DATE 12/14/2006

Columbia County Building Permit

This Permit Expires One Year From the Date of Issue

PERMIT 000025311

APPLICANT CHARESE NORTON PHONE 386.752.3331

ADDRESS 3367 S US HWY 441, STE 101 LAKE CITY FL 32025

OWNER NORTON HOME IMP. CO., INC. PHONE 386.752.3331

ADDRESS 237 SW BRIARBROOK PLACE LAKE CITY FL 32024

CONTRACTOR JAMES H. NORTON PHONE 386.752.3331

LOCATION OF PROPERTY 47-S TO C-242,TR TO MOCKINGBIRD,TL TO BRIARBROOK,TR THE SITE IS THE 5TH LOT ON R.

TYPE DEVELOPMENT SFD/UTILITY ESTIMATED COST OF CONSTRUCTION 95750.00

HEATED FLOOR AREA 1915.00 TOTAL AREA 2929.00 HEIGHT 19.00 STORIES 1

FOUNDATION CONC WALLS FRAMED ROOF PITCH 6'12 FLOOR CONC

LAND USE & ZONING RSF-2 MAX. HEIGHT 35

Minimum Set Back Requirments: STREET-FRONT 25.00 REAR 15.00 SIDE 10.00

NO. EX.D.U. 0 FLOOD ZONE X DEVELOPMENT PERMIT NO.

PARCEL ID 25-4S-16-03121-055 SUBDIVISION PICCADILLY PARK SOUTH

LOT 13 BLOCK D PHASE UNIT TOTAL ACRES 0.50

000001278		RB0031780	Charese J. North	
Culvert Permit No.	Culvert Waiver	Contractor's License Number	Applicant/Owner/Contractor	
18"X32'MITERED	06-01062N	BLK	JTH	N
Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance	New Resident

COMMENTS: NOC ON FILE. 1 FOOT ABOVE ROAD.

Check # or Cash 22634

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power _____ date/app. by

Foundation _____ date/app. by

Monolithic _____ date/app. by

Under slab rough-in plumbing _____ date/app. by

Slab _____ date/app. by

Sheathing/Nailing _____ date/app. by

Framing _____ date/app. by

Rough-in plumbing above slab and below wood floor _____ date/app. by

Electrical rough-in _____ date/app. by

Heat & Air Duct _____ date/app. by

Peri. beam (Lintel) _____ date/app. by

Permanent power _____ date/app. by

C.O. Final _____ date/app. by

Culvert _____ date/app. by

M/H tie downs, blocking, electricity and plumbing _____ date/app. by

Pool _____ date/app. by

Reconnection _____ date/app. by

Pump pole _____ date/app. by

Utility Pole _____ date/app. by

M/H Pole _____ date/app. by

Travel Trailer _____ date/app. by

Re-roof _____ date/app. by

BUILDING PERMIT FEE $ 480.00 CERTIFICATION FEE $ 14.64 SURCHARGE FEE $ 14.64

MISC. FEES $ 0.00 ZONING CERT. FEE $ 50.00 FIRE FEE $ 0.00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ 25.00 **TOTAL FEE** 609.28

INSPECTORS OFFICE _____ CLERKS OFFICE CH

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Columbia County Building Permit Application

Revised 9-23-04

For Office Use Only Application # 0612-26 Date Received 12/7 By JW Permit # 12781/25311

Application Approved by - Zoning Official BLK Date 12.12.06 Plans Examiner OKJTH Date 12-7-06

Flood Zone X Development Permit N/A Zoning RSF-2 Land Use Plan Map Category RES. Low DEN.

Comments SITE PLAN ON PLANS

Applicants Name Charese Norton Phone 386-752-3331

Address 3367 S US HWY 441, Ste 101, LAKE City, FL 32025

Owners Name Norton Home Improvement Co., Inc. Phone 386-752-3331

911 Address 237 SW Briarbrook PL, LAKE City, FL 32024

Contractors Name James H. Norton Phone 386-752-3331

Address 3367 S US HWY 441, Ste 101, LAKE City, FL 32025

Fee Simple Owner Name & Address N/A

Bonding Co. Name & Address N/A

Architect/Engineer Name & Address Tim DeIbere 192 SW Sagewood Gl, LAKE City, FL 32024, Mark Disaway P.O. Box 868, LAKE City, FL 32056

Mortgage Lenders Name & Address N/A

Circle the correct power company - FL Power & Light - (Clay Elec.) - Suwannee Valley Elec. - Progressive Energy

Property ID Number 25-4S-16-03121-055 Estimated Cost of Construction 115000.00

Subdivision Name Picadilly Park South Lot 13 Block D Unit - Phase -

Driving Directions 47 S to CR242, turn Right go to Mockingbird turn left go down to Briarbrook turn Right, house is down on right. (5th lot).

Type of Construction SFD, new home Const. Number of Existing Dwellings on Property 0

Total Acreage 1/2 acre Lot Size _____ Do you need a (Culvert Permit) or Culvert Waiver or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 30' Side 20' Side 75' Rear 78'

Total Building Height 19' Number of Stories 1 Heated Floor Area 1915 TOTAL 2924 Roof Pitch 6/12

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

James H Norton

Owner Builder or Agent (Including Contractor)

James H. Norton

Contractor Signature

Contractors License Number RB0031780

Competency Card Number 5553

STATE OF FLORIDA
COUNTY OF COLUMBIA

PATRICIA T. PEELER
Notary Public, State of Florida
My comm. exp. Sep. 5, 2010
Comm. No. DD 579471

NOTARY STAMP/SEAL

Sworn to (or affirmed) and subscribed before me this 05 day of DEC 20 06.

Personally known ✓ or Produced Identification _____

Patricia T. Peeler

Notary Signature

22631 Jw called Charese 12.14.06

C

Columbia County Building Department
Culvert Permit

Culvert Permit No.
000001278

DATE 12/14/2006 PARCEL ID # 25-4S-16-03121-055

APPLICANT CHARESE NORTON PHONE 386.752.3331

ADDRESS 3367 S US HWY 441, STE 101 LAKE CITY FL 32025

OWNER NORTON HOME IMP. CO., INC. PHONE 386.752.3331

ADDRESS 237 SW BRIARBROOK PLACE LAKE CITY FL 32024

CONTRACTOR JAMES H. NORTON PHONE 386.752.3331

LOCATION OF PROPERTY 47-S TO C-242,TR TO MOCKINGBIRD ,TL TO BRIARBROOK,TR THE SITE IS THE 5TH LOT ON R.

SUBDIVISION/LOT/BLOCK/PHASE/UNIT PICCADILLY PARK SOUT 13 D

SIGNATURE Charese J. Norton

INSTALLATION REQUIREMENTS

[X] Culvert size will be 18 inches in diameter with a total lenght of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4 : 1 slope and poured with a 4 inch thick reinforced concrete slab.

INSTALLATION NOTE: Turnouts will be required as follows:
a) a majority of the current and existing driveway turnouts are paved, or;
b) the driveway to be served will be paved or formed with concrete.
Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concreted turnouts.

[ ] Culvert installation shall conform to the approved site plan standards.

[ ] Department of Transportation Permit installation approved standards.

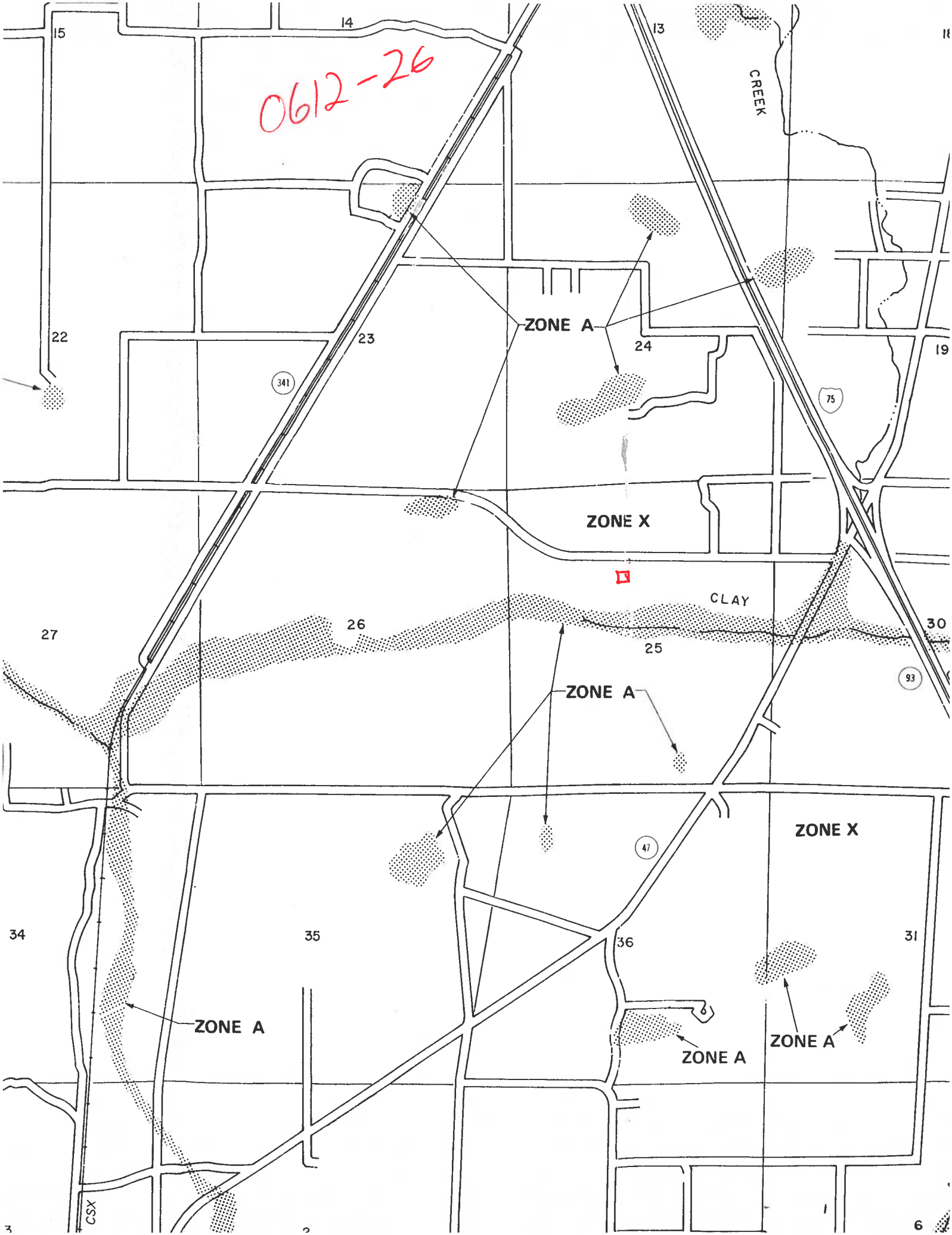
[ ] Other _____

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED DURING THE INSTALATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00

COLUMBIA COUNTY GATEWAY TO FLORIDA


0612-26
CREEK
ZONE A
ZONE X
CLAY
ZONE A
ZONE X
ZONE A
ZONE A
ZONE A
CSX
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36
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341
75
93
47

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@ CAM112M01     S  CamaUSA Appraisal System                    Columbia County
12/07/2006  9:38  Legal Description Maintenance      65600  Land  001   *
Year T Property                          Sel                  AG    000
2007 R 25-4S-16-03121-055 ..... ... ..                        Bldg  000
                                                              Xfea  000
       NORTON HOME IMP CO INC                             65600  TOTAL     B

  1  LOTS 3,, 5,, 11,, 13 BLOCK D ..... PICCADILLY PARK SOUTH S/D. ....   2
  3  ORB 903-2437,, 906-1897,. ..... ..............................   4
  5  .............................. ..............................   6
  7  .............................. ..............................   8
  9  .............................. ..............................  10
 11  .............................. ..............................  12
 13  .............................. ..............................  14
 15  .............................. ..............................  16
 17  .............................. ..............................  18
 19  .............................. ..............................  20
 21  .............................. ..............................  22
 23  .............................. ..............................  24
 25  .............................. ..............................  26
 27  .............................. ..............................  28
                                                Mnt 10/30/2000 TERRY
F1=Task  F3=Exit  F4=Prompt  F10=GoTo  PgUp/PgDn  F24=More
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NOTICE OF COMMENCEMENT FORM
COLUMBIA COUNTY, FLORIDA

******THIS DOCUMENT MUST BE RECORDED AT THE COUNTY CLERKS OFFICE BEFORE YOUR FIRST INSPECTION.******

THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

Tax Parcel ID Number 25-4S-16-03121-055

1. **Description of property: (legal description of the property and street address or 911 address)**
   Picadilly Park South Subdivision Lot 13 Block D

2. **General description of improvement:** New home Construction, Single Family Dwelling

3. **Owner Name & Address** Norton Home Improvement Co., Inc. 3367 S US Hwy 441, Ste 101, Lake City, FL 32025 **Interest in Property** Own

4. **Name & Address of Fee Simple Owner (if other than owner):** NA

5. **Contractor Name** James H. Norton **Phone Number** 386-752-3331
   Address 3367 S US Hwy 441, Ste 101, Lake City, FL 32025

6. **Surety Holders Name** NA **Phone Number**
   Address
   Amount of Bond NA

Inst:2006028851 Date:12/07/2006 Time:08:09
S.T DC,P.DeWitt Cason,Columbia County B:1104 P:579

7. **Lender Name** NA
   Address

8. **Persons within the State of Florida designated by the Owner upon whom notices or other documents may be served as provided by section 718.13 (1)(a) 7; Florida Statutes:**
   Name NA **Phone Number**
   Address

9. **In addition to himself/herself the owner designates** NA **of** _____ **to receive a copy of the Lienor's Notice as provided in Section 713.13 (1) – (a) 7. Phone Number of the designee** NA

10. **Expiration date of the Notice of Commencement (the expiration date is 1 (one) year from the date of recording, (Unless a different date is specified)**

NOTICE AS PER CHAPTER 713, Florida Statutes:
The owner must sign the notice of commencement and no one else may be permitted to sign in his/her stead.

James H. Norton
Signature of Owner

Sworn to (or affirmed) and subscribed before day of 12-05, 20 06

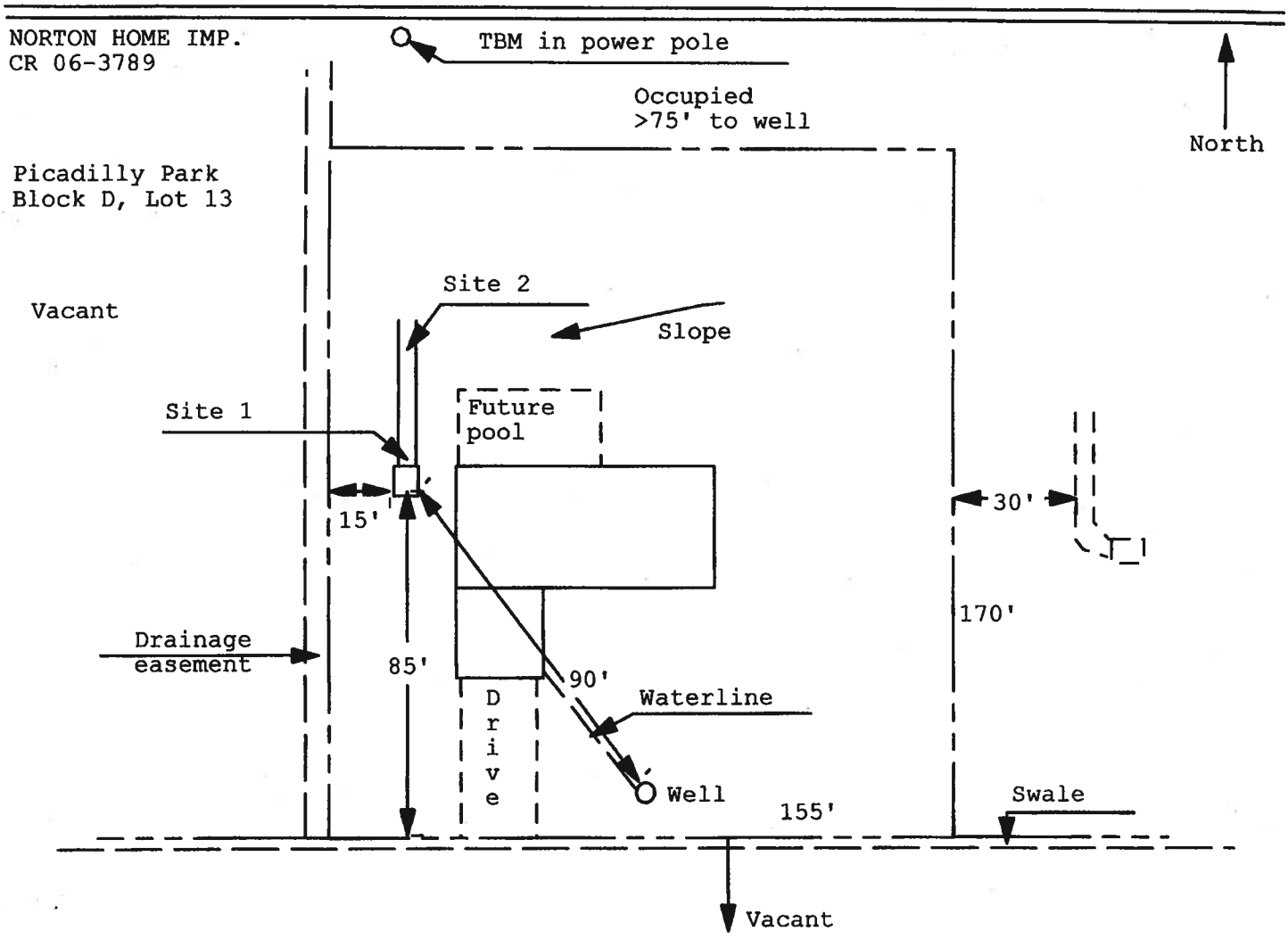
PATRICIA T. PEELER
NOTARY STAMP/SEAL
Notary Public, State of Florida
My comm. exp. Sep. 5, 2010
Comm. No. DD 579471

Patricia T. Peeler
Signature of Notary

Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan

Permit Application Number: 06-01062N

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT



1 inch = 40 feet

Site Plan Submitted By Paul Lloyd **Date** 11/28/06

Plan Approved ✓ **Not Approved** ____ **Date** 12/5/06

By Marsha Jones Columbia **CPHU**

Notes: _____

FORM 600A-2001

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs

Residential Whole Building Performance Method A

Project Name:	**Spec House by Norton**	Builder:	**Norton Home Imp.**
Address:	**Lot: 13, Sub: Piccadilly Pk., Plat:**	Permitting Office:	**Columbia Co.**
City, State:	**Lake City, FL**	Permit Number:	121000
Owner:	**Norton Home Improvements**	Jurisdiction Number:	~~121000~~
Climate Zone:	**North**		

1. New construction or existing		New	
2. Single family or multi-family		Single family	
3. Number of units, if multi-family		1	
4. Number of Bedrooms		3	
5. Is this a worst case?		No	
6. Conditioned floor area (ft²)		1915 ft²	
7. Glass area & type	Single Pane	Double Pane	
a. Clear glass, default U-factor	0.0 ft²	245.0 ft²	
b. Default tint	0.0 ft²	0.0 ft²	
c. Labeled U or SHGC	0.0 ft²	0.0 ft²	
8. Floor types			
a. Slab-On-Grade Edge Insulation		R=0.0, 229.0(p) ft	
b. N/A			
c. N/A			
9. Wall types			
a. Frame, Wood, Exterior		R=13.0, 1545.0 ft²	
b. N/A			
c. N/A			
d. N/A			
e. N/A			
10. Ceiling types			
a. Under Attic		R=30.0, 1915.0 ft²	
b. N/A			
c. N/A			
11. Ducts			
a. Sup: Unc. Ret: Unc. AH: Interior		Sup. R=6.0, 5.0 ft	
b. N/A			

12. Cooling systems		
a. Central Unit	Cap: 35.0 kBtu/hr	
	SEER: 14.00	
b. N/A		
c. N/A		
13. Heating systems		
a. Electric Heat Pump	Cap: 35.0 kBtu/hr	
	HSPF: 7.90	
b. N/A		
c. N/A		
14. Hot water systems		
a. Electric Resistance	Cap: 30.0 gallons	
	EF: 0.90	
b. N/A		
c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)		
15. HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)	PT, CF,	

Glass/Floor Area: 0.13

Total as-built points: 22281
Total base points: 28209

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: Tim Delbene

DATE: 10/30/06

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____

DATE: _____

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL: _____

DATE: _____

FORM 600A-2001

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 13, Sub: Piccadilly Pk., Plat: , Lake City, FL,** PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BSPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	SPM X	SOF	= Points	
.18	1915.0	20.04	6907.8	Double, Clear	N	2.0	7.0	45.0	19.20	0.92	796.8	
				Double, Clear	N	9.0	7.0	30.0	19.20	0.67	387.5	
				Double, Clear	N	13.0	8.0	30.0	19.20	0.64	370.5	
				Double, Clear	S	2.0	5.0	9.0	35.87	0.72	233.5	
				Double, Clear	S	2.0	7.0	36.0	35.87	0.82	1059.0	
				Double, Clear	S	8.0	7.0	36.0	35.87	0.50	645.8	
				Double, Clear	E	2.0	7.0	26.0	42.06	0.89	968.9	
				Double, Clear	E	2.0	5.0	12.0	42.06	0.80	402.2	
				Double, Clear	W	2.0	5.0	6.0	38.52	0.80	184.8	
				Double, Clear	W	10.0	8.0	15.0	38.52	0.48	276.1	
				As-Built Total:				**245.0**			**5325.1**	
WALL TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points	
Adjacent	0.0	0.00	0.0	Frame, Wood, Exterior			13.0	1545.0	1.50		2317.5	
Exterior	1545.0	1.70	2626.5									
Base Total:	**1545.0**		**2626.5**	**As-Built Total:**				**1545.0**			**2317.5**	
DOOR TYPES	Area X	BSPM =	Points	Type				Area X	SPM =		Points	
Adjacent	21.0	2.40	50.4	Exterior Insulated				21.0	4.10		86.1	
Exterior	21.0	6.10	128.1	Adjacent Insulated				21.0	1.60		33.6	
Base Total:	**42.0**		**178.5**	**As-Built Total:**				**42.0**			**119.7**	
CEILING TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM X SCM =		Points	
Under Attic	1915.0	1.73	3312.9	Under Attic			30.0	1915.0	1.73 X 1.00		3312.9	
Base Total:	**1915.0**		**3312.9**	**As-Built Total:**				**1915.0**			**3312.9**	
FLOOR TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points	
Slab	229.0(p)	-37.0	-8473.0	Slab-On-Grade Edge Insulation			0.0	229.0(p	-41.20		-9434.8	
Raised	0.0	0.00	0.0									
Base Total:			**-8473.0**	**As-Built Total:**				**229.0**			**-9434.8**	
INFILTRATION	Area X	BSPM =	Points					Area X	SPM =		Points	
	1915.0	10.21	19552.2					1915.0	10.21		19552.2	

FORM 600A-2001

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 13, Sub: Piccadilly Pk., Plat: , Lake City, FL,** PERMIT #:

BASE			AS-BUILT					
Summer Base Points:		**24104.9**	**Summer As-Built Points:**					**21192.6**
Total Summer Points	X System Multiplier	= Cooling Points	Total Component	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Cooling Points
24104.9	**0.4266**	**10283.1**	21192.6 **21192.6**	1.000 **1.00**	(1.090 x 1.147 x 0.91) **1.138**	0.244 **0.244**	0.902 **0.902**	5304.8 **5304.8**

FORM 600A-2001

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 13, Sub: Piccadilly Pk., Plat: , Lake City, FL,** PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BWPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	WPM X	WOF	= Points	
.18	1915.0	12.74	4391.5	Double, Clear	N	2.0	7.0	45.0	24.58	1.00	1109.7	
				Double, Clear	N	9.0	7.0	30.0	24.58	1.02	753.0	
				Double, Clear	N	13.0	8.0	30.0	24.58	1.02	754.7	
				Double, Clear	S	2.0	5.0	9.0	13.30	1.40	167.6	
				Double, Clear	S	2.0	7.0	36.0	13.30	1.17	560.5	
				Double, Clear	S	8.0	7.0	36.0	13.30	2.96	1419.0	
				Double, Clear	E	2.0	7.0	26.0	18.79	1.05	510.8	
				Double, Clear	E	2.0	5.0	12.0	18.79	1.08	244.3	
				Double, Clear	W	2.0	5.0	6.0	20.73	1.06	131.7	
				Double, Clear	W	10.0	8.0	15.0	20.73	1.19	370.2	
				As-Built Total:				**245.0**			**6021.6**	
WALL TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points	
Adjacent	0.0	0.00	0.0	Frame, Wood, Exterior			13.0	1545.0	3.40		5253.0	
Exterior	1545.0	3.70	5716.5									
Base Total:	**1545.0**		**5716.5**	**As-Built Total:**				**1545.0**			**5253.0**	
DOOR TYPES	Area X	BWPM =	Points	Type				Area X	WPM =		Points	
Adjacent	21.0	11.50	241.5	Exterior Insulated				21.0	8.40		176.4	
Exterior	21.0	12.30	258.3	Adjacent Insulated				21.0	8.00		168.0	
Base Total:	**42.0**		**499.8**	**As-Built Total:**				**42.0**			**344.4**	
CEILING TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM X	WCM =	Points	
Under Attic	1915.0	2.05	3925.8	Under Attic			30.0	1915.0	2.05 X	1.00	3925.8	
Base Total:	**1915.0**		**3925.8**	**As-Built Total:**				**1915.0**			**3925.8**	
FLOOR TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points	
Slab	229.0(p)	8.9	2038.1	Slab-On-Grade Edge Insulation			0.0	229.0(p	18.80		4305.2	
Raised	0.0	0.00	0.0									
Base Total:			**2038.1**	**As-Built Total:**				**229.0**			**4305.2**	
INFILTRATION	Area X	BWPM =	Points					Area X	WPM =		Points	
	1915.0	-0.59	-1129.8					1915.0	-0.59		-1129.8	

FORM 600A-2001

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 13, Sub: Piccadilly Pk., Plat: , Lake City, FL,** PERMIT #:

BASE			AS-BUILT					
Winter Base Points:		**15441.8**	**Winter As-Built Points:**					**18720.1**
Total Winter Points	X System Multiplier	= Heating Points	Total Component	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points
15441.8	**0.6274**	**9688.2**	18720.1 **18720.1**	1.000 **1.00**	(1.069 x 1.169 x 0.93) **1.162**	0.432 **0.432**	0.950 **0.950**	8921.4 **8921.4**

FORM 600A-2001

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 13, Sub: Piccadilly Pk., Plat: , Lake City, FL,** PERMIT #:

BASE				AS-BUILT							
WATER HEATING											
Number of Bedrooms	X	Multiplier	= Total	Tank Volume	EF	Number of Bedrooms	X	Tank Ratio	X Multiplier	X Credit Multiplier	= Total
3		2746.00	8238.0	30.0	0.90	3		1.00	2684.98	1.00	8054.9
				As-Built Total:							**8054.9**

CODE COMPLIANCE STATUS

BASE				AS-BUILT			
Cooling Points +	Heating Points +	Hot Water Points =	Total Points	Cooling Points +	Heating Points +	Hot Water Points =	Total Points
10283	**9688**	**8238**	**28209**	**5305**	**8921**	**8055**	**22281**

PASS

FORM 600A-2001

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 13, Sub: Piccadilly Pk., Plat: , Lake City, FL,** PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	✓
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	✓
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	✓
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	✓
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	✓
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	N/A
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	✓

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 6-12. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	✓
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	N/A
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	✓
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	✓
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	✓
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	✓

Architectural Testing

ANSI/AAMA/NWWDA 101/I.S.2-97
TEST REPORT

Rendered to:

MI WINDOWS AND DOORS, INC.

SERIES/MODEL: 8500/1250
PRODUCT TYPE: PVC Single Hung Window

Title	Summary of Results		
	Test Specimen #1	Test Specimen #2	Test Specimen #3
Rating	H-R25 48 x 78	H-R35* 36 x 72	H-R25* 40 x 83
Operating Force	21 lbf max.	N/A	N/A
Air Infiltration	0.15 cfm/ft^2	N/A	N/A
Water Resistance Test Pressure	6.0 psf	N/A	N/A
Uniform Load Deflection Test Pressure	±25.0 psf	+35.0 psf/-40.0 psf	±25.0 psf
Uniform Load Structural Test Pressure	±37.5 psf	+52.5 psf/60.0 psf	±37.5 psf
Forced Entry Resistance	Grade 10	N/A	N/A

Reference should be made to ATI Report No. 56448.02-122-47 for complete test specimen description and data.

130 Derry Court
York, PA 17402-9405
phone: 717-764-7700
fax: 717-764-4129
www.archtest.com

Architectural Testing

ANSI/AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to:

MI WINDOWS AND DOORS, INC.
P.O. Box 370
Gratz, Pennsylvania 17030-0370

Report No.:	56448.02-122-47
Test Date:	03/17/05
And:	03/18/05
Report Date:	03/29/05
Expiration Date:	03/18/09

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Windows and Doors, Inc. to witness testing on three Series/Model 8500/1250, PVC single hung windows at MI Windows and Doors, Inc. test facility in Elizabethville, Pennsylvania. The samples tested successfully met the performance requirements for the following ratings: Test Specimen #1: H-R25 48 x 78; Test Specimen #2: H-R35* 36 x 72; Test Specimen #3: H-R25* 40 x 83. Test specimen description and results are reported herein.

General Note: *An asterisk (*) next to the performance grade indicates that the size tested for optional performance was smaller than the Gateway test size for the product type and class.*

Test Specification: The test specimens were evaluated in accordance with ANSI/AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 8500/1250

Product Type: PVC Single Hung Windows

Test Specimen #1: H-R25 48 x 78

Overall Size: 4' 0" wide by 6' 6" high

Interior Sash Size: 3' 9-1/2" wide by 3' 1-3/4" high

Daylight Opening Size: 3' 6-7/8" wide by 2' 10-3/4" high

Screen Size: 3' 8" wide by 3' 1" high

130 Derry Court
York, PA 17402-9405
phone: 717-764-7700
fax: 717-764-4129
www.archtest.com

Test Specime Description: (Continued)

Test Spe men #2: H-R35* 36 x 72

Ove ll Size: 3' 0" wide by 6' 0" high

Inte or Sash Size: 2' 9-5/8" wide by 2' 10-3/4" high

Dayl ht Opening Size: 2' 6-3/4" wide by 2' 7-3/4" high

Scre Size: 2' 8" wide by 2' 9-1/2" high

Test Spe men #3: H-R25* 40 x 83 (Oriel)

Ove ll Size: 3' 4-1/4" wide by 6' 11-1/4" high

Inte or Sash Size: 3' 1-1/2" wide by 2' 4-3/4" high

Dayl ht Opening Size: 2' 10-3/4" wide by 4' 0-7/8" high

Scre Size: 3' 0-1/8" wide by 2' 4" high

The following descriptions apply to all specimens.

Finish: A l PVC was white.

Glazing Details: The test specimens utilized 7/8" thick, sealed insulating glass fabricated from two sheets of 3/32" thick, clear annealed glass and a metal reinforced butyl spacer system. The lites were interior glazed onto double-sided adhesive foam tape and secured with PVC snap-in glazing beads.

Weather ripping:

Descri tion	Quantity	Location
0.250" high by 0.187" backed polypi with center fin	1 Row	Interior vertical sill leg, active meeting rail, and stiles
0.290" high by 0.187" backed polypi with center fin	1 Row	Stiles
1/8" ro nd vinyl foam filled polypi with center fin	1 Row	Fixed meeting rail
5/16" r und vinyl foam filled bulb s l	1 Row	Bottom rail

Test Specimen Description: (Continued)

Frame Construction: The frame was constructed of extruded vinyl with mitered and welded corners. End caps were utilized on the ends of the fixed meeting rail and secured with three #6 x 5/8" flat head screws through the end cap into the fixed meeting rail screw boss. The end caps were then secured to the jamb with three #6 x 5/8" flat head screws through the end caps into the jambs. The sill utilized a snap-in sill insert.

Sash Construction: The sashes were constructed of extruded vinyl with mitered and welded corners.

Screen Construction: The screen was constructed of roll-formed aluminum with keyed corners. The fiberglass mesh was secured with a flexible wrap around vinyl spline.

Hardware:

Description	Quantity	Location
Metal sweep locks	2	7" from active sash ends
Tilt latch	2	Active meeting rail ends
Tilt pins	2	Bottom rail
Constant force balance assembly	2	One per jamb

Drainage:

Description	Quantity	Location
1" long by 1/8" high weephole	2	3" from sill ends, on sill face
1/2" long 3/16" high weephole	2	2-1/2" from jambs in sill track
1/2" long by 3/16" high weephole	2	Bottom rail under glazing
1/2" long by 1/16" high weephole	4	Two 2-1/2" from bottom rail ends

Reinforcement: The fixed meeting rail utilized a roll-formed steel reinforcement (Drawing #RF-104). The active meeting rail and the bottom rail utilized a roll-formed steel reinforcement (Drawing #GVL-451).

Installation: The windows were installed into a #2 Spruce-Pine-Fir wood buck. The nail fin was back bedded in silicone and secured utilizing #6 x 1-5/8" drywall screws located 3" from corners and 8" on center. Silicone was utilized around the exterior perimeter.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
Test Specimen #1: H-R25 48 x 78			
2.2.1.6.1	Operating Force	21 lbf	30 lbf max.
2.1.2	Air Infiltration per ASTM E 283 1.57 psf (25 mph)	0.15 cfm/ft^2	0.30 cfm/ft^2 max.

Note #1: The tested specimen meets (or exceeds) the performance levels specified in ANSI/AAMA/NWWDA 101/I.S.2-97 for air infiltration.

2.1.3	Water Resistance per ASTM E 547 (with and without screen) 2.86 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection per ASTM E 330 (Deflections reported were taken on the meeting rail) (Loads were held for 52 seconds)		
	15.0 psf (positive)	0.31"	See Note #2
	15.0 psf (negative)	0.32"	See Note #2
2.1.4.2	Uniform Load Structural per ASTM E 330 (Permanent sets reported were taken on the meeting rail) (Loads were held for 10 seconds)		
	22.5 psf (positive)	0.01"	0.17" max.
	22.5 psf (negative)	0.03"	0.17" max.

Note #2: The Uniform Load Deflection test is not a requirement of ANSI/AAMA/NWWDA 101/I.S.2-97 for this product designation. The deflection data is recorded in this report for special code compliance and information only.

