

ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
 Florida Engineering Certificate of Authorization Number: 0 278
 Florida Certificate of Product Approval # FL1999
 Page 1 of 1 Document ID: ITWN8228Z0110104923

Truss Fabricator: Anderson Truss Company
 Job Identification: 9-188--Doug Edgley Kisa -- , **

Truss Count: 56

Model Code: Florida Building Code 2007 and 2009 Supplement

Truss Criteria: FBC2007Res/TPI-2002(STD)

Engineering Software: Alpine Software, Version 9.02.

Structural Engineer of Record: The identity of the structural EOR did not exist as of

Address: the seal date per section 61615-31.003(5a) of the FAC

Minimum Design Loads: Roof - 40.0 PSF @ 1.25 Duration

Floor - N/A

Wind - 110 MPH ASCE 7-05 - Closed

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR8228

Details: BRCLBSUB-A1101505-GBLLETIN-HIPFRAME-PB120

#	Ref	Description	Drawing#	Date
1	31410--H7A		09314012	11/10/09
2	31411--H9A		09314013	11/10/09
3	31412--H1A		09314022	11/10/09
4	31413--H13A		09314023	11/10/09
5	31414--H15A		09314024	11/10/09
6	31415--H17A		09314025	11/10/09
7	31416--H17AA		09314026	11/10/09
8	31417--A1		09314028	11/10/09
9	31418--A2		09314042	11/10/09
10	31419--A3		09314041	11/10/09
11	31420--A4		09314043	11/10/09
12	31421--H7B		09314004	11/10/09
13	31422--B4		09314036	11/10/09
14	31423--H9B		09314006	11/10/09
15	31424--B1		09314030	11/10/09
16	31425--B2		09314009	11/10/09
17	31426--B3		09314035	11/10/09
18	31427--H1B		09314007	11/10/09
19	31428--H13B		09314008	11/10/09
20	31429--H15B		09314003	11/10/09
21	31430--H17B		09314039	11/10/09
22	31431--C1		09314010	11/10/09
23	31432--C-GE		09314001	11/10/09
24	31433--D1		09314029	11/10/09
25	31434--D-GE		09314018	11/10/09
26	31435--E-GE		09314047	11/10/09
27	31436--E1		09314050	11/10/09
28	31437--F2-GDR		09314002	11/10/09
29	31438--F1		09314052	11/10/09
30	31439--F-GE		09314051	11/10/09
31	31440--J1		09314003	11/10/09
32	31441--HJ2		09314005	11/10/09
33	31442--HJ7		09314017	11/10/09
34	31443--J3		09314019	11/10/09
35	31444--J5		09314016	11/10/09
36	31445--HJ7A		09314001	11/10/09

#	Ref	Description	Drawing#	Date
37	31446--EJ7		09314020	11/10/09
38	31447--EJ7A		09314038	11/10/09
39	31448--J1A		09314015	11/10/09
40	31449--J3A		09314014	11/10/09
41	31450--J5A		09314002	11/10/09
42	31451--EJ2		09314004	11/10/09
43	31452--PF2		09314046	11/10/09
44	31453--PF1		09314040	11/10/09
45	31454--PF4		09314048	11/10/09
46	31455--PF3		09314021	11/10/09
47	31456--PB1		09314031	11/10/09
48	31457--PB2		09314027	11/10/09
49	31458--PB3		09314032	11/10/09
50	31459--PB4		09314033	11/10/09
51	31460--PB5		09314045	11/10/09
52	31461--PB6		09314037	11/10/09
53	31462--PB7		09314044	11/10/09
54	31463--PB8		09314049	11/10/09
55	31464--PB9		09314011	11/10/09
56	31465--PB10		09314034	11/10/09

Seal Date: 11/10/2009
 -Truss Design Engineer-
 Doug Fleming
 Florida License Number: 66648
 1950 Marley Drive
 Haines City, FL 33844

Handwritten signature

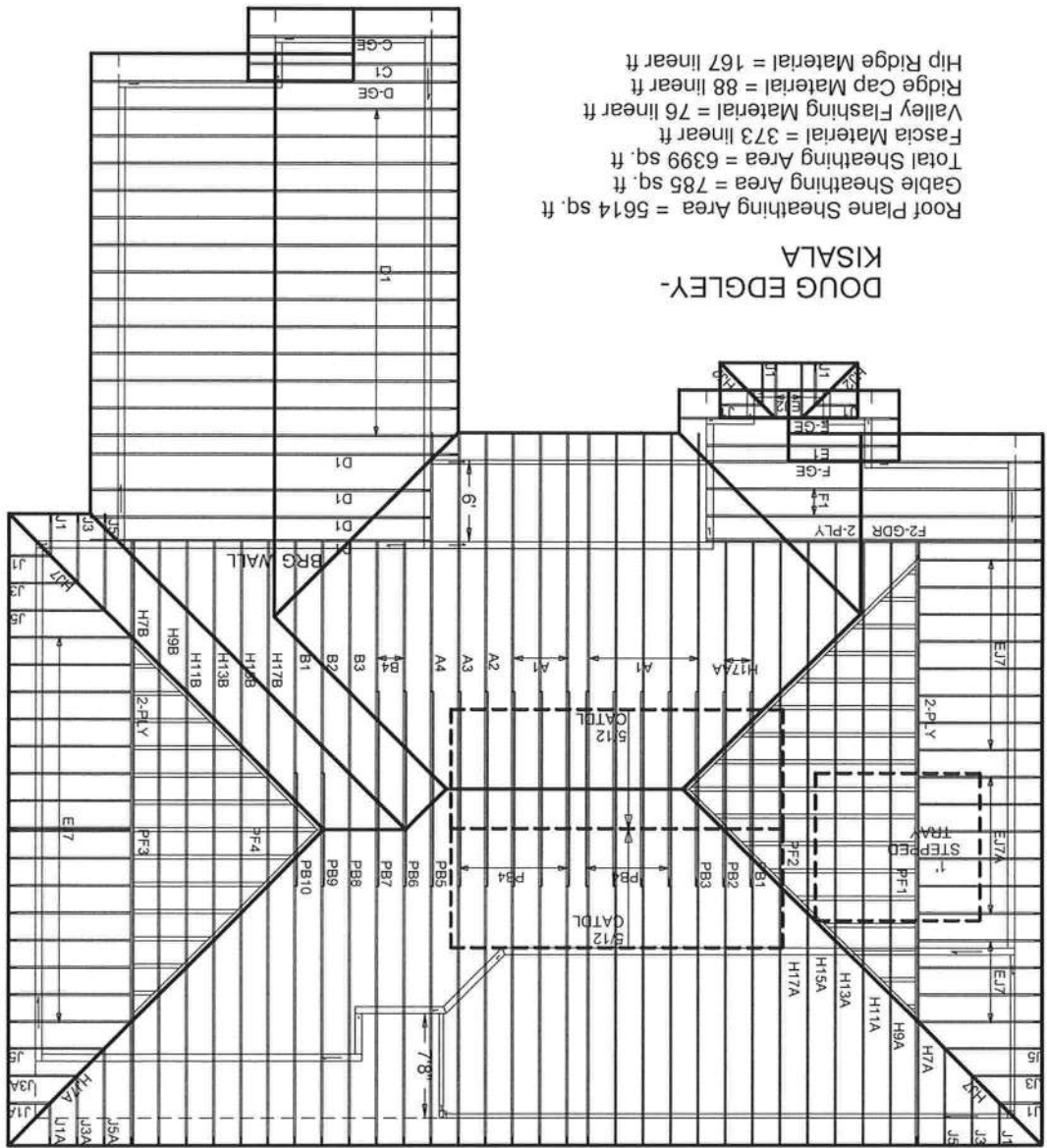


79'6"

26' 36'4" 7'8"

RooF Plane Sheathing Area = 5614 sq. ft
Gable Sheathing Area = 785 sq. ft
Total Sheathing Area = 6399 sq. ft
Fascia Material = 373 linear ft
Valley Flashing Material = 76 linear ft
Ridge Cap Material = 88 linear ft
Hip Ridge Material = 167 linear ft

DOUG EDGLEY-
KISALA



71'8" 42' 37'2" 6'2" 4'4" 24' 7' 7'

33'10" 34'8" 36' 4'2"

10'6" 6' 2' 6' 20'2" 11'6" 11'6" 6' 6'

JOB DESCRIPTION: Doug Edgley
/ Kisala

JOB NO: 9-188

PAGE NO: 1 OF 1

Top chord 2x6 SP #2 :T1 2x4 SP #2 Dense:

Bot chord 2x6 SP #2

Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, PART_ENC, bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 Gcpi (+/-)-0.55

Wind reactions based on MWFRS pressures.

Roof overhang supports 2.00 psf soffit load.

#1 hip supports 6-11-0 jacks W/2 panel TC and no end vert.

Left side jacks have 6-11-0 setback with 0-0-0 cant and 2-0-0 overhang. End jacks have 6-11-0 setback with 0-0-0 cant and 2-0-0 overhang. Right side jacks have 0-0-0 setback with 0-0-0 cant and 0-0-0 overhang.

2 COMPLETE TRUSSES REQUIRED

Nail Schedule: 0.131"x3" nails

Top Chord: 1 Row @12.00" o.c.

Bot Chord: 1 Row @12.00" o.c.

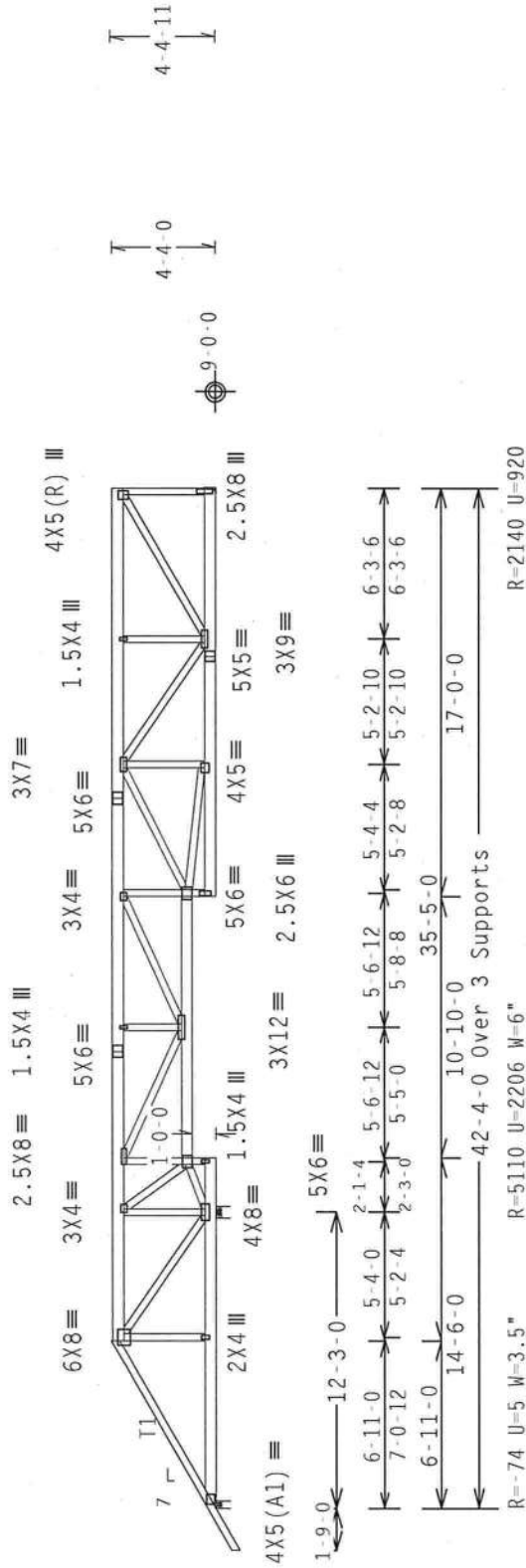
Webs : 1 Row @ 4" O.C.

Use equal spacing between rows and stagger nails in each row to avoid splitting.

Right end vertical not exposed to wind pressure.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.

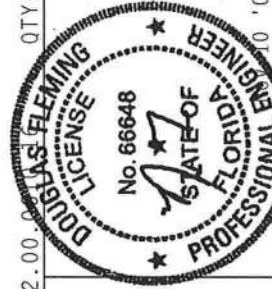


PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

QTY:1 FL/-/4/-/-/R/-
Scale =.125"/Ft.

TC LL	20.0 PSF	REF	R8228- 31410
TC DL	10.0 PSF	DATE	11/10/09
BC DL	10.0 PSF	DRW	HCUSR8228 09314012
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	69915
DUR.FAC.	1.25	FROM	AH



ALPINE

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1110 mph wind, 15.00 ft mean hgt, ASCE 7-05, PART_ENC. bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, lw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

Laterally brace BC at 24" OC in lieu of rigid ceiling. Laterally brace BC above filler at 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3 :W 2x4 SP #2 Dense:
Filler 2x4 SP #2 Dense

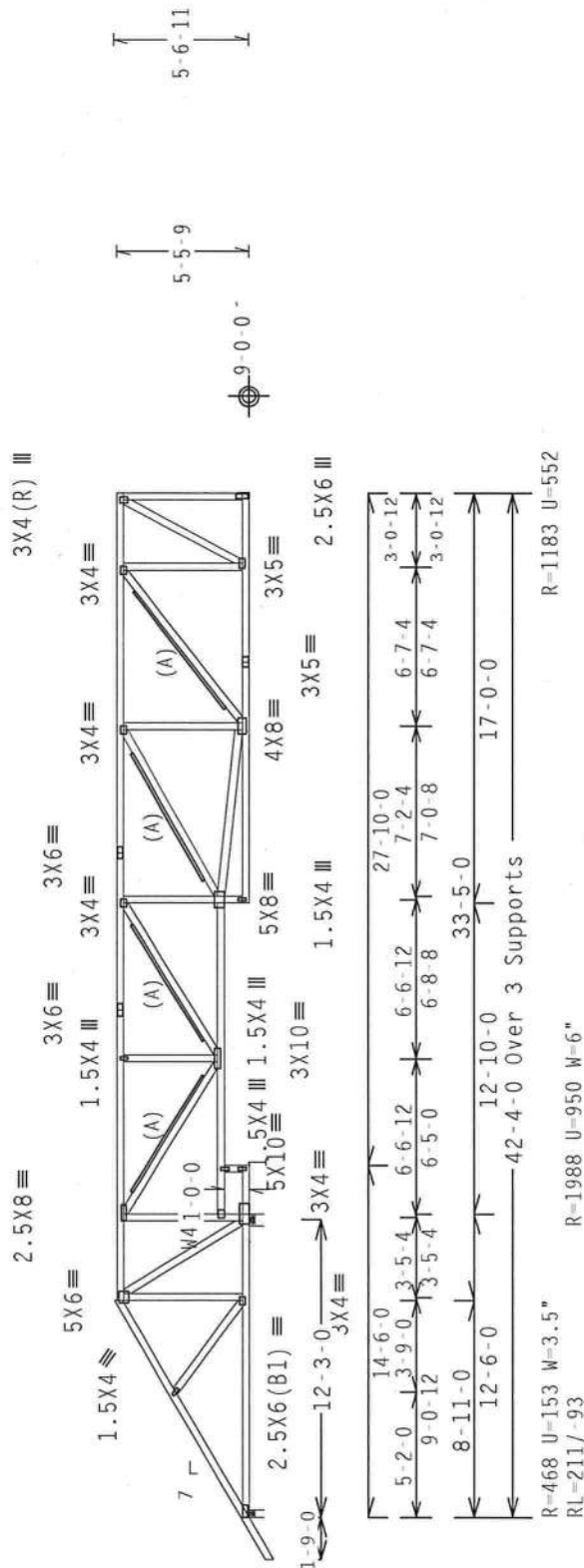
Roof overhang supports 2.00 psf soffit load.

(A) 1x4 #3SDB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.

Laterally brace chord member above filler @ 24" O.C.
Including a lateral brace at chord ends.



PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
 $FT/RT=10\%(0\%)/0(0)$

QTY:1 FL/-/4/-/-/R/-/

Scale = .125"/Ft.

TC LL	20.0	PSF	REF R8228- 31411
TC DL	10.0	PSF	DATE 11/10/09
BC DL	10.0	PSF	DRW HCUSR8228 09314013
BC LL	0.0	PSF	HC-ENG DF/DF
TOT.LD.	40.0	PSF	SEQN- 69923
DUR.FAC.	1.25		FROM AH



****WARNING**** THUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. PRIOR TO BESS' CONSIDERING COMPLETION OF ANY UNFINISHED OR UNCOMPLETED PROJECTS, CONTACT THE BUREAU OF STRUCTURAL ENGINEERING, UNIVERSITY OF CALIFORNIA, BERKELEY, CA 94720-1780, FOR A COMPLETE LIST OF THUSSE CONTRACTORS. WITHOUT THIS PRELIMINARY CONSULTATION, THE USER ASSUMES ALL LIABILITY FOR ANY DAMAGE TO PERSONS OR PROPERTY. IF ANOTHERISE LABEL #6010150N, #1 537/19, IS APPLIED TO A THUSSE, IT INDICATES THAT THE THUSSE IS NOT DESIGNED FOR USE AS AN OTHERISE. OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITN BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DAMAGE TO THE TRUSS IN COMPLIANCE WITH THE FOLLOWING:

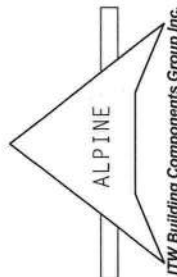
(1) CUTTING OR FABRICATING HANDLING, SHIPPING, INSTALLING, OR DRAGING OF TRUSSES.

(2) DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., BY AF&PA) AND IPT.

(3) CONNECTOR PLATES ARE MADE OF 20/18 TONGA (W-1/55%) ASH 4653 GRADE 40/60 (N-KY-55) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2 THROUGH 160D-2.

(4) ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMERX 43 OF 17-11-2002 SEC. 3.

(5) ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DURING INDICATES.



ITW Building Components Group Inc.

	Top	chord	2x4	SP	#2	Dense
	Bot	chord	2x4	SP	#2	Dense
		webs	2x4	SP	#3	:W5 2x
		Filler	2x4	SP	#2	Dense

1110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

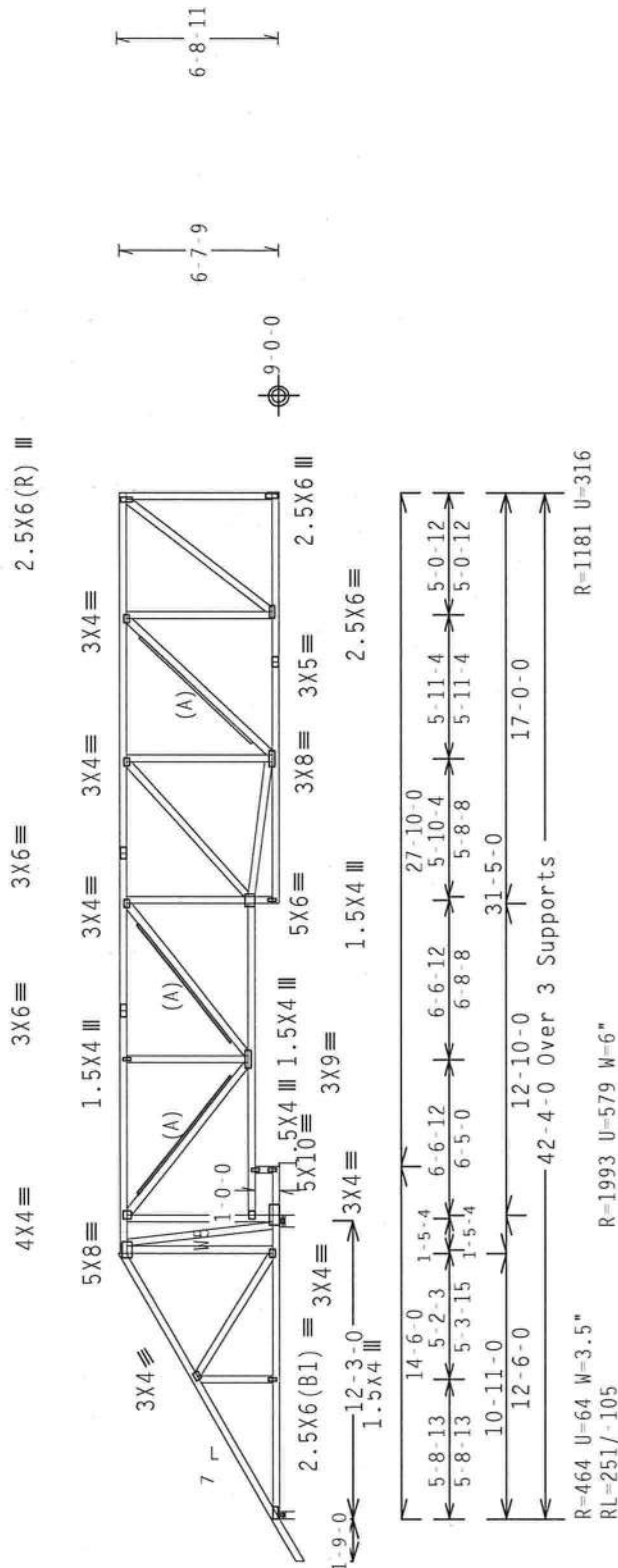
• **Supply** is the amount of a good or service that producers are willing and able to supply at a given price.

(A) 1x4 #3SRB SPF-S or better "J" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.) nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.

Laterally brace chord member above filler @ 24" O.C.
Including a lateral brace at chord ends.



PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
 $FT/RT=10\%(0\%)/0(0)$

00

QTY:1

FL/-/4/-/-/R/-

Scale = .125"/Ft.

REF R8228- 31412

DATE 11/10/09

ORW HCUSR8228 09314022

IC-ENG DF/DF

CFON- 69930

	MUR	AH
0000		

ALL

0	•
1	•
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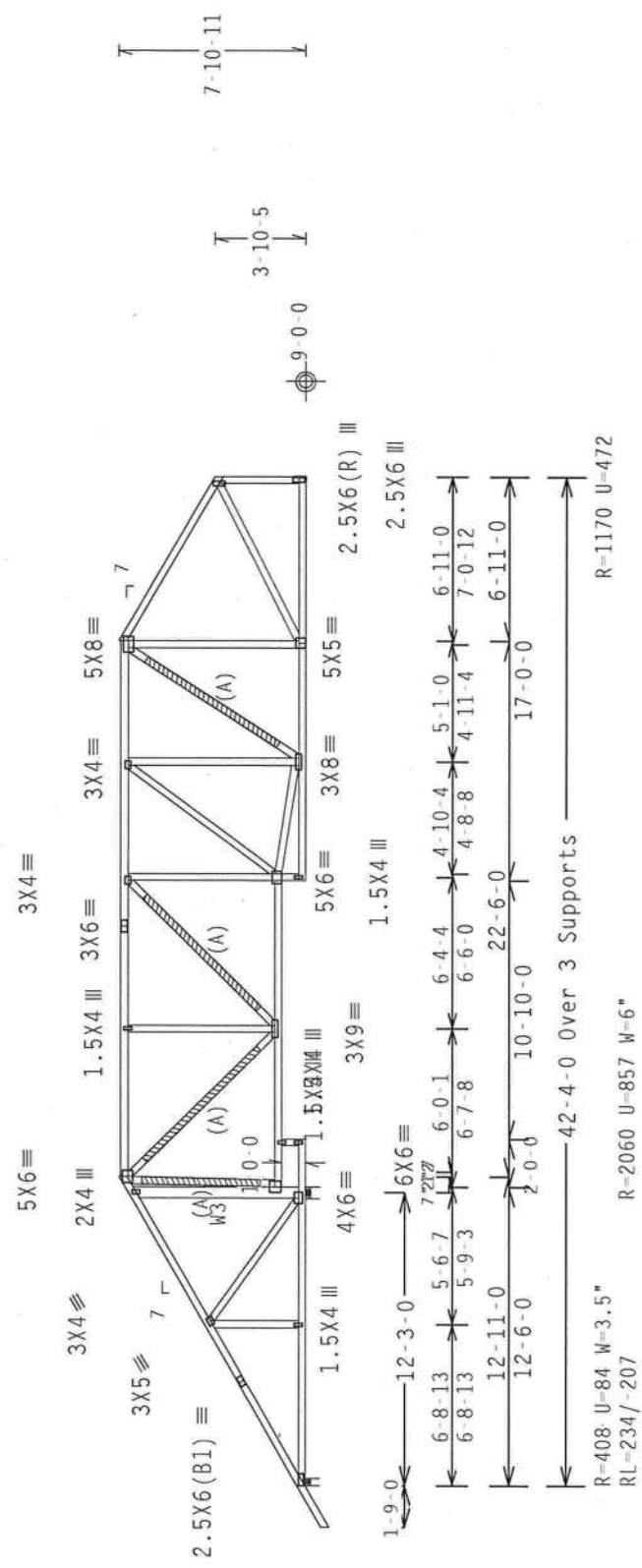
60 OT

DRAMA INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT

1111 Building Components Group Inc.

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3 :W3 2x6 SP #2:
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, PART-ENC. bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, wind IC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ $G_{CPI}(+/-)=0.55$

Wind reactions based on MWFRS pressures.
Right end vertical not exposed to wind pressure.
In lieu of structural panels use purlins to brace all flat IC @ 24" OC.
MWFRS loads based on trusses located at least 15.00 ft. from roof edge.
Roof overhang supports 2.00 psf soffit load.
(A) #3 or better scab brace. Same size & 80% length of web member. Attach with 10d Box or Gun (0.128"x3", min.) nails @ 6" OC.
Bottom chord checked for 10.00 psf non-concurrent live load.
Deflection meets L/240 live and L/180 total load.
Laterally brace chord member above filler @ 24" O.C. Including a lateral brace at chord ends.

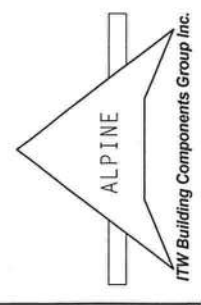


Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave	QTY:1	FL/-4/-1-R/-	Scale = .125"/Ft.
	TC LL	20.0 PSF	REF R8228- 31413
	TC DL	10.0 PSF	DATE 11/10/09
	BC DL	10.0 PSF	DRW HCUR8228 09314023
	BC LL	0.0 PSF	HC-ENG DF/DF
	TOT.LD.	40.0 PSF	SEQN- 69547
	DUR.FAC.	1.25	FROM AH

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO RC51 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN OR PLACING WITH ANY OTHER PROVISIONS OF THIS (NATIONAL DESIGN SPEC. BY AFPA) AND TPI. ITW BCG PLATES TO EACH FACE OF TRUSS AND UNLESS OTHERWISE LOCATED ON THIS DESIGN. POSITION PER DIMENSIONS 100A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ARJF A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT



110 mph wind, 15.00 ft mean hgt, ASCE 7-05, PART. ENC. bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Roof overhang supports 2.00 psf soffit load.

(A) #3 or better scab brace. Same size & 80% length of web member. Attach with 10d Box or Gun (0.128"x3", min.) nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

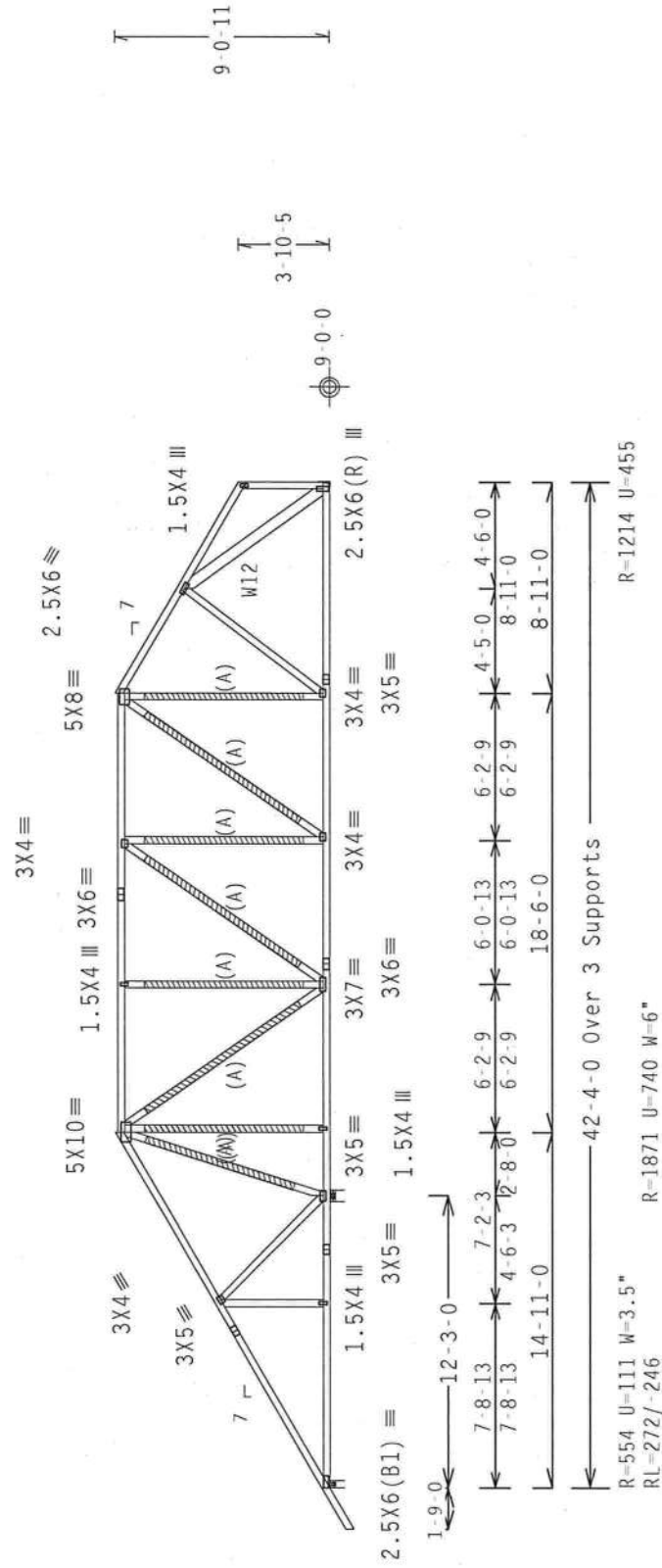
Deflection meets L/240 live and L/180 total load.

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

Design Crit: FBC2007Res/TPI-2002 (STD)
 $FT/RT=10\%(0\%)/0(0)$

PLT TYP. Wave

QTY:1 FL/-/4/-/-/R/-

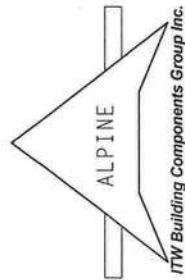
Scale = .125"/Ft.

***WARNING:** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST AVAILABLE COMPONENT SAFETY INFORMATION. PUBLISHED BY IPT (CURTIS PEASE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALTA VISTA, VA 22314) FOR THE AMERICAN COUNCIL OF AMERICA, 6300 UNIVERSITY MICROFILMS INTERNATIONAL, ANN ARBOR, MI 48109-3003.
TERPSE LAKE, MADISON, WI 53719). FOR SAFETY PRACTICES AND PREPAREDNESS FOR THESE FUNCTIONS, UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

TC LL	20.0	PSF	REF	R8228-	31414
TC DL	10.0	PSF	DATE	11/10/09	
BC DL	10.0	PSF	DRW	HCUSR8228	09314024
BC LL	0.0	PSF	HC-ENG	DF/DF	
TOT.LD.	40.0	PSF	SEQN-	69554	
DUR.FAC.	1.25		FROM	AH	

A circular professional engineer seal for the State of Florida. The outer ring contains the text "FLORIDA PROFESSIONAL ENGINEER" at the top and "STATE OF FLORIDA" at the bottom, separated by two stars. Inside the ring, the words "LICENSE" and "No. 66648" are printed. A stylized signature, "D. Williams", is written across the center of the seal.

****IMPORTANT****-TURKISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE OR INABILITY TO BUILD THE TRUSS IN CONFORMANCE WITH THIS DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NON-QUALIFICATION SPEC. BY ATAPRA AND TP1. ITW REG. CORROSION RESISTANT PLATES ARE MADE OF 20/19/16GA H/S/AS/25/85 GRADE 40/60 H/ K/H/SS GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSSES AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. IF ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TP1 2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT.



ITW Building Components Group Inc.

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, PART. ENC. bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi (+/-) 0.55

Roof overhang supports 2.00 psf soffit load.

Wind reactions based on MWFRS pressures.

(A) #3 or better scab brace. Same size & 80% length of web member. Attach with 10d Box or Gun (0.128"x3", min.) nails @ 6" OC.

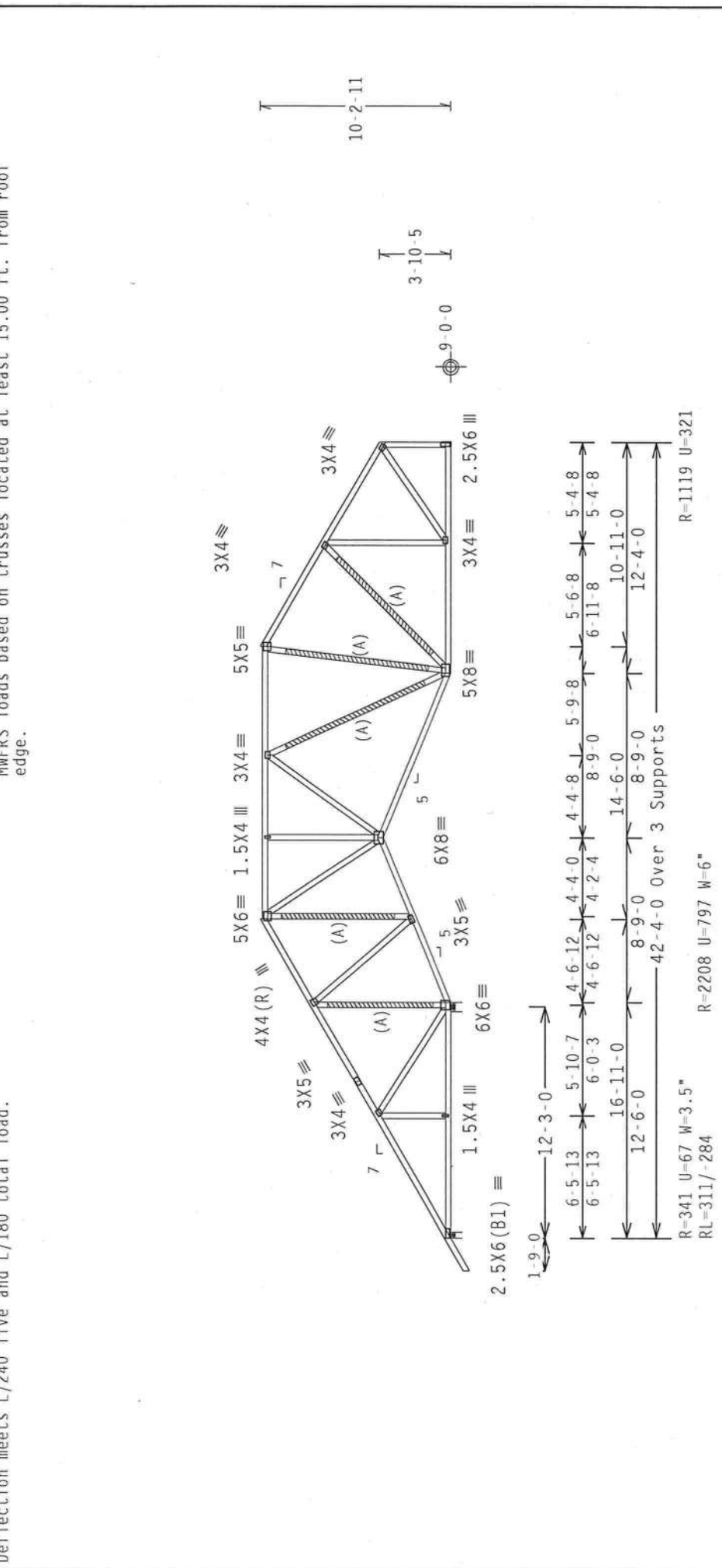
Right end vertical not exposed to wind pressure.

Bottom chord checked for 10.00 psf non-concurrent live load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.



PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10%(0%)/0(0)

Scale = 1/25" / Ft.

ALPINE

ITW Building Components Group Inc.

DTG & S. FLEMING
LICENSE
No. 66648
FLORIDA
PROFESSIONAL ENGINEER

10 '09

TC LL	20.0 PSF	REF	R8228- 31415
TC DL	10.0 PSF	DATE	11/10/09
BC DL	10.0 PSF	DRW	HCUSR8228 09314025
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEON-	69934
DUR.FAC.	1.25	FROM	AH

Top	chord	2x4	SP	#2	Dense
Bot	chord	2x4	SP	#2	Dense
	webs	2x4	SP	#3	

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, PART_ENC. bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. lw=1.00 GCpi(+/-)=0.55

Roof overhang supports 2.00 psf soffit load.

Wind reactions based on MWFRS pressures.

(A) Continuous lateral bracing equally spaced on member.

Right end vertical not exposed to wind pressure.

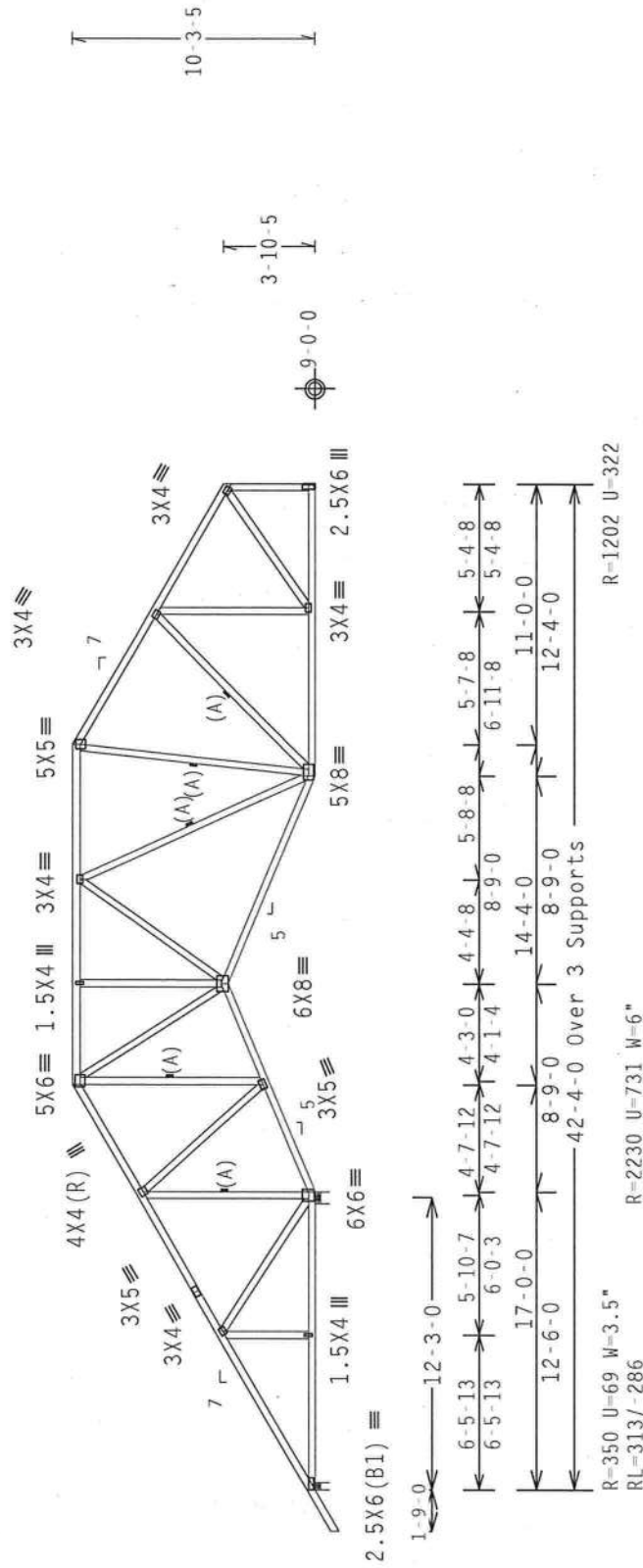
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.



PLT TYP. Wave

Design Crit: FBC2007Res/TPI - 2002(STD)
 $FT/RT=10\%(0\%)/0(0)$

OTY:2 FL/-/4/-/-/R/- Scale = .125"/Ft.

TC LL	20.0	PSF	REF R8228- 31416
TC DL	10.0	PSF	DATE 11/10/09
BC DL	10.0	PSF	DRW HCUR8228 09314026
BC LL	0.0	PSF	HC-ENG DF/DF
TOT.LD.	40.0	PSF	SEQN- 69758
DUR.FAC.	1.25		FROM AH



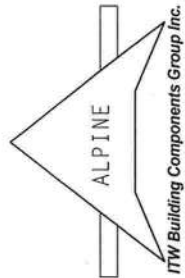
****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST AVAILABLE SUPPORTING INFORMATION, PUBLISHED BY THE TRUSS PLATE INSTITUTE, 2100 ENTERPRISE LANE, MOULDSBORO, OH 43103, FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CELLS.

****IMPORTANT****TURKISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANYTHING OTHER THAN THE THROSS IN CONFORMANCE WITH THE DRAWING INDICATES. ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE DESIGN AND FABRICATION OF THE THROSS COMPONENT.

DESIGNER'S COMMENTS: THE THROSS IS TO BE INSTALLED IN ACCORDANCE WITH THE FOLLOWING REQUIREMENTS:

- 1. THE THROSS SHALL BE MADE OF 1018 STEEL, GALV., STEEL, APPLY PLATES TO EACH FACE OF THROSS AND, OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS T60A-2.
- 2. THE THROSS SHALL BE WELDMENTED TO THE STRUCTURE IN ACCORDANCE WITH THE FOLLOWING REQUIREMENTS:
 - a. THE WELDMENT SHALL BE MADE OF 1018 STEEL, GALV., STEEL, APPLY PLATES TO EACH FACE OF THROSS AND, OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS T60A-2.
 - b. THE WELDMENT SHALL BE WELDMENTED TO THE STRUCTURE IN ACCORDANCE WITH THE FOLLOWING REQUIREMENTS:
 - i. THE WELDMENT SHALL BE MADE OF 1018 STEEL, GALV., STEEL, APPLY PLATES TO EACH FACE OF THROSS AND, OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS T60A-2.

ITW BCG
T60A-2



ITW Building Components Group Inc.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, PART ENC, bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, wind IC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCpi(+/-)=0.55

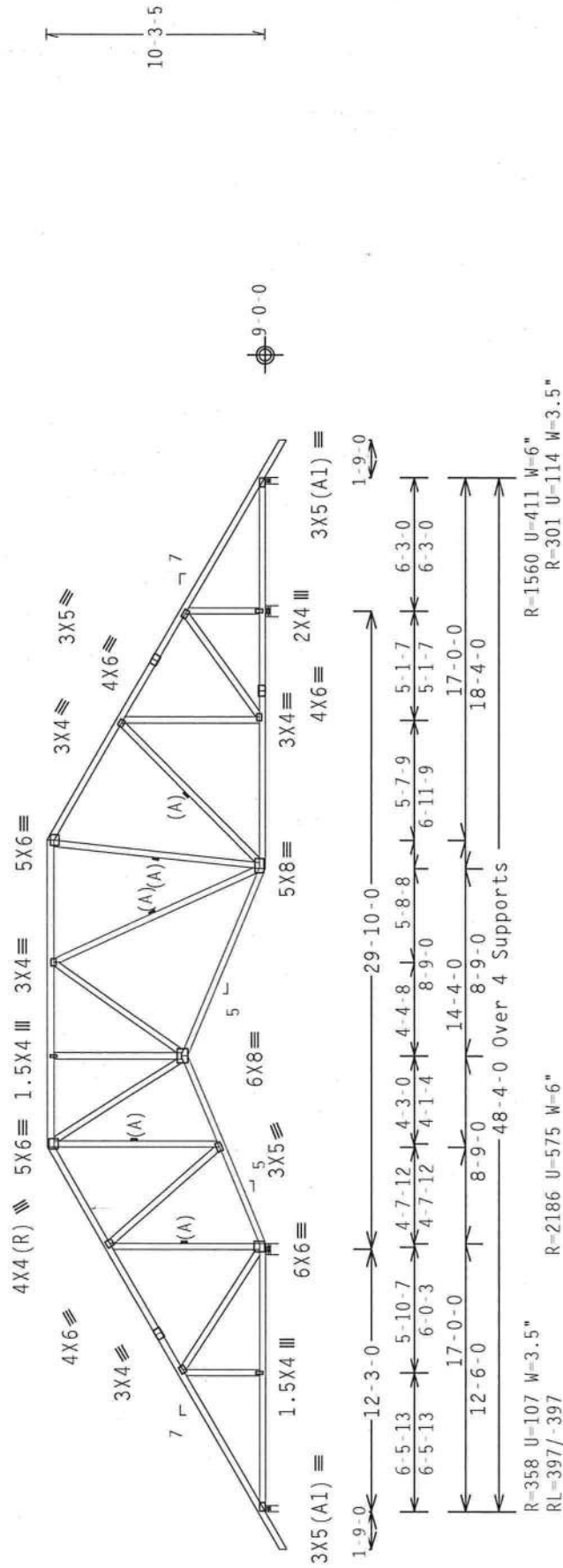
Wind reactions based on MWFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

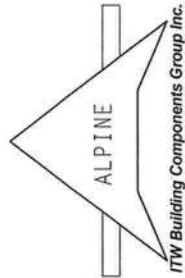


QTY:8 FL/-/4/-/-/R/-/-
Scale = .125" / Ft.

QTY: 8 FL / - / 4 / - / - / R / -

TC LL	20.0 PSF	REF R8228- 31417
TC DL	10.0 PSF	DATE 11/10/09

BC LL	10.0 PSF	DKW MC03NR62Z6 03514022
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 69737
DUR.FAC.	1.25	FROM AH



ITW Building Components Group Inc.

(9-188--Doug Edgley Kisala -- ** - A2)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, PART-ENC. bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, $1w=1.00 G_{Cpi}(+/-)-0.55$

Roof overhang supports 2.00 psf soffit load.

