

REVISIONS	

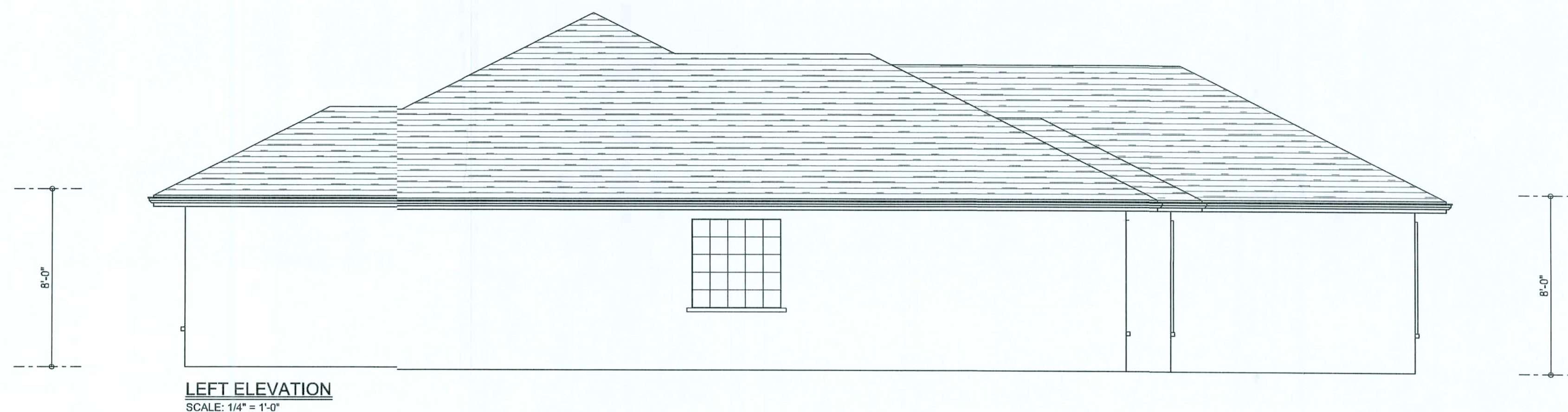
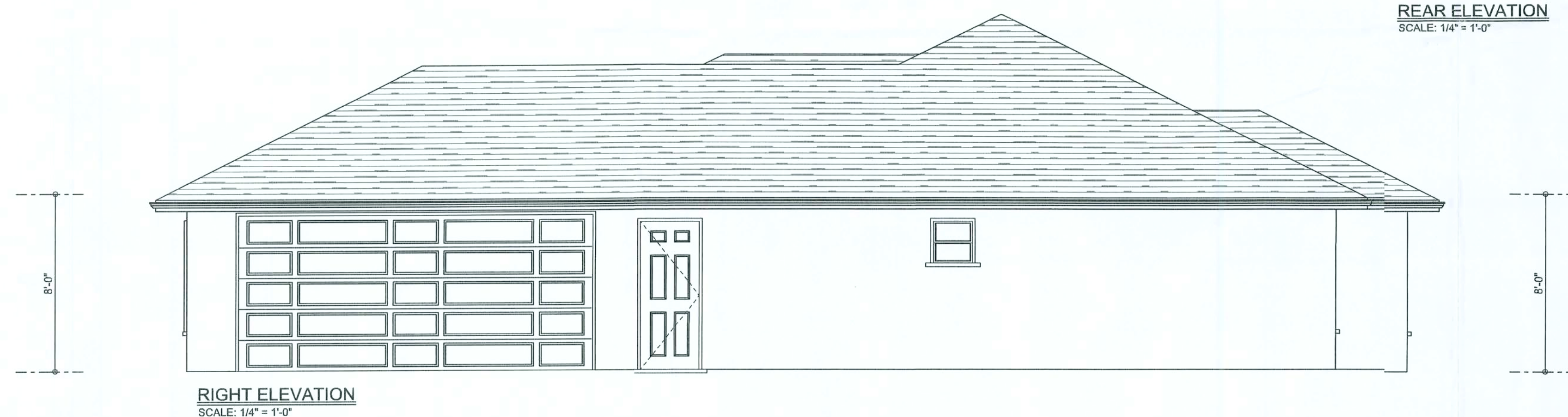
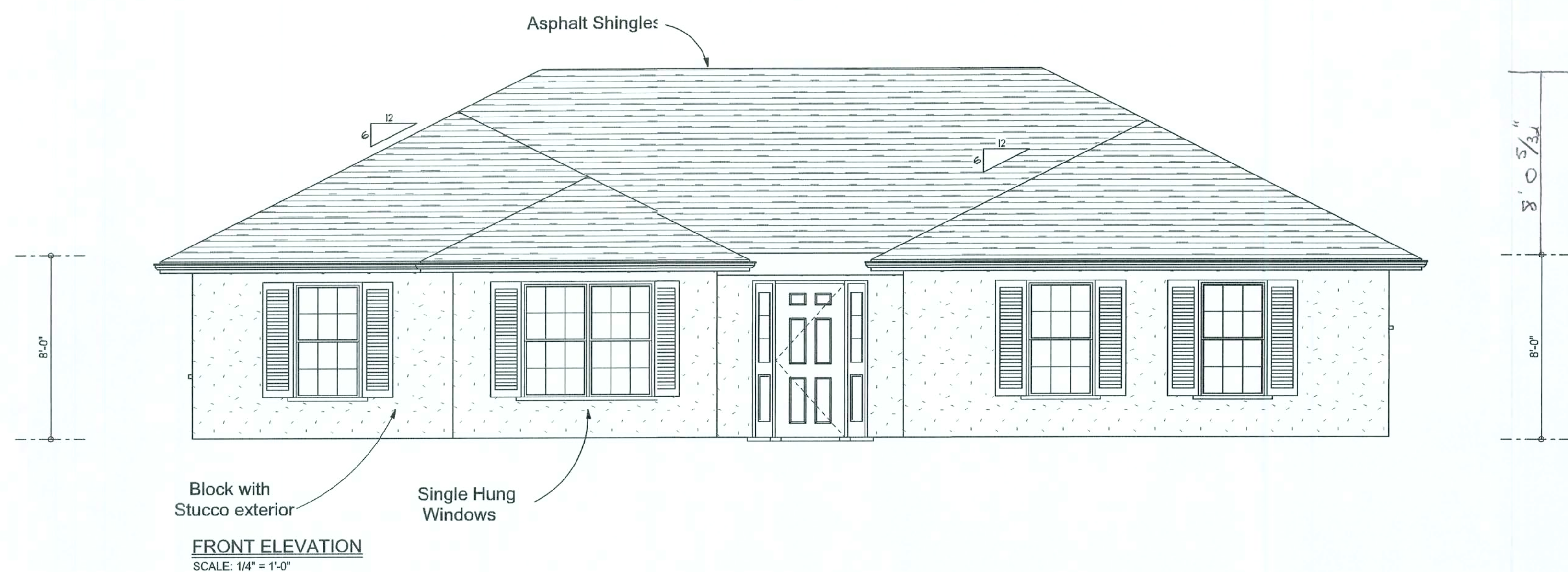
SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

Notes:

R-1 All roof pitches shall be 6/12 unless otherwise noted.

R-2 All overhangs shall be 24" except on gables 18".

R-3 Provide attic ventilation in accordance with code requirements (1/300th insulated attic).



