

DATE 02/12/2008

Columbia County Building Permit
This Permit Must Be Prominently Posted on Premises During Construction**PERMIT**
000026743

APPLICANT CRAIG TIMBERAKE PHONE 352-472-6850
ADDRESS 25370 NW 8TH PLACE NEWBERRY FL 32669
OWNER VIC PATEL PHONE 386-288-4669
ADDRESS 592 SWEET BREEZE TRAIL LAKE CITY FL 32025
CONTRACTOR TIMBERLAKE ALUMINUM PHONE 352-472-6850

LOCATION OF PROPERTY 90 W, L SWEET BREEZE TERR (CYPRESS LAKE S/D),STAY TO THE
RIGHT, ON THE R JUST BEFORE THE DIP ON TE ROAD

TYPE DEVELOPMENT SCREEN ENCLOSURE ESTIMATED COST OF CONSTRUCTION 4437.00

HEATED FLOOR AREA TOTAL AREA HEIGHT STORIES

FOUNDATION WALLS ROOF PITCH FLOOR

LAND USE & ZONING RSF-2 MAX. HEIGHT 35

Minimum Set Back Requirments: STREET-FRONT 25.00 REAR 15.00 SIDE 10.00

NO. EX.D.U. 1 FLOOD ZONE NA DEVELOPMENT PERMIT NO.

PARCEL ID 33-3S-16-02434-117 SUBDIVISION CYPRESS LAKE

LOT 17 BLOCK PHASE 2 UNIT TOTAL ACRES 0.95

SCG056710
Culvert Permit No. Culvert Waiver Contractor's License Number Applicant/Owner/Contractor Craig Timberlake
EXISTING X08-032 CS JH N
Driveway Connection Septic Tank Number LU & Zoning checked by Approved for Issuance New Resident

COMMENTS: NOC ON FILE,

Check # or Cash 1598

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power Foundation Monolithic
date/app. by date/app. by date/app. by

Under slab rough-in plumbing Slab Sheathing/Nailing
date/app. by date/app. by date/app. by

Framing Rough-in plumbing above slab and below wood floor
date/app. by date/app. by

Electrical rough-in Heat & Air Duct Peri. beam (Lintel)
date/app. by date/app. by date/app. by

Permanent power C.O. Final Culvert
date/app. by date/app. by date/app. by

M/H tie downs, blocking, electricity and plumbing Pool
date/app. by date/app. by

Reconnection Pump pole Utility Pole
date/app. by date/app. by date/app. by

M/H Pole Travel Trailer Re-roof
date/app. by date/app. by date/app. by

BUILDING PERMIT FEE \$ 25.00 CERTIFICATION FEE \$ 0.00 SURCHARGE FEE \$ 0.00

MISC. FEES \$ 0.00 ZONING CERT. FEE \$ 50.00 FIRE FEE \$ 0.00 WASTE FEE \$

FLOOD DEVELOPMENT FEE \$ FLOOD ZONE FEE \$ CULVERT FEE \$ **TOTAL FEE** 75.00

INSPECTORS OFFICE L.H. CLERKS OFFICE msb

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED TO BE IN ACTIVE PROGRESS WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Columbia County Building Permit Application

For Office Use Only	Application # <u>0802-05</u>	Date Received <u>2/11</u>	By <u>JW</u>	Permit # <u>26743</u>
Zoning Official <u>ads</u>	Date <u>2/11/08</u>	Flood Zone <u>N/A</u>	FEMA Map # _____	Zoning <u>RSF-2</u>
Land Use <u>RLD</u>	Elevation _____	MFE _____	River _____	Plans Examiner <u>DKJTH</u>
Date <u>2-6-8</u>				
Comments _____				
<input type="checkbox"/> NOC <input checked="" type="checkbox"/> EH <input type="checkbox"/> Deed or PA <input type="checkbox"/> Site Plan <input type="checkbox"/> State Road Info <input type="checkbox"/> Parent Parcel # _____				
<input type="checkbox"/> Dev Permit # _____ <input type="checkbox"/> In Floodway <input type="checkbox"/> Letter of Authorization from Contractor				
<input type="checkbox"/> Unincorporated area <input type="checkbox"/> Incorporated area <input type="checkbox"/> Town of Fort White <input type="checkbox"/> Town of Fort White Compliance letter				

Septic Permit No. _____ Fax 352-472-6855

Name Authorized Person Signing Permit Craig Timberlake Phone 352-472-6850

Address 25370 NW 8th Place Newberry, FL 32669

Owners Name Vic Patel Phone 386-288-4669

911 Address 592 Sweet Breeze Trail

Contractors Name Timberlake Aluminum Const. - C. HEUS Phone 352-472-6850

Address 25370 NW 8th Place Newberry, FL 32669

Fee Simple Owner Name & Address _____ N/A

Bonding Co. Name & Address _____ N/A

Architect/Engineer Name & Address Lawrence Bennett PO Box 214368 South Daytona, FL 32121

Mortgage Lenders Name & Address _____ N/A

Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progress Energy

Property ID Number 33-35-16-02434-117HX Estimated Cost of Construction \$4,437.00

Subdivision Name Cypress Lake Lot 17 Block _____ Unit _____ Phase 2

Driving Directions 90-W TO CYPRESS LAKE. S. D. @ SWEETBREEZE, IRL STAY
to the R. - HOME ON THE R. BDE "Just before Dip on the Road" start
turning @ 592

Number of Existing Dwellings on Property _____

Construction of Screen enclosure Total Acreage .954 Lot Size _____

Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive Total Building Height _____

Actual Distance of Structure from Property Lines - Front 44' Side 80' Side 28' Rear 70'

Number of Stories _____ Heated Floor Area _____ Total Floor Area 55 Roof Pitch _____

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

Columbia County Building Permit Application

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment

According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE:

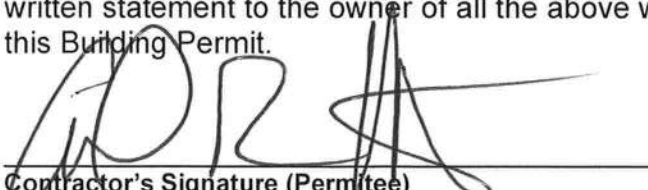
YOU ARE HEREBY NOTIFIED as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

OWNERS CERTIFICATION: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning. I further understand the above written responsibilities in Columbia County for obtaining this Building Permit.



Owners Signature

CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit.



Contractor's Signature (Permittee)

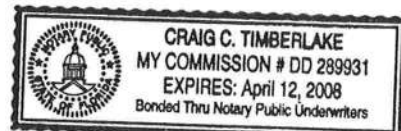
Contractor's License Number SCG 056710
Columbia County
Competency Card Number _____

Affirmed under penalty of perjury to by the Contractor and subscribed before me this 4 day of Feb 2008
Personally known ☒ or Produced Identification _____



State of Florida Notary Signature (For the Contractor)

SEAL:



NOTICE OF COMMENCEMENT

Tax Parcel Identification Number 33-35-16-02434-117-HX

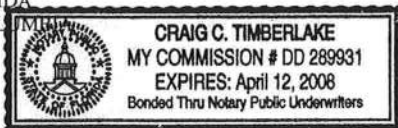
Inst:200812002301 Date:2/6/2008 Time:8:35 AM
~~17~~ DC, P. DeWitt Cason, Columbia County Page 1 of 1

THE UNDERSIGNED hereby gives notice that improvements will be made to certain real property, and in accordance with Section 713.13 of the Florida Statutes, the following information is provided in this NOTICE OF COMMENCEMENT.

1. Description of property (legal description): Lot 17 Cypress Lake Phase 2, ORB 744-672-Pod 744-671,862-2129 890-1117, WD 1060-422
 - a) Street (job) Address: 592 Sweet Breeze Trail Lake City, FL
2. General description of improvements: Screen enclosure.
3. Owner Information
 - a) Name and address: Vic Patel 592 Sweetbreeze Trail, Lake City FL
 - b) Name and address of fee simple titleholder (if other than owner) N/A
 - c) Interest in property Owner
4. Contractor Information
 - a) Name and address: Timberlake Aluminum Const. 25370 NW 8th Place Newberry, FL
 - b) Telephone No.: 852-472-6850 Fax No. (Opt.) _____
5. Surety Information
 - a) Name and address: N/A
 - b) Amount of Bond: _____
 - c) Telephone No.: _____ Fax No. (Opt.) _____
6. Lender
 - a) Name and address: N/A
 - b) Phone No.: _____
7. Identity of person within the State of Florida designated by owner upon whom notices or other documents may be served:
 - a) Name and address: _____
 - b) Telephone No.: _____ Fax No. (Opt.) _____
8. In addition to himself, owner designates the following person to receive a copy of the Lienor's Notice as provided in Section 713.13(1)(b), Florida Statutes:
 - a) Name and address: _____
 - b) Telephone No.: _____ Fax No. (Opt.) _____
9. Expiration date of Notice of Commencement (the expiration date is one year from the date of recording unless a different date is specified): _____

WARNING TO OWNER: ANY PAYMENTS MADE BY THE OWNER AFTER THE EXPIRATION OF THE NOTICE OF COMMENCEMENT ARE CONSIDERED IMPROPER PAYMENTS UNDER CHAPTER 713, PART I, SECTION 713.13, FLORIDA STATUTES, AND CAN RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY; A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT YOUR LENDER OR AN ATTORNEY BEFORE COMMENCING WORK OR RECORDING YOUR NOTICE OF COMMENCEMENT.

STATE OF FLORIDA
 COUNTY OF COL



10. [Signature]
 Signature of Owner or Owner's Authorized Office/Director/Partner/Manager
VIC PATEL
 Print Name

The foregoing instrument was acknowledged before me, a Florida Notary, this 1 day of Feb, 20 08, by: _____ as _____ (type of authority, e.g. officer, trustee, attorney

fact) for _____ (name of party on behalf of whom instrument was executed).

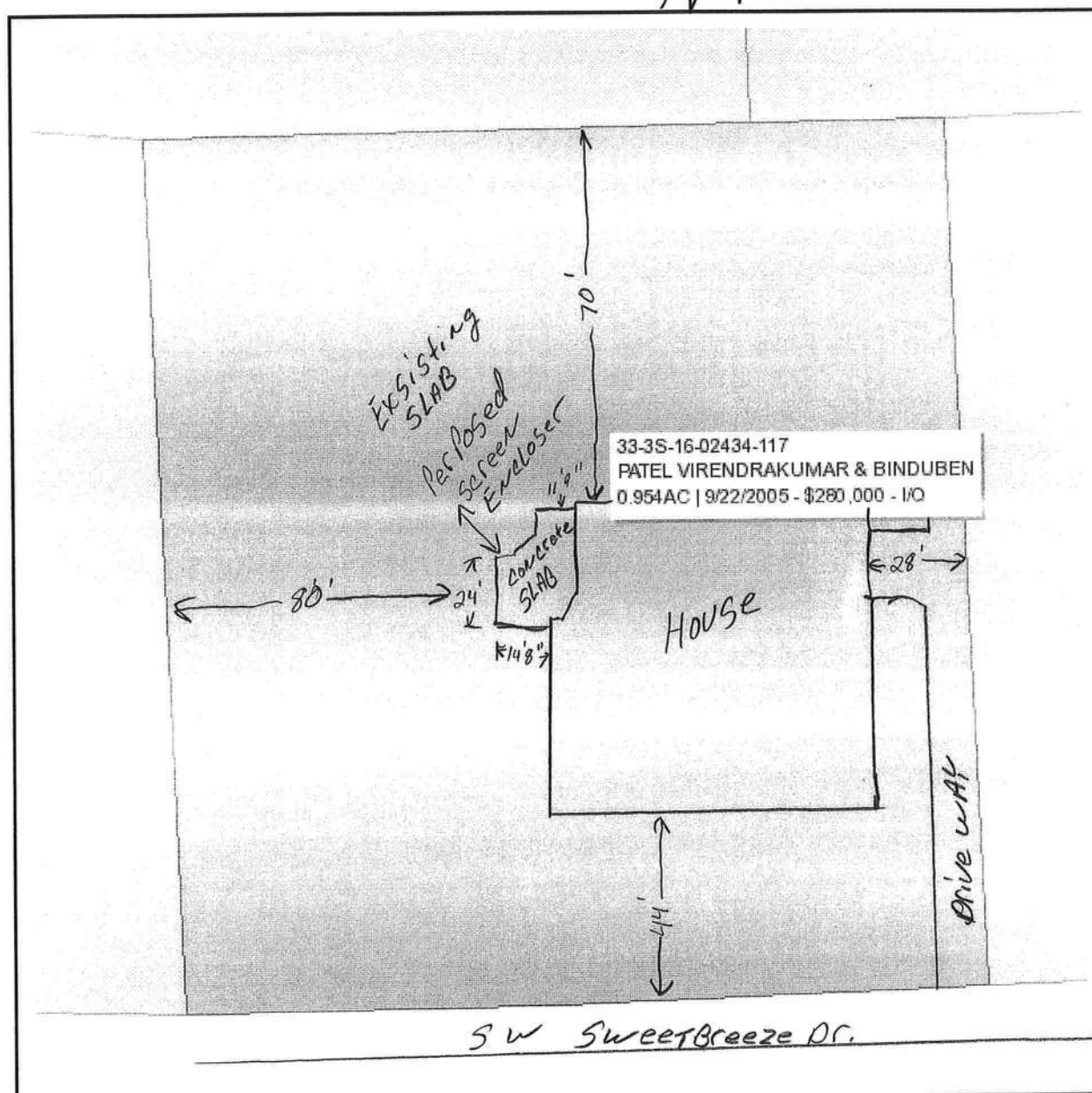
Personally Known ☒ OR Produced Identification _____ Type _____

Notary Signature Craig C Timberlake Notary Stamp or Seal: _____

---AND---

11. Verification pursuant to Section 92.525, Florida Statutes. Under penalties of perjury, I declare that I have read the foregoing and that the facts stated in it are true to the best of my knowledge and belief.