Wind reactions based on MWFRS pressures.

(A) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

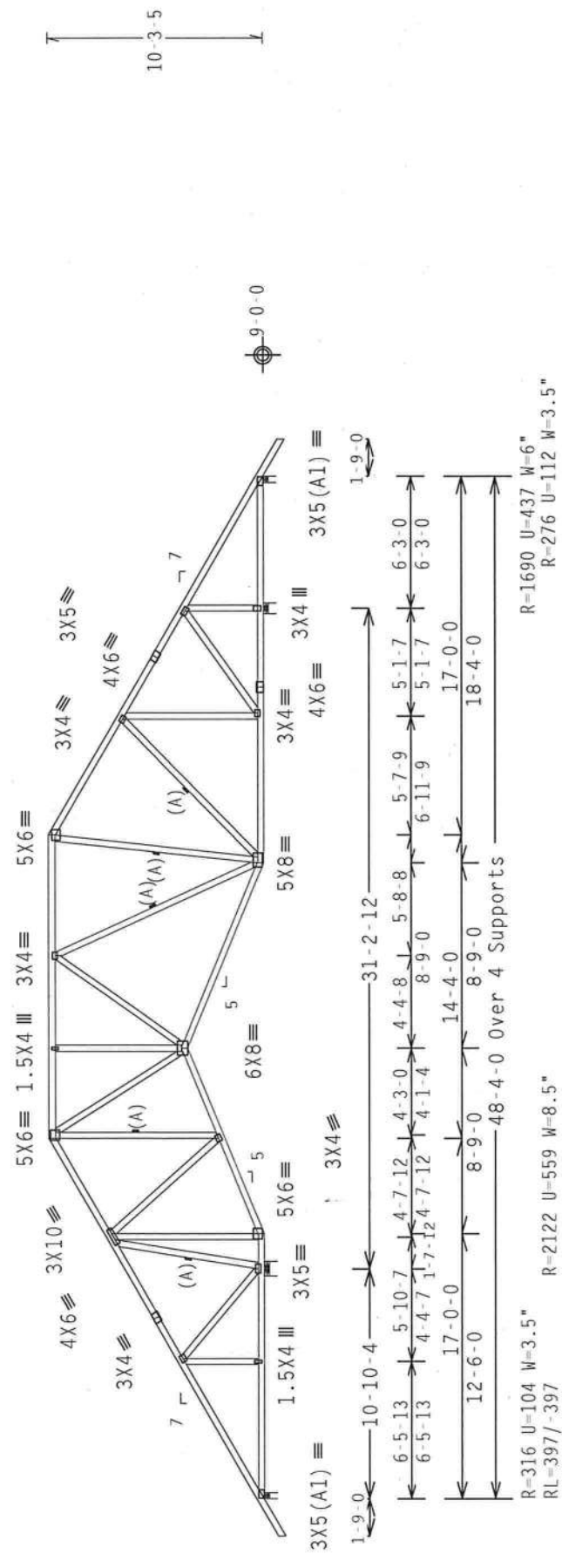
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.



Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave	QTY:1	FL/-/4/-/-/R/-	Scale =.125"/Ft.
	TC LL	20.0 PSF	REF R8228- 31418
	TC DL	10.0 PSF	DATE 11/10/09
	BC DL	10.0 PSF	DRW HCUSR8228 09314042
	BC LL	0.0 PSF	HC-ENG DF/DF
	TOT.LD.	40.0 PSF	SEQN- 69741
	DUR.FAC.	1.25	FROM AH

DOUGLAS FLEMING
LICENSE
No. 66648
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

10 '09

ALPINE

ITW Building Components Group Inc.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSE (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 238 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND MICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. ITW BCG DESIGN CONFORMS WITH ALL APPLICABLE PROVISIONS OF THE INTERNATIONAL BUILDING CODE (IBC) AND THE 2006 INTERNATIONAL RESIDENTIAL CODE (IRC) ON KILN-DRYED LUMBER. ITW BCG SHALL BE RESPONSIBLE FOR THE DESIGN OF THE TRUSS AND THE INSTALLATION OF THE TRUSS. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN.

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, PART-ENC. bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GGpi(+/-)-0.55

Roof overhang supports 2.00 psf soffit load.

Wind reactions based on MWFRS pressures.

(A) Continuous lateral bracing equally spaced on member.

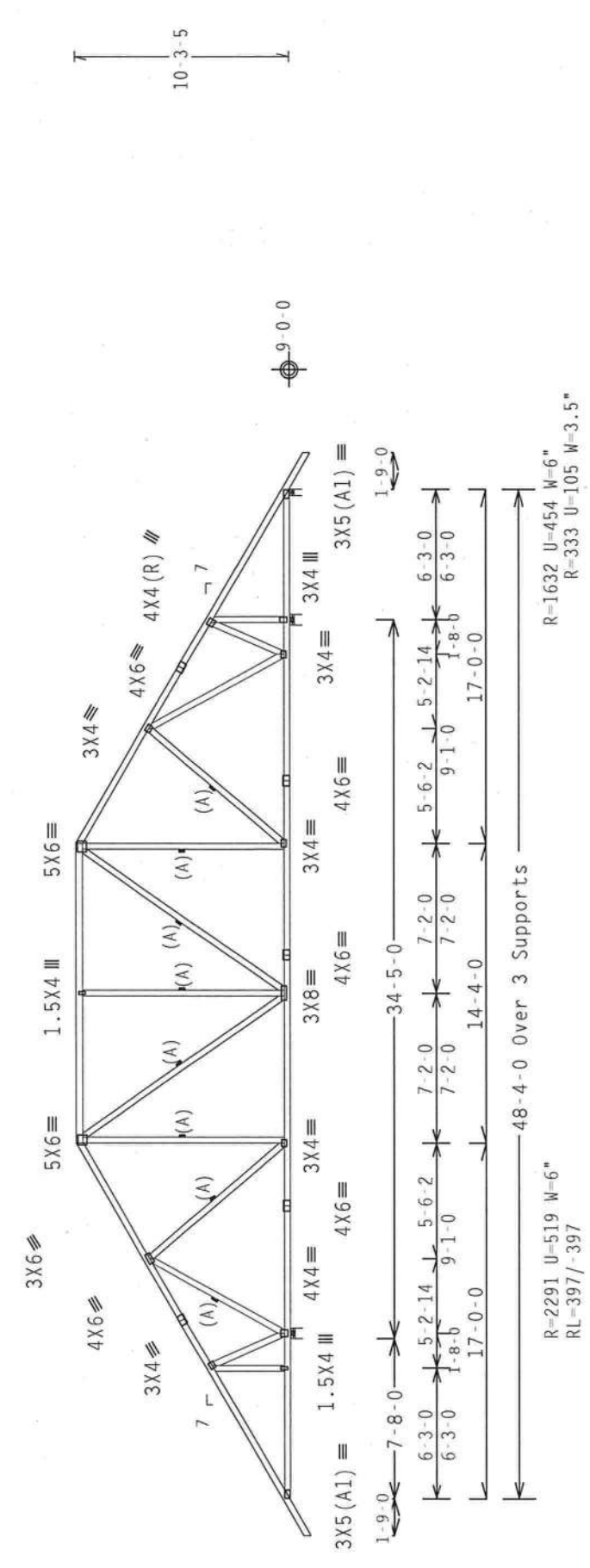
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

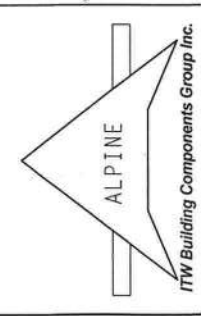
MWFRS loads based on trusses located at least 30.00 ft. from roof edge.



Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0)/0(0)



QTY:1	FL/-4/-1-R/-	Scale = .125"/Ft.
TC LL	20.0 PSF	REF R8228- 31420
TC DL	10.0 PSF	DATE 11/10/09
BC DL	10.0 PSF	DRW HCUSR8228 09314043
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 69771
DUR.FAC.	1.25	FROM AH



PLT TYP. Wave

Top chord 2x6 SP #2 :T1, T5 2x4 SP #2 Dense:
Bot chord 2x6 SP #2
Webs 2x4 SP #3

2 COMPLETE TRUSSES REQUIRED

Nail Schedule: 0.131"x3" nails
Top Chord: 1 Row @ 12.00" o.c.
Bot Chord: 1 Row @ 12.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows
in each row to avoid splitting

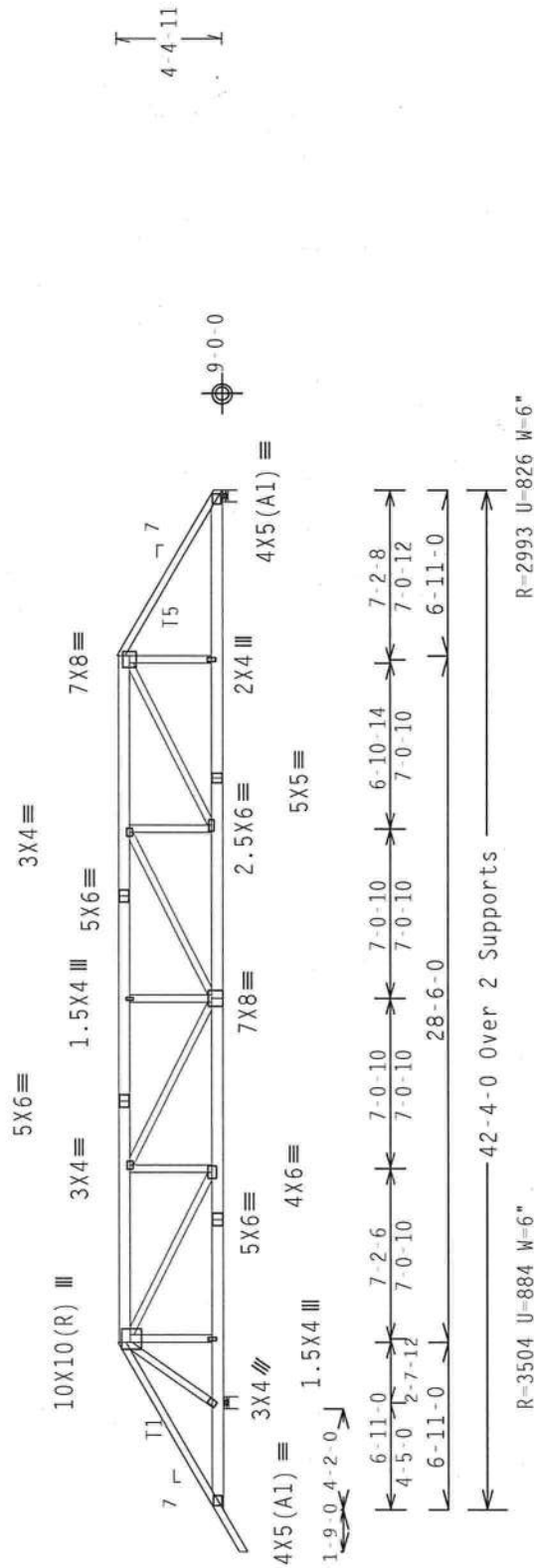
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. lw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Roof overhang supports 2.00 psf soffit load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

THE BUILDING DESIGNER SHALL EVALUATE AND APPROVE LOAD MAGNITUDES AND LOCATIONS. THE TRUSS ENGINEER IS NOT RESPONSIBLE FOR LOAD MAGNITUDES AND LOCATIONS.



PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10%(0%)/0(0)

9.02.00

QTY:1 FL/-/4/-/-/R/-

Scale = .125"/Ft.

WARNING THESE STEEL EXTRUDED C CHANNELS, HANDLING, SHIPPING, UNLOADING AND BRACING REQUIRE SPECIAL PRECAUTIONS. FOR MORE INFORMATION, PUBLISHED BY THE CURTIS PATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND A 8000 TRUSS COMPANY OF AMERICA, 6300 WILLOW LANE, SUITE 505, M1, 53719, FOR SAFETY PRECAUTIONS PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

[illegible]

Top chord	2x4	SP	#2	Dense
Bot chord	2x4	SP	#2	Dense
Webbs	2x4	SP	#3	

110 mph wind, 15.00 ft mean hgt., ASCE 7-05, PART. ENC. bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. lw=1.00 GCpi(+/-)=0.55

Roof overhang supports 2.00 psf soffit load.

Wind reactions based on MWFRS pressures.

(A) Continuous lateral bracing equally spaced on member.

Right end vertical not exposed to wind pressure.

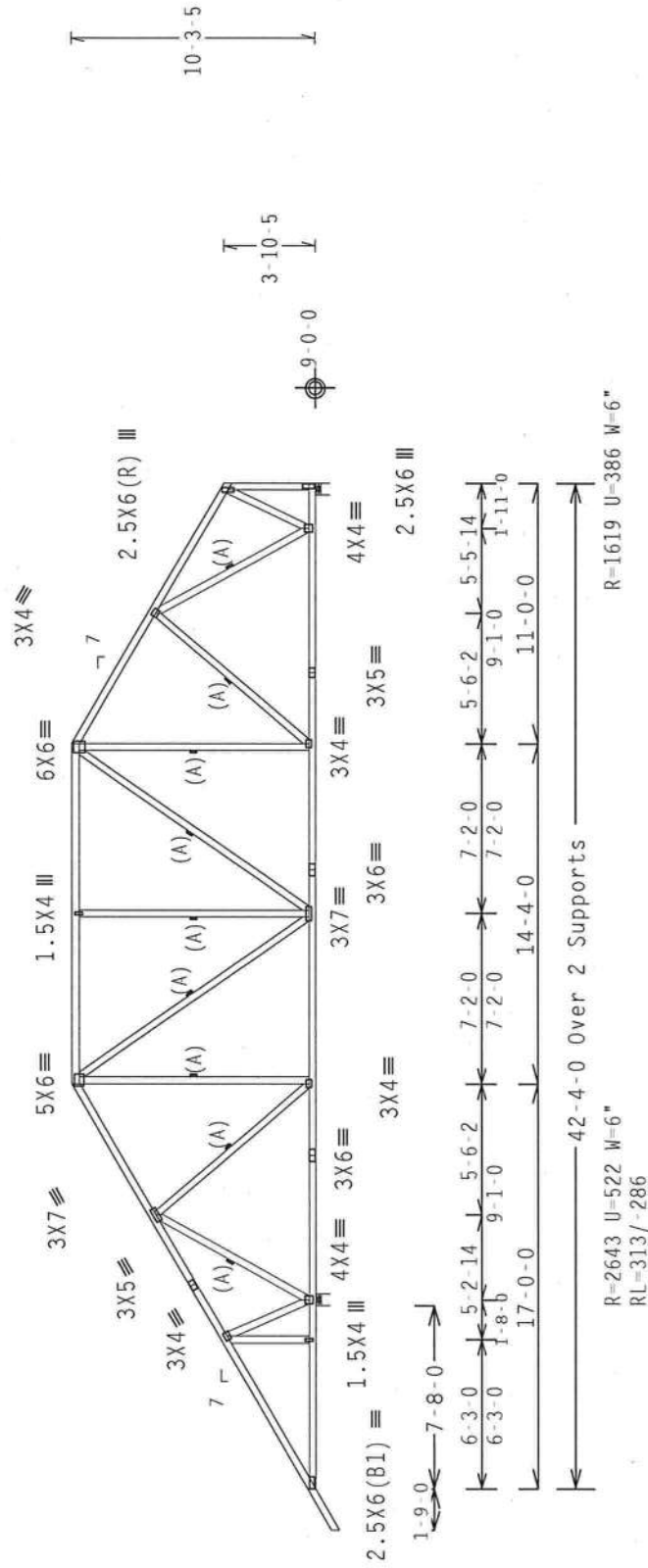
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

MWFRS loads based on trusses located at least 30.00 ft. from roof edge.



Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave

QTY:2 FL/-/4/-/-/R/-

Scale = .125"/Ft.

WARNING THOSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. PROPER USE OF THE PRODUCT IS CRITICAL TO THE SAFETY OF THE USER. FOR MORE INFORMATION, CONTACT US AT 800-679-6300 OR VISIT OUR WEBSITE AT WWW.ALEXANDRIA.COM.
NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA 22304 AND VICA TRUSS, COUNCIL OF AMERICA, UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CELLS.

RFF R8228- 31422

REF. NO.	11/10/09
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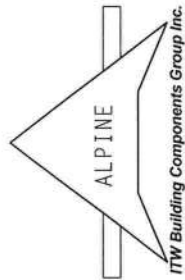
1150

DRW HCUSR8228 09314036

HC-ENG DF/DF

SEQN-	69766
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FROM AH



ITW Building Components Group Inc.

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

(A) #3 or better scab brace. Same size & 80% length of web member.
Attach with 10d Box or Gun (0.128"x3",min.)nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

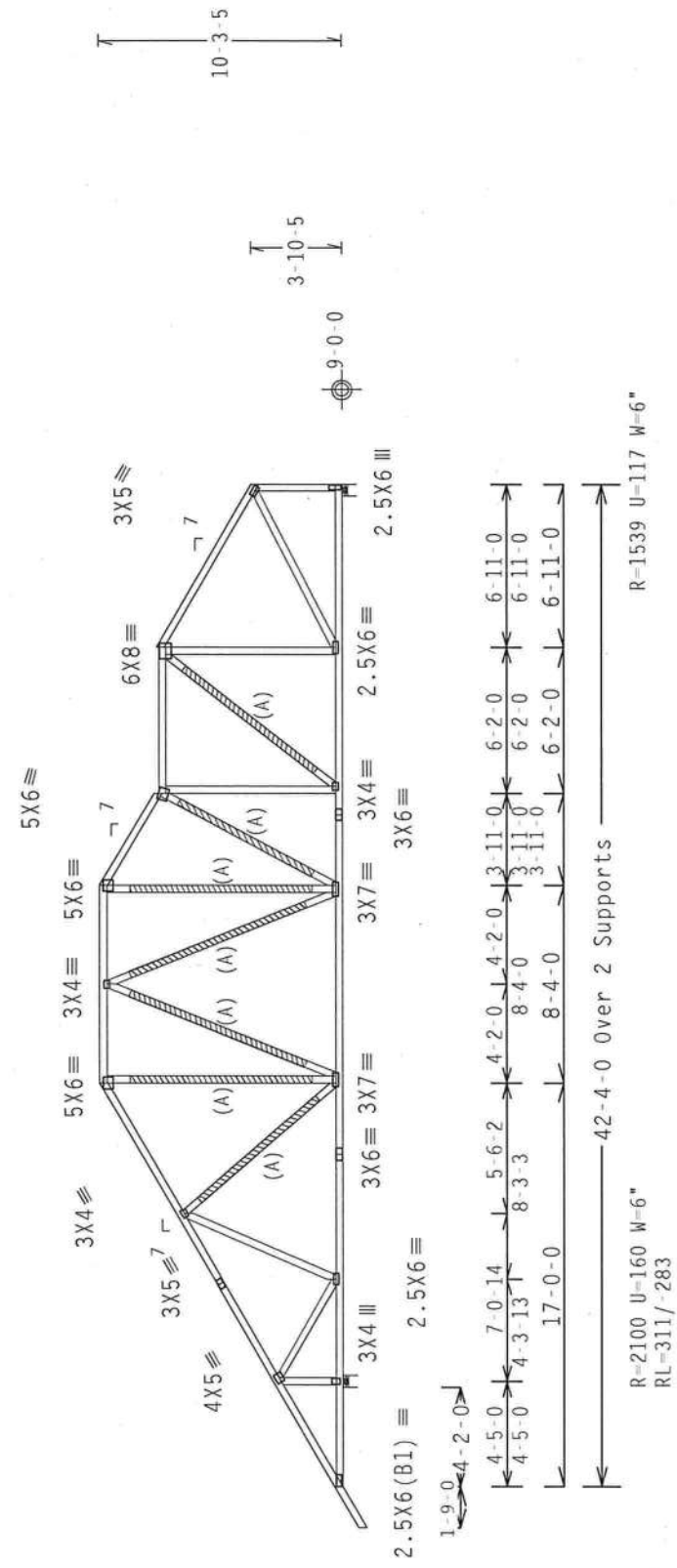
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)-0.18

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

In lieu of structural panels use purlins to brace all flat IC @ 24" OC.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

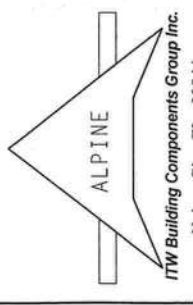


Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave	QTY: 1	FL / - / 4 / - / R / -	Scale = .125" / Ft.
	TC LL	20.0 PSF	REF R8228- 31424
	TC DL	10.0 PSF	DATE 11/10/09
	BC DL	10.0 PSF	DRW HCUSR8228 09314030
	BC LL	0.0 PSF	HC-ENG DF/DF
	TOT.LD.	40.0 PSF	SEQN- 69942
	DUR.FAC.	1.25	FROM AH

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSE (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND MICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY ATTEMPT TO ALTER THE TRUSS IN CONFORMANCE WITH DESIGN CONDITIONS WITH APPLICABLE PROVISIONS OF RDS (NATIONAL DESIGN SPEC. BY AIA/PSA) AND TPI. ITW REG. CONNECTOR PLATES ARE MADE OF 20/18/16GA (H/H/SS/PS) ASIN A653 GRADE 40/60 (H, K/H/SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AIA/PSA 3.3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE DESIGN CHOOSE.



Bot chord 2x4 SP #2 Dense

Web	2x4	SP	#3
Web	2x4	SP	#3

Roof overhang supports 2.00 psf soffit load.

(A) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

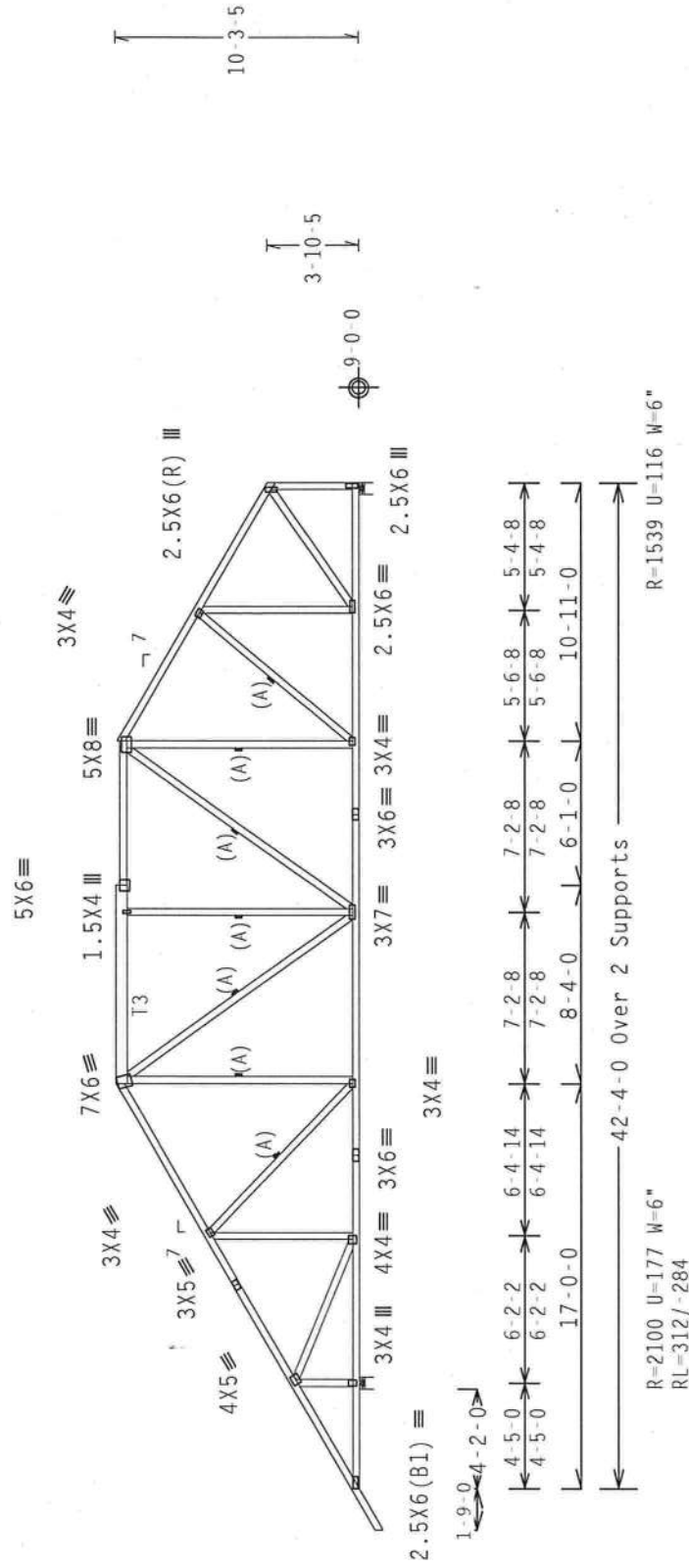
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. lw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.



Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave

QTY:1 FL/-/4/-/-/R/-/

Scale = .125"/Ft.

WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO RESIST BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TYP GIERSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA 22304, AND AISC 4000 TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MOHAWK, MI 48371-9100. FOR SAFETY PRACTICES PRIOR TO PREPARING THESE FUNCTIONS, UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

*****IMPORTANT****FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DETERIORATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THIS DESIGN SHALL BE AT THE USER'S RISK. PROVIDE THE FOLLOWING INFORMATION TO THE USER:

DESIGN CONFORMS WITH ALL APPLICABLE PROVISIONS OF NDS NATIONAL DESIGN SPEC. (FOR AF&P) AND TP1. 1TW BCG CONNECTOR PLATES ARE MADE OF 2018/T166GA OR 2018/T166GAL ALUMINUM GRADE 40/69 (A, K/H/S5) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. IF AN INSPECTION OF PLATES FOLLOWED BY (1). SHALL BE PER ANNEX 43 OF TP11-2002 SEC. 3.

DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT SEAL ON THIS



ITW Building Components Group Inc.

Top	chord	2x4	SP	#2	Dense
Bot	chord	2x4	SP	#2	Dense
	webs	2x4	SP	#3	

Roof overhang supports 2.00 psf soffit load.

(A) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

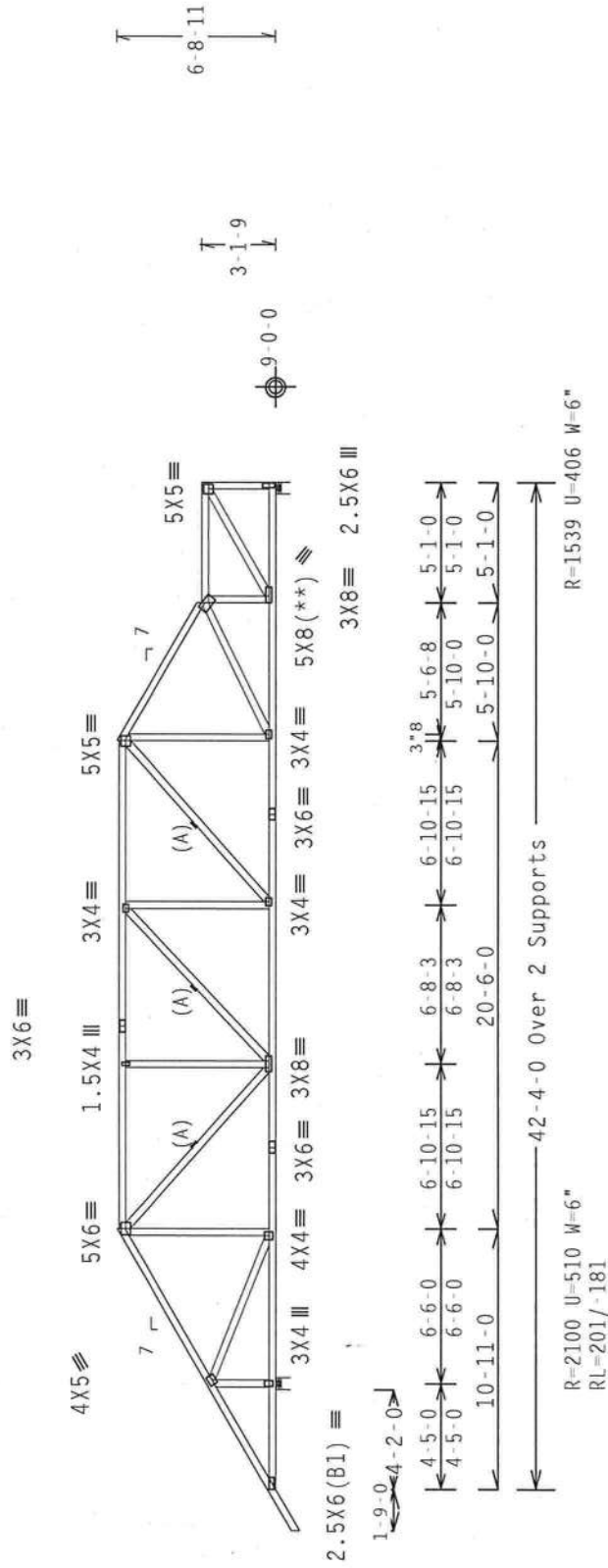
(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. lw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

Deflection meets L/240 live and L/180 total load.



PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10%(0%)/0(0)

.00

QTY:1 FL/-/4/-/-/R/-/-

Scale = .125"/Ft.

***WARNING:** TROUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION PUBLISHED BY PTI (TROUGH PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALXANDRIA, VA 22314) AND ATEA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, MI 48219) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

TC LL	20.0	PSF	REF	R8228 - 31427
TC DL	10.0	PSF	DATE	11/10/09
BC DL	10.0	PSF	DRW	HGUSR8228 09314007
BC LL	0.0	PSF	HC-ENG	DF/DF
TOT.LD.	40.0	PSF	SEQN-	69964
DUR.FAC.	1.25		FROM	AH



*****IMPORTANT***** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATIONS FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN COMPLIANCE WITH THIS DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., BY AISI) AND TPI. 1TM BCG CORRELATOR PLATES ARE MADE OF 2014/T3G6 (AL 6063 GRADE 40360, 0.5% Zn-55% GALV. STEEL - APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMBEX 43 OF 1911-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT



ITW Building Components Group Inc.

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

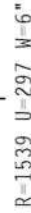
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCp(+/-)=0.18

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.



QTY:1 FL/-/4/-/-/R/-/- Scale =.125"/Ft.

TC LL	20.0 PSF	REF R8228- 31428
TC DL	10.0 PSF	DATE 11/10/09
BC DL	10.0 PSF	DRW HCUSR8228 09314008
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 69953
DUR.FAC.	1.25	FROM AH



ITW Building Components Group Inc.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi (+/-)=0.18

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

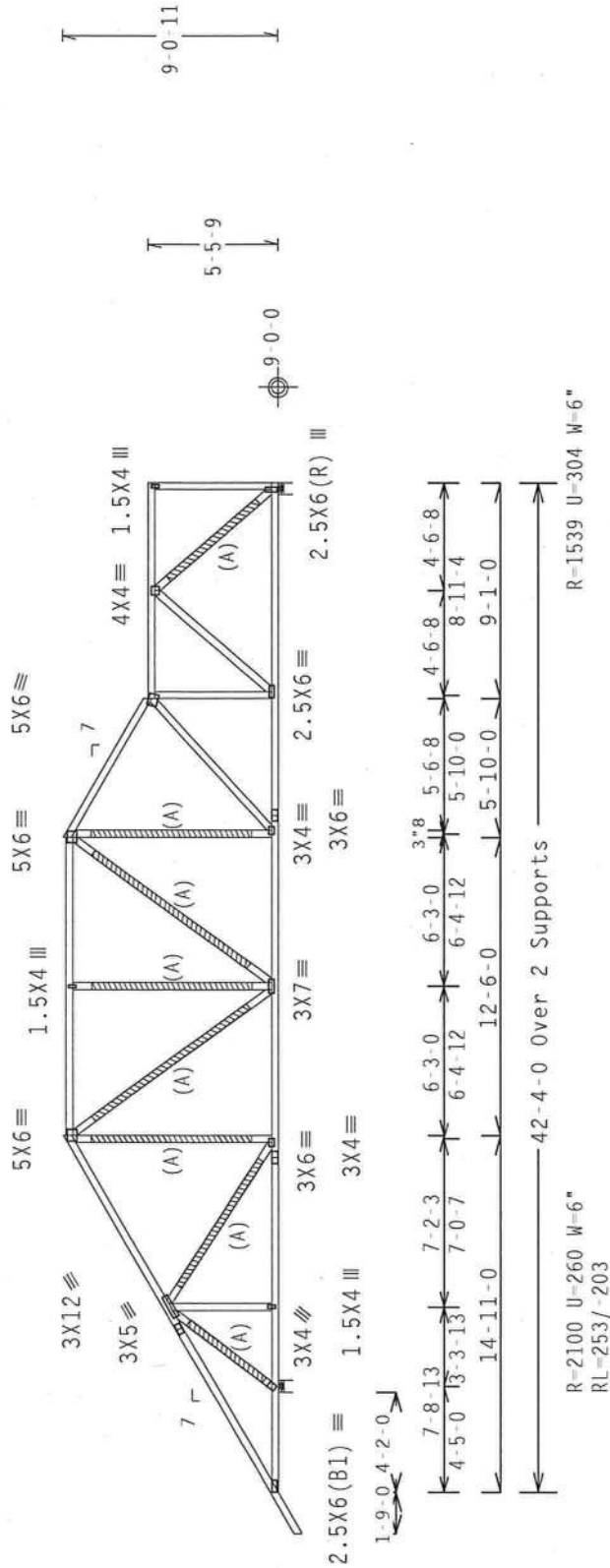
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

(A) #3 or better scab brace. Same size & 80% length of web member.
Attach with 10d Box or Gun (0.128"x3" min.) nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.



Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave

QTY:1 FL/-/4/-/-/R/-/-

Scale = .125"/Ft.

• **WARNING:** THUSSES REQUIRE EXTREME CARE IN FABRICATION, AND THE SHIPPING, INSTALLING AND BRACING OF THESE THUSSES MUST BE DONE IN ACCORDANCE WITH THE FOLLOWING INFORMATION. IF YOU DO NOT HAVE THE NORTH LANE STREET, SUITE 312, ALICIA APOSTOLIS, VA. 22134, AND WCA (WOOD TRUSS COUNCIL OF AMERICA), 6300 ENTERPRISE LANE, SUITE 501, 52719, FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

TC LL	20.0 PSF	REF	R8228- 31429
TC DL	10.0 PSF	DATE	11/10/09
BC DL	10.0 PSF	DRW	HCUSR8228 0931400
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	70023
DUR.FAC.	1.25	FROM	AH

DOUGLAS FLEMING
LICENSE
No. 66648
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

ALPINE

ITW Building Components Group Inc.

Top	chord	2x4	SP	#2	Dense
Bot	chord	2x4	SP	#2	Dense
	webs	2x4	SP	#3	

L10 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Roof overhang supports 2.00 psf soffit load.

Wind reactions based on MWFRS pressures.

(A) #3 or better scab brace. Same size & 80% length of web member. Attach with 10d Box or Gun (0.128"x3", min.) nails @ 6" OC.

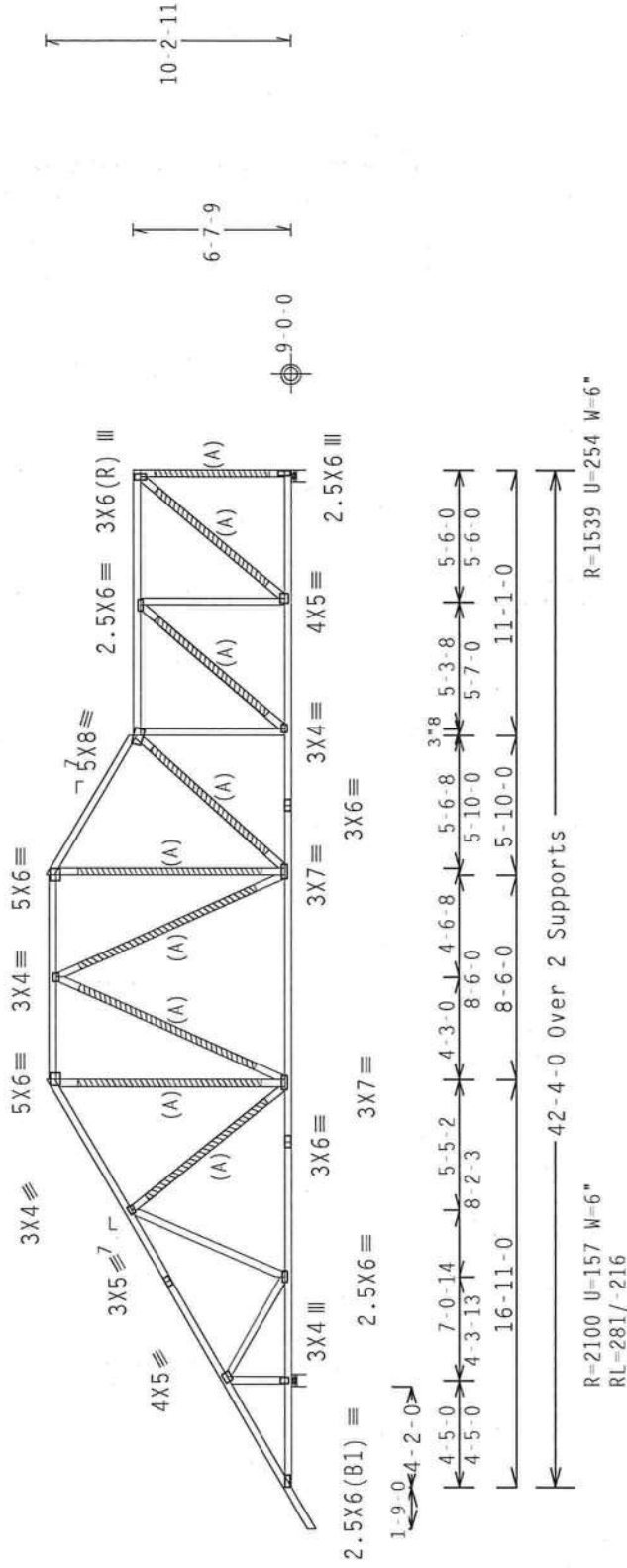
Right end vertical not exposed to wind pressure.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.



PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

QTY:1 FL/-/4/-/-/R/-

Scale = .125"/Ft.

[illegible]

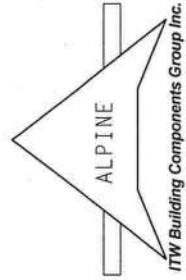
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TC DL	10.0	PSF	DATE	11/10/09	
BC DL	10.0	PSF	DRW	HCUSR8228	0931403
BC LL	0.0	PSF	HC-ENG	DF/DF	
TOT.LD.	40.0	PSF	SEQN-	69946	
DUR.FAC.	1.25		FROM	AH	



**** IMPORTANT **** THROUGH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR TRU INC., INC. SHALL NOT BE RESPONSIBLE FOR ANY VIOLATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN COMBINS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., BY AFAPA) AND TPI. CONNECTOR PLATES ARE MADE OF 20X18/16GA (MIL-H-5757E) ASTM A653 GRADE 40/60 (T/C H/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS T60A-2. ANY INSPECTION OF PLATES FOLLOWED BY (C) SHALL BE PER ABERR A3 OF TPI-2002-SEC.3.

THESE TRUSSES ARE DESIGNED TO BE USED AS AN INTERIOR COMPONENT FOR ALL BUILDING TYPES. THE RESPONSIBILITY OF THE DESIGNER SHOWN ON THE SUBMITTAL IS TO USE IT AS SUCH AND COMPARE FOR ALL BUILDING CODES. THE RESPONSIBILITY OF THE



ITW Building Components Group Inc.

(9-188--Doug Edgley Kisala -- , ** - C1)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

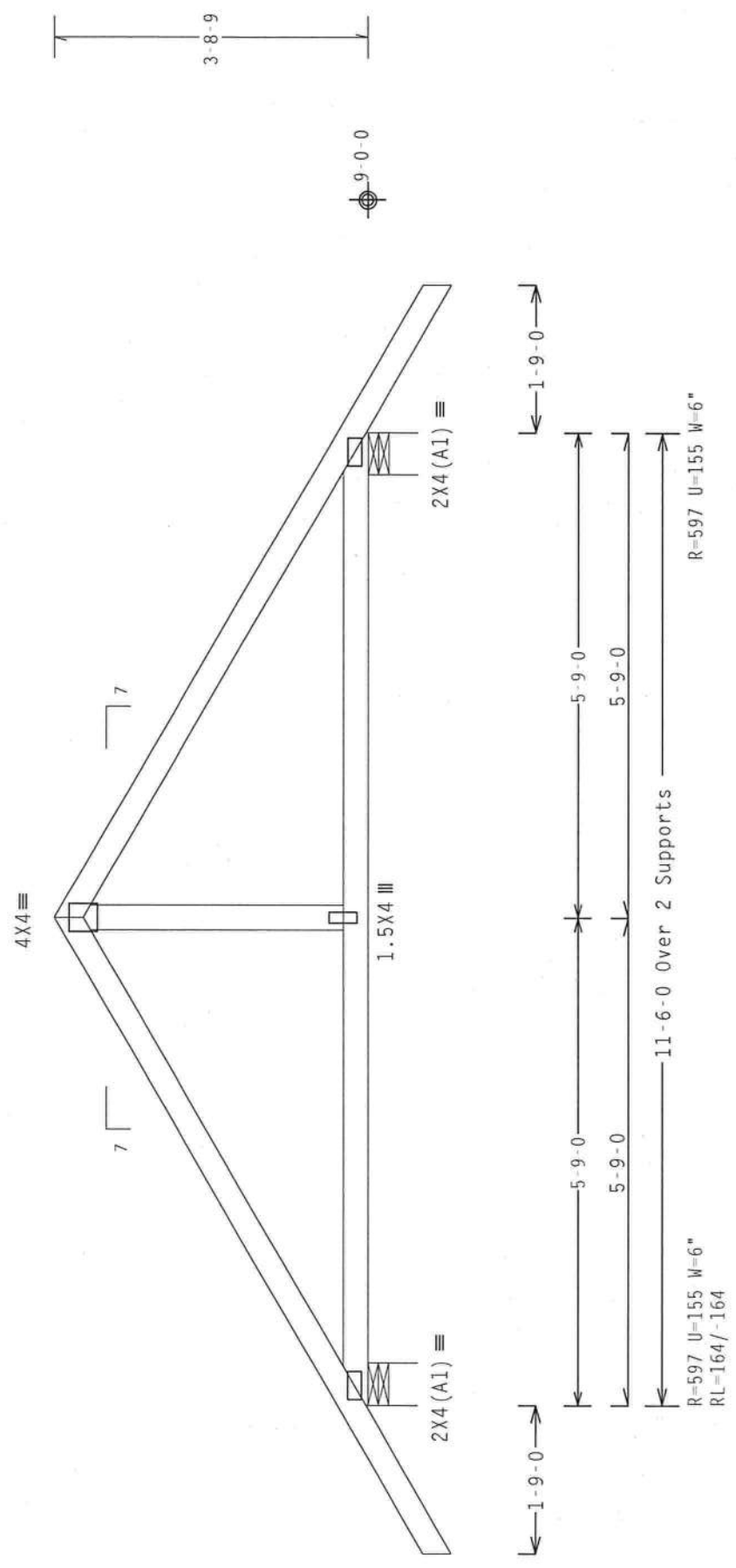
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located
anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0
psf. Iw=1.00 GCpi(+/-)-0.18

Roof overhang supports 2.00 psf soffit load.

Wind reactions based on MWFRS pressures.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

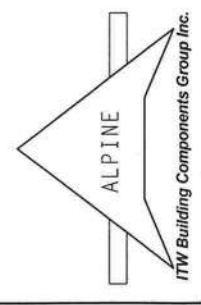


Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave	QTY:1	FL/-4/-/-R/-	Scale = .5"/Ft.
	TC LL	20.0 PSF	REF R8228- 31431
	TC DL	10.0 PSF	DATE 11/10/09
	BC DL	10.0 PSF	DRW HCUSR8228 09314010
	BC LL	0.0 PSF	HC-ENG DF/DF *
	TOT.LD.	40.0 PSF	SEQN- 69630
	DUR.FAC.	1.25	FROM AH

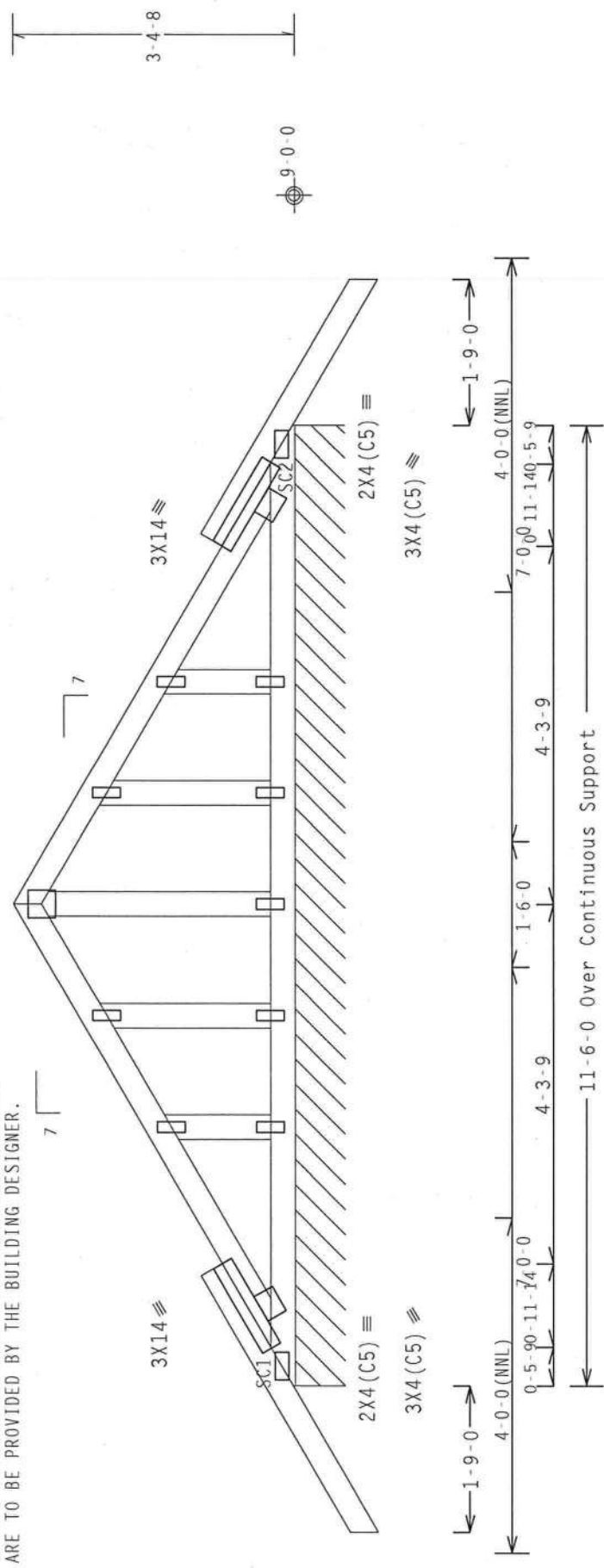
****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND METCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BEG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF BOB (NATIONAL DESIGN SPEC. BY AERPA) AND TPI. STEEL TRUSS BEG PLATES TO EACH FACE OF TRUSS AND UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ARBET A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGNER.



Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
:Stack Chord SC1 2x4 SP #2 Dense::Stack Chord SC2 2x4 SP #2 Dense:
Roof overhang supports 2.00 psf soffit load.
See DWGS A11015050109 & GBLLETIN0109 for more requirements.
Stacked top chord must NOT be notched or cut in area (NNL). Attach stacked top chord (SC) to dropped top chord in notched area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notched area using 3x6.
THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF, FLOOR AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS. DIAPHRAGMS AND SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS ARE TO BE PROVIDED BY THE BUILDING DESIGNER.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi (+/-)=0.18
Wind reactions based on MWFRS pressures.
Truss spaced at 24.0" OC designed to support 2-0-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.
Bottom chord checked for 10.00 psf non-concurrent live load.
Deflection meets L/240 live and L/180 total load.



R-195 PLF U=27 PLF W=11-6-0
RL=14/-14 PLF

Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

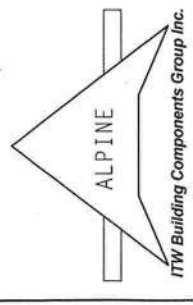
Scale = .5"/Ft.

QTY:1	FL/-4/-/R/-	TC LL	20.0 PSF	REF	R8228- 31432
		TC DL	10.0 PSF	DATE	11/10/09
		BC DL	10.0 PSF	DRW	HCUSR8228 09314001
		BC LL	0.0 PSF	HC-ENG	DF/DF
		TOT.LD.	40.0 PSF	SEQN-	69637
		DUR.FAC.	1.25	FROM	AH



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN COMPLIANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF BCS (NATIONAL DESIGN SPEC. BY AIA/ASA) AND TPI. ITW BCG CONTRACTOR PLATES ARE MADE OF 2018/1066 (A/1/SS/24) WITH A553 GRADE 40/60 (A/4/1/SS) GALV. STEEL. APPLY STATES EACH PLATE SHALL BE PROPERLY DESIGNED FOR THE DESIGNATION PER DRAWING. ITW BCG SHALL BE RESPONSIBLE FOR THE DESIGN OF THE TRUSS COMPONENTS. ANY DEVIATION FROM THIS DESIGN SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. ITW BCG SHALL NOT BE RESPONSIBLE FOR THE DESIGN OF THE TRUSS COMPONENTS. ANY DEVIATION FROM THIS DESIGN SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.



(9-188--Doug Edgley Kisala -- ** - D1)

Top chord 2x4 SP #2 Dense
Bot chord 2x6 SP #1 Dense :B2 2x6 SP #2:
Webs 2x4 SP #3

Special loads

TC - From	Dur.Fac.-1.25 / Plate Dur.Fac.-1.25
TC - From	79 pif at -1.75 to 79 pif at 11.50
TC - From	79 pif at 11.50 to 79 pif at 24.75
BC - From	6 pif at -1.75 to 6 pif at 0.00
BC - From	25 pif at 0.00 to 25 pif at 7.33
BC - From	125 pif at 7.33 to 125 pif at 9.83
BC - From	175 pif at 9.83 to 175 pif at 13.25
BC - From	125 pif at 13.25 to 125 pif at 14.00
BC - From	125 pif at 14.00 to 125 pif at 15.67
BC - From	25 pif at 15.67 to 25 pif at 23.00
BC - From	6 pif at 23.00 to 6 pif at 24.75

Wind reactions based on MWFRS pressures.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP-C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi (+/-)-0.18

Roof overhang supports 2.00 psf soffit load.

In lieu of structural panels use purlins to brace TC @ 24" OC.

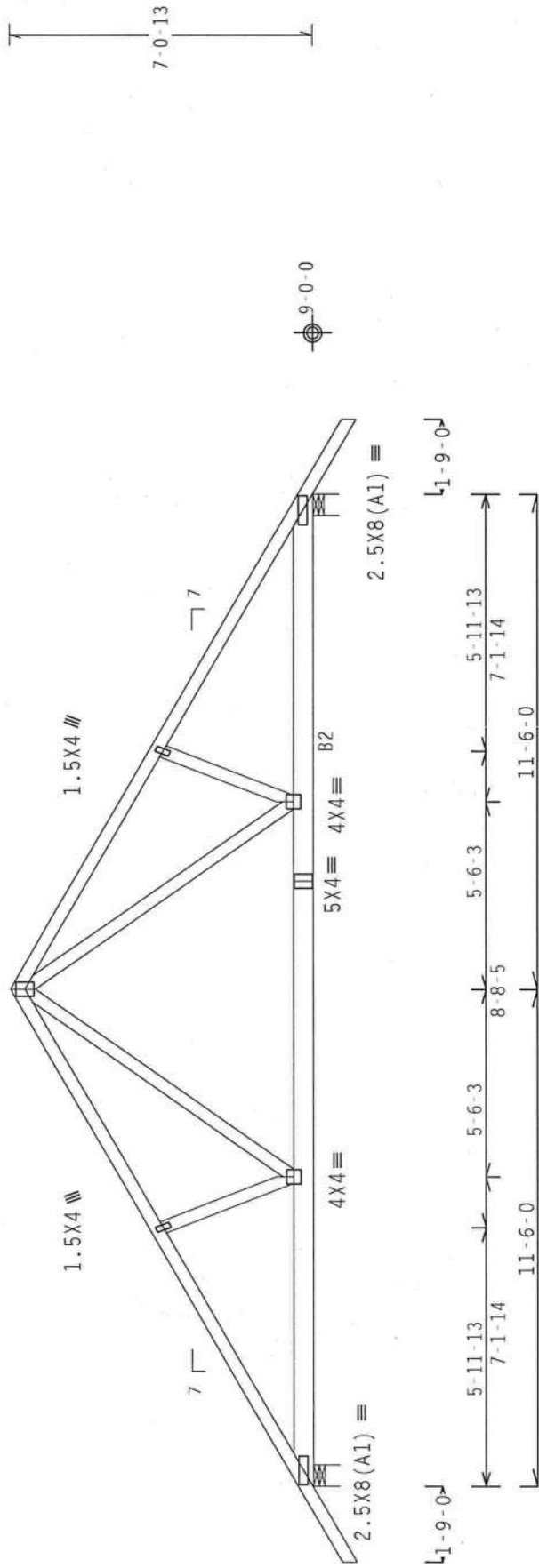
Bottom chord checked for 10.00 psf non-concurrent live load.

Trusses to be spaced at 30.0" OC maximum.

Deflection meets L/240 live and L/180 total load.

THE BUILDING DESIGNER SHALL EVALUATE AND APPROVE LOAD MAGNITUDES AND LOCATIONS. THE TRUSS ENGINEER IS NOT RESPONSIBLE FOR LOAD MAGNITUDES AND LOCATIONS.

4X5(R) III



R-1846 U-513 W-6"
RL=334/-334
23'-0-0 Over 2 Supports
R-1846 U-513 W-6"
Scale = .25"/Ft.

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

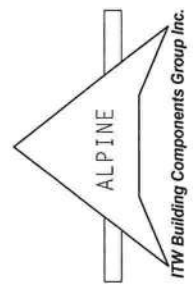
PLT TYP. Wave



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND MICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, MI 48050) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCSI, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. FOR WOOD) AND TPI. THE BCSI CONNECTION PLATES ARE MADE OF 2018/1866 (A1/SS) ASH AND 40760 (A1/SS) GALV. STEEL. APPLY THE FOLLOWING DIMENSIONS TO THE TRUSS. UNLESS OTHERWISE INDICATED, ALL DIMENSIONS ARE IN INCHES. ALL DIMENSIONS SHALL BE FOLLOWED BY THE TRUSS COMPONENT FABRICATOR. ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT FABRICATOR INDICATES.

QTY:17	FL/-4/-1/R/-	Scale = .25"/Ft.
TC LL	20.0 PSF	REF R8228- 31433
TC DL	10.0 PSF	DATE 11/10/09
BC DL	10.0 PSF	DRW HCUSR8228 09314029
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 69642
DUR.FAC.	1.25	FROM AH



Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3 :W3 2x4 SP #2 Dense:
:Stack Chord SC1 2x4 SP #2 Dense::Stack Chord SC2 2x4 SP #2 Dense:

Roof overhang supports 2.00 psf soffit load.

Stacked top chord must NOT be notched or cut in area (NNL). Attach stacked top chord (SC) to dropped top chord in notched area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notched area using 3x6.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF, FLOOR AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS. DIAPHRAGMS AND SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS ARE TO BE PROVIDED BY THE BUILDING DESIGNER.

See DWGS A11015050109 & GBLTIN0109 for more requirements. 6X8(R) III

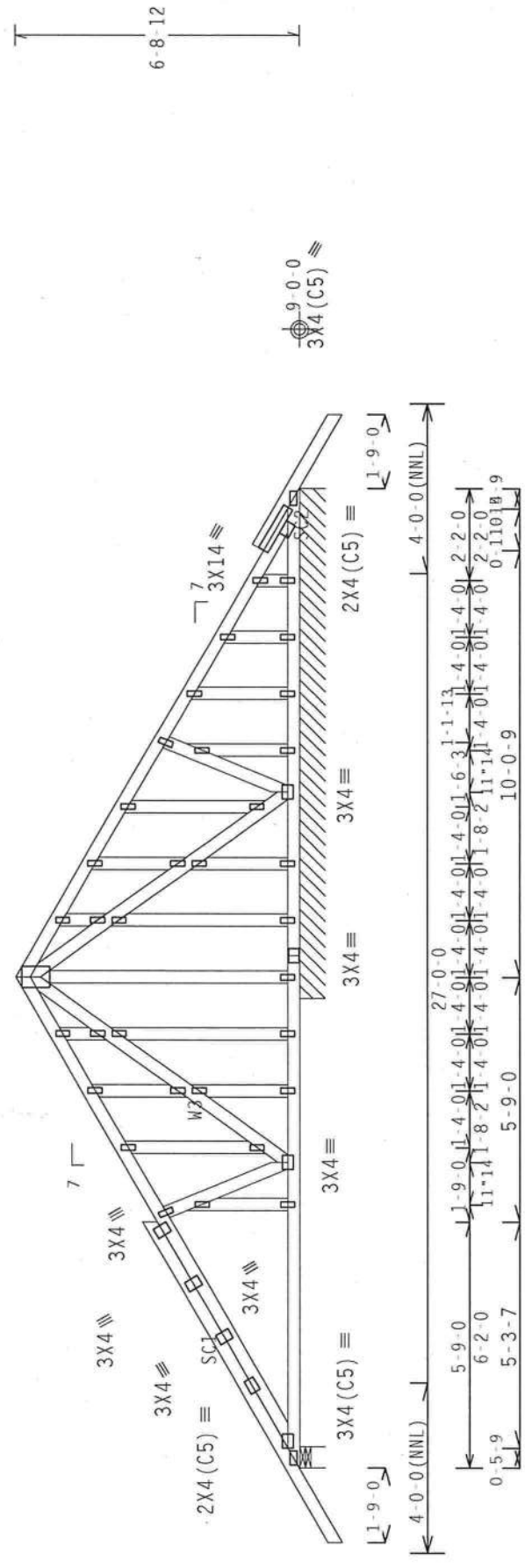
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. IW=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Truss spaced at 24.0" OC designed to support 2-0-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.



R-975 U=100 W=6"
RL=214/-249
R-286 PLF U-26 PLF W-12-0-0

Note: All Plates Are 1.5X4 Except As Shown.

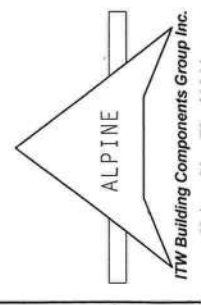
Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10%(0%)/0(0)

QTY: 1 FL/-4/-/R/- Scale = .25"/Ft.



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND METCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, RADFORD, VA 22405) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. THE TRUSS SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE TPI REG. CONNECTIONS ARE MADE OF 20/10/10GA (10 W/5/8") ASTM A653 GRADE 40/60 (40 KSI) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 100A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMBEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGNER. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE DESIGNER.



Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

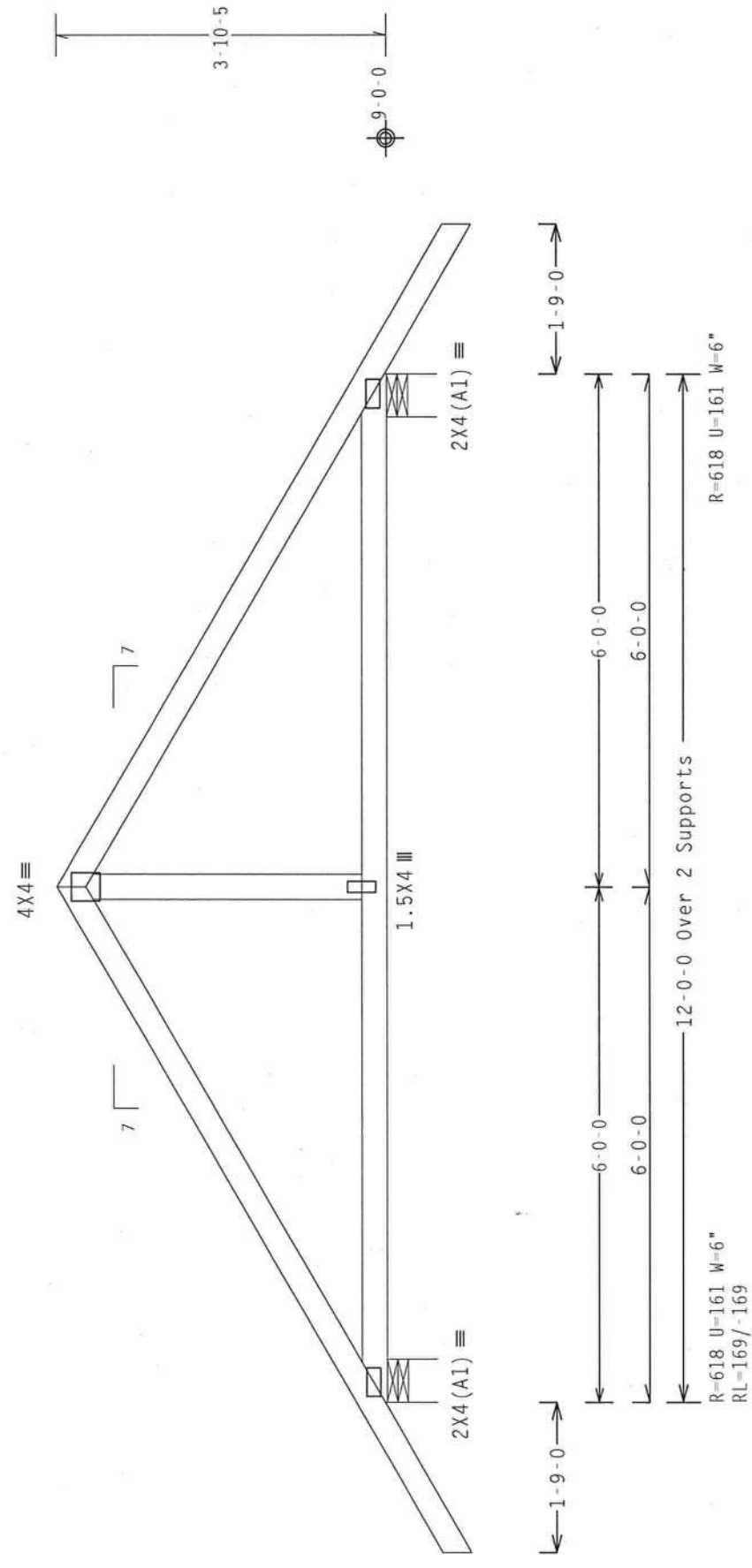
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/-)-0.18

Wind reactions based on MWFRS pressures.

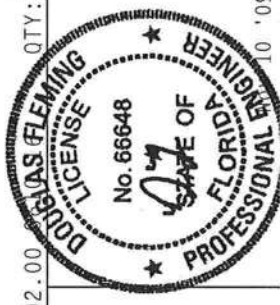
Deflection meets L/240 live and L/180 total load.

Roof overhang supports 2.00 psf soffit load.

Bottom chord checked for 10.00 psf non concurrent live load.



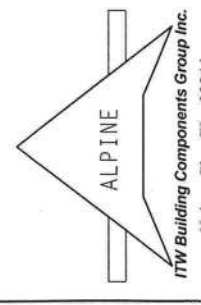
Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%/0(0)



QTY: 1	FL / - / 4 / - / R / -	Scale = .5" / Ft.
TC LL	20.0 PSF	REF R8228- 31436
TC DL	10.0 PSF	DATE 11/10/09
BC DL	10.0 PSF	DRW HCUSR8228 09314050
BC LL	0.0 PSF	HC-ENG DF/DF *
TOT.LD.	40.0 PSF	SEQN - 69616
DUR.FAC.	1.25	FROM AH

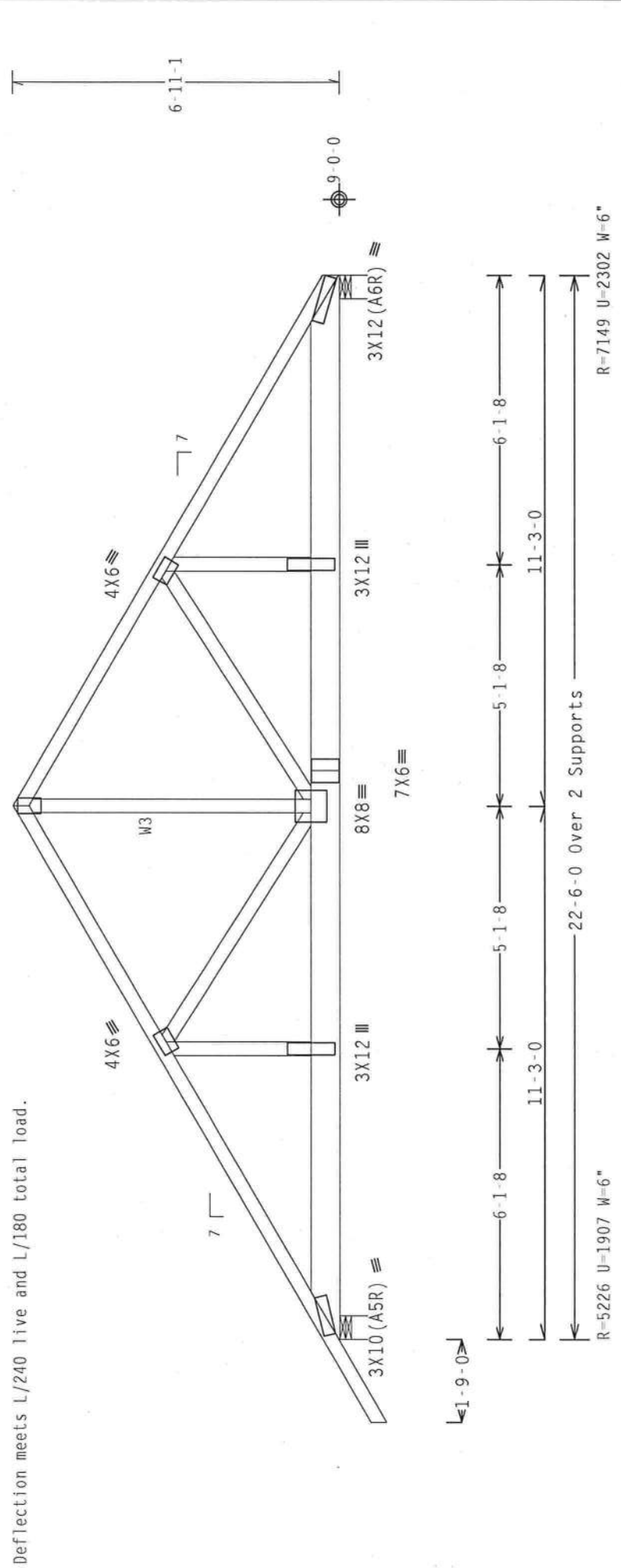
****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND MICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, MI 48071) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR. THE TRUSS SHALL BE FABRICATED, HANDLED, SHIPPED, INSTALLED AND BRACED IN ACCORDANCE WITH THE DESIGN SPEC. THE TRUSS SHALL BE MADE OF 20/18/1604 (A1/SS/ST) ASTM A653 GRADE 40/60 (A1, K1/SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 1604-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMBEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN. SIGN THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE DESIGNER.



Top chord 2x4 SP #2 Dense
Bot chord 2x8 SP #1 Dense
Webs 2x4 SP #3 :W3 2x4 SP #2 Dense:

2 COMPLETE TRUSSES REQUIRED
Nail Schedule: 0.131"x3" nails
Top Chord: 1 Row @ 12.00" o.c.
Bot Chord: 1 Row @ 3.25" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. 1W=1.00 GCpf(+/-)=0.18
Wind reactions based on MWFRS pressures.
4X6(R) III Roof overhang supports 2.00 psf soffit load.



Scale = .3125" / Ft.
REF R8228- 31437
DATE 11/10/09
DRW HCUSR8228 09314002
HC-ENG DF/DF
SEQN- 69726
FROM AH

PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

9.02.00 QTY:1 FL/-/4/-/R/-

TC LL 20.0 PSF
TC DL 10.0 PSF
BC DL 10.0 PSF
BC LL 0.0 PSF
TOT.LD. 40.0 PSF
DUR.FAC. 1.25

10 '09

DOUGLAS FLEMING
LICENSE
No. 66648
STATE OF FLORIDA
PROFESSIONAL ENGINEER

ALPINE
ITW Building Components Group Inc.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WIGA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI- OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

(9-188--Doug Edgley Kisala --, ** - F1)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

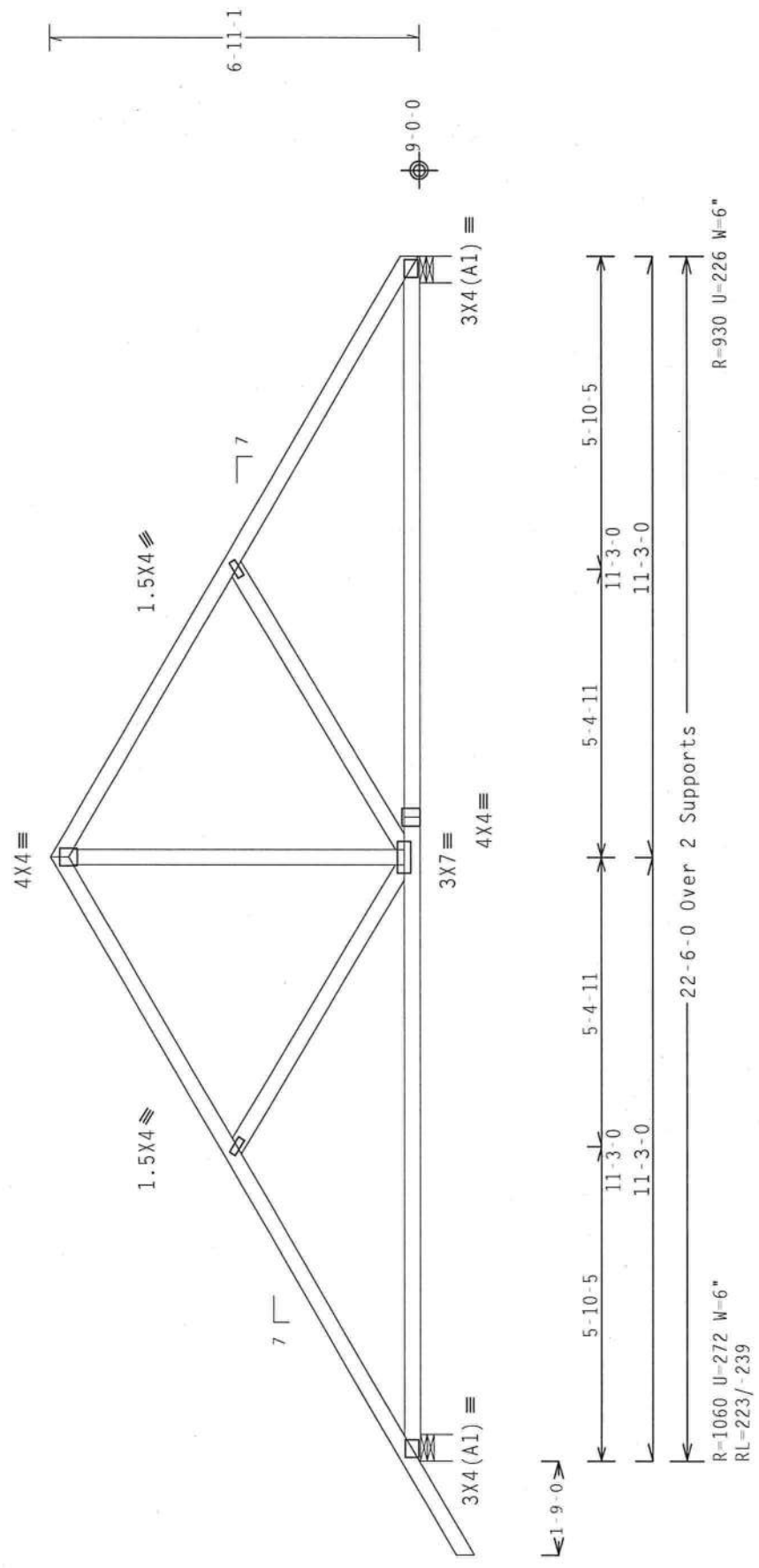
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located
within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf,
wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Roof overhang supports 2.00 psf soffit load.

Wind reactions based on MWFRS pressures.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

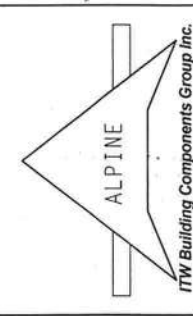


Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0)/0(0)



QTY: 2	FL / - / 4 / - / - / R / -	Scale = .3125" / Ft.
TC LL	20.0 PSF	REF R8228 - 31438
TC DL	10.0 PSF	DATE 11/10/09
BC DL	10.0 PSF	DRW HCUSR8228 09314052
BC LL	0.0 PSF	HC-ENG DF/DF *
TOT.LD.	40.0 PSF	SEQN- 69620
DUR.FAC.	1.25	FROM AH

PLT TYP. Wave



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO RC51 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND MICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND IPI. 1TH BCG CONNECTION PLATES ARE MADE OF 2017B1616GA (4.0/55/8) ASTM A653 GRADE 40/60 (M. 8/4/55) GALV. STEEL. APPLY PLATES TO EACH FACE OF ALL TRUSS MEMBERS. THESE TRUSSES ARE DESIGNED FOR USE IN RESIDENTIAL APPLICATIONS. A PROFESSIONAL ENGINEER'S SEAL AND SIGNATURE ARE REQUIRED FOR ALL TRUSSES. ITW BCG, INC. IS THE SOLE RESPONSIBILITY FOR THE TRUSS COMPONENT DRAWING INDICATES. ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. 1w=1.00 GCpi(+/-)-0.18

Wind reactions based on MMFRS pressures.

Truss spaced at 24.0" OC designed to support 2-0-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

Bottom chord checked for 10.00 psf non-concurrent live load.

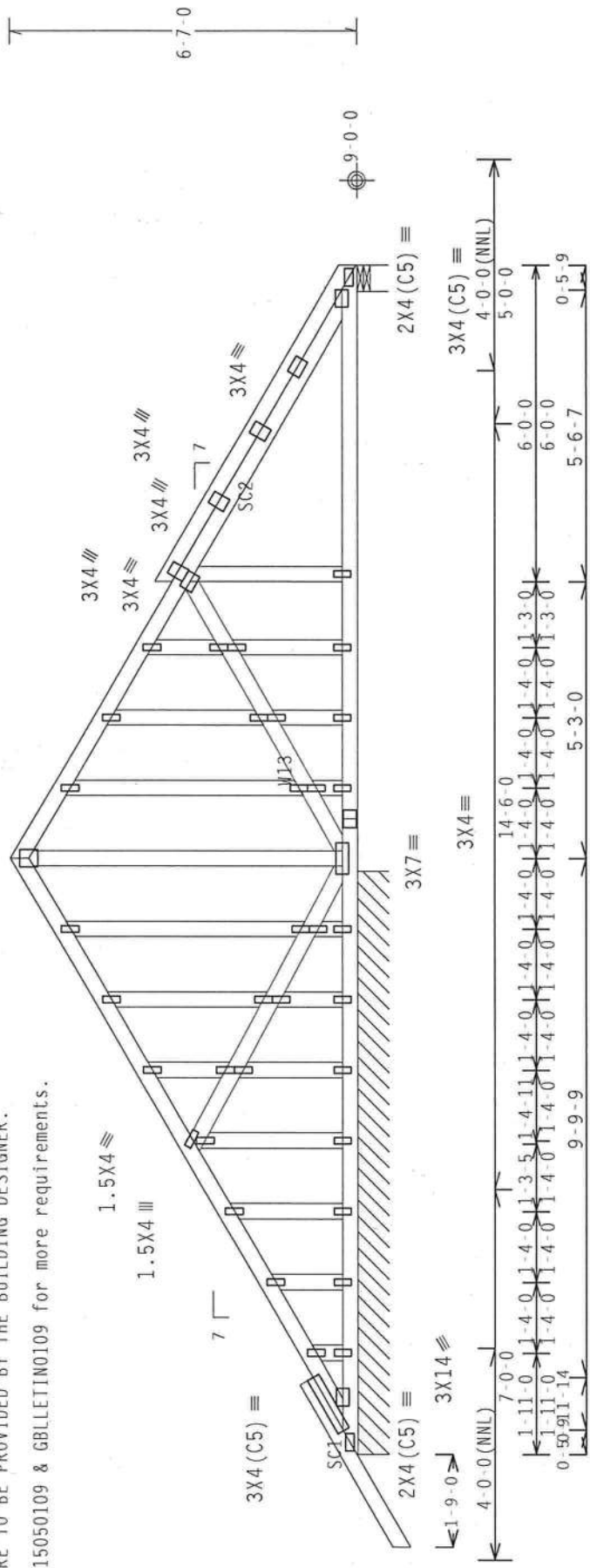
Deflection meets L/240 live and L/180 total load.

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3 W13 2x4 SP #2 Dense:
:Stack Chord SC1 2x4 SP #2 Dense::Stack Chord SC2 2x4 SP #2 Dense:
Roof overhang supports 2.00 psf soffit load.

Stacked top chord must NOT be notched or cut in area (NNL). Attach stacked top chord (SC) to dropped top chord in notched area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notched area using 3x6.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF, FLOOR AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS. DIAPHRAGMS AND SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS ARE TO BE PROVIDED BY THE BUILDING DESIGNER.

See DWGS A11015050109 & GBLLETIN0109 for more requirements.



PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

Note: All Plates Are 1.5x4 Except As Shown.

Scale = .3125"/Ft.

REF R8228- 31439
DATE 11/10/09
DRW HCUSR8228 09314051
HC-ENG DF/DF
SEQN- 69625
FROM AH

PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

Note: All Plates Are 1.5x4 Except As Shown.

Scale = .3125"/Ft.

REF R8228- 31439
DATE 11/10/09
DRW HCUSR8228 09314051
HC-ENG DF/DF
SEQN- 69625
FROM AH

ALPINE

ITW Building Components Group Inc.

DOUGLAS FLEMING
LICENSE
No. 66648
STATE OF FLORIDA
PROFESSIONAL ENGINEER

10 '09

9.02.00

QTY:1

FL/-4/-/-R/-

TC LL 20.0 PSF
TC DL 10.0 PSF
BC DL 10.0 PSF
BC LL 0.0 PSF
TOT.LD. 40.0 PSF
DUR.FAC. 1.25

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. 1w=1.00 GCpi(+/-)-0.18

Wind reactions based on MMFRS pressures.

Truss spaced at 24.0" OC designed to support 2-0-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF, FLOOR AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS. DIAPHRAGMS AND SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS ARE TO BE PROVIDED BY THE BUILDING DESIGNER.

See DWGS A11015050109 & GBLLETIN0109 for more requirements.

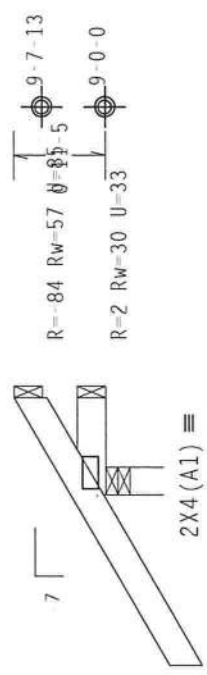
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3 W13 2x4 SP #2 Dense:
:Stack Chord SC1 2x4 SP #2 Dense::Stack Chord SC2 2x4 SP #2 Dense:
Roof overhang supports 2.00 psf soffit load.

Stacked top chord must NOT be notched or cut in area (NNL). Attach stacked top chord (SC) to dropped top chord in notched area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notched area using 3x6.

THIS DWG PREPARED FROM COMPUTER INPUT (LUNAS & DIMENSIONS) SUBMITTED BY KUSSA MEN.

(9-188--Doug Edgley Kisala --- ** - J1)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Roof overhang supports 2.00 psf soffit load.
Bottom chord checked for 10.00 psf non-concurrent live load.
110 mph wind, 15.00 ft mean hgt. ASCE 7-05, PART ENC. bldg. located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCpi (+/-)=0.55
Wind reactions based on MWFRS pressures.
Deflection meets L/240 live and L/180 total load.



← 1-9-0 →
1-0-0 Over 3 Supports
R-309 U=120 W=3.5"
RL=54/-44

Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10%(0%)/0(0)

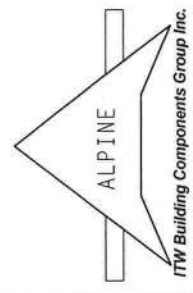
PLT TYP. Wave	QTY: 8	FL / - / 4 / - / R / -	Scale = .5" / Ft.
	TC LL	20.0 PSF	REF R8228- 31440
	TC DL	10.0 PSF	DATE 11/10/09
	BC DL	10.0 PSF	DRW HCUSR8228 09314003
	BC LL	0.0 PSF	HC-ENG DF/DF
	TOT.LD.	40.0 PSF	SEQN- 69452
	DUR.FAC.	1.25	FROM AH

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO RCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND AIGA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AFAPA) AND TPI. 1TH BCG CONNECTOR PLATES ARE MADE OF 20/18/10GA (N.W./SS/X) ASTM A653 GRADE 40/60 (M. K/H-SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS THRU 2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMER AS PER TPI-2002 SECS. 2.3.

DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT



Special loads

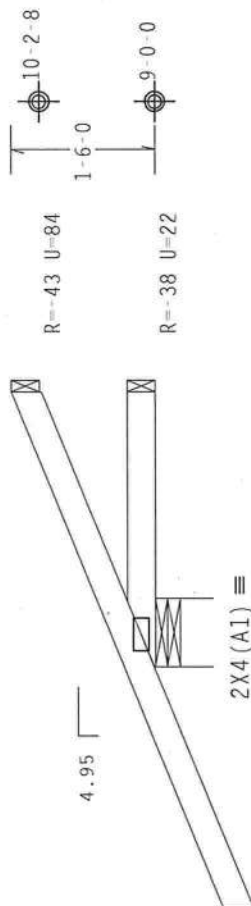
	(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)	
TC - From	62 pif at -2.83 to	62 pif at 2.83
BC - From	4 pif at -2.83 to	4 pif at 0.00
BC - From	20 pif at 0.00 to	20 pif at 2.83
TC -	-168 lb Conc. Load at	1.48
TC -	-47 lb Conc. Load at	1.48

Roof overhang supports 2.00 psf soffit load.

1110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load.



$\longleftrightarrow 2-5-11 \longleftrightarrow$
 $\longleftrightarrow 2-9-15 \text{ Over } 3 \text{ Supports } \longleftrightarrow$
 $R=261 \quad U=273 \quad W=8.485"$

Design Crit: FBC2007Res/TPI -2002(STD)
 $FT/RT=10\%(0\%)/0(0)$

PLT TYP. Wave

QTY:2 FL/-/4/-/-/R/-

Scale = .5"/Ft.

WARNING—TRUSSES PROVIDE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLATION AND BRACING. PLEASE TO: (CCS) CONSULTING CORP., SAFETY INFORMATION, PUBLISHED BY IPI (STEEL PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND AISC (AMERICAN INSTITUTE OF STEEL CONSTRUCTION, INC., 530 N. LAKE STREET, SUITE 100, CHICAGO, IL 60610). TRUSSES SHOULD BE PROPERLY DESIGNED AND MANUFACTURED TO PERFORM THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

**** IMPORTANT **** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. BY ACCEPTING THIS DESIGN, THE USER AGREES TO HOLD ITW BCG, INC. HARMLESS FROM ANY LITIGATION OR DAMAGES OF ANY KIND ARISING OUT OF THE USE OF THIS DESIGN. THE DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC.) AND TP1.
CONNECTION PLATES ARE MADE OF 2708/AL6GA (94%MIN/56% MAX) ALUMINUM GRADE 4046 (N. K/JH/S5). GALV. STEEL - APPLY PLATES TO EACH FACE OF TRUSS AND, WHERE APPROPRIATE, LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. IF ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A OF TP1-2002 SEC.2.

DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SOLELY FOR E-TRUSS COMPONENT



TC LL	20.0	PSF
TC DL	10.0	PSF
BC DL	10.0	PSF
BC LL	0.0	PSF
TOT. LD.	40.0	PSF
DUR. FAC.	1.25	

ATW Building Components Group Inc.

(9-188--Doug Edgley KisaLa -- ** - HJ7)

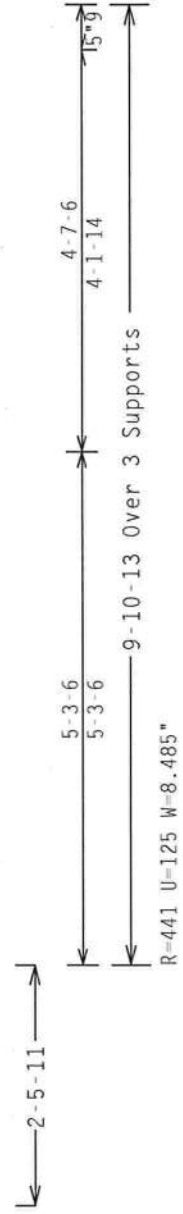
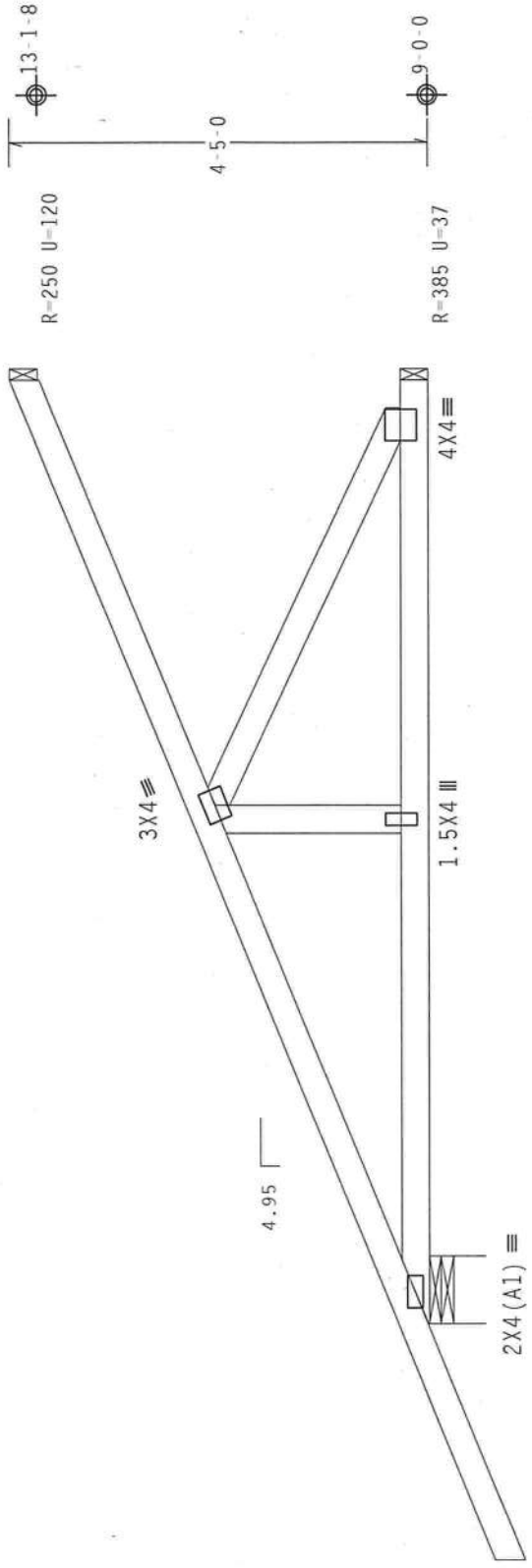
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located
anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0
psf. Iw=1.00 GCpi(+/-)=0.18

Hipjack supports 7-0-0 setback jacks with no webs.

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load.



Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10%(0%)/0(0)

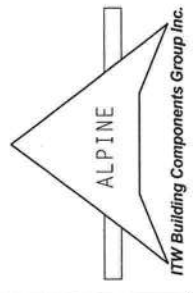
Scale = .5" / Ft.

TC LL	20.0 PSF
TC DL	10.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	40.0 PSF
DUR.FAC.	1.25



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(9-188--Doug Edgley Kisala --, ** - J3)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

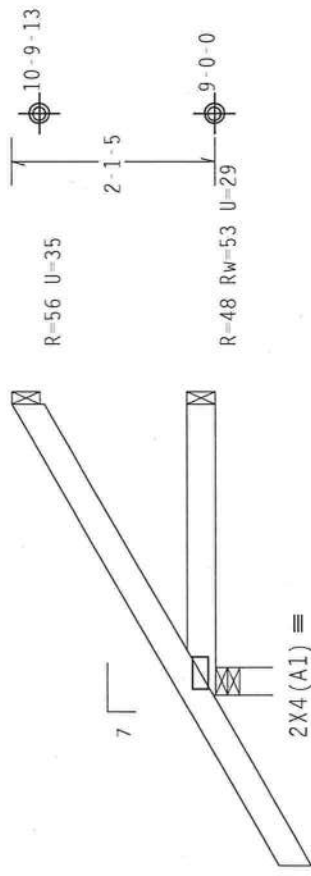
Roof overhang supports 2.00 psf soffit load.

Bottom chord checked for 10.00 psf non-concurrent live load.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, PART-ENC. bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load.



← 1-9-0 →
3-0-0 Over 3 Supports
R-292 U-93 W-3.5"
RL=94/-53

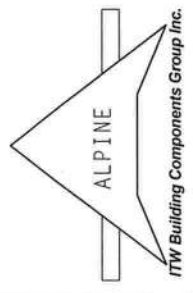
Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10%(0%)/0(0)



QTY: 4	FL / - / 4 / - / - / R / -	Scale = .5" / Ft.
TC LL	20.0 PSF	REF R8228- 31443
TC DL	10.0 PSF	DATE 11/10/09
BC DL	10.0 PSF	DRW HCUSR8228 09314019
BC LL	0.0 PSF	HC-ENG DF / DF
TOT.LD.	40.0 PSF	SEQN- 69457
DUR.FAC.	1.25	FROM AH

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. ITW REG DESIGN COMPLIES WITH APPLICABLE PROVISIONS OF AISC (HISTORICAL DESIGN SEC. 16, K10.1-10.5) GALV. STEEL, APPLY PLATES TO EACH FACE OF TRUSS AND UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 100A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT.



Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

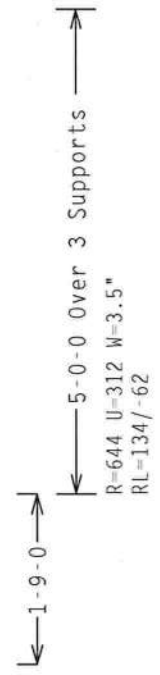
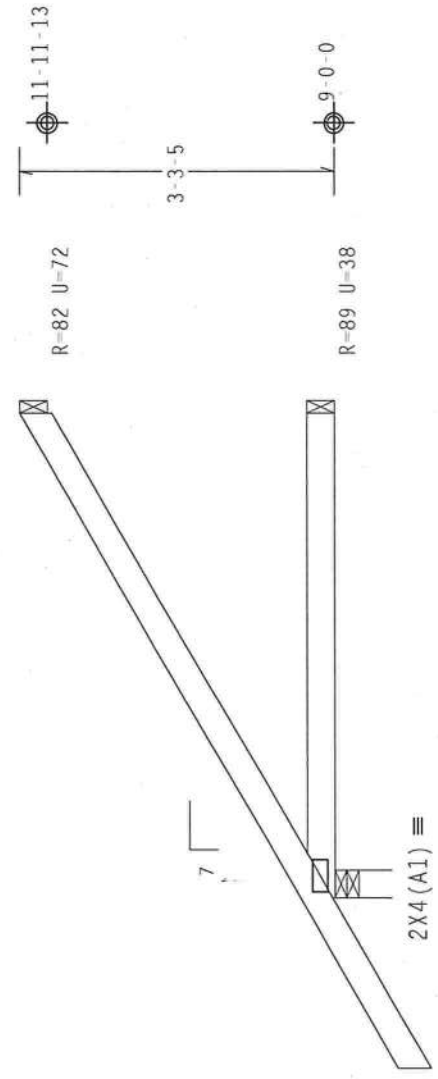
Special loads

----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC - From 63 plf at -1.75 to 63 plf at 5.00
BC - From 5 plf at -1.75 to 5 plf at 0.00
BC - From 20 plf at 0.00 to 20 plf at 5.00
TC - 207 lb Conc. Load at -1.71

Wind reactions based on MWFRS pressures.