ROBINSON RESIDENCE

ADDRESS:
168 SW Tara Ct.,
Ft. White FL 32038

Woodman Park
Builders, Inc.
Lake City, Florida
Phone: (386) 755-8699
Fax: (386) 755-8684

PRINTED DATE:
January 7, 2006

DRAWN BY:
Mark Haddock

CHECKED BY:
Mark Haddock

DESIGNED BY:
Mark Haddock

FINALS DATE:
16 / Jan / 06

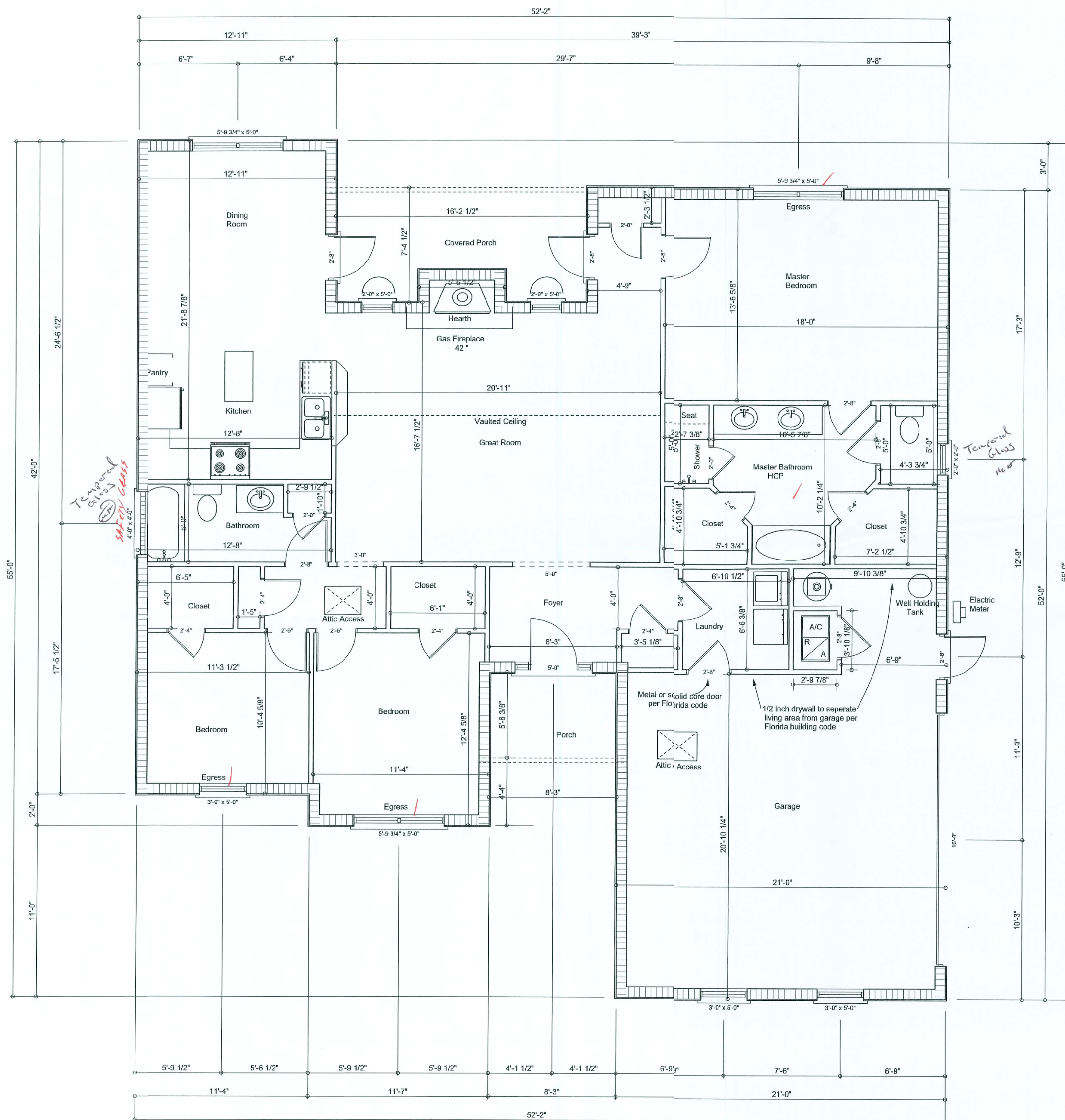
JOB NUMBER:
512061

DRAWINGNUMBER

A-1

OF 3 SHEETS

Office copy



Notes:

- R-1 All ceiling heights shall be 8' unless otherwise noted.
R-2 Pt 4x4 post shall be used on front and rear porch for a beam to slab connection and a decorative cover placed over post.
R-3 Plumbers shall exhaust the dryer vent to the outside.

AREA SUMMARY

Living Area	1706	S.F.
Garage Area	514	S.F.
Porch Area	183	S.F.
Total Area	2403	S.F.

FLOOR PLAN
SCALE: 1/4" = 1'-0"

Well

Consult contractor or homeowner for exact location for well and septic inlet and outlet.

ROBINSON
RESILIENCE

ADDRESS:
168 SW Tara Ct.
Ft. White FL 32038

Woodman Park
Builders, Inc.
Lake City, Florida
Phone: (386) 755-8699
Fax: (386) 755-8684

PRINTED DATE:
January 7, 2006

DRAWN BY: Mark Haddock
CHECKED BY:

DESIGNED BY:
Mark Haddock

FINALES DATE:
16 / Jan / 06

JOB NUMBER:
512061

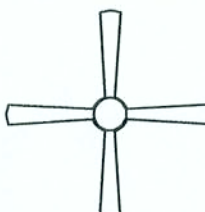
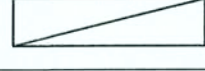
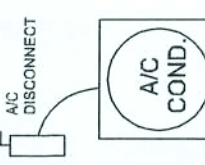
DRAWING NUMBER

