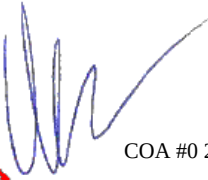


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Alpine, an ITW Company
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www.alpineitw.com

COA #0 278

Florida Certificate of Product Approval #FL 1999
03/06/2023



Site Information:	Page 1:
Customer: W. B. Howland Company, Inc.	Job Number: 23-9006
Job Description: Nickelson Shed	
Address: Nickelson Shed, FL	

Job Engineering Criteria:	
Design Code: FBC 7th Ed. 2020 Res.	IntelliVIEW Version: 22.02.00 JRef #: 1XNR2150007
Wind Standard: ASCE 7-16 Wind Speed (mph): 130 Building Type: Closed	Design Loading (psf): 40.00, 55.00

This package contains general notes pages, 18 truss drawing(s) and 4 detail(s).

Item	Drawing Number	Truss
1	065.23.1509.21327	A1
3	065.23.1509.43430	B1
5	065.23.1509.58270	FL1
7	065.23.1511.16897	FT1
9	065.23.1512.07710	FT2
11	065.23.1512.58673	FT4
13	065.23.1458.20480	FT6
15	065.23.1513.24670	J1
17	065.23.1513.34887	J5
19	160TL	
21	BRCLBSUB0119	

Item	Drawing Number	Truss
2	065.23.1509.41137	A1E
4	065.23.1509.56177	B1E
6	065.23.1510.11873	FL2
8	065.23.1511.54933	FT1A
10	065.23.1512.43193	FT3
12	065.23.1513.14283	FT5
14	065.23.1513.22800	HGJ1
16	065.23.1513.33087	J3
18	065.23.1513.38710	J6
20	A14030ENC160118	
22	GBLLETIN0118	

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed and detailed by the Building Designer.

Connector Plate Information:

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for of all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for of all load cases.

Max Web CSI = Maximum bending and axial Combined Stress Index for Webs for of all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.

W = Width of non-hanger bearing, in inches.

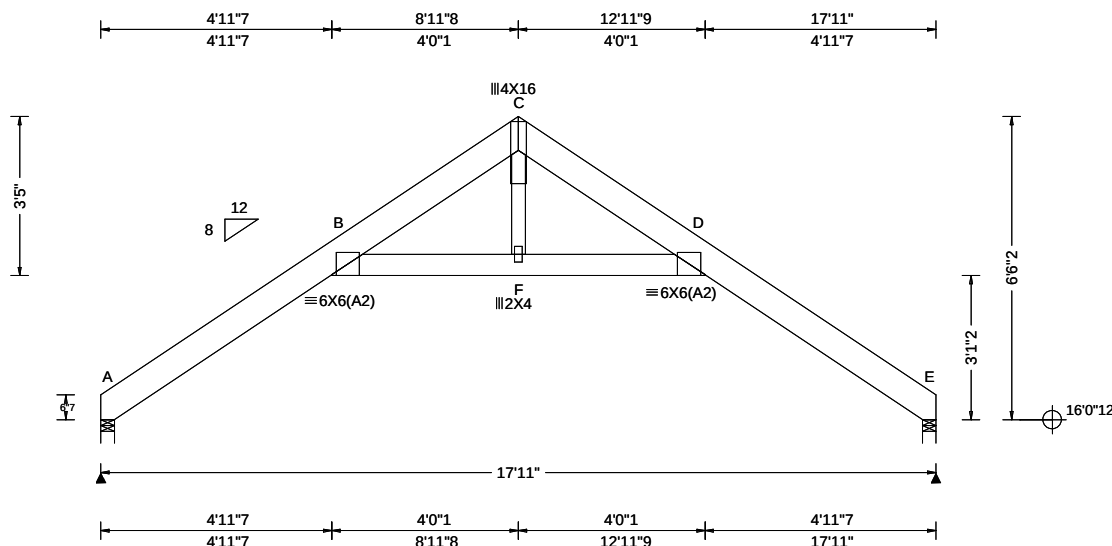
Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcacomponents.com.

SEQN: 543761 FROM: RFG	COMN	Ply: 1 Qty: 8	Job Number: 23-9006 Nickelson Shed Truss Label: A1	Cust: R 215 JRef: 1XNR2150007 T7 DrwNo: 065.23.1509.21327 KD / WHK 03/06/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 19.59 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.424 C 491 360 VERT(CL): 0.774 C 269 240 HORZ(LL): 0.282 D - - HORZ(TL): 0.514 D - - Creep Factor: 2.0 Max TC CSI: 0.473 Max BC CSI: 0.077 Max Web CSI: 0.023 VIEW Ver: 22.02.00.0914.12	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 654 /- /- /343 /218 /168 E 654 /- /- /343 /218 /- Wind reactions based on MWFRS A Brg Wid = 3.5 Min Req = 1.5 (Support) E Brg Wid = 3.5 Min Req = 1.5 (Support) Bearings A & E Fcperp = 565psi. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 279 - 1203 C - D 279 - 1203

Lumber

Top chord: 2x8 SP 2400f-2.0E;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

The overall height of this truss excluding overhang is 14'-6-14.



COA #0278

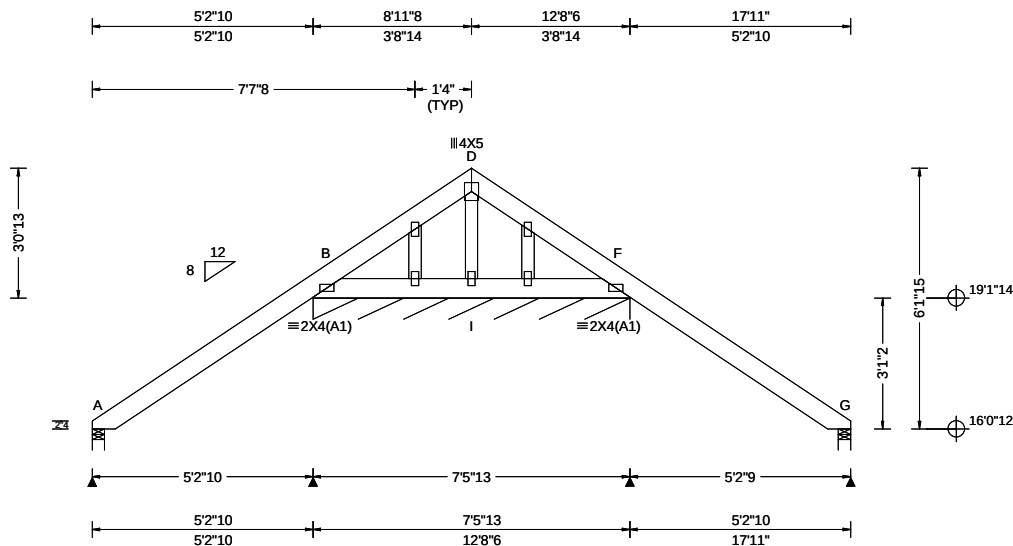
03/06/2023
Florida Certificate of Product Approval #FL 1999

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!
****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.
For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org



SEQN: 543719 FROM: RFG	GABL Ply: 1 Qty: 2	Job Number: 23-9006 Nickelson Shed Truss Label: A1E	Cust: R 215 JRef:1XNR2150007 T8 DrwNo: 065.23.1509.41137 KD / WHK 03/06/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 19.24 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 6.06 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.001 B 999 360 VERT(CL): 0.002 B 999 240 HORZ(LL): 0.002 F - - HORZ(TL): 0.002 F - - Creep Factor: 2.0 Max TC CSI: 0.106 Max BC CSI: 0.011 Max Web CSI: 0.040 VIEW Ver: 22.02.00.0914.12	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 203 - / - /170 /139 /234 B* 193 - / - /103 /48 - G 203 - / - /137 /139 - Wind reactions based on MWFRS A Brg Wid = 3.5 Min Req = 1.5 (Support) B Brg Wid = 89.8 Min Req = - G Brg Wid = 3.5 Min Req = 1.5 (Support) Bearing B is a rigid surface. Bearings A & G Fcperp = 565psi. Members not listed have forces less than 375#

Lumber

Top chord: 2x6 SP 2400f-2.0E;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

Loading

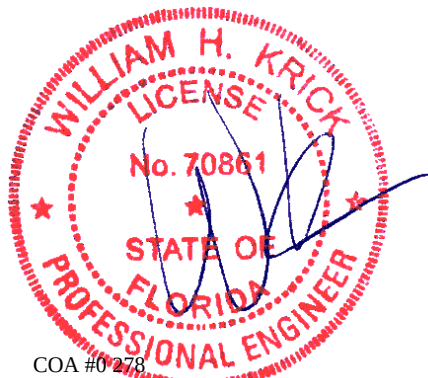
Truss designed to support 1-4-0 top chord outlookers and cladding load not to exceed 7.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.
The overall height of this truss excluding overhang is 14-2-11.



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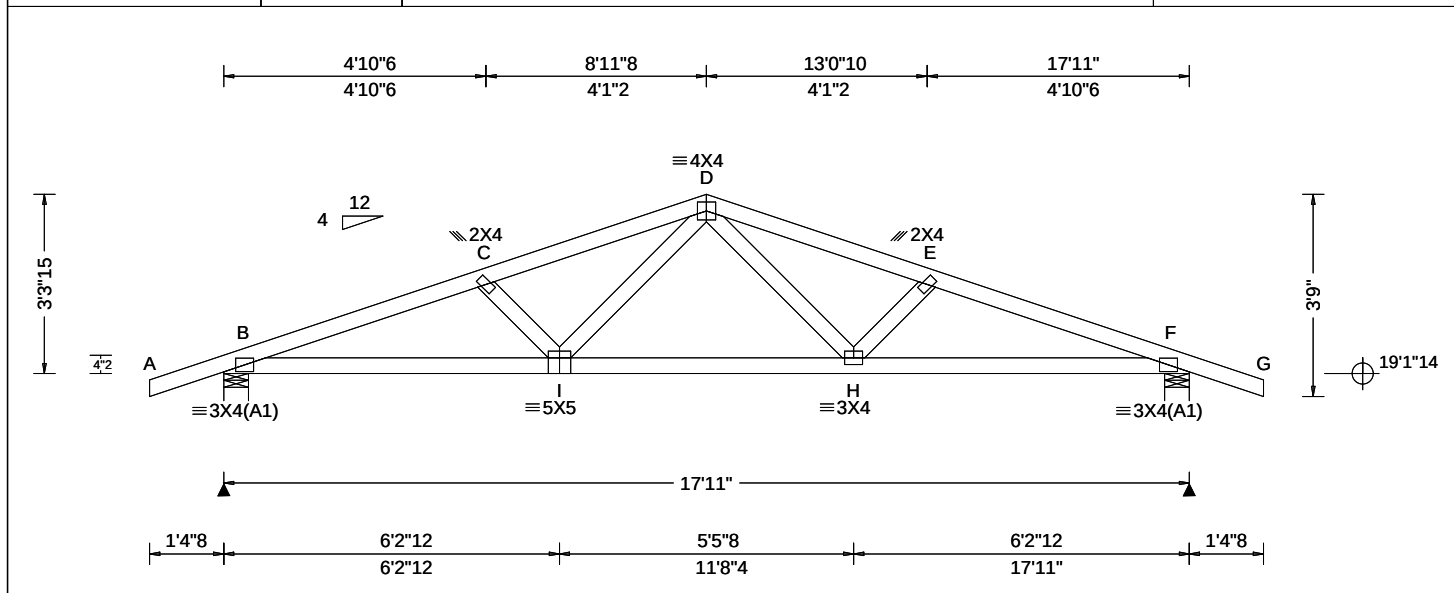
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Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbccomponents.com; ICC: iccsafe.org; AWC: awc.org

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155 Harlem Ave
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Glenview, IL 60025

SEQN: 543758 FROM: RFG	COMN Ply: 1 Qty: 6	Job Number: 23-9006 Nickelson Shed Truss Label: B1	Cust: R 215 JRef: 1XNR2150007 T1 DrwNo: 065.23.1509.43430 KD / WHK 03/06/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 20.76 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.062 D 999 360 VERT(CL): 0.122 D 999 240 HORZ(LL): 0.019 F - - HORZ(TL): 0.037 F - - Creep Factor: 2.0 Max TC CSI: 0.209 Max BC CSI: 0.433 Max Web CSI: 0.156 VIEW Ver: 22.02.00.0914.12	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 816 /- /- /425 /155 /68 F 816 /- /- /425 /155 /- Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.5 (Truss) F Brg Wid = 5.5 Min Req = 1.5 (Truss) Bearings B & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 309 -1585 D - E 277 -1411 C - D 278 -1410 E - F 309 -1586

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 3-3-15.



