

BRACING REACTIONS, PANEL SHEAR					
	± Reactions (k)				Panel Shear (lb/ft)
	± Wind		± Seismic		
Loc	Horz	Vert	Horz	Vert	
1. SW 1	Ridg	Frome	A	Endwall	0.7
2. SW 4	2.4	6.8	A	Endwall	0.7
3. EW 4	Ridg	Frome	A	Endwall	0.6
4. SW 3	3.2	6.3	3.6	1.0	0.6
5. SW A					

GENERAL NOTES

- (1) APPLICATION OF ENGINEERS SEAL IS FOR METAL BUILDING ONLY AND DOES NOT REPRESENT THE PROFESSIONAL OF RECORD.
- (2) FOUNDATION DESIGN AND CONSTRUCTION ARE NOT THE RESPONSIBILITY OF THE METAL BUILDING MANUFACTURER.
- (3) ANCHOR BOLTS SHALL BE ACCURATELY SET TO A TOLERANCE OF $\pm 1/8"$ IN BOTH ELEVATION AND LOCATION.
- (4) THE BUILDING REACTION DATA REPORTS THE LOADS WHICH THIS BUILDING PLACES ON THE FOUNDATION. THE ENGINEER HAS TO DESIGN A BUILDING ENVELOPE TO SUPPORT THE BUILDING REACTIONS IN ADDITION TO OTHER LOADS IMPOSED BY THE BUILDING USE OR OCCUPANCY WITH RESPECT TO JOB SITE CONDITIONS.
- (5) ALL ANCHOR BOLTS TO BE ASTM SPECIFICATION A307 UNLESS OTHERWISE NOTED.
- (6) VALUES GIVEN FOR BENDS AND ANCHOR BOLT TOTAL LENGTHS ARE SUGGESTED LENGTHS ONLY. IT IS THE RESPONSIBILITY OF THE FOUNDATION ENGINEER TO DETERMINE THESE VALUES SINCE THEY ARE A FUNCTION OF THE CONCRETE STRENGTH AS WELL AS OTHER FACTORS.

MEZZ LOAD
DEAD LOAD : 50 psf
LIVE LOAD : 75 psf
TOTAL LOAD : 125 psf

Protection of Opening

This building is located in a wind-borne debris region. Exterior glazing is assumed to be impact resistant and meet the provisions of the missile test, or they should be protected by impact resistant covering meeting the requirements of SSTD 12, ASTM E 1886 and ASTM 1996 or Miami-Dade PA 201, 202, & 203. Openings may also be protected by structural wood panels having a min. thickness of 7/16" and maximum panel span of 8 feet. Attachment hardware and fastening schedule shall be in accordance with the following table.

WIND-BORNE DEBRIS PROTECTION FASTENING SCHEDULE FOR WOOD STRUCTURAL PANELS (PLYWOOD)				
FASTENER TYPE	FASTENER SPACING (in.) ^{1,2}			
	PANEL SPAN ≤ 2ft 4	2ft 4 PANEL SPAN ≤ 4 ft.	4ft 4 PANEL SPAN ≤ 6 ft.	6ft 4 PANEL SPAN ≤ 8 ft.
2 1/2 #6 Wood Screw ^{3,5}	16	16	12	9
2 1/2 #8 Wood Screw ³	16	16	16	12
Double-Headed nails ⁴	12	6	4	3

SI: 1 inch=25.4 mm 1 foot=305 mm

Notes:

- 1.) This table is based on a maximum wind speed of 130 m.p.h. (58 m/s) and mean roof height of 33 feet (10 m) or less.
- 2.) Fasteners shall be installed at opposing ends of the wood structural panel.
- 3.) Where screws are attached to masonry or masonry/stucco, they shall be attached using vibration-resistant anchors having a minimum withdrawal capacity of 490 lb. (2180 kN).
- 4.) Nails shall be 10d common or 12d box double-headed nails.
- 5.) Where screws are attached to pre-engineered metal building components, i.e. Door Jamb, framed openings, etc., they shall be #12 self drilling screws secured to a minimum 16 ga. material. Screws should have a min. withdrawal strength of 500 lbs

[illegible]

REVISIONS						
REV.	DESCRIPTION:	DATE	DTLR	DATE	CHKR	APPL

DRAWING STATUS		FLOOR-N-DECK	
[X] FOR CONSTRUCTION	VULCAN STEEL STRUCT.	ANCHOR BOLT REACTIONS	
[] FOR PERMIT ONLY	PROJECT 35.00' x 50.0' x 20.0' x 23.9	DESIGN	DRAFTSKP/MMD CHECK
[] FOR APPROVAL	ID 18056-2	DATE: 12/12/07	SHEET 2
[] OTHER, EXPLAIN	PROJECT LAKE CITY, FL	ADDRESS	

RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Line Col	Line	Anchor Bolt No	Anchor Bolt Dia (in)	Base Plate Width	Base Plate Length	Base Plate Thickness (in)	GROUT (in)
1	A	4	0.750	8.000	10.63	0.500	0.0
2	B	4	0.750	8.000	10.75	0.500	0.0
3	C	4	0.750	8.000	8.000	0.500	0.0
4	D	4	0.750	8.000	8.000	0.500	0.0

FRAME LINE

COLUMN LINE

Firm Col Line Line	Anc. Bolt No D (in)	Base Plate Wid (in)	Len (in)	Thk (in)	Grout
3 A	4	0.750	8.000	10.63	0.500
3 B	4	0.750	8.000	11.00	0.500
3 C	4	0.750	8.000	8.280	0.500
3 D	4	0.750	8.000	8.280	0.500

Item	Unit	Car	Alt.	Den.	Wd.	Fl.	Side	Wind	Wind
Line	Line	Line	No.	(0.0)	Den.	Fl.	Side	(m)	(m)
1	A	4	0.750	8.000	10.63	0.500	0.0	0.0	0.0
2	B	4	0.750	8.000	11.00	0.500	0.0	0.0	0.0
3	C	4	0.750	8.000	8.280	0.500	0.0	0.0	0.0