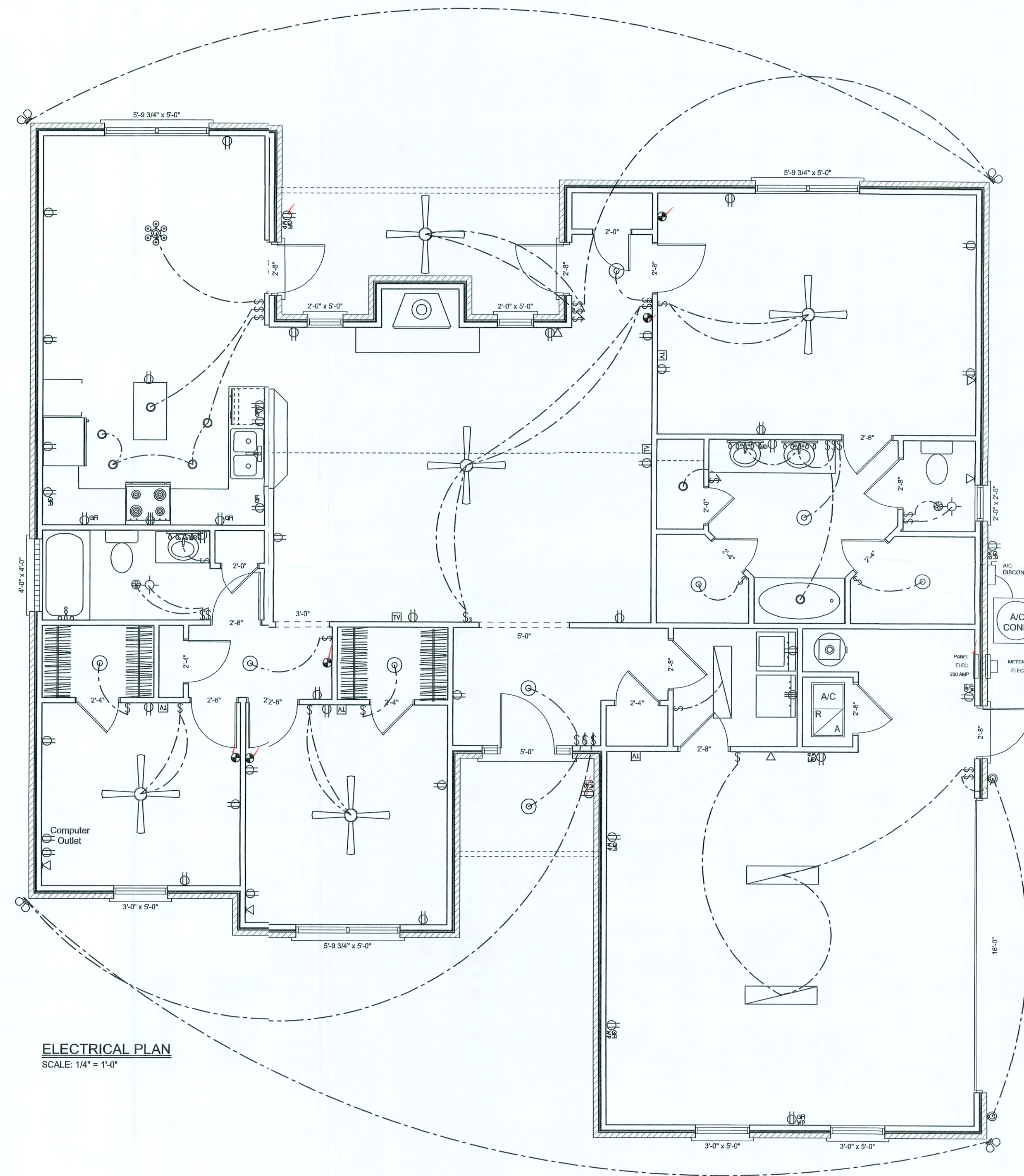
A-2

OF 3 SHEETS

REVISIONS

SOFTPLAN
ARCHITECTURAL DESIGN AND MORE

ELECTRICAL	COUNT	SYMBOL
ceiling fan globe 1	5	
ceiling globe light	9	
chandelier	1	
double spotlight	4	
fluorescent fixture	3	
pot light	7	
vanity bar light	3	
wall mount 1	2	
AC Disconnect	1	
Outlet WP GFI	8	
PANEL METER	1	
cable tv outlet	6	
fan	2	
light	2	
outlet	32	
outlet 220v	2	
outlet gfi	7	
smoke detector	5	
switch	31	
switch 3 way	2	
telephone	7	



ELECTRICAL PLAN
SCALE: 1/4" = 1'-0"

Electrical Plan Notes:

- E-1 Wire all appliances, HVAC units and other equipment per manufactures specifications.
- E-2 Consult the owner for the number or separte telephone lines to be installed. Owner is responsible for all overages not noted on plan.
- E-3 All installations shall be per national code.
- E-4 All smoke detectors shall be 120v with battery back-up of the photoelectric type, and shall be interlocked together. Install inside and near all bedrooms.
- E-5 Telephone, television and other low voltage devices or outlets shall be as per the owners directions and in accordance with applicable sections of the National Electric Codes latest edition. Owner is responsible for all overages not noted on plan.
- E-6 Electrical contractor shall be responsible for the design and sizing of electrical service and circuits.
- E-7 Entry of service (underground or overhead) to be determined by contractor agreement.
- E-8 All bedroom receptacles shall be AFCI (arc fault circuit interrupter).
- E-9 All outlets to be located above base flood elevation.
- E-10 All exterior GFI outlets shall be weatherproof.

Overcurrent protection device shall be installed on the exterior of structures to serve as a disconnecting means. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground.

ROBINSON RESIDENCE

ADDRESS:
168 SW Tina Ct.
Ft. White, FL 32038

Woodman Park
Builders, Inc.
Lake City, Florida
Phone: (386) 755-8699
Fax: (386) 755-8684

PRINTED DATE:
January 17 2006

DRAWN BY: Mark Haddox

CHECKED BY:

DESIGNED BY:
Mark Haddox

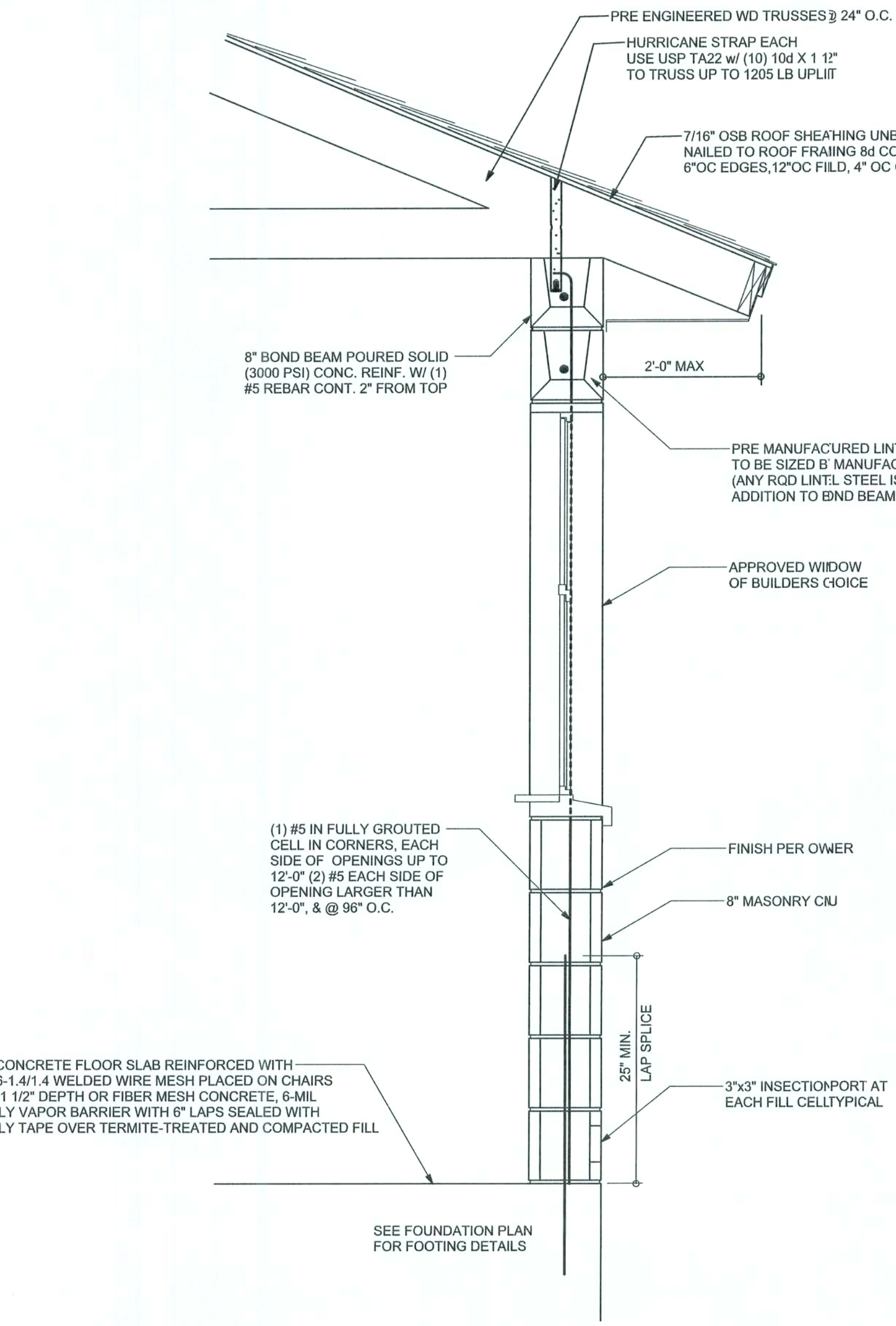
FINALES DATE:
16 / Jan / 06

JOB NUMBER:
51261

DRAWING NUMBER

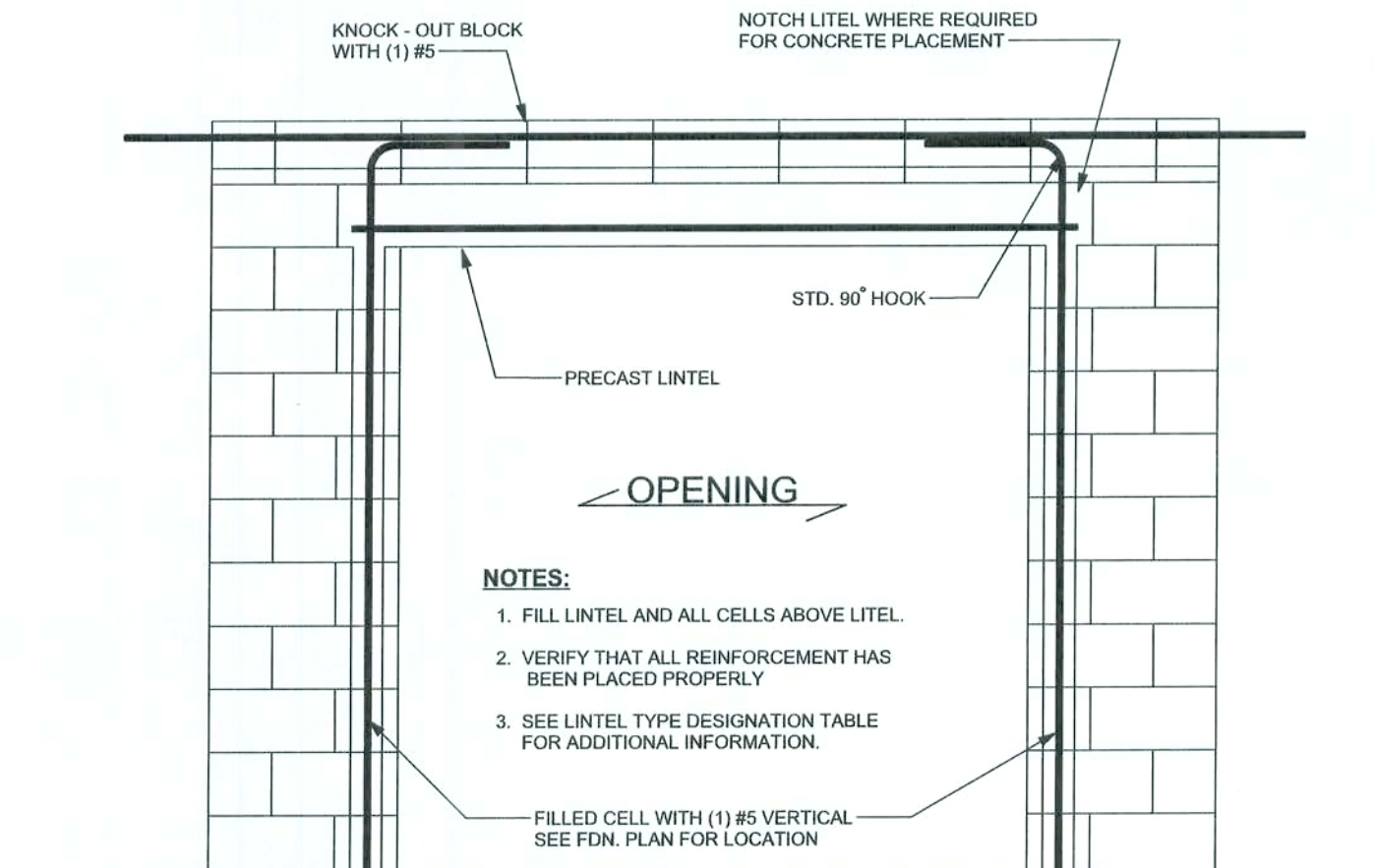
A-3

OF 3 SHEETS

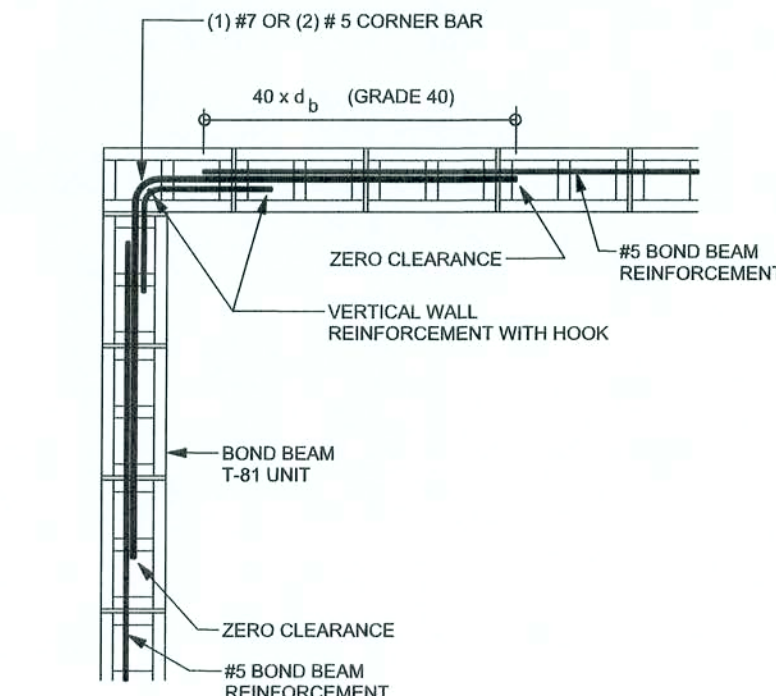


TYPICAL 1 STORY BLOCK WALL SECTION
SCALE: 1" = 1'-0"

BEAM TO BLOCK CONNECTION DETAIL
SCALE: 1/2" = 1'-0"



TYPICAL FILLED LINTEL ASSEMBLY
SCALE: 1/2" = 1'-0"



TYPICAL BOND BEAM CORNER DETAIL
SCALE: 1/2" = 1'-0"

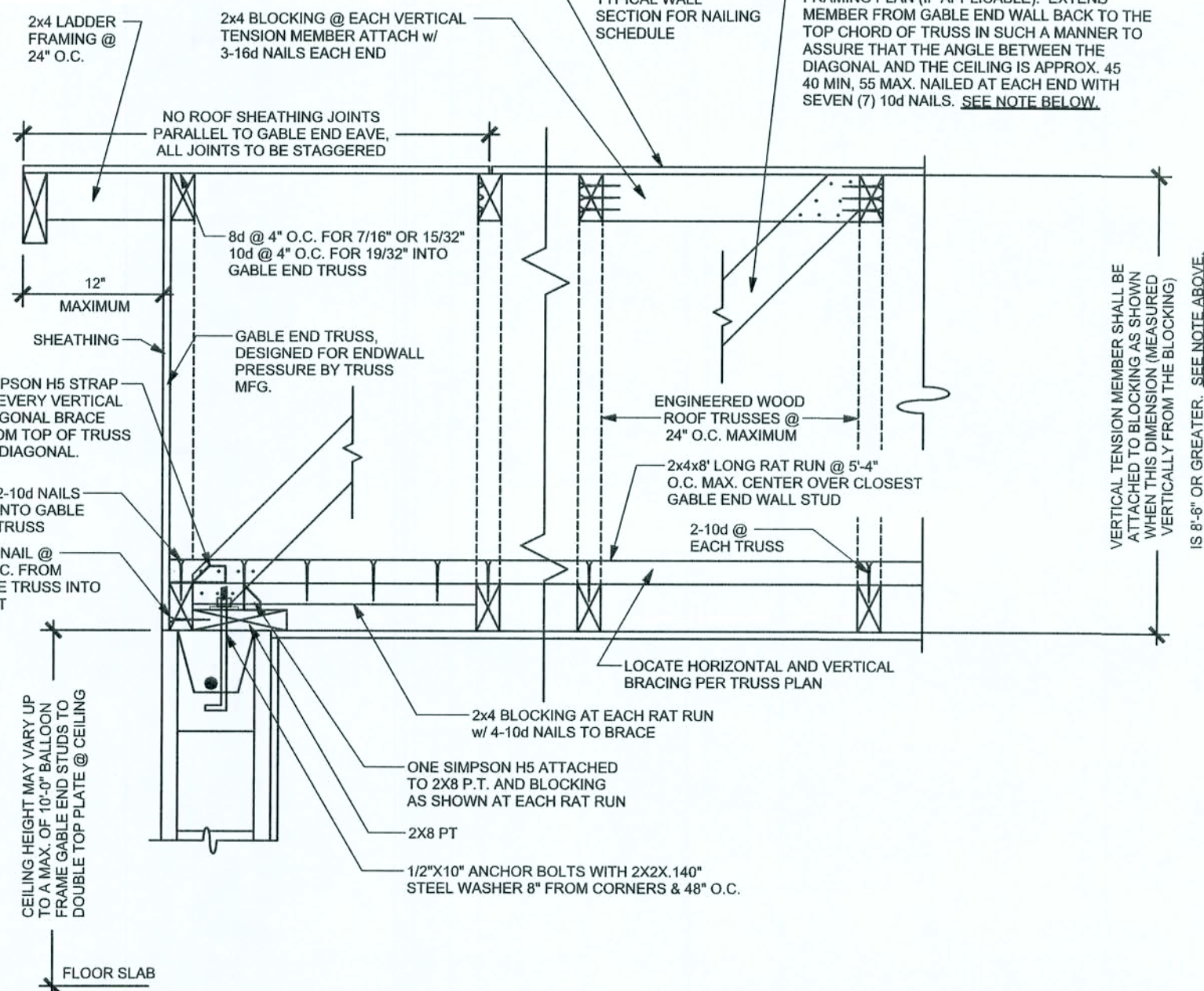
MASONRY TRUSS ANCHOR TABLE
OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS.	TRUSS CONNECTOR MASONRY *	
< 1205	TA22	10-10d x 1 1/2"
< 1605	TA22	11-10d
< 860	MTSM20	4 - 1/4"x2 1/4" TITEN IN BLOCK 7 - 10d IN TRUSS
< 1175	HTSM20	4 - 1/4"x2 1/4" TITEN IN BLOCK 10 - 10d IN TRUSS
< 1040	META20	7-10d, 1 1/2"
< 1490	META20	10-10d, 1 1/2"
< 1780	HETA20	7-16d
< 1780	LGT2	7 - 1/4"x2 1/4" TITEN IN BLOCK 14 - 16d SINKER IN GIRDER
< 2130	HHETA20	17-10d, 1 1/2"
< 2310	HHETA24	21-10d, 1 1/2"
< 3965	MGT	22-10d TO TRUSS 5/8 AB TO WALL 15" EMBEDMENT
< 10980	HGT-2	16-10d TO TRUSS (2) 3/4 AB TO WALL 15" EMBEDMENT
< 10530	HGT-3	16-10d TO TRUSS (2) 3/4 AB TO WALL 15" EMBEDMENT