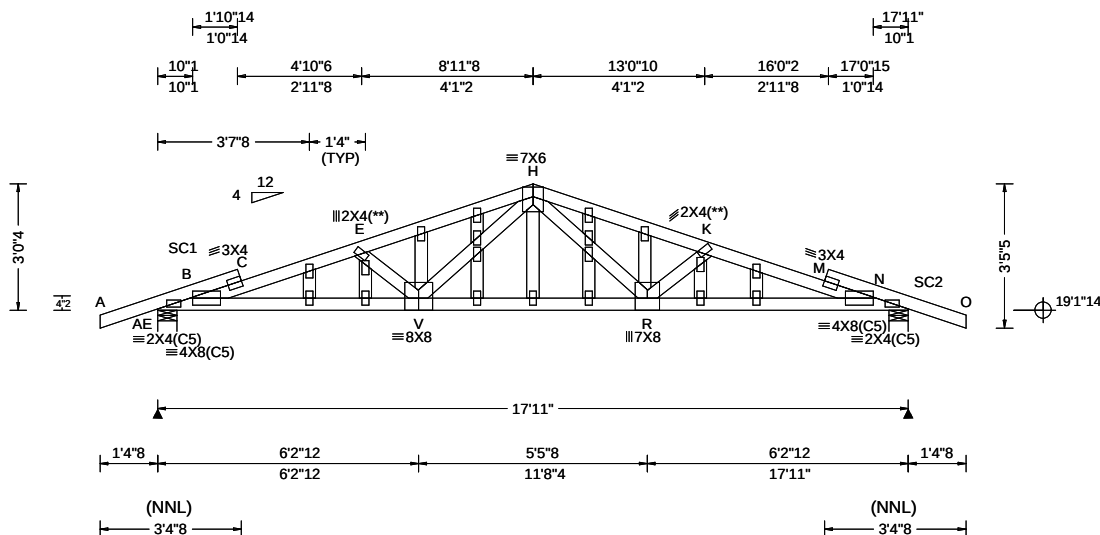
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 543716 FROM: RFG	GABL Ply: 1 Qty: 2	Job Number: 23-9006 Nickelson Shed Truss Label: B1E	Cust: R 215 JRef:1XNR2150007 T2 DrwNo: 065.23.1509.56177 KD / WHK 03/06/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 20.61 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 12.11 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.123 I 999 360 VERT(CL): 0.246 I 846 240 HORZ(LL): 0.019 D - - HORZ(TL): 0.037 D - - Creep Factor: 2.0 Max TC CSI: 0.758 Max BC CSI: 0.528 Max Web CSI: 0.369 VIEW Ver: 22.02.00.0914.12	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity AE 1145 /- /- /524 /461 /94 N 1145 /- /- /524 /461 /- Wind reactions based on MWFRS AE Brg Wid = 5.5 Min Req = 1.5 (Truss) N Brg Wid = 5.5 Min Req = 1.5 (Truss) Bearings AE & N are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 935 -2695 H - K 883 -2240 C - E 960 -2640 K - M 960 -2640 E - H 884 -2241 M - N 935 -2695

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP M-31;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;

Plating Notes

All plates are 2X4 except as noted.

(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Loading

Truss designed to support 1-4-0 top chord outlookers and cladding load not to exceed 7.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Purlins

In lieu of structural panels use purlins to brace TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

The overall height of this truss excluding overhang is 3-0-4.



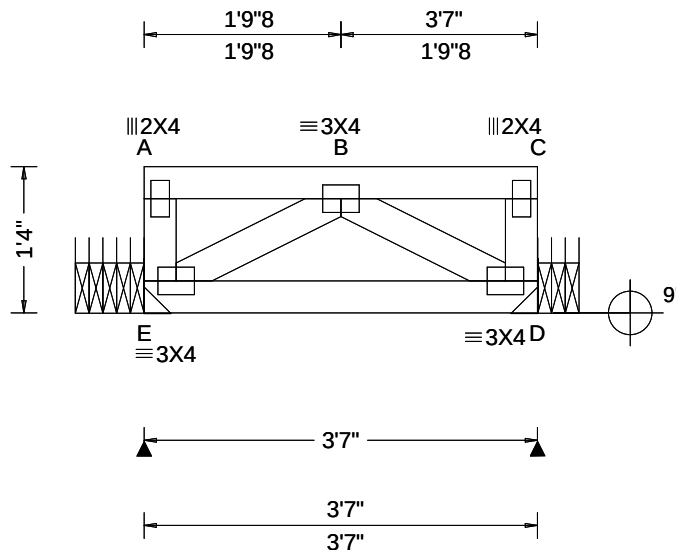
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Glenview, IL 60025

SEQN: 543743 FROM: RFG	FLAT Ply: 1 Qty: 2	Job Number: 23-9006 Nickelson Shed Truss Label: FL1	Cust: R 215 JRef: 1XNR2150007 T19 DrwNo: 065.23.1509.58270 KD / WHK 03/06/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 40.00 TCDL: 10.00 BCLL: 0.00 BCDL: 5.00 Des Ld: 55.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.00 Spacing: 24.0 "	Wind Std: NA Speed: NA mph Enclosure: NA Category: NA EXP: NA Kzt: NA Mean Height: NA ft TCDL: NA psf BCDL: NA psf MWFRS Parallel Dist: NA C&C Dist a: NA ft Loc. from endwall: NA I: NA GCpi: NA Wind Duration: NA	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.002 B 999 480 VERT(CL): 0.002 B 999 360 HORZ(LL): 0.001 D - - HORZ(TL): 0.001 D - - Creep Factor: 2.0 Max TC CSI: 0.065 Max BC CSI: 0.106 Max Web CSI: 0.049 VIEW Ver: 22.02.00.0914.12	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL E 197 /- /- /- /- /- D 197 /- /- /- /- /- E Brg Wid = - Min Req = - D Brg Wid = - Min Req = - Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Hangers / Ties

(J) Hanger Support Required, by others

Additional Notes

Truss must be installed as shown with top chord up.
The overall height of this truss excluding overhang is 1-4-0.



COA #0278

03/06/2023
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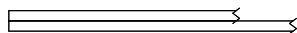
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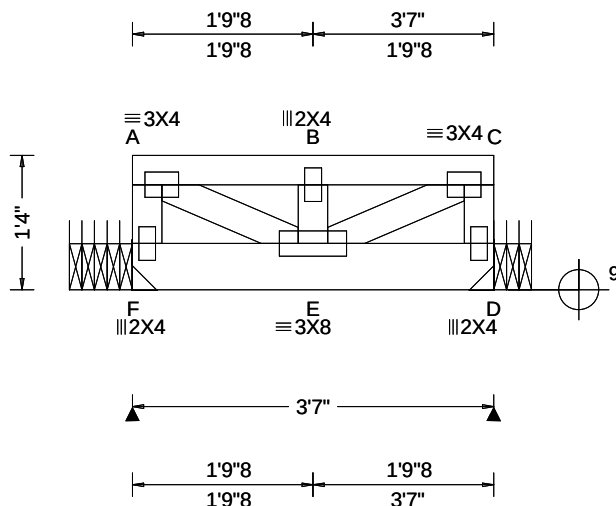


155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 543745 FROM: RFG	FLAT Ply: 2 Qty: 1	Job Number: 23-9006 Nickelson Shed Truss Label: FL2	Cust: R 215 JRef: 1XNR2150007 T5 DrwNo: 065.23.1510.11873 KD / WHK 03/06/2023
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2 Complete Trusses Required



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 40.00 TCDL: 10.00 BCLL: 0.00 BCDL: 5.00 Des Ld: 55.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.00 Spacing: 24.0 "	Wind Std: NA Speed: NA mph Enclosure: NA Category: NA EXP: NA Kzt: NA Mean Height: NA ft TCDL: NA psf BCDL: NA psf MWFRS Parallel Dist: NA C&C Dist a: NA ft Loc. from endwall: NA I: NA GCpi: NA Wind Duration: NA	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.004 B 999 480 VERT(CL): 0.005 B 999 360 HORZ(LL): 0.001 A - - HORZ(TL): 0.001 A - - Creep Factor: 2.0 Max TC CSI: 0.050 Max BC CSI: 0.087 Max Web CSI: 0.189 VIEW Ver: 22.02.00.0914.12	Gravity Loc R+ / R- / Rh / Rw / U / RL F 998 - / - / - / - / - D 762 - / - / - / - / - F Brg Wid = - Min Req = - D Brg Wid = - Min Req = - Members not listed have forces less than 375# Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. A - E 397 0 E - C 398 0

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3;

Nailnote

Nail Schedule: 0.128"x3", min. nails
Top Chord: 1 Row @ 12.00" o.c.
Bot Chord: 2 Rows @ 6.00" o.c. (Each Row)
Webs: 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special Loads

----- (Lumber Dur.Fac.=1.00 / Plate Dur.Fac.=1.00)
TC: From 50 plf at 0.00 to 50 plf at 3.58
BC: From 5 plf at 0.00 to 5 plf at 3.58
BC: 781 lb Conc. Load at 0.52, 2.52

Hangers / Ties

(J) Hanger Support Required, by others

Purlins

The TC of this truss shall be braced with attached spans at 24" oc in lieu of structural sheathing.

Additional Notes

Truss must be installed as shown with top chord up.
The overall height of this truss excluding overhang is 1'-4".