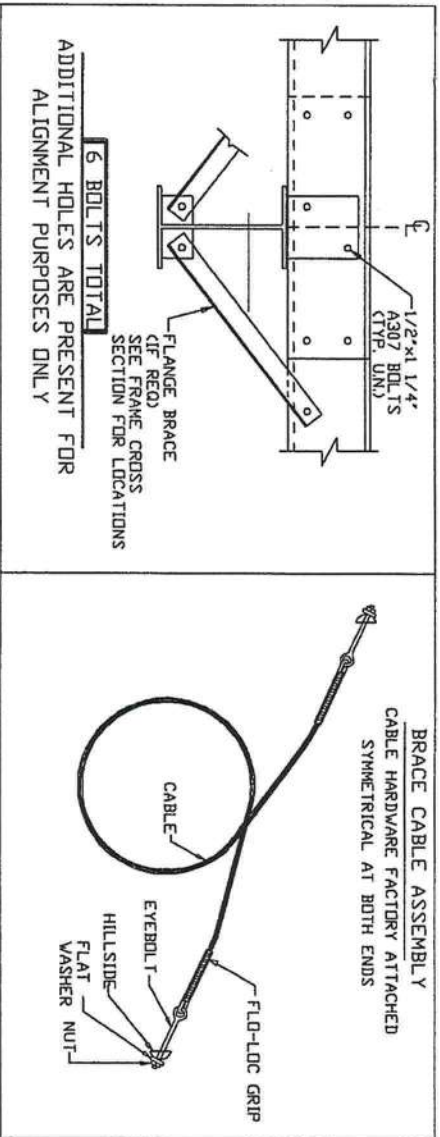
	Closed/Open	= C
	Importance Wind	= 1.00
	Importance Seismic	= 1.00
	Seismic Zone	= B
	Elastic Coeff (Fo/Ss)	= 0.22

Id	Description
1	DL+CL+LL
2	DL+CL+0.75LL+0.75WL1
3	DL+CL+0.75LL+0.75WL1
4	0.60DL+WLR1
5	0.60DL+WLR1
6	0.60DL+WLR2
7	0.60DL+WLR2
8	0.60DL+LWWRd1
9	0.60DL+LWWRd2
10	0.60DL+LWWRd2
11	DL+CL+FAux-2
12	DL+CL+FAux-2
13	DL+CL+LL+FIPAT_LL_1
14	1.03DL+1.03CL+SeisL+FIPAT_LL_4
15	1.03DL+1.03CL+SeisL+FIPAT_LL_5
16	DL+CL+LL+F2PAT_LL_1
17	1.03DL+1.03CL+SeisL+F2PAT_LL_4
18	1.03DL+1.03CL+SeisL+F2PAT_LL_5

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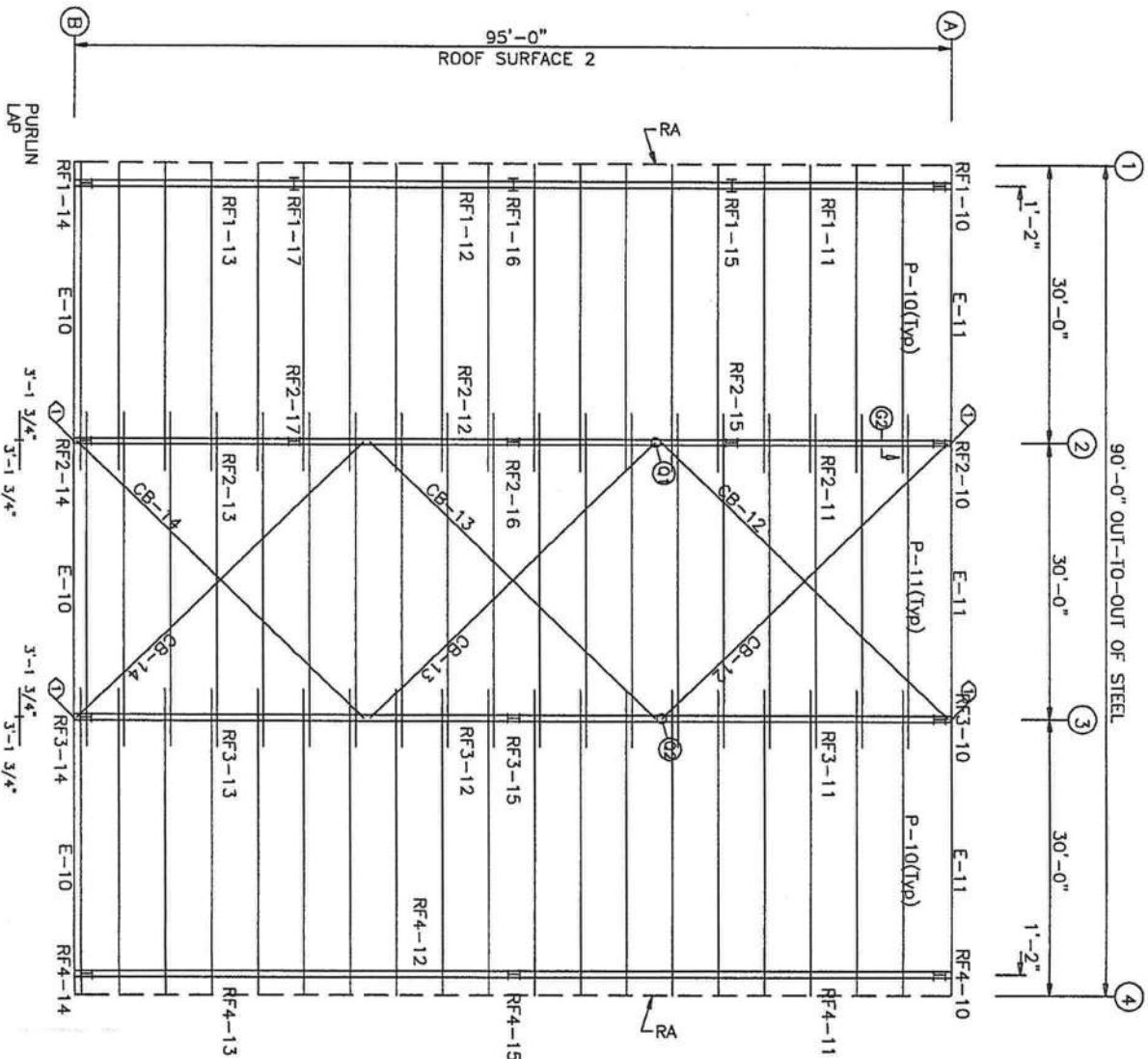
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By Richard T. Smith at 12:49 pm, Dec 12, 2007

