

RE: 6243113 2508-CR-2 Car MiTek, Inc. 16023 Swingley Ridge Rd. Chesterfield, MO 63017 314.434.1200

Site Information:S14.3Customer: Adams Homes-GainesvilleProject Name: 6243113Lot/Block: 096Model: 2508-CR-2 CarAddress: 715 SW Rosemary DrSubdivision: The Preserve at Laurel LakeCity: Lake CityState: fl

# General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2023/TPI2014 Wind Code: ASCE 7-22

Roof Load: 40.0 psf

Design Program: MiTek 20/20 8.7 Wind Speed: 130 mph Floor Load: N/A psf

This package includes 41 individual, dated Truss Design Drawings and 0 Additional Drawings. With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

| No. | Seal#     | Truss Name | Date      | No. | Seal#     | Truss Name | Date      |
|-----|-----------|------------|-----------|-----|-----------|------------|-----------|
| 1   | T34534201 | A01        | 7/25/2024 | 21  | T34534221 | B01        | 7/25/2024 |
| 2   | T34534202 | A02        | 7/25/2024 | 22  | T34534222 | B01X       | 7/25/2024 |
| 3   | T34534203 | A03        | 7/25/2024 | 23  | T34534223 | B02        | 7/25/2024 |
| 4   | T34534204 | A04        | 7/25/2024 | 24  | T34534224 | C1         | 7/25/2024 |
| 5   | T34534205 | A05        | 7/25/2024 | 25  | T34534225 | C3         | 7/25/2024 |
| 6   | T34534206 | A06        | 7/25/2024 | 26  | T34534226 | C5         | 7/25/2024 |
| 7   | T34534207 | A07        | 7/25/2024 | 27  | T34534227 | E01        | 7/25/2024 |
| 8   | T34534208 | A08        | 7/25/2024 | 28  | T34534228 | E01X       | 7/25/2024 |
| 9   | T34534209 | A09        | 7/25/2024 | 29  | T34534229 | E02        | 7/25/2024 |
| 10  | T34534210 | A10        | 7/25/2024 | 30  | T34534230 | E02X       | 7/25/2024 |
| 11  | T34534211 | A11        | 7/25/2024 | 31  | T34534231 | E7         | 7/25/2024 |
| 12  | T34534212 | A11A       | 7/25/2024 | 32  | T34534232 | E7B        | 7/25/2024 |
| 13  | T34534213 | A12        | 7/25/2024 | 33  | T34534233 | E7V        | 7/25/2024 |
| 14  | T34534214 | A13        | 7/25/2024 | 34  | T34534234 | E7VA       | 7/25/2024 |
| 15  | T34534215 | A14        | 7/25/2024 | 35  | T34534235 | E7VB       | 7/25/2024 |
| 16  | T34534216 | A15        | 7/25/2024 | 36  | T34534236 | E7VC       | 7/25/2024 |
| 17  | T34534217 | A16        | 7/25/2024 | 37  | T34534237 | H7         | 7/25/2024 |
| 18  | T34534218 | A17        | 7/25/2024 | 38  | T34534238 | PB1        | 7/25/2024 |
| 19  | T34534219 | A18        | 7/25/2024 | 39  | T34534239 | PB2        | 7/25/2024 |
| 20  | T34534220 | A19        | 7/25/2024 | 40  | T34534240 | PB3        | 7/25/2024 |
|     |           |            |           |     |           |            |           |

The truss drawing(s) referenced above have been prepared by

MiTek USA, Inc under my direct supervision

based on the parameters provided by Tibbetts Lumber Co., LLC.

Truss Design Engineer's Name: Velez, Joaquin

My license renewal date for the state of Florida is February 28, 2025. Florida COA: 6634

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek. Any project specific information included is for MiTek customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



July 25, 2024

Velez, Joaquin



RE: 6243113 - 2508-CR-2 Car

MiTek, Inc. 16023 Swingley Ridge Rd. Chesterfield, MO 63017 314.434.1200

## Site Information:

Project Customer: Adams Homes-GainesvilleProject Name: 6243113Lot/Block: 096Subdivision: The Preserve at Laurel LakeAddress: 715 SW Rosemary DrState: fl

 No.
 Seal#
 Truss Name
 Date

 41
 T34534241
 PB4
 7/25/2024



15

7x8 =

44

14

46

47

45

13

7x8 =

48

|   |  | 7-0-0  | 12-4-9   | 17-7-5                              |                      | 22-10-2              | 28                | -0-14       |                   | 33-3-11             |              | 38-6-7        |                   | 43-11-0               |        |
|---|--|--|--|-------------------------------------|----------------------|----------------------|-------------------|-------------|-------------------|---------------------|--------------|---------------|-------------------|-----------------------|--------|
|   | te (X V)                                     | [3·0-2-4 0-3-0]  | <u>5.0-4-0</u> <u>5.0-4-0</u> <u>15.0-4-0</u>                              | 0-4-81 [0:0-4                       | -0.0-4-8             | 1 [13.0-3-           | 12 0-4-121 [1     | 5.0-4-0     | 0-5-01            | J-2-13<br>[17:0_3_1 | 2 0-5-01     | 5-2-15        |                   | 5-4-5                 |        |
| T late Olise                                      | IS (X, T)                                    | [3.0-2-4,0-3-0]  | , [5.0-4-0,0-4-0], [7.0-4-0  | ,0-4-0], [3.0-4                     | -0,0-4-0             | ], [13.0-3-          | 12,0-4-12], [1    | 5.0-4-0     | ,0- <u>J</u> -0], | [17.0-5-1           | 2,0-5-0]     |               |                   |                       |        |
| LOADING<br>TCLL                                   | (psf)<br>20.0                                | SPACIN<br>Plate G  | <b>IG-</b> 2-0-0<br>rip DOL 1.25   | CSI.<br>TC                          | 0.77                 |                      | DEFL.<br>Vert(LL) | in<br>-0.40 | (loc)<br>15       | l/defl<br>>999      | L/d<br>360   | PL/<br>MT     | <b>ATES</b><br>20 | <b>GRIP</b><br>244/19 | 00     |
| TCDL  | 10.0   | Lumber   | DOL 1.25   | BC                                  | 0.98                 |                      | Vert(CT)          | -0.82       | 15                | >641                | 240          |               |                   |                       |        |
| BCLL  | 0.0 *  | Rep Str  | ess Incr NO  | WB                                  | 0.84                 |                      | Horz(CT)          | 0.17        | 11                | n/a                 | n/a          |               |                   |                       |        |
| BCDL  | 10.0   | Code F   | BC2023/TPI2014   | Matri                               | x-S                  |                      | Wind(LL)          | 0.27        | 15                | >999                | 240          | We            | ight: 597         | 7lb FT=               | 20%    |
|   |  |  |  |                                     |                      |                      |                   |             |                   |                     |              |               |                   |                       |        |
| TOP CHOR  | 2x6.SP                                       | No 2 *Excent*  | r  |                                     |                      |                      | TOP CHOR          | П           | Structu           | al wood s           | sheathing di | rectly applie | d or 3-1          | 0-14 oc purli         | ins    |
|   | 1-3: 2x                                      | 4 SP No.2  |  |                                     |                      |                      |                   | 0           | except            | and vertic          | als.         | roony appire  |                   | 0 1 1 00 pull         |        |
| BOT CHOR  | D 2x6 SP                                     | No.2 *Except*  |  |                                     |                      |                      | BOT CHOR          | D           | Rigid ce          | eiling dire         | ctly applied | or 10-0-0 od  | bracing           | <b>j</b> .            |        |
| WEBS  | 15-17:<br>2x4 SP                             | 2x6 SP DSS<br>9 No.2   |  |                                     |                      |                      |                   |             |                   |                     |              |               |                   |                       |        |
| REACTION  | I <b>S.</b> (size<br>Max H<br>Max U<br>Max G | e) 11=0-4-0,<br>lorz 2=119(LC<br>plift 11=-261(L<br>lrav 11=3631(L | 2=0-4-0<br>27)<br>C 8), 2=-229(LC 8)<br>.C 1), 2=3443(LC 1)                |                                     |                      |                      |                   |             |                   |                     |              |               |                   |                       |        |
| FORCES.<br>TOP CHOR                               | (lb) - Max.<br>D 2-3=-<br>7-8=-              | Comp./Max. Te<br>6883/334, 3-4=<br>8477/597, 8-9=                  | en All forces 250 (lb) c<br>9345/592, 4-5=-9343/5<br>8477/597, 10-11=-310/ | r less except<br>91, 5-6=-115<br>94 | when sh<br>50/791, 6 | own.<br>6-7=-1155    | 0/791,            |             |                   |                     |              |               |                   |                       |        |
| BOT CHOR  | D 2-18=                                      | =-324/6066, 17   | -18=-316/6086, 16-17=-   | 44/11142, 15                        | 5-16=-74             | 4/11142,             |                   |             |                   |                     |              |               |                   |                       |        |
| WEBS  | 14-15  | =-/4//10682,<br>-0/661 3_173                                       | 13-14=-747/10682, 12-1<br>326/3028 /-17715/21                              | 3=-356/4974,                        | 11-12=-<br>/182 5-4  | 356/4974<br>16-0/464 |                   |             |                   |                     |              |               |                   |                       |        |
| WEBS  | 5-15=  | 65/519 6-15=   | -612/200 7-15=-52/103  | , 3-17=-2103<br>3 7-14=0/45         | 7-13-                | .2628/178            |                   |             |                   |                     |              |               |                   |                       |        |
|   | 8-13=  | =-612/194. 9-13  | 3=-287/4173. 9-12=0/49   | . 9-11=-5865                        | /420                 | 2020/110             | ,                 |             |                   |                     |              |               | ull               | un.                   |        |
|   |  | ,  |  | ,                                   |                      |                      |                   |             |                   |                     |              |               | "OU               | IN VE                 | 11,    |
| NOTES-  |  |  |  |                                     |                      |                      |                   |             |                   |                     |              | 11.0          | AUC               |                       | A.L.   |
| 1) 2-ply true                                     | ss to be con                                 | nected togethe   | er with 10d (0.131"x3") n  | ails as follows                     | :                    |                      |                   |             |                   |                     |              | 1. 2          | · C               | ENS                   | 11     |
| Top chor  | ds connecte                                  | ed as follows: 2   | 2x4 - 1 row at 0-9-0 oc, 2   | x6 - 2 rows st                      | aggered              | l at 0-9-0 (         | DC.               |             |                   |                     |              | 3 1           | . 1.              | 5                     | N 20   |
| Bottom c  | hords conn                                   | ected as follow  | s: 2x6 - 2 rows staggere   | d at 0-9-0 oc.                      |                      |                      |                   |             |                   |                     |              | 2 3           | No                | 68182                 | 1 S S  |
| Webs co   | nnected as                                   | follows: 2x4 - 1   | 1 row at 0-9-0 oc.   |                                     |                      |                      |                   |             |                   |                     |              | 2 <u></u>     | 110               | 00102                 |        |
| 2) All loads                                      | are conside                                  | ered equally ap  | plied to all plies, except   | f noted as fro                      | nt (F) or            | back (B)             | face in the LC    |             | ASE(S) s          | section. P          | ly to        | = ^ :         |                   | -                     | 1 T E  |
| ply conn  | ections have                                 | e been provide   | d to distribute only loads   | noted as (F)                        | or (B), u            | nless othe           | erwise indicat    | ed.         |                   |                     |              |               | A                 |                       | 1      |
| 3) Unbalan  | ced roof live                                | e loads have be  | en considered for this d   | esign.                              |                      |                      |                   |             | ~                 |                     |              | = 0:          | - <del>\</del>    | AN I                  | : # =  |
| 4) Wind: AS                                       | SCE 7-22; V                                  | /ult=130mph (3   | -second gust) Vasd=101   | mph; TCDL=                          | 4.2psf; E            | SCDL=6.0             | psf; h=15ft; B    | =45ft; L    | _=24ft; e         | ave=5ft;            | Cat.         | = 1.          | STA               | TE OF                 | : 41 5 |
| II; Exp B   | ; Encl., GCp                                 | 0=0.18; MWFR   | (directional); cantileve   | r left and righ                     | t expose             | a ; Lumbe            | er DOL=1.60       | piate gr    | np DOL=           | =1.60               |              | =0.           | ~ /               | ~                     |        |
| 5) Building                                       | Designer / I                                 | -roject enginee  | er responsible for verifyin  | g applied roo                       | r live loa           | a shown a            | overs rain loa    | ading re    | equireme          | ents spec           | ITIC         | 10            |                   | DID                   | 17.    |
| to the us   | e of this true                               | ss component.  |  |                                     |                      |                      |                   |             |                   |                     |              | 1.0           | 10.00             |                       | 0,1    |
| 6) Provide a                                      | adequate dr                                  | ainage to preve  | ent water ponding.   |                                     |                      |                      |                   |             |                   |                     |              | 11,           | 010               | NALE                  | in     |
| <ol> <li>All plates</li> <li>This true</li> </ol> | s are 2x4 M                                  | 120 Unless oth   | erwise indicated.  |                                     |                      |                      | ath an live !     | مام         |                   |                     |              |               | 1111              | in the second         |        |
| <ol><li>This trus</li></ol>                       | s has been                                   | designed for a   | 10 0 pst bottom chord li   | e load nonce                        | ncurrent             | t with anv           | other live loa    | ds          |                   |                     |              |               |                   |                       |        |

8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

17<sub>39</sub>

7x8 =

40

38

16

41 42 43

18

37

- 9) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide
- will fit between the bottom chord and any other members.

10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 11=261, 2=229.

### Continued on page 2

4x5 =

👠 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. WARNING - verify design parameters and READ NOTES ON THIS AND INCLOSED INTERNETING TO THE REFERENCE FOR UNITY TO THE INSTANCE OF THE ADDRESS OF THE ADDRESS

### July 25,2024

Joaquin Velez PE No.68182

Date:

MiTek Inc. DBA MiTek USA FL Cert 6634

16023 Swingley Ridge Rd. Chesterfield, MO 63017

¥

5x5 =

52 <sup>11</sup>

12

50 51

49



| · · ·                       | -                          |                 | <b>A</b> | <b>D</b> 1  |   |        |
|-----------------------------|----------------------------|-----------------|----------|-------------|---|--------|
| Job                         | Truss                      | Iruss Type      | Qty      | Ріу         | 2508-CR-2 Car   |        |
|                             |                            |                 |          |             | T345  | 534201 |
| 6242112                     | A01                        | Half Hip Cirder | 1        | -           |   |        |
| 0243113                     | AUT                        |                 | 1        | 2           |   |        |
|                             |                            |                 |          | <b>_</b>    | Job Reference (optional)                                    |        |
| Tibbetts Lumber Co., LLC (C | Ocala, FL). Ocala, FL - 34 | 472.            |          | 3.730 s Jul | 11 2024 MiTek Industries, Inc. Wed Jul 24 11:26:43 2024 Pag | le 2   |

NOTES-

ID:Ts3RJ0261\_Xu2fYgSyBHAWzZSLZ-f2cEOvzVzUm0zLfa7K3HvNytQ2NHsjkV9c2TSYyusww

11) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 141 lb down and 86 lb up at 7-0-0, 122 lb down and 83 lb up at 9-0-12, 122 lb down and 83 lb up at 11-0-12, 122 lb down and 83 lb up at 13-0-12, 122 lb down and 83 lb up at 15-0-12, 122 lb down and 83 lb up at 19-0-12, 122 lb down and 83 lb up at 21-0-12, 122 lb down and 83 lb up at 23-0-12, 122 lb down and 83 lb up at 25-0-12, 122 lb down and 83 lb up at 27-0-12, 122 lb down and 83 lb up at 37-0-12, 122 lb down and 83 lb up at 37-0-12, 122 lb down and 83 lb up at 37-0-12, 122 lb down and 83 lb up at 37-0-12, 122 lb down and 83 lb up at 39-0-12, and 122 lb down and 83 lb up at 37-0-12, 95 lb down at 9-0-12, 95 lb down at 13-0-12, 95 lb down at 13-0-12, 95 lb down at 37-0-12, 95 lb down at 33-0-12, 95 lb down at 33-0-12,

### LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-60, 3-10=-60, 2-11=-20

Concentrated Loads (lb)

Vert: 3=122(F) 18=-262(F) 15=-48(F) 6=-122(F) 13=-48(F) 8=-122(F) 19=-122(F) 20=-122(F) 21=-122(F) 23=-122(F) 24=-122(F) 26=-122(F) 26=-122(F) 27=-122(F) 28=-122(F) 29=-122(F) 29=-122(F) 31=-122(F) 33=-122(F) 33=-122(F) 35=-122(F) 35=-122(F) 36=-131(F) 37=-48(F) 38=-48(F) 49=-48(F) 41=-48(F) 42=-48(F) 43=-48(F) 43=-48(F) 43=-48(F) 45=-48(F) 45=

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent outlapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality** Criteria and DBS-22 available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcscomponents.com)







| 6   | -4-12 9-0-0 16-0-14  | 22-11-15  |   | 29-11-1  | 36-10-2  | 43-11-0  |
|---|--|---|---|--|--|--|
| 6   | -4-12 2-7-3 7-0-14   | 6-11-2  |   | 6-11-2   | 6-11-2   | 7-0-14   |
| Plate Offsets (X,Y)   | [4:0-6-0,0-2-8], [6:0-4-0,0-3-0], [8:0-4-0,  | 0-3-0], [12:0-4-0,Edge], [14:0  | J-2-8,Edgej   |  |  |  |
| LOADING (psf)<br>TCLL 20.0<br>TCDL 10.0   | SPACING-2-0-0Plate Grip DOL1.25Lumber DOL1.25  | <b>CSI.</b><br>TC 0.74<br>BC 0.82   | <b>DEFL.</b><br>Vert(LL)<br>Vert(CT)  | in (loc)<br>-0.34 13<br>-0.69 13-14  | l/defl L/d<br>>999 360<br>>761 240   | PLATES         GRIP           MT20         244/190                         |
| BCLL 0.0 *<br>BCDL 10.0   | Rep Stress Incr YES<br>Code FBC2023/TPI2014  | WB 0.86<br>Matrix-S   | Horz(CT)<br>Wind(LL)  | 0.20 10<br>0.20 13   | n/a n/a<br>>999 240  | Weight: 243 lb FT = 20%  |
| LUMBER-           TOP CHORD         2x4 SF           4-6: 2)           BOT CHORD         2x4 SF           12-14:           WEBS         2x4 SF  | P No.2 *Except*<br>44 SP M 31 or 2x4 SP SS<br>P No.2 *Except*<br>2x4 SP M 31 or 2x4 SP SS<br>P No.2  |   | BRACING-<br>TOP CHORI<br>BOT CHORI<br>WEBS  | D Structura<br>except e<br>D Rigid cei<br>1 Row a  | al wood sheathing dire<br>and verticals.<br>Iling directly applied or<br>t midpt 6-7 | ectly applied or 2-2-0 oc purlins,<br>10-0-0 oc bracing.<br>14, 6-12, 8-10 |
| REACTIONS. (siz<br>Max H<br>Max U<br>Max C  | e) 10=0-4-0, 2=0-4-0<br>łorz 2=144(LC 12)<br>Jplift 10=-80(LC 12), 2=-133(LC 12)<br>Grav 10=1741(LC 1), 2=1877(LC 1)   |   |   |  |  |  |
| FORCES. (lb) - Max.<br>TOP CHORD 2-3=<br>7-8=   | Comp./Max. Ten All forces 250 (lb) or<br>-3340/213, 3-4=-3064/231, 4-5=-3923/27<br>-3670/232   | less except when shown.<br>4, 5-6=-3923/275, 6-7=-367   | 0/232,  |  |  |  |
| BOT CHORD 2-16<br>11-1  | =-261/2886, 15-16=-261/2886, 14-15=-2<br> 2=-145/2302, 10-11=-145/2302   | 16/2711, 13-14=-276/4212,   | 12-13=-276/421  | 2,   |  |  |
| WEBS 3-15<br>6-12   | =-292/69, 4-15=0/362, 4-14=-80/1441, 5<br>=-649/52, 7-12=-407/111, 8-12=-106/163   | 14=-447/132, 6-14=-346/16<br>7, 8-11=0/299, 8-10=-2721/   | , 6-13=0/275,<br>171  |  |  |  |
| NOTES-<br>1) Wind: ASCE 7-22; V<br>II; Exp B; Encl., GC<br>Zone1 13-2-15 to 4:<br>Lumber DOL=1.60<br>2) Building Designer /<br>to the use of this tru<br>3) Provide adequate d<br>4) This truss has bee<br>will fit between the b | /ult=130mph (3-second gust) Vasd=101r<br>pi=0.18; MWFRS (directional) and C-C Z<br>3-9-4 zone; cantilever left and right expos<br>plate grip DOL=1.60<br>Project engineer responsible for verifying<br>iss component.<br>rainage to prevent water ponding.<br>designed for a 10.0 psf bottom chord liv<br>en designed for a live load of 20.0psf on t<br>sottom chord and any other members. | nph; TCDL=4.2psf; BCDL=6<br>one3 -2-0-0 to 1-0-0, Zone1<br>sed ;C-C for members and fo<br>applied roof live load show<br>e load nonconcurrent with an<br>he bottom chord in all areas | .0psf; h=15ft; B:<br>1-0-0 to 9-0-0, 2<br>prces & MWFRS<br>n covers rain loa<br>ny other live load<br>where a rectand | =45ft; L=24ft; ea<br>Zone2 9-0-0 to '<br>for reactions sl<br>ading requireme<br>ds.<br>gle 3-6-0 tall by | ave=5ft; Cat.<br>13-2-15,<br>hown;<br>nts specific<br>2-0-0 wide                     | DD. STATE OF   |

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 10 except (jt=lb) 2=133.

