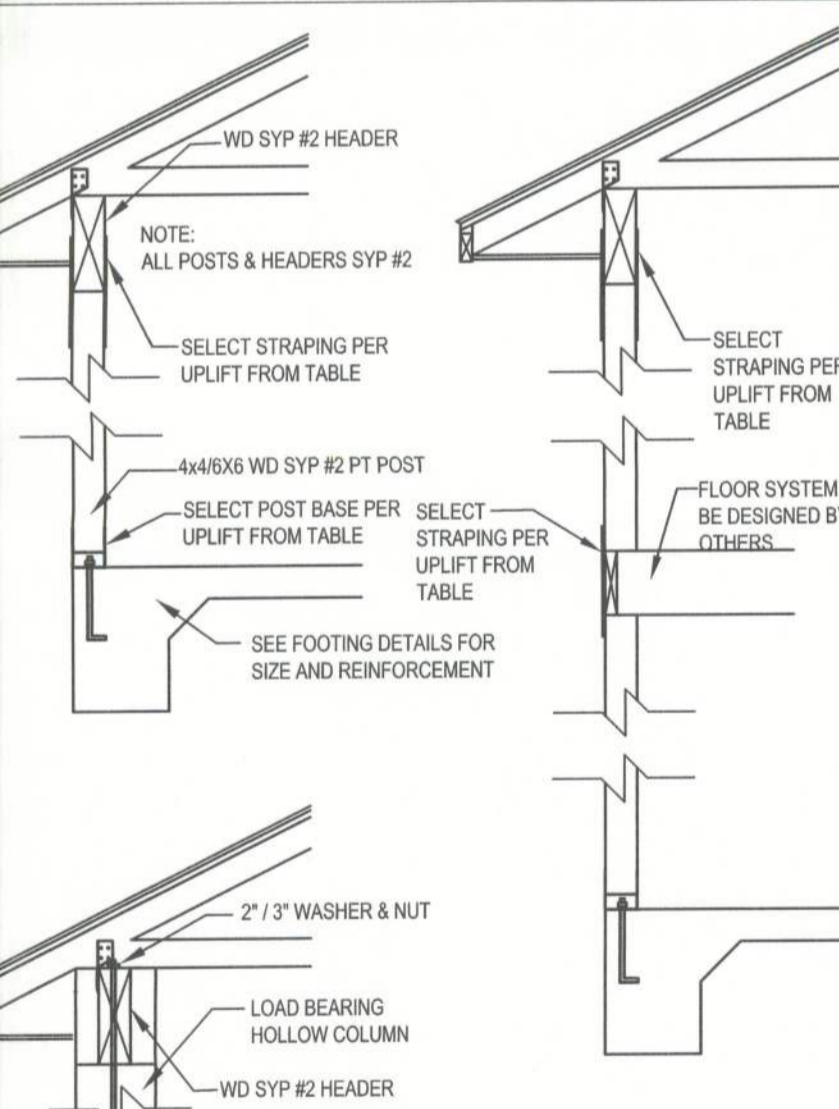


STUD ANCHOR TABLE			
TYPICAL TRUSS UPLIFT & MAX 12" WALL HEIGHT	ANCHOR BOLT SPACING	SP4 / SP5 SPACING	ALTERNATE SP4/SP5/SP6 SPACING
770 LB	48" O.C.	48" O.C.	N/A
850 LB	48" O.C.	32" O.C.	N/A
1270 LB	32" O.C.	16" O.C.	32" O.C.
1550 LB	24" O.C.	16" O.C.	16" O.C.
2200 LB	LTT01 W/ 5" X 7" WEDGE ANCHOR	N/A	(2) HTS20 NAILED TO STUD PACK

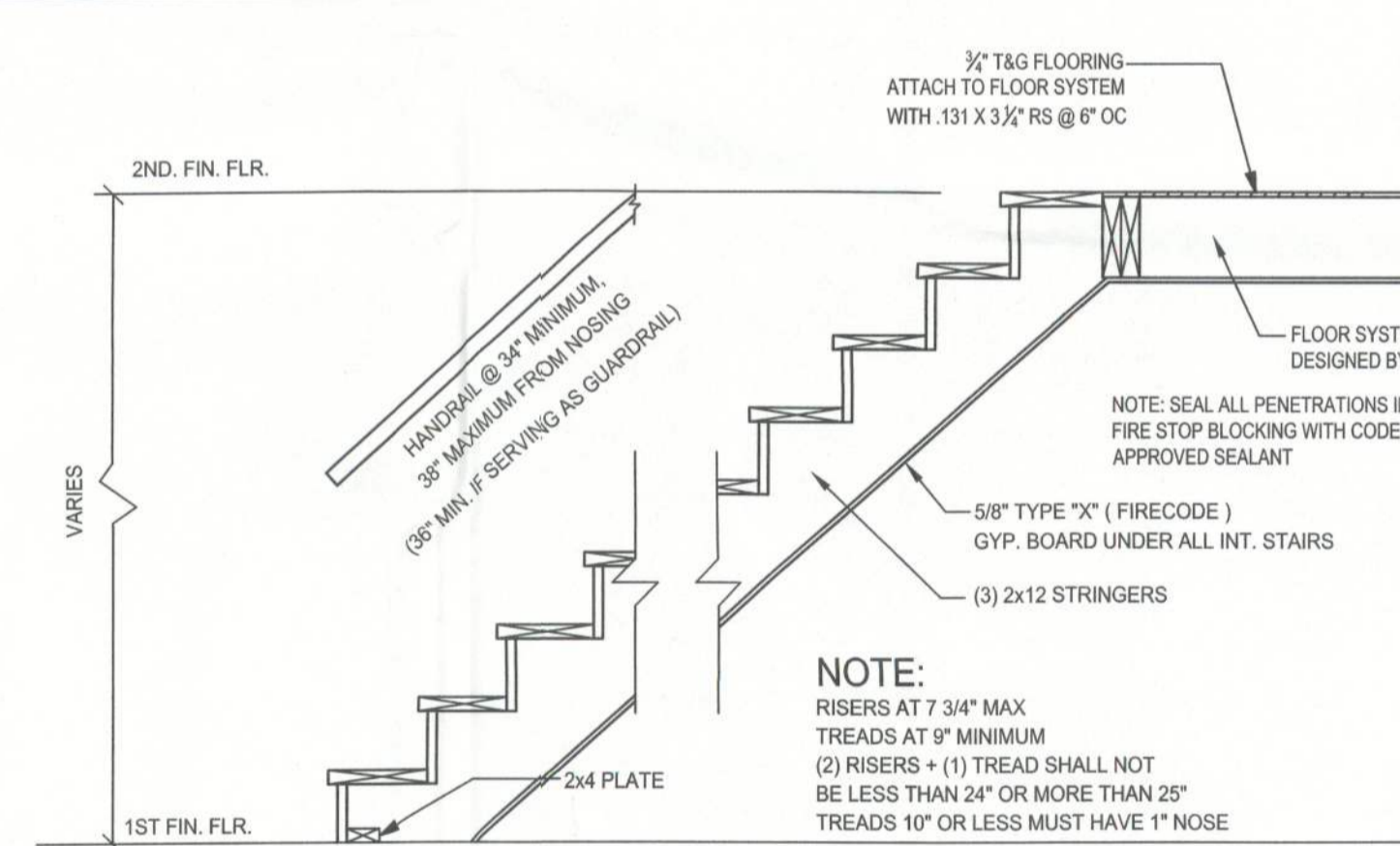
NOTE: SP2 TOP & SP1 BOTTOM ALTERNATE FOR SP4/6