THE BUILDING DESIGNER SHALL EVALUATE AND APPROVE LOAD MAGNITUDES AND LOCATIONS. THE TRUSS ENGINEER IS NOT RESPONSIBLE FOR LOAD MAGNITUDES AND LOCATIONS.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, PART ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCpi (+/-)=0.55
Roof overhang supports 2.00 psf soffit load.
Bottom chord checked for 10.00 psf non-concurrent live load.
Deflection meets L/240 live and L/180 total load.

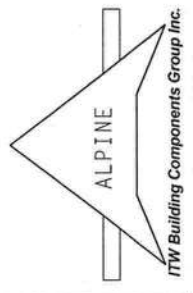


Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave	QTY: 5	FL / - / 4 / - / - / R / -	Scale = .5" / Ft.
	TC LL	20.0 PSF	REF R8228- 31444
	TC DL	10.0 PSF	DATE 11/10/09
	BC DL	10.0 PSF	DRW HCUSR8228 09314016
	BC LL	0.0 PSF	HC-ENG DF/DF
	TOT.LD.	40.0 PSF	SEQN- 12185 REV
	DUR.FAC.	1.25	FROM AH

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI-1 OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE TPI (TRUSS PLATE INSTITUTE) DESIGN SPEC. (BY ACPA) AND TPI-1. ITW REG. PLATES TO EACH FACE OF TRUSS AND UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 100A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SOLELY FOR THE TRUSS COMPONENT DESIGN GROUP. THE SUFFICIENCY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE DESIGN GROUP.



(9-188--Doug Edgley Kisala -- ** - HJ7A)

Top chord 2x6 SP #2
Bot chord 2x6 SP #2
Webs 2x4 SP #3

The following trusses need concentrated loads at the end of their overhangs: 6-8-10 span/setback member on the 4-2-0 cant side requires 207 lbs and the 6-8-10 span/setback member on the 4-2-0 cant side requires 207 lbs.

Sub-fascia beam assumptions: 8-9-0 sub-fascia beam on the 4-2-0 cantilever side. 8-9-0 sub-fascia beam on the 4-2-0 cantilever side.

Deflection meets L/240 live and L/180 total load.

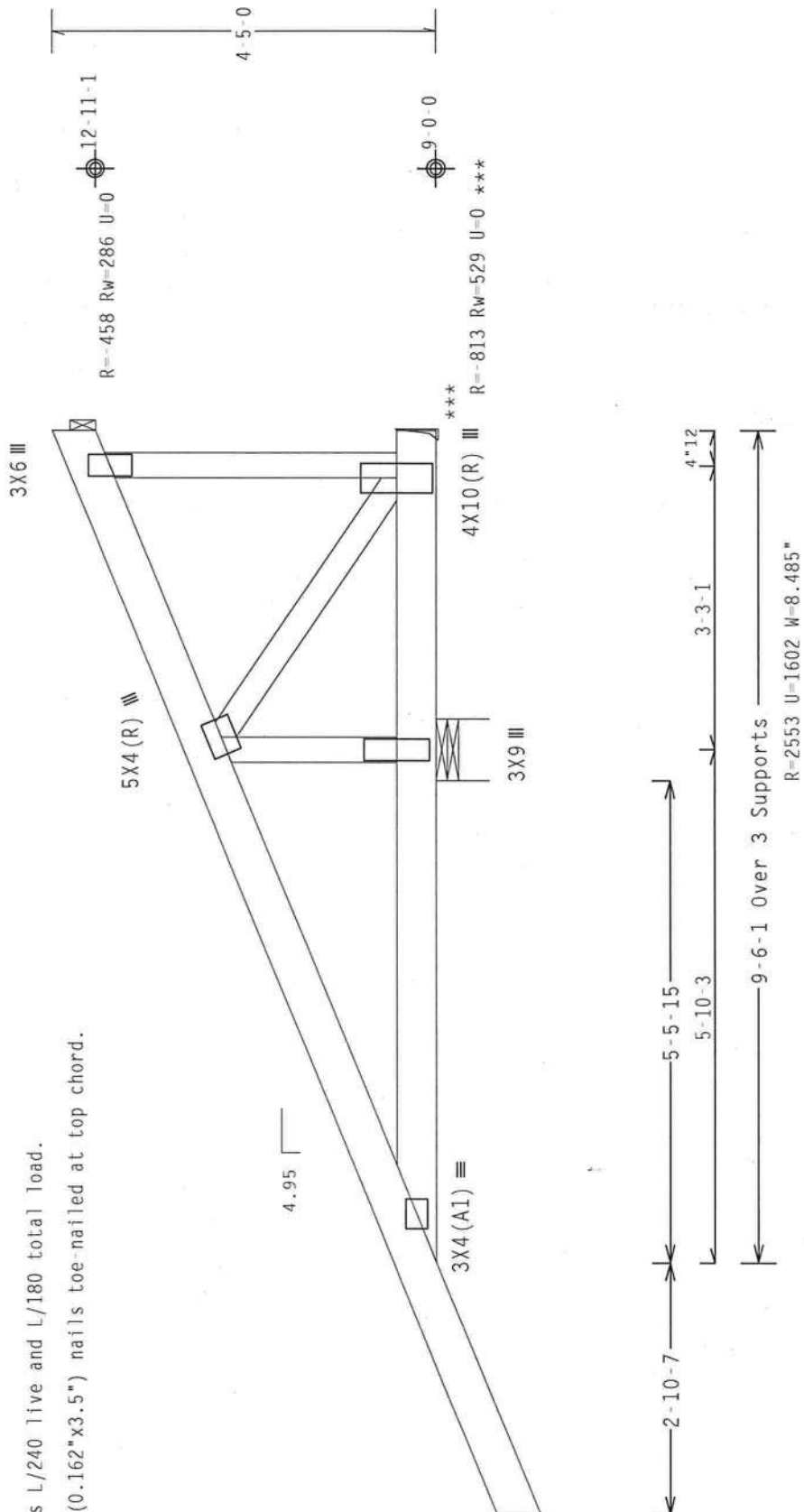
Provide (3)16d (0.162"x3.5") nails toe-nailed at top chord.

***Negative reaction(s) of -812# MAX. (See below) from a non-wind load case requires uplift connection.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, PART. ENC. bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL-5.0 psf, wind BC DL-5.0 psf. Iw=1.00 GCpi (+/-)-0.55

Wind reactions based on MWFRS pressures.

Hipjack supports 6-8-10 setback jacks with 4-2-0 cantilever one face; 4-2-0 cantilever opposite face.



WARNING** TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND MGA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. ITW REG CONNECTION PLATES ARE MADE OF 2024-T3 ALUMINUM (K1555) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWING 100A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL OR THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT.

PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10%(0%)/0(0)

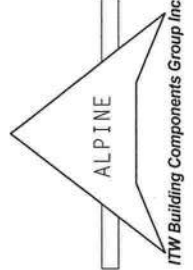
9-02-00

QTY: 1

FL / - / 4 / - / R / -

Scale = .5" / Ft.

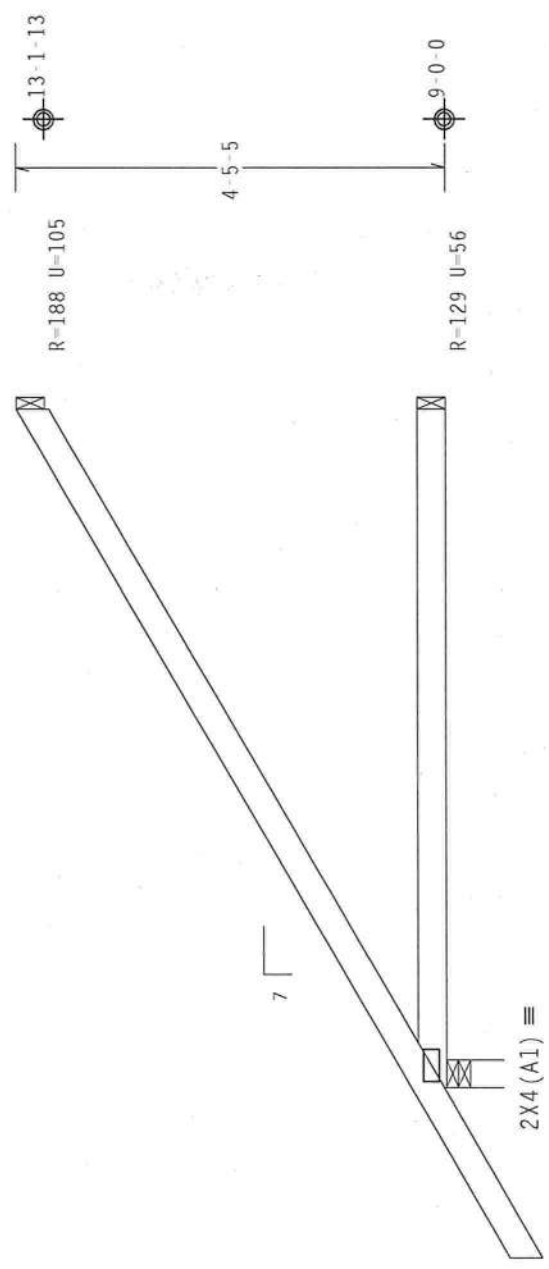
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TC DL	10.0 PSF	DATE	11/10/09
BC DL	10.0 PSF	DRW	HCUSR8228 09314001
BC LL	0.0 PSF	HC-ENG	DF / DF
TOT.LD.	40.0 PSF	SEQN-	70009
DUR.FAC.	1.25	FROM	AH



(9-188--Doug Edgley KisaJa -- , ** - EJ7)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Roof overhang supports 2.00 psf soffit load.
Bottom chord checked for 10.00 psf non-concurrent live load.
MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, PART-ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind IC DL-5.0 psf, wind BC DL-5.0 psf. $I_w=1.00$ GCpi (+/-)=0.55
Wind reactions based on MWFRS pressures.
Deflection meets L/240 live and L/180 total load.



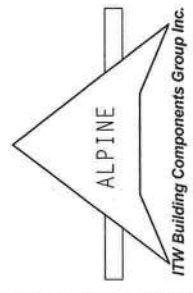
7'-0-0 Over 3 Supports
R-434 U=133 W=3.5"
RL=173/-71

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0)/0(0)

PLT TYP. Wave	QTY: 28	FL / - / 4 / - / R / -	Scale = .5" / Ft.
	TC LL	20.0 PSF	REF R8228- 31446
	TC DL	10.0 PSF	DATE 11/10/09
	BC DL	10.0 PSF	DRW HCUSR8228 09314020
	BC LL	0.0 PSF	HC-ENG DF/DF
	TOT.LD.	40.0 PSF	SEQN- 69465
	DUR.FAC.	1.25	FROM AH

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND NCA (NATIONAL COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIA/PDA) AND TPI. ITM BCG CORRECTOR PLATES ARE MADE OF 2018/1706A (N/1755K) ASH A663 GRADE 40/60 (N/4 K/1.55) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND BRACING. UNLESS OTHERWISE INDICATED ON THIS DRAWING. ALL TRUSSES SHALL BE SEAL OR THIS DRAWING INDICATES. ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DRAWING INDICATES.



(9-188--Doug Edgley Kisala --, ** - EJ7A)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCPI(+/-)=0.18

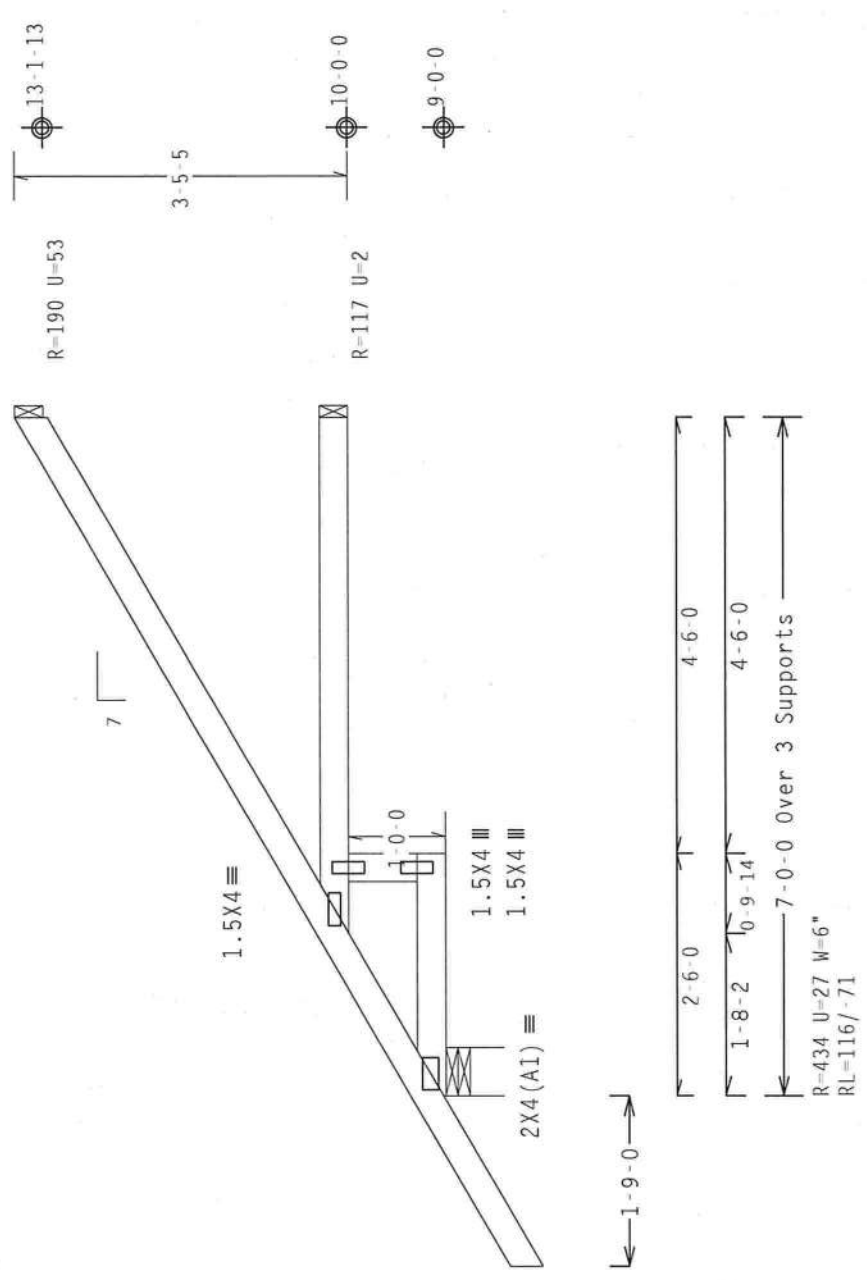
Roof overhang supports 2.00 psf soffit load.

Wind reactions based on MWFRS pressures.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.



Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10%(0%) / 0(0)

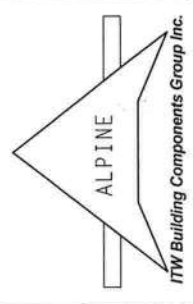
QTY: 6 FL/-/4/-/-/R/- Scale = 5"/Ft.

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TC DL	10.0 PSF	DATE	11/10/09
BC DL	10.0 PSF	DRW	HCUSR8228 09314038
BC LL	0.0 PSF	HC-ENG	DF/DF *
TOT.LD.	40.0 PSF	SEQN-	69479
DUR.FAC.	1.25	FROM	AH



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSE (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND MICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BEG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. ITW BEG DESIGN CONFORMS WITH APPLICABLE PROVISIONS (NATIONAL DESIGN SPEC., BY AISC) AND TPI. ITW BEG CONNECTOR PLATE FABRICATOR SHALL BE RESPONSIBLE FOR THE CORRECTNESS OF THE PLATE DESIGN. ITW BEG SHALL BE RESPONSIBLE FOR THE CORRECTNESS OF THE TRUSS DESIGN. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ABX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT

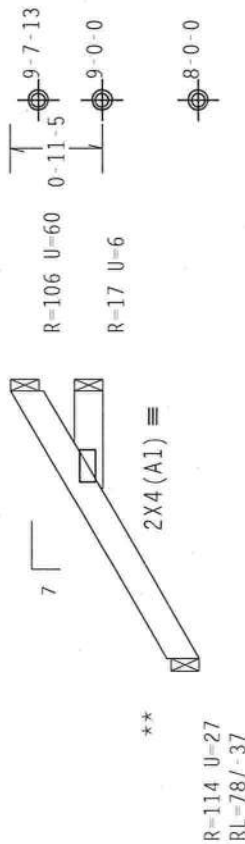


110 mph wind, 15.00 ft mean hgt, ASCE 7-05, PART. ENC. bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi (+/-) 0.55

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load.

** FASCIA BEAM DESIGNED AND FURNISHED BY OTHERS. PROVIDE CONNECTION FOR REACTIONS SHOWN.



2-9-0 Over 3 Supports

Design Crit: FBC2007Res/TPI-2002(STD)
 $FT/RT=10\%(0\%)/0(0)$

PLT TYP. Wave

0.02.00

QTY:2 FL/-/4/-/-/R/-

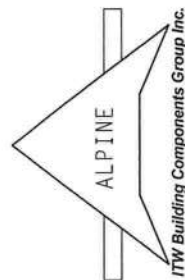
Scale = .5"/Ft.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION AND THE SHIPPING, INSTALLING AND BRACING OF THESE TRUSSES TO PREVENT DAMAGE TO THE TRUSSES BY THE FOLLOWING:

NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA 22314) AND JACOBO TRUSS COMPANY OF AMERICA, 6309 ENTERPRISE LANE, MADISON, MI 48071. F-537(19). FOR SAFETY PURPOSES THESE FUNCTIONS, UNLESS OTHERWISE INDICATED TOP GORED SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM GORED SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

*****WARNING***** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT CARE INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND AISC (AMERICAN INSTITUTE OF STEEL CONSTRUCTION, INC., 500 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) FOR TRUSS CONSTRUCTION PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

*****IMPORTANT***** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. TPI BCG, INC. SHALL NOT BE RESPONSIBLE FOR THE CONSTRUCTION OF THIS DESIGN. THE TRUSS IS TO BE CONSTRUCTED IN CONFORMANCE WITH THE DESIGNING MANUFACTURING AND BRACING SPECIFICATIONS, AND THE MANUFACTURING AND BRACING SPECIFICATIONS OF THE TRUSS SHALL BE IN CONFORMANCE WITH THE DESIGNING MANUFACTURING AND BRACING SPECIFICATIONS OF NOS. NATIONAL DESIGN SPEC. BY AISC AND TPI. TPI BCG CONNECTOR PLATES ARE MADE OF 2018/116GA. (H-K/55KPS) ASTM A653 GRADE 40/46 (H-K/55) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE NOTED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SOLELY FOR TPI TRUSS COMPONENT



TW Building Components Group Inc.

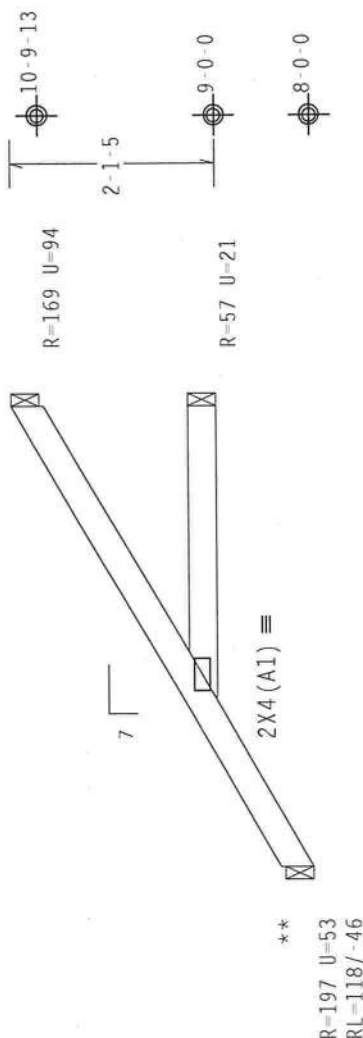
A circular professional seal for the State of Florida. The outer ring contains the text "DOUGLAS FLEMING" at the top and "PROFESSIONAL ENGINEER" at the bottom, separated by two stars. Inside the ring, the words "STATE OF FLORIDA" are written in a semi-circle. In the center, "LICENSE" is written above "No. 66648". A signature, "DF", is written over the license number.

11110 mph wind, 15.00 ft mean hgt, ASCE 7-05, PART. ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCpi (μ)=0.55

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load.

*** FASCIA BEAM DESIGNED AND FURNISHED BY OTHERS. PROVIDE CONNECTION FOR REACTIONS SHOWN.



← 4-9-0 Over 3 Supports →

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

QTY: 2 FL / - / 4 / - / - / R / - Scale = .5" / Ft.

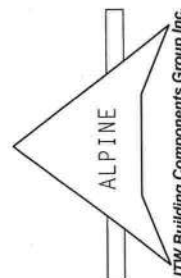
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TC DL	10.0 PSF	DATE 11/10/09
BC DL	10.0 PSF	DRW HCURSR8228 09314014
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 69489
DUR.FAC.	1.25	FROM AH



WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, INSTALLING AND BRACING. REFER TO BCSP (BUILDING COMPONENTS SUPPLY PROMOTION) PUBLICATION, "TRUSSING FOR THE 1980'S", AVAILABLE FROM THE NATIONAL TRUSS MANUFACTURING INSTITUTE, 2718 NORTH LEE STREET, SUITE 314, CHICAGO, ILLINOIS 60640. THE TRUSS COUNCIL OF AMERICA, 6300 WEST 12TH AVENUE, DENVER, COLORADO 80202, PROVIDES ADDITIONAL INFORMATION. THE TRUSS COUNCIL OF AMERICA RECOMMENDS THAT ALL TRUSS MANUFACTURERS BE QUALIFIED TO PERFORM THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID GIRDERS.

****IMPORTANT**** URNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR, ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO FOLLOW THE TROUS IN COMPLIANCE WITH THE DESIGN OR FABRICATING, HANDLING, SHIPPING, INSTALLING, OR BRACING OF TROUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., BY AF&PA) AND TPV (TYPICAL CONNECTION PLATES ARE MADE OF 20/91R16GA (U.S.75S) ASTM A653 GRAIN 80% MIN. STEEL, APPLY PLANTS TO EACH FACE OF TROUSS AND UNLESS OTHERWISE SPECIFIED, POSITION PER DRAWINGS 160A, 2 INSPECTION PLACEMENT SHALL BE AS SHOWN PER ANNEK A3 OF TPV-2009 SEC 3. A SEAL ON THIS DRAWING IS REQUIRED TO PREVENT FALSIFICATION OF THE PROJECT ENGINEERING RESPONSIBILITY. SOLELY FOR E TRUSS COMPONENT



Top	chord	2x4	SP	#2	Dense
Bot	chord	2x4	SP	#2	Dense
	webs	2x4	SP	#3	

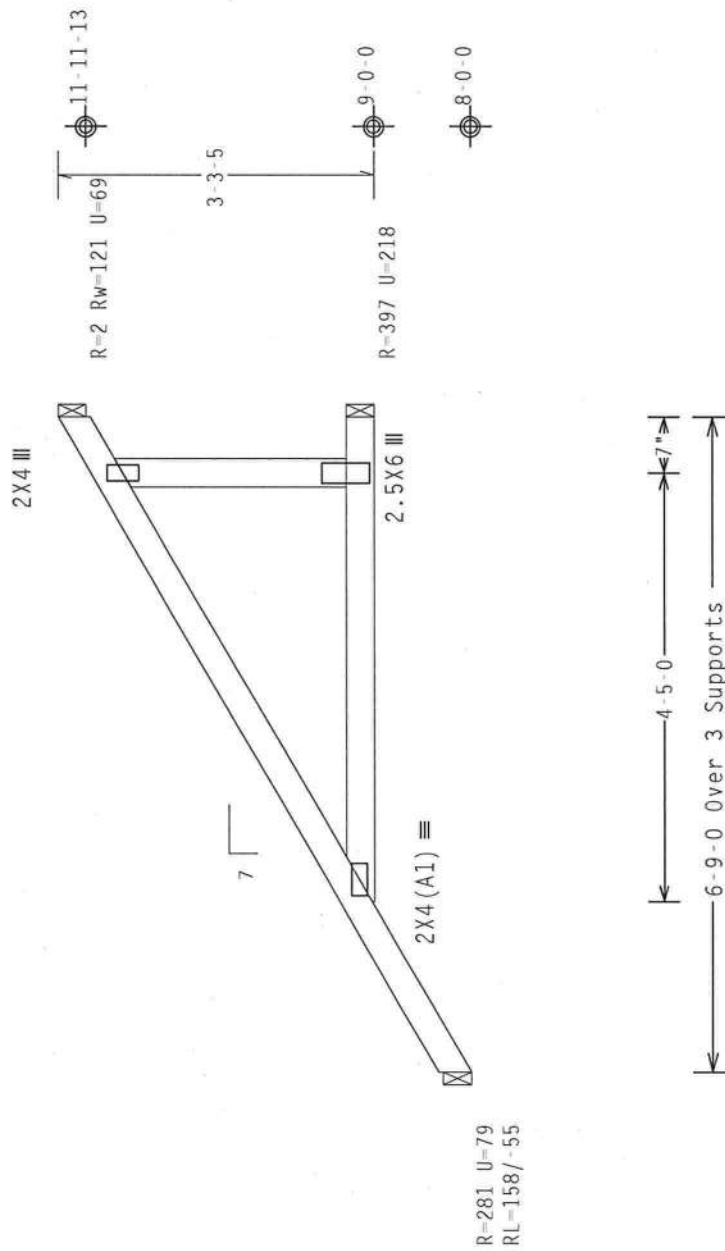
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, PART ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. lw=1.00 GCpi (+/-)=0.55

Roof overhang supports 2.00 psf soffit load.

Wind reactions based on MWFRS pressures.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.



PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

02.00

QTY:1

FL/-/4/-/-/R/-

Scale = .5"/Ft.

WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. TRUSSES MUST BE PROPERLY BRACED TO PREVENT BUCKLING. FOR ADDITIONAL INFORMATION, PUBLISHED BY TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, SAN ANTONIO, TX 78204, OR TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719. FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS, UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

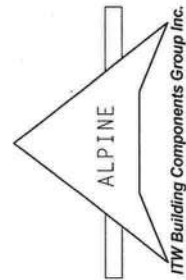
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TC DL	10.0	PSF	DATE	11/10/09
BC DL	10.0	PSF	DRW	HCUSR8228 09314002
BC LL	0.0	PSF	HC-ENG	DF/DF
TOT.LD.	40.0	PSF	SEQN-	69973
DUR.FAC.	1.25		FROM	AH

****IMPORTANT****-FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR, ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DAMAGE TO THE TRUSS IN COMPLIANCE WITH THE FOLLOWING:

(1) ON FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING STRUCTURES.

(2) DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF RDOS (NATIONAL DESIGN SPEC., BY AISC/AISI) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 207/1016GA UNLH55/8K ASH 40/60 (M./K.H-SS) GALV. STEEL. APPLY CONNECTOR PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 16GA-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A-3 OF TPI-2002 SEC.3.

(3) ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DRAWING INDICATES.



ITW Building Components Group Inc.

(9-188--Doug Edgley Kisala -- , ** - EJ2)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

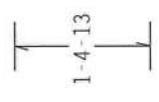
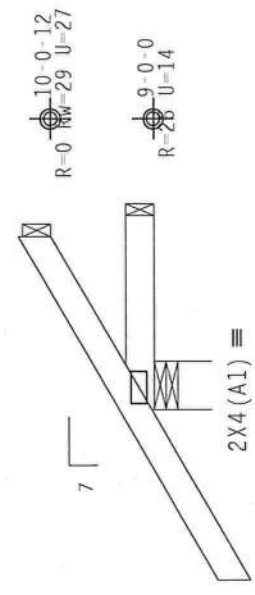
Roof overhang supports 2.00 psf soffit load.

Bottom chord checked for 10.00 psf non-concurrent live load.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load.



1-9-0
2-0-0 Over 3 Supports
R=273 U=77 W=6"
RL=69/-47

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%) / 0(0)

PLT TYP. Wave

9.02.00

QTY:2 FL/-4/-/-R/-

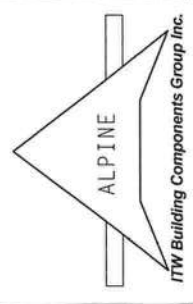
Scale =.5"/Ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. THE BCG DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., BY AIA/ASD) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 2019/16GA (44-H/SS/RS) ASH 4053 GRADE 40/60 (4" X 7/16") GALV. STEEL. ALLY PLATE INSPECTION OF PLATES FOLLOWED BY (3) SHALL BE PER ABX 43 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT



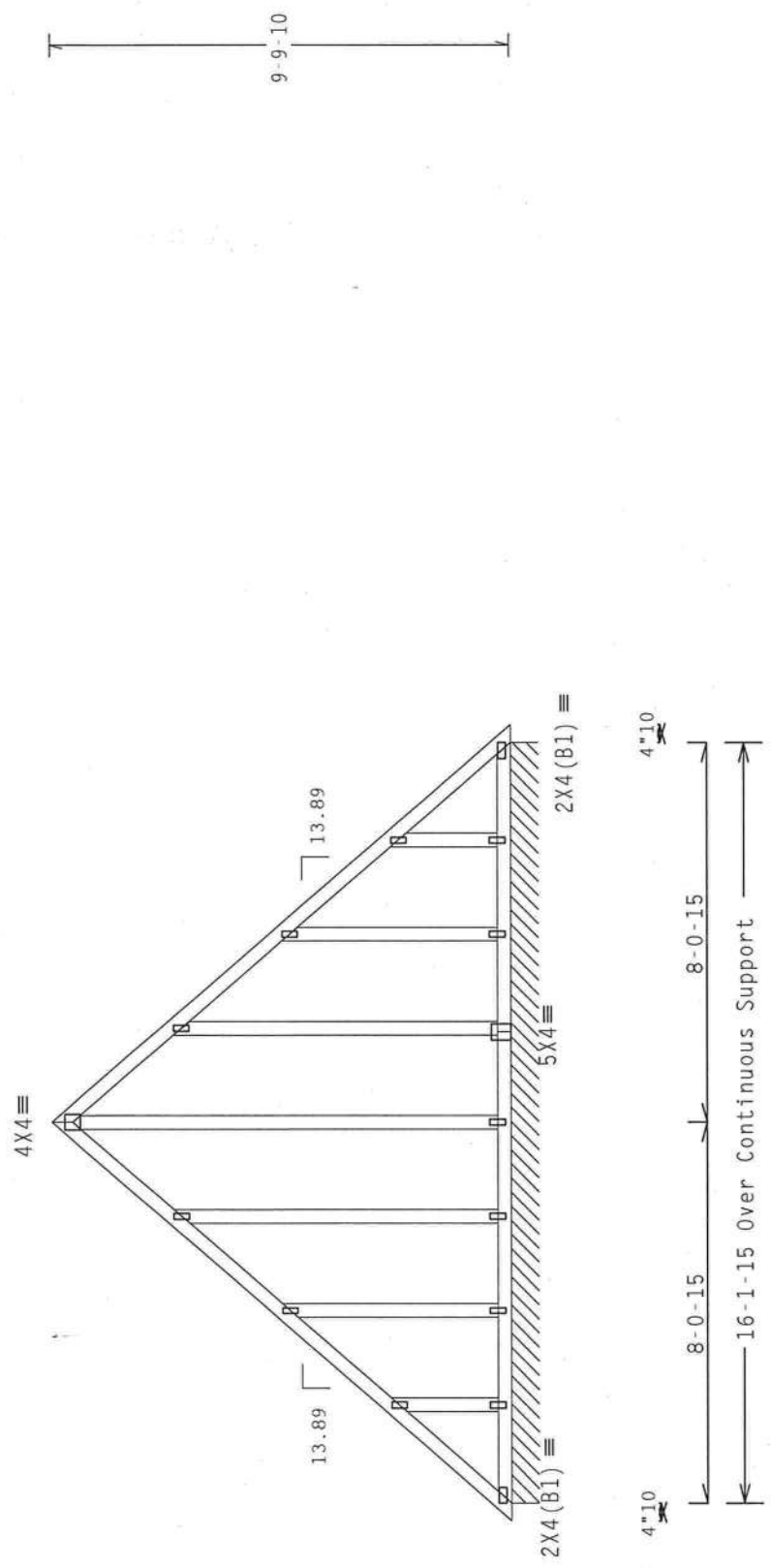
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TC DL	10.0 PSF	DATE	11/10/09
BC DL	10.0 PSF	DRW	HCUSR8228 09314004
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEON-	69605
DUR.FAC.	1.25	FROM	AH



(9-188--Doug Edgley Kisala -- ** - PF2)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

NOTE: THIS DESIGN IS NOT TO BE USED AS A ROOF TRUSS. IT IS
TO BE USED AS A ROOF HIP FRAME.
SEE DETAIL HIPFRAME0109 FOR MORE INFORMATION.



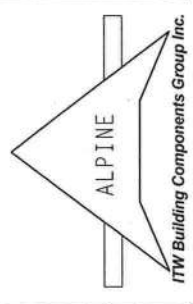
Note: All Plates Are 1.5X4 Except As Shown.
Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%) / 0(0)

PLT TYP. Wave		QTY: 1	FL / - / 4 / - / - / R / -	Scale = .25" / Ft.
		TC LL	20.0 PSF	REF R8228- 31452
		TC DL	10.0 PSF	DATE 11/10/09
		BC DL	10.0 PSF	DRW HCUSR8228 09314046
		BC LL	0.0 PSF	HC-ENG DF/DF
		TOT. LD.	40.0 PSF	SEQN- 39265
		DUR. FAC.	1.25	FROM AH



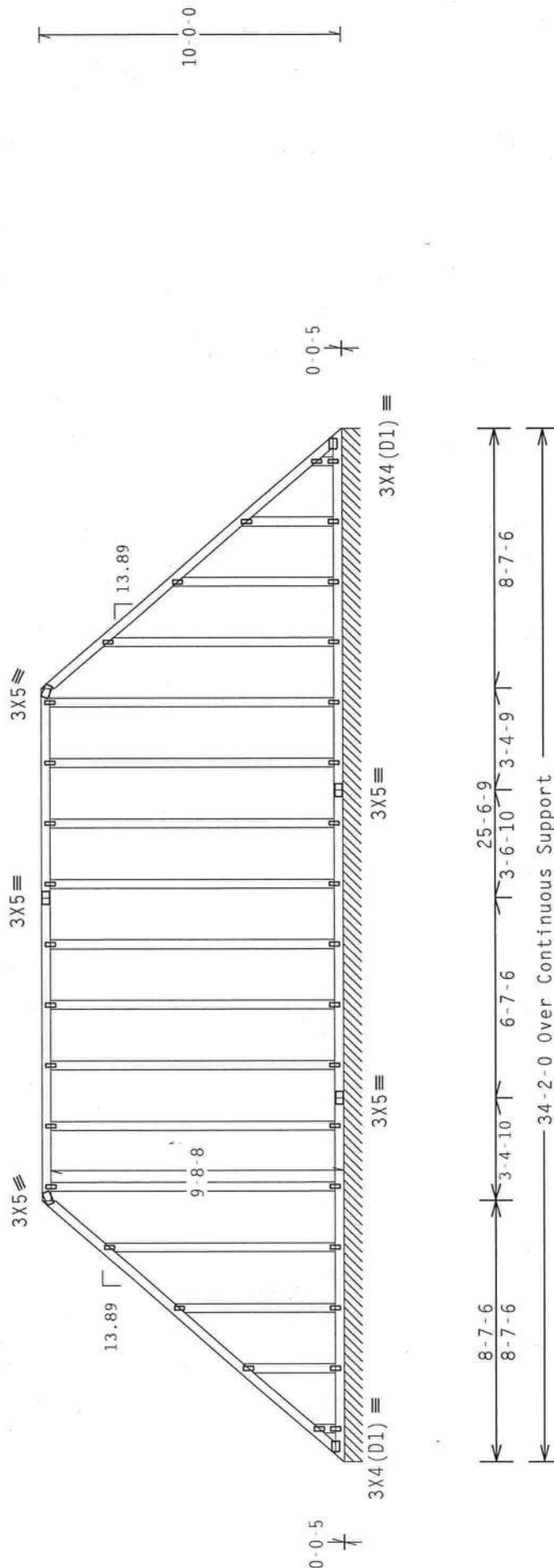
****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND MICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. TPI BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. THE BCG DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. THE BCG CONNECTOR PLATES ARE MADE OF 2024-T3 ALUMINUM (4.0X10.0X.0625) AS PER 2002 SECTION 11.1.1. APPLY PLATES TO EACH FACE OF TRUSS AND WEBS UNLESS OTHERWISE NOTED. THE TRUSS SHALL BE SEaled ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SIGNED FOR THE TRUSS COMPONENT



	Top	chord	2x4	SP	#2	Dense
Bot	chord	2x4	SP	#2	Dense	
		webs	2x4	SP	#3	

NOTE: THIS DESIGN IS NOT TO BE USED AS A ROOF TRUSS. IT IS TO BE USED AS A ROOF HIP FRAME. SEE DETAIL HIPRAME0109 FOR MORE INFORMATION.



Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave

0(0) 9.02.00

QTY:1 FL/-/4/-/-/R/-

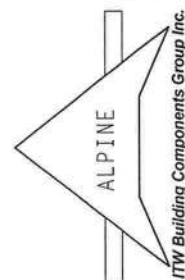
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TC LL	20.0 PSF	REF R8228- 31453
TC DL	10.0 PSF	DATE 11/10/09
BC DL	10.0 PSF	DRW HCUR8228 09314040
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 39296
DUR.FAC.	1.25	FROM AH



WARNING THUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI, STEEL PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ANDOVER, MA 02214, AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, WEST LOSON, MI 53219), FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNDESIRABLE THERMAL INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CHORDS.

IMPORTANT—FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO ADHERE TO THIS DESIGN IN COMPLIANCE WITH THE DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AOS (NATIONAL DESIGN SPEC., BY AIAAPA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 2018/1868 (9-1/2X5/8) ASH 4053 GRADE 40/60 (3/4-5/8) GALV. STEEL, APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. IF ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMEX AS OF 1/11-2002 SEC.2. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT

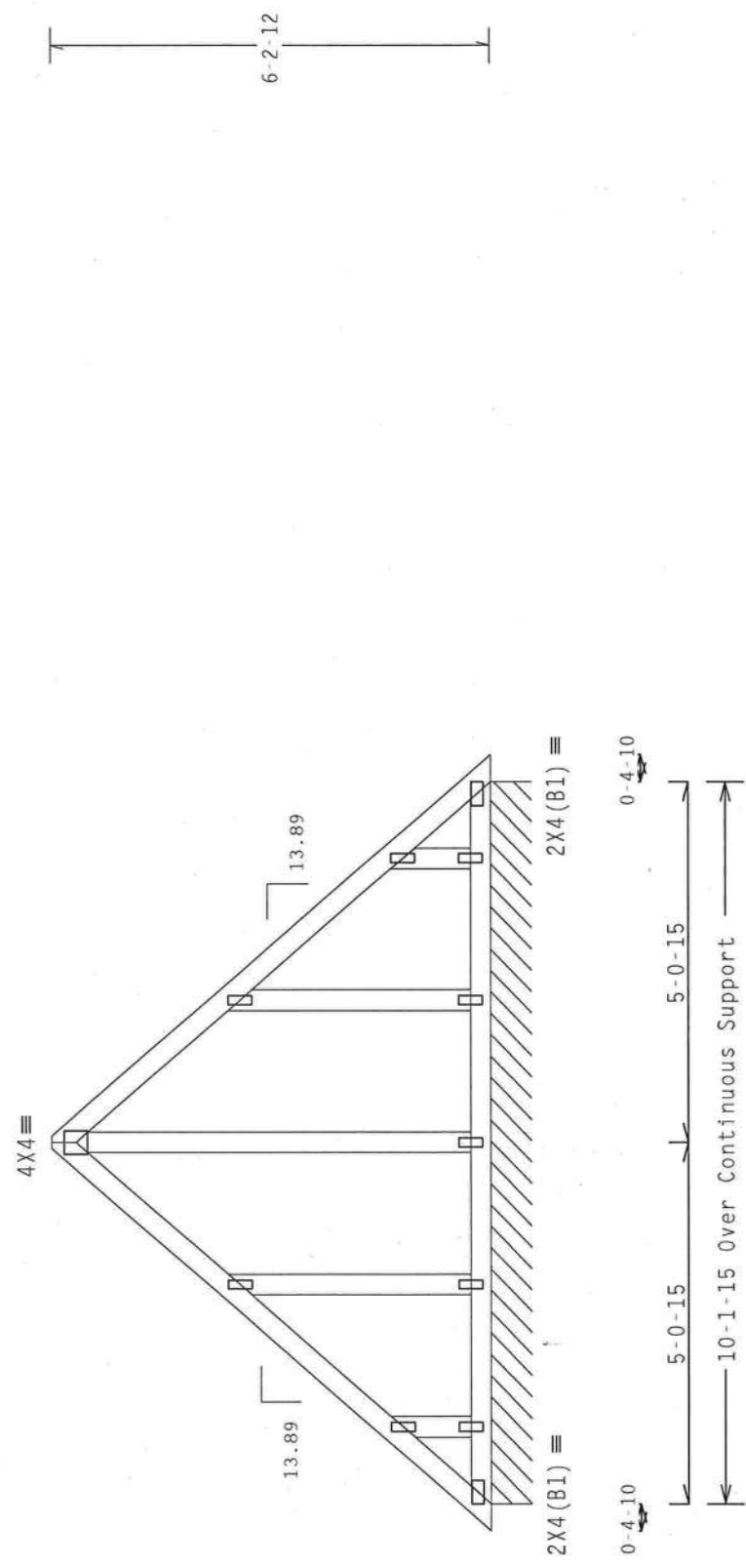


ITW Building Components Group Inc.

(9-188--Doug Edgley Kisala -- ** - PF4)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

NOTE: THIS DESIGN IS NOT TO BE USED AS A ROOF TRUSS. IT IS
TO BE USED AS A ROOF HIP FRAME.
SEE DETAIL HIPFRAME0109 FOR MORE INFORMATION.



Note: All Plates Are 1.5X4 Except As Shown.
Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%) JO(0)

QTY:1

FL/-/4/-/-/R/-

Scale = .375"/Ft.

TC LL

20.0 PSF

REF R8228- 31454

TC DL

10.0 PSF

DATE 11/10/09

BC DL

10.0 PSF

DRW HCUSR8228 09314048

BC LL

0.0 PSF

HC-ENG DF/DF

TOT.LD.

40.0 PSF

SEQN- 39315

DUR.FAC.

1.25

FROM AH

DOUGLAS FLEMING

LICENSE

No 66648

STATE OF

FLORIDA

PROFESSIONAL ENGINEER

10 '09

TC LL	20.0 PSF	REF R8228- 31454
TC DL	10.0 PSF	DATE 11/10/09
BC DL	10.0 PSF	DRW HCUSR8228 09314048
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 39315
DUR.FAC.	1.25	FROM AH

ALPINE

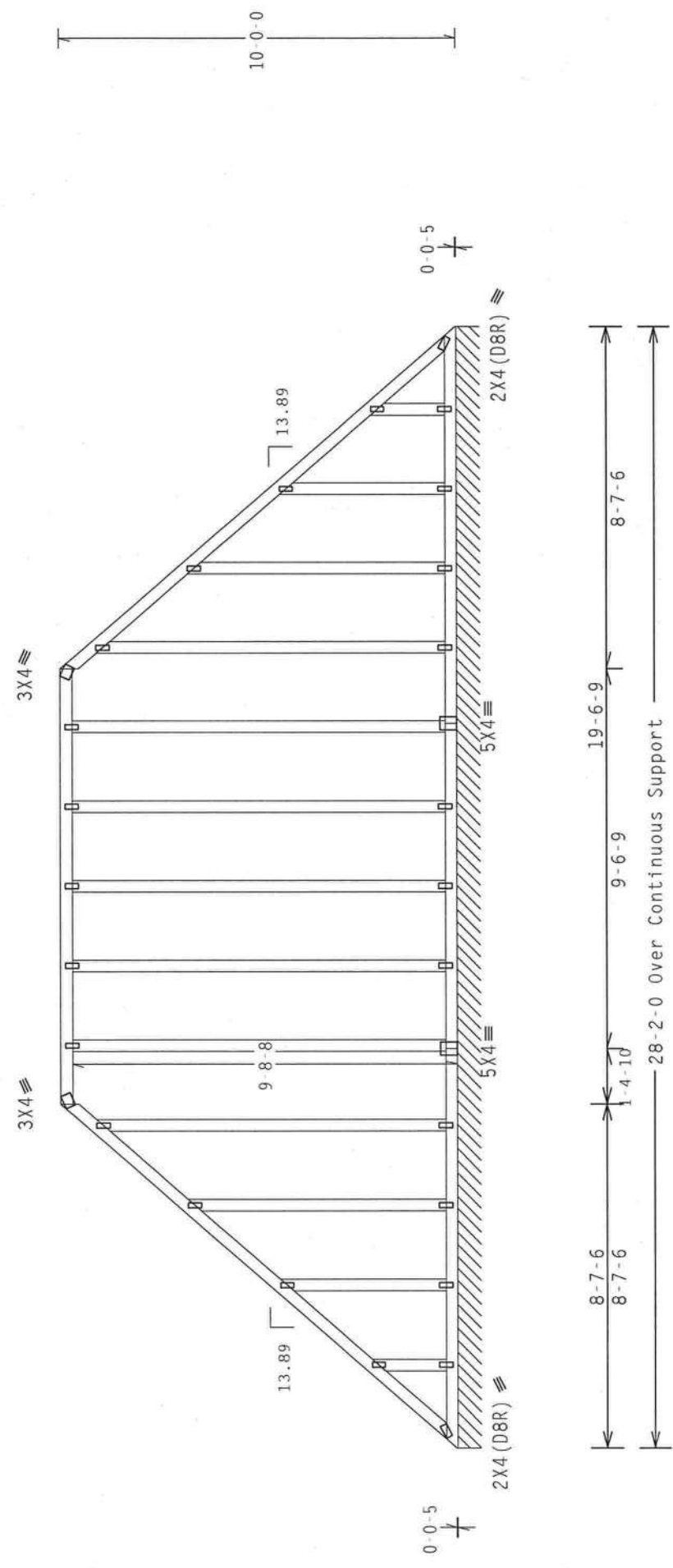
ITW Building Components Group Inc.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING & BRACING. REFER TO BEST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND MICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. ITW BCG DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF ROSS NATIONAL DESIGN SPEC. BY AERPA) AND TPI. ITW BCG CONNECTOR PLATES ARE INSTALLED PER TPI (TPI CONNECTOR PLATE) GALV. STEEL, APPX. 2. ALL DIMENSIONS ARE IN FEET AND INCHES. UNLESS OTHERWISE INDICATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

NOTE: THIS DESIGN IS NOT TO BE USED AS A ROOF TRUSS. IT IS
TO BE USED AS A ROOF HIP FRAME.
SEE DETAIL HIPFRAME0109 FOR MORE INFORMATION.



