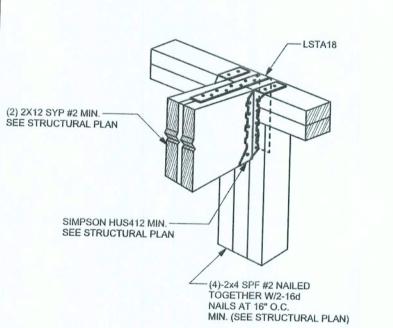


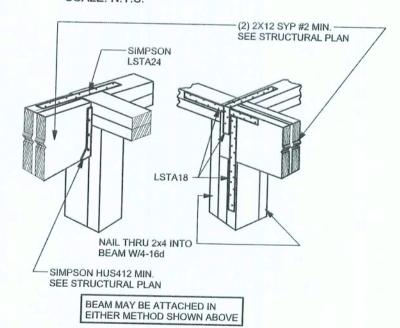
EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 11'-9" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 18'-10' STUD HEIGHT
(1) 2x6 @ 12" OC	TO 20.0' STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3,20B, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.



BEAM MID-WALL CONNECTION DETAIL SCALE: N.T.S.



BEAM CORNER CONNECTION. DETAIL



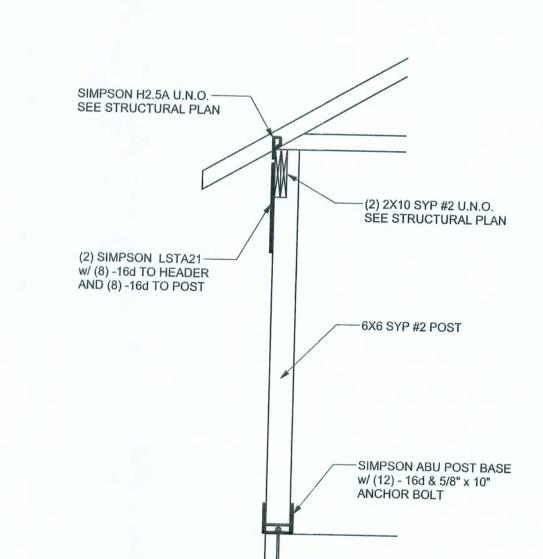
SUPPORTIVE .

2X4 LADDER BEAM

SUPPORTIVE CENTER POST TO BEAM)ETAIL

-3-1/2" P.T.

(2-ONE SIDE,2-ON OTHER SIDE)



MSTA30, 10-10d (1700lb) -(5) NAILS EACH SIDE OF STUD

(OR STRAP STUD TO HEADER 260-10d)

1/2" ANCHOR W/ 7" EMBEDMENT, SIMPSONN AT-

(MAY BE RECESSED BELOW FINISHED FLCOOR)

SCALE: 1/2" = 1'-0"

IF TRUSS TO WALL STRAPS ARE NAILED

TO THE HEADER THE SPH4/6 @ 48" O.C.

ARE NOT REQUIRED

(7) .131 x 3 1/4" GUN NAILS-

INTO KING STUD

TOE NAILED THRU HEADER

LTT20B, 10-16d (17/750lb)

ALTERNATE WALL TIE: CONNECTION WHERE THREADED ROD CANINOT BE PLACED IN WALL

FOR LESS THAN 1500 Ib UPLIFT USE

FOR LESS THAN 3750 Ib UPLIFT USE

-NAIL SHEATHING TO HEADER AND TOP PLATE WITH 8d AT 3" O.C. FOR UPLIFT

— SP4/6 @ 48" O.C. (U.N.O.) /----(7) .131 x 3 1/4" GUN NAILS

TOE NAILED THRU HEADER

INTO KING STUD

2 X 2 X 1/8" WASHER

3 X 3 X 1/8" WASHER

CRIPPLES IF REQUIRED

(5) .131 x 3 1/4" GUN NAILS

TOE NAILED THRU SILL-

INTO JACK STUD U.N.O.

TYPICAL STRAPPING (U.N.O.)

(SEE STRUCTURAL PLAN)

(1) 2×X6 SPF #2 SILL UP TO 7'-6" U.N.O.

(2) 2XX4 SPF #2 SILL UP TO 7'-8" U.N.O.