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-6d	3-6d	
< 455	< 265	H5	4-6d	4-6d	
< 360	< 235	H4	4-6d	4-6d	
< 455	< 320	H3	4-6d	4-6d	
< 415	< 365	H2.5	5-6d	5-6d	
< 600	< 535	H2.5A	5-6d	5-6d	
< 950	< 820	H6	8-6d	8-6d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-6d	12-6d, 1 1/2"	
< 1465	< 1050	H14-2	15-6d	12-6d, 1 1/2"	
< 990	< 850	H10-1	8-6d, 1 1/2"	8-6d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LGT2	14 - 16d	14 - 16d	
HEAVY GIRDER TIEDOWNS*					
< 3965	< 3330	MGT		22 - 10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 - 10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16 - 10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 - 10d	2-5/8" THREADED ROD 12" EMBEDMENT
STUD STRAP CONNECTOR*					
< 435	< 435	SSP DOUBLE TOP PLATE	3 - 10d		4 - 10d
< 455	< 460	SSP SINGLE SILL PLATE	1 - 10d		4 - 10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 - 10d		8 - 10d
< 825	< 690	DSP SINGLE SILL PLATE	2 - 10d		8 - 10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14 - 10d		
< 1235	< 1235	LSTA21	16 - 10d		
< 1030	< 1030	CS20	18 - 6d		
< 1705	< 1705	CS16	28 - 6d		
STUD ANCHORS*					
< 1350	< 1305	LTT19	8 - 16d		1/2" AB
< 2310	< 2310	LTT131	18 - 10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 - 16d		5/8" AB
< 1400	< 1400	PAHD42	16 - 16d		
< 3335	< 3335	HPAHD22	16 - 16d		
< 2200	< 2200	ABU44	12 - 16d		1/2" AB
< 2300	< 2300	ABU66	12 - 16d		1/2" AB
< 2320	< 2320	ABU88	18 - 16d		2-5/8" AB



GABLE END DETAIL

MASONRY NOTES:

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 8/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

	ACI530.1-02 Section	Specific Requirements
1.4A	Compressive strength	8" block bearing walls Fm = 1500 psi
2.1	Mortar	ASTM C 270, Type N, UNO
2.2	Grout	ASTM C 476, admixtures require approval
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 60, Fy = 60 ksi, Lap splices min 48 bar dia. (30" for #5)
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class 060, 0.60 oz/lb or 30MSS
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/lb or 304SS
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

GRADE & SPECIES TABLE

		Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2900	2.0
PSL	PARALAM	2900	2.0

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDERS RESPONSIBILITY TO VERIFY THE TRUSS ENGINEERING FULLY SATISFIES ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END, 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE)

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, Fc = 3000 PSI

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 65KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.R.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1 1/2 INCH TO 2 INCHES. DOSEAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, Fy = 60 KSI. ALL LAP SPICES 48" DB (30" FOR #5 BARS). UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 318-08, U.N.O.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY, 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64", WITH 5/8" BOLTS TO BE 3" x 3" x 9/64", WITH 3/4" BOLTS TO BE 2" x 3" x 9/64", WITH 7/8" BOLTS TO BE 3" x 3" x 5/16", UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMTS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R302.1.2 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2004 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

DESIGN DATA

WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1

(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE
BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

- 1.) BASIC WIND SPEED = 110 MPH
- 2.) WIND EXPOSURE = B
- 3.) WIND IMPORTANCE FACTOR = 1.0
- 4.) BUILDING CATEGORY = II
- 5.) ROOF ANGLE = 10-45 DEGREES
- 6.) MEAN ROOF HEIGHT = <30 FT
- 7.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)
- 8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))

Zone	Effective Wind Area (ft ²)	10	100
1	19.9	21.8	18.1
2	19.9	25.5	18.1
2.0th	-40.6	-40.6	-40.6
3	19.9	25.5	18.1
3.0th	-68.3	-68.3	-42.4
4	21.8	23.6	18.5
5	21.8	29.1	18.5
Doors & Windows Worst Case (Zone 5, 10 ft ²)	21.8	-29.1	
8x7 Garage Door	19.5	-22.9	
16x7 Garage Door	18.5	-21.0	

DESIGN LOADS

FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)
	30 PSF (SLEEPING ROOMS)
	30 PSF (ATTICS WITH STORAGE)
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)
ROOF	20 PSF (FLAT OR <4:12)
	16 PSF (4:12 TO <12:12)
	12 PSF (12:12 AND GREATER)
STAIRS	40 PSF (ONE & TWO FAMILY DWELLINGS)
SOIL BEARING CAPACITY	1000PSF
NOT IN FLOOD ZONE (BUILDER TO VERIFY)	

REVISIONS

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Disoway,
PE No.53915, F026 BB, Lake City, FL
32056, 386-754-5419

DIMENSIONS:
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. by resolution. Do not proceed without clarification.

COPYRIGHTS AND PROPERTY RIGHTS:
Mark Disoway, P.E. hereby expressly reserves its common law copyrights and property right in these instruments of service. This document is not to be reproduced, altered or copied in any form or manner without first the express written permission and consent of Mark Disoway.

CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R601.2.1, Florida building code residential 2004 to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOWAY
P.E. 53915

Mark Disoway
10/5/2005
EAL

Robinson Residence

ADDRESS:
168 SV Tara Ct.,
Ft. White, FL 32038

Mark Disoway P.E.
P.O. Box 868
Lake City, Florida 32056
Phone: (386) 754 - 5419
Fax: (386) 269 - 4871