NOTE: MINIMUM ANCHOR BOLT SPACING FOR WALLS WITH A HEIGHT GREATER THAN 10'-0" AND LESS THAN 14'-0" SHALL BE 32" O.C.

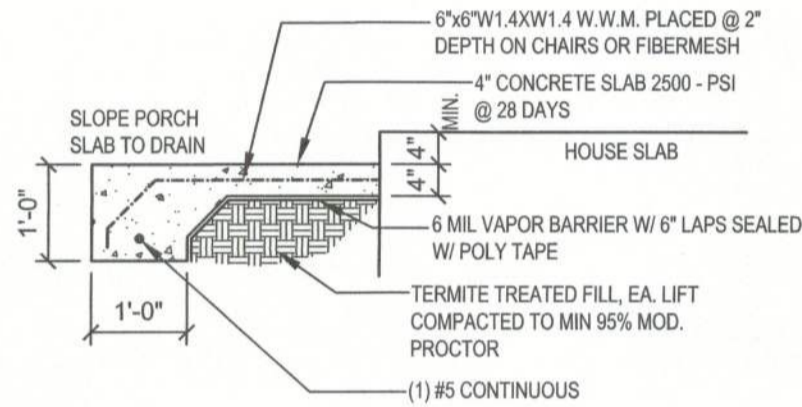
W1 - SINGLE STORY EXT. WALL SECTION
SCALE: 1/2"=1'-0" REV-22-AUG-03



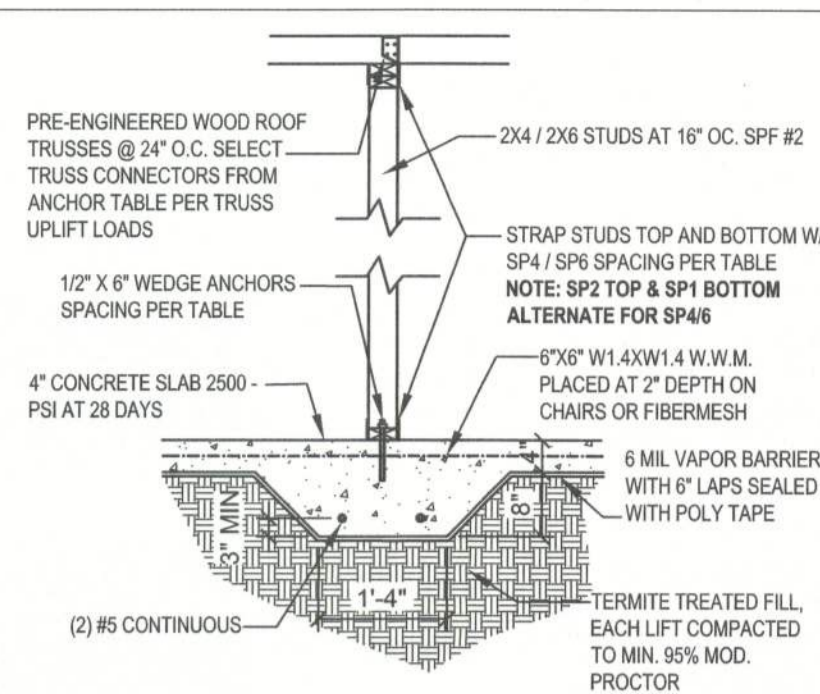
W2 - 2 STORY EXT. WALL SECTION
SCALE: 1/2"=1'-0" REV-22-AUG-03