COA #0278

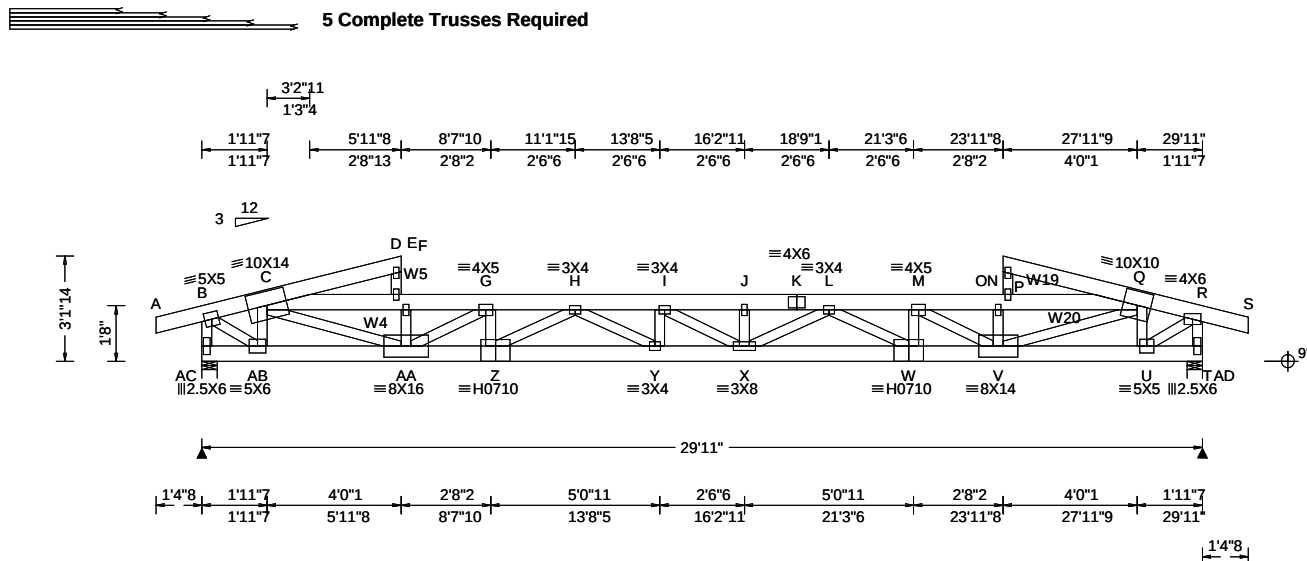
03/06/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 40.00 TCDL: 10.00 BCLL: 0.00 BCDL: 5.00 Des Ld: 55.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.00 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE, HS	PP Deflection in loc L/def L/# VERT(LL): 0.409 I 878 360 VERT(CL): 0.761 I 471 240 HORZ(LL): 0.105 E - - HORZ(TL): 0.197 E - - Creep Factor: 2.0 Max TC CSI: 0.284 Max BC CSI: 0.633 Max Web CSI: 0.931 VIEW Ver: 22.02.00.0914.12	Gravity Loc R+ / R- / Rh / Rw / U / RL AC 6779 - / - / /106 /612 - / AD 6468 - / - / /106 /613 - / Wind reactions based on MWFRS AC Brg Wid = 5.5 Min Req = 1.5 (Truss) AD Brg Wid = 5.5 Min Req = 1.5 (Truss) Bearings AC & AD are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 131 -1510 J - K 604 -7214 C - D 379 -4644 K - L 604 -7214 D - F 378 -4662 L - M 505 -5869 F - G 378 -4662 M - N 379 -4433 G - H 504 -6258 N - P 379 -4433 H - I 602 -7403 P - Q 380 -4415 I - J 604 -7214 Q - R 131 -1437

Lumber

Top chord: 2x6 SP 2400f-2.0E;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3; W4, W20 2x4 SP #2; W5,
W19 2x4 SP M-31;

Nailnote

Nail Schedule: 0.128"x3", min. nails
Top Chord: 1 Row @ 3.75" o.c.
Bot Chord: 1 Row @ 7.25" o.c.
Webs : 1 Row @ 4" o.c.
Repeat nailing as each layer is applied. Use equal
spacing between rows and stagger nails in each row
to avoid splitting.
In addition apply (1) 1/2" bolt, or 0.22"-0.25" min/max dia. X 4.5"
length wood screw from each outside face, at each joint location.

Plating Notes

All plates are 2X4 except as noted.

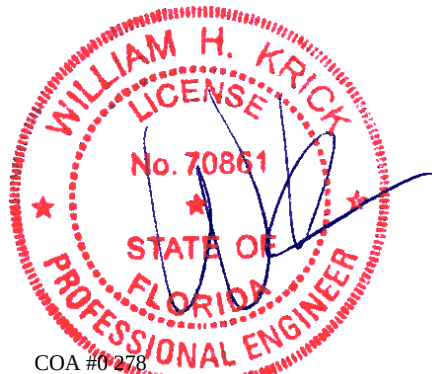
Wind

Wind loads and reactions based on MWFRS.
End verticals exposed to wind pressure. Deflection
meets L/360.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is
3-1-14.

Note: Laterally brace top chord below filler at
20" O.C. Max. including a lateral brace at chord ends.



COA #0278

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North Building, 4th Floor
Glenview, IL 60025

SEQN: 543753	COMN	Ply: 5	Job Number: 23-9006	Cust: R 215 JRef:1XNR2150007 T10
FROM: RFG		Qty: 1	Nickelson Shed	DrwNo: 065.23.1511.16897
Page 2 of 2			Truss Label: FT1	KD / WHK 03/06/2023

Special Loads

----- (Lumber Dur.Fac.=1.00 / Plate Dur.Fac.=1.00)

TC: From 101 plf at -1.38 to 101 plf at 5.96
TC: From 259 plf at 5.96 to 341 plf at 14.96
TC: From 341 plf at 14.96 to 316 plf at 17.79
TC: From 316 plf at 17.79 to 260 plf at 23.93
TC: From 310 plf at 23.93 to 310 plf at 23.96
TC: From 101 plf at 23.96 to 101 plf at 31.29
BC: From 4 plf at -1.38 to 4 plf at 0.00
BC: From 10 plf at 0.00 to 10 plf at 5.99
BC: From 5 plf at 5.99 to 5 plf at 23.93
BC: From 10 plf at 23.93 to 10 plf at 29.92
BC: From 4 plf at 29.92 to 4 plf at 31.29
TC: 689 lb Conc. Load at 6.10,23.81
BC: 767 lb Conc. Load at 5.99,23.93
BC: 230 lb Conc. Load at 8.02,10.02,12.02,14.02
15.90,17.90,19.90,21.90
BC: 998 lb Conc. Load at 10.83
BC: 197 lb Conc. Load at 12.56,14.56



COA #0278

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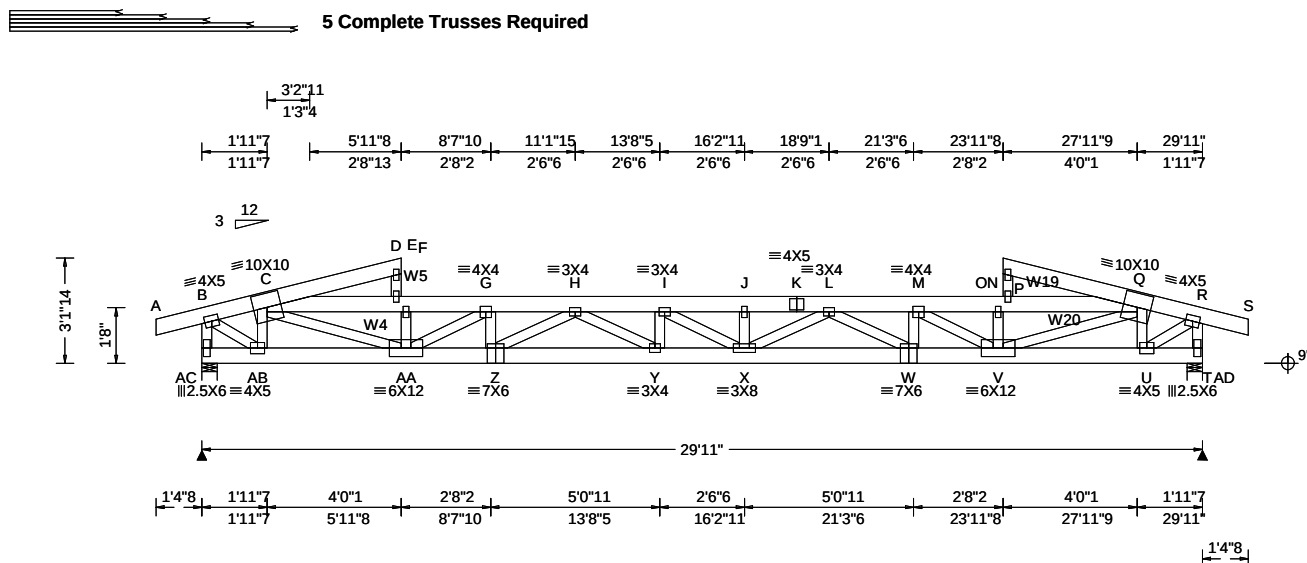
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 543755 FROM: RFG	COMN Ply: 5 Qty: 1	Job Number: 23-9006 Nickelson Shed Truss Label: FT1A	Cust: R 215 JRef:1XNR2150007 T6 DrwNo: 065.23.1511.54933 KD / WHK 03/06/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 40.00 TCDL: 10.00 BCLL: 0.00 BCDL: 5.00 Des Ld: 55.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.00 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.315 I 999 360 VERT(CL): 0.594 I 604 240 HORZ(LL): 0.081 E - - HORZ(TL): 0.152 E - - Creep Factor: 2.0 Max TC CSI: 0.189 Max BC CSI: 0.446 Max Web CSI: 0.701 VIEW Ver: 22.02.00.0914.12	Gravity Loc R+ / R- / Rh / Rw / U / RL AC 5232 - / - / - / 2 / 614 - / - AD 5233 - / - / - / 2 / 614 - / - Wind reactions based on MWFRS AC Brg Wid = 5.5 Min Req = 1.5 (Truss) AD Brg Wid = 5.5 Min Req = 1.5 (Truss) Bearings AC & AD are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 131 -1151 J - K 606 -5750 C - D 381 -3510 K - L 606 -5750 D - F 379 -3523 L - M 506 -4733 F - G 379 -3523 M - N 380 -3523 G - H 506 -4730 N - P 380 -3523 H - I 605 -5731 P - Q 381 -3510 I - J 606 -5750 Q - R 131 -1151

Lumber	Plating Notes	Maximum Bot Chord Forces Per Ply (lbs)
Top chord: 2x6 SP 2400f-2.0E; Bot chord: 2x6 SP 2400f-2.0E; Webs: 2x4 SP #3; W4,W20 2x4 SP #2; W5, W19 2x4 SP M-31;	All plates are 2X4 except as noted. Wind Wind loads and reactions based on MWFRS. End verticals exposed to wind pressure. Deflection meets L/360. Wind loading based on both gable and hip roof types.	Chords Tens.Comp. Chords Tens. Comp. AB-AA 1215 -132 X - W 5361 -560 AA- Z 4629 -493 W - V 4630 -493 Z - Y 5357 -560 V - U 1216 -132 Y - X 5761 -605

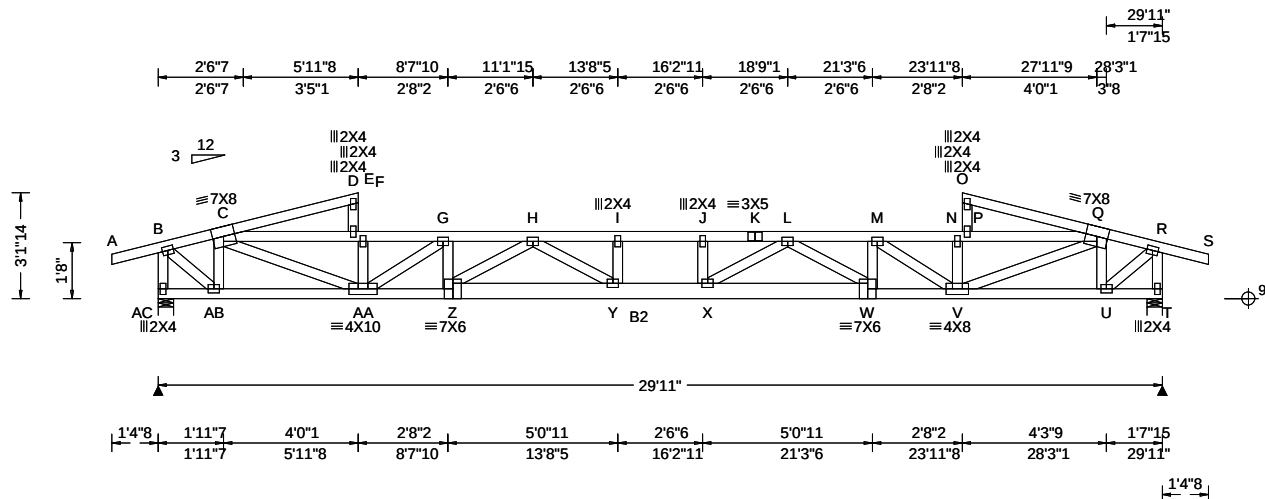
Nailnote	Additional Notes	Maximum Web Forces Per Ply (lbs)
Nail Schedule:0.128"x3", min. nails Top Chord: 1 Row @ 4.50" o.c. Bot Chord: 1 Row @10.00" o.c. Webs :1 Row @ 4" o.c. Repeat nailing as each layer is applied. Use equal spacing between rows and stagger nails in each row to avoid splitting. In addition apply (1) 1/2" bolt, or 0.22"-0.25" min/max dia. X 4" long wood screw from each outside face, at each joint location.	The overall height of this truss excluding overhang is 3'-1.14". Note: Laterally brace top chord below filler at 24" O.C.Max. including a lateral brace at chord ends.	Webs Tens.Comp. Webs Tens. Comp. B - AC 129 -1101 X - L 473 -53 B - AB 1377 -153 L - W 65 -766 AB - C 82 -686 W - M 560 -67 C - AA 2485 -265 M - V 136 -1332 AA - G 136 -1331 V - Q 2485 -265 G - Z 555 -66 Q - U 82 -686 Z - H 66 -764 U - R 1377 -153 H - Y 463 -52 R - T 129 -1101