ADD



SPECIAL BOLTS				
ROOF PLAN				
Q ID	QUAN	TYPE	DIA	LENGTH WASH
1	4	A307	1/2"	1 1/4" 0

MEMBER TABLE			
ROOF PLAN			
MARK	PART	LENGTH	
P-10	10X25Z13	35'-1 1/2"	
P-11	10X25Z14	36'-3 1/2"	
E-10	10E27SD14	29'-11 1/2"	
E-11	10E27SD14	29'-11 1/2"	
CB-12	0.312CBL	43'-2"	
CB-13	0.250CBL	43'-4"	
CB-14	0.312CBL	43'-7"	



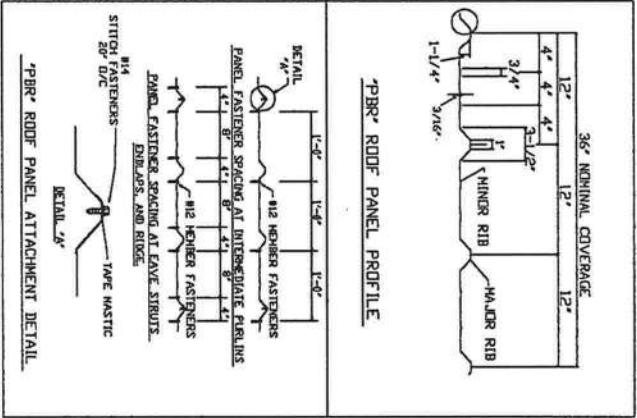
ROOF FRAMING PLAN

25'-8"	20'-5"	25'-5"	25'-4"
(30)	(30)	(30)	(30)

ROOF SHEETING
PANELS: 26 Co. PBR
Galvalume

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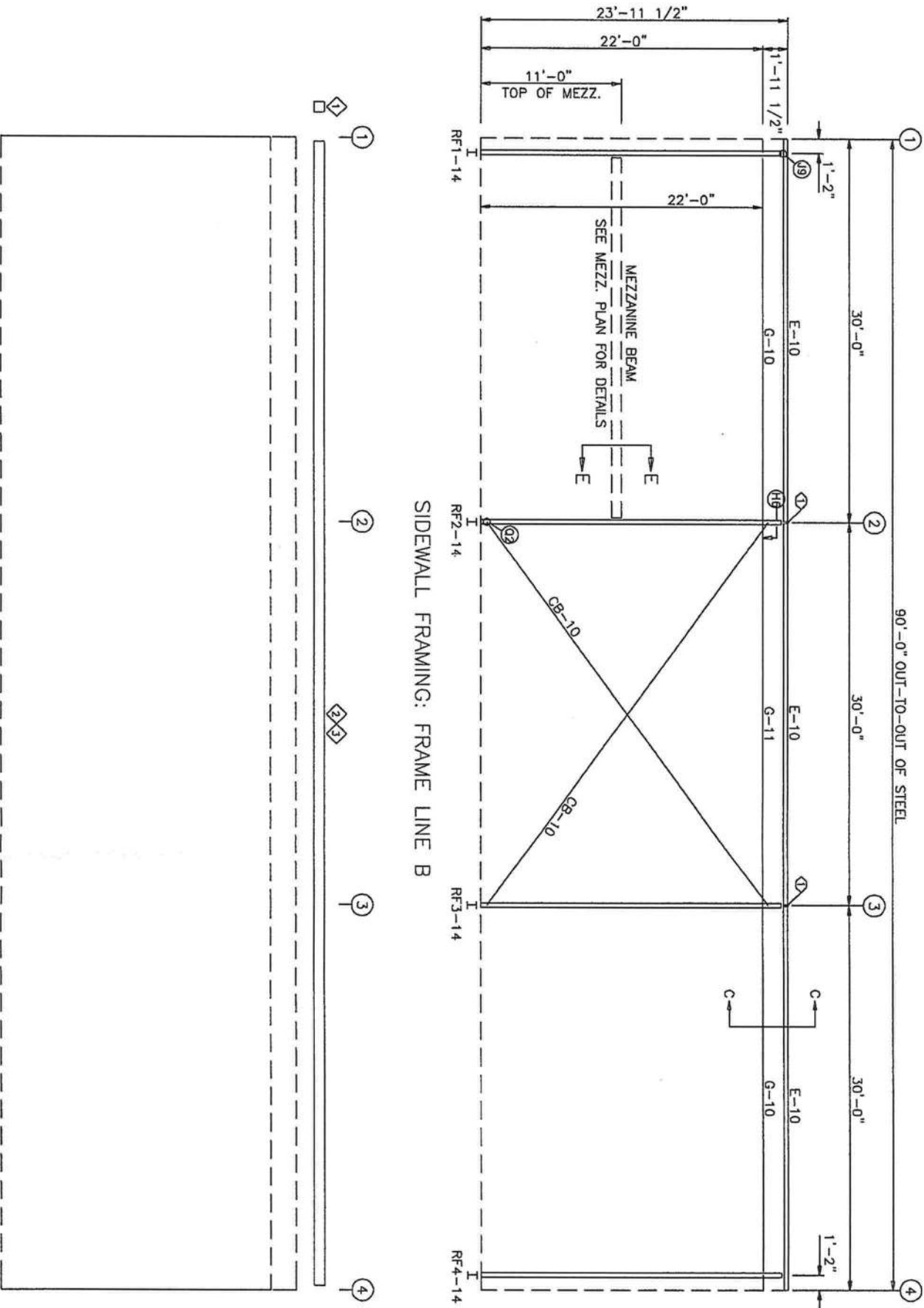
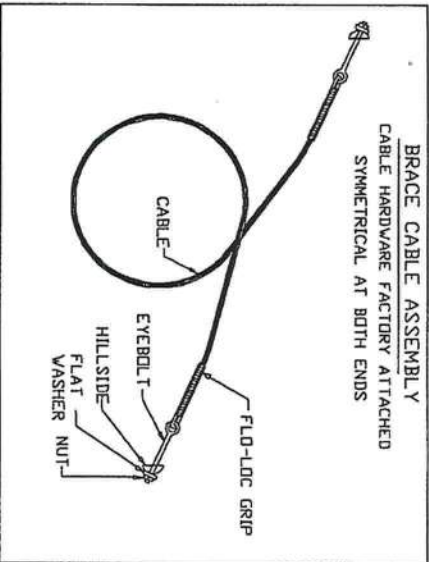
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REVISIONS				
REV.	DESCRIPTION	DATE	DLR	CHKR