F1 - STEM WALL FOUNDATION
SCALE: 1/2"=1'-0" REV-27-MAY-03

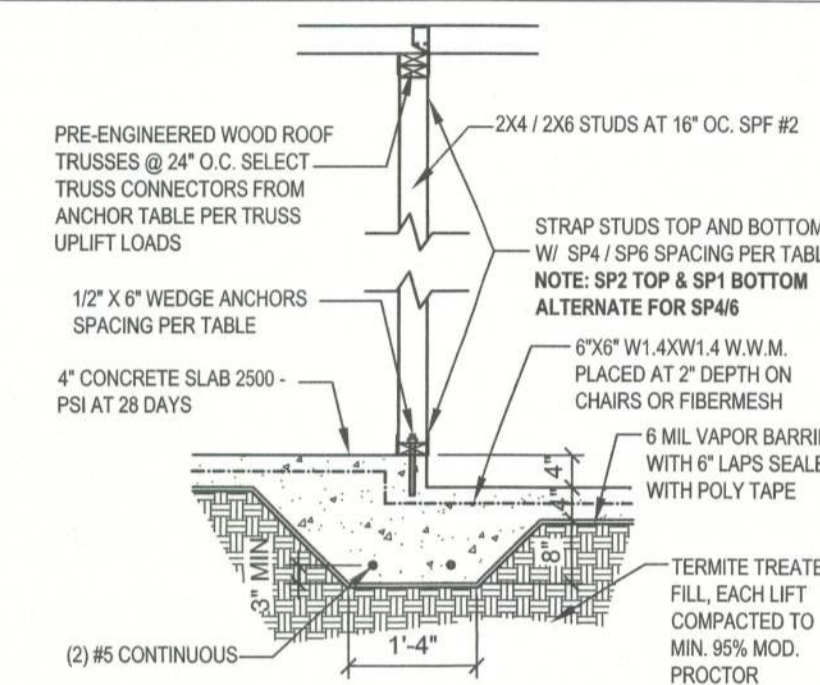


F2 - PORCH SLAB
SCALE: 1/2"=1'-0" REV-22-AUG-03



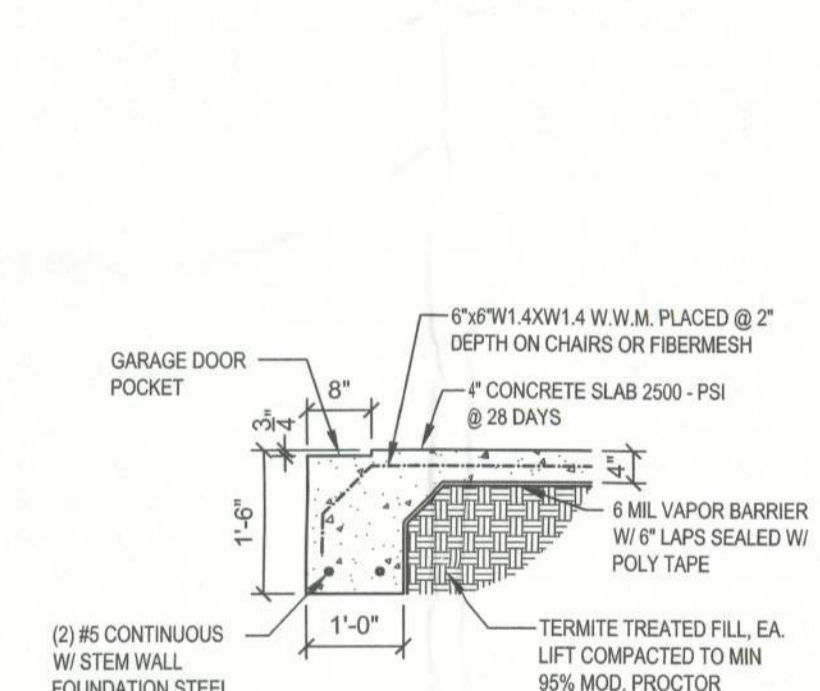
TYPICAL TRUSS UPLIFT	WEDGE ANCHOR SPACING	SP4 / SP5 SPACING	TRUSS CONNECTOR
400 LB	48" O.C.	48" O.C.	H2SA
600 LB	48" O.C.	32" O.C.	H10
1000 LB	32" O.C.	16" O.C.	HTS20
2200 LB	LTT01 W/ 5" X 7" WEDGE ANCHOR	N/A	(2) HTS20 NAILED TO STUD PACK

F4 - INTERIOR BEARING FOOTING
SCALE: 1/2"=1'-0" REV-22-AUG-03

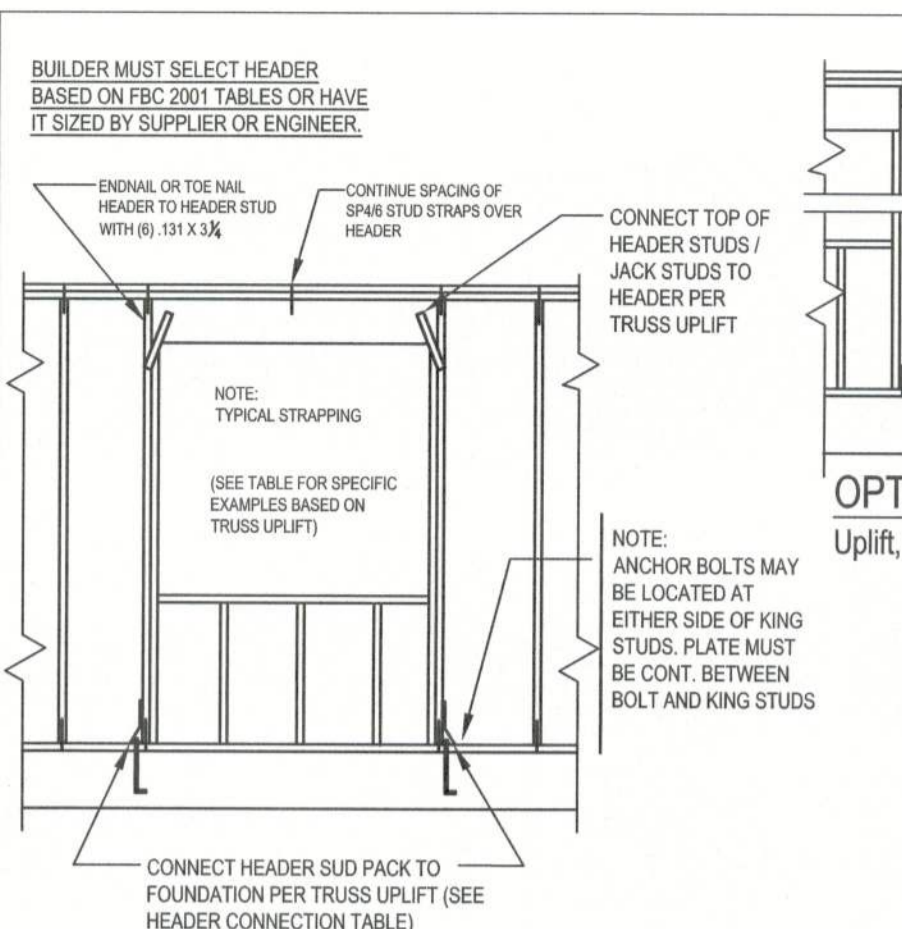


TYPICAL TRUSS UPLIFT	WEDGE ANCHOR SPACING	SP4 / SP5 SPACING	TRUSS CONNECTOR
400 LB	48" O.C.	48" O.C.	H2SA
600 LB	48" O.C.	32" O.C.	H10
1000 LB	32" O.C.	16" O.C.	HTS20
2200 LB	LTT01 W/ 5" X 7" WEDGE ANCHOR	N/A	(2) HTS20 NAILED TO STUD PACK