PRINTED DATE:
January 16, 2006
DRAWN BY: STRUCTURAL BY:
David Disoway

FINALS DATE:
16 / Jan / 06

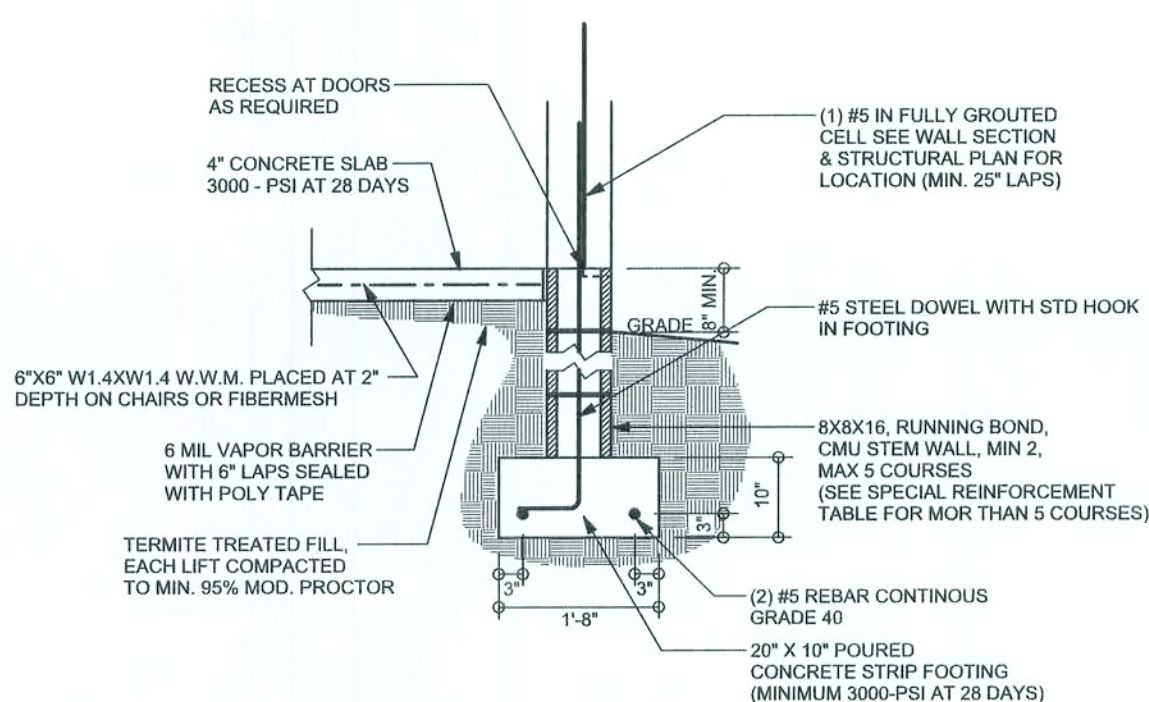
JOB NUMBER:
512061
DRAWING NUMBER

S-1

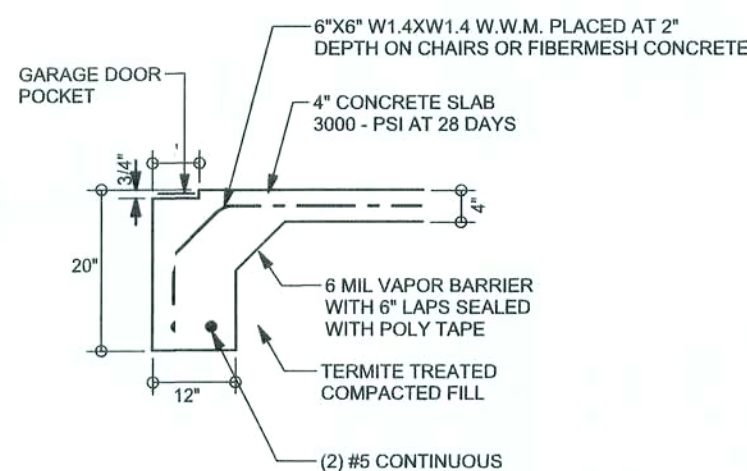
OF 3 SHEETS

REVISIONS

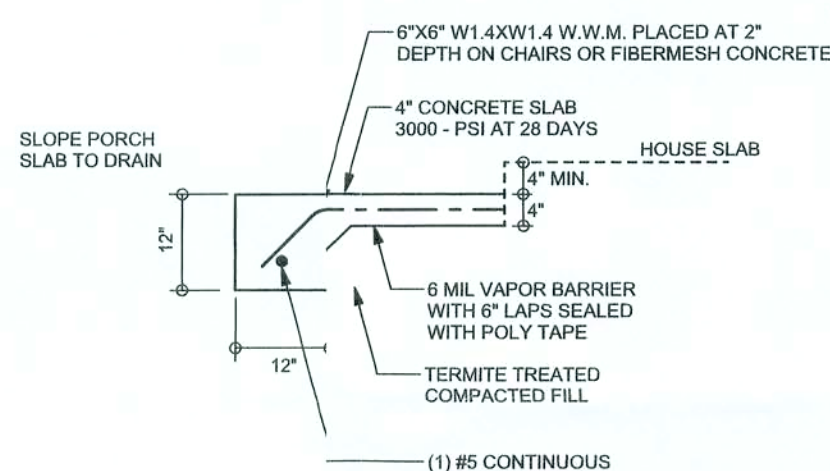
SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE



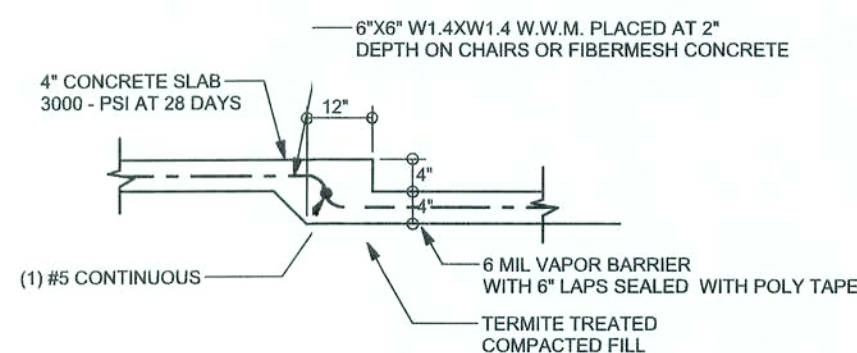
F17 S-2 STEM WALL FOOTING
SCALE: 1/2" = 1'-0"



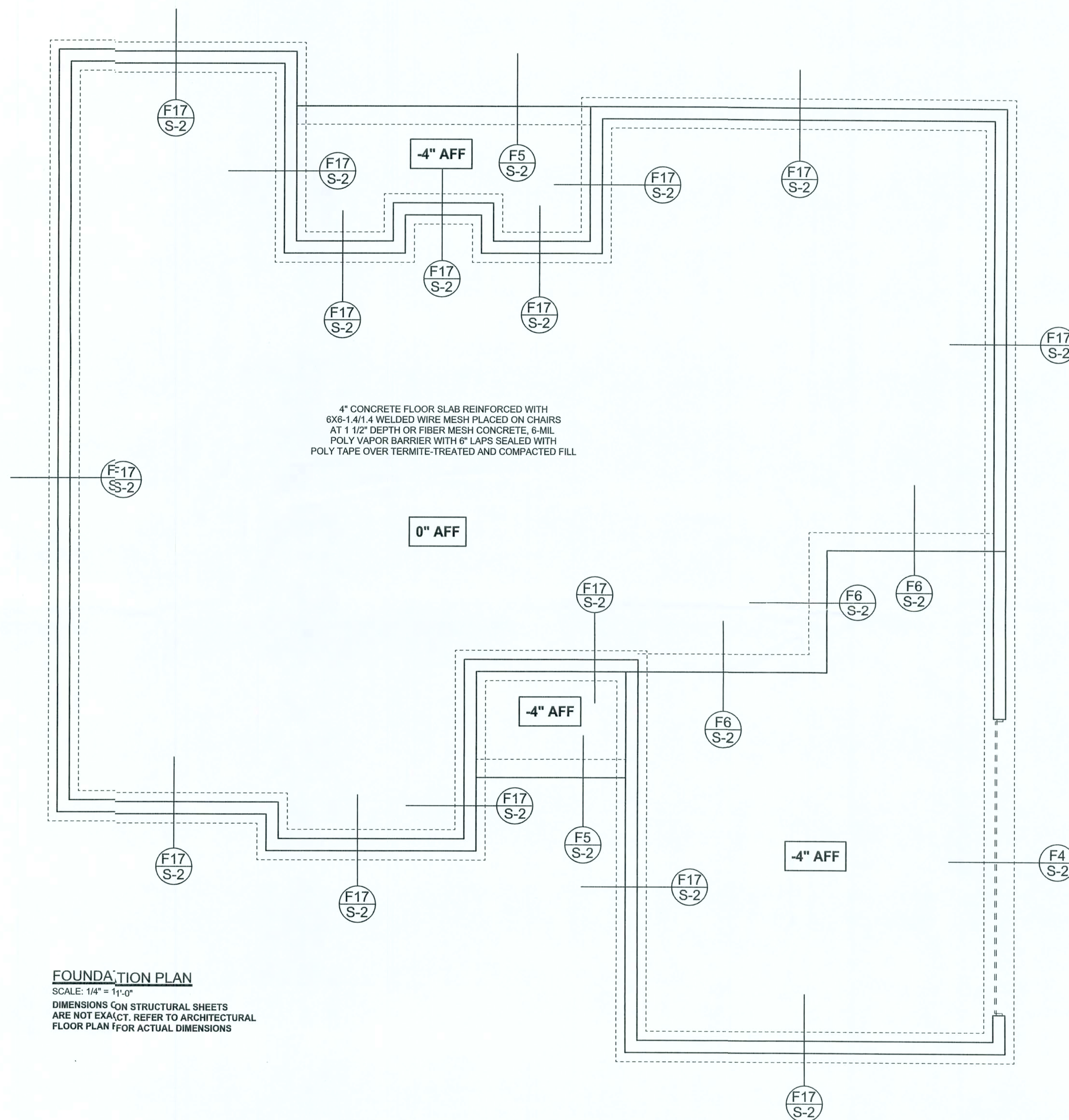
F4 S-2 GARAGE DOOR FOOTING
SCALE: 1/2" = 1'-0"