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
Test Specimen #1: H-R25 48 x 78 (Continued)			
2.2.1.6.2	Deglazing Test per ASTM E 987 In operating direction - 70 lbs		
	Meeting rail	0.13"/25%	0.50"/100%
	Bottom rail	0.13"/25%	0.50"/100%
	In remaining direction - 50 lbs		
	Right stile	0.06"/13%	0.50"/100%
	Left stile	0.06"/13%	0.50"/100%
2.1.7	Welded Corner Test	Meets as stated	Meets as stated
2.1.8	Forced Entry Resistance per ASTM F 588		
	Type: A	Grade: 10	
	Lock Manipulation Test	No entry	No entry
	Test A1 through A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry
Optional Performance			
4.3	Water Resistance per ASTM E 547 (with and without screen) 6.0 psf	No leakage	No leakage

Test Results: ntinued)

Paragrapl	Title of Test - Test Method	Results	Allowed
Test Spe	**n #1**: H-R25 48 x 78 (Continued)		
Optional	ormance: (Continued)		
4.4.1	Uniform Load Deflection per ASTM E 330 (Deflections reported were taken on the meeting rail) (Loads were held for 52 seconds)		
	25.0 psf (positive)	0.62"	See Note #2
	25.0 psf (negative)	0.49"	See Note #2
4.4.2	Uniform Load Structural per ASTM E 330 (Permanent sets reported were taken on the meeting rail) (Loads were held for 10 seconds)		
	37.5 psf (positive)	0.09"	0.17" max.
	37.5 psf (negative)	0.07"	0.17" max.
Test Spec	**#2**: H-R35* 36 x 72		
Optional P	rmance		
4.4.1	Uniform Load Deflection per ASTM E 330 (Deflections reported were taken on the meeting rail) (Loads were held for 52 seconds)		
	35.0 psf (positive)	0.19"	See Note #2
	40.0 psf (negative)	0.21"	See Note #2
4.4.2	Uniform Load Structural per ASTM E 330 (Permanent sets reported were taken on the meeting rail) (Loads were held for 10 seconds)		
	52.5 psf (positive)	0.03"	0.17" max.
	60.0 psf (negative)	0.05"	0.17" max.

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
Test Specimen #3: H-R25* 40 x 83 (Oriel)			
Optional Performance			
4.4.1	Uniform Load Deflection per ASTM E 330		
	(Deflections reported were taken on the meeting rail)		
	(Loads were held for 52 seconds)		
	25.0 psf (positive)	0.33"	See Note #2
	25.0 psf (negative)	0.22"	See Note #2
4.4.2	Uniform Load Structural per ASTM E 330		
	(Permanent sets reported were taken on the meeting rail)		
	(Loads were held for 10 seconds)		
	37.5 psf (positive)	0.02"	0.15" max.
	37.5 psf (negative)	0.02"	0.15" max.

Note: A lead check swab test was performed on all polymeric profiles. The test result was negative for the presence of lead (Pb).

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years from the original test date. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator. This report may not be reproduced, except in full, without the approval of Architectural Testing, Inc.

For ARCHITECTURAL TESTING, INC:

Mark A. Hess vlm

Digitally Signed for: Mark A. Hess by Vicki L. McElwain

Mark A. Hess
Technician

Digitally Signed by: Steven M. Urich

Steven M. Urich, P.E.
Senior Project Engineer

MAH:vlm

Revision Log

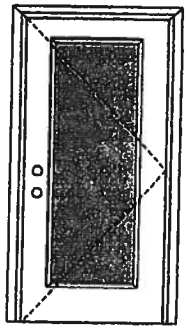
Rev. #	Date	Page(s)	Revision(s)
0	03/29/05	N/A	Original report issue

X
Glazed Inswing Unit

COP-WL-JH4141-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Note:
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".

Warnock Hersey

Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.etlsamko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Single Door
Maximum unit size = 3'0" x 6'8"

Design Pressure
+40.5/-40.5
Limited water unless special threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

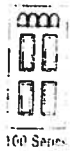
Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0001-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0001-02.

APPROVED DOOR STYLES:

1/4 GLASS:


100 Series


133, 135 Series


136 Series


680 Series


822 Series

1/2 GLASS:


105 Series*


106, 150 Series*


129 Series*


200 Series*


12 R/L, 23 R/L, 24 R/L Series*


107 Series*


108 Series


304 Series

*This glass kit may also be used in the following door styles: 5-panel; 5-panel with scroll; Eyebrow 5-panel; Eyebrow 5-panel with scroll.

Johnson™ EntrySystems

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

PREMDOR Collection Premium Quality Doors

Exclusively from Masonite®
Masonite International Corporation

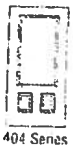
X
Glazed Inswing Unit

COP-WL-JH4141-02

WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES:

3/4 GLASS:


404 Series


410 Series


450 Series

FULL GLASS:


109 Series


114, 120, 122 Series


152 Series


149 Series


300 Series

CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9, 10, 11, 12; NCTL 210-1861-4, 5, 6, 10, 11, 12; NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Evaluation report NCTL-210-2794-1

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey WH

Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.etisamko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Johnson™ EntrySystems

June 17, 2002

Our continuing program of product improvement makes specifications, design and product details subject to change without notice.

PREMDOR Collection Premium Quality Doors

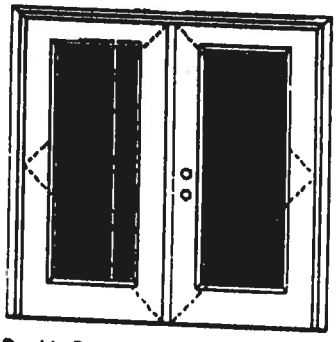
Exclusively from Masonite®
Masonite International Corporation

XX
Glazed Inswing Unit

COP-WL-JH4142-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

Double Door
Maximum unit size = 6'0" x 6'8"

Design Pressure
+40.5/-40.5
Limited water unless special threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0002-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0002-02.

APPROVED DOOR STYLES:

1/4 GLASS:


100 Series


135, 136 Series


136 Series


600 Series


622 Series

1/2 GLASS:


105 Series*


106, 160 Series*


120 Series*


200 Series*


12 R/L, 23 R/L, 24 R/L Series*


107 Series*


108 Series


304 Series

*This glass kit may also be used in the following door styles: 6-panel; 6-panel with scroll; Eyebrow 6-panel; Eyebrow 6-panel with scroll.

Johnson EntrySystems

March 28, 2002
Our continuing program of product improvement makes specifications, design and product data subject to change without notice.

PREMDOR Collection
Masonite
Exclusively from Masonite International Corporation

XX
Glazed Inswing Unit

COP-WI-JH4142-02

WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES:

3/4 GLASS:


404 Series


410 Series


450 Series

FULL GLASS:


109 Series


114, 120, 122 Series


152 Series


149 Series


300 Series

CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9, 10, 11, 12; NCTL 210-1861-4, 5, 6, 10, 11, 12; NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Evaluation report NCTL-210-2794-1

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Bathy

State of Florida, Professional Engineer
Kurt Baithazor, P.E. – License Number 56533

Johnson Entry Systems

March 20, 2002

Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

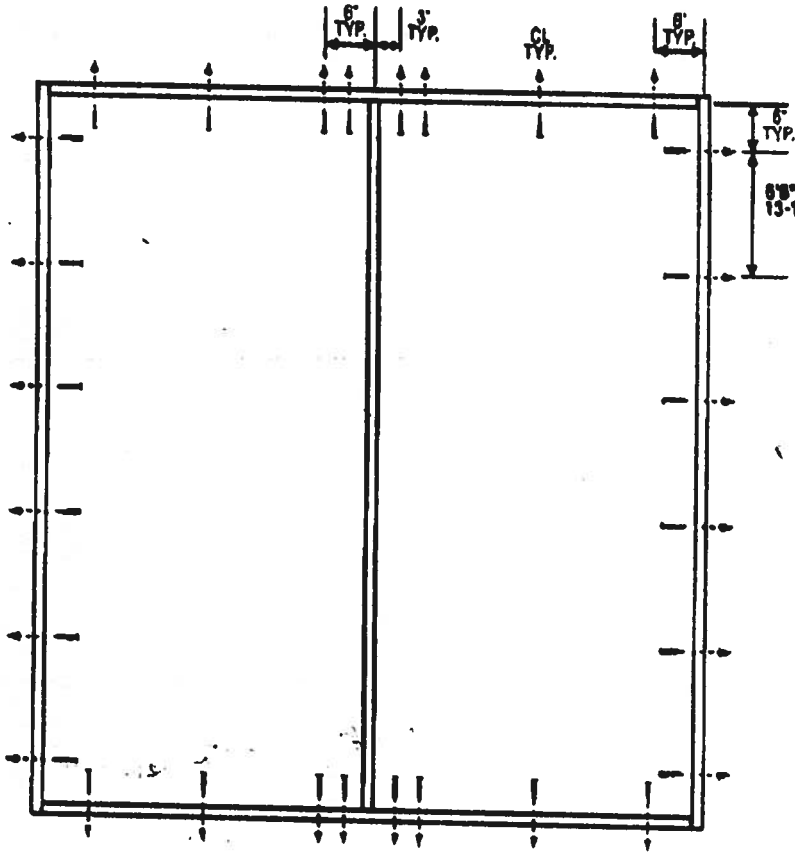
PREMDOOR Collection Premium Quality Doors

Exclusively from Masonite®
Masonite International Corporation

XX
Unit

MID-WL-MA0002-02

DOUBLE DOOR



Minimum Fastener Count

- 6 per vertical framing member
- 8 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Latching Hardware:

- Compliance requires that GRADE 2 or better (ANSI/BHMA A156.2) cylinderical and deadlock hardware be installed.

Notes:

1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons.
2. The wood screw single shear design values come from Table 11.3A of ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and embedment of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Dade Country approvals respectively, each with minimum 1-1/4" embedment.
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

March 29, 2002
Our continuing program of product improvement makes specifications, design and process detail subject to change without notice.

PREMDOR by Masonite Premium Quality Doors
Exclusively from Masonite
Masonite International Corporation

TAMKO®
ROOFING PRODUCTS

January 31, 2002

TO: OUR FLORIDA CUSTOMERS:

Effective February 1, 2002, the following TAMKO shingles, as manufactured at TAMKO's Tuscaloosa, Alabama, facility, comply with ASTM D-3161, Type I modified to 110 mph. Testing was conducted using four nails per shingle. These shingles also comply with Florida Building Code TAS 100 for wind driven rain.

- Glass-Seal AR
- Elite Glass-Seal AR
- ASTM Heritage 30 AR (formerly ASTM Heritage 25 AR)
- Heritage 40 AR (formerly Heritage 30 AR)
- Heritage 50 AR (formerly Heritage 40 AR)

All testing was performed by Florida State certified independent labs.

Please direct all questions to TAMKO's Technical Services Department at 1-800-641-4691.

TAMKO Roofing Products, Inc.

2300 35th STREET P.O. BOX 2149 TUSCALO

MIAMI-DADE

MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING

BUILDING CODE COMPLIANCE OFFICE
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901 FAX (305) 375-2908

CONTRACTOR LICENSING SECTION
(305) 375-2527 FAX (305) 375-2558

CONTRACTOR ENFORCEMENT DIVISION
(305) 375-2966 FAX (305) 375-2908

PRODUCT CONTROL DIVISION
(305) 375-2902 FAX (305) 372-6339

PRODUCT CONTROL NOTICE OF ACCEPTANCE

Premdor Entry Systems
911 E. Jeferson, P.O. Box 76
Pittsburgh ,KS 66762

Your application for Notice of Acceptance (NOA) of:
Series Entergy 6-8 S-W/E Inswing Opaque Residential Insulated Steel Door w/H. M. Frame
under Chapter 8 of the Code of Miami-Dade County governing the use of Alternate Materials and Types of Construction, and completely described herein, has been recommended for acceptance by the Miami-Dade County Building Code Compliance Office (BCCO) under the conditions specified herein.

This NOA shall not be valid after the expiration date stated below. BCCO reserves the right to secure this product or material at any time from a jobsite or manufacturer's plant for quality control testing. If this product or material fails to perform in the approved manner, BCCO may revoke, modify, or suspend the use of such product or material immediately. BCCO reserves the right to revoke this approval, if it is determined by BCCO that this product or material fails to meet the requirements of the South Florida Building Code.

The expense of such testing will be incurred by the manufacturer.

ACCEPTANCE NO.: 01-0313.06
EXPIRES: 02/19/2006

Raul Rodriguez
Chief Product Control Division

THIS IS THE COVERSHEET, SEE ADDITIONAL PAGES FOR SPECIFIC AND GENERAL CONDITIONS

BUILDING CODE & PRODUCT REVIEW COMMITTEE

This application for Product Approval has been reviewed by the BCCO and approved by the Building Code and Product Review Committee to be used in Miami-Dade County, Florida under the conditions set forth above.

RECEIVED
APR 27 2001
PITTSBURG, KANSAS

APPROVED: 04/19/2001

Francisco J. Quintana, R.A.
Director
Miami-Dade County
Building Code Compliance Office

Premdor Entry Systems

ACCEPTANCE NO.: 01-0313.06

APPROVED : APR 19 2001

EXPIRES : February 19, 2006

NOTICE OF ACCEPTANCE: SPECIFIC CONDITIONS

1. DESCRIPTION OF UNIT

1.1 This renews the Notice of Acceptance No. 97-0910.11 which was issued on February 19, 1998. It approves a residential insulated steel door, as described in Section 2 of this Notice of Acceptance, designed to comply with the South Florida Building Code (SFBC), 1994 Edition for Miami-Dade County, for the locations where the pressure requirements, as determined by SFBC Chapter 23, do not exceed the Design Pressure Rating values indicated in the approved drawings.

2. PRODUCT DESCRIPTION

2.1 The Series Entergy 6' 8" S-W/E Inswing Opaque Residential Insulated Steel Door in a Hollow Metal Frame-Impact and its components shall be constructed in strict compliance with the following documents: Drawing No 31-1032-EW-I, Sheets 1 through 5 of 5; titled "Premdor (Entergy Brand Wood Edge) 3'0" x 6'8" Steel door in a Hollow Metal Frame (Inswing)" dated 6/25/97 with revision C. dated 3/20/01, bearing the Miami-Dade County Product Control approval stamp with the Notice of Acceptance number and approval date by the Miami-Dade County Product Control Division. These documents shall hereinafter be referred to as the approved drawings.

3. LIMITATIONS

3.1 This approval applies to single unit applications of single door only, as shown in approved drawings.

3.2 Unit shall be installed only at locations protected by a canopy or overhang such that the angle between the edge of canopy or overhang to sill is less than 45 degrees. Unless unit is installed in non-habitable areas where the unit and the area are designed to accept water infiltration.

4. INSTALLATION

4.1 The residential insulated steel door and its components shall be installed in strict compliance with the approved drawings.

4.2 Hurricane protection system (shutters): the installation of this unit will not require a hurricane protection system.

5. LABELING

5.1 Each unit shall bear a permanent label with the manufacturer's name or logo, city, state and following statement: "Miami-Dade County Product Control Approved".

6. BUILDING PERMIT REQUIREMENTS

6.1 Application for building permit shall be accompanied by copies of the following:

6.1.1 This Notice of Acceptance

6.1.2 Duplicate copies of the approved drawings, as identified in Section 2 of this Notice of Acceptance, clearly marked to show the components selected for the proposed installation.

6.1.3 Any other documents required by the Building Official or the South Florida Building Code (SFBC) in order to properly evaluate the installation of this system.

Manuel Perez, P.E. Product Control Examiner
Product Control Division

AMTROL INC.

WEL-FLO®

Pre-pressurized Water System Tanks

- Proven Diaphragm Design
- Tough Gloss Finish
- Sizes from 14 to 119 Gallons
- Outstanding Value

NEW HOME CONST ONLY

Pump and Tank Code
Section 613
Well Pumps and Tanks used for private potable water systems
~~July 1, 2001~~ March 1, 2002

613.1 Pumps. Well pumps used for potable water shall comply with sections 613.1.1 and 613.1.2
613.1.1 Pump Installation. Pumps shall be installed for operation without re-priming or breaking suction. Pumps shall be connected to the well head by means of a union, companion flange or compression coupling in such a manner that it is accessible for maintenance, repair and, removal.
613.1.2 Pump Sizing. Minimum pump size shall be determined by table 613.1.

Table 613.1
Minimum Private Potable Water System Pump Size

	Bathrooms in Home				
	1	1 ½	2-2 ½	3-4	5-6
Minimum Pump Size	7gpm	10gpm	14gpm	17gpm	21gpm

Notes:
1. Values given are average and do not include high and low extremes
2. Installations over 6 bathrooms shall be approved by the code official

613.2 Pressure Tanks. Tanks relying on expansion of a flexible membrane within a restricting container, or tanks with direct water- to- air interface to provide pressure in the water system shall be used. All pressure tanks for storing potable water under pressure, including those having an airspace for pressure for expansion shall be identified by seal, label, or plate indicating the manufacturer's name and model number and shall meet the following specifications:

1. Pressure tank drawdown shall be a minimum of 1 gallon for every gallon produced by the pump (Example: 20 gallon per minute pump will require a draw of 20 gallons usable). Exceptions: Pump start applications, constant pressure devices and variable speed pumps.
2. Pressure tanks must be constructed of steel, fiberglass, or comparable materials. Tanks to be buried shall have a minimum wall thickness of ¼ inch and be built by the manufacture specifically for underground use. Fiberglass or other non-metallic tanks to be buried shall have the structural strength to prevent collapse.

613.3 Piping. Piping associated with well pumps and tanks shall comply with Sections 613.3.1 through 613.3.
613.3.1 Drop Pipe. The Drop pipe from the submersible pump to the first fitting past the well seal shall be either galvanized steel, stainless steel, or PVC Schedule 80 threaded/coupled or lock joint pipe. The drop pipe for a single (pipe) jet pump shall be either galvanized steel, or stainless steel. The drop pipe for a double (pipe) jet shall be galvanized steel, stainless steel on the suction side and/or minimum PVC Schedule 40 on the pressure side.
613.3.2 Pump Discharge pipe sizing. For submersible pumps, pipe size shall be equal to the pump discharge. Piping for all other types of pumps shall be sized in accordance to the manufacturers' specifications.
613.3.3 Pressure Tank Pipe Sizing. Piping size for the offset of the pressure tank shall use the piping friction loss charts for the piping material used.
613.4 Electrical wiring. All wiring shall be installed in accordance with chapter 27 of the Florida Building code and NFPA 70.
613.5 Disinfection. The pump installer shall disinfect any potable well and water system in accordance with Section 610.
613.6 Valves. A pressure relief valve shall be installed on any pumping system that can produce pressures of 75 psi or greater. A check valve shall be installed at the well head of submersible pumps.

* Cycle Stop Valve's are constant press device.

* Countys may add higher demands

'ROL INC.

essurized Diaphragm Well Tanks

CHAMPION, WEL-FLO, PRO-LINE *See Net Sheet*

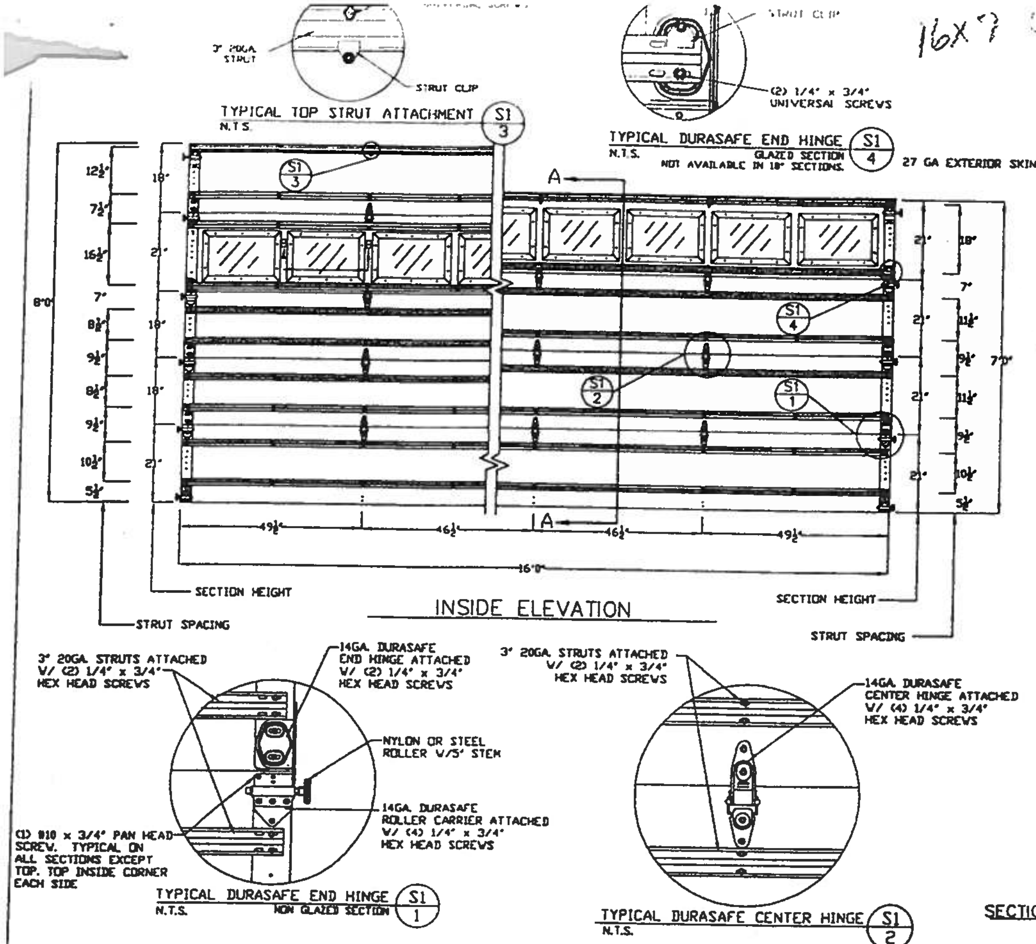
Model / Part No.	List Price ($)	Diameter (ins)	Dimensions Height (ins)	Total Volume (gals)	Max. Accept. Factor	System Drawdown 20/40 (gals)	30/50 (gals)	40/60 (gals)	Shipping Wt. (Vol.) lbs (cu ft)
CH 4202/WF60/CA4202	213.00	15¾	31⅛	20.0	0.57	8.0	6.8	5.9	33 (4.9)
CH 6000/WF80/CA6000	225.00	15¾	38¼	26.0	0.44	10.5	8.8	7.6	36.0
CH 8003/WF100/CA8003	364.00	15¾	46½	32.0	0.35	--	10.9	9.4	43 (7.0)
CH 8205/WF110/CA8205	399.00	22	29⅛	34.0	1.00	13.7	11.6	10.0	61 (9.5)
CH 10050/WF140/CA10050	461.00	22	36	44.0	0.77	17.7	15.0	13.0	69 (11.0)
CH 12051/WF200/CA12051	545.00	22	46¼	62.0	0.55	24.9	21.1	18.3	92 (13.9)
CH 17255/WF255/CA17255	585.00	22	56⅛	81.0	0.41	32.6	27.5	23.9	103
CH 17252/WF252/CA17252	663.00	22	62¼	86.0	0.39	34.6	29.2	25.4	114 (18.1)
CH 17002/WF260/CA17002	647.00	26	47¼	86.0	0.54	34.6	29.2	25.4	123 (18.9)
CH 22050/WF360/CA22050	922.00	26	51⅛	119.0	0.39	47.8	40.5	35.1	166 (24.5)

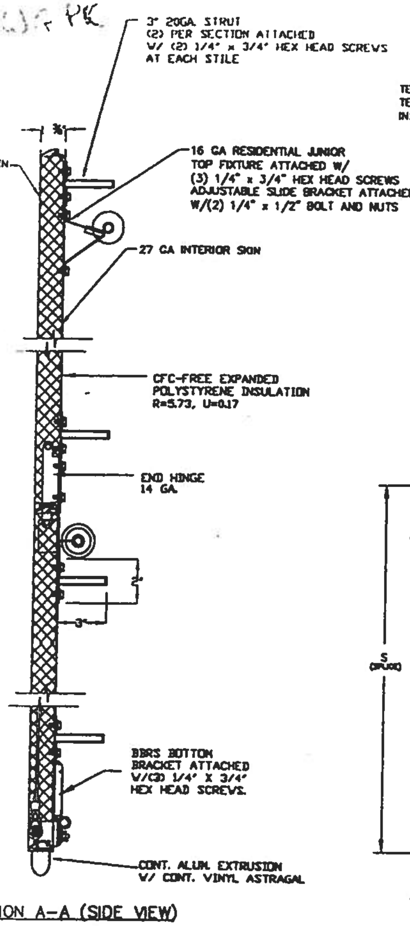
CH4202, CH6000, CH8003, WF60, WF80, WF100, CA 4202, CA6000, & CA8003 have a 1" NPTF system connection and a 28 psig pre-charge.

CH8205, CH22050, WF110, WF260, CA8205, CA22050 have a 1¼" NPTF system connection and a 39

FROM : LYNCH WELL DRILLING 752-1477 PHONE NO. : 7526677 MAR. 13 2002 02:25PM P3

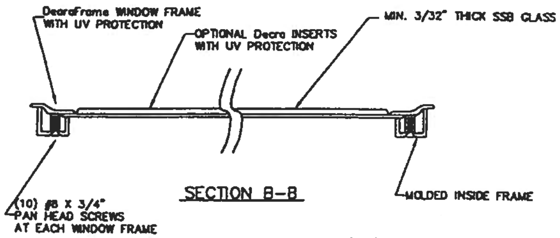
16X7

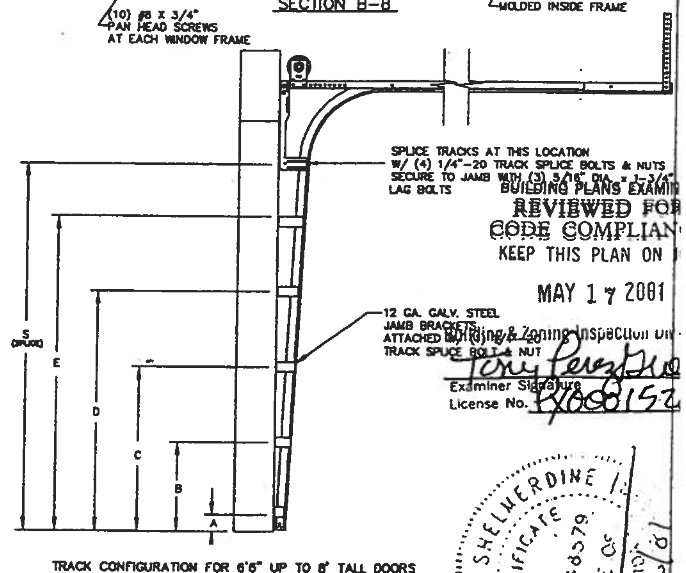




GLAZING OPTION CROSS SECTION

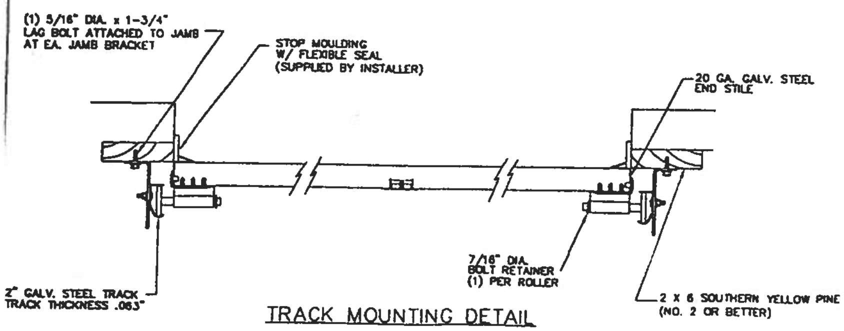
TEST No. SBC-580-020 ON MAY 24, 2000 INCLUDED GLASS WINDOWS IN THE DOOR BEING USED. THE TEST PRESSURES WERE +49.5 PSF AND -51.9 PSF. BY COMPARISION, EIGHT (8) WINDOWS MAY BE INSTALLED IN (1) ONE SECTION OF THE 16' X 7' AND 16' X 8' MODEL 1500-D DOORS.





BUILDING PLANS EXAMI[NER]
REVIEWED FO[R]
CODE COMPLIAN[CE]
KEEP THIS PLAN ON
MAY 17 2001
Building & Zoning Inspection Div.
Examiner Signature
License No.

THOMAS L. SHELMERDINE
CERTIFICATE NO. 48579
STATE OF



WOOD JAMB ATTACHMENT TO STRUCTURE

RATED FOR 110 MPH FASTEST-MILE BASIC WIND SPEEDS

VERTICAL JAMB ATTACHMENT TO WOOD FRAME STRUCTURE
5/16" X 3" LAG SCREWS STARTING 6" FROM ENDS THEN 24" O.C.

VERTICAL JAMB ATTACHMENT TO 2,000 PSI CONCRETE
HILTI KWIK BOLT 3/8" X 4" STARTING 6" FROM ENDS THEN 24" O.C.
HILTI SLEEVE ANCHOR 3/8" X 2-3/4" STARTING 6" FROM ENDS THEN 24" O.C.
ITW/RAMSET RED HEAD 3/8" X 3" STARTING 6" FROM ENDS THEN 24" O.C.

VERTICAL JAMB ATTACHMENT TO C-90 BLOCK
HILTI SLEEVE ANCHOR 3/8" X 2-3/4" STARTING 6" FROM ENDS THEN 24" O.C.
ITW/RAMSET TAPCON 1/4" X 2-3/4" STARTING 6" FROM ENDS, USE PAIRS OF FASTENERS (3" APART) AT 16" O.C.

*LAGS AND BOLTS CAN BE COUNTERSUNK TO PROVIDE A FLUSH MOUNTING SURFACE.
*PREPARATION OF WOOD JAMBS BY OTHERS

JAMB BRACKET LOCATIONS

	A	B	C	D	E	S
6'-6"	4"	21-1/2"	39"	57"		70"
7'-0"	4"	21-1/2"	42"	63"		76"
7'-6"	4"	18-1/2"	36"	54"	72"	82"
8'-0"	4"	21-1/2"	39"	57"	75"	88"

SPECIFICATIONS AND NOTES

1. DOORS AND HARDWARE WILL BE DESIGNED, MANUFACTURED AND INSTALLED WITH STANDARDS AS SET FORTH BY DASMA.
2. DOOR SECTIONS SHALL BE 27 GA. MIN. (.016") INTERIOR AND EXTERIOR ROLLED FORMED LIGHT COMMERCIAL QUALITY, G-40 GALVANIZATION
3. DOORS UPTO 7'0" HIGH CONSIST OF (4) SECTIONS AS SHOWN.
4. DOORS UPTO 8'0" HIGH CONSIST OF (5) SECTIONS AS SHOWN.
5. SUPPORTING STRUCTURAL ELEMENTS SHALL BE DESIGNED BY A REGISTRED PROFESSIONAL ENGINEER FOR WIND LOADS INDICATED ON THIS DRAWING IN ADDITION TO OTHER LOADINGS.
6. THE METHOD OF TESTING WAS IN SUBSTANTIAL CONFORMANCE WITH THE PROCEDURE DESCRIBED IN ASTM E330-90, AND THE SOUTHERN BUILDING CODE SECTION 1606 WIND LOAD DESIGN CRITERIA THE PRESSURES SHOWN ON THE DRAWINGS WERE CALCULATED USING THE FOLLOWING PARAMETERS:
   A. BASIC WIND SPEED OF 110 MPH
   B. DOOR CAN BE INSTALLED WITH 5 FEET OF DOORS WIDTH INSIDE THE EDGE STRIP.
   C. 15' MEAN ROOF HEIGHT AT ANY SLOPE
   D. USE FACTOR OF 1.0
   E. EXPOSURE RATING OF C

MAX SIZE 16' x 8'
DESIGN LOADS +29.5 PSF -29.8 PSF
TEST LOADS +45.3 PSF -45.3 PSF

Amarr
MODEL #1500 WeatherGuard
SIZE B
SBC-580-019-J
SCALE NOT TO SCALE
SHEET 1 OF 1

Architectural Testing

AAMA/WDMA 101/I.S. 2-97
TEST REPORT

Rendered to:

JORDAN COMPANIES

SERIES/MODEL: 8500
TYPE: PVC Single Hung Window

Title of Test	Results
AAMA/WDMA Rating	H-R40 (44 x 84)
Uniform Load Deflection Test Pressure	± 40.0 psf
Operating Force	10 lbs max.
Air Infiltration	0.21 cfm/ft^2
Water Resistance Test Pressure	6.00 psf
Uniform Load Structural Test Pressure	± 60.0 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

Reference should be made to full report for test specimen description and data.

Report No: 02-48976.02
Report Date: 02-26-04
Expiration Date: 02-25-08

848 Western Avenue North
Saint Paul, Minnesota 55117-5245
phone: 651.636.3835
fax: 652.636.3843
www.architest.com

Architectural Testing

AAMA/WDMA 101/I.S.2-97 TEST REPORT

Rendered to:

JORDAN COMPANIES
P.O. Box 18377
Memphis, Tennessee 38118

Report No: 02-48976.02
Test Date: 02/25/04
Report Date: 02/26/04
Expiration Date: 02/25/08

Project Summary: Architectural Testing, Inc. (ATI) was contracted by Jordan Companies to perform tests on a Jordan Companies Series 8500 Single Hung Window. The sample tested successfully met the performance requirements for a H-R40 44 x 84 rating. Test specimen description and results are reported herein.

Test Procedure: The test specimen was evaluated in accordance with AAMA/NWDMA 101/I.S. 2-97, *"Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors."*

Test Specimen Description:

Series/Model: 8500

Type: PVC Single Hung Window

Overall Size: 3' 8" wide by 7' 0" high

Sash Size: 3' 4-3/8" wide by 2' 5" high

Fixed D.L.O. Size: 3' 4-3/4" wide by 4' 5" high

Screen Size: 3' 4-3/4" wide by 2' 4-1/4" high

Finish: All PVC was white

849 Western Avenue North
Saint Paul, Minnesota 55117-5245
phone: 651.636.3835
fax: 652.636.3843
www.architest.com

Test Specimen Description: (Continued)

Glazing Type: The window utilized nominal 3/4" insulating glass comprised of two single-strength annealed sheets in the operating sash and two double-strength sheets in the fixed lite and a desiccant-filled metal spacer system. The glass for the fixed area was set from the interior into a bed of silicone sealant with PVC stops used on the interior. The sash was glazed from the exterior into a bed of silicone sealant with PVC stops used on the exterior.

Weatherstripping:

Description	Quantity	Location
0.260" high by 0.187" backed pile with center fin	1 Row	Sash top and bottom rails
0.260" high by 0.187" backed pile with center fin	2 Rows	Sash stiles

Frame Construction: Frame corners were miter-cut and welded. Aluminum reinforcement was utilized in the fixed meeting rail (Jordan part number H-2447).

Sash Construction: Sash corners were miter-cut and welded. Aluminum reinforcement was utilized in the top rail (Jordan part number H-2448).

Hardware:

Metal cam locks with keepers	2	6" from ends and meeting rail
Plastic tilt latches	2	Sash top rail corners
Metal tilt pins	2	Sash bottom rail corners
Block-and-tackle balances	2	One per jamb

Drainage:

3/16" by 5/8" slots	2	1-3/4" from ends in sill pocket to hollow below
1/8" by 1/2" slots	4	1-3/4" and 2" from each end through sill exterior face

Installation: The unit was installed into a Grade 2 SPF 2" by 8" wood test buck secured through the flange with 1-5/8" screws spaced 4" from corners and 8" on center. The nail fin was sealed to the buck with silicone.