[Signature]
 Signature of Natural Person Signing (in line #10 above.)



Columbia County Property Appraiser

J. Doyle Crews, CFA Lake City, Florida - 386-758-1083

PARCEL: 33-3S-16-02434-117 HX - SINGLE FAM (000100)

Name:	PATEL VIRENDRAKUMAR & BINDUBEN	LandVal	\$42,500.00
Site:	SWEETBREEZE	BldgVal	\$198,448.00
Mail:	592 SW SWEETBREEZE DR	ApprVal	\$246,202.00
	LAKE CITY, FL 32024	JustVal	\$246,202.00
Sales	9/22/2005 \$280,000.00 / Q	Assd	\$246,202.00
Info	10/21/1999 \$30,000.00V / U	Exmpt	\$25,000.00
	7/21/1998 \$19,000.00V / Q	Taxable	\$221,202.00

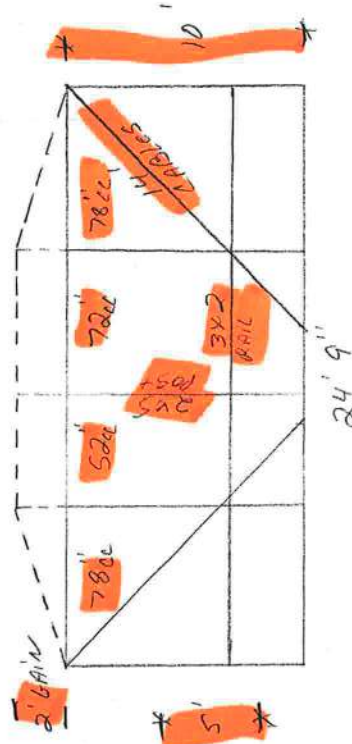
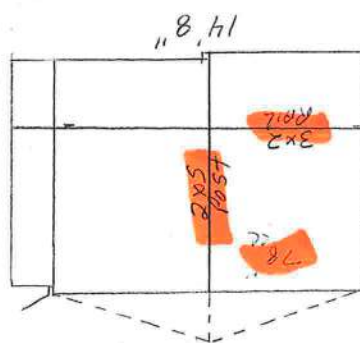
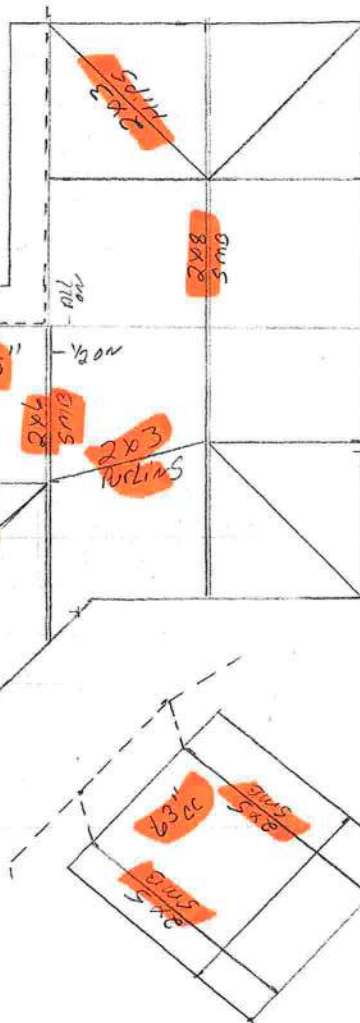
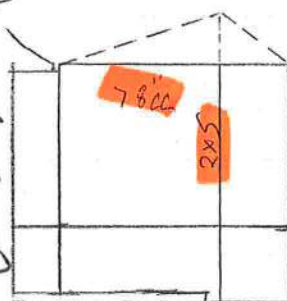
0 17 34 51 ft



This information, GIS Map Updated: 1/15/2008, was derived from data which was compiled by the Columbia County Property Appraiser Office solely for the governmental purpose of property assessment. This information should not be relied upon by anyone as a determination of the ownership of property or market value. No warranties, expressed or implied, are provided for the accuracy of the data herein, its use, or its interpretation. Although it is periodically updated, this information may not reflect the data currently on file in the Property Appraiser's office. The assessed values are NOT certified values and therefore are subject to change before being finalized for ad valorem assessment purposes.

Patel / TAC #181
592 Sweet Breeze Trail
Lake City FL

13'	Beam Span	*
17'	Beam Span	*
23'	Beam Span	†



FILE COPY

Bennet 06

Design Check List for Pool Enclosures (Page 1 of 4)

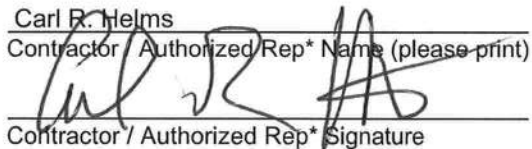
I. Design Statement:

These plans have been designed in accordance with the Aluminum Structures Design Manual by Lawrence E. Bennett and are in compliance with the 2004 Florida Building Code Edition with 2006 Supplements, Chapter 20, ASM35 and The 2005 Aluminum Design Manual Part I-A & II-A; Exposure 'B' ☒ or 'C' ☐ or 'D' ☐; Importance Factor 0.87 for 100 MPH and 0.77 for 110 MPH and higher; Negative I.P.C. 0.00; ___ MPH Wind Zone for 3 second wind gust; Basic Wind Pressure ___; Design pressures are ___ PSF for roofs & ___ PSF for walls. (see page 1ii for wind loads and design pressures) A 300 PLF point load is also considered for screen roof members.

Notes: Wind velocity zones and exposure category is determined by local code. Design pressures and conversion multipliers are on page 1-ii.

II. Host Structure Adequacy Statement:

I have inspected and verify that the host structure is in good repair and attachments made to the structure will be solid.

Carl R. Helms Phone: 352 - 472 - 6850
Contractor / Authorized Rep* Name (please print)
 Date: 2 / 1 / 08
Contractor / Authorized Rep* Signature

PATEL / 592 SWEETBREEZE TRAIL

Job Name & Address

Note: If the total of beam span & upright height exceeds 50' or upright height exceeds 16', site specific engineering is required.

III. Building Permit Application Package contains the following:

	Yes	No
A. Project name & address on plans	<input checked="" type="checkbox"/>	<input type="checkbox"/>
B. Site plan or survey with enclosure location	<input checked="" type="checkbox"/>	<input type="checkbox"/>
C. Contractor's / Designer's name, address, phone number, & signature on plans	<input checked="" type="checkbox"/>	<input type="checkbox"/>
D. Site exposure form completed	<input checked="" type="checkbox"/>	<input type="checkbox"/>
E. Enclosure layout drawing @ 1/8" or 1/10" scale with the following:	<input checked="" type="checkbox"/>	<input type="checkbox"/>
1. Plan view with host structure, enclosure length, projection from host structure, and all dimensions	<input checked="" type="checkbox"/>	<input type="checkbox"/>
2. Front and side elevation views with all dimensions & heights	<input checked="" type="checkbox"/>	<input type="checkbox"/>
Note: All mansard wall drawings shall include mansard panel at the top of the wall.		
3. Beam location (show in plan & elevation view) & size (Table 1.1 & 1.6)	<input checked="" type="checkbox"/>	<input type="checkbox"/>

Roof frame member allowable span conversions from 120 MPH wind zone,
"B" Exposure to 120 MPH wind zone and / or ☐ 'C' or ☐ 'D' Exposure for load
width of ___:

Note: Conversion factors do not apply to members subject to point load (P).

Look up span in appropriate 120 MPH span table and apply the following formula:

$$\begin{array}{c} \text{Span} \\ \text{@ 120 MPH} \end{array} \quad \begin{array}{c} \text{Required Converted} \\ \text{Span / Height} \end{array}$$
$$\frac{0.00}{\text{Wind Zone Multiplier (see page 1ii)}} (b \text{ or } d) \times \frac{1.00}{\text{Exposure Multiplier (see page 1ii)}} (b \text{ or } d) = \underline{\hspace{2cm}}$$

4. Upright location (show in plan & elevation view) & size (Table 1.3 & 1.6)	<input checked="" type="checkbox"/>	<input type="checkbox"/>
5. Chair rail & girt size, length, & spacing (Table 1.4)	<input checked="" type="checkbox"/>	<input type="checkbox"/>
6. Eave rail size, length, spacing and stitching of (Table 1.2)	<input checked="" type="checkbox"/>	<input type="checkbox"/>

* Must have attended Engineer's Continuing Education Class within the past two years.

Design Check List for Pool Enclosures (Page 2 of 4)

Wall frame member allowable span conversions from 120 MPH wind zone, "B" Exposure to _____ MPH wind zone and / or ☐ "C" or ☐ "D" Exposure for load width of 1.00 :
Look up span in appropriate 120 MPH span table and apply the following formula:

$$\begin{array}{c} \text{Span / Height} \\ \text{@ 120 MPH} \\ \text{or _____ MPH} \end{array} \quad \begin{array}{c} \swarrow \\ \text{7.91} \\ \text{Wind Zone} \\ \text{Multiplier **} \end{array} \quad (b \text{ or } d) \times \begin{array}{c} \swarrow \\ \text{1.10} \\ \text{Exposure Multiplier} \\ \text{(see page 1ii)} \end{array} \quad (b \text{ or } d) \times \begin{array}{c} \swarrow \\ \text{1.00} \\ \text{Exposure Multiplier} \\ \text{(see page 1ii)} \end{array} \quad (b \text{ or } d) = \begin{array}{c} \swarrow \\ \text{8.70} \\ \text{Required Converted} \\ \text{Span / Height} \end{array}$$

	Yes	No
7. Enclosure roof diagonal bracing in plan view	<input checked="" type="checkbox"/>	<input type="checkbox"/>
8. Knee braces length, location, & size	<input checked="" type="checkbox"/>	<input type="checkbox"/>
(Table 1.7)		
9. Wall cables or K-bracing sizes shown in wall views	<input checked="" type="checkbox"/>	<input type="checkbox"/>
IV. Highlight details from the Aluminum Structures Design Manual:	Yes	No
A. Beam & purlin tables with size, thickness, spacing, & spans / lengths	<input checked="" type="checkbox"/>	<input type="checkbox"/>
(Tables 1.1 & 1.2 or 1.9.1 & 1.9.2)		
B. Upright & girt tables with size, thickness, spacing, & spans / lengths	<input checked="" type="checkbox"/>	<input type="checkbox"/>
(Tables 1.3 & 1.4)		
C. Table 1.6 with beam & upright combination	<input checked="" type="checkbox"/>	<input type="checkbox"/>
D. Connection details to be use such as:		
1. Beam to upright	<input checked="" type="checkbox"/>	<input type="checkbox"/>
2. Beam to wall	<input checked="" type="checkbox"/>	<input type="checkbox"/>
3. Beam to beam	<input checked="" type="checkbox"/>	<input type="checkbox"/>
4. Chair rail, purlins, & knee braces	<input checked="" type="checkbox"/>	<input type="checkbox"/>
5. Extruded gutter connections	<input checked="" type="checkbox"/>	<input type="checkbox"/>
6. Angle to deck and / or sole plate	<input checked="" type="checkbox"/>	<input type="checkbox"/>
7. Anchors go through pavers into concrete	<input type="checkbox"/>	<input checked="" type="checkbox"/>
8. Minimum footing and / or knee wall details	<input checked="" type="checkbox"/>	<input type="checkbox"/>
9. Cable or K- brace details Section 1	<input checked="" type="checkbox"/>	<input type="checkbox"/>

Wall area calculations for cables:

W = wall width, H = wall height, R = rise

W1 = width @ top of mansard, W2 = width @ top of wall

E. Select footing from examples in manual.

Example 1: Flat Roof

Front wall @ eave: $\frac{\text{W}}{\text{H}} \text{ ft.} \times \frac{\text{H}}{\text{a}} \text{ ft.} = \frac{0.00}{\text{a}} \text{ ft.}^2 @ 100\% = \underline{\hspace{2cm}} \text{ ft.}^2$