Special Loads	Professional Engineer	Alpine
----- (Lumber Dur.Fac.=1.00 / Plate Dur.Fac.=1.00) TC: From 101 plf at -1.38 to 101 plf at 5.96 TC: From 126 plf at 5.96 to 127 plf at 6.00 TC: From 260 plf at 6.00 to 341 plf at 14.96 TC: From 341 plf at 14.96 to 316 plf at 17.79 TC: From 316 plf at 17.79 to 260 plf at 23.92 TC: From 127 plf at 23.92 to 127 plf at 23.93 TC: From 177 plf at 23.93 to 177 plf at 23.96 TC: From 101 plf at 23.96 to 101 plf at 31.29 BC: From 4 plf at -1.38 to 4 plf at 0.00 BC: From 10 plf at 0.00 to 10 plf at 5.99 BC: From 5 plf at 5.99 to 5 plf at 23.93 BC: From 10 plf at 23.93 to 10 plf at 29.92 BC: From 4 plf at 29.92 to 4 plf at 31.29 BC: 767 lb Conc. Load at 5.99,23.93 BC: 230 lb Conc. Load at 8.02,10.02,12.02,14.02 15.90,17.90,19.90,21.90	WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER COA #0278 03/06/2023 Florida Certificate of Product Approval #FL 1999	ALPINE AN ITW COMPANY 155 Harlem Ave North Building, 4th Floor Glenview, IL 60025

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SEQN: 543751 FROM: RFG	COMN Ply: 3 Qty: 1	Job Number: 23-9006 Nickelson Shed Truss Label: FT2	Cust: R 215 JRef: 1XNR2150007 T20 DrwNo: 065.23.1512.07710 KD / WHK 03/06/2023
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3 Complete Trusses Required



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 40.00 TCDL: 10.00 BCLL: 0.00 BCDL: 5.00 Des Ld: 55.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.00 Spacing: 16.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.341 I 999 360 VERT(CL): 0.467 I 769 240 HORZ(LL): 0.098 E - - HORZ(TL): 0.134 E - - Creep Factor: 2.0 Max TC CSI: 0.296 Max BC CSI: 0.686 Max Web CSI: 0.608 VIEW Ver: 22.02.00.0914.12	Gravity Loc R+ / R- / Rh / Rw / U / RL AC 1821 -/- /- /197 -/ T 1595 -/- /- /201 -/ Wind reactions based on MWFRS AC Brg Wid = 5.5 Min Req = 1.5 (Truss) T Brg Wid = 5.5 Min Req = 1.5 (Truss) Bearings AC & T are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2; B2 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3;
Nailnote
Nail Schedule: 0.128"x3", min. nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs : 1 Row @ 4" o.c.
Repeat nailing as each layer is applied. Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special Loads
----- (Lumber Dur.Fac.=1.00 / Plate Dur.Fac.=1.00)
TC: From 67 plf at -1.38 to 67 plf at 10.83
TC: From 34 plf at 10.83 to 34 plf at 14.56
TC: From 67 plf at 14.56 to 67 plf at 31.29
BC: From 3 plf at -1.38 to 3 plf at 0.00
BC: From 7 plf at 0.00 to 7 plf at 10.83
BC: From 3 plf at 10.83 to 3 plf at 14.56
BC: From 7 plf at 14.56 to 7 plf at 29.92
BC: From 3 plf at 29.92 to 3 plf at 31.29
BC: 762 lb Conc. Load at 10.83
BC: 197 lb Conc. Load at 12.56,14.56

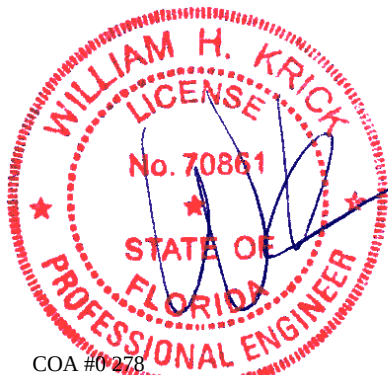
Plating Notes
All plates are 3X4 except as noted.

Additional Notes
The overall height of this truss excluding overhang is 3-1-14.

Wind
Wind loads and reactions based on MWFRS.
End verticals exposed to wind pressure. Deflection meets L/360.
Wind loading based on both gable and hip roof types.
Note: Laterally brace top chord below filler at 2'0" O.C.Max. including a lateral brace at chord ends.

Chords	Tens.Comp.	Chords	Tens. Comp.
AB-AA	583 -51	X - W	2442 -231
AA- Z	2377 -193	W - V	1974 -199
Z - Y	2700 -223	V - U	503 -53
Y - X	2823 -237		

Maximum Web Forces Per Ply (lbs)	Maximum Bot Chord Forces Per Ply (lbs)
Chords Tens.Comp. Webs Tens. Comp.	Chords Tens. Comp. Chords Tens. Comp.
B - AC 66 -613	X - L 549 -6
B - AB 682 -64	L - W 34 -528
AB - C 45 -413	M - V 56 -612
C - AA 1280 -107	V - Q 1082 -110
AA - G 54 -787	U - R 587 -66
G - Z 420 -17	R - T 67 -535

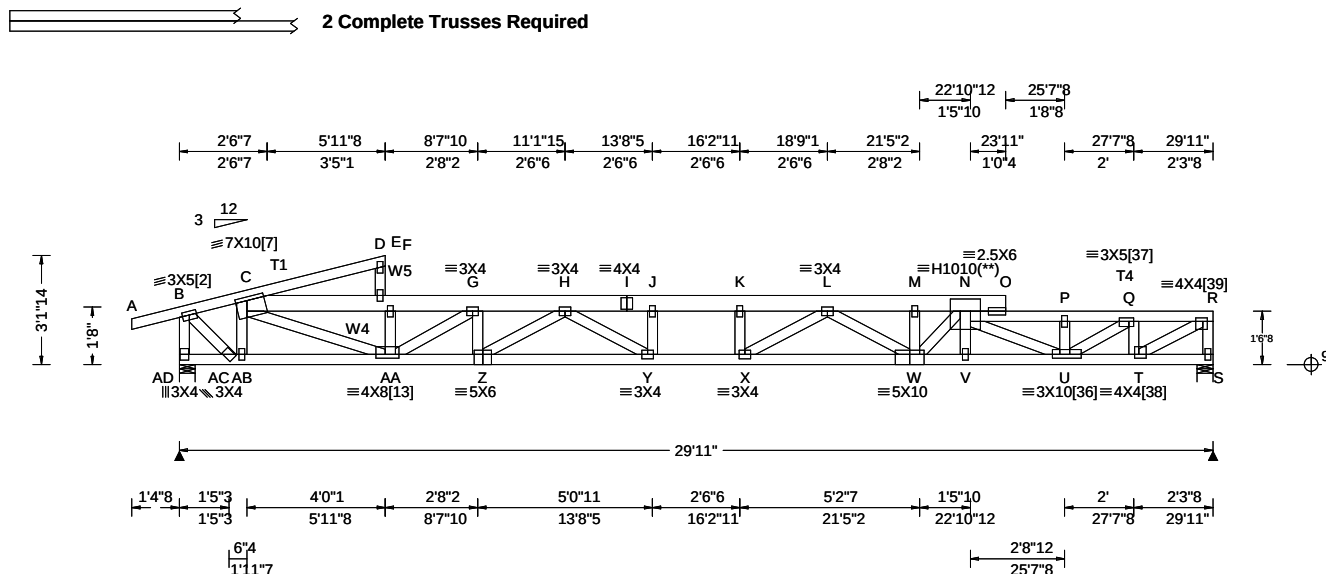


COA #0278
03/06/2023
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AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 543779 FROM: RFG	COMN Ply: 2 Qty: 10	Job Number: 23-9006 Nickelson Shed Truss Label: FT4	Cust: R 215 JRef: 1XNR2150007 T21 DrwNo: 065.23.1512.58673 KD / WHK 03/06/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 40.00 TCDL: 10.00 BCLL: 0.00 BCDL: 5.00 Des Ld: 55.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.00 Spacing: 16.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 3.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT: 20(0)/10(0) Plate Type(s): WAVE, HS	PP Deflection in loc L/def L/# VERT(LL): 0.324 K 999 360 VERT(CL): 0.535 K 670 240 HORZ(LL): 0.087 E - - HORZ(TL): 0.145 E - - Creep Factor: 2.0 Max TC CSI: 0.775 Max BC CSI: 0.490 Max Web CSI: 0.785 VIEW Ver: 22.02.00.0914.12	Gravity Loc R+ / R- / Rh / Rw / U / RL AD 1890 - / - / 348 / 204 / 76 S 1790 - / - / 310 / 192 - / - Wind reactions based on MWFRS AD Brg Wid = 5.5 Min Req = 1.5 (Truss) S Brg Wid = 5.5 Min Req = 1.5 (Truss) Bearings AD & S are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber
Top chord: 2x6 SP 2400F-2.0E; T1 2x4 SP #2;
T4 2x4 SP M-31;
Bot chord: 2x4 SP M-31;
Webs: 2x4 SP #3; W4 2x4 SP #2; W5 2x4 SP M-31;

Nailnote
Nail Schedule: 0.128"x3", min. nails
Top Chord: 1 Row @ 12.00" o.c.
Bot Chord: 1 Row @ 12.00" o.c.
Webs: 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special Loads
----- (Lumber Dur.Fac.=1.00 / Plate Dur.Fac.=1.00)
TC: From 67 plf at -1.38 to 67 plf at 29.92
BC: From 3 plf at -1.38 to 3 plf at 0.00
BC: From 7 plf at 0.00 to 7 plf at 29.92
TC: 689 lb Conc. Load at 6.10, 23.81

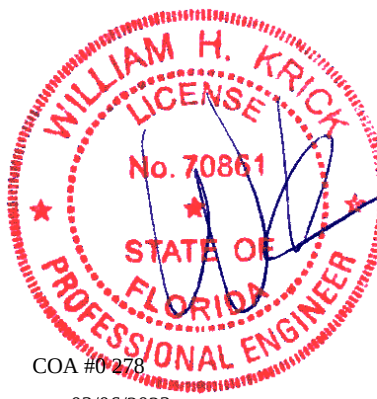
Plating Notes
All plates are 2X4 except as noted.
(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Plate Shift Table

JT	Plate	Lateral	Chord	JT	Plate	Lateral	Chord
No	Size	Shift	Bite	No	Size	Shift	Bite
[2]	3X5	S	1.25	[7]	7X10	1.22	L 1.25
[13]	4X8	2.62	L 1.25	[36]	3X10	2.53	R 1.25
[37]	3X5	1.75	R 1.25	[38]	4X4	2.00	R 1.25
[39]	4X4	1.50	R 1.25				

Wind
Wind loads based on MWFRS with additional C&C member design.
End verticals exposed to wind pressure. Deflection meets L/360.
Wind loading based on both gable and hip roof types.