Test Results: The results are tabulated as follows.

Paragraph	Title of Test	Results	Allowed
2.2.1.6.1	Operating Force		
	Force to initiate motion	10 lbs	30 lbs max.
	Force to keep in motion	8 lbs	30 lbs max.
2.1.2	Air Infiltration per ASTM E 283-97 (See Note #1)		
	@ 1.57 psf (25 mph)	0.21 cfm/ft^2	0.30 cfm/ft^2

Note #1: The tested specimen meets the performance levels specified in AAMA/WDMA 101/I.S.2-97 for air infiltration.

2.1.3	Water Resistance per ASTM 547-97 (See Note #2)		
2.1.4.1	Uniform Load Deflection per ASTM E 330-97 (See Note #2)		
2.1.4.2	Uniform Load Structural per ASTM E 330-97 (See Note #2)		

Note #2: The client opted to start at a pressure higher than the minimum required. Those results are listed under "Optional Performance."

2.2.1.6.2	Deglazing Test per ASTM E 987		
	In operating direction @ 70 lbs		
	Top rail	0.04"/ 8%	0.500"/100%
	Bottom rail	0.06"/12%	0.500"/100%
	In remaining direction @ 50 lbs		
	Left stile	0.04"/8%	0.500"/100%
	Right stile	0.03"/6%	0.500"/100%
2.1.7	Corner Weld Test	Meets as stated	Meets as stated
2.1.8	Forced Entry Resistance per ASTM F 588-97		
	Type A		
	Grade 10		
	Lock Manipulation Test	No entry	No entry
	Tests A1 through A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry

Test Results: (Continued)

Paragraph	Title of Test	Results	Allowed
Optional Performance:			
4.3	Water Resistance per ASTM E 547-97 WTP = 6.00 psf	No leakage	No leakage
4.4.1	Uniform Load Deflection per ASTM E 330-97 (See Note #3) (Measurements reported were taken on the meeting rail) (Loads were held for 60 seconds)		
	@ 40.0 psf (positive)	0.45"	(See Note #3)
	@ 40.0 psf (negative)	0.52"	(See Note #3)
4.4.2	Uniform Load Structural per ASTM E 330-97 (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds)		
	@ 60.0 psf (positive)	0.03"	0.16" max.
	@ 60.0 psf (negative)	0.03"	0.16" max.

Note #3: The Uniform Load Deflection test is not a AAMA/NWWDA 101/I.S. 2-97 requirement for this product designation. The data is recorded in this report for information only.

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator. This report may not be reproduced except in full without the approval of Architectural Testing, Inc.

For ARCHITECTURAL TESTING, INC.

Digitally Signed by: Paul L. Spiess

Paul L. Spiess
Project Manager

Digitally Signed by: Daniel A. Johnson

Daniel A. Johnson
Regional Manager

DAJ/jb
02-48976.02

AAMA/NWWDA 101/I.S.2-97
TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 3500 Twin
TYPE: Mulled PVC Single Hung Window

Title	Summary of Results
AAMA Rating	H-R15 96 x 78
Operating Force	19 lb max.
Air Infiltration	0.10 cfm/ft^2
Water Resistance Test Pressure	5.25 psf
Uniform Load Deflection Test Pressure	15.0 psf
Uniform Load Structural Test Pressure	22.5 psf
Forced Entry Resistance	Grade 10

Reference should be made to ATI Report No. 01-45879.01 for complete test specimen description and data.

Architectural Testing

AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.
P.O. Box 370
650 West Market Street
Gratz, Pennsylvania 17030-0370

Report No: 01-[illegible]5879.01
Test Date: 06/03/03
And: 06/04/03
Report Date: 06/24/03
Expiration Date: 06/03/07

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to perform tests on a Series/Model 3500 Twin, mulled PVC single hung window at their test facility in Elizabethville, Pennsylvania. The sample tested successfully met the performance requirements for an H-R15 96 x 78 rating.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 3500 Twin

Type: Mulled PVC Single Hung Window

Overall Size: 8' 0-1/4" wide by 6' 6-1/8" high

Active Sash Size (2): 3' 10" wide by 3' 2-1/2" high

Fixed Daylight Opening Size (2): 3' 8" wide by 2' 11-1/4" high

Screen Size: 3' 9" wide by 3' 1-3/4" high

Finish: All PVC was white.

Glazing Details: The window utilized 7/8" thick sealed insulating glass constructed from two sheets of 3/32" thick clear annealed glass and a metal reinforced butyl spacer system. The lites were interior glazed onto single-sided adhesive foam tape and secured with PVC snap-in glazing beads.

Test Specimen Description: (Continued)

Weatherstripping:

Description	Quantity	Location
3/16" round foam filled vinyl bulb seal	1 Row	Fixed meeting rail
0.250" high by 0.187" backed polypile with center fin	1 Row	Interior vertical sill leg, interior meeting rail and stiles
1/4" round foam filled vinyl bulb seal with single leaf	1 Row	Bottom rail
0.310" high by 0.187" backed polypile with center fin	1 Row	Stiles

Frame Construction: The frame was constructed of extruded vinyl with mitered and welded corners. End caps were utilized on the ends of the meeting rails and secured with two #6 x 5/8" screws per cap. The fixed meeting rails were then secured to the frame utilizing two #6 x 5/8" screws. The windows were mulled together utilizing interior and exterior snap-in caps. Silicone was utilized at the head and sill mullion points.

Sash Construction: The sash were constructed of extruded vinyl with mitered and welded corners.

Screen Construction: The screen frames were constructed of roll-formed aluminum with keyed corners. The fiberglass mesh was secured with a flexible vinyl spline.

Hardware:

Description	Quantity	Location
Metal sweep locks with keepers	4	7" from stiles on interior meeting rails with keepers adjacent
Constant force balance assembly	4	One per jamb
Tilt latch	4	Each end of active meeting rails
Metal tilt pin	4	Each end of bottom rails
Tension springs	4	5" from top rail ends of screens

Test Specimen Description (Continued)

Drainage:

Description	Quantity	Location
1/8" wide by 1" long weephole	4	2-1/4" from sill ends and mullion on sill face
3/16" wide by 1/2" long weephole	8	Two per corner through sill interior walls
1/16" wide by 1/2" long weephole	8	Two 2-1/2" from bottom rail ends
3/16" wide by 1/2" long weephole	4	1-1/4" from jambs in bottom rail glazing channels

Reinforcement: Sash rails contained a roll-formed steel "I" reinforcement (drawing #GV1-451-020). The fixed meeting rail contained a roll-formed steel reinforcement (drawing #RF-1045-020).

Installation: The windows were installed into a #2 Spruce-Pine-Fir wood buck. The nail fin was back bedded in silicone and secured utilizing #8 x 1-5/8" drywall screws located in the corners and 9" on center around the nail fin perimeter.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.1	Operating Force	19 lbs	20 lbs max.
2.1.2	Air Infiltration (ASTM E 283-91) @ 1.57 psf (25 mph)	0.10 cfm/ft^2	0.3 cfm/ft^2 max.

Note #1: The tested specimen meets the performance levels specified in AAMA/NWWDA 101/I.S.2-97 for air infiltration.

2.1.3	Water Resistance (ASTM E 547-00) (with and without screen) WTP – 2.86 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection (ASTM E 330-97) (Deflections reported were taken on the mullion) (Loads were held for 52 seconds) @ 15.0 psf (positive) @ 15.0 psf (negative)	 1.27" 1.18"	 See Note #2 See Note #2

Note #2: The Uniform Load Deflection test is not an AAMA/NWWDA 101/I.S.2-97 requirement for this product designation. The data is recorded in this report for information only.

st Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
2.1.4.2	Uniform Load Structural (ASTM E 330-97) (Permanent Sets reported were taken on the mullion) (Loads were held for 10 seconds)		
	@ 22.5 psf (positive)	0.17"	0.29" max.
	@ 22.5 psf (negative)	0.18"	0.29" max.
2.2.1.6.2	Deglazing Test (ASTM E 987-88) In operating direction at 70 lbs		
	Right sash, meeting rail	0.13"/25%	0.50" 100%
	Right sash, bottom rail	0.13"/25%	0.50" 100%
	Left sash, meeting rail	0.13"/25%	0.50" 100%
	Left sash, bottom rail	0.13"/25%	0.50" 100%
	In remaining direction at 50 lbs		
	Right sash, right stile	0.06"/13%	0.50"/100%
	Right sash, left stile	0.06"/13%	0.50"/100%
	Left sash, right stile	0.03"/6%	0.50"/100%
	Left sash, left stile	0.03"/6%	0.50"/100%
2.1.7	Welded Corner Test	Meets as stated	Meets as stated
2.1.8	Forced Entry Resistance (ASTM F 588-97)		
	Type: A Grade: 10		
	Lock Manipulation Test	No entry	No entry
	Test A1 thru A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry
Optional Performance			
4.3	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 5.25 psf	No leakage	No leakage

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product which may only be granted by the certification program administrator. This report may not be reproduced except in full without the approval of Architectural Testing.

For: ARCHITECTURAL TESTING, INC:

Mark A. Hess
Technician

Steven M. Urich, P.E.
Senior Project Engineer

JH:baw
45879.01

MIAMI-DADE

BUILDING CODE COMPLIANCE OFFICE (BCCO)
PRODUCT CONTROL DIVISION

MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901 FAX (305) 375-2908

NOTICE OF ACCEPTANCE (NOA)

Therma-Tru Corporation
108 Mutzfeld Road
Butler, IN 46721

SCOPE:

This NOA is being issued under the applicable rules and regulations governing the use of construction materials. The documentation submitted has been reviewed by Miami-Dade County Product Control Division and accepted by the Board of Rules and Appeals (BORA) to be used in Miami Dade County and other areas where allowed by the Authority Having Jurisdiction (AHJ).

This NOA shall not be valid after the expiration date stated below. The Miami-Dade County Product Control Division (In Miami Dade County) and/or the AHJ (in areas other than Miami Dade County) reserve the right to have this product or material tested for quality assurance purposes. If this product or material fails to perform in the accepted manner, the manufacturer will incur the expense of such testing and the AHJ may immediately revoke, modify, or suspend the use of such product or material within their jurisdiction. BORA reserves the right to revoke this acceptance, if it is determined by Miami-Dade County Product Control Division that this product or material fails to meet the requirements of the applicable building code.

This product is approved as described herein, and has been designed to comply with the High Velocity Hurricane Zone of the Florida Building Code.

DESCRIPTION: Series "Landmark" 6'8" W/E Outswing Opaque Insulated Steel Door – Impact Resistant

APPROVAL DOCUMENT: Drawing No. S-2189, dated 11/27/01, with revision 1 dated 01/30/02, titled "Landmark Woodedge Opaque Single 6'8 Outswing Door in Wood Frame", sheets 1 through 5, prepared by R. W. Building Consultants, Inc., bearing the Miami-Dade County Product Control Approval stamp with the Notice of Acceptance number and approval date by the Miami-Dade County Product Control Division.

MISSILE IMPACT RATING: Large and Small Missile Impact Resistant

LABELING: Each unit shall bear a permanent label with the manufacturer's name or logo, city, state and following statement: "Miami-Dade County Product Control Approved", unless otherwise noted herein.

RENEWAL of this NOA shall be considered after a renewal application has been filed and there has been no change in the applicable building code negatively affecting the performance of this product.

TERMINATION of this NOA will occur after the expiration date or if there has been a revision or change in the materials, use, and/or manufacture of the product or process. Misuse of this NOA as an endorsement of any product, for sales, advertising or any other purposes shall automatically terminate this NOA. Failure to comply with any section of this NOA shall be cause for termination and removal of NOA.

ADVERTISEMENT: The NOA number preceded by the words Miami-Dade County, Florida, and followed by the expiration date may be displayed in advertising literature. If any portion of the NOA is displayed, then it shall be done in its entirety.

INSPECTION: A copy of this entire NOA shall be provided to the user by the manufacturer or its distributors and shall be available for inspection at the job site at the request of the Building Official.

This NOA consists of this page 1 as well as approval document mentioned above.

The submitted documentation was reviewed by **Manuel Perez, P.E.**

THERMA-TRU®

"LANDMARK SERIES"

6'8 OUTSWING INSULATED WOOD EDGE STEEL DOOR WITH WOOD FRAME

GENERAL NOTES

1. THIS PRODUCT IS DESIGNED TO COMPLY WITH THE FLORIDA BUILDING CODE.
2. WOOD BUCKS BY OTHERS, MUST BE ANCHORED PROPERLY TO TRANSFER LOADS TO THE STRUCTURE.
3. PRODUCT ANCHORS SHALL BE AS LISTED AND SPACED AS SHOWN ON DETAILS. ANCHOR EMBEDMENT TO BASE MATERIAL SHALL BE BEYOND WALL DRESSING OR STUCCO.
4. DESIGNED PRESSURE RATING SEE TABLE PAGE 1.
5. THIS PRODUCT MEETS THE WATER REQUIREMENTS FOR "HIGH VELOCITY HURRICANE ZONES".

RESIDENTIAL INSULATED STEEL DOOR
(Common to all frame conditions)

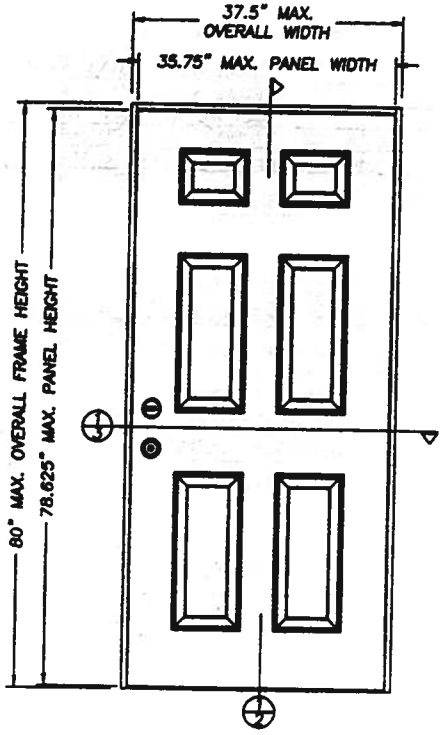
Door Construction:

Door Face Sheets: 25 GA. (0.019") minimum thickness, Galvanized steel A-525 commercial quality – AKDQ per ASTM 620 with yield strength Fy(min.)=23,300 psi

Core design: Polyurethane foam core, with 1.9 lbs. density by BASF.

Door Panel Construction: Flush or embossed type. The vertical edges of the skin, rolled formed to provide a mechanical interlock with finger jointed pine stiles. Wood end rails are butt jointed and pressure fitted with contact cement to the wood stiles at the corners.

Frame Construction: The frame is constructed from finger jointed Ponderosa Pine measuring 4.5625" wide x 1.25" thick. The header is joined to the side jambs with (3) 16ga. 1/2" crown x 2" long staples at each side. The threshold is joined to the side jambs with (2) 16ga. 1/2" crown x 2.5" long staples at each side. The units use an Outswing Bumpface threshold measuring 4.625" long x 1.0" high or a High Dam threshold measuring 5.25" long x 1.75" high.



3-0 x 6-8 OUTSWING
ELEVATION
VIEWED FROM INTERIOR

TABLE OF CONTENTS	
SHEET #	DESCRIPTION
1	TYPICAL ELEVATIONS & GENERAL NOTES
2	VERTICAL CROSS SECTIONS & BILL OF MATERIALS
3	HORIZONTAL CROSS SECTION
4	ANCHORING LOCATIONS & DETAILS
5	UNIT COMPONENTS & DOOR MODELS

DESIGN PRESSURE RATING	
UNIT TYPE	WHERE WATER INFILTRATION REQUIREMENT IS NEEDED
SINGLE (with high dam threshold)	+ 60.0 PSF – 60.0 PSF
SINGLE (with standard threshold)	+ 55.0 PSF – 55.0 PSF

Approved as complying with the Florida Building Code
Date June 6, 2007
NOA# 01-1219.05
Miami Dade Product Control Division
By

THERMA-TRU CORPORATION
1687 WOODLANDS DRIVE
MAUMEE, OHIO
PH. (800) 537-8827

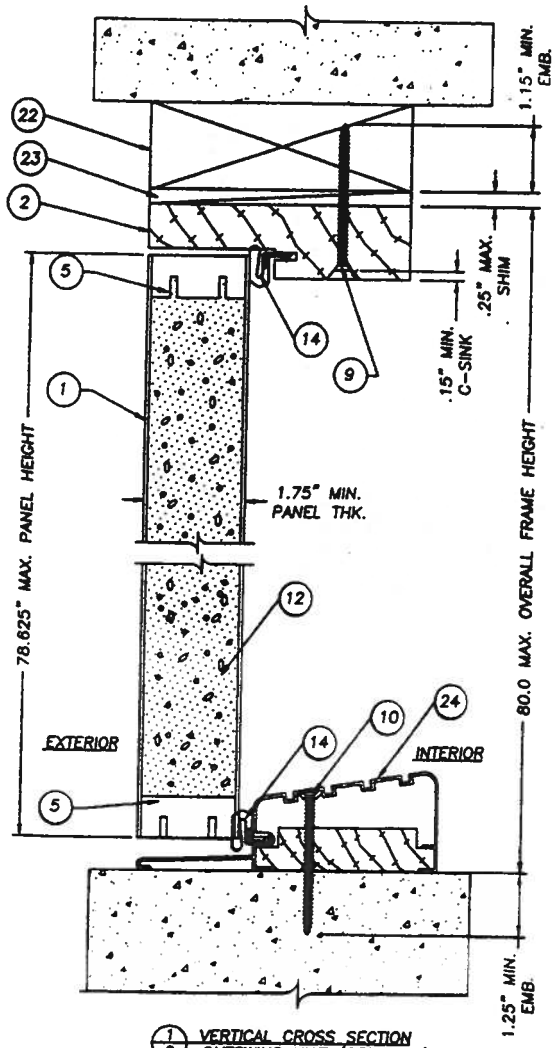
PRODUCT: LANDMARK WOODEDGE OPAQUE SINGLE 6'8 OUTSWING DOOR IN WOOD FRAME

PART OR ASSEMBLY: TYPICAL ELEVATION & GENERAL NOTES

NO	DATE	REVISIONS	BY
1	1/30/02	GENERAL REVISION	WLN

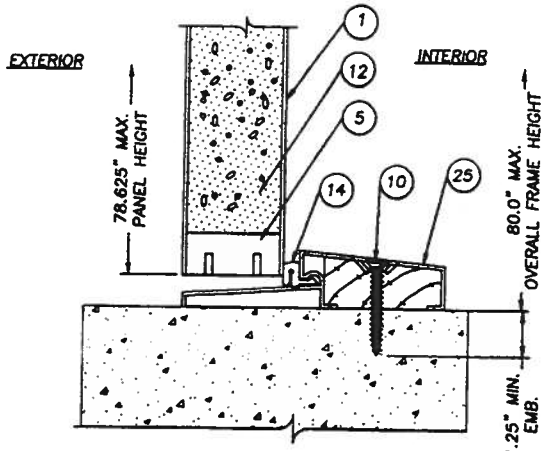
RW BUILDING CONSULTANTS, INC.
813.684.3831

DATE: 11/27/01
SCALE: NTS
DWG. BY: WLN
CHK. BY: RW
DRAWING NO.: S-2189
SHEET 1 OF 5



1/2 VERTICAL CROSS SECTION
OUTSWING UNIT (OPTION 1)
HIGH DAM THRESHOLD
(SEE DESIGN PRESURE TABLE)

Item	DESCRIPTION	Material
1	DOOR SKIN (25 GA. .018" MIN.)	STEEL
2	4 9/16" HEADER	WOOD
3	4 9/16" LATCH JAMB	WOOD
4	4 9/16" HINGE JAMB	WOOD
5	TOP AND BOTTOM RAIL	WOOD
6	RADIUS & SQ. CORNER LEAF 4" X 4" HINGE	STEEL
7	#10 x 3/4" lg. (Hinge to Jamb)	STEEL
8	NOT USED	
9	#8 x 2-1/2" PFH WS	STEEL
10	3/16" TAPCON ANCHOR (ELCO)	STEEL
11	NOT USED	-
12	BASF FOAM CORE	
13	#10 x 3/4" PFH WS (Hinge to Door)	STEEL
14	COMPRESSION WEATHER-STRIP (Therma-Tru)	
15	NOT USED	-
16	#8 x 1/2" LG. TYPE "AB" PANHEAD	STEEL
17	NOT USED	STEEL
18	KWIKSET 700 SERIES PASSAGE	
19	KWIKSET 700 SERIES DEADBOLT	
20	LATCH & DEADBOLT WOODBLOCK 3.0" WIDE x 3.5" HIGH	WOOD
21	#10 x 2" LG. PFH WOOD SCREW (Hinge to Jamb)	STEEL
22	2x WOOD SUB BUCK	WOOD
23	MAX. 1/4" SHIM MATERIAL	WOOD
24	HIGH DAM THRESHOLD (Imperial)	ALUMINUM
25	STANDARD BUMP THRESHOLD	ALUMINUM
26	HINGE STYLE	WOOD
27	LATCH STYLE	WOOD



1/2 VERTICAL CROSS SECTION
OUTSWING UNIT (OPTION 2)
STANDARD BUMP THRESHOLD
(SEE DESIGN PRESURE TABLE)

Approved as complying with the Florida Building Code
Date: June 6, 2002
NOA# 01-1219.08
Miami Dade Product Control Division
By: [signature]

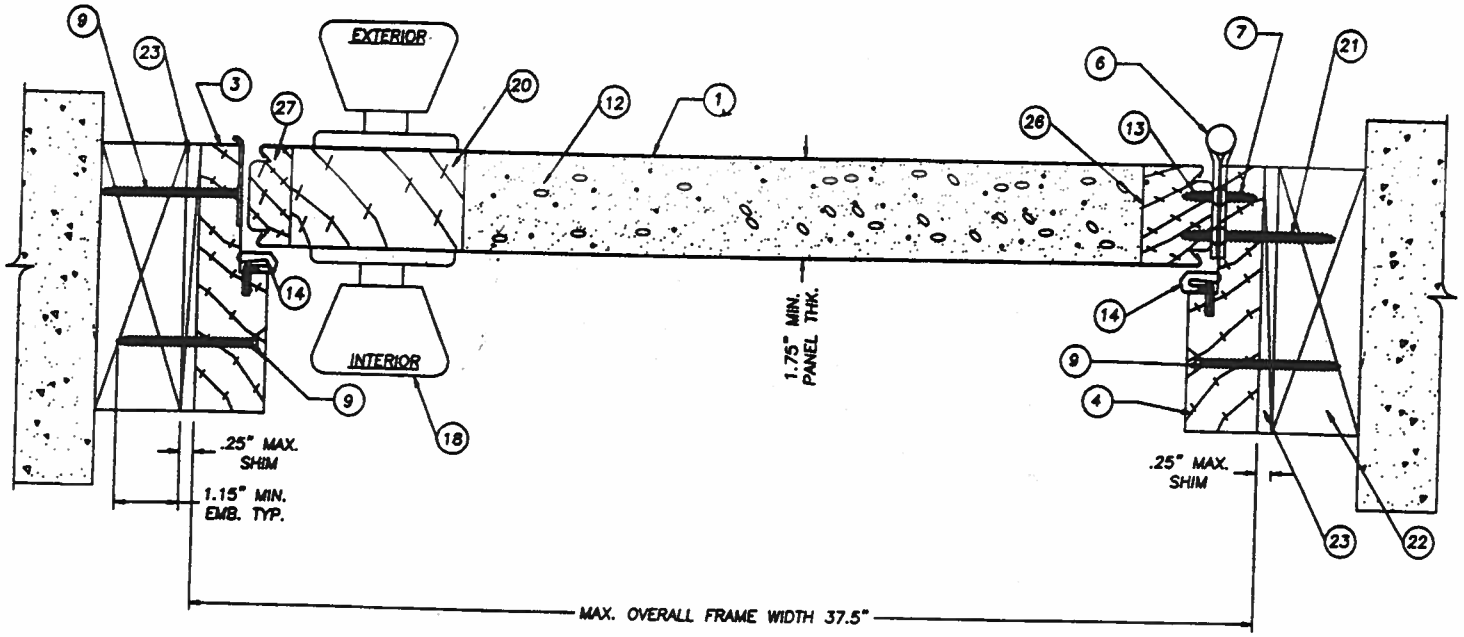
THERMA-TRU CORPORATION
1687 WOODLANDS DRIVE
MAUMEE, OHIO
PH. (800) 537-8827

PRODUCT: LANDMARK WOODEDGE OPAQUE SINGLE 6'8" OUTSWING DOOR IN WOOD FRAME
PART OR ASSEMBLY: VERTICAL CROSS SECTION

NO.	DATE	REVISIONS	BY
1	1/30/02	GENERAL REVISION	WN

RW BUILDING CONSULTANTS, INC.
813.684.3831

DATE: 11/27/01
SCALE: NTS
DWG. BY: WLN
CHK. BY: RW
DRAWING NO.: S-2189
SHEET 2 OF 5



1/3 HORIZONTAL CROSS SECTION
HINGE & LATCH JAMB TO BUCK

THERMA-TRU CORPORATION
1687 WOODLANDS DRIVE
MAUMEE, OHIO
PH. (800) 537-8827

PRODUCT: LANDMARK WOODEDGE OPAQUE SINGLE 6'8 OUTSWING DOOR IN WOOD FRAME

PART OR ASSEMBLY: HORIZONTAL CROSS SECTION

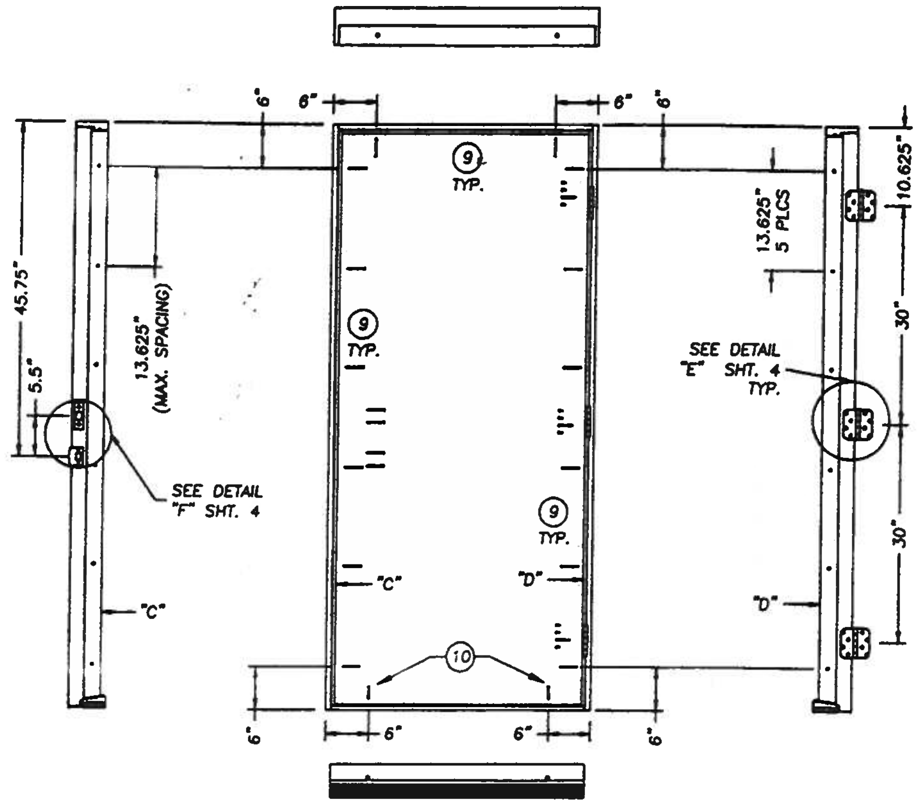
NO	DATE	GENERAL REVISION	BY
1	1/30/02		WN

REVISIONS

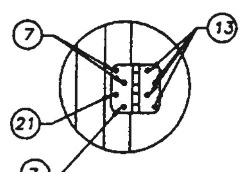
RW BUILDING CONSULTANTS, INC.
813.684.3831

Approved as complying with the Florida Building Code
Date June 6, 2002
NOA# 01-1219.05
Miami Dade Product Control Division
By

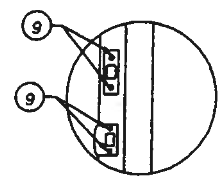
DATE: 11/28/01
SCALE: NTS
DRG. BY: WLN
CHK. BY: RW
DRAWING NO.: 5-2189
SHEET 3 OF 5



3-0 x 6-8 OUTSWING
ANCHORING LOCATIONS



DETAIL "E"



DETAIL "F"

THERMA-TRU CORPORATION
1687 WOODLANDS DRIVE
MAUMEE, OHIO
PH. (800) 537-8827

PRODUCT:
"LANDMARK WOODEDGE OPAQUE" SINGLE 6'8 OUTSWING IN WOOD FRAME

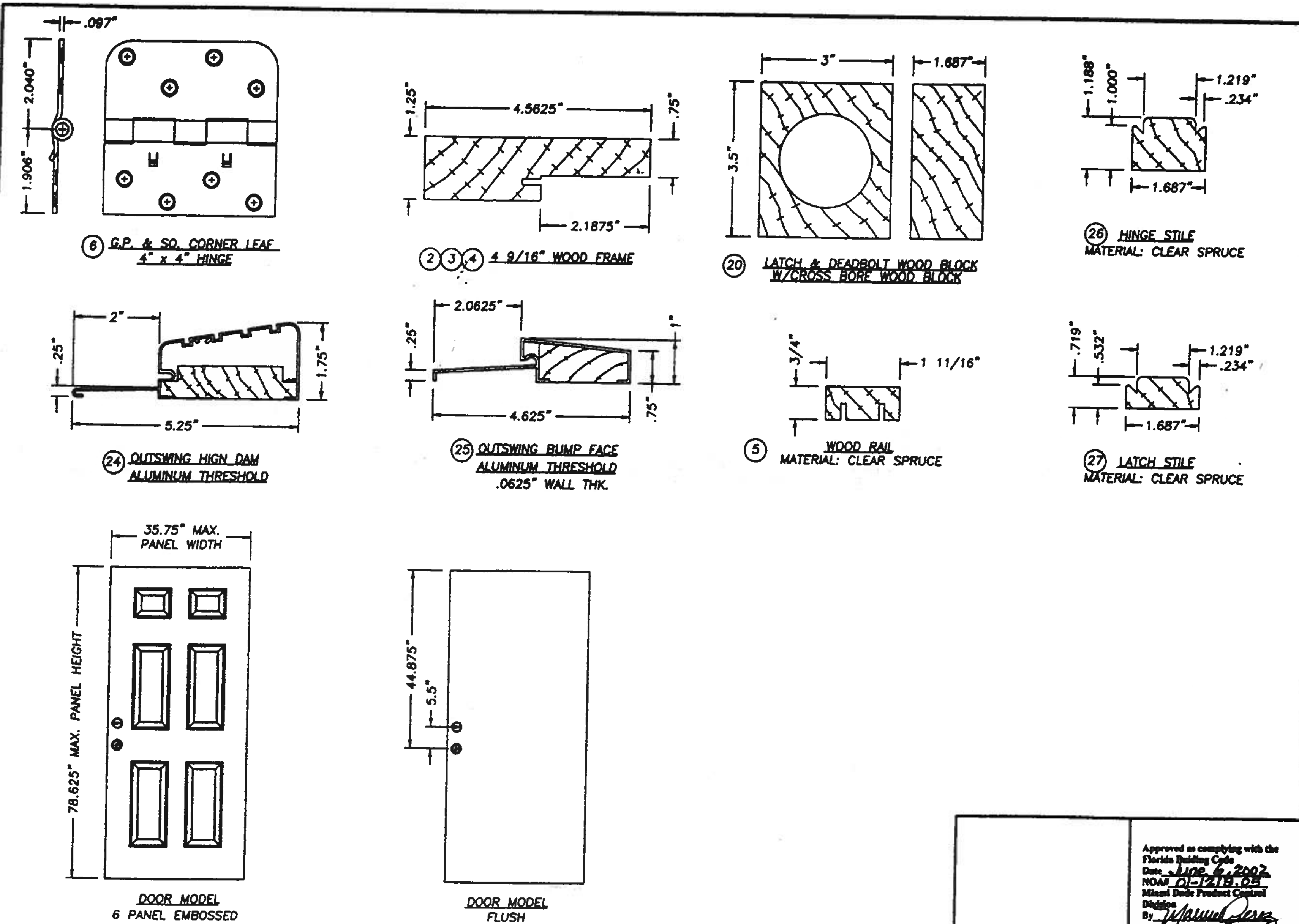
PART OR ASSEMBLY:
ANCHORING LOCATIONS & DETAILS

NO.	DATE	REVISIONS	BY
1	1/30/02	GENERAL REVISION	WN

RW BUILDING CONSULTANTS, INC.
813.684.3831

DATE: 11/27/01
SCALE: NTS
DWG. BY: WLN
CHK. BY: RW
DRAWING NO.: S-2189
SHEET 4 OF 5

Approved as complying with the Florida Building Code
Date June 6 2002
NOA# 01-1219.05
Miami Dade Product Control Division
By [signature]



Approved as complying with the Florida Building Code
Date June 6, 2002
NOA# 01-1219.05
Miami Dade Product Control Division
By [signature]

1	1/30/02	GENERAL REVISION	WN
NO.	DATE		BY

REVISIONS

RW BUILDING CONSULTANTS, INC. 813.684.3831

DATE: 11/28/01
SCALE: NTS
DWG. BY: WLN
CHK. BY: RW
DRAWING NO.: S-2189
SHEET 5 OF 5

PRODUCT: "LANDMARK WOODEDGE OPAQUE" SINGLE 6'8 OUTSWING DOOR IN WOOD FRAME

PART OR ASSEMBLY: UNIT COMPONENTS & DOOR MODELS

THERMA-TRU CORPORATION
1687 WOODLANDS DRIVE
MAUMEE, OHIO
PH. (800) 537-8827

** LAMAR BOOZER **		PROJECT:	CUSTOM
900 EAST PUTNAM STREET		CLIENT:	NORTON BUILDERS
LAKE CITY, FL 32055		DATE:	12 4 06
RESIDENTIAL/LIGHT COMMERCIAL HVAC LOADS		DESIGNER:	LAMAR BOOZER

CLIENT INFORMATION:

NAME: NORTON BUILDERS
ADDRESS:
CITY, STATE: LAKE CITY FL

TOTAL BUILDING LOADS:

BLDG. LOAD DESCRIPTIONS	AREA QUAN	SEN. LOSS	LAT. + GAIN	SEN. = GAIN	TOTAL GAIN
3-C WINDOW DBL PANE CLR GLS METL FR	133	4,342	0	3,862	3,862
9-I FRENCH DOOR DBL CLR GLS METL FR	42	1,425	0	689	689
12-D WALL R-11 +1/2"ASPHLT BRD(R-1.3)	1,497	5,388	0	2,946	2,946
13-C PART R-11 + 1/2" GYPSUM(R-0.5)	112	227	0	161	161
11-C DOOR METAL POLYSTYRENE CORE	42	888	0	486	486
16-G CEILING R-30 INSULATION	1,915	2,824	0	2,824	2,824
22-A SLAB ON GRADE NO EDGE INSUL	182	6,633	0	0	0
SUBTOTALS FOR STRUCTURE:	3,923	21,727	0	10,968	10,968
PEOPLE	19	0	4,370	5,700	10,070
APPLIANCES	0	0	0	1,500	1,500
DUCTWORK	0	2,018	0	2,203	2,203
INFILTRATION W.CFM: 376.5 S.CFM: 167.3	0	18,637	5,576	3,865	9,441
VENTILATION W.CFM: 0.0 S.CFM: 0.0	0	0	0	0	0
SENSIBLE GAIN TOTAL				24,236	
TEMP. SWING MULTIPLIER				X 1.00	
BUILDING LOAD TOTALS		42,381	9,946	24,236	34,182

SUPPLY CFM AT 20 DEG DT:	1,102	CFM PER SQUARE FOOT:	0.579
SQUARE FT. OF ROOM AREA:	1,915	SQUARE FOOT PER TON:	667.720

TOTAL HEATING REQUIRED WITH OUTSIDE AIR: 42.381 MBH
TOTAL COOLING REQUIRED WITH OUTSIDE AIR: 2.849 TONS

CALCULATIONS ARE BASED ON 7TH EDITION OF ACCA MANUAL J.
ALL COMPUTED RESULTS ARE ESTIMATES AS BUILDING USE AND WEATHER MAY VARY.
BE SURE TO SELECT A UNIT THAT MEETS BOTH SENSIBLE AND LATENT LOADS.