DRAWING STATUS				

VULCAN STEEL STRUCT.		FLOOR-N-DECOR	
PROJECT	95007 x 980 x 200 x 239	ROOF FRAMING	
ID	18056-2	DESIGN: DRAFT/SKP/AMD/CHECK	
PROJECT	LAKE CITY FL	DATE: 12/12/07	SHEET 7



SIDEWALL SHEETING & TRIM: FRAME LINE B

TRIM TABLE			
FRAME LINE B		LENGTH	DETAIL
QID MARK			
1	VHSCB	6'-0"	TM8
2	HSET	20'-3"	TM8
3	HSET	10'-3"	TM8

SPECIAL BOLTS			
Q ID	QUAN	TYPE	DIA LENGTH WASH
1	4	A307	1/2" 1 1/4" 0

MEMBER TABLE			
FRAME LINE B		LENGTH	
MARK	PART		
E-10	10E275D14	29'-11 1/2"	
G-10	W8X24	28'-1 1/4"	
G-11	W8X24	29'-3 1/4"	
CB-10	0.500CBL	37'-4"	

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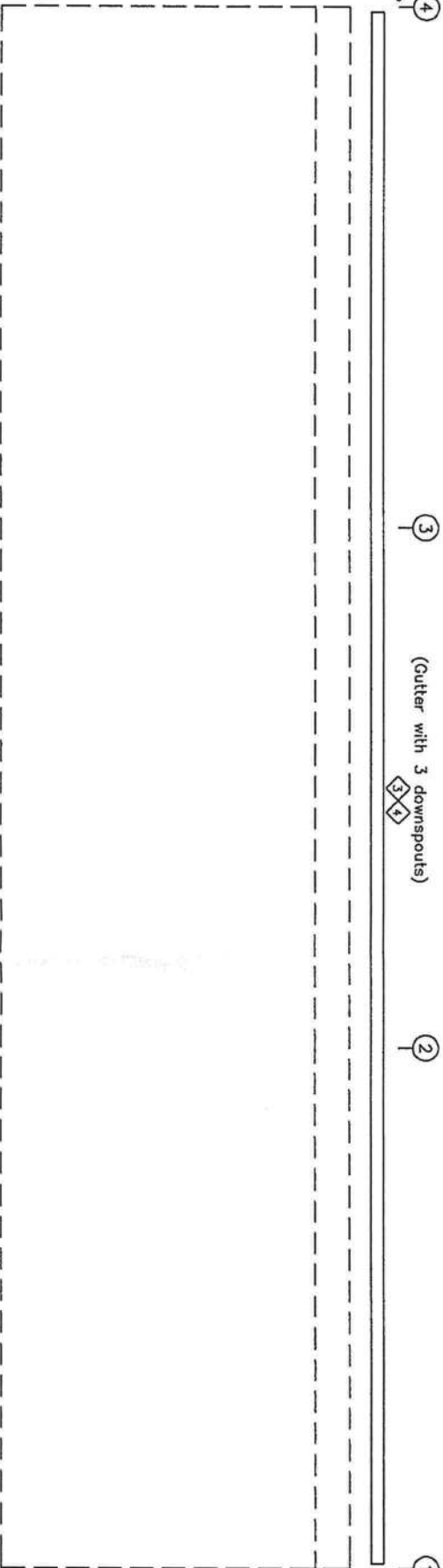
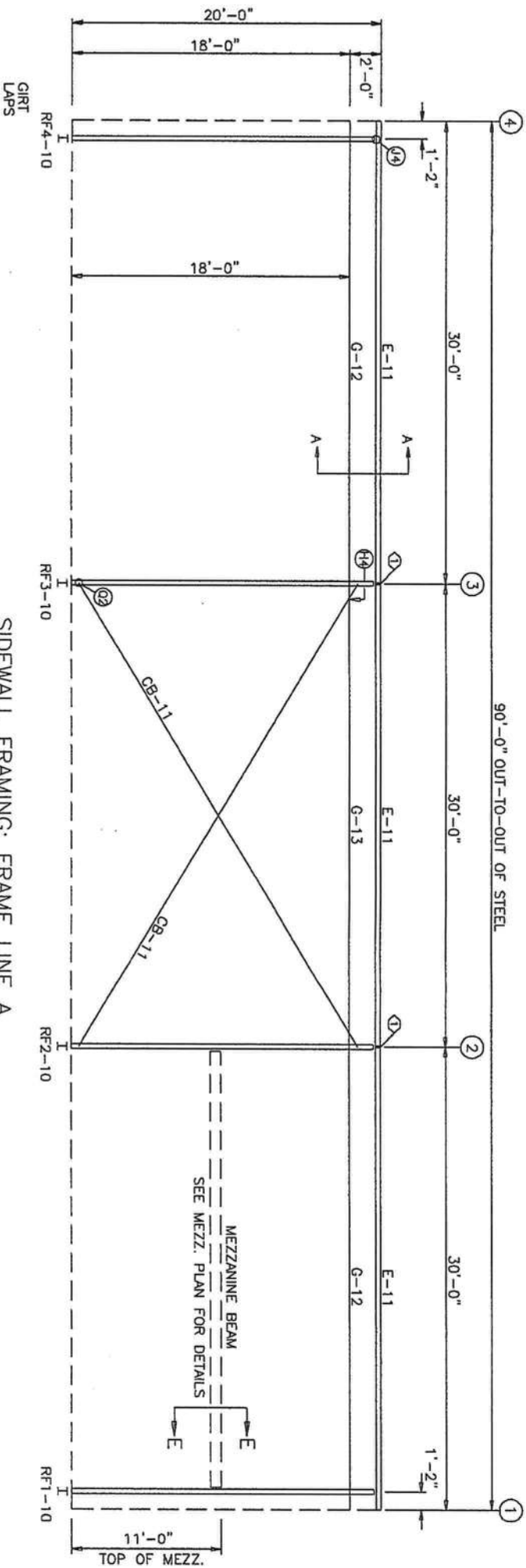
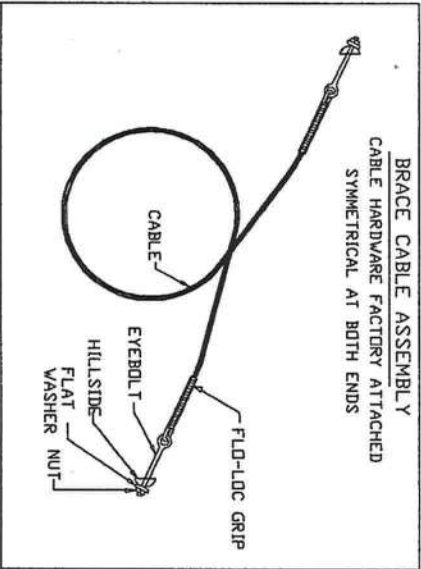
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By Richard T. Smith at 12:49 pm, Dec 12, 2007