F5 S-2 PORCH FOOTING
SCALE: 1/2" = 1'-0"



F6 S-2 TYPICAL NON-BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"



FOUNDATION PLAN

SCALE: 1/4" = 1'-0"
DIMENSIONS ON STRUCTURAL SHEETS
ARE NOT EXACT. REFER TO ARCHITECTURAL
FLOOR PLAN FOR ACTUAL DIMENSIONS

WINDLOAD ENGINEER: Mark Disoway,
P.E. No. 53915, P.O. Box 868, Lake City, FL
32056, 386-754-5419

DIMENSIONS:
Stated dimensions supersede scaled
dimensions. Refer all questions to
Mark Disoway, P.E. for resolution.
Do not proceed without clarification.

COPYRIGHTS AND PROPERTY RIGHTS:
Mark Disoway, P.E. hereby expressly reserves
its common law copyright and property right in
these instruments of service. This document is
not to be reproduced, altered or copied in any
form or manner without the express written
permission and consent of Mark Disoway.

CERTIFICATION: I hereby certify that I have
examined this plan, and that the applicable
portions of the plan, relating to wind engineering
comply with section R3012.1, Florida building
code residential 2004, to the best of my
knowledge.

LIMITATION: This design is valid for one
building, at specified location.

MARK DISOWAY
P.E. 53915

Mark Disoway
16 Jan 05
SEA

Robinson Residence

ADDRESS:
168 SW Tara Ct.,
Ft. White, FL 32038

Mark Disoway P.E.
P.O. Box 868
Lake City, Florida 32056
Phone: (386) 754 - 5419
Fax: (386) 269 - 4871

PRINTED DATE:
January 16 2006

DRAWN BY: STRUCTURAL BY:
David Disoway

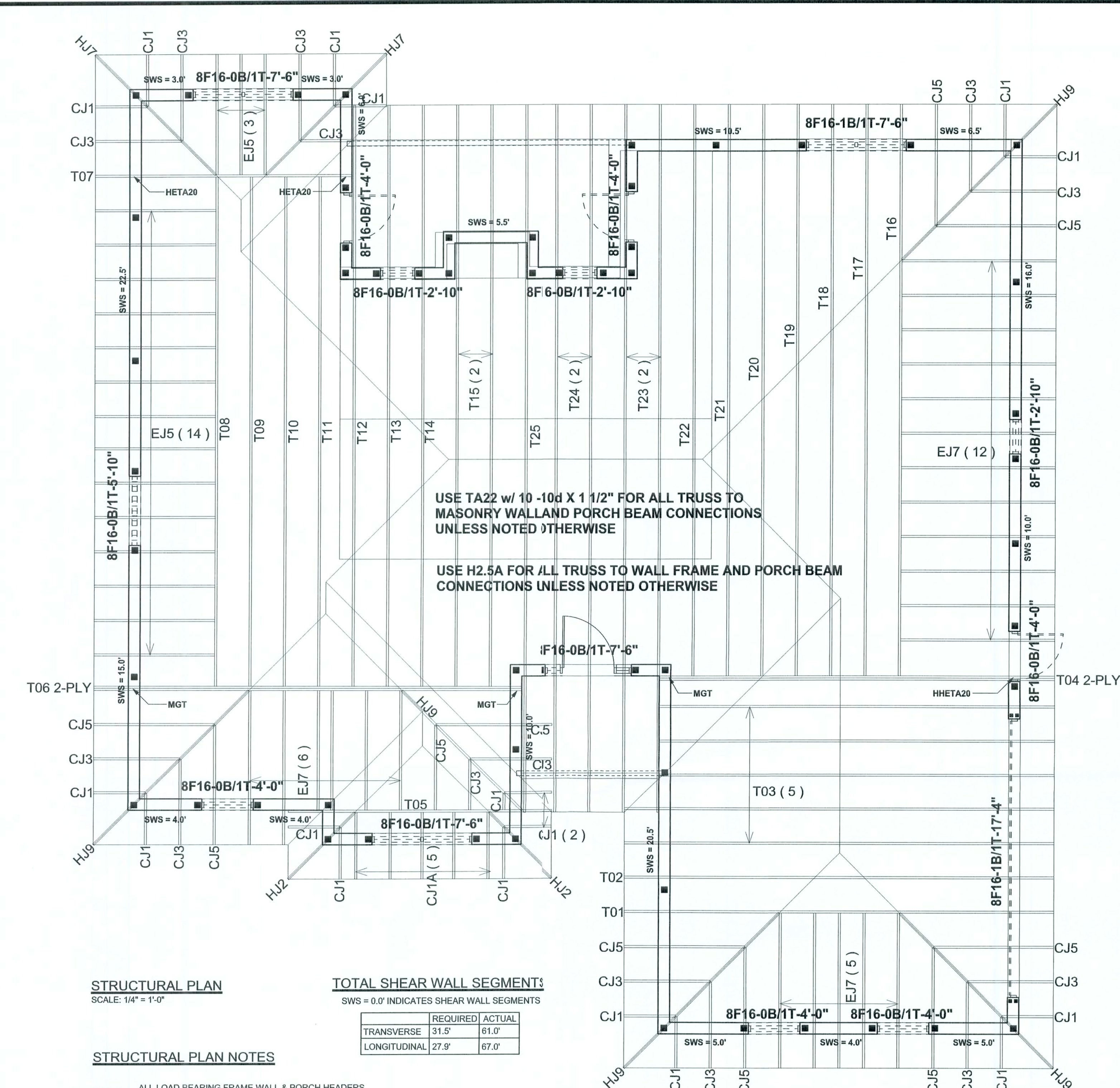
FINALS DATE:
16 / Jan / 06

JOB NUMBER:
51261

DRAWING NUMBER

S-2

OF 3 SHEETS



MATERIALS

1. fc 8" precast lintel = 3500 psi
2. fc prestressed lintel = 9000 psi
3. Grout per ASTM C476 fc = 3000 psi w/ maximum 3/8 inch slump aggregate & 8 to 11 inch slump
4. Concrete Masonry Units (CMU) per ASTM C90 minimum net area compressive strength = 1900 psi
5. Rebar per ASTM A615 grade 60
6. Prestressing strand per ASTM A416 grade 270 low relaxation
7. Mortar per ASTM C270 type M or S

GENERAL NOTES

1. Provide full mortar bed and head joints.
2. Shore filled lintels as required.
3. Installation of lintel must comply with the architectural and/or structural documents.
4. U-Intels are manufactured with 5 1/2" long notches at the ends to accommodate vertical cell reinforcing and grouting.
5. All lintels meet or exceed L/360 deflection, except lintels 17'-4" and longer with a nominal height of 8" meet or exceed L/180 deflection.
6. Bottom field added rebar to be located at the bottom of the lintel cavity.
7. 7/32" diameter wire stirrups are welded to the bottom steel for mechanical anchorage.
8. Cast-in-place concrete may be provided in composite lintel in lieu of concrete masonry units.
9. Safe load rating based on rational design analysis per ACI 318 and ACI 530
10. Product Approvals: Miami-Dade County, Florida No. 03-0606.05
11. The exterior surface of lintel installed in exterior concrete masonry walls shall have a coating of stucco applied in accordance with ASTM C-296/r other approved coating.
12. Lintels loaded simultaneously with vertical (gravity or uplift) and horizontal (lateral) loads should be checked for the combined loading with the following equation:
$$\frac{V_u}{\phi V_n} + \frac{H_u}{\phi H_n} \leq 1.0$$