LYNCH WELL DRILLING, INC.

173 SW Tustenuggee Ave
Lake City, FL. 32025
Phone 386-752-6677
Fax 386-752-1477

Piccadilly Park S. Lot 13 Block D

Building Permit #_____ Owner's Name Norton Home Improvement

Well Depth_____ Ft. Casing Depth _____Ft. Water Level _____Ft

Casing Size 4 inch Steel Pump Installation: Deep Well Submersible

Pump Make Schaffer Pump Model _____ HP 1

System Pressure (PSI) _____ On 30 Off 50 Average Pressure 40

Pumping System GPM at average pressure and pumping level 18 GPM

Tank Installation: (Bladder) Galvanized Make Challenger
Model PC244 Size 81

Tank Draw-down per cycle at system pressure 25.1 Gallons

I HEREBY VERTIFY THAT THIS WATER WELL SYSTEM HAS BEEN INSTALLED AS PER THE ABOVE INFORMATION.

Linda Newcomb
Signature

Linda Newcomb
Print Name

2609
License Number

12-5-06
Date

Dale C. Johns, P.E.
437 SW Thurman Terrace
Lake City, Fl 32024
PH 386.961.8903

Jan. 9, 07

To: Columbia County Building and Zoning

Subject: Finished floor Norton Home Imp. permitted by Charese Norton

To whom it may concern:

The lot owned by Norton Home Imp. permitted by Charese Norton (permit no. 25311), is out of the100 year flood plain. The home will cause no impact to the 100 year flood elevation and have no flooding. The finished floor is 7.25" above the road on the west end and 19" above the road on the east end due to the vertical curve in the road. The water will over top the road before any flooding could occur.

Thanks for your help on this subject. If you have any questions, please call at 386-961-8903 or cell 386-365-3250.

Sincerely,

Dale C Johns

Dale C. Johns, P.E.
PE # 45263

Notice of Treatment

Applicator: Florida Pest Control & Chemical Co. (www.flapest .com)
Address: 536 SE Baya
City Lake City Phone 752-1103

Site Location: Subdivision _____
Lot # _____ Block# _____ Permit # 25311
Address 237 SW Briarbrook pl

Product used	Active Ingredient	% Concentration
☐ Premise	Imidacloprid	0.1%
☒ Termidor	Fipronil	0.12% .06%
☐ Bora-Care	Disodium Octaborate Tetrahydrate	23.0%

Type treatment: ☒ Soil ☐ Wood

Area Treated	Square feet	Linear feet	Gallons Applied
Trenched + Treated + Drilled Pad's All			92

As per Florida Building Code 104.2.6 – If soil chemical barrier method for termite prevention is used, final exterior treatment shall be completed prior to final building approval.

If this notice is for the final exterior treatment, initial this line _____.

5/23/08 (Date) 8:35 (Time) Guy + Ron (Print Technician's Name)

Remarks: _____

Applicator - White Permit File - Canary Permit Holder - Pink
10/05 ©

CERTIFICATE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 25-4S-16-03121-055 **Building permit No.** 000025311

Use Classification SFD/UTILITY **Fire:** 19.26

Permit Holder JAMES H. NORTON **Waste:** 50.25

Owner of Building NORTON HOME IMP. CO., INC. **Total:** 69.51

Location: 237 SW BRIARBROOK PLACE, LAKE CITY, FL

Date: 07/03/2008

Harry Dicks

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)

Project Information for:	L213465		
Builder:	NORTON HOME	Date:	10/10/2006
Lot:	LOT 13 PICCADILLY PARK S(	Start Number:	1254
Subdivision:	N/A	SEI Ref:	L213465
County or City:	COLUMBIA COUNTY		
Truss Page Count:	39		

Truss Design Load Information (UNO) Design Program: MiTek

Gravity		Wind		Building Code:	FBC2004
Roof (psf):	42	Wind Standard:	ASCE 7-02		
Floor (psf):	55	Wind Speed (mph):	110		

Note: See individual truss drawings for special loading conditions

Building Designer, responsible for Structural Engineering: (See attached)

NORTON, JAMES H. RB 0031780
Address: RT 28 BOX 388A
HIGH SPRINGS, FL. 32643 Designer: 114

Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987
Company: Structural Engineering and Inspections, Inc. EB 9196
Address: 16105 N. Florida Ave, Ste B, Lutz, FL 33549 Phone: 813-849-5769

Notes:

1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
4. Trusses designed for veritcal loads only, unless noted otherwise.
5. Where hangers are shown, Carried Member hanger capacity per Simpson C-2006 (SYP/Full Nailing Value) as an individual component. Building Designer shall verify the suitablity and use of Carrying Member hanger capacity.

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	CJ1	1010061254	10/10/2006				
2	CJ3	1010061255	10/10/2006				
3	CJ5	1010061256	10/10/2006				
4	EJ5	1010061257	10/10/2006				
5	EJ5A	1010061258	10/10/2006				
6	EJ7	1010061259	10/10/2006				
7	HJ8	1010061260	10/10/2006				
8	HJ9	1010061261	10/10/2006				
9	T01	1010061262	10/10/2006				
10	T02	1010061263	10/10/2006				
11	T03	1010061264	10/10/2006				
12	T04	1010061265	10/10/2006				
13	T05	1010061266	10/10/2006				
14	T06	1010061267	10/10/2006				
15	T07	1010061268	10/10/2006				
16	T08	1010061269	10/10/2006				
17	T09	1010061270	10/10/2006				
18	T10	1010061271	10/10/2006				
19	T11	1010061272	10/10/2006				
20	T12	1010061273	10/10/2006				
21	T13	1010061274	10/10/2006				
22	T14	1010061275	10/10/2006				
23	T15	1010061276	10/10/2006				
24	T16	1010061277	10/10/2006				
25	T17	1010061278	10/10/2006				
26	T18	1010061279	10/10/2006				
27	T19	1010061280	10/10/2006				
28	T20	1010061281	10/10/2006				
29	T22	1010061282	10/10/2006				
30	T23	1010061283	10/10/2006				
31	T25	1010061284	10/10/2006				
32	T26	1010061285	10/10/2006				
33	T27	1010061286	10/10/2006				
34	T28	1010061287	10/10/2006				
35	T29	1010061288	10/10/2006				
36	T30	1010061289	10/10/2006				
37	T31	1010061290	10/10/2006				
38	T31G	1010061291	10/10/2006				
39	T32	1010061292	10/10/2006				

OCT 10 2006

Dwg.#1010061253

01:57:03 PM 10/6/2004

Licensee Details

Licensee Information

Name:	**NORTON, JAMES H (Primary Name)**
	NORTON HOME IMPROVEMENT COMPANY INC (DBA Name)
Main Address:	**RT 28 BOX 388A LAKE CITY, Florida 32025**
Lic. Location:	**RT 28 BOX 388A LAKE CITY, FL 32025 Columbia**

License Information

License Type:	**Registered Building Contractor**
Rank:	**Reg Building**
License Number:	**RB0031780**
Status:	**Current, Active**
Licensure Date:	**02/16/1978**
Expires:	**08/31/2005**

Special Qualifications	Effective Date
Bldg Code Core Course Credit	
Qualified Business License Required	02/20/2004

View Related License Information
View License Complaint

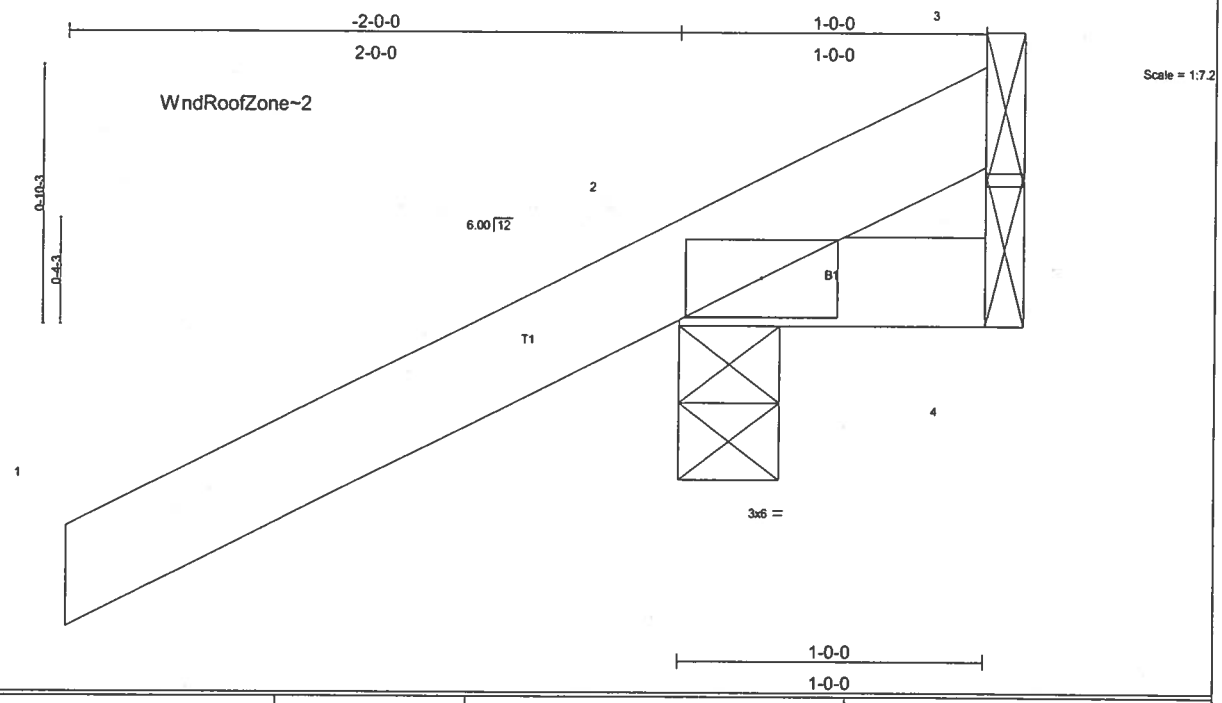
New Search Back

OCTOBER 10, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1010061254

Job	Truss	Truss Type	Qty	Ply	0 0
L213465	CJ1	MONO TRUSS	12	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Oct 10 13:51:06 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.28	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 7 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=267/0-4-0, 4=14/Mechanical, 3=-91/Mechanical
Max Horz 2=87(load case 5)
Max Uplift2=-287(load case 5), 4=-9(load case 3), 3=-91(load case 1)
Max Grav 2=267(load case 1), 4=14(load case 1), 3=128(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-69/76
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 287 lb uplift at joint 2, 9 lb uplift at joint 4 and 91 lb uplift at joint 3.

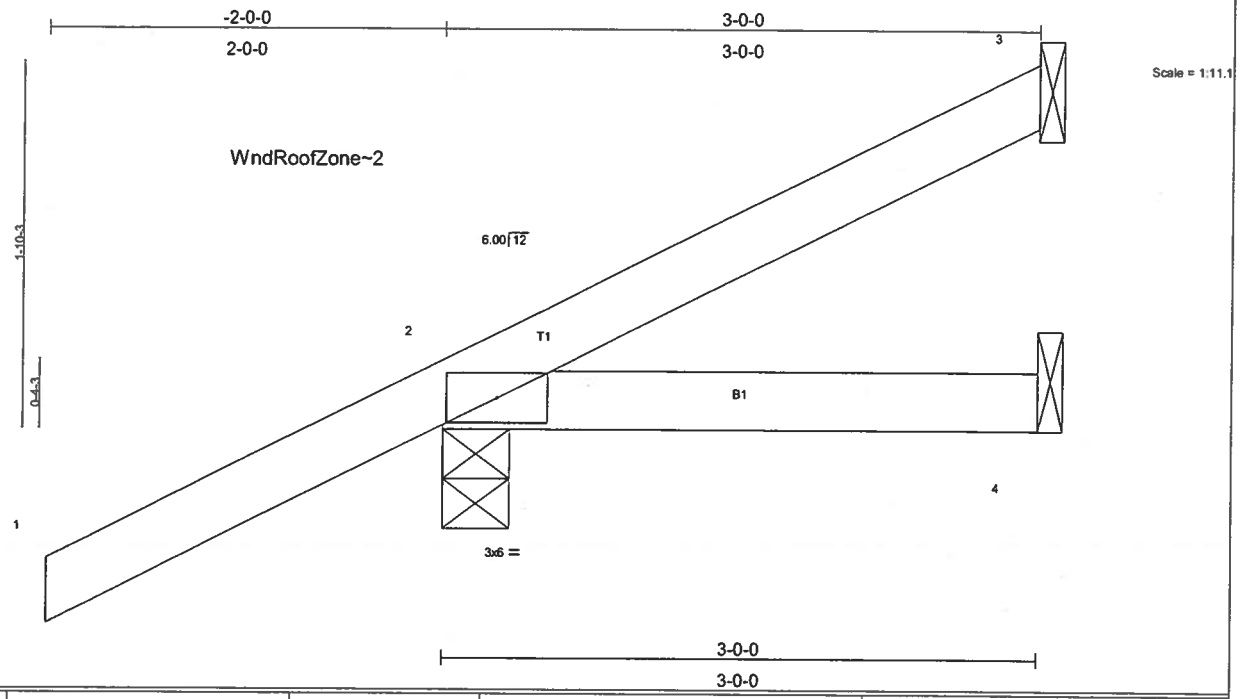
LOAD CASE(S) Standard

OCTOBER 10, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1010061255

Job	Truss	Truss Type	Qty	Ply	0 0
L213465	CJ3	JACK	12	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Oct 10 13:51:07 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	0.01	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	-0.01	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 13 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=29/Mechanical, 2=279/0-4-0, 4=42/Mechanical
Max Horz 2=132(load case 5)
Max Uplift3=-27(load case 6), 2=-240(load case 5), 4=-26(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-58/7
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 27 lb uplift at joint 3, 240 lb uplift at joint 2 and 26 lb uplift at joint 4.

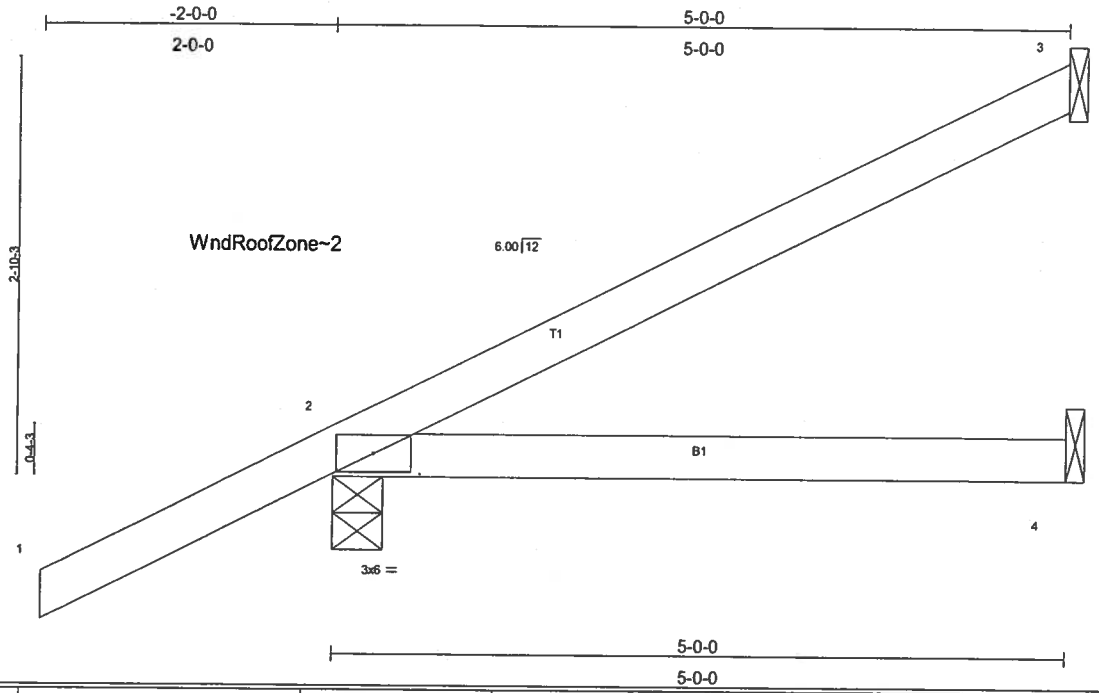
LOAD CASE(S) Standard

OCTOBER 10, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1010061256

Job	Truss	Truss Type	Qty	Ply	0 0
L213465	CJ5	JACK	12	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Oct 10 13:51:08 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	0.09	2-4	>671	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.24	Vert(TL)	0.07	2-4	>784	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 19 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=102/Mechanical, 2=344/0-4-0, 4=72/Mechanical
Max Horz 2=178(load case 5)
Max Uplift3=-86(load case 5), 2=-261(load case 5), 4=-46(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-87/36
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 86 lb uplift at joint 3, 261 lb uplift at joint 2 and 46 lb uplift at joint 4.

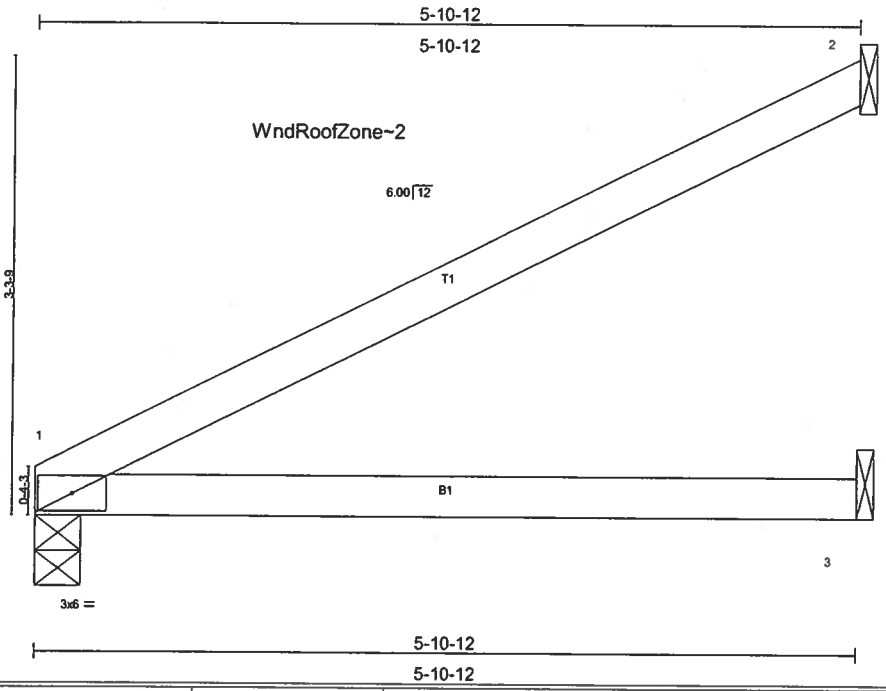
LOAD CASE(S) Standard

OCTOBER 10, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1010061257

Job	Truss	Truss Type	Qty	Ply	0 0
L213465	EJ5	JACK	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Oct 10 13:51:08 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.45	Vert(LL)	-0.05	1-3	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.23	Vert(TL)	-0.09	1-3	>718	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	2	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 19 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-10-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=238/0-4-0, 2=153/Mechanical, 3=85/Mechanical
Max Horz 1=136(load case 5)
Max Uplift1=-54(load case 5), 2=-142(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-122/57
BOT CHORD 1-3=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 54 lb uplift at joint 1 and 142 lb uplift at joint 2.

LOAD CASE(S) Standard

OCTOBER 10, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1010061258

Job	Truss	Truss Type	Qty	Ply	
L213465	EJ5A	SPECIAL	1	2	0 0 Job Reference (optional)

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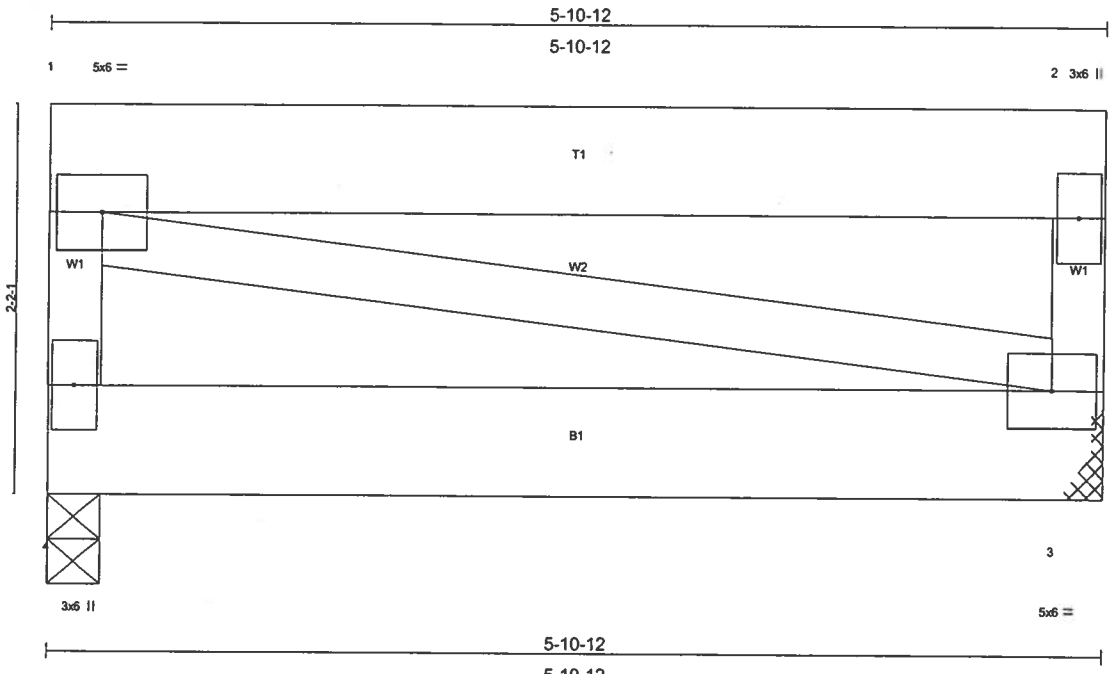

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.03	Vert(LL)	-0.05	3-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.37	Vert(TL)	-0.08	3-4	>821	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 94 lb	

LUMBER
TOP CHORD 2 X 8 SYP 2400F 2.0E
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-10-12 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=1874/0-3-8, 3=1874/Mechanical
Max Uplift4=-708(load case 2), 3=-708(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-4=-151/106, 1-2=0/0, 2-3=-151/106
BOT CHORD 3-4=-0/0
WEBS 1-3=-0/0

NOTES
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc, 2 X 8 - 2 rows at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-7-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
4) Provide adequate drainage to prevent water ponding.
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 708 lb uplift at joint 4 and 708 lb uplift at joint 3.
7) Girder carries tie-in span(s): 30-1-4 from 0-0-0 to 5-10-12

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 3-4=-615(F=-585)

OCTOBER 10, 2006 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1010061259

Job	Truss	Truss Type	Qty	Ply	0 0
L213465	EJ7	JACK	32	1	Job Reference (optional)

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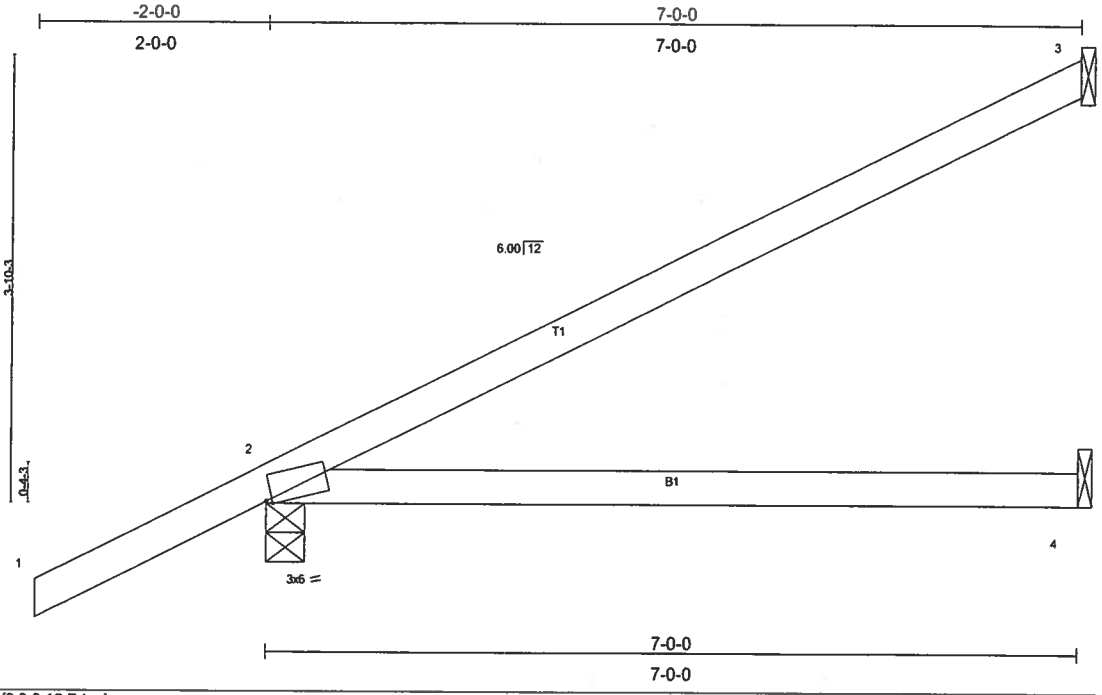

Plate Offsets (X,Y): [2:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	0.26	2-4	>309	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.38	Vert(TL)	0.21	2-4	>379	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 26 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=162/Mechanical, 2=420/0-4-0, 4=104/Mechanical
Max Horz 2=224(load case 5)
Max Uplift3=-143(load case 5), 2=-296(load case 5), 4=-67(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-94/58
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 143 lb uplift at joint 3, 296 lb uplift at joint 2 and 67 lb uplift at joint 4.

LOAD CASE(S) Standard

OCTOBER 10, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1010061260

Job	Truss	Truss Type	Qty	Ply	0 0
L213465	HJ8	MONO TRUSS	1	1	Job Reference (optional)

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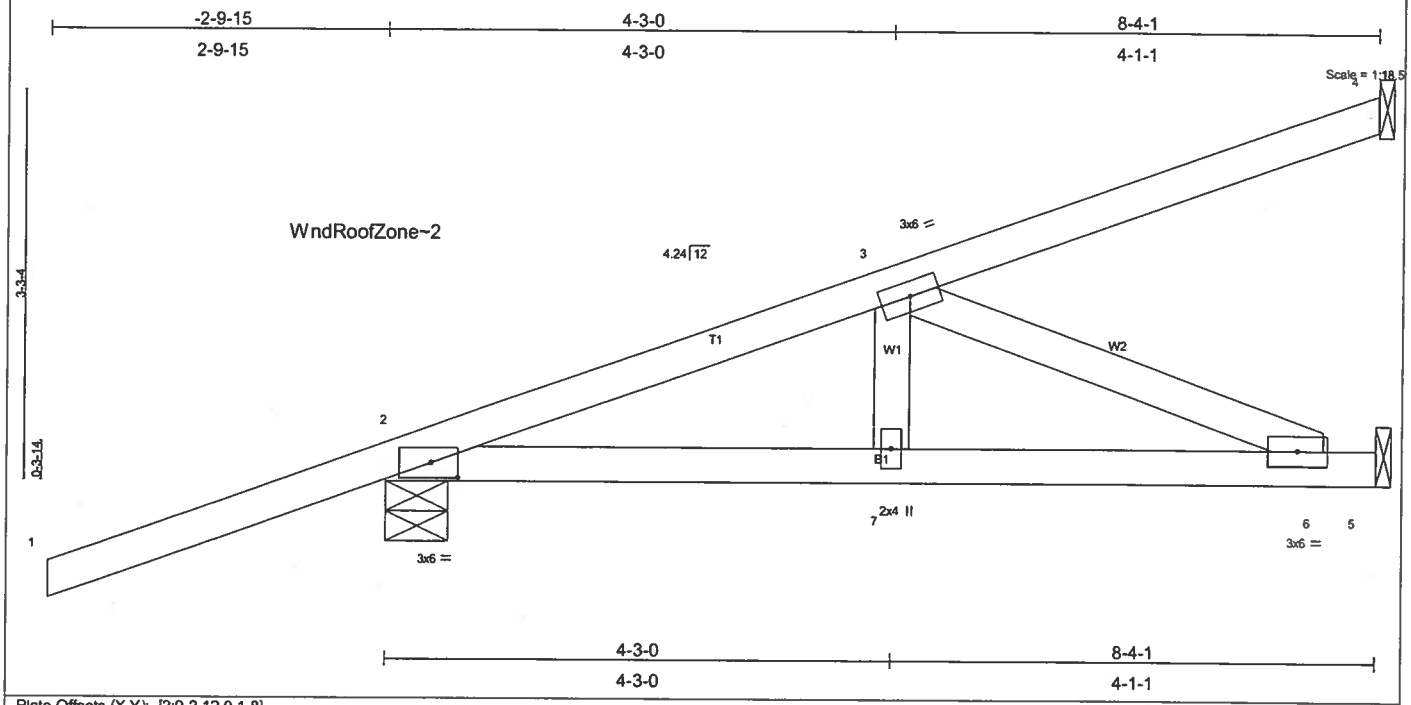


Plate Offsets (X,Y): [2:0-2-12,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.55	Vert(LL)	-0.04	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.34	Vert(TL)	-0.06	6-7	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.15	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 38 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=174/Mechanical, 2=443/0-6-7, 5=266/Mechanical
Max Horz 2=208(load case 2)
Max Uplift4=-149(load case 2), 2=-264(load case 2), 5=-39(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/50, 2-3=-496/25, 3-4=-67/43
BOT CHORD 2-7=-126/444, 6-7=-126/444, 5-6=0/0
WEBS 3-7=0/123, 3-6=-481/137

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 149 lb uplift at joint 4, 264 lb uplift at joint 2 and 39 lb uplift at joint 5.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

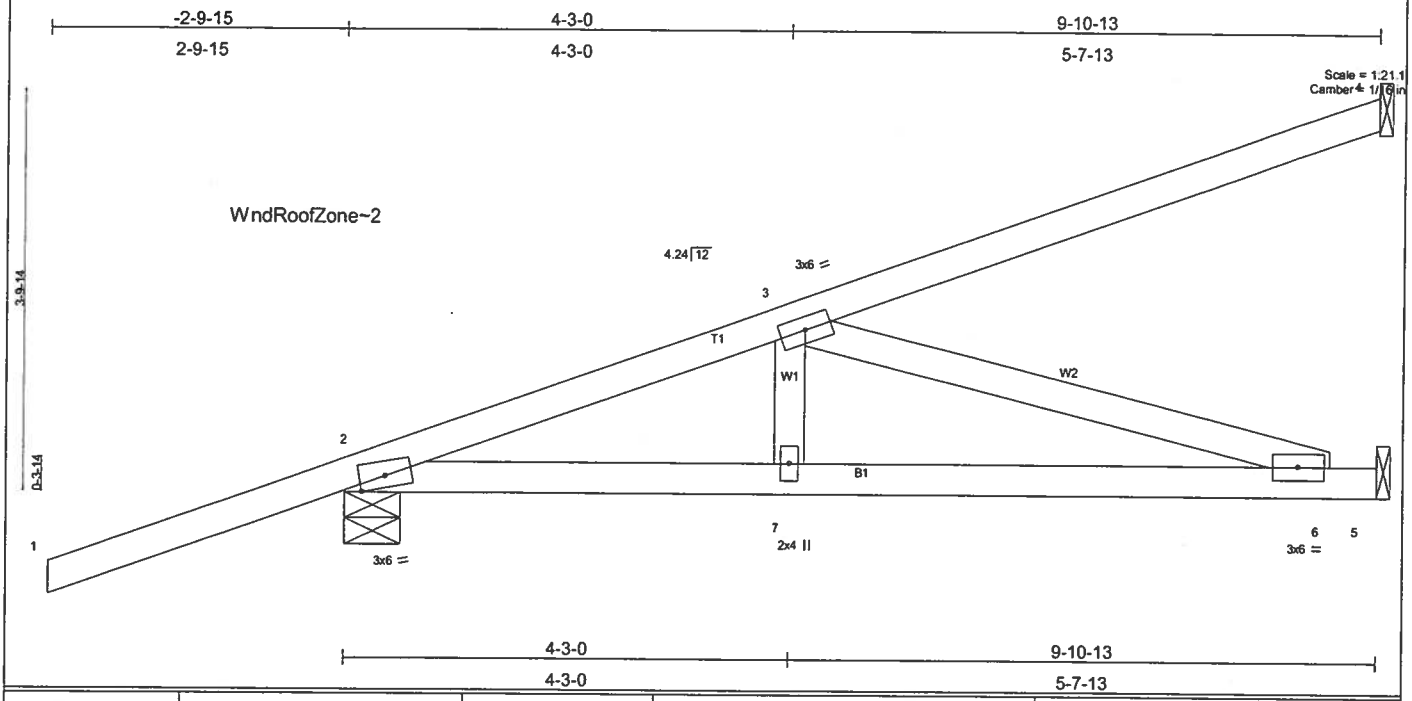
LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-4(F=25, B=25)-to-4=-113(F=-29, B=-29), 2=0(F=15, B=15)-to-5=-63(F=-16, B=-16)

OCTOBER 10, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1010061261

Job	Truss	Truss Type	Qty	Ply	0 0
L213465	HJ9	MONO TRUSS	5	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Oct 10 13:51:11 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.62	Vert(LL)	-0.11	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.61	Vert(TL)	-0.18	6-7	>623	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.46	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 45 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-11-13 oc bracing.