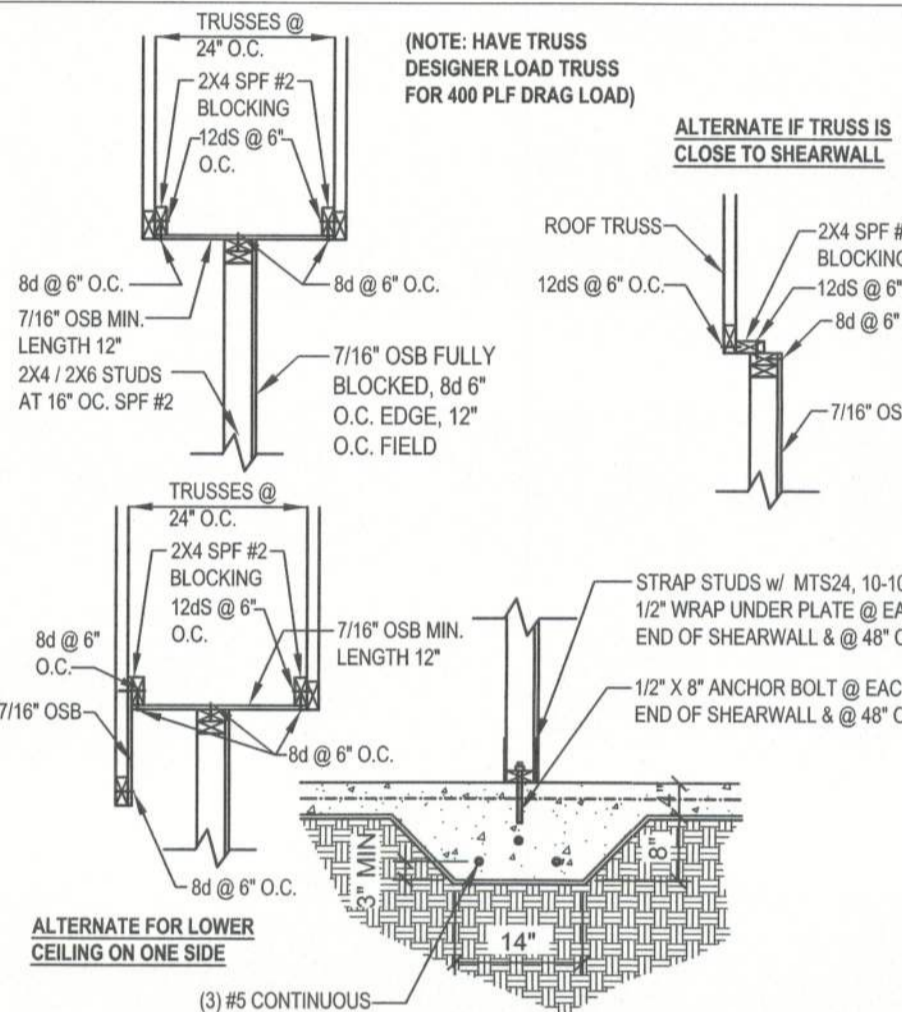
F5 - INTERIOR BEARING STEP FOOTING
SCALE: 1/2"=1'-0" REV-22-AUG-03



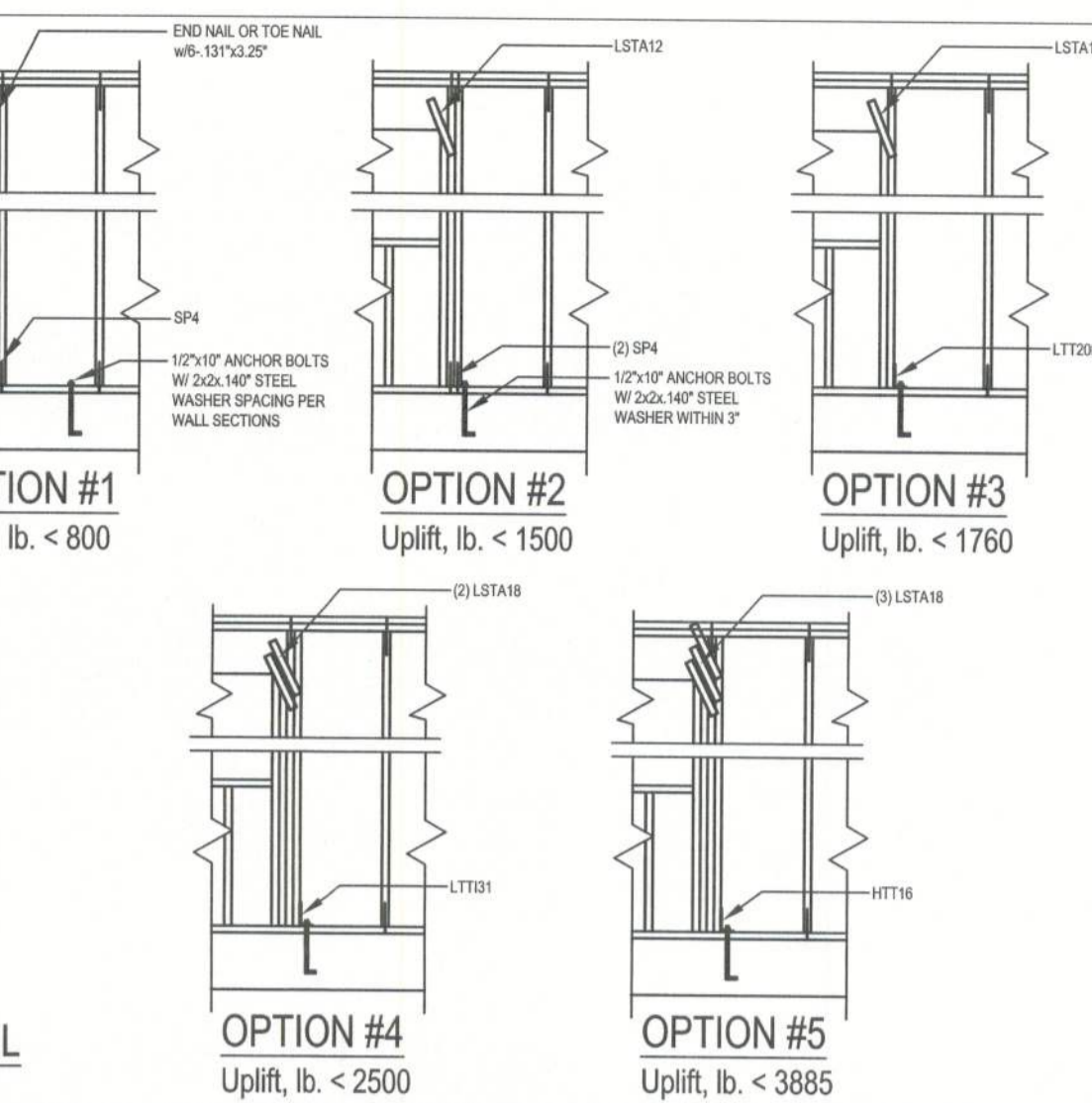
F3 - GARAGE DOOR POCKET
SCALE: 1/2"=1'-0" REV-22-AUG-03