# SSIONA -F Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

Date:

July 25,2024



႔ WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. WARNING - Verify design parameters and KEAU NOTES ON This AND INCLOSED MITCH REFERENCE FACE MILETARS IN TAL2625 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcscomponents.com)





|   | 6-   | -4-11   | 10-2-8  | 17-0   | 0-7  | 23  | -8-10  | 1   | 30-4-14  | 1   | 37-1-1  | 43-11-0  | )                                  |
|---|--|---|---|--|--|---|--|---|--|---|---|--|------------------------------------|
|   | 6-   | -4-11   | 3-9-13  | 6-9-   | -15  | 6   | -8-3   | 1   | 6-8-3  |   | 6-8-3   | 6-9-15   |                                    |
| Plate Offset                                    | s (X,Y)                                    | [4:0-6-0,0-2-8  | 8], [6:0-4-0,0-3  | 8-0], [8:0-4-0,0                                 | )-3-0], [12:0-4                                | 4-0,0-3-0], [                             | [14:0-4-0,0-3                                | 3-0]                                      |  |   |   |  |                                    |
| LOADING<br>TCLL 2<br>TCDL 4<br>BCLL<br>BCDL 4   | (psf)<br>20.0<br>10.0<br>0.0 *<br>10.0     | SPAC<br>Plate C<br>Lumbe<br>Rep S<br>Code                       | ING-<br>Grip DOL<br>er DOL<br>tress Incr<br>FBC2023/TPI | 2-0-0<br>1.25<br>1.25<br>YES<br>2014             | <b>CSI.</b><br>TC<br>BC<br>WB<br>Matrix        | 0.74<br>0.90<br>0.81<br><-S               | DE<br>Ver<br>Ver<br>Hol<br>Wir               | FL.<br>t(LL)<br>t(CT)<br>rz(CT)<br>nd(LL) | in (loc<br>-0.29 13-14<br>-0.61 13-14<br>0.20 11<br>0.18 13-14 | ) l/defl<br>4 >999<br>4 >860<br>0 n/a<br>4 >999           | L/d<br>360<br>240<br>n/a<br>240                 | PLATES<br>MT20<br>Weight: 251 lb                                     | <b>GRIP</b><br>244/190<br>FT = 20% |
| LUMBER-<br>TOP CHOR<br>BOT CHOR<br>WEBS         | D 2x4 SP<br>4-6: 2x<br>D 2x4 SP<br>2x4 SP  | 9 No.2 *Excep<br>4 SP M 31 or<br>9 No.2<br>9 No.2               | t*<br>2x4 SP SS   |  |  |   | BRA<br>TOF<br>BOT<br>WEE                     | CHORI<br>CHORI<br>CHORI                   | D Struc<br>excep<br>D Rigid<br>1 Rov                           | tural wood<br>ot end vertio<br>ceiling dire<br>v at midpt | sheathing dire<br>cals.<br>ctly applied o<br>6· | ectly applied or 2-2-3 or<br>r 10-0-0 oc bracing.<br>·14, 6-12, 8-10 | c purlins,                         |
| REACTION  | <b>S.</b> (size<br>Max H<br>Max U<br>Max G | e) 10=0-4-0<br>lorz 2=158(L0<br>lplift 10=-82(L<br>brav 10=1741 | 9, 2=0-4-0<br>C 12)<br>.C 12), 2=-132<br>(LC 1), 2=187  | 2(LC 12)<br>7(LC 1)                              |  |   |  |   |  |   |   |  |                                    |
| FORCES.<br>TOP CHOR                             | (lb) - Max.<br>D 2-3=-<br>7-8=-            | Comp./Max<br>3343/209, 3-4                                      | Ten All force<br>4=-2982/223, 4                         | es 250 (lb) or  <br>4-5=-3543/25                 | less except v<br>7, 5-6=-3542                  | when shown<br>2/257, 6-7=-3               | n.<br>3177/207,                              |   |  |   |   |  |                                    |
| BOT CHOR  | D 2-16=<br>11-1                            | =-273/2889, 1<br>2=-128/1973,                                   | 5-16=-273/288<br>10-11=-128/1                           | 89, 14-15=-21<br>973                             | 8/2624, 13-1                                   | 14=-251/370                               | 00, 12-13=-                                  | 251/370                                   | Ο,   |   |   |  |                                    |
| WEBS  | 3-15=<br>7-12=                             | =-356/71, 4-15<br>=-393/107, 8-1                                | 5=0/381, 4-14<br>12=-102/1521,                          | =-61/1151, 5-<br>, 8-11=0/289,                   | 14=-433/128<br>8-10=-2464/                     | , 6-13=0/26<br>159                        | 85, 6-12=-66                                 | 1/55,                                     |  |   |   |  |                                    |
| NOTES-<br>1) Wind: AS<br>II; Exp B;<br>Zone1 14 | CE 7-22; V<br>Encl., GCp<br>I-5-7 to 43-   | /ult=130mph (<br>bi=0.18; MWF<br>9-4 zone: can                  | 3-second gus<br>RS (directiona<br>tilever left and      | t) Vasd=101m<br>al) and C-C Zo<br>I right expose | nph; TCDL=4<br>one3 -2-0-0 to<br>d :C-C for me | I.2psf; BCD<br>o 1-0-0, Zor<br>embers and | 0L=6.0psf; h<br>ne1 1-0-0 tc<br>1 forces & N | =15ft; B=<br>10-2-8,<br>WFRS f            | =45ft; L=24ft<br>Zone2 10-2<br>or reactions                    | ; eave=5ft;<br>-8 to 14-5-7<br>shown: Lur                 | Cat.<br>7,<br>mber                              | MAQUIN   | VELEN                              |

DOL=1.60 plate grip DOL=1.60 2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 10 except (jt=lb) 2=132.



Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

July 25,2024



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| LOADING (ps<br>TCLL 20<br>TCDL 10<br>BCLL 0<br>BCDL 10             | osf)<br>0.0<br>0.0<br>0.0 *<br>0.0 | SPACING-<br>Plate Grip DOL<br>Lumber DOL<br>Rep Stress Incr<br>Code FBC2023/TP | 2-0-0<br>1.25<br>1.25<br>YES<br>I2014 | <b>CSI.</b><br>TC<br>BC<br>WB<br>Matrix | 0.81<br>0.80<br>0.73<br>-S | DEFL.<br>Vert(LL)<br>Vert(CT)<br>Horz(CT)<br>Wind(LL) | in (loc)<br>-0.43 18-19<br>-0.87 18-19<br>0.40 12<br>0.26 18-19 | l/defl<br>>999<br>>599<br>n/a<br>>999 | L/d<br>360<br>240<br>n/a<br>240 | PLATES<br>MT20<br>MT20HS<br>Weight: 284 lb | <b>GRIP</b><br>244/190<br>187/143<br>FT = 20% |
|--|------------------------------------|--|---------------------------------------|---|----------------------------|---|---|---------------------------------------|---------------------------------|--|---|
| LUMBER-<br>TOP CHORD 2x4 SP No.2<br>BOT CHORD 2x4 SP No.2 *Except* |                                    |  |                                       |   | BRACING-<br>TOP CHOR       | D Structu<br>except                                   | ral wood :<br>end vertic  | sheathing dir<br>cals.                | rectly applied or 2-2-0 or      | c purlins,                                 |   |

| TOP CHORD | 2x4 SP No.2                           | TOP CHORD | Structural wood sheathing directly applied or 2-2-0 oc purlins, |
|-----------|---------------------------------------|-----------|---|
| BOT CHORD | 2x4 SP No.2 *Except*                  |           | except end verticals.   |
|           | 20-25,17-20: 2x4 SP M 31 or 2x4 SP SS | BOT CHORD | Rigid ceiling directly applied or 10-0-0 oc bracing. Except:    |
| WEBS      | 2x4 SP No.2                           |           | 10-0-0 oc bracing: 21-22, 15-17                                 |
|           |                                       | WEBS      | 1 Row at midpt 9-14   |
|           |                                       | JOINTS    | 1 Brace at Jt(s): 25  |
| DEACTIONS | (cizo) 2-0.4.0.12-0.3.8               |           |   |

ACTIONS. (size) 2=0-4-0, 12=0-3-8 Max Horz 2=108(LC 12) Max Uplift 2=-128(LC 12), 12=-70(LC 12) Max Grav 2=1891(LC 1), 12=1753(LC 1)

 
 FORCES.
 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 2-3=-3382/208, 3-4=-4945/360, 4-5=-4112/312, 5-6=-4470/336, 6-7=-4469/336, 7-8=-4124/310, 8-9=-4093/309, 9-10=-2271/211, 10-11=-1814/153, 11-12=-1692/161

 BOT CHORD
 2-27=-212/2924, 4-25=-80/1222, 22-25=-285/4370, 21-22=-285/4369, 19-21=-211/3708, 18-19=-254/4624, 17-18=-254/4624, 8-17=-297/85, 13-14=-80/1534

 WEBS
 3-27=-1464/186, 25-27=-237/3224, 3-25=-71/1470, 4-21=-1185/127, 5-21=-39/1008,

5-19=-63/1058, 6-19=-347/107, 7-19=-300/4, 7-17=-646/36, 14-17=-135/2444, 9-17=-139/2584, 9-14=-2067/170, 10-14=-82/1336, 10-13=-615/109, 11-13=-82/1671

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=5ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 -2-0-0 to 1-0-0, Zone1 1-0-0 to 12-2-8, Zone2 12-2-8 to 16-5-7, Zone1 16-5-7 to 37-7-8, Zone2 37-7-8 to 41-10-7, Zone1 41-10-7 to 43-9-4 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

Provide adequate drainage to prevent water ponding.

5) All plates are MT20 plates unless otherwise indicated.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 12 except (jt=lb) 2=128.



Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

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a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall
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fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI Uaulity Criteria and DSE-22 available from Truss Plate Institute (www.tpinst.org)
and BCSI Building Component Safety Information
available from the Structural Building Component Association (www.sbcscomponents.com)



| Construction         6-4-13         3-10-4         1-6-0         2-5-8         7-5-4         7-5-4         6-6-8         3-9-12           Plate Offsets (X,Y)         [3:0-2-8,0-3-0], [5:0-6-0,0-2-8], [6:0-2-8,Edge], [8:0-6-0,0-2-8], [16:0-4-12,0-2-4], [23:0-5-8,0-3-0], [25:0-3-8,0-2-0]         Image: Construction of the second s | 4-5-8 0-0-4<br>GRIP<br>244/190 |
|---|--------------------------------|
| Plate Offsets (X,Y)         [3:0-2-8,0-3-0], [5:0-6-0,0-2-8], [6:0-2-8,Edge], [8:0-6-0,0-2-8], [16:0-4-12,0-2-4], [23:0-5-8,0-3-0], [25:0-3-8,0-2-0]           LOADING (psf)         SPACING-         2-0-0         CSI.         DEFL.         in (loc)         I/defl         L/d         PLATES           TCLL         20.0         Plate Grip DOL         1.25         TC         0.76         Vert(LL)         -0.32         17-19         >999         360         MT20           TCDL         10.0         Lumber DOL         1.25         BC         0.80         Vert(CT)         -0.68         17-19         >767         240           BCLL         0.0 *         Rep Stress Incr         YES         WB         0.74         Horz(CT)         0.33         11         n/a         n/a  | <b>GRIP</b><br>244/190         |
| LOADING (psf)         SPACING-         2-0-0         CSI.         DEFL.         in         (loc)         l/defl         L/d         PLATES           TCLL         20.0         Plate Grip DOL         1.25         TC         0.76         Vert(LL)         -0.32         17-19         >999         360         MT20           TCDL         10.0         Lumber DOL         1.25         BC         0.80         Vert(CT)         -0.68         17-19         >767         240           BCLL         0.0 *         Rep Stress Incr         YES         WB         0.74         Horz(CT)         0.33         11         n/a         n/a   | <b>GRIP</b><br>244/190         |
| BCDL 10.0   Code FBC2023/1P12014   Matrix-S   Wind(LL) 0.20 17-19 >999 240   Weight: 28-  | 34 lb FT = 20%                 |
| LUMBER-<br>TOP CHORD 2x4 SP No.2 *Except* Description directly applied or 2-2   | 2-0 oc purlins.                |
| 5-6: 2x4 SP M 31 or 2x4 SP SS<br>BOT CHORD 2x4 SP No.2 *Except*<br>40 02 45 49 0rd SP No.2 *Except*<br>BOT CHORD 2x4 SP No.2 *Except*<br>40 02 45 49 0rd SP No.2 *Except*<br>40 02 45 49 0rd SP SS  | ig. Except:                    |
| WEBS         2x4 SP No.2         WEBS         1 Row at midpt         6-16           JOINTS         1 Brace at Jt(s): 23         2   |                                |
| REACTIONS.         (size)         2=0-4-0, 11=0-3-8           Max Horz         2=108(LC 12)           Max Uplift         2=-128(LC 12), 11=-70(LC 12)           Max Grav         2=1891(LC 1), 11=1753(LC 1)  |                                |

 
 FORCES.
 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 2-3=-3381/206, 3-4=-4941/357, 4-5=-3663/288, 5-6=-3784/308, 6-7=-3356/287, 7-8=-3340/290, 8-9=-1901/191, 9-10=-1538/132, 10-11=-1708/151

 BOT CHORD
 2-25=-209/2923, 4-23=-52/1210, 20-23=-288/4383, 19-20=-289/4381, 17-19=-173/3270, 16-17=-199/3784, 7-16=-437/129, 12-13=-81/1328

 WEBS
 3-25=-1454/179, 23-25=-227/3209, 3-23=-74/1473, 4-19=-1407/144, 5-19=-8/925, 5-17=-42/785, 6-17=-280/122, 6-16=-570/32, 13-16=-82/1659, 8-16=-134/2189, 8-13=-825/88, 9-13=0/595, 9-12=-853/104, 10-12=-96/1600

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=5ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 -2-0-0 to 1-0-0, Zone1 1-0-0 to 14-2-8, Zone2 14-2-8 to 18-5-7, Zone1 18-5-7 to 35-7-8, Zone2 35-7-8 to 39-10-7, Zone1 39-10-7 to 43-9-4 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) Provide adequate drainage to prevent water ponding.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 11 except (jt=lb) 2=128.



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| L   | 6-4-13   | 10-3-0  | 11-9-0   | 16-2-8   | 22-7-12  |  | 29  | 9-1-0                                |  | 33-7-8  | 1  | 39-5-3                                     | 43-                                   | 11-0                               |  |
|---|--|---|--|--|--|--|---|--------------------------------------|--|---|--|--|---------------------------------------|------------------------------------|--|
|   | 6-4-13   | 3-10-4  | 1-6-0  | 4-5-8  | 6-5-4  |  | 6   | -5-4                                 | 1  | 4-6-8   | 1  | 5-9-11                                     | 4-5                                   | 5-13                               |  |
| Plate Offsets (X,Y)   | [3:0-2-8,0-3-0],   | [5:0-6-0,0-2  | 2-8], [6:0-2-8   | 8,0-3-4], [8:0-  | 6-0,0-2-8], [23:0  | 0-3-8,0                                | )-4-0], [25:0   | )-3-8,0                              | -2-8]  |   |  |  |                                       |                                    |  |
| LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0                      | SPACIN<br>Plate Gr<br>Lumber<br>Rep Stre<br>Code Fl  | IG-<br>ip DOL<br>DOL<br>ess Incr<br>BC2023/TP   | 2-0-0<br>1.25<br>1.25<br>YES<br>I2014  | CSI.<br>TC<br>BC<br>WB<br>Mati   | 1.00<br>0.97<br>0.80<br>ix-S   | D<br>V<br>V<br>F<br>V                  | DEFL.<br>Vert(LL)<br>Vert(CT)<br>Horz(CT)<br>Wind(LL) | in<br>-0.37<br>-0.67<br>0.35<br>0.18 | (loc)<br>17-19<br>17-19<br>11<br>19                              | l/defl<br>>999<br>>778<br>n/a<br>>999   | L/d<br>360<br>240<br>n/a<br>240                      | PL<br>MT<br>We                             | . <b>ATES</b><br>T20<br>eight: 291 lb | <b>GRIP</b><br>244/190<br>FT = 20% |  |
| LUMBER-<br>TOP CHORD 2x4<br>BOT CHORD 2x4<br>18-2<br>WEBS 2x4<br>REACTIONS. (<br>Ma<br>Ma<br>Ma   | SP No.2<br>SP No.2 *Except*<br>3,16-18: 2x4 SP N<br>SP No.2<br>size) 2=0-4-0, 1<br>(Horz 2=122(LC<br>(Uplift 2=-128(LC<br>(Grav 2=2117(LC                            | √ 31 or 2x4<br>1=0-3-13<br>11)<br>12), 11=-7(<br>≿ 17), 11=19   | SP SS<br>0(LC 12)<br>044(LC 18)  |  |  | BI<br>TC<br>BC<br>W<br>JC              | RACING-<br>OP CHORE<br>OT CHORE<br>/EBS<br>OINTS      | 0                                    | Structur<br>Rigid ce<br>2-2-0 or<br>10-0-0<br>1 Row a<br>1 Brace | ral wood s<br>eiling direc<br>c bracing:<br>oc bracing<br>at midpt<br>e at Jt(s): 2 | heathing di<br>tly applied (<br>2-25.<br>: 14-16<br> | rectly appli<br>or 10-0-0 c<br>I-19, 6-16, | ed, except of bracing,<br>8-13        | end verticals.<br>Except:          |  |
| FORCES.         (lb)         -M.           TOP CHORD         2-         7-           BOT CHORD         2-         16           WEBS         3-         9- | ax. Comp./Max. Te<br>3=-3832/206, 3-4=<br>8=-3088/268, 8-9=<br>25=-210/3408, 4-2<br>-17=-155/3591, 7-<br>25=-1621/167, 23-<br>17=-27/505, 6-16=<br>13=0/574, 9-12=-8 | en All forc<br>-5700/351,<br>-2177/205,<br>:3=-19/1507<br>:16=-338/10<br>:25=-208/37<br>-624/33, 13<br>:51/128, 10- | es 250 (lb) o<br>4-5=-3769/2<br>9-10=-1709/<br>7, 20-23=-29<br>13, 12-13=-84<br>31, 3-23=-7<br>16=-69/203<br>12=-101/180 | r less except<br>275, 5-6=-356<br>2132, 10-11=-<br>5/5168, 19-20<br>4/1491<br>5/1765, 4-19<br>2, 8-16=-109<br>05 | when shown.<br>i0/289, 6-7=-310<br>1881/147<br>)=-296/5162, 17<br>=-2024/169, 5-1<br>/2156, 8-13=-10 | 04/269<br>7-19=-1<br>19=0/11<br>044/87 | 9,<br>144/3374,<br>130,<br>7,                         |                                      |  |   |  |  | AQUIN                                 | VE                                 |  |

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=5ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 -2-0-0 to 1-0-0, Zone1 1-0-0 to 16-2-8, Zone2 16-2-8 to 20-5-7, Zone1 20-5-7 to 33-7-8, Zone2 33-7-8 to 37-10-7, Zone1 37-10-7 to 43-9-4 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) Provide adequate drainage to prevent water ponding.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

\* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 6) will fit between the bottom chord and any other members, with BCDL = 10.0psf.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 11 except (jt=lb) 2=128

Velez PE No.68182 . DBA MiTek US<sup>3</sup> gley Ridge R<sup>3</sup> 68182

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July 25,2024



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and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcscomponents.com)

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July 25,2024



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July 25,2024



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July 25,2024



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July 25,2024



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5) All plates are 3x6 MT20 unless otherwise indicated.

- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 20, 11.

6 Joaquin Velez PE No.68182

MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

July 25,2024



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| 0-4-5 4-   | -9-12 8-11-0 9-1-0 14-5-11  | 19-0-0 2  | 24-11-0  | 30-10-0   | 35-5-4 3  | 39-5-4 45-2-0  |                     |
|--|---|---|--|---|---|--|---------------------|
| U-4-5 4  | 1-5-7 4-1-4 0-2-0 5-4-10<br>[2:0-0-12 Edge] [2:0-0-4 Edge] [3:0-2-0   | 4-6-5   | 5-11-0<br>3:0-6-0 0-2-81 [10:0                                   | 5-11-0<br>0-2-8 0-3-01                                | 4-7-4   | 4-0-1 5-8-12   | ·                   |
|  | [2.0-0-12,Luge], [2.0-0-4,Luge], [3.0-2-0   | ,0-3-4], [0.0-0-0,0-2-0], [0  | J.0-0-0,0-2-0j, [10.0  | 5-2-0,0-5-0]  |   |  |                     |
| LOADING (psf)  | SPACING- 2-0-0  | CSI.  | DEFL.  | in (loc)  | l/defl L/d  | PLATES   | GRIP                |
| TCLL 20.0  | Plate Grip DOL 1.25   | TC 0.56   | Vert(LL)   | -0.14 15-17   | >999 360  | MT20   | 244/190             |
| TCDL 10.0  | Lumber DOL 1.25   | BC 0.66   | Vert(CT)   | -0.26 15-17   | >999 240  |  |                     |
| BCLL 0.0 *   | Rep Stress Incr YES   | WB 0.47   | Horz(CT)   | 0.08 12   | n/a n/a   |  |                     |
| BCDL 10.0  | Code FBC2023/TPI2014  | Matrix-S  | Wind(LL)   | 0.06 15   | >999 240  | Weight: 283 lb   | FT = 20%            |
| LUMBER-<br>TOP CHORD 2x4 Sf<br>BOT CHORD 2x4 Sf<br>WEBS 2x4 Sf<br>WEDGE<br>Left: 2x4 SP No.2<br>SLIDER Right 2 | P No.2<br>P No.2<br>P No.2<br>P No.2<br>2x4 SP No.2 3-1-12  |   | BRACING-<br>TOP CHOR<br>BOT CHOR<br>WEBS                         | D Structur<br>D Rigid ce<br>1 Row a                   | al wood sheathing di<br>iling directly applied d<br>t midpt 8 | rectly applied or 2-10-1<br>or 3-10-12 oc bracing.<br>3-17 | 5 oc purlins.       |
| REACTIONS. (siz<br>Max H<br>Max L<br>Max C   | te) 2=0-3-8, 22=0-4-0, 12=0-4-0<br>Horz 2=153(LC 11)<br>Jplift 2=-144(LC 12), 22=-164(LC 12), 12:<br>Grav 2=336(LC 23), 22=2242(LC 17), 12=                             | =-61(LC 12)<br>=1585(LC 18)   |  |   |   |  |                     |
| FORCES. (lb) - Max.<br>TOP CHORD 2-3=  | . Comp./Max. Ten All forces 250 (lb) or<br>-27/313, 3-4=-54/850, 4-5=-1369/130, 5-6   | less except when shown.<br>6=-1628/190, 6-7=-1827/2                                     | 219,   |   |   |  |                     |
|  | -1027/219, 0-9=-2007/213, 9-10=-2432/2  | 12, 10-12=-2709/190   | 7 10-0/1450  |   |   |  |                     |
| 15-1   | 7=-19/1775 14-15=-77/2132 13-14=-11(  | )/2336 12-13=-109/2340  | 7 13-0/1400,   |   |   |  |                     |
| WEBS 3-23  | =-32/336, 3-21=-654/173, 4-20=-112/203  | 7. 5-20=-660/111. 5-19=0  | 0/420.   |   |   |  |                     |
| 6-17   | =-52/709, 7-17=-405/117, 8-15=-0/638, 9   | -15=-557/87, 9-14=0/304   | , 10-14=-265/53  |   |   |  | IIIII.              |
| NOTES-<br>1) Unbalanced roof liv<br>2) Wind: ASCE 7-22; V<br>II; Exp B; Encl., GC<br>Zone1 23-2-15 to 30       | e loads have been considered for this de:<br>Vult=130mph (3-second gust) Vasd=101n<br>pi=0.18; MWFRS (directional) and C-C Z<br>0-10-0, Zone2 30-10-0 to 35-0-15, Zone1 | sign.<br>nph; TCDL=4.2psf; BCDL<br>one3 -2-0-0 to 1-0-0, Zon<br>35-0-15 to 45-2-0 zone; | _=6.0psf; h=15ft; B<br>e1 1-0-0 to 19-0-0<br>cantilever left and | =45ft; L=24ft; e<br>, Zone2 19-0-0<br>right exposed ; | ave=6ft; Cat.<br>to 23-2-15,<br>porch left                    | No 6   | VELE<br>NSE<br>8182 |
| exposed;C-C for me   | embers and forces & MWFRS for reaction  | ns shown; Lumber DOL=1  | 1.60 plate grip DOL  | _=1.60  |   |  | · · · · · ·         |

3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) Provide adequate drainage to prevent water ponding.

5) All plates are 3x6 MT20 unless otherwise indicated.

- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide
- will fit between the bottom chord and any other members, with BCDL = 10.0psf. 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 12 except (jt=lb) 2=144, 22=164.



Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

July 25,2024



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| 0- <u>4-5</u> 4-  | <u>9-12</u> <u>8-11-0</u> <u>9-1-0</u> <u>17-0-0</u><br>-5-7 <u>4-1-4</u> <u>0-2-0</u> <u>7-11-0</u>  | 24-11   | -0  | <u>32-10-0</u><br>7-11-0  | 38-10-4  | 45-2-0                              |                                    |
|---|---|---|---|---|--|-------------------------------------|------------------------------------|
| Plate Offsets (X,Y)   | [2:0-0-4,Edge], [2:0-0-12,Edge], [3:0-2-8   | ,0-3-0], [5:0-6-0,0-2-8], [7  | :0-6-0,0-2-8], [18:E  | dge,0-1-8]  |  | 0012                                |                                    |
| LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0  | SPACING-2-0-0Plate Grip DOL1.25Lumber DOL1.25Rep Stress IncrYESCode FBC2023/TPI2014   | CSI.<br>TC 0.89<br>BC 0.85<br>WB 0.46<br>Matrix-S   | DEFL.<br>Vert(LL)<br>Vert(CT)<br>Horz(CT)<br>Wind(LL)   | in (loc) l/d<br>-0.20 12-14 >99<br>-0.38 12-14 >99<br>0.08 10 r<br>0.07 14 >99  | efl L/d<br>99 360<br>99 240<br>1/a n/a<br>99 240   | PLATES<br>MT20<br>Weight: 252 lb    | <b>GRIP</b><br>244/190<br>FT = 20% |
| LUMBER-<br>TOP CHORD 2x4 SF<br>BOT CHORD 2x4 SF<br>WEBS 2x4 SF<br>WEDGE<br>Left: 2x4 SP No.2<br>SLIDER Right 2  | P No.2<br>P No.2<br>P No.2<br>P No.2<br>2x4 SP No.2 3-1-12  |   | BRACING-<br>TOP CHORD<br>BOT CHORD  | Structural wo<br>Rigid ceiling  | ood sheathing direc<br>directly applied or         | ctly applied.<br>3-11-0 oc bracing. |                                    |
| REACTIONS. (siz<br>Max H<br>Max U<br>Max G  | e) 10=0-4-0, 2=0-3-8, 18=0-4-0<br>łorz 2=137(LC 11)<br>Jplift 10=-61(LC 12), 2=-70(LC 12), 18=-8<br>Grav 10=1597(LC 18), 2=360(LC 23), 18=  | 6(LC 12)<br>2258(LC 17)   |   |   |  |                                     |                                    |
| FORCES.         (lb) - Max.           TOP CHORD         3-4=-           8-10:         8-10:           BOT CHORD         17-13:           11-1:         WEBS         3-19:           6-14:         6-14:   | Comp./Max. Ten All forces 250 (lb) or<br>-31/675, 4-5=-1705/157, 5-6=-2163/225,<br>=-2830/196<br>8=-2266/145, 4-17=-1906/197, 16-17=-49<br>2=-123/2404, 10-11=-123/2404<br>=0/281, 3-17=-536/51, 4-16=-48/1968, 5-<br>=-538/148, 7-14=-20/357, 7-12=0/515, 8-   | less except when shown.<br>6-7=-2163/225, 7-8=-231<br>13/46, 14-16=-1/1473, 12-<br>16=-337/116, 5-14=-71/96<br>12=-477/92   | 1/202,<br>14=-39/1985,<br>69,   |   |  |                                     | line.                              |
| NOTES-<br>1) Unbalanced roof live<br>2) Wind: ASCE 7-22; \<br>II; Exp B; Encl., GC[<br>Zone1 21-2-15 to 32<br>members and force:<br>3) Building Designer /<br>to the use of this tru<br>4) Provide adequate d<br>5) This truss has been<br>6) * This truss has been | e loads have been considered for this des<br>/ult=130mph (3-second gust) Vasd=101n<br>pi=0.18; MWFRS (directional) and C-C Zt<br>2-10-0, Zone2 32-10-0 to 37-0-15, Zone1<br>s & MWFRS for reactions shown; Lumbee<br>Project engineer responsible for verifying<br>iss component.<br>rainage to prevent water ponding.<br>designed for a 10.0 psf bottom chord live<br>in designed for a live load of 20.0psf on th | sign.<br>hph; TCDL=4.2psf; BCDL<br>hr=3-2-0-0 to 1-0-0, Zone<br>37-0-15 to 45-2-0 zone; c<br>DCL=1.60 plate grip DO<br>applied roof live load sho<br>load nonconcurrent with<br>he bottom chord in all area | =6.0psf; h=15ft; B=<br>e1 1-0-0 to 17-0-0, z<br>cantilever left and ric<br>L=1.60<br>wm covers rain load<br>any other live loads<br>as where a rectangl | 45ft; L=24ft; eave=<br>Zone2 17-0-0 to 2'<br>ght exposed ;C-C f<br>ding requirements :<br>s.<br>le 3-6-0 tall by 2-0- | -6ft; Cat.<br>I-2-15,<br>for<br>specific<br>0 wide | PROSTATION                          | VELEX<br>182<br>OF                 |

will fit between the bottom chord and any other members, with BCDL = 10.0psf.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 10, 2, 18.



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July 25,2024



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| 0-4-5 4  | -9-12 8-11-0 9-1-0 15-0-0  | 21-7-15  | 28-2-1   |   | 4-10-0  | 39-10-4                                   | 45-2-0                                       |                                    |  |  |
|--|--|--|--|---|---|---|--|------------------------------------|--|--|
| Plate Offsets (X,Y)  | [2:0-0-4,Edge], [2:0-0-12,Edge], [3:0-2-8  | ,0-3-0], [5:0-6-0,0-2-8], [7   | :0-2-4,0-3-4], [8:0-3  | 2-8,0-2-4], [11:  | 0-0-0,0-2-4], [16   | :0-4-0,0-3-0                              | )]   |                                    |  |  |
| LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0         *           BCDL         10.0   | SPACING- 2-0-0<br>Plate Grip DOL 1.25<br>Lumber DOL 1.25<br>Rep Stress Incr YES<br>Code FBC2023/TPI2014  | CSI.<br>TC 0.59<br>BC 0.62<br>WB 0.45<br>Matrix-S  | DEFL.<br>Vert(LL)<br>Vert(CT)<br>Horz(CT)<br>Wind(LL)  | in (loc)<br>-0.14 15<br>-0.30 15-16<br>0.08 11<br>0.08 15               | l/defl L/d<br>>999 360<br>>999 240<br>n/a n/a<br>>999 240 |   | PLATES<br>MT20<br>Weight: 256 lb             | <b>GRIP</b><br>244/190<br>FT = 20% |  |  |
| LUMBER-<br>TOP CHORD 2x4 SI<br>BOT CHORD 2x4 SI<br>WEBS 2x4 SI<br>WEDGE<br>Left: 2x4 SP No.2<br>SLIDER Right :   | P No.2<br>P No.2<br>P No.2<br>2x4 SP No.2 2-11-0   |  | BRACING-<br>TOP CHORI<br>BOT CHORI<br>WEBS   | ) Structur<br>) Rigid ce<br>1 Row a                                     | ral wood sheathi<br>eiling directly app<br>at midpt       | ng directly a<br>blied or 4-0-<br>7-16, 7 | applied or 3-1-8 or<br>10 oc bracing.<br>-13 | c purlins.                         |  |  |
| REACTIONS. (size) 11=0-4-0, 2=0-3-8, 19=0-4-0<br>Max Horz 2=120(LC 11)<br>Max Uplift 11=-61(LC 12), 2=-70(LC 12), 19=-87(LC 12)<br>Max Grav 11=1390(LC 1), 2=272(LC 23), 19=2090(LC 1)   |  |  |  |   |   |   |  |                                    |  |  |
| FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown.         TOP CHORD       2-3=-46/364, 3-4=-42/958, 4-5=-1256/137, 5-6=-1954/212, 6-7=-1954/212, 7-8=-1861/205, 8-9=-2110/204, 9-11=-2463/193         BOT CHORD       2-20=-295/33, 18-19=-2097/148, 4-18=-1923/176, 17-18=-788/74, 16-17=0/1029, 15-16=-83/2224, 13-15=-83/2224, 12-13=-120/2075         WEBS       3-20=0/360, 18-20=-362/55, 3-18=-681/69, 4-17=-79/1976, 5-17=-600/110, 5-16=-86/1237, 6-16=-419/120, 7-16=-375/31, 7-15=0/265, 7-13=-584/35, 8-13=0/571, 9-13=-276/73 |  |  |  |   |   |   |  |                                    |  |  |
| NOTES-<br>1) Unbalanced roof liv<br>2) Wind: ASCE 7-22, '<br>II; Exp B; Encl., GC<br>Zone1 19-2-15 to 3<br>members and force<br>3) Building Designer /<br>to the use of this tru<br>4) Provide adequate of   | re loads have been considered for this des<br>Vult=130mph (3-second gust) Vasd=101n<br>Epi=0.18; MWFRS (directional) and C-C Zo<br>4-10-0, Zone2 34-10-0 to 39-0-15, Zone1<br>as & MWFRS for reactions shown; Lumber<br>Project engineer responsible for verifying<br>use component. | sign.<br>nph; TCDL=4.2psf; BCDL<br>one3 -2-0-0 to 1-0-0, Zone<br>39-0-15 to 45-2-0 zone; c<br>r DOL=1.60 plate grip DO<br>applied roof live load sho | =6.0psf; h=15ft; B=<br>e1 1-0-0 to 15-0-0,<br>cantilever left and r<br>iL=1.60<br>wn covers rain loa | =45ft; L=24ft; e<br>Zone2 15-0-0<br>right exposed ;r<br>iding requireme | eave=6ft; Cat.<br>to 19-2-15,<br>C-C for<br>ents specific | Munt + PR                                 | No 68  | NS 6<br>182<br>OF                  |  |  |

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 11, 2, 19.



Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

July 25,2024



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| <b>⊢</b> −−                        | 6-4-11                           | 13-0-0                           | 20-11-15          |             | 28-10-1                |          | 36-5-8     | 36         | 8-0 41-6-0      | 49-10-0                  |            |
|------------------------------------|----------------------------------|----------------------------------|-------------------|-------------|------------------------|----------|------------|------------|-----------------|--------------------------|------------|
|                                    | <u>6-4-11</u>                    | 6-7-4                            | 7-11-15           | 0.0.01      | 7-10-3                 |          | 1-1-1      | 0-         | 2-8 4-10-0      | 8-4-0                    |            |
| Plate Olisets (X, Y)-              | [3:0-2-8,0-3                     | -0], [4:0-6-0,0-2-8], [9:0-3-0,0 | 1-0-7], [16:0-2-0 | ,0-2-0]     |                        |          |            |            |                 |                          |            |
| LOADING (psf)                      | SPAG                             | <b>CING-</b> 2-0-0               | CSI.              |             | DEFL.                  | in       | (loc)      | l/defl     | L/d             | PLATES                   | GRIP       |
| TCLL 20.0                          | Plate                            | Grip DOL 1.25                    | TC 0.             | .80         | Vert(LL)               | -0.20    | 20-22      | >999       | 360             | MT20                     | 244/190    |
| TCDL 10.0                          | Lumb                             | per DOL 1.25                     | BC 0.             | .83         | Vert(CT)               | -0.37    | 20-22      | >999       | 240             | MT20HS                   | 187/143    |
| BCLL 0.0 *                         | Rep                              | Stress Incr YES                  | WB 0.             | .85         | Horz(CT)               | 0.08     | 17         | n/a        | n/a             |                          |            |
| BCDL 10.0                          | Code                             | FBC2023/TPI2014                  | Matrix-S          |             | Wind(LL)               | 0.07     | 22         | >999       | 240             | Weight: 308 lb           | FT = 20%   |
| LUMBER-                            |                                  |                                  |                   |             | BRACING-               |          |            |            |                 |                          |            |
| TOP CHORD 2x4                      | 4 SP No.2                        |                                  |                   |             | TOP CHORE              | 2        | Structura  | al wood s  | sheathing dire  | ctly applied or 2-2-0 or | c purlins, |
| BOT CHORD 2x4                      | 1 SP No.2                        |                                  |                   |             |                        |          | except e   | nd vertic  | als.            |                          |            |
| WEBS 2x4                           | 4 SP No.2                        |                                  |                   |             | BOT CHORE              | D        | Rigid ce   | iling dire | ctly applied or | 3-5-15 oc bracing.       |            |
|                                    |                                  |                                  |                   |             | WEBS                   |          | 1 Row a    | t midpt    | 4-2             | 20, 7-16                 |            |
| REACTIONS. (                       | (size) 14=0-4-                   | 0, 2=0-4-0, 17=0-5-0             |                   |             |                        |          |            |            |                 |                          |            |
| Ma                                 | ax Horz 2=182(L                  | _C 12)                           |                   |             |                        |          |            |            |                 |                          |            |
| Ma                                 | ax Uplift 14=-41(                | LC 9), 2=-105(LC 12), 17=-1      | 33(LC 12)         |             |                        |          |            |            |                 |                          |            |
| Ma                                 | ax Grav 14=197                   | (LC 24), 2=1616(LC 17), 17=      | 2766(LC 17)       |             |                        |          |            |            |                 |                          |            |
|                                    |                                  |                                  | ( )               |             |                        |          |            |            |                 |                          |            |
| FORCES. (lb) - M                   | lax. Comp./Max.                  | Ten All forces 250 (lb) or       | ess except whe    | en shown.   |                        |          |            |            |                 |                          |            |
| TOP CHORD 2-                       | -3=-2770/162.3                   | -4=-2123/160, 4-5=-1816/15       | 0. 5-7=-1816/15   | 50. 7-8=-9  | 0/1159.                |          |            |            |                 |                          |            |
| 8-                                 | -9=-97/1165 9-1                  | 10=-139/1314 10-11=-63/39        | ) 11-12=-63/38    | 39          |                        |          |            |            |                 |                          |            |
| BOT CHORD 2-                       | -23=-270/2428                    | 22-23=-272/2423 20-22=-17        | 4/1848 18-20=     | -51/916 1   | 16-17=-2650/280        |          |            |            |                 |                          |            |
| 8-                                 | -16831/211 1                     | 5-161087/111                     | 1/10/10, 10/20=   | 01/01/0,    | 10 11 - 2000/200,      |          |            |            |                 |                          |            |
| WEBS 3-                            | -23-0/260 3-22                   | 655/100 1.22-0/501 5.20          | 515/152 7-20      | 0115/11     | 60                     |          |            |            |                 |                          |            |
| VVLDO J-                           | -23-0/203, 3-22<br>6 19 60/025 7 | 2-000,100, 4-22-0,001, 0-20      |                   |             | 03,<br>E 47E/106       |          |            |            |                 |                          |            |
| 10                                 | 0-10=-09/925, 7                  | -16=-2416/176, 10-16=-312/       | 14, 10-15=-73/1   | 1045, 12-1  | 5=-475/106             |          |            |            |                 |                          |            |
| NOTES                              |                                  |                                  |                   |             |                        |          |            |            |                 |                          |            |
| NUIES-                             |                                  | have a second day of the state   |                   |             |                        |          |            |            |                 |                          |            |
| 1) Unbalanced roof                 | f live loads have                | been considered for this des     | lign.             |             |                        |          |            |            | <b>a</b> .      | annun in                 | 11111      |
| 2) Wind: ASCE 7-22                 | 2; Vult=130mph                   | (3-second gust) Vasd=101n        | iph; ICDL=4.2p    | ost; BCDL   | =6.0psf; h=15ft; B=    | =45ft; L | .=24ft; ea | ave=6ft;   | Cat.            | OUIN                     | VE         |
| II; Exp B; Encl., C                | GCpi=0.18; MW                    | FRS (directional) and C-C Zo     | one3 -2-0-0 to 1  | -0-0, Zone  | e1 1-0-0 to 13-0-0,    | Zone2    | 13-0-0     | to 17-2-1  | 5,              | N. OA.                   | -LE II     |
| Zone1 17-2-15 to                   | o 36-10-0, Zone                  | 3 36-10-0 to 37-7-8, Zone1 3     | 7-7-8 to 49-8-4   | zone; car   | ntilever left and righ | nt expo  | sed ;C-C   | C for      |                 | S S CEI                  | VSAT       |
| members and for                    | rces & MWFRS                     | for reactions shown; Lumber      | DOL=1.60 plat     | te grip DO  | L=1.60                 |          |            |            |                 |                          |            |
| <ol><li>Building Designe</li></ol> | er / Project engir               | neer responsible for verifying   | applied roof live | e load sho  | wn covers rain loa     | ding re  | equireme   | nts spec   | ific 🚬          | No 68                    | 182 : -    |
| to the use of this                 | s truss compone                  | nt.                              |                   |             |                        |          |            |            | =               |                          |            |
| <ol><li>Provide adequate</li></ol> | te drainage to pr                | event water ponding.             |                   |             |                        |          |            |            | =               | ×;                       | * =        |
| 5) All plates are MT               | F20 plates unles                 | s otherwise indicated.           |                   |             |                        |          |            |            | =               | : 🔺                      |            |
| 6) This truss has be               | een designed for                 | r a 10.0 psf bottom chord live   | load nonconcu     | irrent with | any other live load    | ls.      |            |            |                 | 7:                       |            |
| 7) * This trues has h              | haan dagianad f                  | or a live load of 20 Opef on th  | e hottom chord    | in all are  | as where a rectand     | 10 3-6-  | 0 tall by  | 2-0-0 wi   | do 💈            |                          |            |

7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

8) Bearing at joint(s) 14 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 14 except (jt=lb) 2=105, 17=133. Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

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| L (   | 6-4-12 11-0-0 17-5-0   | 23-10-0  | 30-3-0  | 36-5-8 36 <sub>7</sub> 8-0 41-6-   | 0 49-10-0  |  |  |  |  |
|---|--|--|---|--|--|--|--|--|--|
| (   | 6-5-0  | 6-5-0  | 6-5-0   | 6-2-8 0-2-8 4-10-  | 0 8-4-0  |  |  |  |  |
| Plate Offsets (X,Y)-  | - [4:0-6-0,0-2-8], [16:0-2-4,0-2-0]  |  |   |  |  |  |  |  |  |
| LOADING (psf)<br>TCLL 20.0  | SPACING- 2-0-0<br>Plate Grip DOL 1.25  | <b>CSI.</b><br>TC 0.64   | <b>DEFL.</b> ir<br>Vert(LL) -0.17   | n (loc) l/defl L/d<br>7 14-15 >931 360   | PLATES         GRIP           MT20         244/190                             |  |  |  |  |
| TCDL 10.0   | Lumber DOL 1.25  | BC 0.64  | Vert(CT) -0.34  | 4 14-15 >464 240   | MT20HS 187/143   |  |  |  |  |
| BCLL 0.0 *  | Rep Stress Incr YES  | WB 0.69  | Horz(CT) 0.09   | 9 17 n/a n/a   |  |  |  |  |  |
| BCDL 10.0   | Code FBC2023/TPI2014   | Matrix-S   | Wind(LL) 0.08   | 8 21 >999 240  | Weight: 300 lb FT = 20%  |  |  |  |  |
| LUMBER-<br>TOP CHORD 2x4<br>BOT CHORD 2x4<br>WEBS 2x4<br>REACTIONS. (Ma<br>Ma<br>Ma   | SP No.2<br>SP No.2<br>SP No.2<br>size) 14=0-4-0, 2=0-4-0, 17=0-5-0<br>x Horz 2=168(LC 12)<br>x Uplift 14=-45(LC 9), 2=-109(LC 12), 17=-<br>x Grav 14=172(LC 1), 2=1459(LC 1), 17=2   | 124(LC 12)<br>461(LC 1)  | BRACING-<br>TOP CHORD<br>BOT CHORD<br>WEBS  | Structural wood sheathing<br>except end verticals.<br>Rigid ceiling directly applie<br>1 Row at midpt                                      | directly applied or 3-1-9 oc purlins,<br>ed or 3-8-8 oc bracing.<br>6-18, 8-16 |  |  |  |  |
| FORCES.         (lb) - M           TOP CHORD         2-           8         8           BOT CHORD         2-           1         1           WEBS         3-           6-         10  | FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown.         TOP CHORD       2-3=-2448/142, 3-4=-2016/154, 4-5=-2013/156, 5-6=-2013/156, 6-8=-707/38,<br>8-9=-96/1247, 9-10=-100/1257, 10-12=-46/323         BOT CHORD       2-24=-224/2096, 23-24=-224/2096, 21-23=-160/1750, 19-21=-112/1651, 18-19=-112/1651,<br>16-17=-2405/199, 9-16=-352/94, 15-16=-341/45         WEBS       3-23=-431/77, 4-23=0/398, 4-21=-6/348, 5-21=-411/121, 6-21=-58/479, 6-19=0/250,<br>6-18=-1248/100, 8-18=0/725, 16-18=-48/749, 8-16=-2246/151, 10-16=-1137/68,<br>10-15=0/436, 12-15=-434/84   |  |   |  |  |  |  |  |  |
| <ol> <li>NOTES-</li> <li>1) Wind: ASCE 7-2<br/>II; Exp B; Encl., (<br/>Zone1 15-2-15 to<br/>Lumber DOL=1.6</li> <li>2) Building Designe<br/>to the use of this</li> <li>3) Provide adequat</li> <li>4) All plates are MT</li> <li>5) All plates are axt</li> <li>6) This truss has be</li> <li>7) * This truss has be</li> <li>7) * This truss has be</li> <li>7) * This truss has in<br/>will fit between th</li> <li>8) Bearing at joint(s<br/>capacity of beari</li> </ol> | 2; Vult=130mph (3-second gust) Vasd=101<br>GCpi=0.18; MWFRS (directional) and C-C 2<br>0 49-8-4 zone; cantilever left and right expo<br>30 plate grip DOL=1.60<br>r / Project engineer responsible for verifying<br>truss component.<br>e drainage to prevent water ponding.<br>20 plates unless otherwise indicated.<br>3 MT20 unless otherwise indicated.<br>4 MT20 unless otherwise indicated.<br>5 MT20 unless otherwise indicated.<br>9 me designed for a 10.0 psf bottom chord liv<br>peen designed for a 10.0 psf bottom chord liv<br>peen designed for a nive load of 20.0psf on<br>the bottom chord and any other members.<br>9 14 considers parallel to grain value using<br>ng surface. | mph; TCDL=4.2psf; BCDL<br>Zone3 -2-0-0 to 1-0-0, Zone<br>sed ;C-C for members and<br>g applied roof live load sho<br>re load nonconcurrent with<br>the bottom chord in all area<br>ANSI/TPI 1 angle to grain | =6.0psf; h=15ft; B=45ft;<br>e1 1-0-0 to 11-0-0, Zone<br>I forces & MWFRS for re<br>wm covers rain loading i<br>any other live loads.<br>as where a rectangle 3-6<br>formula. Building desig | L=24ft; eave=6ft; Cat.<br>22 11-0-0 to 15-2-15,<br>eactions shown;<br>requirements specific<br>6-0 tall by 2-0-0 wide<br>ner should verify | D STORIO PROVINCE  |  |  |  |  |

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 14 except (jt=lb) 2=109, 17=124.

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and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcscomponents.com)



|   | 6-4-1   | 2 9-0-0  | 15-11-0  | 22-10-0  | 29-9-0   |   | 36-6-8  | 36-  | 8-0 41-6-0   | 49-10-0  |                           |
|---|---|--|--|--|--|---|---|--|--|--|---------------------------|
|   | 6-4-1   | 2 2-7-4  | 6-11-0   | 6-11-0   | 6-11-0   | 1   | 6-9-8   | 0-1  | -8 4-10-0  | 8-4-0  |                           |
| Plate Offse   | ts (X,Y)  | [4:0-6-0,0-2-8], [16:0-2   | 2-0,0-2-0], [17:Edg  | e,0-1-8]   |  |   |   |  |  |  |                           |
| LOADING<br>TCLL<br>TCDL   | (psf)<br>20.0<br>10.0   | SPACING-<br>Plate Grip DOL<br>Lumber DOL   | 2-0-0<br>. 1.25<br>1.25  | <b>CSI.</b><br>TC 0.77<br>BC 0.70  | DEFL.<br>Vert(LL)<br>Vert(CT   | ir<br>-0.18<br>0.38   | n (loc)<br>8 19-21<br>8 19-21   | l/defl<br>>999<br>>999                                 | L/d<br>360<br>240                                  | PLATES<br>MT20   | <b>GRIP</b><br>244/190    |
| BCDI  | 10.0  | Code FBC202  | 3/TDI201/  | Matrix-S   | Wind(L   | ) 0.10  | 10-21   | 11/a   | 240  | Weight: 284 lb   | FT - 20%                  |
| LUMBER-<br>TOP CHOF<br>BOT CHOF<br>WEBS<br>REACTION   | RD 2x4 SP<br>RD 2x4 SP<br>2x4 SP<br>IS. (size<br>Max He<br>Max U<br>Max G   | No.2<br>No.2<br>No.2<br>e) 14=0-4-0, 2=0-4-(<br>orz 2=144(LC 12)<br>plift 14=-43(LC 9), 2=-<br>rav 14=238(LC 1), 2=  | 0, 17=0-3-0<br>-114(LC 12), 17=-1<br>1484(LC 1), 17=23   | 14(LC 12)<br>70(LC 1)  | BRACIN<br>TOP CH<br>BOT CH<br>WEBS   | g-<br>DRD<br>DRD  | Structura<br>except e<br>Rigid cei<br>1 Row at                        | al wood s<br>nd vertic<br>ling direc<br>t midpt        | sheathing direct<br>als.<br>ctly applied or<br>6-1 | 204 no<br>204 no<br>20 | c purlins,                |
| FORCES.<br>TOP CHOP<br>BOT CHOP<br>WEBS   | (lb) - Max.<br>RD 2-3=-3<br>8-9=-4<br>RD 2-24=<br>16-13<br>3-23=<br>6-18=<br>11-15  | Comp./Max. Ten All<br>2492/156, 3-4=-2207/<br>93/1368, 9-11=-103/1<br>-210/2134, 23-24=-21<br>7=-2309/187, 9-16=-3<br>312/69, 4-23=0/366,<br>1333/100, 8-18=0/58<br>=0/387, 12-15=-380/8   | forces 250 (lb) or 1<br>174, 4-5=-2545/18-<br>396<br>10/2134, 21-23=-16<br>75/99, 15-16=-261/<br>4-21=-33/719, 5-2<br>37, 16-18=-77/1204   | ess except when sh<br>I, 5-6=-2545/184, 6-<br>5/1945, 19-21=-145<br>41<br> =-444/130, 6-21=-4<br>, 8-16=-2684/165, 1                   | iown.<br>-8=-1143/63,<br>5/2256, 18-19=-145/;<br>16/346, 6-19=0/269,<br>1-16=-1284/70,   | 256,  |   |  |  |  |                           |
| NOTES-<br>1) Wind: A:<br>II; Exp E<br>Zone1 1<br>Lumber<br>2) Building<br>to the us<br>3) Provide<br>4) All plate<br>5) This trus<br>6) * This tru<br>will fit be | SCE 7-22; V<br>;; Encl., GCp<br>3-2-15 to 49<br>DOL=1.60 p<br>Designer / F<br>se of this trus<br>adequate dr.<br>s are 3x6 MT<br>ss has been<br>uss has been<br>tween the b | ult=130mph (3-second<br>i=0.18; MWFRS (dire-<br>-8-4 zone; cantilever I<br>late grip DOL=1.60<br>Project engineer respo<br>ss component.<br>ainage to prevent wat<br>f20 unless otherwise i<br>designed for a 10.0 ps<br>in designed for a live Ic<br>ottom chord and any of | d gust) Vasd=101m<br>ctional) and C-C Zo<br>eft and right expose<br>insible for verifying<br>er ponding.<br>indicated.<br>if bottom chord live<br>boad of 20.0psf on th<br>ther members. | ph; TCDL=4.2psf; E<br>ine3 -2-0-0 to 1-0-0,<br>ed ;C-C for member<br>applied roof live loa<br>load nonconcurren<br>e bottom chord in a | 3CDL=6.0psf; h=15f<br>Zone1 1-0-0 to 9-0<br>s and forces & MWF<br>d shown covers rain<br>t with any other live<br>II areas where a rec | ; B=45ft;<br>0, Zone2<br>RS for re<br>loading r<br>oads.<br>angle 3-6 | L=24ft; ea<br>9-0-0 to 1<br>actions sh<br>equirement<br>6-0 tall by 2 | ave=6ft;<br>13-2-15,<br>nown;<br>nts spec<br>2-0-0 wie | Cat.<br>ific<br>de                                 | No 68  | VELEX<br>NSE<br>182<br>OF |

 Bearing at joint(s) 14 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 14 except (jt=lb) 2=114, 17=114.



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|   | Job                         | Truss                     | Truss Type      | Qty | Ply         | 2508-CR-2 Car   |           |
|---|-----------------------------|---------------------------|-----------------|-----|-------------|---|-----------|
|   | 0040440                     |                           |                 |     |             |   | T34534220 |
|   | 6243113                     | A19                       | Hair Hip Girder | 1   | 2           | Job Reference (optional)                                |           |
| Ì | Tibbetts Lumber Co., LLC (C | cala, FL), Ocala, FL - 34 | 472,            |     | 3.730 s Jul | 11 2024 MiTek Industries, Inc. Wed Jul 24 11:26:59 2024 | Page 2    |

### NOTES-

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11) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 141 lb down and 86 lb up at 7-0-0, 122 lb down and 83 lb up at 9-0-12, 122 lb down and 83 lb up at 11-0-12, 122 lb down and 83 lb up at 13-0-12, 122 lb down and 83 lb up at 13-0-12, 122 lb down and 83 lb up at 13-0-12, 122 lb down and 83 lb up at 23-0-12, 122 lb down and 83 lb up at 25-0-0, 122 lb down and 83 lb up at 21-0-12, 122 lb down and 83 lb up at 23-0-12, 122 lb down and 83 lb up at 25-9-0, 122 lb down and 83 lb up at 21-0-12, 122 lb down and 83 lb up at 23-0-12, 122 lb down and 83 lb up at 25-9-0, 122 lb down and 83 lb up at 21-0-12, 122 lb down and 83 lb up at 33-9-9, 122 lb down and 35 lb up at 33-9-9, 122 lb down and 71 lb up at 47-9-4, and 220 lb down and 30 lb up at 49-8-4 on top chord, and 310 lb down at 7-0-0, 95 lb down at 33-0-12, 95 lb down at 13-0-12, 95 lb down at 25-0-12, 95 lb down at 23-0-12, 95 lb down at 33-9-9, 95 lb down at

### LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf) Vert: 1-3=-60, 3-11=-60, 2-15=-20, 13-14=-20, 12-13=-20

Concentrated Loads (lb)

Vert: 3=-122(B) 11=-220 20=-262(B) 19=-48(B) 4=-122(B) 5=-122(B) 18=-48(B) 17=-48(B) 6=-122(B) 10=-5(B) 21=-122(B) 22=-122(B) 23=-122(B) 24=-122(B) 26=-122(B) 27=-122(B) 28=-122(B) 29=-122(B) 30=-122(B) 31=-122(B) 32=-122(B) 34=-122(B) 35=-122(B) 35=-148(B) 45=-48(B) 45=-48(B) 45=-48(B) 45=-48(B) 45=-48(B) 45=-48(B) 45=-48(B) 45=-48(B) 45=-48(B) 55=-48(B) 55=-48(

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.
 Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not
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 building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing
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 fabrication, storage, delivery, erection and bracing of trusses and truss systems, see
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 and BCSI Building Component Safety Information
 available from the Structural Building Component Association (www.sbcscomponents.com)





BRACING-

TOP CHORD

BOT CHORD

### LUMBER-

 TOP CHORD
 2x4 SP No.2

 BOT CHORD
 2x4 SP M 31 or 2x4 SP SS

 WEBS
 2x4 SP No.2

Structural wood sheathing directly applied or 3-11-2 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 2=0-4-0, 8=0-4-0 Max Horz 2=-114(LC 10)

Max Grav 2=1298(LC 17), 8=1298(LC 18)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2069/0, 3-4=-1864/0, 4-5=-1870/0, 5-6=-1870/0, 6-7=-1864/0, 7-8=-2070/0

BOT CHORD 2-11=0/1859, 10-11=0/1242, 8-10=0/1774

WEBS 5-10=0/885, 5-11=0/885

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 -2-0-0 to 1-0-0, Zone1 1-0-0 to 11-11-0, Zone2 11-11-0 to 16-1-15, Zone1 16-1-15 to 25-10-0 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Load case(s) 1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13, 14, 15, 16, 17, 18, 19, 20, 21, 22, 23, 24, 25, 26 has/have been modified. Building designer must review loads to verify that they are correct for the intended use of this truss.