REACTIONS (lb/size) 4=270/Mechanical, 2=537/0-6-6, 5=372/Mechanical
Max Horz 2=269(load case 2)
Max Uplift4=-233(load case 2), 2=-404(load case 2), 5=-180(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/50, 2-3=-876/358, 3-4=-105/66
BOT CHORD 2-7=-530/809, 6-7=-530/809, 5-6=0/0
WEBS 3-7=-94/187, 3-6=-843/552

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 233 lb uplift at joint 4, 404 lb uplift at joint 2 and 180 lb uplift at joint 5.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-4(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=0(F=15, B=15)-to-5=-74(F=-22, B=-22)

OCTOBER 10, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1010061262

Job	Truss	Truss Type	Qty	Ply	0 0
L213465	T01	HIP	1	2	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Oct 10 13:51:13 2006 Page 1

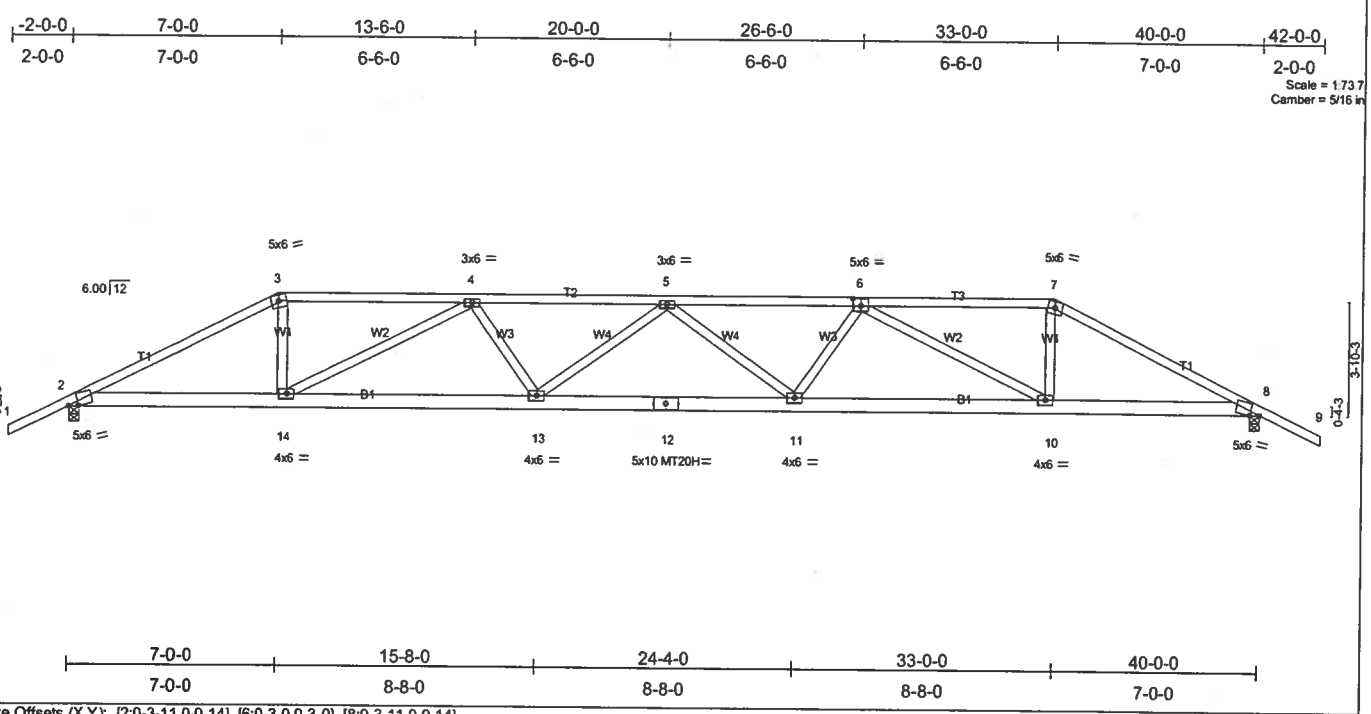


Plate Offsets (X,Y): [2:0-3-11,0-0-14], [6:0-3-0,0-3-0], [8:0-3-11,0-0-14]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.67	Vert(LL)	-0.51	11-13	>934	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.64	Vert(TL)	-0.82	11-13	>582	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.77	Horz(TL)	0.16	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 439 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-8-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-5-2 oc bracing.

REACTIONS (lb/size) 2=3591/0-4-0, 8=3591/0-4-0
Max Horz 2=89(load case 4)
Max Uplift2=-1495(load case 4), 8=-1495(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/51, 2-3=-7215/3076, 3-4=-6483/2834, 4-5=-10223/4441, 5-6=-10223/4441, 6-7=-6483/2834, 7-8=-7215/3076, 8-9=0/51
BOT CHORD 2-14=-2710/6371, 13-14=-4249/9631, 12-13=-4723/10693, 11-12=-4723/10693, 10-11=-4210/9631, 8-10=-2671/6371
WEBS 3-14=-1020/2673, 4-14=-3670/1777, 4-13=-235/1142, 5-13=-629/498, 5-11=-629/498, 6-11=-235/1142, 6-10=-3670/1777, 7-10=-1020/2673

NOTES
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.
6) All plates are MT20 plates unless otherwise indicated.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1495 lb uplift at joint 2 and 1495 lb uplift at joint 8.
8) Girder carries hip end with 7-0-0 end setback.
9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 33-0-0, and 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-7=-117(F=-63), 7-9=-54, 2-14=-30, 10-14=-65(F=-35), 8-10=-30
Concentrated Loads (lb)
Vert: 14=-539(F) 10=-539(F)

OCTOBER 10, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

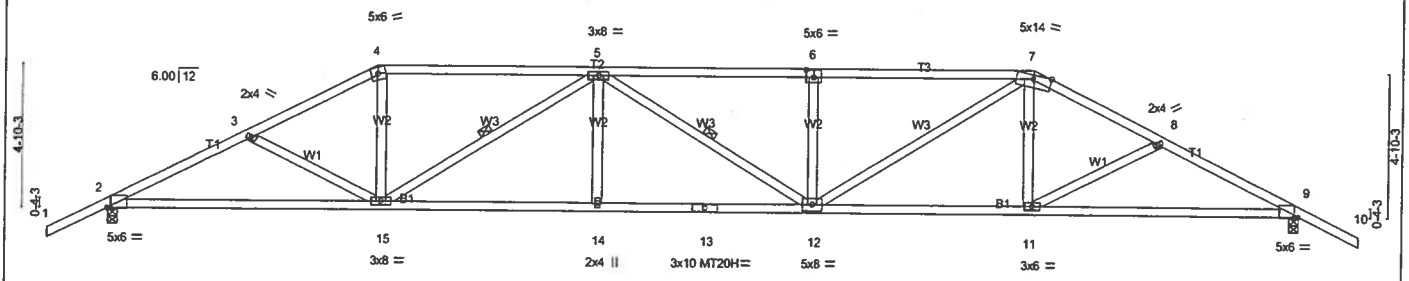
Dwg.#1010061263

Job	Truss	Truss Type	Qty	Ply	0 0
L213465	T02	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Oct 10 13:51:14 2006 Page 1

-2-0-0	4-9-4	9-0-0	16-4-9	23-7-7	31-0-0	35-2-12	40-0-0	42-0-0
2-0-0	4-9-4	4-2-12	7-4-9	7-2-13	7-4-9	4-2-12	4-9-4	2-0-0

Scale = 1:73.7
Camber = 1/4 in



9-0-0	16-4-9	23-7-7	31-0-0	40-0-0
9-0-0	7-4-9	7-2-13	7-4-9	9-0-0

Plate Offsets (X,Y): [2:0-1-11,Edge], [6:0-3-0,0-3-0], [9:0-1-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.52	Vert(LL)	-0.41	12-14	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.84	Vert(TL)	-0.67	12-14	>712	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	YES	WB	0.62	Horz(TL)	0.19	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 205 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-11-1 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-7 oc bracing.
WEBS 1 Row at midpt 5-15, 5-12

REACTIONS (lb/size) 2=1783/0-4-0, 9=1783/0-4-0
Max Horz 2=-101(load case 6)
Max Uplift2=-582(load case 5), 9=-582(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-3100/911, 3-4=-2905/894, 4-5=-2592/841, 5-6=-3553/1189, 6-7=-3553/1189, 7-8=-2905/895, 8-9=-3101/911, 9-10=0/47
BOT CHORD 2-15=-794/2707, 14-15=-1097/3553, 13-14=-1097/3553, 12-13=-1097/3553, 11-12=-685/2572, 9-11=-739/2708
WEBS 3-15=-166/176, 4-15=-197/938, 5-15=-1212/489, 5-14=0/207, 5-12=-67/68, 6-12=-403/282, 7-12=-494/1230, 7-11=-24/335, 8-11=-167/176

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 582 lb uplift at joint 2 and 582 lb uplift at joint 9.

LOAD CASE(S) Standard

OCTOBER 10, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

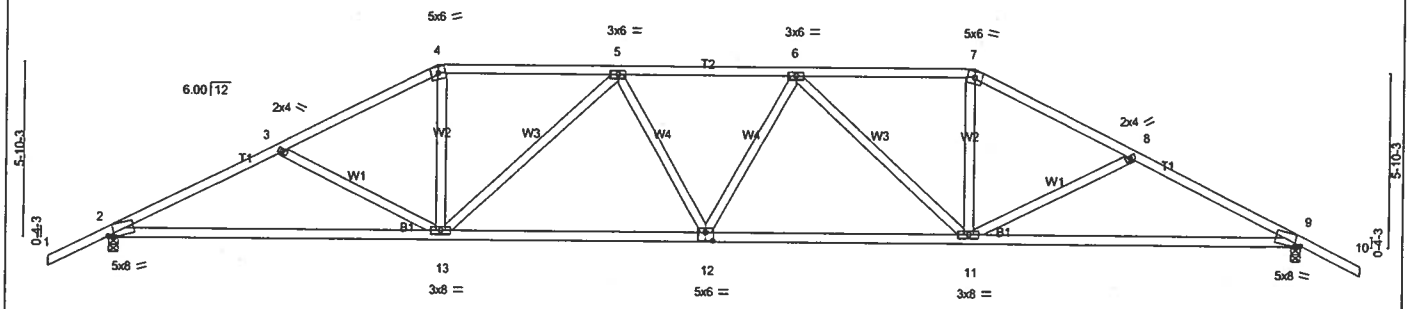
Dwg.#1010061264

Job	Truss	Truss Type	Qty	Ply	0 0
L213465	T03	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Oct 10 13:51:15 2006 Page 1

-2-0-0	5-9-4	11-0-0	17-0-0	23-0-0	29-0-0	34-2-12	40-0-0	42-0-0
2-0-0	5-9-4	5-2-12	6-0-0	6-0-0	6-0-0	5-2-12	5-9-4	2-0-0

Scale = 1:73.7
Camber = 5/16 in



11-0-0	20-0-0	29-0-0	40-0-0
11-0-0	9-0-0	9-0-0	11-0-0

Plate Offsets (X,Y): [2:0-2-7,Edge], [9:0-2-7,Edge], [12:0-3-0,0-3-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.48	Vert(LL)	-0.41	9-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.87	Vert(TL)	-0.70	9-11	>683	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.85	Horz(TL)	0.18	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 204 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-14 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-11-11 oc bracing.

REACTIONS (lb/size) 2=1783/0-4-0, 9=1783/0-4-0
Max Horz 2=-115(load case 6)
Max Uplift2=-601(load case 5), 9=-601(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-3063/860, 3-4=-2762/784, 4-5=-2435/749, 5-6=-2960/911, 6-7=-2435/749, 7-8=-2762/784, 8-9=-3063/860, 9-10=0/47
BOT CHORD 2-13=-733/2682, 12-13=-810/2897, 11-12=-781/2897, 9-11=-660/2682
WEBS 3-13=-308/260, 4-13=-171/901, 5-13=-736/345, 5-12=-4/194, 6-12=-4/194, 6-11=-736/345, 7-11=-171/901, 8-11=-308/261

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 601 lb uplift at joint 2 and 601 lb uplift at joint 9.

LOAD CASE(S) Standard

OCTOBER 10, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1010061265

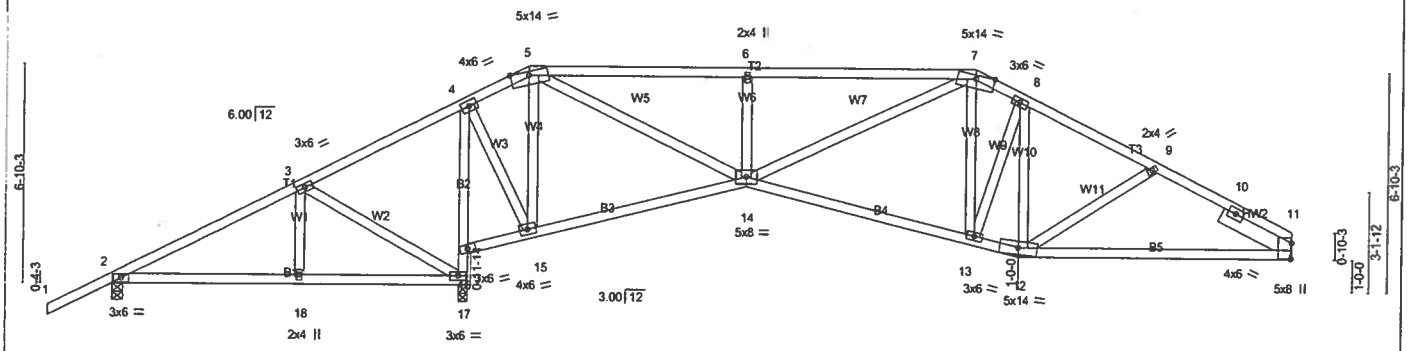
Job	Truss	Truss Type	Qty	Ply	0 0
L213465	T04	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Oct 10 13:51:17 2006 Page 1

-2-0-0 | 5-10-2 | 11-1-12 | 13-0-0 | 19-9-6 | 27-0-0 | 28-4-8 | 32-6-8 | 37-0-0

2-0-0 | 5-10-2 | 5-3-10 | 1-10-4 | 6-9-6 | 7-2-10 | 1-4-8 | 4-2-0 | 4-5-8

Scale = 1:68.8
Camber = 1/16 in



5-10-2 | 11-1-12 | 13-0-0 | 19-9-6 | 27-0-0 | 28-4-8 | 37-0-0

5-10-2 | 5-3-10 | 1-10-4 | 6-9-6 | 7-2-10 | 1-4-8 | 8-7-8

Plate Offsets (X,Y): [11:0-6-0,0-0-2]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.36	Vert(LL)	-0.14	13-14	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.75	Vert(TL)	-0.23	13-14	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.54	Horz(TL)	0.05	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 212 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
SLIDER Right 2 X 6 SYP No.1D 2-6-2

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-9-4 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-0-10 oc bracing.

REACTIONS (lb/size) 11=1001/Mechanical, 2=366/0-4-0, 17=1843/0-3-8
Max Horz 2=186(load case 5)
Max Uplift11=-326(load case 6), 2=-374(load case 5), 17=-638(load case 5)
Max Grav 11=1007(load case 10), 2=390(load case 9), 17=1843(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-193/290, 3-4=-142/527, 4-5=-57/137, 5-6=-1539/562, 6-7=-1539/562, 7-8=-1300/480, 8-9=-1364/459, 9-10=-1488/531, 10-11=-1549/522
BOT CHORD 2-18=-237/111, 17-18=-237/111, 16-17=-1525/445, 4-16=-1308/369, 15-16=-499/196, 14-15=-10/127, 13-14=-286/1249, 12-13=-268/1222, 11-12=-375/1286
WEBS 3-18=-211/217, 3-17=-558/434, 4-15=-298/1068, 5-15=-1010/365, 5-14=-507/1677, 6-14=-393/278, 7-14=-183/447, 7-13=-92/145, 8-13=-130/126, 8-12=-70/87, 9-12=-117/169

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 326 lb uplift at joint 11, 374 lb uplift at joint 2 and 638 lb uplift at joint 17.

LOAD CASE(S) Standard

OCTOBER 10, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

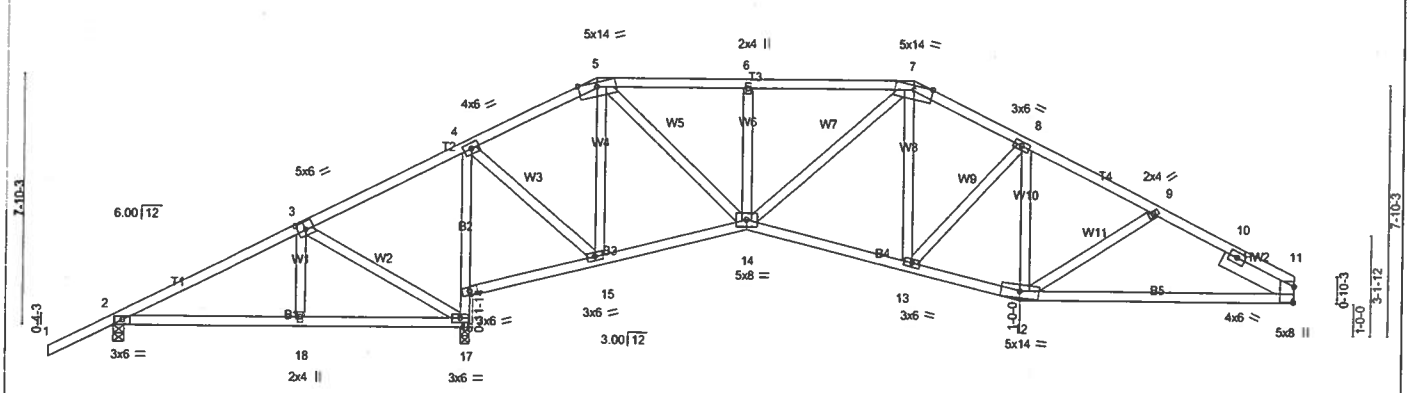
Dwg.#1010061266

Job	Truss	Truss Type	Qty	Ply	0 0
L213465	T05	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Oct 10 13:51:18 2006 Page 1

-2-0-0	5-10-2	11-1-12	15-0-0	19-9-6	25-0-0	28-4-8	32-6-8	37-0-0
2-0-0	5-10-2	5-3-10	3-10-4	4-9-6	5-2-10	3-4-8	4-2-0	4-5-8

Scale = 1:68.8
Camber = 1/16 in



5-10-2	11-1-12	15-0-0	19-9-6	25-0-0	28-4-8	37-0-0
5-10-2	5-3-10	3-10-4	4-9-6	5-2-10	3-4-8	8-7-8

Plate Offsets (X,Y): [3:0-3-0,0-3-0], [11:0-6-0,0-0-2]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.12	11-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.92	Vert(TL)	-0.20	11-12	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.41	Horz(TL)	0.04	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 215 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3
SLIDER Right 2 X 6 SYP No.1D 2-6-2

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-8-14 oc purlins.
BOT CHORD Rigid ceiling directly applied or 3-7-0 oc bracing.

REACTIONS (lb/size) 11=1032/Mechanical, 2=440/0-4-0, 17=1739/0-3-8
Max Horz 2=200(load case 5)
Max Uplift11=-352(load case 6), 2=-391(load case 5), 17=-634(load case 5)
Max Grav 11=1032(load case 1), 2=463(load case 9), 17=1739(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-343/359, 3-4=-72/358, 4-5=-569/272, 5-6=-1248/459, 6-7=-1248/459, 7-8=-1304/492, 8-9=-1413/512, 9-10=-1531/577, 10-11=-1592/568
BOT CHORD 2-18=-288/243, 17-18=-283/238, 16-17=-1418/388, 4-16=-1255/366, 15-16=-323/134, 14-15=-95/478, 13-14=-222/1190, 12-13=-293/1278, 11-12=-413/1323
WEBS 3-18=-209/209, 3-17=-537/422, 4-15=-210/997, 5-15=-737/211, 5-14=-292/1095, 6-14=-284/203, 7-14=-116/242, 7-13=-127/235, 8-13=-154/160, 8-12=-59/76, 9-12=-105/155

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 352 lb uplift at joint 11, 391 lb uplift at joint 2 and 634 lb uplift at joint 17.

LOAD CASE(S) Standard

OCTOBER 10, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1010061267

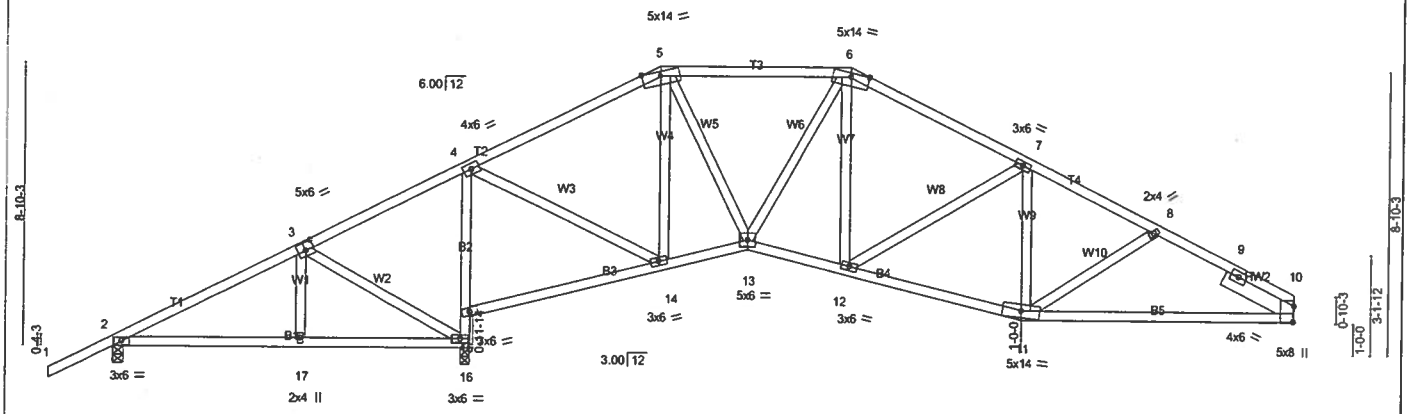
Job	Truss	Truss Type	Qty	Ply	0 0
L213465	T06	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Oct 10 13:51:19 2006 Page 1

-2-0-0 | 5-10-2 | 11-1-12 | 17-0-0 | 23-0-0 | 28-4-8 | 32-6-8 | 37-0-0

2-0-0 | 5-10-2 | 5-3-10 | 5-10-4 | 6-0-0 | 5-4-8 | 4-2-0 | 4-5-8

Scale = 1:68.8
Camber = 1/15 in



5-10-2 | 11-1-12 | 17-0-0 | 19-9-6 | 23-0-0 | 28-4-8 | 37-0-0

5-10-2 | 5-3-10 | 5-10-4 | 2-9-6 | 3-2-10 | 5-4-8 | 8-7-8

Plate Offsets (X,Y): [3:0-3-0,0-3-0], [10:0-6-0,0-0-2]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.11	10-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.76	Vert(TL)	-0.19	10-11	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.37	Horz(TL)	0.04	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 214 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except* B2 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3
SLIDER Right 2 X 6 SYP No.1D 2-6-2

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-8-11 oc purlins.
BOT CHORD Rigid ceiling directly applied or 3-10-3 oc bracing.

REACTIONS (lb/size) 10=1040/Mechanical, 2=459/0-4-0, 16=1712/0-3-8
Max Horz 2=214(load case 5)
Max Uplift10=-370(load case 6), 2=-398(load case 5), 16=-642(load case 5)
Max Grav 10=1040(load case 1), 2=481(load case 9), 16=1712(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-380/394, 3-4=-44/311, 4-5=-863/357, 5-6=-995/423, 6-7=-1234/478, 7-8=-1437/552, 8-9=-1543/605, 9-10=-1606/597
BOT CHORD 2-17=-319/275, 16-17=-313/270, 15-16=-1393/401, 4-15=-1227/402, 14-15=-262/97, 13-14=-116/718, 12-13=-168/1091, 11-12=-337/1315, 10-11=-436/1331
WEBS 3-17=-208/203, 3-16=-524/413, 4-14=-146/1044, 5-14=-558/140, 5-13=-151/670, 6-13=-163/118, 6-12=-136/312, 7-12=-257/221, 7-11=-53/66, 8-11=-89/133

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 370 lb uplift at joint 10, 398 lb uplift at joint 2 and 642 lb uplift at joint 16.

LOAD CASE(S) Standard

OCTOBER 10, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1010061268

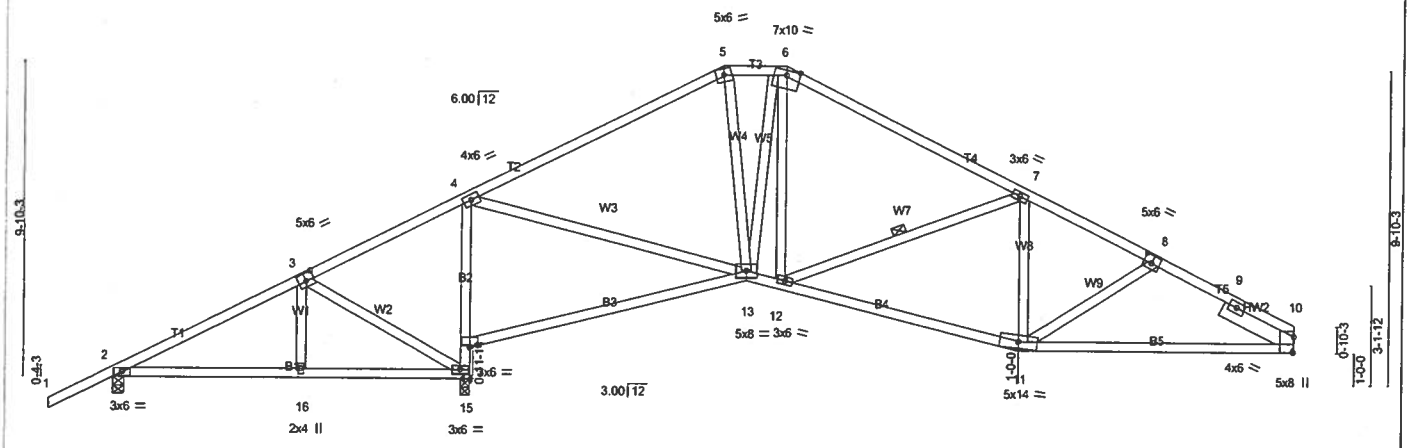
Job	Truss	Truss Type	Qty	Ply	0 0
L213465	T07	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Oct 10 13:51:21 2006 Page 1

-2-0-0 | 5-10-2 | 11-1-12 | 19-0-0 | 21-0-0 | 28-4-8 | 32-6-8 | 37-0-0

2-0-0 | 5-10-2 | 5-3-10 | 7-10-4 | 2-0-0 | 7-4-8 | 4-2-0 | 4-5-8

Scale = 1:68.8
Camber = 1/16 in



5-10-2 | 11-1-12 | 19-9-6 | 21-0-0 | 28-4-8 | 37-0-0

5-10-2 | 5-3-10 | 8-7-10 | 1-2-10 | 7-4-8 | 8-7-8

Plate Offsets (X,Y): [3:0-3-0,0-3-0], [8:0-3-0,0-3-0], [10:0-6-0,0-0-2]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.43	Vert(LL)	-0.11	11-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.94	Vert(TL)	-0.18	11-12	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.36	Horz(TL)	0.04	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 213 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3
SLIDER Right 2 X 6 SYP No.1D 2-6-2

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-9-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 3-8-6 oc bracing.
WEBS 1 Row at midpt 7-12

REACTIONS (lb/size) 2=454/0-4-0, 10=1038/Mechanical, 15=1719/0-3-8
Max Horz 2=228(load case 5)
Max Uplift2=-406(load case 5), 10=-385(load case 6), 15=-646(load case 5)
Max Grav 2=475(load case 9), 10=1038(load case 1), 15=1719(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-362/426, 3-4=-20/314, 4-5=-1084/433, 5-6=-900/449, 6-7=-1111/452, 7-8=-1449/591, 8-9=-1535/624, 9-10=-1599/618
BOT CHORD 2-16=-346/259, 15-16=-341/253, 14-15=-1407/413, 4-14=-1217/437, 13-14=-239/55, 12-13=-111/960, 11-12=-382/1350, 10-11=-450/1320
WEBS 3-16=-209/201, 3-15=-505/400, 4-13=-119/1112, 5-13=-102/348, 6-13=-247/172, 6-12=-171/308, 7-12=-426/301, 7-11=-52/87, 8-11=-53/99

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 406 lb uplift at joint 2, 385 lb uplift at joint 10 and 646 lb uplift at joint 15.

LOAD CASE(S) Standard

OCTOBER 10, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

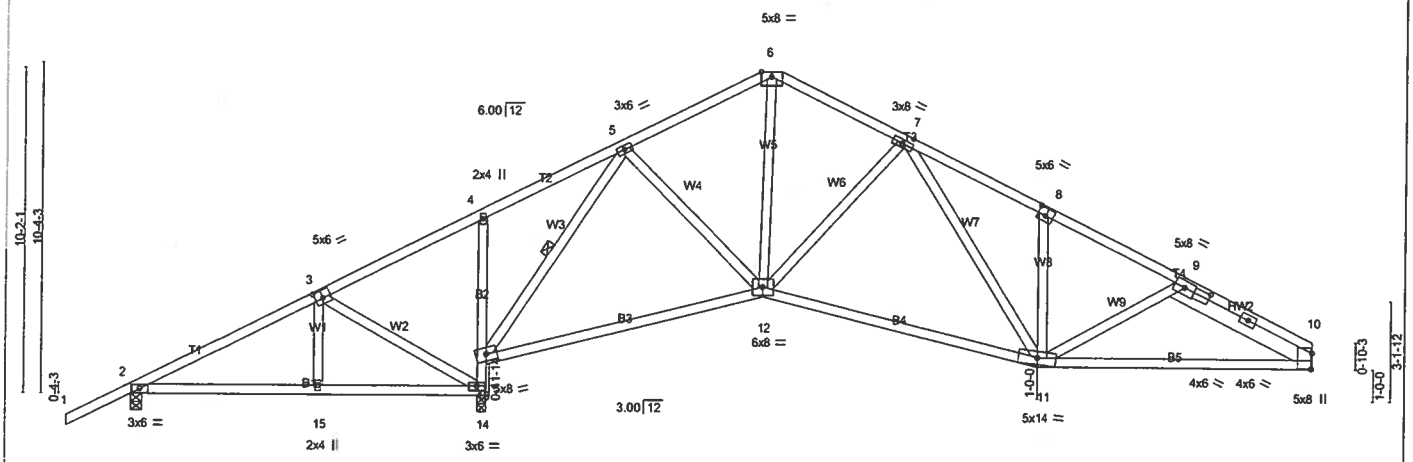
Dwg.#1010061269

Job	Truss	Truss Type	Qty	Ply	0 0
L213465	T08	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Oct 10 13:51:22 2006 Page 1

-2-0-0	5-10-2	11-1-12	15-4-12	20-0-0	24-4-6	28-4-8	32-6-8	37-0-0
2-0-0	5-10-2	5-3-10	4-3-0	4-7-4	4-4-6	4-0-2	4-2-0	4-5-8

Scale = 1:68.8
Camber = 1/16 in



5-10-2	11-1-12	19-9-6	20-0-0	28-4-8	37-0-0
5-10-2	5-3-10	8-7-10	0-2-10	8-4-8	8-7-8

Plate Offsets (X,Y): [3:0-3-0,0-3-0], [8:0-3-0,0-3-0], [10:3-7-13,0-2-0], [10:0-6-0,0-0-2]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.13	11-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.82	Vert(TL)	-0.22	11-12	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.44	Horz(TL)	0.04	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 218 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3
SLIDER Right 2 X 6 SYP No.1D 4-10-12

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-2-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 3-3-14 oc bracing.
WEBS 1 Row at midpt 5-13

REACTIONS (lb/size) 10=1033/Mechanical, 2=441/0-4-0, 14=1738/0-3-8
Max Horz 2=235(load case 5)
Max Uplift10=-383(load case 6), 2=-390(load case 5), 14=-676(load case 5)
Max Grav 10=1033(load case 1), 2=466(load case 9), 14=1738(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-348/405, 3-4=-39/351, 4-5=0/347, 5-6=-974/442, 6-7=-949/422, 7-8=-1455/685, 8-9=-1459/577, 9-10=-1616/641
BOT CHORD 2-15=-329/248, 14-15=-323/243, 13-14=-1415/431, 4-13=-258/221, 12-13=-118/519, 11-12=-233/1139, 10-11=-479/1352
WEBS 3-15=-209/202, 3-14=-529/418, 5-13=-1359/310, 5-12=0/508, 6-12=-244/567, 7-12=-429/322, 7-11=-220/371, 8-11=-238/214, 9-11=-120/157

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 383 lb uplift at joint 10, 390 lb uplift at joint 2 and 676 lb uplift at joint 14.

LOAD CASE(S) Standard

OCTOBER 10, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

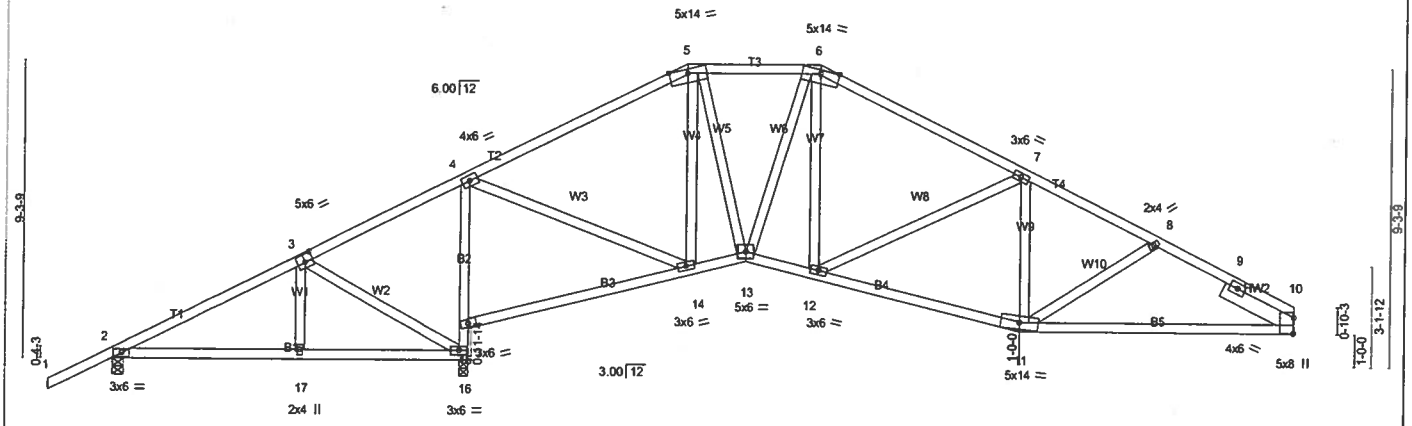
Dwg.#1010061270

Job	Truss	Truss Type	Qty	Ply	0 0
L213465	T09	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Oct 10 13:51:23 2006 Page 1

-2-0-0	5-10-2	11-1-12	17-10-12	22-1-4	28-4-8	32-6-8	37-0-0
2-0-0	5-10-2	5-3-10	6-9-0	4-2-8	6-3-4	4-2-0	4-5-8

Scale = 1:68.8
Camber = 1/16 in



5-10-2	11-1-12	17-10-12	19-9-6	22-1-4	28-4-8	37-0-0
5-10-2	5-3-10	6-9-0	1-10-10	2-3-14	6-3-4	8-7-8

Plate Offsets (X,Y): [3:0-3-0,0-3-0], [10:0-6-0,0-0-2]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.35	Vert(LL)	-0.11	10-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.80	Vert(TL)	-0.19	10-11	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.34	Horz(TL)	0.04	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 217 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3
SLIDER Right 2 X 6 SYP No.1D 2-6-2

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-8-13 oc purlins.
BOT CHORD Rigid ceiling directly applied or 3-11-0 oc bracing.