Additional Notes
The overall height of this truss excluding overhang is 3-1-14.
Note: Laterally brace top chord below filler at 20" O.C. Max. including a lateral brace at chord ends.

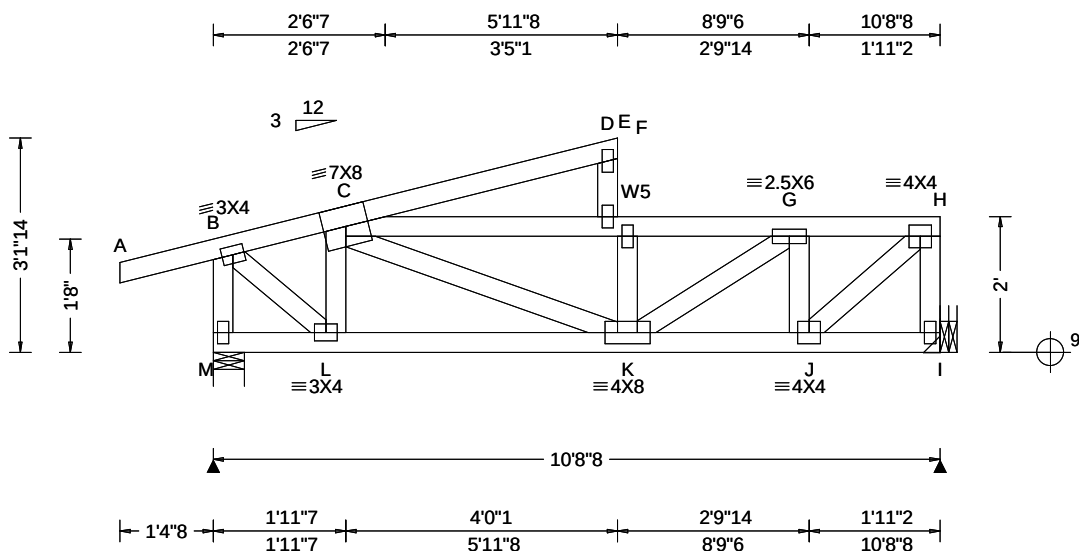


COA #0-278

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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 40.00 TCDL: 10.00 BCLL: 0.00 BCDL: 5.00 Des Ld: 55.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.00 Spacing: 16.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 3.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.039 E 999 360 VERT(CL): 0.068 E 999 240 HORZ(LL): 0.014 E - - HORZ(TL): 0.024 E - - Creep Factor: 2.0 Max TC CSI: 0.285 Max BC CSI: 0.346 Max Web CSI: 0.510 VIEW Ver: 22.02.00.0914.12	Gravity Loc R+ / R- / Rh / Rw / U / RL M 793 -/- /- /149 /84 /69 I 781 -/- /- /112 /77 -/ Wind reactions based on MWFRS M Brg Wid = 5.5 Min Req = 1.5 (Truss) I Brg Wid = - Min Req = - Bearing M is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 68 -635 F - G 127 -1629 C - D 127 -1620 G - H 73 -806 D - F 127 -1629

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3; W5 2x4 SP #2;

Special Loads

----- (Lumber Dur.Fac.=1.00 / Plate Dur.Fac.=1.00)
TC: From 67 plf at -1.38 to 67 plf at 10.71
BC: From 3 plf at -1.38 to 3 plf at 0.00
BC: From 7 plf at 0.00 to 7 plf at 10.71
TC: 689 lb Conc. Load at 6.10

Plating Notes

All plates are 2X4 except as noted.

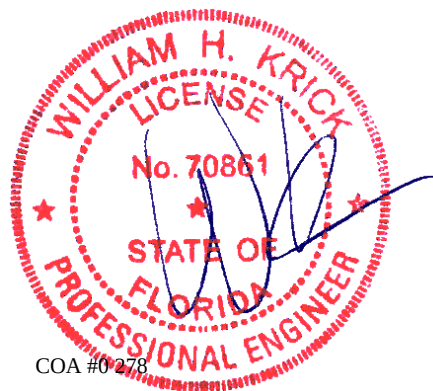
Wind

Wind loads based on MWFRS with additional C&C member design.
Left end vertical exposed to wind pressure. Deflection meets L/360.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 3'-1-14".

Note: Laterally brace top chord below filler at 20" O.C.Max. including a lateral brace at chord ends.



COA #0278

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SEQN: 543686	COMN	Ply: 1	Job Number: 23-9006	Cust: R 215 JRef:1XNR2150007 T4
FROM: RFG		Qty: 2	Nickelson Shed	DrwNo: 065.23.1513.14283
Page 2 of 2			Truss Label: FT5	KD / WHK 03/06/2023

Hangers / Ties

Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

Bearing at location x=10'5"8 uses the following support conditions: 10'5"8

Bearing I (10'5"8, 9') HUS26

Supporting Member: (2)2x6 SP 2400f-2.0E

(14) 0.148"x3" nails into supporting member,

(4) 0.148"x3" nails into supported member.



COA #0278

03/06/2023
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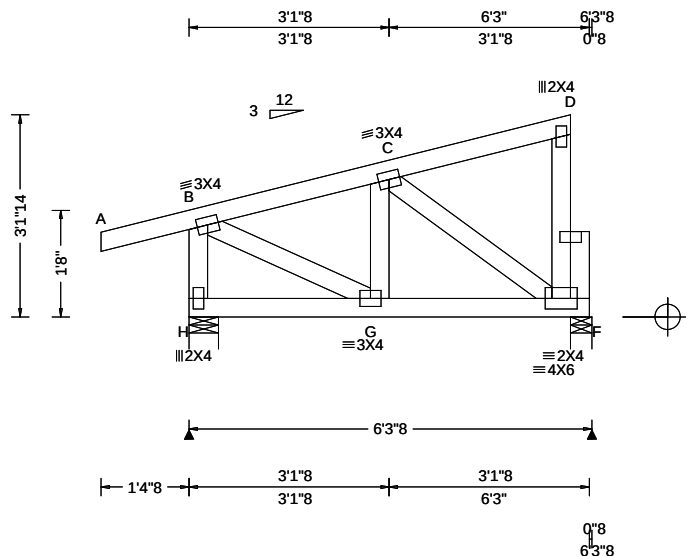
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 543081 / FROM: RFG	MONO Ply: 1 Qty: 1	Job Number: 23-9006 Nickelson Shed Truss Label: FT6	Cust: R 215 JRef:1XNR2150007 T11 / DrwNo: 065.23.1458.20480 SSB / YK 03/06/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 40.00 TCDL: 10.00 BCLL: 0.00 BCDL: 5.00 Des Ld: 55.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.00 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 3.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.005 C 999 360 VERT(CL): 0.007 C 999 240 HORZ(LL): 0.001 F - - HORZ(TL): 0.001 F - - Creep Factor: 2.0 Max TC CSI: 0.248 Max BC CSI: 0.098 Max Web CSI: 0.333 VIEW Ver: 22.02.00.0914.12	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL H 505 /- /- /122 /104 /103 F 301 /- /- /62 /93 /- Wind reactions based on MWFRS H Brg Wid = 5.5 Min Req = 1.5 (Truss) F Brg Wid = 4.0 Min Req = 1.5 (Truss) Bearings H & F are a rigid surface. Members not listed have forces less than 375# Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. B - H 104 -491

Lumber

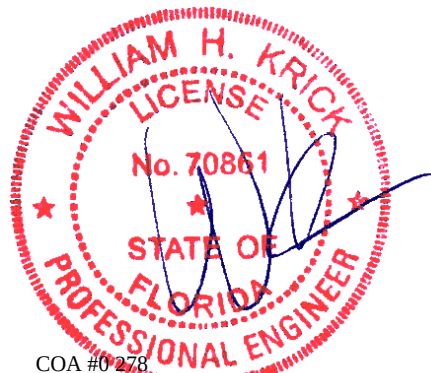
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Rt Bearing Leg: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Left end vertical exposed to wind pressure. Deflection meets L/360.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 3-1-14.



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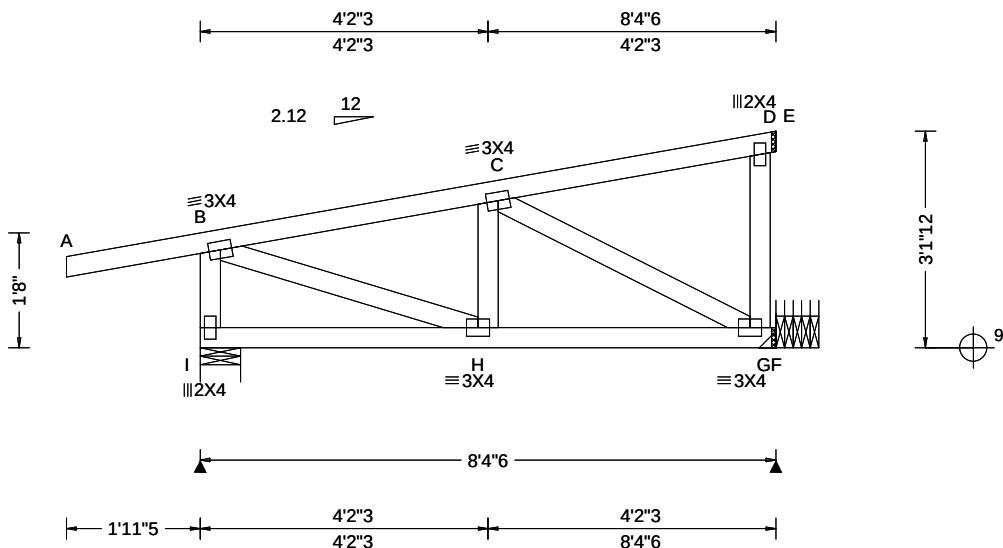
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SEQN: 543776 FROM: RFG	HIP_	Ply: 1 Qty: 4	Job Number: 23-9006 Nickelson Shed Truss Label: HGJ1	Cust: R 215 JRef:1XNR2150007 T18 DrwNo: 065.23.1513.22800 KD / WHK 03/06/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.007 H 999 360 VERT(CL): 0.014 H 999 240 HORZ(LL): -0.002 E - - HORZ(TL): 0.003 E - - Creep Factor: 2.0 Max TC CSI: 0.556 Max BC CSI: 0.510 Max Web CSI: 0.190 VIEW Ver: 22.02.00.0914.12	Gravity Loc R+ / R- / Rh / Rw / U / RL I 303 /- /- /- /86 /41 F 537 /- /- /- /43 /- Non-Gravity Wind reactions based on MWFRS I Brg Wid = 7.0 Min Req = 1.5 (Truss) F Brg Wid = - Min Req = - Bearing I is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. B - C 53 -450

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Special Loads

----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 0 plf at -1.94 to 60 plf at 0.00
TC: From 2 plf at 0.00 to 2 plf at 8.36
BC: From 0 plf at -1.94 to 4 plf at 0.00
BC: From 2 plf at 0.00 to 2 plf at 8.36
TC: -31 lb Conc. Load at 1.32
TC: 136 lb Conc. Load at 4.15
TC: 274 lb Conc. Load at 6.98
BC: 37 lb Conc. Load at 1.32
BC: 117 lb Conc. Load at 4.15
BC: 197 lb Conc. Load at 6.98

Hangers / Ties

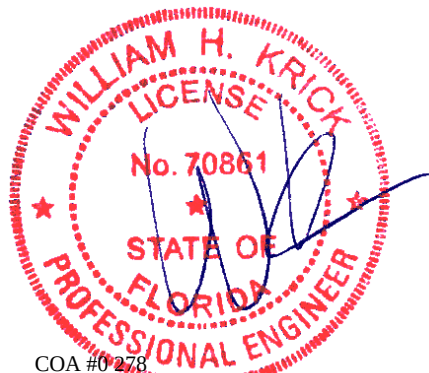
(J) Hanger Support Required, by others

Wind

Wind loads and reactions based on MWFRS.
Left end vertical exposed to wind pressure. Deflection meets L/360.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 3-1-12.