Note: All Plates Are 1.5X4 Except As Shown.
Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%) / 0(0)

PLT TYP. Wave QTY: 1 FL / - / 4 / - / - / R / - Scale = .25" / Ft.

DOUGLAS FLEMING
LICENSE
No. 66648
STATE OF FLORIDA
PROFESSIONAL ENGINEER

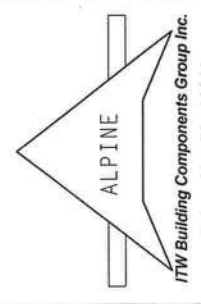
REF R8228- 31455
DATE 11/10/09
DRW HCUSR8228 09314021
HC-ENG DF/DF
SEQN- 39337
FROM AH

TC LL 20.0 PSF
TC DL 10.0 PSF
BC DL 10.0 PSF
BC LL 0.0 PSF
TOT. LD. 40.0 PSF
DUR. FAC. 1.25

10 '09

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND MTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN OR WITH APPLICABLE PROVISIONS OF IBCS (NATIONAL DESIGN SPEC. BY AIA/ASA) AND TPI. ITW BCG CORP. PLATES ARE MADE OF 2018/18GA (H/H/SS/PS) ASH 6051 GRADE 40/60 (W. K/H/SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE DESIGN ENGINEER.



Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

(**) 6 plate(s) require special positioning. Refer to scaled plate
plot details for special positioning requirements.

110 mph wind, 19.83 ft mean hgt, ASCE 7-05, CLOSED bldg, not located
within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf,
wind BC DL=2.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Refer to DWG PB1200109 for piggyback details.

Special loads

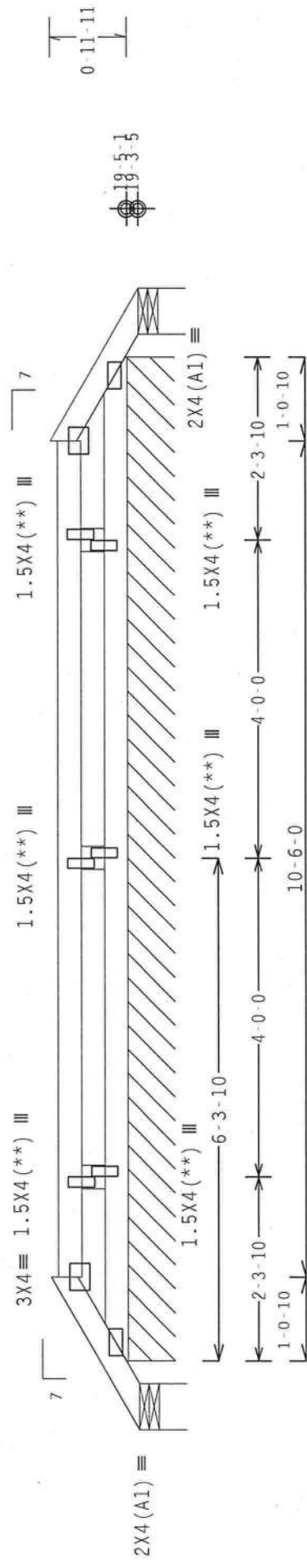
TC - From	Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
63 plf at 0.00 to	63 plf at 2.00
TC - From	63 plf at 2.00 to
63 plf at 12.33 to	63 plf at 14.33
BC - From	4 plf at 0.00 to
4 plf at 14.33	

In lieu of structural panels use purlins to brace all flat TC @ 24"

Deflection meets L/240 live and L/180 total load.

MWFRS loads based on trusses located at least 19.83 ft. from roof
edge.

3X4=



R=18 U=15 W=6.946"
RL=30/-30

R=70 PLF U=32 PLF W=12-7-4

R=18 U=7 W=6.946"

Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10%(0%) I/O(0)

Scale = .5"/Ft.

QTY:1

9.02.00

FL/-4/-/-/R/-

TC LL

TC DL

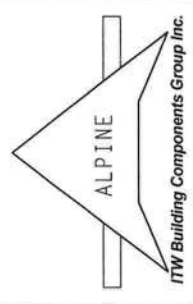
BC DL

BC LL

TOT.LD.

DUR.FAC.

REF	R8228- 31456
DATE	11/10/09
DRW	HCUSR8228 09314031
HC-ENG	DF/DF
SEQN-	39981
FROM	AH



Top	chord	2x4	SP	#2	Dense
Bot	chord	2x4	SP	#2	Dense
	webs	2x4	SP	#3	

1110 mph wind, 20.42 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=2.0 psf. $I_w=1.00$ GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load.

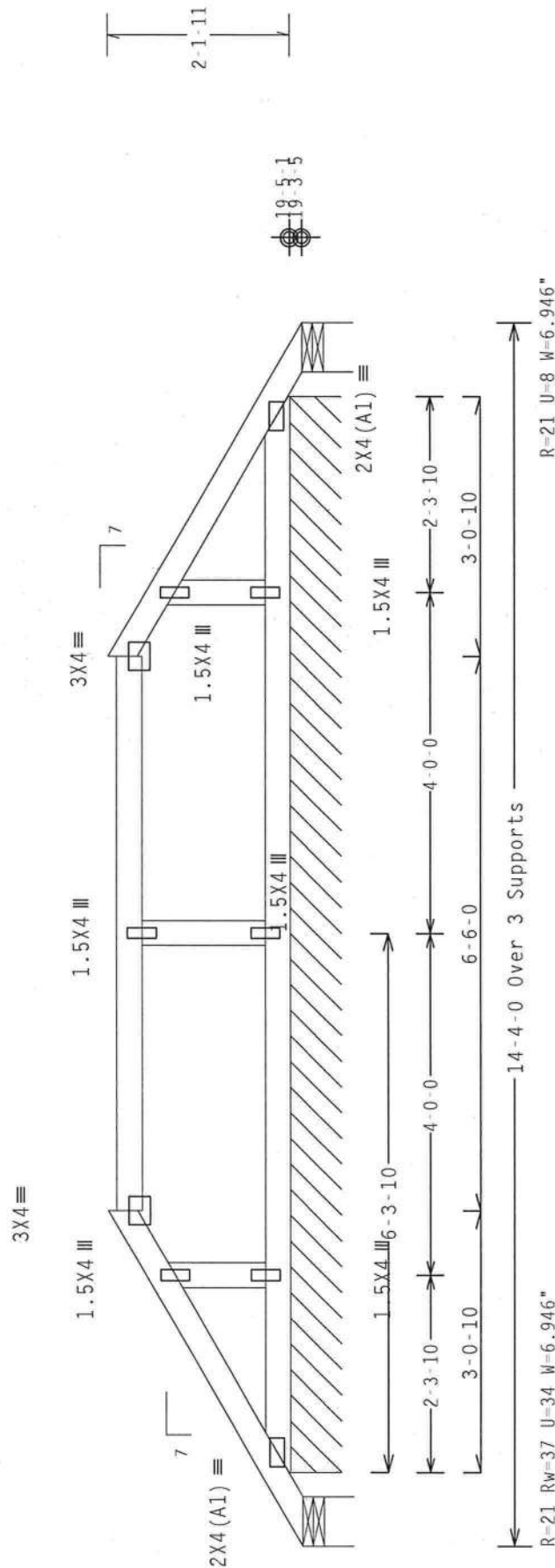
Refer to DWG PB1200109 for piggyback details.

Special loads

TC	- From	63 pif at 0.00 to	Dur.Fac.=1.25 / plate	Dur.Fac.=1.25
TC	- From	63 pif at 4.00 to	63 pif at 4.00	
TC	- From	63 pif at 4.00 to	63 pif at 10.33	
BC	- From	63 pif at 10.33 to	63 pif at 14.33	
BC	- From	4 pif at 0.00 to	4 pif at 14.33	

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" OC, all BC @ 24" OC.

MWFRS loads based on trusses located at least 20.42 ft. from roof edge.



R=21 RW=37 U=34 W=6.946"

RL=69/-8970 PLF U=25 PLF W=12-7-4

Design Crit: FBC2007Res/TPI-2002(STD)
 $FT/RT=10\%(0\%)/0(0)$

PLT TYP. Wave

.00

QTY:1 FL/-/4/-/-/R/-

Scale = .5"/Ft.

[illegible]

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR DETAILING ANY OF THE TRUSS DESIGN. ANY CHANGES TO THE TRUSS IN COMPLIANCE WITH THE DESIGNER'S REQUIREMENTS FOR BRACING OF TRUSSES, SHALL BE THE RESPONSIBILITY OF THE TRUSS CONTRACTOR. THE DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AASHTO HANDBOOK SPEC. FOR AEP/A AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/10/16GA. 0.415/25KSI ALUM 6053 GRADE 40/60 (H. K/H-55) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, IF NECESSARY, LOCATED ON THIS DESIGN, POSITION PER DRAWING 160A-2. IF ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A-3 OF TPII-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE CROSS COMPONENT



TC LL	20.0 PSF	REF R8228- 31457
TC DL	10.0 PSF	DATE 11/10/09
BC DL	10.0 PSF	DRW HCU8R8228 09314027
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 39987
DUR.FAC.	1.25	FROM AH

Top	chord	2x4	SP	#2	Dense
Bot	chord	2x4	SP	#2	Dense
	webs	2x4	SP	#3	

1110 mph wind, 21.37 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=2.0 psf. $I_w=1.00$ GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

MMWFRS loads based on trusses located at least 21.37 ft. from roof edge.

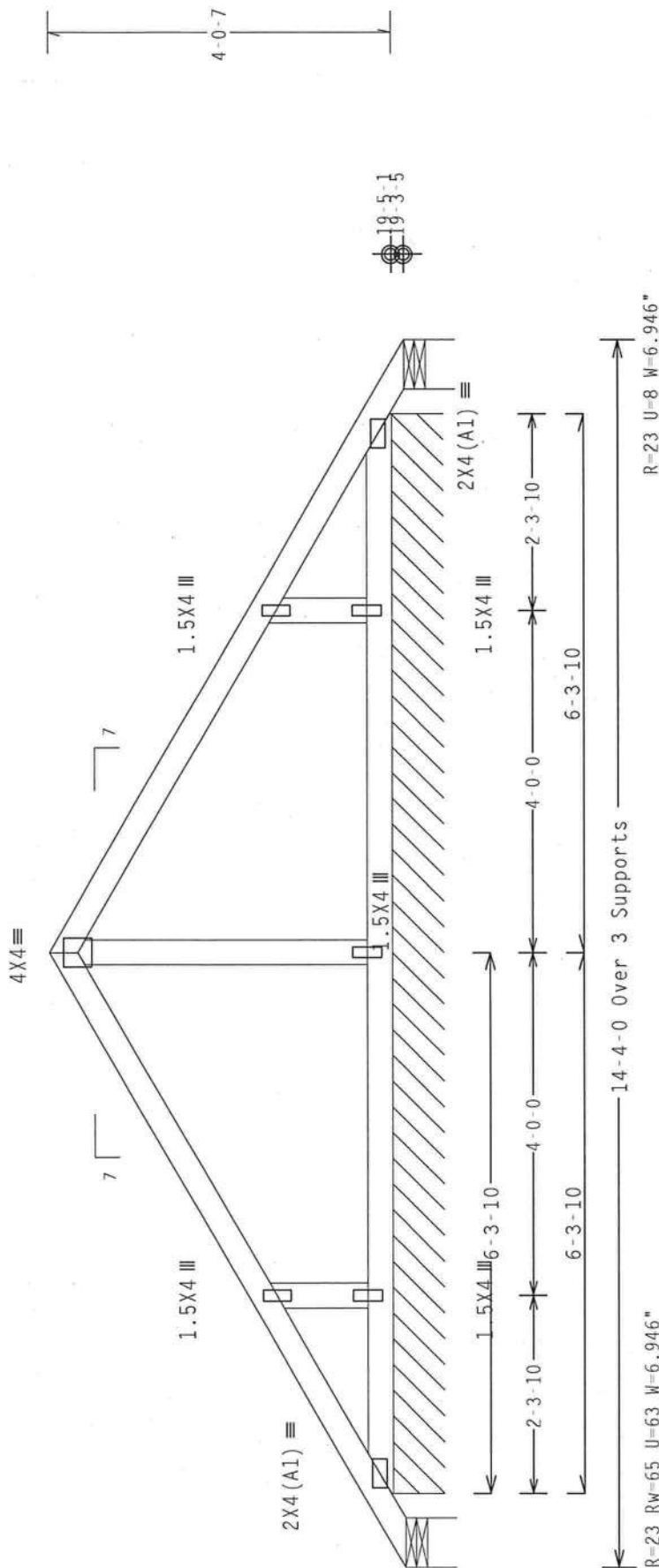
Refer to DWG PB1200109 for piggyback details.

Special loads

	(Lumber)	Dur.Fac.=1.25 /	Plate Dur.Fac.=1.25)
TC - From	63 plf at	0.00 to	63 plf at 7.17
TC - From	63 plf at	7.17 to	63 plf at 14.33
BC - From	4 plf at	0.00 to	4 plf at 14.33

In lieu of rigid ceiling use purlins to brace BC @ 24" OC.

Deflection meets L/240 live and L/180 total load.



R=23 RW=65 U=63 W=6.946"

RL=133/R180 PLF U=18 PLF W=12-7-4

Design Crit: $\text{FBC2007Res/TPI-2002(STD)}$
 $\text{FT/RT}=10\%(0\%)/0(0)$

PLT TYP. Wave

9.02.00

QTY: 9 FL / - / 4 / - / - / R / -

Scale = .5"/Ft.

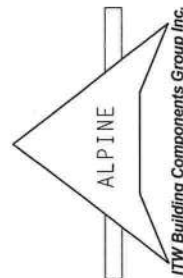
TC LL	20.0	PSF	REF R8228- 31459
TC DL	10.0	PSF	DATE 11/10/09
BC DL	10.0	PSF	DRW HCUSR8228 09314033
BC LL	0.0	PSF	HC-ENG DF/DF
TOT.LD.	40.0	PSF	SEQN- 39995
DUR.FAC.	1.25		FROM AH



*WARNING** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING TO PREVENT DAMAGE TO THE STRUCTURE. CONSULT WITH THE MANUFACTURER FOR THE FOLLOWING INFORMATION:
MORTIS LEE STEEL, SUITE 312, ALABAMA, VA 22314, AND WCA (WOOD) TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LAKE ROAD, SUITE 41, 57219, FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS.
OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT****-FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO COMPLY IN CONFORMANCE WITH THE DESIGN OR SPECIFICATIONS WILL BE AT THE USER'S RISK. PROVIDING THIS INFORMATION DOES NOT CONSTITUTE AN OFFER OF INSURANCE OR GUARANTEE. THE USER ASSUMES ALL LIABILITY FOR THE PROPER USE OF THIS INFORMATION AND THE RESULTS THEREOF.

DESIGN OF CORROSION RESISTANT APPLICATIONS: PROVIDING OF THIS NATIONAL DESIGN SPEC. BY AFPPA AND TP1. 1TH BCG CONNECTION PLATES ARE MADE OF 2018/16GGA (MIL-MS/K) GRADE 40/60 (H./KJLSS) GALV. STEEL, APPLY PLATES TO EACH FACE OF BRUUS AND QUINCY/STAS LOCATED ON THIS DESIGN. POSITION PER DRAWINGS 160A-Z. IF ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TP1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING SOLELY FOR E TRINS COMPONENT



ITW Building Components Group Inc.

(9-188--Doug Edgley Kisala -- . ** - PB5)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind. 21.00 ft mean hgt. ASCE 7-05, CLOSED bldg. not located within 4.50 ft from roof edge. CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=2.0 psf. $I_w=1.00$ GCPI(+/-)=0.18

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load.

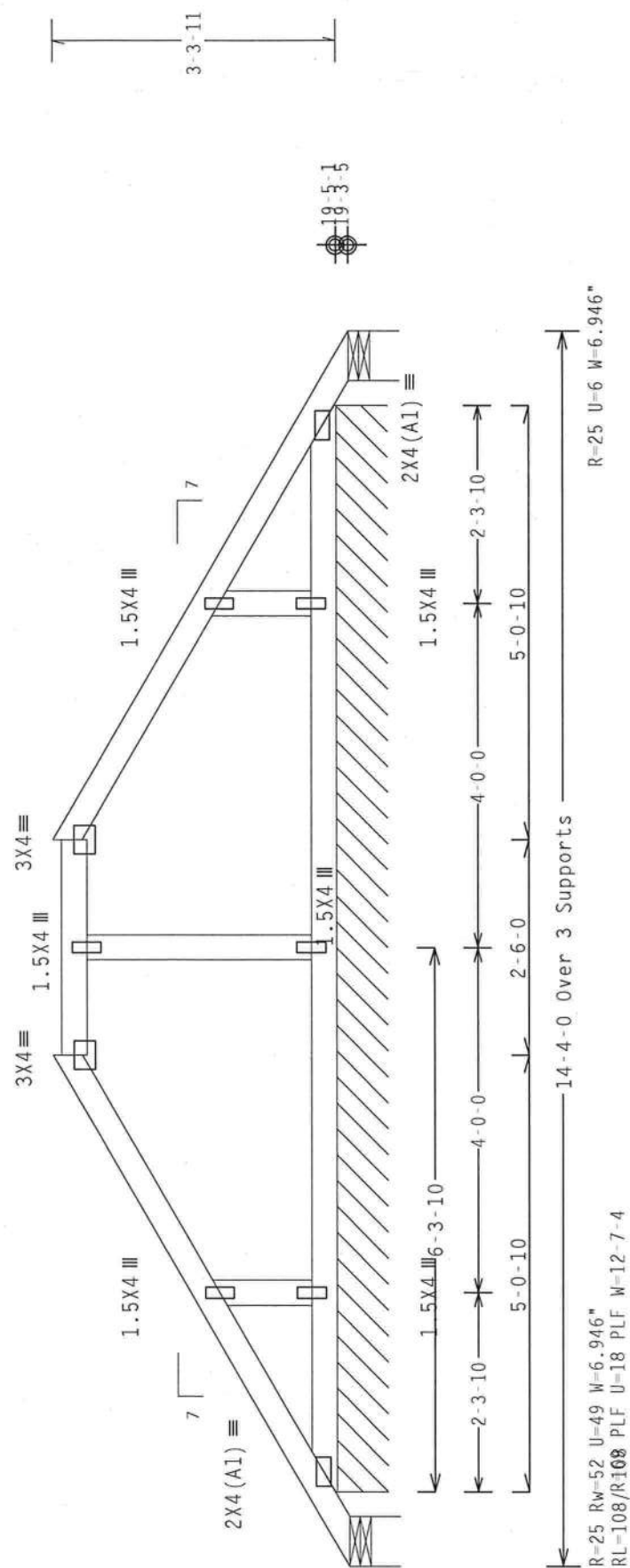
Refer to DWG PB1200109 for piggyback details.

Special loads

TC - From	63 plf at 0.00 to 63 plf at 6.00
TC - From	63 plf at 6.00 to 63 plf at 8.33
TC - From	63 plf at 8.33 to 63 plf at 14.33
BC - From	4 plf at 0.00 to 4 plf at 14.33

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" OC, all BC @ 24" OC.

MWFRS loads based on trusses located at least 21.00 ft. from roof edge.



Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10%(0%) / 0(0)

PLT TYP. Wave

QTY: 1 FL/-/4/-/-/R/-

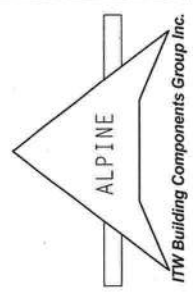
Scale = .5" / Ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIA/AS) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/10/16GA (44/45/55/59) ASH A653 GRADE 40/60 (4, 6/7/25) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE INDICATED, WITH DESIGN PER DIAGRAM ON THIS DRAWING. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROPER INSTALLATION OF THE TRUSS COMPONENTS. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SOLELY FOR THE TRUSS COMPONENT



TC LL	20.0 PSF	REF	R8228- 31460
TC DL	10.0 PSF	DATE	11/10/09
BC DL	10.0 PSF	DRW	HCUSR8228 09314045
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	40006
DUR. FAC.	1.25	FROM	AH



Top chord	2x4	SP	#2	Dense
Bot chord	2x4	SP	#2	Dense
Webbs	2x4	SP	#3	

110 mph wind, 20.49 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=2.0 psf. $I_w=1.00$ GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load.

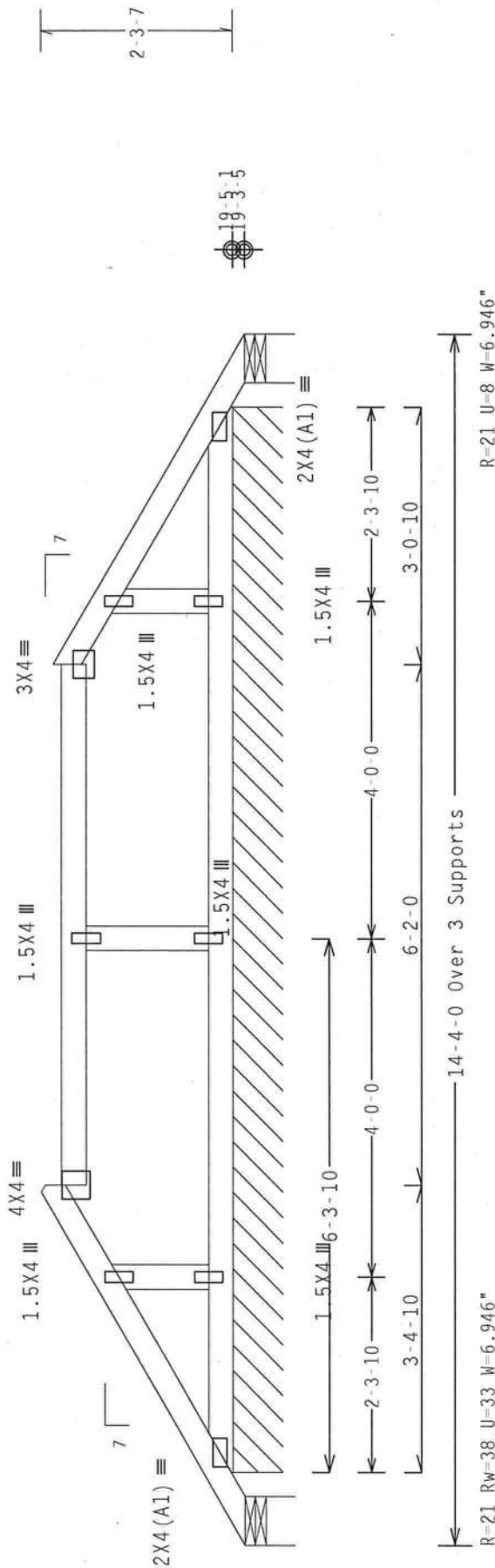
Refer to DWG PB1200109 for piggyback details.

Special loads

----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)			
TC	- From	63 plf at 0.00 to	63 plf at 4.17
TC	- From	63 plf at 4.17 to	63 plf at 10.42
TC	- From	63 plf at 10.42 to	63 plf at 14.33
BC	- From	4 plf at 0.00 to	4 plf at 14.33

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" OC, all BC @ 24" OC.

MMFRS loads based on trusses located at least 20.49 ft. from roof edge.



R=21 RW=38 U=33 W=6.946"

RL=75/-R270 PLF U=25 PLF W=12-7-4

14-4-0 Over 3 Supports

R=21 U=8 W=6.946"

PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

9.02.00

QTY:1 FL/-/4/-/-/R/-

Scale = .5"/Ft.

TC LL	20.0 PSF	REF R8228- 31461
TC DL	10.0 PSF	DATE 11/10/09
BC DL	10.0 PSF	DRW HCU8R8228 09314037
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 40013
DUR.FAC.	1.25	FROM AH



WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, SHIPPING, INSTALLING AND BRACING. THESE TRUSSES ARE A COMPLEX COMPONENT. SAFETY INFORMATION PUBLISHED BY IPI CONSULTING, INC., 100 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314, AND WCA GROUP TRUSS, COUNCIL OF AMERICA, 5300 ENTERPRISE LANE, WAUWATOSA, WI 53219, MUST BE FOLLOWED PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

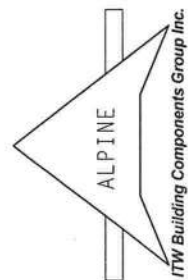
*** IMPORTANT *** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR THE DESIGN OF THE TRUSS IN CONFORMANCE WITH THE FOLLOWING REQUIREMENTS:

1. THE TRUSS SHALL BE DESIGNED TO BE USED FOR THE FOLLOWING APPLICATIONS: ERECTING, TRANSPORTING, HANDLING, SHIPPING, UNLOADING, AND BRACING OF TRUSSES, WITH BCG DESIGN COMPLIANT WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., BY AIA/ASD) AND TPI.

2. THE TRUSS SHALL BE DESIGNED TO SUPPORT THE FOLLOWING LOADS: 100 PSF (40 KN/SQ M) GALV. STEEL, APPLY TO PLATES TO EACH FACE OF TRUSS, AND UNLESS OTHERWISE NOTED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2.

3. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A OF TPI-2002 SEC. 3.

4. THE DESIGN OF THE TRUSS SHALL BE THE SOLE RESPONSIBILITY OF THE USER FOR THE TRUSS COMPONENT BEING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY FOR THE TRUSS DESIGN.



ITW Building Components Group Inc.

	Top	chord	2x4	SP	#2	Dense
	Bot	chord	2x4	SP	#2	Dense
		webs	2x4	SP	#3	

(**) 4 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

1110 mph wind, 20.49 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=2.0 psf. $I_w=1.00$ GCPI (+/-)=0.18

Wind reactions based on MwFRS pressures.

MMFRS loads based on trusses located at least 20.49 ft. from roof edge.

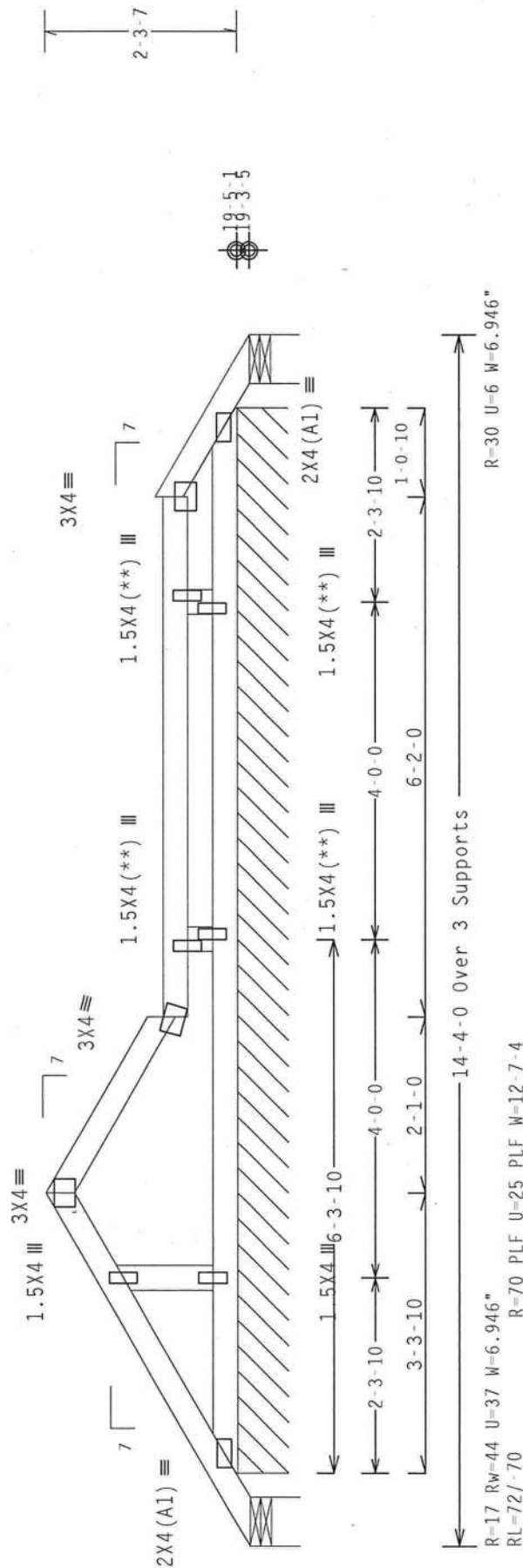
Refer to DWG PB1200109 for piggyback details.

Special loads

		(Lumber Dur.Fac.=1.25 / plate Dur.Fac.=1.25)	
TC - From	63 pif at 0.00 to	63 pif at 4.17	
IC - From	63 pif at 4.17 to	63 pif at 6.33	
TC - From	63 pif at 6.33 to	63 pif at 12.33	
IC - From	63 pif at 12.33 to	63 pif at 14.33	
BC - From	4 pif at 0.00 to	4 pif at 14.33	

In lieu of structural panels use purlins to brace all flat TC @ 24" O.C.

Deflection meets L/240 live and L/180 total load.



PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

.00

QTY:1 FL/-/4/-/-/R/-

Scale = .5"/Ft.

TC LL	20.0	PSF	REF	R8228-	31462
TC DL	10.0	PSF	DATE	11/10/09	
BC DL	10.0	PSF	DRW	HCUSR8228	09314044
BC LL	0.0	PSF	HC-ENG	DF/DF	
TOT.LD.	40.0	PSF	SEQN-	40022	
DUR.FAC.	1.25		FROM	AH	

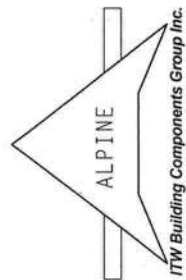


WARNING THUSSES REQUIRE EXTREME CARE IN FABRICATION, SHIPPING, HANDLING, AND BRACING. BECAUSE OF THE BUILDING COMPONENT SAFETY INFORMATION PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND THUSS, COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, MI 48051, FOR SAFETY PRACTICES PRIOR TO PERFORMING THE SECTIONS, UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

**** IMPORTANT *****FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW HCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DETAILATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN COMPLIANCE WITH THE FOLLOWING DRAWINGS AND SPECIFICATIONS WILL BE AT THE INSTALLER'S RISK.

DESIGN LOADS: FLOORING, CEILING, MECHANICAL, ETC. (SEE ARCHITECTURAL DESIGN SPEC. BY ARBP) AND TOL. 17W REG. CORRELATOR PLATES ARE MADE OF 2010/17H6GA, OR #2525/AS ASH #653 GRADE 40/60 U, X-Y-Z=51 GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DIAGRAMS 160A-2.

ANY INSPECTION OF PLATES FOLLOWED BY (1). SHALL BE PER ANNEX A-3 OF TP11-2902 SEC.3., A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT



Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

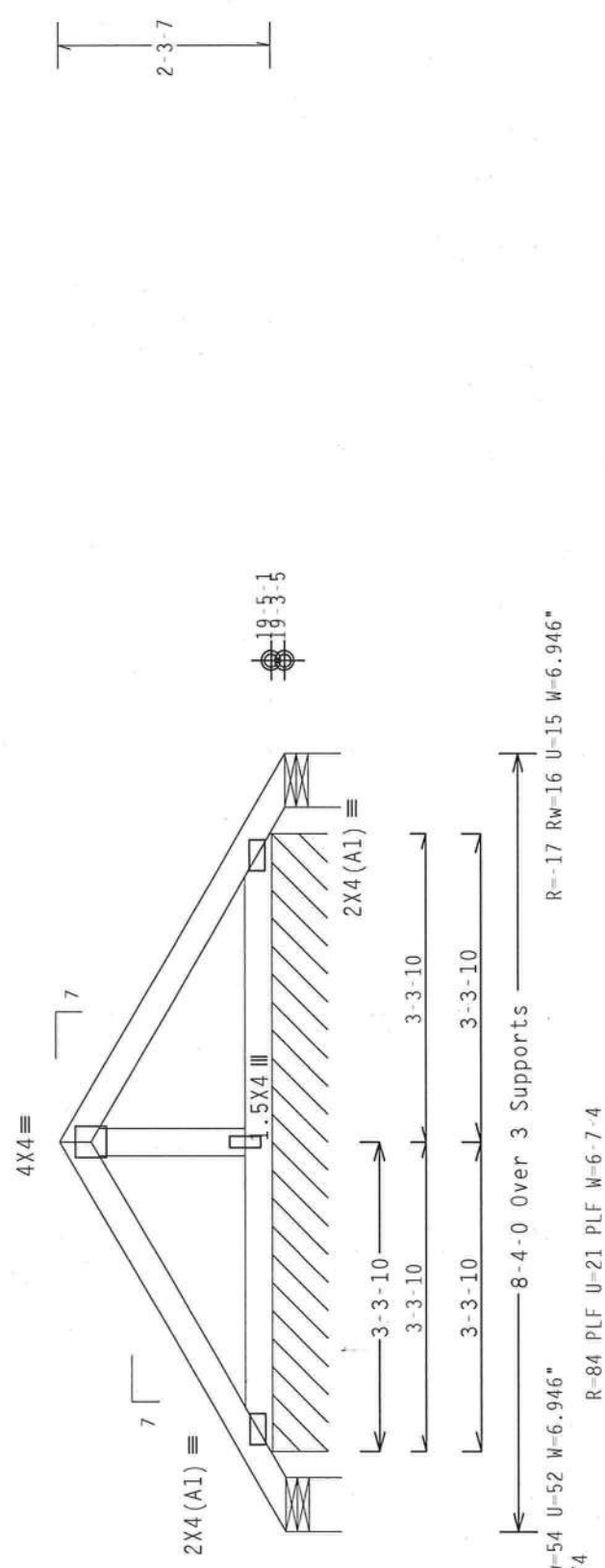
110 mph wind, 20.49 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=2.0 psf. IW=1.00 GCpi(+/-)=0.18

Deflection meets L/240 live and L/180 total load.

Wind reactions based on MWFRS pressures.

MWFRS loads based on trusses located at least 20.49 ft. from roof edge.

Refer to DWG PB1200109 for piggyback details.



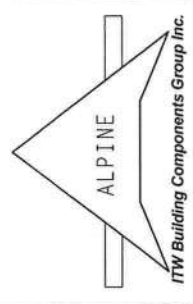
Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10%(0%) / 0(0)

PLT TYP. Wave	QTY: 1	FL/-4/-/-R/-	Scale = .5"/Ft.
	TC LL	20.0 PSF	REF R8228- 31463
	TC DL	10.0 PSF	DATE 11/10/09
	BC DL	10.0 PSF	DRW HCUSR8228 09314049
	BC LL	0.0 PSF	HC-ENG DF/DF
	TOT.LD.	40.0 PSF	SEQN- 40034
	DUR.FAC.	1.25	FROM AH



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND MICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. ITW BCG DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/10/16GA (24-H/55/K) ASTM A653 GRADE 40/60 (4. K/P/55) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE INDICATED, THE POSITION PER DRAWING. ON THIS DRAWING, INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT.



Special loads

-----Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC - From 63 plf at 0.00 to 63 plf at 4.00
TC - From 63 plf at 4.33 to 63 plf at 8.33
BC - From 4 plf at 0.00 to 4 plf at 8.33

110 mph wind, 20.42 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=2.0 psf. lw=1.00 GCpl(+/-)=0.18

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" OC, all BC @ 24" OC.

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load.

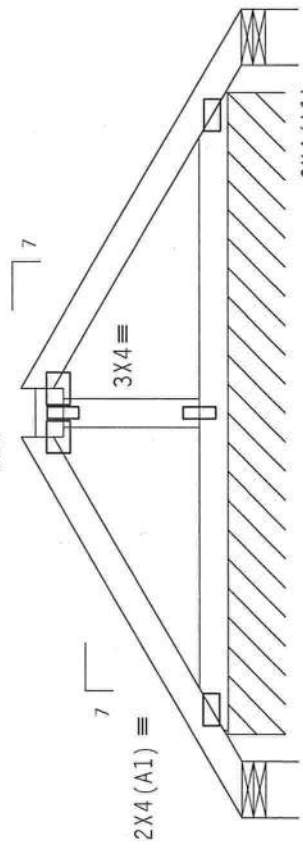
MWFRS loads based on trusses located at least 20.42 ft. from roof edge.

Refer to DWG PB1200109 for piggyback details.

1.5X4 III

3X4 III

2-1-11



R=9 Rw=49 U=50 W=6.946"
RL=69/-69
R=79 PLF U=22 PLF W=6-7-4
R=13 Rw=17 U=18 W=6.946"

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

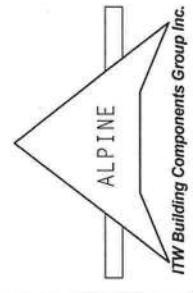
QTY:1 FL/-4/-/-R/- Scale =.5"/Ft.

TC LL	20.0 PSF	REF	R8228- 31464
TC DL	10.0 PSF	DATE	11/10/09
BC DL	10.0 PSF	DRW	HCUSR8228 09314011
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	40041
DUR.FAC.	1.25	FROM	AH



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND MICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, MI 48371) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 2018/106GA (M-37/55/28) ASTM A653 GRADE 40/90 (W, X, Y, Z) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWING. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT.



(9-188--Doug Edgley Kisala -- ** - PB10)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

110 mph wind, 19.83 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, Exp C, wind TC DL=5.0 psf, wind BC DL=2.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Refer to DWG PB1200109 for piggyback details.

Special loads

-----Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC - From 63 plf at 0.00 to 63 plf at 2.00
TC - From 63 plf at 2.00 to 63 plf at 6.33
TC - From 63 plf at 6.33 to 63 plf at 8.33
BC - From 4 plf at 0.00 to 4 plf at 8.33

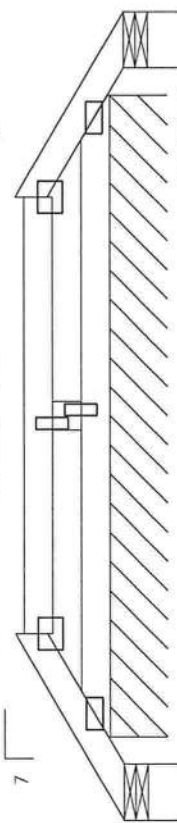
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.

MWFRS loads based on trusses located at least 19.83 ft. from roof edge.

3X4=

3X4= 1.5X4(**) III



0-11-11

R=13 RW=18 U=16 W=6.946"
RL=30/-30 R=75 PLF U=30 PLF W=6-7-4
R=13 U=5 W=6.946"

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%) / 0(0)

PLT TYP. Wave

QTY: 1

FL/-4/-/-/R/-

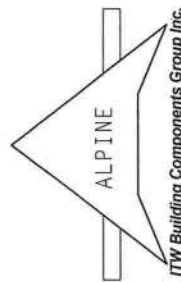
Scale = .5"/Ft.

TC LL	20.0 PSF	REF	R8228- 31465
TC DL	10.0 PSF	DATE	11/10/09
BC DL	10.0 PSF	DRW	HCUSR8228 09314034
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	40051
DUR.FAC.	1.25	FROM	AH



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND MICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF BCS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ITM BCG CORRELATOR PLATES ARE MADE OF 2024-T3 ALUMINUM (4"X4"X1/8") UNLESS OTHERWISE INDICATED. PLATES ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SOLELY FOR THE TRUSS COMPONENT



ITW Building Components Group Inc.

CLB WEB BRACE SUBSTITUTION

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON A TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

NOTES:

THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB BRACING.

ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE. FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE BRACING.

WEB MEMBER SIZE	SPECIFIED CLB BRACING	T OR L-BRACE	ALTERNATIVE BRACING SCAB BRACE
2X3 OR 2X4	1 ROW	2X4	1-2X4
2X3 OR 2X4	2 ROWS	2X6	2-2X4
2X6	1 ROW	2X4	1-2X6
2X6	2 ROWS	2X6	2-2X4(*)
2X8	1 ROW	2X6	1-2X8
2X8	2 ROWS	2X6	2-2X6(*)

T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

(*) CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH FACE OF WEB.

T-BRACING

OR

L-BRACING:

APPLY TO EITHER SIDE OF WEB NARROW FACE.

ATTACH WITH 10d BOX OR GUN

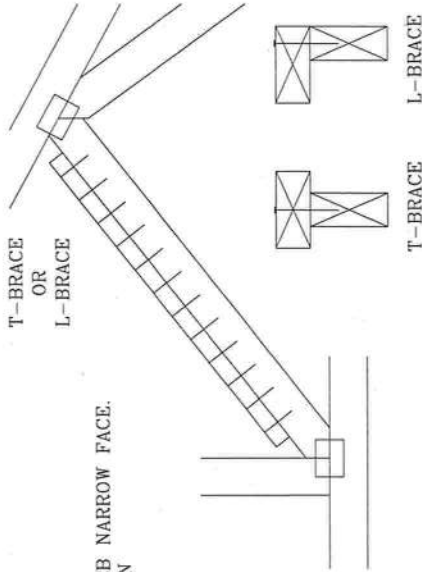
(0.128" x 3" MIN) NAILS.

AT 6" O.C.

BRACE IS A

MINIMUM 80% OF WEB

MEMBER LENGTH



T-BRACE

L-BRACE

SCAB BRACING:

APPLY SCAB(S) TO WIDE FACE OF WEB.

NO MORE THAN (1) SCAB PER FACE.

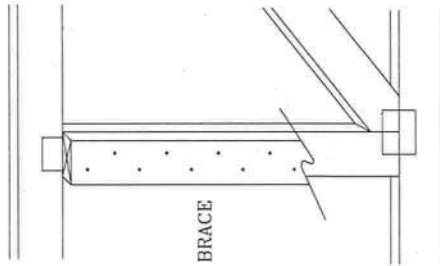
ATTACH WITH 10d BOX OR GUN

(0.128" x 3" MIN) NAILS.

AT 6" O.C.

BRACE IS A MINIMUM

80% OF WEB MEMBER LENGTH



SCAB BRACE



Building Components Group Inc.

Earth City, MO 63045

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET. THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON A TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED. REFER TO AND FOLLOW THE BUILDING COMPONENTS GROUP INC. (ITWBCG) TRUSS BRACING MANUAL (TBM) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. INSTALLERS SHALL PROVIDE TEMPORARY BRACING PER BCSI. UNLESS NOTED OTHERWISE, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING. LOCATIONS SHOWN FOR PERMANENT LATERAL RESTRAINT OF WEBS SHALL HAVE BRACING INSTALLED PER BCSI SECTIONS B3 & B7. SEE THIS JOB'S GENERAL NOTES PAGE FOR MORE INFORMATION.

****IMPORTANT**** FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design, including but not limited to, fabricating, handling, shipping, installing & bracing of trusses. ITWBCG connector plates are made of 2018/10GA (W/H/S/K) ASTM A653 grade 37/40 (K/W/H/S) galv. steel. Apply plates to each face of truss, positioned as shown above and on Joint Details. A seal on this drawing or cover page indicates acceptance and professional engineering responsibility solely for the truss component design shown. The suitability and use of this component for any building is the responsibility of the user. ITWBCG is not responsible for the design of the building. ITWBCG: www.itwbcg.com; TPI: www.tpinet.com; WCA: www.abcdindustry.com; ICC: www.iccsafe.org



TC LL	PSF	REF	CLB SUBST.
TC DL	PSF	DATE	1/1/09
BC DL	PSF	DRWG	BRCILBSUB0109
BC LL	PSF		
TOT. LD.	PSF		
DUR. FAC.			
SPACING			

ASCE 7-05: 110 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C, Kzt = 1.00

GABLE STUD REINFORCEMENT DETAIL

2X4 GABLE VERTICAL SPACING		BRACE		NO BRACES		(1) 1X4 "L" BRACE •		(1) 2X4 "L" BRACE •		(2) 2X4 "L" BRACE ••		(1) 2X6 "L" BRACE •		(2) 2X6 "L" BRACE •		(1) 2X6 "L" BRACE ••		(2) 2X6 "L" BRACE •		(1) 2X6 "L" BRACE ••		(2) 2X6 "L" BRACE •		(1) 2X6 "L" BRACE ••		(2) 2X6 "L" BRACE •		(1) 2X6 "L" BRACE ••		(2) 2X6 "L" BRACE •		(1) 2X6 "L" BRACE ••		(2) 2X6 "L" BRACE •		(1) 2X6 "L" BRACE ••		(2) 2X6 "L" BRACE •		(1) 2X6 "L" BRACE ••		(2) 2X6 "L" BRACE •		(1) 2X6 "L" BRACE ••		(2) 2X6 "L" BRACE •		(1) 2X6 "L" BRACE ••		(2) 2X6 "L" BRACE •		(1) 2X6 "L" BRACE ••		(2) 2X6 "L" BRACE •		(1) 2X6 "L" BRACE ••		(2) 2X6 "L" BRACE •		(1) 2X6 "L" BRACE ••		(2) 2X6 "L" BRACE •		(1) 2X6 "L" BRACE ••		(2) 2X6 "L" BRACE •		(1) 2X6 "L" BRACE ••		(2) 2X6 "L" BRACE •		(1) 2X6 "L" BRACE ••		(2) 2X6 "L" BRACE •		(1) 2X6 "L" BRACE ••		(2) 2X6 "L" BRACE •		(1) 2X6 "L" BRACE ••		(2) 2X6 "L" BRACE •		(1) 2X6 "L" BRACE ••		(2) 2X6 "L" BRACE •		(1) 2X6 "L" BRACE ••		(2) 2X6 "L" BRACE •		(1) 2X6 "L" BRACE ••		(2) 2X6 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BRACING GROUP SPECIES AND GRADES:

GROUP A:

SPRUCE-PINE-FIR	HEM-FIR
#1 / #2	#3
STUD	STUD
STANDARD	STANDARD

DOUGLAS FIR-LARCH	SOUTHERN PINE
#3	#3
STUD	STUD
STANDARD	STANDARD

GROUP B:

HEM-FIR	DOUGLAS FIR-LARCH
#1 & BTR	#1
#1	#2

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 80 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.

* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.

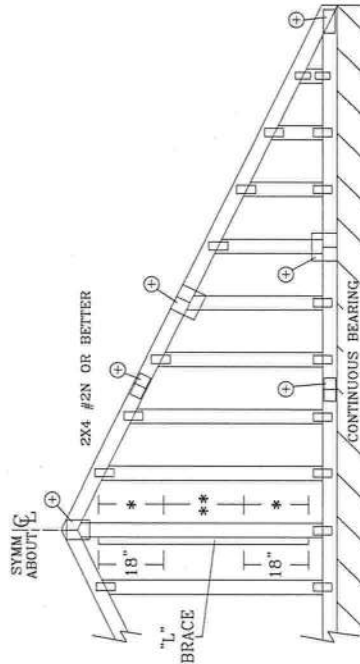
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

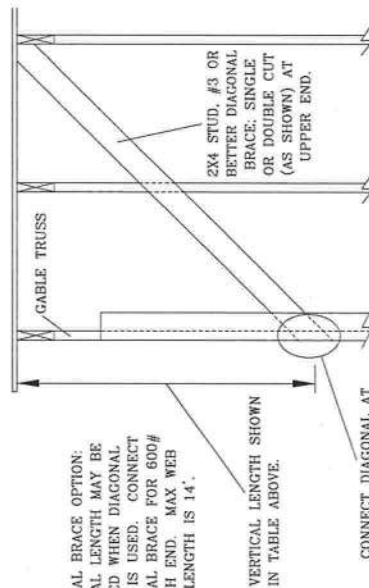
GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0" BUT LESS THAN 11' 6"	2.5X4
GREATER THAN 11' 6"	3X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.



REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.



WARNING: READ AND FOLLOW ALL NOTES ON THIS SHEET. ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design, including but not limited to, fabrication, handling, shipping, installing and bracing. Refer to and follow BCSI (Building Component Safety Information, by TPI and WCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural panels and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B5 & B7. See this job's general notes page for more information.

IMPORTANT: FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design, including but not limited to, fabrication, handling, shipping, installing and bracing of trusses. ITWBCG connector plates are made of 2018/16GA (W/H/S/K) ASTM A653/ASTM 37/40/40 (K/N/LS) galv. steel. Apply plates to each face of truss, positioned as shown above and on Joint Details. A seal on this drawing or cover page indicates acceptance and professional engineering responsibility solely for the time shown. This drawing is the property of ITW Building Components Group Inc. and its use without the written consent of ITW Building Components Group Inc. is prohibited. ITW-BCSI: www.itwbcg.com, TPI: www.tpiind.com, WCA: www.abindustry.com, ICC: www.iccsafe.org



Building Components Group Inc.

Earth City, MO 63045



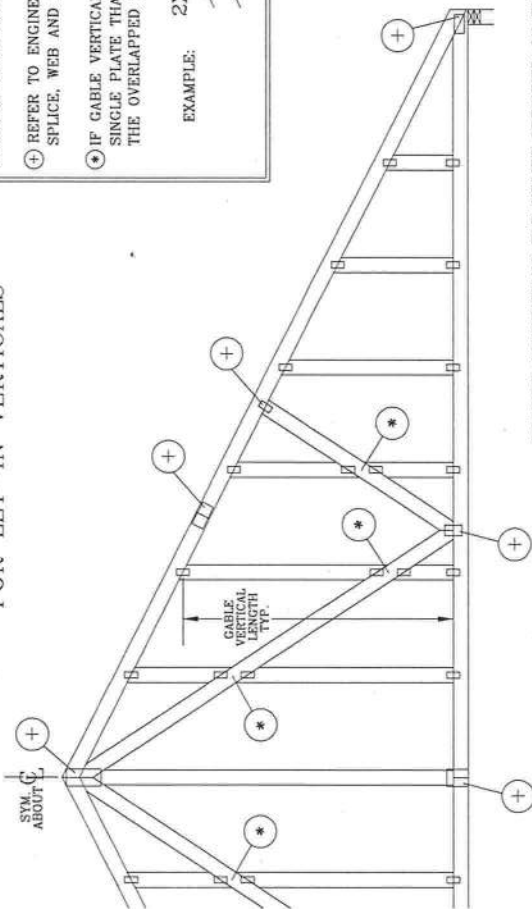
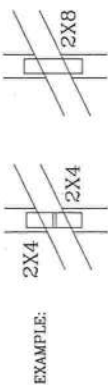
REF ASCE7-05-CAB1015
DATE 1/1/09
DRWG A11015050109

MAX. TOT. LD. 60 PSF
MAX. SPACING 24.0"

GABLE DETAIL FOR LET-IN VERTICALS

GABLE TRUSS PLATE SIZES

- REFER TO APPROPRIATE ITW GABLE DETAIL FOR MINIMUM PLATE SIZES FOR VERTICAL STUDS.
- ⊕ REFER TO ENGINEERED TRUSS DESIGN FOR PEAK, SPLICE, WEB AND HEEL PLATES.
- ⊙ IF CABLE VERTICAL PLATES OVERLAP, USE A SINGLE PLATE THAT COVERS THE TOTAL AREA OF THE OVERLAPPED PLATES TO SPAN THE WEB.



PROVIDE CONNECTIONS FOR UPLIFT SPECIFIED ON THE ENGINEERED TRUSS DESIGN.

ATTACH EACH "T" REINFORCING MEMBER WITH

END DRIVEN NAILS:

- 10d COMMON (0.148"x 3".MIN) NAILS AT 4" O.C. PLUS
- (4) NAILS IN TOP AND BOTTOM CHORD.

TOENAIL NAILS:

- 10d COMMON (0.148"x 3".MIN) TOENAILS AT 4" O.C. PLUS
- (4) TOENAILS IN TOP AND BOTTOM CHORD.

THIS DETAIL TO BE USED WITH THE APPROPRIATE ITW GABLE DETAIL FOR ASCE

WIND LOAD.

ASCE 7-98 GABLE DETAIL DRAWINGS

- A13015980109, A12015980109, A11015980109, A10015980109,
- A13030980109, A12030980109, A11030980109, A10030980109

ASCE 7-02 GABLE DETAIL DRAWINGS

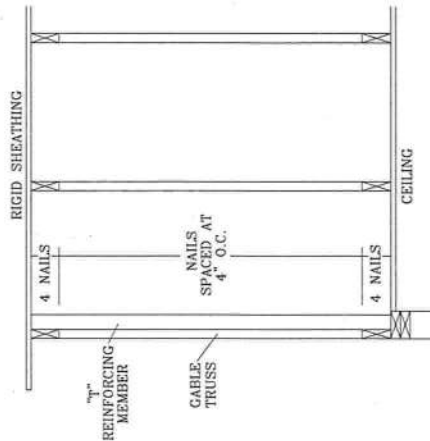
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ASCE 7-05 GABLE DETAIL DRAWINGS

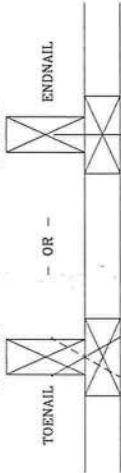
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- A13030050109, A12030050109, A11030050109, A10030050109

SEE APPROPRIATE ITW GABLE DETAIL FOR MAXIMUM

UNREINFORCED GABLE VERTICAL LENGTH.



"T" REINFORCEMENT ATTACHMENT DETAIL



TO CONVERT FROM "L" TO "T" REINFORCING MEMBERS, MULTIPLY "T" INCREASE BY LENGTH (BASED ON APPROPRIATE ITW GABLE DETAIL).

MAXIMUM ALLOWABLE "T" REINFORCED GABLE VERTICAL LENGTH IS 14' FROM TOP TO BOTTOM CHORD.

WEB LENGTH INCREASE W/ "T" BRACE

WIND SPEED AND MRH	"T" REINF. MBR. SIZE	"T" INCREASE
140 MPH	2x4	10 %
15 FT	2x6	50 %
140 MPH	2x4	10 %
30 FT	2x6	50 %
130 MPH	2x4	10 %
15 FT	2x6	50 %
130 MPH	2x4	10 %
30 FT	2x6	50 %
120 MPH	2x4	10 %
15 FT	2x6	50 %
120 MPH	2x4	10 %
30 FT	2x6	40 %
110 MPH	2x4	10 %
15 FT	2x6	40 %
110 MPH	2x4	10 %
30 FT	2x6	50 %
100 MPH	2x4	20 %
15 FT	2x6	30 %
100 MPH	2x4	10 %
30 FT	2x6	40 %
90 MPH	2x4	20 %
15 FT	2x6	20 %
90 MPH	2x4	20 %
30 FT	2x6	30 %

EXAMPLE:

- ASCE WIND SPEED = 100 MPH
- MEAN ROOF HEIGHT = 30 FT, Kz1 = 1.00
- GABLE VERTICAL = 24' O.C. SP #3
- "T" REINFORCING MEMBER SIZE = 2X4
- "T" INCREASE (FROM ABOVE) = 10% = 1.10
- (1) 2X4 "L" BRACE LENGTH = 6' 7"
- MAXIMUM "T" REINFORCED GABLE VERTICAL LENGTH 1.10 x 6' 7" = 7' 3"

WARNING: READ AND FOLLOW ALL NOTES ON THIS SHEET. Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow BCSI (Building Component Safety Information, by TPI and WCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural panels and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections E3 & E7. See this job's general notes page for more information.

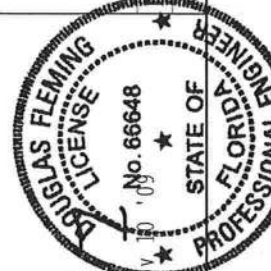
IMPORTANT: FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design, any failure to build the truss in conformance with TPI, or fabricating, handling, shipping, installing & bracing of trusses. ITWBCG connector plates are made of 20/18/16GA (W/H/S/K) ASTM A653 grade 37/40/50 (K/W/H/S) galv. steel. Apply plates to each face of truss, positioned as shown above and on Joint Details. A seal on this drawing or cover page indicates acceptance and professional engineering responsibility solely for the use of this component for the use of this component for the use of this building is the responsibility of the Building Designer per ANSI/TPI 1, Sec. 2. ITW-BGC: www.itwbcg.com; TPI: www.tpinet.com; WCA: www.abindustry.com; ICC: www.iccsafe.org



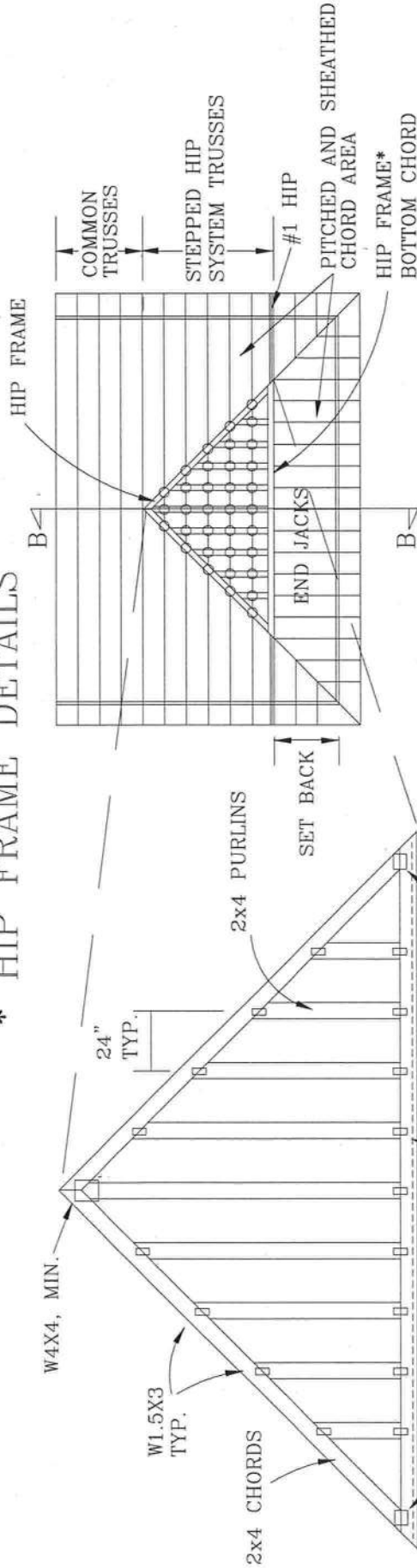
Earth City, MO 63045

REF	LET-IN VERT
DATE	1/1/09
DRWG	GBLETTN0109

TAX TOT. LD. 60 PSF
UR. FAC. ANY
MAX SPACING 24.0"

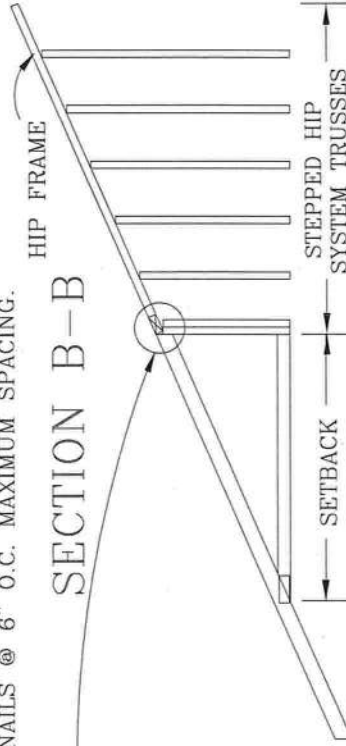


* HIP FRAME DETAILS



o- ATTACH HIP FRAME TO FLAT CHORDS OF STEPPED HIP AT ALL OVERLAPPING POINTS WITH 2-10d (0.148"x3") COMMON NAILS. BOTTOM CHORD OF HIP FRAME TO BE ATTACHED TO #1 HIP WITH 10d COMMON (0.148"x3") NAILS @ 6" O.C. MAXIMUM SPACING.

SECTION B-B



HIP FRAME STOPS AT PLUMB CUT OF JACKS TO MAINTAIN PITCH CONTINUITY.

USE THIS DETAIL FOR ASCE 7-02 OR ASCE 7-05 WIND, 110 MPH, 30' MEAN HEIGHT, ENCLOSED BUILDING, EXP. C, RESIDENTIAL, WIND TC DL=4.2 PSF, Kzt=1.00

* HIP FRAME LUMBER IS SPF, SO. PINE, HF, OR DFL STANDARD, STUD GRADE, OR BETTER.

SEE ENGINEER'S SEALED DESIGN FOR SETBACK, LUMBER, PLATING, LOADING AND DURATION FACTOR REQUIRED.

'R' HIP FRAME CHORDS MAY BE TRIMMED UP TO 2" TO FIT. PURLINS MUST BE INTACT AND PROPERLY ATTACHED.

HIP FRAME - PROVIDED BY TRUSS MANUFACTURER. HIP FRAME IS DESIGNED TO PROVIDE BRACING FOR FLAT TOP CHORDS OF HIP FRAME SYSTEM WHERE INDICATED. STRUCTURAL PANELS MUST BE PROPERLY ATTACHED DIRECTLY TO HIP FRAME PURLINS.

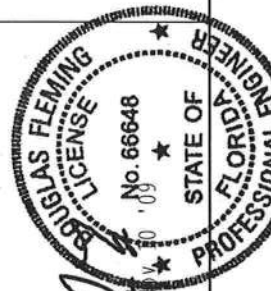
WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET. Trussing, bracing, purlins, and bracing. Refer to and follow the Building Components Group Inc. (ITWBCG) design and BCS (Building Component Safety) information, by TPI and WTA. Unless noted otherwise, top chord shall have properly attached structural panels and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCS sections B3 & B7. See this job's general notes page for more information.

IMPORTANT** FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design. any failure to build the truss in conformance with TPI, or fabricating, handling, shipping, installing & bracing of trusses. ITWBCG connector plates are made of 2018/16GA (W/H/S/K) ASTM A653 grade 37/40 (K/W/H/S) galv. steel. Apply plates to each face of truss, positioned as shown above and on Joint Detail A seal on this drawing or cover page indicates acceptance and professional engineering responsibility solely for the truss component design shown. The suitability and use of this component for any building is the responsibility of the Building Designer per ASCE 7-02, Section 16.2. See also ITWBCG website: www.itwbcg.com, TPI: www.tpi.com, WTA: www.abindustry.com; ICC: www.iccsafe.org



Earth City, MO 63045

REF	HIP FRAME
DATE	1/1/09
DRWG	HIPFRAME0109



120 PIGGYBACK DETAIL

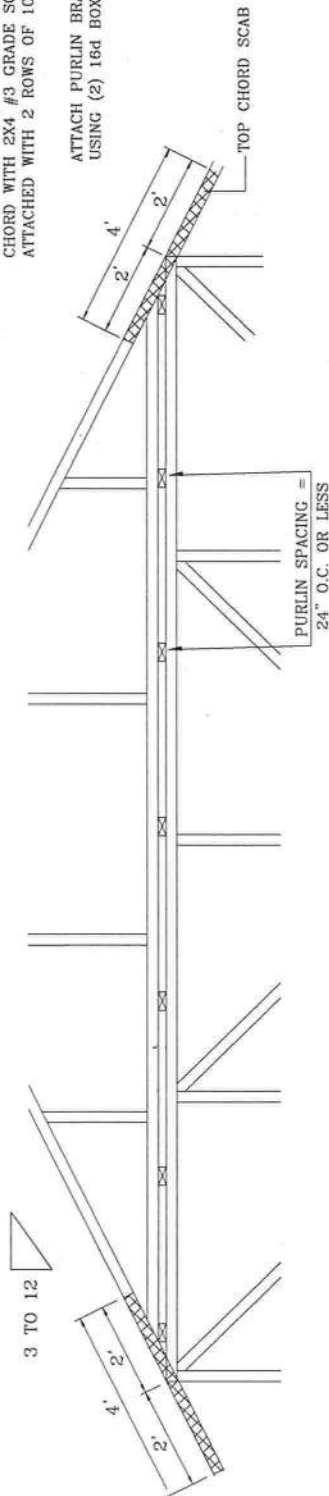
UP TO 120 MPH WIND, 30.00 FT MEAN HGT. ASCE 7-02 OR ASCE 7-05, ENCLOSED BLDG. LOCATED ANYWHERE IN ROOF, CAT II, EXP C, WIND DL= 5.0 PSF KZ1=1.0.

MAXIMUM TRUSS SPACING IS 24" O.C. DETAIL IS NOT APPLICABLE IF CAP SUPPORTS ADDITIONAL LOADS SUCH AS CUPOLA, STEEPLE, CHIMNEY OR DRAG STRUT LOADS.

NOTE: TOP CHORDS OF TRUSSES SUPPORTING PIGGYBACK CAP TRUSSES MUST BE ADEQUATELY BRACED BY SHEATHING OR PURLINS. THE BUILDING ENGINEER OF RECORD SHALL PROVIDE DIAGONAL BRACING OR OTHER SUITABLE ANCHORAGE TO PERMANENTLY RESTRAIN PURLINS.

** REFER TO ENGINEER'S SEALED TRUSS DESIGN DRAWING FOR PIGGYBACK AND BASE TRUSS SPECIFICATIONS.

DETAIL A

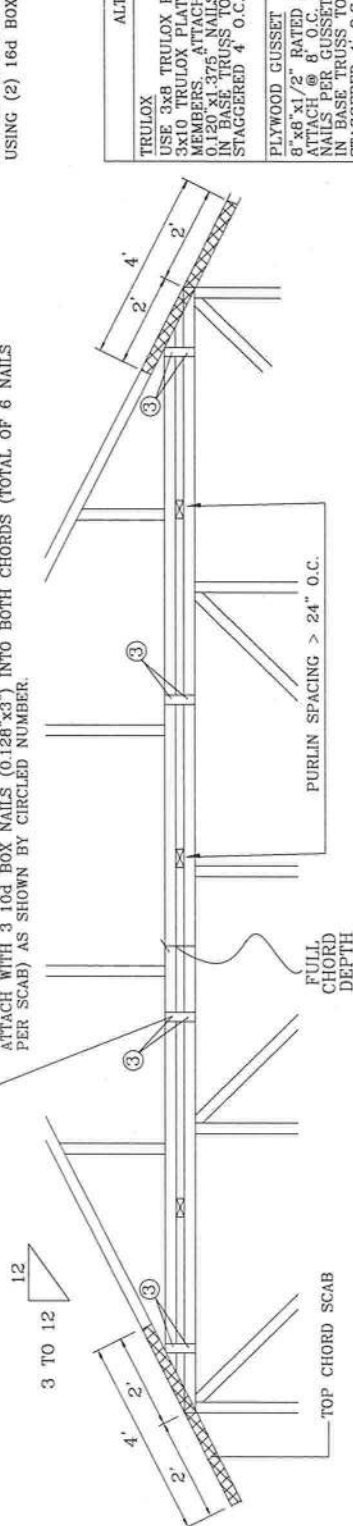


PIGGYBACK CAP TRUSS SLANT NAILED TO ALL TOP CHORD PURLIN BRACING WITH (2) 16d BOX NAILS (0.135"x3.5") AND SECURE TOP CHORD WITH 2X4 #3 GRADE SCAB (1 SIDE ONLY AT EACH END), ATTACHED WITH 2 ROWS OF 10d BOX NAILS (0.128"x3.0") AT 4" O.C.

ATTACH PURLIN BRACING TO THE FLAT TOP CHORD USING (2) 16d BOX NAILS (0.135"x3.5")

DETAIL B

PIGGYBACK CAP TRUSS NAILED TO TOP CHORD OF BASE TRUSS AND SECURED WITH 2x4 SPF#2, FULL CHORD DEPTH SCABS @ 8" O.C. EACH FACE, STAGGERED 4" O.C. ATTACH WITH 3 10d BOX NAILS (0.128"x3") INTO BOTH CHORDS (TOTAL OF 6 NAILS PER SCAB) AS SHOWN BY CIRCLED NUMBER.



PIGGYBACK CAP TRUSS SLANT NAILED TO ALL TOP CHORD PURLIN BRACING WITH (2) 16d BOX NAILS (0.135"x3.5") AND SECURE TOP CHORD WITH 2X4 #3 GRADE SCAB (1 SIDE ONLY AT EACH END), ATTACHED WITH 2 ROWS OF 10d BOX NAILS (0.128"x3.0") AT 4" O.C.

ATTACH PURLIN BRACING TO THE FLAT TOP CHORD USING (2) 16d BOX NAILS (0.135"x3.5")

ALTERNATE ATTACHMENTS

TRULUX
USE 3x8 TRULUX PLATES FOR 2x4 CHORD MEMBER, AND 3x10 TRULUX PLATES FOR 2x6 AND LARGER CHORD MEMBERS. ATTACH TO EACH FACE @ 8" O.C. WITH (4) 10d BOX NAILS INTO CAP TRUSS TOP CHORD. TRULUX PLATES MAY BE STAGGERED 4" O.C. FRONT TO BACK FACES.

PLYWOOD GUSSET
8"x8"x1/2" RATED SHEATHING GUSSETS (EACH FACE). ATTACH @ 8" O.C. WITH (8) 6d COMMON (0.113"x2") NAILS PER GUSSET. (4) IN CAP BOTTOM CHORD AND (4) IN BASE TRUSS TOP CHORD. GUSSETS MAY BE STAGGERED 4" O.C. FRONT TO BACK FACES.

****WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET**
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design, any failure to build the truss in conformance with TPI, or fabricating, handling, shipping, installing & bracing of trusses. ITWBCG connector plates are made of 2018/166A (W.H/S/K) ASTM A653 grade 37/40/80 (K/W/H/S) galv. steel. Apply plates to each face of truss, positioned as shown above and on Joint Details. A seal on this drawing or cover page indicates acceptance and professional engineering responsibility solely for the truss component design shown. The suitability and use of this component for any building is the responsibility of the building designer per ASCE 7-16 Section 1.5. See also ITWBCG website for more information.
ITW-BCG: www.itwbcg.com, TPI: www.tpiusa.com, WTCA: www.abindustry.com; ICC: www.iccsafe.org



Earth City, MO 63045



REF PIGGYBACK
DATE 1/1/09
DRWG PB1200109

SPACING 24.0"

KISAIA



COLUMBIA COUNTY BUILDING DEPARTMENT RESIDENTIAL CHECK LIST REQUIREMENTS

MINIMUM PLAN REQUIREMENTS FOR THE FLORIDA BUILDING CODE RESIDENTIAL 2007 ONE (1) AND TWO (2) FAMILY DWELLINGS

ALL REQUIREMENTS ARE SUBJECT TO CHANGE

ALL BUILDING PLANS MUST INDICATE COMPLIANCE with the Current 2007 FLORIDA BUILDING CODES RESIDENTIAL. ALL PLANS OR DRAWINGS SHALL PROVIDE CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS.

FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEEDS ARE PER FIGURE R301.2(4) of the FLORIDA BUILDING CODES RESIDENTIAL (Florida Wind speed map) SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ----- 100 MPH
ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE -----110 MPH
NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

GENERAL REQUIREMENTS: APPLICANT - PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

Items to Include-
Each Box shall be
Circled as
Applicable

		Yes	No	N/A
1	Two (2) complete sets of plans containing the following:	☒		
2	All drawings must be clear, concise, drawn to scale, details that are not used shall be marked void	☒		
3	Condition space (Sq. Ft.) <u>2583</u>			
	Total (Sq. Ft.) under roof <u>4014</u>			

Designers name and signature shall be on all documents and a licensed architect or engineer, signature and official embossed seal shall be affixed to the plans and documents as per the FLORIDA BUILDING CODES RESIDENTIAL R101.2.1

Site Plan information including:

4	Dimensions of lot or parcel of land	☒		
5	Dimensions of all building set backs	☒		
6	Location of all other structures (include square footage of structures) on parcel, existing or proposed well and septic tank and all utility easements.	☒		
7	Provide a full legal description of property. <u>warranty deed</u>	☒		

Wind-load Engineering Summary, calculations and any details required

GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL		Items to Include- Each Box shall be Circled as Applicable		
8	Plans or specifications must show compliance with FBCR Chapter 3	IIIIII	IIII	IIIIII
		YES	NO	N/A
9	Basic wind speed (3-second gust), miles per hour <i>110 mph</i>	☒		
10	(Wind exposure – if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated)	☒		
11	Wind importance factor and nature of occupancy	☒		
12	The applicable internal pressure coefficient, Components and Cladding	☒		
13	The design wind pressure in terms of psf (kN/m ²), to be used for the design of exterior component, cladding materials not specifically designed by the registered design professional.	☒		

Elevations Drawing including:

14	All side views of the structure	☒		
15	Roof pitch	☒		
16	Overhang dimensions and detail with attic ventilation	☒		
17	Location, size and height above roof of chimneys <i>non-vented</i>			☒
18	Location and size of skylights with Florida Product Approval			☒
18	Number of stories	☒		
20A	Building height from the established grade to the roofs highest peak	☒		

Floor Plan including:

20	Dimensioned area plan showing rooms, attached garage, breeze ways, covered porches, deck, balconies	☒		
21	Raised floor surfaces located more than 30 inches above the floor or grade	☒		
22	All exterior and interior shear walls indicated	☒		
23	Shear wall opening shown (Windows, Doors and Garage doors)	☒		
24	Emergency escape and rescue opening shown in each bedroom (net clear opening shown)	☒		
25	Safety glazing of glass where needed	☒		
26	Fireplaces types (gas appliance) (vented or non-vented) or wood burning with Hearth (see chapter 10 of FBCR)	☒		
27	Stairs with dimensions (width, tread and riser and total run) details of guardrails, Handrails (see FBCR SECTION 311)			☒
28	Identify accessibility of bathroom (see FBCR SECTION 322)	☒		

All materials placed within opening or onto/into exterior walls, soffits or roofs shall have Florida product approval number and mfg. installation information submitted with the plans (see Florida product approval form)

**GENERAL REQUIREMENTS:
APPLICANT - PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL**

Items to Include-
Each Box shall be
Circled as
Applicable

FBCR 403: Foundation Plans

		YES	NO	N/A
29	Location of all load-bearing walls footings indicated as standard, monolithic, dimensions, size and type of reinforcing.	✓		
30	All posts and/or column footing including size and reinforcing	✓		
31	Any special support required by soil analysis such as piling.			✓
32	Assumed load-bearing value of soil _____ Pound Per Square Foot	✓		
33	Location of horizontal and vertical steel, for foundation or walls (include # size and type)	✓		

FBCR 506: CONCRETE SLAB ON GRADE

34	Show Vapor retarder (6mil. Polyethylene with joints lapped 6 inches and sealed)	✓		
35	Show control joints, synthetic fiber reinforcement or welded fire fabric reinforcement and Supports	✓		

FBCR 320: PROTECTION AGAINST TERMITES

36	Indicate on the foundation plan if soil treatment is used for subterranean termite prevention or submit other approved termite protection methods. Protection shall be provided by registered termiticides <i>Treat Soil</i>	✓		
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FBCR 606: Masonry Walls and Stem walls (load bearing & shear Walls)

37	Show all materials making up walls, wall height, and Block size, mortar type	✓		
38	Show all Lintel sizes, type, spans and tie-beam sizes and spacing of reinforcement <i>Wood Frame</i>			✓

Metal frame shear wall and roof systems shall be designed, signed and sealed by Florida Prof. Engineer or Architect

Floor Framing System: First and/or second story

39	Floor truss package shall including layout and details, signed and sealed by Florida Registered Professional Engineer			✓
40	Show conventional floor joist type, size, span, spacing and attachment to load bearing walls, stem walls and/or piers			✓
41	Girder type, size and spacing to load bearing walls, stem wall and/or piers			✓
42	Attachment of joist to girder			✓
43	Wind load requirements where applicable			✓
44	Show required under-floor crawl space			✓
45	Show required amount of ventilation opening for under-floor spaces			✓
46	Show required covering of ventilation opening			✓
47	Show the required access opening to access to under-floor spaces			✓
	Show the sub-floor structural panel sheathing type, thickness and fastener schedule on the edges &			✓

48	intermediate of the areas structural panel sheathing			✓
49	Show Draftstopping, Fire caulking and Fire blocking			✓
50	Show fireproofing requirements for garages attached to living spaces, per FBCR section 309			✓
51	Provide live and dead load rating of floor framing systems (psf).			✓

FBCR CHAPTER 6 WOOD WALL FRAMING CONSTRUCTION

GENERAL REQUIREMENTS: APPLICANT - PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL		Items to Include- Each Box shall be Circled as Applicable		
		YES	NO	N/A

52	Stud type, grade, size, wall height and oc spacing for all load bearing or shear walls	✓		
53	Fastener schedule for structural members per table FBCR 602.3 are to be shown	✓		
54	Show Wood structural panel's sheathing attachment to studs, joist, trusses, rafters and structural members, showing fastener schedule attachment on the edges & intermediate of the areas structural panel sheathing	✓		
55	Show all required connectors with a max uplift rating and required number of connectors and oc spacing for continuous connection of structural walls to foundation and roof trusses or rafter systems	✓		
56	Show sizes, type, span lengths and required number of support jack studs, king studs for shear wall opening and girder or header per FBCR Table 502.5 (1)	✓		
57	Indicate where pressure treated wood will be placed	✓		
58	Show all wall structural panel sheathing, grade, thickness and show fastener schedule for structural panel sheathing edges & intermediate areas	✓		
59	A detail showing gable truss bracing, wall balloon framing details or/ and wall hinge bracing detail	✓		

FBCR :ROOF SYSTEMS:

60	Truss design drawing shall meet section FBCR 802.10 Wood trusses	✓		
61	Include a layout and truss details, signed and sealed by Florida Professional Engineer	✓		
62	Show types of connector's assemblies' and resistance uplift rating for all trusses and rafters	✓		
63	Show gable ends with rake beams showing reinforcement or gable truss and wall bracing details	✓		
64	Provide dead load rating of trusses	✓		

FBCR 802:Conventional Roof Framing Layout

65	Rafter and ridge beams sizes, span, species and spacing	✓		
66	Connectors to wall assemblies' include assemblies' resistance to uplift rating	✓		
67	Valley framing and support details	✓		
68	Provide dead load rating of rafter system	✓		

FBCR Table 602,3(2) & FBCR 803 ROOF SHEATHING

69	Include all materials which will make up the roof decking, identification of structural panel sheathing, grade, thickness	✓		
70	Show fastener Size and schedule for structural panel sheathing on the edges & intermediate areas	✓		

FBCR ROOF ASSEMBLIES FRC Chapter 9

71	Include all materials which will make up the roof assemblies covering	☒		
72	Submit Florida Product Approval numbers for each component of the roof assemblies covering	☒		

FBCR Chapter 11 Energy Efficiency Code for residential building

Residential construction shall comply with this code by using the following compliance methods in the FBCR chapter 11 Residential buildings compliance methods. *Two of the required forms are to be submitted, showing dimensions condition area equal to the total condition living space area*

GENERAL REQUIREMENTS: APPLICANT - PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL		Items to Include- Each Box shall be Circled as Applicable		
		YES	NO	N/A
73	Show the insulation R value for the following areas of the structure	☒		
74	Attic space <i>R-38</i>	☒		
75	Exterior wall cavity <i>R-19</i>	☒		
76	Crawl space <i>Concrete Floor.</i>			☒

HVAC information

77	Submit two copies of a Manual J sizing equipment or equivalent computation study	☒		
78	Exhaust fans locations in bathrooms	☒		
79	Show clothes dryer route and total run of exhaust duct	☒		

Plumbing Fixture layout shown

80	All fixtures waste water lines shall be shown on the foundation plan			☒
81	Show the location of water heater	☒		

Private Potable Water

82	Pump motor horse power <i>1 1/2 H.P.</i>	☒		
83	Reservoir pressure tank gallon capacity <i>36 Gal.</i>	☒		
84	Rating of cycle stop valve if used <i>30 Gal. per min.</i>	☒		

Electrical layout shown including

85	Switches, outlets/receptacles, lighting and all required GFCI outlets identified	☒		
86	Ceiling fans	☒		
87	Smoke detectors & Carbon dioxide detectors	☒		
88	Service panel, sub-panel, location(s) and total ampere ratings	☒		
89	On the electrical plans identify the electrical service overcurrent protection device for the main electrical service. This device shall be installed on the exterior of structures to serve as a disconnecting means for the utility company electrical service. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground. Indicate if the utility company service entrance cable will be of the overhead or underground type.	☒		

90	Appliances and HVAC equipment and disconnects	☒		
91	Arc Fault Circuits (AFCI) in bedrooms	☒		

Disclosure Statement for Owner Builders *If you as the applicant will be acting as an owner/builder under section 489.103(7) of the Florida Statutes, submit the required owner builder disclosure statement form.*

Notice Of Commencement

A notice of commencement form **recorded** in the Columbia County Clerk Office is required to be filed with the building department Before Any Inspections can be preformed.

GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL	Items to Include- Each Box shall be Circled as Applicable
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THE FOLLOWING ITEMS MUST BE SUBMITTED WITH BUILDING PLANS

		YES	NO	N/A
92	Building Permit Application A current Building Permit Application form is to be completed and submitted for all residential projects	☒		
93	Parcel Number The parcel number (Tax ID number) from the Property Appraiser (386) 758-1084 is required. A copy of property deed is also requested	☒		
94	Environmental Health Permit or Sewer Tap Approval A copy of a approved Columbia County Environmental Health (386) 758-1058	☒		
95	City of Lake City A permit showing an approved waste water sewer tap			☒
96	Toilet facilities shall be provided for all construction sites	☒		
97	Town of Fort White (386) 497-2321 If the parcel in the application for building permit is within the Corporate city limits of Fort White an approval land use development letter issued by the Town of Fort is required to be submitted with the application for a building permit.			☒
98	Flood Information: All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting a application to this office. Any project located within a flood zone where the base flood elevation (100 year flood) has been established shall meet the requirements of Section 8.5.2 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.5.3 of the Columbia County Land Development Regulations			☒
99	CERTIFIED FINISHED FLOOR ELEVATIONS will be required on any project where the base flood elevation (100 year flood) has been established			☒
100	A development permit will also be required. Development permit cost is \$50.00			☒
101	Driveway Connection: If the property does not have an existing access to a public road, then an application for a culvert permit (\$25.00) must be made. If the applicant feels that a culvert is not needed, they may apply for a culvert waiver (\$50.00). All culvert waivers are sent to the Columbia County Public Works Department for approval or denial.	☒		
102	911 Address: If the project is located in an area where a 911 address has not been issued, then application for a 911 address must be applied for and received through the Columbia County Emergency Management Office of 911 Addressing Department (386) 758-1125	☒		

Section R101.2.1 of the Florida Building Code Residential:

The provisions of Chapter 1, Florida Building Code, Building shall govern the administration and enforcement of the Florida Building Code, Residential.

Section 105 of the Florida Building Code defines the:

Time limitation of application.

An application for a permit for any proposed work shall be deemed to have been abandoned 180 days after the date of filing, unless such application has been pursued in good faith or a permit has been issued; except that the building official is authorized to grant one or more extensions of time for additional periods not exceeding 90 days each. The extension shall be requested in writing and justifiable cause demonstrated.

Single-family residential dwelling.

Section 105.3.4 A building permit for a single-family residential dwelling must be issued within 30 working days of application therefor unless unusual circumstances require a longer time for processing the application or unless the permit application fails to satisfy the Florida Building Code or the enforcing agency's laws and ordinances.

Permit intent.

Section 105.4.1: A permit issued shall be constructed to be a license to proceed with the work and not as an authority to violate, cancel, alter or set aside any of the provisions of the technical codes, nor shall issuance of a permit prevent the building official from thereafter requiring a correction of errors in plans, construction or violations of this code. Every permit issued shall become invalid unless the work authorized by such permit is commenced within six months after its issuance, or if the work authorized by such permit is suspended or abandoned for a period of six months after the time the work is commenced.

If work has commenced.

Section 105.4.1.1: If work has commenced and the permit is revoked, becomes null and void, or expires because of lack of progress or abandonment, a new permit covering the proposed construction shall be obtained before proceeding with the work.

New Permit.

Section 105.4.1.2: If a new permit is not obtained within 180 days from the date the initial permit became null and void, the building official is authorized to require that any work which has been commenced or completed be removed from the building site. Alternately, a new permit may be issued on application, providing the work in place and required to complete the structure meets all applicable regulations in effect at the time the initial permit became null and void and any regulations which may have become effective between the date of expiration and the date of issuance of the new permit.

Work Shall Be:

Section 105.4.1.3: Work shall be considered to be in active progress when the permit has received an approved inspection within 180 days. This provision shall not be applicable in case of civil commotion or strike or when the building work is halted due directly to judicial injunction, order or similar process.

The Fee:

Section 105.4.1.4: The fee for renewal reissuance and extension of a permit shall be set forth by the administrative authority.

When the submitted application is approved for permitting the applic will be notified by phone as to the date and time a building permit wil prepared and issued by the Columbia County Building & Zoning Department

PRODUCT APPROVAL SPECIFICATION SHEET

Location: _____

Project Name: LOUIS + ELONA KUALA

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and the product approval number(s) on the building components listed below if they will be utilized on the construction project for which you are **applying for a building permit on or after April 1, 2004**. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. More information about statewide product approval can be obtained at www.floridabuilding.org

Category/Subcategory	Manufacturer	Product Description	Approval Number(s)
A. EXTERIOR DOORS			
1. Swinging			
2. Sliding			
3. Sectional			
4. Roll up			
5. Automatic			
6. Other			
B. WINDOWS			
1. Single hung		<u>Atrium Series 150</u>	<u>FL6208.4</u>
2. Horizontal Slider			
3. Casement			
4. Double Hung			
5. Fixed			
6. Awning			
7. Pass-through			
8. Projected			
9. Mullion			
10. Wind Breaker			
11. Dual Action			
12. Other <u>Fixed Octagon</u>			<u>FL7834.1</u>
C. PANEL WALL			
1. Siding			
2. Soffits			
3. EIFS			
4. Storefronts			
5. Curtain walls			
6. Wall louver			
7. Glass block		<u>Copies turned in</u>	
8. Membrane		<u>with plans</u>	
9. Greenhouse			
10. Other			
D. ROOFING PRODUCTS			
1. Asphalt Shingles			
2. Underlayments			
3. Roofing Fasteners			
4. Non-structural Metal Rf			
5. Built-Up Roofing			
6. Modified Bitumen			
7. Single Ply Roofing Sys			
8. Roofing Tiles			
9. Roofing Insulation			
10. Waterproofing			
11. Wood shingles /shakes			
12. Roofing Slate			

Louis + Elona KISALA

Category/Subcategory (cont.)	Manufacturer	Product Description	Approval Number
13. Liquid Applied Roof Sys			
14. Cements-Adhesives – Coatings			
15. Roof Tile Adhesive			
16. Spray Applied Polyurethane Roof			
17. Other			
E. SHUTTERS			
1. Accordion			
2. Bahama			
3. Storm Panels			
4. Colonial			
5. Roll-up			
6. Equipment			
7. Others			
F. SKYLIGHTS			
1. Skylight			
2. Other			
G. STRUCTURAL COMPONENTS			
1. Wood connector/anchor			
2. Truss plates			
3. Engineered lumber		Copies turned in with plans	
4. Railing			
5. Coolers-freezers			
6. Concrete Admixtures			
7. Material			
8. Insulation Forms			
9. Plastics			
10. Deck-Roof			
11. Wall			
12. Sheds			
13. Other			
H. NEW EXTERIOR ENVELOPE PRODUCTS			
1.			
2.			

The products listed below did not demonstrate product approval at plan review. I understand that at the time of inspection of these products, the following information must be available to the inspector on the jobsite; 1) copy of the product approval, 2) the performance characteristics which the product was tested and certified to comply with, 3) copy of the applicable manufacturers installation requirements.

I understand these products may have to be removed if approval cannot be demonstrated during inspection.

Kimmy Edgley
Contractor or Contractor's Authorized Agent Signature

Kimmy Edgley 8-12-09
Print Name Date

Permit # (FOR STAFF USE ONLY)

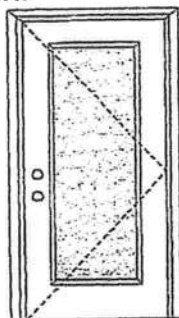
X

Glazed Inswing Unit

COP-WL-JH4141-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Note:
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".

Single Door
Maximum unit size = 3'0" x 6'8"

Design Pressure
+40.5/-40.5
Limited water unless special threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is **REQUIRED**.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.



Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.itswh.com), the Masonite website (www.masonite.com) or the Masonite technical center.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed - see MAD-WL-MA0001-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed - see MID-WL-MA0001-02.

APPROVED DOOR STYLES:

1/4 GLASS:



100 Series



133, 135 Series



136 Series



680 Series



822 Series

1/2 GLASS:



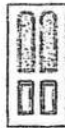
105 Series*



106, 160 Series*



129 Series*



200 Series*



12 R/L, 23 R/L, 24 R/L Series*



107 Series*



108 Series



304 Series

* This glass kit may also be used in the following door styles: 5-panel; 5-panel with scroll; Eyebrow 5-panel; Eyebrow 5-panel with scroll.

XX

Opaque Inswing Unit

COP-WL-JH4102-02

WOOD-EDGE STEEL DOORS

CERTIFIED TEST REPORTS:

NCTL 210-1905-7, 8, 9, 10, 11, 12; NCTL 210-1861-4, 5, 6, 10, 11, 12;
NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA201, PA202 and PA203.

Evaluation report NCTL-210-2794-1

Door panels constructed from 28-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH
MIAMI-DADE BCCO
PA201, PA202 & PA203

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. - License Number 56533

Woodsbury
W

Test Data Review Certificate #302647A
and COP/First Report Validation Matrix
#302647A-001 provide additional
information - available from the ITSMH
website (www.itsmh.com), the
Masonite website (www.masonite.com)
or the Masonite technical center.

Johnson
EntrySystems

June 17, 2002

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development subject to change without notice.

PREMIER
Premium Quality Doors



Exclusively from

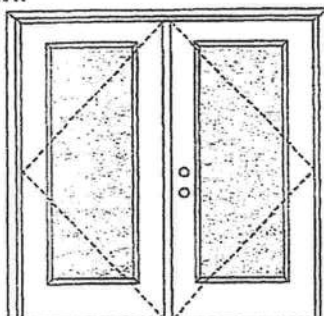
Masonite
Masonite International Corporation

XX
Glazed Inswing Unit

COP-WL-JH4142-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Test Data Review Certificate #3026447A
and COP/Test Report Validation Matrix
#3026447A-001 provides additional
information - available from the ITS/WH
website (www.itswh.com), the
Masonite website (www.masonite.com)
or the Masonite technical center.

Note:
Units of other sizes are covered by this
report as long as the panels used do not
exceed 3'0" x 6'8".

Double Door
Maximum unit size = 6'0" x 6'8"

Design Pressure
+40.5/-40.5
Limited water unless special threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is **REQUIRED**.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0002-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0002-02.

