

ENGINEERING DESIGN CRITERIA

VULCAN STEEL STRUCT.  
Job Number: 17679R12  
End User: SEVENTH DAY ADVENTIST CHURCH  
Job Location: LAKE CITY,FL

DATE: 10/ 1/07  
DESIGNED BY:  
DETAILED BY: TSP  
CHECKED BY: ???

DESIGN PARAMETERS

BUILDING DESCRIPTION:	SNOW :
NOMINAL WIDTH:	30 feet
NOMINAL LENGTH:	28.917 feet
EAVE HEIGHT, BACK S.W:	13 feet
EAVE HEIGHT, FRONT S.W:	13 feet
ROOF SLOPE, LEFT:	6.0:12
ROOF SLOPE, RIGHT:	6.0:12

DESIGN LOADS

BUILDING CODE: FBC 04  
(WITH 2006 AMENDMENTS)

ENGINEER'S STAMP

Richard T. Smith  
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Lagrange, Georgia Pk. (706) 888-4874  

REVIEWED  
By Richard T. Smith At 5:37 PM, Oct 16, 2007

WIND EXPOSURE: B  
CLOSURE "C, 0, P": Closed  
IMP. FACTOR, WIND : 1.00  
BUILDING CATEGORY : III

SPECIAL NOTE:

BUILDING IS NOT STRUCTURALLY SOUND UNTIL ALL WALL COVERING, ROOF SHEETS, AND PERMANENT BRACING IS INSTALLED. BUILDER / CONTRACTOR IS RESPONSIBLE FOR SUPPORTS OR TEMPORARY BRACING DURING ERECTION. HE SHALL FURNISH, AND INSTALL THESE TEMPORARY SUPPORTS WHERE NECESSARY. TEMPORARY SUPPORTS ARE NOT PROVIDED BY THE METAL BUILDING MANUFACTURER.

GENERAL NOTES

1. MATERIALS
- STRUCTURAL STEEL PLATE  
GRADE 50  
ASTM A572 OR A577 OR A570 OR A607  
GRADE 65, UNLESS NOTED  
FY= 36 KSI OR GRADE 50  
ASTM A572  
A577  
A36 OR A572  
A653 OR A792  
A307, A325, AND A490  
ASTM A570
2. STRUCTURAL PRIMER
- SNAP PRIMER PAINT IS A RUST INHIBITIVE PRIMER AND IT'S COLOR IS RED OXIDE.  
THIS PAINT IS NOT INTENDED FOR LONG TERM EXPOSURE TO THE ELEMENTS.
3. A325 BOLT TIGHTENING REQUIREMENTS
- ALL HIGH STRENGTH BOLTS ARE A325-N UNLESS SPECIFICALLY NOTED OTHERWISE.
- STRUCTURAL BOLTS SHALL BE TIGHTENED BY THE TURN-OF-THE-NUT METHOD IN ACCORDANCE WITH THE 13th EDITION, AISC "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS", PER SECTION 8.2. A325 BOLTS MAY BE INSTALLED WITHOUT WASHERS WHEN TIGHTENED BY THE TURN-OF-THE NUT METHOD.
- ALL HIGH STRENGTH BOLTS, EXCEPT AS NOTED OTHERWISE, ARE SUBJECT TO DIRECT TENSION AND MAY REQUIRE INSPECTION AS DEFINED BY THE APPLICABLE BUILDING CODE OR STANDARD. IT IS THE RESPONSIBILITY OF THE ERECTOR TO ASSURE PROPER TIGHTNESS.
4. BUILDER/CONTRACTOR RESPONSIBILITIES
- THE METAL BUILDING MANUF.'S STANDARD PRODUCT SPECIFICATIONS APPLY AND UNLESS STIPULATED OTHERWISE IN THE CONTRACT DOCUMENTS, THE METAL BUILDING MANUF.'S DESIGN, FABRICATION, QUALITY CRITERIA STANDARDS AND TOLERANCES WILL GOVERN THE WORK.
- IN CASE OF DISCREPANCIES BETWEEN METAL BUILDINGS MANUF. STRUCTURAL PLANS AND PLANS FOR OTHER TRADES, THE METAL BUILDING MANUF.'S PLANS SHALL GOVERN.
- IT IS THE RESPONSIBILITY OF THE BUILDER / CONTRACTOR TO OBTAIN APPROPRIATE APPROVALS AND NECESSARY PERMITS FROM CITY, COUNTY, STATE, OR FEDERAL AGENCIES, AS REQUIRED.
- APPROVAL OF METAL BUILDING MANUF.'S DRAWINGS CONSTITUTES THE BUILDER / CONTRACTOR'S ACCEPTANCE OF THE METAL BUILDING MANUF.'S INTERPRETATION OF THE CONTRACT PURCHASE ORDER.
- ONCE THE BUILDER / CONTRACTOR OR A/E FIRM HAS SIGNED MANUF.'S APPROVAL PACKAGE, CHANGES FROM THE PURCHASE ORDER BY THE BUILDER WILL BE BILLED TO THE BUILDER / CONTRACTOR FOR MATERIAL, ENGINEERING AND HANDLING FEES. SUCH CHANGES MAY CAUSE THE PROJECT TO BE MOVED FROM THE FABRICATION AND / OR SHIPPING SCHEDULE. A PENALTY FEE MAY BE CHARGED IF THE PROJECT MUST BE MOVED FROM THE FABRICATION AND / OR SHIPPING SCHEDULE, AS LONG AS THE MANUF.'S DESIGN AND DETAILED APPROACH COMPLIES WITH THE PURCHASE ORDER.
- THE BUILDER / CONTRACTOR OR A/E FIRM ARE RESPONSIBLE FOR THE OVERALL PROJECT CONDITION. ALL INTERFACE AND COMPATIBILITY CONCERNING ANY MATERIALS NOT FURNISHED BY THE MANUF. ARE TO BE CONSIDERED AND COORDINATED BY THE BUILDER / CONTRACTOR OR A/E FIRM. UNLESS SPECIFIC DESIGN CRITERIA CONCERNING THIS INTERFACE BETWEEN MATERIALS IS FURNISHED AS PART OF THE PURCHASE ORDER, THE METAL BUILDING MANUF.'S ASSUMPTIONS WILL GOVERN.
- THE BUILDER / CONTRACTOR IS RESPONSIBLE TO INSURE THAT ALL OTHER PROJECT PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE REQUIREMENTS OF ANY GOVERNING BUILDING AUTHORITY. SUPPLYING SEALED ENGINEERING DESIGN DATA AND DRAWINGS BY THE BUILDING MANUF. DOES NOT IMPLY OR CONSTITUTE AN AGREEMENT THAT THE BUILDING MANUF. OR ITS DESIGN ENGINEER IS ACTING AS THE ENGINEER OF RECORD OR DESIGN PROFESSIONAL FOR THE CONSTRUCTION PROJECT. THESE DRAWINGS AND DESIGN DATA ARE SEALED AS TO THE STRUCTURAL SYSTEM FURNISHED BY THE METAL BUILDING MANUF. IN COMPLIANCE WITH ALL REQUIREMENTS OF THE PURCHASE ORDER.
- THE BUILDER / CONTRACTOR IS RESPONSIBLE FOR SETTING OF ANCHOR BOLTS AND ERECTION OF STEEL BUILDING COMPONENTS IN ACCORDANCE WITH THE METAL BUILDING MANUF.'S "FOR CONSTRUCTION" DRAWINGS. TEMPORARY SUPPORTS OR BRACING REQUIRED FOR THE BUILDING ERECTION WILL BE THE RESPONSIBILITY OF THE ERECTOR TO DETERMINE, FURNISH, AND INSTALL.
- THE METAL BUILDING MANUF. DOES NOT WARRANT STRUCTURAL INTEGRITY OF ANY COMPONENTS FIELD MODIFIED OR DESIGNED AND FABRICATED BY OTHERS. NEITHER DO WE ACCEPT DESIGN RESPONSIBILITY FOR THE EFFECTS NON STANDARD COMPONENTS DESIGNED BY OTHERS MAY HAVE ON THE SYSTEM IN GENERAL.