SEAL

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GENERAL NOTES:
(1.) IF CABLE BRACING, WIND BENTS, WIND COLUMNS, OR WEAK AXIS DESIGN OF SIDE WALL COLUMNS WERE NOT PROVIDED IT HAS BEEN DETERMINED THAT DIAPHRAGM PANEL ACTION IS SUFFICIENT TO RESIST LONGITUDINAL FORCES. TEMPORARY BRACING SHOULD BE PROVIDED BY ERECTOR UNTIL ALL WALL AND ROOF PANELS ARE INSTALLED.

REVISIONS						DRAWING STATUS		VULCAN STEEL STRUCT.		FLOOR-N-DECK	
REV.	DESCRIPTION	DATE	DLR	DATE	CHKR	APPD		PROJECT ID	ADDRESS	SIDEWALL FRAMING	DATE
								18056-2	LAKE CITY FL	DRAFT-SKP/RMD/CHECK	10



TRIM TABLE			
QID	MARK	LENGTH	DETAIL
1	RVCB	6"	TM52
2	RVEG	6"	TM50
3	RVEG	20'-3"	TM50
4	RVEG	10'-3"	TM50

MEMBER TABLE			
MARK	PART	LENGTH	DETAIL
E-11	10E2/5D14	29'-11"	1/2"
G-12	W8X24	29'-11"	1/2"
G-13	W8X24	29'-11"	1/2"
CB-11	0.500CBL	35'-3"	1/2"

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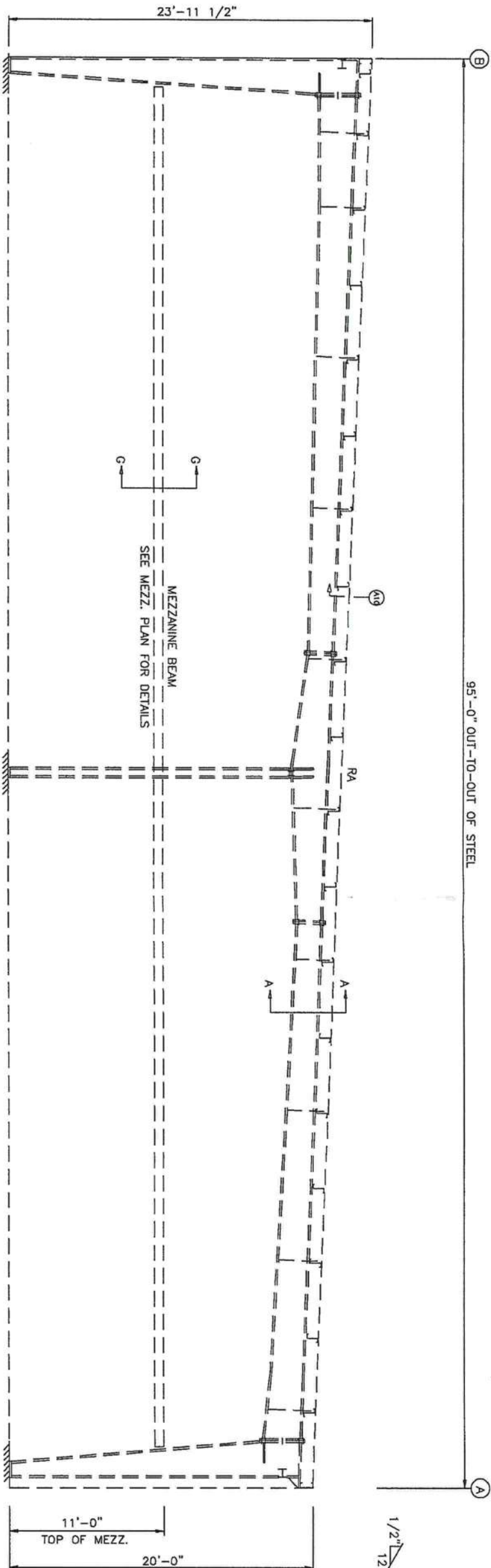
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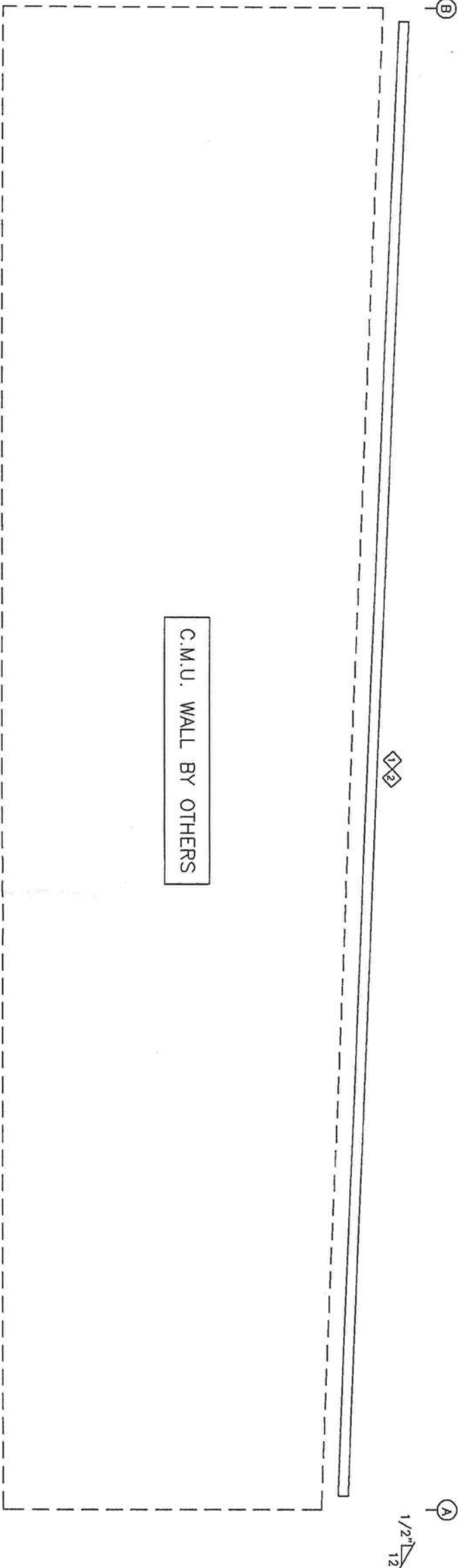
GENERAL NOTES:
(1) IF CABLE BRACING, WIND BENTS, WIND COLUMNS, OR WEAK AXIS DESIGN OF SIDE WALL COLUMNS WERE NOT PROVIDED IT HAS BEEN DETERMINED THAT DIAPHRAGM PANEL ACTION IS SUFFICIENT TO RESIST LONGITUDINAL FORCES. TEMPORARY BRACING SHOULD BE PROVIDED BY ERECTOR UNTIL ALL WALL AND ROOF PANELS ARE INSTALLED.

