DATE 07/3	0/2008			uilding Permit on Premises During Co		PERMIT 000027218
A DDL LC A NIT	DATRICK		e Frommently Posted (00002/218
APPLICANT ADDRESS	PATRICK 6800	SOUTHPOINT PKW	/V #200	PHONE JACKSONVILLE	904.296.1490	FL 32216
OWNER		DA HOMES INC. OF FI	- T	PHONE	904.296.1490	<u>1L</u> <u>32210</u>
ADDRESS	254	SW TIMBE RIDGE I	100000 00000 matrix	LAKE CITY	904.290.1490	FL 32024
CONTRACTO	-	EODORE BROCK	DRIVE	PHONE	407.227.3504	<u> </u>
LOCATION O	-	*	R 247-S TL TO C-252.	B,TR TO TIMBER RII	-	
DOCATION	I TROTER	4TH LOT C		b, in to invidenti	700,10	
TYPE DEVEL	OPMENT	SFD/UTILITY	EST	TIMATED COST OF C	ONSTRUCTION	114400.00
HEATED FLO	OOR AREA	2236.00	TOTAL ARE	A 2288.00	HEIGHT	STORIES 1
FOUNDATIO	N CONC	WALL	S FRAMED R	OOF PITCH 6'12	FLC	OOR CONC
LAND USE &	ZONING	RSF-2	27.11.11.11.11.11.11.11.11.11.11.11.11.11	MA	X. HEIGHT 35	5
Minimum Set	Back Requir	rments: STREET-F	FRONT 25.00	REAR	15.00	SIDE 10.00
NO. EX.D.U.	0	FLOOD ZONE	<u>X</u>	DEVELOPMENT PER	RMIT NO.	
PARCEL ID	10-4S-16-	02856-121	SUBDIVISIO	N TIMBERLANDS		
LOT <u>21</u>	BLOCK	PHASE _	UNIT _	тот	ALACRES 0.5	0
000001642			CBC1256382		Then h	ula-
Culvert Permit	No.	Culvert Waiver Co	ontractor's License Nun	nber	Applicant/Owner/O	Contractor
18"X32'MITE	RED	08-401	BLK	′	WR	<u>N</u>
Driveway Con	nection	Septic Tank Number	LU & Zonir	ng checked by Ap	proved for Issuance	New Resident
	ELEVAT	ION LETTER CONFIR	MATION LETTER RE	QUIRED BEFORE SL	AB. MFE @	
96.00'.					G 1 " G	1 019697
					Check # or Ca	sh 918687
		FOR BU	ILDING & ZONIN	IG DEPARTMEN	T ONLY	(footer/Slab)
Temporary Pov	wer		Foundation		Monolithic	
		date/app. by		date/app. by		date/app. by
Under slab rou	igh-in plumb	date/app		date/app. by	Sheathing/N	date/app. by
Framing		————		pove slab and below woo	od floor	date app. by
-	date/ap	pp. by	,			date/app. by
Electrical roug	gh-in		Heat & Air Duct		Peri. beam (Lintel)
Darmonant nous	/A=	date/app. by	COF	date/app. by	0.1	date/app. by
Permanent pow		nte/app. by	C.O. Final	late/app. by	Culvert	date/app. by
M/H tie downs.	, blocking, e	lectricity and plumbing			Pool	
Reconnection			date/app Pump pole	o. by Utility P	ole	date/app. by
		date/app. by	date	/app. by	date/app. by	_
M/H Pole da	ate/app. by	Trav	vel Trailer	ate/app. by	Re-roof	date/app. by
	e de Mannes est	D 575 00			OLD OIL DO	PPP 6
BUILDING PE			CERTIFICATION FE		SURCHARGE	
MISC. FEES S	0.00	ZONING	CERT. FEE \$ 50.00	FIRE FEE \$ 0.0	0 WASTE	E FEE \$
		_//				
FLOOD DEVE	LOPMENT	FEE \$ FLOO	OD ZONE FEE \$ 25.0	0 CULVERT FEE \$		AL FEE 697.88

PERMIT

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED TO BE IN ACTIVE PROGESS WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Columbia County Building Permit Application

For Office Use Only Application # 0806-53 Date Received 127 By W Permit # 2728 /1642
Zoning Official 131K Date 14,07.08 Flood Zone FEMA Map # NA Zoning RSF-2
Land Use Delevation NA MFE River NA Plans Examiner Date 7/2 08
Comments Elevation Confirmation Letter required at slab
NOC ± EH ⊕ Deed or PA
□ Dev Permit # □ In Floodway □ Letter of Authorization from Contractor
□ Unincorporated area □ Incorporated area □ Town of Fort White □ Town of Fort White Compliance letter
Septic Permit No. (904)-332-6367
Name Authorized Person Signing Permit Theodore C. Brock/Phraick Wilson Phone (904)-296-1490
Address 6800 Southpoint Pkwy. #300 Jacksonville, FL 32216
Owners Name Maronda Homes Inc. of Florida Phone (904)-296-1490
911 Address 254 SW Timber Ridge Dr, LC St 32024
Contractors Name Theodore C. Brock Phone (407)-227-3504
Address6800 Southpoint Pkwy. #300 Jacksonville, FL 32216
Fee Simple Owner Name & AddressN/A
그는 사람들은 그들이 아니라 회사를 하는 아니라 있는데 하면 하는데 아니라 아니라 아니는 아니는 아니라 아니라 하는데 아니라 가는데 없었다. 그는 것은 것
Bonding Co. Name & Address N/A
Architect/Engineer Name & Address Tomas Ponce 4005 Maronda Way Sanford, FL 32771
Mortgage Lenders Name & Address Bank of America 250 Park Ave. S. #400 Winter Park, FL 32789
Circle the correct power company — FL Power & Light — Clay Elec. — Suwannee Valley Elec. — Progress Energy
Property ID Number 10-48-10-02850-121 Estimated Cost of Construction 91,890
Subdivision Name Timber lands Lot 2 Block Unit Phase
Driving Directions Hwy 90, Left on 247 South; Right on 252B; Left on Timber Ridge, 4th for on
Left.
Number of Existing Dwellings on Property
Construction of Residential Single Family Dwelling Total Acreage50 Lot Size
Do you need a - <u>Culvert Permit</u> or <u>Culvert Waiver</u> or <u>Have an Existing Drive</u> Total Building Height
Actual Distance of Structure from Property Lines - Front 50.0 Side 35.0 Side 35.0 Rear 57.0
Number of Stories Heated Floor Area 230 Total Floor Area 2788 Roof Pitch 126
Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

Page 1 of 2 (Both Pages must be submitted together.)

CKHP. / 918698 697.88

Revised 11-30-07

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment

According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE:

YOU ARE HEREBY NOTIFIED as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

OWNERS CERTIFICATION: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning. I further understand the above written responsibilities in Columbia County for obtaining this Building Permit. Owners Signature Theodore C. Brock CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit. Contractor's License Number (BC 1250382 Contractor's Signature (Permitee) **Columbia County** Competency Card Number Theodore C. Brock Affirmed under penalty of perjury to by the Contractor and subscribed before me this W day of Wat Personally known XXX or Produced Identification Notary Public State of Florida

> Notary Public State of Florida Melissa L McKague My Commission DD493647

Page 2 of 2 (Both Pages must be submitted together.) Expires 11/22/2001

State of Florida Notary Signature (For the Contractor)

Melissa L. McKaque

Revised 11-30-07

Meilssen wicklague

Exp : //22/2009

My Lammission DD493647

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owners certification: I hereby certify that all the foregoing information is accurate and all work will be

the above written responsibilities in Columbia Cou	unty for obtaining	this Buildi	ng Permit.	
	. 1 . d. a.g. e. j	•	T.,	E 8
Owners Signature STANE HOCK		₹.	*	
	*			
CONTRACTORS AFFIDAVIT: By my signature I unde	rstand and agree	that I have	e informed and	provided this
written statement to the owner of all the above wr				
this Building Permit.	~ w		(5 .)	
10 1				
No bearing	E E 23 EX EX EX		· Adais	7 -10700
	_ Contractor's	s License Nu	mber (BC)	150387
Contractor's Signature (Permitee)	Columbia C			
. Theodore C. Brock	Competency	y Card Numb	ber	
· · · · · · · · · · · · · · · · · · ·				89.1
Affirmed under penalty of perjury to by the Contractor			24 14	Ne 2008.
Affirmed under penalty of perjury to by the Contractor a	and subscribed bet	ore me this	All day of VI	20/5
Personally known XXX or Produced Identification			*	3
N. 1	v : 20			•
Mellosa i Maragle	SEAL:			
State of Florida Notary Signature (For the Contractor)				100
Melissa L. McKague	*	Dratit State	Notary Public State of Flor Melissa L McKague	lda
A.		3 6 8	My Commission DD49364	17
		TO FROM	Expires 11/22/2009	

Page 2 of 2 (Both Pages must be submitted together.)

Revised 11-30-07

This Instrument Prepared by and Return to:

Amy Wesp SOUTHERN TITLE

HOLDING

COMPANY, LLC. 3943 BAY MEADOWS ROAD JACKSONVILLE, Florida 32217

as a necessary incident to the fulfillment of conditions contained in a title insurance commitment issued by it.

Property Appraisers Parcel L.D. (Folio) Number(s): R02856-000

Grantee(s) LD.#(s): File No:JX0812085

WARRANTY DEED (CORPORATION)

This Warranty Deed Made this 27th day of May, 2008, by RML HOLDINGS INC., A FLORIDA CORPORATION, and having its place of business at 703 NW BLACKBERRY CIRCLE, LAKE CITY, Florida 32055, hereinafter called the grantor,

to MARONDA HOMES, INC. OF FLORIDA, A FLORIDA CORPORATION, whose post office address is: 11200 ST. JOHNS INDUSTRIAL PARKWAY, JACKSONVILLE, FLORIDA 32246, hereinafter called the grantee,

\$899,000.00

WOETH

Printed Name: Notary Public

Serial Number

WITNESSETH: That said grantor, for and in consideration of the sum of \$30.00 Dollars and other valuable considerations, receipt whereof is hereby acknowledged, by these presents grants, bargains, sells, aliens, remises, releases, conveys and confirms unto the grantee, all that certain land situate in Columbia County, Florida, viz: LOTS 1, 9, 10, 11, 12, 13, 14, 15, 16, 17, 18, 19, 20, 24, 27, 28, 29, 30, 31, 32, 33, 34, 35, 36, 37, 38, 39, 40, AND 41, OF TIMBERLANDS, PHASE 1, ACCORDING TO PLAT THEREOF AS RECORDED IN PLAT BOOK 9, PAGE 26 AND 27 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.

TOGETHER with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining. To Have and to Hold, the same in fee simple forever.

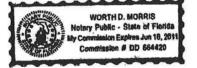
And the grantor hereby covenants with said grantee that the grantor is lawfully seized of said land in fee simple; that the grantor has good right and lawful authority to sell and convey said land; that the grantor hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever; and that said land is free of all encumbrances, except taxes accruing subsequent to December 31, 2007, reservations, restrictions and easements of record, if any.

(Werever used herefi the terms "grantor" and "grantee" included all the parties to this instrument, and the heltrs, legal representatives and exigns of individuals, and the successors and assigns of corporation.)

In Witness Whereof, the Grantor has caused these presents to be executed in its name, and its corporate seal to be hereinto attitud, by its proper officers thereunto duly authorized, the day and year first above written.

-Signed seafed and delivered in our presence:

ATTEST:	
Secretary	RML HOLDINGS INC.
Witness Signature: North D. Marr. Printed Name: WORTH D. HORRES	- BY: Dant Rfail
Witness Signature: Jodym. Coole. Av Printed Name: Jodym. Coble	ROBERTAL LARDIZABAL, PRESIDENT
STATE OF FLORIDA	
COUNTY OF DUVAL:	
The foregoing instrument was acknowledged before ROBERT R. LARDIZABAL as PRESIDENT of I behalf of the corporation. He/she is personally known	re me this 28 ^E day of May , 200 f, by RML HOLDINGS INC., A FLORIDA CORPORATION, on n to me or who has produced driver license(s) as identification.
My Commission Expires:	· Thered D. Mains





STATE OF FLORIDA DEPARTMENT OF HEALTH

08-401

Permit Application Number ____

APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

PART II - SITE PLAN	
Scale: Each block represents 5 feet and 1 inch = 50 feet.	
Notes:	
Lot	F7.1
	-1
Site Plan submitted by: Signature	Title
Plan Approved Not Approved	Date 6-23-0
By Colubia Col	unty Health Depart

Project Name:

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs
Residential Whole Building Performance Method A

Duildon

ALISTIN A RDR GAINESVILLE

21/1 TM

Address: Z54 SW TIMDER RIGGEDY. City, State: LOW City, In 32 W5 Owner: ELECTRIC Climate Zone: North	Permitting Office: COUNDIA Permit Number: 272/8 Jurisdiction Number: 22/000
New construction or existing	12. Cooling systems
2. Single family or multi-family Single family	a. Central Unit Cap: 40.5 kBtu/hr
3. Number of units, if multi-family 4. Number of Bedrooms	SEER: 13.00
5. Is this a worst case? Yes	b. N/A
6. Conditioned floor area (ft²) 2236 ft²	c. N/A
7. Glass type ¹ and area: (Label reqd. by 13-104.4.5 if not default)	0.10/2
a. U-factor: Description Area	13. Heating systems
(or Single or Double DEFAULT) 7a(Sngle Default) 177.0 ft ²	a. Electric Heat Pump Cap: 40.5 kBtu/hr
b. SHGC:	HSPF: 8.10
(or Clear or Tint DEFAULT) 7b. (Clear) 177.0 ft ²	b. N/A
8. Floor types .	
a. Slab-On-Grade Edge Insulation R=0.0, 219.0(p) ft	c. N/A
b. N/A c. N/A	
9. Wall types	14. Hot water systems
a. Concrete, Int Insul, Exterior R=4.1, 1199.0 ft ²	a. Electric Resistance Cap: 50.0 gallons
b. Frame, Steel, Adjacent R=13.0, 320.0 ft ²	b. N/A
c. N/A	
d. N/A	c. Conservation credits
e. N/A	(HR-Heat recovery, Solar
10. Ceiling types	DHP-Dedicated heat pump)
a. Under Attic R=19.0, 2450.0 ft ²	15. HVAC credits PT,
b. N/A	(CF-Ceiling fan, CV-Cross ventilation,
c. N/A	HF-Whole house fan,
11. Ducts a. Sup: Unc. Ret: Con. AH(Sealed):Interior Sup. R=6.0, 200.0 ft	PT-Programmable Thermostat,
b. N/A	MZ-C-Multizone cooling,
J. 1011	MZ-H-Multizone heating)
Glass/Floor Area: 0.08 Total as-built p	points: 28950 PASS
I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code. PREPARED BY:	Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code.

PREPARED BY:

DATE: WMWL COMPONI ON IV D8

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENP: MCOME

DATE: MCOMPONI ON IV D8

With the Florida Energy Code.

Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL:

DATE:

DATE:

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , , PERMIT #:

	BASE		AS-BUILT									
	GLASS TYPES .18 X Conditioned X BSPM = Points Floor Area				Overhang Type/SC Ornt Len Hgt Area X SPM X SO						SOF	= Points
.18 2236.0		18.59	7482.0	1.Single, Clear 2.Single, Clear 3.Single, Clear 4.Single, Clear 5.Single, Clear 6.Single, Clear 7.Single, Clear 8.Single, Clear As-Built Total:	W N W W E E E	1.0 1.0 1.0 1.0 1.0 1.0 1.0	6.0 6.0 3.0 7.0 8.0 6.0 6.0	16.0 16.0 5.0 30.0 40.0 20.0 20.0 30.0	43.8 43.8 43.8 47.9 47.9	34 73 34 34 34 32	0.97 0.97 0.85 0.98 0.99 0.97 0.97	680.0 338.0 186.0 1294.0 1734.0 927.0 927.0 1394.0
WALL TYPES	Area X	BSPM	= Points	Туре		R-	√alue	Area	Х	SPM	=	Points
Adjacent Exterior	320.0 1199.0	0.70 1.70	224.0 2038.3	1. Concrete, Int Insul, 2. Frame, Steel, Adja			4.1 13.0	1199.0 320.0		1.13 0.90		1360.9 288.0
Base Total:	1519.0		2262.3	As-Built Total:				1519.0				1648.9
DOOR TYPES	Area X	BSPM	= Points	Туре		8)		Area	Х	SPM	=	Points
Adjacent Exterior	18.0 20.0	2.40 6.10	43.2 122.0	1.Adjacent Wood 2.Exterior Insulated	Westernament of Allenda		542 014 011 750	18.0 20.0		2.40 4.10	or game, to the	43.2 82.0
Base Total:	38.0	S180885, S1400	165.2	As-Built Total:				38.0				125.2
CEILING TYPES	Area X	BSPM	= Points	Туре		R-Valu	e A	Area X S	SPM	x sc	M =	Points
Under Attic 2	2236.0	1.73	3868.3	1. Under Attic		1	9.0	2450.0 2	.34 X	(1.00	Υı	5733.0
Base Total:	2236.0		3868.3	As-Built Total:				2450.0	9			5733.0
FLOOR TYPES	Area X	BSPM	= Points	Туре	5.	R-V	/alue	Area	Х	SPM	=	Points
Slab 21 Raised	19.0(p) 0.0	-37.0 0.00	-8103.0 0.0	1. Slab-On-Grade Ed	ge Insulation		0.0	219.0(p		41.20		-9022.8
Base Total:			-8103.0	As-Built Total:				219.0	-1,			-9022.8
INFILTRATION	Area X	BSPM	= Points					Area	Χ	SPM	=	Points
	2236.0	10.21	22829.6					2236.0		10.21		22829.6

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , , PERMIT #:

	BASE		AS-BUILT							
Summer Ba	se Points: 2	8504.3	Summer As-Built Points: 2879	93.8						
Total Summer Points	X System = Multiplier	Cooling Points	Total X Cap X Duct X System X Credit = Coc Component Ratio Multiplier Multiplier Multiplier Poi (System - Points) (DM x DSM x AHU)	oling ints						
28504.3	0.3250	9263.9	(sys 1: Central Unit 40500btuh ,SEER/EFF(13.0) Ducts:Unc(S),Con(R),Int(AH),R6.0(INS) 28794 1.00 (1.08 x 1.147 x 0.86) 0.260 0.950 762 28793.8 1.00 1.072 0.260 0.950 762							

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , , PERMIT #:

BASE		AS-BUILT							
GLASS TYPES .18 X Conditioned X BWPM = Floor Area	Points	Overhang Type/SC Ornt Len Hgt Area X WPM X WOF =	Points						
.18 2236.0 20.17	8118.0	1.Single, Clear W 1.0 6.0 16.0 28.84 1.01	465.0						
		2.Single, Clear N 1.0 6.0 16.0 33.22 1.00	531.0						
		3.Single, Clear W 1.0 3.0 5.0 28.84 1.04	150.0						
		4.Single, Clear W 1.0 7.0 30.0 28.84 1.00	869.0						
	93	5.Single, Clear W 1.0 8.0 40.0 28.84 1.00	1157.0						
	1	6.Single, Clear E 1.0 6.0 20.0 26.41 1.02	536.0						
	-	7.Single, Clear E 1.0 6.0 20.0 26.41 1.02	536.0						
		8.Single, Clear E 1.0 6.0 30.0 26.41 1.02	804.0						
		As-Built Total: 177.0	5048.0						
WALL TYPES Area X BWPM	= Points	Type R-Value Area X WPM = F	oints						
Adjacent 320.0 3.60	1152.0	1. Concrete, Int Insul, Exterior 4.1 1199.0 6.42	7697.6						
Exterior 1199.0 3.70	4436.3	2. Frame, Steel, Adjacent 13.0 320.0 4.90	1568.0						
Base Total: 1519.0	5588.3	As-Built Total: 1519.0	9265.6						
DOOR TYPES Area X BWPM	= Points	Type Area X WPM = F	oints						
Adjacent 18.0 11.50	207.0	1.Adjacent Wood 18.0 11.50	207.0						
Exterior 20.0 12.30	246.0	2.Exterior Insulated 20.0 8.40	168.0						
Base Total: 38.0	453.0	As-Built Total: 38.0	375.0						
CEILING TYPES Area X BWPM	= Points	Type R-Value Area X WPM X WCM = F	oints						
Under Attic 2236.0 2.05	4583.8	1. Under Attic 19.0 2450.0 2.70 X 1.00	6615.0						
Base Total: 2236.0	4583.8	As-Built Total: 2450.0	6615.0						
FLOOR TYPES Area X BWPM	= Points	Type R-Value Area X WPM = F	oints						
Slab 219.0(p) 8.9	1949.1	1. Slab-On-Grade Edge Insulation 0.0 219.0(p 18.80	4117.2						
Raised 0.0 0.00	0.0	AVE	Construction Construction						
Base Total:	1949.1	As-Built Total: 219.0	4117.2						
INFILTRATION Area X BWPM	= Points	Area X WPM = F	oints						
2236.0 -0.59	-1319.2	2236.0 -0.59 -	1319.2						

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

	 The state of the s	
ADDRESS:,,,	PERMIT #:	
		_

	BASE	i	AS-BUILT							
Winter Base Points: 19373.0			Winter As-Built Points: 24	24101.5						
Total Winter X Points	System = Multiplier	Heating Points		Heating Points						
19373.0	0.5540	10732.6	(基準) 보고 있었다면 보면 있었다	R6.0 10552.7)552.7						

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: , , , PERMIT #:

BASE				AS-BUILT									
WATER HEA Number of Bedrooms	100		=	Total	Tank Volume	EF	Number of Bedrooms	х	Tank X Ratio	Multiplier		Credit Multiplie	
4		2635.00		10540.0	50.0 As-Built To	0.90 otal:	4 .		1.00	2693.56	5	1.00	10774.2 10774.2

	CODE COMPLIANCE STATUS												
	BASE									AS	-BUILT		
Cooling Points	+	Heating Points	+	Hot Water Points	Sel Consumeration	Total Points	Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points
9264		10733		10540	Marajad	30537	7623	15.	10553	3.	10774		28950

PASS



Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS:,,,	PERM	літ #:
	No.	

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	1
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	V
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	\checkmark
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	V
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	V
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	1
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	/

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked cirbreaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	1
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	1
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	V
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	V
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	V

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 85.8

The higher the score, the more efficient the home.

ELECTRIC, , ,

1.	New construction or existing	New	12.	Cooling systems	
2.	Single family or multi-family	Single family	a	. Central Unit	Cap: 40.5 kBtu/hr
3.	Number of units, if multi-family	1	A		SEER: 13.00
4.	Number of Bedrooms	\$ 4	—	. N/A	_
5.	Is this a worst case?	Yes			_
6.	Conditioned floor area (ft²)	2236 ft²	с.	. N/A	_
7.	Glass type 1 and area: (Label reqd. by 13-10	04.4.5 if not default)	(1770))		-
a.		escription Area	13.	Heating systems	
	(or Single or Double DEFAULT) 7a(Sngl	e Default) 177.0 ft²	a.	. Electric Heat Pump	Cap: 40.5 kBtu/hr
b.	SHGC:	1			HSPF: 8.10
	(or Clear or Tint DEFAULT) 7b.	(Clear) 177.0 ft ²	b	. N/A	
8.	Floor types	the state of the s			-
a.	Slab-On-Grade Edge Insulation	R=0.0, 219.0(p) ft	_ c.	. N/A	
b.	N/A	£.			
c.	N/A	3	14.	Hot water systems	
9.	Wall types	6	a.	. Electric Resistance	Cap: 50.0 gallons
a.	Concrete, Int Insul, Exterior	R=4.1, 1199.0 ft ²		€	EF: 0.90
b.	Frame, Steel, Adjacent	R=13.0, 320.0 ft ²	b.	. N/A	
c.	N/A	4) 1	-	*	
d.	N/A		- с.	. Conservation credits	-
e.	N/A	4	3 	(HR-Heat recovery, Solar	2.
10.	Ceiling types			DHP-Dedicated heat pump)	
a.	Under Attic	R=19.0, 2450.0 ft ²	15.	HVAC credits	PT,
b.	N/A			(CF-Ceiling fan, CV-Cross ventilation,	180000 19 000
c.	N/A	. }		HF-Whole house fan,	
11.	Ducts		Const	PT-Programmable Thermostat,	
a.	Sup: Unc. Ret: Con. AH(Sealed):Interior	Sup. R=6.0, 200.0 ft		MZ-C-Multizone cooling,	
ъ.	N/A	7	240	MZ-H-Multizone heating)	
			•	\$700	

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant feature

Builder Signature: |

Involer Rigge Gir FL Zip: Lake Cody, Pl

*NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar Mesignation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.

DC,P DeWitt Cason, Columbia County Page 1 of 1 B 1155 P 1861 NOTICE OF COMMENCEMENT 10-02850-12 County Clerk's Office Stamp or Seal THE UNDERSIGNED hereby gives notice that improvements will be made to certain real property, and in accordance with Section 713.13 of the Florida Statutes, the following information is provided in this NOTICE OF COMMENCEMENT. 1. Description of property (legal description): a) Street (job) Address: 2. General description of improvements: 3. Owner Information b) Name and address of fee simple titleholder (if other than owner) c) Interest in property 4. Contractor Information a) Name and address: MAYONGA HOMES INC & FL USOD SOUTHOUNT PLOWA b) Telephone No. (Opt.) (101) 332 b) Telephone No. [0 5. Surety Information a) Name and address: b) Amount of Bond: c) Telephone No.: 6. Lender Name and address; b) Phone No._ 7. Identity of person within the State of Florida designated by owner upon whom notices or other documents may be served: a) Name and address: SWHMYN THE HOWING CO LUC 3043 BUTWOWNS b) Telephone No.: (1024) 739-2205 Fax No. (Opt.) Fax No. (OpL) 8. In addition to himself, owner designates the following person to receive a copy of the Lienor's Notice as provided in Section 713.13(I)(b), Florida Statutes: a) Name and address: b) Telephone No.: 9. Expiration date of Notice of Commencement (the expiration date is one year from the date of recording unless a different date is specified): WARNING TO OWNER: ANY PAYMENTS MADE BY THE OWNER AFTER THE EXPIRATION OF THE NOTICE OF COMMENCEMENT ARE CONSIDERED IMPROPER PAYMENTS UNDER CHAPTER 713, PART I, SECTION 713.13, FLORIDA STATUTES, AND CAN RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY; A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT YOUR LENDER OR AN ATTORNEY BEFORE COMMENCING WORK OR RECORDING YOUR NOTICE OF COMMENCEMENT. STATE OF FLORIDA COUNTY OF COLUMBIA Signature of Owner or Owner's Authorized Office/Director/Partner/Manager Print Name The foregoing instrument was acknowledged before me, a Plorida Notary, this _ type of authority, e.g. officer, trustee, attorney (name of party on behalf of whom instrument was executed). OR Produced Identification ____ Type Notary Public State of Florida Melissa L McKague My Commission DD493647 Expires 11/22/2009 Notary Stamp or Scal:

11. Verification pursuant to Section 92.525, Florida Statutes. Under penalties of perjury, I declare that I have read the foregoing and that the

Signature of Natural Person Signing (in line #10 above.)

facts stated in it are true to the best of my knowledge and belief.

Columbia County Building Department Culvert Permit

Culvert Permit No.

000001642

DATE 07/3	30/2008 PARCEL ID #	10-4S-16-02856-121	
APPLICANT	PATRICK WILSON	PHONE	904.296.1490
ADDRESS _	6800 SOUTHPOINT PKWY. #300	JACKSONVILLE	FL 32216
OWNER M	IARONDA HOMES,INC. OF FLORIDA	PHONE	904.296.1490
ADDRESS _2	254 SW TIMBE RIDGE DRIVE	LAKE CITY	FL 32024
CONTRACTO	OR THEODORE BROCK	PHONE	407.227.3504
LOCATION O	OF PROPERTY 90-W TO SR. 247-S,TL TO	C-252-B,TR TO TIMBER R	IDGE,TL
4TH LOT ON L.			
SUBDIVISION	N/LOT/BLOCK/RHASE/UNIT TIMBERL	ANDS	21
SIGNATURE	INSTALLATION REQUIREMENT Culvert size will be 18 inches in diamete driving surface. Both ends will be mitered thick reinforced concrete slab. INSTALLATION NOTE: Turnouts will be an amajority of the current and existing by the driveway to be served will be partially to the concrete or paved concrete or paved driveway, which is current and existing paved or concrete or concrete or paved or concrete or pave	er with a total lenght of 3 and 4 foot with a 4 : 1 sloped 4 foot with a 4 : 1 sloped 4 foot with a 5 follows: and driveway turnouts are aved or formed with cond a minimum of 12 feet wever is greater. The width	pe and poured with a 4 inch paved, or; crete. wide or the width of the
	Culvert installation shall conform to the	e approved site plan star	ndards.
	Department of Transportation Permit in	nstallation approved star	ndards.
	Other		

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED DURING THE INSTALATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21 Lake City, FL 32055

Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00



GEO-TECH, INC.

