

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

RE: 2061167 - IC CONST. - LOT # HP

MiTek USA, Inc.

6904 Parke East Blvd. Tampa, FL 33610-4115

Site Information:

Customer Info: IC Construction Project Name: Custom Model: Wesley

Subdivision: High Pointe

Lot/Block: 10 Address: TBD, TBD

City: Columbia Cty

State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

License #: Name:

Address:

City:

State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special

Loading Conditions):

Design Code: FBC2017/TPI2014

Design Program: MiTek 20/20 8.2

Wind Code: ASCE 7-10 Roof Load: 37.0 psf

Wind Speed: 130 mph Floor Load: N/A psf

This package includes 37 individual, Truss Design Drawings and 0 Additional Drawings. With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.



The truss drawing(s) referenced above have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Builders FirstSource-Jacksonville.

Truss Design Engineer's Name: Finn, Walter

My license renewal date for the state of Florida is February 28, 2021.

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Walter P. Finn PE No.22839 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

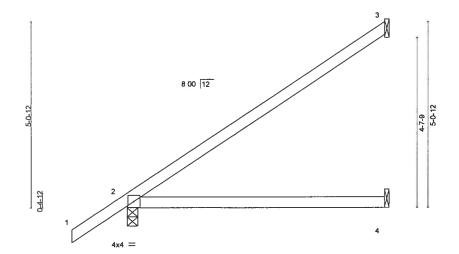
August 26,2019

C T

157

Job	Truss	Truss Type	Qty	Ply	IC CONST LOT 10 HP	
					T179	954027
2061167	EJ01	Jack-Partial	13	1		
					Job Reference (optional)	
Builders FirstSource,	Jacksonville, FL - 32244,			8 240 s Jul	14 2019 MiTek Industries, Inc. Mon Aug 26 06 58 40 2019 Pag	je 1
			ID:R1i8Y_54Jqc	JZT2xK_nQF	PtykgBF-4bqRA8sFyvZ9XSK6X5Ca5UQDLxsF2bVvIWx3xNyk2n	πD
		-1-6-0	7-0-0			
		1-6-0	7-0-0			

Scale = 1 30 2



LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES Code FBC2017/TPI2014	CSI. TC 0.73 BC 0.55 WB 0.00 Matrix-MS	DEFL. in (loc) Vert(LL) 0.17 4-7 Vert(CT) -0.25 4-7 Horz(CT) 0.01 3	 PLATES GRIP MT20 244/190 Weight: 26 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.2 BOT CHORD

2x4 SP No.2

(lb/size) 3=163/Mechanical, 2=346/0-3-8, 4=85/Mechanical

Max Horz 2=267(LC 12)

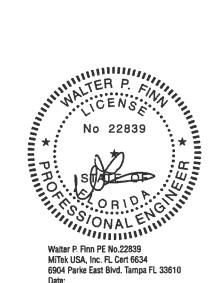
Max Uplift 3=-176(LC 12), 2=-99(LC 12), 4=-12(LC 12) Max Grav 3=190(LC 19), 2=346(LC 1), 4=127(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl. GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4 except (jt=lb) 3=176.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

6904 Parke East Blvd. Tampa FL 33610 Date:

August 26,2019

🛕 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent uccliagse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N Lee Street, Suite 312, Alexandria, VA 22314.

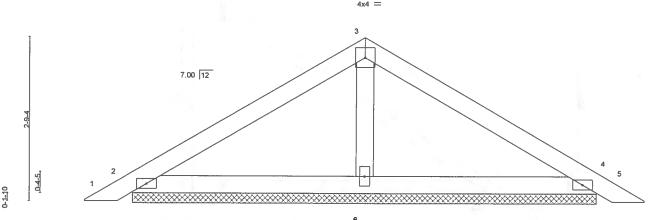


Job Truss Truss Type IC CONST. - LOT 10 HP Qty T17954028 2061167 PB01 Piggyback 17 Job Reference (optional) Builders FirstSource. Jacksonville, FL - 32244, 8 240 s Jul 14 2019 MiTek Industries, Inc. Mon Aug 26 06 58 41 2019 Page 1 ID.R1i8Y_54JqcJZT2xK_nQPtykgBF-YoOpNUttjCh09cvI5pjpehzXsLHqn2A3X9gcTpyk2mC 4-9-0 9-6-0

Scale = 1:18.8

14.5

11-1



2x4TOP CHORD UNDER PIGGYBACKS2x4 ||
TO BE LATERALLY BRACED BY

PURLING AT 2-0-0 OC MAY

2x4 ==

TYPICAL.					9-6-0							
LOADING TCLL TCDL BCLL BCDL	(psf) 20.0 7.0 0.0 *	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code FBC2017/T	2-0-0 1.25 1.25 YES PI2014	CSI. TC BC WB Matri	0.16 0.15 0.04 x-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in 0.01 0.01 0.00	(loc) 5 5 4	l/defl n/r n/r n/a	L/d 120 120 n/a	PLATES MT20 Weight: 31 lb	GRIP 244/190 FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 OTHERS 2x4 SP No.3 BRACING-

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 2=167/7-9-11, 4=167/7-9-11, 6=302/7-9-11

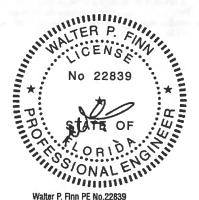
Max Horz 2=-81(LC 10)

Max Uplift 2=-83(LC 12), 4=-94(LC 13), 6=-83(LC 12) Max Grav 2=167(LC 1), 4=170(LC 20), 6=302(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4, 6.
- See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



Walter P. Finn PE No.22839 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

August 26,2019



Job	Truss	Truss Type	Qty	Ply	IC CONST LOT 10 HP
2061167	PB01G	GABLE	2	1	T17954029
2001101		G, SEE			Job Reference (optional)
Builders FirstSource,	Jacksonville, FL - 32244,			8.240 s Jul	14 2019 MiTek Industries, Inc. Mon Aug 26 06 58 42 2019 Page 1
			ID:R1i8Y_54Jqc	JZT2xK_nQP	tykgBF-0_yBbqtWUWptnlUUeWE2BvViPlecWVZCmpQ90Fyk2mB
	1	4-2-1		_	8-4-2
		4-2-1	1		4-2-1
					Scale = 1:17.6
			4×4 =		

3 7.00 12 6 2x4 = 2x4 || 2x4 =

			8-4-2	<u> </u>
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES Code FBC2017/TPI2014	CSI. TC 0.18 BC 0.11 WB 0.03 Matrix-P	DEFL. in (loc) l/defl L/d Vert(LL) 0.01 5 n/r 120 Vert(CT) 0.01 5 n/r 120 Horz(CT) 0.00 4 n/a n/a	PLATES GRIP MT20 244/190 Weight: 27 lb FT = 20%

8-4-2

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 OTHERS

2x4 SP No.3

BRACING-TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(lb/size) 2=159/6-7-13, 4=159/6-7-13, 6=232/6-7-13

Max Horz 2=-70(LC 10)

Max Uplift 2=-85(LC 12), 4=-94(LC 13), 6=-48(LC 12) Max Grav 2=159(LC 1), 4=160(LC 20), 6=232(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ff; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Gable requires continuous bottom chord bearing.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4, 6.
- 10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



Walter P. Finn PE No.22839 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

August 26,2019

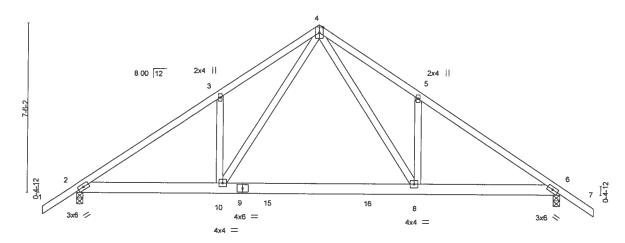
🛕 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.



Job	Truss	Truss Type		Qty	Ply	IC CONST LOT 10 HP	
2061167	T01	Common		10	1		T17954030
						Job Reference (optional)	
Builders FirstSource,	Jacksonville, FL - 3	2244,		8	240 s Jul 1	14 2019 MiTek Industries, Inc. Mo	n Aug 26 06 58 43 2019 Page 1
						nQPtykgBF-UAVZoAu8EqxkOv3h0	
	-1-6-0	6-3-11	10-8-0	15-0-5		21-4-0	,22-10-0
	1-6-0	6-3-11 '	4-4-6	4-4-5		6-3-11	1-6-0

4x6 ||

Scale = 1 49.3



	6-3-11 6-3-11	15-0-5 8-8-10		21-4-0 6-3-11	
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr NO Code FBC2017/TPI2014	CSI. DEFL TC 0.40 Vert(L BC 0.73 Vert(C WB 0.71 Horz(C Matrix-MS	L) -0.14 8-10 >999 T) -0.27 8-10 >948	240 N 3 180 1 n/a	PLATES GRIP MT20 244/190 Weight: 129 lb FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x6 SP No.2 WEBS 2x4 SP No.3 BRACING-

TOP CHORD

Structural wood sheathing directly applied or 4-2-14 oc purlins.

Rigid ceiling directly applied or 9-10-1 oc bracing.

REACTIONS. (lb/size) 2=1132/0-3-8, 6=1132/0-3-8

Max Horz 2=253(LC 11)

Max Uplift 2=-457(LC 12), 6=-457(LC 13) Max Grav 2=1151(LC 19), 6=1151(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1731/698, 3-4=-1789/892, 4-5=-1789/892, 5-6=-1731/698

BOT CHORD 2-10=-522/1531, 8-10=-237/958, 6-8=-425/1399

WEBS 4-8=-545/1097, 5-8=-371/338, 4-10=-545/1097, 3-10=-371/338

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vuit=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=457.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)

Vert: 1-4=-54, 4-7=-54, 2-10=-20, 8-10=-80(F=-60), 6-8=-20



Walter P. Finn PE No.22839 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

August 26,2019

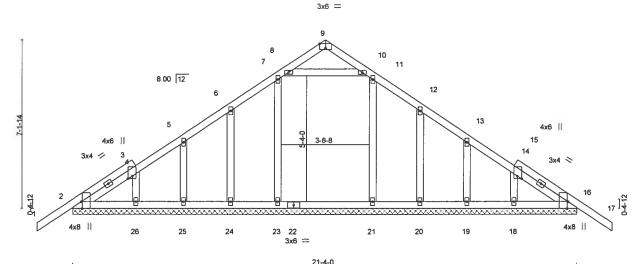
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-1473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSUTH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type	Qty	Ply	IC CONST LOT 10 HP	
ļ						T17954031
2061167	T01G	Common Supported Gable	1	1		
				1	Job Reference (optional)	
Builders FirstSource,	Jacksonville, FL - 32244,			8 240 s Jul	14 2019 MiTek Industries, Inc.	Mon Aug 26 06 58 45 2019 Page 1
			ID:R1i8Y_54Jq	JZT2xK_n	QPtykgBF-RZdKDswOmRBSe	eDD3KfnloX7Dzyfvjr7eSneqdayk2m8
	-1-6-0	10-8-0	1		21-4-0	22-10-0
	1-6-0	10-8-0	1		10-8-0	1-6-0

Scale = 1 47 0



		Г					21-4-0					7	
Plate Offsets (X,Y) [2:0-3-8,Edge], [3:0-1-5,0-2-4], [9:0-3-0,Edge], [15:0-1-5,0-2-4], [16:0-3-8,Edge]													
												1	
LOADING	G (psf)	SP	ACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Pla	te Grip DOL	1.25	тс	0.16	Vert(LL)	-0.01	17	n/r	120	MT20	244/190
TCDL	7.0	Lur	nber DOL	1.25	ВС	0.14	Vert(CT	-0.01	17	n/r	120		
BCLL	0.0 *	Re	Stress Incr	YES	WB	0.10	Horz(C1	0.01	16	n/a	n/a		
BCDL	10.0	Co	de FBC2017/T	PI2014	Matri	x-S	,	•				Weight: 128 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3

OTHERS 2x4 SP No.3

All bearings 21-4-0.

(lb) - Max Horz 2=-242(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 2, 16, 23 except 24=-117(LC 12), 25=-107(LC 12), 26=-104(LC 12),

20=-118(LC 13), 19=-110(LC 13), 18=-107(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 2, 16, 24, 25, 26, 20, 19, 18 except 23=329(LC 19), 21=284(LC

FORCES. (ib) - Max. Comp./Max. Ten. - All forces 250 (ib) or less except when shown.

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 9) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 16, 23 except (jt=lb) 24=117, 25=107, 26=104, 20=118, 19=110, 18=107.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

6904 Parke East Blvd. Tampa FL 33610 Date:

August 26,2019

🛕 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters and properly incorporate this design into the overall building design is based only upon parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly anage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314



4.1	T	·					
Jop	Truss	Truss Type	Qt	y Piy	IC CONST LOT 10 HP		
2061167	T02	Roof Special	1	1			T17954032
L			_		Job Reference (optional)		
Builders FirstSource,	Jacksonville, FL - 32244			8.240 s Jul	14 2019 MiTek Industries, I	nc Mon Aug 26 06 58	46 2019 Page 1
			ID R1i8\	_54JqcJZT2xK_i	nQPtykgBF-vlBiQCw0XIKJF	NnFtMI LigHsMuhS8t	ohRON90vk2m7
r-1-6-0		14-0-13	20-7-2	24-4-1	29-1-6	34-8-0	36-2-0
'1-6-0	7-6-8	6-6-5	6-6-5	3-8-14	4-9-6	5-6-10	1-6-0

Scale = 1:68.3

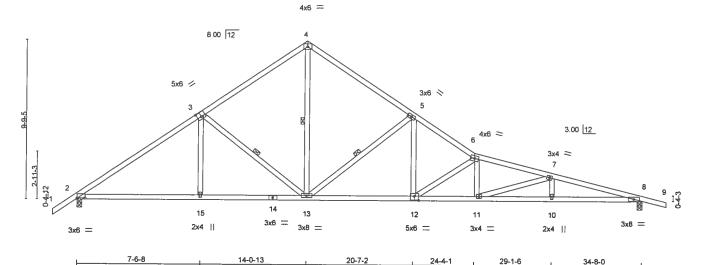


Plate Offsets (X,Y)-	[2:0-6-0,0-0-4], [3:0-3-0,0-3-4], [12:0-3-	0,0-3-0]	0-0-3	3-0-14	4-2-0	5-6-10
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES Code FBC2017/TPI2014	CSI. TC 0.60 BC 0.61 WB 0.71 Matrix-MS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) I/defl 0.29 11 >999 -0.52 10-11 >806 0.12 8 n/a	L/d 240 180 n/a	PLATES GRIP MT20 244/190 Weight: 186 lb FT = 20%

20-7-2 6-6-5

BRACING-

WEBS

TOP CHORD

BOT CHORD

29-1-6

Rigid ceiling directly applied or 5-8-8 oc bracing.

1 Row at midpt

Structural wood sheathing directly applied or 2-5-12 oc purlins.

3-13, 4-13, 5-13

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD

2x4 SP No.2 *Except*

8-12: 2x4 SP M 31

WEBS 2x4 SP No.3

REACTIONS. (lb/size) 2=1364/0-3-8, 8=1364/0-3-8

Max Horz 2=-322(LC 10)

Max Uplift 2=-489(LC 12), 8=-557(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - Ali forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1949/850, 3-4=-1488/774, 4-5=-1481/768, 5-6=-2604/1222, 6-7=-3674/1695,

7-8=-4268/1998

BOT CHORD 2-15=-577/1557, 13-15=-577/1559, 12-13=-814/2130, 11-12=-1507/3517,

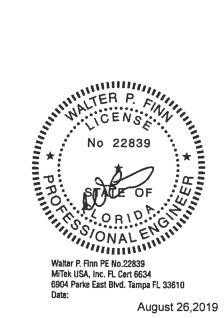
10-11=-1874/4112, 8-10=-1874/4112

WEBS 3-15=0/294, 3-13=-634/404, 4-13=-617/1272, 5-13=-1248/714, 6-12=-1688/846,

6-11=-98/285, 7-11=-630/378, 5-12=-460/1095

NOTES.

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (|t=|b|) 2=489, 8=557,



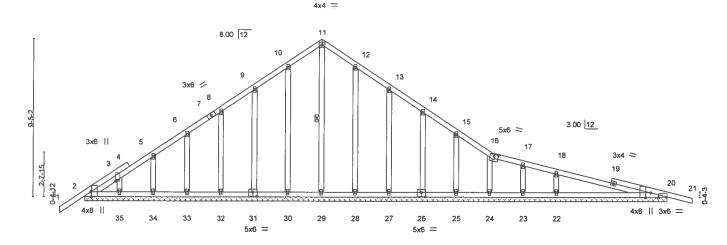
August 26,2019

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-1473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly dranage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job		Truss	Truss Type	Qty	Ply	IC CONST LOT 10 HP		
			_					T17954033
206	1167	T02G	GABLE	1	1			
						Job Reference (optional)		
Bu	ilders FirstSource, Ja	acksonville, FL - 32244,			240 s Jul	14 2019 MiTek Industries, In	c Mon Aug 26 06 58 48	2019 Page 1
				ID:R1i8Y_54	JqcJZT2xK	_nQPtykgBF-r8JSrtyG3Ma1	Vgxe?nLSQAlfcAdlwA?5	8ltUDvyk2m5
	_[-1-6-0 _]	14-()-13	24-4-1			34-8-0	36-2-0
	1-6-0	14-0)-13	10-3-4		1	10-3-15	1-6-0

Scale = 1:66.2



						0-4-0-0						
						34-8-0						1
Plate Off	sets (X,Y)-	[2:0-3-8,Edge], [3:0-0-9,0	-1-01. [16:0-3-	0.0-1-71, [20:	0-3-8.Edge1	[20:0-5-12 Edgel	26:0-3-0	0.0-3-0	1.131:0-3	-0.0-3-01		
	(-1,-7,	International Property		1	,=-g-,	, [20:0 0 :2,2299],]			1, 10 1.0 0	5,5 5 51		
LOADING	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.45	Vert(LL)	0.02	21	n/r	120	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	ВС	0.32	Vert(CT)	0.03	21	n/r	120		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.16	Horz(CT)	0.01	20	n/a	n/a	1	
BCDL	10.0	Code FBC2017/TI	PI2014	Matri	ix-S	\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \					Weight: 215 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

WEBS

LUMBER-

TOP CHORD 2x4 SP No.2

2x4 SP No.2 **BOT CHORD**

OTHERS 2x4 SP No.3

REACTIONS. All bearings 34-8-0. Max Horz 2=-310(LC 10) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 2, 34, 35, 24, 23 except 20=-189(LC 9), 30=-109(LC 12),

31=-116(LC 12), 32=-111(LC 12), 33=-114(LC 12), 28=-105(LC 13), 27=-118(LC 13), 26=-106(LC 13),

25=-127(LC 13), 22=-227(LC 9)

Max Grav All reactions 250 lb or less at joint(s) 2, 30, 31, 32, 33, 34, 35, 28, 27, 26, 25, 24, 23 except

20=291(LC 1), 29=253(LC 22), 22=497(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD

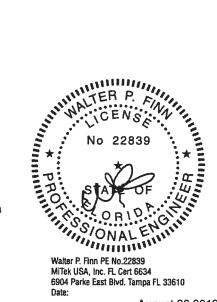
2-3-279/232, 10-11-193/265, 11-12-193/265

WEBS 18-22=-347/288

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 34, 35, 24, 23 except (jt=lb) 20=189, 30=109, 31=116, 32=111, 33=114, 28=105, 27=118, 26=106, 25=127, 22=227.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

11-29

Rigid ceiling directly applied or 6-0-0 oc bracing.

