10-24=161, 9-24=-42, 8-24=-7, 8-26=-269, 7-26=114, 7-28=-456, 6-28=753, 6-29=-1125

NOTES

Unbalanced floor live loads have been considered for this design.
 All plates are MT20 plates unless otherwise indicated.
 All plates are 3x6 MT20 unless otherwise indicated.

4) The following joint(s) require plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection: 11, 22, 25, 27 and 3.

5) Required 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

LOAD CASE(S) Standard



Robert W. Wall, PE 46021 24710 State Road 54 Lutz, Fl 33559

- 1	Job	Truss					
	, ,	liuss		Truss Type	Qty	Ply	Burton L. Fish Const. Inc_Riopelle Fi
٠,	MASTER	ETT					
١		1712		FLOOR	2	1	
ı	Tampa, Builders FirstSource, Ta	mna Florida 22640	Duildon Find				Job Reference (optional)
-1		inpa, rionua 330 (9	, Duilders First	Source			6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Aug 09 07:50 03 2006, Page 1

944 2-0-0 1000

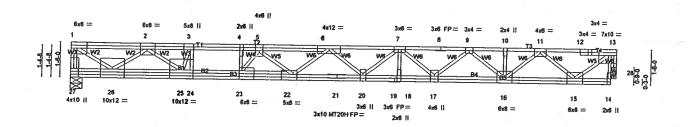


Plate Offsets (X,Y): [3:0-3-0,Edge], [4:0-3-0,0-0-0], [12:0-1-2,Edge], [13:0-4-8,Edge], [14:0-3-0,Edge], [22:0-2-0,Edge], [23:0-1-8,Edge], [27:Edge,0-1-8] LOADING (psf) TCLL 40.0 SPACING CSI TC 0.94 BC 0.87 WB 0.77 L/d 360 **PLATES** 1.00 1.00 YES Vert(LL) Vert(TL) 20 20 >620 TCDL BCLL 10.0 MT20 244/190 Lumber Increase -0.71>397 240 MT20H 187/143 0.0 Rep Stress Incr Horz(TL) BCDL n/a n/a 5.0 Code FBC2004/TPI2002 Weight: 188 lb LUMBER

TOP CHORD 4 X 2 SYP No.2
BOT CHORD 4 X 2 SYP No.1D *Except*
B4 4 X 2 SYP No.2 **WEBS** 4 X 2 SYP No.3 OTHERS 4 X 2 SYP No.3

BRACING TOP CHORD **BOT CHORD**

Structural wood sheathing directly applied or 2-2-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 27=1289/0-6-0, 28=1288/0-2-8

FORCES (b) - First Load Case Only
TOP CHORD
BOT CHORD
BOT CHORD
WEBS

TOP CHORD
BOT CHORD
BOT CHORD
BOT CHORD
WEBS

TOP CHORD
BOT CHORD
BO

NOTES

1) Unbalanced floor live loads have been considered for this design.

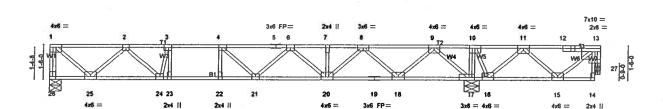
 1) Unbalanced floor live loads have been considered for this design.
 2) All plates are MT20 plates unless otherwise indicated.
 3) The following joint(s) require plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection: 19, 21 and 8.
 4) Bearing at joint(s) 28 considers parallel to grain value using ANSUTPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 28.
 6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means. restrained by other means.
7) CAUTION, Do not erect truss backwards.

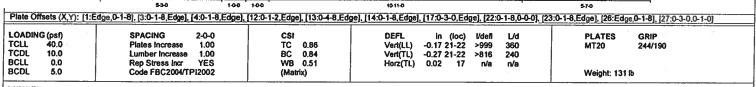
LOAD CASE(S) Standard



Robert W. Wall, PE 46021 24710 State Road 54 Lutz, Fl 33559

Job , ,	Truss	Truss Type		Qty	Pty	Burton L. Fish Const. Inc_Riopelle_FI
MASTER	FT3	FLOOR		2	1	
Tampa, Builders FirstSource	, Tampa, Florida 33619, Builden	s FirstSource	11 1.0	l		Job Reference (optional) 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Aug 09 07:50:03 2006 Page 1
•	180	2-0-0			J	188 070