FLORIDA PRODUCT APPROVAL INFORMATION

24 GA. ULTRA-DEK PANEL ~ NEED KYMAR COLOR	APPROVAL #
26 GA. R PANEL ~ NEED COLOR	FL5671,4
26 GA. U PANEL ~ NEED COLOR	FL5335,1
	FL5670,4

SHEETING

SKYLIGHTS

FRAMED OPENINGS

VENTILATORS

LINER PANELS

DRAWING RELEASE HISTORY

DRAWING REVISIONS

24 GA. ULTRA-DEK PANEL ~ NEED KYMAR COLOR	ROOF NO.	DESCRIPTION	COLOR	NO.	SIZE	DESCRIPTION	NO.	DESCRIPTION	WALL	THICK	TYPE	LIN FT.	HT.	COLOR	DATE SENT	REASON FOR RELEASE	REVISION	DATE	BY	DESCRIPTION
26 GA. R PANEL ~ NEED COLOR	N/A	3' X 10'-8 W/TRIM	WHITE	N/A	3' X 7'	FO.	N/A	9" X 10' VENTS W/BS & DMP.	L.E.W.	26 GA	"R"	0'	0'	PW	A					
26 GA. U PANEL ~ NEED COLOR	SOFFIT								F.S.W.	26 GA	"R"	0'	0'	PW	B					
TRIM									R.E.W.	26 GA	"R"	0'	0'	PW	C					
26 GA. NEED COLOR	GUTTER							BLANKET TYPE INSULATION	B.S.W.	26 GA	"R"	0'	0'	PW	D					
26 GA. NEED COLOR	DOWN	NO.	DESCRIPTION	COLOR											E					
26 GA. NEED COLOR	RAKE	N/A	N/A	WHITE											F					
26 GA. NEED COLOR	EAVE														G					
26 GA. NEED COLOR	CORNER														H					
26 GA. NEED COLOR	ACCESS.														I					
															J					
															K					
															L					
															M					



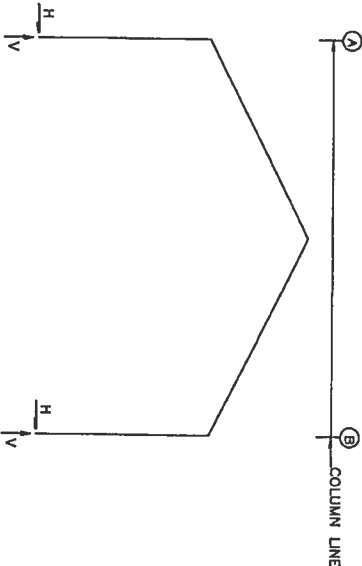
NOTES FOR REACTIONS

Building reactions are based on the following building data:

Length (ft)	= 30.0
Roof Slope (rise/run)	= 13.0/13.0
Roof Slope (rise/12)	= 6.0/6.0
Roof Live Load (psf)	= 20.0
Roof Dead Load (psf)	= 14.0
Wind Speed (mph)	= 110.0
Wind Code (3 sec gust)	= FBC 04 W/06
Exposure	= B
Importance Wind	= C
Importance Seismic	= 1.00
Seismic Zone	= A
Seismic Coeff (F <sub>a</sub> /S <sub>s</sub> )	= 0.20
AMENDMENTS	
1 DL+CL+LL	
2 DL+CL+0.75LL+0.75WL1	
3 DL+CL+0.75LL+0.75WL1	
4 0.60DL+WL1	
5 0.60DL+WL1	
6 0.60DL+WL2	
7 0.60DL+WL2	

GENERAL NOTES

- (1) APPLICATION OF ENGINEERS SEAL IS FOR METAL BUILDING ONLY AND DOES NOT REPRESENT THE PROFESSIONAL OF RECORD.
- (2) FOUNDATION DESIGN AND CONSTRUCTION ARE NOT THE RESPONSIBILITY OF THE METAL BUILDING MANUFACTURER.
- (3) ANCHOR BOLTS SHALL BE ACCURATELY SET TO A TOLERANCE OF +/- 1/8" IN BOTH ELEVATION AND LOCATION.
- (4) THE BUILDING REACTION DATA REPORTS THE LOADS WHICH THIS BUILDING PLACES ON THE FOUNDATION. THE FOUNDATION IS TO BE DESIGNED BY QUALIFIED ENGINEERS TO SUPPORT THE BUILDING REACTIONS IN ADDITION TO OTHER LOADS IMPOSED BY THE BUILDING USE OR OCCUPANCY WITH RESPECT TO JOB SITE CONDITIONS.
- (5) ALL ANCHOR BOLTS TO BE ASTM SPECIFICATION A307 UNLESS OTHERWISE NOTED.
- (6) VALUES GIVEN FOR BEAMS AND ANCHOR BOLT TOTAL LENGTHS ARE SUGGESTED LENGTHS ONLY. IT IS THE RESPONSIBILITY OF THE FOUNDATION MANAGER TO DETERMINE THESE VALUES SINCE THEY ARE A FUNCTION OF CONCRETE STRENGTH AS WELL AS OTHER FACTORS.



RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Col Line	Anchor Bolt No	Base Plate (in)	Groat (in)
2 A	4	0.750 10.00 10.63 0.625	-4.0

RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Col Line	Anchor Bolt No	Base Plate (in)	Groat (in)
3 A	4	0.750 10.00 10.63 0.625	-4.0

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Column Line	Dead	Colateral	Live	Snow	Wind L1	Wind R1
2 A	0.21	1.12	0.26	0.85	1.06	0.00
3 A	0.21	1.12	0.26	0.85	1.06	0.00
4 A	0.21	1.12	0.26	0.85	1.06	0.00
5 A	0.21	1.12	0.26	0.85	1.06	0.00
6 A	0.21	1.12	0.26	0.85	1.06	0.00
7 A	0.21	1.12	0.26	0.85	1.06	0.00
8 A	0.21	1.12	0.26	0.85	1.06	0.00
9 A	0.21	1.12	0.26	0.85	1.06	0.00
10 A	0.21	1.12	0.26	0.85	1.06	0.00
11 A	0.21	1.12	0.26	0.85	1.06	0.00
12 A	0.21	1.12	0.26	0.85	1.06	0.00
13 A	0.21	1.12	0.26	0.85	1.06	0.00
14 A	0.21	1.12	0.26	0.85	1.06	0.00
15 A	0.21	1.12	0.26	0.85	1.06	0.00
16 A	0.21	1.12	0.26	0.85	1.06	0.00
17 A	0.21	1.12	0.26	0.85	1.06	0.00
18 A	0.21	1.12	0.26	0.85	1.06	0.00
19 A	0.21	1.12	0.26	0.85	1.06	0.00
20 A	0.21	1.12	0.26	0.85	1.06	0.00
21 A	0.21	1.12	0.26	0.85	1.06	0.00
22 A	0.21	1.12	0.26	0.85	1.06	0.00
23 A	0.21	1.12	0.26	0.85	1.06	0.00
24 A	0.21	1.12	0.26	0.85	1.06	0.00
25 A	0.21	1.12	0.26	0.85	1.06	0.00
26 A	0.21	1.12	0.26	0.85	1.06	0.00
27 A	0.21	1.12	0.26	0.85	1.06	0.00
28 A	0.21	1.12	0.26	0.85	1.06	0.00
29 A	0.21	1.12	0.26	0.85	1.06	0.00
30 A	0.21	1.12	0.26	0.85	1.06	0.00
31 A	0.21	1.12	0.26	0.85	1.06	0.00
32 A	0.21	1.12	0.26	0.85	1.06	0.00
33 A	0.21	1.12	0.26	0.85	1.06	0.00
34 A	0.21	1.12	0.26	0.85	1.06	0.00
35 A	0.21	1.12	0.26	0.85	1.06	0.00
36 A	0.21	1.12	0.26	0.85	1.06	0.00
37 A	0.21	1.12	0.26	0.85	1.06	0.00
38 A	0.21	1.12	0.26	0.85	1.06	0.00
39 A	0.21	1.12	0.26	0.85	1.06	0.00
40 A	0.21	1.12	0.26	0.85	1.06	0.00
41 A	0.21	1.12	0.26	0.85	1.06	0.00
42 A	0.21	1.12	0.26	0.85	1.06	0.00
43 A	0.21	1.12	0.26	0.85	1.06	0.00
44 A	0.21	1.12	0.26	0.85	1.06	0.00
45 A	0.21	1.12	0.26	0.85	1.06	0.00
46 A	0.21	1.12	0.26	0.85	1.06	0.00
47 A	0.21	1.12	0.26	0.85	1.06	0.00
48 A	0.21	1.12	0.26	0.85	1.06	0.00
49 A	0.21	1.12	0.26	0.85	1.06	0.00
50 A	0.21	1.12	0.26	0.85	1.06	0.00
51 A	0.21	1.12	0.26	0.85	1.06	0.00
52 A	0.21	1.12	0.26	0.85	1.06	0.00
53 A	0.21	1.12	0.26	0.85	1.06	0.00
54 A	0.21	1.12	0.26	0.85	1.06	0.00
55 A	0.21	1.12	0.26	0.85	1.06	0.00
56 A	0.21	1.12	0.26	0.85	1.06	0.00
57 A	0.21	1.12	0.26	0.85	1.06	0.00
58 A	0.21	1.12	0.26	0.85	1.06	0.00
59 A	0.21	1.12	0.26	0.85	1.06	0.00
60 A	0.21	1.12	0.26	0.85	1.06	0.00
61 A	0.21	1.12	0.26	0.85	1.06	0.00
62 A	0.21	1.12	0.26	0.85	1.06	0.00
63 A	0.21	1.12	0.26	0.85	1.06	0.00
64 A	0.21	1.12	0.26	0.85	1.06	0.00
65 A	0.21	1.12	0.26	0.85	1.06	0.00
66 A	0.21	1.12	0.26	0.85	1.06	0.00
67 A	0.21	1.12	0.26	0.85	1.06	0.00
68 A	0.21	1.12	0.26	0.85	1.06	0.00
69 A	0.21	1.12	0.26	0.85	1.06	0.00
70 A	0.21	1.12	0.26	0.85	1.06	0.00
71 A	0.21	1.12	0.26	0.85	1.06	0.00
72 A	0.21	1.12	0.26	0.85	1.06	0.00
73 A	0.21	1.12	0.26	0.85	1.06	0.00
74 A	0.21	1.12	0.26	0.85	1.06	0.00
75 A	0.21	1.12	0.26	0.85	1.06	0.00
76 A	0.21	1.12	0.26	0.85	1.06	0.00
77 A	0.21	1.12	0.26	0.85	1.06	0.00
78 A	0.21	1.12	0.26	0.85	1.06	0.00
79 A	0.21	1.12	0.26	0.85	1.06	0.00
80 A	0.21	1.12	0.26	0.85	1.06	0.00
81 A	0.21	1.12	0.26	0.85	1.06	0.00
82 A	0.21	1.12	0.26	0.85	1.06	0.00
83 A	0.21	1.12	0.26	0.85	1.06	0.00
84 A	0.21	1.12	0.26	0.85	1.06	0.00
85 A	0.21	1.12	0.26	0.85	1.06	0.00
86 A	0.21	1.12	0.26	0.85	1.06	0.00
87 A	0.21	1.12	0.26	0.85	1.06	0.00
88 A	0.21	1.12	0.26	0.85	1.06	0.00
89 A	0.21	1.12	0.26	0.85	1.06	0.00
90 A	0.21	1.12	0.26	0.85	1.06	0.00
91 A	0.21	1.12	0.26	0.85	1.06	0.00
92 A	0.21	1.12	0.26	0.85	1.06	0.00
93 A	0.21	1.12	0.26	0.85	1.06	0.00
94 A	0.21	1.12	0.26	0.85	1.06	0.00
95 A	0.21	1.12	0.26	0.85	1.06	0.00
96 A	0.21	1.12	0.26	0.85	1.06	0.00
97 A	0.21	1.12	0.26	0.85	1.06	0.00
98 A	0.21	1.12	0.26	0.85	1.06	0.00
99 A	0.21	1.12	0.26	0.85	1.06	0.00
100 A	0.21	1.12	0.26	0.85	1.06	0.00