1 Row at midpt

Date:

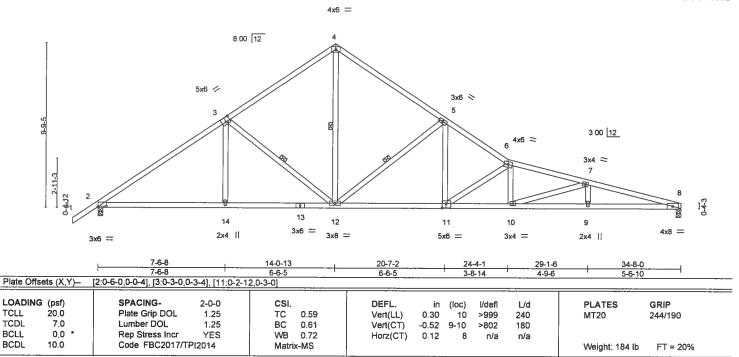
August 26,2019

🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building designer must verify the applicability of design parameters and property incorporate this design into the overall building designer. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N Lee Street, Suite 312, Alexandria, VA 22314.



i	Job	Truss	Truss Type	Qty	Ply	IC CONST LOT 10 HP	
	2061167	тоз	Roof Special	5	1		T17954034
						Job Reference (optional)	
	Builders FirstSource, J	acksonville, FL - 32244			8 240 s Jul	14 2019 MiTek Industries, I	nc. Mon Aug 26 06 58 50 2019 Page 1
				ID:R1i8Y_54JqcJ	ZT2xK_nQF	tykgBF-nWQDGZzXbzqkk	516CNwVbrzvzFdOxoNc3Maloyk2m3
	₁ -1-6-0 ,	7-6-8	14-0-13	20-7-2	24-	4-1 , 29-1-6	34-8-0
	1-6-0	7-6-8	6-6-5	6-6-5	3-8	-14 4-9-6	5-6-10

Scale = 1 66 2



BRACING-

TOP CHORD

BOT CHORD

WEBS

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD

2x4 SP No.2 *Except*

8-11: 2x4 SP M 31

WEBS 2x4 SP No.3

REACTIONS. (lb/size) 8=1281/0-3-8, 2=1365/0-3-8

Max Horz 2=-313(LC 10)

Max Uplift 8=-506(LC 13), 2=-490(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1952/854, 3-4=-1491/779, 4-5=-1484/773, 5-6=-2613/1233, 6-7=-3690/1713,

7-8=-4313/2037

BOT CHORD 2-14=-586/1552, 12-14=-586/1553, 11-12=-840/2137, 10-11=-1542/3532,

9-10=-1930/4157, 8-9=-1930/4157

3-14=0/294, 3-12=-634/403, 4-12=-622/1274, 5-12=-1254/720, 5-11=-466/1101,

6-11=-1698/855, 6-10=-103/287, 7-10=-661/401

NOTES.

WEBS

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl. GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 8=506, 2=490.



Structural wood sheathing directly applied or 2-5-8 oc purlins.

3-12, 4-12, 5-12

Rigid ceiling directly applied or 5-7-7 oc bracing.

1 Row at midpt

Walter P. Finn PE No.22839 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

August 26,2019

A WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Components available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314



Job Truss Truss Type Qty IC CONST. - LOT 10 HP T17954035 2061167 T04 Common Girder Job Reference (optional) 8.240 s Jul 14 2019 MiTek Industries, Inc. Mon Aug 26 06 58 51 2019 Page 1 Builders FirstSource, Jacksonville, FL - 32244, ID_R1i8Y_54JqcJZT2xK_nQPtykgBF-Fi_bUv_9MHybM8gDgvu92oNC4Nfp7LPXqj58qEyk2m2

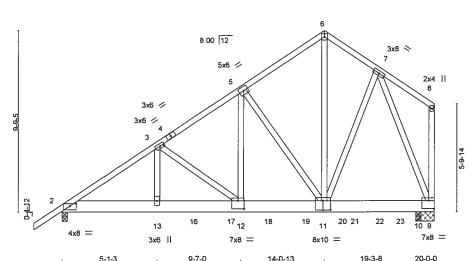
9-7-0 4-5-13 14-0-13 4-5-13 16-10-11 2-9-13 20-0-0 3-1-5

> 4x4 = Scale = 1 59.7

Structural wood sheathing directly applied or 4-10-4 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.



5-1-3 4-5-13 [2:0-4-0,0-1-9], [9:Edge,0-5-4], [11:0-5-0,0-6-0], [12:0-4-0,0-5-4] Plate Offsets (X,Y)-**PLATES** GRIP LOADING (psf) SPACING-DEFL (loc) Ľ∕d Plate Grip DOL 0.12 12-13 >999 240 MT20 244/190 TCLL 20.0 1.25 TC 0.37 Vert(LL) TCDL 1.25 ВС 0.36 Vert(CT) -0.16 12-13 >999 180 7.0 Lumber DOL WB 0.95 0.02 **BCLL** 0.0 Rep Stress Inci NO Horz(CT) Code FBC2017/TPI2014 Matrix-MS Weight: 350 lb FT = 20% **BCDL** 10.0

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x8 SP 2400F 2.0E

WEBS 2x4 SP No.3

REACTIONS. (lb/size) 2=3632/0-3-8, 9=728/1-0-0, 10=4894/0-3-8

Max Horz 2=391(LC 27)

Max Uplift 2=-1926(LC 8), 9=-544(LC 8), 10=-1970(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-6360/3418, 3-5=-4966/2589, 5-6=-2680/1356, 6-7=-2605/1370

2-13=-3088/5236, 12-13=-3088/5236, 11-12=-2261/4081, 10-11=-683/1379, **BOT CHORD**

9-10=-683/1379 WEBS

3-13=-903/1407, 3-12=-1438/1029, 5-12=-2189/3745, 5-11=-3330/2071, 6-11=-1377/2660,

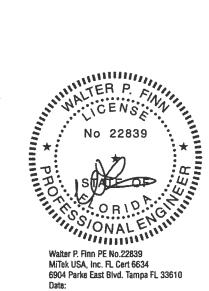
7-11=-1034/2116, 7-9=-3744/1854

NOTES-

- 1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
 - Top chords connected as follows: 2x4 1 row at 0-9-0 oc.
 - Bottom chords connected as follows: 2x8 2 rows staggered at 0-5-0 oc.
 - Webs connected as follows: 2x4 1 row at 0-9-0 oc.
- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- 3) Unbalanced roof live loads have been considered for this design.
- 4) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=1926, 9=544, 10=1970.
- 9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 2114 lb down and 1498 lb up at 7-0-12, 938 lb down and 460 lb up at 9-0-12, 965 lb down and 462 lb up at 11-0-12, 985 lb down and 457 lb up at 13-0-12, 931 lb down and 349 lb up at 15-0-12, and 931 lb down and 352 lb up at 17-0-12, and 933 lb down and 350 lb up at 19-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25



6904 Parke East Bivd. Tampa FL 33610 Date:

August 26,2019

Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design entering the property incorporate this design into the overall building design. Bracing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314



Job Truss Truss Type IC CONST. - LOT 10 HP Qty Ply T17954035 2061167 T04 Common Girder Job Reference (optional)

Builders FirstSource,

Jacksonville, FL - 32244,

8.240 s Jul 14 2019 MiTek Industries, Inc. Mon Aug 26 06,58 51 2019 Page 2 ID.R1i8Y_54JqcJZT2xK_nQPtykgBF-Fj_bUv_9MHybM8gDgvu92oNC4Nfp7LPXqi58qEyk2m2

LOAD CASE(S) Standard

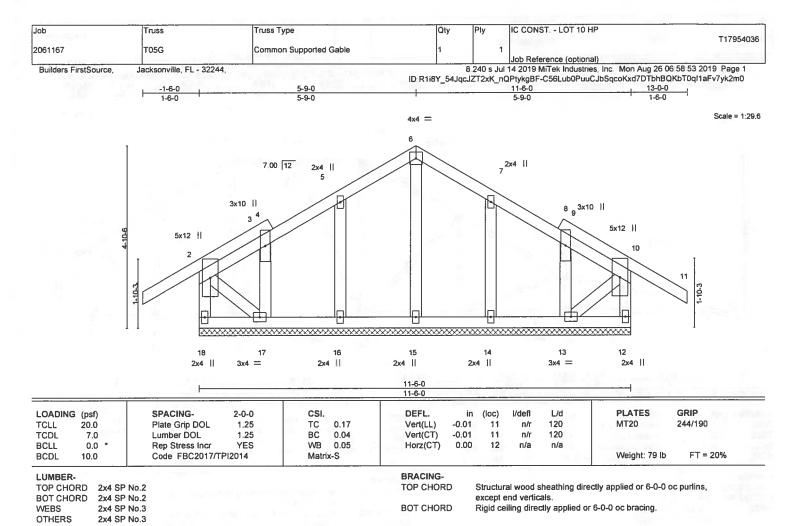
Uniform Loads (plf)

Vert: 1-6=-54, 6-8=-54, 2-9=-20

Concentrated Loads (lb)

Vert: 10=-933(B) 16=-2114(B) 17=-931(B) 18=-931(B) 19=-931(B) 20=-931(B) 22=-931(B)





REACTIONS. All bearings 11-6-0

(lb) - Max Horz 18-187(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 18, 12 except 16=-106(LC 12), 17=-136(LC 12), 14=-106(LC 13),

13=-132(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 18, 12, 15, 16, 17, 14, 13

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

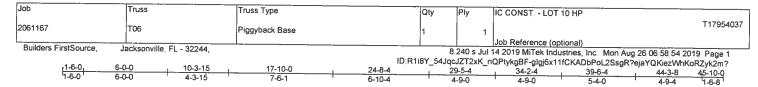
- Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Gable requires continuous bottom chord bearing.
- 5) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 18, 12 except (jt=lb) 16=106, 17=136, 14=106, 13=132.



Waiter P. Finn PE No.22839 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

August 26,2019





Scale = 1:79.7

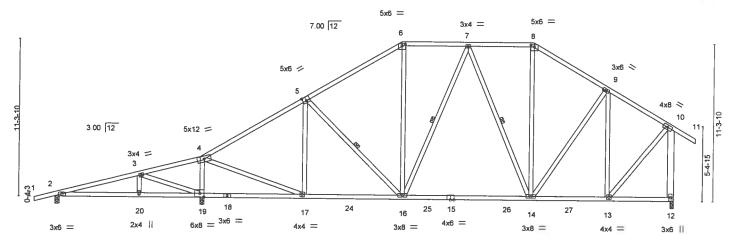


Plate Offsets (X,Y)-	6-0-0 4-3-15		7-4-4) 0-1-121 [19:0-3-8 0-3-0	6-10-4	9-6-0	39-6-4 5-4-0	4-3-8
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code FBC2017/TF	2-0-0 1.25 1.25 YES	CSI. TC 0.60 BC 0.88 WB 0.92 Matrix-MS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) I/defl L/d -0.29 14-16 >999 240 -0.47 14-16 >863 180 0.03 12 n/a n/a	PLATES MT20 Weight: 304	GRIP 244/190 lb FT = 20%

24 8 4

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3 *Except*

10-12: 2x6 SP No.2

6-0-0

BRACING-TOP CHORD

WEBS

Structural wood sheathing directly applied or 4-1-9 oc purlins,

except end verticals.

ROT CHORD Rigid ceiling directly.

Rigid ceiling directly applied or 6-0-0 oc bracing. 1 Row at midpt 5-16, 7-16, 7-14

REACTIONS.

(lb/size) 2=376/0-3-8, 19=1752/0-3-8, 12=1310/0-3-0

Max Horz 2=457(LC 11)

Max Uplift 2=-368(LC 8), 19=-699(LC 12), 12=-433(LC 13) Max Grav 2=378(LC 23), 19=1752(LC 1), 12=1317(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=400/522, 3-4=-479/388, 4-5=-1216/509, 5-6=-1172/642, 6-7=-969/620,

7-8=-867/586, 8-9=-1017/611, 9-10=-785/467, 10-12=-1271/646

10-3-15 10-5-12

17-10-0

BOT CHORD 2-20=-564/369, 19-20=-564/369, 17-19=-386/435, 16-17=-421/1160, 14-16=-360/928,

13-14=-286/637 WEBS 3-19=-707/893.4

3-19=-707/893, 4-19=-1394/800, 4-17=-789/1430, 5-17=-337/311, 5-16=-278/242,

6-16=-116/345, 7-16=-130/261, 7-14=-363/248, 8-14=-118/298, 9-14=-185/356,

9-13=-613/308, 10-13=-369/963

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf, BCDL=3.0psf, h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical right exposed; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (it=lb) 2=368, 19=699, 12=433.



Walter P. Finn PE No.22839 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

August 26,2019



Job Truss Truss Type Qty IC CONST. - LOT 10 HP T17954038 2061167 T06G GABLE Job Reference (optional) Builders FirstSource, Jacksonville, FL - 32244. 8 240 s Jul 14 2019 MiTek Industries, Inc. Mon Aug 26 06 59 01 2019 Page 1 39-6-4 5-10-15 33-7-5 4-2-1

Scale = 1.87.5

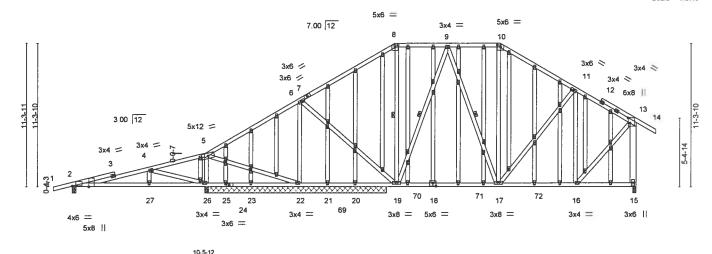


Plate Offsets (X,Y)-	[2:0-3-4,0-0-5], [2:0-0-9,Edge], [8:0-3-0,0	-1-13], [10:0-3-0,0-1-12], [13:0-5-	,0-1-12], [18:0-3-0,0-	-3-0], [24:0-2-8,0-1-8], [5	7:0-1-11,0-1-0], [59:0-1-11,0-1-0]
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES Code FBC2017/TPI2014	TC 0.67 V BC 0.64 V	FL. in (loc rt(LL) -0.17 17-1 rt(CT) -0.27 17-1 rz(CT) -0.02 1	9 >999 240	PLATES GRIP MT20 244/190 Weight: 487 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

WEBS

LUMBER-

TOP CHORD 2x4 SP No.2

2x4 SP No.2 BOT CHORD

2x4 SP No.3 *Except* WEBS

13-15: 2x6 SP No.2 OTHERS 2x4 SP No.3

All bearings 14-3-8 except (it=length) 2=0-3-8, 26=0-3-8, 26=0-3-8, 15=0-3-0. REACTIONS.

10-4-0 0-0-1

(lb) - Max Horz 2=452(LC 11)

Max Uplift All uplift 100 lb or less at joint(s) 20, 25 except 2=-348(LC 8), 26=-410(LC 8), 22=-488(LC 12),

15=-375(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 20, 25, 23, 21 except 2=380(LC 23), 26=774(LC 23),

26=768(LC 1), 22=1146(LC 19), 15=1035(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-4=-420/466, 4-5=-557/401, 6-8=-629/399, 8-9=-563/417, 9-10=-656/476, 10-11=-730/478, 11-13=-652/378, 13-15=-994/514

BOT CHORD

2-27=-565/416, 26-27=-565/416, 25-26=-391/496, 23-25=-391/496, 22-23=-391/496,

21-22=-222/294, 20-21=-222/294, 19-20=-222/294, 17-19=-237/549, 16-17=-227/514

4-26=-807/941, 5-26=-463/371, 5-22=-321/369, 6-22=-1158/726, 6-19=-244/672, 9-19=-321/239, 11-16=-426/238, 13-16=-274/739

NOTES-

WEBS

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl. GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical right exposed; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Provide adequate drainage to prevent water ponding.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 9) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 20, 25 except (it=lb) 2=348, 26=410, 22=488, 15=375.



Structural wood sheathing directly applied or 6-0-0 oc purlins,

8-19, 9-19, 9-17

Rigid ceiling directly applied or 6-0-0 oc bracing.

except end verticals.