LUMBER

TOP CHORD 4 X 2 SYP No.2 BOT CHORD 4 X 2 SYP No.1D *Except* B2 4 X 2 SYP No.2

WERS 4 X 2 SYP No.3 OTHERS 4 X 2 SYP No.3 BRACING TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 6-0-0 oc bracing: 17-18,16-17,15-16. **BOT CHORD**

Scale: 1/4"=

REACTIONS (ib/size) 26=788/0-6-0, 27=-386/0-2-8, 17=2175/0-8-0 Max Uplift27=-597(load case 2)

Max Grav26=790(load case 2), 27=106(load case 3), 17=2175(load case 1)

FORCES (ib) - First Load Case Only

TOP CHORD BOT CHORD WEBS

1-26=-783, 14-27=14, 13-27=389, 1-2⁻770, 2-3=-1829, 3-4=-2000, 4-5=-1917, 5-6=-1917, 6-7=-1186, 7-8=-1186, 8-9=353, 9-10=2641, 10-11=2193, 11-12=627, 12-13=626 25-26=-0, 24-25=1450, 23-24=1983, 22-23=2000, 21-22=2000, 20-21=1730, 19-20=554, 18-19=554, 17-18=-1292, 16-17=-2641, 15-16=-1245, 14-15=65 3-23=372, 4-22=-147, 10-17=-1020, 1-25=1009, 2-25=-922, 2-24=512, 3-24=-612, 4-21=-111, 6-21=254, 6-20=-724, 7-20=-54, 8-20=841, 8-18=-1231, 9-18=1273, 9-17=-1749, 13-15=-877,

11-15=838, 11-16=-1286, 10-16=978

NOTES

1) Unbalanced floor live loads have been considered for this design.

1) Unbalanced floor live loads have been considered for this design.
2) All plates are 3x4 MT20 unless otherwise indicated.
3) The following joint(s) require plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection: 19 and 5.
4) Bearing at joint(s) 27 considers parallel to grain value using ANS/ITPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 27.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 597 lb uplift at joint 27.
7) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

restrained by other means.

8) CAUTION, Do not erect truss backwards.

LOAD CASE(S) Standard



Robert W. Wall, PE 46021 24710 State Road 54 Lutz, Fl 33559

12	Job	Truss	Truss Type	Qty	Ply	Burton L. Fish Const. Inc_Riopelle_FI
~	MASTER	FT4	FLOOR	13	1	Zanovi za i sanovi sa inic_Nobalie_Fi
	Tampa, Builders FirstSource, Ta	mpa, Florida 33619, Builders First	Source			Job Reference (optional) 5.200 s Jul 13 2005 MiTek Industries, Inc. Wed Aug 09 07:50.04 2006 Page 1
		. 160 .	048 200 400			Page 1

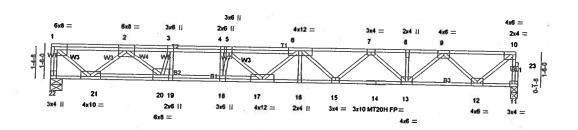


Plate Offsets (X,Y): [4:	530 530 0-3-0,0-0-0], [10:0-1-8,Edge], [19:0-3-0,Edge	1-630 7-30 1-00 1-00	20.1-8 12-10-8	
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING 2-0-0 Plates Increase 1.00 Lumber Increase 1.00 Rep Stress Incr YES Code FBC2004/TPI2002	CSI TC 0.60 BC 0.83 WB 0.67 (Matrix)	DEFL in (loc) I/defi L/d Vert(LL) -0.30 17-18 >800 360 Vert(TL) -0.47 17-18 >512 240 Horz(TL) 0.07 11 n/a n/a	PLATES GRIP MT20 244/190 MT20H 187/143 Weight: 134 lb
LUMBER TOP CHORD 4 X 2 SYR BOT CHORD 4 X 2 SYR B3 4 X 2 SYR WEBS 4 X 2 SYR	P No.1D *Except* SYP No.2	F 3 00 190452	BRACING TOP CHORD Structural wood sheathing dire Rigid ceiling directly applied or	ectiv applied or 6-0-0 oc purting agreed and water to

REACTIONS (lb/size) 22=1093/0-6-0, 11=1087/0-3-8

FORCES (ib) - First Load Case Only
TOP CHORD
1-22=-1085, 11-23=-1081, 10-23=-1080, 1-2=-1369, 2-3=-3218, 3-4=-3665, 4-5=-3708, 5-6=-4277, 6-7=-3796, 7-8=-2838, 8-9=-2838, 9-10=-1134
BOT CHORD
21-22=0, 20-21=2333, 19-20=3619, 18-19=3665, 17-18=3945, 16-17=4217, 15-16=4181, 14-15=3446, 13-14=3446, 12-13=2148, 11-12=49
WEBS
3-19=899, 4-18=857, 1-21=1663, 2-21=1331, 2-20=1150, 3-20=-1429, 10-12=1446, 9-12=-1376, 9-13=919, 8-13=-40, 7-13=-810, 7-15=470, 6-15=-482, 6-16=-176, 6-17=84, 5-17=456, 5-18=-1264

NOTES

1) Unbalanced floor live loads have been considered for this design.