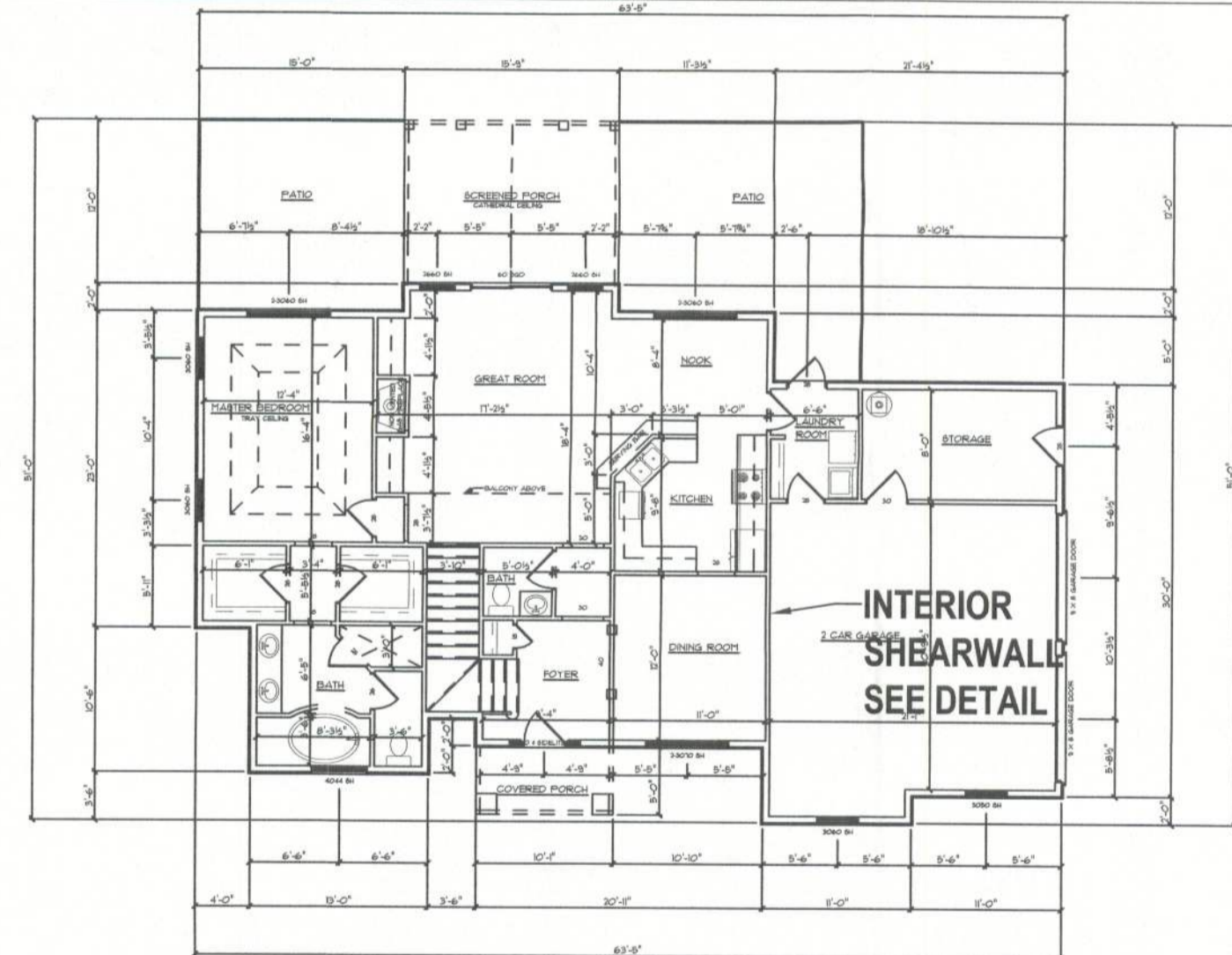
W3-TYPICAL HEADER SIZING & STRAPING DETAIL
SCALE: N.T.S. REV-22-AUG-03



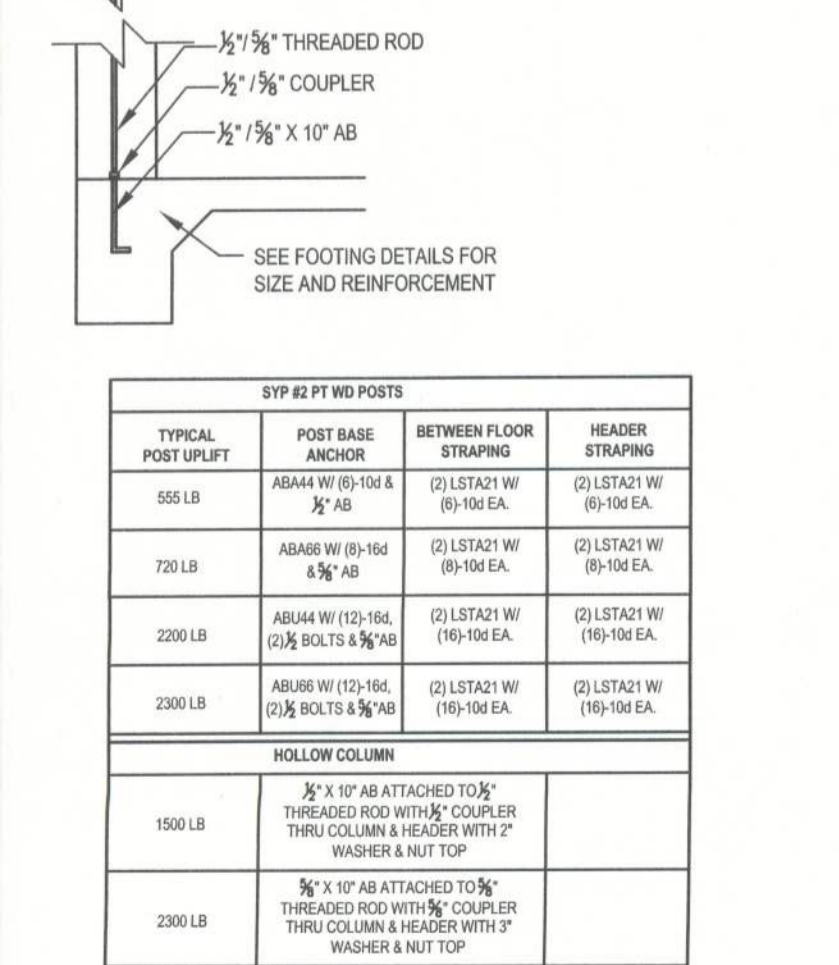