Engineering Consultants in Geotechnical • Environmental • Construction Materials Testing

27218

FIELD DENSITY WORKSHEET

CLIENT MATONDA	HO ME	23			DATE	28	JUH	608
- To 1 1 1		1	. 1 -	e-1 -	PROJE	CT NO		
PROJECT NAME Timber LA	405	Los	14.	174	PERMI	T NO		
EARTH CONTRACTOR Lof #					-		1 / / / /	the state of the s
COMPACTION REQUIREMENT (%)			andard Foodified P	roctor	P	9 tricic	FIELD (CONTACT
TOTAL ON-SITE TIME	- 0.				MILES	FROM OF	FICE	
□ Limerock □ Subgrade □ Pipe Backfill □	Building	Pad 📮	Building	Footing	Oth	ner		
	LAB PR	OCTOR			7.00	WET	DRY	
TEST LOCATION	DENS.	ОМС	TEST DEPTH	PROBE DEPTH	MOIST.	DENSITY (PCF)	DENSITY (PCF)	COMP.
CAKIUS PAD	1049	10.1	FL	120	6.6	110.6	103.8	98.9
CHR. OF E. FIL	1	1	1	ſ	5.8	167,5	101.6	96-8
CAR OS AN FILE	1	F	1	1	63	108.2	101.8	97.0
REMARKS	****		<u> </u>	maria ka	1		sity failed to mum projec	
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							ined. It is aware	of
							it is aware itisfactory to	

Columbia County

Building Department

Residential Plan Review Checklist

Application #0806-53

Date Received: 6/27/08

911 Address: 254 SW Timber Ridge Dr.

Applicant Name: Theodore Brock/Patrick Wilson

Phone # 904-296-1490

Owner Name: Moronda Homes

Phone # 904-296-1490

Contractor Name: Theodore Brock

Engineer/Architect Name: Tomas Ponce

Phone # 407-321-0064

Fax Numbers: 904-332-6367

Application and Processing Forms

1.	Application & Checklist complete	Yes
2.	Notice of Commencement recorded at Columbia County Clerk office.	Yes
3.	Section 10 Township 4s Range16 Parcel Number 02856-121 Subdivision Name Timberlands lot 21 block 1	
4.	A copy of a approved Columbia County Environmental Health waste disposal system	Yes
5.	Owner Builder disclosure statement	N/A

6.	Front Setback 50 Side Setback 35 Side Setback 35 Rear Setback 57	
7.	Height of proposed structure measured from existing grade. 23'3"	Yes
8.	Under roof square footage 3061	
9.	Condition heated/cooled area 2236 Florida energy code Matches	Yes
10.	Do worksheet calculations agree with calculations on plans?	Yes
11.	Is the structure within the floodway?	No

Structure Code Compliance

12.	Are plans sealed by Architect or Engineer? Elmer Bergman P. E. # 50158	Yes
13.	Is correct wind speed shown? (FBC R301.2(4)) 110 MPH (3 second gust) 125 MPH	Yes
14.	Is exposure category 'B' shown? (FBC R301.2.1.4)	Yes
15.	Is Importance Factor 1 shown? (FBC 1604.5)	Yes
16.	Is internal pressure coefficient shown? (fully enclosed .18/Partial .55) ASCE 7	Yes
17.	Are pressures for wind loading on components and cladding shown? (FBC R301.2)	Yes
18.	Is there a proposed detach accessory structure on the same lot?	No
19.	Are the exterior walls, less than 6' apart, protected by 1 hr fire-resistance rating? (FBC R302.1)	N/A

20.	Are the projections extending into the 6' separation distance protected by 1 hr fire-resistance rating? (FBC R302.1	N/A
21.	Are penetrations located in the exterior wall of a dwelling separated by less than 6' protected in accordance with FBC R317.3?	N/A

Energy Code Information

22.	Is current energy code form completed properly and signed by designer and owner/agent, address, climate zone(3), Jurisdiction (Columbia County) and jurisdiction number (221000)? (FBC 13-600)	Yes
23.	Does conditioned square feet area on plans match square feet shown on energy forms?	Yes
24.	Manuel J submitted?	No

Construction Plans

25.	Is Designer's name, address and phone number shown on plans?	Yes
26.	Are current codes used for design listed?	Yes

Foundation Plan

27.	Are all footings shown, including interior bearing walls, column pads and concentrated loads? Sheet FTG & 2M	Yes
28.	Are all locations of vertical reinforcement and anchor bolts shown with spacing and size? 1 floor 8" Block	N/A
29.	Are all elevation changes in slab shown? Sheet FTG	Yes
30.	Is horizontal reinforcement shown or specified? Sheet 2M	Yes
31.	Is minimum concrete PSI shown? (FBC R402.2) 2500 PSI sheet 2m General notes	Yes
32.	Wire meshes size and gauges shown?	N/A
33.	Fiber meshes reinforcement? Sheet 2m	Yes
34.	Is vapor barrier, minimum 6 mil. shown? (FBC R320.1.4 & R506.2.3) Need to specify 6 mill Visqueen Sheet 2m	No
35.	Is minimum slab on grade thickness shown? (FBC R506.1) 3 1/2 " Sheet 2m General Notes	Yes
36.	Is type of soil treatment for termites shown? (FBC R320) Sheet 2m General Notes	Yes
37.	Is perimeter slab reinforcement shown? Sheet FTG	Yes
38.	Do plans show bottom of foundation minimum of 12" below finish grade? (FBC R403.1.4) Sheet FTG	Yes
39.	Do plans show concrete footings have a specified compressive strength of not less than 2500 PSI at 28 days? (FBC TABLE R402.2) Sheet FTG	Yes

Typical Wall Section

40.	Is finished grade shown? See Sheet FTG Drawing A one story Typical	Yes
41.	Is minimum floor elevation shown?	No
A.	Minimum 6" above adjacent grade? (FBC R319.1(5)) See Sheet FTG Drawing A one story Typical	Yes
B.	Minimum 12" above crown of road or drainage plan submitted? See subdivision plat (timberland subdivision)	Yes
C.	Flood protection elevation? Zone X	No

Typical Wall Section

D.	Base flood elevation?	N/A
E.	Are engineered floor elevations shown?	N/A
42.	Is minimum footing depth beneath finished grade shown? (FBC R403.1.4)	Yes
43.	Are all footing sizes shown? (FBC TABLE R403.1 for minimum) See Sheet FTG Drawing A one story Typical	Yes

44.	Are all horizontal reinforcements shown? (FBC R606.8)	Yes
A.	Number and size of reinforcement? See Sheet FTG Drawing A one story Typical	Yes
B.	Minimum lap? See Sheet FTG Drawing A one story Typical	Yes
43.	Is vertical reinforcement shown with spacing? (FBC R606.8) see sheet 2M	Yes
44.	Masonry construction: 8" Block	Yes
A.	Is exterior wall finish shown? Simulated stacked stone sheet 1M	Yes
B.	Is interior wall finish shown? ½" drywall Sheet CSU	Yes
C.	Is interior furring shown? 3/4" PT onto 8" CMU Sheet CSU	Yes
D.	Is insulation shown for exterior walls, floors, and roofs? See Sheet WS-B1 drawing B Walls R-4 Attic R-19	Yes
45.	Wood frame construction: (FBC R602.3)	N/A
A.	Is stud size, spacing, grade and lumber species shown? (FBC R602.3.1)	
В.	Are exterior sheathing (type and thickness) and attachment details shown? (FBC R602.3)	
c.	Are nailing requirements (size and spacing) shown? (FBC R602.3(1) through R602.3(4))	
D.	Is exterior wall finish shown?	
E.	Is interior wall finish shown?	
F.	Is wall insulation shown?	
G.	Is minimum clearance between wood siding and finished grade shown? (FBC R319.1(5)) 6"	
н.	Are shear wall segments shown with detailed drawings?	
	Type of hold-downs with locations, number and type of fasteners shown?	
46.	Are all hurricane anchorage and hold-downs specified and labeled?	
47.	Is connector schedule showing connector type, max uplift, number and size of fasteners provided?	
48.	Is ceiling type shown, drywall thickness?	
49.	Are ceiling heights shown? (FBC R305)	

Roof Framing:

50.	Are engineered trusses shown? (FBC R802.10.2)	Yes
51.	Are conventional frame rafters used? (FBC R802.2)	N/A
52.	Rafter size shown?	
53.	Are all hurricane anchors and hold downs shown and specified?	
54.	Species of lumber shown?	
55.	Grade of lumber shown?	
56.	Type of roof sheathing shown? (FBC R803) See Sheet WS-B1 drawing B 7/16" OSB	Yes
57.	Thickness of roof sheeting shown?	Yes
58.	Grade of roof sheeting shown? Hip Roof See Sheet RSIm & Sheet TRIm	Yes
59.	Nailing pattern of roof sheeting shown? (FBC Table R602.3(1)) See Sheet WS-B1 drawing B	Yes
60.	Type of roof cover shown? See Sheet WS-B1 drawing B & Product Approval sheet with plans	Yes
61.	Attachment asphalt/fiberglass shingles shown? (FBC R905.2) Product Approval sheet with plans	Yes
62.	Other roof covering and attachments shown?	No
63.	Length of roof overhang shown? See Sheet WS-B1 drawing B	Yes
64.	Type of soffit and fascia shown? Vented See Sheet WS-B1 drawing B	Yes
65.	Attic ventilation shown? (FBC R806) See Sheet RSIm	
		Yes

Floor Plan

66.	Does square footage on plan match square footage shown on energy form and site plan?	Yes
67.	Are square footage calculations shown for total square footage under roof?	Yes
68.	Are all room dimensions shown? (FBC R304.3)	Yes
69.	Are all door and window sizes shown?	Yes

70.	Are all exterior and adjacent doors shown to be insulated or solid core (other than glass doors)?	Yes
71.	Is garage separated from the residence and its attic area by not less than 1/2 inch gypsum board? (FBC R309.2)	Yes
72.	Are habitable rooms above the garage separated by not less than 5/8 inch Type X gypsum board? (FBC R309.2)	N/A
73.	Is door between garage and living space equipped with solid wood door not less than 1 3/8 inches, solid or honeycomb core steel door not less than 1 3/8 inches thick, or 20 minute fire-rated door? (FBC R309.1)	Yes
74.	Are all emergency egress openings shown (egress windows and doors)? (FBC R310.1)	Yes
75.	Is required tempered glass shown at all hazardous locations? (FBC R308.4)	Yes
76.	Are all vertical reinforcements shown for shear walls shown?	Yes
77.	Are all shear wall segments shown?	Yes
78.	Are all hold-downs and hurricane anchorage shown and identified?	Yes
79.	Is required attic access shown? - 22" x 30" (FBC R807.7)	Yes
80.	Does one (1) bathroom on the first habitable floor level have a 29" net clear door opening and handicap accessible route? (FBC 11-11) (minimum door size 32")	Yes
81.	Does bedroom not opening directly into garage? (FBC R309.1)	No
82.	Is at least one 3' 0" wide, side hinged egress door shown? (FBC R311.4)	Yes
83.	Do doors and landings comply with FBC R311.4.3?	Yes
84.	Are habitable rooms shown with the minimum light and ventilation requirements of FBC R303.1? (8% natural light)	Yes
85.	Are garage doors, all windows, doors, sky lights and other openings shown as meeting wind load requirements for components and cladding per FBC R301.2.1? Are design pressures specified?	Yes
86.	Does floor plan show fireplace?	No
87.	Does masonry fireplace have a detailed for a load-bearing foundation?	N/A
88.	Are copies of pre-fabricated fireplace manufacturer's specifications included?	N/A
89.	Is hearth size and detail shown? (FBC R1003.9)	N/A

Stairs Details

90.	Is minimum stair width shown? (FBC R311.5.1)	N/A
91.	Are tread and riser sizes shown? (FBC R311.5.3)	N/A
92.	Do spiral stairways comply with FBC R311.5.8.1?	N/A
93.	Are required landings and size shown? (Max vertical rise is 12' between floors.) (FBC R311.5.4)	Yes
94.	Are handrail height, spacing and grasp ability details shown? (FBC R311.5.6)	N/A
95.	Is required headroom clearance shown? (FBC R311.5.2) 6' 8"	Yes
96.	Are guardrail height, spacing and details shown (max openings less than 4")? (FBC R312)	N/A
97.	Are exterior door landing shown?	Yes

Porches

98.	Are all columns and beams shown for porches and lanais?	N/A
99.	Are column type, size and anchorage shown?	N/A
100.	Are beam type, size, span and anchorage shown including garage opening and porch beams?	Yes
101.	Are all lintel and beam details shown? (FBC R611.7.3)	Yes

Floor Framing (FBC-2307)

102.	Is joist plan provided, showing size, spacing, span, species, grade of lumber and connections?	N/A
103.	Is floor sheathing indicated, showing type, thickness and nailing pattern? (FBC R503)	N/A
104.	Is crawl space opening shown (18" x 24" minimum)? (FBC R408.3)	N/A
105.	Is the crawl space showed to be insulated, showing R rating?	N/A

106.	Is joist plan provided, showing size, spacing, span, species, grade of lumber and connections?	N/A
107.	Is floor sheathing indicated, showing type, thickness and nailing pattern? (FBC R503)	N/A
108.	Is crawl space opening shown (18" x 24" minimum)? (FBC R408.3)	N/A
109.	Is the correct amount of area of ventilation openings shown? (FBC R408.2)	N/A

Elevations

110.	Does plan show four (4) elevations?	Yes
111.	Are attic ventilation requirements shown? (FBC R806.1)	Yes
112.	Are roof pitch and overhang shown for sloped and flat roofs?	Yes
113.	Is chimney height and location shown? (FBC R1001.6)	N/A
114.	Are all lanai/porch details shown?	Yes
115.	Are roof drainage provisions shown? (FBC R801.3)	N/A
116.	Does the front elevation show the existing grade elevation?	No
117.	Is total height shown from the existing grade, not from finished floor?	Yes

Structural Details (also see Structural Code Compliance section)

118.	Are gable end bracing details shown?	N/A
119.	Are roof sheeting nailing zones shown? (FBC FIGURE R301.2(8))	Yes
120.	Are wind design pressures for components and cladding shown? (FBC R301.2)	Yes
121.	Are exterior windows and glass doors shown as approved by independent testing laboratory and do they bear a label by AAMA or WDMA or other approved label? (FBC R613.3.1)	Yes
122.	Are exterior window and door manufacturer's specifications and installation details which meet the specified design pressures provided?	Yes
123.	Are window and door installation and buck details shown?	Yes
124.	Are mullion installation details and design criteria provided	Yes
125.	Are garage door positive and negative design wind pressures shown as meeting requirements of 1.5 x pressure? (FBC R613.4)	Yes
126.	Are number and size of fasteners for all connections shown?	Yes

Electrical (NEC)

127.	Is underground service specified? (WPA 106.3.4)	Yes
128.	Is an exterior service disconnect shown?	No
129.	Is service size (amps) and location shown? (NEC 230) 150 Amp	Yes
123.	Are panel locations shown with proper clearances (NEC 384)?	Yes
124.	Are disconnects shown (WH and A/C equipment) (NEC 440-14)? Include exterior 110 Volt receptacle GFI Near Ac Compressor	Yes
125.	Are GFI receptacles (kitchen, bath, exteriors, basements and garage) shown? (NEC 210-8)	Yes

Electrical (NEC)

126.	Are all smoke detectors shown (bedroom halls, top & bottom of stairs)? (FBC R313.1)	Yes
127.	Are the required carbon monoxide alarm shown within 10 feet of each room used for sleeping room, with the dwelling having a fossil-fuel-burning heater or appliance, a fireplace, or an attached garage carbon monoxide alarm installed purposes.	NO
128.	Are receptacle locations shown? (NEC 210-52) The <u>2008 National Electric Code</u> expands the Combination Type AFCI requirement beyond bedroom circuits to include <i>additional circuits in the home</i> , (i.e. family rooms, dining rooms, living rooms, hallways, libraries, dens, sun rooms, recreation rooms, and similar rooms.	NO
129.	NEC Article 406.11 states that all 125-volt. 15- and 20-ampere receptacles shall be listed tamper-resistant receptacles the effective date will be upon the adoption of the 2008 NEC	NO
130.	Walls receptacles (12 ft. o.c. & 6 ft from openings)?	Yes
140.	Kitchen counter tops (12 in. widths, 48 in. o.c. and 2in. from edge of counter) and islands?	N/A
141.	Plan shows GFI – receptacles, with water proof or unattended type covers front and rear of dwelling.	Yes
142.	Is switched lighting shown? (NEC 210-70)	Yes

143.	A. Top and Bottom of stairs?	N/A
144.	B. Attic access?	Yes
145.	C. Exterior doors?	Yes
146.	D. Occupiable rooms (light or switched receptacle)?	Yes
147.	Are all electrical fixtures shown?	Yes

Plumbing (FPC)

148.	Are all plumbing fixtures shown on the foundation plan and floor plan?	Yes
149.	Is water heater size and location shown?	Yes
150.	Is the potable well shown on the site plan to include the size of pump motor, size of pressure tank and cycle stop valve?	City

Mechanical (FMC)

151.	Are all mechanical fixtures shown?	Yes
152.	Are the clothes dryer vent route shown not to exceed 25 feet from the dryer location to the outlet terminal.(Fuel & Gas code 504.4)	Yes

COLUMBIA COUNTY BUILDING DEPARTMENT RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR THE FLORIDA RESIDENTIAL BUILDING CODE 2004 with 2005 & 2006 Supplements and One (1) and Two (2) Family Dwellings

ALL REQUIREMENTS ARE SUBJECT TO CHANGE

ALL BUILDING PLANS MUST INDICATE COMPLIANCE with the Current FLORIDA BUILDING CODES and the Current FLORIDA RESIDENTIAL CODE. ALL PLANS OR DRAWING SHALL PROVIDED CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS.

FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEEDS ARE PER FIGURE R301.2(4) of the Residential Code (Florida Wind speed map) SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

- 1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ------ 100 MPH
- 2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE ------110 MPH
- 3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

GENERAL REQUIREMENTS;

- Two (2) complete sets of plans containing the following:
- All drawings must be clear, concise and drawn to scale, details that are not used shall be marked void
- O Condition space (Sq. Ft.) and total (Sq. Ft.) under roof shall be shown on the plans.
- Designers name and signature shall be on all documents and a licensed architect or engineer, signature and official embossed seal shall be affixed to the plans and documents per FBC 106.1.

Site Plan information including:

- Dimensions of lot or parcel of land
- Dimensions of all building set backs
- Location of all other structures (include square footage of structures) on parcel, existing or proposed well and septic tank and all utility easements.
- Provide a full legal description of property.

Wind-load Engineering Summary, calculations and any details required:

- Plans or specifications must meet state compliance with FRC Chapter 3
- The following information must be shown as per section FRC
- Basic wind speed (3-second gust), miles per hour
- Wind importance factor and nature of occupancy
- Wind exposure if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated
- The applicable internal pressure coefficient, Components and Cladding The design wind pressure in terms of psf (kN/m²), to be used for the design of exterior component and cladding materials not specifally designed by the registered design professional.

Elevations Drawing including:

- All side views of the structure
- Roof pitch
- Overhang dimensions and detail with attic ventilation
- Location, size and height above roof of chimneys
- Location and size of skylights with Florida Product Approval
- Number of stories
- e) Building height from the established grade to the roofs highest peak

Floor Plan including:

- Dimensioned area plan showing rooms, attached garage, breeze ways, covered porches, deck, balconies and raised floor surfaces located more than 30 inches above the floor or grade
- All exterior and interior shear walls indicated
- Shear wall opening shown (Windows, Doors and Garage doors
- Emergency escape and rescue opening in each bedroom (net clear opening shown)

Safety glazing of glass where needed

- Fireplaces types (gas appliance) (vented or non-vented) or wood burning with Hearth (see chapter 10 of FRC)
- Stairs with dimensions (width, tread and riser and total run) details of guardrails, Handrails (see FRC 311)
- Plans must show and identify accessibility of bathroom (see FRC 322)

 All materials placed within opening or onto/into exterior shear walls, soffits or roofs shall have Florida product approval number and mfg. installation information submitted with the plans (see Florida product approval form)

Foundation Plans Per FRC 403:

- a) Location of all load-bearing walls footings indicated as standard, monolithic, dimensions, size and type of reinforcing.
- o b) All posts and/or column footing including size and reinforcing
- o c) Any special support required by soil analysis such as piling.
- d) Assumed load-bearing valve of soil (psf)
- e) Location of horizontal and vertical steel, for foundation or walls (include # size and type)

CONCRETE SLAB ON GRADE Per FRC R506

- Show Vapor retarder (6mil. Polyethylene with joints lapped 6 inches and sealed)
- Show control joints, synthetic fiber reinforcement or welded fire fabric reinforcement and Supports

PROTECTION AGAINST TERMITES Per FRC 320:

Indicate on the foundation plan if soil treatment is used for subterranean termite prevention or submit other approved termite protection methods. Protection shall be provided by registered termiticides

Masonry Walls and Stem walls (load bearing & shear Walls) FRC Section R606

- Show all materials making up walls, wall height, and Block size, mortar type
- Show all Lintel sizes, type, spans and tie-beam sizes and spacing of reinforcement

Metal frame shear wall and roof systems shall be designed, signed and sealed by Florida Prof. Engineer or Architect

Floor Framing System: First and/or second story

- Floor truss package shall including layout and details, signed and sealed by Florida Registered Professional Engineer
- Show conventional floor joist type, size, span, spacing and attachment to load bearing walls, stem walls and/or priers
- o Girder type, size and spacing to load bearing walls, stem wall and/or priers
- Attachment of joist to girder
- Wind load requirements where applicable
- Show required under-floor crawl space
- Show required amount of ventilation opening for under-floor spaces
- Show required covering of ventilation opening.
- Show the required access opening to access to under-floor spaces
- Show the sub-floor structural panel sheathing type, thickness and fastener schedule on the edges & intermediate of the areas structural panel sheathing
- Show Draft stopping, Fire caulking and Fire blocking
- Show fireproofing requirements for garages attached to living spaces, per FRC section R309
- o Provide live and dead load rating of floor framing systems (psf).

WOOD WALL FRAMING CONSTRUCTION FRC CHAPTER 6

- O Stud type, grade, size, wall height and oc spacing for all load bearing or shear walls.
- o Fastener schedule for structural members per table R602.3 (1) are to be shown.
- Show wood structural panel's sheathing attachment to studs, joist, trusses, rafters and structural members, showing fastener schedule attachment on the edges & intermediate of the areas structural panel sheathing
- Show all required connectors with a max uplift rating and required number of connectors and oc spacing for continuous connection of structural walls to foundation and roof trusses or rafter systems.
- Show sizes, type, span lengths and required number of support jack studs, king studs for shear wall
 opening and girder or header per FRC Table R502.5 (1)
- Indicate where pressure treated wood will be placed.
- Show all wall structural panel sheathing, grade, thickness and show fastener schedule for structural panel sheathing edges & intermediate areas
- A detail showing gable truss bracing, wall balloon framing details or/ and wall hinge bracing detail

ROOF SYSTEMS:

- Truss design drawing shall meet section FRC R802.10 Wood trusses. Include a layout and truss details and be signed and sealed by Fl. Pro. Eng.
- Show types of connector's assemblies' and resistance uplift rating for all trusses and rafters
- Show gable ends with rake beams showing reinforcement or gable truss and wall bracing details
- Provide dead load rating of trusses

Conventional Roof Framing Layout Per FRC 802:

- Rafter and ridge beams sizes, span, species and spacing
- Connectors to wall assemblies' include assemblies' resistance to uplift rating.
- Valley framing and support details
- Provide dead load rating of rafter system.

ROOF SHEATHING FRC Table R602,3(2) FRC 803

Include all materials which will make up the roof decking, identification of structural panel sheathing, grade, thickness and show fastener schedule for structural panel sheathing on the edges & intermediate areas

ROOF ASSEMBLIES FRC Chapter 9

Include all materials which will make up the roof assembles covering; with Florida Product Approval numbers for each component of the roof assembles covering.

FCB Chapter 13 Florida Energy Efficiency Code for Building Construction

- Residential construction shall comply with this code by using the following compliance methods in the FBC Subchapter 13-6, Residential buildings compliance methods. Two of the required forms are to be submitted, showing dimensions condition area equal to the total condition living space area
- Show the insulation R value for the following areas of the structure: Attic space, Exterior wall cavity and Crawl space (if applicable)

HVAC information shown

- Manual J sizing equipment or equivalent computation
- Exhaust fans locations in bathrooms

Plumbing Fixture layout shown

All fixtures waste water lines shall be shown on the foundation plan

Electrical layout shown including:

- Switches, outlets/receptacles, lighting and all required GFCI outlets identified
- Ceiling fans
- Smoke detectors
- Service panel, sub-panel, location(s) and total ampere ratings

- On the electrical plans identify the electrical service overcurrent protection device for the main electrical service. This device shall be installed on the exterior of structures to serve as a disconnecting means for the utility company electrical service. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground. Indicate if the utility company service entrance cable will be of the overhead or underground type.
- Appliances and HVAC equipment and disconnects
- Arc Fault Circuits (AFCI) in bedrooms
- Notarized Disclosure Statement for Owner Builders
- Notice of Commencement Recorded (in the Columbia County Clerk Office) <u>Notice</u>
 <u>Of Commencement is required to be filed with the building department Before Any</u>
 Inspections Will Be Done.

Private Potable Water

- Size of pump motor
- Size of pressure tank
- Cycle stop valve if used

THE FOLLOWING ITEMS MUST BE SUBMITTED WITH BUILDING PLANS

- Building Permit Application: A current Building Permit Application form is to be completed and submitted for all residential projects.
- Parcel Number: The parcel number (Tax ID number) from the Property Appraiser (386) 758-1084 is required. A copy of property deed is also requested.
- Environmental Health Permit or Sewer Tap Approval: A copy of the Environmental Health permit,
 existing septic approval or sewer tap approval is required before a building permit can be issued. (386)
 758-1058 (Toilet facilities shall be provided for construction workers)
- <u>City Approval:</u> If the project is to be located within the city limits of the Town of Fort White, prior approval is required. The Town of Fort White approval letter is required to be submitted by the owner or contractor to this office when applying for a Building Permit. (386) 497-2321
- Flood Information: All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting application to this office. Any project located within a flood zone where the base flood elevation (100 year flood) has been established shall meet the requirements of Section 8.8 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.7 of the Columbia County Land Development Regulations. CERTIFIED FINISHED FLOOR ELEVATIONS WILL BE REQUIRED ON ANY PROJECT WHERE THE BASE FLOOD ELEVATION (100 YEAR FLOOD) HAS BEEN ESTABLISHED. A development permit will also be required. The permit cost is \$50.00.
- <u>Driveway Connection:</u> If the property does not have an existing access to a public road, then an application for a culvert permit (\$25.00) must be made. If the applicant feels that a culvert is not needed, they may apply for a culvert waiver (\$50.00). All culvert waivers are sent to the Columbia County Public Works Department for approval or denial.
- 911 Address: If the project is located in an area where the 911 address has been issued, then the proper Paper work from the 911 Addressing Departments must be submitted. (386) 758-1125

ALL REQUIRED INFORMATION IS TO BE SUBMITTED FOR REVIEW. NOTIFICATION WILL BE GIVEN WHEN THE APPLICATION AND PLANS ARE APPROVED AND READY TO PERMIT.

PRODUCT APPROVAL SPECIFICATION SHEET

Location:	Project Name: tute 553.842 and Florida Administrative Code 9B-72, please provide the information and the s) on the building components listed below if they will be utilized on the construction project for or a building permit on or after April 1, 2004. We recommend you contact your local product low the product approval number for any of the applicable listed products. More information approval can be obtained at the product approval can be obtained at the product approval.				
product approval number(s) of which you are applying for a supplier should you not know					
Category/Subcategory	Manufacturer	Product Description	Approval Number(s)		
A. EXTERIOR DOORS					
1. Swinging					
2. Sliding					
3. Sectional					
4. Roll up 5. Automatic					
6. Other					
B. WINDOWS					
Single hung Horizontal Slider					
3. Casement					
Double Hung					
5. Fixed					
6. Awning					
7. Pass-through			+		
8. Projected					
9. Mullion					
10. Wind Breaker					
11 Dual Action					
12. Other					
C. PANEL WALL					
1. Siding					
2. Soffits					
3. EIFS					
4. Storefronts					
5. Curtain walls					
6. Wall louver					
7. Glass block					
8. Membrane					
9. Greenhouse					
10. Other					
D. ROOFING PRODUCTS					
Asphalt Shingles					
2. Underlayments					
Roofing Fasteners					
4. Non-structural Metal Rf					
5. Built-Up Roofing					
6. Modified Bitumen					
7. Single Ply Roofing Sys					
8. Roofing Tiles					
Roofing Insulation Waterproofing					

02/02/04 - 1 of 2

12. Roofing Slate

11. Wood shingles /shakes

Category/Subcategory (cont.)	Manufacturer	Product Description	Approval Number(s)
13. Liquid Applied Roof Sys			
 Cements-Adhesives – Coatings 			
15. Roof Tile Adhesive			
16. Spray Applied			
Polyurethane Roof			
17. Other			
E. SHUTTERS			
Accordion			
2. Bahama			
Storm Panels			
4. Colonial			
5. Roll-up			
6. Equipment			
7. Others			
F. SKYLIGHTS			
Skylight			
2. Other			
G. STRUCTURAL			
COMPONENTS			
 Wood connector/anchor 			
Truss plates			
Engineered lumber			
Railing			
Coolers-freezers			
Concrete Admixtures			
7. Material			
Insulation Forms			
9. Plastics			
10. Deck-Roof			
11. Wall			
12. Sheds			
13. Other			
I. NEW EXTERIOR			
ENVELOPE PRODUCTS			
1.			_
2.			
me of inspection of these probbsite; 1) copy of the production of the production certified to comply with, 3	oducts, the follow t approval, 2) the s) copy of the app	e product approval at plan review ving information must be available e performance characteristics who olicable manufacturers installation emoved if approval cannot be de	e to the inspector on the ich the product was tested n requirements.
0/.			
1 1/1/1		D . V 11	1/2
Janus IV More	Actual Control of State Actual Control		ilson 6/27/08
Intractor or Contractor's Authorized	Agent Signature	Print Name	Øate
ocation		Permit # (FOR STA)	FF USE ONLY)

Website:

Effective April 1, 2004

02/02/04 - 2 of 2

Maronda Systems

MODEL

AUSM4 LEFT

Maronda Systems

4005 Maronda Way

Sanford FL 32771

(407) 321-0064

Fax (407) 321-3913

56

16.0 psf 7.0 psf 10.0 psf 10.0 psf 43.0 psf

Engineer/Architect of Record:

Tomas Ponce, P.E.

254 SW TIMBER

RIDGE DR

ADDRESS

367 Medallion Pl.

Chuluota, Fl. 32766

FL PE # 50068,

MiTek software **DIV/SUB**

De	esign Crit	eria:	TPI
	PLAN J	JOB :	#

9TM02101

Design: Matrix Analysis

JAX-9TM

AUSTIN	M4	

This structure was designed in accordance with, and meets the requirements of TPI standards and the FLORIDA 2004 BUILDING CODE for 125 M.P.H. Wind Zone. Truss loading is in accordance with ASCE 7-02. These trusses are designed for an enclosed building.

The Truss Engineering package for the above referenced site was generated by the Truss Designer/Architect/MiTek/Trenco.

I, Tomas Ponce, P.E.

the Architect/Engineer of Record for the above referenced lot

Have reviewed the package and confirmed that it matches the physical and structural

Parameters found on the set of permit drawings.

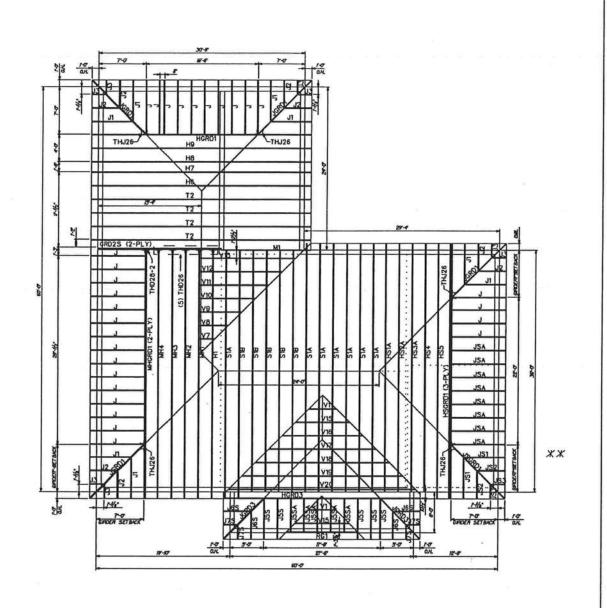
LOT

21-1

rarameters lou	nu on the set	or berning drawn	ngs.			
Truss ID	Run Date	Drawing Reviewed	Truss ID	Run Date	Drawing Reviewed	No. of Eng. Dwgs:
Layout	10/10/07		MHGRD1	11/21/07		Roof Loads-
V	07/27/05)	RG1	11/21/07	1	TC Live:
HIP	11/02/06		S1A	11/21/07		TC Dead:
GRD2S	01/03/08		S1B	11/21/07		BC Live:
H1	11/21/07		T2	11/21/07		BC Dead:
H6	11/21/07		V1	11/21/07		Total
H7	11/21/07		V10	11/21/07		
H8	11/21/07		V11	11/21/07		DurFac- Lbr: 1.25
H9	11/21/07		V12	11/21/07		DurFac- Plt: 1.25
HGRD1	11/21/07		V13	11/21/07		O.C. Spacing: 24.0"
HGRD3	11/21/07		V15	11/21/07		
HS1A	11/21/07		V16	11/21/07		
HS2A	11/21/07		V17	11/21/07		
HS3A	11/21/07		V18	11/21/07		- 1111 Da
HS4	11/21/07		V19	11/21/07		8UILDIA
HS5	11/21/07		V20	11/21/07		/S/queelvec
HSGRD1	11/21/07		V7	11/21/07		
J	11/21/07		V8	11/21/07	1_	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
J1	11/21/07		V9	11/21/07		FILE OC
J2	11/21/07					181
J3	11/21/07					C Marie
J5S	11/21/07					CLA
J5SA	11/21/07					14.0
J6S	11/21/07					5t# 1-2
J7S	11/21/07					
JGRD1	11/21/07					
JGRD3	11/21/07					
JS1	11/21/07		INV#	DESC	QNTY	
JS2	11/21/07		18331	THD26	5	
JS3	11/21/07		18330	THD28		
JSA	11/21/07		18357	SKHH26R		
JSGRD1	11/21/07		18357	SKHH26L		
M1	11/21/07		18363	JUS26		
MH1	11/21/07		18370	THJ26	5] /(
MH2	11/21/07		18336	THD28-2	1	1 /
MH3	11/21/07	1,				/
MH4	11/21/07	V	SEAT PLAT	ΓES		DATE: MAY 0 9



9 2008



HARDWARE LEGEND

1 HUS26

2 HUS28

3 JUS26

4 MP6F

5 MPA1 & MPA1F

6 SKH26 L/R

7 SKHH26 L/R

8 SUS26

9 SUS28

10 THD26

[11] THD28

12 THD28-2

13 THDH28-3

14 THD48

15 THJ26**

16 LTW12



HARDWARE MANUFACTURED BY USP

* HARDWARE MANUFACTURED BY SIMPSON

* HARDWARE MANUFACTURED BY CLEVELAND

AUSTIN 4 "M" - FL

DESIGNER: CP CHECKER: MIKE DRAWN BY: EJ

SCALE: 1/8" = 1'-0" DATE: 10/10/2007

GARAGE: LEFT LOADING-FBC2004/TPI2002

TC LNE 16.00 SNOW LOAD 0.00 TC DEAD 7.00 LUMBER DOL 1.25 125 PLATE DOL BC LNE 10,00 BC DEAD 10.00 WIND 125 TOTAL 2-0 43.00 SPACING



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Maronda Homes

(407) 321-0064 A005 MARONDA NAY SANFORD, FLORIDA

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GENERAL NOTES

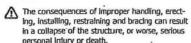
Trusses are not marked in any way to identify the frequency or location of temporary lateral restraint and diagonal bracing. Follow the recommendations for handling, installing and temporary restraining and bracing of trusses. Refer to BCST Guide to Good Practice for Handling. Installing. Restraining & Bracing of Metal Plate Connected Wood Trusses for more detailled Information.