BRACING REACTIONS, PANEL SHEAR

Loc Line	Col Line	Reactions (k)	Panel Shear (lb/ft)
1 A	1	0.00	0.00
2 A	2	0.00	0.00
3 A	3	0.00	0.00
4 A	4	0.00	0.00
5 A	5	0.00	0.00
6 A	6	0.00	0.00
7 A	7	0.00	0.00
8 A	8	0.00	0.00
9 A	9	0.00	0.00
10 A	10	0.00	0.00
11 A	11	0.00	0.00
12 A	12	0.00	0.00
13 A	13	0.00	0.00
14 A	14	0.00	0.00
15 A	15	0.00	0.00
16 A	16	0.00	0.00
17 A	17	0.00	0.00
18 A	18	0.00	0.00
19 A	19	0.00	0.00
20 A	20	0.00	0.00
21 A	21	0.00	0.00
22 A	22	0.00	0.00
23 A	23	0.00	0.00
24 A	24	0.00	0.00
25 A	25	0.00	0.00
26 A	26	0.00	0.00
27 A	27	0.00	0.00
28 A	28	0.00	0.00
29 A	29	0.00	0.00
30 A	30	0.00	0.00
31 A	31	0.00	0.00
32 A	32	0.00	0.00
33 A	33	0.00	0.00
34 A	34	0.00	0.00
35 A	35	0.00	0.00
36 A	36	0.00	0.00
37 A	37	0.00	0.00
38 A	38	0.00	0.00
39 A	39	0.00	0.00
40 A	40	0.00	0.00
41 A	41	0.00	0.00
42 A	42	0.00	0.00
43 A	43	0.00	0.00
44 A	44	0.00	0.00
45 A	45	0.00	0.00
46 A	46	0.00	0.00
47 A	47	0.00	0.00
48 A	48	0.00	0.00
49 A	49	0.00	0.00
50 A	50	0.00	0.00
51 A	51	0.00	0.00
52 A	52	0.00	0.00
53 A	53	0.00	0.00
54 A	54	0.00	0.00
55 A	55	0.00	0.00
56 A	56	0.00	0.00
57 A	57	0.00	0.00
58 A	58	0.00	0.00
59 A	59	0.00	0.00
60 A	60	0.00	0.00
61 A	61	0.00	0.00
62 A	62	0.00	0.00
63 A	63	0.00	0.00
64 A	64	0.00	0.00
65 A	65	0.00	0.00
66 A	66	0.00	0.00
67 A	67	0.00	0.00
68 A	68	0.00	0.00
69 A	69	0.00	0.00
70 A	70	0.00	0.00
71 A	71	0.00	0.00
72 A	72	0.00	0.00
73 A	73	0.00	0.00
74 A	74	0.00	0.00
75 A	75	0.00	0.00
76 A	76	0.00	0.00
77 A	77	0.00	0.00
78 A	78	0.00	0.00
79 A	79	0.00	0.00
80 A	80	0.00	0.00
81 A	81	0.00	0.00
82 A	82	0.00	0.00
83 A	83	0.00	0.00
84 A	84	0.00	0.00
85 A	85	0.00	0.00
86 A	86	0.00	0.00
87 A	87	0.00	0.00
88 A	88	0.00	0.00
89 A	89	0.00	0.00
90 A	90	0.00	0.00
91 A	91	0.00	0.00
92 A	92	0.00	0.00
93 A	93	0.00	0.00
94 A	94	0.00	0.00
95 A	95	0.00	0.00
96 A	96	0.00	0.00
97 A	97	0.00	0.00
98 A	98	0.00	0.00
99 A	99	0.00	0.00
100 A	100	0.00	0.00

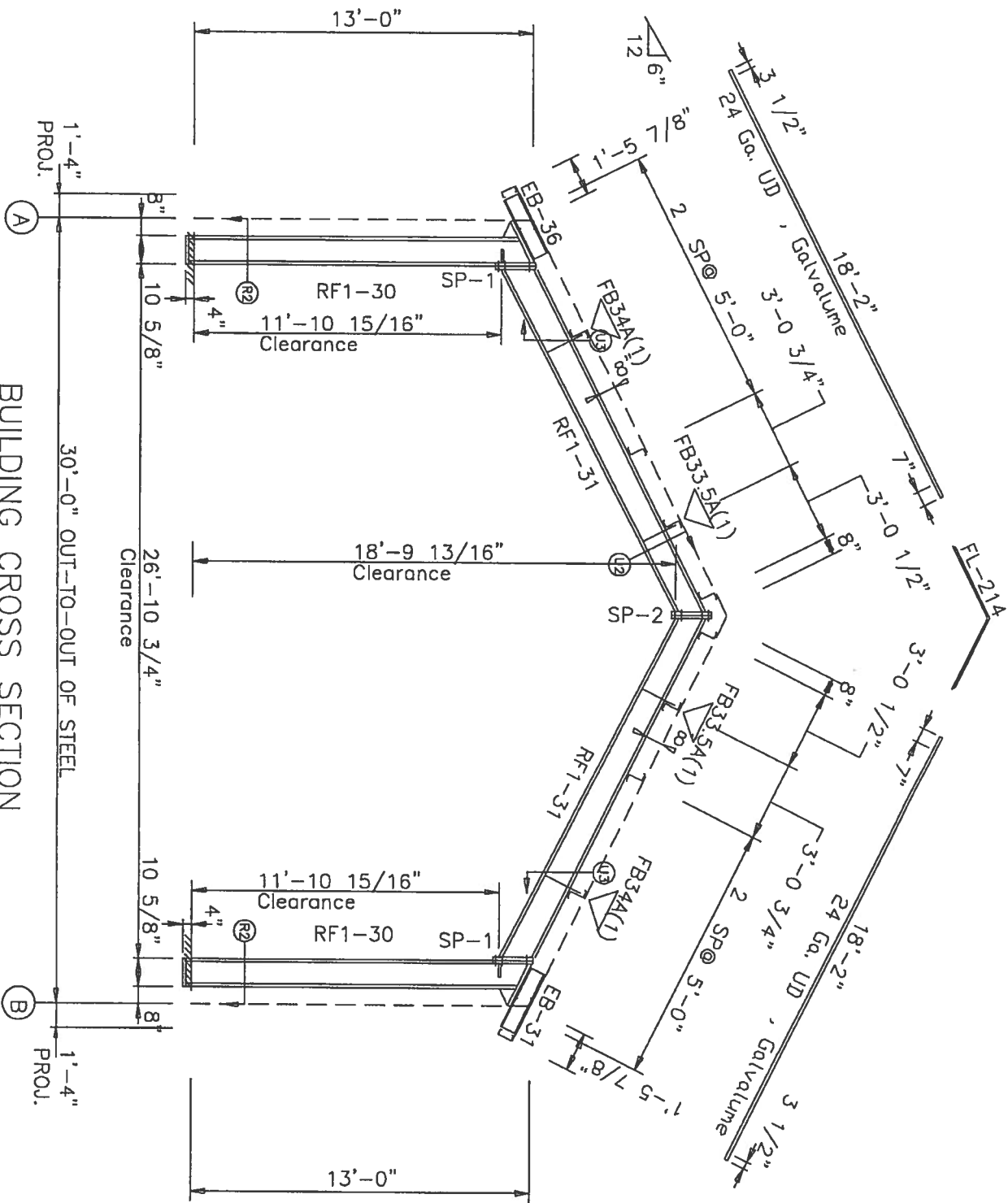
RIGID FRAME REACTIONS: WEAK AXIS BENDING