### LOAD CASE(S) Standard

- 1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (olf)
  - Vert: 1-5=-60, 5-9=-60, 2-11=-20, 10-11=-60, 8-10=-20
- Dead + 0.75 Roof Live (balanced) + 0.75 Uninhab. Attic Storage: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)
- Vert: 1-5=-50, 5-9=-50, 2-11=-35, 11-16=-75, 16-17=-90, 10-17=-75, 8-10=-35
- Dead + Uninhabitable Attic Without Storage: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)
- Vert: 1-5=-20, 5-9=-20, 2-11=-40, 10-11=-80, 8-10=-40

4) Dead + 0.6 C-C Wind (Pos. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.60

### Continued on page 2

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and BCSI Building Component Safety Information
available from the Structural Building Component Association (www.sbcscomponents.com)



Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

July 25,2024



| Job                         | Truss                      | Truss Type | Qty | Ply         | 2508-CR-2 Car   |           |
|-----------------------------|----------------------------|------------|-----|-------------|---|-----------|
|                             |                            |            |     |             |   | T34534221 |
| 6243113                     | B01                        | Common     | 9   | 1           |   |           |
|                             |                            |            |     |             | Job Reference (optional)                                |           |
| Tibbetts Lumber Co., LLC (C | Dcala, FL). Ocala, FL - 34 | 472.       |     | 8.730 s Jul | 11 2024 MiTek Industries. Inc. Wed Jul 24 11:27:00 2024 | Page 2    |

 Ocala, FL - 34472,
 8.730 s Jul 11 2024 MiTek Industries, Inc.
 Wed Jul 24 11:27:00 2024 Page 2

 ID:Ts3RJ0261\_Xu2fYgSyBHAWzZSLZ-fK8gyjA9zivbVyTrcPtG5y9ujvJFKa3?3lfsY3yuswf

LOAD CASE(S) Standard Uniform Loads (plf) Vert: 1-2=47, 2-12=32, 5-12=17, 5-14=26, 8-14=17, 8-9=12, 2-11=-12, 10-11=-52, 8-10=-12 Horz: 1-2=-55, 2-12=-40, 5-12=-25, 5-14=35, 8-14=25, 8-9=21 5) Dead + 0.6 C-C Wind (Pos. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=12, 2-13=17, 5-13=26, 5-15=17, 8-15=32, 8-9=47, 2-11=-12, 10-11=-52, 8-10=-12 Horz: 1-2=-21, 2-13=-25, 5-13=-35, 5-15=25, 8-15=40, 8-9=55 6) Dead + 0.6 C-C Wind (Neg. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-8, 2-5=-32, 5-8=-32, 8-9=-28, 2-11=-20, 10-11=-60, 8-10=-20 Horz: 1-2=-12, 2-5=12, 5-8=-12, 8-9=-8 7) Dead + 0.6 C-C Wind (Neg. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-28, 2-5=-32, 5-8=-32, 8-9=-8, 2-11=-20, 10-11=-60, 8-10=-20 Horz: 1-2=8, 2-5=12, 5-8=-12, 8-9=12 8) Dead + 0.6 MWFRS Wind (Pos. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=15, 2-5=3, 5-8=9, 8-9=4, 2-11=-12, 10-11=-52, 8-10=-12 Horz: 1-2=-24, 2-5=-11, 5-8=17, 8-9=13 9) Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=4, 2-5=9, 5-8=3, 8-9=15, 2-11=-12, 10-11=-52, 8-10=-12 Horz 1-2=-13 2-5=-17 5-8=11 8-9=24 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-24, 2-5=-28, 5-8=-12, 8-9=-7, 2-11=-20, 10-11=-60, 8-10=-20 Horz: 1-2=4, 2-5=8, 5-8=8, 8-9=13 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-7, 2-5=-12, 5-8=-28, 8-9=-24, 2-11=-20, 10-11=-60, 8-10=-20 Horz: 1-2=-13, 2-5=-8, 5-8=-8, 8-9=-4 12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=28, 2-5=15, 5-8=15, 8-9=28, 2-11=-12, 10-11=-52, 8-10=-12 Horz: 1-2=-37, 2-5=-24, 5-8=24, 8-9=37 13) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=15, 2-5=3, 5-8=3, 8-9=15, 2-11=-12, 10-11=-52, 8-10=-12 Horz: 1-2=-24, 2-5=-11, 5-8=11, 8-9=24 14) Dead + 0.6 MWFRS Wind (Neg. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-16, 2-5=-21, 5-8=-21, 8-9=-16, 2-11=-20, 10-11=-60, 8-10=-20 Horz: 1-2=-4, 2-5=1, 5-8=-1, 8-9=4 15) Dead + 0.6 MWFRS Wind (Neg. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-16, 2-5=-21, 5-8=-21, 8-9=-16, 2-11=-20, 10-11=-60, 8-10=-20 Horz: 1-2=-4, 2-5=1, 5-8=-1, 8-9=4 16) Dead + Uninhabitable Attic Storage: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf) Vert: 1-5=-20, 5-9=-20, 2-11=-40, 11-16=-80, 16-17=-100, 10-17=-80, 8-10=-40 17) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) Left): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-53, 2-5=-56, 5-8=-44, 8-9=-40, 2-11=-35, 11-16=-75, 16-17=-90, 10-17=-75, 8-10=-35 Horz: 1-2=3, 2-5=6, 5-8=6, 8-9=10 18) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) Right): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-40, 2-5=-44, 5-8=-56, 8-9=-53, 2-11=-35, 11-16=-75, 16-17=-90, 10-17=-75, 8-10=-35 Horz: 1-2=-10, 2-5=-6, 5-8=-6, 8-9=-3 19) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) 1st Parallel): Lumber Increase=1.60 Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-47, 2-5=-51, 5-8=-51, 8-9=-47, 2-11=-35, 11-16=-75, 16-17=-90, 10-17=-75, 8-10=-35 Horz: 1-2=-3, 2-5=1, 5-8=-1, 8-9=3 20) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) 2nd Parallel): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-47, 2-5=-51, 5-8=-51, 8-9=-47, 2-11=-35, 11-16=-75, 16-17=-90, 10-17=-75, 8-10=-35 Horz: 1-2=-3, 2-5=1, 5-8=-1, 8-9=3 21) Dead + 0.6 C-C Wind Min. Down: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 1-2=8, 2-5=-25, 5-9=-25, 2-11=-12, 10-11=-52, 8-10=-12 Horz: 1-2=-16, 2-5=16, 5-9=-16

22) Dead + 0.6 C-C Wind Min. Upward: Lumber Increase=1.60, Plate Increase=1.60

### Continued on page 3

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality** Criteria **and DSE-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcscomponents.com)



| מסנ     | Truss | Truss Type | Qty | Ply | 2508-CR-2 Car            |          |
|---------|-------|------------|-----|-----|--------------------------|----------|
| 2040440 | DO1   | Common     | 0   | 1   | Т                        | 34534221 |
| 0243113 | DUI   | Common     | 9   | 1   | Job Reference (optional) |          |

Tibbetts Lumber Co., LLC (Ocala, FL), Ocala, FL - 34472,

8.730 s Jul 11 2024 MiTek Industries, Inc. Wed Jul 24 11:27:00 2024 Page 3 ID:Ts3RJ0261\_Xu2fYgSyBHAWzZSLZ-fK8gyjA9zivbVyTrcPtG5y9ujvJFKa3?3lfsY3yuswf

### LOAD CASE(S) Standard

Uniform Loads (plf) Vert: 1-5=8, 5-9=8, 2-11=-12, 10-11=-52, 8-10=-12

- Horz: 1-5=-16, 5-9=16
- 23) 1st Dead + Roof Live (unbalanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)
- Vert: 1-5=-60, 5-9=-20, 2-11=-20, 10-11=-60, 8-10=-20
- 24) 2nd Dead + Roof Live (unbalanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)
- Vert: 1-5=-20, 5-9=-60, 2-11=-20, 10-11=-60, 8-10=-20
- 25) 3rd Dead + 0.75 Roof Live (unbalanced) + 0.75 Uninhab. Attic Storage: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)
  - Vert: 1-5=-50, 5-9=-20, 2-11=-35, 11-16=-75, 16-17=-90, 10-17=-75, 8-10=-35
- 26) 4th Dead + 0.75 Roof Live (unbalanced) + 0.75 Uninhab. Attic Storage: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-5=-20, 5-9=-50, 2-11=-35, 11-16=-75, 16-17=-90, 10-17=-75, 8-10=-35

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and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcscomponents.com)





| Plate Offsets (X,Y)                     | [2:0-4-0,0-2-1], [14:0-4-0,0-2-1], [20:0-2-              | 8,0-3-0]                          |  |  |
|---|--|-----------------------------------|--|--|
| LOADING (psf)<br>TCLL 20.0<br>TCDL 10.0 | SPACING- 2-0-0<br>Plate Grip DOL 1.25<br>Lumber DOL 1.25 | <b>CSI.</b><br>TC 0.26<br>BC 0.10 | <b>DEFL.</b> in (loc) I/defl L/d<br>Vert(LL) -0.01 15 n/r 120<br>Vert(CT) -0.02 15 n/r 120 | PLATES         GRIP           MT20         244/190 |
| BCLL 0.0 *<br>BCDL 10.0                 | Rep Stress Incr YES<br>Code FBC2023/TPI2014              | WB 0.07<br>Matrix-S               | Horz(CT) 0.00 14 n/a n/a   | Weight: 129 lb FT = 20%                            |
| LUMBER-                                 |  |                                   | BRACING-   |  |

TOP CHORD

BOT CHORD

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 OTHERS 2x4 SP No.2

REACTIONS. All bearings 23-10-0.

Max Horz 2=-109(LC 10) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 2, 14, 21, 22, 23, 19, 18, 17

Max Grav All reactions 250 lb or less at joint(s) 20, 21, 22, 23, 19, 18, 17 except 2=285(LC 23), 14=286(LC 24), 24=250(LC 23), 16=250(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=2ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

5) All plates are 2x4 MT20 unless otherwise indicated.

6) Gable requires continuous bottom chord bearing.

7) Gable studs spaced at 2-0-0 oc.

8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

9) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 14, 21, 22, 23, 19, 18, 17.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 6-0-0 oc bracing.

Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

July 25,2024



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|   | 8-1-8  | 1   | 15-5-4   | 19-2-0   | 23-9-8   |
|---|--|---|--|--|--|
| F   | 8-1-8  | I   | 7-3-12   | 3-8-12   | 4-7-8  |
| Plate Offsets (X,Y)                                   | [8:0-3-8,0-3-0], [11:0-4-0,0-3-0]  |   |  |  |  |
| LOADING (psf)<br>TCLL 20.0<br>TCDL 10.0<br>BCLL 0.0 * | SPACING- 2-0-0<br>Plate Grip DOL 1.25<br>Lumber DOL 1.25<br>Rep Stress Incr NO | CSI.<br>TC 0.58<br>BC 0.48<br>WB 0.41<br>Matrix S | DEFL. in (loc<br>Vert(LL) -0.10 11-1<br>Vert(CT) -0.31 11-1<br>Horz(CT) 0.02 1<br>Wind(L) 0.04 111 | <ul> <li>) I/defl L/d</li> <li>2 &gt;999 360</li> <li>2 &gt;904 240</li> <li>4 n/a n/a</li> <li>2 &gt;902 240</li> </ul> | PLATES         GRIP           MT20         244/190 |

| LU | JMBER- | - |
|----|--------|---|
|    |        |   |

| TOP CHORD | 2x4 SP No.2              |
|-----------|--------------------------|
| BOT CHORD | 2x4 SP M 31 or 2x4 SP SS |
| WEBS      | 2x4 SP No.2              |
| OTHERS    | 2x4 SP No.2              |

 BRACING 

 TOP CHORD
 Structural wood sheathing directly applied or 3-11-0 oc purlins, except end verticals.

 BOT CHORD
 Rigid ceiling directly applied or 10-0-0 oc bracing.

- REACTIONS. (size) 2=0-4-0, 14=0-3-8 Max Horz 2=92(LC 12) Max Grav 2=1304(LC 17), 14=1132(LC 19)
- FORCES. (Ib) Max. Comp./Max. Ten. All forces 250 (Ib) or less except when shown.
- TOP CHORD 2-3=-2085/0, 3-4=-1880/0, 4-5=-1887/0, 5-6=-1870/0, 6-7=-1862/0, 7-8=-1757/0
- BOT CHORD 2-12=0/1849, 11-12=0/1227, 10-11=0/1812
- WEBS 5-12=0/897, 5-11=0/882, 7-10=-904/0, 8-10=0/1837, 8-14=-1311/0

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 -2-0-0 to 1-0-0, Zone1 1-0-0 to 11-11-0, Zone2 11-11-0 to 16-1-15, Zone1 16-1-15 to 23-4-4 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) Provide adequate drainage to prevent water ponding.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Bearing at joint(s) 14 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Load case(s) 1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13, 14, 15, 16, 17, 18, 19, 20, 21, 22, 23, 24, 25, 26 has/have been modified. Building designer must review loads to verify that they are correct for the intended use of this truss.

### LOAD CASE(S) Standard

- 1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
  - Uniform Loads (plf) Vert: 1-5=-60, 5-7=-60, 7-8=-60, 2-12=-20, 11-12=-60, 9-11=-20
- Dead + 0.75 Roof Live (balanced) + 0.75 Uninhab. Attic Storage: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)
  - Vert: 1-5=-50, 5-7=-50, 7-8=-50, 2-12=-35, 12-18=-75, 18-19=-90, 11-19=-75, 9-11=-35

### Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality** Criteria **and DSE-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcscomponents.com)



Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

July 25,2024



| Job   | Truss   | Truss Type   | Qty                                | Ply         | 2508-CR-2 Car                      | T34534223                     |  |  |  |
|---|---|--|------------------------------------|-------------|------------------------------------|-------------------------------|--|--|--|
| 6243113   | B02   | Roof Special   | 1                                  | 1           | lah Deferance (antional)           |                               |  |  |  |
| Tibbetts Lumber Co., LLC (  | Ocala, FL), Ocala, FL - 34  | 472,   |                                    | 8.730 s Jul | 11 2024 MiTek Industries, Inc. Wed | d Jul 24 11:27:01 2024 Page 2 |  |  |  |
|   |   | IC   | D:Ts3RJ0261_Xu                     | 2fYgSyBHA   | WzZSLZ-7Wi2A3Ank01S7611A6O         | /d9i?rJeH3zq8IPPQ5Wyuswe      |  |  |  |
| LOAD CASE(S) Standar<br>3) Dead + Uninhabitable A<br>Uniform Loads (plf)<br>Vert: 1-5=-20, i<br>4) Dead + 0.6 C-C Wind (l<br>Uniform Loads (plf)                      | d<br>Attic Without Storage: Lumber<br>5-7=-20, 7-8=-20, 2-12=-40, 1<br>Pos. Internal) Case 1: Lumbe | Increase=1.25, Plate Increase=1.25<br>1-12=-80, 9-11=-40<br>r Increase=1.60, Plate Increase=1.60 |                                    |             |                                    |                               |  |  |  |
| Uniform Loads (pit)<br>Vert: 1-2=47, 2-15=32, 5-15=17, 5-17=26, 7-17=17, 7-8=17, 2-12=-12, 11-12=-52, 9-11=-12<br>Horz: 1-2=-55, 2-15=-40, 5-15=-25, 5-17=35, 7-17=25 |   |  |                                    |             |                                    |                               |  |  |  |
| Drag: 7-8=-0<br>5) Dead + 0.6 C-C Wind (I<br>Uniform Loads (plf)  | Pos. Internal) Case 2: Lumbe  | r Increase=1.60, Plate Increase=1.60   |                                    |             |                                    |                               |  |  |  |
| Vert: 1-2=12, 2<br>Horz: 1-2=-21,<br>Drag: 7-8=-0   | 2-16=17, 5-16=26, 5-7=17, 7-8<br>2-16=-25, 5-16=-35, 5-7=25   | 3=17, 2-12=-12, 11-12=-52, 9-11=-12  |                                    |             |                                    |                               |  |  |  |
| 6) Dead + 0.6 C-C Wind (I<br>Uniform Loads (plf)  | Neg. Internal) Case 1: Lumbe  | r Increase=1.60, Plate Increase=1.60   |                                    |             |                                    |                               |  |  |  |
| Horz: 1-2=-0, 2<br>Horz: 1-2=-12,<br>Drag: 7-8=0  | 2-5=12, 5-7=-12   | z 20, 11-1200, 3-1120  |                                    |             |                                    |                               |  |  |  |
| Uniform Loads (plf)<br>Vert: 1-2=-28, 2   | 2-5=-32, 5-7=-32, 7-8=-32, 2-   | r Increase=1.60, Plate Increase=1.60<br>12=-20, 11-12=-60, 9-11=-20                              |                                    |             |                                    |                               |  |  |  |
| Horz: 1-2=8, 2-<br>Drag: 7-8=0<br>8) Dead + 0.6 MWFRS Wi  | -5=12, 5-7=-12<br>ind (Pos. Internal) Left: Lumb  | er Increase=1.60, Plate Increase=1.60  |                                    |             |                                    |                               |  |  |  |
| Uniform Loads (plf)<br>Vert: 1-2=15, 2<br>Horz: 1-2=-24.  | 2-5=3, 5-7=9, 7-8=8, 2-12=-12<br>2-5=-11. 5-7=17  | 2, 11-12=-52, 9-11=-12   |                                    |             |                                    |                               |  |  |  |
| Drag: 7-8=-0<br>9) Dead + 0.6 MWFRS Wi<br>Uniform Loads (plf)   | ind (Pos. Internal) Right: Lum  | ber Increase=1.60, Plate Increase=1.60   |                                    |             |                                    |                               |  |  |  |
| Vert: 1-2=4, 2-<br>Horz: 1-2=-13,   | 5=9, 5-7=3, 7-8=18, 2-12=-12<br>2-5=-17, 5-7=11   | 2, 11-12=-52, 9-11=-12   |                                    |             |                                    |                               |  |  |  |
| 10) Dead + 0.6 MWFRS V<br>Uniform Loads (plf)   | Vind (Neg. Internal) Left: Lum  | ber Increase=1.60, Plate Increase=1.60   |                                    |             |                                    |                               |  |  |  |
| Vert: 1-2=-24<br>Horz: 1-2=4, :<br>11) Dead + 0.6 MWFRS V   | , 2-5=-28, 5-7=-12, 7-8=-21, 2<br>2-5=8, 5-7=8<br>Vind (Neg. Internal) Right: Lu                    | 2-12=-20, 11-12=-60, 9-11=-20<br>mber Increase=1.60, Plate Increase=1.6(                         | 0                                  |             |                                    |                               |  |  |  |
| Uniform Loads (plf)<br>Vert: 1-2=-7,<br>Horz: 1-213   | 2-5=-12, 5-7=-28, 7-8=-21, 2-   | 12=-20, 11-12=-60, 9-11=-20  |                                    |             |                                    |                               |  |  |  |
| 12) Dead + 0.6 MWFRS V<br>Uniform Loads (plf)   | Vind (Pos. Internal) 1st Parall   | el: Lumber Increase=1.60, Plate Increase   | e=1.60                             |             |                                    |                               |  |  |  |
| Vert: 1-2=28,<br>Horz: 1-2=-37<br>Drag: 7-8=-0  | 2-5=15, 5-7=15, 7-8=15, 2-12<br>7, 2-5=-24, 5-7=24  | 2=-12, 11-12=-52, 9-11=-12   |                                    |             |                                    |                               |  |  |  |
| 13) Dead + 0.6 MWFRS V<br>Uniform Loads (plf)<br>Vert: 1-2=15   | Vind (Pos. Internal) 2nd Paral  | lel: Lumber Increase=1.60, Plate Increas   | se=1.60                            |             |                                    |                               |  |  |  |
| Horz: 1-2=-24<br>Drag: 7-8=-0   | 4, 2-5=-11, 5-7=11  |  | - 1.00                             |             |                                    |                               |  |  |  |
| Uniform Loads (plf)<br>Vert: 1-2=-16  | , 2-5=-21, 5-7=-21, 7-8=-21, 2  | 2-12=-20, 11-12=-60, 9-11=-20  | e=1.00                             |             |                                    |                               |  |  |  |
| Horz: 1-2=-4,<br>15) Dead + 0.6 MWFRS V<br>Uniform Loads (plf)  | 2-5=1, 5-7=-1<br>Vind (Neg. Internal) 2nd Para  | llel: Lumber Increase=1.60, Plate Increas  | se=1.60                            |             |                                    |                               |  |  |  |
| Vert: 1-2=-16<br>Horz: 1-2=-4,<br>16) Dead + Uninhabitable  | , 2-5=-21, 5-7=-21, 7-8=-21, 2<br>2-5=1, 5-7=-1<br>Attic Storage: Lumber Increa                     | 2-12=-20, 11-12=-60, 9-11=-20  |                                    |             |                                    |                               |  |  |  |
| Uniform Loads (plf)<br>Vert: 1-5=-20  | , 5-7=-20, 7-8=-20, 2-12=-40,   | 12-18=-80, 18-19=-100, 11-19=-80, 9-1  | 1=-40                              |             |                                    |                               |  |  |  |
| Increase=1.60<br>Uniform Loads (plf)  | : (Dai.) + 0.75 Uninnab. Attic \$   | ວເບເລge + ບ.7ວ(ປ.6 MWFRS WIND (Neg. I  | ini, Leii): Lumbe                  | I INCLEASE: | = 1.00, Male                       |                               |  |  |  |
| Vert: 1-2=-53<br>Horz: 1-2=3, 1<br>18) Dead + 0.75 Roof Live  | , 2-5=-56, 5-7=-44, 7-8=-51, 2<br>2-5=6, 5-7=6<br>e (bal.) + 0.75 Uninhab. Attic \$                 | 2-12=-35, 12-18=-75, 18-19=-90, 11-19=-<br>Storage + 0.75(0.6 MWFRS Wind (Neg. I                 | -75, 9-11=-35<br>Int) Right): Lumb | er Increas  | e=1.60,                            |                               |  |  |  |
| Plate Increase=1.60<br>Uniform Loads (plf)<br>Vert: 1-2=-40   | , 2-5=-44, 5-7=-56, 7-8=-51, 2  | 2-12=-35, 12-18=-75, 18-19=-90, 11-19=-  | -75, 9-11=-35                      |             | ,                                  |                               |  |  |  |
| Horz: 1-2=-10<br>19) Dead + 0.75 Roof Live<br>, Plate Increase=1.60   | 0, 2-5=-6, 5-7=-6<br>e (bal.) + 0.75 Uninhab. Attic s   | Storage + 0.75(0.6 MWFRS Wind (Neg. I  | Int) 1st Parallel):                | Lumber In   | crease=1.60                        |                               |  |  |  |