2) All plates are MT20 plates unless otherwise indicated.

3) The following joint(s) require plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection: 14.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) CAUTION, Do not erect truss backwards.

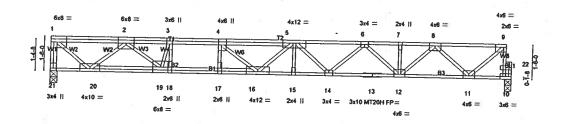
LOAD CASE(S) Standard



- 1	Job	Truss	Toron Trans			
. [Truss Type	Qty	Ply	Burton L. Fish Const. Inc_Riopelle_FI
1	MASTER	FT4A	FLOOR	4	1	
	Tampa, Builders FirstSource, Ta	mpa, Florida 33619, Builders First	Source			Job Reference (optional) 6.200 s Jul 13 2005 MiTek Industries Inc. Wed Aug 09 07:50-04-2005 Description

1-6-0 048 2-00 1-7-8

930



530 530 Plate Offsets (X,Y): [4:0-3-0,Edge], [9:0-1-8,Edge], [17:0-3-0,0-0-0], [18:0-3-0,Edge], [21:Edge,0-1-8] LOADING (psf) TCLL 40.0 2-0-0 CSI DEFL TCLL in (loc) -0.31 16-17 L/d 360 240 TC 0.66 BC 0.94 WB 0.67 **PLATES** Plates Increase Lumber Increase 1.00 GRIP Vert(LL) Vert(TL) Horz(TL) >769 10.0 244/190 -0.48 16-17 0.0 MT20H Rep Stress Incr YES 187/143 0.07 BCDL 10 Code FBC2004/TPI2002 (Matrix) Weight: 132 lb

LUMBER

TOP CHORD 4 X 2 SYP No.2 BOT CHORD 4 X 2 SYP No.1D *Except* B3 4 X 2 SYP No.2

WEBS 4 X 2 SYP No.3

BRACING TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing. Except: 2-2-0 oc bracing: 18-19,17-18.

REACTIONS (lb/size) 21=1090/0-3-8, 10=1077/0-3-8

FORCES (ib) - First Load Case Only TOP CHORD 1-21=-1082, 10-22=-1 BOT CHORD 20-21=0, 19-20=2326

WEBS

- First Load Case Uniy
1-21=-1082, 10-22=-1070, 9-22=-1070, 1-2=-1365, 2-3=-3196, 3-4=-3687, 4-5=-4195, 5-6=-3793, 6-7=-2854, 7-8=-2854, 8-9=-1171
20-21=0, 19-20=2326, 18-19=3631, 17-18=3687, 16-17=3687, 15-16=4212, 14-15=4176, 13-14=3442, 12-13=3442, 11-12=2174, 10-11=97
3-18=1084, 4-17=-488, 1-20=1659, 2-20=-1326, 2-19=1130, 3-19=-1551, 9-11=1430, 8-11=-1361, 8-12=906, 7-12=-49, 6-12=-783, 8-14=471, 5-14=-480, 5-15=-182, 5-16=-17, 4-16=664

1) Unbalanced floor live loads have been considered for this design.
2) All plates are MT20 plates unless otherwise indicated.
3) The following joint(s) require plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection: 13.
4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means. restrained by other means.

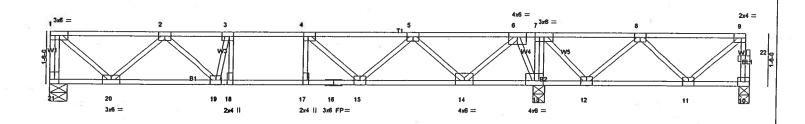
5) CAUTION, Do not erect truss backwards.