1 Row at midpt

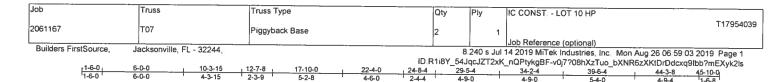
Walter P. Finn PE No.22839 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

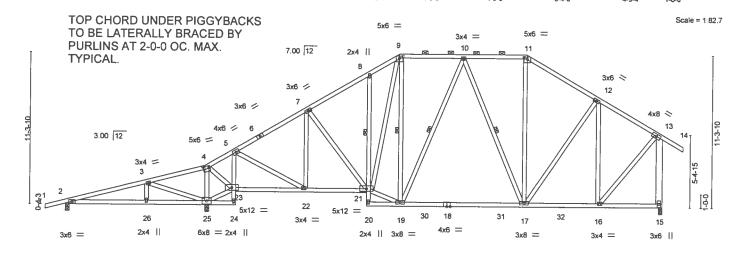
August 26,2019

▲ WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MTek® connectors. This design is based only upon parameters and individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building designer must verify the applicability of design parameters and property incorporate this design into the overall building designer must verify the applicability of design parameters and property incorporate this design into the overall building designer must verify the applicability of design parameters and property designers only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information. available from Truss Plate Institute, 218 N Lee Street, Suite 312, Alexandria, VA 22314.







-	6-0-0	10-3-15	10-5-12	17-10-0	22-4-0	24-8-4	34-2-4	39-6-4	44-3-8	- 124
	6-0-0	4-3-15	0-11/13 2-1-12	5-2-8	4-6-0	2-4-4	9-6-0	5-4-0	4-9-4	- 11
Plate Offsets (X,Y)-	[4:0-5-0,0-1	1-12], [9:0-3-0,0-	-1-12], [11:0-	-3-0,0-1-12]						

LOADING (psf) TCLL 20,0 TCDL 7.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES Code FBC2017/TPI2014	CSI. TC 0.45 BC 0.87 WB 0.71 Matrix-MS	DEFL. in (loc) Vert(LL) -0.26 17-19 Vert(CT) -0.43 17-19 Horz(CT) 0.04 15	Vdefi L/d >999 240 >933 180 n/a n/a	PLATES GRIP MT20 244/190 Weight: 340 lb FT = 20%
---	---	--	--	--	--

BRACING-

TOP CHORD

BOT CHORD

WEBS

1 Row at midpt

1 Row at midpt

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.2 **BOT CHORD**

2x4 SP No.2 *Except*

5-24,8-20; 2x4 SP No.3

2x4 SP No.3 *Except* **WEBS**

13-15: 2x6 SP No.2

(lb/size) 2=270/0-3-8, 25=1890/0-3-8, 15=1277/0-3-0

Max Horz 2=457(LC 11)

Max Uplift 2=-384(LC 8), 25=-755(LC 12), 15=-426(LC 13) Max Grav 2=277(LC 23), 25=1890(LC 1), 15=1277(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-153/592, 3-4=-703/863, 4-5=-370/49, 5-7=-1158/497, 7-8=-1166/627,

8-9=-1184/723, 9-10=-916/594, 10-11=-841/574, 11-12=-958/598, 12-13=-754/458,

17.7.8

13-15=-1232/630

BOT CHORD 2-26=-375/21, 25-26=-375/21, 5-23=-1098/690, 22-23=-154/474, 21-22=-394/1082,

17-19=-340/857, 16-17=-277/616

3-25=-797/929, 4-25=-1121/534, 23-25=-919/662, 4-23=-618/1240, 5-22=-469/876,

19-21=-321/983, 9-21=-348/643, 10-17=-334/238, 11-17=-114/271, 12-17=-179/324,

12-16-591/297, 13-16-355/915, 7-22-302/256

NOTES-

WEBS

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical right exposed; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (it=lb) 2=384, 25=755, 15=426.
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



Structural wood sheathing directly applied or 5-3-8 oc purlins,

except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 9-11.

8-21

9-19, 10-19, 10-17

Rigid ceiling directly applied or 6-0-0 oc bracing. Except:

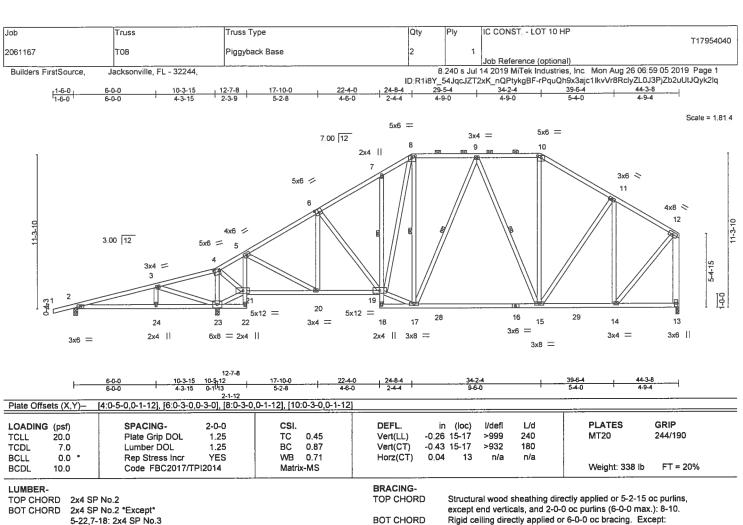
Walter P. Finn PE No.22839 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

August 26,2019

📤 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MTIek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSITPH Quality Criteria, DSB-89 and BCSI Building Comp. Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





WEBS

1 Row at midpt

1 Row at midpt

7-19

8-17, 9-17, 9-15

5-22,7-18: 2x4 SP No.3

2x4 SP No.3 *Except* WEBS

12-13: 2x6 SP No.2

(lb/size) 2=270/0-3-8, 23=1892/0-3-8, 13=1179/0-3-0

Max Horz 2=447(LC 11)

Max Uplift 2=-380(LC 8), 23=-758(LC 12), 13=-369(LC 13)

Max Grav 2=277(LC 23), 23=1892(LC 1), 13=1182(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD

2-3=-145/577, 3-4=-756/840, 4-5=-368/32, 5-6=-1161/487, 6-7=-1170/620,

7-8=-1179/714, 8-9=-909/585, 9-10=-834/563, 10-11=-964/586, 11-12=-754/434, 12-13=-1137/524

BOT CHORD 2-24-397/8, 23-24-397/8, 5-21-1100/700, 20-21-163/464, 19-20-452/1072,

15-17=-394/860, 14-15=-322/615

3-23=-797/930, 4-23=-1123/553, 21-23=-920/651, 4-21=-653/1235, 5-20=-479/877,

17-19=-356/973, 8-19=-353/634, 9-15=-333/239, 10-15=-111/273, 11-15=-183/320,

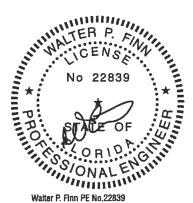
11-14=-596/333, 12-14=-400/921, 6-20=-303/261

NOTES-

WEBS

REACTIONS.

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl. GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical right exposed; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=380, 23=758, 13=369,
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord

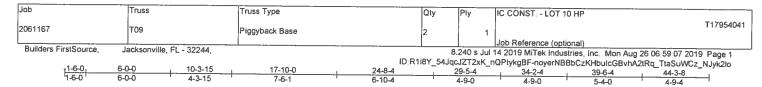


Walter P. Finn PE No.22839 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

August 26,2019

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N Lee Street, Suite 312, Alexandria, VA 22314





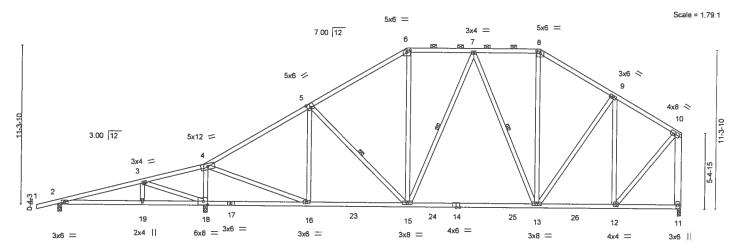


Plate Offsets (X,Y)–	6-0-0 10-3-15 10-5-12 6-0-0 4-3-15 0-1-13 [5:0-3-0,0-3-4], [6:0-3-0,0-1-12], [8:0-	17-10-0 7-4-4 3-0,0-1-12], [18:0-3-8,0-3-0	24-8-4 6-10-4	34-2-4 9-6-0	39-6-4 5-4-0	44-3-8 4-9-4
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES Code FBC2017/TPI2014	CSI. TC 0.60 BC 0.88 WB 0.94 Matrix-MS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) I/defl L -0.29 13-15 >999 24 -0.47 13-15 >863 18 0.03 11 n/a n.	00 MT20	GRIP 244/190 FT = 20%

BRACING-

TOP CHORD

BOT CHORD

WEBS

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3 *Except*

10-11: 2x6 SP No.2

(lb/size) 2=376/0-3-8, 18=1754/0-3-8, 11=1212/0-3-0

Max Horz 2=447(LC 11)

Max Uplift 2=-365(LC 8), 18=-701(LC 12), 11=-377(LC 13) Max Grav 2=378(LC 23), 18=1754(LC 1), 11=1237(LC 2)

FORCES. (lb) - Max. Comp./Max, Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-400/509, 3-4=-521/370, 4-5=-1218/496, 5-6=-1175/632, 6-7=-963/612, 7-8=-860/575, 8-9=-1023/600, 9-10=-790/443, 10-11=-1191/540

BOT CHORD 2-19=-596/369, 18-19=-596/369, 16-18=-385/417, 15-16=-473/1150, 13-15=-416/932,

12-13=-331/646

WEBS 3-18=-707/894, 4-18=-1397/807, 4-16=-812/1432, 5-16=-338/318, 5-15=-275/241,

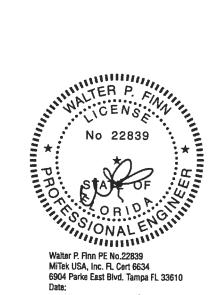
6-15=-111/346, 7-15=-132/259, 7-13=-363/249, 8-13=-115/299, 9-13=-189/353,

9-12=-618/344, 10-12=-415/968

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18, MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical right exposed; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
 This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (it=lb) 2=365, 18=701, 11=377.
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



Structural wood sheathing directly applied or 4-1-9 oc purlins,

except end verticals, and 2-0-0 oc purlins (5-11-4 max.): 6-8.

5-15, 7-15, 7-13

Rigid ceiling directly applied or 6-0-0 oc bracing.

1 Row at midpt

Date:

August 26,2019

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314



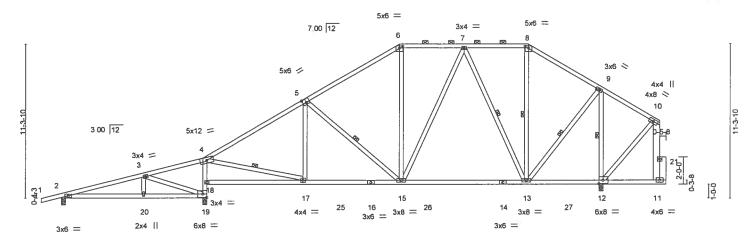
Job Truss Truss Type Qty IC CONST. - LOT 10 HP T17954042 2061167 T10 Piggyback Base Job Reference (optional) Builders FirstSource, Jacksonville, FL - 32244 8.240 s Jul 14 2019 MiTek Industries, Inc. Mon Aug 26 06:59 08 2019 Page 1 ID:R1i8Y_54JqcJZT2xK_nQPtykgBF-G_W02jBpMV5BulTUA_i8EOa2eEK8c501ksjXwlyk2ln 1-6-0 10₇7-8 0-3-9 17-10-0 24-8-4 29-5-4 34-2-4 39-6-4 44-3-8

6-10-4

4-9-0

7-2-8

Scale = 1.81.5



<u> </u>		2 10-7-8	17-10-0	24-8-4	34-2-4		39-6-4 39-8-0 44-3	
	6-0-0 4-5-12	2 0-1-12	7-2-8	6-10-4	9-6-0	'	5-4-0 0-1-12 4-7-	-8 '
Plate Offsets (X,Y)	[5:0-3-0,0-3-4], [6:0-3-0,	0-1-12], [8:0-3-	0,0-1-12], [12:0-3	3-8,0-3-0]				
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code FBC2017/7	2-0-0 1.25 1.25 YES PI2014	BC 0.	DEFL. 64 Ven(LL) 85 Ven(CT) 67 Horz(CT)	in (loc) I/defl -0.29 13-15 >999 -0.46 13-15 >753 -0.03 12 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 290 lb	GRIP 244/190 FT = 20%

BRACING-

TOP CHORD

BOT CHORD

1 Row at midpt

WEBS

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.2

2x4 SP No.2 *Except* BOT CHORD 4-19: 2x4 SP No.3

2x4 SP No.3 *Except* WEBS

10-11,11-21: 2x6 SP No.2

2=451/0-3-8, 19=1467/0-3-8, 12=1389/0-3-8 (lb/size)

Max Horz 2=430(LC 9)

Max Uplift 2=-422(LC 8), 19=-854(LC 9), 12=-568(LC 9) Max Grav 2=451(LC 23), 19=1467(LC 1), 12=1403(LC 2)

FORCES. (ib) - Max. Comp./Max. Ten. - All forces 250 (ib) or less except when shown.

2-3=-699/726, 3-4=-373/249, 4-5=-1235/1212, 5-6=-996/1087, 6-7=-786/1009, TOP CHORD

7-8=-489/686, 8-9=-613/727

BOT CHORD 2-20=-792/659, 19-20=-792/659, 18-19=-1188/1240, 4-18=-1116/1108, 15-17=-1096/994, 13-15=-680/681

3-19=-759/907, 4-17=-1227/1132, 5-15=-419/427, 6-15=-339/253, 7-15=-285/303,

7-13=-546/506, 9-13=-782/851, 9-12=-1221/1190

NOTES-

WEBS

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever right exposed; end vertical right exposed; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb)
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



Structural wood sheathing directly applied or 4-2-13 oc purlins,

4-17, 5-15, 7-13, 8-13, 9-13, 9-12

except end verticals, and 2-0-0 oc purlins (6-0-0 max.); 6-8. Rigid ceiling directly applied or 4-10-11 oc bracing.

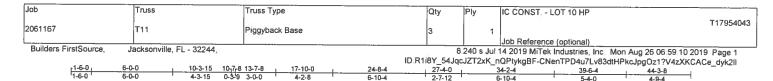
> Walter P. Finn PE No.22839 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

August 26,2019

🛕 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





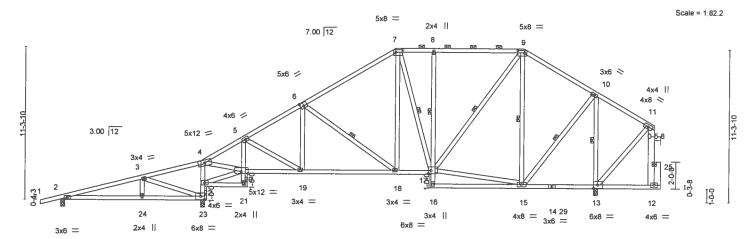


Plate Offsets (X,Y)-	6-0-0 10-5-12 6-0-0 4-5-12 [6:0-3-0,0-3-0], [7:0-6-0,0	10-7-8 13-7-8 0-1 ¹ 12 3-0-0 -2-4], [9:0-6-0,0	17-10-0 4-2-8 -2-4], [13:0-3-8,0-3	6-10-4	27-4-0 2-7-12	34-2-4 6-10-4		39-6-4 5-4-0	39-8-0 0-1-12	44-3-8 4-7-8	
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code FBC2017/TF	2-0-0 1.25 1.25 YES	CSI. TC 0.52 BC 0.92 WB 0.79 Matrix-MS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) 0.15 18-19 -0.16 18-19 -0.04 13	l/defi >999 >999 n/a	L/d 240 180 n/a		PLATES MT20 Weight: 3		GRIP 244/190 FT = 20%

BRACING-

TOP CHORD

BOT CHORD

WEBS

1 Row at midpt

1 Row at midpt

LUMBER.

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2 *Except*

4-23,5-21,8-16: 2x4 SP No.3

WEBS 2x4 SP No.3 *Except*

11-12,12-25: 2x6 SP No.2

REACTIONS. (lb/size) 2=407/0-3-8, 23=1527/0-3-8, 13=1374/0-3-8

Max Horz 2=526(LC 9)

Max Uplift 2=-394(LC 8), 23=-915(LC 9), 13=-557(LC 8) Max Grav 2=408(LC 23), 23=1527(LC 1), 13=1374(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-587/614, 3-4=-729/349, 4-5=-1135/1075, 5-6=-1310/1338, 6-7=-1035/1111,

7-8=-782/1003, 8-9=-785/1005, 9-10=-567/700 **BOT CHORD**

2-24=-666/500, 23-24=-666/500, 22-23=-1249/1358, 4-22=-1036/1108, 5-20=-334/337,

19-20=-1163/951, 18-19=-1321/1098, 17-18=-964/810, 8-17=-324/261 3-23=-786/939, 20-22=-481/525, 4-20=-1713/1405, 6-18=-448/465, 7-18=-506/404,

WEBS 15-17=-393/360, 9-17=-622/624, 9-15=-548/407, 10-15=-751/792, 10-13=-1194/1149

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever right exposed; end vertical right exposed; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=394, 23=915, 13=557.
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



Structural wood sheathing directly applied or 4-8-4 oc purlins,

except end verticals, and 2-0-0 oc purlins (5-7-3 max.): 7-9.