COA #0278

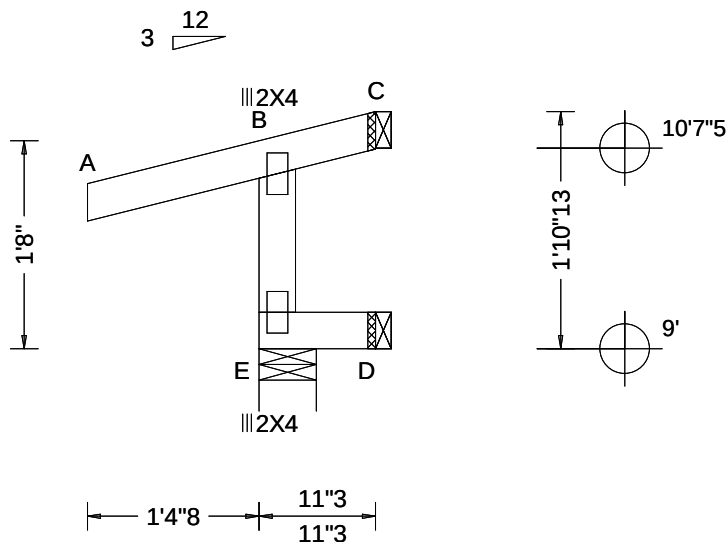
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SEQN: 543764 FROM: RFG	JACK Ply: 1 Qty: 8	Job Number: 23-9006 Nickelson Shed Truss Label: J1	Cust: R 215 JRef: 1XNR2150007 T16 DrwNo: 065.23.1513.24670 KD / WHK 03/06/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.000 B 999 360 VERT(CL): 0.000 B 999 240 HORZ(LL): -0.000 B - - HORZ(TL): 0.000 B - - Creep Factor: 2.0 Max TC CSI: 0.123 Max BC CSI: 0.008 Max Web CSI: 0.052 VIEW Ver: 22.02.00.0914.12	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL E 192 /- /- /125 /97 /- D 19 /- /- /9 /- /22 C - /-37 /- /34 /40 /39 Wind reactions based on MWFRS E Brg Wid = 5.5 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = - Bearing E is a rigid surface. Members not listed have forces less than 375#

Lumber

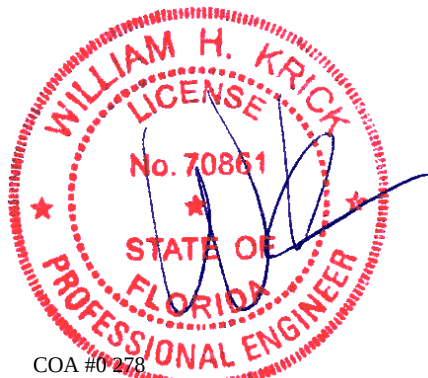
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Left end vertical exposed to wind pressure. Deflection meets L/360.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 1'-10-13.



COA #0278

03/06/2023
Florida Certificate of Product Approval #FL 1999

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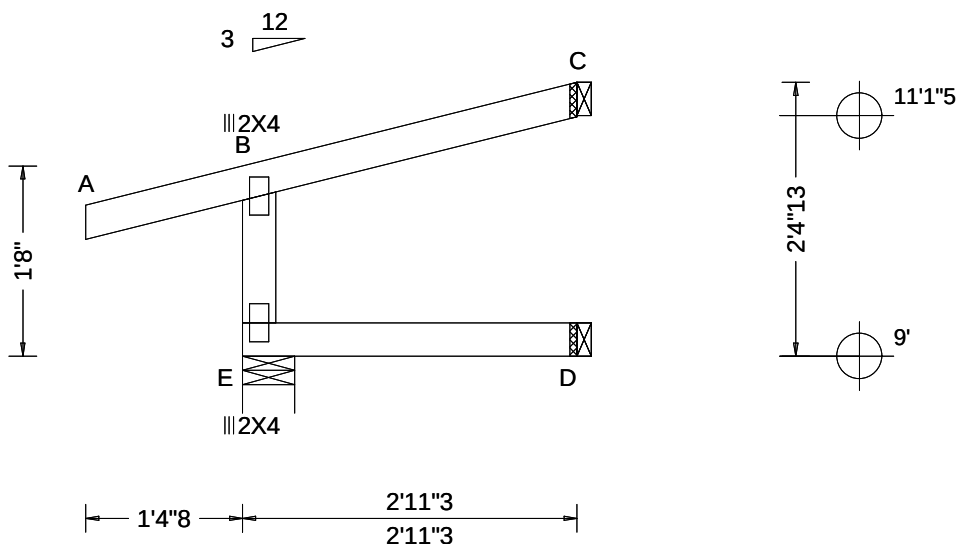
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Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 543767 FROM: RFG	JACK Ply: 1 Qty: 8	Job Number: 23-9006 Nickelson Shed Truss Label: J3	Cust: R 215 JRef:1XNR2150007 T14 DrwNo: 065.23.1513.33087 KD / WHK 03/06/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.000 B 999 360 VERT(CL): 0.000 B 999 240 HORZ(LL): -0.000 B - - HORZ(TL): 0.000 B - - Creep Factor: 2.0 Max TC CSI: 0.134 Max BC CSI: 0.094 Max Web CSI: 0.053 VIEW Ver: 22.02.00.0914.12	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL E 228 /- /- /138 /82 /- D 59 /- /- /29 /- /22 C 68 /- /- /30 /19 /57 Wind reactions based on MWFRS E Brg Wid = 5.5 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = - Bearing E is a rigid surface. Members not listed have forces less than 375#

Lumber

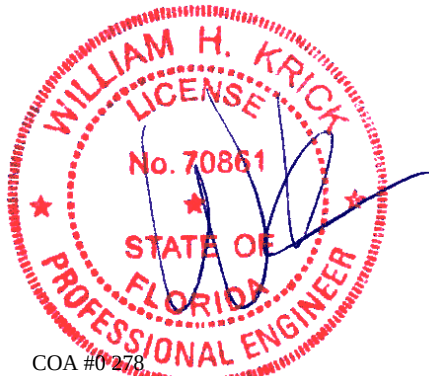
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Left end vertical exposed to wind pressure. Deflection meets L/360.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 2'-4"-13."



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03/06/2023
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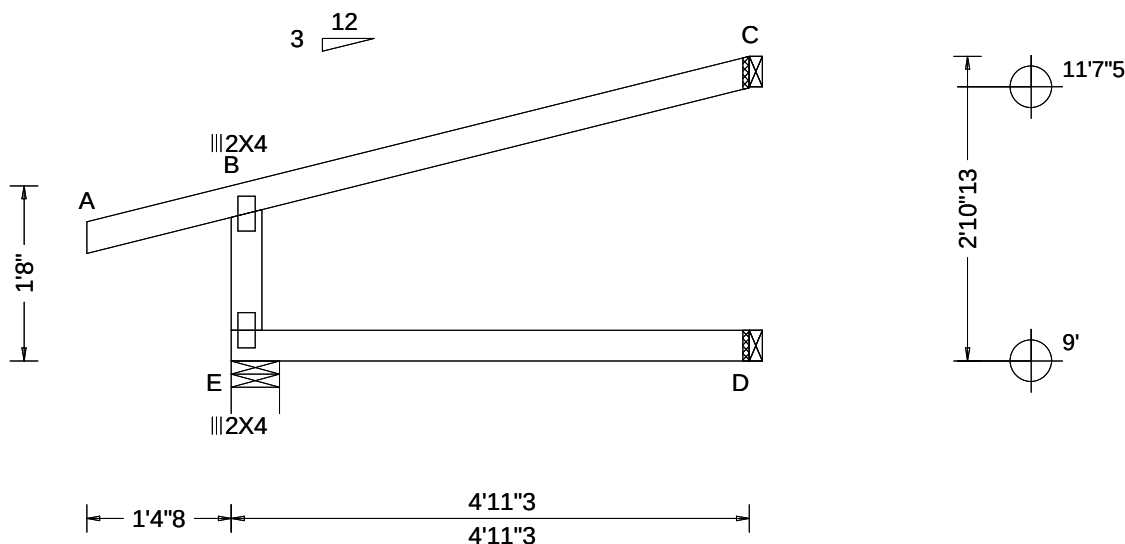
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SEQN: 543770 FROM: RFG	JACK Ply: 1 Qty: 8	Job Number: 23-9006 Nickelson Shed Truss Label: J5	Cust: R 215 JRef:1XNR2150007 T13 DrwNo: 065.23.1513.34887 KD / WHK 03/06/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.000 B 999 360 VERT(CL): 0.001 B 999 240 HORZ(LL): -0.000 B - - HORZ(TL): 0.000 B - - Creep Factor: 2.0 Max TC CSI: 0.370 Max BC CSI: 0.289 Max Web CSI: 0.050 VIEW Ver: 22.02.00.0914.12	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL E 300 /- /- /173 /87 /- D 99 /- /- /49 /- /22 C 137 /- /- /54 /45 /74 Wind reactions based on MWFRS E Brg Wid = 5.5 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = - Bearing E is a rigid surface. Members not listed have forces less than 375#

Lumber

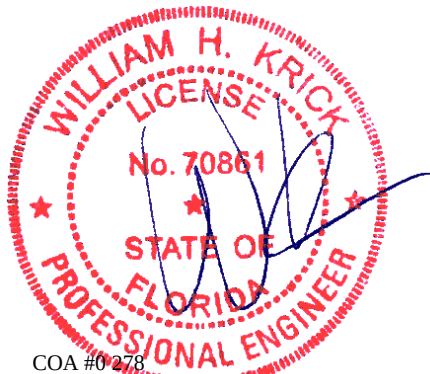
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Left end vertical exposed to wind pressure. Deflection meets L/360.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 2-10-13.



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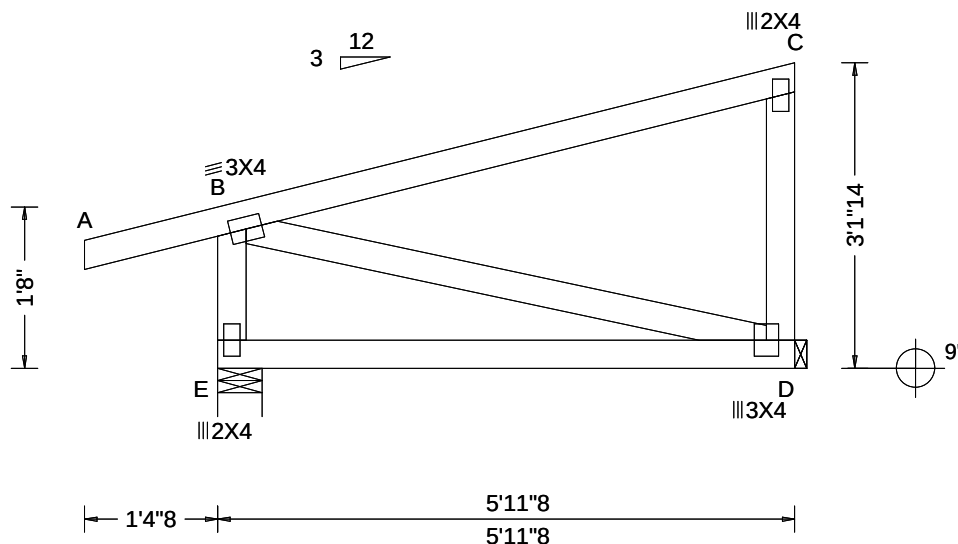
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 543773 FROM: RFG	EJAC Ply: 1 Qty: 20	Job Number: 23-9006 Nickelson Shed Truss Label: J6	Cust: R 215 JRef: 1XNR2150007 T17 DrwNo: 065.23.1513.38710 KD / WHK 03/06/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.000 C 999 360 VERT(CL): 0.001 C 999 240 HORZ(LL): -0.001 C - - HORZ(TL): 0.002 C - - Creep Factor: 2.0 Max TC CSI: 0.503 Max BC CSI: 0.374 Max Web CSI: 0.161 VIEW Ver: 22.02.00.0914.12	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL E 339 /- /- /178 /70 /103 D 230 /- /- /125 /68 /- Wind reactions based on MWFRS E Brg Wid = 5.5 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = - Bearing E is a rigid surface. Members not listed have forces less than 375#

Lumber

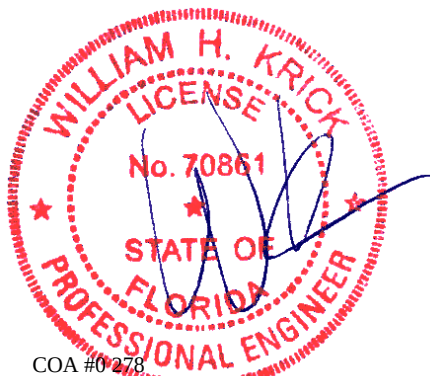
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Left end vertical exposed to wind pressure. Deflection meets L/360.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 3-1-14.