REACTIONS (lb/size) 10=1042/Mechanical, 2=463/0-4-0, 16=1706/0-3-8
Max Horz 2=220(load case 5)
Max Uplift10=-376(load case 6), 2=-401(load case 5), 16=-646(load case 5)
Max Grav 10=1042(load case 1), 2=485(load case 9), 16=1706(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-385/406, 3-4=-34/298, 4-5=-954/386, 5-6=-935/436, 6-7=-1186/465, 7-8=-1449/569, 8-9=-1544/615, 9-10=-1607/607
BOT CHORD 2-17=-329/279, 16-17=-324/274, 15-16=-1391/408, 4-15=-1223/418, 14-15=-235/79, 13-14=-98/776, 12-13=-139/1024, 11-12=-358/1337, 10-11=-443/1330
WEBS 3-17=-209/202, 3-16=-514/406, 4-14=-122/1049, 5-14=-439/126, 5-13=-170/591, 6-13=-244/118, 6-12=-139/381, 7-12=-333/260, 7-11=-57/76, 8-11=-71/117

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 376 lb uplift at joint 10, 401 lb uplift at joint 2 and 646 lb uplift at joint 16.

LOAD CASE(S) Standard

OCTOBER 10, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

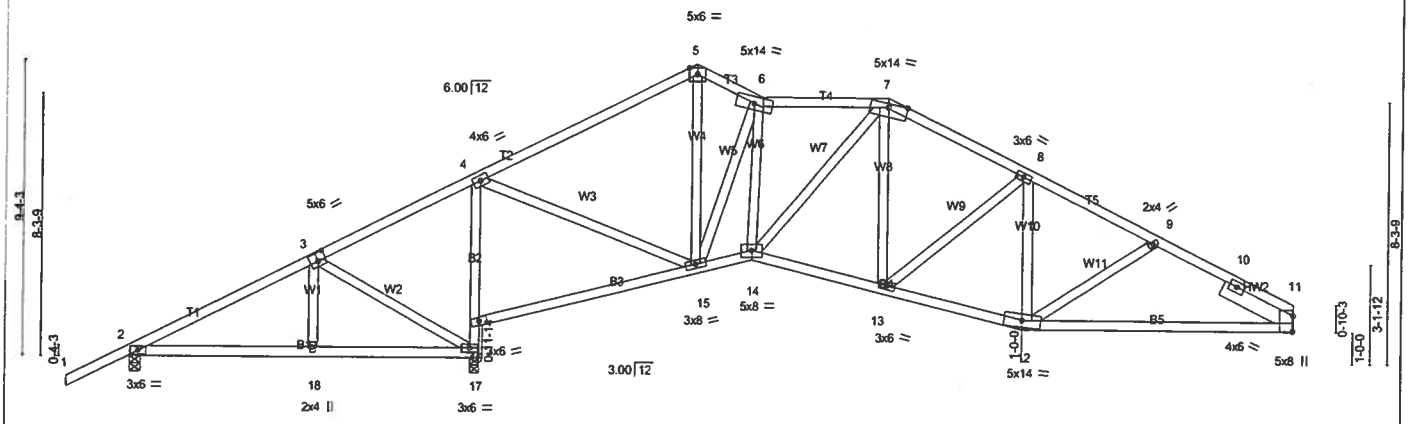
Dwg.#1010061271

Job	Truss	Truss Type	Qty	Ply	0 0
L213465	T10	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Oct 10 13:51:24 2006 Page 1

-2-0-0	5-10-2	11-1-12	18-0-0	20-1-4	24-1-4	28-4-8	32-6-8	37-0-0
2-0-0	5-10-2	5-3-10	6-10-4	2-1-4	4-0-0	4-3-4	4-2-0	4-5-8

Scale = 1:69.7
Camber = 1/16 in



5-10-2	11-1-12	18-0-0	19-9-6	24-1-4	28-4-8	37-0-0
5-10-2	5-3-10	6-10-4	1-9-6	4-3-14	4-3-4	8-7-8

Plate Offsets (X,Y): [3:0-3-0,0-3-0], [11:0-6-0,0-0-2]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	-0.12	11-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.79	Vert(TL)	-0.20	11-12	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.62	Horz(TL)	0.04	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 221 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except* B2 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3
SLIDER Right 2 X 6 SYP No.1D 2-6-2

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-9-3 oc purlins.
BOT CHORD Rigid ceiling directly applied or 3-5-15 oc bracing.

REACTIONS (lb/size) 11=1030/Mechanical, 2=434/0-4-0, 17=1748/0-3-8
Max Horz 2=221(load case 5)
Max Uplift11=-390(load case 6), 2=-394(load case 5), 17=-657(load case 5)
Max Grav 11=1030(load case 1), 2=477(load case 9), 17=1748(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-369/387, 3-4=-45/366, 4-5=-909/415, 5-6=-807/417, 6-7=-1094/510, 7-8=-1261/563, 8-9=-1411/590, 9-10=-1525/646, 10-11=-1587/638
BOT CHORD 2-18=-313/265, 17-18=-307/259, 16-17=-1434/419, 4-16=-1246/424, 15-16=-305/115, 14-15=-207/1163, 13-14=-236/1129, 12-13=-367/1283, 11-12=-471/1317
WEBS 3-18=-211/209, 3-17=-521/409, 4-15=-161/1084, 5-15=-199/500, 6-15=-1092/411, 6-14=-143/583, 7-14=-39/59, 7-13=-118/295, 8-13=-217/165, 8-12=-54/60, 9-12=-103/139

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 390 lb uplift at joint 11, 394 lb uplift at joint 2 and 657 lb uplift at joint 17.

LOAD CASE(S) Standard

OCTOBER 10, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1010061272

Job	Truss	Truss Type	Qty	Ply	0 0
L213465	T11	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Oct 10 13:51:26 2006 Page 1

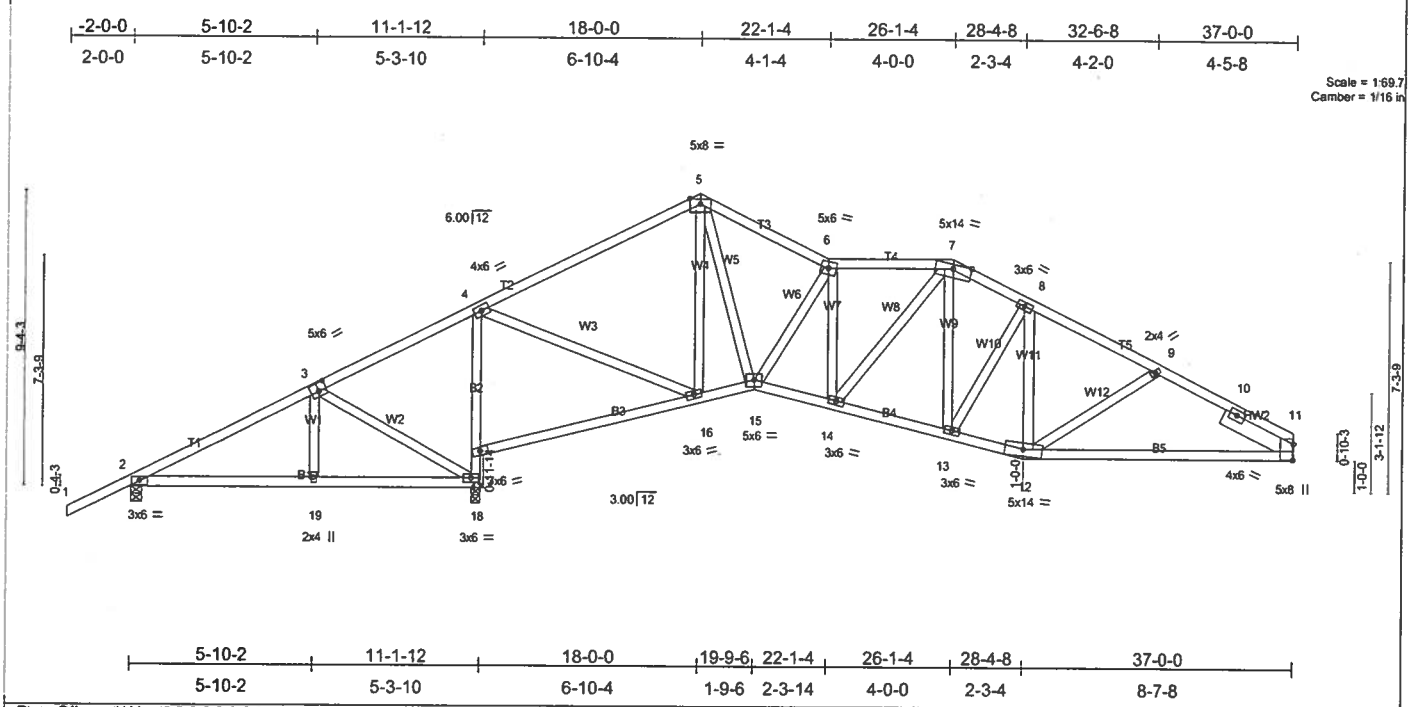


Plate Offsets (X,Y): [3:0-3-0,0-3-0], [11:0-6-0,0-0-2]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.38	Vert(LL)	-0.12	11-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.81	Vert(TL)	-0.20	11-12	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.35	Horz(TL)	0.04	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 224 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3
SLIDER Right 2 X 6 SYP No.1D 2-6-2

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-8-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 3-5-8 oc bracing.

REACTIONS (lb/size) 11=1028/Mechanical, 2=431/0-4-0, 18=1752/0-3-8
Max Horz 2=221(load case 5)
Max Uplift11=-390(load case 6), 2=-392(load case 5), 18=-660(load case 5)
Max Grav 11=1028(load case 1), 2=475(load case 9), 18=1752(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-364/384, 3-4=-47/373, 4-5=-906/413, 5-6=-1197/542, 6-7=-1365/611, 7-8=-1324/608, 8-9=-1407/585, 9-10=-1526/647, 10-11=-1587/638
BOT CHORD 2-19=-310/260, 18-19=-304/255, 17-18=-1440/423, 4-17=-1251/427, 16-17=-312/118, 15-16=-91/743, 14-15=-350/1412, 13-14=-300/1210, 12-13=-361/1260, 11-12=-473/1318
WEBS 3-19=-211/210, 3-18=-521/409, 4-16=-163/1085, 5-16=-466/149, 5-15=-408/1068, 6-15=-671/340, 6-14=-153/92, 7-14=-74/316, 7-13=-109/185, 8-13=-159/111, 8-12=-45/63, 9-12=-112/149

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 390 lb uplift at joint 11, 392 lb uplift at joint 2 and 660 lb uplift at joint 18.

LOAD CASE(S) Standard

OCTOBER 10, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

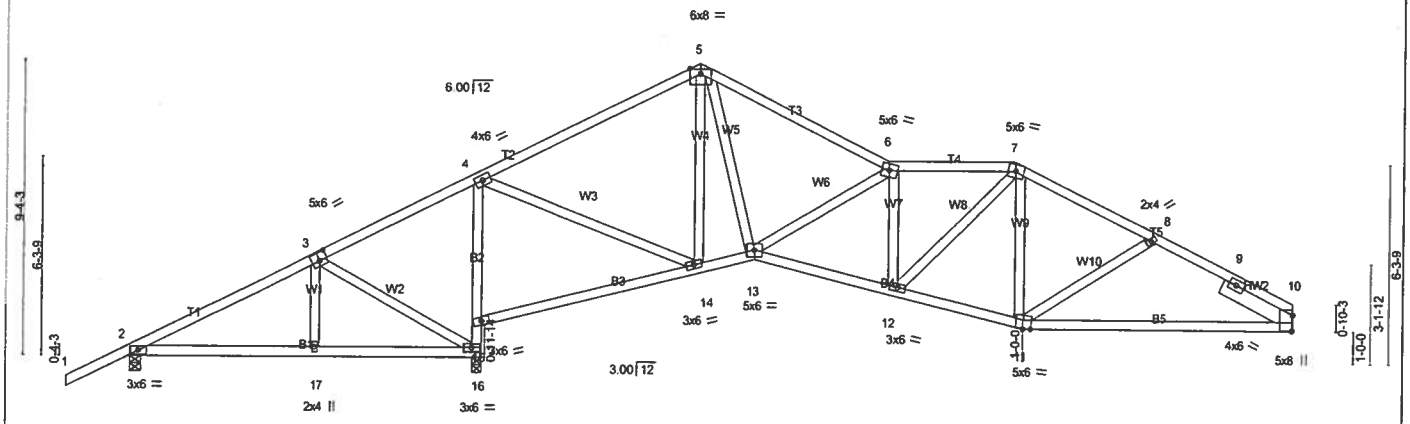
Dwg.#1010061273

Job	Truss	Truss Type	Qty	Ply	0 0
L213465	T12	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Oct 10 13:51:27 2006 Page 1

-2-0-0	5-10-2	11-1-12	18-0-0	24-1-4	28-1-4	32-4-14	37-0-0
2-0-0	5-10-2	5-3-10	6-10-4	6-1-4	4-0-0	4-3-10	4-7-2

Scale = 1:69.7
Camber = 1/16 in



5-10-2	11-1-12	18-0-0	19-9-6	24-1-4	28-4-8	37-0-0
5-10-2	5-3-10	6-10-4	1-9-6	4-3-14	4-3-4	8-7-8

Plate Offsets (X,Y): [3:0-3-0,0-3-0], [10:0-6-0,0-0-2]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.38	Vert(LL)	-0.13	10-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.85	Vert(TL)	-0.22	10-11	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.38	Horz(TL)	0.04	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 210 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3
SLIDER Right 2 X 6 SYP No.1D 2-7-0

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-9-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 3-1-13 oc bracing.

REACTIONS (lb/size) 10=1022/Mechanical, 2=416/0-4-0, 16=1772/0-3-8
Max Horz 2=221(load case 5)
Max Uplift10=-387(load case 6), 2=-388(load case 5), 16=-665(load case 5)
Max Grav 10=1022(load case 1), 2=467(load case 9), 16=1772(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-347/375, 3-4=-50/407, 4-5=-888/408, 5-6=-1187/497, 6-7=-1594/702, 7-8=-1393/581, 8-9=-1522/648, 9-10=-1585/639
BOT CHORD 2-17=-302/245, 16-17=-296/240, 15-16=-1460/428, 4-15=-1260/429, 14-15=-349/136, 13-14=-87/737, 12-13=-495/1664, 11-12=-350/1261, 10-11=-474/1318
WEBS 3-17=-212/213, 3-16=-525/411, 4-14=-181/1110, 5-14=-521/157, 5-13=-351/1044, 6-13=-746/406, 6-12=-363/189, 7-12=-195/543, 7-11=-28/73, 8-11=-132/164

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 387 lb uplift at joint 10, 388 lb uplift at joint 2 and 665 lb uplift at joint 16.

LOAD CASE(S) Standard

OCTOBER 10, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1010061274

Job	Truss	Truss Type	Qty	Ply	0 0
L213465	T13	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Oct 10 13:51:28 2006 Page 1

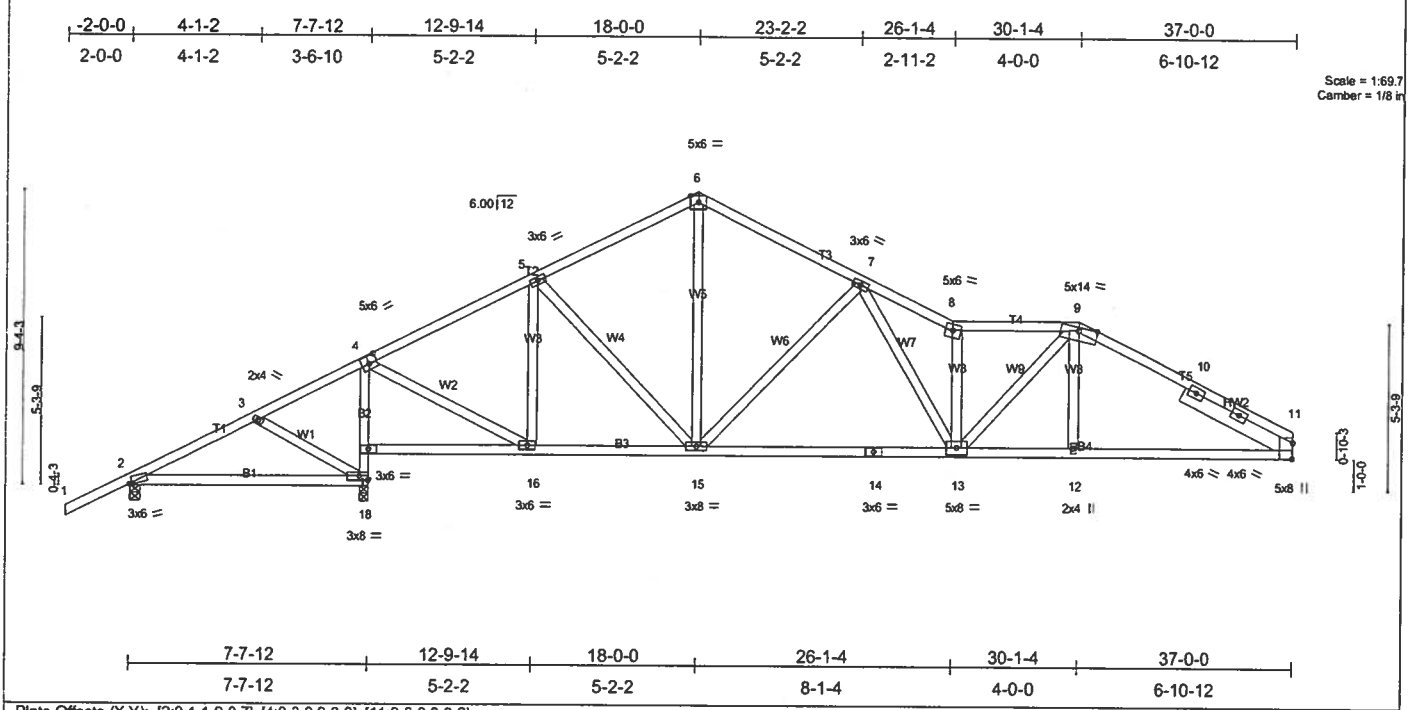


Plate Offsets (X,Y): [2:0-1-1,0-0-7], [4:0-3-0,0-3-0], [11:0-6-0,0-0-2]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	0.10	2-18	>898	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.67	Vert(TL)	-0.34	13-15	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.85	Horz(TL)	0.04	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 216 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3
SLIDER Right 2 X 6 SYP No.1D 3-11-8

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-11-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 3-8-11 oc bracing.

REACTIONS (lb/size) 11=1218/Mechanical, 2=359/0-4-0, 18=1634/0-3-8
Max Horz 2=221(load case 5)
Max Uplift11=-437(load case 6), 2=-307(load case 5), 18=-605(load case 5)
Max Grav 11=1218(load case 1), 2=389(load case 9), 18=1634(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-184/254, 3-4=-18/207, 4-5=-1098/467, 5-6=-1211/536, 6-7=-1210/519, 7-8=-2396/939, 8-9=-2089/798, 9-10=-1868/706, 10-11=-1953/687
BOT CHORD 2-18=-178/136, 17-18=-1422/453, 4-17=-1320/463, 16-17=-89/6, 15-16=-251/924, 14-15=-447/1614, 13-14=-447/1614, 12-13=-502/1635, 11-12=-502/1630
WEBS 3-18=-222/180, 4-16=-253/1152, 5-16=-437/156, 5-15=-3/235, 6-15=-298/737, 7-15=-849/439, 7-13=-376/1056, 8-13=-1202/490, 9-13=-161/653, 9-12=0/136

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 437 lb uplift at joint 11, 307 lb uplift at joint 2 and 605 lb uplift at joint 18.

LOAD CASE(S) Standard

OCTOBER 10, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1010061275

Job	Truss	Truss Type	Qty	Ply	0 0
L213465	T14	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Oct 10 13:51:29 2006 Page 1

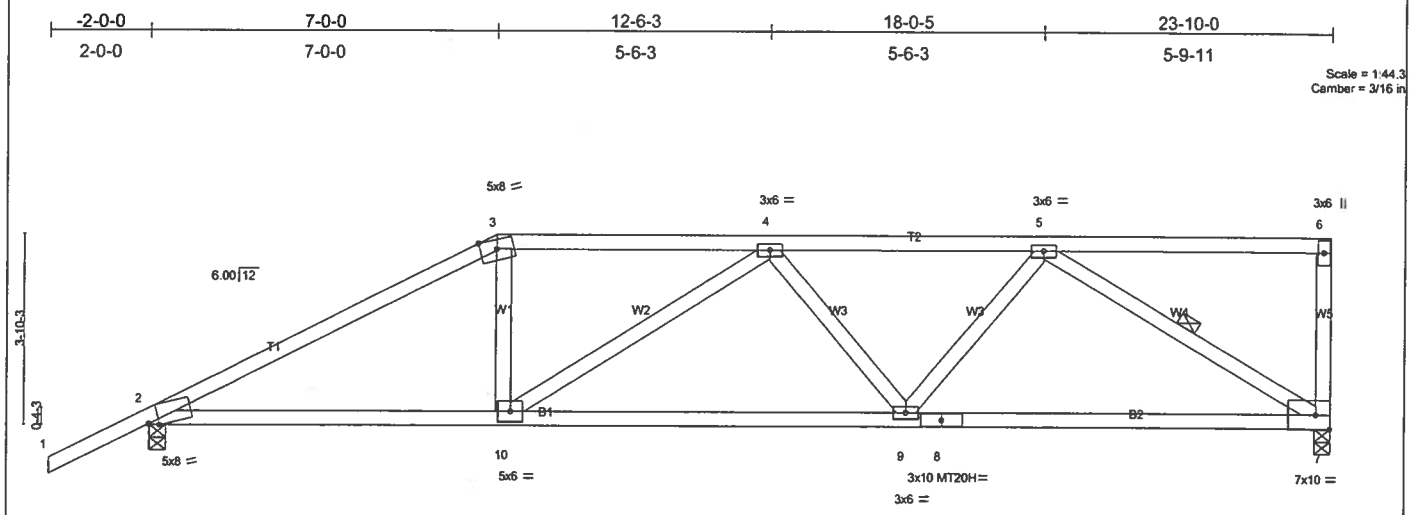


Plate Offsets (X,Y): [2:0-2-7,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.67	Vert(LL)	-0.30	9-10	>926	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.97	Vert(TL)	-0.50	9-10	>561	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.76	Horz(TL)	0.11	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 117 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B1 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except*
W5 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-10-12 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-0-6 oc bracing.
WEBS 1 Row at midpt 5-7

REACTIONS (lb/size) 7=2196/0-4-0, 2=2066/0-4-0
Max Horz 2=227(load case 4)
Max Uplift7=-987(load case 3), 2=-901(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-3790/1570, 3-4=-3345/1454, 4-5=-3453/1456, 5-6=-166/40, 6-7=-329/253
BOT CHORD 2-10=-1439/3295, 9-10=-1730/3746, 8-9=-1280/2719, 7-8=-1280/2719
WEBS 3-10=-409/1188, 4-10=-482/392, 4-9=-479/446, 5-9=-287/1198, 5-7=-3015/1464

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide adequate drainage to prevent water ponding.
3) All plates are MT20 plates unless otherwise indicated.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 987 lb uplift at joint 7 and 901 lb uplift at joint 2.
5) Girder carries hip end with 0-0-0 right side setback, 7-0-0 left side setback, and 7-0-0 end setback.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-6=-117(F=-63), 2-10=-30, 7-10=-65(F=-35)
Concentrated Loads (lb)
Vert: 10=-539(F)

OCTOBER 10, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1010061276

Job	Truss	Truss Type	Qty	Ply	0 0
L213465	T15	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Oct 10 13:51:30 2006 Page 1

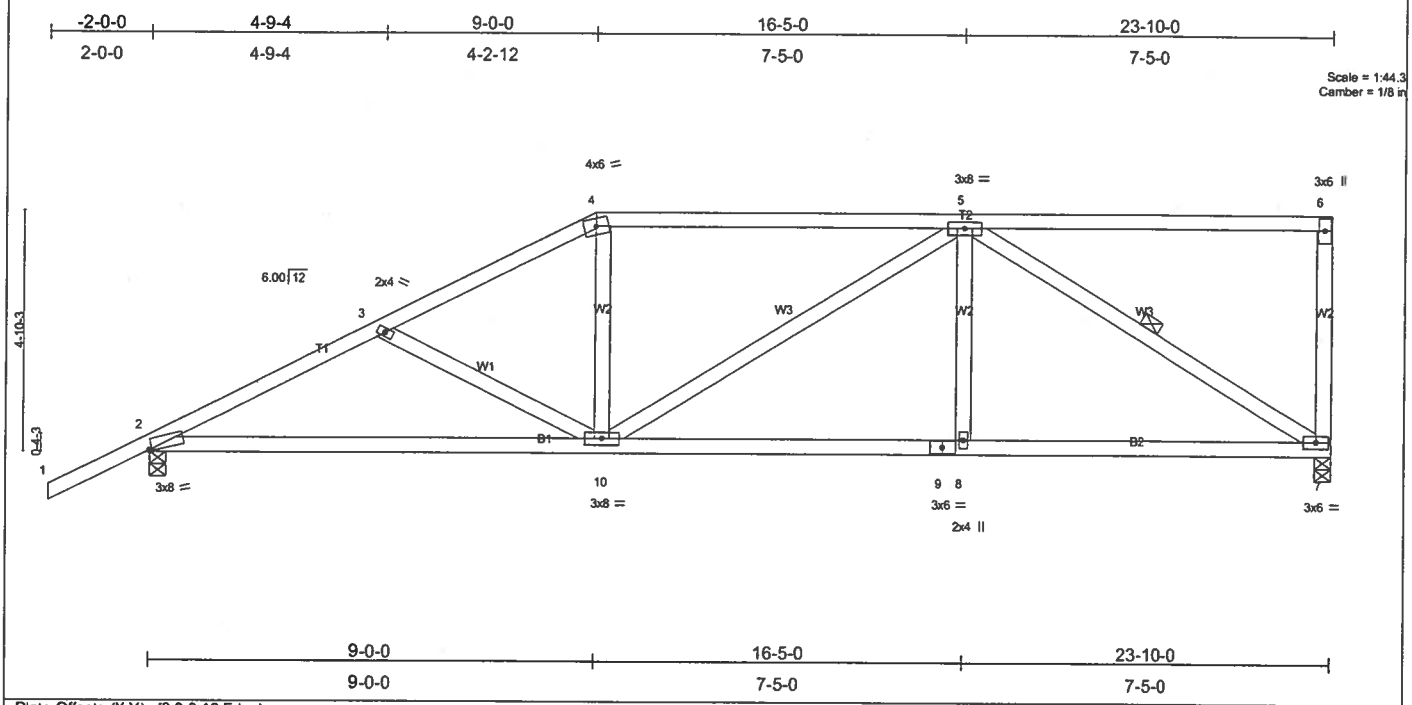


Plate Offsets (X,Y): [2:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.60	Vert(LL)	-0.16	2-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.49	Vert(TL)	-0.27	2-10	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.47	Horz(TL)	0.05	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 127 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-7-2 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-5-15 oc bracing.
WEBS 1 Row at midpt 5-7

REACTIONS (lb/size) 7=982/0-4-0, 2=1110/0-4-0
Max Horz 2=272(load case 5)
Max Uplift7=-354(load case 4), 2=-417(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1669/465, 3-4=-1443/420, 4-5=-1262/409, 5-6=-62/22, 6-7=-181/122
BOT CHORD 2-10=-549/1445, 9-10=-417/1165, 8-9=-417/1165, 7-8=-417/1165
WEBS 3-10=-217/189, 4-10=0/297, 5-10=-86/114, 5-8=0/207, 5-7=-1302/466

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 354 lb uplift at joint 7 and 417 lb uplift at joint 2.

LOAD CASE(S) Standard

OCTOBER 10, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1010061277

Job	Truss	Truss Type	Qty	Ply	0 0
L213465	T16	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Oct 10 13:51:31 2006 Page 1

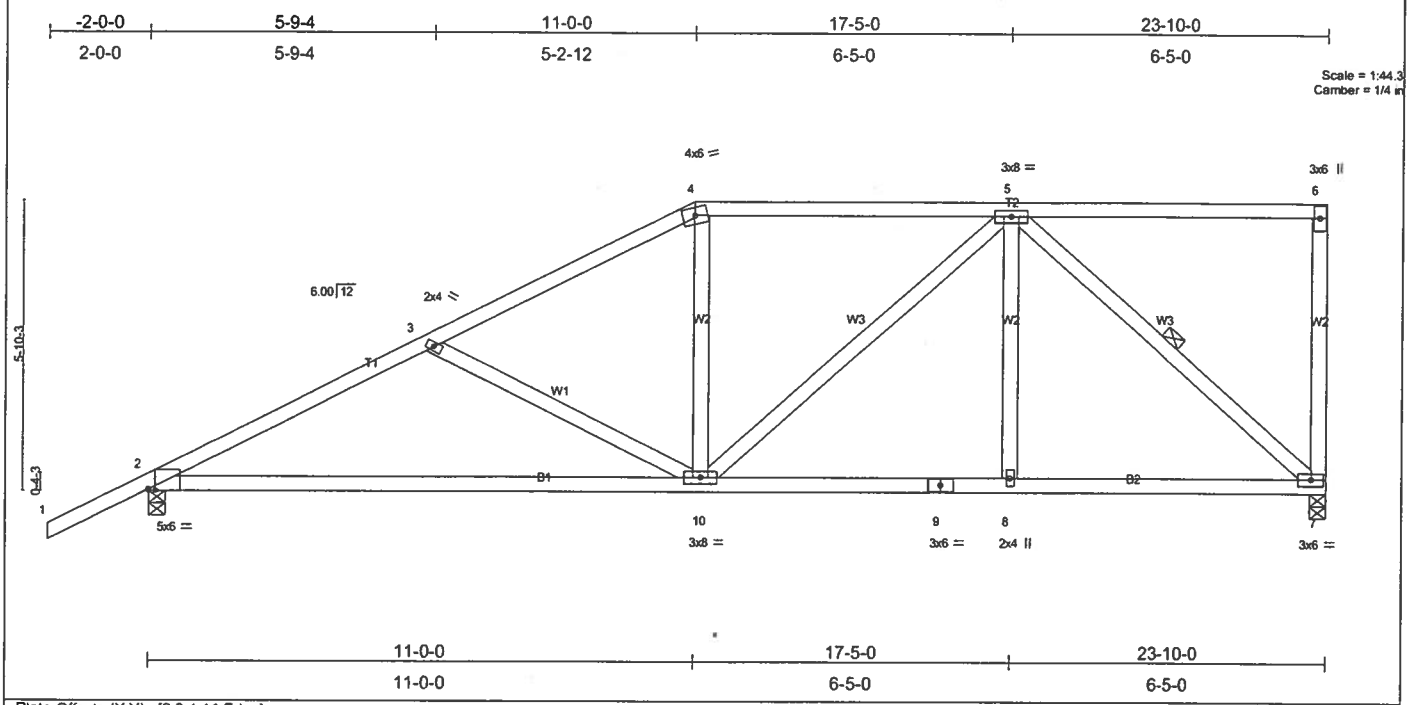


Plate Offsets (X,Y): [2:0-1-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.41	Vert(LL)	-0.33	2-10	>865	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.63	Vert(TL)	-0.56	2-10	>501	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.38	Horz(TL)	0.04	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 132 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-9 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-1-3 oc bracing.
WEBS 1 Row at midpt 5-7

REACTIONS (lb/size) 7=982/0-4-0, 2=1110/0-4-0
Max Horz 2=318(load case 5)
Max Uplift7=-345(load case 4), 2=-423(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1613/478, 3-4=-1289/341, 4-5=-1098/357, 5-6=-36/12, 6-7=-154/104
BOT CHORD 2-10=-597/1400, 9-10=-300/858, 8-9=-300/858, 7-8=-300/858
WEBS 3-10=-346/269, 4-10=0/241, 5-10=-144/321, 5-8=0/150, 5-7=-1098/386

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 345 lb uplift at joint 7 and 423 lb uplift at joint 2.