(1) 2XX4 SPF #2 SILL UP TO 5'-1" U.N.O. (FOR: 1220 MPH, 10'-0" WALL HEIGHT U.N.O.)

TYPICAL 1 STORY HEADER STRAPING DETAIL
SCALE: 1/2" = 1'-0"

-SEE FOOTING DETAILS



UPLIFT LBS. SY	P UPLIFT LBS. SP	F TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUS	SS TO STUDS
< 420	< 245	H5A	3-8d	3-8d	1031003
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	НЗ	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2'		
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2'		
< 1470	< 1265	H16-2	10-10d, 1 1/2"		
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	12-100 1 1/2	
< 2050	< 1785	LGT2	14 -16d	14 -16d	
		HEAVY GIRDER TIEDOWNS		14-16d	
		TIEAVI GIRDER HEDOWNS			TO FOUNDATION
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED RO 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED RO 12" EMBEDMENT
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED RO 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED ROI 12" EMBEDMENT
< 435		STUD STRAP CONNECTOR*			TO STUDS
< 455	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d
< 825	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d
< 885	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d
< 1240	< 760	SP4			6-10d, 1 1/2"
< 885	< 1065	SPH4			10-10d, 1 1/2"
< 1240	< 760	SP6			6-10d, 1 1/2"
< 1235	< 1065	SPH6			10-10d, 1 1/2"
	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
		STUD ANCHORS*	TO STUDS		TO FOUNDATION
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 - 16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		SIG IND
< 3335	< 3335	HPAHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		
< 2320	< 2320	ABU88	18 - 16d		1/2" AB 2-5/8" AB

GRADE & SPECIES TABLE

		Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2900	2.0
PSL	PARALAM	2900	2.0

UPLIFT LBS. SYP	UPLIFT LBS. SPI	F TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	70.07
< 420	< 245	H5A	3-8d	3-8d	TO STUDS
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	НЗ	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"		
< 1465	< 1050	H14-1	13-8d	5-10d, 1 1/2" 12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d		
< 990	< 850	H10-1	8-8d, 1 1/2"	12-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	8-8d, 1 1/2"	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	6-10d	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	2-10d, 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	7-10d 1 1/2"	
< 2900	< 2490	2 - HTS24	12-100 1 1/2	12-10d 1 1/2"	
< 2050	< 1785	LGT2	14 -16d	14 -16d	
		HEAVY GIRDER TIEDOWNS*		14-100	
2 2400		The second of th			TO FOUNDATION
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED ROI 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
425		STUD STRAP CONNECTOR*			TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d 1 1/2"

		OUTOEL OILL LEVIE	1-100	1	4 401	11451
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		4 -10d	ANG
< 825	< 600	DSP SINGLE SILL PLATE			8 -10d	ANC LES
< 885		DSF SINGLE SILL PLATE	2 -10d		8 -10d	
	< 760	SP4			6-10d, 1 1/2"	WAS 3/4"
< 1240	< 1065	SPH4		-		3/4
< 885	< 760	SP6			10-10d, 1 1/2"	NAIL
< 1240	< 1065	SPH6			6-10d, 1 1/2"	REP
< 1235	10 10 10				10-10d, 1 1/2"	m.
	< 1165	LSTA18	14-10d			BL
< 1235	< 1235	LSTA21	16-10d			
< 1030	< 1030	CS20	18-84			TI

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBC 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER ALL BEAKING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

GENERAL NOTES:

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: $6" \times 6" \times 1.4 \times 1.4$, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'. FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT.
FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD
DED THE MANUFACTURE DIS DECOMMENDATIONS. FIRED TO COMMEND AND THE PROPERTY OF PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 48 * DB (30" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"0C INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

(NCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

ASHERS: WASHERS USED WITH 1/2" BOLTS TO BE $2" \times 2" \times 9/64"$; WITH 5/8" BOLTS TO BE $3" \times 3" \times 9/64"$; WITH 4" BOLTS TO BE $3" \times 3" \times 9/64"$; WITH 4" BOLTS TO BE $3" \times 3" \times 5/16"$; UNO.

ILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST EPORTS AS HAVING EQUAL STRUCTURAL VALUES.

UILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK. CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE. PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBC 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES. PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

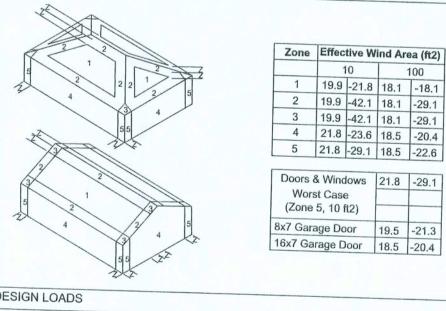
VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBC 2004, SECTION 1609 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN RUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2004 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED

DESIGN DATA

ME ON	ND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1 NCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; EAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT IN UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% OPE AND UNOBSTRUCTED LIPWIND FOR 50% HIS EXP. B, 30FT IN EXP. C AND >10%
	- THE STAND FOR SUX HEIGHT OR 1 MILE WHICHEVER IS I ESS
BUI	ILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE
BUI	ILDING IS NOT IN THE WIND-BORNE DEBRIS REGION
1.)	
2.)	WIND EXPOSURE = B
3.)	WIND IMPORTANCE FACTOR = 1.0
4.)	BUILDING CATEGORY = II
5.)	ROOF ANGLE = 10-45 DEGREES
6.)	MEAN ROOF HEIGHT = <30 FT
7.)	INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)
8.)	COMPONENTS AND CLADDING DESIGN WIND PRESSURES [TABLE R301.2(2)]
	24
	Zone Effective Wind Area (ft2)



	2XX
DESIGN	LOADS
FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)
	30 PSF (SLEEPING ROOMS)
	20 PSF (ATTICS WITH STORAGE)
	10 PSF (ATTICS WITHOUT STORAGE, <3:1
	40 PSF (DECKS)
	60 PSF (EXTERIOR BALCONIES)
ROOF	20 DOE /FLAT OD 110

ROOF 20 PSF (FLAT OR <4:12) 16 PSF (4:12 TO <12:12) 12 PSF (12:12 AND GREATER)

STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS) SOIL BEARING CAPACITY 1000PSF

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

27 / Apr / 06 604213 DRAWNG NUMBER

> 5-1 OF6 SHEETS

REVISIONS

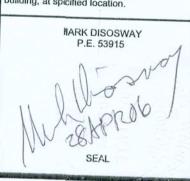
SOFTPLAN

WINDLOAD :NGINEER: Mark Disosway, PE No.53915 POB 868, Lake City, FL 32056, 386-754-5419 IMENSION: stated dimensions supercede scaled

dimensions. Refer all questions to Mark Disoswy, P.E. for resolution. Do not proceed without clarification. COPYRIGHT; AND PROPERTY RIGHTS: Mark Disosway, P.E. hereby expressly reserve its common law copyrights and property right in these instruments of service. This document is not to be repriduced, altered or copied in any form or mannir without first the express written

permission and consent of Mark Disosway. CERTIFICATION: I hereby certify that I have examined thisolan, and that the applicable portions of theplan, relating to wind engineer comply with section R301.2.1, florida building code residential 2004, to the best of my

LIMITATION: his design is valid for one building, at specified location.



RICHARD KEEN

SPEC HOUSE

ADDRESS: Lot 12 Century Oaks S/D Columlia County, Florida

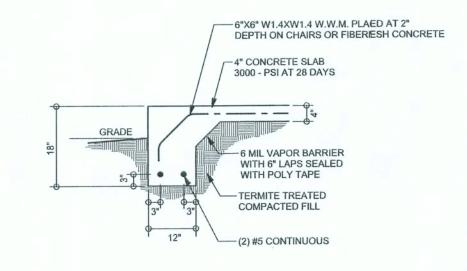
Mark Disosway P.E. P.0. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (336) 269 - 4871 PRINTED DATE:

April28, 2006 DRAWN BY: STRUCTURAL BY Ben Sparks Ben Sparks

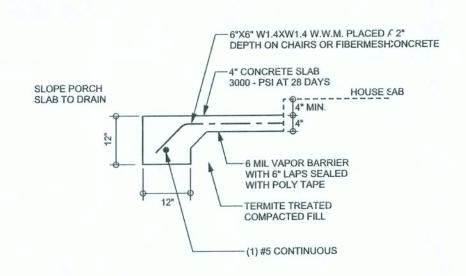
FINALS DATE JOB NUMBER:

REVISIONS

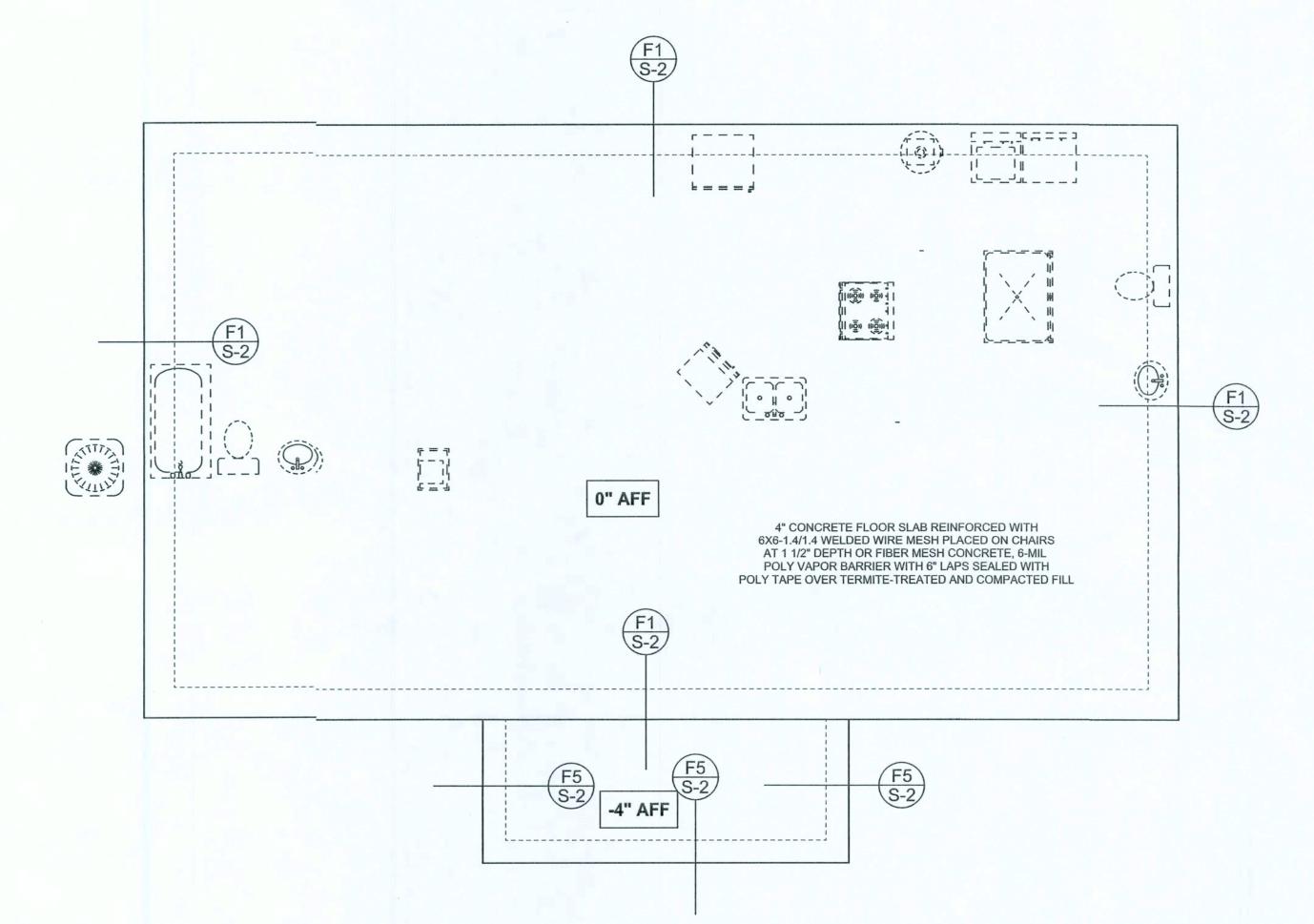
SOFTPI ACHITECTURAL DESIGN SOFTWARE



F1 MONOLITHIC FOOTING S-2 SCALE: 1/2" = 1'-0"