Truss Design Drawings may specify locations of permanent lateral restraint or reinforcement for individual truss members. Refer to the <u>BCSI-B3 Summary Sheet — Permanent Restraint/Bracing of Chords & Web Members</u> for more information. All other permanent bracing design is the responsibility of the Building Designer.

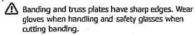
NOTAS GENERALES

Los trusses no están marcados de ningún modo que identifique la frecuencia o localización de restricción lateral y arriostre diagonal temporales. Use las recomendadones de manejo, instalación, restricción y arriostre temporal de los trusses. Vea el folleto <u>BCSI Quía de Buena Práctica</u> para el Manejo. Instalación, Restricción y Arriostre de los Trusses de Madera Conectados con Placas de Metal para información más detallada.

Los dibujos de diseño de los trusses pueden especificar las localizadones de restricción lateral permanente o refuerzo en los miembros individuales del truss. Vea la hoja resumen <u>BCSI-B3 — Restricción/Arriostre Permanente de Cuerdas y Miembros Secundarios para más información.</u> El resto de los diseños de arriostres permanentes son la responsabilidad del Diseñador del Edificio.

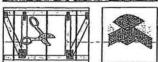


El resultado de un manejo, levantamiento, Instalación, restricción y arrisotre incorrecto puede ser la caída de la estructura o aún peor, heridos o muertos.



Empaques y placas de metal tienen bordes afilados. Use guantes y lentes protectores cuando corte los empaques.





HOISTING RECOMMENDATIONS FOR TRUSS BUNDLES RECOMENDACIONES PARA LEVANTAR PAQUETES DE TRUSSES.

Warning! Don't overload the crane. iAdvertencial iNo sobrecarga la grúa!

Never use banding alone to lift a bundle.

Do not lift a group of Individually banded bundles.

Nunca use sôlo los empaques para levantar un paquete.

No levante un grupo de empaques Individuales.

A single lift point may be used for bundles with trusses up to 45'.

Two lift points may be used for bundles with

busses up to 60'. Usé at least 3 lift points for bundles with trusses greater than 60'.

Puede usar un solo lugar de levantar para paquetes de trusses hasta 45 pies. Puede usar dos puntos de levantar para paquetes más de 60 pies.

Use por lo menos tres puntos de levantar para paquetes más de 60 ples.



Warning! Do not over load supporting structure with truss bundle.

l'Advertencial No sobrecargue la estructura apoyada con el paquete de trusses.

Place truss bundles in stable position.

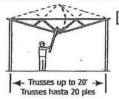
Puse paquetes de trusses en una posición estable

INSTALLATION OF SINGLE TRUSSES BY HAND INSTALACIÓN POR LA MANO DE TRUSSES INDIVIDUALES

Trusses 20' or less, support at peak.

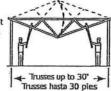
INDIVIDUALES

Levante del pico los trusses de 20 pies o



Trusses 30° or less, support at quarter points.

Levante de los cuartos de tramo los trusses de 30 ples o menos.



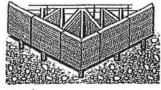
HANDLING — MANEJO

Avoid lateral bending. — Evite la flexión lateral.



The contractor is responsible for properly receiving, unloading and storing the trusses at the jobsite.

El contratista tiene la responsabilidad de recibir, descargar y almacenar adecuadamente los trusses en la obra.



If trusses are to be stored horizontally, place blocking of sufficient height beneath the stack of trusses at 8' to 10' on center.

For trusses stored for more than one week, cover bundles to prevent moisture gain but allow for ventilation.

Refer to BCSI Guide to Good Practice for Handling. Installing. Restraining & Bracing. of Metal Plate Connected Wood Trusses for more detailed information pertaining to handling and jobsite storage of trusses.

Si los trusses estarán guardados horizontalmente, ponga bloqueando de altura suficiente detrás de la pila de los trusses.

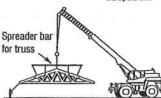
Para trusses guardados por más de una semana, cubra los paquetes para prevenir aumento de humedad pero permita venti-

Vea el folleto <u>BCSI Guía de Buena Práctica</u> para el Maneia. Instalación, Restricción y Arriostres de los Trusses de Hadera Conectados con Placas de Metal para información más detallada sobre el manejo y almacenado de los trusses en área de trabaio.



Use special care in windy weather or near power lines and airports.

Utilice cuidado especial en días ventosos o cerca de cables eléctricos o de aeropuertos.



Use proper rigging and hoisting equipment. Use equipo apropiado para levantar e improvisar.



O Do not store unbraced bundles upright. No almacene verticalmente los



O Do not store on uneven ground.

No almacene en tierra desigual.



HOISTING OF SINGLE TRUSSES — LEVANTAMIENTO DE TRUSSES INDIVIDUALI

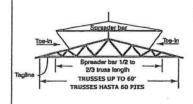
Hold each truss in position with the erection equipment until top chord temporary lateral restraint is installed and the truss is fastened to the bearing points.

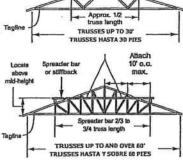
Sostenga cada bruss en posición con equipo de grúa hasta que la restricción lateral temporal de la cuerda superior esté instalado y el truss está asegurado en los soportes.

Marningi Using a single pick-point at the peak can damage the truss.

¡Advertencia! El uso de un solo lugar para levantar en el pico puede hacer daño al truss.







Top Chord Temporal

Lateral Restraint

(TCTLR)

TEMPORARY RESTRAINT & BRACING RESTRICCIÓN Y ARRIOSTRE TEMPORAL

A Refer to <u>BCSI-B2 Summary Sheet – Truss</u>, <u>Installation & Temporary Restraint/Bracing</u> for more information.

Vea el resumen <u>BCSI 82 – Restricción/</u> <u>Arriostre Temporal y Instalación de los</u> <u>Trusses</u> para más información.

Locate ground braces for first truss directly in line with all rows of top chord temporary lateral restraint (see table in the next column).

Coloque los arriostres de tierra para el primer truss directamente en linea con cada una de las filas de restricción lateral temporal de la cuerda superior (vea la table en la próxima columna).



O not walk on unbraced trusses.

No camine en trusses

sueltos.

Brace first truss securely before erection of addition trusses.

TARVISTENCTAT HOM RESUMEN DE LA GUVA DE BUENA PRÁCTICA PAR

ARCHES DE LE COURT E VE UNE LA CENTRALITÉ D'ALLEMENT D'ALLEMENT D'ALLEMENT D'ALLEMENT D'ALLEMENT D'ALLEMENT DE

TEPS TO SETTING TRUSSES AS MEDIDAS DE LA INSTALLACIÓN DE LOS TRUSSES

- 1) Install ground bracing. 2) Set first truss and attach securely to ground bracing. 3) Set next 4 trusses with short member temporary lateral restraint (see below). 4) Install top chord diagonal bracing (see below). 5) Install web member plane diagonal bracing to stabilize the first five trusses (see below). 6) Install bottom chord temporary lateral restraint and diagonal bracing (see below). 7) Repeat process on groups of four trusses until all trusses are set.
 - 1) Instale los arriostres de tierra. 2) Instale el primero truss y ate seguramente al arriostre de tierra. 3) Instale los próximos cuatro truses con restricción lateral temporal de miembro corto (vea abajo). 4) Instale el arriostre diagonal de la cuerda superior (vea abajo). 5) Instale arriostre diagonal para los planos de los miembros secundarios para estable los primeros cinco trusses (vea abajo). 6) Instale la restricción lateral temporal y arriostre diagonal para la cuerda inferior (vea abajo). 7) Repita éste procedimiento en grupos de cuatro trusses hasta que todos los trusses estén instalados.
- Refer to <u>BCSI-B2 Summary Sheet Truss Installation & Temporary Restraint/Bracing</u> for more information.

Vea el resúmen <u>BCSI-B2 - Instalación de Trusses v Arriostre Temporal</u> para mayor información.

ESTRAINT/BRACING FOR ALL PLANES OF TRUSSES

- RESTRICCIÓN/ARRIOSTRE EN TODOS PLANOS DE TRUSSES.
- This restraint & bracing method is for all trusses except 3x2 and 4x2 parallel chord trusses.

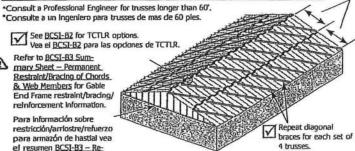
Este método de restricción y arriostre es para todo trusses excepto trusses de cuerdas paralelas 3x2 y 4x2.

) TOP CHORD - CUERDA SUPERIOR

stricción/Arriostre

Permanente de Cuerdas y

Truss Span Longitud de Tramo	Top Chord Temporary Lateral Restraint (TCTLR) Spacing Espaciamiento del Arriostre Temporal de la Cuerda Superio	
Up to 30'	10' o.c. max.	
Hasta 30 pies	10 ples máximo	
30' to 45'	8' o.c. max.	
30 a 45 pies	8 pies máximo	
45' to 60'	6' o.c. max.	
45 a 60 ples	6 ples máximo	
60° to 80°*	4' o.c. max.	
60 a 80 pies*	4 ples máximo	

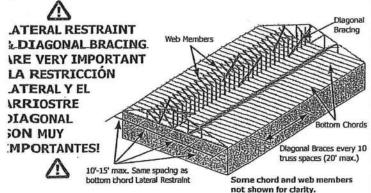


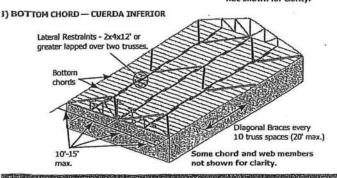
Ground bracing not shown for clarity.

Miembros Secundarios.

Repita los arrisotres diagonales para cada grupo de 4 trusses.

) WEB MEMBER PLANE - PLANO DE LOS MIEMBROS SECUNDARIOS

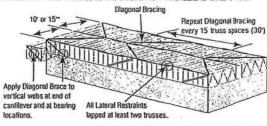




RESTRAINT & BRACING FOR 3x2 AND 4x2 PARALLEL CHORD TRUSSES LA RESTRICCIÓN Y EL ARRIOSTRE PARA TRUSSES DE CUERDAS PARALLELAS 3x2 Y 4X2

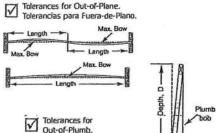
Refer to BCSI-B7 Summary Sheet - Temporary & Permanent Restraint/ Bracing for Parallel Chord Trusses for more information.

> Vea el resumen BCSI-B7 — Restricción y Arriostre Temporal y. Permanente de Trusses de Cuerdas Paralelas para más información.



*Top chord Temporary Lateral Restraint spacing shall be 10 o.c. max. for 3x2 chords and 15 o.c. for 4x2 chords.

INSTALLING — INSTALACION



Tolerancias para Fuera-de-Plomada.

	D/50	D (fL)
	1/4"	1"
	1/2"	2"
	3/4"	3,
	1"	4'
	1-1/4"	5
	1-1/2*	6'
	1-3/4*	7
	2*	≥8.

中央公司	源的部位原
Max. Bow	Truss Length
3/4"	12.5'
7/8*	14.6'
1"	15.7
1-1/8"	18.8*
1-1/4*	20.8
1-3/8:	22.9*
1-1/2"	25.0*
1-3/4"	29,2*
2-	≥33.3'
	Max. Bow 3/4* 7/8* 1* 1-1/8* 1-1/4* 1-1/2* 1-3/4*

CONSTRUCTION LOADING — CARGA DE CONSTRUCCION

⚠ Do not proceed with construction until all lateral restraint and bracing is securely and properly in place.

No proceda con la construcción hasta que todas las restricciones laterales y los arriostres estén colocados en forma aproplada y segura.

O Do not exceed maximum stack heights. Refer to <u>BCSI-B4</u>
Summary Sheet - Construction Loading for more information.

No exceda las máximas alturas recomendadas. Vea el resúme <u>BCSI-B4 Carga de Construcción</u> paja mayor información.

CASE OF THE PROPERTY OF THE PARTY OF THE PAR	TANKS BERNES ROYAL
Material	Helght
Gypsum Board	12"
Plywood or OSB	16*
Asphalt Shingles	2 bundles
Concrete Block	В*
Clay Tile	3-4 tiles high

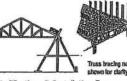
第二人员员





- Do not overload small groups or single trusses. No sobrecargue pequeños grupos o trusses Individuales.
- Never stack materials near a peak.

 Nunca amontone material cerca del pico.
- Place loads over as many trusses as possible.
 Coloque las cargas sobre tantos trusses como sea posible
- Position loads over load bearing walls.
 Coloque las cargas sobre las paredes soportantes.

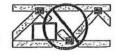


ALTERATIONS - ALTERACIONES

- Refer to BCSI-B5 Summary Sheet Truss Darnage, Jobsite Modifications & Installation Errors.

 Vea el resúmen BCSI-B5 Daños de trusses. Modificaciones en la Obra y Errores de Instalación.
- O Do not cut, alter, or drill any structural member of a truss unless specifically permitted by the Truss Design Drawing.

No corte, altere o perfore ningún miembro estructural de los trusses, a menos que esté específicamente permitido en el dibujo del diseño del truss.



Trusses that have been overloaded during coristruction or altered without the Truss Manufacturer's prior approval may render the Truss Manufacturer's limited warranty null and void.

Trusses que se han sobrecargado durante la construcción o han sido alterados sin una autorización previa del Fabricante de Trusses, pueden reducir o eliminar la garantia del Fabricante de Trusses.

NOTE: The Thuss Manufacturer and Truss Designer rely on the presumption that the Contractor and crane operator (If applicable) are professionals with the capability to undertake the work they have agreed to do on any given project. If the Contractor believes it needs assistance in some aspect of the construction project, it should seek assistance from a competent party. The methods and procedures outlined in this document are bitshed to ensure that the overeit construction beniques employed will put the trusses into piece SAFEIX. These recommendations for handling, installing, restraining and bracing trusses are based upon the collective experience of leading personnel involved with trust seelsing, manufacture and installation, but must, due to the nature of responsibility involved, be presented only as a GUIDE for use by a qualified building Designer or Contractor. It is not intended that these recommendations be interpreted as superior to the Building Designer's design specification for handling, installing, installing, and bracing trusses and it does not preclude the use of other equivalent methods for restraining/bracing and providing stability for the wells, puthurs, floors, roots and all the intervelbul structural building components as determined by the Contractor. Thus, VITCA and TPI expressly disclaim any responsibility for demages arising from the uses, application, or reflaces on the recommendations and information curbated herein.



6300 Enterprise Lane • Madison, WI 53719 608/274-4899 • www.sbcindustry.com



B1WARN11x17 20061115



MARONDA SYSTEMS

4005 Maronda Way

Sanford, FL 32771

(407) 321-0064

Fax (407) 321-3913

Date: November 1, 2006

To: Building Department

From: Maronda Systems

Tomas Ponce

Professional Engineer State of Florida #0050068

Subject: Valley Trusses

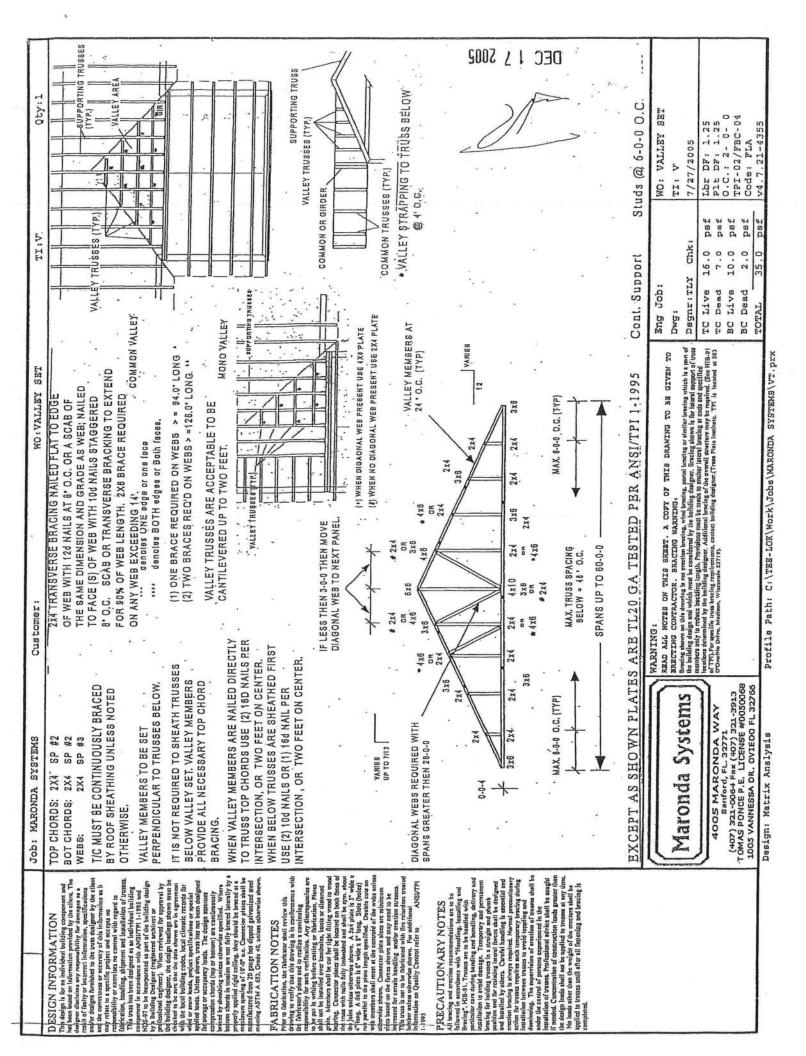
All valley trusses labeled V-1 through 100 are covered under the general valley sheet provided in the truss package signed and sealed by the engineer of record. The connections are noted on the structural info sheet of the plans. All criteria of the valley trusses are noted on the general sheet.

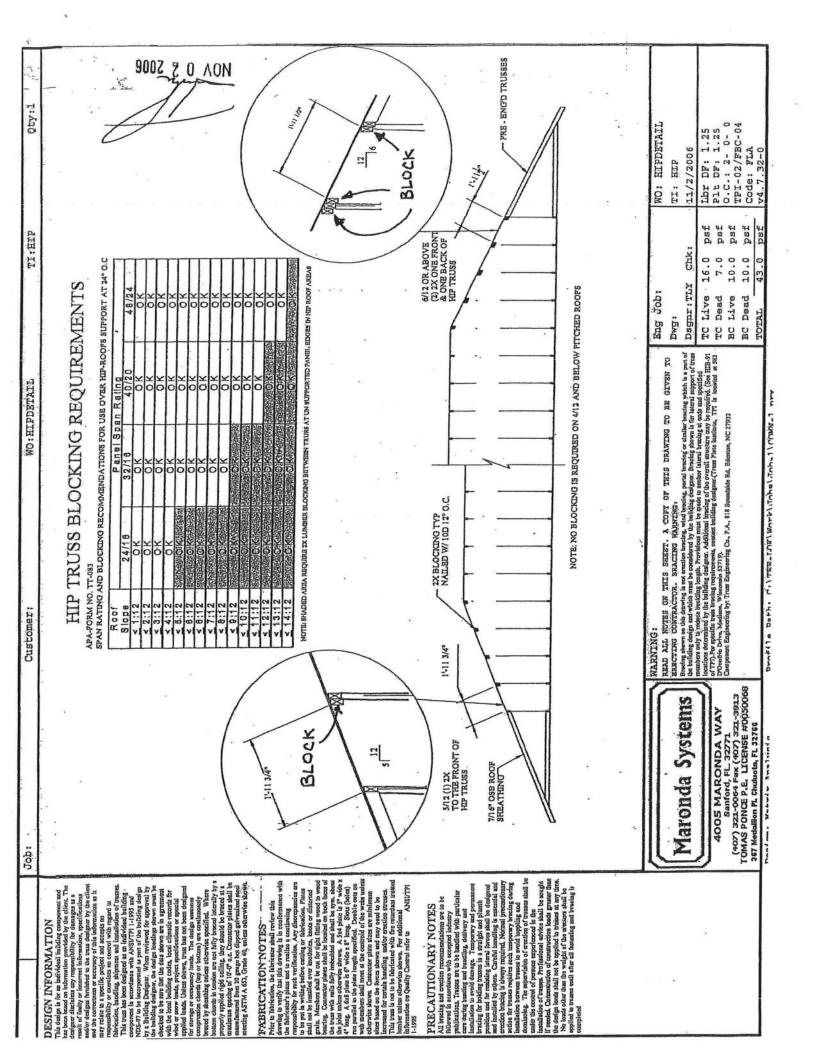
If you have any questions please feel free to call at 407-321-0064.

Sincerely,

Tomas Ponce, P.E.

Date: 11/1/06





Qty AUSTIN FL 125 russ Type Job ้านรร F4595120 COMMON AUSTIN 2 Job Reference (optional) s Nov 9 2007 MiTek Industries, Inc. Thu Jan 03 16:55:13 2008 Page 1 nda Homes Inc., Sanford, Florida 15-4-0 18-4-0 4-6-15 4-D-D 3-1-8 3-7-9 4x6 = Scale = 1:51.4 8x10 > 6.00 12 5x8 = 3x4 = 3x4 = 10 4x10 = 11 13 14 12 8x10 = 3x6 11 8x10 = 10x10 = 3x12 || 10-9-1 4-0-0 3-1-8 Plate Offsets (X,Y): [1:0-3-9.0-2-0]. [7:0-2-3.Edge]. [10:0-3-8.0-5-8]. [11:0-5-0.0-6-0] L/d PLATES GRIP DEFL in (loc) SPACING 2-0-0 LOADING (psf) Vert(LL) -0.15 10-11 >999 240 244/190 16.0 Plates Increase 1.25 TC 0.41 TCLL 1.25 BC 0.92 Vert(TL) -0.28 10-11 >779 180 Lumber Increase TCDL 7.0 NO WB 0.99 Horz(TL) 0.05 n/a n/a 10.0 Rep Stress Incr BCLL Weight: 302 lb Code FBC2004/TPI2002 (Matrix) BCDL 10.0 BRACING LUMBER Structural wood sheathing directly applied or 3-9-5 oc purlins, except TOP CHORD 2 X 4 SYP No.2 TOP CHORD BOT CHORD 2 X 8 SYP No.2 Rigid ceiling directly applied or 8-11-14 oc bracing. **BOT CHORD** 2 X 4 SYP No.2 WEBS 1 Row at midpt WEBS REACTIONS (lb/size) 1=5327/0-8-0, 8=8121/0-8-0 Max Horz 1=276(LC 5) Max Uplift1=2359(LC 5), 8=-3272(LC 5) FORCES (lb) - Maximum Compression/Maximum Tension 1-2=10711/4798, 2-3=-11019/5006, 3-4=-10999/5014, 4-5=-7773/3268, 5-6=-3111/1256, 6-7=-3096/1280, TOP CHORD

7-8=-7232/2967 BOT CHORD

1-12=-4494/9484, 11-12=-4494/9484, 11-13=-4647/9838, 10-13=-4647/9838, 10-14=-3018/6934, 14-15=-3018/6934,

9-15=3018/6934, 9-16=-18/43, 8-16=-18/43 2-12=381/241, 2-11=291/506, 4-11=-2226/4064, 4-10=-4003/2246, 5-10=-2926/6707, 5-9=-6389/2884,

6-9=1026/2606, 7-9=2625/6389

NOTES

WEBS

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:

Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

Bottom chords connected as follows: 2 X 8 - 2 rows at 0-3-0 oc. Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

3) Unbalanced roof live loads have been considered for this design.

4) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Calegory II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2359 lb uplift at joint 1 and 3272 lb uplift

7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 3454 lb down and 2152 lb up at 7-1-8, 1706 lb down and 813 lb up at 9-0-12, 1706 lb down and 631 lb up at 11-0-12, 1706 lb down and 645 lb up at 13-0-12, and 1706 lb down and 656 lb up at 15-0-12, and 1632 lb down and 601 lb up at 17-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

Continued on page 2



January 4,2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USB. Design vold for usery sessign parameters will assume that a control of this substitution and is for an individual building component.

Design vold for use only with Millek connection. This design is based only upon parameters shown, and is for an individual building component.

Applicability of design parameters and proper incorporation of component is responsibility of building designer, and state and proper incorporation of component is responsibility of building designer, for construction is the responsibility of the building construction is designer, and construction is the responsibility of the building designer, for general guidance regarding floatication, quality control, storage, delivery, erection and bracting, consult. ANSI/TP11 Quality Criteria, DS8-89 and BCS11 Building Component Salety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Modison, Wi 53719.



Job	Truss	Truss Type	Qty	Ply	AUSTIN_FL_125	E4595120
AUSTIN	GRD2S	соммон	1	2	Job Reference (optional)	

Maronda Homes Inc., Sanford, Florida

7,020 s Nov 9 2007 MiTek Industries, Inc. Thu Jan 03 16:55:13 2008 Page 2

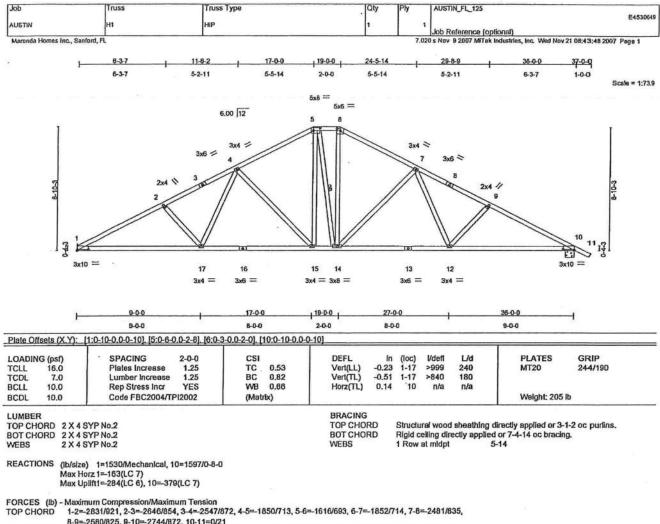
LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-6=-46, 6-7=-46, 1-8=-40
Concentrated Loads (lb)

Vert: 11=3454(F) 9=1706(F) 13=1706(F) 14=1706(F) 15=1706(F) 16=-1632(F)



818 Soundside Road



BOT CHORD

8-9=2580/825, 9-10=2744/872, 10-11=0/21 1-17=688/2491, 16-17=468/2060, 15-16=468/2060, 14-15=242/1612, 13-14=453/2030, 12-13=453/2030,

10-12=-630/2389

2-17=271/263, 4-17=105/640, 4-15=659/329, 5-15=174/634, 5-14=143/172, 6-14=169/618, 7-14=614/307,

7-12=-65/574, 9-12=-209/226

WEBS

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grlp DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) Refer to girder(s) for truss to truss connections.

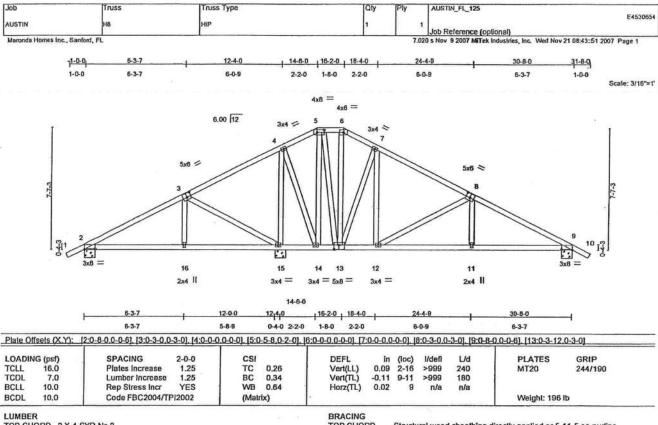
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 284 lb uplift at joint 1 and 379 lb uplift at Joint 10.

LOAD CASE(S) Standard

This document was originally issued by Lassiter, Frank on November 21,2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated November 21,2007 above.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEE REFERENCE PAGE MII-7473 BEFORE USE. AM MANUALY - VETUS design parameters and NEAL NOTES ON TAILS AND INCLUDED WHITE REFERENCE PAGE MILTAYS DEFORE USE. Design volid for use only with Miles connection. This design is bosed only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer—not inus designer. Bracing shown is tor fallers to support of Individual web membes only. Additional temporary bracing to insue solibility during construction is the responsibility of the vertex. Additional permanent bracing of the overal structure is the responsibility of the building designer. For general guidance regarding (abscicling, quality control, stronge, delivery, erection and bracing, consult. AMSUTPIT Quality Criteria, DSS-89 and BCS11 Building Component Salely Internation available from Trus Plote Institute, 583 D'Onotrio Drive, Madison, WI S3719.





TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.2

TOP CHORD **BOT CHORD**

Structural wood sheathing directly applied or 5-11-5 oc purlins. Rigid celling directly applied or 10-0-0 oc bracing, Except: 6-0-0 oc bracing: 14-15,13-14.

REACTIONS (lb/size) 2=407/0-8-0, 15=1579/0-8-0, 9=723/0-8-0

Max Horz 2=135(LC 7)

Max Uplift2=-310(LC 6), 15=-535(LC 6), 9=-224(LC 7) Max Grav2=436(LC 10), 15=1579(LC 1), 9=740(LC 11)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/21, 2-3=-392/406, 3-4=-137/401, 4-5=-13/105, 5-6=-101/127, 6-7=-157/140, 7-8=-436/128, 8-9=-1036/199,

9-10=0/21

2-16—228/297, 15-16—220/290, 14-15—308/389, 13-14—79/336, 12-13=0/331, 11-12—67/857, 9-11=-65/864 3-16=-317/272, 3-15=-618/692, 4-15=-1084/432, 4-14=-167/764, 5-14=-653/256, 5-13=-257/617, 6-13=-39/28, 7-13=-644/281, 7-12=-62/493, 8-12=-600/257, 8-11=0/274 **BOT CHORD**

NOTES

WEBS

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

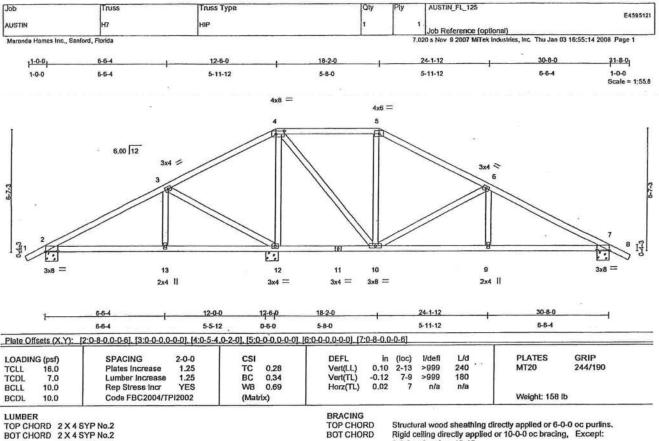
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 310 lb uplift at joint 2, 535 lb uplift at joint 15 and 224 lb uplift at joint 9.