8-17

6-18, 7-17, 9-17, 9-15, 10-15, 10-13

Rigid ceiling directly applied or 2-2-0 oc bracing. Except:

MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

August 26,2019

🕰 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Paracing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Composately Information available from Truss Plate Institute, 218 N Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type	Qty	Ply	IC CONST - LOT 10 HP T17954044
2061167	T12	Piggyback Base	1	1	Job Reference (optional)
Builders FirstSource,	Jacksonville, FL - 32244,	ID:R:	8. 1i8Y 54Ja	240 s Jul 1 cJZT2xK i	4 2019 MiTek Industries, Inc. Mon Aug 26 06 59 12 2019 Page 1 nQPtykgBF-8lmXu5FKPkbdNMnFPqm4OElk0riPYwTdfUhl3Wyk2lj
1 4 4		13-7-8 17-10-0 24-8-4 3-0-0 4-2-8 6-10-4	27-4-0	34	10-4 39-6-4 44-3-8 10-4 5-4-0 4-9-4

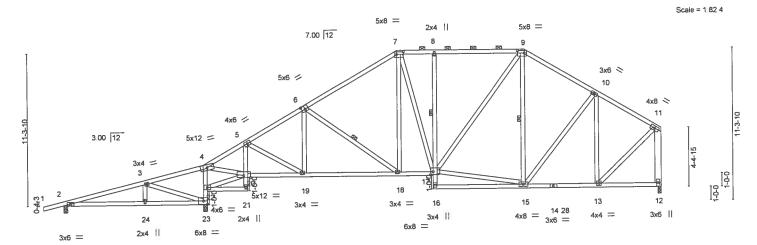


Plate Offsets (X,Y)-	6-0-0 10-4-0 6-0-0 4-4-0 [6:0-3-0,0-3-0], [7:0-6-0,0-	10-7 ₇ 8 13-7-8 0-3-8 3-0-0 -2-4], [9:0-6-0,0	17-10-0 4-2-8 0-2-4], [17:0-2-4,0-2-12]	24-8-4 6-10-4	27-4-0	34-2-4 6-10-4		39-6-4 5-4-0	44-3-8	
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code FBC2017/TF	2-0-0 1.25 1.25 YES PI2014	CSI. TC 0.55 BC 0.89 WB 0.64 Matrix-MS	DEFL Vert(C Vert(C Horz(L) -0.08 T) -0.18		L/d 240 180 n/a	MT	TES 20 ght: 313 lb	GRIP 244/190 FT = 20%

BRACING-

TOP CHORD

BOT CHORD

WEBS

1 Row at midpt

1 Row at midpt

LUMBER-

REACTIONS.

2x4 SP No.2

TOP CHORD 2x4 SP No.2 *Except* **BOT CHORD**

4-23,5-21,8-16; 2x4 SP No.3

2x4 SP No.3 *Except* **WEBS**

11-12: 2x6 SP No.2

(lb/size) 2=397/0-3-8, 23=1726/0-3-8, 12=1219/0-3-0 Max Horz 2=426(LC 9)

Max Uplift 2=-430(LC 8), 23=-703(LC 12), 12=-378(LC 13) Max Grav 2=401(LC 23), 23=1726(LC 1), 12=1219(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-508/754, 3-4=-424/458, 4-5=-1363/556, 5-6=-1608/739, 6-7=-1368/712, TOP CHORD

7-8=-1126/711, 8-9=-1129/714, 9-10=-1072/635, 10-11=-883/477, 11-12=-1175/551 2-24=-722/473, 23-24=-722/473, 22-23=-1449/736, 4-22=-1207/632, 5-20=-401/263

BOT CHORD 19-20=-588/1201, 18-19=-678/1356, 17-18=-487/1097, 8-17=-322/261, 13-15=-364/717

3-23=-789/924, 20-22=-556/310, 4-20=-883/1674, 6-18=-450/302, 7-18=-140/394, **WEBS**

7-17=-278/242, 15-17=-378/781, 9-17=-275/508, 10-15=-189/293, 10-13=-558/324,

11-13=-427/959

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical right exposed; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=430, 23=703, 12=378,
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



Structural wood sheathing directly applied or 3-11-0 oc purlins,

except end verticals, and 2-0-0 oc purlins (4-7-4 max.): 7-9.

8-17

6-18, 9-15

Rigid ceiling directly applied or 2-9-7 oc bracing. Except:

Watter P. Finn PE No.22839 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

August 26,2019

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Comp Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type	Qty	Ply	IC CONST - LOT 10 HP	_
2061167	T13	Piggyback Base	2	1	T1795404	5
Builders FirstSource,	Jacksonville, FL - 32244,				Job Reference (optional) 14 2019 MiTek Industries, Inc. Mon Aug 26 06 59 15 2019 Page 1]
T1-6-0 +		7-8 13-7-8 17-10-0 3-9 3-0-0 4-2-8	ID R1 8Y_54J 24-8-4 29-5 6-10-4 4-9	qcJZT2xK_	_nQPtykgBF-YKRgW6HCifzBEqVq4yKn0sNFJ2jHIEE3LSvPfryk2lg	

Scale = 1:82 4

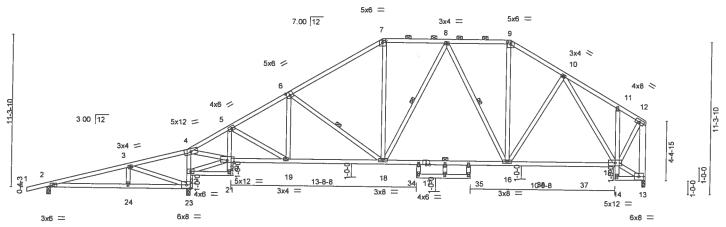


Plate Offsets (X,Y)-	6-0-0 10-5-1 6-0-0 4-5-12 [6:0-3-0,0-3-0], [7:0-3-0,0	2 0-1-12 3-0-0	17-10-0 4-2-8 0-1-12]	24-8-4 6-10-4	27-4-0 29-5-4 2-7-12 2-1-4	31-4-0 34- 1-10-12 2-1			44-3-8 2-3-0
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code FBC2017/T	2-0-0 1.25 1.25 YES PI2014	CSI. TC 0.55 BC 0.88 WB 0.83 Matrix-MS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.22 16-18 -0.35 16-18 0.06 13	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 303 lb	GRIP 244/190 FT = 20%

BRACING-

TOP CHORD

BOT CHORD

WEBS

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.2

2x4 SP No.2 *Except* **BOT CHORD**

4-23,5-21,11-14,25-26; 2x4 SP No.3, 15-17; 2x4 SP M 31

2x4 SP No.3 *Except* **WEBS**

12-13: 2x6 SP No 2

(lb/size) 2=398/0-3-8, 23=1724/0-3-8, 13=1219/0-3-0

Max Horz 2=426(LC 9)

Max Uplift 2=-429(LC 8), 23=-702(LC 12), 13=-378(LC 13)

Max Grav 2=402(LC 23), 23=1724(LC 1), 13=1219(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-512/752, 3-4=-422/453, 4-5=-1369/558, 5-6=-1607/741, 6-7=-1374/711,

7-8=-1104/688, 8-9=-990/621, 9-10=-1188/669, 10-11=-678/431, 11-12=-614/328,

12-13=-1212/552

BOT CHORD 2-24=-724/477, 23-24=-724/477, 22-23=-1447/735, 4-22=-1209/632, 5-20=-394/264,

19-20=-589/1284, 18-19=-679/1405, 16-18=-502/1103, 15-16=-443/895

3-23=-789/924, 20-22=-549/308, 4-20=-882/1674, 6-18=-442/307, 7-18=-135/398, 8-16=-377/255, 9-16=-179/403, 10-15=-704/299, 12-15=-480/1020

NOTES-

WEBS

- 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl.,

 (Constitution of the constitution of t GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical right exposed; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=429, 23=702, 13=378.
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



Structural wood sheathing directly applied or 3-11-2 oc purlins,

6-18, 8-18, 8-16

except end verticals, and 2-0-0 oc purlins (5-5-12 max.): 7-9.

Rigid ceiling directly applied or 2-10-4 oc bracing. Except:

8-4-0 oc bracing: 16-18

1 Row at midpt

Walter P. Finn PE No.22839 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

August 26,2019

📤 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MTEk® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly demage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITPH Quality Criterie, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



6904 Parke East Blvd Tampa, FL 36610

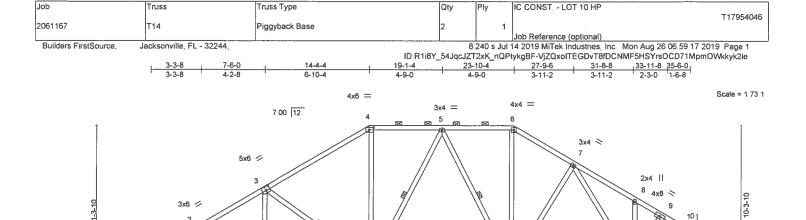


Plate Offsets (X,Y)-	3-3-8 7-6-0 3-3-8 4-2-8	14-4-4 6-10-4 ,0-2-4], [20:0-4-8,0-1-8]	23-10-4 9-6-0	31-8-8 7-10-4	33-11-8 2-3-0
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES Code FBC2017/TPI2014	CSI. TC 0.74 BC 0.92 WB 0.87 Matrix-MS	DEFL. in (loc) Vert(LL) -0.26 14-16 Vert(CT) -0.42 14-16 Horz(CT) -0.15 11	Vdefl L/d >999 240 >955 180 n/a n/a	PLATES GRIP MT20 244/190 Weight: 255 lb FT = 20%

22 15

4x6 =

BRACING-

TOP CHORD

BOT CHORD

WEBS

23

3x8 =

1 Row at midpt

25

Structural wood sheathing directly applied or 3-9-12 oc purlins, except end verticals, and 2-0-0 oc purlins (5-4-5 max.): 4-6. Rigid ceiling directly applied or 2-2-0 oc bracing.

3-16, 5-16, 5-14

12

2x4 ||

6x8 =

16

3x8 =

LUMBER-

2x4 SP No.2

TOP CHORD 2x4 SP No.2 *Except* BOT CHORD

2-19.8-12: 2x4 SP No.3

WEBS 2x4 SP No.3 *Except*

1-21: 2x4 SP No.2, 9-11: 2x6 SP No.2

3x6 /

21

348

1-11-3 2-11-3

REACTIONS. (lb/size) 11=1341/0-3-0, 21=1240/0-3-8

Max Horz 21=369(LC 11)

Max Uplift 11=-439(LC 13), 21=-425(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

19

2x4 ||

1-2=-1736/856, 2-3=-1746/866, 3-4=-1429/755, 4-5=-1160/726, 5-6=-1008/635, TOP CHORD

6-7=-1209/686, 7-8=-688/449, 8-9=-618/359, 20-21=-1240/552, 1-20=-1238/618,

17

3x4 =

BOT CHORD 2-18=-322/210, 17-18=-831/1696, 16-17=-752/1600, 14-16=-482/1136, 13-14=-409/906 WEBS 3-16=-573/383, 4-16=-157/422, 5-16=-119/284, 5-14=-404/290, 6-14=-188/413,

7-14=-162/264, 1-18=-621/1406, 9-13=-432/1022, 7-13=-742/308, 18-20=-548/475

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18, MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Bearing at joint(s) 21 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 11=439, 21=425.
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



Walter P. Finn PE No.22839 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd, Tampa FL 33610 Date:

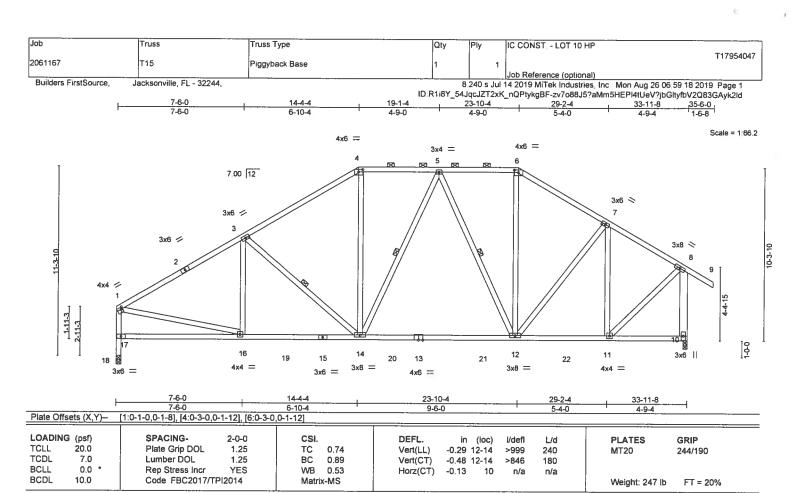
August 26,2019

읂

🛕 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MT els6 connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/THIS Quality Criteria, DSB-89 and BCSI Building Component Safety Information. available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





BRACING-

TOP CHORD

BOT CHORD

WEBS

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3 *Except*

1-18: 2x4 SP No.2, 8-10: 2x6 SP No.2

REACTIONS. (lb/size) 10=1341/0-3-0, 18=1240/0-3-8

Max Horz 18=369(LC 11)

Max Uplift 10=-439(LC 13), 18=-425(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-3=-1533/704, 3-4=-1317/712, 4-5=-1065/683, 5-6=-930/621, 6-7=-1124/652,

7-8=-898/505, 17-18=-1240/552, 1-17=-1168/567, 8-10=-1295/686

BOT CHORD 16-17=-565/550, 14-16=-639/1430, 12-14=-434/1042, 11-12=-334/734 WEBS

3-14=-448/340, 4-14=-149/393, 5-14=-113/279, 5-12=-401/286, 6-12=-139/346,

7-12=-194/331, 7-11=-581/308, 1-16=-496/1187, 8-11=-410/993

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) *This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Bearing at joint(s) 18 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 10=439, 18=425,
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



Structural wood sheathing directly applied or 3-7-13 oc purlins,

3-14, 5-14, 5-12

except end verticals, and 2-0-0 oc purlins (5-7-3 max.): 4-6.

Rigid ceiling directly applied or 7-4-9 oc bracing.

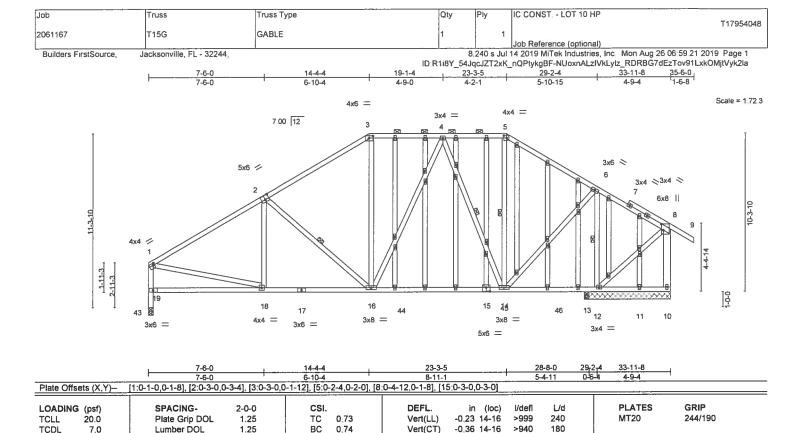
1 Row at midpt

Walter P. Finn PF No.22839 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

August 26,2019

⚠ WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters show, and is for an individual building component, not a inuss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property anage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP/1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandra, VA 22314.





LUMBER-

BCLL

BCDL

TOP CHORD 2x4 SP No.2

0.0

10.0

2x4 SP No.2 BOT CHORD

2x4 SP No.3 *Except* WEBS

1-43: 2x4 SP No.2, 8-10: 2x6 SP No.2

OTHERS 2x4 SP No.3 **BRACING-**

Horz(CT)

-0.12

10

TOP CHORD Structural wood sheathing directly applied or 4-2-15 oc purlins,

n/a

except end verticals, and 2-0-0 oc purfins (6-0-0 max.); 3-5.

Weight: 357 lb

FT = 20%

BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing WEBS 1 Row at midpt 2-16, 4-14, 5-14, 6-12

n/a

Rep Stress Incr

Code FBC2017/TPI2014

REACTIONS. All bearings 5-7-0 except (jt=length) 43=0-3-8, 13=0-3-8. (lb) - Max Horz 43=364(LC 11)

Max Uplift All uplift 100 ib or less at joint(s) except 12-486(LC 13), 10-131(LC 25), 43-367(LC 12) Max Grav All reactions 250 lb or less at joint(s) 10, 11, 13 except 12=1365(LC 2), 43=1042(LC 1)

WB 0.47

Matrix-MS

FORCES. (ib) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown

1-2=-1252/563, 2-3=-975/561, 3-4=-809/551, 4-5=-552/423, 5-6=-620/418, TOP CHORD

19-43=-1042/459, 1-19=-971/475

BOT CHORD 18-19=-558/544, 16-18=-518/1201, 14-16=-325/663 WEBS

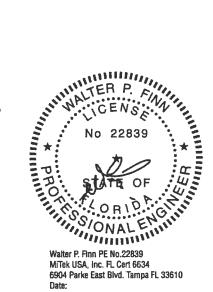
2-16-495/354, 4-16-165/418, 4-14-551/314, 6-14-311/869, 6-12-1250/657,

YES

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl. GCpi=0.18, MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical right exposed C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Provide adequate drainage to prevent water ponding.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 9) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 10) Bearing at joint(s) 43 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 486 lb uplift at joint 12, 131 lb uplift at joint 10 and 367 lb uplift at joint 43.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

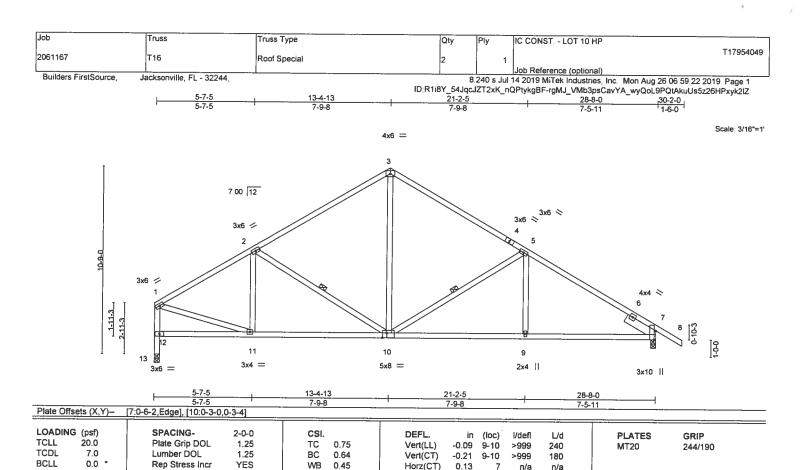


August 26,2019

🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITeK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSITH1 Quality Criteria, DSB-69 and BCSI Building Component Safety Information. available from Truss Plate Institute, 218 N Lee Street, Suite 312, Alexandria, VA 22314.