Continued on page 3

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| Ì | Job                         | Truss                     | Truss Type   | Qty | Ply         | 2508-CR-2 Car   |           |
|---|-----------------------------|---------------------------|--------------|-----|-------------|---|-----------|
|   |                             |                           |              |     |             |   | T34534223 |
|   | 6243113                     | B02                       | Roof Special | 1   | 1           |   |           |
|   |                             |                           |              |     |             | Job Reference (optional)                                |           |
|   | Tibbetts Lumber Co., LLC (C | cala, FL). Ocala, FL - 34 | 472.         | 8   | 3.730 s Jul | 11 2024 MiTek Industries, Inc. Wed Jul 24 11:27:01 2024 | Page 3    |

Tibbetts Lumber Co., LLC (Ocala, FL), Ocala, FL - 34472,

ID:Ts3RJ0261\_Xu2fYgSyBHAWzZSLZ-7Wi2A3Ank01S7611A6OVd9i?rJeH3zq8IPPQ5Wyuswe

### LOAD CASE(S) Standard

Uniform Loads (plf)

Vert: 1-2=-47, 2-5=-51, 5-7=-51, 7-8=-51, 2-12=-35, 12-18=-75, 18-19=-90, 11-19=-75, 9-11=-35

Horz: 1-2=-3. 2-5=1. 5-7=-1

20) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) 2nd Parallel): Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf) Vert: 1-2=-47, 2-5=-51, 5-7=-51, 7-8=-51, 2-12=-35, 12-18=-75, 18-19=-90, 11-19=-75, 9-11=-35

Horz: 1-2=-3, 2-5=1, 5-7=-1

21) Dead + 0.6 C-C Wind Min. Down: Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-2=8, 2-5=-25, 5-7=-25, 7-8=-25, 2-12=-12, 11-12=-52, 9-11=-12 Horz: 1-2=-16, 2-5=16, 5-7=-16

Drag: 7-8=0

22) Dead + 0.6 C-C Wind Min. Upward: Lumber Increase=1.60, Plate Increase=1.60

- Uniform Loads (plf)
  - Vert: 1-5=8, 5-7=8, 7-8=8, 2-12=-12, 11-12=-52, 9-11=-12

Horz: 1-5=-16, 5-7=16 Drag: 7-8=-0

- 23) 1st Dead + Roof Live (unbalanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)
- Vert: 1-5=-60, 5-7=-20, 7-8=-20, 2-12=-20, 11-12=-60, 9-11=-20
- 24) 2nd Dead + Roof Live (unbalanced): Lumber Increase=1.25, Plate Increase=1.25
- Uniform Loads (plf)

Vert: 1-5=-20, 5-7=-60, 7-8=-60, 2-12=-20, 11-12=-60, 9-11=-20

25) 3rd Dead + 0.75 Roof Live (unbalanced) + 0.75 Uninhab. Attic Storage: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-5=-50, 5-7=-20, 7-8=-20, 2-12=-35, 12-18=-75, 18-19=-90, 11-19=-75, 9-11=-35 26) 4th Dead + 0.75 Roof Live (unbalanced) + 0.75 Uninhab. Attic Storage: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-5=-20, 5-7=-50, 7-8=-50, 2-12=-35, 12-18=-75, 18-19=-90, 11-19=-75, 9-11=-35

🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. WARNING - verify design parameters and READ NOTES ON THIS AND INCLOSED INTERNETING TO THE REFERENCE FOR UNITY TO THE INSTANCE OF THE ADDRESS OF THE ADDRESS





|              |               |                                       |                     |              |            | 1-     | 0-0         |            |              |             |
|--------------|---------------|---------------------------------------|---------------------|--------------|------------|--------|-------------|------------|--------------|-------------|
|              | G (psf)       | SPACING- 2-0-0<br>Plate Grip DOI 1.25 | <b>CSI.</b>         | DEFL.        | in         | (loc)  | l/defl      | L/d        | PLATES       | <b>GRIP</b> |
| TCDL         | 10.0          | Lumber DOL 1.25                       | BC 0.01             | Vert(CT) -0. | .00        | 2      | >999        | 240        | 10120        | 244/190     |
| BCLL<br>BCDL | 0.0 *<br>10.0 | Code FBC2023/TPI2014                  | WB 0.00<br>Matrix-P | Wind(LL) 0   | .00<br>.00 | 3<br>2 | n/a<br>**** | n/a<br>240 | Weight: 7 lb | FT = 20%    |

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2

REACTIONS. (size) 3=Mechanical, 2=0-4-0, 4=Mechanical

Max Horz 2=48(LC 12)

Max Uplift 3=-101(LC 1), 2=-134(LC 12)

Max Grav 3=68(LC 12), 2=290(LC 1), 4=19(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

### NOTES-

1) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 zone; cantilever left and right exposed ;C-C for members and

forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

 Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 3=101, 2=134.



Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

July 25,2024



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Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not
a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall
building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing
is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the
fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TFI Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org)
and **BCSI Building Component Safety Information**available from the Structural Building Component Association (www.sbcscomponents.com)

BRACING-

TOP CHORD BOT CHORD 1-0-0

Structural wood sheathing directly applied or 1-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.



|  |   |  | 3-0-0   |   |
|--|---|--|---|---|
| LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0         *           BCDL         10.0 | SPACING- 2-0-0<br>Plate Grip DOL 1.25<br>Lumber DOL 1.25<br>Rep Stress Incr YES<br>Code FBC2023/TPI2014 | <b>CSI.</b><br>TC 0.33<br>BC 0.09<br>WB 0.00<br>Matrix-P | DEFL.         in         (loc)         l/defl         L/d           Vert(LL)         -0.00         2-4         >999         360           Vert(CT)         -0.01         2-4         >999         240           Horz(CT)         -0.00         3         n/a         n/a           Wind(LL)         0.00         2         ****         240 | PLATES         GRIP           MT20         244/190           Weight: 13 lb         FT = 20% |

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

REACTIONS. 3=Mechanical, 2=0-4-0, 4=Mechanical (size)

Max Horz 2=71(LC 12)

Max Uplift 3=-14(LC 9), 2=-86(LC 12)

Max Grav 3=35(LC 17), 2=292(LC 1), 4=55(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

### NOTES-

1) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 -2-0-0 to 1-0-0, Zone1 1-0-0 to 2-11-4 zone; cantilever left and

right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2.



Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

July 25,2024



👠 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only using the mathematical network of the intervention of the in



TOP CHORD BOT CHORD

200

Structural wood sheathing directly applied or 3-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.



|  |   |   | 5-0-0  |   |
|--|---|---|--|---|
| LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0         *           BCDL         10.0 | SPACING- 2-0-0<br>Plate Grip DOL 1.25<br>Lumber DOL 1.25<br>Rep Stress Incr YES<br>Code FBC2023/TPI2014 | CSI.<br>TC 0.32<br>BC 0.28<br>WB 0.00<br>Matrix-P | DEFL.         in         (loc)         l/defl         L/d           Vert(LL)         -0.03         2-4         >999         360           Vert(CT)         -0.06         2-4         >921         240           Horz(CT)         -0.00         3         n/a         n/a           Wind(LL)         0.00         2         *****         240 | PLATES         GRIP           MT20         244/190           Weight: 19 lb         FT = 20% |

BRACING-

TOP CHORD

BOT CHORD

### LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2

REACTIONS. (size) 3=Mechanical, 2=0-4-0, 4=Mechanical

Max Horz 2=95(LC 12)

Max Uplift 3=-35(LC 12), 2=-71(LC 12) Max Grav 3=114(LC 1), 2=350(LC 1), 4=95(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

### NOTES-

1) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 -2-0-0 to 1-0-0, Zone1 1-0-0 to 4-11-4 zone; cantilever left and

right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

 Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2.



Structural wood sheathing directly applied or 5-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

July 25,2024



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|  | 4-7-4<br>4-7-4  | 8-0-4 8-6-0<br>3-5-0 0-5-12                       | <u>14-8-12</u><br>6-2-12  | 21-4-0   |  |
|--|---|---|---|--|--|
| Plate Offsets (X,Y)  | [4:0-5-8,0-2-8], [5:0-2-8,0-3-4]  |   |   |  |  |
| LOADING (psf)<br>TCLL 20.0<br>TCDL 10.0<br>BCLL 0.0 *<br>BCDL 10.0 | SPACING- 2-0-0<br>Plate Grip DOL 1.25<br>Lumber DOL 1.25<br>Rep Stress Incr YES<br>Code FBC2023/TPI2014 | CSI.<br>TC 0.46<br>BC 0.39<br>WB 0.44<br>Matrix-S | DEFL.         in         (loc)           Vert(LL)         -0.04         6-8           Vert(CT)         -0.08         6-8           Horz(CT)         0.00         6           Wind(LL)         -0.01         6-8 | :) I/defl L/d<br>8 >999 360<br>8 >999 240<br>6 n/a n/a<br>8 >999 240 | PLATES         GRIP           MT20         244/190           Weight: 121 lb         FT = 20% |

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2

WEBS 2x4 SP No.2

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 6-0-0 oc bracing. Except: 10-0-0 oc bracing: 9-11

REACTIONS. (size) 2=0-3-8, 6=0-3-8, 12=0-3-8 Max Horz 2=-104(LC 10)

Max Uplift 2=-96(LC 12), 6=-106(LC 12)

Max Grav 2=362(LC 23), 6=593(LC 24), 12=1057(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 3-4=0/357, 4-5=0/265, 5-6=-602/99

BOT CHORD 6-8=-16/463

WEBS 3-12=-348/79, 8-11=-34/448, 5-11=-620/136, 4-12=-603/45

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 -2-0-0 to 1-0-0, Zone1 1-0-0 to 8-7-12, Zone2 8-7-12 to 12-10-11, Zone1 12-10-11 to 23-4-0 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

 Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb)

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb) 6=106.



Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

July 25,2024



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|  | 1-9-7         1-11-1         4-7-4         7-1           1-9-7         0-1-9         2-8-3         3- | 0-8 8-6-0<br>3-4 0-7-8                                   | 14-8-12<br>6-2-12   | <u>19-4-15</u><br>4-8-3  | <u>19⊤</u> 6-8 21-4-0<br>0-1-9 1-9-7   |
|--|---|--|---|--|--|
| Plate Offsets (X, Y)   | [2:0-4-0,0-2-1], [5:0-5-14,0-3-0], [8:0-4-0   | ,0-2-1], [20:0-1-9,0-1-0], ]                             | [ <u>22:0-1-11,0-1-0]</u> , [ <u>25:0-1-9</u> ,0  | 0-1-0]   | I  |
| LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0 | SPACING-2-0-0Plate Grip DOL1.25Lumber DOL1.25Rep Stress IncrYESCode FBC2023/TPI2014                   | <b>CSI.</b><br>TC 0.51<br>BC 0.41<br>WB 0.47<br>Matrix-S | DEFL.         in         (           Vert(LL)         -0.04         8           Vert(CT)         -0.10         8           Horz(CT)         -0.00         wind(LL)         0.05 | (loc) I/defl L/d<br>8-10 >999 360<br>8-10 >999 240<br>8 n/a n/a<br>8-10 >999 240 | PLATES         GRIP           MT20         244/190           Weight: 141 lb         FT = 20% |
| LUMBER-<br>TOP CHORD 2x4 SF<br>BOT CHORD 2x4 SF<br>WEBS 2x4 SF   | P No.2<br>P No.2<br>P No.2<br>P No.2  |  | BRACING-<br>TOP CHORD St<br>BOT CHORD Ri  | ructural wood sheathing dire<br>gid ceiling directly applied o                   | ectly applied or 6-0-0 oc purlins.<br>r 6-0-0 oc bracing.                                    |

REACTIONS. (size) 2=0-3-8, 8=0-3-8, 13=0-3-8 Max Horz 2=173(LC 11) Max Uplift 2=-115(LC 12), 8=-187(LC 12), 13=-254(LC 12) Max Grav 2=328(LC 23), 8=562(LC 24), 13=1141(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- TOP CHORD 2-4=-91/261, 4-5=-101/522, 5-6=-76/405, 6-8=-548/220
- BOT CHORD 12-13=-355/225, 8-10=-134/439

2x4 SP No.2

WEBS 10-12=-131/438, 6-12=-709/336, 4-13=-366/173, 5-13=-692/186

### NOTES-

OTHERS

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 -2-0-0 to 1-0-0, Zone1 1-0-0 to 8-7-12, Zone2 8-7-12 to 12-10-11, Zone1 12-10-11 to 23-4-0 zone; cantilever left and right exposed ; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

5) All plates are 2x4 MT20 unless otherwise indicated.

6) Gable studs spaced at 2-0-0 oc.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=115, 8=187, 13=254.



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|  |  |   | 8-2-0  | -   |
|--|--|---|--|---|
| LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0         *           BCDL         10.0 | SPACING-2-0-0Plate Grip DOL1.25Lumber DOL1.25Rep Stress IncrYESCodeFBC2023/TPI2014 | CSI.<br>TC 0.36<br>BC 0.40<br>WB 0.09<br>Matrix-P | DEFL.         in         (loc)         l/defl         L/d           Vert(LL)         -0.18         2-5         >527         360           Vert(CT)         -0.36         2-5         >263         240           Horz(CT)         0.00         5         n/a         n/a           Wind(LL)         0.00         2         ****         240 | PLATES         GRIP           MT20         244/190           Weight: 41 lb         FT = 20% |

BRACING-

TOP CHORD

BOT CHORD

### LUMBER-

TOP CHORD 2x4 SP No 2 2x4 SP M 31 or 2x4 SP SS BOT CHORD WEBS 2x4 SP No.2

REACTIONS. 5=0-4-0, 2=0-3-8 (size) Max Horz 2=132(LC 12) Max Uplift 5=-28(LC 12), 2=-60(LC 12) Max Grav 5=297(LC 1), 2=461(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-322/70

WEBS 3-5=-259/240

NOTES-

1) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 -2-0-0 to 1-0-0, Zone1 1-0-0 to 8-0-4 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 2.



Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

July 25,2024



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|  |  | <u>1-9-7</u><br>1-9-7                  | <u>1-11-1</u><br>0-1-9          |                             | 6-2-15<br>4-3-15                                      |                                      |                               | 6 <sub>7</sub> 4 <sub>7</sub> 9<br>0-1-9 | 8-2-0<br>1-9-7                  |  |                                    |
|--|--|--|---------------------------------|-----------------------------|---|--------------------------------------|-------------------------------|--|---------------------------------|--|------------------------------------|
| Plate Offsets (X,Y)  | [2:0-4-13,Edge], [6:0-4-13   | 3,Edge]                                |                                 |                             |   |                                      |                               |  |                                 |  |                                    |
| LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0         *           BCDL         10.0 | SPACING-<br>Plate Grip DOL<br>Lumber DOL<br>Rep Stress Incr<br>Code FBC2023/TI | 2-0-0<br>1.25<br>1.25<br>YES<br>PI2014 | CSI.<br>TC<br>BC<br>WB<br>Matri | 0.36<br>0.40<br>0.00<br>x-P | DEFL.<br>Vert(LL)<br>Vert(CT)<br>Horz(CT)<br>Wind(LL) | in<br>-0.18<br>-0.36<br>0.00<br>0.00 | (loc)<br>2-6<br>2-6<br>6<br>2 | l/defl<br>>527<br>>263<br>n/a            | L/d<br>360<br>240<br>n/a<br>240 | PLATES<br>MT20<br>Weight: 39 lb                  | <b>GRIP</b><br>244/190<br>FT = 20% |
| LUMBER-<br>TOP CHORD 2x4 S<br>BOT CHORD 2x4 S  | P No.2<br>P M 31 or 2x4 SP SS  |  |                                 |                             | BRACING-<br>TOP CHOR<br>BOT CHOR                      | D S<br>D F                           | Structur<br>Rigid ce          | al wood :<br>iling dire                  | sheathing dir<br>ctly applied o | rectly applied or 6-0-0<br>or 10-0-0 oc bracing. | oc purlins.                        |

REACTIONS. (size) 2=0-3-8, 6=0-3-8 Max Horz 2=-45(LC 10) Max Uplift 2=-74(LC 12), 6=-74(LC 12)

2x4 SP No.2

Max Uplift 2=-74(LC 12), 6=-74(LC 12) Max Grav 2=444(LC 1), 6=444(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-4=-259/134, 4-6=-259/135

NOTES-

OTHERS

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 -2-0-0 to 1-0-0, Zone1 1-0-0 to 4-1-0, Zone2 4-1-0 to 8-3-15, Zone1 8-3-15 to 10-2-0 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6.