REVISIONS						DRAWING STATUS		VULCAN STEEL STRUCT.		FLDOR-N-DECOR	
REV.	DESCRIPTION	DATE	DLR	DATE	CHKR	APPD	☐ FOR CONSTRUCTION ☒ FOR PERMIT ONLY ☐ FOR APPROVAL ☐ OTHER, EXPLAIN	PROJECT	18056-2	SIDEWALL FRAMING	DESIGN
								PROJECT	LAKE CITY,FL		DRAFT/SKP/AMD/CHECK
								ADDRESS			SHEET 11

TRIM TABLE			
FRAME LINE 4			
ID	MARK	LENGTH	DETAIL
1	HIL0	20'-3"	SEE DETAIL
2	HIL0	15'-4"	



ENDWALL FRAMING: FRAME LINE 4



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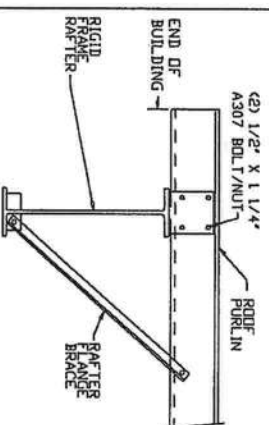
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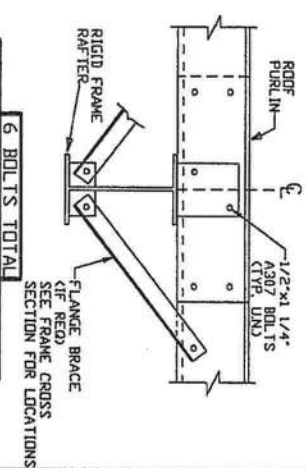
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GENERAL NOTES:
(1.) IF CABLE BRACING FOR END WALL IS NOT SHOWN ON ERECTION DRAWINGS IT HAS BEEN DETERMINED THAT DIAPHRAGM PANEL ACTION IS SUFFICIENT TO RESIST LONGITUDINAL FORCES. TEMPORARY BRACING SHOULD BE PROVIDED BY ERECTOR UNTIL ALL WALL AND ROOF PANELS ARE INSTALLED.

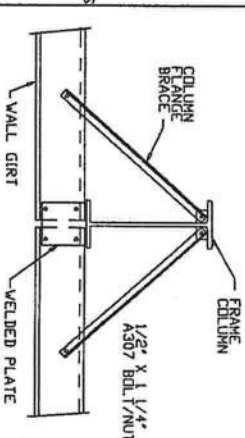
REVISIONS						DRAWING STATUS		VULCAN STEEL STRUCT.		FLOOR-N-DECK	
REV.	DESCRIPTION	DATE	DLR	DATE	CHKR	APPD		PROJECT	ID	DESIGN	SHEET
							[] FOR CONSTRUCTION [X] FOR PERMIT ONLY [] FOR APPROVAL [] OTHER, EXPLAIN	18056-2	18056-2	13	13



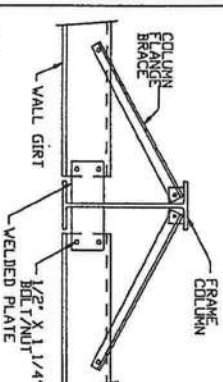
A10 ROOF PURLIN TO EXPANDABLE ENDWALL RIGID FRAME