LOAD CASE(S) Standard



Robert W. Wall, PE 46021 24710 State Road 54 Lutz, Fl 33559

Job	Truss	Truss Type	Qty	Ply	Burton L. Fish Const. Inc_Riopelle_FI	
MASTER	FT5	FLOOR	1	1		- 1
			l		Job Reference (optional)	- 1
Tampa, Builders FirstSource, T	ampa, Florida 33619, Builders First	Source			6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Aug 09 07:50:05 2006 P	age 1
						٠
1-60	<u> 048</u>	200		1	1-0-0 1-3-0 0 ₁ 1-0	



TCLL 40.0 Plates Increase 1.00 TC 0.54 Vert(LL) -0.06 18 >999 360 MT20 244/190 TCDL 10.0 Lumber Increase 1.00 BC 0.52 Vert(TL) -0.09 18 >999 240 BCLL 0.0 Rep Stress Incr YES WB 0.40 Horz(TL) 0.01 13 r/s r/s		530	100	1-0-0	690	+	20-1-8 6-1-8	
TCLL 40.0 Plates Increase 1.00 TC 0.54 Vert(LL) -0.06 18 >999 360 MT20 244/190 TCDL 10.0 Lumber Increase 1.00 BC 0.52 Vert(TL) -0.09 18 >999 240 BCLL 0.0 Rep Stress Incr YES WB 0.40 Horz(TL) 0.01 13 n/a n/a	Plate Offsets (X,Y): [3:0	-1-8,Edge], [4:0-1-8,Edge], [9:0	-1-8,Edge], [17:0-1-8,0-0-0] <u>, [18:0-1-8,</u> E	dge], [21:Edge,0-1-8], [22:0-1-8,0-1	-0]		
	TCDL 10.0 BCLL 0.0	Plates Increase 1. Lumber Increase 1. Rep Stress Incr Y	.00 .00 ES	TC 0.54 BC 0.52 WB 0.40	Vert(LL) -0.06 18 Vert(TL) -0.09 18	>999 360 >999 240	MT20 244/190	

LUMBER
TOP CHORD 4 X 2 SYP No.2
BOT CHORD 4 X 2 SYP No.2
WEBS 4 X 2 SYP No.3

BRACING TOP CHORD **BOT CHORD**

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (ib/size) 21=639/0-6-0, 10=37/0-3-8, 13=1503/0-4-0 Max Uplift10=-187(load case 2)

Max Grav21=644(load case 2), 10=236(load case 3), 13=1503(load case 1)

FORCES (lb) - First Load Case Only
TOP CHORD
BOT CHORD
BOT CHORD
BOT CHORD
WEBS

1-21=-634, 10-22=-33, 9-22=-33, 1-2=-597, 2-3=-1296, 3-4=-1326, 4-5=-976, 5-6=87, 6-7=1209, 7-8=728, 8-9=98
20-21=0, 19-20=1121, 18-19=1326, 16-17=1326, 16-17=1326, 14-15=617, 13-14=-818, 12-13=-1209, 11-12=-223, 10-11=2
3-18=-17, 4-17=24, 7-13=-620, 1-20=782, 2-20=-710, 2-19=237, 3-19=-121, 4-15=-466, 5-15=487, 5-14=-955, 6-14=991, 6-13=-945, 9-11=-133, 8-11=170, 8-12=-685, 7-12=681

NOTES

- 1) Unbalanced floor live loads have been considered for this design.
 2) All plates are 3x4 MT20 unless otherwise indicated.
 3) The following joint(s) require plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection: 16.
 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 187 lb uplift at joint 10.
 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or extrained by other capable. restrained by other means.
 6) CAUTION, Do not erect truss backwards.

LOAD CASE(S) Standard

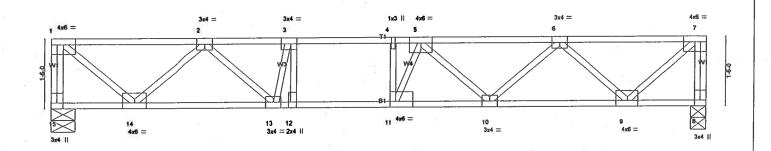


Robert W. Wall, PE 46021 24710 State Road 54 Lutz, Fl 33559

Job	Truss	Truss Type	Qty	Ply	Burton L. Fish Const. Inc_Riopelle_FI	
MASTER	FT6	FLOOR	3	1	Job Reference (optional)	
Tampa, Builders FirstSource, Tampa, Florida 33619, Builders FirstSource				6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Aug 09 07:50:05 2006 Page		