Largest side wall: $\frac{\text{W}}{\text{H}} \text{ ft.} \times \frac{\text{H}}{\text{b}} \text{ ft.} = \frac{0.00}{\text{b}} \text{ ft.}^2 @ 50\% = \underline{\hspace{2cm}} \text{ ft.}^2$

TOTAL = $\underline{\hspace{2cm}} \text{ ft.}^2$

Total area / (233 ft.² / cable for 3/32") = 0 cable pairs

or

Total area / (445 ft.² / cable for 1/8") = 0 cable pairs

Side wall cable calculation: $\frac{0.00}{\text{b}} \text{ ft.}^2 @ 100\% = \underline{\hspace{2cm}} \text{ ft.}^2$

Side wall area / (233 ft.² / cable for 3/32") = 0 cable(s)

or

Side wall area / (445 ft.² / cable for 1/8") = 0 cable(s)

Design Check List for Pool Enclosures (Page 4 of 4)

Example 4: Mansard Roof

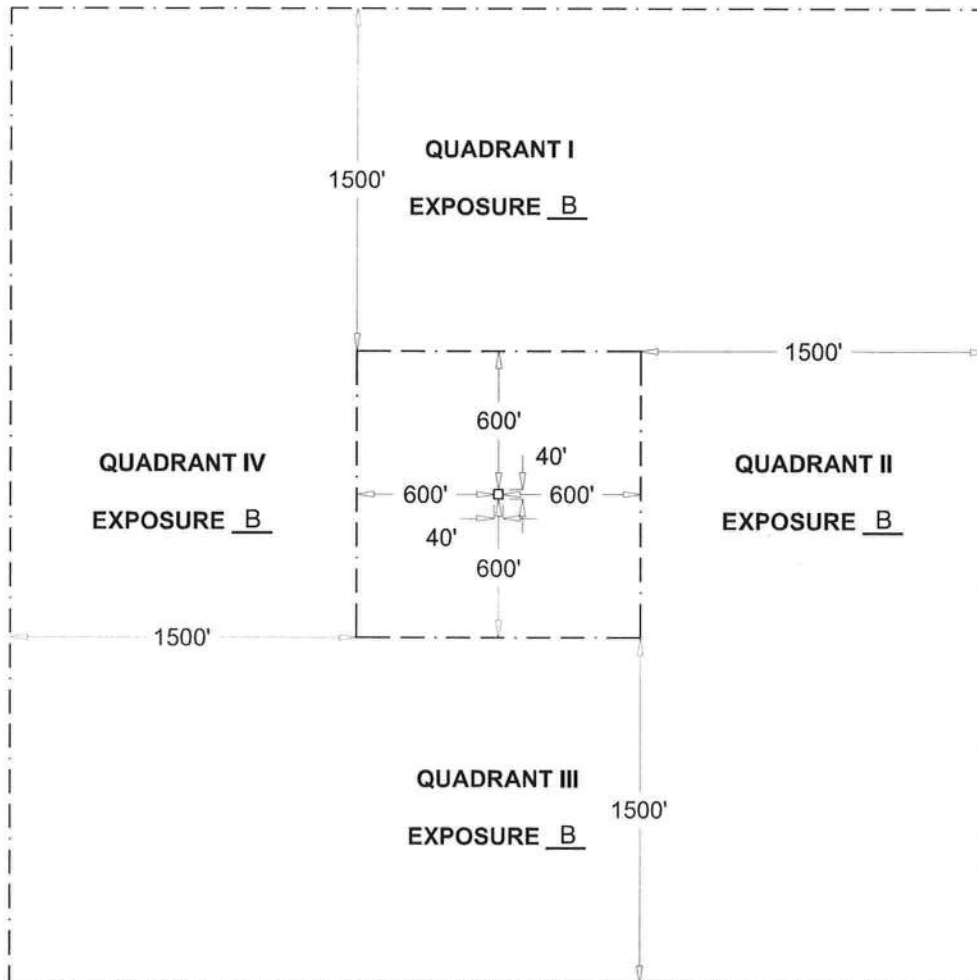
$$\begin{aligned}
 \text{Front wall @ eave: } & \frac{24.90}{W} \text{ ft.} \times \frac{10.00}{H} \text{ ft.} = \frac{249.00}{a} \text{ ft.}^2 @ 100\% = \underline{249.00} \text{ ft.}^2 \\
 \text{Front mansard rise: } & \frac{2.00}{R} \text{ ft.} \times \frac{1}{2} \left(\frac{13.00}{W1} \text{ ft.} + \frac{24.90}{W2} \text{ ft.} \right) = \frac{37.90}{b} \text{ ft.}^2 @ 100\% = \underline{37.90} \text{ ft.}^2 \\
 \text{Largest side wall: } & \frac{14.80}{W} \text{ ft.} \times \frac{10.00}{H} \text{ ft.} = \frac{148.00}{c} \text{ ft.}^2 @ 50\% = \underline{74.00} \text{ ft.}^2 \\
 \text{Largest side mansard rise: } & \frac{2}{R} \text{ ft.} \times \frac{1}{2} \left(\frac{7.00}{W1} \text{ ft.} + \frac{14.00}{W2} \text{ ft.} \right) = \frac{21.00}{d} \text{ ft.}^2 @ 50\% = \underline{10.50} \text{ ft.}^2 \\
 & \text{TOTAL} = \underline{371.40} \text{ ft.}^2 \\
 \text{Total area / (233 ft.}^2 \text{ / cable for 3/32")} & = \underline{2} \text{ cable pairs} \\
 \text{or} \\
 \text{Total area / (445 ft.}^2 \text{ / cable for 1/8")} & = \underline{1} \text{ cable pairs} \\
 \text{Side wall cable calculation: } & \frac{148.00}{c} \text{ ft.}^2 + \frac{21.00}{d} \text{ ft.}^2 = \frac{169.00}{d} \text{ ft.}^2 @ 100\% = \underline{169.00} \text{ ft.}^2 \\
 \text{Side wall area / (233 ft.}^2 \text{ / cable for 3/32")} & = \underline{1} \text{ cable(s)} \\
 \text{or} \\
 \text{Side wall area / (445 ft.}^2 \text{ / cable for 1/8")} & = \underline{0} \text{ cable(s)}
 \end{aligned}$$

Example 5: Dome Roof

$$\begin{aligned}
 \text{Front dome wall @ eave: } & \frac{\quad}{W} \text{ ft.} \times \frac{\quad}{H} \text{ ft.} = \frac{0.00}{a} \text{ ft.} @ 100\% = \underline{0.00} \text{ ft.}^2 \\
 \text{Front dome rise: } & \frac{\quad}{R} \text{ ft.} \times \frac{1}{2} \left(\frac{\quad}{W} \text{ ft.} \right) = \frac{0.00}{b} \text{ ft.}^2 @ 100\% = \underline{0.00} \text{ ft.}^2 \\
 \text{Largest side wall: } & \frac{\quad}{W} \text{ ft.} \times \frac{\quad}{H} \text{ ft.} = \frac{0.00}{c} \text{ ft.}^2 @ 50\% = \underline{0.00} \text{ ft.}^2 \\
 \text{Largest side dome rise: } & \frac{\quad}{R} \text{ ft.} \times \frac{\quad}{W} \text{ ft.} = \frac{0.00}{d} \text{ ft.}^2 @ 50\% = \underline{0.00} \text{ ft.}^2 \\
 & \text{TOTAL} = \underline{0.00} \text{ ft.}^2 \\
 \text{Total area / (233 ft.}^2 \text{ / cable for 3/32")} & = \underline{0} \text{ cable pairs} \\
 \text{or} \\
 \text{Total area / (445 ft.}^2 \text{ / cable for 1/8")} & = \underline{0} \text{ cable pairs} \\
 \text{Side wall cable calculation: } & \frac{0.00}{c} \text{ ft.}^2 + \frac{0.00}{d} \text{ ft.}^2 = \frac{0.00}{d} \text{ ft.}^2 @ 100\% = \underline{0.00} \text{ ft.}^2 \\
 \text{Side wall area / (233 ft.}^2 \text{ / cable for 3/32")} & = \underline{0} \text{ cable(s)} \\
 \text{or} \\
 \text{Side wall area / (445 ft.}^2 \text{ / cable for 1/8")} & = \underline{0} \text{ cable(s)}
 \end{aligned}$$

Notes:

SITE EXPOSURE EVALUATION FORM



NOTE: ZONES ARE MEASURED FROM STRUCTURE OUTWARD

SITE

SCALE: 1" = 800'

USING THE FOLLOWING CRITERIA, EVALUATE EACH QUADRANT AND MARK IT AS 'B', 'C', OR 'D' EXPOSURE. 'C' OR 'D' EXPOSURE IN ANY QUADRANT MAKE THE SITE THAT EXPOSURE.

- EXPOSURE C:
1. OPEN TERRAIN FOR MORE THAN 1,500 FEET IN ANY QUADRANT.
 2. ANY 'C' EXPOSURE FOR GREATER THAN 600 FEET IN ANY QUADRANT.
 3. NO SHORT TERM CHANGES IN 'B', 2 YEARS BEFORE SITE EVALUATION AND BUILD OUT WITHIN 3 YEARS, SITE WILL BE 'B'.
 4. FLAT, OPEN COUNTRY, GRASSLANDS, PONDS AND OCEAN OR SHORELINES IN ANY QUADRANT FOR GREATER THAN 1,500 FEET.

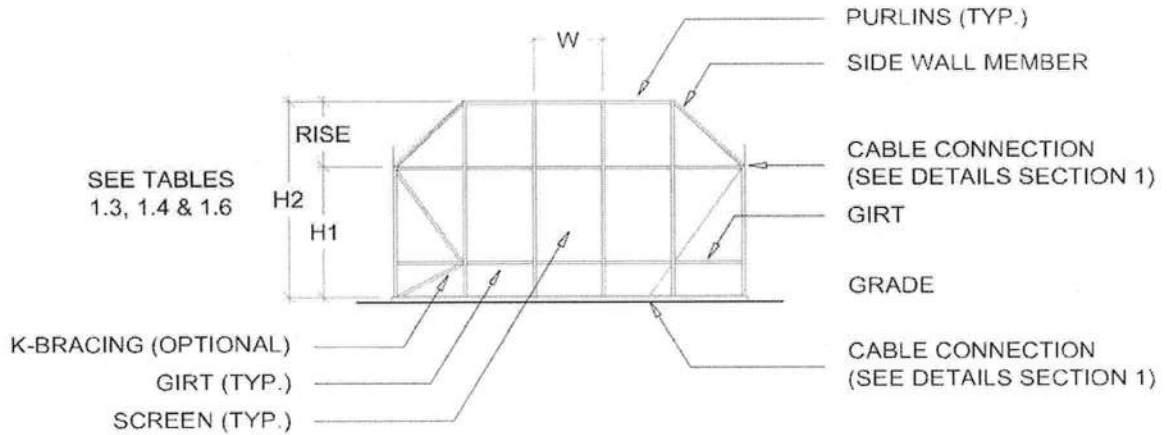
EXPOSURE D: FLAT, UNOBSTRUCTED AREAS THAT ARE 1,500 FT INLAND FROM THE SHORE LINE AND ARE EXPOSED TO WIND FLOWING OVER WATER FOR A DISTANCE OF AT LEAST 1 MILE.

SITE IS EXPOSURE: B EVALUATED BY: Carl R. Helms DATE: 2/1/08

SIGNATURE: [Signature] LICENSE #: SCC056710

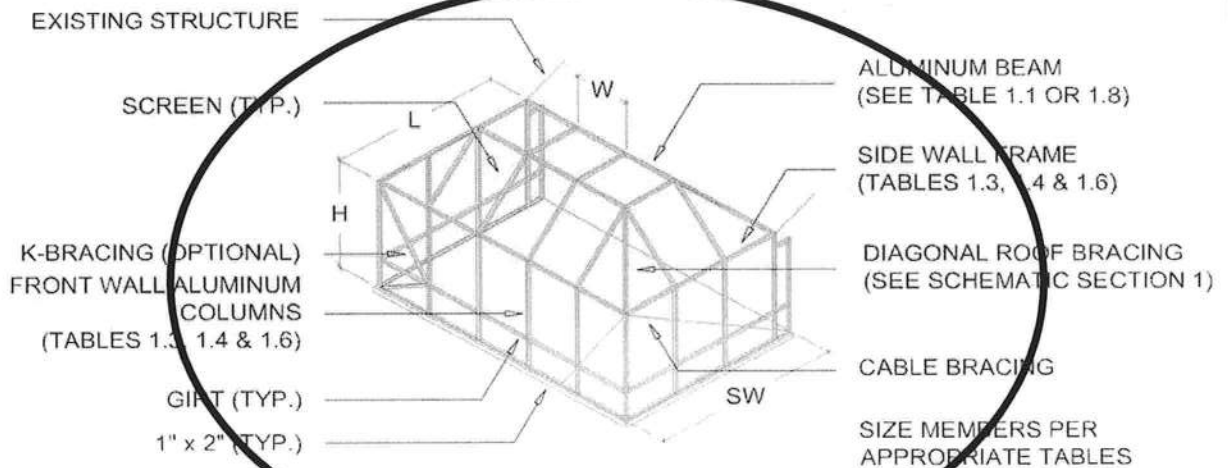
SECTION 1

SCREENED ENCLOSURES



TYPICAL MANSARD ROOF - FRONT WALL ELEVATION

SCALE: N.T.S.