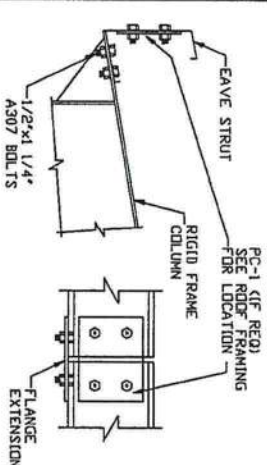
G2 ROOF PURLIN TO
RIGID FRAME



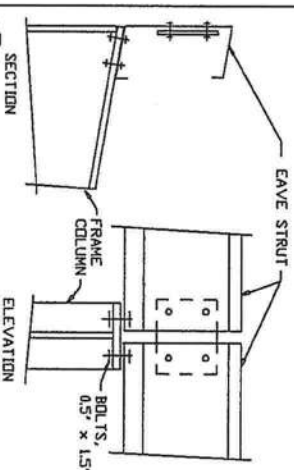
④ H4 WALL GIRT TO FRAME COLUMN



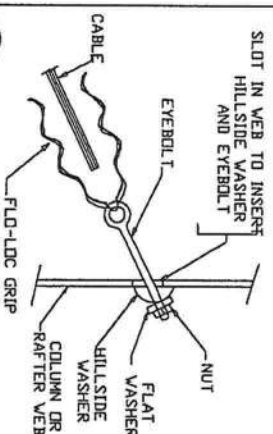
WALL GIRT TO FRAME COLUMN



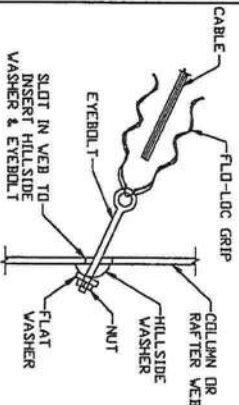
4 EAVE STRUT TO
RIGID FRAME



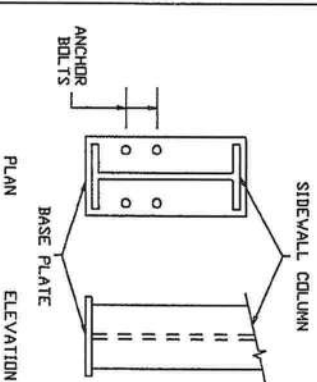
99 EAVE STRUT TO RIGID FRAME



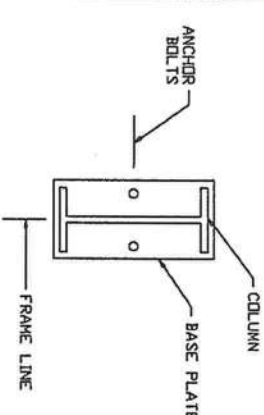
Q1 DIAGONAL CABLE, EYEBOLT END



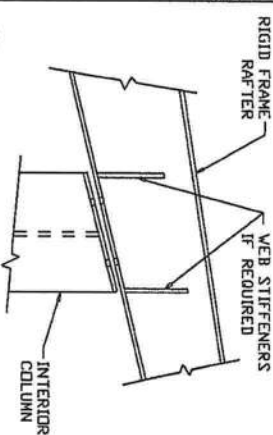
Q2 DIAGONAL CABLE
EYEBOLT END



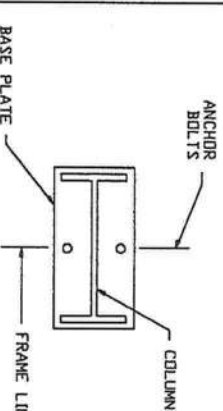
(R2) ANCHOR BOLTS AT SIDEWALL COLUMN



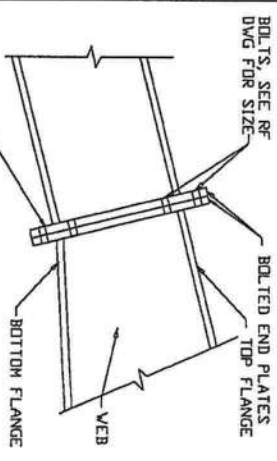
S1 INTERIOR COLUMN ANCHOR BOLTS



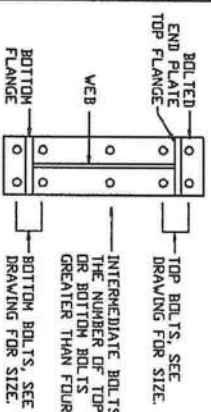
V2 INTERIOR COLUMN TO RAFTER



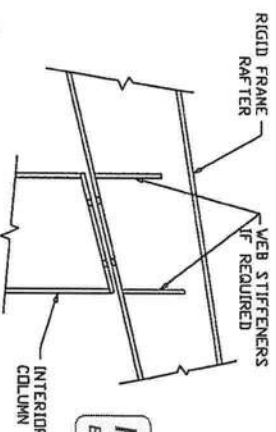
(S2) INTERIOR COLUMN ANCHOR BOLTS



U1 BOLTED END PLATE RAFTER SPLICE



U3 BOLTS FOR RAFTER TO
COLUMN CONNECTION