W60 - INTERIOR SHEAR WALL DETAIL
SCALE: 1/2"=1'-0"



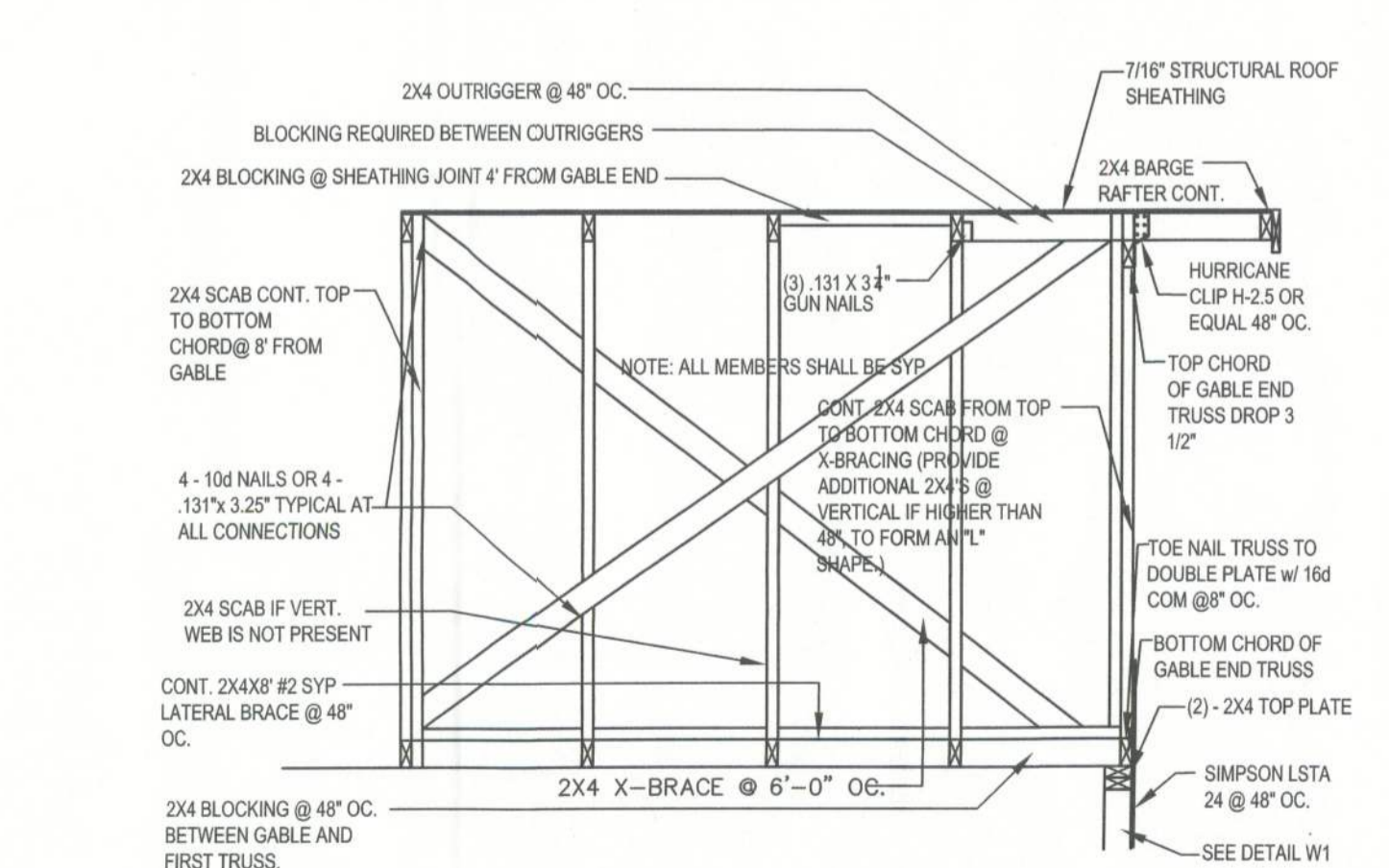
W71 - HEADER SPANS FOR ROOF/CEILING LOAD