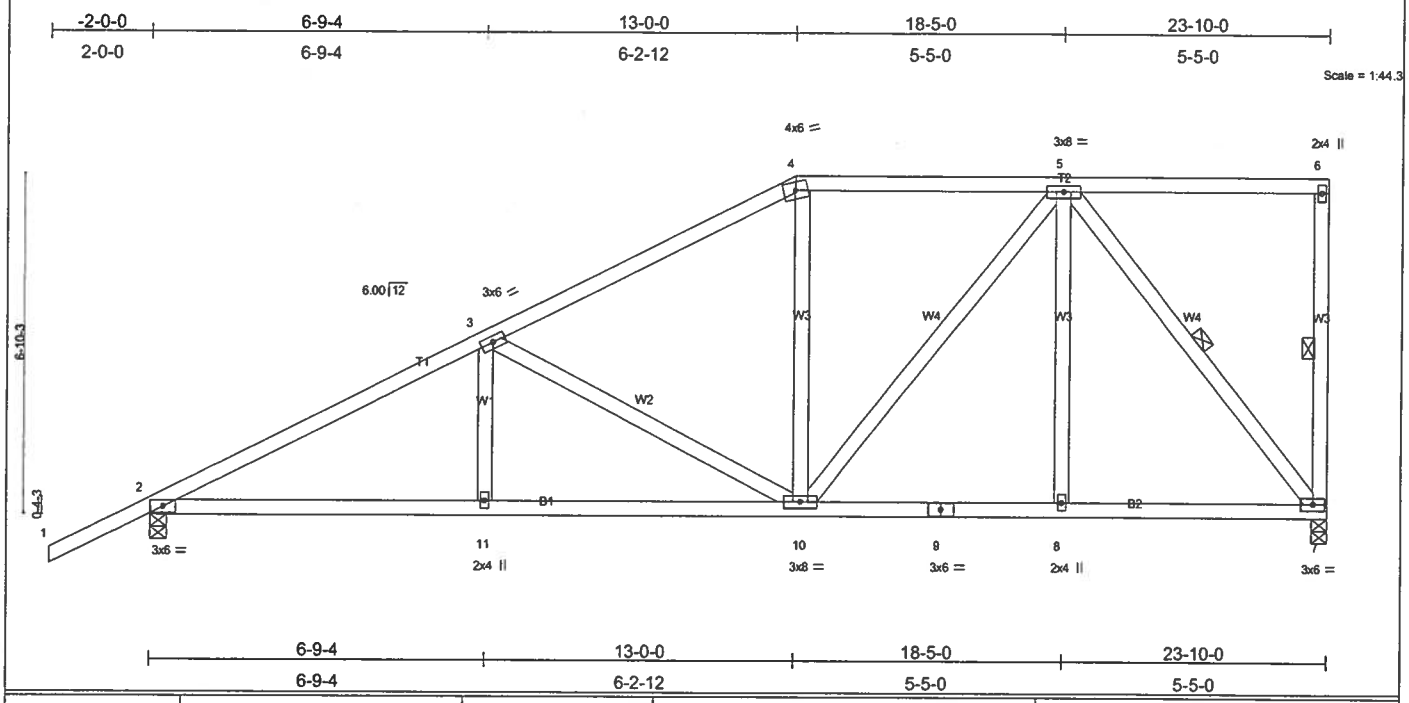
LOAD CASE(S) Standard

OCTOBER 10, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1010061278

Job	Truss	Truss Type	Qty	Ply	0 0
L213465	T17	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Oct 10 13:51:32 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.09	2-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.40	Vert(TL)	-0.14	2-11	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.56	Horz(TL)	0.04	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 143 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-6-9 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-2-0 oc bracing.
WEBS 1 Row at midpt 6-7, 5-7

REACTIONS (lb/size) 7=982/0-4-0, 2=1110/0-4-0
Max Horz 2=364(load case 5)
Max Uplift7=-334(load case 4), 2=-425(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1693/441, 3-4=-1083/310, 4-5=-901/334, 5-6=-19/7, 6-7=-130/87
BOT CHORD 2-11=-603/1436, 10-11=-603/1436, 9-10=-214/642, 8-9=-214/642, 7-8=-214/642
WEBS 3-11=0/215, 3-10=-611/304, 4-10=0/140, 5-10=-210/412, 5-8=0/157, 5-7=-995/332

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 334 lb uplift at joint 7 and 425 lb uplift at joint 2.

LOAD CASE(S) Standard

OCTOBER 10, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1010061279

Job	Truss	Truss Type	Qty	Ply	0 0
L213465	T18	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Oct 10 13:51:33 2006 Page 1

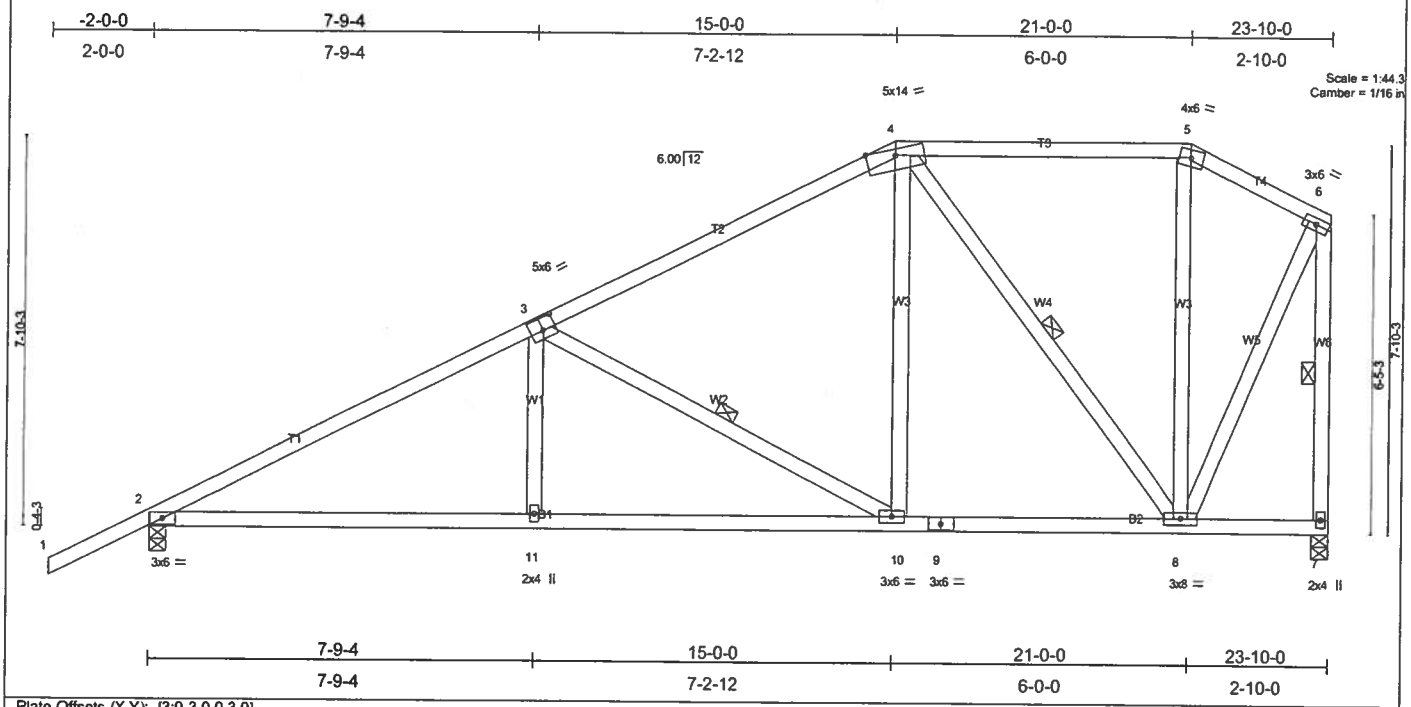


Plate Offsets (X,Y): [3:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	-0.14	2-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.56	Vert(TL)	-0.22	2-11	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.29	Horz(TL)	0.04	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 149 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-14 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-0-12 oc bracing.
WEBS 1 Row at midpt 3-10, 4-8, 6-7

REACTIONS (lb/size) 2=1110/0-4-0, 7=982/0-4-0
Max Horz 2=367(load case 5)
Max Uplift2=-436(load case 5), 7=-310(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1643/448, 3-4=-922/286, 4-5=-342/148, 5-6=-401/143, 6-7=-952/312
BOT CHORD 2-11=-600/1384, 10-11=-601/1378, 9-10=-280/753, 8-9=-280/753, 7-8=-3/4
WEBS 3-11=0/258, 3-10=-724/367, 4-10=-134/543, 4-8=-680/287, 5-8=-150/136, 6-8=-246/817

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 436 lb uplift at joint 2 and 310 lb uplift at joint 7.

LOAD CASE(S) Standard

OCTOBER 10, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1010061280

Job	Truss	Truss Type	Qty	Ply	0 0
L213465	T19	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Oct 10 13:51:34 2006 Page 1

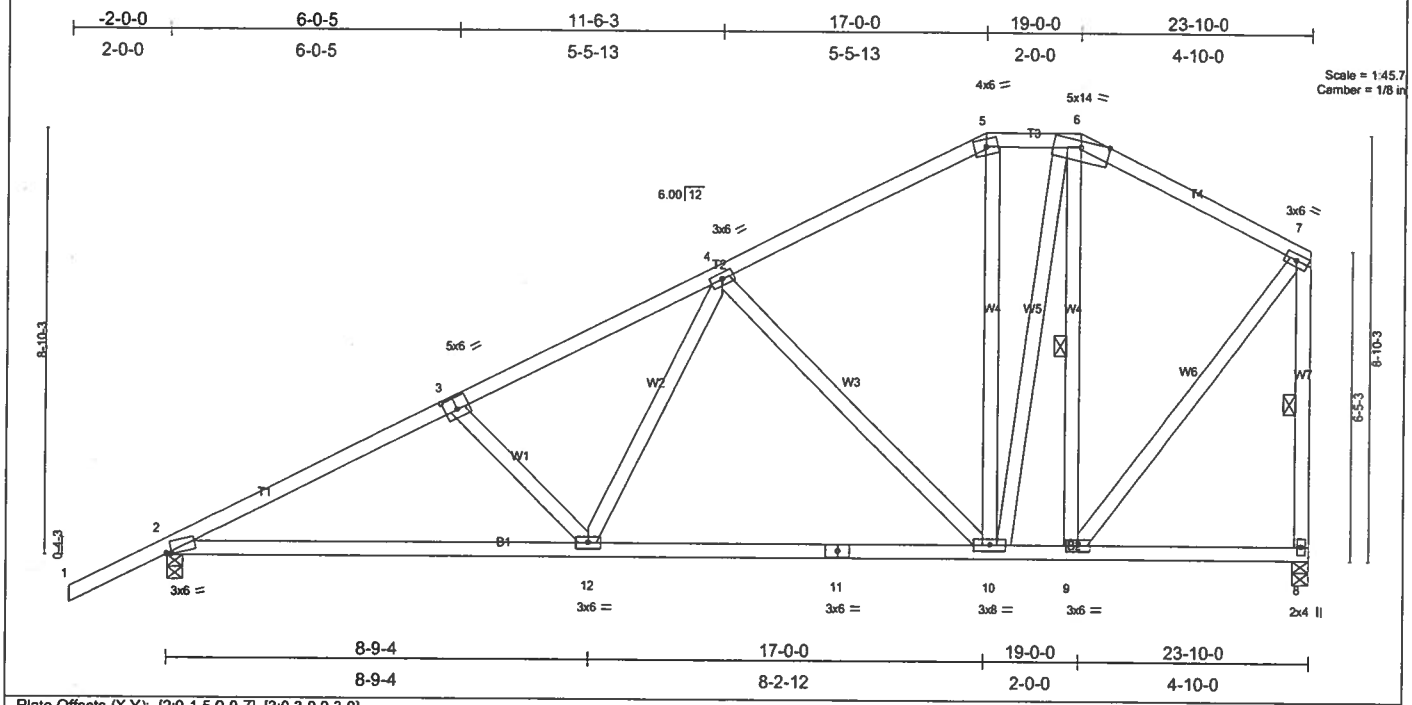


Plate Offsets (X,Y): [2:0-1-5,0-0-7], [3:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.14	2-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.52	Vert(TL)	-0.24	2-12	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.73	Horz(TL)	0.04	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 162 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-7-7 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-7-10 oc bracing.
WEBS 1 Row at midpt 6-9, 7-8

REACTIONS (lb/size) 2=1110/0-4-0, 8=982/0-4-0
Max Horz 2=381(load case 5)
Max Uplift2=-447(load case 5), 8=-355(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1668/508, 3-4=-1447/464, 4-5=-701/274, 5-6=-566/295, 6-7=-563/240, 7-8=-909/367
BOT CHORD 2-12=-686/1424, 11-12=-466/1004, 10-11=-466/1004, 9-10=-166/445, 8-9=-9/19
WEBS 3-12=-281/240, 4-12=-131/537, 4-10=-632/353, 5-10=-25/149, 6-10=-252/621, 6-9=-502/219, 7-9=-267/706

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 447 lb uplift at joint 2 and 355 lb uplift at joint 8.

LOAD CASE(S) Standard

OCTOBER 10, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1010061281

Job	Truss	Truss Type	Qty	Ply	0 0
L213465	T20	COMMON	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Oct 10 13:51:35 2006 Page 1

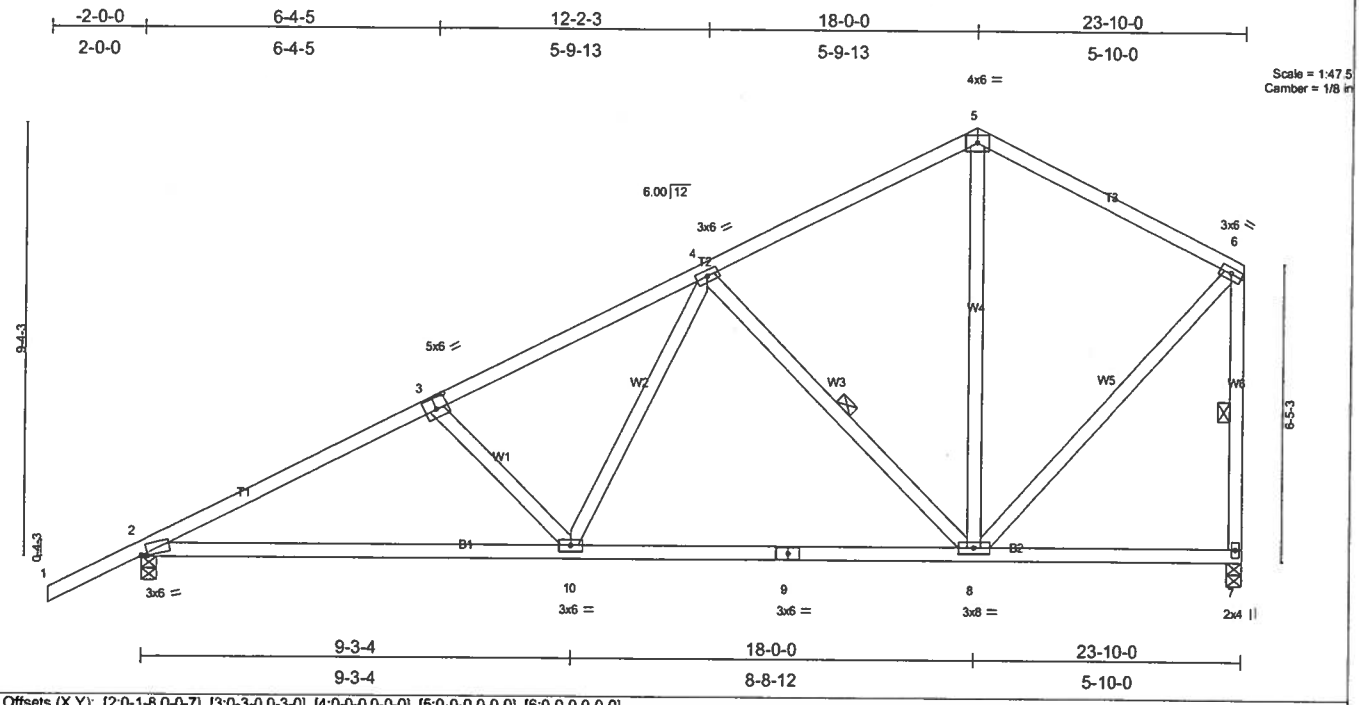


Plate Offsets (X,Y): [2:0-1-8,0-0-7], [3:0-3-0,0-3-0], [4:0-0-0,0-0-0], [5:0-0-0,0-0-0], [6:0-0-0,0-0-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.35	Vert(LL)	-0.17	2-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.55	Vert(TL)	-0.29	2-10	>976	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.32	Horz(TL)	0.03	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 141 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-7-6 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-8-6 oc bracing.
WEBS 1 Row at midpt 4-8, 6-7

REACTIONS (lb/size) 2=1110/0-4-0, 7=982/0-4-0
Max Horz 2=388(load case 5)
Max Uplift2=-441(load case 5), 7=-347(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1634/493, 3-4=-1412/443, 4-5=-629/243, 5-6=-623/258, 6-7=-913/357
BOT CHORD 2-10=-676/1405, 9-10=-436/950, 8-9=-436/950, 7-8=-14/23
WEBS 3-10=-307/260, 4-10=-146/572, 4-8=-666/368, 5-8=-27/237, 6-8=-260/712

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 441 lb uplift at joint 2 and 347 lb uplift at joint 7.

LOAD CASE(S) Standard

OCTOBER 10, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1010061282

Job	Truss	Truss Type	Qty	Ply	0 0
L213465	T22	COFFER	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Oct 10 13:51:36 2006 Page 1

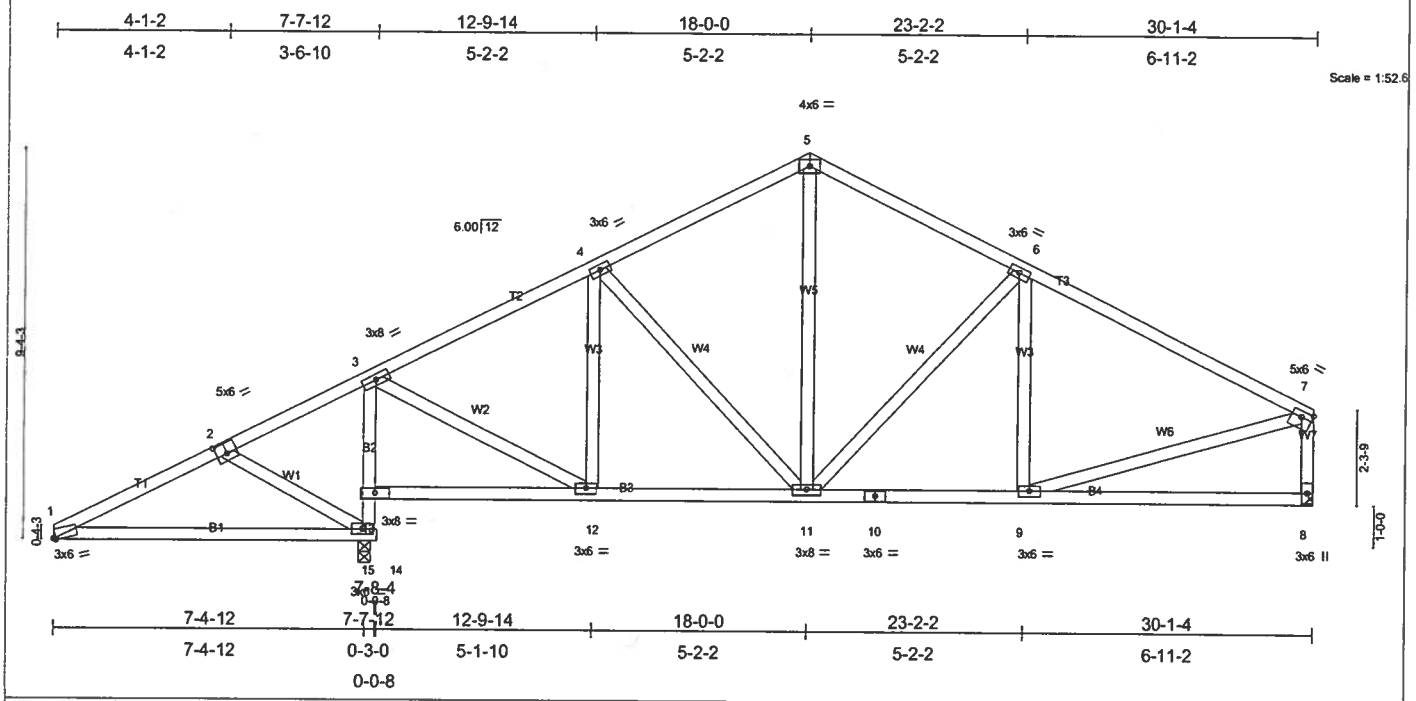


Plate Offsets (X,Y): [1:0-0-10,Edge], [2:0-3-0,0-3-0], [7:Edge,0-1-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.72	Vert(LL)	-0.05	8-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.85	Vert(TL)	-0.08	8-9	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.40	Horz(TL)	0.10	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 176 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 15=1684/0-3-8, 8=838/Mechanical
Max Horz 15=225(load case 5)
Max Uplift15=-775(load case 5), 8=-284(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-389/479, 2-3=-476/695, 3-4=-508/222, 4-5=-648/323, 5-6=-651/311, 6-7=-905/318, 7-8=-730/298
BOT CHORD 1-15=-363/356, 14-15=0/0, 13-15=-1454/596, 3-13=-1330/587, 12-13=-719/554, 11-12=-95/398, 10-11=-196/735, 9-10=-196/735, 8-9=-77/154
WEBS 2-15=-245/221, 3-12=-518/1259, 4-12=-482/323, 4-11=-112/179, 5-11=-129/282, 6-11=-338/218, 6-9=-2/82, 7-9=-124/607

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; cantilever left exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 775 lb uplift at joint 15 and 284 lb uplift at joint 8.

LOAD CASE(S) Standard

OCTOBER 10, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1010061283

Job	Truss	Truss Type	Qty	Ply	0 0
L213465	T23	SPECIAL	1	2	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Oct 10 13:51:38 2006 Page 1

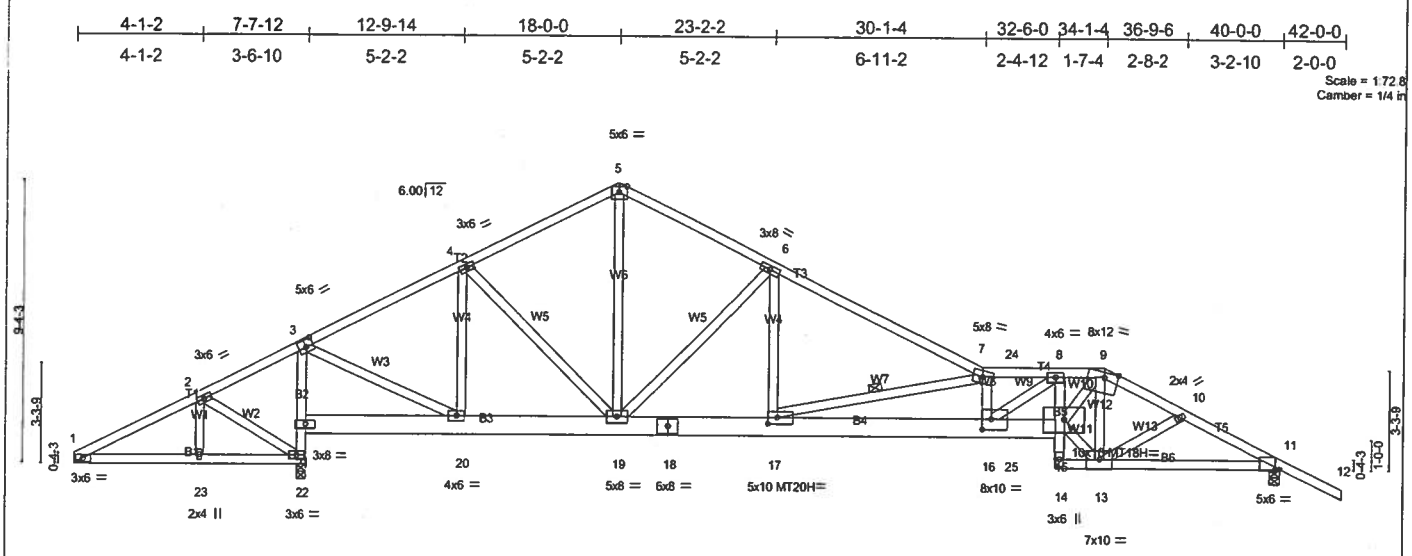


Plate Offsets (X,Y): [3:0-3-0,0-3-0], [9:0-5-3,Edge], [11:0-1-11,Edge], [16:0-3-8,0-4-0], [17:0-3-8,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.59	Vert(LL)	-0.42	16-17	>927	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.87	Vert(TL)	-0.67	16-17	>579	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.95	Horz(TL)	0.19	11	n/a	n/a	MT18H	244/190
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 541 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B3 2 X 8 SYP 2400F 2.0E, B4 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3 *Except*
W11 2 X 4 SYP No.2, W10 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-4-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 7-17

REACTIONS (lb/size) 11=3200/0-4-0, 22=2731/0-3-8
Max Horz 22=-189(load case 5)
Max Uplift11=-1327(load case 5), 22=-1137(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-195/340, 2-3=-434/761, 3-4=-1674/615, 4-5=-2215/893, 5-6=-2226/884, 6-7=-4482/1656, 7-24=-11938/4513, 8-24=-11937/4513, 8-9=-11364/4313, 9-10=-6078/2362, 10-11=-6085/2341, 11-12=0/47
BOT CHORD 1-23=-255/213, 22-23=-255/213, 21-22=-2521/972, 3-21=-2310/902, 20-21=-572/390, 19-20=-317/1438, 18-19=-1195/3962, 17-18=-1195/3962, 16-17=-4325/11952, 16-25=-4260/11757, 15-25=-4259/11767, 14-15=-127/250, 8-15=-48/47, 13-14=-300/796, 11-13=-1933/5302
WEBS 2-23=-151/223, 2-22=-450/355, 3-20=-741/2237, 4-20=-1016/473, 4-19=-240/697, 5-19=-643/1672, 6-19=-2913/1203, 6-17=-947/2717, 7-17=-8266/3239, 7-16=-185/169, 8-16=-123/361, 13-15=-2292/6452, 9-15=-3324/9116, 9-13=-4517/1670, 10-13=-95/192

NOTES
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc, 2 X 8 - 2 rows at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc, Except member 16-7 2 X 4 - 1 row at 0-4-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left exposed ; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.
6) All plates are MT20 plates unless otherwise indicated.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1327 lb uplift at joint 11 and 1137 lb uplift at joint 22.
8) Girder carries hip end with 5-10-12 right side setback, 31-0-0 left side setback, and 5-10-12 end setback.
9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1874 lb down and 708 lb up at 30-3-0, and 363 lb down and 187 lb up at 34-1-4 on bottom chord. The design/selection of such connection device(s) is the responsiblity of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-54, 5-7=-54, 7-24=-54, 9-24=-103(F=-49), 9-12=-54, 1-22=-30, 21-25=-30, 15-25=-57(F=-27), 13-14=-57(F=-27), 11-13=-30
Concentrated Loads (lb)
Vert: 16=-1874(F) 13=-363(F)

OCTOBER 10, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

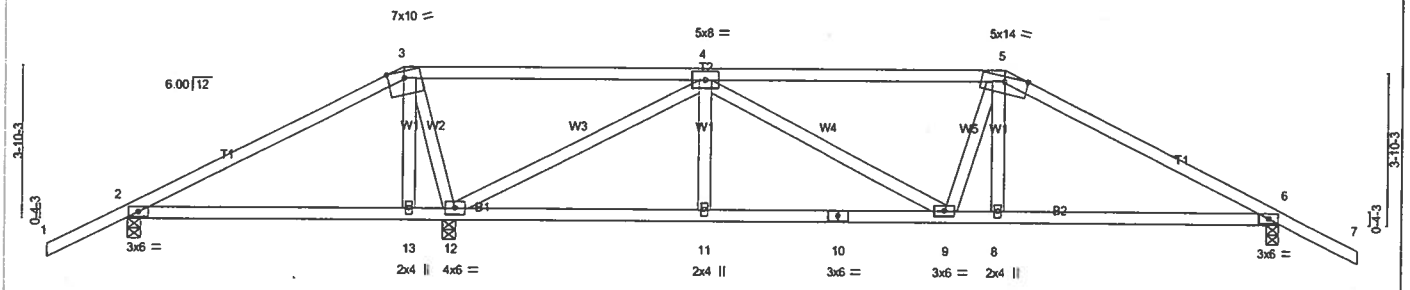
Dwg.#1010061284

Job	Truss	Truss Type	Qty	Ply	0 0
L213465	T25	HIP	1	2	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Oct 10 13:51:39 2006 Page 1

-2-0-0	3-9-4	7-0-0	14-6-14	22-1-12	25-4-8	29-1-12	31-1-12
2-0-0	3-9-4	3-2-12	7-6-14	7-6-14	3-2-12	3-9-4	2-0-0

Scale = 1:55.5



7-0-0	8-2-0	14-6-14	20-11-12	22-1-12	29-1-12
7-0-0	1-2-0	6-4-14	6-4-14	1-2-0	7-0-0

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.59	Vert(LL)	-0.08	9-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.39	Vert(TL)	-0.14	9-11	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.71	Horz(TL)	0.03	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 283 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=-37/0-4-0, 12=3662/0-4-0, 6=1581/0-4-0
Max Horz 2=87(load case 4)
Max Uplift2=-280(load case 9), 12=-1654(load case 3), 6=-732(load case 5)
Max Grav 2=27(load case 2), 12=3662(load case 1), 6=1586(load case 9)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-316/947, 3-4=-514/1254, 4-5=-2380/1049, 5-6=-2728/1161, 6-7=0/47
BOT CHORD 2-13=-784/350, 12-13=-764/334, 11-12=-771/1793, 10-11=-771/1793, 9-10=-771/1793, 8-9=-971/2390, 6-8=-962/2365
WEBS 3-13=-427/531, 3-12=-1658/1033, 4-12=-3482/1611, 4-11=0/403, 4-9=-242/693, 5-9=-38/158, 5-8=-234/613

NOTES
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 280 lb uplift at joint 2, 1654 lb uplift at joint 12 and 732 lb uplift at joint 6.
7) Girder carries hip end with 7-0-0 end setback.
8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 22-1-12, and 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-5=-117(F=-63), 5-7=-54, 2-13=-30, 8-13=-65(F=-35), 6-8=-30
Concentrated Loads (lb)
Vert: 13=-539(F) 8=-539(F)

OCTOBER 10, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

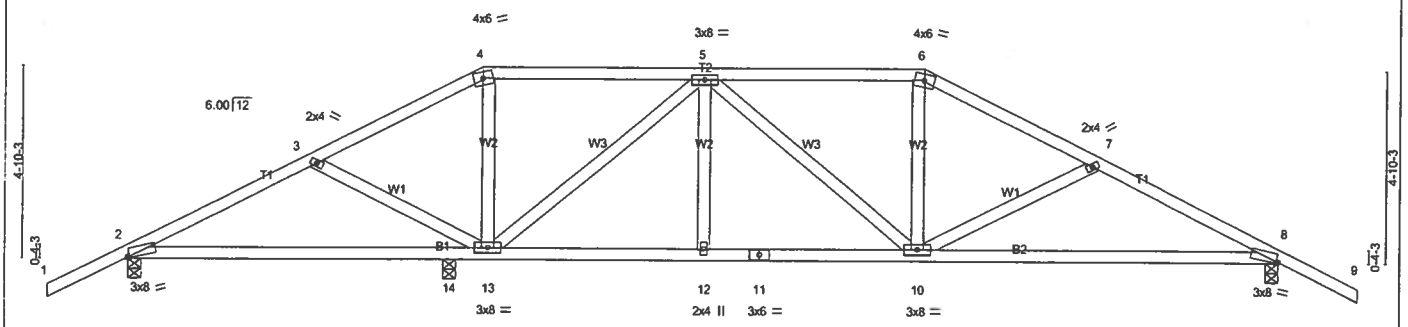
Dwg.#1010061285

Job	Truss	Truss Type	Qty	Ply	0 0
L213465	T26	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Oct 10 13:51:40 2006 Page 1

-2-0-0	4-9-4	9-0-0	14-6-14	20-1-12	24-4-8	29-1-12	31-1-12
2-0-0	4-9-4	4-2-12	5-6-14	5-6-14	4-2-12	4-9-4	2-0-0

Scale = 1:55.5
Camber = 1/8 in



8-2-0	9-0-0	14-6-14	20-1-12	29-1-12
8-2-0	0-10-0	5-6-14	5-6-14	9-0-0

Plate Offsets (X,Y): [2:0-0-10,Edge], [8:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	0.11	2-14	>883	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.84	Vert(TL)	-0.31	8-10	>815	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.70	Horz(TL)	0.06	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 150 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-6-10 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=798/0-4-0, 8=1124/0-4-0, 14=733/0-4-0
Max Horz 2=-101(load case 6)
Max Uplift2=-427(load case 5), 8=-424(load case 6), 14=-323(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1078/335, 3-4=-841/216, 4-5=-712/234, 5-6=-1268/388, 6-7=-1457/385, 7-8=-1699/483, 8-9=0/47
BOT CHORD 2-14=-273/900, 13-14=-273/900, 12-13=-279/1297, 11-12=-279/1297, 10-11=-279/1297, 8-10=-310/1474
WEBS 3-13=-221/233, 4-13=0/101, 5-13=-807/294, 5-12=0/208, 5-10=-145/110, 6-10=-25/360, 7-10=-248/200

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 427 lb uplift at joint 2, 424 lb uplift at joint 8 and 323 lb uplift at joint 14.

LOAD CASE(S) Standard

OCTOBER 10, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

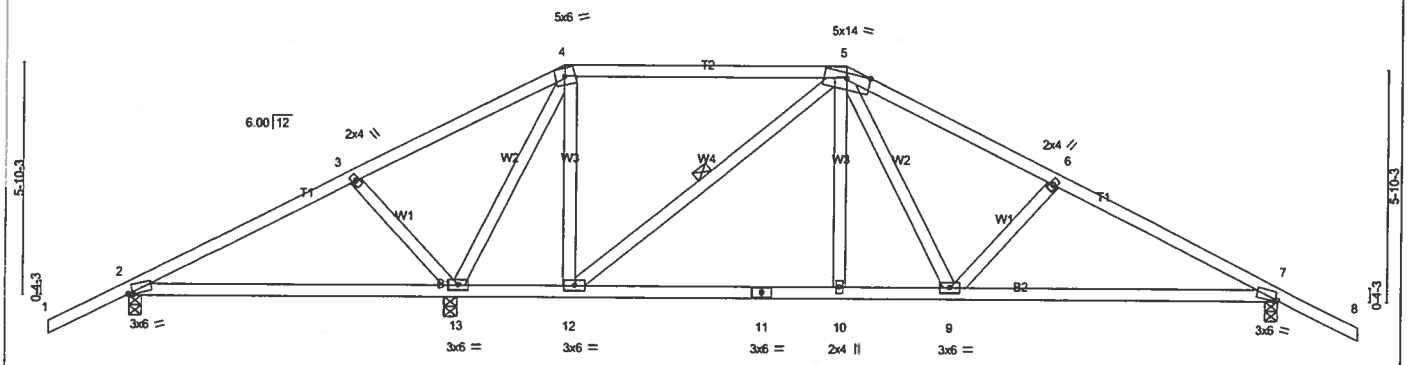
Dwg.#1010061286

Job	Truss	Truss Type	Qty	Ply	0 0
L213465	T27	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Oct 10 13:51:41 2006 Page 1

-2-0-0	5-9-4	11-0-0	18-1-12	23-4-8	29-1-12	31-1-12
2-0-0	5-9-4	5-2-12	7-1-12	5-2-12	5-9-4	2-0-0

Scale = 1:55.5
Camber = 1/16 in



8-2-0	11-0-0	18-1-12	20-11-12	29-1-12
8-2-0	2-10-0	7-1-12	2-10-0	8-2-0

Plate Offsets (X,Y): [2:0-1-5,0-0-7], [7:0-0-13,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.42	Vert(LL)	0.18	2-13	>533	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.37	Vert(TL)	0.14	2-13	>683	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.76	Horz(TL)	0.02	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 154 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-4-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 6-0-0 oc bracing: 2-13.
WEBS 1 Row at midpt 5-12

REACTIONS (lb/size) 2=300/0-4-0, 13=1431/0-4-0, 7=923/0-4-0
Max Horz 2=-115(load case 6)
Max Uplift2=-291(load case 5), 13=-530(load case 5), 7=-397(load case 6)
Max Grav 2=342(load case 9), 13=1431(load case 1), 7=926(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-21/245, 3-4=-65/410, 4-5=-205/142, 5-6=-1066/365, 6-7=-1257/407, 7-8=0/47
BOT CHORD 2-13=-194/151, 12-13=0/193, 11-12=-84/722, 10-11=-84/722, 9-10=-85/719, 7-9=-218/1073
WEBS 3-13=-286/264, 4-13=-1132/244, 4-12=-57/525, 5-12=-660/227, 5-10=0/130, 5-9=-136/419, 6-9=-258/231

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 291 lb uplift at joint 2, 530 lb uplift at joint 13 and 397 lb uplift at joint 7.