BRACING-

TOP CHORD

BOT CHORD

WEBS

LUMBER-TOP CHORD

10.0

BCDI

BOT CHORD WEBS

2x4 SP No.2 2x4 SP No.2 2x4 SP No.3 *Except*

1-13: 2x4 SP No.2

SLIDER Right 2x6 SP No.2 1-11-8

REACTIONS. (lb/size) 7=1138/0-3-8, 13=1053/0-3-8

Max Horz 13=-291(LC 8)

Max Uplift 7=-431(LC 13), 13=-367(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

Code FBC2017/TPI2014

TOP CHORD 1-2=-1232/526, 2-3=-1076/548, 3-5=-1066/545, 5-7=-1511/659, 12-13=-1053/454,

1-12=-1005/461 **BOT CHORD**

11-12=-414/469, 10-11=-448/1156, 9-10=-419/1235, 7-9=-419/1235 2-10=-362/318, 3-10=-264/668, 5-10=-616/414, 5-9=0/278, 1-11=-452/1023 **WEBS**

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

Matrix-MS

- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 6) Bearing at joint(s) 13 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 431 lb uplift at joint 7 and 367 lb uplift at joint 13.



Weight: 165 lb

Structural wood sheathing directly applied or 2-4-4 oc purlins,

2-10, 5-10

Rigid ceiling directly applied or 8-9-14 oc bracing.

except end verticals.

1 Row at midpt

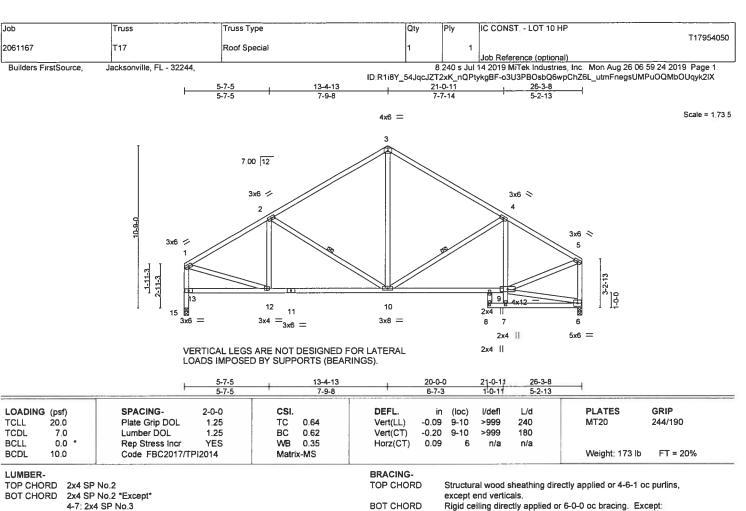
FT = 20%

Walter P. Finn PE No.22839 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

August 26,2019

🔼 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters show, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314





4-7: 2x4 SP No.3 2x4 SP No.3 *Except*

WEBS

1-15: 2x4 SP No.2

10-0-0 oc bracing: 7-9

WEBS 1 Row at midpt 2-10, 4-10

REACTIONS. (lb/size) 6=981/0-3-8, 15=967/0-3-8

Max Horz 15=240(LC 9)

Max Uplift 6=-328(LC 13), 15=-335(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-1122/494, 2-3=-930/484, 3-4=-932/484, 4-5=-1029/445, 13-15=-967/407, 1-13=-920/418, 5-6=-934/414

12-13=-386/402, 10-12=-486/1043, 9-10=-342/869 BOT CHORD

2-10=-388/339, 3-10=-201/530, 4-10=-306/285, 1-12=-350/926, 5-9=-347/902 WEBS

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 6) Bearing at joint(s) 15 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 328 lb uplift at joint 6 and 335 lb uplift at joint 15.



Walter P. Finn PE No.22839 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

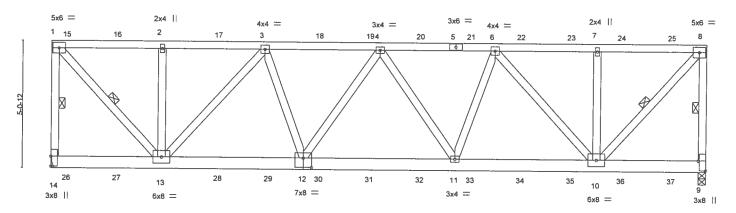
August 26,2019

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job Truss Truss Type IC CONST. - LOT 10 HP Qty T17954051 2061167 T18 Flat Girder 1 Job Reference (optional) Builders FirstSource Jacksonville, FL - 32244, 8 240 s Jul 14 2019 MiTek Industries, Inc. Mon Aug 26 06 59 26 2019 Page 1 ID.R1i8Y_54JqcJZT2xK_nQPtykgBF-kScqqtP661Md3WrxDm0MzBK5_UXEq9Mhtg4UYiyk2IV 13-0-0 17-6-12 21-7-10 26-0-0 4-6-12 4-6-12

Scale = 1 44 2



4-4-6 4-4-6 Plate Offsets (X,Y)—		10-0-0 5-7-10 , 0-4- 8]	-	16-0-0 6-0-0	-	21-7-10 5-7-10		26-0-0 4-4-6	
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code FBC2017/T	2-0-0 1.25 1.25 NO PI2014	CSI. TC 0.65 BC 0.60 WB 0.99 Matrix-MS	Vert(LL) 0.1	in (loc) 17 11-12 20 11-12 05 9	l/defl L/ >999 24 >999 18 n/a n/	0	PLATES MT20 Weight: 189 lb	GRIP 244/190
LUMBER-				BRACING-		-			

TOP CHORD

BOT CHORD

WEBS

NOTES-

TOP CHORD 2x4 SP No 2 BOT CHORD 2x6 SP No 2

2x4 SP No.3 WEBS

REACTIONS.

(lb/size) 14=2134/Mechanical, 9=2048/0-3-8 Max Uplift 14=-1478(LC 4), 9=-1412(LC 4)

FORCES. (lb) - Max, Comp./Max, Ten, - All forces 250 (lb) or less except when shown

TOP CHORD 1-14=-2006/1452, 1-2=-1606/1105, 2-3=-1606/1105, 3-4=-2682/1827, 4-6=-2685/1830,

6-7=-1605/1104, 7-8=-1605/1104, 8-9=-1951/1392

BOT CHORD 12-13=1734/2492, 11-12=1991/2847, 10-11=1738/2496 WEBS

1-13=-1626/2369, 2-13=-488/490, 3-13=-1352/960, 3-12=-293/611, 4-12=-305/304, 4-11=-299/299, 6-11=-289/611, 6-10=-1359/967, 7-10=-482/484, 8-10=-1626/2369

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Provide adequate drainage to prevent water ponding.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1478 lb uplift at joint 14 and 1412 lb
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 159 lb down and 158 lb up at 0-7-12, 165 lb down and 156 lb up at 2-7-12, 165 lb down and 156 lb up at 4-7-12, 165 lb down and 156 lb up at 6-7-12, 165 lb down and 156 lb up at 8-7-12, 165 lb down and 156 lb up at 10-7-12, 165 lb down and 156 lb up at 12-7-12, 165 lb down and 156 lb up at 14-7-12, 165 lb down and 156 lb up at 16-7-12, 165 lb down and 156 lb up at 18-7-12, 165 lb down and 156 lb up at 20-7-12, and 165 lb down and 156 lb up at 22-7-12, and 165 lb down and 156 lb up at 24-7-12 on top chord, and 97 lb down and 27 lb up at 0-7-12, 87 lb down and 32 lb up at 2-7-12, 87 lb down and 32 lb up at 4-7-12, 87 lb down and 32 lb up at 6-7-12, 87 lb down and 32 lb up at 8-7-12, 87 lb down and 32 lb up at 10-7-12, 87 lb down and 32 lb up at 12-7-12, 87 lb down and 32 lb up at 14-7-12, 87 lb down and 32 lb up at 16-7-12, 87 lb down and 32 lb up at 18-7-12, 87 lb down and 32 lb up at 20-7-12, and 87 lb down and 32 lb up at 22-7-12, and 87 lb down and 32 lb up at 24-7-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 9) in the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

No 22839 No 22839 No 22839 Walter P. Finn PE No. 22839

Structural wood sheathing directly applied or 3-3-1 oc purlins,

1-14, 8-9, 1-13, 8-10

Rigid ceiling directly applied or 5-3-5 oc bracing

except end verticals

1 Row at midpt

Walter P. Finn PF No 22839 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

August 26,2019

Continued on page 2

A WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters show, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and permanent bracing of the prevent collapse with possible personal injury and permanent bracing the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Sefety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type	Qty	Ply	IC CONST, - LOT 10 HP
			l		T17954051
2061167	T18	Flat Girder	1	1	
					Job Reference (optional)

Builders FirstSource,

Jacksonville, FL - 32244,

8 240 s Jul 14 2019 MiTek Industries, Inc. Mon Aug 26 06 59:26 2019 Page 2 ID.R1i8Y_54JqcJZT2xK_nQPtykgBF-kScqqtP661Md3WrxDm0MzBK5_UXEq9Mhtg4UYiyk2IV

LOAD CASE(S) Standard

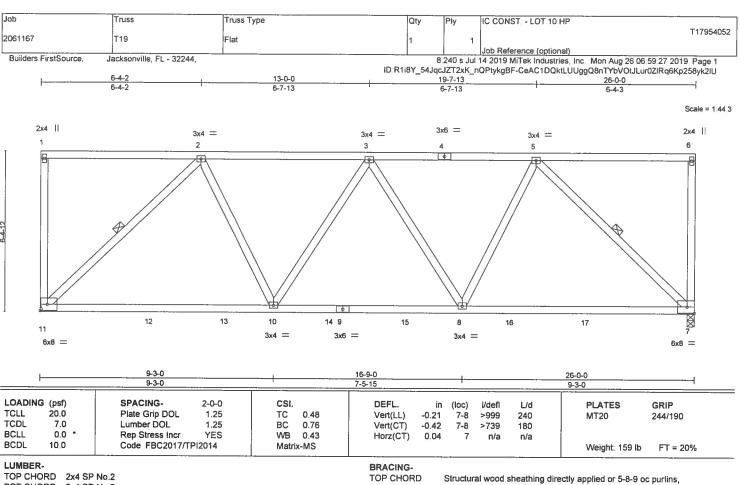
1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-8=-54, 9-14=-20

Concentrated Loads (lb)

Vert: 13=-65(B) 2=-109(B) 3=-109(B) 15=-123(B) 16=-109(B) 17=-109(B) 18=-109(B) 19=-109(B) 20=-109(B) 21=-109(B) 22=-109(B) 23=-109(B) 24=-109(B) 25=-109(B) 25=-109(





BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3

except end verticals.

BOT CHORD

Rigid ceiling directly applied or 8-3-6 oc bracing. 2-11.5-7

WEBS

1 Row at midot

REACTIONS. (lb/size) 11=951/Mechanical, 7=951/0-3-8

Max Uplift 11=-440(LC 8), 7=-440(LC 8) Max Grav 11=958(LC 2), 7=958(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-969/420, 3-5=-969/420

BOT CHORD 10-11=-382/766, 8-10=-503/1057, 7-8=-382/766

WEBS 2-11=-1045/531, 2-10=-89/473, 5-8=-89/473, 5-7=-1045/531

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 440 lb uplift at joint 11 and 440 lb uplift at joint 7.



Walter P. Finn PE No.22839 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

August 26,2019

🔔 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design, Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPIT Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Swite 312, Alexandria, VA 22314.



Job Truss Truss Type Qty IC CONST. - LOT 10 HP T17954053 2061167 T20 Roof Special Job Reference (optional) 8 240 s Jul 14 2019 MiTek Industries, Inc. Mon Aug 26 06 59 28 2019 Page 1 Builders FirstSource, Jacksonville, FL - 32244, ID_R1iBY_54JqcJZT2xK_nQPtykgBF-gqjaFZRMefcLlq?KLB3q2cPVQl8flBuzLzZbdbyk2lT 6-3-8 <u>11-</u>11-15 23-0-0 26-0-0 3-0-0 6-3-8 5-8-8 Scale 1/4"=1" 2x4 || 3x6 = 3x4 = 3x4 = 5x6 = 7.00 12 2x4 || 3x6 < 3-2-13 4x8 = 2x 2 15 16 13 18 12 11 10 3x4 = 3x6 = 6x8 == 3x8 = 2x4 || VERTICAL LEGS ARE NOT DESIGNED FOR LATERAL LOADS IMPOSED BY SUPPORTS (BEARINGS). 9-1-12 18-3-8 9-1-12 9-1-11 4-8-8 [5:0-3-0,0-1-12], [9:0-2-12,0-2-0] Plate Offsets (X,Y)-DEFL L/d **PLATES** GRIP LOADING (psf) SPACING-CSI TCLL Plate Grip DOL 1.25 TÇ 0.39 Vert(LL) -0.20 13-14 >999 240 MT20 244/190 20.0 TCDL 7.0 Lumber DOL 1.25 вс 0.93 Vert(CT) -0.37 13-14 >841 180 0.0 Rep Stress Incr YES WB 0.48 Horz(CT) 0.04 BCLL **BCDL** Code FBC2017/TPI2014 Matrix-MS Weight: 184 lb FT = 20% 10.0 LUMBER-**BRACING-**TOP CHORD 2x4 SP No.2 TOP CHORD Structural wood sheathing directly applied or 5-3-14 oc purlins, **BOT CHORD** 2x4 SP No.2 *Except* except end verticals. BOT CHORD 6-10: 2x4 SP No.3 Rigid ceiling directly applied or 2-2-0 oc bracing, WEBS 2x4 SP No.3 *Except* WEBS 1 Row at midpt 2-14, 4-11

REACTIONS.

7-15: 2x4 SP No.2

(lb/size) 14=951/Mechanical, 15=951/0-3-8

Max Horz 14=-216(LC 13)

Max Uplift 14=-442(LC 8), 15=-280(LC 8)

Max Grav 14=985(LC 2), 15=951(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-4=-822/391, 4-5=-716/423, 5-6=-1134/600, 6-7=-1112/468, 8-15=-951/389,

7-8=-905/390

BOT CHORD 13-14=-262/639, 11-13=-316/861, 6-9=-278/252

2-14=-978/525, 2-13=-150/511, 4-11=-251/204, 9-11=-229/761, 5-9=-244/378, WEBS

7-9=-366/928

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Provide adequate drainage to prevent water ponding.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Bearing at joint(s) 15 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 442 lb uplift at joint 14 and 280 lb uplift at joint 15.

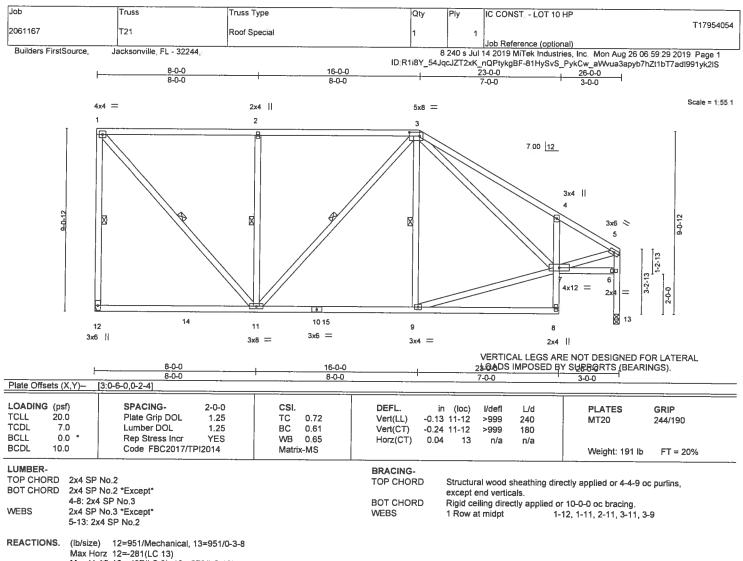


Walter P. Finn PE No.22839 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

August 26,2019

🛕 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





Max Uplift 12=-437(LC 8), 13=-273(LC 13) Max Grav 12=1006(LC 2), 13=951(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-12=-882/526, 1-2=-649/358, 2-3=-649/358, 3-4=-1218/667, 4-5=-1125/451,

6-13=-951/364, 5-6=-900/363

BOT CHORD 11-12=-212/278, 9-11=-157/693, 4-7=-417/374

WEBS 1-11=-528/952, 2-11=-495/383, 7-9=-162/685, 3-7=-367/504, 5-7=-382/970

NOTES-

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Provide adequate drainage to prevent water ponding.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Bearing at joint(s) 13 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 437 lb uplift at joint 12 and 273 lb uplift at joint 13.



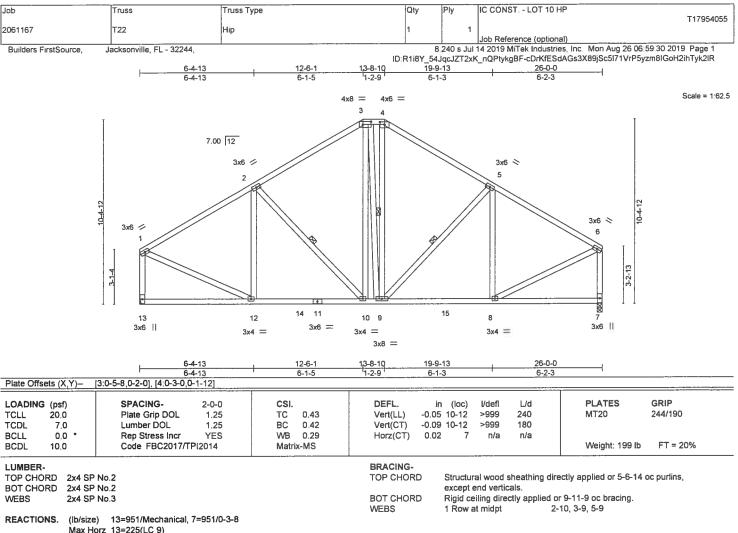
Walter P. Finn PE No.22839 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

August 26,2019

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





Max Horz 13=225(LC 9)

Max Uplift 13=-329(LC 12), 7=-327(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown 1-2=-924/420, 2-3=-835/482, 3-4=-762/478, 4-5=-836/482, 5-6=-898/411,

TOP CHORD 1-13=-895/421, 6-7=-897/421

BOT CHORD 10-12-351/875, 9-10-177/672, 8-9-274/716

WEBS 2-10=-299/254, 3-10=-142/262, 4-9=-148/276, 5-9=-270/241, 1-12=-283/771,

6-8=-285/766

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 329 lb uplift at joint 13 and 327 lb uplift at joint 7.