Well- Loc Line		Reactions (k)		Wind	
Line	Line	Horiz	AB, Vert	Moment	Load
E SW	B	0.9	18.2	12.1	Wind
F SW	B	0.0	1.9	0.4	Satelli
G SW	B	0.0	0.0	0.4	Satelli
H SW	B	0.0	0.6	0.4	Satelli
I SW	B	0.9	18.2	12.1	Wind
J SW	A	0.0	0.6	0.4	Wind
K SW	A	0.9	18.2	12.1	Wind
L SW	A	0.0	0.6	0.4	Wind

SPLICE BOLTS				
Splice Mark	Quan	Top/Bot	Int Type	Bolt Dia Len
SP-1	2	4	0	A325 0.750 2.00
SP-2	4	4	0	A325 0.625 1.75

FLANGE BRACES:  
FBxxxA(1): xx=inches, (1)=one side only  
A - L2X2X14G

MEMBER SIZE TABLE (in)				
PIECE	WEB DEPTH		WEB PLATE	
	START/END	THICK	LENGTH	
RF1-30	10.0/10.0	0.134	141.6	
RF1-31	10.0/10.0	0.164	17.8	
RF1-31	12.0/9.5	0.120	185.2	



BUILDING CROSS SECTION  
FOR FRAME LINE 2

GENERAL NOTES:

- \* NOTICE TO ERECTOR \*
- (A)It is IMPORTANT that for members exceeding 30 ft. in length that a spreader bar be used when lifting.
- (B)ALL flange braces and wind bracing must be installed prior to exterior finishes being applied.

REVISIONS				
REV.	DESCRIPTION	DATE	DLR	CHKR

DRAWING STATUS				
[ ] FOR CONSTRUCTION	[ ] FOR PERMIT ONLY	[ ] FOR APPROVAL	[ ] OTHER, EXPLAIN	

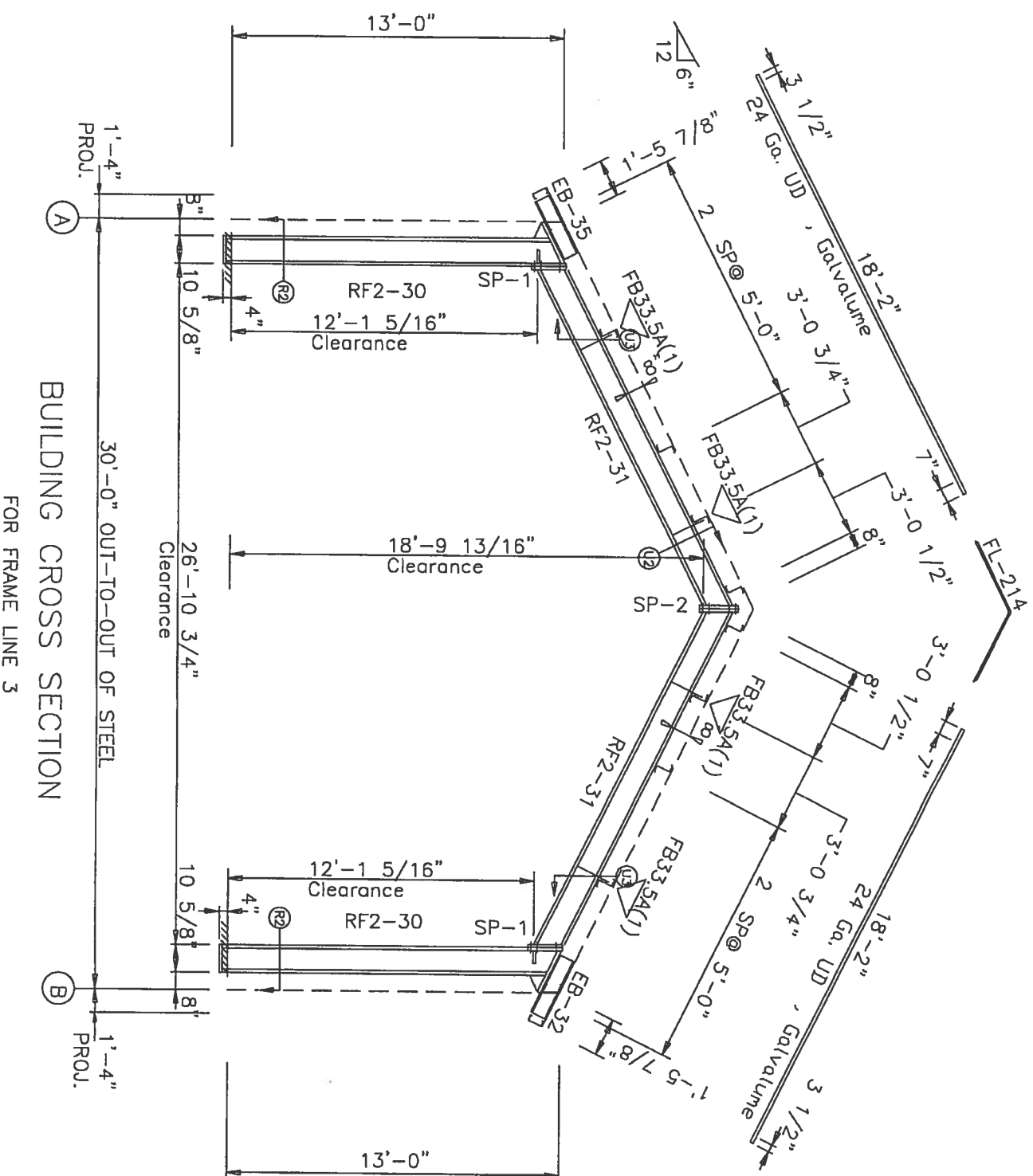
VULCAN STEEL STRUCT.				
PROJECT	3000' x 289' x 130'	BUILDING CROSS SECTION	DESIGN	DATE: 10/1/07
PROJECT	LAKE CITY FL	DRAFT: TSP	CHECK: 777	SHEET 3