APPROVED DOOR STYLES:

1/4 GLASS:



100 Series



133, 135 Series



136 Series



680 Series



822 Series

1/2 GLASS:



105 Series*



106, 160 Series*



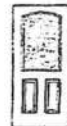
129 Series*



200 Series*



12 R/L, 23 R/L, 24 R/L
Series*



107 Series*



108 Series



304 Series

*This glass kit may also be used in the following door styles: 5-panel; 5-panel with scroll; Eyebrow 5-panel; Eyebrow 5-panel with scroll.

WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES:

3/4 GLASS:



404 Series



410 Series



450 Series

FULL GLASS:



109 Series



114, 120, 122 Series



152 Series



149 Series



300 Series

CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9, 10, 11, 12; NCTL 210-1861-4, 5, 6, 10, 11, 12; NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Evaluation report NCTL-210-2794-1

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. - License Number 56533



Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.itswh.com), the Masonite website (www.masonite.com) or the Masonite technical center.

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June 17, 2002

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Exclusively from

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Masonite International Corporation

OXO
Glazed Inswing Unit

COP-WL-JH4144-02

WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES:

3/4 GLASS:



404 Series



410 Series



450 Series

FULL GLASS:



109 Series



114, 120, 122
Series



152 Series



149 Series



300 Series

APPROVED SIDELITE STYLES:



680 Series



129 Series



200 Series



12R, 12L, 23R,
23L, 24R, 24L
Series



450 Series



152 Series



149 Series



109 Series



120, 122 Series



300 Series

CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9, 10, 11, 12; NCTL 210-1861-4, 5, 6, 10, 11, 12; NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Evaluation report NCTL-210-2794-1

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab and sidelite panels glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. - License Number 56533



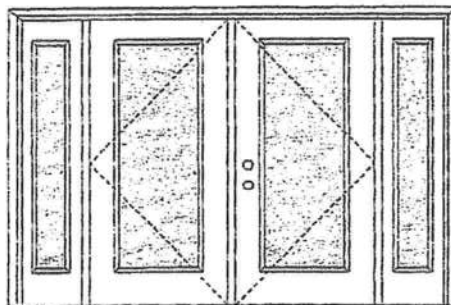
Test Data Review Certificate #3025447A and COP/Test Report Validation Matrix #3025447A-001 provides additional information - available from the ITS/WH website (www.itswh.com), the Masonite website (www.masonite.com) or the Masonite technical center.

OXXO
Glazed Inswing Unit

COP-WL-JH4145-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Warnock Hersey



Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.itswh.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

Double Door with 2 Sidelites
Minimum unit size = 12'0" x 5'8"

Design Pressure
+40.5/-40.5

Limited water unless special threshold design is used.

Large Missile Impact Resistance

Hurricane protective system (shutters) is **REQUIRED**.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed - see MAD-WL-MA0005-02 or MAD-WL-MA0008-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed - see MID-WL-MA0005-02.

APPROVED DOOR STYLES:

1/4 GLASS:



100 Series



133, 135 Series



136 Series



680 Series



822 Series

1/2 GLASS:



105 Series*



106, 160 Series*



129 Series*



200 Series*



12 R/L, 23 R/L, 24 R/L
Series*



107 Series*



108 Series



304 Series

*This glass kit may also be used in the following door styles: 5-panel; 5-panel with scroll; Eyebrow 5-panel; Eyebrow 5-panel with scroll.

Johnson™

WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES:

3/4 GLASS:



404 Series



410 Series



450 Series

FULL GLASS:



109 Series



114, 120, 122 Series



152 Series



149 Series



300 Series

CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9, 10, 11, 12; NCTL 210-1861-4, 5, 6, 10, 11, 12; NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Evaluation report NCTL-210-2794-1

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

State of Florida, Professional Engineer
Kurt Balthazor, P.E. - License Number 56533

Warnock Hersey



Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITSAWH website (www.itsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

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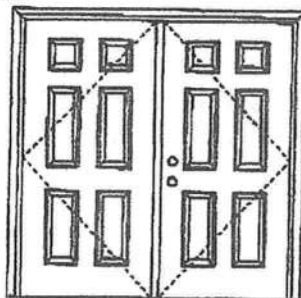
Masonite International Corporation

XX
Opaque Inswing Unit

COP-WL-JH4102-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Test Data Review Certificate #3025447A and COP/TEST Report Validation Matrix #3025447A-001 provides additional information - available from the ITG/AM website (www.itg-am.com), the Masonite website (www.masonite.com) or the Masonite regional center.

Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

Double Door
Maximum unit size = 6'0" x 6'8"

Design Pressure
+45.0/-45.0

(limited water unless special threshold design is used)

Large Missile Impact Resistance

Hurricane protective system (shutters) is NOT REQUIRED.

Actual design pressure and impact resistance requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed - see MAD-WL-MA0002-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed - see MID-WL-MA0002-02.

APPROVED DOOR STYLES:



Flush



Arch Top 3-panel



3-panel



6-panel



New England 4-panel



eyebrow 4-panel



8-panel



9-panel



15-panel



5-panel



5-panel with scroll



eyebrow 5-panel



eyebrow 5-panel with scroll

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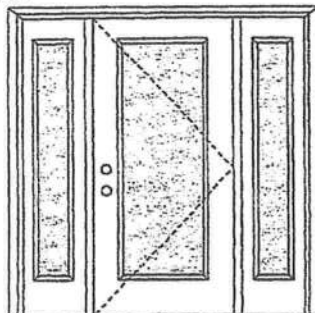
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Masonite
Masonite International Corporation

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Single Door with 2 Sidelites
Maximum unit size = 9'0" x 6'8"

Design Pressure
+40.5/-40.5

Limited water unless special threshold design is used.

Large Missile Impact Resistance

Hurricane protective system (shutters) is **REQUIRED**.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.



Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/NWH website (www.etsenro.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Note:

Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed - see MAD-WL-MA0004-02 or MAD-WL-MA0007-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed - see MID-WL-MA0004-02.

APPROVED DOOR STYLES:

1/4 GLASS:



100 Series



133, 135 Series



136 Series



680 Series



822 Series

1/2 GLASS:



105 Series*



106, 160 Series*



129 Series*



200 Series*



12 R/L, 23 R/L, 24 R/L Series*



107 Series*



108 Series



304 Series

*This glass kit may also be used in the following door styles: 5-panel; 5-panel with scroll; Eyebrow 5-panel; Eyebrow 5-panel with scroll.

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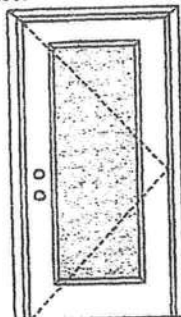
X

Glazed Inswing Unit

COP-WL-JH4141-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Note:
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".



Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.etsamko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Single Door
Maximum unit size = 3'0" x 6'8"

Design Pressure
+40.5/-40.5
Limited water unless special threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the action required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed - see MAD-WL-MA0001-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed - see MID-WL-MA0001-02.

APPROVED DOOR STYLES:

1/4 GLASS:



100 Series



133, 135 Series



135 Series



680 Series

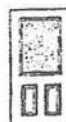


822 Series

1/2 GLASS:



105 Series*



106, 160 Series*



129 Series*



200 Series*



12 R/L, 23 R/L, 24 R/L Series*



107 Series*



108 Series



304 Series

XX

Opaque Inswing Unit

COP-WL-JH4102-02

WOOD-EDGE STEEL DOORS

CERTIFIED TEST REPORTS:

NCTL 210-1905-7, 8, 9, 10, 11, 12; NCTL 210-1861-4, 5, 6, 10, 11, 12;
NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA201, PA202 and PA203.

Evaluation report NCTL-210-2794-1

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH
MIAMI-DADE BCCO
PA201, PA202 & PA203

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthaz

State of Florida, Professional Engineer
Kurt Balthazor, P.E. - License Number 56533



Test Data Review Certificate #0020447A
and COP/Ins Report Validation Matrix
#0020447A-001 provides additional
information - available from the ITSMH
website (www.gpssm.com), the
Masonite website (www.masonite.com)
or the Masonite technical center.

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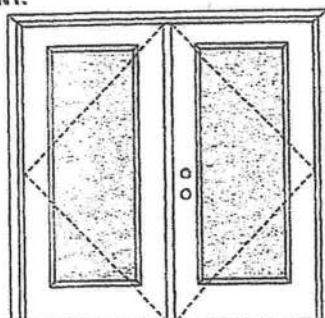
XX

Glazed Inswing Unit

COP-WL-JH4142-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Double Door
Maximum unit size = 6'0" x 6'8"

Design Pressure
+40.5/-40.5

Limited water unless special threshold design is used.

Large Missile Impact Resistance

Hurricane protective system (shutters) is **REQUIRED**.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.



Test Data Review Certificate #3026447A
and COP/Test Report Validation Matrix
#3026447A-001 provides additional
information - available from the ITSAWH
website (www.itsawh.com), the
Masonite website (www.masonite.com)
or the Masonite technical center.

Note:
Units of other sizes are covered by this
report as long as the panels used do not
exceed 3'0" x 6'8".

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed - see MAD-WL-MA0002-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed - see MID-WL-MA0002-02.

APPROVED DOOR STYLES:

1/4 GLASS:



100 Series



133, 135 Series



136 Series



680 Series



822 Series

1/2 GLASS:



105 Series*



106, 160 Series*



129 Series*



200 Series*



12 R/L, 23 R/L, 24 R/L
Series*



107 Series*



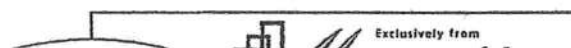
108 Series



304 Series

*This glass kit may also be used in the following door styles: 5-panel; 5-panel with scroll; Eyebrow 5-panel; Eyebrow 5-panel with scroll.

Johnson™



WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES:

3/4 GLASS:



404 Series



410 Series



450 Series

FULL GLASS:



109 Series



114, 120, 122 Series



152 Series



149 Series



300 Series

CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9, 10, 11, 12; NCTL 210-1861-4, 5, 6, 10, 11, 12; NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Evaluation report NCTL-210-2794-1

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

State of Florida, Professional Engineer
Kurt Balthazor, P.E. - License Number 56533



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OXO
Glazed Inswing Unit

COP-WL-JH4144-02

WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES: 3/4 GLASS:



404 Series



410 Series



450 Series

FULL GLASS:



109 Series



114, 120, 122
Series



152 Series



149 Series



300 Series

APPROVED SIDELITE STYLES:



680 Series



129 Series



200 Series



12R, 12L, 23R,
23L, 24R, 24L
Series



450 Series



152 Series



149 Series



109 Series



120, 122 Series



300 Series

CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9, 10, 11, 12; NCTL 210-1861-4, 5, 6, 10, 11, 12; NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Evaluation report NCTL-210-2794-1

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab and sidelite panels glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthaz

State of Florida, Professional Engineer
Kurt Balthazor, P.E. - License Number 56533



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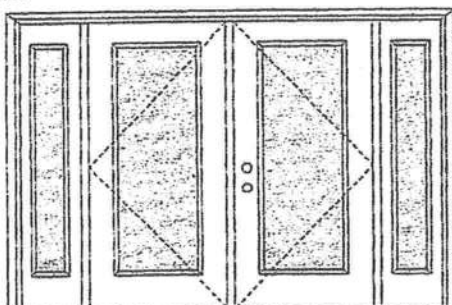
Exclusively from

OXO
Glazed Inswing Unit

COP-WL-JH4145-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Warnock Hersey



Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.itswh.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

Double Door with 2 Sidelites
Maximum unit size = 12'0" x 6'8"

Design Pressure
+40.5/-40.5
Limited water unless special threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is **REQUIRED**.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed - see MAD-WL-MA0005-02 or MAD-WL-MA0008-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed - see MID-WL-MA0005-02.

APPROVED DOOR STYLES:

1/4 GLASS:



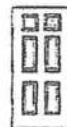
100 Series



133, 135 Series



136 Series



680 Series



822 Series

1/2 GLASS:



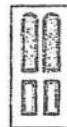
105 Series*



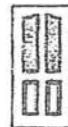
105, 160 Series*



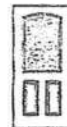
129 Series*



200 Series*



12 R/L, 23 R/L, 24 R/L
Series*



107 Series*



108 Series



304 Series

*This glass kit may also be used in the following door styles: 5-panel; 5-panel with scroll; Eyebrow 5-panel; Eyebrow 5-panel with scroll.

WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES:

3/4 GLASS:



404 Series



410 Series



450 Series

FULL GLASS:



109 Series



114, 120, 122 Series



152 Series



149 Series



300 Series

CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9, 10, 11, 12; NCTL 210-1861-4, 5, 6, 10, 11, 12; NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Evaluation report NCTL-210-2794-1

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

State of Florida, Professional Engineer
Kurt Balthazor, P.E. - License Number 56533

Warnock Hersey



Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITSAWH website (www.eltcmko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Johnson™
EntrySystems

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

PREMDOR Collection
Premium Quality Doors



Exclusively from

Masonite®
Masonite International Corporation

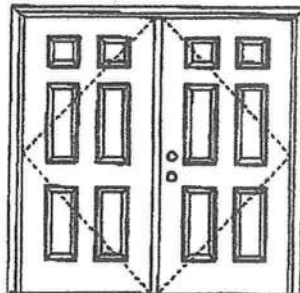
XX

Opaque Inswing Unit

COP-WL-JH4102-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Test Data Review Certificate #3026447A
and COP/Last Report Validation Matrix
#3026447A-001 provides additional
information - available from the ITG/WH
website (www.itg-wh.com), the
Masonite website (www.masonite.com)
or the Masonite technical center.

Note:

Units of other sizes are covered by this
report as long as the panels used do not
exceed 3'0" x 6'8".

Double Door

Maximum unit size = 6'6" x 6'8"

Design Pressure

+45.0/-45.0

Emitted water unless special threshold design is used.

Large Missile Impact Resistance

Hurricane protective system (shutters) is NOT REQUIRED.

Actual design pressure and impact resistance requirements for a specific building design and geographic location is determined by ASCE 7-national,
state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed - see MAD-WL-MA0002-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed - see MID-WL-MA0002-02.

APPROVED DOOR STYLES:



Flush



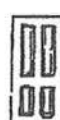
Arch Top 3-panel



3-panel



6-panel



New England 4-panel



Eyebrow 4-panel



8-panel



9-panel



15-panel



5-panel



5-panel with scroll



Eyebrow 5-panel



Eyebrow 5-panel with scroll

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are subject to change without notice.



Exclusively from

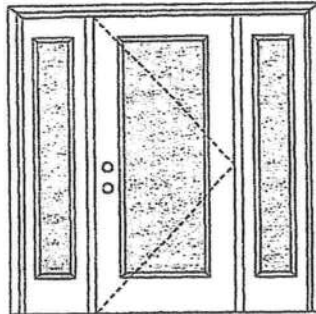
Masonite
Masonite International Corporation

OXO
Glazed Inswing Unit

COP-WL-JH4144-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Single Door with 2 Sidelites
Maximum unit size = 9'0" x 6'8"

Design Pressure
+40.5/-40.5

Limited water unless special threshold design is used.

Large Missile Impact Resistance

Hurricane protective system (shutters) is **REQUIRED**.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.



Test Data Review Certificate #3026447A
and COP/Test Report Validation Matrix
#3026447A-001 provides additional
information - available from the ITS/WH
website (www.itswh.com), the
Masonite website (www.masonite.com)
or the Masonite technical center.

Note:

Units of other sizes are covered by this
report as long as the panels used do not
exceed 3'0" x 6'8".

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed - see MAD-WL-MA0004-02 or
MAD-WL-MA0007-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed - see MID-WL-MA0004-02.

APPROVED DOOR STYLES:

1/4 GLASS:



100 Series



133, 135 Series



136 Series



680 Series



822 Series

1/2 GLASS:



105 Series*



106, 160 Series*



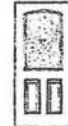
129 Series*



200 Series*



12 R/L, 23 R/L, 24 R/L
Series*



107 Series*



108 Series



304 Series

*This glass kit may also be used in the following door styles: 5-panel; 5-panel with scroll; Eyebrow 5-panel; Eyebrow 5-panel with scroll.

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detail subject to change without notice.

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Premium Quality Doors



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Masonite International Corporation



Zone 3 Supplemental Instructions

Pan Doors: Raised Panel

18'-0" wide

Design pressure: 18.7 pos / 20.8 neg

Test pressure: 28.1 pos / 31.2 neg

CAUTION

Higher wind pressures and larger doors require additional reinforcement.

Premature failure of door system may result from improper application.

Use these instructions only for the wind pressures and door sizes as listed above.

WARNING

These supplemental instructions do not contain basic door installation steps and related safety information.

Failure to follow basic installation steps and related safety information may result in injury or death.

Door installers must follow a primary instruction manual for basic door installation steps and related safety information.

Garage door reinforcement details include:

- Top fixture type and attachment.
- Strut attachment.
- Flag bracket attachment to the wall and track system.
- Track bracket quantity and placement.
- End hinge type and attachment.
- Strut type and placement.

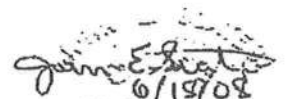
A locking system must be installed if the door is not electrically operated.

Stop molding is required. A minimum 2-1/2" long nail or screw must be used on an 8" spacing.

The correct selection of door and framing materials is the responsibility of the building owner/designer following local building code directives. Use of a reinforced garage door does not constitute automatic compliance with any building code. Local building code officials determine compliance criteria.

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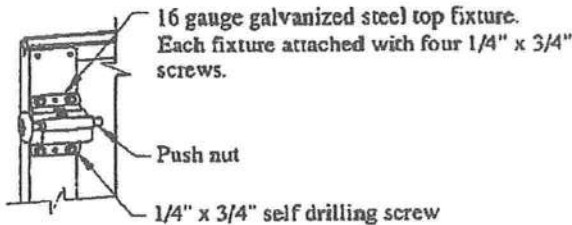
Z3-18045-pan-051608


John E. Segley, P.E.
1411 LeMay Street #205
Carrollton, Texas 75007
Florida P.E. # 51737
This document includes 4 pages.
Professional Engineer's seal
provided only for verification of
windload construction details

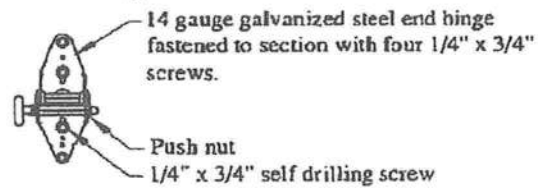
FL 10474

Top Fixture Detail

(Strut, if applicable, not shown for clarity.)

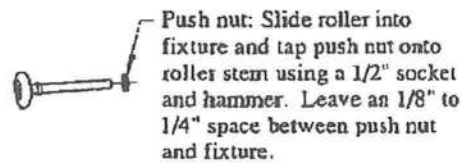


End Hinge Detail

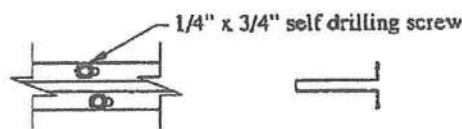


Push Nut Detail (use on all rollers)

use 3/8" I. D. on bottom fixture roller stem
use 7/16" I. D. on end hinge and top fixture roller stems

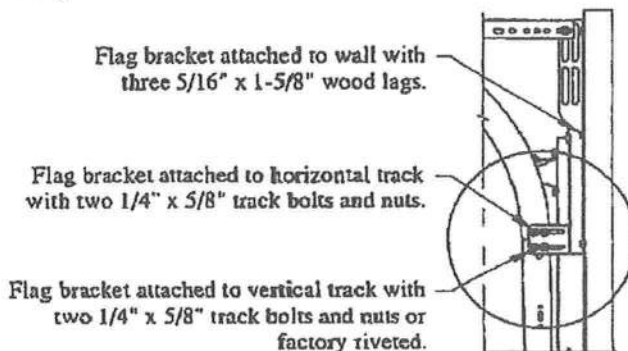


Strut Attachment Detail

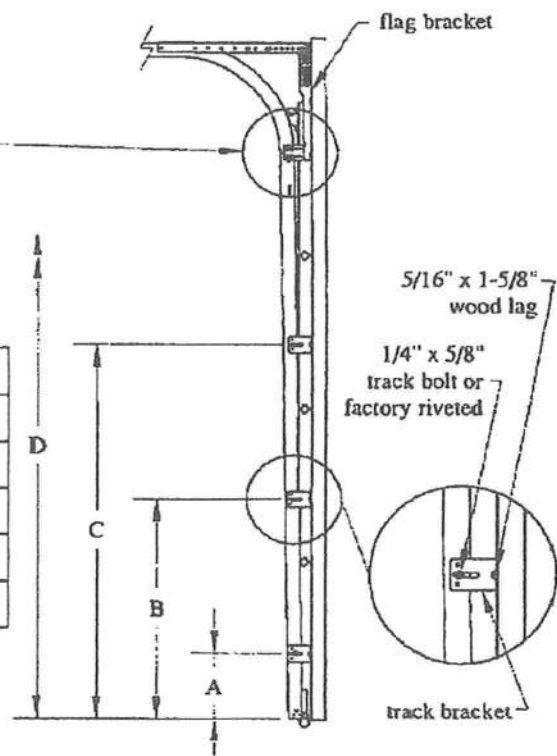
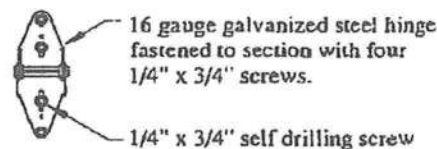


17 gauge 50ksi galvanized steel 3" strut attached with two 1/4" x 3/4" screws to every stile.

Flag Bracket Detail



Intermediate Hinge Detail



Track Bracket Locations

Manufacturing tolerances and actual field conditions may result in variances of +/- 1"

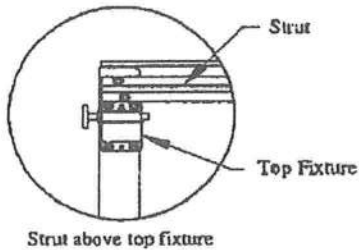
	door height / four sections			door height / five sections					
	6'-6"	6'-9"	7'-0"	7'-5"	7'-9"	8'-0"	8'-3"	8'-6"	8'-9"
D	n/a	n/a	n/a	72"	69"	72"	81"	84"	87"
C	60"	63"	66"	58"	55"	58"	60"	63"	66"
B	35"	35"	38"	34"	31"	34"	32"	35"	38"
A	10"	7"	10"	10"	7"	10"	4"	7"	10"

Track bracket locations shown above are for doors up to five sections high. Additional door sections may be added for a maximum door height of 14'-0". One track bracket (per track) must be added for each section and spaced at a distance not greater than the corresponding section height.

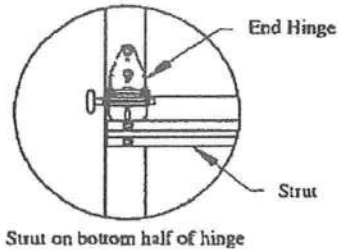
Z3-18045-part-051608

Four Section High Doors 18'-0" wide

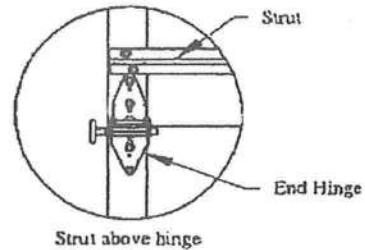
Strut Placement



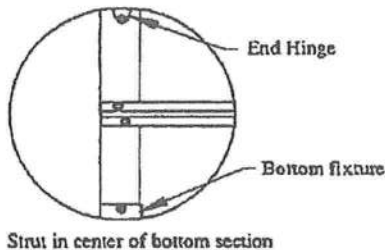
Detail 1



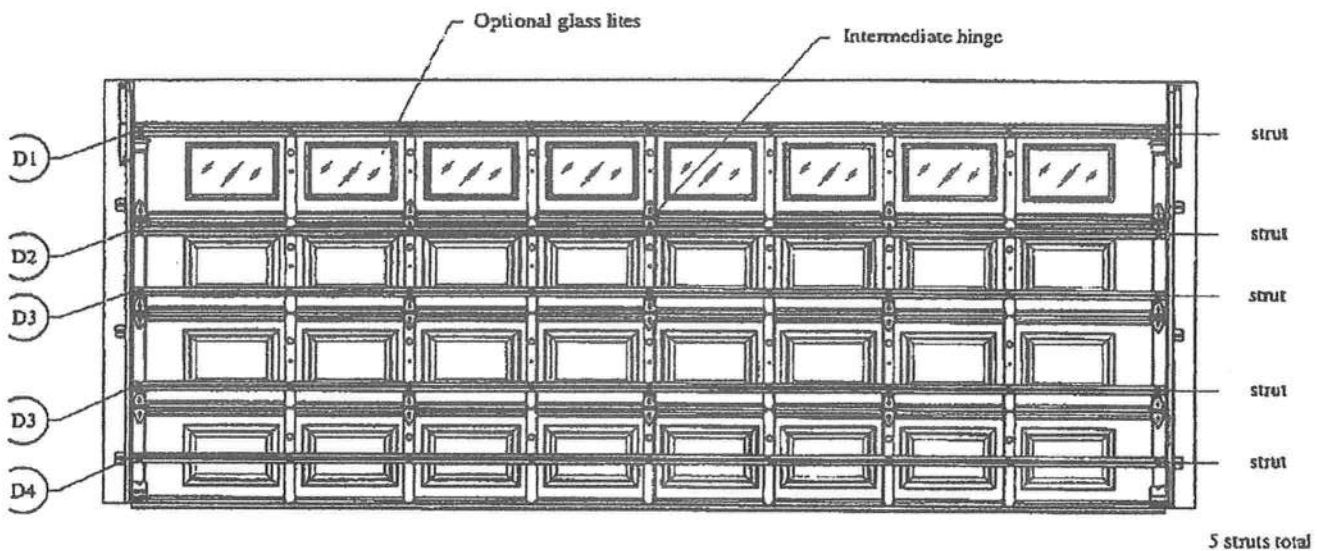
Detail 2



Detail 3



Detail 4



Z3-18045-pan-051608

P. 7

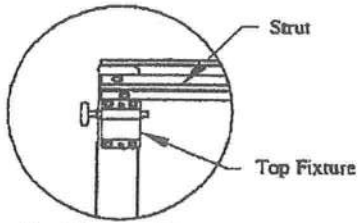
1-800-738-5006

Eldon Plank CHI

Aug 11 09 01:17p

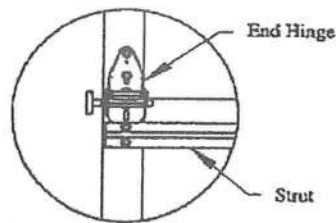
Five Section High Doors 18'-0" wide

Strut Placement



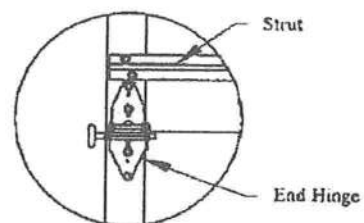
Strut above top fixture

Detail 1



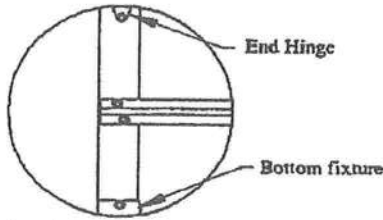
Strut on bottom half of hinge

Detail 2



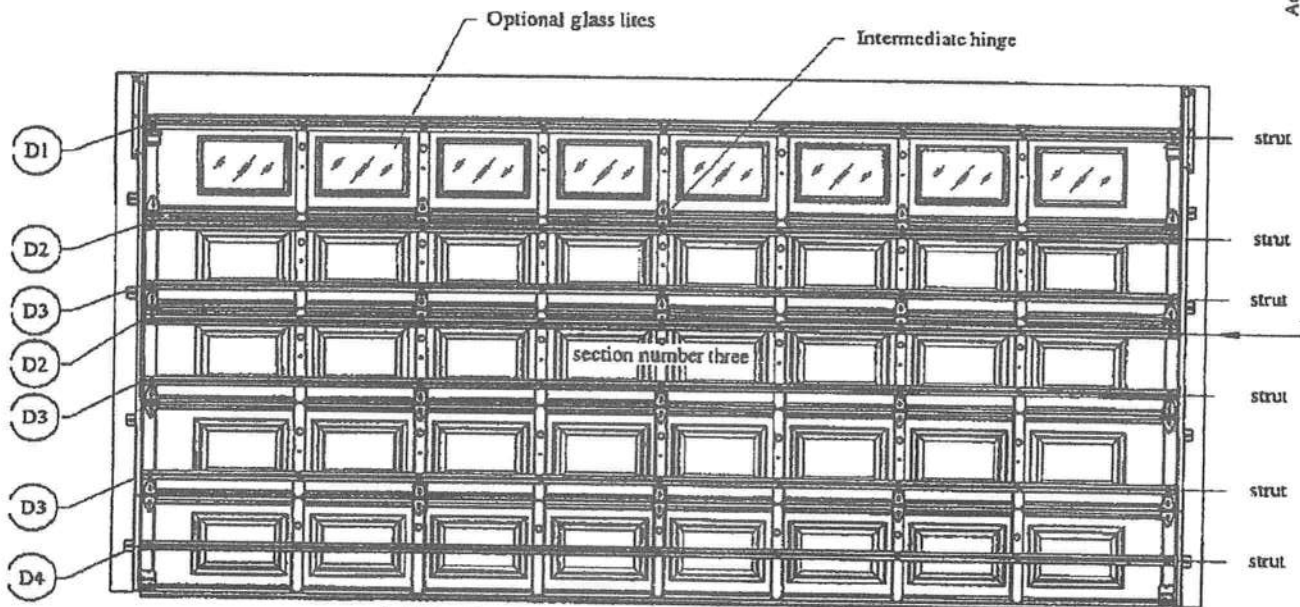
Strut above hinge

Detail 3



Strut in center of bottom section

Detail 4



Additional sections may be added for a maximum door height of 14'-0". Each additional section must have two struts as shown above in section number four and be installed above the third section.

6 struts total 7'-6" thru 8'-0"
7 struts total 8'-3" thru 8'-9"

Z3-18045-pan-051608

P. 8

1-800-738-5006

ELDON PLANK CH

Aug 11 09 01:17p



Zone 3 Supplemental Instructions

Pan Doors: Raised Panel

18'-0" wide

Design pressure: 18.7 pos / 20.8 neg

Test pressure: 28.1 pos / 31.2 neg

CAUTION

Higher wind pressures and larger doors require additional reinforcement.

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Use these instructions only for the wind pressures and door sizes as listed above.

WARNING

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Garage door reinforcement details include:

- Top fixture type and attachment.
- Strut attachment.
- Flag bracket attachment to the wall and track system.
- Track bracket quantity and placement.
- End hinge type and attachment.
- Strut type and placement.

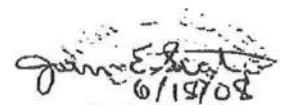
A locking system must be installed if the door is not electrically operated.

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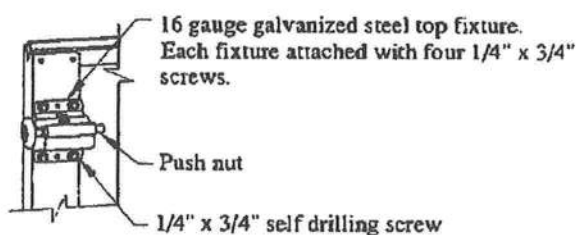
Z3-18045-pan-051608


 John E. Santos, P.E.
 1411 LeMay Street #205
 Carrollton, Texas 75007
 Florida P.E. # 51737
 This document includes 4 pages.
 Professional Engineer's seal
 provided only for verification of
 windload construction details

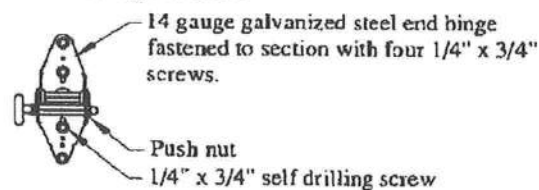
FL 10474

Top Fixture Detail

(Strut, if applicable, not shown for clarity.)

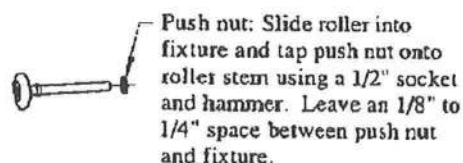


End Hinge Detail

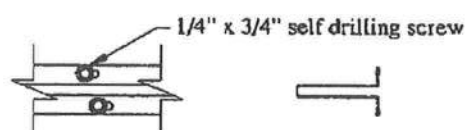


Push Nut Detail (use on all rollers)

use 3/8" I. D. on bottom fixture roller stem
use 7/16" I. D. on end hinge and top fixture roller stems

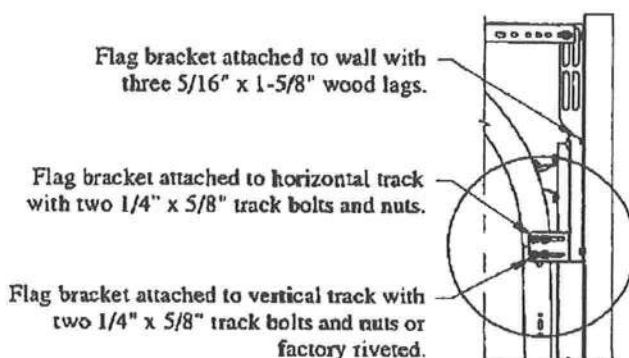


Strut Attachment Detail

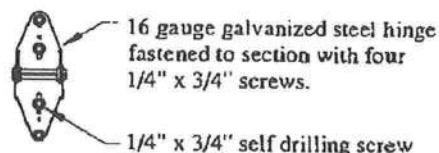


17 gauge 50ksi galvanized steel 3" strut attached with two 1/4" x 3/4" screws to every stile.

Flag Bracket Detail



Intermediate Hinge Detail



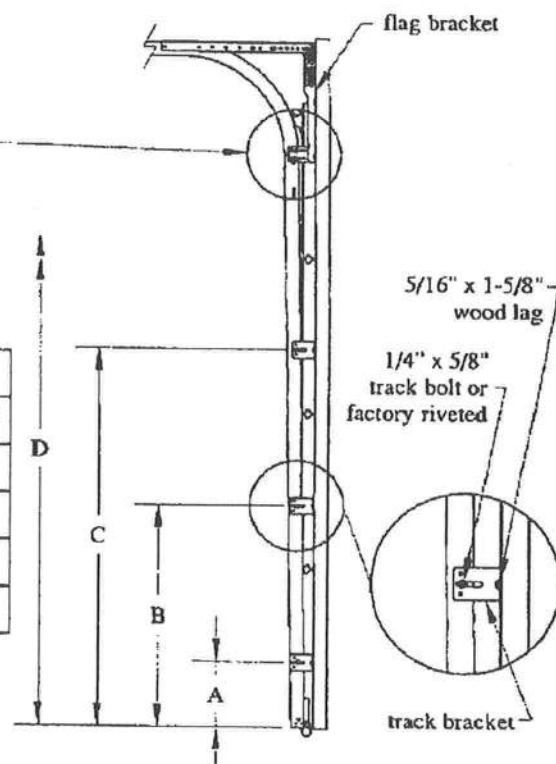
Track Bracket Locations

Manufacturing tolerances and actual field conditions may result in variances of +/- 1"

	door height / four sections			door height / five sections					
	6'-6"	6'-9"	7'-0"	7'-6"	7'-9"	8'-0"	8'-3"	8'-6"	8'-9"
D	n/a	n/a	n/a	72"	69"	72"	81"	84"	87"
C	60"	63"	66"	58"	55"	58"	60"	63"	66"
B	35"	35"	38"	34"	31"	34"	32"	35"	38"
A	10"	7"	10"	10"	7"	10"	4"	7"	10"

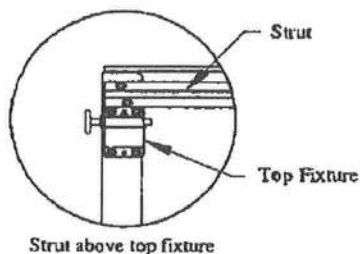
Track bracket locations shown above are for doors up to five sections high. Additional door sections may be added for a maximum door height of 14'-0". One track bracket (per track) must be added for each section and spaced at a distance not greater than the corresponding section height.

Z3-18045-par-051608



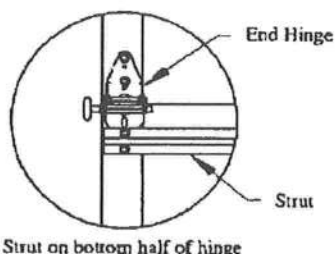
Four Section High Doors 18'-0" wide

Strut Placement



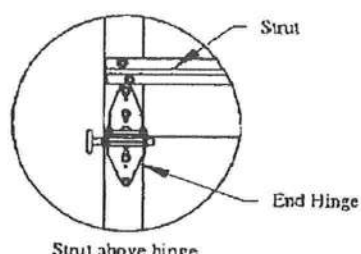
Strut above top fixture

Detail 1



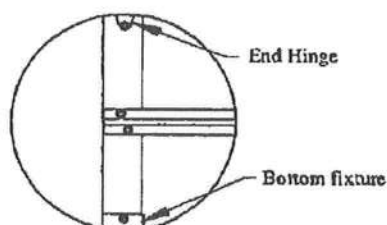
Strut on bottom half of hinge

Detail 2



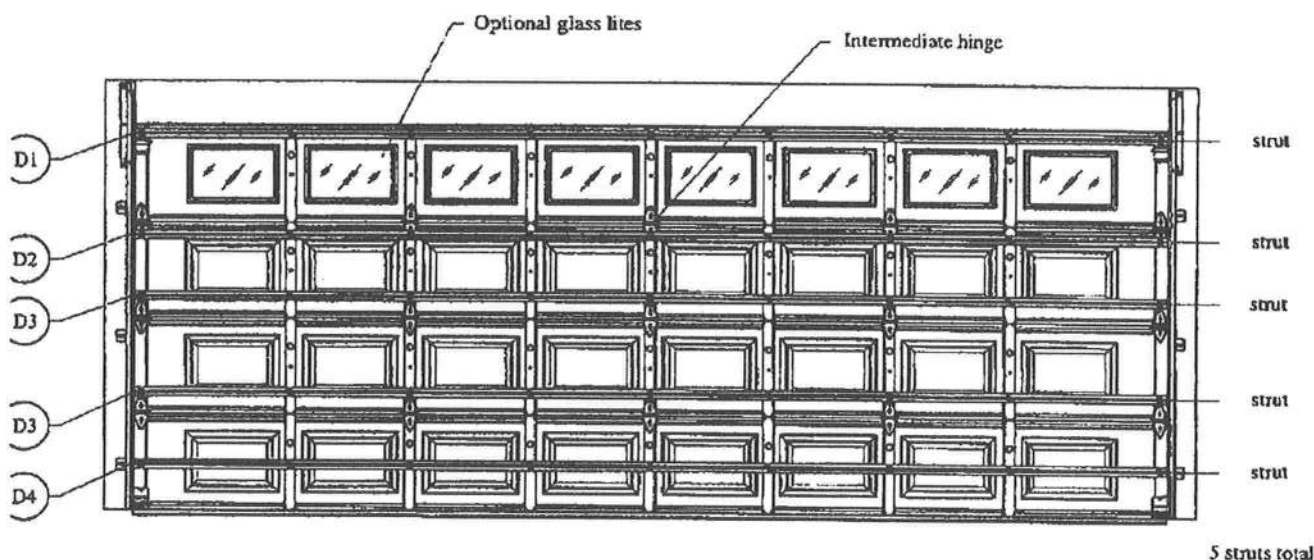
Strut above hinge

Detail 3



Strut in center of bottom section

Detail 4



Z3-18045-pan-051608

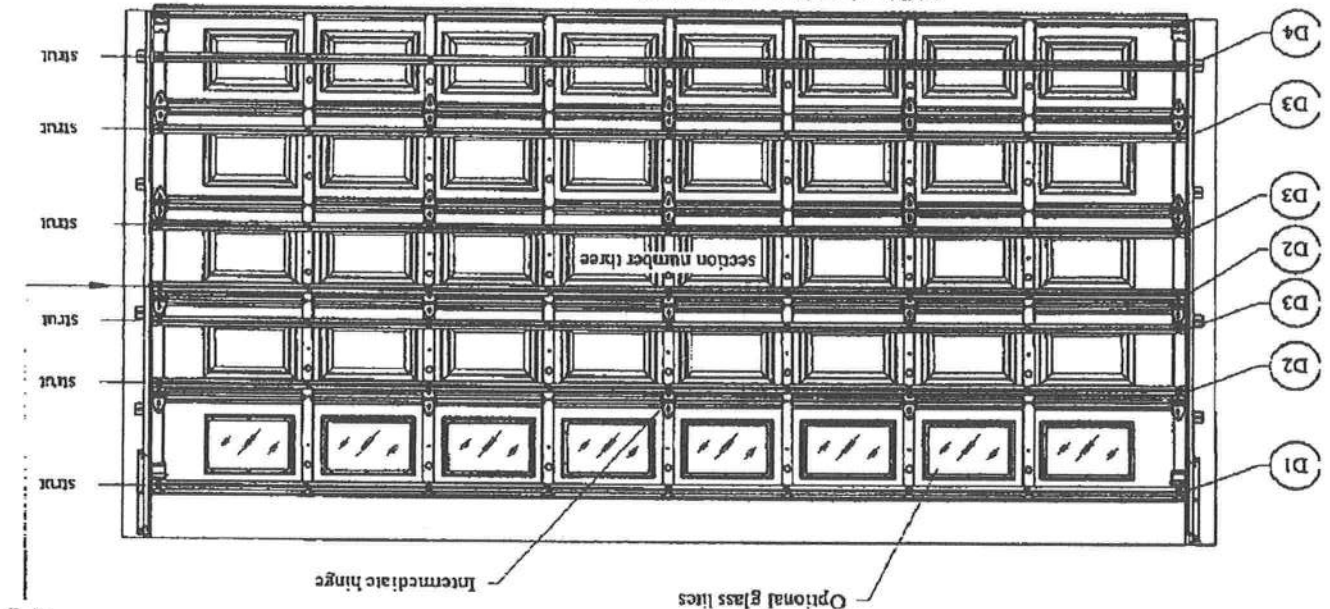
P. 7

1-800-738-5006

Eldon Plank CHI

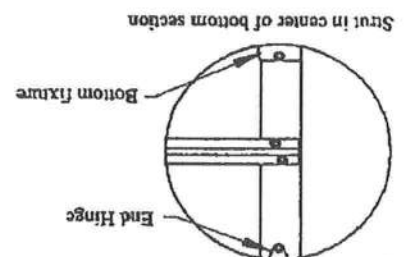
Aug 11 09 01:17p

Additional sections may be added for a maximum door height of 14'-0".
Each additional section must have two struts as shown above in section
number four and be installed above the third section.
6 struts total 7'-6" thru 8'-0"
7 struts total 8'-3" thru 8'-9"

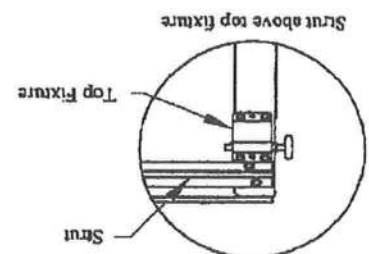


Add this strut for all five
section doors over 8'-0" high

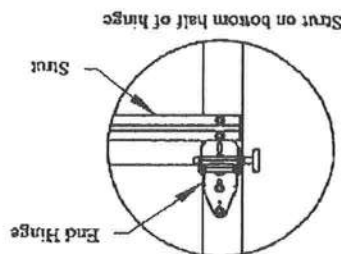
Detail 4



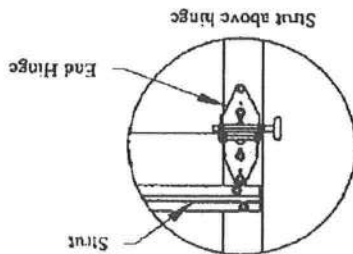
Detail 1



Detail 2



Detail 3



Strut Placement

Five Section High Doors 18'-0" wide

Residential System Sizing Calculation

Summary

Project Title:
808041aKisalaMANJ_Revised_6-24-09

Class 3 Rating
Registration No. 0
Climate: North

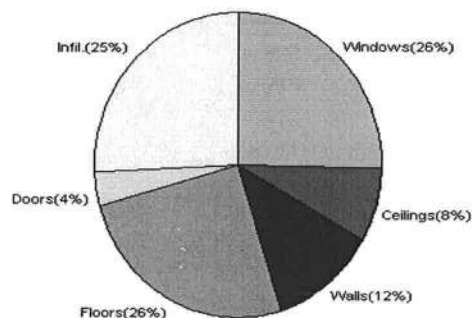
6/24/2009

Location for weather data: Gainesville - Defaults: Latitude(29) Altitude(152 ft.) Temp Range(M)			
Humidity data: Interior RH (50%) Outdoor wet bulb (77F) Humidity difference(54gr.)			
Winter design temperature	33 F	Summer design temperature	92 F
Winter setpoint	70 F	Summer setpoint	75 F
Winter temperature difference	37 F	Summer temperature difference	17 F
Total heating load calculation	40697 Btuh	Total cooling load calculation	38968 Btuh
Submitted heating capacity	% of calc Btuh	Submitted cooling capacity	% of calc Btuh
Total (Electric Heat Pump)	117.9 48000	Sensible (SHR = 0.75)	109.2 36000
Heat Pump + Auxiliary(0.0kW)	117.9 48000	Latent	199.5 12000
		Total (Electric Heat Pump)	123.2 48000

WINTER CALCULATIONS

Winter Heating Load (for 2583 sqft)

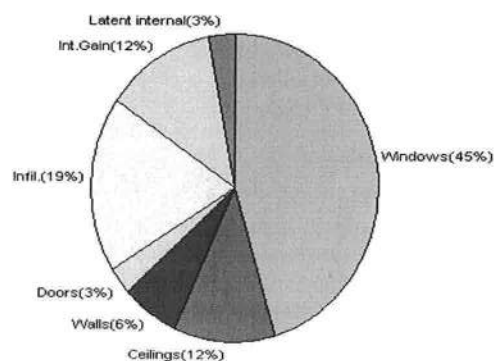
Load component		Load	
Window total	323 sqft	10397	Btuh
Wall total	1695 sqft	4845	Btuh
Door total	113 sqft	1467	Btuh
Ceiling total	2711 sqft	3194	Btuh
Floor total	239 sqft	10435	Btuh
Infiltration	256 cfm	10358	Btuh
Duct loss		0	Btuh
Subtotal		40697	Btuh
Ventilation	0 cfm	0	Btuh
TOTAL HEAT LOSS		40697	Btuh



SUMMER CALCULATIONS

Summer Cooling Load (for 2583 sqft)

Load component		Load	
Window total	323 sqft	17616	Btuh
Wall total	1695 sqft	2506	Btuh
Door total	113 sqft	1110	Btuh
Ceiling total	2711 sqft	4490	Btuh
Floor total		0	Btuh
Infiltration	132 cfm	2452	Btuh
Internal gain		4780	Btuh
Duct gain		0	Btuh
Sens. Ventilation	0 cfm	0	Btuh
Total sensible gain		32954	Btuh
Latent gain(ducts)		0	Btuh
Latent gain(infiltration)		4814	Btuh
Latent gain(ventilation)		0	Btuh
Latent gain(internal/occupants/other)		1200	Btuh
Total latent gain		6014	Btuh
TOTAL HEAT GAIN		38968	Btuh



For Florida residences only

EnergyGauge® System Sizing

PREPARED BY:

DATE: 6/24/09

System Sizing Calculations - Winter

Residential Load - Whole House Component Details

Project Title:
808041aKisalaMANJ_Revised_6-24-09

Class 3 Rating
Registration No. 0
Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F

6/24/2009

This calculation is for Worst Case. The house has been rotated 315 degrees.