F5 PORCH FOOTING
S-2 SCALE: 1/2" = 1'-0"



FOUNDATION PLAN SCALE: 1/4" = 1'-0" DIMENSIONS ON STRUCTURAL SHEETS
ARE NOT EXACT. REFER TO ARCHITECTURAL
FLOOR PLAN FOR ACTUAL DIMENSIONS WINDLOAL ENGINEER: Mark Disosway, PE No.539'5, POB 868, Lake City, FL 32056, 386754-5419

dimensions Refer all questions to Mark Disosvay, P.E. for resolution. Do not proced without clarification.

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these instruments of service. This document is
not to be reproduced, altered or copied in any
form or mainer without first the express written
permissionand consent of Mark Disosway. CERTIFIC/FION: I hereby certify that I have examined tils plan, and that the applicable portions of he plan, relating to wind engineering comply with section R301.2.1, florida building code residential 2004, to the best of my

LIMITATIOI: This design is valid for one building, at specified location. MARK DISOSWAY P.E. 53915

RICHARD KEEN

SPEC HOUSE

ADDRESS: Lo 12 Century Oaks S/D Coumbia County, Florida

Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phore: (386) 754 - 5419 Fax: (386) 269 - 4871

> PRINTED DATE: April 28, 2006

DRAWI BY: STRUCTURAL BY Ben Spirks Ben Sparks

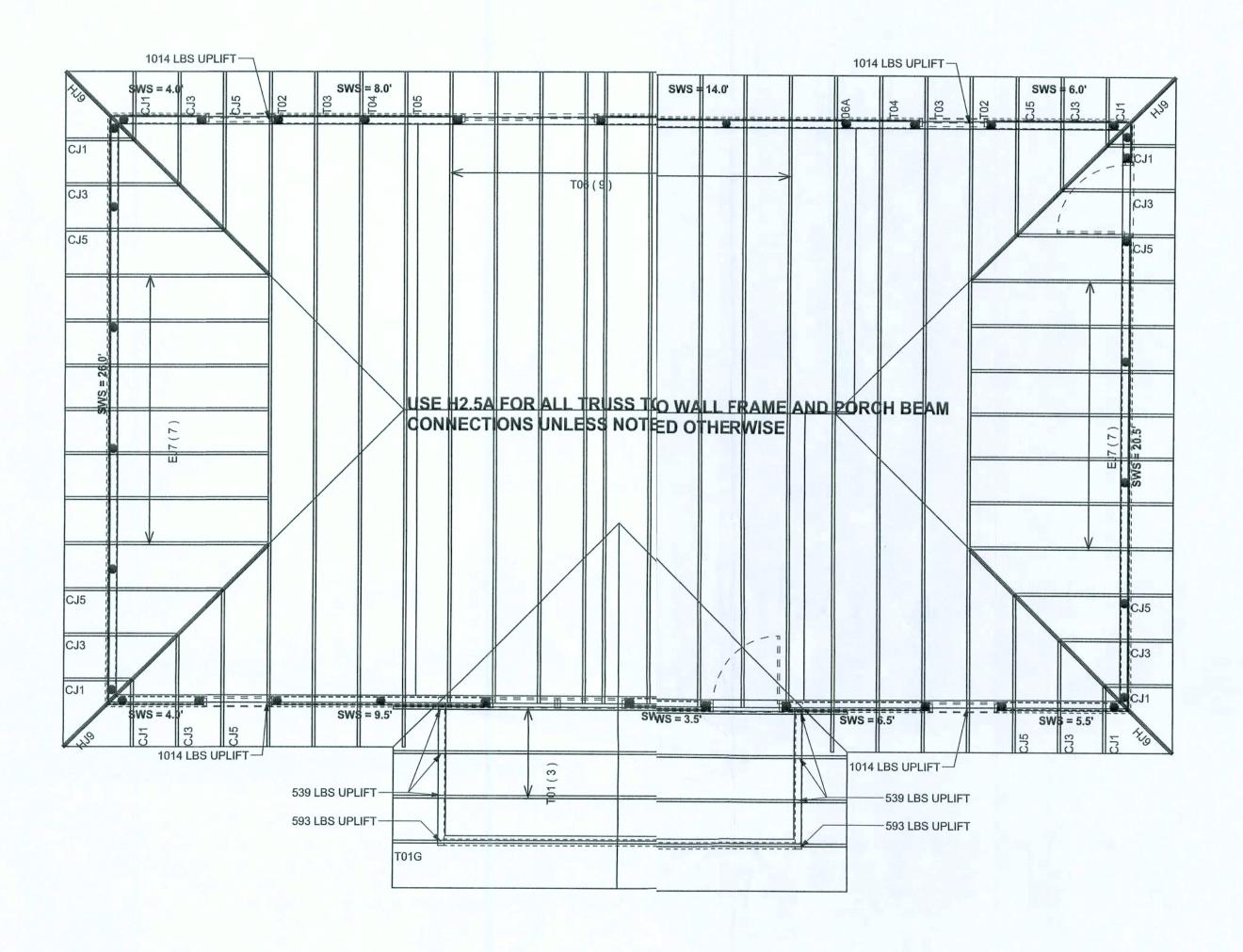
FINALSDATE: 27 / Apr / 06

JDB NUMBER: 604213 **PRAWING NUMBER**

> **S-2** OF 6 SHEETS

REVISIONS

SCFTPIAN



STRUCTURAL PLAN
SCALE: 1/4" = 1'-0"

STRUCTURAL PLAN NOTES

SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL E A MINIMUM OF (2) 2X12 SYP#2 (U.N.O.)

SN-2 ALL LOD BEARING FRAME WALL HEADERS SHALL AVE (1) JACK STUD & (1) KING STUD EACH SDE (U.N.O.)

SN-3 DIMENSONS ON STRUCTURAL SHEETS ARE NO EXACT. REFER TO ARCHITECTURAL FLOOR LAN FOR ACTUAL DIMENSIONS