TYPICAL MANSARD ROOF - ISOMETRIC

SCALE: N.T.S.

CONNECTION DETAILS AND NOTES ARE FOUND IN THE SUBSEQUENT PAGES.

Lawrence E. Bennett, P.E. FL # 16644

CIVIL & STRUCTURAL ENGINEERING

P.O. Box 214368, South Daytona, FL 32121

Telephone #: (386) 767-4774 Fax #: (386) 767-6556

Email: lebpe@bellsouth.net

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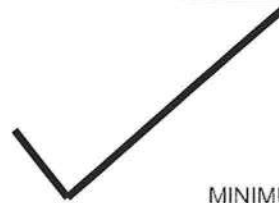
1-2

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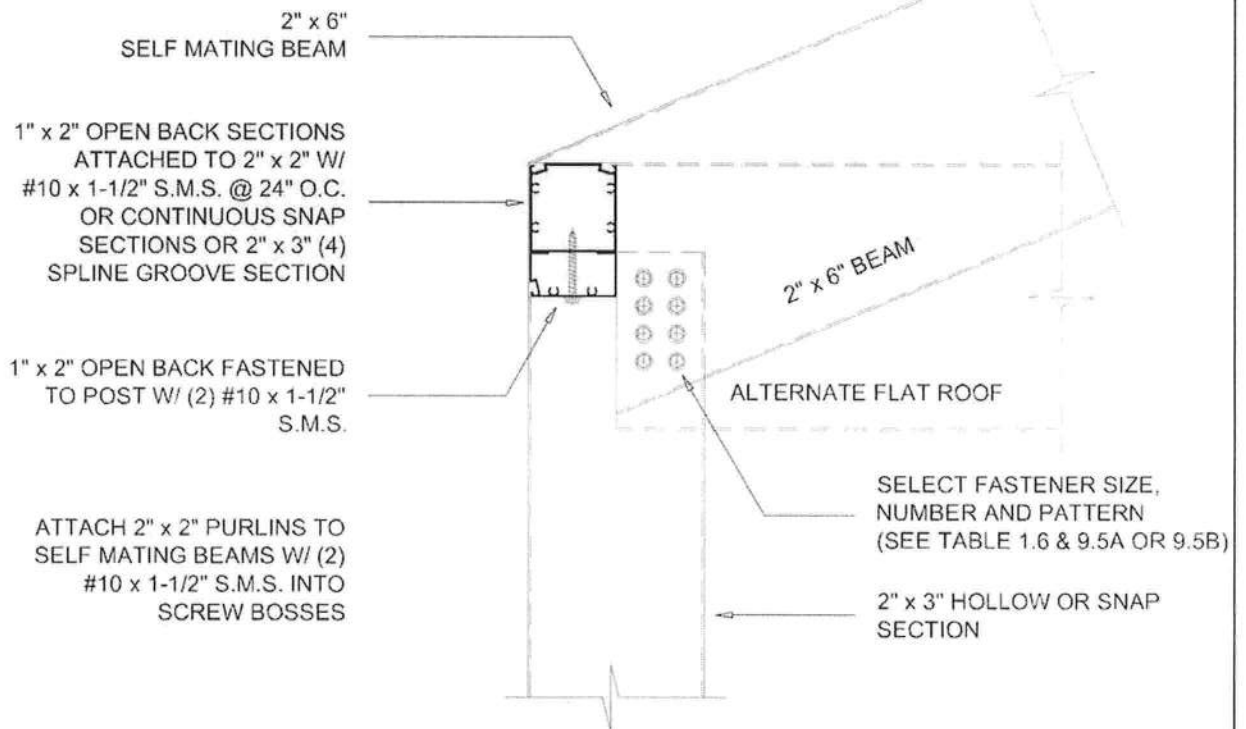
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SCREENED ENCLOSURES

SECTION 1



MINIMUM POST SIZES
REQUIRED FOR EACH BEAM
SIZE (SEE TABLE 1.6)



SLOPING BEAM TO UPRIGHT CONNECTION DETAIL (PARTIAL LAP)

SCALE: 3" = 1'-0"

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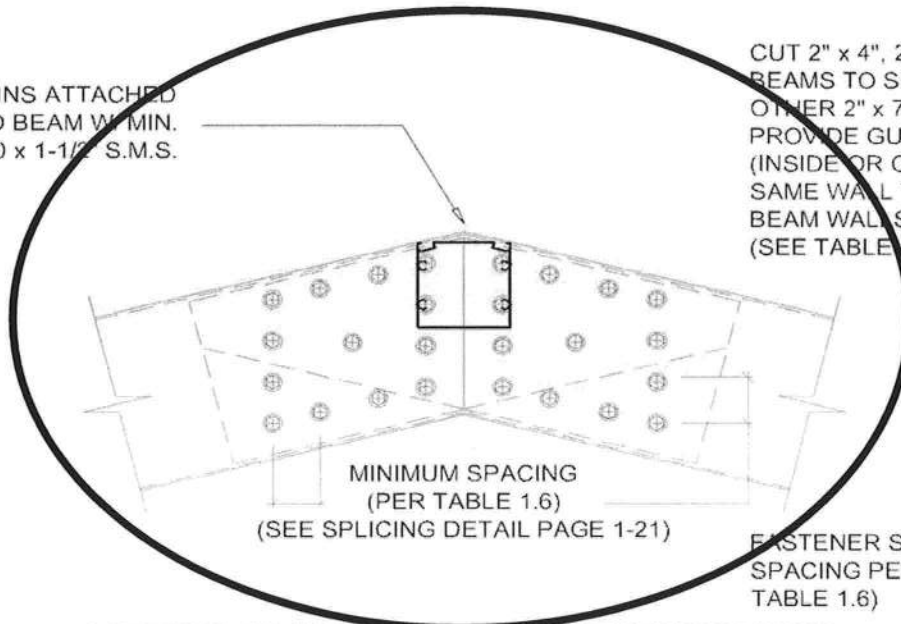
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Telephone #: (386) 767-4774 Fax #: (386) 767-6556

Email: lebpe@bellsouth.net

SECTION 1**SCREENED ENCLOSURES**

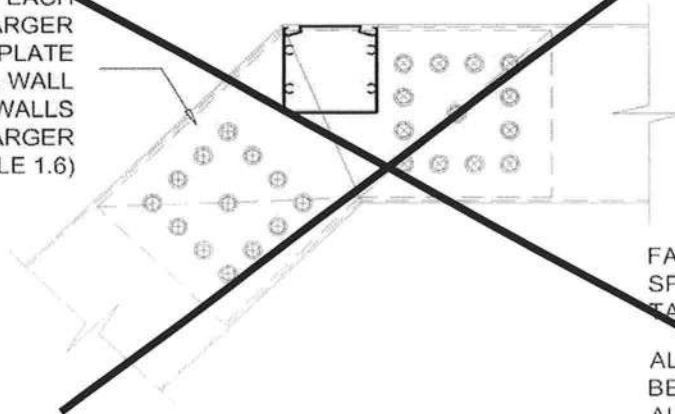
2" x 2" PURLINS ATTACHED
TO BEAM W/ MIN.
(3) #10 x 1-1/2" S.M.S.



**ALTERNATE SIDE PLATE CONNECTION DETAIL
GUSSET PLATE MOUNTED INTERNALLY**

SCALE: 3" = 1'-0"

CUT 2" x 4", 2" x 5", OR 2" x 6"
BEAMS TO SLIDE OVER EACH
OTHER 2" x 7" & LARGER
PROVIDE GUSSET PLATE
(INSIDE BEAM) SAME WALL
THICKNESS AS BEAM WALLS
OR LARGER
(SEE TABLE 1.6)



FASTENER SIZE, NUMBER AND
SPACING PER PAGE 1-20(SEE
TABLE 1.6)

ALL GUSSET PLATES SHALL
BE A MINIMUM OF 5052 H-32
ALLOY OR HAVE AN ULTIMATE
YIELD STRENGTH OF 30 KSI

**ALTERNATE SIDE PLATE CONNECTION DETAIL - MANSARD ROOF
GUSSET PLATE MOUNTED INTERNALLY**

SCALE: 3" = 1'-0"

Lawrence E. Bennett, P.E. FL # 16644
CIVIL & STRUCTURAL ENGINEERING
P.O. Box 214368, South Daytona, FL 32121
Telephone #: (386) 767-4774 Fax #: (386) 767-6556
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SCREENED ENCLOSURES

SECTION 1

1/4" x 2" LAG SCREWS @ 24"
O.C. OR #10 x 2" SCREWS @
12" O.C. MIN. AND (2) @ EACH
STRAP
OPTIONAL 1" x 2" OR 2" x 2"
FOR SCREEN

SELF-MATING
BEAM
(SIZE VARIES)

SUPER OR
EXTRUDED
GUTTER

ANGLE, INTERIOR OR
EXTERIOR RECEIVING
CHANNEL (SEE SECTION 9)

2" x " x 0.050" STRAP
@ EACH BEAM CONNECTION
AND @ 1/2 BEAM SPACING W/
(2) #8 x 1/2" S.M.S. PER STRAP
MAX. DISTANCE FROM FASCIA
TO HOST STRUCTURE WALL
(SEE TABLE 1.11)

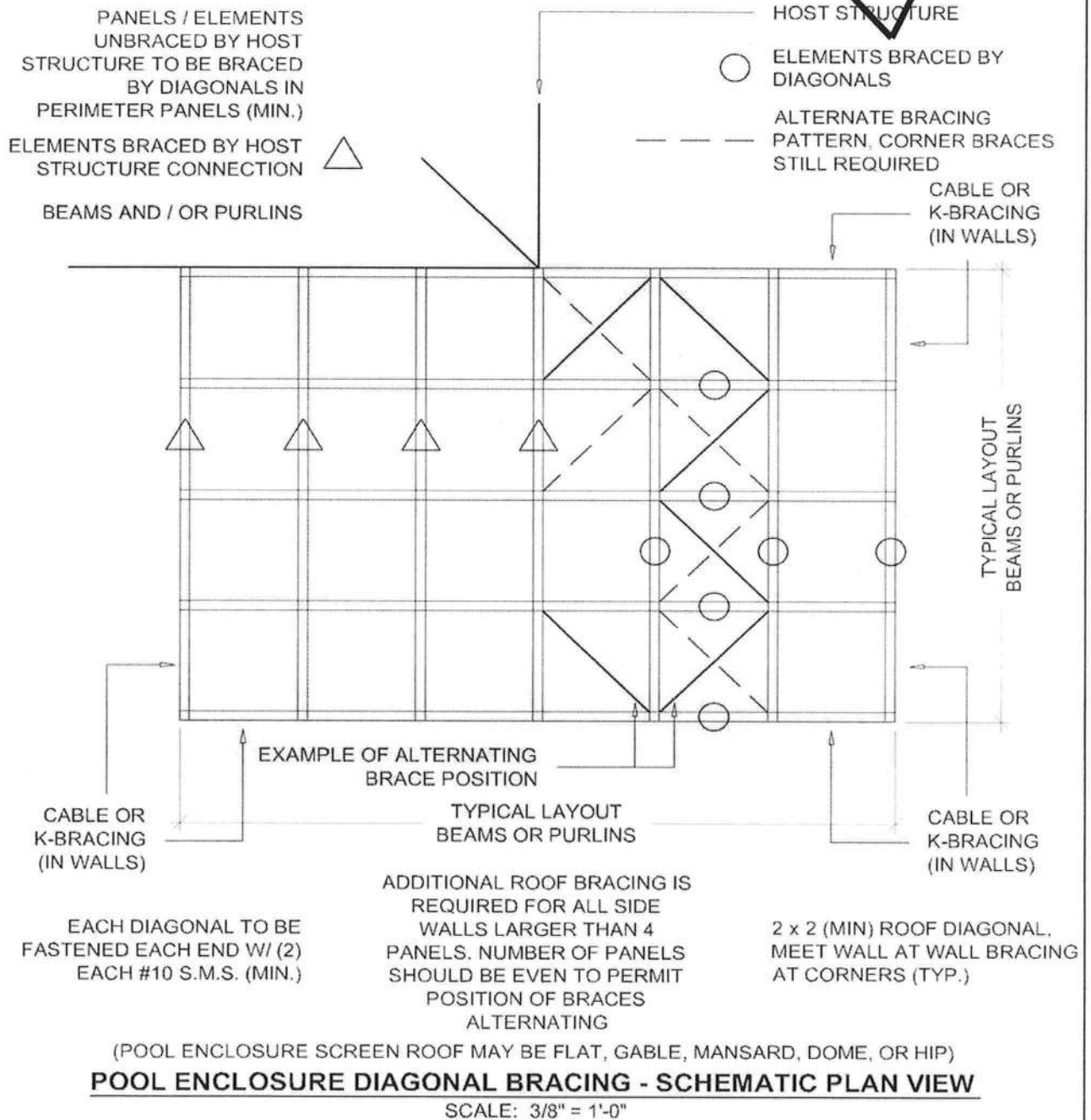
ALTERNATE SELF MATING BEAM CONNECTION TO SUPER OR EXTRUDED GUTTER

SCALE: 3" = 1'-0"