Component Loads for Whole House

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft)	X	HTM=	Load
1	2, Clear, Metal, 0.87	NW	30.0		32.2	966 Btuh
2	2, Clear, Metal, 0.87	NW	20.0		32.2	644 Btuh
3	2, Clear, Metal, 0.87	NW	40.0		32.2	1288 Btuh
4	2, Clear, Metal, 0.87	W	10.0		32.2	322 Btuh
5	2, Clear, Metal, 0.87	NW	10.0		32.2	322 Btuh
6	2, Clear, Metal, 0.87	SW	10.0		32.2	322 Btuh
7	2, Clear, Metal, 0.87	NW	20.0		32.2	644 Btuh
8	2, Clear, Metal, 0.87	NW	30.0		32.2	966 Btuh
9	2, Clear, Metal, 0.87	NE	20.0		32.2	644 Btuh
10	2, Clear, Metal, 0.87	NE	6.0		32.2	193 Btuh
11	2, Clear, Metal, 0.87	NE	30.0		32.2	966 Btuh
12	2, Clear, Metal, 0.87	SE	30.0		32.2	966 Btuh
13	2, Clear, Metal, 0.87	SE	3.0		32.2	97 Btuh
14	2, Clear, Metal, 0.87	NE	10.0		32.2	322 Btuh
15	2, Clear, Metal, 0.87	SE	16.0		32.2	515 Btuh
16	2, Clear, Metal, 0.87	SW	10.0		32.2	322 Btuh
17	2, Clear, Metal, 0.87	SE	3.0		32.2	97 Btuh
18	2, Clear, Metal, 0.87	SW	16.0		32.2	515 Btuh
19	2, Clear, Metal, 0.87	SW	9.0		32.2	290 Btuh
Window Total			323(sqft)			10397 Btuh
Walls	Type	R-Value	Area	X	HTM=	Load
1	Frame - Wood - Ext(0.08)	19.0	1508		2.9	4311 Btuh
2	Frame - Wood - Adj(0.08)	19.0	187		2.9	535 Btuh
Wall Total			1695			4845 Btuh
Doors	Type		Area	X	HTM=	Load
1	Insulated - Adjacent		20		12.9	259 Btuh
2	Insulated - Exterior		93		12.9	1208 Btuh
Door Total			113			1467Btuh
Ceilings	Type/Color/Surface	R-Value	Area	X	HTM=	Load
1	Vented Attic/D/Shin)	30.0	2711		1.2	3194 Btuh
Ceiling Total			2711			3194Btuh
Floors	Type	R-Value	Size	X	HTM=	Load
1	Slab On Grade	0	239.0 ft(p)		43.7	10435 Btuh
Floor Total			239			10435 Btuh
Zone Envelope Subtotal:						30339 Btuh
Infiltration	Type	ACH	Zone Volume		CFM=	
	Natural	0.66	23247		255.7	10358 Btuh
Ductload	Partially sealed, R6.0, Supply(Attic), Return(Attic) (DLM of 0.00)					0 Btuh

Manual J Winter Calculations

Residential Load - Component Details (continued)

Project Title:
808041aKisalaMANJ_Revised_6-24-09

Class 3 Rating
Registration No. 0
Climate: North

6/24/2009

Zone #1	Sensible Zone Subtotal	40697 Btuh
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WHOLE HOUSE TOTALS

	Subtotal Sensible	40697 Btuh
	Ventilation Sensible	0 Btuh
	Total Btuh Loss	40697 Btuh

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)



For Florida residences only

System Sizing Calculations - Winter

Residential Load - Room by Room Component Details

Project Title:
808041aKisalaMANJ_Revised_6-24-09

Class 3 Rating
Registration No. 0
Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F

6/24/2009

This calculation is for Worst Case. The house has been rotated 315 degrees.

Component Loads for Zone #1: Main

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft)	X	HTM=	Load
1	2, Clear, Metal, 0.87	NW	30.0		32.2	966 Btuh
2	2, Clear, Metal, 0.87	NW	20.0		32.2	644 Btuh
3	2, Clear, Metal, 0.87	NW	40.0		32.2	1288 Btuh
4	2, Clear, Metal, 0.87	W	10.0		32.2	322 Btuh
5	2, Clear, Metal, 0.87	NW	10.0		32.2	322 Btuh
6	2, Clear, Metal, 0.87	SW	10.0		32.2	322 Btuh
7	2, Clear, Metal, 0.87	NW	20.0		32.2	644 Btuh
8	2, Clear, Metal, 0.87	NW	30.0		32.2	966 Btuh
9	2, Clear, Metal, 0.87	NE	20.0		32.2	644 Btuh
10	2, Clear, Metal, 0.87	NE	6.0		32.2	193 Btuh
11	2, Clear, Metal, 0.87	NE	30.0		32.2	966 Btuh
12	2, Clear, Metal, 0.87	SE	30.0		32.2	966 Btuh
13	2, Clear, Metal, 0.87	SE	3.0		32.2	97 Btuh
14	2, Clear, Metal, 0.87	NE	10.0		32.2	322 Btuh
15	2, Clear, Metal, 0.87	SE	16.0		32.2	515 Btuh
16	2, Clear, Metal, 0.87	SW	10.0		32.2	322 Btuh
17	2, Clear, Metal, 0.87	SE	3.0		32.2	97 Btuh
18	2, Clear, Metal, 0.87	SW	16.0		32.2	515 Btuh
19	2, Clear, Metal, 0.87	SW	9.0		32.2	290 Btuh
Window Total			323(sqft)			10397 Btuh
Walls	Type	R-Value	Area	X	HTM=	Load
1	Frame - Wood - Ext(0.08)	19.0	1508		2.9	4311 Btuh
2	Frame - Wood - Adj(0.08)	19.0	187		2.9	535 Btuh
Wall Total			1695			4845 Btuh
Doors	Type		Area	X	HTM=	Load
1	Insulated - Adjacent		20		12.9	259 Btuh
2	Insulated - Exterior		93		12.9	1208 Btuh
Door Total			113			1467Btuh
Ceilings	Type/Color/Surface	R-Value	Area	X	HTM=	Load
1	Vented Attic/D/Shin)	30.0	2711		1.2	3194 Btuh
Ceiling Total			2711			3194Btuh
Floors	Type	R-Value	Size	X	HTM=	Load
1	Slab On Grade	0	239.0 ft(p)		43.7	10435 Btuh
Floor Total			239			10435 Btuh
Zone Envelope Subtotal:						30339 Btuh
Infiltration	Type	ACH X	Zone Volume		CFM=	
	Natural	0.66	23247		255.7	10358 Btuh
Ductload	Partially sealed, R6.0, Supply(Attic), Return(Attic) (DLM of 0.00)					0 Btuh

Manual J Winter Calculations

Residential Load - Component Details (continued)

Project Title:
808041aKisalaMANJ_Revised_6-24-09

Class 3 Rating
Registration No. 0
Climate: North

6/24/2009

Zone #1	Sensible Zone Subtotal	40697 Btuh
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WHOLE HOUSE TOTALS

	Subtotal Sensible	40697 Btuh
	Ventilation Sensible	0 Btuh
	Total Btuh Loss	40697 Btuh

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)



For Florida residences only

System Sizing Calculations - Summer

Residential Load - Whole House Component Details

Project Title:
808041aKisalaMANJ_Revised_6-24-09

Class 3 Rating
Registration No. 0
Climate: North

Reference City: Gainesville (Defaults) Summer Temperature Difference: 17.0 F

6/24/2009

This calculation is for Worst Case. The house has been rotated 315 degrees.

Component Loads for Whole House

Window	Type*		Overhang		Window Area(sqft)			HTM		Load	
	Pn/SHGC/U/InSh/ExSh/IS	Ornt	Len	Hgt	Gross	Shaded	Unshaded	Shaded	Unshaded		
1	2, Clear, 0.87, None,N,N	NW	13.5f	7ft.	30.0	0.0	30.0	29	60	1801	Btuh
2	2, Clear, 0.87, None,N,N	NW	13.5f	8ft.	20.0	0.0	20.0	29	60	1201	Btuh
3	2, Clear, 0.87, None,N,N	NW	13.5f	8ft.	40.0	0.0	40.0	29	60	2401	Btuh
4	2, Clear, 0.87, None,N,N	W	14ft.	8ft.	10.0	10.0	0.0	29	80	290	Btuh
5	2, Clear, 0.87, None,N,N	NW	9.16	8ft.	10.0	0.0	10.0	29	60	600	Btuh
6	2, Clear, 0.87, None,N,N	SW	49.6	7ft.	10.0	10.0	0.0	29	63	290	Btuh
7	2, Clear, 0.87, None,N,N	NW	5.66	6ft.	20.0	0.0	20.0	29	60	1201	Btuh
8	2, Clear, 0.87, None,N,N	NW	5.66	8ft.	30.0	0.0	30.0	29	60	1801	Btuh
9	2, Clear, 0.87, None,N,N	NE	1.5ft	7ft.	20.0	0.0	20.0	29	60	1201	Btuh
10	2, Clear, 0.87, None,N,N	NE	1.5ft	3.5ft	6.0	0.0	6.0	29	60	360	Btuh
11	2, Clear, 0.87, None,N,N	NE	1.5ft	7ft.	30.0	0.0	30.0	29	60	1801	Btuh
12	2, Clear, 0.87, None,N,N	SE	7.5ft	7ft.	30.0	30.0	0.0	29	63	869	Btuh
13	2, Clear, 0.87, None,N,N	SE	7.5ft	2ft.	3.0	3.0	0.0	29	63	87	Btuh
14	2, Clear, 0.87, None,N,N	NE	1.5ft	7ft.	10.0	0.0	10.0	29	60	600	Btuh
15	2, Clear, 0.87, None,N,N	SE	0ft.	0ft.	16.0	0.0	16.0	29	63	1001	Btuh
16	2, Clear, 0.87, None,N,N	SW	1.5ft	7ft.	10.0	1.0	9.0	29	63	590	Btuh
17	2, Clear, 0.87, None,N,N	SE	1.5ft	3ft.	3.0	1.6	1.4	29	63	135	Btuh
18	2, Clear, 0.87, None,N,N	SW	1.5ft	6ft.	16.0	2.1	13.9	29	63	930	Btuh
19	2, Clear, 0.87, None,N,N	SW	1.5ft	3.5ft	9.0	3.1	5.9	29	63	457	Btuh
Window Total					323 (sqft)					17616 Btuh	
Walls	Type	R-Value/U-Value		Area(sqft)		HTM		Load			
1	Frame - Wood - Ext	19.0/0.08		1507.7		1.5		2260 Btuh			
2	Frame - Wood - Adj	19.0/0.08		187.0		1.3		246 Btuh			
Wall Total					1695 (sqft)			2506 Btuh			
Doors	Type			Area (sqft)		HTM		Load			
1	Insulated - Adjacent			20.0		9.8		196 Btuh			
2	Insulated - Exterior			93.3		9.8		914 Btuh			
Door Total					113 (sqft)			1110 Btuh			
Ceilings	Type/Color/Surface	R-Value		Area(sqft)		HTM		Load			
1	Vented Attic/DarkShingle	30.0		2711.0		1.7		4490 Btuh			
Ceiling Total					2711 (sqft)			4490 Btuh			
Floors	Type	R-Value		Size		HTM		Load			
1	Slab On Grade	0.0		239 (ft(p))		0.0		0 Btuh			
Floor Total					239.0 (sqft)			0 Btuh			
Zone Envelope Subtotal:										25722 Btuh	
Infiltration	Type	ACH		Volume(cuft)		CFM=		Load			
SensibleNatural		0.34		23247		131.7		2452 Btuh			
Internal gain	Occupants		Btuh/occupant		Appliance		Load				
		6		X 230 +		3400		4780 Btuh			
Duct load	Partially sealed, R6.0, Supply(Attic), Return(Attic)							DGM = 0.00		0.0 Btuh	
Sensible Zone Load										32954 Btuh	

Manual J Summer Calculations

Residential Load - Component Details (continued)

Project Title:
808041aKisalaMANJ_Revised_6-24-09

Class 3 Rating
Registration No. 0
Climate: North

6/24/2009

WHOLE HOUSE TOTALS

Whole House Totals for Cooling	Sensible Envelope Load All Zones	32954 Btuh
	Sensible Duct Load	0 Btuh
	Total Sensible Zone Loads	32954 Btuh
	Sensible ventilation	0 Btuh
	Blower	0 Btuh
	Total sensible gain	32954 Btuh
	Latent infiltration gain (for 54 gr. humidity difference)	4814 Btuh
	Latent ventilation gain	0 Btuh
	Latent duct gain	0 Btuh
	Latent occupant gain (6 people @ 200 Btuh per person)	1200 Btuh
	Latent other gain	0 Btuh
	Latent total gain	6014 Btuh
	TOTAL GAIN	38968 Btuh

*Key: Window types (Pn - Number of panes of glass)

(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)

(U - Window U-Factor or 'DEF' for default)

(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R))

(ExSh - Exterior shading device: none(N) or numerical value)

(BS - Insect screen: none(N), Full(F) or Half(H))

(Ornt - compass orientation)



For Florida residences only

System Sizing Calculations - Summer

Residential Load - Room by Room Component Details

Project Title:
808041aKisalaMANJ_Revised_6-24-09

Class 3 Rating
Registration No. 0
Climate: North

Reference City: Gainesville (Defaults) Summer Temperature Difference: 17.0 F
This calculation is for Worst Case. The house has been rotated 315 degrees.

6/24/2009

Component Loads for Zone #1: Main

Window	Type*		Overhang		Window Area(sqft)			HTM		Load	
	Pn/SHGC/U/InSh/ExSh/IS	Ornt	Len	Hgt	Gross	Shaded	Unshaded	Shaded	Unshaded		
1	2, Clear, 0.87, None,N,N	NW	13.5f	7ft.	30.0	0.0	30.0	29	60	1801	Btuh
2	2, Clear, 0.87, None,N,N	NW	13.5f	8ft.	20.0	0.0	20.0	29	60	1201	Btuh
3	2, Clear, 0.87, None,N,N	NW	13.5f	8ft.	40.0	0.0	40.0	29	60	2401	Btuh
4	2, Clear, 0.87, None,N,N	W	14ft.	8ft.	10.0	10.0	0.0	29	80	290	Btuh
5	2, Clear, 0.87, None,N,N	NW	9.16	8ft.	10.0	0.0	10.0	29	60	600	Btuh
6	2, Clear, 0.87, None,N,N	SW	49.6	7ft.	10.0	10.0	0.0	29	63	290	Btuh
7	2, Clear, 0.87, None,N,N	NW	5.66	6ft.	20.0	0.0	20.0	29	60	1201	Btuh
8	2, Clear, 0.87, None,N,N	NW	5.66	8ft.	30.0	0.0	30.0	29	60	1801	Btuh
9	2, Clear, 0.87, None,N,N	NE	1.5ft	7ft.	20.0	0.0	20.0	29	60	1201	Btuh
10	2, Clear, 0.87, None,N,N	NE	1.5ft	3.5ft	6.0	0.0	6.0	29	60	360	Btuh
11	2, Clear, 0.87, None,N,N	NE	1.5ft	7ft.	30.0	0.0	30.0	29	60	1801	Btuh
12	2, Clear, 0.87, None,N,N	SE	7.5ft	7ft.	30.0	30.0	0.0	29	63	869	Btuh
13	2, Clear, 0.87, None,N,N	SE	7.5ft	2ft.	3.0	3.0	0.0	29	63	87	Btuh
14	2, Clear, 0.87, None,N,N	NE	1.5ft	7ft.	10.0	0.0	10.0	29	60	600	Btuh
15	2, Clear, 0.87, None,N,N	SE	0ft.	0ft.	16.0	0.0	16.0	29	63	1001	Btuh
16	2, Clear, 0.87, None,N,N	SW	1.5ft	7ft.	10.0	1.0	9.0	29	63	590	Btuh
17	2, Clear, 0.87, None,N,N	SE	1.5ft	3ft.	3.0	1.6	1.4	29	63	135	Btuh
18	2, Clear, 0.87, None,N,N	SW	1.5ft	6ft.	16.0	2.1	13.9	29	63	930	Btuh
19	2, Clear, 0.87, None,N,N	SW	1.5ft	3.5ft	9.0	3.1	5.9	29	63	457	Btuh
Window Total					323 (sqft)					17616 Btuh	
Walls	Type	R-Value/U-Value			Area(sqft)			HTM		Load	
1	Frame - Wood - Ext	19.0/0.08			1507.7			1.5		2260 Btuh	
2	Frame - Wood - Adj	19.0/0.08			187.0			1.3		246 Btuh	
Wall Total					1695 (sqft)					2506 Btuh	
Doors	Type				Area (sqft)			HTM		Load	
1	Insulated - Adjacent				20.0			9.8		196 Btuh	
2	Insulated - Exterior				93.3			9.8		914 Btuh	
Door Total					113 (sqft)					1110 Btuh	
Ceilings	Type/Color/Surface	R-Value			Area(sqft)			HTM		Load	
1	Vented Attic/DarkShingle	30.0			2711.0			1.7		4490 Btuh	
Ceiling Total					2711 (sqft)					4490 Btuh	
Floors	Type	R-Value			Size			HTM		Load	
1	Slab On Grade	0.0			239 (ft(p))			0.0		0 Btuh	
Floor Total					239.0 (sqft)					0 Btuh	
Zone Envelope Subtotal:										25722 Btuh	
Infiltration	Type	ACH			Volume(cuft)			CFM=		Load	
	SensibleNatural	0.34			23247			131.7		2452 Btuh	
Internal gain	Occupants			Btuh/occupant			Appliance		Load		
	6			X 230			+ 3400		4780 Btuh		
Duct load	Partially sealed, R6.0, Supply(Attic), Return(Attic)							DGM = 0.00		0.0 Btuh	
Sensible Zone Load										32954 Btuh	

Manual J Summer Calculations

Residential Load - Component Details (continued)

Project Title:
808041aKisalaMANJ_Revised_6-24-09

Class 3 Rating
Registration No. 0
Climate: North

6/24/2009

WHOLE HOUSE TOTALS

Whole House Totals for Cooling	Sensible Envelope Load All Zones	32954 Btuh
	Sensible Duct Load	0 Btuh
	Total Sensible Zone Loads	32954 Btuh
	Sensible ventilation	0 Btuh
	Blower	0 Btuh
	Total sensible gain	32954 Btuh
	Latent infiltration gain (for 54 gr. humidity difference)	4814 Btuh
	Latent ventilation gain	0 Btuh
	Latent duct gain	0 Btuh
	Latent occupant gain (6 people @ 200 Btuh per person)	1200 Btuh
	Latent other gain	0 Btuh
	Latent total gain	6014 Btuh
	TOTAL GAIN	38968 Btuh

*Key: Window types (Pn - Number of panes of glass)
 (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
 (U - Window U-Factor or 'DEF' for default)
 (InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R))
 (ExSh - Exterior shading device: none(N) or numerical value)
 (BS - Insect screen: none(N), Full(F) or Half(H))
 (Ornt - compass orientation)



For Florida residences only

Residential Window Diversity

MidSummer

Project Title:
808041aKisalaMANJ_Revised_6-24-09

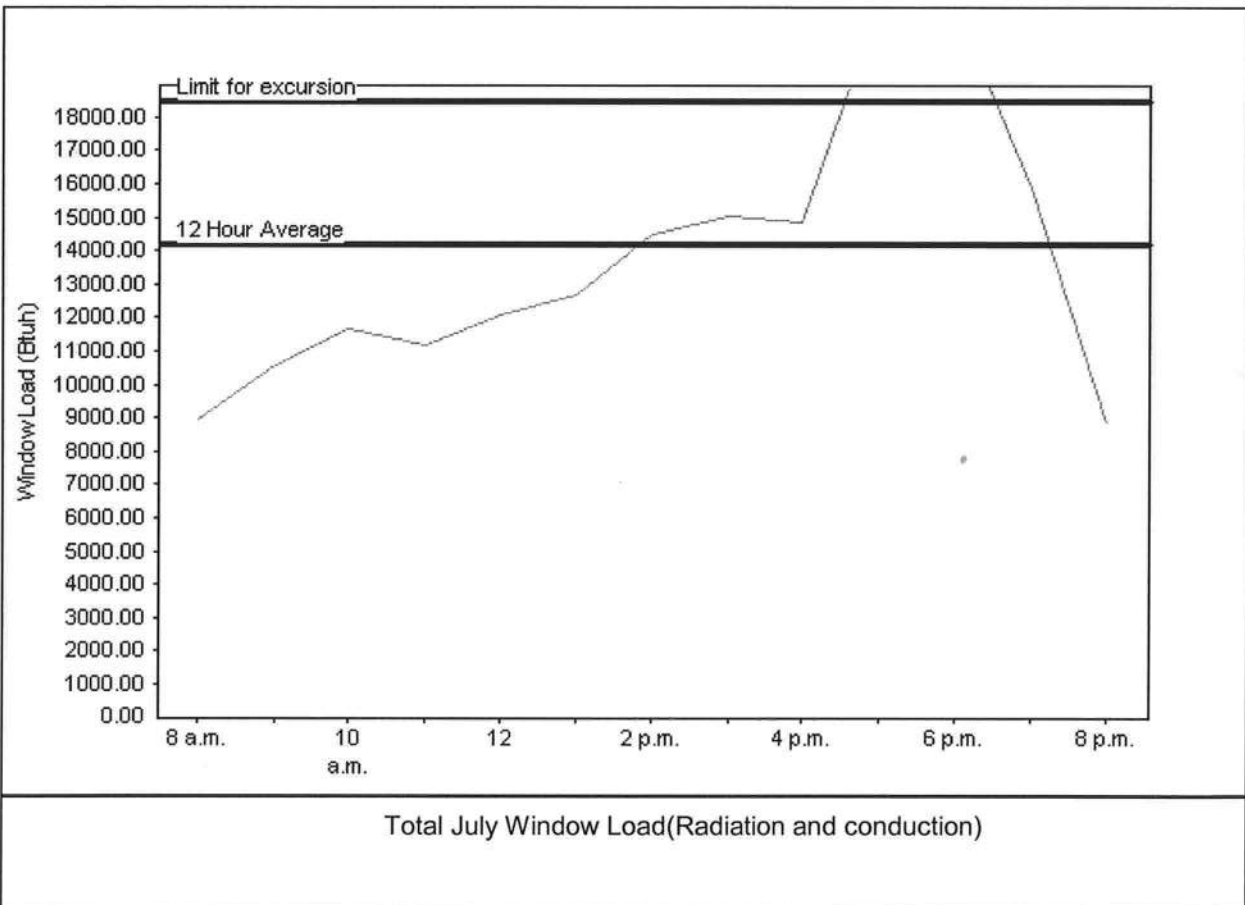
Class 3 Rating
Registration No. 0
Climate: North

6/24/2009

Weather data for: Gainesville - Defaults

Summer design temperature	92 F	Average window load for July	14198 Btu
Summer setpoint	75 F	Peak window load for July	21727 Btu
Summer temperature difference	17 F	Excursion limit(130% of Ave.)	18457 Btu
Latitude	29 North	Window excursion (July)	3270 Btu

WINDOW Average and Peak Loads



This application has glass areas that produce large heat gains for part of the day. Variable air volume devices are required to overcome spikes in solar gain for one or more rooms. Install a zoned system or provide zone control for problem rooms. Single speed equipment may not be suitable for the application.

EnergyGauge® System Sizing for Florida residences only

PREPARED BY: _____

DATE: _____

EnergyGauge® FLR2PB v4.1



Permit # 28074



- Engineering
- Geotechnical
- Environmental

Laboratories

Cal-Tech Testing, Inc.

P.O. Box 1625 • Lake City, FL 32056-1525 • Tel(386)755-3633 • Fax(386)752-5456
4784 Rosselle St. Jacksonville, FL 32254 • Tel(904)381-8901 • Fax(904)381-8902

REPORT OF IN-PLACE DENSITY TEST

JOB NO.: 09-00323-01

DATE TESTED: 8/13/09

DATE REPORTED: 8/18/09

PROJECT:	Louis & Elona Kisala Residence Lot 31 High Point Subdivision
CLIENT:	Dale's Excavation, Inc. 6139 SW SR 47, Lake City, FL 32024
GENERAL CONTRACTOR:	Dale's Excavation, Inc
EARTHWORK CONTRACTOR:	Dale's Excavation, Inc
INSPECTOR:	Louis Roberts
ASTM METHOD (D-2922) Nuclear	SOIL USE BUILDING FILL
SPECIFIED REQUIREMENTS: 95%	

TEST NO.	TEST LOCATION	LIFT	TEST DEPTH	WET DENSITY (lb/ft ³)	MOISTURE PERCENT	DRY DENSITY (lb/ft ³)	PROCTOR TEST NO.	PROCTOR VALUE	MAXIMUM DENSITY
1	SE Corner of Building 45' West x 30' North	1	12"	119.6	6.9	111.9	1	111.0	101%
2	SW Corner of Building 20' East x 20' North	2	12"	121.2	12.1	108.1	1	111.0	97%
3	SW Corner of Building 15' East x 15' North	1	12"	115.3	6.7	108.1	1	111.0	97%
4	SE Corner of Building 30' North x 30' North	2	12"	117.4	3.2	113.8	1	111.0	102%

REMARKS: The Above Tests Meet Specified Requirements.

PROCTORS

PROCTOR NO.	SOIL DESCRIPTION	MAXIMUM DRY UNIT WEIGHT (lb/ft ³)	OPT. MOIST.	TYPE
1	Gray Fine Sand	111.0	10.0	MODIFIED (ASTM D-1557)

Respectfully Submitted,
CAL-TECH TESTING, INC.

Reviewed By:

Linda Creamer, CEO, DBE

Linda M. Creamer
President - CEO

ee

[Signature]
Date: 8/19/09
Licensed, Florida No: 57842

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Cal-Tech Testing, Inc.

- Engineering
 - Geotechnical
 - Environmental
- Laboratories

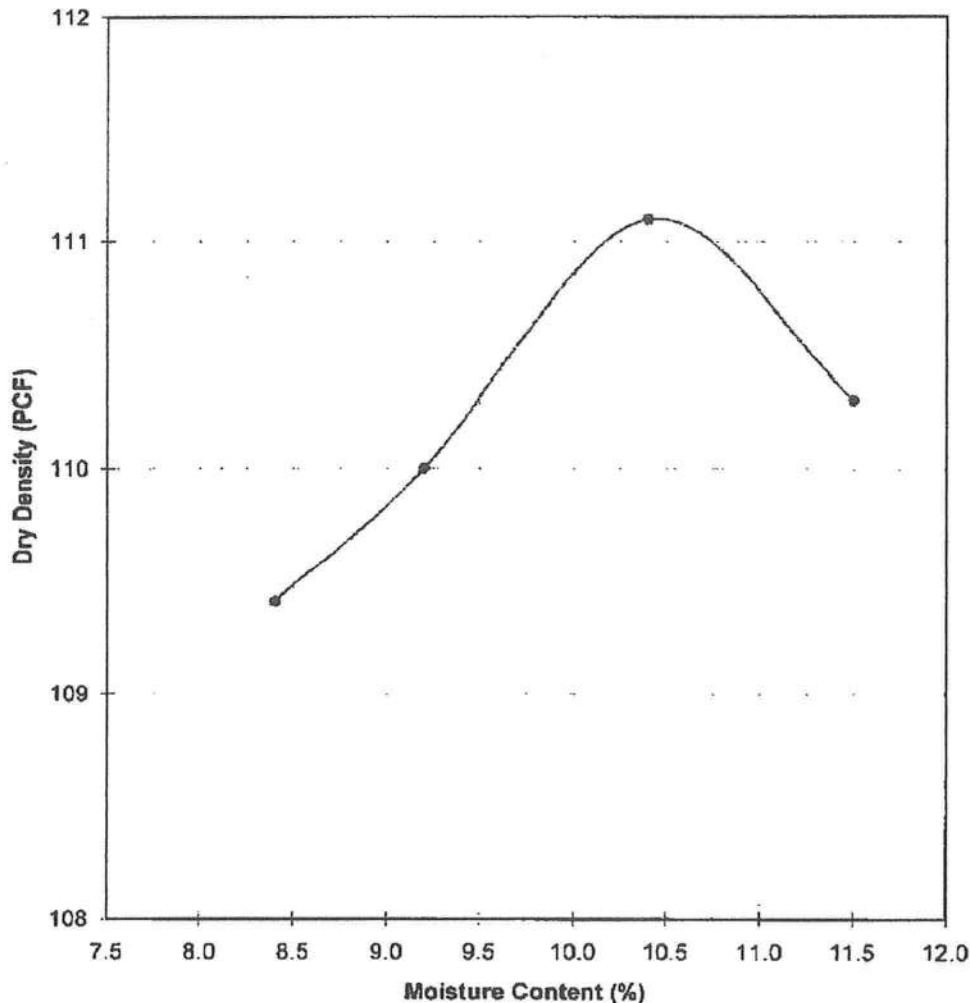
P.O. Box 1625 • Lake City, FL 32056-1625 • Tel(386)755-3833 • Fax(386)752-5456
4784 Rosselle St., Jacksonville, FL 32254 • Tel(904)381-8901 • Fax(904)381-8902

REPORT OF LABORATORY COMPACTION TEST

Client:
Project Name:
Project Location:
Contractor:

Dale's Excavation, Inc. 6139 SW SR 47, Lake City, FL 32024
Louis & Elona Kisala Residence Lot 31 High Point Subdivision
Lake City, FL
Dale's Excavation, Inc.

File No: 09-00323-01
Date: 8/18/2009
Lab No: 12490



PROCTOR DATA

Proctor No.: 1

Modified Proctor ☒
(ASTM D-1557)

Standard Proctor ☐
(ASTM D-698)

Maximum Dry
Dens. Pcf: 111

Optimum Moisture
Percent: 10

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Sample Description:
Sample Location:
Proposed Use:
Sampled By:
Tested By:
Remarks:

Gray Fine Sand
House Pad
House Pad
David Brown Date: 8/12/2009
Tim Cassidy Date: 8/18/2009
1cc: Client
1cc: File

Linda Creamer, CEO
Linda M. Creamer
President - CEO
Reviewed By: *[Signature]*
Date: 8/19/09
Licensed, FL No: 57842



- Engineering
 - Geotechnical
 - Environmental
- Laboratories

Cal-Tech Testing, Inc.

P.O. Box 1625 • Lake City, FL 32056-1625 • Tel(386)755-3633 • Fax(386)752-5456
 4784 Rosselle St., Jacksonville, FL 32254 • Tel(904)361-8921 • Fax(904)361-8502

REPORT OF IN-PLACE DENSITY TEST

JOB NO.: 09-00323-01
 DATE TESTED: 8/17/09
 DATE REPORTED: 8/21/09

PROJECT:	Louis & Elona Kisala Residence Lot 31 High Point Subdivision
CLIENT:	Dale's Excavation, Inc. 6139 SW SR 47, Lake City, FL 32024
GENERAL CONTRACTOR:	Dale's Excavation, Inc
EARTHWORK CONTRACTOR:	Dale's Excavation, Inc
INSPECTOR:	Louis Roberts
ASTM METHOD (D-2922) Nuclear	SOIL USE BUILDING FILL
SPECIFIED REQUIREMENTS: 95%	

TEST NO.	TEST LOCATION	LIFT	TEST DEPTH	WET DENSITY (lb/ft ³)	MOISTURE PERCENT	DRY DENSITY (lb/ft ³)	PROCTOR TEST NO.	PROCTOR VALUE	MAXIMUM DENSITY
8	SW Corner of Building 10' East x 10' North	6	12"	116.2	4.9	110.8	2	111.0	100%
9	SW Corner of Building 12' East x 12' North	5	12"	118.1	5.5	111.9	2	111.0	101%
10	SE Corner of Building 10' West x 10' North	5	12"	121.4	15.7	104.9	2	111.0	95%
11	SE Corner of Building 12' West x 12' North	6	12"	119.4	5.4	113.3	2	111.0	102%
12	Center of Building Pad	2	12"	122.0	10.8	110.1	2	111.0	99%

REMARKS: The Above Tests Meet Specified Requirements.

PROCTORS				
PROCTOR NO.	SOIL DESCRIPTION	MAXIMUM DRY UNIT WEIGHT (lb/ft ³)	OPT. MOIST.	TYPE
2	Brown & Gray Sand	111.0	10.0	MODIFIED (ASTM D-1557)

Respectfully Submitted,
 CAL-TECH TESTING, INC.

Linda Creamer, CEO, DSE
 Linda M. Creamer
 President - CEO

Reviewed By:

[Signature]
 Date: 8/24/09
 Licensed, Florida No: 57842

The test results presented in this report are specific only to the samples tested at the time of testing. The tests were performed in accordance with generally accepted methods and standards. Since material conditions can vary between test locations and change with time, sound judgement should be exercised with regard to the use and interpretation of the data.



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Laboratories

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4784 Roselle St., Jacksonville, FL 32254 • Tel(904)381-8901 • Fax(904)381-8902

REPORT OF IN-PLACE DENSITY TEST

JOB NO.: 09-00323-01

DATE TESTED: 8/14/09

DATE REPORTED: 8/21/09

PROJECT:	Louis & Elona Kisala Residence Lot 31 High Point Subdivision
CLIENT:	Dale's Excavation, Inc. 6139 SW SR 47, Lake City, FL 32024
GENERAL CONTRACTOR:	Dale's Excavation, Inc
EARTHWORK CONTRACTOR:	Dale's Excavation, Inc
INSPECTOR:	Pam Geiger
ASTM METHOD (D-2922) Nuclear	SOIL USE BUILDING FILL
SPECIFIED REQUIREMENTS: 95%	

TEST NO.	TEST LOCATION	LIFT	TEST DEPTH	WET DENSITY (lb/ft ³)	MOISTURE PERCENT	DRY DENSITY (lb/ft ³)	PROCTOR TEST NO.	PROCTOR VALUE	MAXIMUM DENSITY
5	15' North x 30' East of SW Corner	4	12"	120.0	9.7	109.4	2	111.0	99%
6	25' North x 20' West of SE Corner	4	12"	119.6	10.0	108.7	2	111.0	98%
7	20' North x 25' East of SW Corner	3	12"	121.6	8.7	111.9	2	111.0	101%

REMARKS: The Above Tests Meet Specified Requirements.

PROCTORS

PROCTOR NO.	SOIL DESCRIPTION	MAXIMUM DRY UNIT WEIGHT (lb/ft ³)	OPT. MOIST.	TYPE
2	Brown & Gray Sand	111.0	10.0	MODIFIED (ASTM D-1557)

Respectfully Submitted,
CAL-TECH TESTING, INC.

Linda M. Creamer
President - CEO

Reviewed By:

Date: 8/24/09
Licensed, Florida No: 57842

The test results presented in this report are specific only to the samples tested at the time of testing. The tests were performed in accordance with generally accepted methods and standards. Since material conditions can vary between test locations and change with time, sound judgement should be exercised with regard to the use and interpretation of the data.



Cal-Tech Testing, Inc.

- Engineering
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- Environmental

laboratories

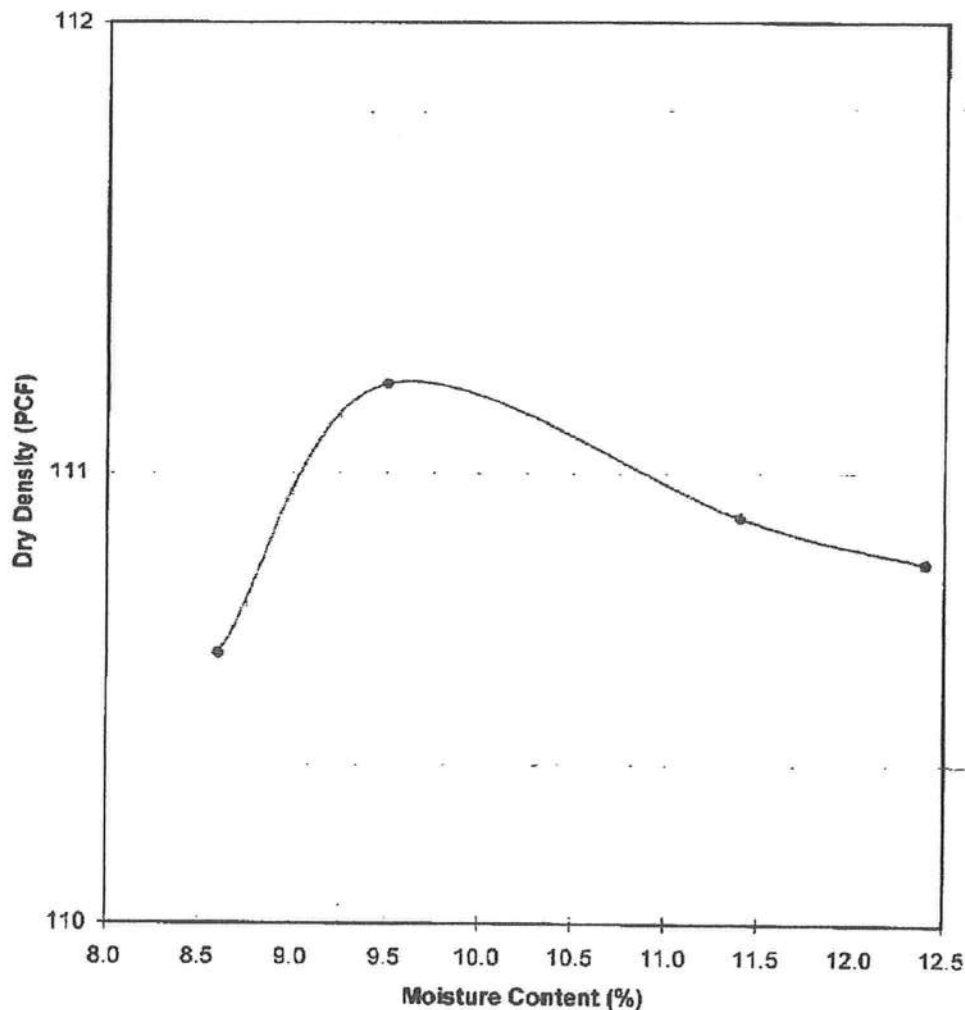
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4784 Roselle St., Jacksonville, FL 32254 • Tel(904)381-8901 • Fax(904)381-8902

REPORT OF LABORATORY COMPACTION TEST

Client:
Project Name:
Project Location:
Contractor:

Dale's Excavation, Inc. 6139 SW SR 47, Lake City, FL 32024
Louis & Elona Kisala Residence Lot 31 High Point Subdivision
Lake City, FL
Dale's Excavation, Inc.

File No: 09-00323-01
Date: 8/21/2009
Lab No: 12499



PROCTOR DATA

Proctor No.: 2

Modified Proctor ☒
(ASTM D-1557)

Standard Proctor ☐
(ASTM D-698)

Maximum Dry
Dens. Pcf: 111

Optimum Moisture
Percent: 10

The test results presented in this report are specific only to the samples tested at the time of testing. The tests were performed in accordance with generally accepted methods and standards. Since material conditions can vary between test locations and change with time, sound judgement should be exercised with regard to the use and interpretation of the data.

Sample Description:
Sample Location:
Proposed Use:
Sampled By:
Tested By:
Remarks:

Brown & Gray Sand
C.A. Boone Project Site
Building Fill

Louis Roberts Date: 8/13/2009
Melissa Ayers Date: 8/21/2009
1cc: Client
1cc: File

Linda Creamer, CEO, DBE

Linda M. Creamer
President - CEO

Reviewed By: *[Signature]*
Date: 8/24/09
Licensed, FL No: 57842

New Construction Subterranean Termite Service Record

OMB Approval No. 2502-0525
(exp. 02/29/2012)

This form is completed by the licensed Pest Control Company.

Public reporting burden for this collection of information is estimated to average 15 minutes per response, including the time for reviewing instructions, searching existing data sources, gathering and maintaining the data needed, and completing and reviewing the collection of information. This information is required to obtain benefits. HUD may not collect this information, and you are not required to complete this form, unless it displays a currently valid OMB control number.

Section 24 CFR 200.926d(b)(3) requires that the sites for HUD insured structures must be free of termite hazards. This information collection requires the builder to certify that an authorized Pest Control company performed all required treatment for termites, and that the builder guarantees the treated area against infestation for one year. Builders, pest control companies, mortgage lenders, homebuyers, and HUD as a record of treatment for specific homes will use the information collected. The information is not considered confidential, therefore, no assurance of confidentiality is provided.

This report is submitted for informational purposes to the builder on proposed (new) construction cases when treatment for prevention of subterranean termite infestation is specified by the builder, architect, or required by the lender, architect, FHA, or VA.

All contracts for services are between the Pest Control Company and builder, unless stated otherwise.

Section 1: General Information (Pest Control Company Information)

Company Name Agren Pest Control Inc.
Company Address P.O. Box 1755
Company Business License No. JB10076
FHA/VA Case No. (if any) _____

Section 2: Builder Information

Company Name Edgley Construction
Phone No. 752-0580

Section 3: Property Information

Location of Structure(s) Treated (Street Address or Legal Description, City, State and Zip) 353 NW High Pointe Drive Lake City, FL 32055
Louis and Elong Kisala

Section 4: Service Information

Date(s) of Service(s) 10-28-2009
Type of Construction (More than one box may be checked) ☒ Slab ☐ Basement ☐ Crawli ☐ Other _____

Check all that apply:

☐ A. Soil Applied Liquid Termiticide
Brand Name of Termiticide: Bifen XTS
EPA Registration No. 53883-189
Approx. Dilution (%): 6
Approx. Total Gallons Mix Applied: 630
Treatment completed on exterior: ☒ Yes ☐ No

☐ B. Wood Applied Liquid Termiticide
Brand Name of Termiticide: _____
EPA Registration No. _____
Approx. Dilution (%): _____
Approx. Total Gallons Mix Applied: _____

☐ C. Bait System Installed
Name of System: _____
EPA Registration No. _____
Number of Stations Installed: _____

☐ D. Physical Barrier System Installed
Name of System: _____
EPA Registration No. _____
Attach installation information (required) _____

Service Agreement Available? ☐ Yes ☐ No

Note: Some state laws require service agreements to be issued. This form does not preempt state law.

Attachments (List) _____

Comments _____

Name of Applicator(s) Cliff Lacey
Certification No. (if required by State law) JP104378

The applicator has used a product in accordance with the product label and state requirements. All materials and methods used comply with state and federal regulations.

Authorized Signature _____

Date 10-28-2009

Warning: HUD will prosecute false claims and statements. Conviction may result in criminal and/or civil penalties. (18 U.S.C. 1001, 1010, 1012; 31 U.S.C. 3729, 3802)

Form NPCA-99-B may still be used

Reorder Product #2581 From • CROWNMAX • 1-800-252-4011

Form HUD-NPMA-99-B

FD-
See notes on
back

COLUMBIA COUNTY OFFICE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 20-3S-16-02202-131

Building permit No. 000028074

Use Classification SFD/UTILITY

Fire: 19.26

Permit Holder DOUG EDGLEY

Waste: 50.25

Owner of Building LOUIS & ELONA KISALA

Total: 69.51

Location: 353 NW HIGH POINTE DRIVE

Date: 07/07/2010

Steve Lee

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)



*Sube on original dec 7.7.10
- Resd one - 8.17.10*

COLUMBIA COUNTY

OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 20-3S-16-02202-131 Building permit No. 000028074

Use Classification SFD, UTILITY Fire: 19.26

Permit Holder DOUG EDGLEY Waste: 50.25

Owner of Building LOUIS & ELONA KISALA Total: 69.51

Location: 363 NW HIGH POINTE DRIVE

Date: 07/07/2010 *Jacy New*



POST IN A CONSPICUOUS PLACE
(Business Places Only)

Building Inspector

COLUMBIA COUNTY OFFICE CALVIN

OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

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Parcel Number 20-3S-16-02202-131

Building permit No. 000028074

Use Classification SFD, UTILITY

Fire: 19.26

Permit Holder DOUG EDGLEY

Waste: 50.25

Owner of Building LOUIS & ELONA KISALA

Total: 69.51

Location: 353 NW HIGH POINTE DRIVE

Date: 06/17/2011

Shay Cur

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)



N.W. High Pointe Drive (Co. Right of way)

222.0

Set Back 30'

119

5' x 5'

84.2'

58.2'

421

71.8

16.1

12.2

47'

134

37.2'

26.5

8.24

23'

49

DRIVE

Set Back 25'

528.36

W. 411

52

Lot #31

LOUIS + ELONA
K15010

HL. HH 7
Set Back 25'

Set Back 25'

454.45