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By Richard T. Smith at 5:37 pm, Oct 09, 2007

SP-LICE BOLTS				
Splice Mark	Quon	Top/Bot/Int	Type	Diag Bolt Len
SP-1	2	4	0	A325 0.750 2.50
SP-2	4	4	0	A325 0.625 1.75

▽FLANGE BRACES:  
FBxxA(1): xx=inches, (1)=one side only  
A - L2X2X14G

PIECE	MEMBER SIZE TABLE (in)			
	WEB DEPTH		WEB PLATE	
	START/END	THICK	LENGTH	
RF2-30	10.0/10.0	0.134	144.0	
	10.0/10.0	0.164	15.4	
RF2-31	10.0/9.5	0.120	184.0	



GENERAL NOTES:

\* NOTICE TO ERECTOR \*

(A)It is IMPORTANT that for members exceeding 30 ft. in length that a spreader bar be used when lifting.

(B) ALL flange braces and wind bracing must be installed prior to exterior finishes being applied.

REVIEWS							DRAWING STATUS				VILCAN STEEL STRUCT.		SEVENTH DAY ADVENTIST CHURCH					
REV.	DESCRIPTION	DATE	DLR	DATE	CHKR	APPD	<input type="checkbox"/> FOR CONSTRUCTION	<input type="checkbox"/> FOR PERMIT ONLY	<input type="checkbox"/> FOR APPROVAL	<input type="checkbox"/> OTHER, EXPLAIN	PROJECT	30'00" x 28'9 x 13'0	BUILDING CROSS SECTION	DESIGN	DRAFT	TSP	CHECK	???
											ID	1/67/9R12						
											PROJECT	LAKE CITY,FL		DATE: 10/ 1/07			SHEET	4
											ADDRESS							

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By Richard T. Smith at 5:37 pm Oct-09, 2007

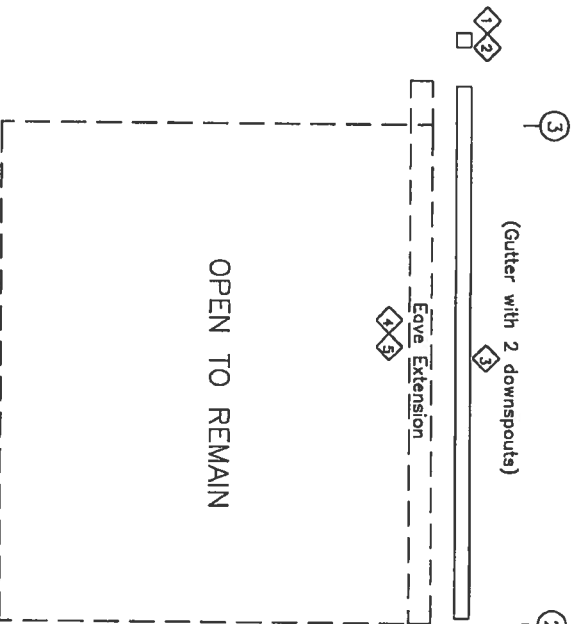
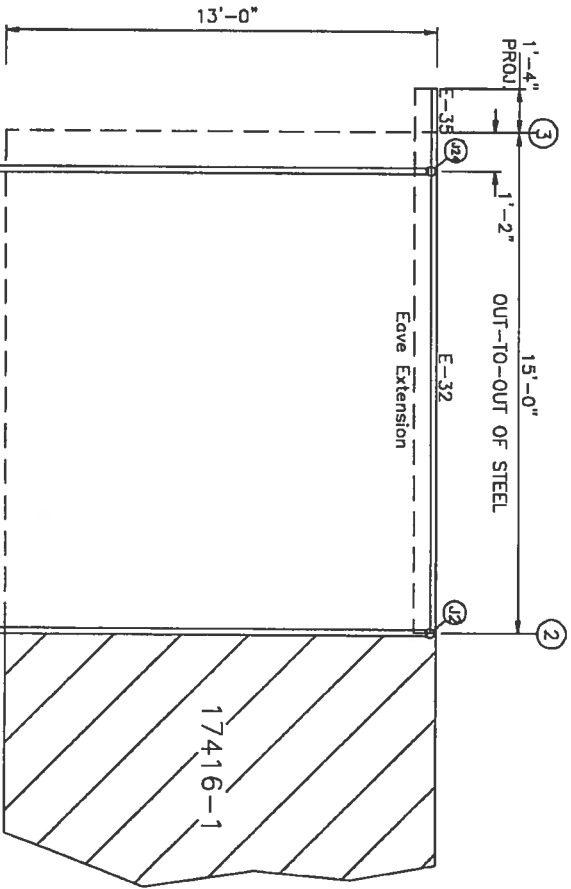
SEAL





TRIM TABLE			
FRAME LINE B			
QID	MARK	LENGTH	DETAIL
1	FL-132	6"	
2	FL-245	6"	
3	FL-240B	10'-2"	
4	CS01	15'-3"	
5	SRT	15'-3"	

MEMBER TABLE			
FRAME LINE B			
MARK	PART	LENGTH	
E-32	9E275D16	13'-2	1/2"
E-35	9E275D16	1'-10	1/2"