V1 INTERIOR COLUMN TO RAFTER

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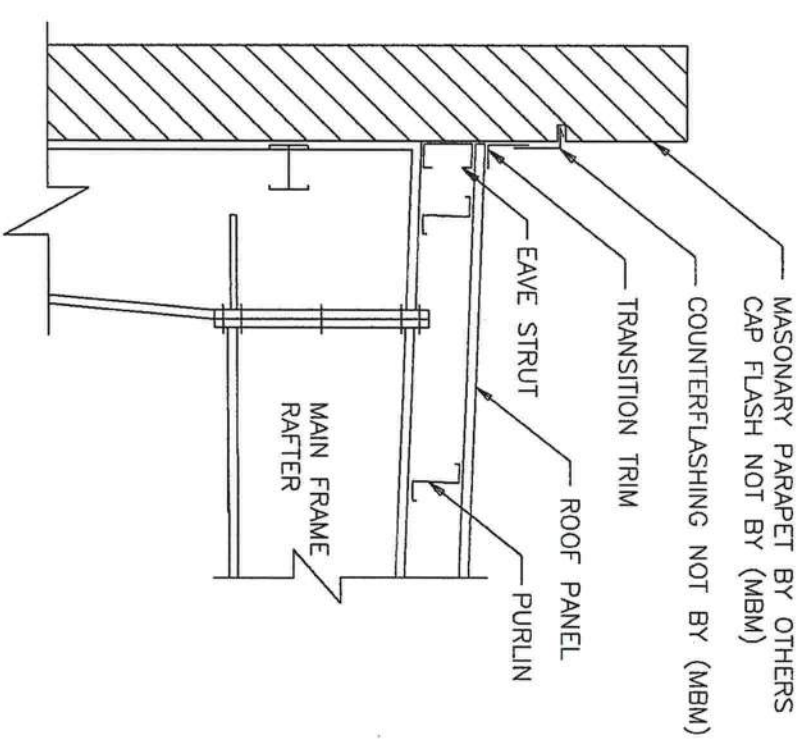
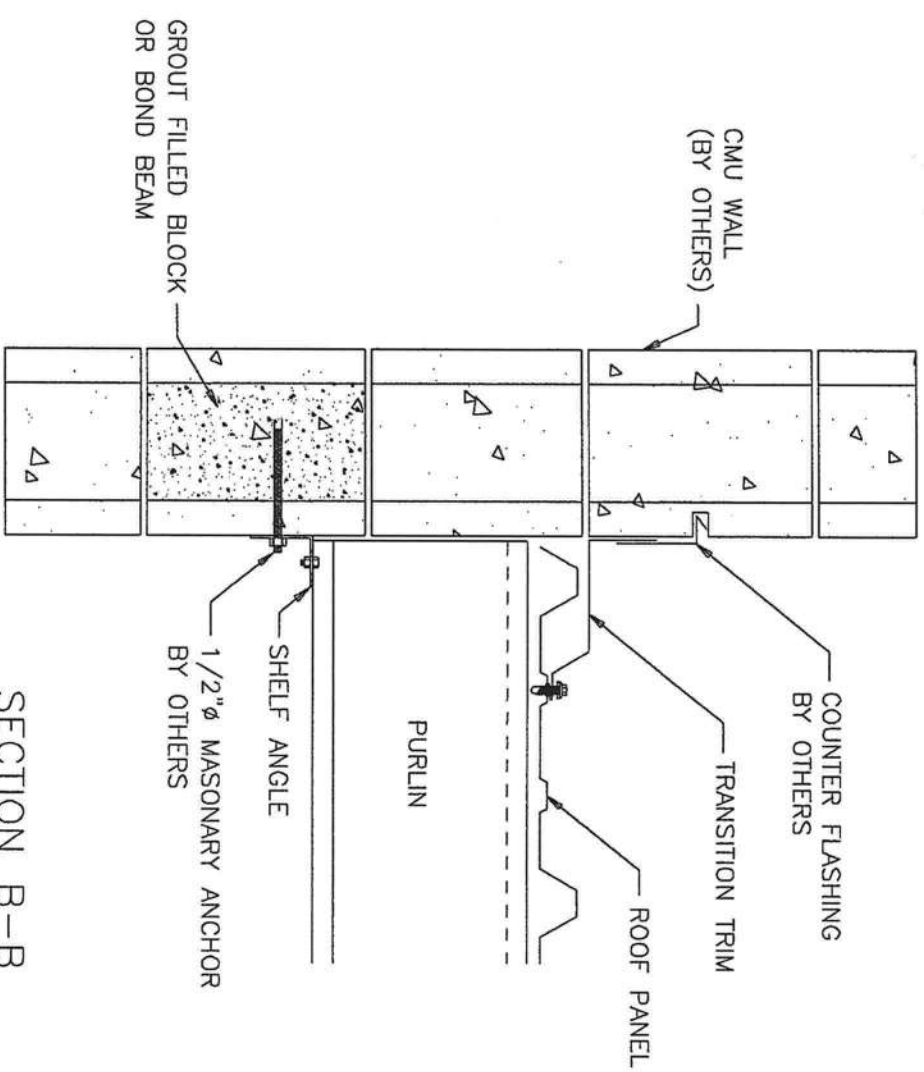
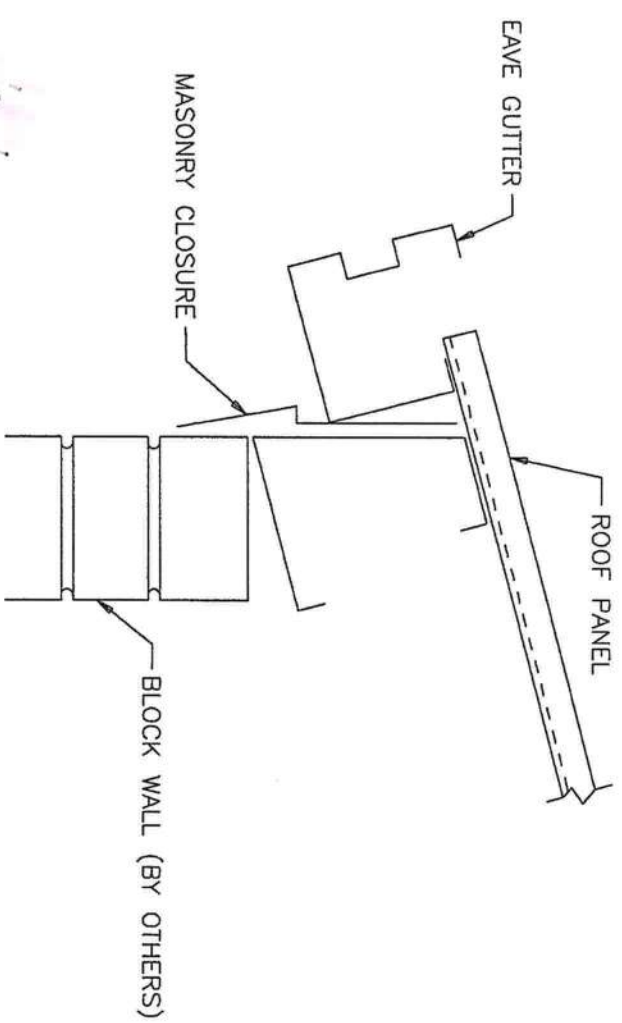
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REVISIONS						DRAWING STATUS		VULCAN STEEL STRUCT.	FLOOR-N-DECK
REV.	DESCRIPTION	DATE	DLR	DATE	CHKR	APPD			Detail Page
							☐ FOR CONSTRUCTION ☒ FOR PERMIT ONLY ☐ FOR APPROVAL ☐ OTHER, EXPLAIN _____	PROJECT 95'0" x 90'0" x 20.0' x 23.9'	DESIGN: [DRAFTSKP/AMND] CHECK:
								ID 18056-2	
								PROJECT LAKE CITY, FL	
								ADDRESS	DATE: 12/12/07 SHEET 14



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REVIEWS				DRAWING STATUS		VULCAN STEEL STRUCT.		FLOOR-N-DECOR	
REV.	DESCRIPTION	DATE	DTLR	DATE	CHKR	APPD	PROJECT	Detail Page	
							ID	18056-2	DRAFT(SKP/RMD) CHECK: 15
							PROJECT	LAKE CITY, FL	
							ADDRESS		