0-8-8

Scale: 1/2"=1



	530	1-0-0	1-0-0	6-11-	0				
Plate Offsets (X,Y): [1:Edge,0-1-8], [3:0-1-8,Edge], [11:0-1-8,Edge], [12:0-1-8,Edge], [15:Edge,0-1-8]									
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING 2-0-0 Plates Increase 1.00 Lumber Increase 1.00 Rep Stress Incr YES Code FBC2004/TPI2002	CSI TC 0.50 BC 0.86 WB 0.39 (Matrix)	1	PEFL in (loc) Vdefl L/d lent(LL) -0.13 10-11 >999 360 lent(TL) -0.19 10-11 >902 240 lorz(TL) 0.03 8 n/a n/a	PLATES GRIP MT20 244/190 Weight: 78 lb				

LUMBER TOP CHORD 4 X 2 SYP No.2 BOT CHORD 4 X 2 SYP No.2 WEBS 4 X 2 SYP No.3 BRACING TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 15=765/0-6-0, 8=765/0-4-0

FORCES (b) - First Load Case Only
TOP CHORD
BOT CHORD
WEBS
1-15=-760, 7-8=-743, 2-3=-1752, 3-4=-1879, 4-5=-1879, 5-6=-1701, 6-7=-749
1-15=-0, 13-14=1398, 12-13=1869, 11-12=1879, 10-11=1919, 9-10=1419, 8-9=0
3-12=210, 4-11=33, 1-14=973, 2-14=-889, 2-13=477, 3-13=-465, 7-9=981, 6-9=-909, 6-10=382, 5-10=-296, 5-11=-100

NOTES

1) Unbalanced floor live loads have been considered for this design.

2) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

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Job Truss Truss Type Qty Burton L. Fish Const. Inc_Riopelle_Fi MASTER F17 FLOOR 13 Job Reference (optional) 6.200 s Jul 13 2005 MITek Industries, Inc. Wed Aug 09 07:50:06 2006 Page 1 Tampa, Builders FirstSource, Tampa, Florida 33619, Builders FirstSource 1-6-0 Scale: 1/2"=1 1x3 II 7x10 = 5 14 12 10 3x6 == 1x3 || 1x3 [[698 Plate Offsets (X,Y): [1:Edge,0-1-8]. [3:0-1-8,Edge]. [7:0-1-2,Edge]. [8:0-4-8,Edge]. [12:0-1-8,Edge]. [16:Edge,0-1-8] LOADING (psf) TCLL 40.0 TCDL 10.0 **SPACING** DEFL in (loc) -0.11 11-12 L/d 360 240 **PLATES** GRIP TC BC WB 1.00 1.00 Vert(LL) Vert(TL) Plates Increase 0.49 >999 MT20 244/190 Lumber Increase 0.81 -0.17 11-12 0.0 Rep Stress Incr YES 0.38 Horz(TL) 0.01 17 BCDL Code FBC2004/TPI2002 (Matrix) Weight: 78 lb LUMBER BRACING TOP CHORD 4 X 2 SYP No.2 BOT CHORD 4 X 2 SYP No.2 WEBS 4 X 2 SYP No.3 Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing. TOP CHORD BOT CHORD OTHERS 4 X 2 SYP No.3 REACTIONS (lb/size) 16=755/0-6-0, 17=754/0-2-8 FORCES (Ib) - First Load Case Only TOP CHORD 1-16=-750, 9-17=15, I BOT CHORD 15-16=-0, 14-15=1370 7-1151_1063_053_0519 1-16=-750_9-17=15, 8-17=-747, 1-2=-731, 2-3=-1714, 3-4=-1833, 4-5=-1840, 5-6=-1670, 6-7=-788, 7-8=-794 15-16=-0, 14-15=1376, 13-14=1824, 12-13=1833, 11-12=1884, 10-11=1395, 9-10=46 3-13=188, 4-12=161, 1-15=957, 2-15=-875, 2-14=458, 3-14=-435, 8-10=940, 6-10=-823, 6-11=374, 5-11=-289, 5-12=-223 WEBS 1) Unbalanced floor live loads have been considered for this design. 1) Unbalanced noor live loads have been considered for this design.
2) All plates are 3x4 MT20 unless otherwise indicated.
3) Bearing at joint(s) 17 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 17.
5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or

NOTES

restrained by other means.

6) CAUTION, Do not erect truss backwards.

LOAD CASE(S) Standard



Robert W. Wall, PE 46021 24710 State Road 54 Lutz, Fl 33559