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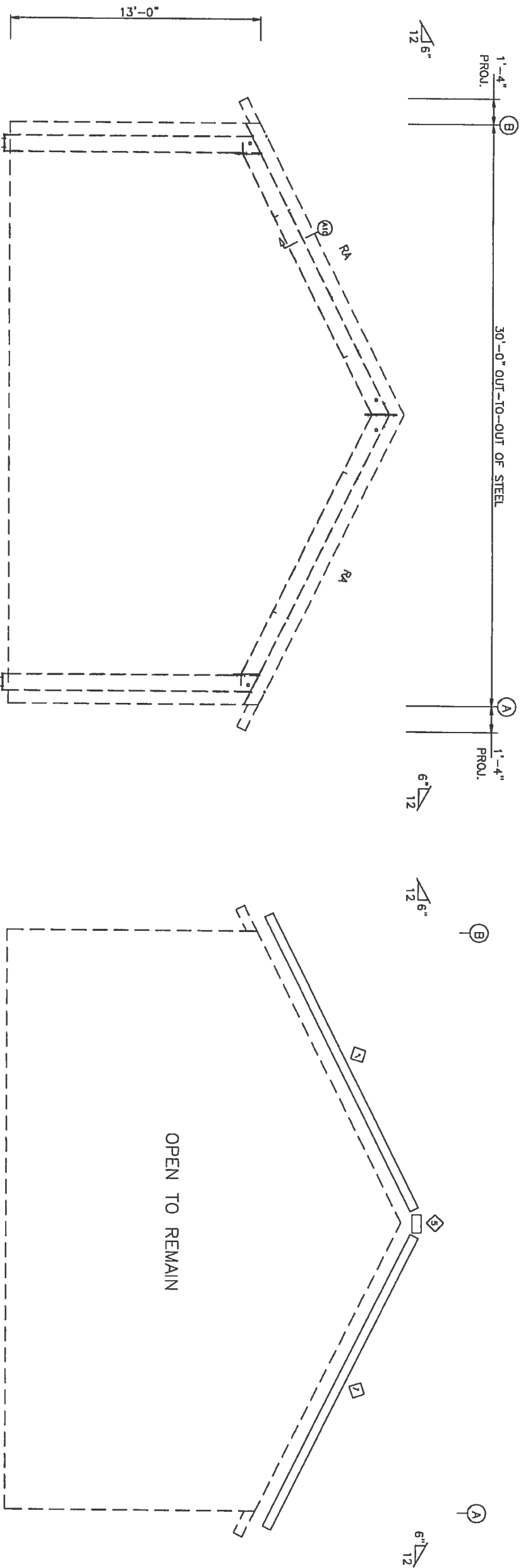
GENERAL NOTES:

(1.) IF CABLE BRACING, WIND BENIS, WIND COLUMNS, OR WEAK AXIS DESIGN OF SIDE WALL COLUMNS WERE NOT PROVIDED IT HAS BEEN DETERMINED THAT DIAPHRAGM PANEL ACTION IS SUFFICIENT TO RESIST LONGITUDINAL FORCES. TEMPORARY BRACING SHOULD BE PROVIDED BY ERECTOR UNTIL ALL WALL AND ROOF PANELS ARE INSTALLED.

REVISIONS						DRAWING STATUS		VULCAN STEEL STRUCT.		SEVENTH DAY ADVENTIST CHURCH		
REV.	DESCRIPTION	DATE	DTLR	DATE	CHKR	APPD	<input type="checkbox"/> FOR CONSTRUCTION <input type="checkbox"/> FOR PERMIT ONLY <input type="checkbox"/> FOR APPROVAL <input type="checkbox"/> OTHER, EXPLAIN -----	PROJECT	ID	SIDEWALL FRAMING	DRAFT. TSP	CHECK. ???
								3000' x 26.9 x 13.0	17679R12			
								LAKE CITY, FL				7



TRIM TABLE		
FRAME LINE 3		
OLD MARK	LENGTH	DETAIL
1 FL-110	10'-2"	
5 FL-125	2'-1"	



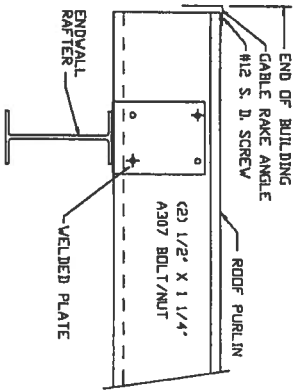
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**REVIEWED**  
By Richard T. Smith at 5:37 PM, Oct 09, 2007

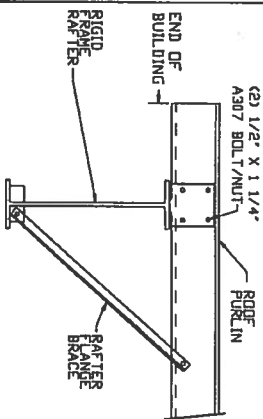
SEAL

**GENERAL NOTES:**  
(1.) IF CABLE BRACING FOR END WALL IS NOT SHOWN ON ERECTION DRAWINGS IT HAS BEEN DETERMINED THAT DIAPHRAGM PANEL ACTION IS SUFFICIENT TO RESIST LONGITUDINAL FORCES. TEMPORARY BRACING SHOULD BE PROVIDED BY ERECTOR UNTIL ALL WALL AND ROOF PANELS ARE INSTALLED.

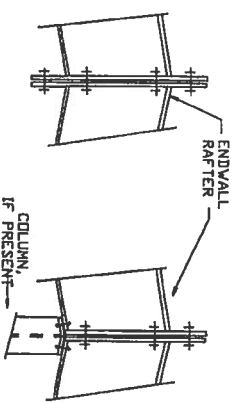
REVISIONS						DRAWING STATUS		VULCAN STEEL STRUCT.		SEVENTH DAY ADVENTIST CHURCH		
REV.	DESCRIPTION	DATE	DLR	DATE	CHKR	APPD	<input type="checkbox"/> FOR CONSTRUCTION <input type="checkbox"/> FOR PERMIT ONLY <input type="checkbox"/> FOR APPROVAL <input type="checkbox"/> OTHER, EXPLAIN	PROJECT ID	3000' x 20.9 x 13.0	ENDWALL FRAMING	DRAFT	TSP
								PROJECT	LAKE CITY, FL	DESIGN	CHECK	777
								ADDRESS		DATES	10/1/07	SHEET 8