Applied vertical load + Applied horizontal load
Safe vertical load Safe horizontal load
13. Additional lateral load capacity can be obtained by the designer by providing additional reinforced concrete masonry above the lintel. See detail at right:

SAFE LOAD TABLE NOTES

1. All values based on minimum 4 inch nominal bearing.
2. N.R. = Not Rated
3. Safe loads are superimposed allowable loads.
4. Safe loads based on grade 40 or grade 60 field rebar.
5. One #7 rebar may be substituted for two #5 rebars in 8" lintels only
6. The designer may evaluate concentrated loads from the safe load tables by calculating the maximum resisting moment and shear at d-away from face of support.
7. For composite lintel heights not shown, use safe load from next lower height shown.
8. For lintel lengths not shown, use safe load from next longest length shown
9. All safe loads in units of pounds per linear foot
10. All safe loads based on simply supported span.
11. The number in the the parenthesis indicates the percent reduction for grade 40 field added rebar.
Example 7'-5" lintel type 8F32-1B safe gravity load = 6472 (H0.0469) (15) (H0.0781; w/ 15% reduction 6472 $\Rightarrow$ (.85) = 5501 plf

SAFE GRAVITY LOADS FOR 8" PRECAST & PRESTRESSED U-INTELS

LENGTH	TYPE	SAFE LOAD - POUNDS PER LINEAR FOOT					
		8F8-0B	8F12-0B	8F16-0B	8F20-0B	8F24-0B	8F28-0B
2'-10" (34")	PRECAST	2231	3069	4605	6113	7547	8974
3'-6" (42")	PRECAST	2231	3069	4605	6113	7547	8974
4'-0" (48")	PRECAST	1986	2693	4605	6113	7547	8974
4'-6" (54")	PRECAST	1599	1989	2110	2831	3753	4576
5'-4" (64")	PRECAST	1217	1269	1438	1999	2560	3123
5'-10" (70")	PRECAST	1062	1105	1173	1631	2090	2549
6'-6" (78")	PRECAST	908	1236	2177	3480	3031	3707
7'-6" (90")	PRECAST	743	1011	1729	2632	2205	2698
9'-4" (112")	PRECAST	554	699	1160	1625	2564	3486
10'-6" (126")	PRECAST	475	643	1052	1533	2093	2781
11'-4" (136")	PRECAST	362	582	945	1366	1846	2423
12'-0" (144")	PRECAST	337	540	873	1254	1684	2193
13'-4" (160")	PRECAST	296	471	755	1075	1428	1838
14'-0" (168")	PRECAST	279	424	706	1002	1326	1697
14'-8" (176")	PRESTRESSED	N.R.	NR	NR	NR	NR	NR
15'-4" (184")	PRESTRESSED	N.R.	NR	NR	NR	NR	NR
17'-4" (208")	PRESTRESSED	N.R.	NR	NR	NR	NR	NR
19'-4" (232")	PRESTRESSED	N.R.	NR	NR	NR	NR	NR
21'-4" (256")	PRESTRESSED	N.R.	NR	NR	NR	NR	NR
22'-0" (264")	PRESTRESSED	N.R.	NR	NR	NR	NR	NR
24'-0" (288")	PRESTRESSED	N.R.	NR	NR	NR	NR	NR

SAFE UPLIFT LOADS FOR 8" PRECAST & PRESTRESSED U-INTELS

LENGTH	TYPE	SAFE LOAD - POUNDS PER LINEAR FOOT					
		8F8-1T	8F12-1T	8F16-1T	8F20-1T	8F24-1T	8F28-1T
2'-10" (34")	PRECAST	1972	3173	4460	5747	7034	8321
3'-6" (42")	PRECAST	1599	2524	3547	4569	5591	6613
4'-0" (48")	PRECAST	1363	2192	3079	3966	4853	5740
4'-6" (54")	PRECAST	1207	1940	2724	3508	4292	5077
5'-4" (64")	PRECAST	1016	1632	2290	2949	3607	4265
5'-10" (70")	PRECAST	809	1492	2093	2694	3295	3897
6'-6" (78")	PRECAST	835	1340	1880	2419	2959	3498
7'-6" (90")	PRECAST	727	1168	1634	2102	2571	3039
9'-4" (112")	PRECAST	591	881	1135	1471	1811	2152
10'-6" (126")	PRECAST	530	552	914	1185	1458	1732
11'-4" (136")	PRECAST	474	485	798	1034	1272	1510
12'-0" (144")	PRECAST	470	441	722	926	1151	1386
13'-4" (160")	PRECAST	418	373	606	783	962	1141
14'-0" (168")	PRECAST	384	346	559	723	887	1052
14'-8" (176")	PRESTRESSED	239	223	519	671	823	976
15'-4" (184")	PRESTRESSED	246	390	655	968	1324	1625
17'-4" (208")	PRESTRESSED	187	255	404	520	637	754
19'-4" (232")	PRESTRESSED	142	198	308	393	480	567
21'-4" (256")	PRESTRESSED	142	230	369	531	713	903
22'-0" (264")	PRESTRESSED	137	192	295	378	461	545
24'-0" (288")	PRESTRESSED	124	175	267	341	416	491

SAFE GRAVITY LOADS FOR 8" PRECAST w/ 2" RECESS DOOR U-INTELS

LENGTH	TYPE	SAFE LOAD - POUNDS PER LINEAR FOOT					
		8RF8-0B	8RF10-0B	8RF14-0B	8RF18-0B	8RF22-0B	8RF26-0B
4'-4" (52")	PRECAST	1635	1749	3355	3280	4340	5421
4'-6" (54")	PRECAST	1494	1591	3069	3206	4639	5690
5'-8" (68")	PRECAST	866	1167	2481	4567	6389	8060
5'-10" (70")	PRECAST	810	859	1653	1600	2124	2649
6'-8" (80")	PRECAST	797	1113	2342	4242	6539	8060
7'-6" (90")	PRECAST	669	801	1825	3120	5048	7915
9'-8" (116")	PRECAST	411	526	999	1568	2253	3129

SAFE UPLIFT LOADS FOR 8" PRECAST w/ 2" RECESS DOOR U-INTELS

LENGTH	TYPE	SAFE LOAD - POUNDS PER LINEAR FOOT					
		8RF8-1T	8RF10-1T	8RF14-1T	8RF18-1T	8RF22-1T	8RF26-1T
4'-4" (52")	PRECAST	905	905	1748	2635	3522	4409
4'-6" (54")	PRECAST	867	867	1675	2525	3374	4224
5'-8" (68")	PRECAST	875	875	1301	1990	2618	3277
5'-10" (70")	PRECAST	675	675	1301	1990	2618	3277
6'-8" (80")	PRECAST	570	570	1012	1651	2204	2756
7'-6" (90")	PRECAST	506	506	967	1462	1952	2442
9'-8" (116")	PRECAST	395	395	589	1135	1514	1883

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. BUILDERS FIRST SOURCE JOB #144253

REVISIONS

NO.	DESCRIPTION	DATE



WINDLOAD ENGINEER: Mark Discoway, PE No. 53915, F08868, Lake City, FL 32056, 386-754-543

DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer a questions to Mark Discoway, P.E. for resolution. Do not proceed without clarification.

CERTIFICATION: I hereby certify that I have examined this plan and that the applicable portions of the plan relating to wind engineering comply with section R301.2.1, Florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISCOWAY
PE, 53915

163805
SEAL

Robinson Residence

ADDRESS:
168 1st Ave. S.E.,
Lake City, FL 32056

Mark Discoway P.E.
P.O. Box 868
Lake City Florida 32056
Phone: (386) 754 - 5419
Fax: (386) 269 - 4871

PRINTED DATE:
January 16, 2006

DRAWN BY: STRUCTURAL BY:
David Discoway

FINALS DATE:
16 / Jan / 06

JOB NUMBER:
512061

DRAWING NUMBER

S-3
OF: SHEETS