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|  | F   |  | 7-0-0  |  |                               |                                 | -                               |                                    |
|--|---|--|--|--|-------------------------------|---------------------------------|---------------------------------|------------------------------------|
| LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0 | SPACING-2-0-0Plate Grip DOL1.25Lumber DOL1.25Rep Stress IncrYESCode FBC2023/TPI2014 | <b>CSI.</b><br>TC 0.71<br>BC 0.60<br>WB 0.00<br>Matrix-P | DEFL.<br>Vert(LL) -0<br>Vert(CT) -0<br>Horz(CT) -0<br>Wind(LL) 0 | in (loc)<br>0.13 2-4<br>0.25 2-4<br>0.00 3<br>0.00 2 | l/defl<br>>645<br>>322<br>n/a | L/d<br>360<br>240<br>n/a<br>240 | PLATES<br>MT20<br>Weight: 26 lb | <b>GRIP</b><br>244/190<br>FT = 20% |

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

REACTIONS. 3=Mechanical, 2=0-4-0, 4=Mechanical (size)

Max Horz 2=119(LC 12)

Max Uplift 3=-62(LC 12), 2=-63(LC 12) Max Grav 3=182(LC 1), 2=422(LC 1), 4=135(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

### NOTES-

1) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 -2-0-0 to 1-0-0, Zone1 1-0-0 to 6-11-4 zone; cantilever left and

right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2.



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BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins.



Rigid ceiling directly applied or 10-0-0 oc bracing.



|  |   |  | 7-0-0   |   |
|--|---|--|---|---|
| LOADING (psf)<br>TCLL 20.0<br>TCDL 10.0<br>BCLL 0.0 *<br>BCDL 10.0 | SPACING- 2-0-0<br>Plate Grip DOL 1.25<br>Lumber DOL 1.25<br>Rep Stress Incr YES<br>Code FBC2023/TPI2014 | <b>CSI.</b><br>TC 0.79<br>BC 0.59<br>WB 0.00<br>Matrix-P | DEFL.         in         (loc)         l/defl         L/d           Vert(LL)         -0.12         2-4         >663         360           Vert(CT)         -0.24         2-4         >331         240           Horz(CT)         0.00         n/a         n/a           Wind(LL)         0.00         2         *****         240 | PLATES         GRIP           MT20         244/190           Weight: 30 lb         FT = 20% |

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.2

BRACING-TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 4=0-3-8, 2=0-3-8 Max Horz 2=118(LC 12) Max Uplift 4=-21(LC 12), 2=-63(LC 12) Max Grav 4=248(LC 1), 2=418(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 3-4=-181/252

### NOTES-

1) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=2ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 zone; cantilever left and right exposed ;C-C for members and

forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

 Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 2.



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fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANS/ITPH Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org)
and BCSI Building Component Safety Information
available from the Structural Building Component Association (www.sbcscomponents.com)



|         | <b>2</b> (nof) | SDACING          | 2.0.0 | 001   |      | DEEL     |       | (10.0) | المما  | I /al |               | CDID     |
|---------|----------------|------------------|-------|-------|------|----------|-------|--------|--------|-------|---------------|----------|
| LUADING | J (psi)        | SPACING-         | 2-0-0 | ບອາ.  |      | DEFL.    | In    | (100)  | i/deli | L/a   | PLAIES        | GRIP     |
| TCLL    | 20.0           | Plate Grip DOL   | 1.25  | TC    | 0.71 | Vert(LL) | -0.13 | 2-4    | >625   | 360   | MT20          | 244/190  |
| TCDL    | 10.0           | Lumber DOL       | 1.25  | BC    | 0.60 | Vert(CT) | -0.26 | 2-4    | >313   | 240   |               |          |
| BCLL    | 0.0 *          | Rep Stress Incr  | YES   | WB    | 0.00 | Horz(CT) | -0.00 | 3      | n/a    | n/a   |               |          |
| BCDL    | 10.0           | Code FBC2023/TPI | 2014  | Matri | x-P  | Wind(LL) | 0.00  | 2      | ****   | 240   | Weight: 26 lb | FT = 20% |

TOP CHORD 2x4 SP No 2 BOT CHORD 2x4 SP No.2

REACTIONS. 3=Mechanical, 2=0-4-0, 4=Mechanical (size)

Max Horz 2=118(LC 12)

Max Uplift 3=-63(LC 12), 2=-63(LC 12) Max Grav 3=182(LC 1), 2=422(LC 1), 4=135(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

### NOTES-

1) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 -2-0-0 to 1-0-0, Zone1 1-0-0 to 6-11-4 zone; cantilever left and

right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

Refer to girder(s) for truss to truss connections.

6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2.



Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

July 25,2024



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TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.





|  | ł   |   | <u>6-1-3</u><br><u>6-1-3</u>                                    |  | 7-0-0<br>0-10-13  | 1                               |                                    |
|--|---|---|---|--|---|---------------------------------|------------------------------------|
| LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0         *           BCDL         10.0 | SPACING- 2-0-0<br>Plate Grip DOL 1.25<br>Lumber DOL 1.25<br>Rep Stress Incr YES<br>Code FBC2023/TPI2014 | CSI.<br>TC 0.76<br>BC 0.46<br>WB 0.03<br>Matrix-P | DEFL.<br>Vert(LL) -C<br>Vert(CT) -C<br>Horz(CT) C<br>Wind(LL) C | in (loc)<br>0.08 2-6<br>0.19 2-6<br>0.03 5<br>0.08 6 | l/defl L/d<br>>966 360<br>>434 240<br>n/a n/a<br>>999 240 | PLATES<br>MT20<br>Weight: 28 lb | <b>GRIP</b><br>244/190<br>FT = 20% |

BRACING-

TOP CHORD

BOT CHORD

### LUMBER-

TOP CHORD 2x4 SP No 2 2x4 SP No.2 BOT CHORD

WEBS 2x4 SP No.2

REACTIONS. (size) 4=Mechanical, 2=0-4-0, 5=Mechanical Max Horz 2=118(LC 12) Max Uplift 4=-27(LC 12), 2=-63(LC 12) Max Grav 4=242(LC 1), 2=422(LC 1), 5=17(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 -2-0-0 to 1-0-0, Zone1 1-0-0 to 6-11-4 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 2.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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|  | H   | <u>4-1-8</u><br>4-1-8   | 7-0-0<br>2-10-8   |   |
|--|---|---|---|---|
| LOADING (psf)<br>TCLL 20.0<br>TCDL 10.0<br>BCLL 0.0 *<br>BCDL 10.0 | SPACING- 2-0-0<br>Plate Grip DOL 1.25<br>Lumber DOL 1.25<br>Rep Stress Incr YES<br>Code FBC2023/TPI2014 | CSI.         D           TC         0.31         V           BC         0.25         V           WB         0.06         H           Matrix-P         V | FL.         in         (loc)         l/defl         L/d           rrt(LL)         -0.02         2-7         >999         360           rrt(CT)         -0.04         2-7         >999         240           rrz(CT)         0.01         5         n/a         n/a           nd(LL)         0.01         7         >999         240 | PLATES         GRIP           MT20         244/190           Weight: 31 lb         FT = 20% |

BRACING-

TOP CHORD

BOT CHORD

### LUMBER-

TOP CHORD 2x4 SP No 2 2x4 SP No.2 BOT CHORD

WEBS 2x4 SP No.2 REACTIONS. (size)

4=Mechanical, 2=0-4-0, 5=Mechanical Max Horz 2=118(LC 12) Max Uplift 4=-24(LC 12), 2=-63(LC 12) Max Grav 4=65(LC 1), 2=422(LC 1), 5=185(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-480/106

BOT CHORD 2-7=-210/380, 6-7=-198/341

WEBS 3-6=-381/221

NOTES-

1) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 -2-0-0 to 1-0-0, Zone1 1-0-0 to 6-11-4 zone; cantilever left and

right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

\* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 4) will fit between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 2.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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|  |   | 2-1-8<br>2-1-8                                    | 7-0-0<br>4-10-8  | 1   |
|--|---|---|--|---|
| LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0         *           BCDL         10.0 | SPACING-2-0-0Plate Grip DOL1.25Lumber DOL1.25Rep Stress IncrYESCode FBC2023/TPI2014 | CSI.<br>TC 0.82<br>BC 0.29<br>WB 0.03<br>Matrix-P | DEFL.         in         (loc)         l/defl         L/d           Vert(LL)         -0.16         6         >517         360           Vert(CT)         -0.31         6         >264         240           Horz(CT)         0.08         5         n/a         n/a           Wind(LL)         0.19         6         >437         240 | PLATES         GRIP           MT20         244/190           Weight: 26 lb         FT = 20% |

TOP CHORD 2x4 SP No 2 2x4 SP No.2 BOT CHORD WEBS 2x4 SP No.2

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 4=Mechanical, 2=0-4-0, 5=Mechanical Max Horz 2=118(LC 12) Max Uplift 4=-51(LC 12), 2=-63(LC 12) Max Grav 4=202(LC 1), 2=422(LC 1), 5=96(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 -2-0-0 to 1-0-0, Zone1 1-0-0 to 6-11-4 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

- 6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 2.



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|  |   | <u>4-5-10</u><br>4-5-10   | <u>9-10-1</u><br>5-4-7  |   |
|--|---|---|---|---|
| LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0 *           BCDL         10.0 | SPACING-2-0-0Plate Grip DOL1.25Lumber DOL1.25Rep Stress IncrNOCodeFBC2023/TPI2014 | CSI.         DEFL.           TC         0.83         Vert(LL)           BC         0.66         Vert(CT)           WB         0.34         Horz(CT)           Matrix-S         Wind(LL) | in (loc) I/defl L/d<br>-0.06 6-7 >999 360<br>-0.14 6-7 >843 240<br>0.01 5 n/a n/a<br>-0.03 2-7 >999 240 | PLATES         GRIP           MT20         244/190           Weight: 44 lb         FT = 20% |

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.2

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 5-3-4 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 4=Mechanical, 2=0-5-5, 5=Mechanical Max Horz 2=119(LC 27) Max Uplift 4=-50(LC 8), 2=-171(LC 8) Max Grav 4=164(LC 1), 2=583(LC 31), 5=271(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-785/22

BOT CHORD 2-7=-54/667, 6-7=-54/667

WEBS 3-7=0/287, 3-6=-702/57

NOTES-

1) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional); cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60

 Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=171.

7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 88 lb down and 186 lb up at 1-4-15, 88 lb down and 186 lb up at 1-4-15, 54 lb down and 23 lb up at 4-2-15, 54 lb down and 23 lb up at 4-2-15, and 82 lb down and 56 lb up at 7-0-14, and 82 lb down and 56 lb up at 7-0-14 on top chord, and at 1-4-15, at 1-4-15, 11 lb down at 4-2-15, 11 lb down at 4-2-15, and 39 lb down at 7-0-14, and 39 lb down at 7-0-14 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

### LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf) Vert: 1-4=-60, 2-5=-20

Concentrated Loads (lb)

Vert: 8=124(F=62, B=62) 9=-58(F=-29, B=-29) 11=-39(F=-19, B=-19)



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building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing
is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the
fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TFI Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org)
and **BCSI Building Component Safety Information**available from the Structural Building Component Association (www.sbcscomponents.com)



| L   |   |   | 9-5-0  |                                 |                      |                             |                          |                         |                                    |
|---|---|---|--|---------------------------------|----------------------|-----------------------------|--------------------------|-------------------------|------------------------------------|
|   |   |   | 9-5-0  |                                 |                      |                             |                          |                         |                                    |
| Plate Offsets (X,Y)   | [3:0-2-0,0-2-8], [5:0-2-0,0-2-8]  |   |  |                                 |                      |                             |                          |                         |                                    |
| LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           *         BCDI | SPACING- 2-0-0<br>Plate Grip DOL 1.25<br>Lumber DOL 1.25<br>Rep Stress Incr YES<br>Code FBC2023/TPI2014 | CSI.<br>TC 0.<br>BC 0.<br>WB 0.<br>Matrix-S | 0.12 <b>DEFL.</b><br>0.14 Vert(LL<br>0.02 Horz(C | in<br>0.00<br>) 0.00<br>T) 0.00 | (loc)<br>7<br>7<br>6 | l/defl<br>n/r<br>n/r<br>n/a | L/d<br>120<br>120<br>n/a | PLATES<br>MT20          | <b>GRIP</b><br>244/190<br>FT = 20% |
|   |   |   |  |                                 |                      |                             |                          | Wolght. 20 lb           | 11-2070                            |
| LUMBER-   |   |   | BRACIN   | G-                              | <b>.</b> .           |                             |                          |                         |                                    |
| TOP CHORD 2x4 SI  | - No.2  |   | TOP CH   | URD                             | Structur             | al wood s                   | sheathing dire           | ctly applied or 6-0-0 o | oc purlins.                        |

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.2

REACTIONS. (size) 2=7-6-6, 6=7-6-6, 8=7-6-6

Max Horz 2=-14(LC 10) Max Uplift 2=-29(LC 12), 6=-29(LC 12), 8=-8(LC 9)

Max Grav 2=184(LC 1), 6=188(LC 1), 8=297(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 0-4-11 to 2-0-0, Zone2 2-0-0 to 6-2-15, Zone1 6-2-15 to 7-5-0, Zone3 7-5-0 to 9-0-5 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) Provide adequate drainage to prevent water ponding.
- 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
  - 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6, 8.
  - 9) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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|   |   |                                       | 9-5-0   |
|---|---|---------------------------------------|---|
| Plate Offsets (X,Y)                                   | [3:0-2-0,0-2-8], [5:0-2-0,0-2-8]  |                                       |   |
| LOADING (psf)<br>TCLL 20.0<br>TCDL 10.0<br>BCLL 0.0 * | SPACING- 2-0-0<br>Plate Grip DOL 1.25<br>Lumber DOL 1.25<br>Rep Stress Incr YES | CSI.<br>TC 0.08<br>BC 0.14<br>WB 0.02 | DEFL.         in         (loc)         l/defl         L/d         PLATES         GRIP           Vert(LL)         0.00         7         n/r         120         MT20         244/190           Vert(CT)         0.00         7         n/r         120         MT20         244/190           Horz(CT)         0.00         6         n/a         n/a         1         1 |
| BCDL 10.0   | Code FBC2023/TPI2014  | Matrix-S                              | Weight: 27 lb FT = 20%  |
| LUMBER-<br>TOP CHORD 2x4                              | SP No.2   |                                       | BRACING-<br>TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.   |

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

9-5-0

BOT CHORD 2x4 SP No.2 OTHERS 2x4 SP No.2

REACTIONS. (size) 2=7-6-6, 6=7-6-6, 8=7-6-6

Max Horz 2=-20(LC 10) Max Uplift 2=-35(LC 12), 6=-35(LC 12)

Max Grav 2=209(LC 1), 6=209(LC 1), 8=252(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

### NOTES-

- 2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) Provide adequate drainage to prevent water ponding.

5) Gable requires continuous bottom chord bearing.

- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6.
- 9) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

July 25,2024



🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTeKe connectors. This design is based only ucon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcscomponents.com)

<sup>1)</sup> Unbalanced roof live loads have been considered for this design.



|               |                       |          | 9-5-0         |              |     |                        |
|---------------|-----------------------|----------|---------------|--------------|-----|------------------------|
|               |                       | 1        | 9-5-0         |              |     | 1                      |
| LOADING (psf) | <b>SPACING-</b> 2-0-0 | CSI.     | DEFL. in      | (loc) l/defl | L/d | PLATES GRIP            |
| TCLL 20.0     | Plate Grip DOL 1.25   | TC 0.25  | Vert(LL) 0.01 | 5 n/r        | 120 | MT20 244/190           |
| TCDL 10.0     | Lumber DOL 1.25       | BC 0.14  | Vert(CT) 0.01 | 5 n/r        | 120 |                        |
| BCLL 0.0 *    | Rep Stress Incr YES   | WB 0.02  | Horz(CT) 0.00 | 4 n/a        | n/a |                        |
| BCDL 10.0     | Code FBC2023/TPI2014  | Matrix-P |               |              |     | Weight: 29 lb FT = 20% |
| LUMBER-       |                       | •        | BRACING-      |              |     |                        |

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No 2 BOT CHORD 2x4 SP No.2 2x4 SP No.2

OTHERS

REACTIONS. 2=7-6-6, 4=7-6-6, 6=7-6-6 (size) Max Horz 2=-36(LC 10) Max Uplift 2=-38(LC 12), 4=-38(LC 12) Max Grav 2=188(LC 1), 4=188(LC 1), 6=294(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat.

- II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 0-4-11 to 3-4-11, Zone1 3-4-11 to 4-8-8, Zone3 4-8-8 to 9-0-5 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.
- 8) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

July 25,2024



🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTeKe connectors. This design is based only ucon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcscomponents.com)



|            | 1         |                                  |        |       |      | 9-5-0    |      |       |        |     |               | 1        |
|------------|-----------|----------------------------------|--------|-------|------|----------|------|-------|--------|-----|---------------|----------|
|            | 1         |                                  |        |       |      | 9-5-0    |      |       |        |     |               |          |
| Plate Offs | ets (X,Y) | [3:0-2-0,0-2-8], [5:0-2-0,0-2-8] | 2-8]   |       |      |          |      |       |        |     |               |          |
| LOADING    | (psf)     | SPACING-                         | 2-0-0  | CSI.  |      | DEFL.    | in   | (loc) | l/defl | L/d | PLATES        | GRIP     |
| TCLL       | 20.0      | Plate Grip DOL                   | 1.25   | тс    | 0.13 | Vert(LL) | 0.00 | 7     | n/r    | 120 | MT20          | 244/190  |
| TCDL       | 10.0      | Lumber DOL                       | 1.25   | BC    | 0.14 | Vert(CT) | 0.01 | 7     | n/r    | 120 |               |          |
| BCLL       | 0.0 *     | Rep Stress Incr                  | YES    | WB    | 0.02 | Horz(CT) | 0.00 | 6     | n/a    | n/a |               |          |
| BCDL       | 10.0      | Code FBC2023/TF                  | 912014 | Matri | k-S  |          |      |       |        |     | Weight: 28 lb | FT = 20% |

BRACING-

TOP CHORD

BOT CHORD

### LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2OTHERS2x4 SP No.2

**REACTIONS.** (size) 2=7-6-6, 6=7-6-6, 8=7-6-6

Max Horz 2=-30(LC 10) Max Uplift 2=-38(LC 12), 6=-38(LC 12)

Max Grav 2=203(LC 1), 6=203(LC 1), 8=264(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 0-4-11 to 3-4-11, Zone1 3-4-11 to 4-0-0, Zone3 4-0-0 to 9-0-5 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

 Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

- 4) Provide adequate drainage to prevent water ponding.
- 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
  - 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6.
  - See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

# HORSE STATE OF

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

July 25,2024



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.
Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not
a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall
building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing
is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the
fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANS/ITPH1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org)
and BCSI Building Component Safety Information
available from the Structural Building Component Association (www.sbcscomponents.com)



General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

- 1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor1 bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- 5. Cut members to bear tightly against each other
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- 11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- 12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- 14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- 15. Connections not shown are the responsibility of others.
- Do not cut or alter truss member or plate without prior approval of an engineer.
- 17. Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- 20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.
- 21. The design does not take into account any dynamic or other loads other than those expressly stated.