Walter P. Finn PE No.22839 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

August 26,2019

🛕 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information. available from Truss Plate Institute, 218 N Lee Street, Suite 312, Alexandria, VA 22314.



Job Truss Truss Type IC CONST. - LOT 10 HP Qtv Ply T17954056 2061167 T23 Common 1 ob Reference (optional) Builders FirstSource, Jacksonville, FL - 32244, 8.240 s Jul 14 2019 MiTek Industries, Inc. Mon Aug 26 06 59 31 2019 Page 1 ID_R1i8Y_54JqcJZT2xK_nQPtykgBF-4PPjtaTFxa_w9Hkv0JcXgE1z9VGCVaQQ1xnFEwyk2IQ 13-1-5 7-9-8 20-10-13 Scale: 3/16"=1" 4x6 = 7.00 12 3x6 / 3x6 < 10-9-0 3x6 💸 5 3-2-13 12 9 13 10 8 7 3x6 || 3x6 = 3x4 = 3x8 = 3x4 = 3x6 || 20-10-13 26-0-0 7-9-8 7-9-8 LOADING (psf) SPACING-2-0-0 CSI. DEFL. in (loc) l/defl L/d **PLATES** GRIP TCLL 20.0 Plate Grip DOL 1.25 TC 0.62 Vert(LL) -0.077-8 >999 240 MT20 244/190 1.25 TCDL 7.0 Lumber DOL BC 0.55 Vert(CT) -0.14 7-8 >999 180 BCLL 0.0 Rep Stress Incr YES WB 0.37 0.02 Horz(CT) 6 n/a n/a BCDI 10.0 Code FBC2017/TPI2014 Matrix-MS Weight: 168 lb FT = 20%LUMBER-**BRACING-**TOP CHORD 2x4 SP No.2 TOP CHORD Structural wood sheathing directly applied or 4-10-7 oc purlins, except end verticals.

BOT CHORD

WEBS

Rigid ceiling directly applied or 9-6-6 oc bracing.

2-8.4-8

1 Row at midpt

2x4 SP No.2 **BOT CHORD** 2x4 SP No.3 WERS

(lb/size) 11=951/Mechanical, 6=951/0-3-8

Max Horz 11=235(LC 9)

Max Uplift 11=-332(LC 12), 6=-331(LC 13) Max Grav 11=955(LC 19), 6=953(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-886/401, 2-3=-842/460, 3-4=-842/460, 4-5=-850/389, 1-11=-923/414,

5-6=-925/415

BOT CHORD 8-10=-384/900, 7-8=-282/703

2-10=-258/200, 2-8=-276/261, 3-8=-174/437, 4-7=-284/211, 1-10=-321/817, WEBS

5-7=-324/813

REACTIONS.

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

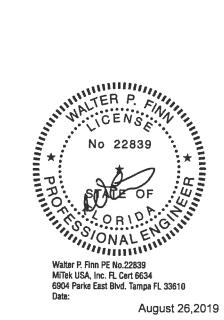
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.

6) Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 332 lb uplift at joint 11 and 331 lb uplift at joint 6.



August 26,2019

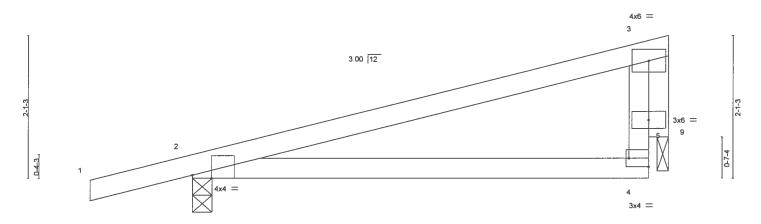
MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters show, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property danage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Ansi/TPH Quality Criteria, DSB-99 and BCSI Building Compor Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



6904 Parke East Blvd Tampa, FL 36610

Job	Truss	Truss Type	(Qty	Pły	IC CONST LOT 10 HP	
						T1795	54057
2061167	T24	MONO TRUSS	[1	6	1		
						Job Reference (optional)	
Builders FirstSource,	Jacksonville, FL - 32244,			8.	240 s Jul	14 2019 MiTek Industries, Inc. Mon Aug 26 06:59:32 2019 Page	e 1
			ID:R1i8	Y_54Jqc	IZT2xK_n	QPtykgBF-Zcz54wUtit6nnRJ5a07mCSaA0vcgE1pZGbXpmMyk2	IP.
L	-1-6-0		7-	-0-0			
	1-6-0		7-	0-0			

Scale = 1 16.3



		L				7-0-	-0					1
						7-0	0					1
Plate Off	sets (X,Y)-	[2:0-3-6.Edge], [4:Edge.0	1-81									
-		T		1							1	
LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.48	Vert(LL)	0.17	4-8	>499	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	ВС	0.47	Vert(CT)	0.14	4-8	>581	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.36	Horz(CT)	-0.00	2	n/a	n/a		
BCDL	10.0	Code FBC2017/TF	12014	Matri	x-MR	, ,					Weight: 27 lb	FT = 20%
				1								

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

OTHERS 2x4 SP No.3

REACTIONS. (lb/size) 2=345/0-3-8, 9=222/0-2-0

Max Horz 2=104(LC 8)

Max Uplift 2=-282(LC 8), 9=-179(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

BOT CHORD 2-4=-286/165

WEBS 3-9=-243/412

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Bearing at joint(s) 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 9.
 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 282 lb uplift at joint 2 and 179 lb uplift at joint 9.



Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 8-10-10 oc bracing.

except end verticals.

6904 Parke East Blvd. Tampa FL 33610 Date:

August 26,2019

🛕 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overail building design, Bracing indicated is to prevent bucking of individual truss web and/or chard members only. Additional temporary and permanent bracing is always required for stability and to prevent ucbling of individual truss web and/or chard members only. Additional temporary and permanent bracing is always required for stability and to prevent ucbling of individual truss web and/or chard members only. Additional temporary and permanent bracing is always required for stability and to prevent ucbling of individual truss with the property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type	Qty	Ply	IC CONST LOT 10 HP	
2061167	T24G	Monopitch Supported Gable	2	1	1	T17954058
L					Job Reference (optional)	
Builders FirstSource,	Jacksonville, FL - 32244,			8 240 s Jul	14 2019 MiTek Industries, Inc. Mon Aug 26 06 59	33 2019 Page 1
			ID:R1i8Y_54	4JqcJZT2xi	K_nQPtykgBF-1oXTIGVVTBEeObtl8ke?lf7OxJ0uz*	Y2iUFGMloyk2IO
	1-6-0		7-0-0			,
1	1-6-0		7-0-0			 -

Scale = 1:15.8

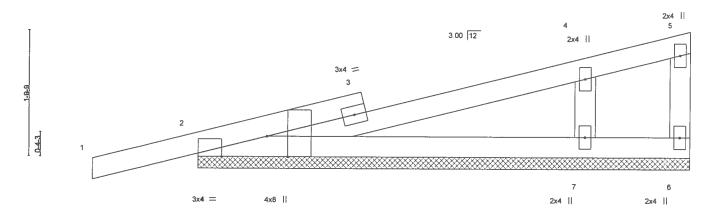


Plate Offsets (X,Y)—	2:0-3-8,Edge), [2:0-7-12,Edge]		7-0-0	
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES Code FBC2017/TPI2014	CSI. TC 0.28 BC 0.22 WB 0.10 Matrix-S	DEFL. in (loc) I/defl L/d Vert(LL) -0.00 1 n/r 120 Vert(CT) 0.01 1 n/r 120 Horz(CT) -0.00 6 n/a n/a	PLATES GRIP MT20 244/190 Weight: 29 lb FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x4 SP No.2

WEBS 2x4 SP No.3 **OTHERS**

2x4 SP No.3

BRACING-

TOP CHORD **BOT CHORD**

7-0-0

Structural wood sheathing directly applied or 7-0-0 oc purlins,

except end verticals.

Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. (lb/size) 2=256/7-0-0, 6=-112/7-0-0, 7=444/7-0-0

Max Horz 2=92(LC 8)

Max Uplift 2=-166(LC 8), 6=-112(LC 1), 7=-208(LC 12) Max Grav 2=256(LC 1), 6=48(LC 12), 7=444(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WEBS 4-7=-305/340

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable requires continuous bottom chord bearing.
- 4) Gable studs spaced at 2-0-0 oc.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 7) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 166 lb uplift at joint 2, 112 lb uplift at joint 6 and 208 lb uplift at joint 7.



Walter P. Finn PE No.22839 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

August 26,2019

🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 rev., 10:03/2015 BEFORE USE. Design valid for use only with MTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Compositions available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type	Qty	Ply		IC CONST LOT 10 HP	
2061167	V01	GABLE	2		1		T17954059
						Job Reference (optional)	
Builders FirstSource,	Jacksonville, FL - 32244,			8.240 s	Jul	14 2019 MiTek Industries, Inc. Mon Aug 26 (06 59 34 2019 Page 1
			ID:R1i8Y	54JqcJZ1	2xK	_nQPtykgBF-V_5rVcW7EVNV0ISUhRAEHtfl	JUIO5i0tsjv0vqEyk2IN
		. 7.	4-2		- 1	9-10-2	

7-4-2

2x4 || 8 00 12 2x4 ||

LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 °	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES	CSI. TC 0.61 BC 0.09 WB 0.06	DEFL. in (loc) I/defl L/d Vert(LL) 0.06 5 n/r 120 Vert(CT) 0.02 5 n/r 120 Horz(CT) -0.00 1 n/a n/a	PLATES GRIP MT20 244/190
BCDL 10.0	Code FBC2017/TPI2014	Matrix-S		Weight: 40 lb FT = 20%

2x4 ||

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No.3 WEBS 2x4 SP No.3 OTHERS

BRACING-TOP CHORD

2x4 ||

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. BOT CHORD

Rigid ceiling directly applied or 6-0-0 oc bracing, Except:

10-0-0 oc bracing: 1-9,

2-6-0

REACTIONS. All bearings 7-4-2.

(lb) - Max Horz 9=274(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 9, 7 except 6=-308(LC 9), 8=-177(LC 12) Max Grav All reactions 250 lb or less at joint(s) 1, 9, 7 except 6=338(LC 19), 8=253(LC 19)

2x4 /

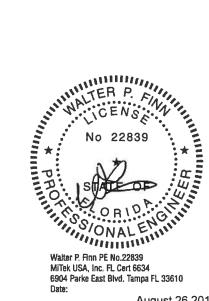
FORCES. (ib) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

1-2=-387/256, 2-3=-255/144, 3-4=-358/201, 4-6=-383/589 TOP CHORD

1-9=-348/230 BOT CHORD

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Gable requires continuous bottom chord bearing.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 9, 7 except (jt=lb) 6=308, 8=177.



Date:

August 26,2019

Scale = 1 35 7

🛕 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property anaage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314



Job Truss Truss Type Qty IC CONST. - LOT 10 HP T17954060 2061167 V02 Valley Job Reference (optional) Builders FirstSource Jacksonville, FL - 32244, 8 240 s Jul 14 2019 MITek Industries, Inc. Mon Aug 26 06 59 35 2019 Page 1 ID R1i8Y_54JqcJZT2xK_nQPtykgBF-zAeDjyWr?oVMev1gF9hTq4Cm66kvRSr?yZITNhyk2lM 16-2-5 4x4 = Scale = 1 34 4 3 8.00 12 2x4 || 2x4 || 3x6 / 9 8 7 6 3x6 N 2x4 || 3x6 = 2x4 || 2x4 || 16-2-5 16-1-15 LOADING (psf) SPACING-2-0-0 CSI. DEFL. in I/defl L/d (loc) **PLATES** GRIP TCLL 20.0 Plate Grip DOL 1.25 TC 0.17 Vert(LL) n/a n/a 999 MT20 244/190 TCDL 7.0 Lumber DOL 1.25 ВС 0.12 Vert(CT) n/a 999 n/a **BCLL** 0.0 Rep Stress Incr YES WB 0.08 0.00 Horz(CT) 5 n/a n/a **BCDL** 10.0 Code FBC2017/TPI2014 Matrix-S Weight: 65 lb FT = 20% LUMBER-**BRACING-**TOP CHORD 2x4 SP No 2 TOP CHORD

BOT CHORD 2x4 SP No.2 **OTHERS** 2x4 SP No.3

BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 16-1-9.

(lb) - Max Horz 1=158(LC 9)

Max Uplift All uplift 100 lb or less at joint(s) 1, 5 except 9=-257(LC 12), 6=-257(LC 13) Max Grav All reactions 250 lb or less at joint(s) 1, 5, 7 except 9=374(LC 19), 6=374(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WEBS 2-9=-306/274, 4-6=-306/274

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 9=257, 6=257.



Walter P. Finn PE No.22839 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

August 26,2019

🛕 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



T17954061 al) es, Inc. Mon Aug 26 06:59:36 2019 Page 1 6dDF3cspsCiNlixGW44AvG9BDV0v7yk2iL Scale = 1:27.0
es, Inc. Mon Aug 26 06 59 36 2019 Page 1 6dDF3cspsCiNIIxGW44AvG9BDV0v7yk2lL -
Scale = 1,27.0
Scale = 1,27.0
5
3x6 ≪
I
PLATES GRIP MT20 244/190 Weight: 48 lb FT = 20%

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 OTHERS 2x4 SP No.3 TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing

REACTIONS. All bearings 12-7-9.

(lb) - Max Horz 1=122(LC 9)

Max Uplift All uplift 100 lb or less at joint(s) 1, 5, 7 except 8=-214(LC 12), 6=-214(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 5, 7 except 8=307(LC 19), 6=306(LC 20)

FORCES. (ib) - Max. Comp./Max. Ten. - All forces 250 (ib) or less except when shown.

WEBS 2-8=-263/237, 4-6=-263/237

NOTES-

- Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) "This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5, 7 except (it=lb) 8=214, 6=214.



Walter P. Finn PE No.22839 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

August 26,2019

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and its for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job Truss Truss Type Qty IC CONST. - LOT 10 HP Ply T17954062 2061167 V04 Valley Job Reference (optional) Builders FirstSource. Jacksonville, FL - 32244, 8 240 s Jul 14 2019 MiTek Industries, Inc. Mon Aug 26 06 59 37 2019 Page 1 ID:R1i8Y_54JqcJZT2xK_nQPtykgBF-vZm_7eY0XQI4tCB3NajxvVH5CwPcvNyIPtEaRZyk2IK Scale = 1:20.7 4x4 = 2 8.00 12 F 90. ************************ 2x4 / 2x4 < 2x4 || 9-1-15 9-1-15 LOADING (psf) SPACING-2-0-0 CSI. DEFL in (loc) I/defl L/d **PLATES** GRIP TCLL 20.0 Plate Grip DOL 1.25 TC 0.20 Vert(LL) n/a n/a 999 MT20 244/190 TCDL 7.0 Lumber DOL 1.25 вс 0.17 Vert(CT) n/a n/a 999 BCLL 0.0 Rep Stress Incr YES WB 0.04 Horz(CT) 0.00 3 n/a n/a BCDL 10.0 Code FBC2017/TPI2014 Matrix-S Weight: 32 lb FT = 20% LUMBER-BRACING-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 OTHERS 2x4 SP No.3

TOP CHORD **BOT CHORD**

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 1=149/9-1-9, 3=149/9-1-9, 4=311/9-1-9

Max Horz 1=-85(LC 8)

Max Uplift 1=-67(LC 12), 3=-78(LC 13), 4=-86(LC 12) Max Grav 1=149(LC 1), 3=150(LC 20), 4=311(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

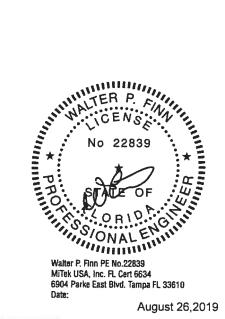
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4.



August 26,2019

📤 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job Truss Truss Type Qty IC CONST. - LOT 10 HP T17954063 2061167 V05 Valley Job Reference (optional) 8 240 s Jul 14 2019 MiTek Industries, Inc. Mon Aug 26 06 59 38 2019 Page 1 ID.R1i8Y_54JqcJZT2xK_nQPtykgBF-NIKML_ZeHjtwVMmFwHEASjqlfKmZeqYSeX_7z0yk2IJ Builders FirstSource, Jacksonville, FL - 32244, 2-10-2 2-10-2 Scale = 1:14.4 4x4 = 8.00 12 504 0-0-4 2x4 // 2x4 || 2x4 🔷 LOADING (psf) SPACING-2-0-0 CSI. DEFL. in l/defi L/d **PLATES** GRIP (loc) TCLL Plate Grip DOL 20.0 1.25 TC 0.09 Vert(LL) 999 244/190 n/a n/a MT20 TCDL 7.0 Lumber DOL 1.25 BC 0.06 Vert(CT) n/a n/a 999 BÇLL 0.0 * Rep Stress Incr WB YES 0.02 Horz(CT) 0.00 3 n/a n/a **BCDL** 10.0 Code FBC2017/TPI2014 Matrix-P Weight: 19 lb FT = 20% LUMBER-BRACING-

TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x4 SP No.2 **OTHERS** 2x4 SP No.3 TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 5-8-5 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 1=94/5-7-9, 3=94/5-7-9, 4=163/5-7-9

Max Horz 1=49(LC 9)

Max Uplift 1=-46(LC 12), 3=-53(LC 13), 4=-34(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4.



Walter P. Finn PE No.22839 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

August 26,2019

🛕 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing individual et all property incorporate this design into the overall building design. Bracing individual temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/THY Quality Criteria, DSB-89 and BCSI Building Component

Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

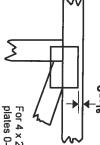


Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated.
Dimensions are in ft-in-sixteenths.
Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- 1 ₁₀" from outside edge of truss.