Lawrence E. Bennett, P.E. FL # 16644
CIVIL & STRUCTURAL ENGINEERING
P.O. Box 214368, South Daytona, FL 32121
Telephone #: (386) 767-4774 Fax #: (386) 767-6556
Email: labpe@bellsouth.net

SECTION 1

SCREENED ENCLOSURES



Lawrence E. Bennett, P.E. FL # 16644

CIVIL & STRUCTURAL ENGINEERING

P.O. Box 214368, South Daytona, FL 32121

Telephone #: (386) 767-4774 Fax #: (386) 767-6556

Email: lebpe@bellsouth.net

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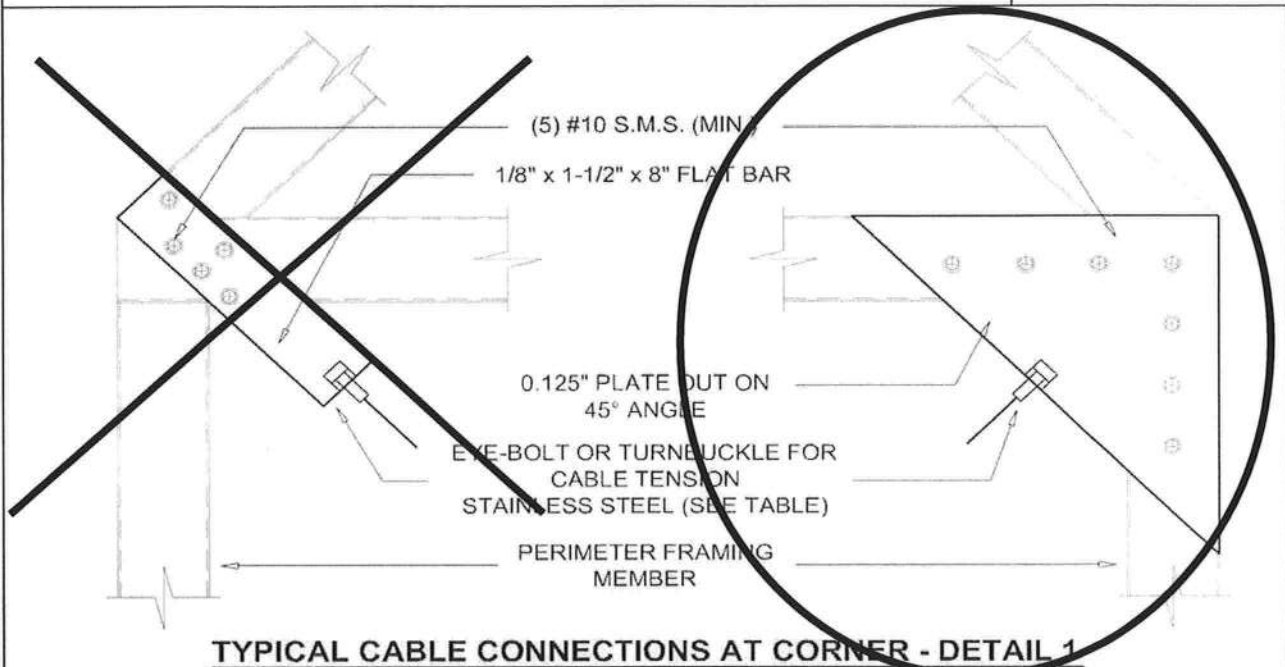
1-48

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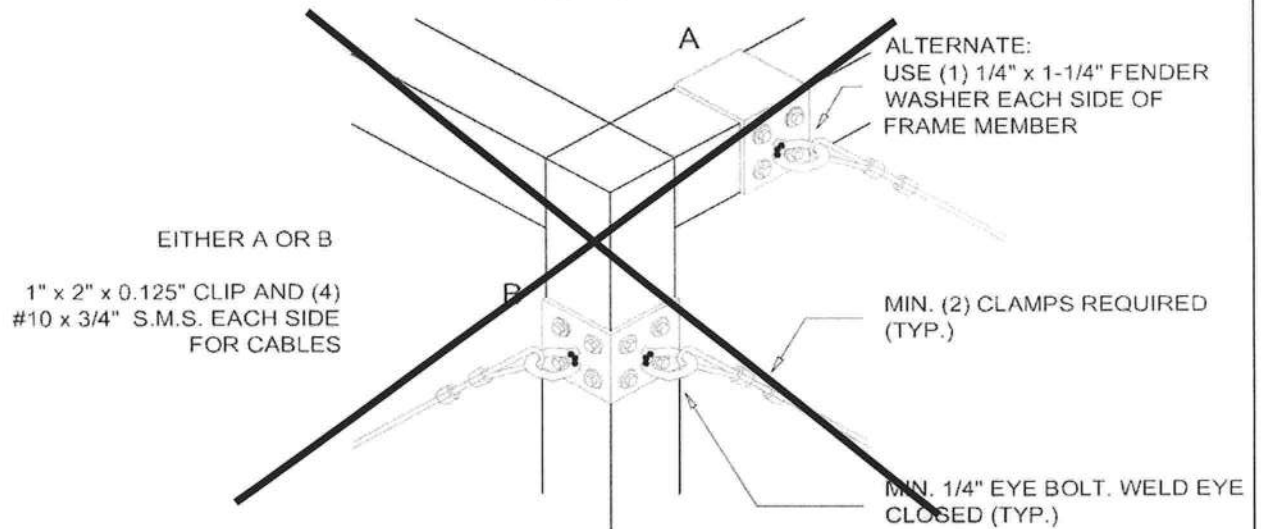
SCREENED ENCLOSURES

SECTION 1



TYPICAL CABLE CONNECTIONS AT CORNER - DETAIL 1

SCALE: 3" = 1'-0"



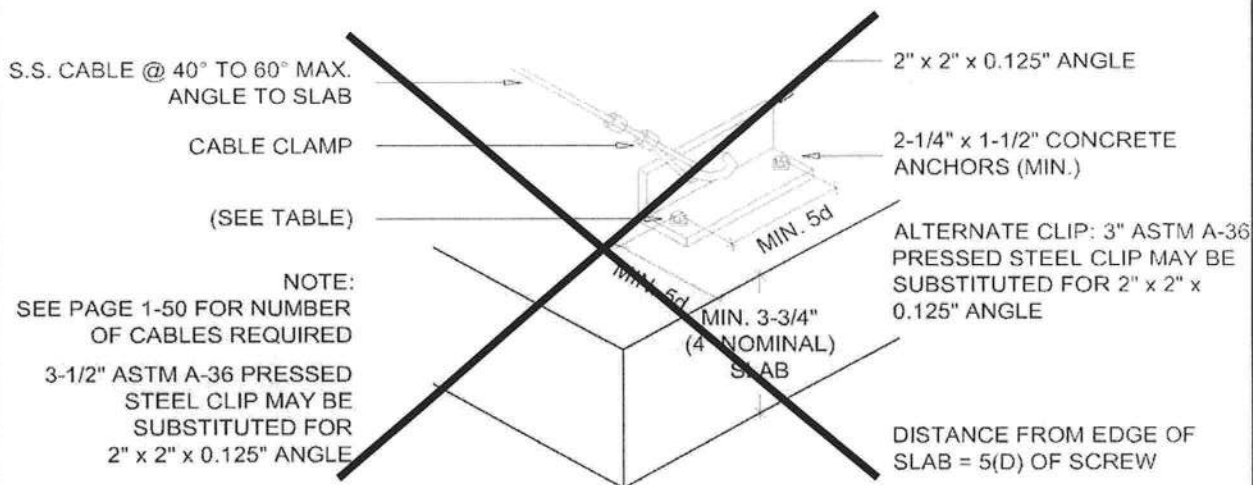
ALTERNATE TOP CORNER OF CABLE CONNECTION - DETAIL 1A

SCALE: 3" = 1'-0"

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 CIVIL & STRUCTURAL ENGINEERING
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 Telephone #: (386) 767-4774 Fax #: (386) 767-6556
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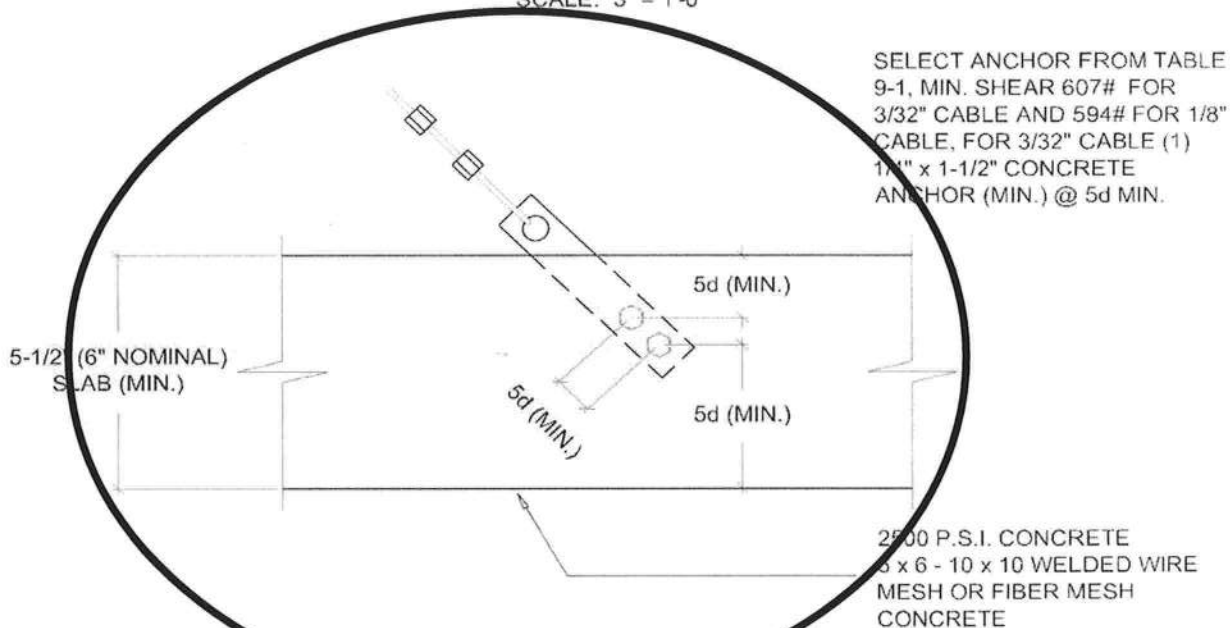
SCREENED ENCLOSURES

SECTION 1



ALTERNATE CABLE CONNECTION AT SLAB DETAIL - DETAIL 2B

SCALE: 3" = 1'-0"