COA #0278

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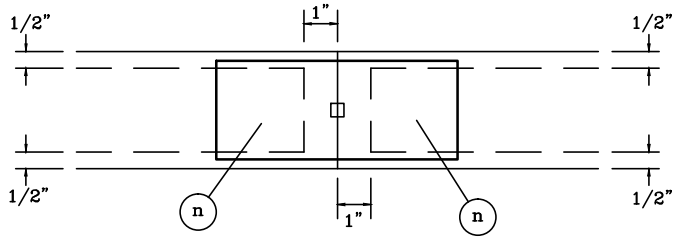
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TRULOX INFORMATION DETAIL

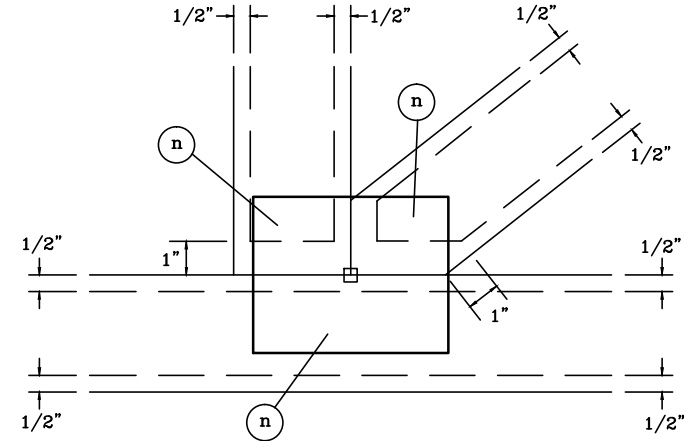
TYPICAL OFF PANEL SPLICE



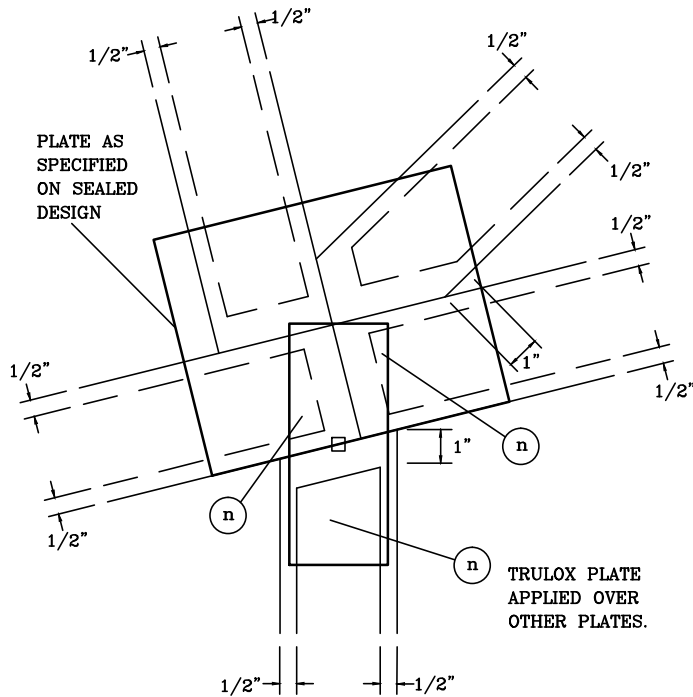
DO NOT APPLY NAILS WITHIN 1/2" OF LUMBER EDGES OR 1" OF LUMBER ENDS ON EACH FACE, AS SHOWN BY DASHED LINES.

NAILS MUST NOT SPLIT LUMBER.

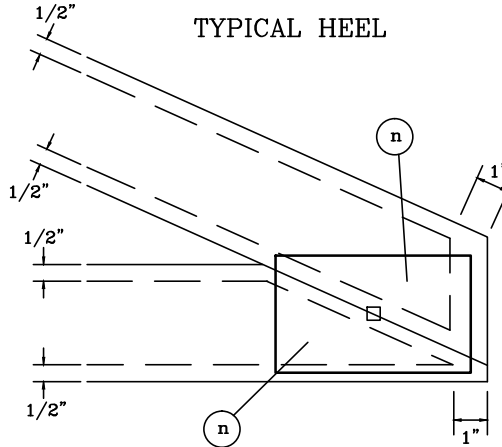
TYPICAL PANEL POINT WITHOUT SPLICE



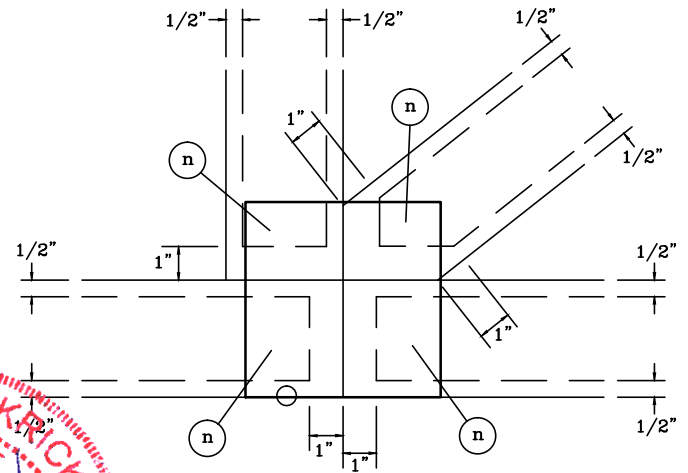
TYPICAL FILLER



TYPICAL HEEL



TYPICAL PANEL POINT SPLICE



TRULOX PLATE
APPLIED OVER
OTHER PLATES.

NOTES:

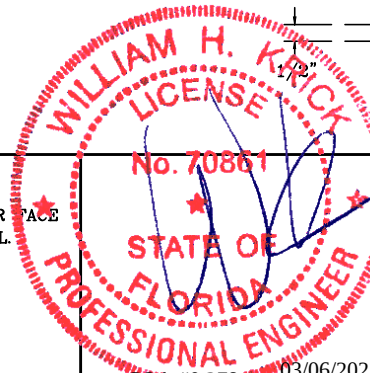
(n) IS THE REQUIRED NUMBER OF 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY AS SPECIFIED ON THE SEALED DESIGN REFERENCING THIS DETAIL.

○ LOCATES PLATE CORNER OR FLUSH EDGE.

□ LOCATES PLATE CENTER.



155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025



COA #0-278

03/06/2023

Florida Certificate of Product Approval #FL 1999

TRULOX PLATING

160
TL

PAGE 1 OF 1

DATE 10/01/14

Gable Stud Reinforcement Detail

ASCE 7-16: 140 mph Wind Speed, 30' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or: 120 mph Wind Speed, 30' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00

Or: 120 mph Wind Speed, 30' Mean Height, Enclosed, Exposure D, Kzt = 1.00

Or: 100 mph wind speed, 30' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

Max Gable Vertical Length	2x4 Gable Vertical		Brace Grade	No Braces	(1) 1x4 'L' Brace *		(1) 2x4 'L' Brace *		(2) 2x4 'L' Brace **		(1) 2x6 'L' Brace *		(2) 2x6 'L' Brace **	
	Spacing	Species			Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
24" O.C.	SPF	#1 / #2	#1	4' 1"	6' 11"	7' 2"	8' 2"	8' 6"	9' 9"	10' 2"	12' 10"	13' 4"	14' 0"	14' 0"
			#3	3' 10"	6' 2"	6' 7"	8' 1"	8' 5"	9' 8"	10' 0"	12' 8"	13' 2"	14' 0"	14' 0"
			Stud	3' 10"	6' 2"	6' 6"	8' 1"	8' 5"	9' 8"	10' 0"	12' 8"	13' 2"	14' 0"	14' 0"
		Standard	#1	4' 2"	7' 0"	7' 3"	8' 3"	8' 7"	9' 10"	10' 3"	13' 0"	13' 6"	14' 0"	14' 0"
			#2	4' 1"	6' 11"	7' 2"	8' 2"	8' 6"	9' 9"	10' 2"	12' 10"	13' 4"	14' 0"	14' 0"
			#3	4' 0"	5' 7"	5' 11"	7' 5"	7' 11"	9' 8"	10' 1"	11' 7"	12' 5"	14' 0"	14' 0"
	SP	DFL	Stud	4' 0"	5' 7"	5' 11"	7' 5"	7' 11"	9' 8"	10' 1"	11' 7"	12' 5"	14' 0"	14' 0"
			Standard	3' 9"	4' 11"	5' 13"	6' 6"	7' 0"	8' 10"	9' 6"	10' 3"	11' 0"	13' 11"	14' 0"
		Standard	#1 / #2	4' 8"	7' 11"	8' 3"	9' 4"	9' 9"	11' 2"	11' 7"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	4' 5"	7' 6"	8' 0"	9' 3"	9' 7"	11' 0"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	4' 5"	7' 6"	8' 0"	9' 3"	9' 7"	11' 0"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	#1	4' 10"	8' 0"	8' 4"	9' 6"	9' 10"	11' 3"	11' 9"	14' 0"	14' 0"	14' 0"	14' 0"
16" O.C.	SPF	#2	#2	4' 8"	7' 11"	8' 3"	9' 4"	9' 9"	11' 2"	11' 7"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	4' 7"	6' 10"	7' 3"	8' 3"	8' 7"	10' 10"	11' 3"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	4' 7"	6' 10"	7' 3"	8' 3"	8' 7"	10' 10"	11' 3"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	#1 / #2	5' 2"	8' 9"	9' 1"	10' 4"	10' 9"	12' 2"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	4' 10"	8' 7"	8' 11"	10' 2"	10' 7"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	4' 10"	8' 7"	8' 11"	10' 2"	10' 7"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
	SP	DFL	#1	5' 4"	8' 10"	9' 2"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	5' 2"	8' 9"	9' 1"	10' 4"	10' 9"	12' 2"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	5' 0"	7' 10"	8' 4"	10' 3"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	Stud	5' 0"	7' 10"	8' 4"	10' 3"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
			Standard	4' 10"	6' 11"	7' 4"	9' 3"	9' 10"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
			Standard	4' 10"	6' 11"	7' 4"	9' 3"	9' 10"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"

Bracing Group Species and Grades:

Group A:			
Spruce-Pine-Fir		Hem-Fir	
#1 / #2	Standard	#2	Stud
#3	Stud	#3	Standard
Douglas Fir-Larch		Southern Pine***	
#3		#3	
Stud		Stud	
Standard		Standard	

Group B:			
Hem-Fir			
#1 & Btr			
#1			
Douglas Fir-Larch		Southern Pine***	
#1		#1	
#2		#2	

1x4 Braces shall be SRB (Stress-Rated Board).