LOAD CASE(S) Standard

This document was originally issued by Lassiter, Frank on November 21,2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated November 21,2007 above.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not Inus designer. Bracing show is for lateral support of Individual web membes only. Additional temporary bracing to insure building designer - not Inus designer. Bracing show is for lateral support of Individual web membes only. Additional temporary bracing to lateral building designer. For general guidance regarding (abracian, quality control, storage, desivery, execution and bracing, consult...) AMSI/TP1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Piale Institute, S83 D'Onotifo Drive, Modison, WI S3719.





BOT CHORD

6-0-0 oc bracing: 10-12.

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.2

REACTIONS (lb/size) 2=378/0-8-0, 12=1656/0-8-0, 7=675/0-8-0

Max Horz 2=119(LC 6)

Max Uplift2=323(LC 6), 12=508(LC 6), 7=-211(LC 7) Max Grav2=416(LC 10), 12=1656(LC 1), 7=701(LC 11)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

BOT CHORD

WEBS

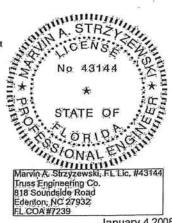
1-2=0/21, 2-3=-334/414, 3-4=-147/487, 4-5=-235/131, 5-6=-330/93, 6-7=-937/184, 7-8=0/21 2-13=-234/244, 12-13=-234/244, 11-12=-409/413, 10-11=-409/413, 9-10=-40/776, 7-9=-40/776 3-13=-337/277, 3-12=-633/721, 4-12=-1129/516, 4-10=-301/897, 5-10=-166/198, 6-10=-616/265, 6-9=0/280

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Calegory II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

- Provide adequate drainage to prevent water ponding.
 This truss has been designed for a 10.0 psf bollom chord live load nonconcurrent with any other live loads.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 323 lb uplift at joint 2, 508 lb uplift at joint 12 and 211 lb uplift at joint 7.

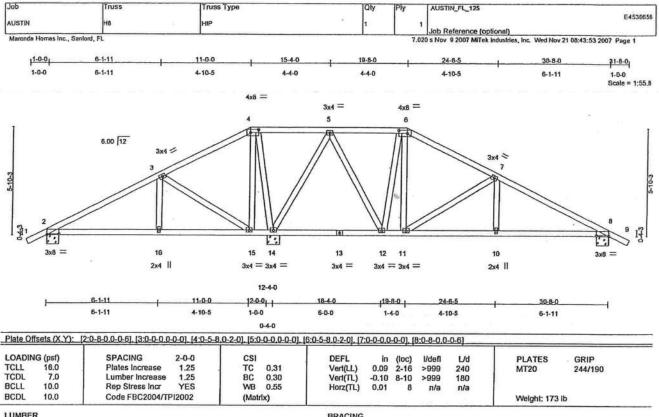
LOAD CASE(S) Standard



January 4,2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design volid for use only with Miles connectors. This design is broaded only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not invividual web members only. Additional temporary broading to insure structure in the responsibility of building designer - not invividual web members only. Additional temporary broading to insure storaging construction is the responsibility of the suiting designer. For general guidance regarding fobrication, quality control, storage, definery, exection and broading, consult. AMSI/IPIT Quality Criteria, DSB-89 and BCSIT Building Component Safety Intermation available from Trus Plate Institute, 583 D'Onotrio Drive, Madison. WI 53719.





LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 2 X 4 SYP No.2 BRACING

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=325/0-8-0, 14=1710/0-8-0, 8=674/0-8-0

Max Horz 2=108(LC 7)
Max Uplift2=321(LC 6), 14=471(LC 6), 8=209(LC 7)
Max Grav2=365(LC 10), 14=1710(LC 1), 8=697(LC 11)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/21, 2-3=-247/387, 3-4=-60/471, 4-5=-141/530, 5-6=-264/127, 6-7=-430/144, 7-8=-945/205, 8-9=0/21 2-16=215/169, 15-16=215/169, 14-15=374/292, 13-14=102/268, 12-13=102/268, 11-12=0/342, 10-11=45/784,

BOT CHORD

8-10=45/784

3-16=-296/263, 3-15=-560/609, 4-15=-497/294, 4-14=-700/715, 5-14=-886/381, 5-12=-173/655, 6-12=-377/220,

6-11=94/317, 7-11=531/215, 7-10=0/257

NOTES

WEBS

- Unbalanced roof live loads have been considered for this design.
 Wind: ASCE 7-02; 125mph (3-second gust); h=25fl; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

 3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

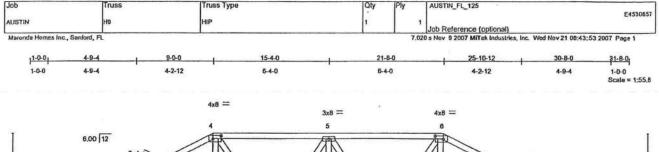
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 321 lb uplift at joint 2, 471 lb uplift at joint 14 and 209 lb uplift at Joint 8.

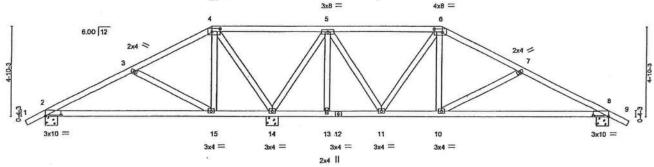
LOAD CASE(S) Standard

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	1	9-0-0		12-0-0	12,4,0	15-4-0	18-4-0	-	21-8-	0		30-8-0	
		9-0-0	114	3-0-0	0-4-0	3-0-0	3-0-0		3-4-0			9-0-0	
Plate Of	fsels (X.Y):	[2:0-10-0.0-0-10]. [3:0-0-0	0.0-0-0]. [4:0	-5-4.0-2-0].	[5:0-0-0	0.0-0-0]. [6:0-5-4.0-2-0].	[7:0-0-(0.0-0-0	[8:0-10	-0,0-0-10]		
LOADIN	IG (psf)	SPACING	2-0-0	csi	i		DEFL	in	(loc)	Vdefl	L/d	PLATES	GRIP
TCLL	16.0	Plates Increase	1.25	TC	0.34		Vert(LL)	0.35	2-15	>417	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.54		Vert(TL)	0.24	2-15	>605	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.48		Horz(TL)	0.02	8	n/a	n/a		
BCDL	10.0	Code FBC2004/TI	212002	(Ma	trix)							Weight: 162 lb)

LUMBER

Llob

Truss

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.2 BRACING

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid celling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=364/0-8-0, 14=1651/0-8-0, 8=694/0-8-0

Max Horz 2=35(LC 6) Max Uplift2=331(LC 6), 14=499(LC 5), 8=202(LC 7) Max Grav2=394(LC 10), 14=1651(LC 1), 8=712(LC 11)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD BOT CHORD 1-2=0/21, 2-3=-254/370, 3-4=-14/222, 4-5=-202/600, 5-8=-314/127, 6-7=-691/191, 7-8=-912/261, 8-9=0/21 2-15=-234/213, 14-15=-96/72, 13-14=-11/218, 12-13=-11/218, 11-12=-11/218, 10-11=0/591, 8-10=-122/789 3-15=-274/289, 4-15=-516/445, 4-14=-894/778, 5-14=-1070/452, 5-13=0/162, 5-11=-146/504, 6-11=-493/160,

6-10=0/435, 7-10=-239/186

NOTES

WEBS

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Calegory II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 331 lb uplift at joint 2, 499 lb uplift at joint 14 and 202 lb uplift at joint 8.

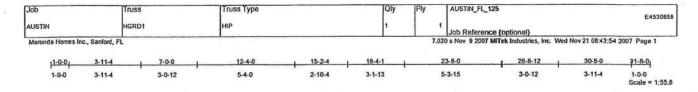
LOAD CASE(S) Standard

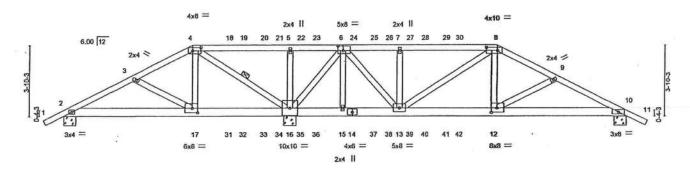
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MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with Miles connectors. This design is based only upon parameters above, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer—not inus designer. Bracing show is for later's support of individual web members only. Additional temporary bracing to Insure slopely during constitutions the responsibility of the exector. Additional permanent bracing of the overal structure is the responsibility of the building designer, for general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult. ANSI/TPI Quality Criteria, DSB-89 and 8CS11 Building Component Salety Intermation available from Trass Plate Institute, SS3 D'Chorifia Driva, MadSon, W1SS719.



enton, NC 27932





	_	7-0-0	1	6-9-0	19-6-4	10-9-1	_	23-0-0		30-0-0	
		7-0-0	. 5	4-0	2-10-4	3-1-13		5-3-15		7-0-0	
Plate Off	sets (X,Y):	[2:0-0-0,0-0-0], [3:0-0-0,0 .0-3-0], [17:0-3-8.0-3-0]	-0-0], [4:0-5-4	1,0-2-0], [5:0	-0-0,0-0-0], [6:	0-3-8,0-3-0], [7	:0-0-0,0-0-0], [1	3:0-7-0,0	-2-0], [9: 0 -0-	0,0-0-0], [10:0-0-0,0)-0-0], [12:0-3-8
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL	16.0	Plates Increase	1.25	TC	0.65	Vert(LL)	-0.06 12-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.44	Vert(TL)	-0.13 12-13	>999	180	1	
BCLL	10.0	Rep Stress Incr	NO	WB	0.64	Horz(TL)	-0.03 2	n/a	n/a		

BRACING TOP CHORD

WEBS

BOT CHORD

LUMBER

BCDL

TOP CHORD 2 X 4 SYP No.2 *Except*

4-6 2 X 4 SYP No.1D, 6-8 2 X 4 SYP No.1D

Code FBC2004/TPI2002

BOT CHORD 2 X 6 SYP No.2

10.0

WEBS 2 X 4 SYP No.2

REACTIONS (lb/size) 2=546/0-8-0, 16=3961/0-8-0, 10=1390/0-8-0

Max Horz 10=80(LC 7) Max Uplift2=-444(LC 5), 16=-2380(LC 8), 10=-672(LC 8) Max Grav2=603(LC 9), 16=3961(LC 1), 10=1398(LC 10)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/24, 2-3=-744/504, 3-4=-629/514, 4-18=-704/1391, 18-19=-704/1392, 19-20=-704/1392, 20-21=-705/1392, TOP CHORD

5-21=-705/1392, 5-22=-704/1392, 22-23=-704/1392, 6-23=-704/1392, 6-24=-1407/811, 24-25=-1407/811,

(Matrix)

25-26-1407/811, 7-26-1407/811, 7-27-1406/810, 27-28-1406/811, 28-29-1406/811, 29-30-1407/811,

8-30=-1407/811, 8-9=-2343/1236, 9-10=-2420/1181, 10-11=0/24

2-17--396/629, 17-31--409/595, 31-32--409/595, 32-33--409/595, 33-34--409/595, 16-34--409/595, 16-35-0/141, 35-36-0/141, 15-36-0/141, 14-15-0/141, 14-37-0/141, 37-38-0/141, 13-38-0/141, 13-38--1062/2136, **BOT CHORD**

39-40=-1062/2136, 40-41=-1062/2136, 41-42=-1062/2136, 12-42=-1062/2136, 10-12=-1008/2093

3-17=-112/89, 4-17=-494/1018, 4-16=-2242/1426, 5-16=-584/653, 6-16=-2222/1186, 6-15=0/105, 6-13=-1135/2025, 7-13=-563/653, 8-13=-884/449, 8-12=-267/1015, 9-12=-102/87

WEBS

1) Unbalanced roof live loads have been considered for this design.

Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise); porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60.
 Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 444 lb uplift at joint 2, 2380 lb uplift at joint 16 and 672 lb uplift at joint 10.

Continued on page 2

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Weight: 186 lb

Structural wood sheathing directly applied or 3-8-14 oc purlins.

4-16

Rigid ceiling directly applied or 7-7-10 oc bracing.

1 Row at midnt

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Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	AUSTIN_FL_125
	HGRD1				E4530658
AUSTIN	HGRD1	HIP	1	- 1	Job Reference (optional)

Maronda Homes Inc., Sanford, FL

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NOTES

6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 116 lb down and 194 lb up at 23-8-0, 116 lb down and 194 lb up at 21-7-4, 116 lb down and 194 lb up at 20-11-4, 116 lb down and 194 lb up at 18-11-4, 116 lb down and 194 lb up at 16-11-4, 116 lb down and 194 lb up at 14-11-4, 116 lb down and 194 lb up at 12-11-4, 116 lb down and 194 lb up at 12-11-4, 116 lb down and 194 lb up at 19-11-4, 116 lb

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

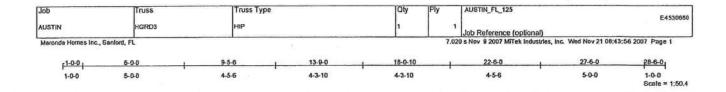
Uniform Loads (plf)

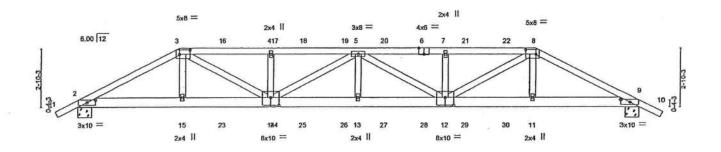
Vert: 1-4=-46, 4-8=-46, 8-11=-46, 2-10=-40

Concentrated Loads (lb)

Vert: 4=116(F) 8=116(F) 17=644(F) 6=116(F) 15=92(F) 12=644(F) 18=116(F) 20=116(F) 22=116(F) 25=116(F) 27=116(F) 29=116(F) 30=116(F) 31=92(F) 33=92(F) 37=92(F) 37=92(F) 41=92(F) 42=92(F) 42=92(F)







	-	5-0-0	9-5-0		13-3-0	10	-0-10	_		22.0.0	27-0-0	
		5-0-0	4-5-6		4-3-10	4	3-10			4-5-6	5-0-0	•
Plate Off	sets (X.Y):	[2:0-5-0.0-1-7], [3:0-6-0.0	2-8] [6:0-3-0	.Edge]. [8:0	6-0.0-2-8]. [9	9:0-5-0,0-1-7]. [1	2:0-5-0.	0-4-8].	[14:0-5-0	0.0-4-8]	,	
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	ln	(loc)	I/defl	L∕d	PLATES	GRIP
TCLL	16.0	Plates Increase	1.25	TC	0.56	Vert(LL)	0.34	13	>944	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.74	Vert(TL)	-0.60	13	>535	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.37	Horz(TL)	0.11	9	n/a	n/a		
BCDL	10.0	Code FBC2004/TF	12002	(Matr	ix)				138		Weight: 153 I	b

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 6 SYP No.1D

2 X 4 SYP No.2 WEBS

BRACING

TOP CHORD **BOT CHORD**

Structural wood sheathing directly applied or 2-5-13 oc purlins.

Rigid celling directly applied or 4-10-7 oc bracing.

REACTIONS (lb/size) 2=2087/0-8-0, 9=2108/0-8-0

Max Horz 2=65(LC 7)
Max Uplift2=1071(LC 7), 9=1091(LC 8)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/25, 2-3=-3979/2011, 3-16=-5219/2699, 16-17=-5218/2699, 4-17=-5218/2699, 4-18=-5218/2699, TOP CHORD

18-19=-5218/2699, 5-19=-5218/2699, 5-20=-5240/2718, 6-20=-5240/2718, 6-7=-5240/2718, 7-21=-5240/2717,

21-22-5240/2717, 8-22-5241/2718, 8-9-4027/2058, 9-10-0/25 2-15=-1749/3479, 15-23=-1746/3504, 23-24=-1746/3504, 14-24=-1746/3504, 14-25=-2952/5861, 25-26=-2952/5861, 13-27=-2952/5861, 27-28=-2952/5861, 12-29=-1750/3547, 29-30=-17 **BOT CHORD**

11-30=-1750/3547, 9-11=-1754/3522

3-15=0/415, 3-14=-1029/1992, 4-14=-354/388, 5-14=-745/396, 5-13=0/350, 5-12=-720/373, 7-12=-352/383,

8-12=1009/1967, 8-11=0/430

NOTES

WEBS

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60.

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1071 ib uplift at joint 2 and 1091 ib uplift

- 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 236 lb down and 306 lb up at 5-0-0, 69 lb down and 132 lb up at 7-0-12, 77 lb down and 145 lb up at 9-0-12, 77 lb down and 145 lb up at 11-0-12, 77 lb down and 145 lb up at 13-0-12, 77 lb down and 145 lb up at 13-0-12, 69 lb down and 132 lb up at 19-0-12, and 69 lb down and 132 lb up at 21-0-12, and 236 lb down and 306 lb up at 22-6-0 on top chord, and 162 lb down at 5-0-0, 57 lb down at 7-0-12, 57 lb down at 9-0-12, 57 lb down at 11-0-12, 57 lb down at 13-0-12, 57 lb down at 15-0-12, 57 lb down at 17-0-12, 57 lb down at 19-0-12, and 57 lb down at 21-0-12, and 162 lb down at 22-5-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

Continued on page 2

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MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE. A WARNING - Verty design parameters and READ MOLES ON THIS AND INCLUDED at LEAR REFIGERIOR SIDE AND ACCOUNTS. DESCRIPTION OF THE AND INCLUDED AND ACCOUNTS AND AC



nton, NC 27932

Job	Truss	Truss Type	Qty	Ply	AUSTIN_FL_125	
ALICTIN	UCDD1	uin.			E	E4530660
AUSTIN	nGKD3	HIP	ľ	1	Job Reference (optional)	

Maronda Homes Inc., Sanford, FL

7.020 s Nov 9 2007 MiTek Industries, Inc. Wed Nov 21 08:43:56 2007 Page 2

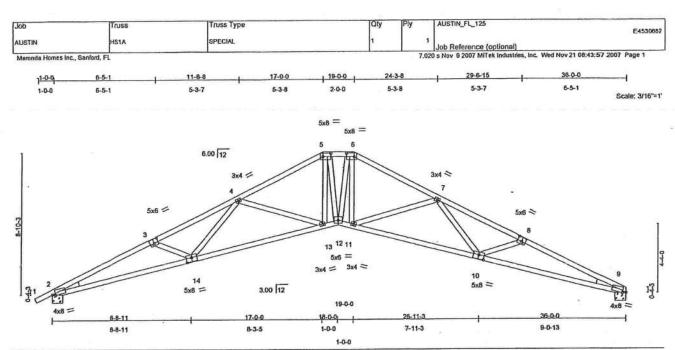
LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf) Vert: 1-3=-46, 3-8=-46, 8-10=-46, 2-9=-40

Concentrated Loads (lb)

Vert: 3=-196(F) 6=-77(F) 8=-196(F) 15=-162(F) 11=-162(F) 16=-69(F) 17=-77(F) 18=-77(F) 19=-77(F) 20=-77(F) 21=-69(F) 22=-69(F) 23=-57(F) 24=-57(F) 25=-57(F) 26=-57(F) 26=-57(F)





OADING (psf)	SPACING 2-0-0	CSI	DEFL	In (loc)	1/def1	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase 1.25	TC 0.88	Vert(LL)	-0.56 13-14	>762	240	MT20	244/190
CDL 7.0	Lumber Increase 1.25	BC 0.96	Vert(TL)	-1.13 13-14	>375	180	İ	
3CLL 10.0	Rep Stress Incr YES	WB 0.40	Horz(TL)	0.75 9	n/a	n/a	l	
3CDL 10.0	Code FBC2004/TPI2002	(Matrix)					Weight: 183	lb

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied.

Rigid ceiling directly applied or 2-2-0 oc bracing.

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 "Except"

2-14 2 X 4 SYP No.1D, 9-10 2 X 4 SYP No.1D

WEBS 2 X 4 SYP No.2

WEDGE

Left: 2 X 4 SYP No.3, Right: 2 X 4 SYP No.3

REACTIONS (lb/size) 9=1518/0-8-0, 2=1585/0-8-0

Max Horz 2=159(LC 6) Max Uplift9=280(LC 7), 2=362(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/18, 2-3=-4842/1457, 3-4=-4669/1334, 4-5=-3450/989, 5-6=-3172/985, 6-7=-3443/991, 7-8=-4648/1350, TOP CHORD

8-9=-4840/1507

2-14=1234/4384, 13-14=959/3887, 12-13=569/3082, 11-12=571/3084, 10-11=968/3899, 9-10=1284/4388. **BOT CHORD** 3-14=117/242, 4-14=84/680, 4-13=749/408, 5-13=97/897, 5-12=212/510, 6-12=225/537, 6-11=-104/861, WEBS

7-11=771/415, 7-10=-96/661, 8-10=-140/272

NOTES

 Unbalanced roof live loads have been considered for this design.
 Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

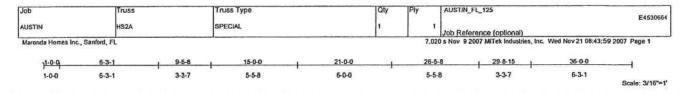
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 5) Bearing at joint(s) 9, 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 280 lb uplift at Joint 9 and 362 lb uplift at Joint 2.

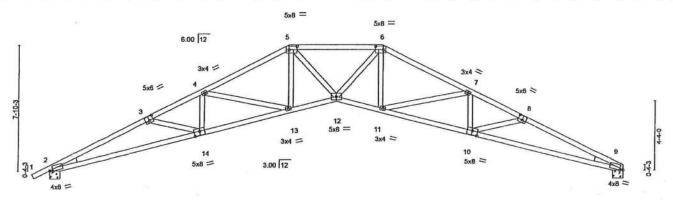
LOAD CASE(S) Standard

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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE. Design void for use only with Milek connection. This design is based only upon parameters that sub-FRACE s







LOADING (psf)	SPACING	2-0-0	CSI		DEFL	In	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC	0.89	Vert(LL)	-0.52	12	>814	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC	0.84	Vert(TL)	-1.03	12	>411	180	(0) (0) (1) (1) (1) (1) (1) (1) (1) (1) (1) (1	
BCLL 10.0	Rep Stress Incr	YES	WB	0.40	Horz(TL)	0.75	9	n/a	n/a		
BCDL 10.0	Code FBC2004/TF	12002	(Matr	ix)	1411 000 000					Weight: 178 lb	

3.0.0

BRACING

TOP CHORD

BOT-CHORD

5.5.8

Structural wood sheathing directly applied.

Rigid ceiling directly applied or 5-8-9 oc bracing.

9-6-8

3.0.0

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.1D WEBS 2 X 4 SYP No.2

WEDGE

Left: 2 X 4 SYP No.3, Right: 2 X 4 SYP No.3

REACTIONS (lb/size) 9=1518/0-8-0, 2=1585/0-8-0 Max Horz 2=143(LC 6)

Max Uplift9=-267(LC 7), 2=-348(LC 6)

FORCES (Ib) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/18, 2-3=-4769/1492, 3-4=-4607/1362, 4-5=-3753/1147, 5-6=-4143/1242, 6-7=-3755/1151, 7-8=-4619/1391,

0.6.8

2-14=-1264/4321, 13-14=-1068/4230, 12-13=-758/3438, 11-12=-762/3440, 10-11=-1091/4240, 9-10=-1311/4342 3-14=-91/197, 4-14=-0/386, 4-13=-802/303, 5-13=-104/513, 5-12=-226/1190, 6-12=-220/1188, 6-11=-108/515, **BOT CHORD**

7-11=-811/323, 7-10=-18/395, 8-10=-101/219

NOTES

WEBS

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

5.5.8

- Provide adequate drainage to prevent water ponding.
 This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 Bearing at joint(s) 9, 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 267 lb uplift at Joint 9 and 348 lb uplift at joint 2.

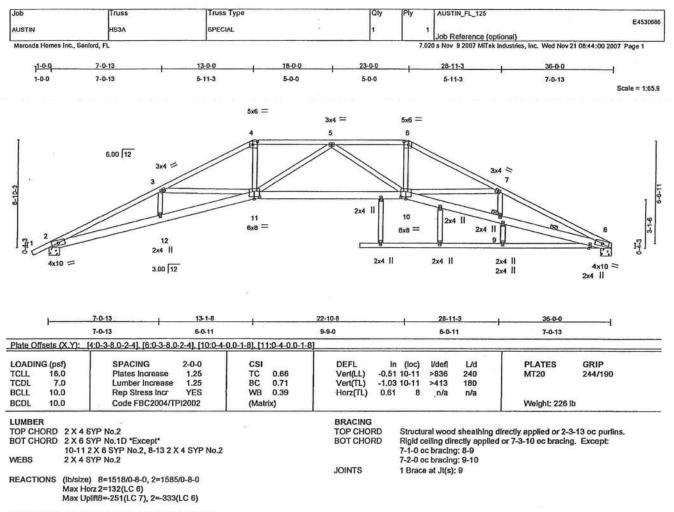
LOAD CASE(S) Standard

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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REPERENCE PAGE MIL-7473 BEFORE USE.

Design volid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual butding component. Applicability of design parameters and proper incorporation of component is responsibility of butding designer - not invisides shore. Bracing shown to roll to support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the execution, Additional permanent bracing of the overall structure is the responsibility of the butding designer. For general guidance regarding tablaciation, quality control, storage, delivery, exection and bracing, consult — MSU/IPII Quality Criteria, DSB-89 and 8CSI1 Butlating Component Safety Information available from Trus Plate Institute, 583 D'Onotrio Drive, Madison, WI 53719.





FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/22, 2-3=-5019/1526, 3-4=-4233/1280, 4-5=-3790/1214, 5-6=-3793/1221, 6-7=-4237/1288, 7-8=-5042/1580 2-12=-1299/4530, 11-12=-1300/4548, 10-11=-1051/3935, 9-10=-1349/4569, 8-9=-1353/4553 TOP CHORD

BOT CHORD WEBS

3-12=0/208, 3-11=-666/357, 4-11=-341/1603, 5-11=-327/240, 5-10=-325/237, 6-10=-346/1605, 7-10=-683/399,

7-9=0/209

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Inlenor(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Provide adequate drainage to prevent water ponding.

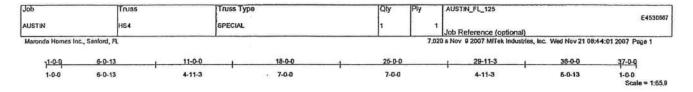
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) Bearing at joint(s) 8, 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify
- capacity of bearing surface.
 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 251 lb uplift at joint 8 and 333 lb uplift at

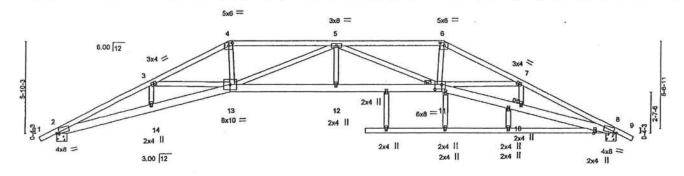
LOAD CASE(S) Standard

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MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE. Design volid for use only with Millex connectors. This design is based only upon parameters shown, and is for on individual building component, Applicability of design parameters and proper incorporation of component is responsibility of building designer - not trust designer. Roccing st for lateral support of individual web members only. Additional temporary brocking to insure slobely during construction is the responsibility of establishing designer. For general guidance regarding responsibility of the building designer, For general guidance regarding tobrication, quality control, storage, defivery, eraction and brocking, consult. AMSI/IPII Quality Criteria, DSB-89 and BCSIT Building Compone Safety Intermediate them Truss Plate Institute, 583 D'Orotho Drive, Madison, WI 53719.







		-0-13	1-1-8	1	8-0-0		4-10-8			29-11-3	36-0-0	
	6	-0-13 5-	0-11	6	-10-8		-10-8			5-0-11	5-0-13	
Plate Of	sets (X.Y):	[2:0-1-15.0-0-9], [4:0-3-0,	0-2-0]. [6:0-3	-0.0-2-0], [8:	0-1-15.0-0-9]. [11:0-5-4,0-3-8]						
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defi	L/d	PLATES	GRIP
TCLL	16.0	Plates Increase	1.25	TC	0.49	Vert(LL)	-0.50	12	>850	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.78	Vert(TL)	-0.99	12-13	>428	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.74	Horz(TL)	0.61	8	n/a	n/a		
BCDL	10.0	Code FBC2004/TF	P12002	(Matr	íx)					*********	Weight: 229 lb	ì

JOINTS

TOP CHORD

BOT CHORD

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 6 SYP No.1D *Except*

8-15 2 X 4 SYP No.2

2 X 4 SYP No.2 WEBS

REACTIONS (lb/size) 2=1584/0-8-0, 8=1584/0-8-0

Max Horz 2=108(LC 6) Max Uplift2=315(LC 6), 8=315(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/22, 2-3=-4994/1481, 3-4=-4488/1368, 4-5=-4049/1292, 5-6=-4049/1292, 6-7=-4488/1368, 7-8=-4994/1481, 3-4=-4488/1368, 4-5=-4049/1292, 5-6=-4049/1292, 6-7=-4488/1368, 7-8=-4994/1481, 3-4=-4488/1368, 4-5=-4049/1292, 5-6=-4049/1292, 6-7=-4488/1368, 7-8=-4994/1481, 3-4=-4488/1368, 4-5=-4049/1292, 5-6=-4049/1292, 6-7=-4488/1368, 7-8=-4994/1481, 3-4=-4488/1368, 4-5=-4049/1292, 5-6=-4049/1292, 6-7=-4488/1368, 7-8=-4994/1481, 3-4=-4488/1368, 4-5=-4049/1292, 5-6=-4049/1292, 6-7=-4488/1368, 7-8=-4994/1481, 3-4=-4488/1368, 4-5=-4049/1292, 5-6=-4049/1292, 6-7=-4488/1368, 7-8=-4994/1481, 3-4=-4488/1368, 4-5=-4049/1292, 5-6=-4049/1292, 6-7=-4488/1368, 7-8=-4994/1481, 3-4=-4488/1368, 3-8=-4994/1481, 3-4=-4488/1368, 3-8=-4994/1481, 3-4=-4488/1368, 3-8=-4994/1481, 3-4=-4488/1368, 3-8=-4994/1481, 3-4=-4488/1368, 3-8=-4994/1481, 3-4=-4488/1368, 3-8=-4049/1292, 3-TOP CHORD

8-9=0/22

BOT CHORD

2-14=-1208/4494, 13-14=-1214/4515, 12-13=-1248/4774, 11-12=-1248/4774, 10-11=-1214/4515, 8-10=-1208/4494 3-14=-0/167, 3-13=-391/249, 5-12=-0/282, 5-11=-930/298, 7-11=-391/261, 7-10=-0/167, 5-13=-930/300, 6-11=-364/1703,

4-13=-364/1703

NOTES

WEBS

- Unbalanced roof live loads have been considered for this design.
 Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 ps bottom chord live load nonconcurrent with any other live loads.

 5) Bearing at Joint(s) 2, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 315 lb uplift at joint 2 and 315 lb uplift at ioint 8.