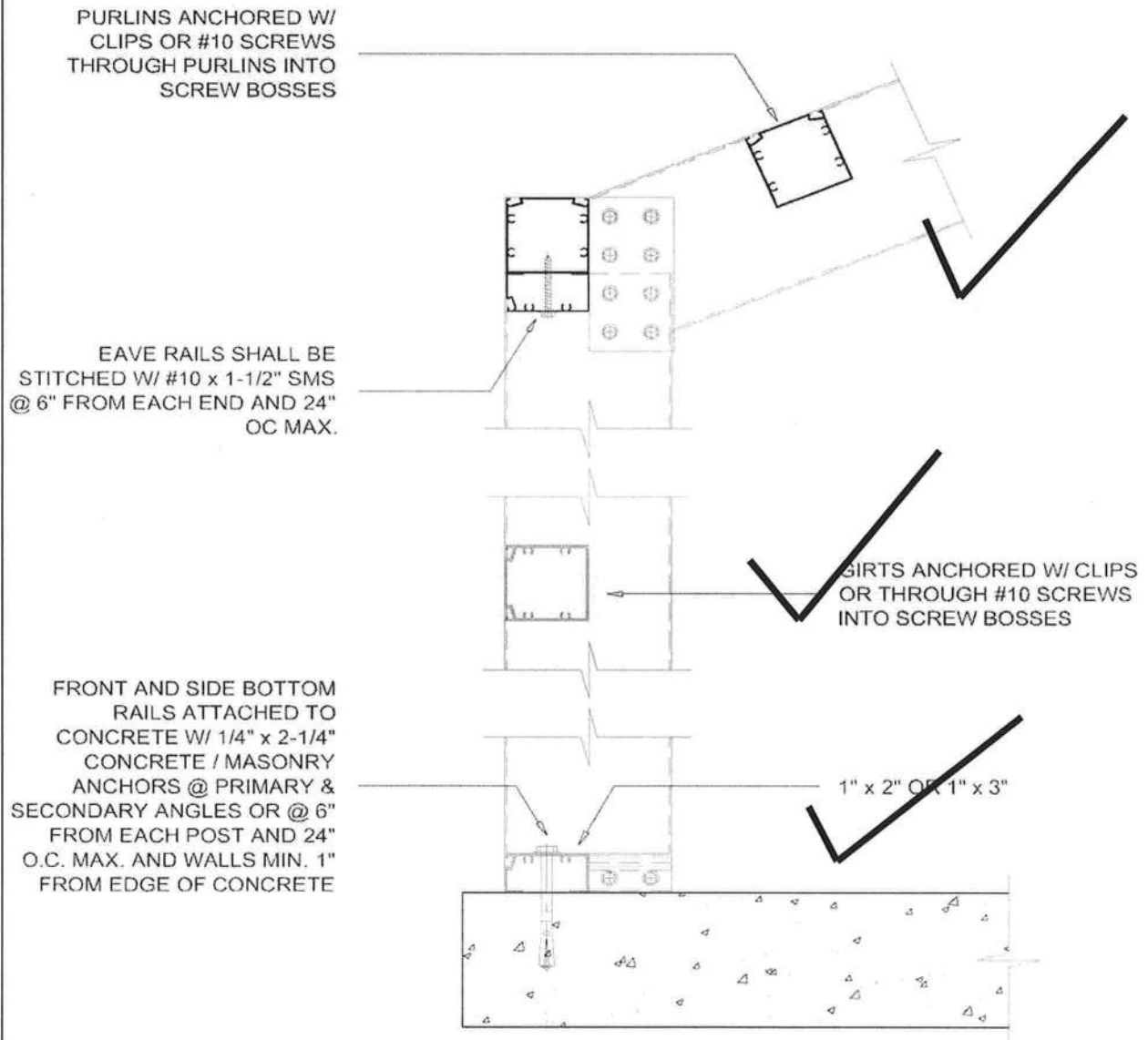
ALTERNATE CABLE CONNECTIONS AT FOUNDATION - DETAIL 2C

SCALE: 3" = 1'-0"

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CIVIL & STRUCTURAL ENGINEERING
P.O. Box 214368, South Daytona, FL 32121
Telephone #: (386) 767-4774 Fax #: (386) 767-6556
Email: lebpe@bellsouth.net

SECTION 1

SCREENED ENCLOSURES



PURLIN & CHAIR RAIL DETAIL

SCALE: 3" = 1'-0"

Lawrence E. Bennett, P.E. FL # 16644

CIVIL & STRUCTURAL ENGINEERING

P.O. Box 214368, South Daytona, FL 32121

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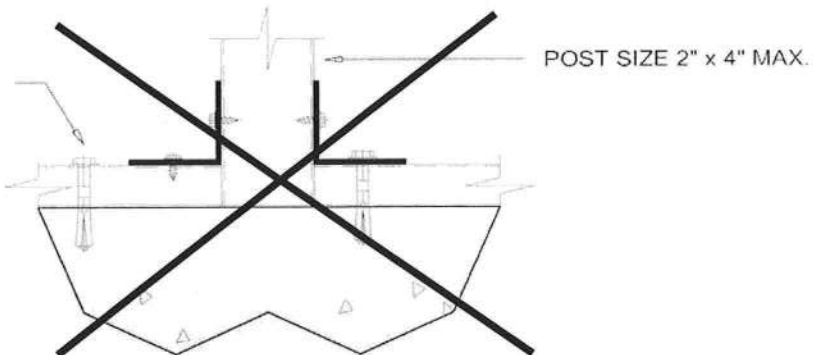
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SCREENED ENCLOSURES

SECTION 1

1" x 2" EXTRUSION ANCHOR
TO CONCRETE W/ CONCRETE
ANCHORS OR THRU PRIMARY
ANGLE 6" MAX. EACH SIDE OF
EACH POST AND @
24" O.C. MAX.
SELECT CONCRETE ANCHORS
FROM SECTION 9

MIN. 3-1/2" SLAB 2500 P.S.I.
CONCRETE 6 x 6 - 10 x 10
WELDED WIRE MESH OR
FIBER MESH CONCRETE

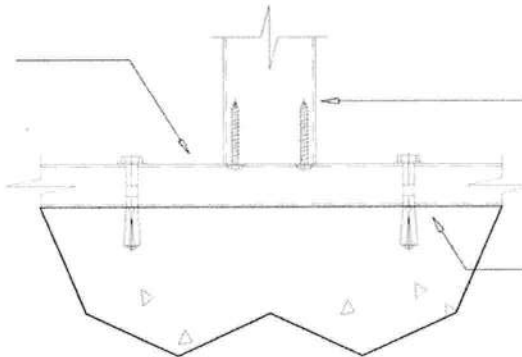


SIDE WALL POST TO PLATE TO CONCRETE DETAIL

SCALE: 3" = 1'-0"

1" x 2" EXTRUSION ANCHOR
TO CONC. W/ CONC. ANCH. 6"
MAX. EA. SIDE OF EA. POST
AND @ 24" O.C. MAX. SELECT
CONCRETE ANCHORS FROM
SECTION 9

MIN. 3-1/2" SLAB 2500 P.S.I.
CONC. 6 x 6 - 10 x 10 W.W.M.
OR FIBER MESH CONC.



2" x 2", 2" x 3" OR 2" x 4"
HOLLOW SECTION
(SEE TABLES)

MIN. (3) #10 x 1-1/2" S.M.S. INTO
SCREW BOSSES

MASONRY ANCHOR @ 6" EA.
SIDE OF POST AND @ 24" O.C.
MAX. SELECT CONCRETE
ANCHORS FROM SECTION 9

SIDE WALL HOLLOW POST TO BASE DETAIL

SCALE: 3" = 1'-0"

POOL ENCLOSURE UPRIGHT TO DECK ANCHOR REQUIREMENTS

General Notes and Specifications:

1. The uplift load on a pool enclosure upright is calculated as 1/2 the beam span x the beam spacing x the screen load of 7# / Sq. Ft.

EXAMPLE:

FOR A 2" x 6" BEAM WITH A SPAN OF 23' AND A BEAM & UPRIGHT SPACING
OF 7' USE: $1/2 \times 17'-11" \times 7' \times 10\# / \text{Sq. Ft.} = 627.2\#$ UPLIFT

2. Table 1.6 of this manual uses the worst case loads for all cases.

3. In all cases there must be a primary anchor within 6" of each side of the upright.

4. For attachment to wood deck (min. 2" nominal thickness) use wood anchors with details shown above (min. 1-3/8" embedment).

Lawrence E. Bennett, P.E. FL # 16644

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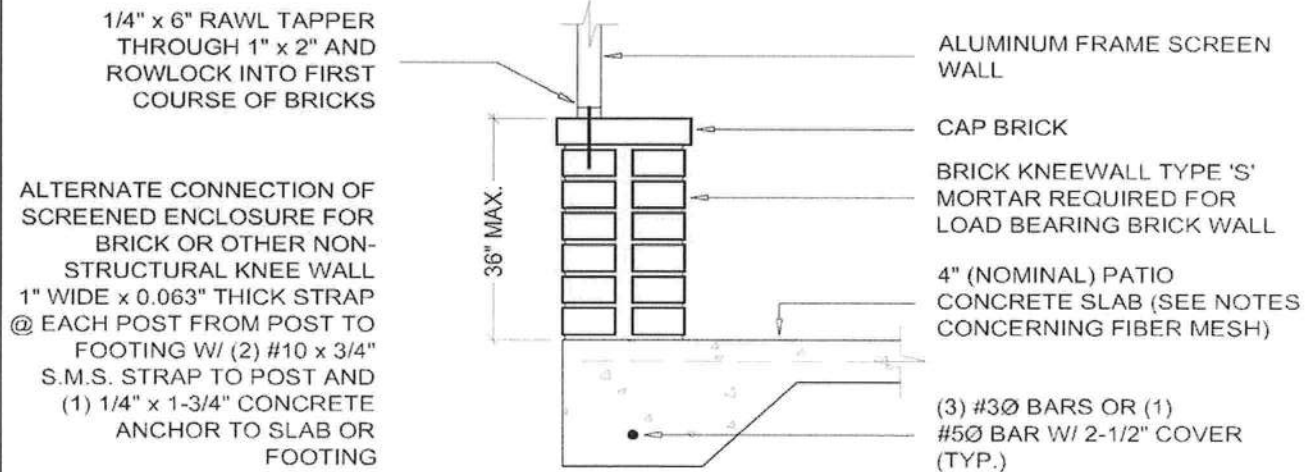
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Telephone #: (386) 767-4774 Fax #: (386) 767-6556

Email: lebpe@bellsouth.net

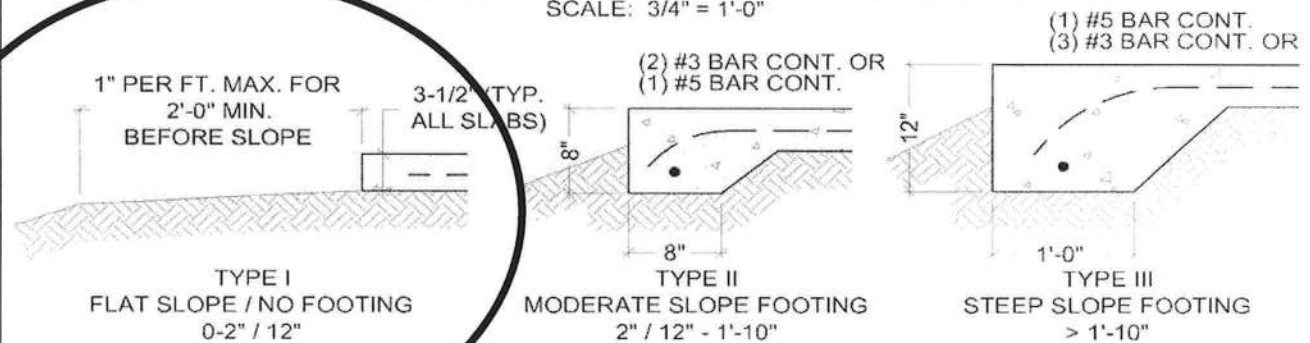
SCREENED ENCLOSURES

SECTION 1



BRICK KNEEWALL AND FOUNDATION FOR SCREEN WALLS

SCALE: 3/4" = 1'-0"



Notes for all foundation types:

1. The foundations shown are based on a minimum soil bearing pressure of 1,500 PSF. Bearing capacity of soil shall be verified prior to placing slab by field soil test (soil penetrometer) or a soil testing lab.
2. The slab / foundation shall be cleared of debris, roots and compacted prior to placement of concrete.
3. No footing is required except when addressing erosion until the slab width in the direction of the primary beams exceeds the span per table on page 1-69, then a type II slab is required under the load bearing wall only unless the side wall exceeds 16' in height or the enclosure is in a "C" exposure category in which case a type II footing is required.
4. Monolithic slabs and footings shall be minimum 2,500 psi concrete with 6 x 6 - 10 x 10 welded wire mesh or crack control fiber mesh; Fibermesh® Mesh, InForce™ e3™ (Formerly Fibermesh MD) per manufacturer's specification may be used in lieu of wire mesh. All slabs / footings shall be allowed to cure for 7 days before installing anchors.
5. If local codes require a minimum footing use Type II footing or footing section required by local code. Local codes govern.

SLAB-FOOTING DETAILS

SCALE: 3/4" = 1'-0"

Lawrence E. Bennett, P.E. FL # 16644
CIVIL & STRUCTURAL ENGINEERING
P.O. Box 214368, South Daytona, FL 32121
Telephone #: (386) 767-4774 Fax #: (386) 767-6556
Email: lebpe@bellsouth.net

SECTION 1

SCREENED ENCLOSURES

Table 1.1 120 Allowable Spans for Primary Screen Roof Frame Members
Aluminum Alloy 6063 T-6
 For Wind Zones up to 120 M.P.H., Exposure "B" and Latitudes Below 30°-30'-00" North (Jacksonville, FL)
 Uniform Load = 4 #/SF, a Point Load of 300 #/SF over (1) linear ft. is also considered

For Wind Zones up to 120 M.P.H., Exposure Category B, and a Design Temperature of 20°F or below, the following table provides the allowable span lengths and point loads for various hollow section profiles. The design is based on a tributary load width 'W' equal to the beam spacing. The uniform load is 4 #/SF, and the point load is 300 #/SF over (1) linear ft. It is also considered that the design is based on a tributary load width 'W' equal to the beam spacing.