![](_page_50_Figure_0.jpeg)

![](_page_50_Figure_1.jpeg)

LUMBER CO.,LLC

6100 SE 68th Street, Ocala, FL 34472

Phone (352) 347-7661 Fax: (347) 347-7797

\*\* Signature of this document acknowledges that the client has reviewed this truss placement diagram in its entirety as in agreement with the following terms, including, but not limited to

a) The client is responsible to verify the accuracy of information submitted for use in design, fabrication and scheduling. Any labor, material or time delay incurred from inadequate or incorrect information supplied from the client, will be at the client's expense. Any field measurements, by an associate of Tibbetts Lumber Co., LLC, are performed as a courtesy to the client and shall be verified by the client.

b) Design Criteria: The client acknowledges that the truss design criteria noted on this truss placement diagram meets or exceeds the design criteria specified by the building designer, engineer of record, and local and state building requirements

c) Fabrication and Delivery: One approved truss placement diagram must be returned to the truss manufacturer before fabrication and delivery will be scheduled. It is the client's responsibility to co-ordinate deliver dates with the truss manufacturer. The client shall provide a marked location for delivery, which must be accessible, level and clear of materials and debris. In lieu of this, truss will be delivered in the best available location at our deliveried dismetic discussion. Core and headling of the thruse following is the averearishing of the adjuster.

driver's discretion. Care and handling of the trusses following delivery is the responsibility of the client. d) Installation & Bracing: BCSI 2008 (Building Component Safety Information) WTCA/TPI guidelines shall be followed when handling, installing & bracing trusses. Temporary and/or permanent bracing and blocking is not included in this truss package. Trusses shall be braced to prevent rotation and provide lateral stability in accordance with the requirements specified in the construction documents for the building and on the individual truss design

drawings. The overall stability of the truss system is the responsibility of the building designer.

| ** | Approved | By: |  |
|----|----------|-----|--|
|    |          |     |  |

Please Print

Name

|                 |             |                  |  |                  |                       |                     |                |                              |             |                 |             |                 |                              | Reaction Summary        |             |                      |               |  |
|-----------------|-------------|------------------|--|------------------|-----------------------|---------------------|----------------|------------------------------|-------------|-----------------|-------------|-----------------|------------------------------|-------------------------|-------------|----------------------|---------------|--|
|                 |             |                  |  |                  | Tibbetts Lumber Ocala |                     |                |                              |             |                 |             |                 | Job Number: <b>6243113-R</b> |                         |             |                      |               |  |
|                 |             |                  |  |                  |                       | 61                  | 00 SE 6        | S8th S                       | St          |                 |             | Quoted On:      |                              |                         |             |                      |               |  |
| Ocala, FL 34472 |             |                  |  |                  |                       |                     |                | Ordered On: <b>11/4/2024</b> |             |                 |             |                 |                              |                         |             |                      |               |  |
|                 |             |                  |  |                  | nce<br>949            |                     | Phon           | ie: 352-3                    | 347-7       | 661             |             |                 | Sch                          | neduled Delive          | erv On:     |                      |               |  |
|                 | LC<br>www.  | JIVIDI           | ER U<br>Slumber.                                 | 0.<br>сом        |                       |                     | www.t          | Ibbettslu                    | umbe        | r.com           |             |                 |                              | Pi                      | oduct: Roof |                      |               |  |
| Customer Inf    | formation   |                  |  |                  |                       |                     |                |                              |             | lob Informati   | on          |                 |                              |                         |             |                      |               |  |
| Adam            | IS HO       | omes             | of NV  | VFL -            | Gaine                 | sville              |                |                              |             | The P           | res         | erve at L       | aurel L                      | ake 096                 | ion         |                      |               |  |
|                 |             |                  |  |                  |                       |                     |                |                              |             |                 |             |                 |                              | 096 The F               | Preserve at | Laurel Lak           | e             |  |
| Phone           | -           |                  |  |                  |                       |                     |                |                              |             | 715 SV          | VRo<br>itv  | semary Dr       | C                            | hris Adam               |             | Istomer P.O. No.     |               |  |
|                 |             |                  |  |                  |                       |                     |                |                              |             | Lake U          | ity i       | L 32024         | Esi                          | timator<br>Steven Rober | ts S        | signer<br>Steven Rot | perts         |  |
|                 |             |                  |  | i                | 1                     |                     |                |                              |             |                 |             |                 | 1                            | 1                       |             | 1 140                |               |  |
| TCLL            | LOa<br>TCDL | BCLL             | BCDL   | Bui              | Iding Co              | de                  |                | Wind                         | l Desi      | gn Metl         | nod         |                 |                              | Velocity                | Exp Cat     | TCDL                 | d Max<br>BCDL |  |
| 20              | 10          | 0                | 10   | FBC2             | 023/TPI               | 2014                | MWFRS ([       | Directiona                   | al)/C-0     | C hybrid        | Wir         | d ASCE 7-2      | 22                           | 130 mph                 |             | 4.2                  | 6             |  |
| Roof T          | russe       | s                |  | 1                |                       | <b>I</b>            | <u> </u>       |                              | ,           |                 |             |                 |                              | <u> </u>                | <u> </u>    | 1                    | •             |  |
|                 |             |                  |  |                  | Qty                   | Span                | TC Pitch       | TC                           |             |                 |             |                 |                              |                         |             |                      |               |  |
| Label           |             | Pr               | ofile  |                  | Ply                   | Height              | BC Pitch       | BC                           |             |                 |             |                 | Re                           | actions                 |             |                      |               |  |
| A01             |             |                  |  |                  | 2_nlv                 | 43-11-00            | 6/12           | 2 X 6                        | Joint<br>36 | 11 Joi<br>31 3  | nt 2<br>443 |                 |                              |                         |             |                      |               |  |
|                 | -           |                  |  |                  | 2-piy                 | 43-11-00            | 6/12           | 2 x 4                        | -2<br>Joint | 61 -<br>10 Joi  | 229<br>nt 2 |                 |                              |                         |             |                      |               |  |
| A02             |             |                  |  |                  | 1-ply                 | 5-09-15             |                | 2 x 4                        | 17          | 41 1            | 877         |                 |                              |                         |             |                      |               |  |
| A 0.2           |             |                  | <u> </u>   | <u> </u>         | 1                     | 43-11-00            | 6 /12          | 2 x 4                        | Joint       | 10 Joi          | nt 2        |                 |                              |                         |             |                      |               |  |
| A03             |             |                  |  |                  | 1-ply                 | 6-05-03             |                | 2 x 4                        | 17          | 41 1<br>82 -    | 877<br>132  |                 |                              |                         |             |                      |               |  |
| A04             |             |                  |  | 17               | 1                     | 43-11-00            | 6 /12          | 2 x 4                        | Joint       | 12 Joi          | nt 2        |                 |                              |                         |             |                      |               |  |
|                 |             | 21               |  |                  | 1-ply                 | 7-05-03             |                | 2 x 4                        | -           | 70 -            | 128         |                 |                              |                         |             |                      |               |  |
| A05             |             | $\sqrt{\Sigma}$  |  | 7                | 1<br>1 ph/            | 43-11-00            | 6 /12          | $2 \times 4$                 | Joint<br>17 | 11 Joi<br>53 1  | nt 2<br>891 |                 |                              |                         |             |                      |               |  |
|                 |             | <u> </u>         |  |                  | 1-piy                 | 43-11-00            | 6 /12          | $2 \times 4$<br>$2 \times 4$ | Joint       | - 70<br>11 Joi  | 128<br>nt 2 |                 |                              |                         |             |                      |               |  |
| A06             |             | لکچ              | ЛĂ   | $\square$        | 1-plv                 | 9-05-03             | 0712           | 2 x 4                        | 19          | 44 2            | 117         |                 |                              |                         |             |                      |               |  |
| 4.07            | -           |                  |  |                  | 1                     | 43-11-00            | 6 /12          | 2 x 4                        | Joint       | 13 Joi          | nt 2        |                 |                              |                         |             |                      |               |  |
|                 |             | لالك             | <u>v</u>   | $\bigtriangleup$ | 1-ply                 | 10-05-03            |                | 2 x 4                        | 19          | 84 2<br>68 -    | 139<br>116  |                 |                              |                         |             |                      |               |  |
| A08             |             | $\overline{}$    | $\overline{N/}$                                  |                  | 1                     | 43-11-00            | 6 /12          | 2 x 4                        | Joint       | 12 Joi<br>04    | nt 2<br>357 | Joint 23        |                              |                         |             |                      |               |  |
|                 | _           |                  |  |                  | 1-ply                 | 11-05-03            | 0.440          | 2 x 4                        |             | 58 -            | 110         | -137            |                              |                         |             |                      |               |  |
| A09             |             | $\checkmark$     | M  |                  |                       | 43-11-00            | 6/12           | $2 \times 4$                 | Joint<br>17 | 12 Joi<br>04    | nt 2<br>357 | 2044            |                              |                         |             |                      |               |  |
|                 | ~           | ,                |  |                  | 1-piy                 | 45-10-00            | 6/12           | 2 x 4                        | -<br>Joint  | 58<br>12 Joi    | -56<br>nt 2 | -88<br>Joint 23 |                              |                         |             |                      |               |  |
| A10             |             |                  | $\mathbb{Z}$                                     |                  | 1-ply                 | 11-05-03            | 0712           | 2 x 4                        | 18          | 80              | 353         | 2080            |                              |                         |             |                      |               |  |
|                 |             |                  |  | <u> </u>         | 4                     | 45-10-00            | 6 /12          | 2 x 4                        | Joint       | 32 -<br>11 Joi  | nt 2        | Joint 21        |                              |                         |             |                      |               |  |
|                 |             | لالك             |  |                  | 1-ply                 | 11-05-03            |                | 2 x 4                        | 17<br>-1    | 16 -<br>26 -    | 327<br>155  | 2281<br>-150    |                              |                         |             |                      |               |  |
| A11A            |             |                  | $\overline{\Lambda}$                             |                  | 1                     | 45-10-00            | 6 /12          | 2 x 4                        | Joint       | 11 Joi<br>89    | nt 2<br>326 | Joint 21        |                              |                         |             |                      |               |  |
|                 |             |                  |  |                  | 1-ply                 | 11-05-03            | 0./10          | 2 x 4                        | -1          | 28 -            | 146         | -135            |                              |                         |             |                      |               |  |
| A12             |             | $\checkmark$     | $\mathbb{N}$                                     |                  |                       | 45-02-00            | 6/12           | $2 \times 4$                 | Joint<br>15 | 11 Joi<br>72 :  | nt 2<br>350 | 2283            |                              |                         |             |                      |               |  |
|                 |             |                  |  |                  | 1-piy                 | 45-02-00            | 6/12           | 2 x 4                        | -<br>Joint  | 62<br>12 Joi    | -76<br>nt 2 | -79<br>Joint 22 |                              |                         |             |                      |               |  |
| A13             |             | $\Delta$         | VZ   | $\frown$         | 1-ply                 | 10-09-15            |                | 2 x 4                        | 15          | 85              | 336         | 2242            |                              |                         |             |                      |               |  |
| A14             | -           |                  |  |                  | 1                     | 45-02-00            | 6 /12          | 2 x 4                        | Joint       | 10 Join         | 144         | Joint 2         |                              |                         |             |                      |               |  |
|                 |             |                  |  |                  | 1-ply                 | 9-09-15             |                | 2 x 4                        | 15          | 97 2<br>61      | 258<br>-86  | 360<br>-70      |                              |                         |             |                      |               |  |
| A15             |             |                  | 1 45-02-00 6 /12 2 x 4 Joint 11 Joint 19 Joint 2 |                  |                       |                     | Joint 2<br>272 |                              |             |                 |             |                 |                              |                         |             |                      |               |  |
|                 |             |                  |  |                  | 1-ply                 | 8-09-15             | 6 /10          | 2 x 4                        | -           | 61              | -87         | -70             |                              |                         |             |                      |               |  |
| A16             |             | $\langle \nabla$ | $\sim$   |                  | 1-nlv                 | 49-10-00<br>7_09_15 | -3/12          | $2 \times 4$<br>$2 \times 4$ | Joint<br>1  | 97 2            | 765         | 1616            |                              |                         |             |                      |               |  |
| <u> </u>        |             |                  |  |                  | 1 · P'y               | 49-10-00            | 6/12           | 2 x 4                        | -<br>Joint  | 41 -<br>14 Join | 133<br>t 17 | -105<br>Joint 2 |                              |                         |             |                      |               |  |
| A17             |             |                  | $\Delta \Delta$                                  |                  | 1-ply                 | 6-09-15             | -3 /12         | 2 x 4                        | 1           | 72 2<br>45      | 461<br>124  | 1459<br>-109    |                              |                         |             |                      |               |  |
| ٨10             |             |                  |  |                  | 1                     | 49-10-00            | 6 /12          | 2 x 4                        | Joint       | 14 Join         | t 17        | Joint 2         |                              |                         |             |                      |               |  |
|                 |             |                  |  |                  | 1-ply                 | 5-09-15             | -3 /12         | 2 x 4                        | 2           | 38 2<br>43 -    | 370<br>114  | 1484<br>-114    |                              |                         |             |                      |               |  |

![](_page_51_Picture_1.jpeg)

6243113-R

# The Preserve at Laurel Lake 096

| Page: | 2 | of | 2 |
|-------|---|----|---|
|-------|---|----|---|

| Roof T     | Roof Trusses |       |          |          |              |                |                |                |                    |
|------------|--------------|-------|----------|----------|--------------|----------------|----------------|----------------|--------------------|
|            |              | Qty   | Span     | TC Pitch | тс           |                |                |                |                    |
| Label      | Profile      | Ply   | Height   | BC Pitch | BC           |                |                |                | Reactions          |
| A10        |              | 1     | 49-10-00 | 6 /12    | 2 x 6        | Joint 12       | Joint 15       | Joint 2        |                    |
|            |              | 2-ply | 4-09-15  | -3 /12   | 2 x 6        | 901<br>-34     | 4624<br>-318   | 2758<br>-181   |                    |
| B01        |              | 9     | 23-10-00 | 6 /12    | 2 x 4        | Joint 2        | Joint 8        |                |                    |
| 501        |              | 1-ply | 7-03-07  |          | 2 x 4        | 1298<br>39     | 1298<br>39     |                |                    |
| B01Y       |              | 1     | 23-10-00 | 6 /12    | 2 x 4        |                |                |                |                    |
|            |              | 1-ply | 6-11-08  |          | 2 x 4        |                |                |                | Continuous Support |
| B02        |              | 1     | 23-09-08 | 6 /12    | 2 x 4        | Joint 14       | Joint 2        |                |                    |
| 002        |              | 1-ply | 7-03-07  |          | 2 x 4        | 1132<br>102    | 1304<br>38     |                |                    |
| C1         |              | 4     | 1-00-00  | 6 /12    | 2 x 4        | Joint 2        | Joint 3        | Joint 4        |                    |
|            |              | 1-ply | 1-09-15  |          | 2 x 4        | -134           | -101           | 19<br>6        |                    |
| <b>C</b> 3 |              | 4     | 3-00-00  | 6 /12    | 2 x 4        | Joint 2        | Joint 3        | Joint 4        |                    |
|            |              | 1-ply | 2-09-15  |          | 2 x 4        | -86            | -14            | 55<br>17       |                    |
| C5         |              | 4     | 5-00-00  | 6 /12    | 2 x 4        | Joint 2        | Joint 3        | Joint 4        |                    |
|            |              | 1-ply | 3-09-15  |          | 2 x 4        | -71            | -35            | 95<br>29       |                    |
| F01        |              | 2     | 21-04-00 | 6 /12    | 2 x 4        | Joint 12       | Joint 2        | Joint 6        |                    |
|            |              | 1-ply | 7-07-15  |          | 2 x 4        | 1057           | -96            | -106           |                    |
| F01X       |              | 1     | 21-04-00 | 6 /12    | 2 x 4        | Joint 13       | Joint 2        | Joint 8        |                    |
|            |              | 1-ply | 7-04-00  |          | 2 x 4        | -254           | -115           | -187           |                    |
| F02        |              | 3     | 8-02-00  | 6 /12    | 2 x 4        | Joint 2        | Joint 5        |                |                    |
|            |              | 1-ply | 5-04-15  |          | 2 x 4        | 461<br>-60     | -28            |                |                    |
| F02X       |              | 1     | 8-02-00  | 6 /12    | 2 x 4        | Joint 2        | Joint 6        |                |                    |
|            |              | 1-ply | 3-00-08  |          | 2 x 4        | 444<br>-74     | -74            |                |                    |
| F7         |              | 35    | 7-00-00  | 6 /12    | 2 x 4        | Joint 2        | Joint 3        | Joint 4        |                    |
|            |              | 1-ply | 4-09-15  |          | 2 x 4        | -63            | -62            | 41             |                    |
| E7B        |              | 1     | 7-00-00  | 6 /12    | 2 x 4        | Joint 2        | Joint 4        |                |                    |
|            |              | 1-ply | 4-09-15  |          | 2 x 4        | -63            | -21            |                |                    |
| E7V        |              | 3     | 7-00-00  | 6 /12    | 2 x 4        | Joint 2        | Joint 3        | Joint 4        |                    |
|            |              | 1-ply | 4-09-15  | 3 /12    | 2 x 4        | -63            | -63            | 41             |                    |
| E7VA       |              | 1     | 7-00-00  | 6 /12    | 2 x 4        | Joint 2<br>422 | Joint 4<br>242 | Joint 5        |                    |
|            |              | 1-ply | 4-09-15  | 3 /12    | 2 x 4        | -63            | -27            | 5              |                    |
| E7VB       |              | 1     | 7-00-00  | 6 /12    | 2 x 4        | Joint 2<br>422 | Joint 4<br>65  | Joint 5<br>185 |                    |
|            |              | 1-ply | 4-09-15  | 3/12     | 2 x 4        | -63            | -24            | 2              |                    |
| E7VC       |              | 1     | 7-00-00  | 6/12     | 2 x 4        | Joint 2<br>422 | Joint 4<br>202 | Joint 5<br>96  |                    |
|            |              | 1-ply | 4-09-15  | 3/12     | 2 x 4        | -63            | -51            | 29             |                    |
| H7         |              | 2     | 9-10-01  | 4.24 /12 | 2 x 4        | Joint 2<br>583 | Joint 4<br>164 | Joint 5<br>271 |                    |
|            |              | 1-ply | 4-09-07  | 0./10    | 2 x 4        | -171           | -50            | 18             |                    |
| PB1        |              | 1     | 9-05-00  | 6/12     | 2 x 4        | Joint 2<br>184 | Joint 6<br>188 | Joint 8<br>297 |                    |
|            |              | 1-ріу | 11-12    | 0./10    | 2 x 4        | -29            | -29            | -8             |                    |
| PB2        |              | 1     | 9-05-00  | 6/12     | 2 X 4        | Joint 2<br>209 | Joint 6<br>209 | Joint 8<br>252 |                    |
|            |              | т-ріу | 0.05.00  | 6/10     | $2 \times 4$ | -35            | -35            | 7              |                    |
| PB3        |              | 4     | 9-00-00  | 0/12     | $2 \times 4$ | Joint 2<br>188 | Joint 4<br>188 | Joint 6<br>294 |                    |
|            |              | т-ріу | 2-04-00  | 6 /40    | 2 X 4        | -38            | -38            | 18             |                    |
| PB4        |              | 1     | 9-05-00  | 6/12     | 2 X 4        | Joint 2<br>203 | Joint 6<br>203 | Joint 8<br>264 |                    |
|            |              | 1-ply | 1-11-12  |          | 2 x 4        | -38            | -38            | 19             |                    |