***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:

Wind Load deflection criterion is L/240.

Provide uplift connections for 100 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12' plywood overhang.

Attach 'L' braces with 10d (0.128"x3.0" min) nails.

* For (1) 'L' brace: space nails at 2' o.c. in 18' end zones and 4' o.c. between zones.

** For (2) 'L' braces: space nails at 3' o.c. in 18' end zones and 6' o.c. between zones.

'L' bracing must be a minimum of 80% of web member length.

Gable Vertical Plate Sizes

Vertical Length	No Splice
Less than 4' 0"	2X4
Greater than 4' 0", but less than 11' 6"	3X4
Greater than 11' 6"	4X4

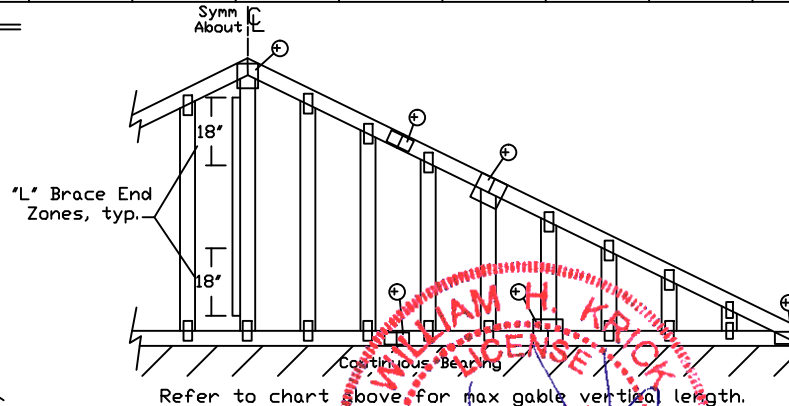
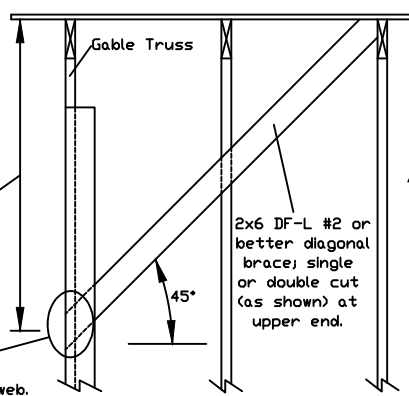
+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.

Diagonal brace option: vertical length may be doubled when diagonal brace is used. Connect diagonal brace for 525# at each end. Max web total length is 14'.

Vertical length shown in table above.

Connect diagonal at midpoint of vertical web.



Refer to chart above for max gable vertical length.

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For more information see this job's general notes page and these web sites:

ALPINE: www.alpineitw.com; TPI: www.tpinet.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org



155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

No. 70861

STATE OF

FLORIDA

PROFESSIONAL ENGINEER

COA #0278

03/06/2023

Florida Certificate of Product Approval

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF ASCE7-16-GAB14030

DATE 01/26/2018

DRWG A14030ENC160118

CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforcement or scab reinforcement.

Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

Use scabs instead of L- or T- reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

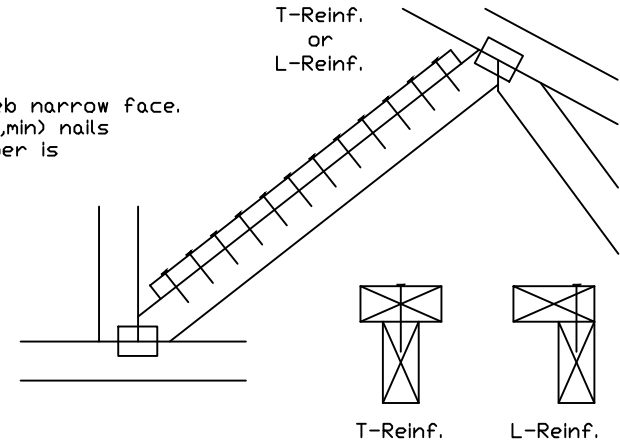
Web Member Size	Specified CLR Restraint	Alternative Reinforcement T- or L- Reinf.	Scab Reinf.
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4(*)
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6(*)

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

(*) Center scab on wide face of web. Apply (1) scab to each face of web.

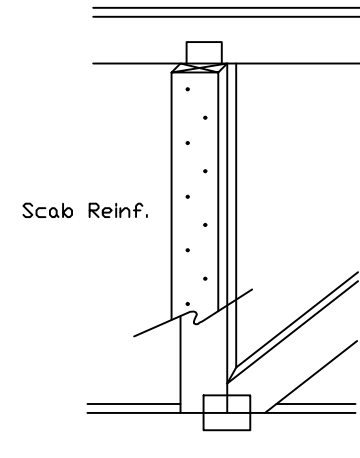
T-Reinforcement or L-Reinforcement:

Apply to either side of web narrow face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



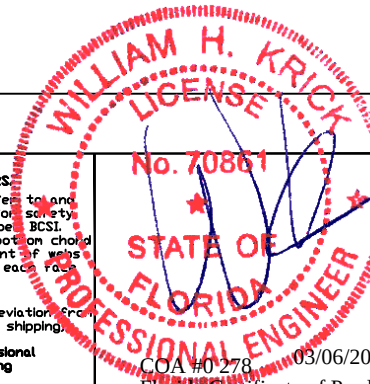
Scab Reinforcement:

Apply scab(s) to wide face of web. No more than (1) scab per face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



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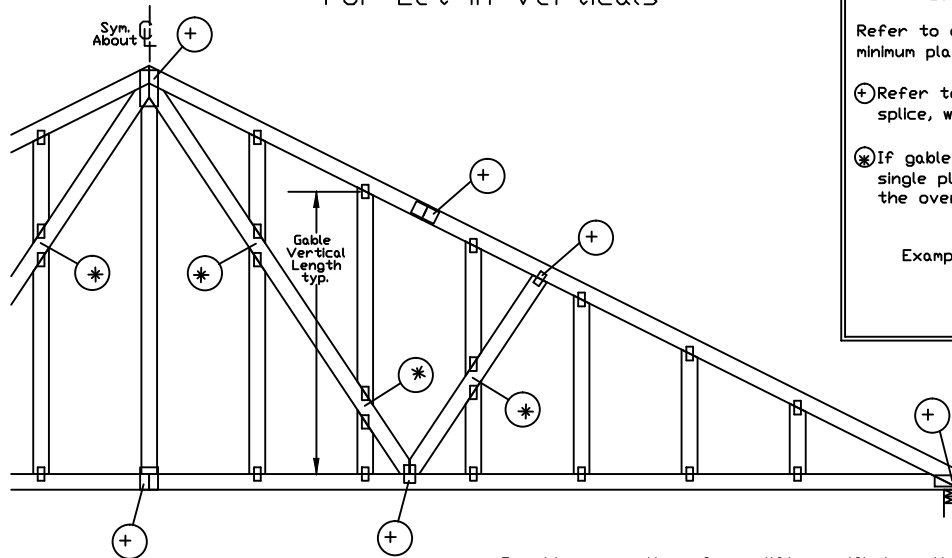
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COA #0278 03/06/2023
Florida Certificate of Product Approval

TC LL	PSF	REF CLR Subst.
TC DL	PSF	DATE 01/02/19
BC DL	PSF	DRWG BRCLBSUB0119
BC LL	PSF	
TOT. LD.	PSF	
DUR. FAC.		
SPACING		

Gable Detail For Let-in Verticals

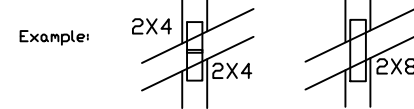


Gable Truss Plate Sizes

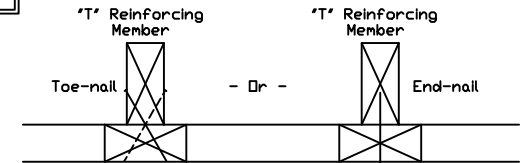
Refer to appropriate Alpine gable detail for minimum plate sizes for vertical studs.

⊕ Refer to Engineered truss design for peak, splice, web, and heel plates.

⊗ If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.



'T' Reinforcement Attachment Detail



To convert from 'L' to 'T' reinforcing members, multiply 'T' increase by length (based on appropriate Alpine gable detail).

Maximum allowable 'T' reinforced gable vertical length is 14' from top to bottom chord.

'T' reinforcing member material must match size, specie, and grade of the 'L' reinforcing member.

Web Length Increase w/ 'T' Brace

'T' Reinf. Mbr. Size	'T' Increase
2x4	30 %
2x6	20 %

Example:

ASCE 7-10 Wind Speed = 120 mph

Mean Roof Height = 30 ft, Kzt = 1.00

Gable Vertical = 24' o.c. SP #3

'T' Reinforcing Member Size = 2x4

'T' Brace Increase (From Above) = 30% = 1.30

(1) 2x4 'L' Brace Length = 8' 7"

Maximum 'T' Reinforced Gable Vertical Length
1.30 x 8' 7" = 11' 2"

Provide connections for uplift specified on the engineered truss design.

Attach each 'T' reinforcing member with

End Driven Nails:

10d Common (0.148"x 3", min) Nails at 4' o.c. plus
(4) nails in the top and bottom chords.

Toenailed Nails:

10d Common (0.148"x 3", min) Toenails at 4' o.c. plus
(4) toenails in the top and bottom chords.

This detail to be used with the appropriate Alpine gable detail for ASCE wind load.

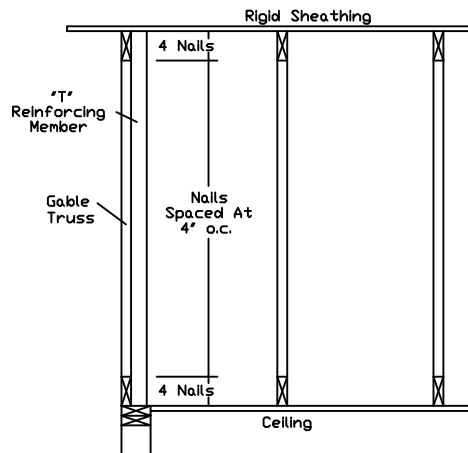
ASCE 7-05 Gable Detail Drawings

A13015051014, A12015051014, A11015051014, A10015051014, A14015051014,
A13030051014, A12030051014, A11030051014, A10030051014, A14030051014

ASCE 7-10 & ASCE 7-16 Gable Detail Drawings

A11515ENC100118, A12015ENC100118, A14015ENC100118, A16015ENC100118,
A18015ENC100118, A20015ENC100118, A20015END100118, A20015PED100118,
A11530ENC100118, A12030ENC100118, A14030ENC100118, A16030ENC100118,
A18030ENC100118, A20030ENC100118, A20030END100118, A20030PED100118,
S11515ENC100118, S12015ENC100118, S14015ENC100118, S16015ENC100118,
S18015ENC100118, S20015ENC100118, S20015END100118, S20015PED100118,
S11530ENC100118, S12030ENC100118, S14030ENC100118, S16030ENC100118,
S18030ENC100118, S20030ENC100118, S20030END100118, S20030PED100118

See appropriate Alpine gable detail for maximum allowable gable vertical length.



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ALPINE
AN ITW COMPANY

155 Harlem Ave
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Glenview, IL 60025

No. 70861

STATE OF
FLORIDA
PROFESSIONAL ENGINEER

COA #0 278 03/06/2023
Florida Certificate of Product

MAX. TOT. LD. 60 PSF

DUR. FAC. ANY

Approval #FL 1999

MAX. SPACING 24.0"

REF LET-IN VERT

DATE 01/02/2018

DRWG GBLLETIN0118