LOAD CASE(S) Standard

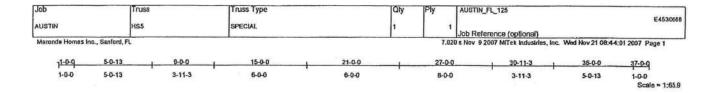
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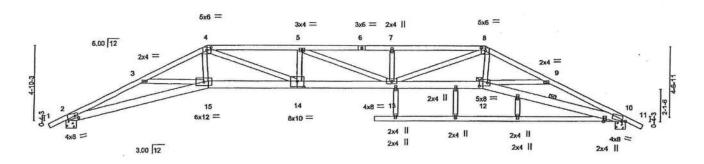
Structural wood sheathing directly applied or 2-6-0 oc purlins. Rigid celling directly applied or 7-6-2 oc bracing.

1 Brace at Jt(s): 11, 10

WARNING - Verify dealign parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIL-7473 BEFORE USB. Design volid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design programmers and proper incorporation of component is responsibility of building designer - not furs designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to building designer - not furs designer, Bracing shown is for lateral support of individual web members only. Additional temporary bracing to flause stability duting construction is the responsibility of the building designer. For general guidance regarding closivers, and in the control of the overal shucture is the responsibility of the building designer. For general guidance regarding tobication, quality control, storage, delivery, eraction and bracing, consult. ANSI/TPII Quality Criteria, DSB-89 and BCSII Building Component Safety Intermation available from Truss Plate Institute. 583 D'Onofrio Drive, Modeon, WI 63719.







_							1				
	9-1	1-8	*	5-10-8		6-0-0	5-10-	-6		9-1-8	
Plate Offsets	(X.Y): [2:0-2-4.0	0-2-0]. [4:0-3-0.0	2-0]. [8:0-3-	0.0-2-0]. [10:	0-2-11.0-0-	9]. [12:0-6-0.0-2-1	2]. [14:0-5-0.0-	4-8]			
LOADING (F	sf) S	PACING	2-0-0	CSI		DEFL	in (loc)	Vdefl	L/d	PLATES	GRIP
TCLL 1	3.0 P	lates Increase	1.25	TC	0.57	Vert(LL)	-0.64 13-14	>665	240	MT20	244/190
TCDL	7.0 L1	umber Increase	1.25	BC	0.69	Vert(TL)	-1.26 13-14	>335	180		
BCLL 1	0.0 R	ep Stress Incr	YES	WB	0.34	Horz(TL)	0.62 10	n/a	n/a	l	
BCDL 10	0.0 C	ode FBC2004/TF	12002	(Matr	ix)					Weight: 228 I	b

JOINTS

TOP CHORD

BOT CHORD

21.0.0

25,10.8

1 Brace at Jt(s): 12

Structural wood sheathing directly applied or 2-4-3 oc purlins. Rigid ceiling directly applied or 6-7-11 oc bracing. Except: 7-3-0 oc bracing: 10-12

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 6 SYP No.1D *Except*

10-16 2 X 4 SYP No.2

WEBS 2'X 4 SYP No.2

REACTIONS (lb/size) 2=1584/0-8-0, 10=1584/0-8-0

9.1.8

Max Horz 2=93(LC 7)

Max Uplift2=295(LC 6), 10=-295(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/22, 2-3=-4835/1610, 3-4=-4753/1444, 4-5=-5526/1846, 5-6=-5514/1843, 6-7=-5514/1843, 7-8=-5514/1843, TOP CHORD

8-9-4762/1448, 9-10-4839/1611, 10-11-0/22

BOT CHORD

2-15-1336/4371, 14-15=1088/4288, 13-14-1595/5541, 12-13-1091/4308, 10-12-1337/4375
3-15-37/270, 4-14-532/1417, 5-14-307/255, 5-13-115/71, 7-13-276/257, 8-13-523/1393, 9-12-44/277, WEBS

15.0.0

8-12=-230/1390, 4-15=-223/1372

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members end forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) Bearing at joint(s) 2, 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 295 lb uplift at joint 2 and 295 lb uplift at

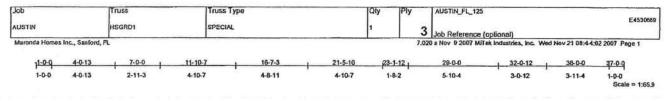
LOAD CASE(S) Standard

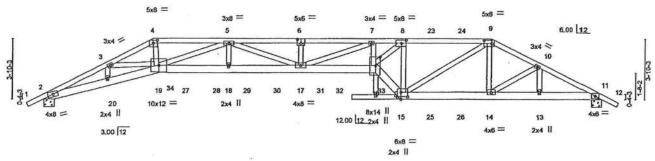
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WARNING - Yerlfy design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 BEFORE USE.

Design valid for use only with Milek connection. This design is based only upon parameters shown, and is for an included building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer—not huss designer, Roccing sh for talestot support of included were members only. Additional temporary brocing to insure shifty during construction is the responsibility of erector. Additional permanent brocking of the overall structure is the responsibility of the building designer. For general guidance regarding tobiccions, questly control, strange, delivery, erection and brocking, consult —AMSI/IPII Quality Criteria, DSB-89 and BCSII Building Componer Safety Information available from Trus Plaje Institute, SSB D'Onotrio Drive. Modison, WI S3719.







	4-3-1	1-4-8	11-10-/	10	1-3	21-5-10	23-1-12	59-1	PO	32-0-12	38-0-0	
	4-3-1	3-1-7	4-5-15	4-8	-11	4-10-7	1-8-2	5-10	14	3-0-12	3-11-4	
Plate Off	sels (X.Y): 1	4:0-3-0,0-2-0], [6:0-3-0,0	3-0]. [8:0-3-8	.0-2-8]. [9:0	6-0.0-2-8].	[15:0-1-12.0-2-12]. [15:0-5-4.0-3	-8], [16:0	-6-12.0-3-8	I		
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in (loc)	l/defi	L/d	PLATES	GRIP	
TCLL	16.0	Plates Increase	1.25	TC	0.46	Vert(LL)	0.75 16-17	>563	240	MT20	244/190	
TCDL	7.0	Lumber increase	1.25	BC	0.88	Vert(TL)	-1.26 16-17	>338	180	A SECURITION		
BCLL	10.0	Rep Stress Incr	NO	WB	0.79	Horz(TL)	-0.47 2	n/a	n/a	1		
BCDL	10.0	Code FBC2004/TF	212002	(Matr	ix)					Weight: 6	53 lb	

TOP CHORD

BOT CHORD

LUMBER

TOP CHORD 2 X 4 SYP No.1D

BOT CHORD 2 X 6 SYP No.2 *Except*

16-19 2 X 6 SYP No.1D, 15-21 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.2

REACTIONS (lb/size) 11=3534/0-8-0, 2=3592/0-8-0

Max Horz 11=-80(LC 6)
Max Uplift11=-1665(LC 8), 2=-1691(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

9-10=-6957/3391, 10-11=-6833/3194, 11-12=0/25, 4-5=-12589/6331, 5-6=-18762/9386, 6-7=-18762/9386, 7-8=-18144/9016, 8-23=-9223/4588, 23-24=-9223/4588, 9-24=-9223/4588, 1-2=0/22, 2-3=-11630/5630,

15-25=3021/6267, 25-26=-3021/6264, 14-26=-3020/6262, 13-14=-2612/5981, 11-13=-2812/5981, 15-16=-5882/11965, 19-27=-8362/16846, 27-28=-8362/16846, 18-28=-8362/16846, 18-29=-8362/16846, 29-30=-8362/16846, **BOT CHORD**

17-30=-8362/16846, 17-31=-8962/18164, 31-32=-8964/18168, 32-33=-8965/18171, 16-33=-8967/18173,

WEBS

2-20=-4992/10394, 20-34=-5040/10490, 19-34=-5156/10675 10-13=-118/155, 10-14=-311/381, 9-14=-408/1039, 9-15=-1790/3464, 8-15=-9174/4681, 8-16=-6409/12898, 7-16=-324/209, 7-17=-452/686, 6-17=-208/154, 6-17=-1025/2102, 5-18=-369/769, 5-19=-4643/2383, 4-19=-2772/5570,

3-19=-901/1444, 3-20=-282/241

NOTES

1) 3-ply truss to be connected together with 10d (0.131"x3") nails as follows:

Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

Bottom chords connected as follows: 2 X 6 - 2 rows at 0-6-0 oc. Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

2) All loads are considered equally applied to all piles, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

Unbalanced roof live loads have been considered for this design.

4) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Calegory II; Exp B; enclosed; MWFRS (low-rise);

Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.

7) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity on November 21,2007. This is not of heading surface. of bearing surface

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1665 lb uplift at joint 11 and 1691 lb uplift document. Official sealed at joint 2.

Continued on page 2

This document was originally issued by Lassiter, Frank considered a sealed

Structural wood sheathing directly applied or 5-6-9 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

drawings are available upon request from the manufacturer indicated November 21,2007 above.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIL-7473 BEFORE USE. A WARNING - Veryy design parameters and READ INCLUSE ON THIS AND INCLUDED HER REFERENCE PARES HER 1994 DESCRIPTION DESCRIPTION DESCRIPTION OF THE REPRESENCE PARES HER 1994 DESCRIPTION


Edenton, NC 27932

Job	Truss	Truss Type		Qly	Ply	AUSTIN_FL_125	
AUSTIN	HSGRD1	SPECIAL	2	1	2		E4530669
		111111111111111111111111111111111111111	Ç.		3	Job Reference (optional)	

7.020 s Nov 9 2007 MiTek Industries, Inc. Wed Nov 21 08:44:02 2007 Page 2

NOTES

9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 19 lb down and 72 lb up at 29-0-0, 19 lb down and 72 lb up at 27-0-12, and 19 lb down and 72 lb up at 25-0-12 and 19 lb down and 72 lb up at 25-0-12 and 19 lb down and 72 lb up at 25-0-12, and 19 lb down and 93 lb up at 25-0-12, 190 lb down and 93 lb up at 25-0-12, 190 lb down and 93 lb up at 25-0-12, 258 lb down and 185 lb up at 19-0-12, 258 lb down and 185 lb up at 17-0-12, 258 lb down and 185 lb up at 17-0-12, and 258 lb down and 185 lb up at 11-0-12, and 258 lb do 9-0-12, and 768 lb down and 519 lb up at 7-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

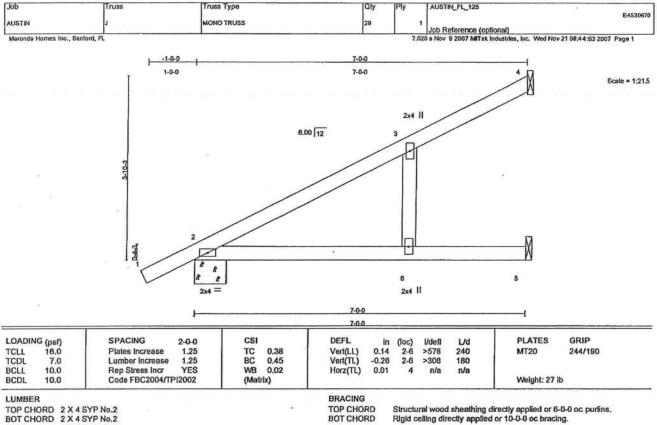
Uniform Loads (plf)

Vert: 9-12=46, 4-9=46, 1-4=46, 11-15=40, 15-16=40, 16-19=40, 2-19=40

Concentrated Loads (lb)

Vert: 9=.19(F) 15=.190(F) 14=.738(F) 8=.19(F) 23=.19(F) 24=.19(F) 25=.190(F) 26=.190(F) 27=.258(F) 28=.258(F) 29=.258(F) 30=.258(F) 31=.258(F) 32=.258(F) 33=.258(F) 34=.766(F)





2 X 4 SYP No.2

REACTIONS (lb/size) 4=136/Mechanical, 2=355/0-8-0, 5=141/Mechanical

Max Horz 2=178(LC 6)

Max Uplift4=64(LC 6), 2=-120(LC 6), 5=-34(LC 6)

FORCES (Ib) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/21, 2-3=-126/10, 3-4=-46/56 BOT CHORD 2-6=0/0, 5-6=0/0

WEBS

3-6=14/132

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) Refer to girder(s) for truss to truss connections.

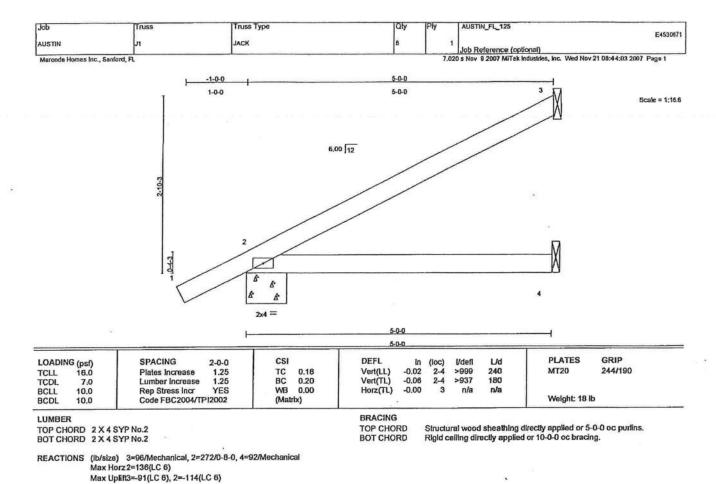
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 64 lb uplift at joint 4, 120 lb uplift at joint 2 and 34 lb uplift at joint 5.
- 5) Attach with (2) 16d Common Toe-Nalls (0.162"x3.5") at joints 4 and 5.

LOAD CASE(S) Standard

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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. AMARITING - Yer/fly destign parameters and READ NOTES ON THIS AND INCLUDED MITTER REFERENCE PAGE MIT-7473 BEFORE USE. Design volid for use only with Mitter connectors. This design is based only upon porometers show, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer, Bracing shown to relate the support of included web members only. Additional temporary bracing to insure stability during construction is the responsibility of the street. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding tobication, qualify control, storage, delevery, exaction and bracing, consult. AMSI/IPII Quality Criteria, DSS-89 and BCSII Building Component Safety Information available from Truss Plate Institute, SS3 D'Onofrio Drive, Modison, WI S3719.





FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/21, 2-3=-72/36

BOT CHORD 2-4=0/0

NOTES

- NOTES (5)

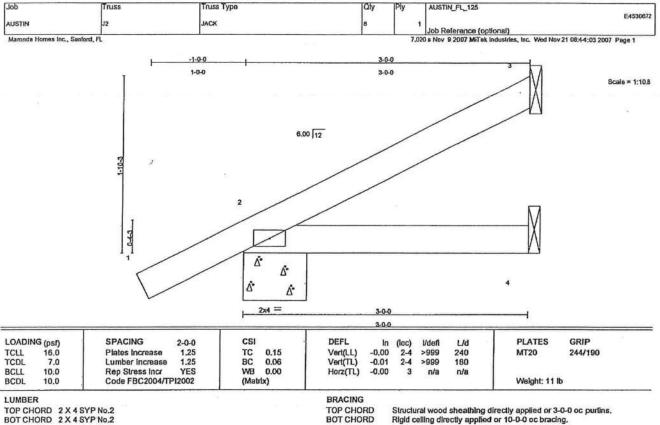
 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interfor(1) zone; Lumber DOL=1.60 plate grtp DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 3) Refer to girder(s) for truss to truss connections.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 91 ib uplift at joint 3 and 114 ib uplift at joint 2.
 5) Attach with (2) 16d Common Toe-Nalls (0.162"x3.5") at joints 3 and 4.

LOAD CASE(S) Standard

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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIL-1473 BEFORE USE. AN WARRING - Yer/fy design parameters and READ NOTES ON THIS AND INCLUDED MITTER REFERENCE PAGE MILTON'S BEFORE USE. Design void for use only with Milter connectors. This design is based only upon pormeters show, and is for on individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not have designer. Bracing shows the related support of inclinidation when members only. Additional temporary bracing to insure substituting designer - not insured in the responsibility of the sector. Additional paramental bracing of the overal structure is the responsibility of the building designer. For general guidance regarding (obtaction, quality control, strategy, delivery, execution and bracing, consult. AMSI/PHI Quality Criteria, DSS-89 and BCSI1 Building Component Safety Information available from Tuss Plate Institute, SSS D'Onotrio Drive, Madison, Wi SS719.





BOT CHORD

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

REACTIONS (lb/size) 3=43/Mechanical, 2=194/0-8-0, 4=52/Mechanical Max Horz 2=95(LC 6)

Max Uplift3=37(LC 6), 2=-113(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/21, 2-3=-39/15

BOT CHORD 2-4=0/0

NOTES

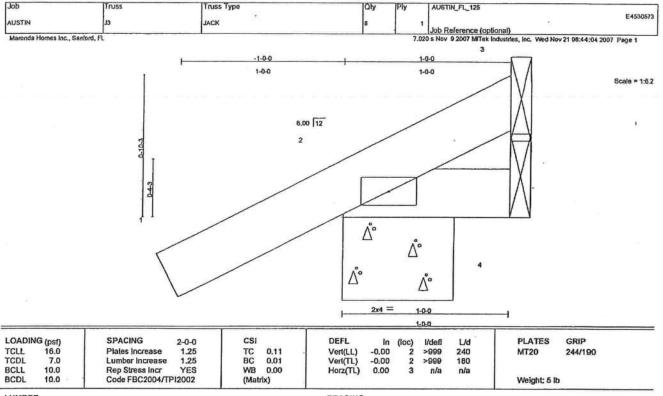
- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 3) Refer to girder(s) for truss to truss connections.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 37 lb uplift at joint 3 and 113 lb uplift at
- 5) Attach with (2) 16d Common Toe-Nalls (0.162"x3.5") at joints 3 and 4.

LOAD CASE(S) Standard

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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE. As warning - yerry gasing parameters and account of the solid in the control of the solid in the





LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 1-0-0 oc purlins. Rigid celling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/slze) 2=124/0-8-0, 4=18/Mechanical, 3=-11/Mechanical Max Horz 2=54(LC 6)

Max Uplift2=-116(LC 6), 3=-11(LC 1) Max Grav2=124(LC 1), 4=18(LC 1), 3=27(LC 6)

FORCES (Ib) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/21, 2-3=27/26 BOT CHORD 2-4=0/0

NOTES

- NOTES (6)

 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for
- MWFRS for reactions specified.

 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) Refer to girder(s) for truss to truss connections.

- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 116 ib uplift at Joint 2 and 11 ib uplift at
- 5) Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at Joints 3 and 4.

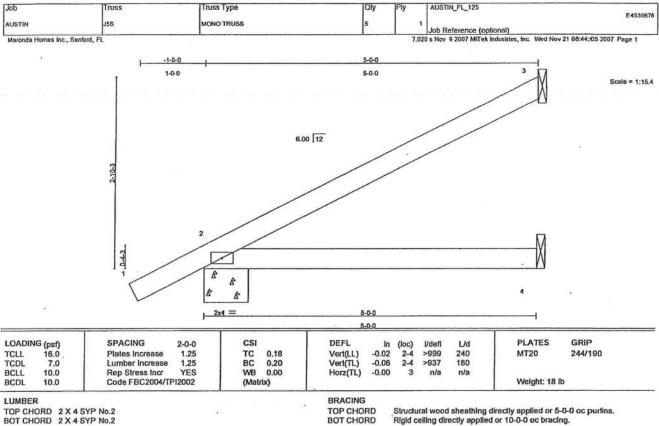
LOAD CASE(S) Standard

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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIT-7473 BEFORE USE. AMARTING - Verity design percenters and READ NOTES ON THIS AND INCLUDED BITER REPRESENCE YAS BULLY473 BISTORS USE.

Design valid for use only with Millex connection. This design is based only upon parameters shown and is for on individual bublishing component.
Applicability of design parameters and proper incorporation of component is responsibility of building designer - not trus designer. Bracing shown is for baleral support of Individual wab members only. Additional temporary bracing to Insure shows insulation is the responsibility of the sector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding tobrication, quality control, storage, delivery, section and bracing, commit. AMS/IPTH Quelly Criteria, DS8-89 and BCSI1 Building Component Salely Information available from Trus Pole Intitule, S83 D'Chatto Drive, Madison, WI 53719.





REACTIONS (lb/size) 3=96/Mechanical, 2=272/0-8-0, 4=92/Mechanical Max Horz 2=136(LC 6) Max Uplift3=91(LC 6), 2=114(LC 6)

FORCES (Ib) - Maximum Compression/Maximum Tension

1-2=0/21, 2-3=-72/36 2-4=0/0 TOP CHORD

BOT CHORD

NOTES

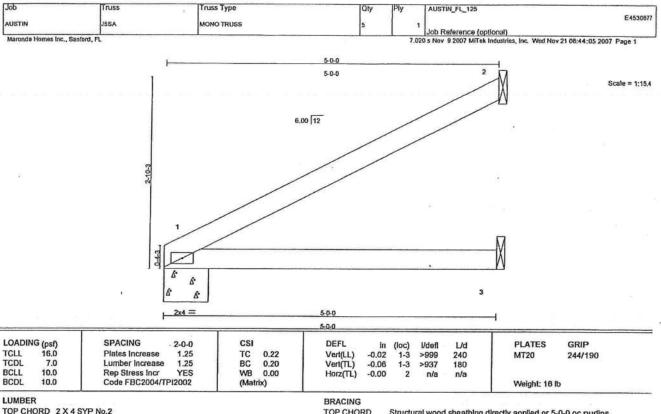
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 3) Refer to girder(s) for truss to truss connections.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 91 lb uplift at joint 3 and 114 lb uplift at
- 5) Attach with (2) 16d Common Toe-Nalls (0.162"x3.5") at joints 3 and 4.

LOAD CASE(S) Standard

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TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 TOP CHORD **BOT CHORD**

Structural wood sheathing directly applied or 5-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=198/0-8-0, 2=106/Mechanical, 3=92/Mechanical Max Horz 1=104(LC 6) Max Uplift1=20(LC 6), 2=104(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD

1-2=81/41 BOT CHORD

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Refer to girder(s) for truss to truss connections.
 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 20 ib uplift at joint 1 and 104 ib uplift at Joint 2, 5) Atlach with (2) 18d Common Toe-Nails (0.162"x3.5") at joints 2 and 3.

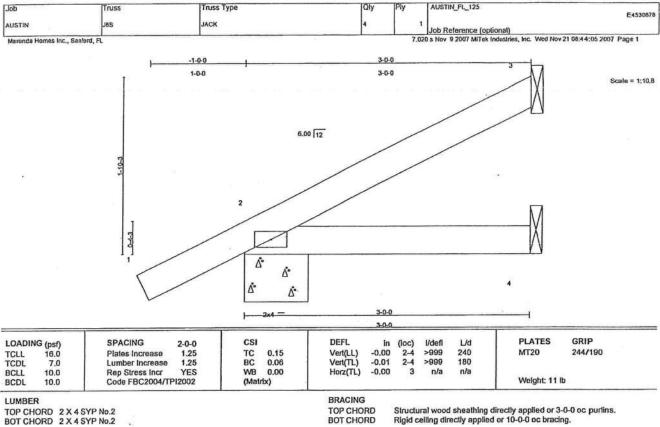
LOAD CASE(S) Standard

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Edenton, NC 27932



REACTIONS (lb/size) 3=43/Mechanical, 2=194/0-8-0, 4=52/Mechanical Max Horz 2=95(LC 6) Max Uplift3=37(LC 6), 2=-113(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/21, 2-3=-39/15

BOT CHORD 2-4=0/0

NOTES

- NOTES (5)

 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Calegory II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for
- MWFRS for reactions specified.

 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) Refer to girder(s) for truss to truss connections.

- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 37 lb uplift at Joint 3 and 113 lb uplift at
- 5) Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at Joints 3 and 4.

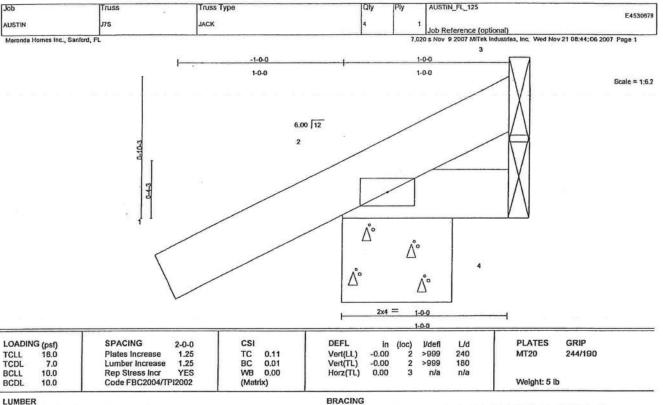
LOAD CASE(S) Standard

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Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not it use designer. Broching shown to raisolate paper of individual web members only. Additional temporary broching to insure stability during constantialculos is the responsibility of the erector. Additional permanent broching of the overall structure is the responsibility of the building designer. For general guidance regarding lobification, quality contino), storage, delivery, excellation and broching, consult. AMSUTPIT Quality Criteria, DSS-89 and BCSI1 Building Component Safety Information available from Truss Plate Institute, SS3 D'Onohin Drive, Madison, WI 53719.





TOP CHORD

BOT CHORD

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

REACTIONS (lb/size) 2=124/0-8-0, 4=18/Mechanical, 3=-11/Mechanical

Max Horz 2=54(LC 6)

Max Uplift2=-116(LC 6), 3=-11(LC 1)

Max Grav2=124(LC 1), 4=18(LC 1), 3=27(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/21, 2-3=-27/26 2-4=0/0 TOP CHORD BOT CHORD

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 116 lb uplift at joint 2 and 11 lb uplift at ioint 3.
- 5) Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at joints 3 and 4.

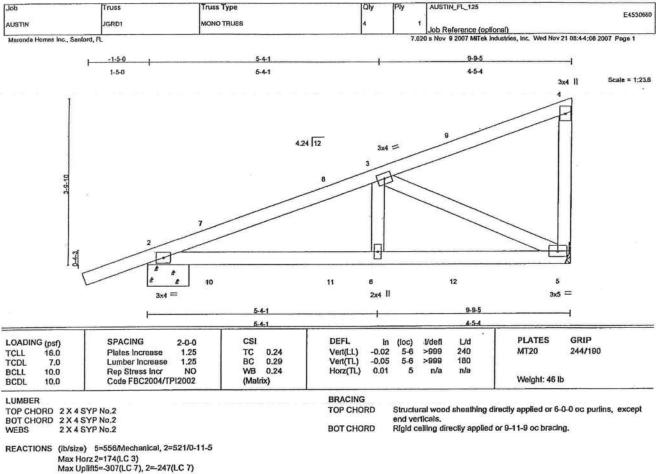
LOAD CASE(S) Standard

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Structural wood sheathing directly applied or 1-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with Mifek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not fluss designer, Bracing shows it for lateral support of individual web membes only. Additional temporary bracing to insure shorting the creation construction is the responsibility of the erector. Additional permanent bracing of the overal structure is the responsibility of the building designer, for general guidance regarding to building control storage, defivery, erection and bracing, consult. AMSU/PIT Quality Criteria, DSB-89 and BCSIT Building Component Safety Intermetien available from Truss Plate Institute, 583 D'Onotifo Drive, Modison, WI 53719.





FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1.2=0/21, 2-7=818/291, 7-8=-780/309, 3-8=-745/296, 3-9=-116/64, 4-9=-48/0, 4-5=-126/155 2-10=-392/728, 10-11=-392/728, 8-11=-392/728, 6-12=-392/728, 5-12=-392/728

BOT CHORD

3-6=0/282, 3-5=-737/387 WEBS

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 307 lb uplift at joint 5 and 247 lb uplift at joint 2.
- 5) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 43 lb up at 4-4-0, 43 lb up at 4-4-0, 43 lb up at 4-4-0, 59 lb down and 122 lb up at 7-1-15, 59 lb down and 122 lb up at 7-1-15, and 48 lb down at 1-6-1, 22 lb up at 1-6-1, 22 lb up at 1-6-1, 12 lb down at 4-4-0, 12 lb down at 4-4-0, and 52 lb down at 7-1-15, and 52 lb down at 7-1-15 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=-46, 2-5=-40

Concentrated Loads (lb)

Vert: 8=8(F=4, B=4) 9=-118(F=-59, B=-59) 10=43(F=22, B=22) 11=-24(F=-12, B=-12) 12=-104(F=-52, B=-52)

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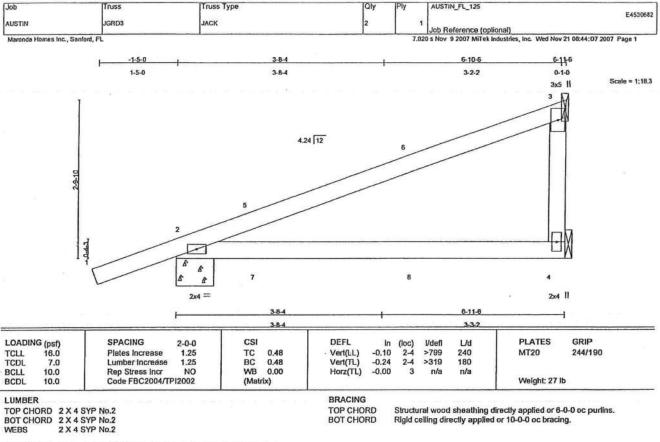
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE. AM WARKING - PETITY design parameters and READ TOTES ON THIS BATH INCLUDED MITTER REFERENCE PAGE BILL-1973 BEFORE USE.

Design void for use only with Mitek connector. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not inus designer. Broating is for lateral support of individual web members only. Additional temporary broating to insure stability during construction is the responsibility of learning to designer, for general guidance regarding erector. Additional permanent broating of the overall shucture is the responsibility of the building designer, for general guidance regarding to place for quality critical, possibly during control, storage, delivery, erection and broating, consult.

ANS/IPPI Quality Critical, DSB-89 and BCSII Building Component Safety Intermettion available from Tress Plate Institute, 583 D'Onotrio Drive, Macésar, WI 53719.



Edenton, NC 27932



REACTIONS (lb/size) 2=358/0-8-0, 4=142/Mechanical, 3=149/Mechanical Max Horz 2=132(LC 3) Max Uplift2=-170(LC 7), 3=-164(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/21, 2-5=-79/0, 5-6=-34/0, 3-6=-66/42 BOT CHORD 2-7=0/0, 7-8=0/0, 4-8=0/0

WEBS 3-4=0/0

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Calegory II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60.
- 2) This truss has been designed for a 10.0 psf boltom chord live load nonconcurrent with any other live loads.

3) Refer to girder(s) for truss to truss connections.

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 170 lb uplift at joint 2 and 164 lb uplift at

- 5) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.
 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 9 lb down and 57 lb up at 4-4-0, 9 lb down and 57 lb up at 4-4-0, and 47 lb down at 1-6-1, and 47 lb down at 1-6-1 on top chord, and 21 lb up at 1-6-1, 21 lb up at 1-6-1, and 17 lb down at 4-4-0, and 17 lb down at 4-4-0 on bottom chord. The design/selection of such connection device(s) is the
- responsibility of others.