INTERIOR
SHEARWALL
SEE DETAIL



W2 - PORCH HEADER ANCHORS
SCALE: N.T.S. REV-18-JUL-03



SG2 - TYPICAL STAIR DETAIL



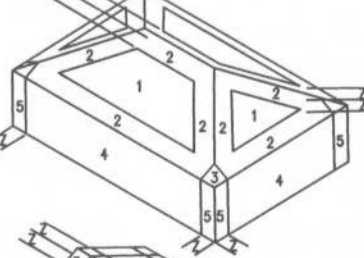
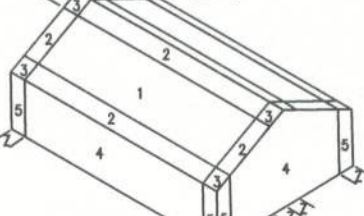
W10 - TYPICAL GABLE END (X-BRACING)
SCALE: 1/2"=1'-0"

N5 - TRUSS UPLIFT CONNECTOR TABLE				REV-18-NOV-04	
All connectors are Simpson Strong-Tie, Inc. Select top and bottom connections from this table or SST catalog to meet truss uplift. Use fasteners as specified.					
Uplift SPF	Uplift SYP	Truss Connector	To Plate	To Truss / Rafter	
320	455	H3	4-8d	4-8d	
245	350	H5A	3-8d	3-8d	
535	600	H2SA	5-8d	5-8d	
620	720	H10	6-10d@1 1/2"	6-10d@1 1/2"	
850	990	LTS12	8-10d@1 1/2"	8-8d@1 1/2"	
1245	1450	HTS20	10-10d or 12-10d@1 1/2"	10-10d or 12-10d@1 1/2"	
1285	1470	H16, H16-2	10-10d@1 1/2"	2-10d@1 1/2"	
1785	2050	LG12	14-10d@1 1/2"	14-10d@1 1/2"	
3555	4200	MGT	3/4" Thd Rod	22-10d	
SPF	SYP	Strap Connector	To One Member	To Other Member	
760	885	SP4	6-10d@1 1/2"	N/A	
865	1005	CS20	9-8d or 7-10d	9-8d or 7-10d	
1085	1265	LSTA18-24	7-10d	7-10d	
1170	1360	SPH4	12-10d@1 1/2"	N/A	
1420	1650	SSP	4-10d	3-10d to double plate or 1-10d to single	
600	825	DSP	8-10d	6-10d to double plate or 2-10d to single	
1420	1650	CS16	14-8d or 11-10d	14-8d or 11-10d	
SPF	SYP	Column Anchor	To Foundation	To Column / Truss	
1160	1350	LTT19	3/4" x 16" AB	8-16d Sinkers	
1985	2310	LTT01	3/4" x 16" AB	18-10d@1 1/2"	
2385	2775	HD2A	3/4" x 16" AB	2-3/4" Bolts	
3590	4175	HTT16	3/4" x 16" AB	18-16d	
1975	2300	ABUR6	3/4" x 16" AB	12-16d	

Truss Supporting Trusses: The builder is responsible for gravity loads, but you should put an extra 2x4 stud under truss bearing location for each 3000 lb of reaction. Check the minimum bearing requirements of the truss and top plate (SPF, F=2400psi/2200psi).

Manufacturer and product number are listed for example not endorsement. An equivalent device of the same or other manufacturer can be substituted for any device listed in the example tables as long as it meets the required load capacities. Manufacturer's installation instructions must be followed to achieve rated loads. All connectors exposed directly to the weather shall be hot dipped galvanized after fabrication. Loads are increased for wind duration. Strip uplift may be reduced proportionally to number of nails. See spec sheet for alternate nail sizes (10d=14d, 16d=18d, 18d=20d, 20d=24d).

10d=14d, 16d=18d, 18d=20d, 20d=24d
SPF=2400psi, SYP=2200psi

N4-WIND LOAD DESIGN DATA																											
(Wind loads are per FBC 2001, Section 1606.2 for enclosed simple diaphragm buildings with mean roof height less than 60' or the least horizontal dimension; not sited on the upper half of an unobstructed 60' high hill with >10% slope.)																											
Basic Wind Speed	110 MPH																										
Wind Exposure	B																										
Wind Importance Factor	I																										
Building Category	IIA (Enclosed)																										
Internal pressure Coefficient	N/A																										
Building not in the high velocity hurricane zone																											
Building not in the wind-borne debris region																											
Mean Roof Height	< 30 ft																										
Roof Angle	10-45 degrees																										
Components And Cladding Wind Pressures (FBC Table 1606.2 B6C)																											
	<table><tr><th>Zone</th><th>Effective Wind Area (ft²)</th></tr><tr><td>10</td><td>100</td></tr><tr><td>4</td><td>21.8</td></tr><tr><td>5</td><td>21.8</td></tr><tr><td>6</td><td>21.8</td></tr><tr><td>7</td><td>21.8</td></tr><tr><td>8</td><td>21.8</td></tr><tr><td>9</td><td>21.8</td></tr><tr><td>1</td><td>21.8</td></tr><tr><td>2</td><td>21.8</td></tr><tr><td>3</td><td>21.8</td></tr></table>	Zone	Effective Wind Area (ft ²)	10	100	4	21.8	5	21.8	6	21.8	7	21.8	8	21.8	9	21.8	1	21.8	2	21.8	3	21.8				
	Zone	Effective Wind Area (ft ²)																									
10	100																										
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3	21.8																										
	<table><tr><th>Zone</th><th>Effective Wind Area (ft²)</th></tr><tr><td>12</td><td>100</td></tr><tr><td>11</td><td>21.8</td></tr><tr><td>10</td><td>21.8</td></tr><tr><td>9</td><td>21.8</td></tr><tr><td>8</td><td>21.8</td></tr><tr><td>7</td><td>21.8</td></tr><tr><td>6</td><td>21.8</td></tr><tr><td>5</td><td>21.8</td></tr><tr><td>4</td><td>21.8</td></tr><tr><td>3</td><td>21.8</td></tr><tr><td>2</td><td>21.8</td></tr><tr><td>1</td><td>21.8</td></tr></table>	Zone	Effective Wind Area (ft ²)	12	100	11	21.8	10	21.8	9	21.8	8	21.8	7	21.8	6	21.8	5	21.8	4	21.8	3	21.8	2	21.8	1	21.8
	Zone	Effective Wind Area (ft ²)																									
12	100																										
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5	21.8																										
4	21.8																										
3	21.8																										
2	21.8																										
1	21.8																										