Hollow Sections	Tributary Load Width 'W' = Beam Spacing													
	3'-0"		4'-0"		5'-0"		6'-0"		7'-0"		8'-0"		9'-0"	
	Allowable Span 'L' / Point Load (P) or Uniform Load (U), bending (b), deflection (d)													
2" x 2" x 0.044"	4'-5"	Pb	4'-5"	Pb	4'-5"	Pb	4'-5"	Pb	4'-5"	Pb	4'-5"	Pb	4'-5"	Pb
2" x 2" x 0.050"	5'-2"	Pb	5'-2"	Pb	5'-2"	Pb	5'-2"	Pb	5'-2"	Pb	5'-2"	Pb	5'-2"	Pb
2" x 2" x 0.090"	7'-6"	Pb	7'-6"	Pb	7'-6"	Pb	7'-6"	Pb	7'-6"	Pb	7'-6"	Pb	7'-6"	Pb
2" x 3" x 0.045"	7'-7"	Pb	7'-7"	Pb	7'-7"	Pb	7'-7"	Pb	7'-7"	Pb	7'-7"	Pb	7'-7"	Pb
2" x 4" x 0.050"	9'-1"	Pb	9'-1"	Pb	9'-1"	Pb	9'-1"	Pb	9'-1"	Pb	9'-1"	Pb	9'-1"	Pb
2" x 5" x 0.062"	20'-5"	Pb	20'-5"	Pb	20'-5"	Pb	20'-4"	Ud	19'-4"	Ud	18'-6"	Ud	17'-9"	Ud

Self Mating Sections	Tributary Load Width 'W' = Beam Spacing													
	3'-0"		4'-0"		5'-0"		6'-0"		7'-0"		8'-0"		9'-0"	
	Allowable Span 'L' / Point Load (P) or Uniform Load (U), bending (b), deflection (d)													
2" x 4" x 0.044 x 0.100"	11'-8"	Pd	11'-8"	Pd	11'-8"	Pd	11'-8"	Pd	11'-8"	Pd	11'-8"	Pd	11'-8"	Pd
2" x 5" x 0.050 x 0.100"	16'-1"	Pd	16'-1"	Pd	16'-1"	Pd	16'-1"	Pd	16'-1"	Pd	15'-9"	Ud	15'-1"	Ud
2" x 6" x 0.050 x 0.120"	20'-4"	Pd	20'-4"	Pd	20'-4"	Pd	20'-3"	Ud	19'-3"	Ud	18'-5"	Ud	17'-8"	Ud
2" x 7" x 0.055 x 0.120"	24'-9"	Pd	24'-9"	Pd	24'-6"	Ud	23'-1"	Ud	21'-11"	Ud	20'-11"	Ud	20'-2"	Ud
2" x 8" x 0.072 x 0.224"	34'-2"	Pd	32'-9"	Ud	30'-5"	Ud	28'-7"	Ud	27'-2"	Ud	25'-11"	Ud	24'-11"	Ud
2" x 9" x 0.072 x 0.224"	39'-3"	Pd	35'-11"	Ud	33'-4"	Ud	31'-5"	Ud	29'-10"	Ud	28'-6"	Ud	27'-5"	Ud
2" x 9" x 0.082 x 0.310"	42'-5"	Ud	38'-7"	Ud	35'-10"	Ud	33'-8"	Ud	31'-11"	Ud	30'-7"	Ud	29'-5"	Ud
2" x 10" x 0.092 x 0.369"	49'-3"	Ud	44'-9"	Ud	41'-7"	Ud	39'-1"	Ud	37'-2"	Ud	35'-6"	Ud	34'-2"	Ud

Snap Sections	Tributary Load Width 'W' = Beam Spacing													
	3'-0"		4'-0"		5'-0"		6'-0"		7'-0"		8'-0"		9'-0"	
	Allowable Span 'L' / Point Load (P) or Uniform Load (U), bending (b), deflection (d)													
2" x 2" x 0.044"	4'-10"	Pd	4'-10"	Pd	4'-10"	Pd	4'-10"	Pd	4'-10"	Pd	4'-10"	Pd	4'-10"	Pd
2" x 3" x 0.045"	7'-6"	Pd	7'-6"	Pd	7'-6"	Pd	7'-6"	Pd	7'-6"	Pd	7'-6"	Pd	7'-6"	Pd
2" x 4" x 0.045"	10'-8"	Pd	10'-8"	Pd	10'-8"	Pd	10'-8"	Pd	10'-8"	Pd	10'-8"	Pd	10'-8"	Pd
2" x 6" x 0.062"	22'-2"	Pd	22'-2"	Pd	22'-2"	Pd	21'-5"	Ud	20'-5"	Ud	19'-6"	Ud	18'-9"	Ud
2" x 7" x 0.062"	26'-8"	Pd	26'-8"	Pd	25'-9"	Ud	24'-3"	Ud	23'-0"	Ud	22'-0"	Ud	21'-2"	Ud

Note:

1. Thicknesses shown are "nominal" industry standard tolerances. No wall thickness shall be less than 0.040".
2. The structures designed using this section shall be limited to a maximum combined span and upright height of 50' and a maximum upright height of 16'. Structures larger than these limits shall have site specific engineering.
3. Span is measured from center of beam and upright connection to fascia or wall connection.
4. Above spans do not include length of knee brace. Add horizontal distance from upright to center of brace to beam connection to the above spans for total beam spans.
5. Tables are based on a maximum wall height of 16' including a 4' max. mansard or gable. Other conditions may offer better spans w/ enclosure site specific engineering.
6. Spans may be interpolated.
7. To convert spans to "C" and "D" exposure categories see exposure multipliers and example on page 1-ii.

Example: Max. 'L' for 2" x 4" x 0.050" hollow section with 'W' = 5'-0" = 9'-1"

Lawrence E. Bennett, P.E. FL # 16644
 CIVIL & STRUCTURAL ENGINEERING

P.O. Box 214368, South Daytona, FL 32121
 Telephone #: (386) 767-4774 Fax #: (386) 767-6556
 Email: lebpe@bellsouth.net

SCREENED ENCLOSURES

SECTION 1

Table 1.2 120 Allowable Spans for Secondary Screen Roof Frame Members

Aluminum Alloy 6063 T-6

For Wind Zones up to 120 M.P.H., Exposure "B", and Latitudes Below 30°-30'-00" North (Jacksonville, FL)

Uniform Load = 4 #/SF, a Point Load of 300 #/SF over (1) linear ft. is also considered

A. Sections Fastened To Beams With Clips

Hollow Sections	Tributary Load Width "W" = Purlin Spacing													
	3'-6"		4'-0"		4'-6"		5'-0"		5'-6"		6'-0"		6'-8"	
	Allowable Span 'L' / Point Load (P) or Uniform Load (U), bending (b), deflection (d)													
2" x 2" x 0.044"	4'-5"	Pb	4'-5"	Pb	4'-5"	Pb	4'-5"	Pb	4'-5"	Pb	4'-5"	Pb	4'-5"	Pb
2" x 2" x 0.050"	5'-2"	Pb	5'-2"	Pb	5'-2"	Pb	5'-2"	Pb	5'-2"	Pb	5'-2"	Pb	5'-2"	Pb
2" x 2" x 0.090"	7'-4"	Pd	7'-4"	Pd	7'-4"	Pd	7'-4"	Pd	7'-4"	Pd	7'-4"	Pd	7'-4"	Pd
3" x 2" x 0.045"	5'-8"	Pb	5'-8"	Pb	5'-8"	Pb	5'-8"	Pb	5'-8"	Pb	5'-8"	Pb	5'-8"	Pb
3" x 2" x 0.070"	7'-8"	Pd	7'-8"	Pd	7'-8"	Pd	7'-8"	Pd	7'-8"	Pd	7'-8"	Pd	7'-8"	Pd
2" x 3" x 0.045"	7'-4"	Pd	7'-4"	Pd	7'-4"	Pd	7'-4"	Pd	7'-4"	Pd	7'-4"	Pd	7'-4"	Pd
2" x 4" x 0.050"	9'-1"	Pb	9'-1"	Pb	9'-1"	Pb	9'-1"	Pb	9'-1"	Pb	9'-1"	Pb	9'-1"	Pb
2" x 5" x 0.062"	14'-1"	Pd	14'-1"	Pd	14'-1"	Pd	14'-1"	Pd	14'-1"	Pd	14'-1"	Pd	14'-1"	Pd

Snap Sections	Tributary Load Width "W" = Purlin Spacing														
	3'-6"		4'-0"		4'-6"		5'-0"		5'-6"		6'-0"		6'-8"		
	Allowable Span 'L' / Point Load (P) or Uniform Load (U), bending (b), deflection (d)														
2" x 2" x 0.044	4'-11"	Pb	4'-11"	Pb	4'-11"	Pb	4'-11"	Pb	4'-11"	Pb	4'-11"	Pb	4'-11"	Pb	4'-11"
2" x 3" x 0.045"	7'-3"	Pd	7'-3"	Pd	7'-3"	Pd	7'-3"	Pd	7'-3"	Pd	7'-3"	Pd	7'-3"	Pd	7'-3"
2" x 4" x 0.045"	9'-2"	Pd	9'-2"	Pd	9'-2"	Pd	9'-2"	Pd	9'-2"	Pd	9'-2"	Pd	9'-2"	Pd	9'-2"

B. Sections Fastened Through Beam Webs Into Screw Bosses

Hollow Sections	Tributary Load Width "W" = Purlin Spacing													
	3'-6"		4'-0"		4'-6"		5'-0"		5'-6"		6'-0"		6'-8"	
	Allowable Span 'L' / Point Load (P) or Uniform Load (U), bending (b), deflection (d)													
	2" x 3" x 0.050"	11'-5"	Pb	11'-5"	Pb	11'-5"	Pb	11'-4"	Ud	10'-11"	Ud	10'-8"	Ud	10'-3"
2" x 4" x 0.050"	13'-8"	Pb	13'-8"	Pb	13'-8"	Pb	13'-8"	Pb	13'-8"	Pb	13'-8"	Pb	13'-8"	Pb
2" x 5" x 0.062"	22'-4"	Pd	22'-4"	Pd	22'-4"	Pd	21'-7"	Ud	20'-11"	Ud	20'-4"	Ud	19'-7"	Ud

Snap Sections	Tributary Load Width "W" = Purlin Spacing													
	3'-6"		4'-0"		4'-6"		5'-0"		5'-6"		6'-0"		6'-8"	
	Allowable Span 'L' / Point Load (P) or Uniform Load (U), bending (b), deflection (d)													
	2" x 2" x 0.044"	4'-4"	Pb	4'-4"	Pb	4'-4"	Pb	4'-4"	Pb	4'-4"	Pb	4'-4"	Pb	4'-4"

Notes:

1. Thicknesses shown are "nominal" industry standard tolerances. No wall thickness shall be less than 0.040".
 2. Span is measured from center of beam and upright connection to fascia or wall connection.
 3. Tables are based on a maximum wall height of 16' including a 4' max. mansard or gable. Other conditions may offer better spans w/ enclosure site specific engineering.
 4. Spans may be interpolated.
 5. 2" x 4" & 2" x 5" Hollow Girts shall be connected w/ an internal or external 1-1/2" x 1-1/2" x 0.044" angle.
 6. To convert spans to "C" and "D" exposure categories see exposure multipliers and example on page 1-ii.
- CHECK TABLE 1.6 FOR MINIMUM UPRIGHT SIZE FOR BEAMS.
- Example: Max. "L" for 2" x 4" x 0.050" hollow section fastened to beam with clips with "W" = 5'-0" = 9'-1"

Lawrence E. Bennett, P.E. FL # 16644

CIVIL & STRUCTURAL ENGINEERING

P.O. Box 214368, South Daytona, FL 32121

Telephone #: (386) 767-4774 Fax #: (386) 767-6556

Email: lebpe@bellsouth.net

SECTION 1

SCREENED ENCLOSURES

Table 1.3 110 Allowable Post / Upright Heights for Primary Screen Wall Frame Members
Aluminum Alloy 6063 T-6

For 3 second wind gust at a velocity of 110 MPH, Exposure "B" or an applied load of 13 #/sq. ft.