 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

8) Attach with (2) 16d Common Toe-Nalls (0,162"x3.5") at joints 3 and 4.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf) Vert: 1-3=48, 2-4=40

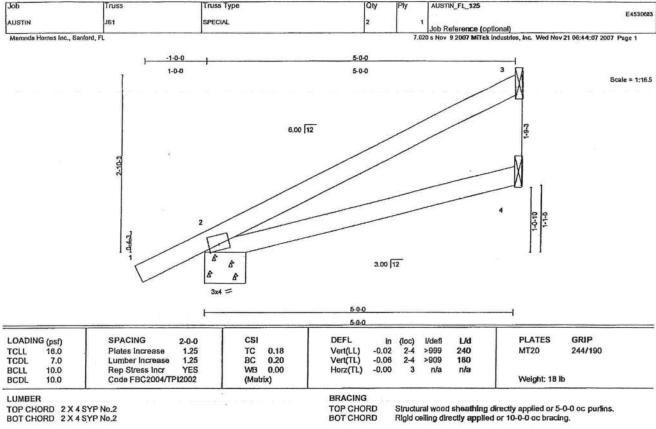
Concentrated Loads (lb)

Vert; 6=-19(F=-9, B=-9) 7=41(F=21, B=21) 8=-33(F=-17, B=-17)

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MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE. Design vold for use only with Milek connectors. This design is based only upon parameters hown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not inviside signer. Bracing shown is for lateral support of individual web members only. Additional temporary brocking to Insure slittly during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding labsication, quality control. storage, delevery, erection and bracing, consult. AMSI/IPII Quality Criteria, DSS-89 and BCSI1 Building Component Sately Information.





REACTIONS (lb/size) 3=96/Mechanical, 2=272/0-8-0, 4=92/Mechanical Max Horz 2=134(LC 6)

Max Uplift3=93(LC 6), 2=-111(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/18, 2-3=-75/37

BOT CHORD 2-4=-18/18

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

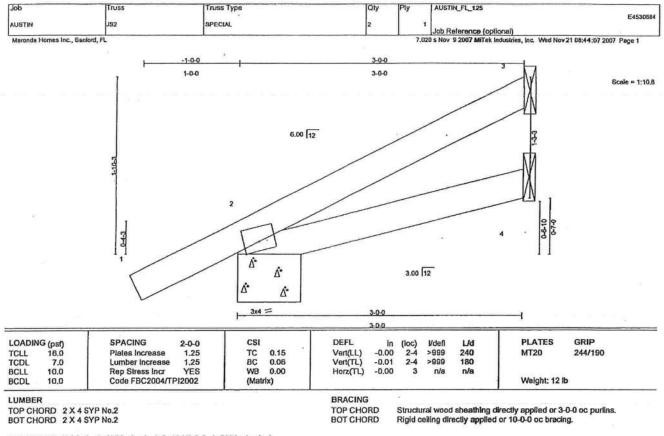
- 3) Refer to girder(s) for truss to truss connections.
 4) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 93 lb uplift at joint 3 and 111 lb uplift at
- 6) Attach with (2) 16d Common Toe-Nalls (0.162"x3.5") at joints 3 and 4.

LOAD CASE(S) Standard

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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 REFORE USE. Design volid for use only with Mijek connectors. This design is broad only upon porameters are marked to the connectors. This design is broad only upon porameters shown, and is for an individual building component. Applicability of design porameters and proper incorporation of component is responsibility of building designer - not trust designer. Enough shown is for laters support of individual web members only. Additional temporary bracting to insure stability during construction is the responsibility of the building designer, for general guidence reporting to include the property stability of the building designer, for general guidence regarding (obtained) and produce of the overall shadows and shall produce the stability during construction. Shall produce the produce of the overall shall produce on the stability of the building designer, for general guidence regarding (obtained) and produce of the constitution of the stability during constitution. Shall produce the stability during component safety intermediate over the stability of the stability of the stability during component safety.





REACTIONS (lb/size) 3=43/Mechanical, 2=194/0-8-0, 4=52/Mechanical Max Horz 2=93(LC 6) Max Uplift3=40(LC 6), 2=-110(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/18, 2-3=-40/16

BOT CHORD 2-4=-10/10

NOTES (6)

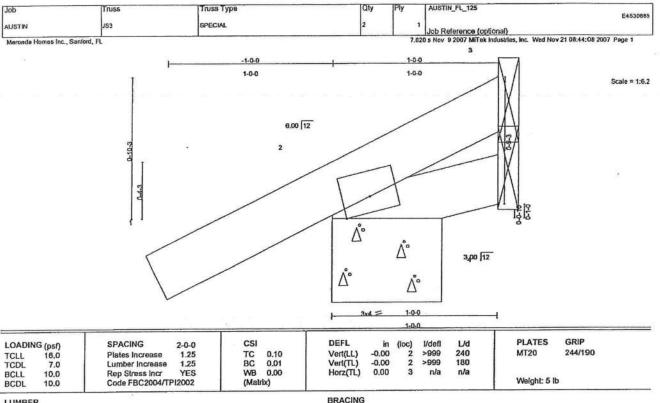
- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 40 lb uplift at joint 3 and 110 lb uplift at
- 6) Attach with (2) 16d Common Toe-Nalls (0.162"x3.5") at joints 3 and 4.

LOAD CASE(S) Standard

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LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 1-0-0 oc purlins, Rigid celling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=124/0-8-0, 4=18/Mechanical, 3=-11/Mechanical Max Horz 2=51(LC 6) Max Uplift2=108(LC 6), 3=11(LC 1) Max Grav2=124(LC 1), 4=18(LC 1), 3=18(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/15, 2-3=-32/24

2-4-4/4 BOT CHORD

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) Refer to girder(s) for truss to truss connections.

- 4) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 108 lb uplift at joint 2 and 11 lb uplift at
- 6) Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at joints 3 and 4.

LOAD CASE(S) Standard

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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REPERENCE PAGE MII-7473 BEFORE USE. AN WARRING - Verify design perameters and READ DIVISS ON THIS AND INCLUDED BRITCH REPLECENCE PAGE BUT-4743 BEFORE USE.

Design vold for use only with Millek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsiblely of building designer - not trust designar. Bracking shown is for falteral support of individual web members only. Additional temporary bracking to insure stability during construction is the responsibility of the execution permanent bracking of the overall shucture is the responsibility of the building designer. For general guidance regarding tobiocation, quality control, storage, delivery, erection and bracking. ARSUTPTI Quality Criteria, DSS-89 and BCS11 Building Component Selety information available from Trust Plate Institute, 593 D'Onosito Drive, Madden, Wt 52719.



Truss Type Job AUSTIN_FL_125 E4530688 ALISTIN ISA SPECIAL Job Reference (optional) 7.020 s Nov 9 2007 MiTek Industries, Inc. Wed Nov 21 08:44:08 2007 Page 1 -1-0-0 7-0-0 1-0-0 7-0-0 Scale: 1/2"=1" 3x5 || 6.00 12 2x4 = Æ 3.00 12 3x4 = 6-11-5 7-0-0 0-0-11 B-11-5 SPACING LOADING (psf) DEFL PLATES GRIP 2-0-0 In (loc) 2-4 l/defl L/d 16.0 Plates Increase 1.25 -0.10 >754 240 TCLL TC 0.49 Vert(LL) MT20 244/190 BC WB TCDL 7.0 Lumber Increase 1.25 0.45 Vert(TL) -0.262-4 >302 180 0.00 BCLL 10.0 Rep Stress Incr YES Horz(TL) -0.00n/a n/a Code FBC2004/TPI2002 Weight: 27 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. BOT CHORD 2 X 4 SYP No.2 **BOT CHORD** Rigid ceiling directly applied or 6-0-0 oc bracing. 2 X 4 SYP No.2 REACTIONS (lb/slze) 2=338/0-8-0, 3=155/Mechanical, 4=143/Mechanical Max Horz 2=174(LC 6) Max Uplift2=122(LC 8), 3=191(LC 8) FORCES (Ib) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/18, 2-5=-120/0, 5-6=-93/0, 3-6=-101/60 BOT CHORD 2-7=-21/0, 7-8=-0/0, 4-8=0/28

WEBS 3-4=0/0

- 1) Vinid: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) Refer to girder(s) for truss to truss connections.

- 4) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 122 ib uplift at joint 2 and 191 ib uplift at
- 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 9 to down and 57 lb up at 4-4-0. 9 lb down and 57 lb up at 4-4-0, and 47 lb down at 1-8-1, and 47 lb down at 1-6-1 on top chord, and 21 lb up at 1-6-1, 21 lb up at 1-6-1, and 17 lb down at 4-4-0, and 17 lb down at 4-4-0 on bottom chord. The design/selection of such connection device(s) is the
- responsibility of others.

 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

8) Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at Joints 3 and 4.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf) Vert: 1-3=46, 2-4=-40

Concentrated Loads (lb) Vert: 6=-19(F=-9, B=-9) 7=41(F=21, B=21) 8=-33(F=-17, B=-17)

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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEE REFERENCE PAGE MII-7473 REFORE USE. Design valid for use only with Milex connectors. This design is based only upon porometer shown, and it for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not inus designer. Bracing shown is for lateral support of Individual web members only. Additional temporary bracing to have shotly during continuction is the responsibility of the erection. Additional permanent bracing of the overall structure is the responsibility of the building designer, for general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult. AMSI/IPTI Geality Citterla, DSB-89 and BCSII Building Component Safety Information.



AUSTIN FL 125 Job Truss Truss Type F4530887 SPECIAL AUSTIN JSGRD1 Job Reference (optional) 7.020 s Nov 9 2007 MiTek Industries, Inc. Wed Nov 21 08:44:09 2007 Page 1 Maronda Homes Inc., Sanford, FI 9-9-5 5-4-4 1-5-0 4-5-1 Scale = 1:23.0 5x8 = 4.24 12 3x4 = 11 043 2x4 || 2.12 12 10 3x4 4-5-1 4-5-1 Plate Offsets (X.Y): [5:0-3-0,0-0-1] PLATES GRIP LOADING (psf) SPACING CSI DEFL 2-0-0 TC BC 244/190 TCLL Plates Increase 1.25 0.41 Vert(LL) -0.05 5-6 >999 240 MT20 180 -0.11 5-8 >999 TCDL 7.0 Lumber Increase 1.25 0.48 Vert(TL) NO WB 0.30 Horz(TL) 10.0 Rep Stress Incr BCLL Weight: 46 lb Code FBC2004/TPI2002 BCDL 10.0 (Matrix) BRACING LUMBER TOP CHORD Structural wood sheathing directly applied or 5-4-11 oc purlins, except TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 end verticals. BOT CHORD Rigid ceiling directly applied or 7-4-11 oc bracing. WEBS 2 X 4 SYP No.2 OTHERS 2 X 6 SYP No.2 REACTIONS (lb/size) 2=522/0-10-9, 7=516/Mechanical Max Horz 2=165(LC 7) Max Uplift2=-260(LC 7), 7=-305(LC 7) FORCES (Ib) - Maximum Compression/Maximum Tension 1-2=0/17, 2-8=-1374/611, 3-8=-1337/637, 3-9=-445/261, 4-9=-364/188, 4-5=-132/439 2-10=-698/1253, 6-10=-688/1268, 8-11=-711/1261, 5-11=-701/1300 TOP CHORD BOT CHORD 3-8=0/199, 3-5=-854/425, 4-7=-557/355, 5-7=-217/218 WEBS NOTES 1) Wind: ASCE 7-02; 125mph (3-second gusl); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Calegory II; Exp B; enclosed; MWFRS (low-rise);

- Lumber DOL=1.60 plate grip DOL=1.60.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Refer to girder(s) for truss to truss connections.
 4) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 260 lb uplift at joint 2 and 305 lb uplift at
- 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 47 lb up at 4-4-0, 47 lb up at 4.4-0, 59 lb down and 128 lb up at 7-1-15, 59 lb down and 126 lb up at 7-1-15, and 44 lb down at 1-6-1 on top chord, and 22 lb up at 1-6-1, 22 lb up at 1-6-1, 12 lb down at 4.4-0, 12 lb down at 4.4-0, and 52 lb down at 7-1-15, and 52 lb down at 7-1-15 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-4=-46, 2-5=-40

Concentrated Loads (lb) Vert: 3=8(F=4, B=4) 6=-24(F=-12, B=-12) 9=-118(F=-59, B=-59) 10=43(F=22, B=22) 11=-104(F=-52, B=-52)

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Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building componer Applicability of design parameters and proper incorporation of componers it responsibility of building designer—not fixes designer. Bracing is for follered support of Individual was membas only. Additional temporary broading to insure shally during constitution is the responsibility of the building designer. For general guidance responsibility of the building designer, For general guidance regarding flobrication, quasity control, storage, delivery, rescion and broading, consult.

MASI/FIT Quality Citierla, DSS-89 and SCS11 Building Componitation available from Trus Flate Institute, 563 D'Onolifo Drive, Madison, Wi 537 19.



Job Truss Truss Type AUSTIN_FL_125 E4530688 MONO TRUSS AUSTIN Job Reference (optional) 7.020 s Nov 9 2007 MiTek Industries, Inc. Wed Nov 21 08:44:09 2007 Page 1 Maronda Homes Inc., Sanford, F 1-0-0 1-0-0 6-3-8 6-0-8 6.00 12 3x4 = 2x4 11 3v4 = 6-3-8 12-4-0 Plate Offsets (X.Y): [2:0-2-10.0-1-8] SPACING PLATES LOADING (psf) 2-0-0 CSI DEFL GRIP in (loc) I/defi L/d 16.0 Plates Increase 1.25 TC 0.25 0.01 120 244/190 TCLL Vert(LL) MT20 n/r TCDL 7.0 Lumber Increase 1.25 BC 0.34 Vert(TL) 0.04 120 BCLL 10.0 Rep Stress Incr YES WB 0.05 Horz(TL) -0.00 5 n/a n/a Code FBC2004/TPI2002 BCDL 10.0 (Matrix) Weight: 63 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing.

WEBS

1 Row at midpt

2 X 4 SYP No.2

REACTIONS (lb/size) 5=215/12-4-0, 2=288/12-4-0, 6=594/12-4-0 Max Horz 2=286(LC 6)

Max Uplift5=117(LC 6), 2=61(LC 6), 6=140(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/21, 2-3=-151/28, 3-4=-93/28, 4-5=-105/126

BOT CHORD 2-6=-98/43, 5-6=-98/43

3-6=-303/255, 3-5=-25/99 WEBS

- Mind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) Gable requires continuous bottom chord bearing.

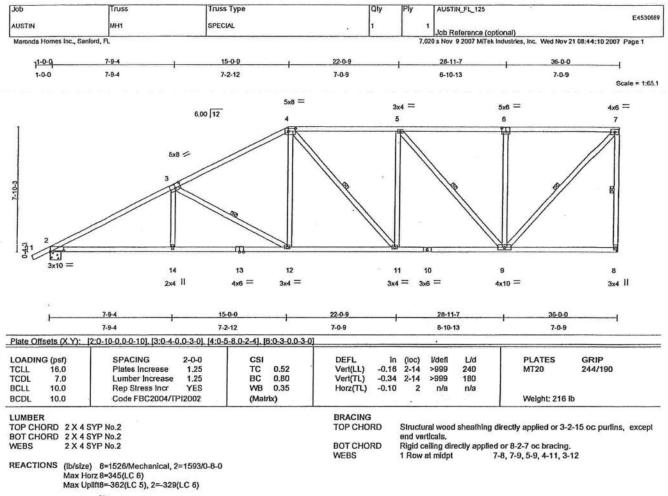
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 117 lb uplift at joint 5, 61 lb uplift at joint 2 and 140 lb uplift at joint 6.

LOAD CASE(S) Standard

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FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 7-8=-1396/603, 4-5=-1748/668, 5-6=-1170/441, 6-7=-1170/441, 1-2=0/21, 2-3=-2771/812, 3-4=-2063/706

BOT CHORD 2-14=-562/2390, 13-14=-564/2383, 12-13=-564/2383, 11-12=-298/1787, 10-11=-264/1748, 9-10=-264/1748, 8-9=0/393

7-9=637/1689, 6-9=330/317, 5-9=856/337, 5-11=9/315, 4-11=-103/121, 4-12=-58/610, 3-12=-691/304, 3-14=0/333

WEBS NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Calegory II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 362 lb uplift at joint 8 and 329 lb uplift at joint 2.

LOAD CASE(S) Standard

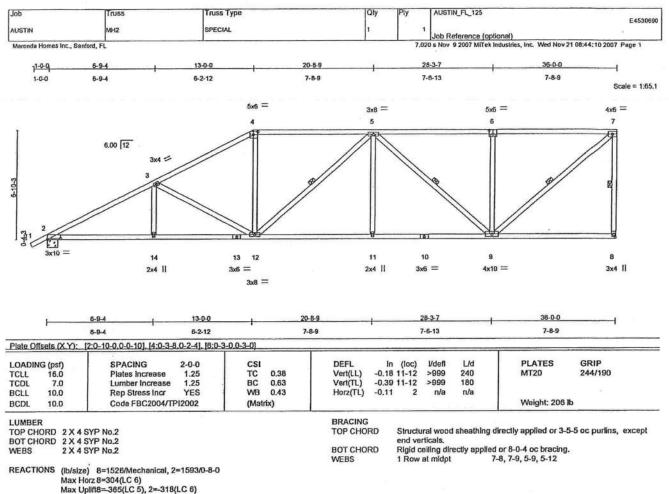
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Edenton, NC 27932



FORCES (lb) - Maximum Compression/Maximum Tension

7-8=-1381/601, 4-5=-1957/757, 5-6=-1451/541, 6-7=-1451/541, 1-2=0/21, 2-3=-2807/852, 3-4=-2234/774

2-14=.608/2424, 13-14=.608/2424, 12-13=.608/2424, 11-12=.430/2084, 10-11=.430/2084, 9-10=.430/2084, 8-9=0/341 7-9=.693/1858, 6-9=.358/343, 5-9=.837/327, 5-11=0/305, 5-12=.168/139, 4-12=.54/649, 3-12=.547/238, 3-14=0/273 **BOT CHORD** WEBS

NOTES 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) Provide adequate drainage to prevent water ponding.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) Refer to girder(s) for truss to truss connections.

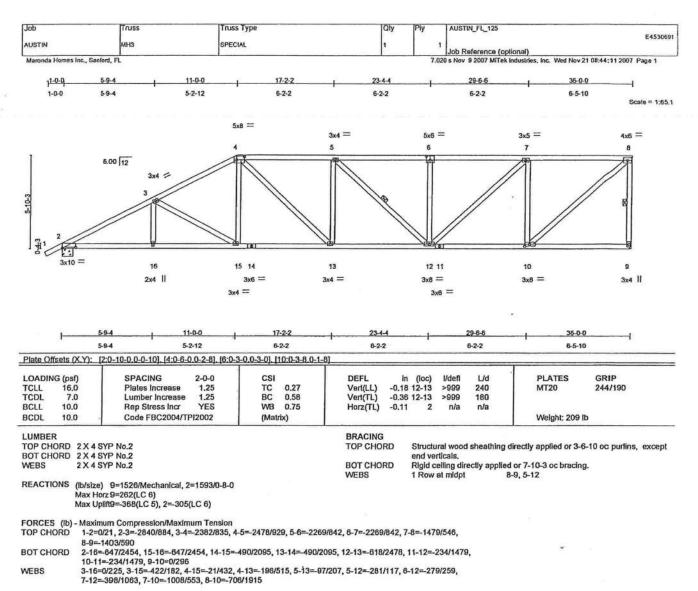
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 365 lb uplift at joint 8 and 318 lb uplift at joint 2.

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MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE AM WARLING - VERTIJY design parameters and NEAD NOTES ON THIS AND INCLUDED BITCH REFERENCE FALS MULTIPYS DEPONE OBE.
Design volid for use only with Millex connection. This design is based only upon poromeles show, and is for an individual building component
Applicability of design parameters and proper incorporation of component is responsibility of building designer-not loss designer. Roading st
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NOTES

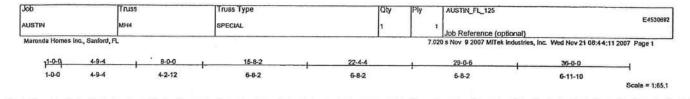
- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25fl; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
 This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 368 lb uplift at joint 9 and 305 lb uplift at Joint 2.

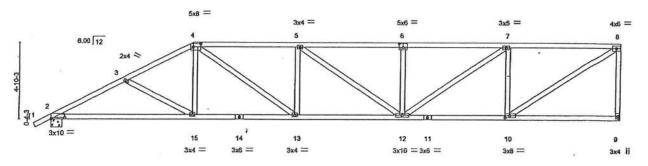
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	1	9-0-0		15-8-2		22-4-4		29-0	-6	38-0-0	
		9-0-0	2.1	6-8-2		6-8-2		6-8-	2	8-11-10	*
Plate Off	sets (X.Y):	2:0-10-0.0-0-14], [4:0-6-0	,0-2-8], [6:0-3	1.[0-2-0,0	10:0-3-8.0-1-	8]					
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL	16.0	Plates Increase	1.25	TC	0.85	Vert(LL)	-0.24 12-13	>999	240	MT20	244/190
CDL	7.0	Lumber Increase	1.25	BC	0.82	Vert(TL)	-0.49 12-13	>868	180	100000000000000000000000000000000000000	
BCLL	10.0	Rep Stress Incr	YES	WB	0.82	Horz(TL)	-0.12 2	n/a	n/a		
BCDL	10.0	Code FBC2004/TF	P12002	(Matr	ix)	0 5 8				Weight: 195 lt)

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS

2 X 4 SYP No.2

BRACING TOP CHORD

Structural wood sheathing directly applied or 3-5-3 oc purlins, except

BOT CHORD Rigid ceiling directly applied or 6-10-14 oc bracing.

REACTIONS (lb/size) 9=1526/Mechanical, 2=1593/0-8-0

Max Horz 9=221(LC 6)

Max Uplift9=-370(LC 5), 2=-296(LC 5)

FORCES (Ib) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/21, 2-3=-2733/947, 3-4=-2573/980, 4-5=-2978/1107, 5-6=-2867/1052, 6-7=-2867/1052, 7-8=-1917/702, R-9=-1391/589

2-15=704/2382, 14-15=575/2276, 13-14=575/2276, 12-13=843/2978, 11-12=-437/1917, 10-11=-437/1917, BOT CHORD

3-15=133/171, 4-15=0/394, 4-13=-324/849, 5-13=-237/255, 5-12=-134/67, 6-12=-302/280, 7-12=-424/1150,

7-10=-966/545, 8-10=-817/2230

NOTES

WEBS

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 370 lb uplift at joint 9 and 296 lb uplift at Joint 2.

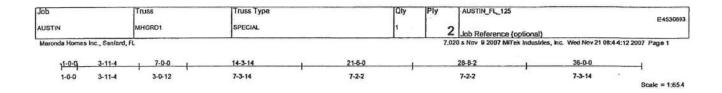
LOAD CASE(S) Standard

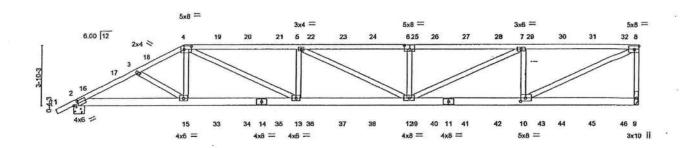
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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE. A WARDING - Very design parameters and READ NOTING ON THIS AND INCLUDED BITEK REPAIRING PAIR BROKE USE.

Design voted for use only with Milak connection. This design is based only upon pouraneters show, and is for on Individual bublishing component, Applicability of design programments and proper incorporation of component is responsibility of building designer - not inus designer. Rocing shown is for folleral support of individual web members only. Additional temporary brocing to inuse riskly during construction is the responsibility of the erector. Additional permanent brocing of the event structure is the responsibility of the building designer. For general guidonce regarding tabrication, quality control, storage, delivery, erection and brocking, consult. AMSUTPIT Quality Criteria, DSS-89 and BCSI1 Building Component Safety Indomnation available from Trus Plate Institute, S83 D'Onotiro Drive, Modison, WI 53719.







-	3-11-4	7-0-0	14-3-14		+	21-6-0		28-8-2		36-0-0	
	3-11-4	3-0-12	7-3-14			7-2-2		7-2-2		7-3-14	360
Plate Offse	ts (X.Y): [2:0	-3-10.0-2-0]. [4:0-5-8.	0-2-4], [6:0-4-0,	0-3-0]. [10	:0-3-8.0-2-	8]					
LOADING ((psf)	SPACING	2-0-0	CSI		DEFL	In (loc)	I/defl	L/d	PLATES	GRIP
TCLL '	16.0	Plates Increase	1.25	TC	0.73	Vert(LL)	0.41 12-13	>999	240	MT20	244/190
TCDL.	7.0	Lumber Increase	1.25	BC	0.71	Vert(TL)	-0.70 12-13	>605	180		
BCLL 1	10.0	Rep Stress Incr	NO	WB	0.73	Horz(TL)	-0.10 2	n/a	n/a		
BCDL 1	10.0	Code FBC2004/TF	212002	(Matr	ix)	1/4/12/2012/2015/201				Weight: 428 I	b

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 6 SYP No.2

WEBS 2 X 4 SYP No.2 BRACING

TOP CHORD

Structural wood sheathing directly applied or 3-9-12 oc purlins, except

end verticals.

BOT CHORD Rigid ceiling directly applied or 8-0-0 oc bracing.

REACTIONS (lb/size) 9=3456/Mechanical, 2=3341/0-8-0

fax Horz 9=181(LC 5)

Max Upliff9=-1881(LC 7), 2=-1885(LC 8)

FORCES (Ib) - Maximum Compression/Maximum Tension

1-2=0/25, 2-16=-6512/3586, 16-17=-6456/3503, 3-17=-6444/3424, 3-18=-6568/3513, 4-18=-6510/3523, TOP CHORD

4-19=-8802/4793, 19-20=-8802/4793, 20-21=-8801/4792, 5-21=-8801/4792, 5-22=-8720/4742, 22-23=-8720/4742, 23-24--8720/4742, 24-25--8720/4742, 6-25--8720/4742, 6-26--8720/4742, 26-27--8720/4742, 27-28--8720/4742, 7-28=-8720/4742, 7-29=-5894/3288, 29-30=-5894/3288, 30-31=-5894/3288, 31-32=-5894/3288, 8-32=-5894/3288,

8-9=-3127/1878

2-15=3023/5685, 15-33=-3059/5940, 33-34=-3059/5940, 14-34=-3059/5940, 14-35=-3059/5940, 13-35=-3059/5940, BOT CHORD

13-36=-4611/8801, 36-37=-4611/8801, 37-38=-4611/8801, 38-39=-4611/8801, 12-39=-4611/8801, 12-40=-3172/5894, 11-40=-3172/5894, 11-41=-3172/5894, 41-42=-3172/5894, 10-42=-3172/5894, 10-43=-86/217, 43-44=-86/217,

44-45=-86/217, 45-46=-86/217, 9-46=-86/217

3-15=285/250, 4-15=189/957, 4-13=1829/3168, 5-13=-724/874, 5-12=-90/74, 6-12=-733/859, 7-12=-1718/3140,

7-10=-2127/1602, 8-10=-3605/6306

NOTES

WERS

2-ply truss to be connected together with 10d (0.131"x3") nalls as follows: Top chords connected as follows: 2 X 4 - 1 row at 0-7-0 oc.

Bottom chards connected as follows: 2 X 6 - 2 rows at 0-9-0 oc.

Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

3) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise);

Lumber DOL=1.60 plate grip DOL=1.60.

4) Provide adequate drainage to prevent water ponding.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1881 lb uplift at joint 9 and 1885 lb uplift at joint 2.

Continued on page 2

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mion, NC 27932

Job	Truss	Truss Type	Qty	Ply	AUSTIN_FL_125	D-2002-00/49-40/
AUSTIN	MHGRD1	SPECIAL				E4530693
				2	Job Reference (optional)	
Marrada Hamas Inc. Canford				7.00	On New C 2007 Mittak industries Inc. Mind No.	P. CO. 44-40 DOOT D D

NOTES

8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 116 lb down and 194 lb up at 35-0-12, 116 lb down and 194 lb up at 31-0-12, 116 lb down and 194 lb up at 27-0-12, 116 lb down and 194 lb up at 25-0-12, 116 lb down and 194 lb up at 23-0-12, 116 lb down and 194 lb up at 21-0-12, 116 lb down and 194 lb up at 19-0-12, 116 lb down and 194 lb up at 17-0-12, 116 lb down and 194 lb up at 17-0-12, 116 lb down and 194 lb up at 17-0-12, 116 lb down and 194 lb up at 17-0-12, 116 lb down and 194 lb up at 19-0-12, 116 lb down and 194 lb up at 17-0-12, 116 lb down and 194 lb up at 19-0-12, 116 lb down and 194 lb up at 19-0-12, 116 lb down and 194 lb up at 17-0-12, 116 lb down and 194 lb up at 19-0-12, 1 down and 194 ib up at 23-0-12, 116 ib down and 194 ib up at 21-0-12, 116 ib down and 194 ib up at 17-0-12, 116 ib down and 194 ib up at 17-0-12, 116 ib down and 194 ib up at 17-0-12, 116 ib down and 194 ib up at 17-0-12, 116 ib down and 194 ib up at 17-0-12, 116 ib down and 194 ib up at 17-0-12, 116 ib down and 194 ib up at 17-0-12, 116 ib down and 194 ib up at 17-0-12, 116 ib down and 194 ib up at 19-0-12, 116 ib down and 19-0-12, 116 ib down and 194 ib up at 19-0-12, 116 ib down and 194 ib up at 19-0-12, 116 ib down and 194 ib up at 19-0-12, 116 ib down and 194 ib up at 19-0-12, 116 ib down and 194 ib up at 19-0-12, 116 ib down and 194 ib up at 19-0-12, 116 ib down and 194 ib up at 19-0-12, 116 ib down and 194 ib up at 19-0-12, 116 ib down and 194 ib up at 19-0-12, 116 ib down and 19-0-12, 116 ib down and 194 ib up at 19-0-12, 116 ib down and 19-0-12, 116 ib down and 19-0-12, 116 ib up at 19-0-12, 116 ib down and 19-0-

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

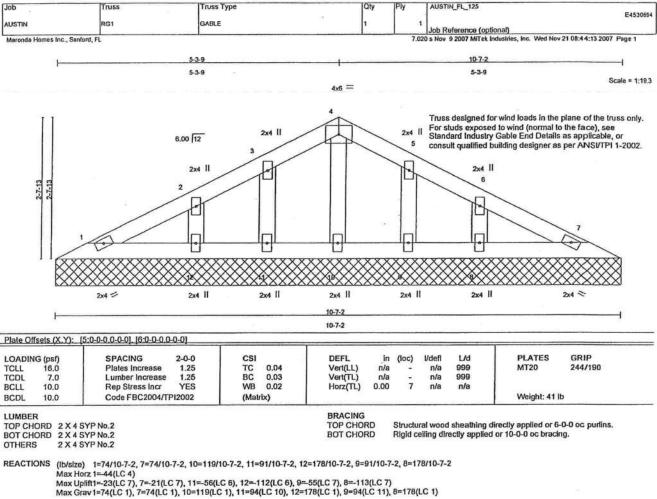
Uniform Loads (plf)

Vert: 1-4=46, 4-8=46, 2-9=40

Concentrated Loads (lb)

Veri: 4=-116(B) 15=-644(B) 19=-116(B) 20=-116(B) 21=-116(B) 22=-116(B) 23=-116(B) 24=-116(B) 25=-116(B) 26=-116(B) 27=-116(B) 28=-116(B) 29=-116(B) 21=-116(B) 21=-11 30=-116(B) 31=-116(B) 32=-116(B) 33=-92(B) 34=-92(B) 34=-92(B) 36=-92(B) 37=-92(B) 38=-92(B) 39=-92(B) 40=-92(B) 41=-92(B) 42=-92(B) 43=-92(B) 44=-92(B) 45=-92(B) 46=-92(B)





FORCES (lb) - Maximum Compression/Maximum Tension

1-2=-42/27, 2-3=-25/65, 3-4=-20/112, 4-5=-20/112, 5-6=-25/65, 6-7=-24/19 1-12=0/51, 11-12=0/51, 10-11=0/51, 9-10=0/51, 8-9=0/51, 7-8=0/51 **BOT CHORD**

WEBS 4-10=-59/7, 3-11=-53/94, 2-12=-92/161, 5-9=-53/94, 6-8=-92/161

TOP CHORD

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) Gable requires continuous bottom chord bearing.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 23 lb uplift at joint 1, 21 lb uplift at joint 7, 56 lb uplift at joint 11, 112 lb uplift at joint 12, 55 lb uplift at joint 9 and 113 lb uplift at joint 8.