Total Shear Wall Segments	
Z=10m to 40.7m wall, Z=10m for 10-15' wall	
Required	Transverse
Actual	68.5
Actual	65.2
All exterior walls are type I shear walls	
ACTUAL SHEAR WALL length is the total length of wall segments with full height sheathing and width to height ratio greater than 1: 3.5 (plus special shear wall segments if noted) REQUIRED SHEAR WALL length is from WFCM 2001, table 5.17A 1.5 x 17B with table 5.17E adjustment for type I shear wall (or equivalent calculation)	
REV-27-Jun-02	

N3-WINDLOAD ENGINEER'S SCOPE OF WORK: The wind load engineer is engineer of record for compliance of structure to wind load requirements of FBC 2001, Section 1606. If trusses are used, the wind load engineer is not engineer of record for the trusses and did not design the trusses or delegate to the truss designer.

BUILDER'S RESPONSIBILITY: The builder and owner are responsible for the following, which are specifically not of the wind load engineer's scope of work.

- * Confirm that the foundation design & site conditions meet gravity load requirements (assume 1000 PSF bearing capacity unless visual observation or soils test proves otherwise)
- * Provide materials and construction techniques, which comply with FBC 2001 requirements for the stated wind velocity and design pressures.
- * Provide a continuous load path from roof to foundation. If you believe the plan permits a continuous load path connection, call the wind load engineer immediately.
- * Verify the truss engineering includes truss design, placement plans, temporary and permanent bracing details, truss-to-truss connections, and load reactions for all bearing locations.
- * Select uplift connections, walls, columns, and footings based on truss engineering bearing locations and reactions including interior bearing walls.
- * Size headers for gravity loads; headers sized by the builder for gravity loads will also satisfy wind loads.

DOCUMENT CONTROL AND PRIORITY: Structural requirements on S-1 control unless the building code or architectural sheets have more stringent requirements. Non-structural requirements on architectural sheets control. Specific requirements take precedence over general requirements. Revision control is by the latest signature date and is the responsibility of the builder.

COPYRIGHTS AND PROPERTY RIGHTS: Mark Disoway, P.E. hereby expressly reserves his common law copyrights and property right in these instruments of service. This document is not to be reproduced, altered or copied in any form or manner without the express written permission and consent of Mark Disoway.

DIMENSIONS:

Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

WINDLOAD ENGINEER: Mark Disoway, PE No. 53915

CERTIFICATION: The attached plans and "Windload Engineering", sheet S-1, comply with FBC 2001, Section 16 wind loads, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location. This drawing is not valid for construction unless raised seal is affixed.

Load Bearing Header Sizing Methods (BY BUILDER)

- Determine header size from FBC 2001, Tables 2308.3 A, B, & C, or 2308.5.
- Use supplier published data or Southern pine span tables.
- For engineered lumber beams have suppliers engineer size beam.

Jack Studs and King Studs (BY BUILDER)

- Lookup jack studs from FBC 2001, Tables 2308.3 A, B, & C, or 2308.5.
- Use one jack stud for every 3000 lb vertical load.
- Total king plus jack studs = studs needed to be there if no opening was there.

Header Uplift Connections (BY BUILDER)

- Calculate the uplift at each end of the header by summing the moments of all truss uplifts and dividing by the length of the header.
- Select header connections from table below or nrg catalog to connect header to stud (top connection) and stud to foundation (bottom connection).

Option #	Uplift, lb.	Top Connector	Bottom Connector
#1	< 800	End nail or tie nail w/ 10d x 1 1/2" x 16"	SP4, 6-16d x 1 1/2" x 16"
#2	< 1500	LSTA12, 10-10d	755 2" SP4, 6-16d x 1 1/2" x 16" AB
#3	< 1750	LSTA18, 14-10d	1055 L17B26, 10-16d x 1 1/2" AB
#4	< 2500	LSTA18, 14-10d	2110 L17D1, 10-16d x 1 1/2" x 16" AB
#5	< 3885	3" LSTA18, 14-10d	3480 H4T16, 18-16d, 1 1/2" x 16" AB
#6	greater than 3885	3" LSTA18, 14-10d	4175

Uplift greater than 3885 lb requires engineering design

FBC2001, TABLE 2308.3A

Header Spans For Exterior Beating Walls Supporting Roof-Ceiling (200d/20dps)

	Header Spans (ft-in)	Building Width / Truss Span (ft)				
		20' Building Width		36' Building Width		
		Span	NJ	Span	NJ	
	2-2x4	3-6	1	3-2	1	2-10
	2-2x6	5-5	2	4-8	1	4-2
	2-2x8	6-10	1	5-11	2	5-4
	2-2x10	8-5	1	7-3	2	6-6
	2-2x12	9-9	2	8-5	2	7-8
	3-2x8	8-4	1	7-5	1	6-4
	3-2x10	10-6	1	9-1	2	8-2
	3-2x12	12-2	2	10-7	2	9-5
	4-2x8	1-2	1	8-4	1	7-4
	4-2x10	11-8	1	10-6	1	9-5
	4-2x12	14-1	1	12-2	2	10-11

NOTES: NJ = Number of jacks needed to support each end. Building width is measured perpendicular to the ridge. For widths between those shown, spans may be interpolated. Spans are based on uniform loads on header.