LOAD CASE(S) Standard

OCTOBER 10, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

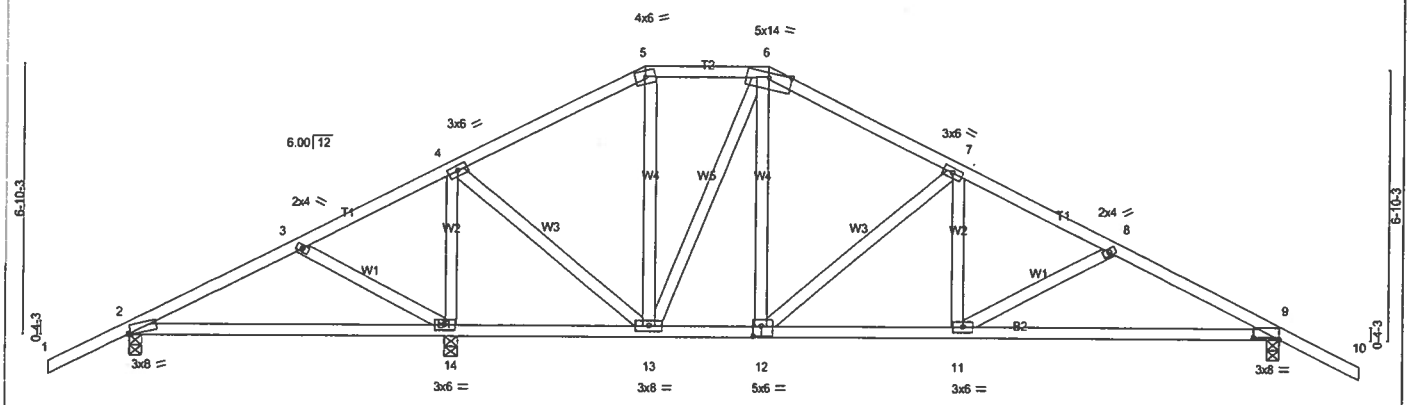
Dwg.#1010061287

Job	Truss	Truss Type	Qty	Ply	0 0
L213465	T28	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Oct 10 13:51:42 2006 Page 1

-2-0-0	4-4-4	8-2-0	13-0-0	16-1-12	20-11-12	24-9-8	29-1-12	31-1-12
2-0-0	4-4-4	3-9-12	4-10-0	3-1-12	4-10-0	3-9-12	4-4-4	2-0-0

Scale = 1:55.5
Camber = 1/16 in



8-2-0	13-0-0	16-1-12	20-11-12	29-1-12
8-2-0	4-10-0	3-1-12	4-10-0	8-2-0

Plate Offsets (X,Y): [2:0-0-10,Edge], [9:0-8-0,0-0-10], [12:0-2-8,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	0.17	2-14	>565	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.40	Vert(TL)	0.14	2-14	>689	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.42	Horz(TL)	0.02	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 169 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-4-14 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=281/0-4-0, 14=1449/0-4-0, 9=925/0-4-0
Max Horz 2=-129(load case 6)
Max Uplift2=-286(load case 5), 14=-550(load case 5), 9=-409(load case 6)
Max Grav 2=335(load case 9), 14=1449(load case 1), 9=925(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-59/229, 3-4=-95/391, 4-5=-441/217, 5-6=-339/224, 6-7=-634/283, 7-8=-1094/374, 8-9=-1305/447, 9-10=0/47
BOT CHORD 2-14=-177/119, 13-14=-305/261, 12-13=0/524, 11-12=-128/938, 9-11=-264/1123
WEBS 3-14=-259/212, 4-14=-1103/375, 4-13=-157/838, 5-13=-93/82, 6-13=-478/186, 6-12=-161/447, 7-12=-551/252, 7-11=-18/318, 8-11=-212/154

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 286 lb uplift at joint 2, 550 lb uplift at joint 14 and 409 lb uplift at joint 9.

LOAD CASE(S) Standard

OCTOBER 10, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

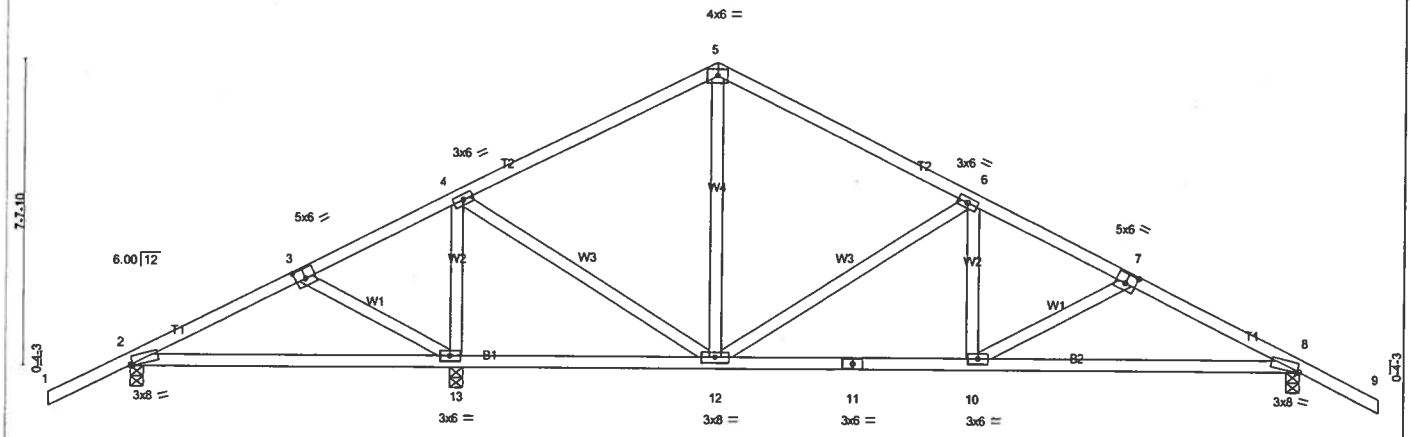
Dwg.#1010061288

Job	Truss	Truss Type	Qty	Ply	0 0
L213465	T29	COMMON	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Oct 10 13:51:43 2006 Page 1

-2-0-0	4-4-4	8-2-0	14-6-14	20-11-12	24-9-8	29-1-12	31-1-12
2-0-0	4-4-4	3-9-12	6-4-14	6-4-14	3-9-12	4-4-4	2-0-0

Scale = 1:54.8
Camber = 1/16 in



8-2-0	14-6-14	20-11-12	29-1-12
8-2-0	6-4-14	6-4-14	8-2-0

Plate Offsets (X,Y): [2:0-0-10,Edge], [3:0-3-0,0-3-0], [7:0-3-0,0-3-0], [8:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.33	Vert(LL)	0.18	2-13	>523	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.38	Vert(TL)	0.15	2-13	>625	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.66	Horz(TL)	0.02	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 155 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-4-11 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=289/0-4-0, 13=1438/0-4-0, 8=928/0-4-0
Max Horz 2=-140(load case 6)
Max Uplift2=-285(load case 5), 13=-563(load case 5), 8=-416(load case 6)
Max Grav 2=342(load case 9), 13=1438(load case 1), 8=928(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-63/212, 3-4=-92/365, 4-5=-549/276, 5-6=-549/256, 6-7=-1108/397, 7-8=-1305/456, 8-9=0/47
BOT CHORD 2-13=-162/122, 12-13=-279/259, 11-12=-153/956, 10-11=-153/956, 8-10=-270/1122
WEBS 3-13=-238/192, 4-13=-1087/405, 4-12=-180/839, 5-12=-16/123, 6-12=-650/309, 6-10=-17/322, 7-10=-190/133

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 285 lb uplift at joint 2, 563 lb uplift at joint 13 and 416 lb uplift at joint 8.

LOAD CASE(S) Standard

OCTOBER 10, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1010061289

Job	Truss	Truss Type	Qty	Ply	0 0
L213465	T30	COMMON	6	1	Job Reference (optional)

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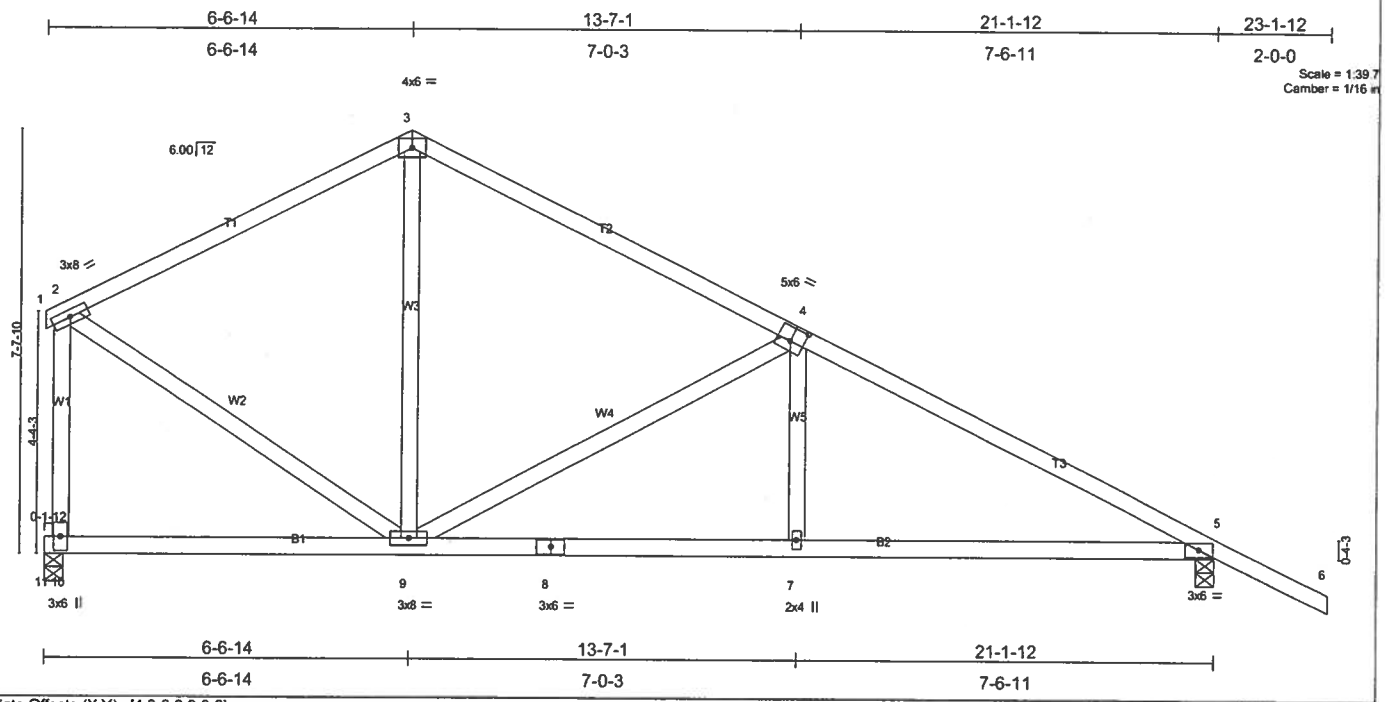

Plate Offsets (X,Y): [4:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.55	Vert(LL)	-0.12	5-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.49	Vert(TL)	-0.19	5-7	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.79	Horz(TL)	0.03	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 114 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-10-5 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 10=876/0-4-0, 5=992/0-4-0
Max Horz 10=-295(load case 6)
Max Uplift10=-280(load case 6), 5=-413(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/2, 2-3=-690/270, 3-4=-708/250, 4-5=-1395/403, 5-6=0/47, 2-10=-776/295
BOT CHORD 10-11=0/0, 9-10=0/283, 8-9=-195/1159, 7-8=-195/1159, 5-7=-195/1164
WEBS 3-9=-10/244, 4-9=-708/351, 4-7=0/246, 2-9=-206/609

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 280 lb uplift at joint 10 and 413 lb uplift at joint 5.

LOAD CASE(S) Standard

OCTOBER 10, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1010061290

Job	Truss	Truss Type	Qty	Ply	0 0
L213465	T31	COMMON	3	1	Job Reference (optional)

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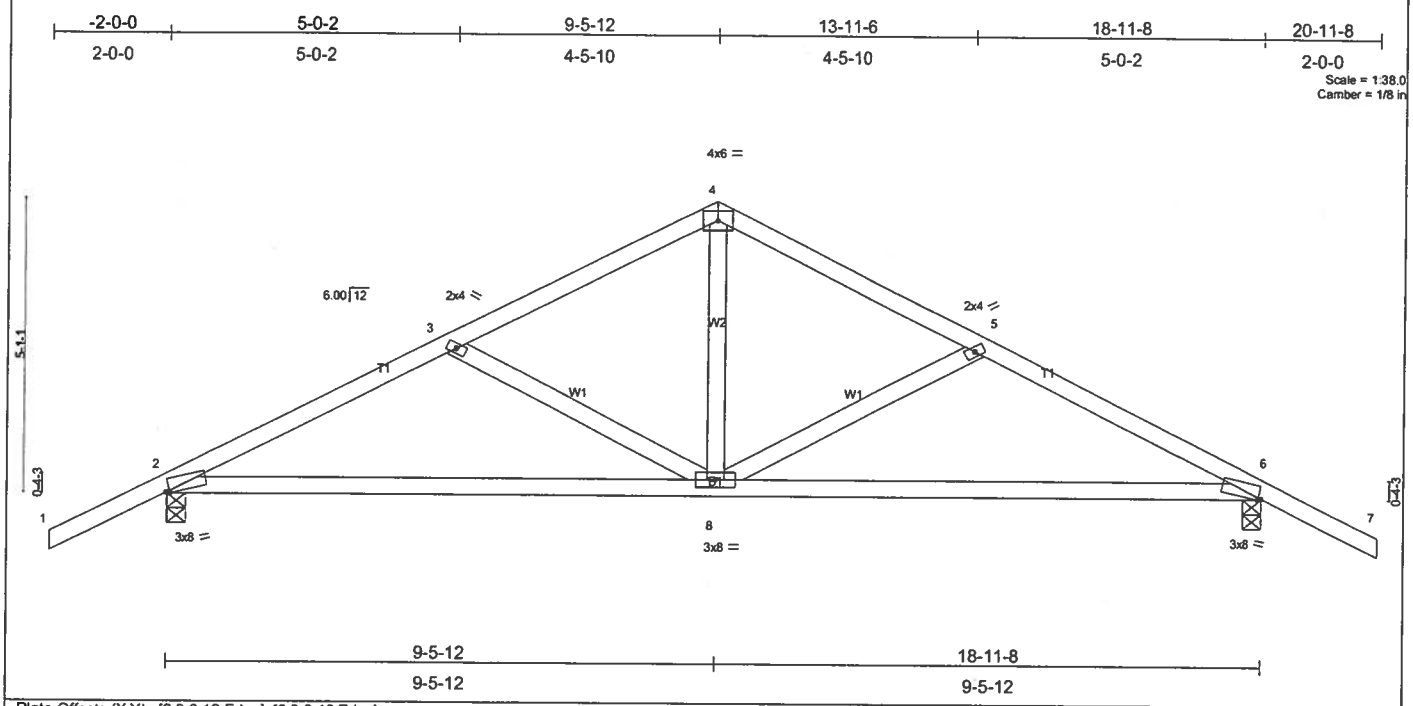

Plate Offsets (X,Y): [2:0-0-10,Edge], [6:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	0.33	2-8	>673	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.51	Vert(TL)	0.27	2-8	>822	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.28	Horz(TL)	0.03	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 88 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-4-11 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-6-12 oc bracing.

REACTIONS (lb/size) 2=899/0-4-0, 6=899/0-4-0
Max Horz 2=-105(load case 6)
Max Uplift2=-615(load case 5), 6=-615(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1224/1022, 3-4=-959/920, 4-5=-959/920, 5-6=-1224/1022, 6-7=0/47
BOT CHORD 2-8=-787/1052, 6-8=-787/1052
WEBS 3-8=-299/250, 4-8=-699/565, 5-8=-299/251

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 615 lb uplift at joint 2 and 615 lb uplift at joint 6.

LOAD CASE(S) Standard

OCTOBER 10, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1010061291

Job	Truss	Truss Type	Qty	Ply	0 0
L213465	T31G	GABLE	1	1	Job Reference (optional)

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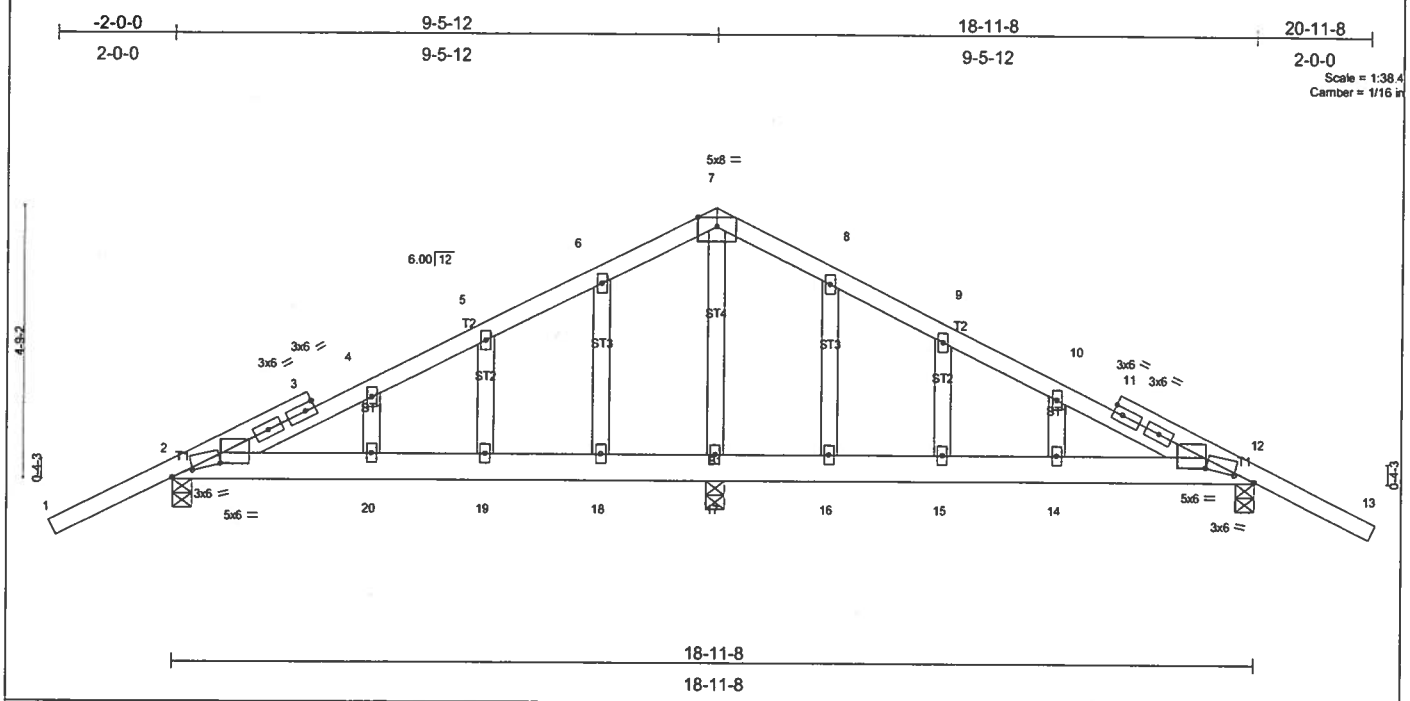

Plate Offsets (X,Y): [2:0-4-7,0-0-7], [2:0-10-0,0-3-0], [12:0-4-7,0-0-7], [12:0-10-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.61	Vert(LL)	0.19	19-20	>575	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.80	Vert(TL)	-0.24	19-20	>470	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.32	Horz(TL)	0.01	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 112 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 8-0-4 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=566/0-4-0, 12=566/0-4-0, 17=2060/0-4-0
Max Horz 2=-103(load case 6)
Max Uplift2=-435(load case 5), 12=-447(load case 6), 17=-1309(load case 5)
Max Grav 2=617(load case 9), 12=617(load case 10), 17=2060(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-46/111, 2-3=-540/561, 3-4=-555/609, 4-5=-545/633, 5-6=-530/666, 6-7=-503/677, 7-8=-503/677, 8-9=-530/666, 9-10=-545/633, 10-11=-555/609, 11-12=-540/561, 12-13=-46/111
BOT CHORD 2-20=-525/622, 19-20=-525/622, 18-19=-525/622, 17-18=-525/622, 16-17=-525/622, 15-16=-525/622, 14-15=-525/622, 12-14=-525/622
WEBS 7-17=-927/741, 6-18=-205/158, 5-19=-176/143, 4-20=-196/138, 8-16=-205/157, 9-15=-176/143, 10-14=-196/141

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) All plates are 2x4 MT20 unless otherwise indicated.
5) Gable studs spaced at 2-0-0 oc.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 435 lb uplift at joint 2, 447 lb uplift at joint 12 and 1309 lb uplift at joint 17.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-7=-114(F=-60), 7-13=-114(F=-60), 2-12=-30

OCTOBER 10, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1010061292

Job	Truss	Truss Type	Qty	Ply	
L213465	T32	COMMON	1	2	0 0 Job Reference (optional)

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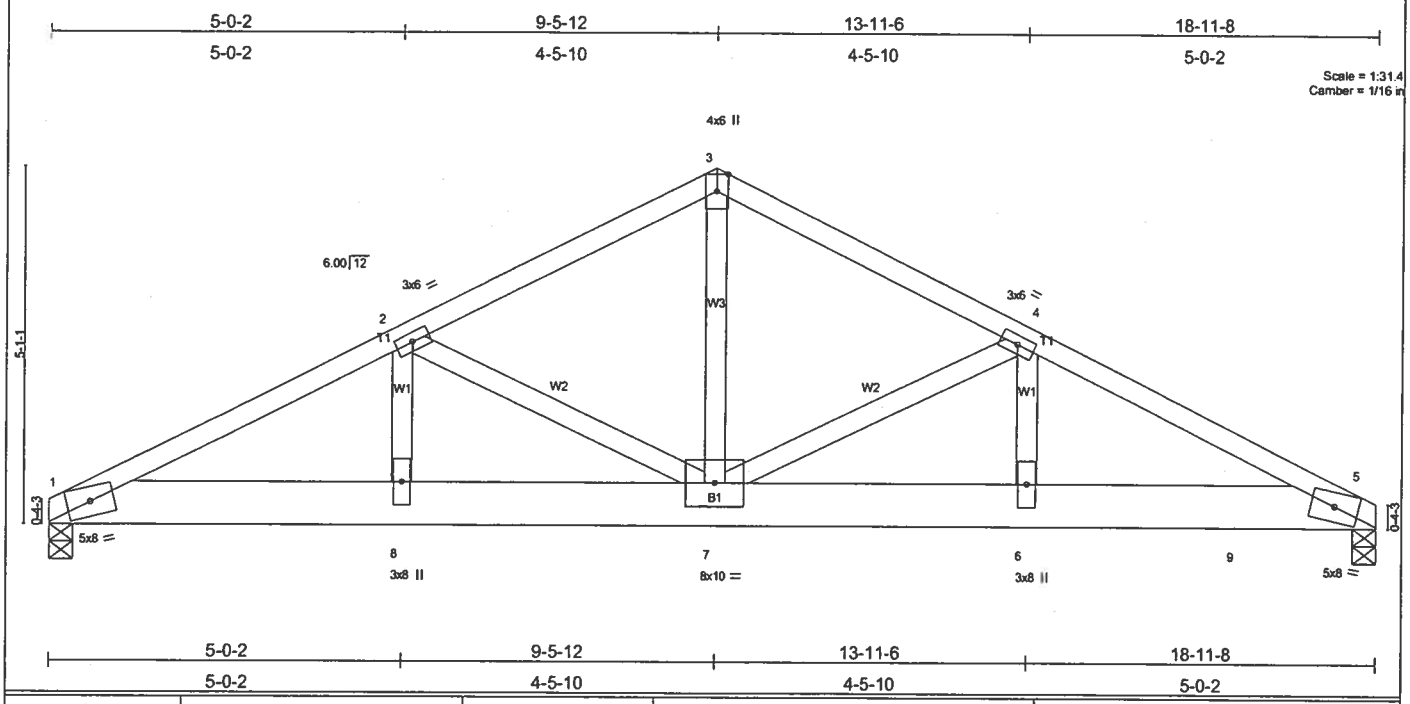


LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.40	Vert(LL)	-0.14	7-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.39	Vert(TL)	-0.23	7-8	>991	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.86	Horz(TL)	0.05	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 232 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-1-14 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=5380/0-4-0, 5=5499/0-4-0
Max Horz 1=67(load case 3)
Max Uplift1=-1991(load case 4), 5=-2036(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-9081/3344, 2-3=-6373/2384, 3-4=-6373/2384, 4-5=-9117/3359
BOT CHORD 1-8=-3002/8105, 7-8=-3002/8105, 6-7=-2951/8137, 6-9=-2951/8137, 5-9=-2951/8137
WEBS 2-8=-774/2239, 2-7=-2779/1116, 3-7=-1967/5376, 4-7=-2816/1131, 4-6=-787/2271

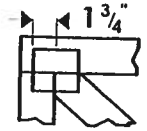
NOTES
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-7-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1991 lb uplift at joint 1 and 2036 lb uplift at joint 5.
6) Girder carries tie-in span(s): 25-9-12 from 0-0-0 to 17-0-0; 29-3-12 from 17-0-0 to 18-11-8

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-5=-54, 1-9=-523(F=-493), 5-9=-597(F=-567)

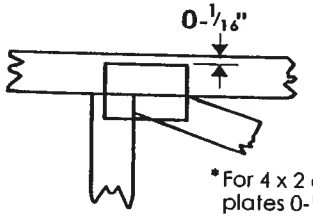
OCTOBER 10, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and securely seat.



*For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

*This symbol indicates the required direction of slots in connector plates.

***Plate location details available in MiTek 20/20 software or upon request.**

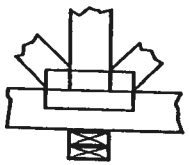
PLATE SIZE

4 X 4 — The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

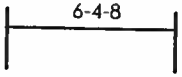


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

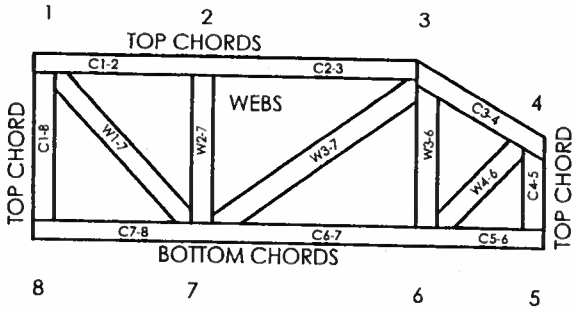
Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI1: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 95-43, 96-20-1, 96-67, 84-32
ICBO	4922, 5243, 5363, 3907
SBCCI	9667, 9730, 9604B, 9511, 9432A

MiTek®

TEE-LOK®

MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI1.
2. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
3. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
4. Cut members to bear tightly against each other.
5. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI1.
6. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI1.
7. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
8. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
9. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
10. Plate type, size, orientation and location dimensions shown indicate minimum plating requirements.
11. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
12. Top chords must be sheathed or purlins provided at spacing shown on design.
13. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
14. Connections not shown are the responsibility of others.
15. Do not cut or alter truss member or plate without prior approval of a professional engineer.
16. Install and load vertically unless indicated otherwise.

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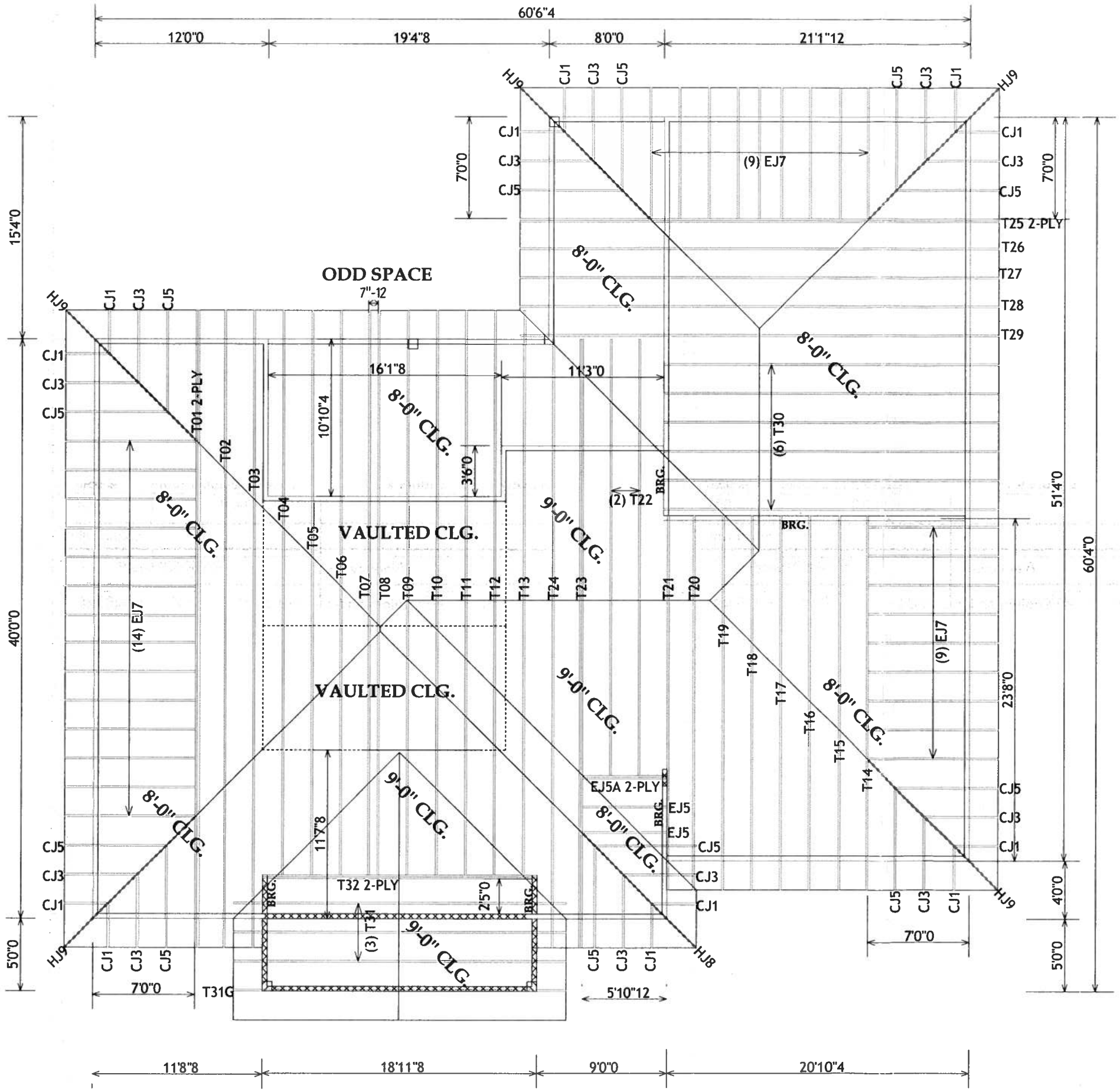

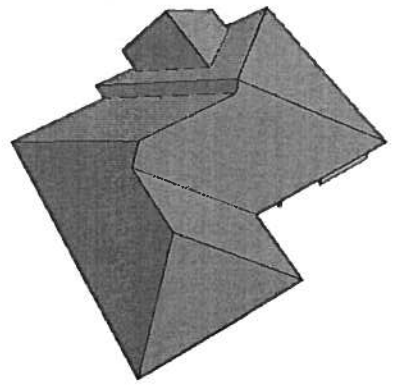

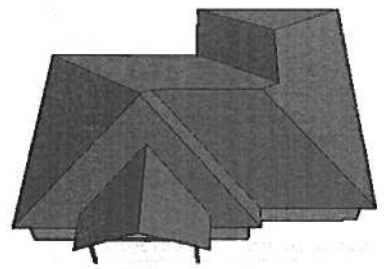

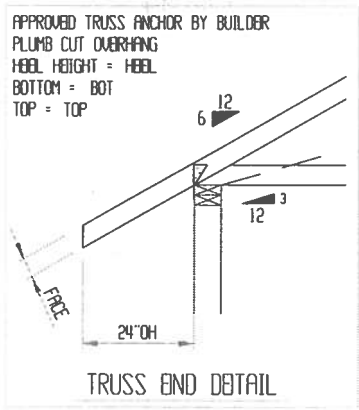

TRUSS END DETAIL

HANGER SCHEDULE

(1) HGUS26-2
(12) HTU26

BEARING HEIGHT SCHEDULE

8'-0"

9'-0"

OVERHANG

2'-0"

ROOF PITCH(S)

6/12

NOTES:

1) REFER TO HIB 91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BRACING.) REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED.

2.) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL VIO5 FOR ALTERNATE BRACING REQUIREMENTS.

3.) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.

4.) ALL TRUSSES ARE DESIGNED FOR 2' o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.

5.) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.

6.) SY42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.

7.) ALL ROOF TRUSS HANGERS TO BE SIMPSON HUS26 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSS HANGERS TO BE SIMPSON THA422 UNLESS OTHERWISE NOTED.

8.) BEAM/HEADER/LINTEL (HDR) TO BE FURNISHED BY BUILDER.

SHOP DRAWING APPROVAL

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION OF TRUSSES AND VOIDS ALL PREVIOUS ARCHITECTURAL OR OTHER TRUSS LAYOUTS. REVIEW AND APPROVAL OF THIS LAYOUT MUST BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VERIFY ALL CONDITIONS TO INSURE AGAINST CHANGES THAT WILL RESULT IN EXTRA CHARGES TO YOU.

Requested Delivery Date : _____

Approved by _____ Date: _____

Builders FirstSource

Bunnell
PHONE: 904-437-3349 FAX: 904-437 3994

Jacksonville
PHONE: 904-772-6100 FAX: 904-772 1973

Lake City
PHONE: 904-755-6894 FAX: 904-755-7973

Sanford
PHONE: 407-322-0059 FAX: 407-322-5553

BUILDER: **NORTON PICCADILY PARK SOUTH**		
LEGAL ADDRESS: **LOT 13**		
MODEL: **CUSTOM**		REVISION:
		SCALE: NTS
DATE: **10/10/06**	DRAWN BY: AM	JOB #: **L213465**