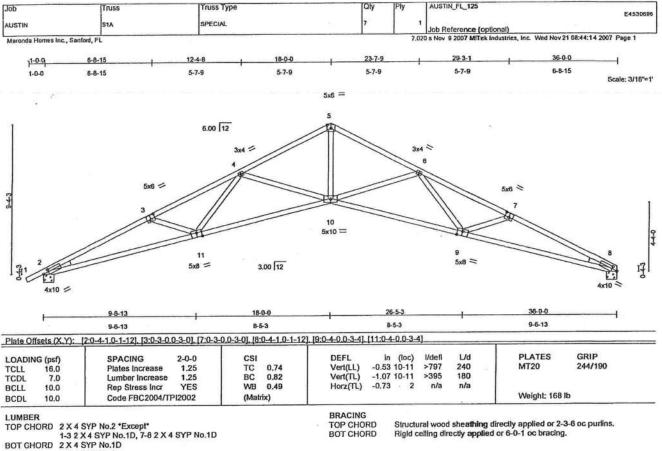
LOAD CASE(S) Standard

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enton, NC 27932



2 X 4 SYP No.2 WEBS

WEDGE

Left: 2 X 4 SYP No.3, Right: 2 X 4 SYP No.3

REACTIONS (lb/size) 8=1518/0-8-0, 2=1585/0-8-0

Max Horz 8=166(LC 6)

Max Uplift8=-286(LC 7), 2=-368(LC 6)

FORCES (ib) - Maximum Compression/Maximum Tension

1-2=0/18, 2-3=-4813/1409, 3-4=-4589/1239, 4-5=-3320/885, 5-6=-3320/885, 6-7=-4602/1269, 7-8=-4831/1449 2-11=-1145/4368, 10-11=-811/3809, 9-10=-822/3814, 8-9=-1186/4387 3-11=-176/290, 4-11=-93/711, 4-10=-807/417, 5-10=-563/2647, 6-10=-812/428, 8-9=-118/722, 7-9=-183/306

BOT CHORD

WEBS

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ff; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) Bearing at joint(s) 8, 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 286 lb uplift at joint 8 and 368 lb uplift at

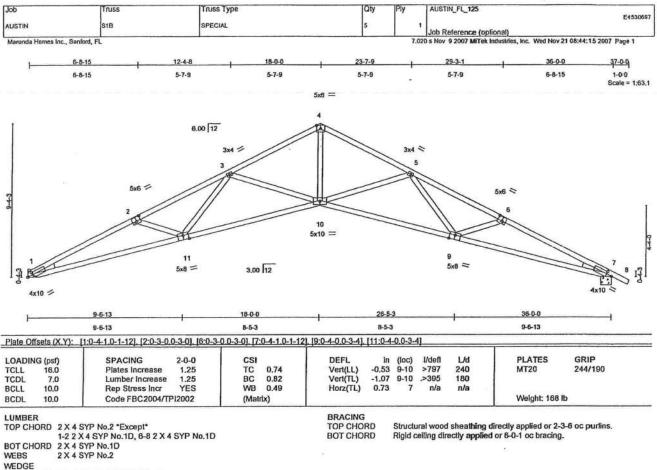
LOAD CASE(S) Standard

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ANSI/TPII Quality Criteria, DSB-89 and BCS11 Suitding Component Safely Intermation available from Truss Plate Institute, SS3 D'Onofrio Drive, Madison, Wi SS319.





Left: 2 X 4 SYP No.3, Right: 2 X 4 SYP No.3

REACTIONS (lb/size) 1=1518/Mechanical, 7=1585/0-8-0

Max Horz 1=-166(LC 7) Max Upliff1=-286(LC 6), 7=-368(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=-4831/1449, 2-3=-4802/1269, 3-4=-3320/885, 4-5=-3320/885, 5-6=-4589/1239, 6-7=-4813/1409, 7-8=0/18

BOT CHORD

1-11=-1186/4387, 10-11=-822/3814, 9-10=-811/3809, 7-9=-1145/4368 2-11=-183/306, 3-11=-118/722, 3-10=-812/428, 4-10=-563/2647, 5-10=-807/417, 5-9=-93/711, 6-9=-176/290 WEBS

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6,0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

Refer to girder(s) for truss to truss connections.

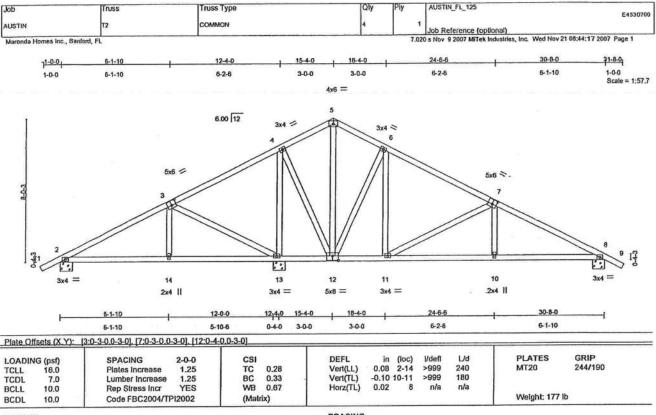
- 5) Bearing at Joint(s) 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 286 lb uplift at joint 1 and 368 lb uplift at joint 7.

LOAD CASE(S) Standard

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LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.2 BRACING

TOP CHORD **BOT CHORD**

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 6-0-0 oc bracing; 12-13.

REACTIONS (lb/size) 2=364/0-8-0, 13=1651/0-8-0, 8=694/0-8-0

Max Horz 2=141(LC 6)

Max Uplift2=302(LC 6), 13=556(LC 6), 8=212(LC 7) Max Grav2=408(LC 10), 13=1651(LC 1), 8=708(LC 11)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/21, 2-3=-345/382, 3-4=-178/495, 4-5=-24/101, 5-6=-35/96, 6-7=-370/102, 7-8=-979/172, 8-9=0/21 2-14=-228/257, 13-14=-224/250, 12-13=-390/422, 11-12=0/271, 10-11=-48/808, 8-10=-46/815

TOP CHORD BOT CHORD

3-14=318/269, 3-13=-628/701, 4-13=-1143/476, 4-12=-194/808, 5-12=-99/28, 6-12=-649/296, 6-11=-69/487, WEBS

7-11=-605/258, 7-10=0/272

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2pst; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 302 lb uplift at joint 2, 556 lb uplift at joint 13 and 212 lb uplift at joint 8.

LOAD CASE(S) Standard

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nion, NC 27932

Job	Truss	Truss Type	Qly	Ply	AUSTIN_FL_125		F4500704
AUSTIN	V1	VALLEY	2				E4530701
Maronda Homes Inc.				7.	Job Reference (option 010 s Oct 18 2007 MITek Ind	BI) lustries, Inc. Wed Nov 21	10:00:13 2007 Page 1
Material France me.	, outroid, i c				7.22		
	1	2-0-0			4-0-0 2-0-0		
		2-0-0	2 3x4 =		2-0-0		Scale: 1.5"=1"
		6.00 12		\			
	9 1			_	3		
				• • • • •			×
		214 =		***	2×4 ≈	*****	\boxtimes
			4-0-0				
	1		4-0-0				
Plate Offsets (X,)); [2:0-2-0,Edge]						
LOADING (psf) TCLL 16.0 TCDL 7.0 BCLL 10.0 BCDL 10.0	SPACING Plates Increase Lumber Increase Rep Stress Incr Code FBC2004/TP	2-0-0 CSI 1.25 TC 0.03 1.25 BC 0.08 YES WB 0.00 2002 (Matrix)	Vert(TL)	In (loc) n/a - n/a - 0.00 3	Vdefl L/d n/a 999 n/a 999 n/a n/a	PLATES MT20 Weight: 11 lb	GRIP 244/190
LUMBER TOP CHORD 2 BOT CHORD 2	X 4 SYP No.2 X 4 SYP No.2	W .	BRACING TOP CHORD BOT CHORD	Structu Rigid o	ral wood sheathing directly applied o	ectly applied or 4-0-0 r 10-0-0 oc bracing.	oc purlins.
M	o/size) 1=119/4-0-0, 3=119/4 ax Horz 1=-11(LC 4) ax Uplift 1=-22(LC 6), 3=-22(LC						
TOP CHORD	Maximum Compression/Maxim 1-2=82/69, 2-3=82/69 1-3=42/64	m Tension					

Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interfor(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 3) This truss has hear designed for a 400 and before the control of the co

- MWH-KS for reactions specified.

 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

 4) Gable requires continuous bottom chord bearing.

 5) WARNING: Top chord roof live load is below minimum required by ASCE 7. The building design professional for the overall
- structure to verify adequacy of top chord live load.

 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 22 lb uplift at joint 1 and 22 lb uplift at

LOAD CASE(S) Standard

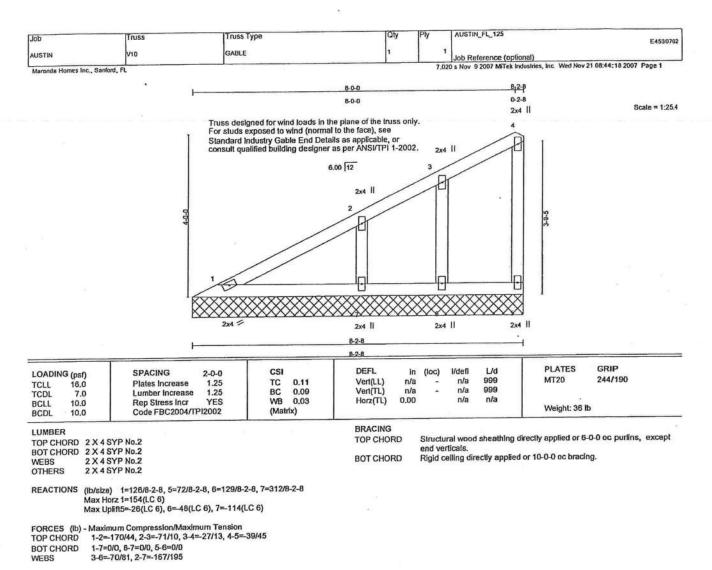
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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED BITTER REFERENCE PAGE MIL-1473 BEFORE USE.

Design valid for use only with Mife connectors. This design is based only upon parameters shown, and is for an individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not trust designer. Bracing shown is for fateral support of individual web members only. Additional temporary bracing to truste stability during construction is the responsibility of the beding designer. For general guidance regarding errors, Additional permonent bracing of line overall structure is the responsibility of the building designer. For general guidance regarding clothication, quality control, storage, desivery, erection and bracing, consult.

ANSJITTI Quality Criteria, DSS-89 and BCS11 Building Component Safety Information avoilable from Truss Plate Institute, S83 D'Onofrio Drive, Maddon, WI S37 19.





NOTES

- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Gable requires continuous bottom chord bearing.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 26 lb uplift at joint 5, 48 lb uplift at joint 6 and 114 lb uplift at joint 7.

LOAD CASE(S) Standard

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Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	AUSTIN_FL_125		E4530703
AUSTIN	V11	GABLE	1		1 Job Reference (optional)		24330703
Maronda Homes Inc., Sanford	i, FL			7.0	020 s Nov 9 2007 MiTek Industri	es, Inc. Wed Nov 21 08:44	:18 2007 Page 1
	Ĩ		10-0-0		10-2 8		
			10-0-0	75	0-2-8		
					2x4 II		Scale = 1:31.5
	Foi Size con	22	al to the face), see ails as applicable, or	2.	2x4 2x4	\$-84	
	1		10-2-8				
LOADING (pst) TCLL 16.0 TCDL 7.0 BCLL 10.0 BCDL 10.0	SPACING 2-0-1 Plates Increase 1.2 Lumber Increase 1.2 Rep Stress Incr YE Code FBC2004/TPI2002	TC 0.09 BC 0.10 WB 0.02	- CONTRACTOR	/a -	l/defi L/d n/a 999 n/a 999 n/a n/a	PLATES GR MT20 244 Weight: 49 lb	IP /190
LUMBER TOP CHORD 2 X 4 SY BOT CHORD 2 X 4 SY WEBS 2 X 4 SY OTHERS 2 X 4 SY	/P No.2 /P No.2 /P No.2	,	BRACING TOP CHORD BOT CHORD	end ve	rral wood sheathing direct rticals. Jeiling directly applied or 1		purlins, except
Max Hor	1=127/10-2-8, 6=66/10-2-8, z 1=196(LC 6) ft6=-26(LC 6), 7=-66(LC 6), 8	7=191/10-2-8, 8=114/10-2-8, 9=43(LC 6), 9=-114(LC 6)	9=315/10-2-8				
FORCES (lb) - Maximu	rm Compression/Maximum To 14/40, 2-3=-126/8, 3-4=-82/2	ension 1, 4-5=-24/14, 5-6=-35/41			=		

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

BOT CHORD 1-9=-2/2, 8-9=-2/2, 7-8=-2/2, 6-7=-2/2 WEBS 4-7=-101/113, 3-8=-64/82, 2-9=-164/168

3) Gable requires continuous bottom chord bearing.

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 26 lb uplift at joint 6, 66 lb uplift at joint 7, 43 lb uplift at joint 8 and 114 lb uplift at joint 9.

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Design voiid for use only with Milek connecton. This design is based only upon parameters shown, and is for an individual building component, Applicability of design parameters and proper incorporation of component is responsibility of building designer - not trus designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the wind designer. For general guidance regarding lateral control, and of the version of the versio



Truss Type AUSTIN_FL_125 Job Truss E453070 GABLE AUSTIN V12 Job Reference (optional) 7.020 s Nov 9 2007 MiTek Industries, Inc. Wed Nov 21 08:44:18 2007 Page 1 12 2 8 0-2-8 12-0-0 2x4 || Scale = 1:37.7 Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see 2x4 || Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002. 2x4 || 6.00 12 2x4 II 2x4 || 2x4 || 2x4 || 2x4 11 244 11 2×4 11 12-2-8 LOADING (psf) SPACING DEFL **PLATES** GRIP CSI V/defl L/d 2-0-0 (loc) 999 MT20 244/190 0.09 Vert(LL) n/a n/a Plates Increase 1.25 TC TCLL 16.0 1.25 0.10 Vert(TL) n/a 999 TCDL Lumber Increase 7.0 Rep Stress Incr YES WB 0.03 Horz(TL) 0.00 n/a n/a BCLL 10.0 Code FBC2004/TPI2002 Weight: 64 lb BCDL 10.0 (Matrix) BRACING Structural wood sheathing directly applied or 6-0-0 oc purlins, except TOP CHORD 2 X 4 SYP No.2 TOP CHORD BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.2 end verticals. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. OTHERS 2 X 4 SYP No.2 REACTIONS (lb/slze) 1=127/12-2-8, 7=69/12-2-8, 8=173/12-2-8, 9=186/12-2-8, 10=114/12-2-8, 11=315/12-2-8 Max Horz 1=237(LC 6) Max Uplift7=-28(LC 6), 8=-60(LC 6), 9=-69(LC 6), 10=-42(LC 6), 11=-115(LC 6) FORCES (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=265/41, 2-3=-178/8, 3-4=-135/21, 4-5=-78/18, 5-6=-24/15, 6-7=-37/42

BOT CHORD

1-11=-2/2, 10-11=-2/2, 9-10=-2/2, 8-9=-2/2, 7-8=-2/2

WEBS

5-8=-93/104, 4-9=-99/109, 3-10=-64/80, 2-11=-164/165

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) Gable requires continuous bottom chord bearing.

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 7, 60 lb uplift at joint 8, 69 lb uplift at joint 9, 42 lb uplift at joint 10 and 115 lb uplift at joint 11.

LOAD CASE(S) Standard

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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 BEFORE USE. Design volt for use only with Milex connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not trust designer, Brocing shown is for fallers a support of individual web members only. Additional temporary brocing to insure shifting designer - not trust designer, Brocing shown is for fallers a support of individual web members only. Additional temporary brocing to insure shifting designer. For general guidance regarding clothocaling, quality control, storage, delivery, sercicion and brocing, consult. AMSI/IPII Quality Criteria, DS8-89 and BCSI1 Building Component Safety Intermation.



Truss Type AUSTIN_FL_125 Job russ E4530705 AUSTIN V13 GABLE Job Reference (optional) 7,020 s Nov 9 2007 MiTek Industria stries, Inc. Wed Nov 21 05:44:19 2007 Page 1 Maronda Homes Inc., Sanford, FL 1428 0-2-8 14-0-0 2x4 || Scale = 1:43 8 Truss designed for wind loads in the plane of the truss only. 2x4 || For stude exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002, 6.00 12 2x4 || 2x4 || 2x4 || 2x4 || 2x4 II 2x4 11 2x4 || 2x4 || 2×4 11 LOADING (psf) DEFL PLATES GRIP SPACING CSI 2-0-0 (loc) **V**defl L/d 999 244/190 Vert(LL) n/a n/a MT20 Plates Increase TC 0.09 TCLL 16.0 1.25 1.25 BC 0.10 Vert(TL) n/a n/a 999 TCDL 7.0 Lumber Increase Rep Stress Incr BCLL 10.0 YES WR 0.05 Horz(TL) -0.00n/a n/a Code FBC2004/TPI2002 Weight: 80 lb (Matrix) BCDL 10.0 BRACING LUMBER Structural wood sheathing directly applied or 6-0-0 oc purlins, except TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 TOP CHORD end verticals. 2 X 4 SYP No.2 **BOT CHORD** Rigid celling directly applied or 10-0-0 oc bracing. WEBS 2 X 4 SYP No.2 WEBS 1 Row at midpt REACTIONS (lb/size) 1=127/14-2-8, 8=68/14-2-8, 9=178/14-2-8, 10=168/14-2-8, 11=187/14-2-8, 12=114/14-2-8, 13=315/14-2-8 Max Horz 1=279(LC 6) Max Upilft8=-28(LC 6), 9=-61(LC 6), 10=-63(LC 6), 11=-67(LC 6), 12=-42(LC 6), 13=-115(LC 6) FORCES (lb) - Maximum Compression/Maximum Tension 1-2=315/41, 2-3=-229/9, 3-4=-186/22, 4-5=-131/19, 5-6=-78/19, 6-7=-24/15, 7-8=-36/41 1-13=-1/1, 12-13=-1/1, 11-12=-1/1, 10-11=-1/1, 9-10=-1/1, 8-9=-1/1 **BOT CHORD** WEBS 6-9=94/104, 5-10=91/101, 4-11=99/107, 3-12=64/79, 2-13=164/163

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2pst; BCDL=6.0psf; Calegory II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) Gable requires continuous bottom chord bearing.

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 8, 61 lb uplift at joint 9, 63 lb uplift at joint 10, 67 lb uplift at joint 11, 42 lb uplift at joint 12 and 115 lb uplift at joint 13.

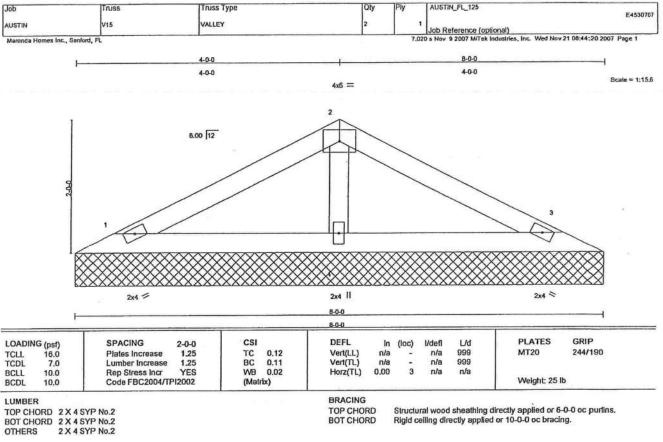
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Edenton, NC 27932



REACTIONS (lb/size) 1=136/8-0-0, 3=136/8-0-0, 4=310/8-0-0

Max Horz 1=28(LC 4)

Max Uplift1=44(LC 6), 3=49(LC 7), 4=-21(LC 6)

FORCES (Ib) - Maximum Compression/Maximum Tension

1-2=-44/39, 2-3=-44/38 TOP CHORD

BOT CHORD

2-4=-141/111 WEBS

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Whird: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) Gable requires continuous bottom chord bearing.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 44 lb uplift at joint 1, 49 lb uplift at joint 3 and 21 lb uplift at joint 4.

LOAD CASE(S) Standard

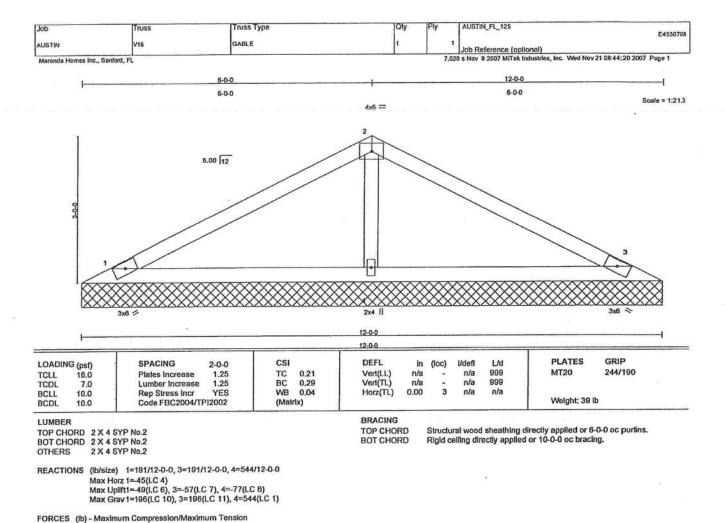
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Design valid for use only with Millek connectors. This design is based only upon parameters shown, and is for an includual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for taleral support of individual web members only. Additional temporary bracing to insure solitability during construction is the responsibility of the erector. Additional permanent bracing of the overal structure is the responsibility of the building designer, For general guidance regarding labitation, quality control, storage, delivery, erection and bracing, consult. AMSI/IPII Quality Criteria, DSS-89 and BCSII Building Component Salety Information available from Truss Piate Institute, SS3 D'Onatio Drive, Modison, Wi S3719.



Edenion NC 27932



WEBS

TOP CHORD

BOT CHORD

NOTES 1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 4) Gable requires continuous bottom chord bearing.

1-2=-70/62, 2-3=-70/60

1-4=-3/34, 3-4=-3/34 2-4=-268/190

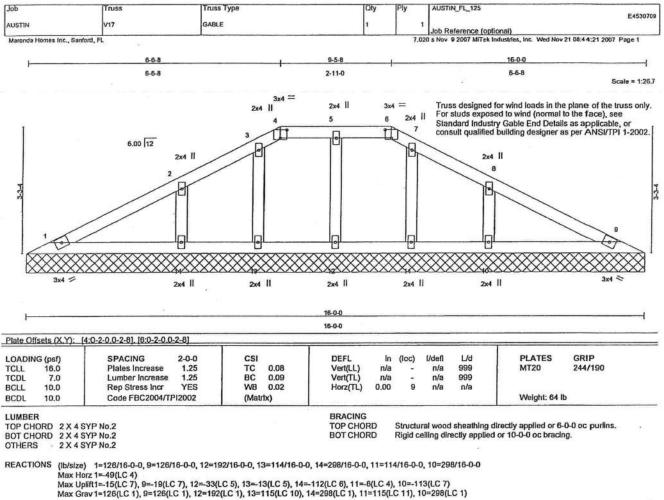
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 49 lb uplift at joint 1, 57 lb uplift at joint 3 and 77 lb uplift at joint 4.

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FORCES (lb) - Maximum Compression/Maximum Tension

1-2=-42/38, 2-3=-47/79, 3-4=-33/100, 4-5=-15/97, 5-6=-15/97, 6-7=-33/100, 7-8=-47/79, 8-9=-39/24 1-14=0/49, 13-14=0/49, 12-13=0/49, 11-12=0/49, 10-11=0/49, 9-10=0/49 TOP CHORD

BOT CHORD

WEBS

5-12=99/64, 3-13=-62/41, 2-14=-156/157, 7-11=-62/41, 8-10=-156/157

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified, 3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) Gable requires continuous bottom chord bearing.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 15 lb uplift at joint 1, 19 lb uplift at joint 9, 33 lb uplift at joint 12, 13 lb uplift at joint 13, 112 lb uplift at joint 14, 6 lb uplift at joint 11 and 113 lb uplift at joint 10.

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AUSTIN_FL_125 Truss Truss Type E4530710 AUSTIN VIB GARLE Job Reference (optional) 7.020 s Nov 9 2007 MiTek Industries, Inc. Wed Nov 21 08:44:22 2007 Page 1 Maronda Homes Inc., Sanford, FL 20-0-0 6-6-8 Fruss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002. 6-11-0 6-6-8 Scale = 1:33.3 3x4 = 2x4 || 2x4 || 2x4 || 3x4 = II 2x4 II B 6 7 6.00 12 9 2x4 || 2x4 || 10 3x4 = 2x4 11 2x4 || 2x4 || 2x4 || 2x4 || 2x4 || 5x6 = 20-0-0 20-0-0 Plate Offsets (X.Y): [4:0-2-0.0-2-8]. [8:0-2-0.0-2-8]. [15:0-3-0.0-3-0] PLATES LOADING (psf) SPACING 2-0-0 CSI DEFL In (loc) I/defl L/d 999 MT20 244/190 n/a 16.0 1.25 TC 0.08 Vert(LL) TCLL Plates Increase TCDL 1.25 BC 0.09 Vert(TL) n/a n/a 999 7.0 Lumber Increase WB 0.00 n/a n/a BCLL 10.0 Rep Stress Incr YES 0.02 Horz(TL) 11 Weight: 83 lb Code FBC2004/TPI2002 BCDL 10.0 (Matrix) BRACING LUMBER Structural wood sheathing directly applied or 6-0-0 oc purlins. TOP CHORD TOP CHORD 2 X 4 SYP No.2 **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing. BOT CHORD 2 X 4 SYP No.2 2 X 4 SYP No.2 OTHERS REACTIONS (lb/size) 1=125/20-0-0, 11=125/20-0-0, 15=167/20-0-0, 16=183/20-0-0, 17=117/20-0-0, 18=298/20-0-0, 14=183/20-0-0, 13=117/20-0-0, 12=298/20-0-0 Max Horz 1=49(LC 5) Max Uplift1=14(LC 7), 11=17(LC 7), 15=46(LC 4), 16=39(LC 5), 17=13(LC 5), 18=113(LC 6), 14=37(LC 5), 13=5(LC 4), 12=-113(LC 7)
Max Grav1=125(LC 1), 11=125(LC 1), 15=168(LC 10), 16=183(LC 1), 17=118(LC 10), 18=298(LC 1), 14=183(LC 1), 13=118(LC 11), 12=298(LC 1) FORCES (Ib) - Maximum Compression/Maximum Tension

1-2=46/35, 2-3=44/72, 3-4=32/93, 4-5=13/90, 5-6=13/90, 6-7=13/90, 7-8=13/90, 8-9=-32/93, 9-10=-44/72, TOP CHORD

10-11=-37/27

1-18=0/52, 17-18=0/52, 16-17=0/52, 15-16=0/52, 14-15=0/52, 13-14=0/52, 12-13=0/52, 11-12=0/52 WEBS

6-15=91/91, 5-16=96/75, 3-17=63/41, 2-18=-156/156, 7-14=-96/75, 9-13=-63/41, 10-12=-156/156

NOTES

Unbalanced roof live loads have been considered for this design.

 Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C-Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) Gable requires continuous bottom chord bearing.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 14 lb uplift at joint 1, 17 lb uplift at joint 11 , 46 lb uplift at joint 15, 39 lb uplift at joint 16, 13 lb uplift at joint 17, 113 lb uplift at joint 18, 37 lb uplift at joint 14, 5 lb uplift at Joint 13 and 113 lb uplift at Joint 12.

LOAD CASE(S) Standard

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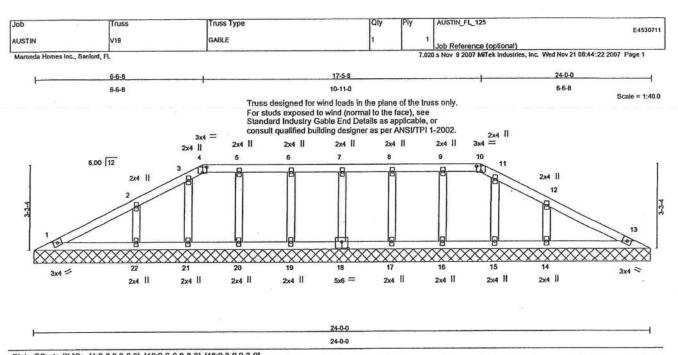
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ABSI/IPIT Quality control, stronge, deferver, erection ond broding, consult.

Safely Information available from Trus Plate Institute, 583 D'Onotrio Drive, Modison, WI 53719.



Inton, NC 27932



LOADING (psf)	SPACING 2-0-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase 1.25	TC 0.08	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL 7.0	Lumber Increase 1,25	BC 0.09	Vert(TL)	n/a		n/a	999	10000000	
BCLL 10.0	Rep Stress Incr YES	WB 0.02	Horz(TL)	0.00	13	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002	(Matrix)	2.7					Weight: 103	b

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

OTHERS 2 X 4 SYP No.2 BRACING

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=124/24-0-0, 13=124/24-0-0, 18=173/24-0-0, 19=169/24-0-0, 20=183/24-0-0, 21=118/24-0-0, 22=298/24-0-0,

17=169/24-0-0, 16=183/24-0-0, 15=118/24-0-0, 14=298/24-0-0

Max Horz 1=49(LC 4)
Max Uplift1=13(LC 7), 13=-16(LC 7), 18=-42(LC 5), 19=-44(LC 4), 20=-39(LC 5), 21=-13(LC 5), 22=-113(LC 6),
17=-44(LC 4), 16=-37(LC 5), 15=-5(LC 4), 14=-113(LC 7)
Max Grav 1=124(LC 1), 13=124(LC 1), 18=173(LC 1), 19=170(LC 11), 20=183(LC 10), 21=119(LC 10), 22=298(LC 1),

17=170(LC 10), 16=183(LC 11), 15=119(LC 11), 14=298(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD

1-2=48/34, 2-3=42/69, 3-4=30/90, 4-5=-11/87, 5-6=-11/87, 6-7=-11/87, 7-8=-11/87, 8-9=-11/87, 9-10=-11/87, 10-11=-30/90, 11-12=-42/69, 12-13=-35/29

1-22=0/53, 21-22=0/53, 20-21=0/53, 19-20=0/53, 18-19=0/53, 17-18=0/53, 16-17=0/53, 15-16=0/53, 14-15=0/53, **BOT CHORD**

13-14=0/53

WEBS

7-18=-92/85, 6-19=-92/89, 5-20=-96/76, 3-21=-64/42, 2-22=-155/156, 8-17=-92/89, 9-16=-96/76, 11-15=-64/42,

12-14=-155/156

Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) Gable requires continuous bottom chord bearing.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 13 lb uplift at joint 1, 16 lb uplift at joint 13 , 42 lb uplift at joint 18, 44 lb uplift at joint 19, 39 lb uplift at joint 20, 13 lb uplift at joint 21, 113 lb uplift at joint 22, 44 lb uplift at joint 17

, 37 lb uplift at joint 18, 5 lb uplift at joint 15 and 113 lb uplift at joint 14.