8

BOTTOM CHORDS

6

G

This symbol indicates the required direction of slots in connector plates.

* Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE

4 × 4

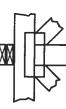
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only

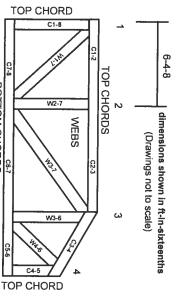
Industry Standards:

ANSI/TPI1: National Design Specification for Metal
Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing

BCSI: Desig

Design Standard for Bracing.
Building Component Safety Information
Guide to Good Practice for Handling,
Installing & Bracing of Metal Plate

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988 ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

© 2012 MiTek® All Rights Reserved



MiTek Engineering Reference Sheet: MII-7473 rev. 10/03/2015

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

- . Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- Cut members to bear tightly against each other.
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire relardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- Connections not shown are the responsibility of others
- Do not cut or alter truss member or plate without prior approval of an engineer.
- 17. Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.

T-BRACE / I-BRACE DETAIL WITH 2X BRACE ONLY

MII-T-BRACE 2

MiTek USA, Inc. Page 1 of 1



MiTek USA, Inc.

ENGINEERED BY

Note:	T-Bracing / I-B	racing to b	e used	when	contin	uous	lateral	bracing
	is impractical.	T-Brace / I	-Brace	must o	cover	90% c	of web	length.

Note: This detail NOT to be used to convert T-Brace / I-Brace webs to continuous lateral braced webs.

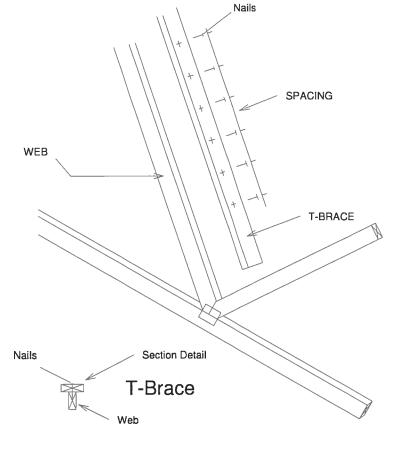
N	Nailing Pattern	
T-Brace size	Nail Size	Nail Spacing
2x4 or 2x6 or 2x8	10d (0.131" X 3")	6" o.c.

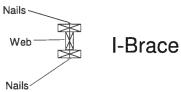
Note: Nail along entire length of T-Brace / I-Brace (On Two-Ply's Nail to Both Plies)

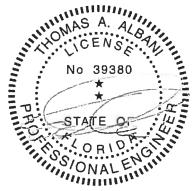
	Brace Size for One-Ply Truss			
		Continuous Iteral Bracing		
Web Size	1	2		
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace		
2x6	2x6 T-Brace	2x6 I-Brace		
2x8	2x8 T-Brace	2x8 I-Brace		

		e Size -Ply Truss
		Continuous teral Bracing
Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

T-Brace / I-Brace must be same species and grade (or better) as web member.







Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

SCAB-BRACE DETAIL

MII-SCAB-BRACE

MiTek USA, Inc.

Page 1 of 1

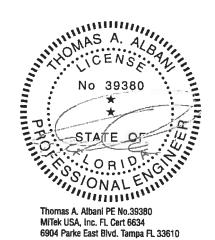


Note: Scab-Bracing to be used when continuous lateral bracing at midpoint (or T-Brace) is impractical. Scab must cover full length of web +/- 6".

THIS DETAIL IS NOT APLICABLE WHEN BRACING IS *** REQUIRED AT 1/3 POINTS OR I-BRACE IS SPECIFIED.

APPLY 2x SCAB TO ONE FACE OF WEB WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 6" O.C. SCAB MUST BE THE SAME GRADE, SIZE AND SPECIES (OR BETTER) AS THE WEB. MAXIMUM WEB AXIAL FORCE = 2500 lbs MAXIMUM WEB LENGTH = 12'-0" 2x4 MINIMUM WEB SIZE SCAB BRACE MINIMUM WEB GRADE OF #3 Nails Section Detail Scab-Brace Web

Scab-Brace must be same species grade (or better) as web member.

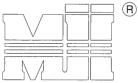


Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

STANDARD REPAIR TO REMOVE END VERTICAL (RIBBON NOTCH VERTICAL)

MII-REP05

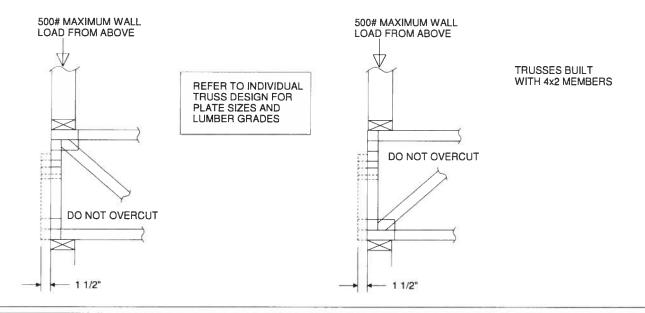
MiTek USA, Inc. Page 1 of 1

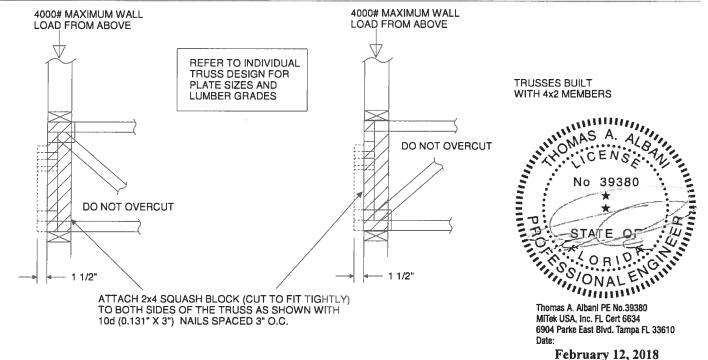


MiTek USA, Inc. ENGINEERED BY

- 1. THIS IS A SPECIFIC REPAIR DETAIL TO BE USED ONLY FOR ITS ORIGINAL INTENTION. THIS REPAIR DOES NOT IMPLY THAT THE REMAINING PORTION OF THE TRUSS IS UNDAMAGED. THE ENTIRE TRUSS SHALL BE INSPECTED TO VERIFY THAT NO FURTHER REPAIRS ARE REQUIRED. WHEN THE REQUIRED REPAIRS ARE PROPERLY APPLIED, THE TRUSS WILL BE CAPABLE OF SUPPORTING THE LOADS INDICATED.
- 2. ALL MEMBERS MUST BE RETURNED TO THEIR ORIGINAL POSITIONS BEFORE APPLYING REPAIR AND HELD IN PLACE DURING APPLICATION OF REPAIR.

 3. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID SPLITTING OF THE WOOD.
- 4. LUMBER MUST BE CUT CLEANLY AND ACCURATELY AND THE REMAINING WOOD MUST BE UNDAMAGED.
 5. THIS REPAIR IS TO BE USED FOR SINGLE PLY TRUSSES IN THE 4X_ORIENTATION ONLY.
 6. CONNECTOR PLATES MUST BE FULLY IMBEDDED AND UNDISTURBED.





1別對/

Standard Gable End Detail

MII-GE130-D-SP

Page 1 of 2

(2) - 10d NAILS

∕Trusses @ 24" o.c.

2x6 DIAGONAL BRACE SPACED 48" O.C.

ATTACHED TO VERTICAL WITH (4) -16d

HORIZONTAL BRACE

(SEE SECTION A-A)

TO BLOCKING WITH (5) - 10d NAILS.

NAILS AND ATTACHED

MiTek USA, Inc.



Typical x4 L-Brace Nailed To 2x_ Verticals W/10d Nails spaced 6" o.c. Vertical Stud SECTION B-B

Vertical Stud DIAGONAL BRACE (4) - 16d Nails 16d Nails Spaced 6" o.c. (2) - 10d Nails into 2x6 2x6 Stud or 2x4 No.2 of better Typical Horizontal Brace Nailed To 2x_ Verticals w/(4)-10d Nails SECTION A-A 2x4\Stud

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SPF BLOCK

- 10¢

NAILS

Roof Sheathing

1'-3"

Max.

24" Max

Diag. Brace

at 1/3 points

End Wall

if needed

DIAGONAL BRACE 4'-0" O.C. MAX TRUSS GEOMETRY AND CONDITIONS SHOWN ARE FOR ILLUSTRATION ONLY. Varies to Common Truss SEE INDIVIDUAL MITEK ENGINEERING DRAWINGS FOR DESIGN CRITERIA ** 3x4 =

PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

Diagonal Bracing Refer to Section A-A

L-Bracing Refer to Section B-B

1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS. 2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND

WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.

3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG.
ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.

4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 1x4 SRB OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.

 DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.
 CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)

7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR

TYPE TRUSSES.

10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.

11. NAILS DESIGN NAILS DESIGN	NATED 10d NATED 16d	ARE (0.13 ARE (0.13	1" X 3") AN 1" X 3.5")	D		
Minimum Stud Size Species	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
and Grade		Maximum Stud Length				
2x4 SP No. 3 / Stud	12" O.C.	3-9-13	4-1-1	5-9-6	7-1-3	11-5-7
2x4 SP No. 3 / Stud		3-5-4	3-6-8	5-0-2	6-10-8	10-3-13
2x4 SP No. 3 / Stud	24" O.C.	2-9-11	2-10-11	4-1-1	5-7-6	8-5-1

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING EXPOSURE D ASCE 7-98, ASCE 7-02, ASCE 7-05 130 MPH ASCE 7-10 160 MPH **DURATION OF LOAD INCREASE: 1.60**

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.



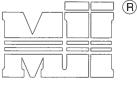
Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

Standard Gable End Detail

MII-GE130-SP

Page 1 of 2

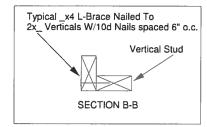
MiTek USA, Inc.



MiTek USA, Inc.

ENGINEERED BY 7140

DIAGONAL BRACE 4'-0" O.C. MAX



2

TRUSS GEOMETRY AND CONDITIONS SHOWN ARE FOR ILLUSTRATION ONLY.

Vertical Stud DIAGONAL BRACE (4) - 16d Nails 16d Nails Spaced 6" o.c. (2) - 10d Nails into 2x6 2x6 Stud or 2x4 No.2 of better Typical Horizontal Brace Nailed To 2x_ Verticals w/(4)-10d Nails SECTION A-A

> PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

Varies to Common Truss SEE INDIVIDUAL MITEK ENGINEERING DRAWINGS FOR DESIGN CRITERIA 3x4 =- Diagonal Bracing ** - L-Bracing Refer

Refer to Section A-A

to Section B-B

NOTE:

1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS. 2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND

WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.

3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.

4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 1x4 SRB

OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.

5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF

DIAPHRAM AT 4'-0" O.C. 6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6* O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)

GABLE STUD DEFLECTIÓN MEETS OR EXCEEDS L/240.

THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.

10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.

NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Roof Sheathi	ng
24" Max	
1'-3" Max.	(2) - 10d NAILS
	/*/ / · · · · · · · · · · · · · · · · ·
A	Trusses @ 24" o.d
Diag. Brace	2x6 DIAGONAL BRACE SPACED 48" O.C. ATTACHED TO VERTICAL WITH (4) -16d
at 1/3 points if needed	NAILS AND ATTACHED TO BLOCKING WITH (5) - 10d NAILS.
End Wall	HORIZONTAL BRACE
	(SEE SECTION A-A)

(4) - 8d (0.131" X2.5") NAILS MINIMUM, PLYWOOD

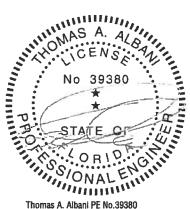
SHEATHING TO 2x4 STD SPF BLOCK

	Minimum Stud Size Species	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
Ì	and Grade		Maximum Stud Length				
	2x4 SP No. 3 / Stud	12" O.C.	4-0-7	4-5-6	6-3-8	8-0-15	12-1-6
	2x4 SP No. 3 / Stud	16" O.C.	3-8-0	3-10-4	5-5-6	7-4-1	11-0-1
	2x4 SP No. 3 / Stud	24" O.C.	3-0-10	3-1-12	4-5-6	6-1-5	9-1-15

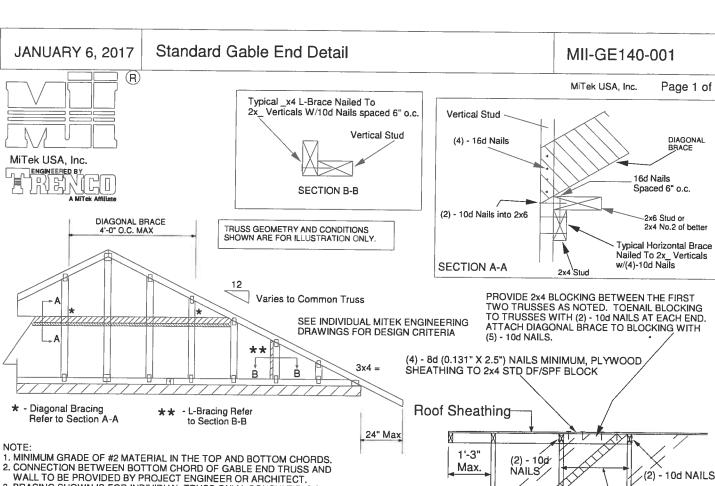
Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING EXPOSURE B or C ASCE 7-98, ASCE 7-02, ASCE 7-05 130 MPH ASCE 7-10 160 MPH **DURATION OF LOAD INCREASE: 1.60**

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610



Diag. Brace

at 1/3 points

End Wall

if needed

3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT

BRACING OF ROOF SYSTEM.

4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 1x4 SRB
OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF

DIAPHRAM AT 4'-0" O.C.

6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6* O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)

GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES

DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR

TYPE TRUSSES.

10. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Minimum Stud Size Species	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
and Grade		Maximum Stud Length				
2x4 DF/SPF Std/Stud	12" O.C.	3-10-1	3-11-7	5-7-2	7-8-2	11-6-4
2x4 DF/SPF Std/Stud	16" O.C.	3-3-14	3-5-1	4-10-2	6-7-13	9-11-11
2x4 DF/SPF Std/Stud	24" O.C.	2-8-9	2-9-8	3-11-7	5-5-2	8-1-12

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of web with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAXIMUM WIND SPEED = 140 MPH MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING EXPOSURE B or C ASCE 7-98, ASCE 7-02, ASCE 7-05 **DURATION OF LOAD INCREASE: 1.60**

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.



/Trusses @ 24" o.c.

2x6 DIAGONAL BRACE SPACED 48" O.C.

ATTACHED TO VERTICAL WITH (4) -16d

HORIZONTAL BRACE

(SEE SECTION A-A)

TO BLOCKING WITH (5) - 10d NAILS.

NAILS AND ATTACHED

Page 1 of 2

DIAGONAL

Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

Standard Gable End Detail

MII-GE170-D-SP



Page 1 of 2

2X6 SP OR SPF No. 2 DIAGONAL BRACE

2X6 SP OR SPF No. 2

(2) - 10d NAILS

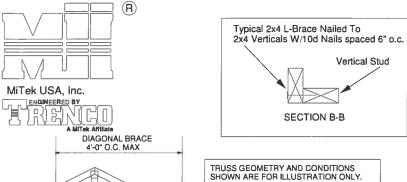
∕Trusses @ 24" o.c.

16d Nails

Spaced 6" o.c.

Typical Horizontal Brace

Nailed To 2x4 Verticals w/(4)-10d Nails



(2) - 10d Nails into 2x6 SECTION A-A Varies to Common Truss

SEE INDIVIDUAL MITEK ENGINEERING

24" Max

Diag. Brace

at 1/3 points

End Wall

if needed

DRAWINGS FOR DESIGN CRITERIA

3x4 =

Vertical Stud

(4) - 16d Nails

PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD,

(2) - 10d/

NAILS

SHEATHING TO 2x4 STD SPF BLOCK

Roof Sheathing

1'-0"

Max.

2X4 SP OR SPF No. 2

* - Diagonal Bracing Refer to Section A-A

- L-Bracing Refer to Section B-B

**

B

Ě

1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG.

ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.

4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH, SPF or SP No.3

OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C. 5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF

DIAPHRAM AT 4'-0" O.C.

6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 AND A 2x4 AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST GABLE STUD. ATTACH TO VERTICAL GABLE STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.

DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.

10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE

06-01-13 BY SPIB/ALSC.

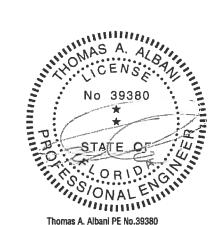
11. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Minimum Stud Size Species	Stud Spacing	Without Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS			
and Grade		Maximum Stud Length						
2x4 SP No. 3 / Stud	12" O.C.	3-9-7	5-8-8	6-11-1	11-4-4			
2x4 SP No. 3 / Stud	16" O.C.	3-4-12	4-11-15	6-9-8	10-2-3			
2x4 SP No. 3 / Stud	24" O.C.	2-9-4	4-0-7	5-6-8	8-3-13			
2x4 SP No. 2	12" O.C.	3-11-13	5-8-8	6-11-1	11-11-7			
2x4 SP No. 2	16" O.C.	3-7-7	4-11-5	6-11-1	10-10-5			
2x4 SP No. 2	24" O.C.	3-1-15	4-0-7	6-3-14	9-5-14			

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 6" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length. T or I braces must be 2x4 SPF No. 2 or SP No. 2.

MAX MEAN ROOF HEIGHT = 30 FEET EXPOSURE D ASCE 7-10 170 MPH **DURATION OF LOAD INCREASE: 1.60**

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.



2x6 DIAGONAL BRACE SPACED

48" O.C. ATTACHED TO VERTICAL WITH (4) -16d NAILS, AND ATTACHED TO BLOCKING WITH (5) -10d NAILS.