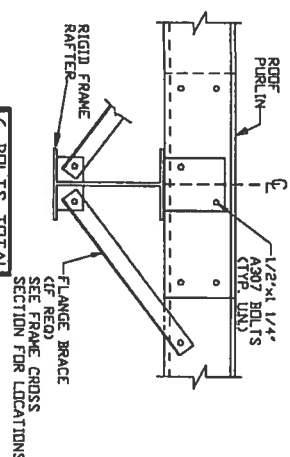
A7 SECTION THRU ENDWALL RAFTER



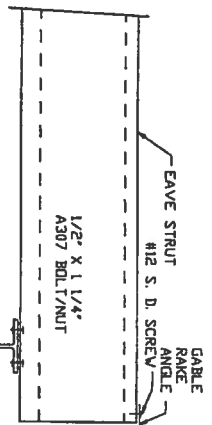
A10 ROOF PURLIN TO EXPANDABLE ENDWALL



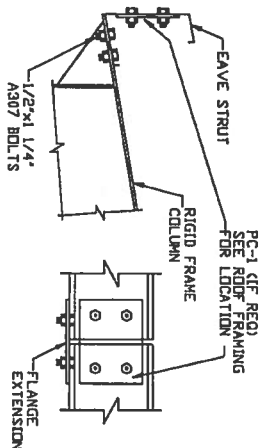
F12 RAFTER SPLICE AT SURFACE CHANGE



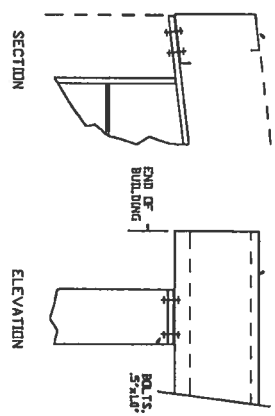
G2 ROOF PURLIN TO RIGID FRAME



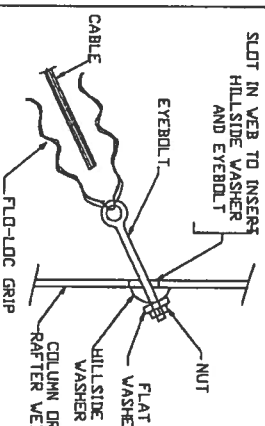
18 EAVE STRUT TO ENDWALL RAFTER



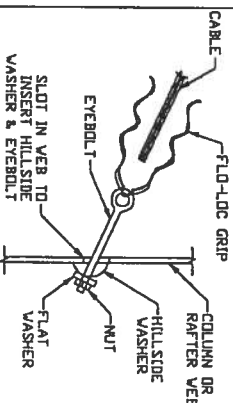
J2 EAVE STRUT TO RIGID FRAME



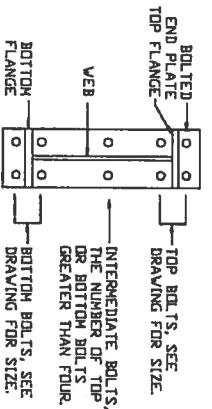
J24 EAVE STRUT TO RIGID FRAME



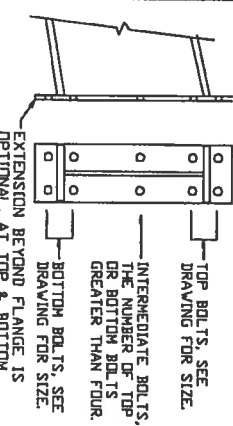
Q1 DIAGONAL CABLE, EYEBOLT END



Q2 DIAGONAL CABLE EYEBOLT END



U3 BOLTS FOR RAFTER TO COLUMN CONNECTION

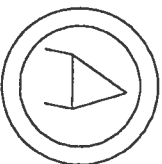
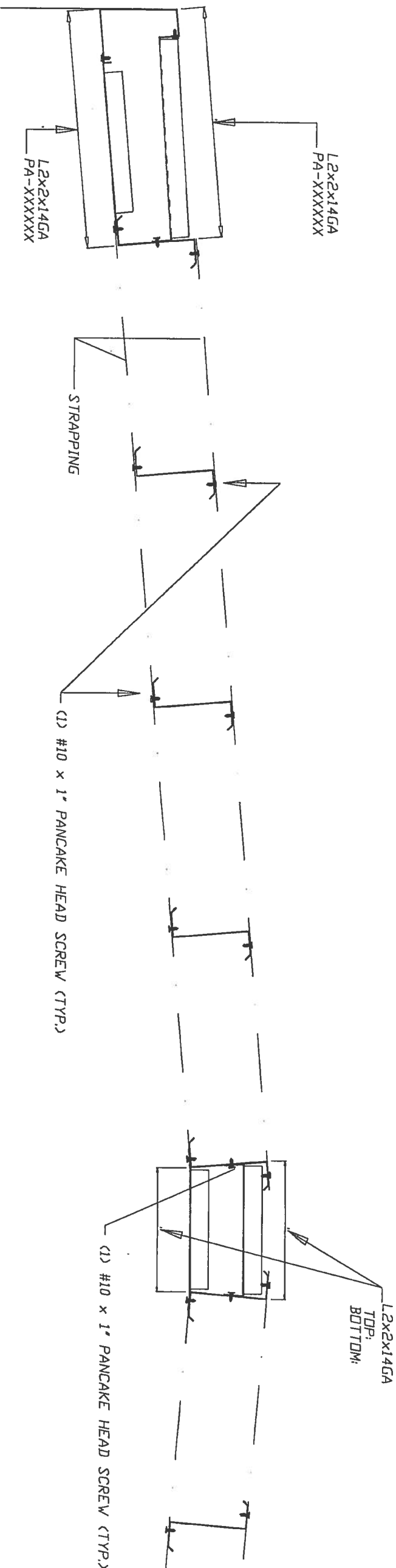


U2 BOLTED END PLATE CONNECTION AT BUILDING PEAK

REVISIONS						DRAWING STATUS				VULCAN STEEL STRUCT.		SEVENTH DAY ADVENTIST CHURCH									
REV.	DESCRIPTION	DATE	DLR	DATE	CHKR	APPD	<input type="checkbox"/> FOR CONSTRUCTION	<input checked="" type="checkbox"/> FOR PERMIT ONLY	<input type="checkbox"/> FOR APPROVAL	<input type="checkbox"/> OTHER, EXPLAIN	PROJECT	3000' x 289' x 130	DESIGN	DRAFT	TSP	CHECK	???	DATE	10/1/07	SHEET	9
											ID	17679R12									
											PROJECT	LAKE CITY, FL									
											ADDRESS										

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REVIEWED  
By Richard T. Smith at 5:37 pm, Oct 09, 2007

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PURLIN STRAPPING DETAIL

SEE ROOF PLAN FOR QUANTITY AND LOCATION  
BUILDER NOTE: KEEP ANGLES BELOW ROOF LINE,  
FIELD BEND TABS & ATTACH WITH #10 SCREWS  
(ALL ROOF PITCHES)

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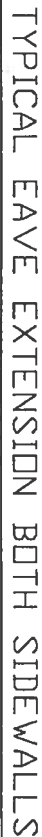
REVIEWED  
By Richard T. Smith at 6:37 pm Oct 03 2007

SEAL

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REVISIONS					DRAWING STATUS			VULCAN STEEL STRUCT.			SEVENTH DAY ADVENTIST CHURCH		
REV.	DESCRIPTION	DATE	DTLR	DATE	CHKR	APPR	<input type="checkbox"/> FOR CONSTRUCTION	<input type="checkbox"/> FOR PERMIT ONLY	<input type="checkbox"/> FOR APPROVAL	<input type="checkbox"/> OTHER, EXPLAIN	PROJECT	DATE	SHEET
											LAKE CITY, FL	10/1/07	10





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SEVENTH DAY ADVENTIST CHURCH			
Eave Ext. and Purlin Ext. Detail Page			
DESIGN:	DRAFT	TSP	CHECK: 777
DATE: 10/1/07	SHEET		12