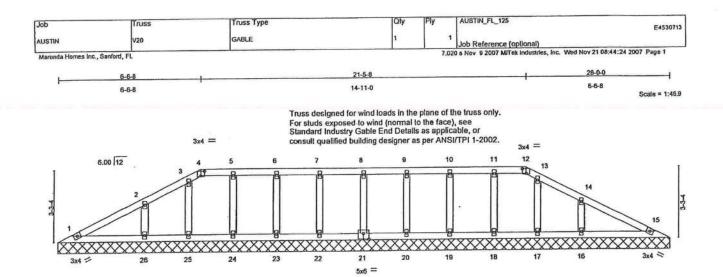
LOAD CASE(S) Standard

This document was originally issued by Lassiter, Frank on November 21,2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated November 21,2007 above.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIJ-7473 BEFORE USE. Design void for use only with Milex connectors. This design is based only upon porameters shown, and is for an Individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not Inus designer. Broching shows it for lateral support of individual web members only. Additional temporary bracking to insure stobility designer - not Inus designer. Broching shows it for lateral support of individual web members only. Additional temporary bracking to insure stobility during construction is the responsibility of like erector. Additional permanent broching of the overall structure is the responsibility of like building designer. For general guidance regarding lobication, quotity control, storage, desirey, execution and broching, consult. ANSI/IPTI Quality Criteria, DSB-89 and BCSI1 Building Component Safely Information available from Trus Plate Institute, SSB D'Onofrio Drive, Modison, WI S3719.



ton. NC 27932



			28-0-0						
Plate Offsets (X.Y): [4	1:0-2-0.0-2-8]. [12:0-2-0.0-2-8]. [21:0-	3-0.0-3-0)				-		·	
LOADING (psf) TCLL 16.0 TCDL 7.0 BCLL 10.0 BCDL 10.0	SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr YES Code FBC2004/TPI2002	CSI TC 0.08 BC 0.09 WB 0.02 (Malrix)	DEFL Vert(LL) Vert(TL) Horz(TL)	In n/a n/a 0.00	(loc) - - 15	I/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 123	GRIP 244/190

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

2 X 4 SYP No.2

BRACING

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=123/28-0-0, 15=123/28-0-0, 21=172/28-0-0, 22=173/28-0-0, 23=169/28-0-0, 24=183/28-0-0, 25=119/28-0-0, 26=298/28-0-0, 20=173/28-0-0, 19=169/28-0-0, 18=183/28-0-0, 17=119/28-0-0, 16=298/28-0-0

Max Horz 1=49(LC 4)
Max Uplift1=-12(LC 7), 15=-16(LC 7), 21=-42(LC 4), 22=-42(LC 5), 23=-44(LC 4), 24=-40(LC 5), 25=-14(LC 5), 25=-14(LC 5), 25=-12(LC 5), 19=-44(LC 4), 18=-36(LC 5), 17=-6(LC 4), 16=-113(LC 7)
Max Grav1=123(LC 1), 15=123(LC 1), 21=172(LC 11), 22=173(LC 1), 23=170(LC 11), 24=183(LC 10), 25=119(LC 10), 26=298(LC 1), 20=173(LC 1), 19=170(LC 10), 18=183(LC 11), 17=119(LC 11), 16=298(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=50/33, 2-3=-40/66, 3-4=-28/88, 4-5=-10/85, 5-6=-10/85, 6-7=-10/85, 7-8=-10/85, 8-9=-10/85, 9-10=-10/85, TOP CHORD

10-11=-10/85, 11-12=-10/85, 12-13=-28/88, 13-14=-40/68, 14-15=-33/30

1-26=0/54, 25-26=0/54, 24-25=0/54, 23-24=0/54, 22-23=0/54, 21-22=0/54, 20-21=0/54, 19-20=0/54, 18-19=0/54, **BOT CHORD**

17-18=0/54, 16-17=0/54, 15-16=0/54

8-21=92/86, 7-22=92/86, 6-23=92/88, 5-24=97/77, 3-25=64/43, 2-26=155/156, 9-20=92/86, 10-19=92/88, 11-18=97/77, 13-17=64/43, 14-16=155/156

NOTES

WEBS

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Provide adequate drainage to prevent water ponding.
 This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 All plates are 2x4 MT20 unless otherwise indicated.

Gable requires continuous bottom chord bearing.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 12 lb uplift at joint 1, 16 lb uplift at joint 15

42 lb uplift at joint 21, 42 lb uplift at joint 22, 44 lb uplift at joint 23, 40 lb uplift at joint 24, 14 lb uplift at joint 25, 112 lb uplift at joint 26 This document was originally 42 lb uplift at joint 20, 44 lb uplift at joint 19, 38 lb uplift at joint 18, 6 lb uplift at joint 17 and 113 lb uplift at joint 16. issued by Lassiter, Frank

LOAD CASE(S) Standard

on November 21,2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated November 21,2007 above.

MARNING - Verify design parameters and read notes on this and included miter reference page mil-1473 before use. As warming - verty design parameters and reads on the parameters and more on the parameters and minimal parameters and proper incorporation of components it responsibility of design parameters and proper incorporation of components it responsibility of building designer - not inside designer - not inside designer - not inside designer - not inside parameters and proper incorporation of components in the responsibility of responsibility of the responsibil



Truss Type AUSTIN_FL_125 Truss Job E4530718 VALLEY AUSTIN Job Reference (optional)
7.010 s Oct 16 2007 MiTek Industries, Inc. Wed Nov 21 10:01:45 2007 Page 1 Maronda Homes Inc., Sanford, FL 0-2-8 2-0-0 2 Scale: 1.5'=1' 2x4 | 6.00 12 2x4 || 2x4 % 2-2-8 2-2-8 GRIP I/defl L/d PLATES DEFL (loc) SPACING 2-0-0 CSI In LOADING (psf) 244/190 Vert(LL) n/a n/a 999 MT20 TC 0.01 16.0 Plates Increase 1.25 TCLL 999 n/a 1.25 BC 0.01 Vert(TL) n/a Lumber Increase TCDL 7.0 n/a n/a 0.00 Rep Stress Incr YES WB 0.00 Horz(TL) BCLL 10.0 Weight: 7 lb Code FBC2004/TP12002 (Matrix) BCDL BRACING LUMBER TOP CHORD Structural wood sheathing directly applied or 2-2-8 oc purlins, TOP CHORD 2 X 4 SYP No.2 except end verticals. BOT CHORD 2 X 4 SYP No.2 Rigid ceiling directly applied or 10-0-0 oc bracing. **BOT CHORD** 2 X 4 SYP No.2 WEBS REACTIONS (lb/size) 1=48/2-2-8, 3=48/2-2-8 Max Horz 1=30(LC 6) Max Uplift 1=2(LC 6), 3=-21(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=-24/11, 2-3=-26/36 1-3=0/0 BOT CHORD

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25fl; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

Gable requires continuous bottom chord bearing.

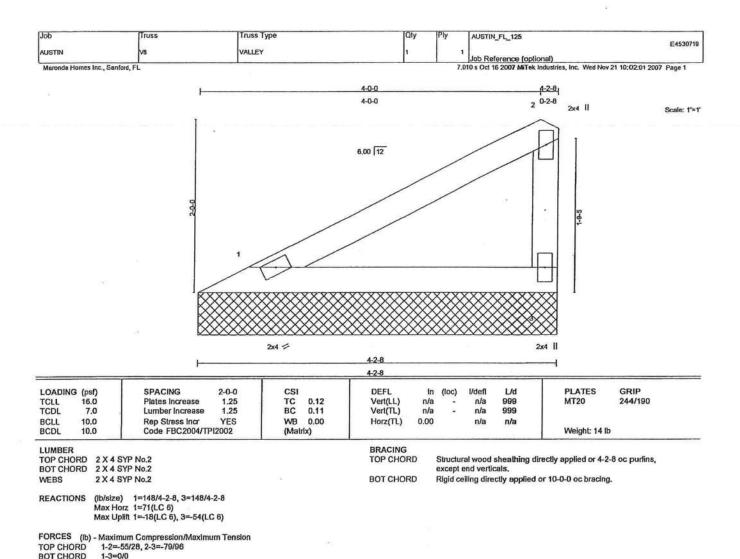
- 4) WARNING: Top chord roof live load is below minimum required by ASCE 7. The building design professional for the overall structure to verify adequacy of top chord live load.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2 lb uplift at joint 1 and 21 lb uplift at

LOAD CASE(S) Standard

This document was originally issued by Lassiter, Frank on November 21,2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated November 21,2007 above.

WARNING - Verify design paramaters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE. Design void for use only with Milek connections. This same includes not the second only upon parameters shown, and is for an individual building component. Applicability of design promenters and proper incorporation of component is responsibility of building designer not laus designer, Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the building designer, for general guidence regarding erector. Additional permanent bracing of the overall structure is the responsibility of the building designer, for general guidence regarding clockcofton, quality control, strage, detivery, erection and bracing, consult. ANSI/TPT Quality Criteria, DSS-89 and 8CST1 Building Component Safely Intermation available from Truss Plate Institute, S83 D'Onotifo Drive, Modison, WI 537 19.





NOTES

BOT CHORD

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for

- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 Gable requires continuous bottom chord bearing.
 WARNING: Top chord roof live load is below minimum required by ASCE 7. The building design professional for the overall structure to verify adequacy of top chord live load.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 18 lb uplift at joint 1 and 54 lb uplift at joint 3.

LOAD CASE(S) Standard

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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with Mifex connectors. This design is based only upon porometers shown, and is for an individual bublishing component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not trust designer, Bracing shown is for talend support of individual web members only. Additional temporary bracing to Insure Jobbs during consultations in the responsibility of the erector. Additional permanent bracing of the overal structure is the responsibility of the building designer, for general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult. AMSI/IPTI Quality Criteria, DSB-89 and BCSII Building Component Safety Information available from Truss Plate Institute, 583 D'Onafrio Drive, Mackson, Wt S3719.



AUSTIN FL 125 Truss Truss Type Qty E4530720 VALLEY AUSTIN Job Reference (optional)
7.020 s Nov 9 2007 MiTek Industries, Inc. Wed Nov 21 08:44:26 2007 Page 1 Homes Inc., Sanford, Fl 6-0-0 6-2-8 0-2-8 6-0-0 Scale = 1:20.0 2x4 11 6.00 12 2x4 || 2x4 = 2x4 || 2x4 || 6-2-8 6-2-8 GRIP SPACING DEFL PLATES LOADING (psf) 2-0-0 (loc) l/defi L/d 244/190 1.25 TC BC Vert(LL) n/a n/a 999 MT20 TCLL 16.0 Plates Increase 0.11 999 TCDL 7.0 Lumber Increase 1,25 0.08 Vert(TL) n/a n/a WB 0.03 0.00 n/a n/a BCLL 10.0 Rep Stress Incr YES Horz(TL) Code FBC2004/TPI2002 Weight: 24 lb BCDL 10.0 BRACING LUMBER TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. 2 X 4 SYP No.2 WEBS **OTHERS** 2 X 4 SYP No.2 REACTIONS (lb/size) 1=44/6-2-8, 4=118/6-2-8, 5=317/6-2-8 Max Horz 1=113(LC 6) Max Uplift4=70(LC 8), 5=167(LC 8) Max Grav 1=88(LC 6), 4=118(LC 1), 5=317(LC 1) FORCES (lb) - Maximum Compression/Maximum Tension 1-6=135/0, 2-6=125/34, 2-7=43/15, 3-7=44/20, 3-4=-59/84 1-8=0/0, 5-8=0/0, 5-9=0/0, 4-9=0/0 TOP CHORD BOT CHORD WEBS NOTES 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise)

- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Cetegory II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) Gable requires continuous bottom chord bearing.

- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 70 lb uplift at Joint 4 and 167 lb uplift at Joint 5
- 5) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 9 lb down and 57 lb up at 4-4-0, 9 lb down and 57 lb up at 4-4-0, and 47 lb down at 1-6-1, and 47 lb down at 1-6-1 on top chord, and 21 lb up at 1-6-1, 21 lb up at 1-6-1, and 17 lb down at 4-4-0, and 17 lb down at 4-4-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf) Vert: 1-3=-46, 1-4=-40

Concentrated Loads (lb) Vert: 7=-19(F=-9, B=-9) 8=41(F=21, B=21) 9=-33(F=-17, B=-17) This document was originally issued by Lassiter, Frank on November 21,2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated above.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIL-7473 BEFORE USE.

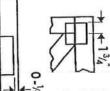
Design volid for use only with Mifek connectors. This design is based only upon parameters shown, and is for an individual building component.

Applicability of design parameters and proper incorporation of component is responsibility of building designer- not frus designer. Bracking shows its for folleral support of Individual web members only. Additional temporary bracking to insure stability during construction is the responsibility of the erector. Additional permanent brocking of the overal structure is the responsibility of the building designer. For general guidance regarding fobication, quotify contino, storage, delivery, erection and bracking, consult. AMSVIPTI Quality Criteria, DSB-89 and SCSI1 Building Component Safety Information available from Truss Plate Institute, S83 D'Onotifo Drive, Modison, WI 537 19.

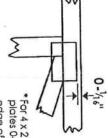


Symbols

PLATE LOCATION AND ORIENTATION



Apply plates to both sides of truss and fully embed teeth. offsets are indicated. Center plate on joint unless x, y Dimensions are in ft-in-sixteenths.



*For 4 x 2 orientation, locate plates 0-1/4" from outside

edge of truss.

This symbol indicates the required direction of slots in connector plates

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* Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE

4 × 4

width measured perpendicular to slots. Second dimension is The first dimension is the plate the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



Indicates location where bearings

ANSI/TPII: Industry Standards:

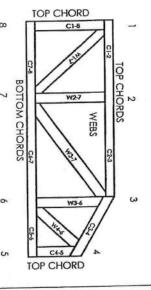
DSB-89

Plate Connected Wood Truss Construction. Design Standard for Bracing. National Design Specification for Metal

(supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Connected Wood Trusses. Installing & Bracing of Metal Plate Guide to Good Practice for Handling, **Building Component Safety Information**

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

NER-487, NER-561 95110, 84-32, 96-67, ER-3907, 9432A ESR-1311, ESR-1352, ER-5243, 9604B 9730, 95-43, 96-31, 9667A

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MITek Engineering Reference Sheet: MII-7473

dimensions shown in tt-in-sixteenths (Drawings not to scale)

General Safety Notes

Damage or Personal Injury Failure to Follow Could Cause Property

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSII.
- Truss bracing must be designed by an engineer, For wide itus spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.

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- Provide copies of this truss design to the building designer, erection supervisor, properly owner and all other interested parties.
- Cut members to bear tightly against each other.
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that
- Top chords must be sheathed or purins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- Connections not shown are the responsibility of others.
- 16. Do not cut or alter truss member or plate without prior approval of an engineer.
- 17. Install and load vertically unless indicated otherwise.
- Use of green or freated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.



OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection
This Certificate of Occupancy is issued to the below named permit holder for the building

and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 10-4S-16-02856-121

Building permit No. 000027218

Use Classification SFD/UTILITY

Permit Holder THEODORE BROCK

Waste: 184.25

Fire:

70.62

Owner of Building MARONDA HOMES INC. OF FLORIDA

Date: 11/14/2008

Location:

254 SW TIMBERRIDGE DRIVE, LAKE CITY, FL

Total: 254.87

Building Inspector

POST IN A CONSPICUOUS PLACE (Business Places Only)

LEGAL DESCRIPTION:

LOT TWENTY-ONE (21) OF "TIMBERLANDS, PHASE 1" AS PER PLAT THEREOF, AS RECORDED IN PLAT BOOK '9', PAGES 26-27 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.



BOUNDARY SURVEY

 \bar{z} SECTION EAST, 10, TOWNSHIP 4 SOUTH, RAIL, COLUMBIA COUNTY, FLORIDA TOWNSHIP RANGE

30

11

TIMBER RIDGE DRIVE

PAVEMENT

60' RIGHT-OF-WAY ±20' ASPHALT ROAD

BUILDING SETBACK INFORMATION FOR "TIMBERLANDS" IS AS FOLLOWS: FRONT 25', REAR 15', SIDE 10'

BUILDING SETBACK NOTE:

1) MARONDA HOMES

CERTIFIED TO:

ZZ BEARING BASIS 89"43"38"W 520.00" 89"43"38"W 520.06"

E

N 89'43'38'W 390.00' N 89'43'54'W 389.98' LOT 20 E S 00'16'20"W 167.53' (M) S 00'16'22"W 167.50' (P) 34.9 34.9 SETBACK LINES (TYP) 20.0' P.U.E 36.0 ZZ 89"42"41"W 130.08" WOOD FORMBOARDS
FOR CONC. 41.4 ELEV.= 96.65' 60.1 ĒĪ 24.0 18.7 ,0'09 25.0 10.0 S 00'16'22"W 167.49' S 00'17'21"W 167.49'

LOT 22

(a) (b) (ii)

TELEPHONE PEDESTAL CATY RISER

WOOD POWER POLE

IN THE OPINION OF THIS SURVEYOR, ACCORDING TO THE NATIONAL FLOOD INSURANCE PROGRAM, FLOOD INSURANCE RATE MAP COMMUNITY PANEL NO. 120070-0175-B, DATED 1-6-8B, THIS PROPERTY IS IN FLOOD ZONE "X" WHICH IS AN AREA DETERMINED TO BE OUTSIDE 500-YEAR FLOOD PLAIN, AS SCALED FROM SAID MAP. INFORMATION FROM THE FEDERAL EMERGENCY MANAGEMENT AGENCY (FEMA) FLOOD INSURANCE RATE MAPS, SHOWN ON THIS MAP, WAS CURRENT AS OF THE REFERENCED DATE. MAP REVISIONS AND AMENDMENTS ARE PERIODICALLY MADE BY LETTER AND MAY NOT BE REFLECTED ON THE MOST CURRENT MAP.

FLOOD NOTE:

TILE NOTE:

LOT 29

15.0

5)

THIS MAP OF SURVEY REFLECTS CONDITIONS LOCATED AS OF THE DATE OF FIELD WORK COMPLETION (SEE

AREAS OF ENVIRONMENTAL CONCERN HAVE NOT BEEN LOCATED BY THIS SURVEYOR, UNLESS OTHERWISE DEPICTED HEREON.

£

BUILDING SETBACK LINES DEPICTED HEREON ARE SUBJECT SHOWN AS PER THE RECORD PLAT, BUT ARE SUBJECT TO CHANGE. PRIOR TO ANY NEW CONSTRUCTION, THE APPROPRIATE GOVERNING AUTHORITY SHOULD BE CONTACTED FOR THE CURRENT SETBACK REQUIREMENTS.

3 2)

IN THE OPINION OF THIS SURVEYOR THE BOUNDARY SHOWN HEREON BEST REPRESENTS THE LOCATION OF THE SUBJECT PROPERTY IN RELATION TO THE DESCRIPTION AND THOSE PROPERTY CORNERS FOUND TO BE ACCEPTABLE TO THIS SURVEYOR.

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SURVEYOR NOTES:

TO THE BEST OF MY KNOWLEDGE, THERE ARE NO ENCROACHMENTS, BOUNDARY LINE DISPUTES, EASEMENTS, OR CLAIMS OF EASEMENTS, OTHER THAN ARE DEPICTED ON THIS DRAWING.

ALL UTILITIES AND OR IMPROVEMENTS, IF ANY, MAY NOT BE SHOWN ON THIS DRAWING.

THIS SURVEY IS SUBJECT TO ANY FACTS THAT MAY BE DISCLOSED BY A FULL AND ACCURATE TITLE SEARCH. THIS SURVEYOR HAS NOT PERFORMED A SEARCH OF THE PUBLIC RECORDS ON THIS PARCEL FOR ANY CLIMAS OF TITLE, EASEMENTS, OR RESTRICTIONS THAT MAY EFFECT THIS PARCEL THE PRESENCE OR ABSENCE OF ANY SUCH CLAIMS ARE NOT CERTIFIED HEREON.

BENCHMARK NOTE:

89"43"38"W 130.00" 89"43"39"W 130.03"

EE

28

ELEVATIONS SHOWN HEREON ARE BASED UPON A BENCHMARK SET IN A 8" PINE AT THE FRONT OF LOT 2, WITH AN ELEVATION OF 98.76'. THIS INFORMATION WAS PROVIDED TO THIS SURVEYOR BY BRITT SURVEYING (PLATTING SURVEYOR) DATUM UNKNOWN.

LEGEND:

- = FOUND 1/2" REBAR NO IDENTIFICATION
- 0 = FOUND 1/2" REBAR & CAP
- O = SET 1/2" REBAR & CAP LB. 6894
- = @ = FOUND 3/4" IRON PIPE FOUND 4" X 4" CONC. MON. NO IDENTIFICATION
- SET 4" X 4" CONC. MON. P.S.M. 5582
- X = SET NAIL & DISK P.S FOUND 6" X 6" S.R.D. R/W MON. SET NAIL & DISK P.S.M. 5582

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- ABBREVIATIONS:

 A/C = AR CONDITIONER

 ASPH = ASPHALTED FRY

 C = CALCULATED FRY

 CATY = CASBE TELEVISIO

 C/B = CONCRETE BLOC

 CLF = CHAIN LINK FEN

 CM = CONCRETE MONI Y(= AR CONDITIONER
 PH = ASPHALT
 PH = ASPHALT
 C = CALCULATED FROM MEASURED
 TV = CABLE TELEVISION
 /B = CONCRETE BLOCK
 LF = CHAIN LINK FENCE
 LF = CHAIN LINK FENCE
- = ELECTRIC = CONCRETE
- LB = LICENSED SURVEYOR BUSINESS

 (i) = FIELD MEASURED

 H = MANHOLE

 L = OVERHEAD UTILITIES

 = PLAT
- PLAT BOOK
 PUBLIC UTILITIES EASEMENT
 TRANSFORMER
 TYPICAL
 WATER METER
 WATER VALVE

CERTIFICATE OF SUKVEYUR.

NOT VALID WITHOUT THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER, ADDITIONS OR DELETIONS TO THIS MAP BY ANYONE OTHER THAN THIS SURVEYOR IS PROHIBITED.

I HEREBY CERTIFY THAT THE SURVEY DATA SHOWN HEREON, IS A TRUE AND CORRECT REPRESENTATION OF A SURVEY PERFORMED UNDER MY SUPERVISION OF THE HEREON DESCRIBED PROPERTY, AND IT MEETS THE MINIMUM TECHNICAL STANDARDS AS SET FORTH BY THE FLORIDA BOARD, OF LAND SURVEYORS, PURSUANT TO SECTION 472.027, FLORIDA STATUTES, AND CHAPPER 61617—6, FLORIDA ADMINISTRATIVE CODE.

BY.

DANTE: BRINKMAN, PM. – FLA. CERT# 5582



BRINKMAN SURVEYING & MAPPING INC.

4607 NW 6th STREET SUITE C, GAINESVILLE, FL 32609

DATE: 8/6/08 SCALE: 1" = 30" PHONE: (352) 374-7707 "THE BENCHMARK IN QUALITY SERVICE" FAX: (352) 374-8757 CHECKED BY: J.B. DRAWN BY: ZL

FIELD WORK COMPLETED ON 7/31/08 FIELDBOOK 98, PAGE 24

FOR: MARONDA

PREPARED

DRAWING NUMBER 104-08

LOT TWENTY-ONE (21) OF "TIMBERLANDS" AS PER PLAT THEREOF, AS RECORDED IN PLAT BOOK '9', PAGE 27 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA. LEGALDESCRIPTION:

ತ

MARONDA HOMES

CERTIFIED TO:

BUILDING SETBACK NOTE: BUILDING SETBACK INFORMATION FOR "TIMBERLANDS" IS AS FOLLOWS: FRONT 25', REAR 15', SIDE 10'

ROPOSEDB

jd

IN SECTION 10, TOWNSHIP 4 SOUTH, RAN 16 EAST, COLUMBIA COUNTY, FLORIDA TOWNSHIP 4 SOUTH, RANGE

30

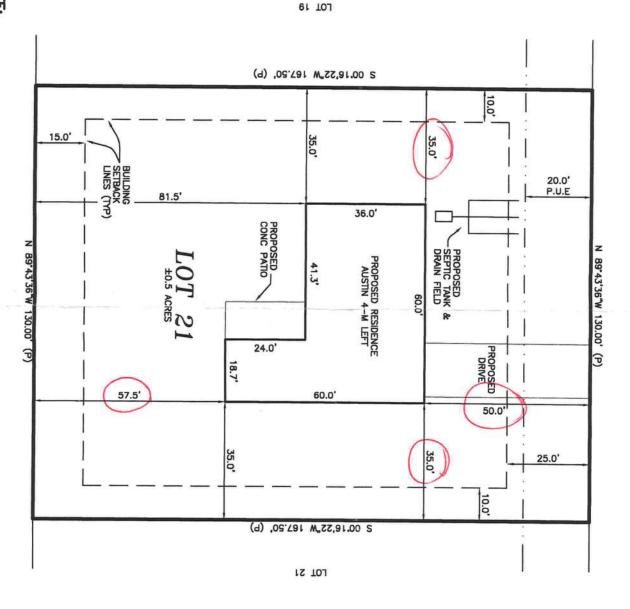
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90

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S.W. TIMBER RIDGE DRIVE



FLOOD NOTE:

IN THE OPINION OF THIS SURVEYOR, ACCORDING TO THE NATIONAL FLOOD INSURANCE PROGRAM, FLOOD INSURANCE RATE MAP COMMUNITY PAWEL NO. 120070—0175—B, DATED 1—6—8B, THIS PROPERTY IS IN FLOOD ZONE "X" WHICH IS AN AREA DETERMINED TO BE OUTSIDE 500—YEAR FLOOD PLAIN, AS SCALED FROM SAID MAP. INFORMATION FROM THE FEDERAL EMERGENCY MANAGEMENT AGENCY (FEMA), FLOOD INSURANCE RATE MAPS, SHOWN ON THIS MAP, WAS CURRENT AS OF THE REFERENCED DATE. MAP REVISIONS AND AMENDMENTS ARE PERIODICALLY MADE BY LETTER AND MAY NOT BE REFLECTED ON THE MOST CURRENT MAP.

5)

THIS MAP OF SURVEY REFLECTS CONDITIONS LOCATED AS OF THE DATE OF FIELD WORK COMPLETION (SEE

BUILDING SETBACK LINES DEPICTED HEREON ARE SHOWN AS PER THE RECORD PLAT, BUT ARE SUBJECT TO CHANGE. PRIOR TO ANY NEW CONSTRUCTION, THE APPROPRIATE GOVERNING AUTHORITY SHOULD BE CONTACTED FOR THE CURRENT SETBACK REQUIREMENTS.

4

3)

IN THE OPINION OF THIS SURVEYOR THE BOUNDARY SHOWN HEREON BEST REPRESENTS THE LOCATION OF THE SUBJECT PROPERTY IN RELATION TO THE DESCRIPTION AND THOSE PROPERTY CORNERS FOUND TO BE ACCEPTABLE TO THIS SURVEYOR.

TO THE BEST OF MY KNOWLEDGE, THERE ARE NO ENCROACHMENTS, BOUNDARY LINE DISPUTES, EASEMENTS, OR CLAIMS OF EASEMENTS, OTHER THAN ARE DEPICTED ON THIS DRAWING.

SURVEYOR NOTES:

ALL UTILITIES AND OR IMPROVEMENTS, IF ANY, MAY NOT BE SHOWN ON THIS DRAWING.

6)

TITLE BLOCK).

AREAS OF ENVIRONMENTAL CONCERN HAVE NOT BEEN LOCATED BY THIS SURVEYOR, UNLESS OTHERWISE DEPICTED HEREON.

THE NOTE:

THIS SURVEY IS SUBJECT TO ANY FACTS THAT MAY BE DISCLOSED BY A FULL AND ACCURATE TITLE SEARCH. THIS SURVEYOR HAS NOT PERFORMED A SEARCH OF THE PUBLIC RECORDS ON THIS PARCEL FOR ANY CLAIMS OF TITLE, EASEMENTS, OR RESTRICTIONS THAT MAY EFFECT THIS PARCEL THE PRESENCE OR ABSENCE OF ANY SUCH CLAIMS ARE NOT CERTIFIED HEREON.

LEGEND: ABBREVIATIONS:

- 0 | FOUND 1/2" REBAR & CAP FOUND 1/2" REBAR NO IDENTIFICATION
- 11 0 0 = SEI 1/2 REBAR & CAP

 CONC =

 FOUND 3/4" IRON PIPE

 ELEC =

 FOUND 4" X 4" CONC. MON. FNC =

 NO IDENTIFICATION

 SET 4" X 4" CONC. MON. FNC =

 SET 4" X 4" CONC. MON. (M) =

 P.S.M. 5582

 SET NAIL & DISK P.S.M. 5582

 O.U. = LB. 6894 SET 1/2" REBAR & CAP LB. 6894
- $\underset{\parallel}{\times} \underset{\parallel}{\times}$ **⊠** FOUND 6" X 6" S.R.D. R/W MON. FOUND NAIL & DISK

0 ||

® B CATV RISER ELEPHONE PEDESTAL

VOOD POWER POLE

- A/C = AIR CONDITIONER
 ASPH = ASPHALT
 C = CALCULATED FROM MEASURED
 CATY = CABLE TELEVISION
 C/B = CONCRETE BLOCK
 CLF = CHAIN LINK FENCE
 CM = CONCRETE MONUMENT
- = CONCRETE = ELECTRIC = ELEVATION = FOUND = FENCE
- B = LICENSED SURVEYOR BUSINESS
 A) = FIELD MEASURED
 H = MANHOLE
 J. = OVERHEAD UTILITIES
 P = PLAT
- = TRANSFORMER = TYPICAL = PLAT BOOK = PUBLIC UTILITIES EASEMENT WATER METER WATER VALVE

'HIS IS NOT A BOUNDARY SURVEY

CERTIFICATE OF SURVEYOR:

NOT VALID WITHOUT THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER. ADDITIONS OR DELETIONS TO THIS MAP BY ANYONE OTHER THAN THIS SURVEYOR IS PROHIBITED.

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	DATE: S
BRINKMAN SURVEYING & MAI	1 8
SURVEY	INKMAN, PSM = FLA.
ING	CERT
80	55
MA	82

TING & MAPPING INC.

PHONE: (352) 374-7707 4607 NW 6th STREET SUITE C. GAINESVILLE, FL 32609 FAX: (352) 374-8757

DATE: 5, SCALE: 1" = 30' /19/08 "THE BENCHMARK IN QUALITY SERVICE" CHECKED BY: J.B. DRAWN BY: ZL

PREPARED FOR: MARONDA FIELD WORK COMPLETED ON **** FIELDBOOK **, PAGE ** DRAWING NUMBER 104-08



GT 23 / TCI/

HOMETEAM

TREATMENT WORKORDER

12/05

☐ Termite Baiting System w/Tubes-under-the slab									
☐ Treat Only	☐ Tubes-under-the slab and Treat ☐ Bora-Care								
	DATE CALLED IN:	710	DATE OF SCHEDULE:	91.					
	DATE CALLED IN.	1130	DATE OF GONEDOLE.	011					
	TIME CALLED IN:		TIME SCHEDULE:	10:00					
JOB NAME:	0		SUBD						
JOB ADDRESS:	nda		limber						
22	545W	Timbor	Ridge Pr						
			- 1						
BILLING NAME:			BILLI	NG PHONE:					
BILLING ADDRESS:									
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16			000027	2/8					
CALLED IN BY:	PHONE:		PERMIT NUMBER						
		7.							
	UMBER:								
DATE & TIME CO	OMPLETED: 8	14/08							
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			Addition D	niveway					
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PRICE PER SQ. F	T.=	TOTAL FOR	P.T.	THE REAL PROPERTY.					
- Medical Control	tartification of the transfer of	ADDITIONA	Les de la constant d						
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I nereby acknowledge th	ne satisfactory completion	of the above describ	ed work.	/					

SCHOOL IMPACT FEE 00100003632900 CORRECTIONS IMPACT FEE 00100003632200 FIRE PROTECTION IMPACT FEE 10200003632220 EMS IMPACT FEE 10300003632210 ROAD IMPACT FEE 10100003632400 FEES: 646.00 CODE

TOTAL FEES CHARGED

,063, 67 CHECK NUMBER