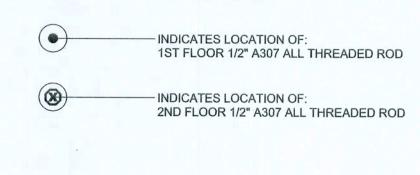
PERMARNT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS.

LATERAIBRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FUNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS FACKAGE

WALL LEGEND

SWS = 0.0'	1ST FLOOR EXTERIOR WALL
SWS = 0.0'	2ND FLOOR EXTERIOR
IBW	1ST FLOOR INTERIOR BEARING VWALLS SEE DETAILS ON SHEET S-1
IBW	2ND FLOOR INTERIOR BEARING VWALLS SEE DETAILS ON SHEET S-1

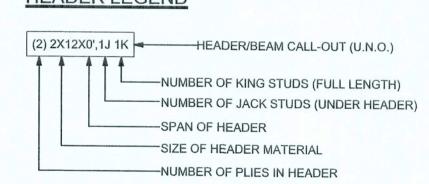
THREADED ROD LEGEND



TOTAL SHEAR WALL SEGMENTS SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	35.0'	46.5'
LONGITUDINAL	15.0'	61.0'

HEADER LEGEND



CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. BUILDERS FIRST SOURCE (JOB #L161233) WINDLOAD EN(INEER: Mark Disosway, PE No.53915, P0B 868, Lake City, FL 32056, 386-754-i419

DIMENSIONS:

Stated dimensions supercede scaled dimensions. Refir all questions to Mark Disosway, 1.E. for resolution. Do not proceed vithout clarification.

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permission and onsent of Mark Disosway.

CERTIFICATION I hereby certify that I have

CERTIFICATION I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, florida building code residential 2004, to the best of my knowledge.

code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY
P.E. 53915

RICHARD KEEN

SPEC HOUSE

ADDRESS: Lot 12 Century Oaks S/D Columba County, Florida

Mark Disosway P.E. P.0. Box 868 Lake Cit/, Florida 32056 Phone: (386) 754 - 5419 Fax: (336) 269 - 4871

PRNTED DATE:
April28, 2006

DRAWN BY: STRUCTURAL BY:
Ben Sparks Ben Sparks

FINALS DATE

JOBNUMBER: 604213 DRAVING NUMBER

> S-3 OF 6 SHEETS