Hollow Sections	Tributary Load Width 'W' = Upright Spacing													
	3'-0"		4'-0"		5'-0"		6'-0"		7'-0"		8'-0"		9'-0"	
	Allowable Height "H" / bending (b), deflection (d)													
2" x 2" x 0.044"	7'-5"	d	6'-5"	b	5'-8"	b	5'-1"	b	4'-8"	b	4'-3"	b	3'-11"	b
2" x 2" x 0.050"	7'-10"	d	7'-1"	b	6'-3"	b	5'-8"	b	5'-2"	b	4'-9"	b	4'-5"	b
2" x 2" x 0.090"	8'-11"	d	8'-2"	d	7'-10"	d	7'-1"	b	6'-7"	b	6'-1"	b	5'-9"	b
2" x 3" x 0.045"	8'-4"	d	7'-7"	d	7'-9"	d	6'-11"	d	6'-5"	d	5'-11"	b	5'-6"	b
2" x 4" x 0.050"	11'-2"	b	9'-7"	b	8'-6"	b	7'-9"	b	7'-1"	b	6'-7"	b	6'-1"	b
2" x 5" x 0.062"	17'-3"	b	14'-10"	b	13'-2"	b	11'-11"	b	11'-0"	b	10'-3"	b	9'-7"	b

Self Mating Sections	Tributary Load Width 'W' = Upright Spacing													
	3'-0"		4'-0"		5'-0"		6'-0"		7'-0"		8'-0"		9'-0"	
	Allowable Height "H" / bending (b), deflection (d)													
2" x 4" x 0.044 x 0.100"	11'-11"	d	10'-10"	d	10'-0"	d	9'-5"	b	8'-8"	b	8'-0"	b	7'-6"	b
2" x 5" x 0.050" x 0.100"	14'-9"	d	13'-5"	d	12'-5"	d	11'-7"	b	10'-8"	b	9'-11"	b	9'-4"	b
2" x 6" x 0.050" x 0.120"	17'-3"	d	15'-8"	d	14'-4"	b	13'-1"	b	12'-0"	b	11'-3"	b	10'-6"	b
2" x 7" x 0.055" x 0.120"	19'-8"	d	17'-6"	b	15'-7"	b	14'-2"	b	13'-1"	b	12'-2"	b	11'-5"	b
2" x 8" x 0.072" x 0.224"	24'-4"	d	22'-1"	d	20'-6"	d	19'-4"	d	18'-4"	d	17'-6"	d	16'-10"	d
2" x 9" x 0.072" x 0.224"	26'-8"	d	24'-3"	d	22'-6"	d	21'-2"	d	20'-1"	d	19'-3"	d	18'-2"	b
2" x 9" x 0.082" x 0.310"	28'-8"	d	26'-0"	d	24'-2"	d	22'-9"	d	21'-7"	d	20'-8"	d	19'-10"	d
2" x 10" x 0.092" x 0.369"	33'-3"	d	30'-3"	d	28'-1"	d	26'-5"	d	25'-1"	d	23'-11"	d	23'-1"	d

Snap Sections	Tributary Load Width 'W' = Upright Spacing													
	3'-0"		4'-0"		5'-0"		6'-0"		7'-0"		8'-0"		9'-0"	
	Allowable Height "H" / bending (b), deflection (d)													
2" x 2" x 0.044"	6'-7"	d	5'-11"	d	5'-7"	d	5'-3"	d	4'-10"	b	4'-5"	b	4'-1"	b
2" x 3" x 0.045"	8'-10"	d	8'-1"	d	7'-6"	d	6'-11"	b	6'-3"	b	5'-9"	b	5'-3"	b
2" x 4" x 0.045"	11'-2"	d	10'-2"	d	9'-2"	b	8'-2"	b	7'-5"	b	6'-9"	b	6'-2"	b
2" x 6" x 0.062"	18'-3"	d	16'-7"	d	15'-5"	d	14'-6"	d	13'-9"	d	13'-2"	d	12'-8"	d
2" x 7" x 0.062"	20'-7"	d	18'-9"	d	17'-5"	d	16'-4"	d	15'-7"	d	14'-10"	d	14'-2"	d

Notes:

1. Thicknesses shown are "nominal" industry standard tolerances. No wall thickness shall be less than 0.040".
2. Using screen panel width 'W' select upright length 'H'.
3. Above heights do not include length of knee brace. Add vertical distance from upright to center of brace to beam connection to the above spans for total beam spans.
4. Site specific engineering required for pool enclosures over 30' in mean roof height.
5. Height is to be measured from center of beam and upright connection to fascia or wall connection.
6. Chair rails of 2" x 2" x 0.044" min. and set @ 36" in height are designed to be residential guardrails provided they are attached with min. (3) #10 x 1-1/2" S.M.S. into the screw bosses and do not exceed 8'-0" in span.
7. Max. beam size for 2" x 5" is 2" x 7" x 0.055" x 0.120"
8. Spans may be interpolated.
9. To convert spans to "C" and "D" exposure categories see exposure multipliers and example on page 1-ii.

Lawrence E. Bennett, P.E. FL # 16644

CIVIL & STRUCTURAL ENGINEERING

P.O. Box 214368, South Daytona, FL 32121

Telephone #: (386) 767-4774 Fax #: (386) 767-6556

Email: lebpe@bellsouth.net

SECTION 1

SCREENED ENCLOSURES

Table 1.4 110 Allowable Post / Girt / Chair Rail Spans, Header Spans & Upright Heights for Secondary Screen Wall Frame Members
Aluminum Alloy 6063 T-6

For 3 second wind gust at a velocity of 110 MPH, Exposure "B" or an applied load of 13 # / sq. ft.

A. Sections As Horizontals Fastened To Posts With Clips

Hollow Sections	Tributary Load Width "W" = Upright Spacing						
	3'-0"	4'-0"	5'-0"	6'-0"	7'-0"	8'-0"	9'-0"
	Allowable Height "H" or Span "L" / bending (b), deflection (d)						
2" x 2" x 0.044"	7'-5" d	6'-5" b	5'-8" b	5'-1" b	4'-8" b	4'-3" b	3'-11" b
2" x 2" x 0.050"	7'-10" d	7'-1" b	6'-3" b	5'-8" b	5'-2" b	4'-9" b	4'-5" b
2" x 2" x 0.090"	8'-11" d	8'-2" d	7'-10" d	7'-1" b	6'-7" b	6'-1" b	5'-9" b
3" x 2" x 0.045"	8'-4" d	7'-4" b	6'-6" b	5'-10" b	5'-4" b	4'-11" b	4'-7" b
3" x 2" x 0.070"	9'-5" d	8'-6" d	7'-9" b	7'-0" b	6'-5" b	5'-11" b	5'-7" b
2" x 3" x 0.045"	8'-4" d	7'-7" d	7'-9" d	6'-11" d	6'-5" d	5'-11" b	5'-6" b
2" x 4" x 0.050"	11'-2" b	9'-7" b	8'-6" b	7'-9" b	7'-1" b	6'-7" b	6'-1" b
2" x 5" x 0.062"	17'-3" b	14'-10" b	13'-2" b	11'-11" b	11'-0" b	10'-3" b	9'-7" b

Snap Sections	Tributary Load Width "W" = Upright Spacing						
	3'-0"	4'-0"	5'-0"	6'-0"	7'-0"	8'-0"	9'-0"
	Allowable Height "H" or Span "L" / bending (b), deflection (d)						
2" x 2" x 0.044"	6'-7" d	5'-11" d	5'-7" d	5'-3" d	4'-10" b	4'-5" b	4'-1" b

B. Sections As Horizontals Fastened To Posts Through Side Into Screw Bosses

Hollow Sections	Tributary Load Width "W" = Upright Spacing						
	3'-0"	4'-0"	5'-0"	6'-0"	7'-0"	8'-0"	9'-0"
	Allowable Height "H" or Span "L" / bending (b), deflection (d)						
2" x 2" x 0.044"	8'-4" b	7'-2" b	6'-4" b	5'-8" b	5'-2" b	4'-9" b	4'-5" b
3" x 2" x 0.045"	9'-7" b	8'-3" b	7'-3" b	6'-6" b	5'-11" b	5'-6" b	5'-1" b
3" x 2" x 0.070"	11'-5" b	9'-10" b	8'-8" b	7'-10" b	7'-2" b	6'-8" b	6'-3" b
2" x 3" x 0.045"	11'-2" d	9'-9" b	8'-8" b	7'-10" b	7'-2" b	6'-8" b	6'-2" b
2" x 4" x 0.050"	12'-6" b	10'-9" b	9'-6" b	8'-7" b	7'-11" b	7'-4" b	6'-10" b
2" x 5" x 0.062"	19'-3" b	16'-7" b	14'-9" b	13'-5" b	12'-4" b	11'-6" b	10'-9" b

Snap Sections	Tributary Load Width "W" = Upright Spacing						
	3'-0"	4'-0"	5'-0"	6'-0"	7'-0"	8'-0"	9'-0"
	Allowable Height "H" or Span "L" / bending (b), deflection (d)						
2" x 2" x 0.044"	8'-10" d	7'-8" b	6'-9" b	6'-0" b	5'-5" b	4'-11" b	4'-7" b

Note:

1. Thicknesses shown are "nominal" industry standard tolerances. No wall thickness shall be less than 0.040".
2. Using screen panel width "W" select girt lengths.
3. Site specific engineering required for pool enclosures over 30' in mean roof height.
4. Span/height is to be measured from center of beam and upright connection to fascia or wall connection.
5. Chair rails of 2" x 2" x 0.044" min. and set @ 36" in height are designed to be residential guardrails provided they are attached with min. (3) #10 x 1-1/2" s.m.s. into the screw bosses and do not exceed 8'-0" o.c.
6. Girt spacing shall not exceed 6'-8".
7. Max. beam size for 2" x 5" is 2" x 7" x 0.055" x 0.120"
8. 2" x 4" & 2" x 5" hollow girts shall be connected w/ an internal or external 1-1/2" x 1-1/2" x 0.044" angle.
9. Spans/heights may be interpolated.
10. To convert spans to "C" and "D" exposure categories see exposure multipliers and example on page 1-ii.

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Lawrence E. Bennett, P.E. FL # 16644

CIVIL & STRUCTURAL ENGINEERING

P.O. Box 214368, South Daytona, FL 32121

Telephone #: (386) 767-4774 Fax #: (386) 767-6556

Email: lebpe@bellsouth.net

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SECTION 1

SCREENED ENCLOSURES

Table 1.6 Minimum Upright Sizes and Number of Screws for
Connection of Roof Beams To Wall Uprights or Beam Splicing

Beam/Upright or Post	Upright or Post/Beam	Minimum Purlin, Girt & Knee Brace Size	Notes	Minimum Number of Screws*			Beam Stitching Screw at 24" OC
				#8 x 1/2"	#10 x 1/2"	#12 x 1/2"	
2 x 4 SMB	2 x 3 SMB or H	2" x 2" x 0.044"	Partial Lap	8	6	4	#10
2 x 5 SMB	2 x 3 SMB or H	2" x 2" x 0.044"	Partial Lap	8	6	4	#8
2 x 6 SMB	2 x 3 SMB or H	2" x 2" x 0.044"	Partial Lap	10	8	6	#10
2 x 7 SMB	2 x 4 SMB or H	2" x 3" x 0.044"	Full Lap	14	12	10	#12
2 x 8 SMB	2 x 5 SMB or H	2" x 3" x 0.044"	Full Lap	16	14	12	#14
2 x 9 SMB	2 x 6 SMB	2" x 3" x 0.045"	Full Lap	18	16	14	#14**
2 x 9 SMB *	2 x 7 SMB	2" x 4" x 0.050"	Full Lap	20	18	16	#14**
2 x 10 SMB	2 x 8 SMB	2" x 5" x 0.050"	Full Lap	20	18	16	#14**

Screw Size	Minimum Distance and Spacing of Screws		Gusset Plate Thickness	
	Edge To Center	Center To Center	Beam Size	Thickness
#8	5/16"	5/8"	2" x 7" x 0.055" x 0.120"	0.063"
#10	3/8"	3/4"	2" x 8" x 0.072" x 0.224"	0.125"
#12	1/2"	1"	2" x 9" x 0.072" x 0.224"	0.125"
#14 or 1/4"	3/4"	1-1/2"	2" x 9" x 0.082" x 0.306"	0.190"
5/16"	7/8"	1-3/4"	2" x 10" x 0.092" x 0.369"	0.250"
3/8"	1"	2"		

* 0.082" wall thickness, 0.310" flange thickness

** (1) Stitching screw at 16" O.C. max.

Connection Example:

2" x 7" beam & 2" x 5" at beam & gusset plate, (14) #8 x 1/2" sms & upright & gusset plate
(14) #8 x 1/2" sms ea. side of beam & upright.

Note:

1. Connection of 2" x 6" to 2" x 4" shall use a full lap cut or 1/16" gusset plate.
2. For beam splice connections the number of screws shown is the total for each splice with 1/2 the screws on each side of the cut.
3. The number of screws is based on the maximum allowable moment of the beam.
4. The number of deck anchors is based on RAWL R Tapper allowable load data for 2,500 psi concrete and / or equal anchors may be used. The number shown is the total use 1/2 per side.
5. Hollow splice connections can be made provided the connection is approved by the engineer.
6. If a larger than minimum upright is used the number of screws is the same for each splice with 1/2 the screws on each side of the cut.
7. The side wall upright shall have a minimum beam size as shown above, i.e., a 2" x 4" upright shall have a 2" x 3" beam.
8. For minimum girt size read upright size as a beam and purlin size is minimum girt size. (i.e. 2" x 9" x 0.072" x 0.224" s.m.b. w/ 2" x 6" x 0.050 x 0.120" s.m.b. upright requires a 2" x 3" x 0.045" girt / chair rail.)

Lawrence E. Bennett, P.E. FL # 16644

CIVIL & STRUCTURAL ENGINEERING

P.O. Box 214368, South Daytona, FL 32121

Telephone #: (386) 767-4774 Fax #: (386) 767-6556

Email: lebpe@bellsouth.net