HORIZONTAL BRACE

(SEE SECTION A-A)

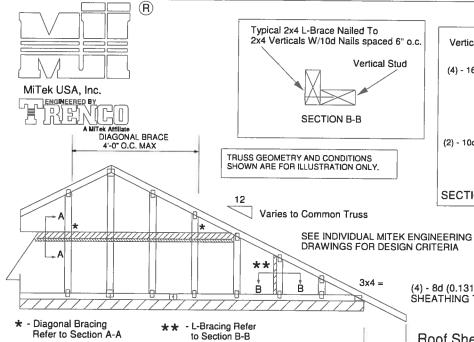
Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

Standard Gable End Detail

MII-GE180-D-SP

Page 1 of 2

MiTek USA, Inc.



Vertical Stud 2X6 SP OR SPF No. 2 DIAGONAL BRACE (4) - 16d Nails 16d Nails Spaced 6" o.c. (2) - 10d Nails into 2x6 2X6 SP OR SPF No. 2 Typical Horizontal Brace Nailed To 2x4 Verticals w/(4)-10d Nails

> PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH

2X4 SP OR SPF No. 2

SECTION A-A

Roof Sheathing

1'-0"

Max.

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SPF BLOCK

(2) - 10d

NAILS

24" Max 1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.

2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.

3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.

"L" BRACES SPECIFIED ARE TO BE FULL LENGTH, SPF or SP No.3

OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.

5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.

6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 AND A 2x4 AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST GABLE STU ATTACH TO VERTICAL GABLE STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)

GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.

THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.

DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR

TYPE TRUSSES.

SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.

NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

JD.	Diag. Brace at 1/3 points if needed
IAGON	
ACES A	·

2x6 DIAGONAL BRACE SPACED 48" O.C. ATTACHED TO VERTICAL WITH (4) -16d NAILS, AND ATTACHED TO BLOCKING WITH (5) -10d NAILS.

(2) - 10d NAILS

Trusses @ 24" o.c.

HORIZONTAL BRACE (SEE SECTION A-A)

Minimum Stud Size Species	Stud Spacing	Without Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS	
and Grade	!	Maximum Stud Length				
2x4 SP No. 3 / Stud	12" O.C.	3-7-12	5-4-11	6-2-1	10-11-3	
2x4 SP No. 3 / Stud	16" O.C.	3-2-8	4-8-1	6-2-1	9-7-7	
2x4 SP No. 3 / Stud	24" O.C.	2-7-7	3-9-12	5-2-13	7-10-4	
2x4 SP No. 2	12" O.C.	3-10-0	5-4-11	6-2-1	11-6-1	
2x4 SP No. 2	16" O.C.	3-5-13	4-8-1	6-2-1	10-5-7	
2x4 SP No. 2	24" O.C.	3-0-8	3-9-12	6-1-1	9-1-9	

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 6in o.c., with 3in minimum end distance. Brace must cover 90% of diagonal length. T or I braces must be 2x4 SPF No. 2 or SP No. 2.

MAX MEAN ROOF HEIGHT = 30 FEET EXPOSURE D ASCE 7-10 180 MPH **DURATION OF LOAD INCREASE: 1.60**

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

MiTek USA, Inc. Page 1 of 1

(R)

MiTek USA, Inc.

ENGINEERED B 別以

MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E MAX MEAN ROOF HEIGHT = 30 FEET MAX TRUSS SPACING = 24 " O.C. CATEGORY II BUILDING EXPOSURE B or C ASCE 7-10 **DURATION OF LOAD INCREASE: 1.60**

DETAIL IS NOT APPLICABLE FOR TRUSSES TRANSFERING DRAG LOADS (SHEAR TRUSSES). ADDITIONAL CONSIDERATIONS BY BUILDING ENGINEER/DESIGNER ARE REQUIRED.

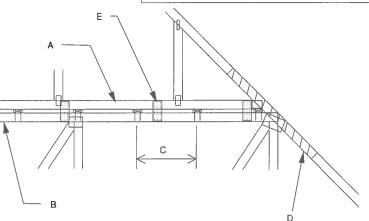
A - PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
SHALL BE CONNECTED TO EACH PURLIN
WITH (2) (0.131" X 3.5") TOE-NAILED.
B - BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
C - PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24" O.C.
UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING.
CONNECT TO BASE TRUSS WITH (2) (0.131" X 3.5") NAILS EACH.
D - 2 X __ X 4"-0" SCAB, SIZE TO MATCH TOP CHORD OF
PIGGYBACK TRUSS, MIN GRADE #2, ATTACHED TO ONE FACE, CENTERED.
ON INTERSECTION, WITH (2) ROWS OF (0.131" X 3") NAILS @ 4" O.C.
SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING
IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH
DIRECTIONS AND: DIRECTIONS AND:

DIRECTIONS AND:

1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR

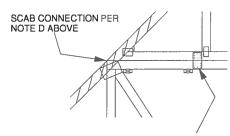
2. WIND SPEED OF 116 MPH TO 160 MPH WITH A MAXIMUM
PIGGYBACK SPAN OF 12 ft.

E - FOR WIND SPEEDS BETWEEN 126 AND 160 MPH, ATTACH
MITEK 3X8 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT
72° O.C. W/ (4) (0.131° X 1.5") NAILS PER MEMBER. STAGGER NAILS
FROM OPPOSING FACES. ENSURE 0.5" EDGE DISTANCE.
(MIN. 2 PAIRS OF PLATES REQ. REGARDLESS OF SPAN)

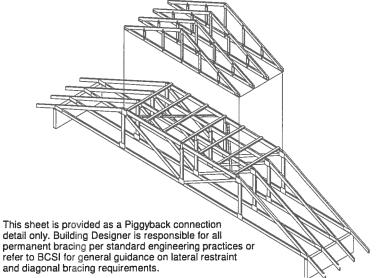


WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

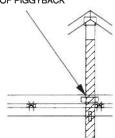
REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH Nail-On PLATES AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.



FOR ALL WIND SPEEDS, ATTACH MITEK 3X6 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 48" O.C. W/ (4) (0.131" X 1.5") PER MEMBER. STAGGER NAILS FROM OPPOSING FACES ENSURE 0.5" EDGE DISTANCE.



VERTICAL WEB TO EXTEND THROUGH **BOTTOM CHORD** OF PIGGYBACK



FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

1) VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL.

AS SHOWN IN DETAIL.
ATTACH 2 x 4-0" SCAB TO EACH FACE OF
TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS
SPACED 4" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH
VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.) (MINIMUM 2X4)

THIS CONNECTION IS ONLY VALID FOR A MAXIMUM CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS GREATER THAN 4000 LBS. FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS.

NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS.

CONCENTRATED LOAD MUST BE APPLIED TO BOTH THE PIGGYBACK AND THE BASE TRUSS DESIGN.



Thomas A. Albani PE No.39380 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

STANDARD PIGGYBACK TRUSS CONNECTION DETAIL

MII-PIGGY-ALT 7-10

MiTek USA, Inc. Page 1 of 1

MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E MAX MEAN ROOF HEIGHT = 30 FEET MAX TRUSS SPACING = 24 " O.C. CATEGORY II BUILDING

EXPOSURE B or C ASCE 7-10

DURATION OF LOAD INCREASE: 1.60

DETAIL IS NOT APPLICABLE FOR TRUSSES TRANSFERING DRAG LOADS (SHEAR TRUSSES). ADDITIONAL CONSIDERATIONS BY BUILDING ENGINEER/DESIGNER ARE REQUIRED.



A - PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.

A - PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
SHALL BE CONNECTED TO EACH PURLIN
WITH (2) 0(0.131" X 3.5") TOE-NAILED.

B - BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
C - PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24" O.C.
UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING.
CONNECT TO BASE TRUSS WITH (2) (0.131" X 3.5") NAILS EACH.
D - 2 X _ X 4"-0" SCAB, SIZE TO MATCH TOP CHORD OF
PIGGYBACK TRUSS, MIN GRADE #2, ATTACHED TO ONE FACE, CENTERED ON
INTERSECTION, WITH (2) ROWS OF (0.131" X 3") NAILS @ 4" O.C.
SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING
IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH
DIRECTIONS AND:

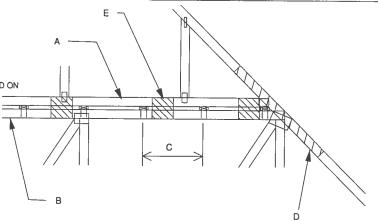
IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH DIRECTIONS AND:

1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR

2. WIND SPEED OF 116 MPH TO 160 MPH WITH A MAXIMUM PIGGYBACK SPAN OF 12 ft.

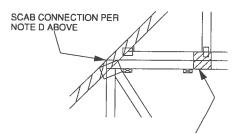
E - FOR WIND SPEED IN THE RANGE 126 MPH - 160 MPH ADD 9" x 9" x 172" PLYWOOD (or 7/16" OSB) GUSSET EACH SIDE AT 48" O.C. OR LESS. ATTACH WITH

3 - 6d (0.113" X 2") NAILS INTO EACH CHORD FROM EACH SIDE (TOTAL - 12 NAILS)

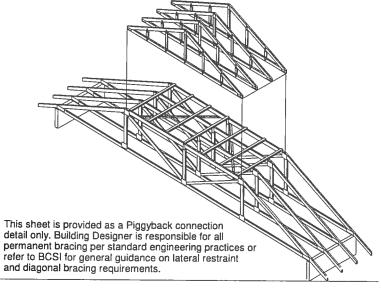


WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

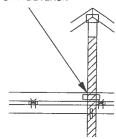
REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH PLYWOOD GUSSETS AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.



 $7"\times 7"\times 1/2"$ PLYWOOD (or 7/16" OSB) GUSSET EACH SIDE AT 24" O.C. ATTACH WITH 3 - 6d (0.113" X 2") NAILS INTO EACH CHORD FROM EACH SIDE (TOTAL - 12 NAILS)



VERTICAL WEB TO EXTEND THROUGH BOTTOM CHORD OF PIGGYBACK



FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

1) VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL.

AS SHOWN IN DETAIL.

ATTACH 2 × x 4-0" SCAB TO EACH FACE OF
TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS
SPACED 4" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH
VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.)
(MINIMUM 2X4)
THIS CONNECTION IS ONLY VALID FOR A MAXIMUM
CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW
BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS
GREATER THAN 4000 LBS.
FOR PIGGYBACK TRUSSES CARRYING CURRED LOADS

FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS. NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS.
5) CONCENTRATED LOAD MUST BE APPLIED TO BOTH THE PIGGYBACK AND THE BASE TRUSS DESIGN.



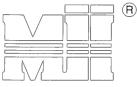
Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

STANDARD REPAIR DETAIL FOR BROKEN CHORDS, WEBS AND DAMAGED OR MISSING CHORD SPLICE PLATES

MII-REP01A1

MiTek USA, Inc.

Page 1 of 1



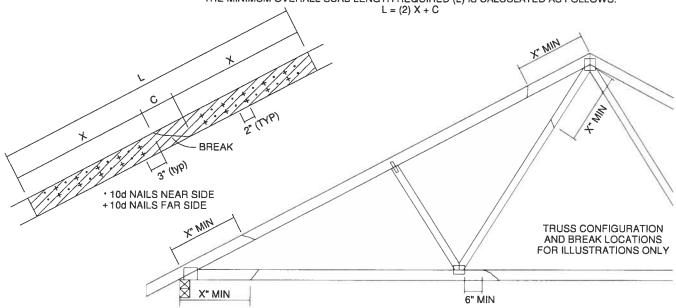
MiTek USA, Inc.

TOTAL NUMBER OF NAILS EACH SIDE OF BREAK * X INCHES			MAXIMUM FORCE (lbs) 15% LOAD DURATION								
			SP		DF		SPF		HF		
2x4	2x6		2x4	2x6	2x4	2x6	2x4	2x6	2x4	2x6	
20	30	24"	1706	2559	1561	2342	1320	1980	1352	2028	
26	39	30"	2194	3291	2007	3011	1697	2546	1738	2608	
32	48	36"	2681	4022	2454	3681	2074	3111	2125	3187	
38	57	42"	3169	4754	2900	4350	2451	3677	2511	3767	
44	66	48"	3657	5485	3346	5019	2829	4243	2898	4347	

* DIVIDE EQUALLY FRONT AND BACK

ATTACH 2x_SCAB OF THE SAME SIZE AND GRADE AS THE BROKEN MEMBER TO EACH FACE OF THE TRUSS (CENTER ON BREAK OR SPLICE) WITH 10d (0.131" X 3") NAILS (TWO ROWS FOR 2x4, THREE ROWS FOR 2x6) SPACED 4" O.C. AS SHOWN. STAGGER NAIL SPACING FROM FRONT FACE AND BACK FACE FOR A NET 0-2-0 O.C. SPACING IN THE MAIN MEMBER. USE A MIN. 0-3-0 MEMBER END DISTANCE.

THE LENGTH OF THE BREAK (C) SHALL NOT EXCEED 12". (C=PLATE LENGTH FOR SPLICE REPAIRS) THE MINIMUM OVERALL SCAB LENGTH REQUIRED (L) IS CALCULATED AS FOLLOWS:



THE LOCATION OF THE BREAK MUST BE GREATER THAN OR EQUAL TO THE REQUIRED X DIMENSION FROM ANY PERIMETER BREAK OR HEEL JOINT AND A MINIMUM OF 6" FROM ANY INTERIOR JOINT (SEE SKETCH ABOVE)

DO NOT USE REPAIR FOR JOINT SPLICES

- NOTES:

 1. THIS REPAIR DETAIL IS TO BE USED ONLY FOR THE APPLICATION SHOWN. THIS REPAIR DOES

 NOT IMPLY THAT THE REMAINING PORTION OF THE TRUSS IS UNDAMAGED. THE ENTIRE TRUSS

 SHALL BE INSPECTED TO VERIFY THAT NO FURTHER REPAIRS ARE REQUIRED. WHEN THE REQUIRED

 REPAIRS ARE PROPERLY APPLIED, THE TRUSS WILL BE CAPABLE OF SUPPORTING THE LOADS INDICATED.

 2. ALL MEMBERS MUST BE RETURNED TO THEIR ORIGINAL POSITIONS BEFORE APPLING REPAIR

 AND HELD IN PLACE DURING APPLICATION OF REPAIR.

 3. THE END DISTANCE, EDGE DISTANCE AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID

 UNUSUAL SPLITTING OF THE WOOD.

 4. WHEN NAILING THE SCABS, THE USE OF A BACKUP WEIGHT IS RECOMMENDED TO AVOID

 LOOSENING OF THE CONNECTOR PLATES AT THE JOINTS OR SPLICES.

 5. THIS REPAIR IS TO BE USED FOR SINGLE PLY TRUSSES IN THE 2x_ ORIENTATION ONLY.

 6. THIS REPAIR IS LIMITED TO TRUSSES WITH NO MORE THAN THREE BROKEN MEMBERS.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

LATERAL TOE-NAIL DETAIL

MII-TOENAIL SP

MiTek USA, Inc.

Page 1 of 1



MiTek USA, Inc. ENGINEERED BY

- NOTES:

 1. TOE-NAILS SHALL BE DRIVEN AT AN ANGLE OF 45 DEGREES WITH THE MEMBER AND MUST HAVE FULL WOOD SUPPORT. (NAIL MUST BE DRIVEN THROUGH AND EXIT AT THE BACK CORNER OF THE MEMBER END AS SHOWN.

 2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.

 3. ALLOWABLE VALUE SHALL BE THE LESSER VALUE OF THE TWO SPECIES FOR MEMBERS OF DIFFERENT SPECIES.

THIS DETAIL APPLICABLE TO THE THREE END DETAILS SHOWN BELOW

NEAR SIDE

VIEWS SHOWN ARE FOR

-	TOE-NAIL SINGLE SHEAR VALUES PER NDS 2001 (lb/nail)								
	DIAM.	SP	DF	HF	SPF	SPF-S			
<u>0</u>	.131	88.0	80.6	69.9	68.4	59.7			
LONG	.135	93.5	85.6	74.2	72.6	63.4			
້າດ	.162	108.8	99.6	86.4	84.5	73.8			
က်									
LONG	.128	74.2	67.9	58.9	57.6	50.3			
	.131	75.9	69.5	60.3	59.0	51.1			
3.25"	.148	81.4	74.5	64.6	63.2	52.5			
က်									

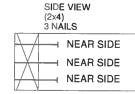
ILLUSTRATION PURPOSES ONLY SIDE VIEW (2x3) 2 NAILS NEAR SIDE

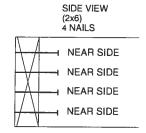
VALUES SHOWN ARE CAPACITY PER TOE-NAIL APPLICABLE DURATION OF LOAD INCREASES MAY BE APPLIED.

(3) - 16d (0.162" X 3.5") NAILS WITH SPF SPECIES BOTTOM CHORD

For load duration increase of 1.15:

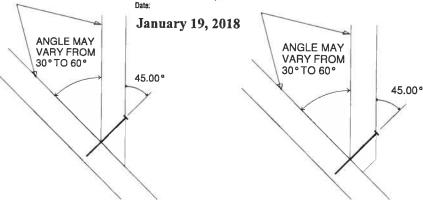
3 (nails) X 84.5 (lb/nail) X 1.15 (DOL) = 291.5 lb Maximum Capacity

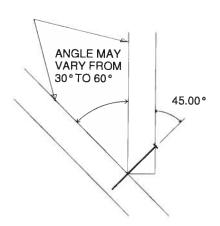






Thomas A. Albani PE No.39380 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610



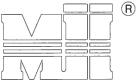


TRUSSED VALLEY SET DETAIL

MII-VALLEY HIGH WIND1

MiTek USA, Inc.

Page 1 of 1

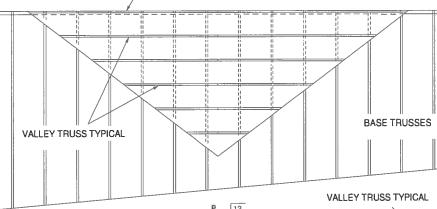


MiTek USA, Inc. ENGINEERED BY A MITek Atfillate

GABLE END, COMMON TRUSS OR GIRDER TRUSS

GENERAL SPECIFICATIONS

- 1. NAIL SIZE 10d (0.131" X 3")
 2. WOOD SCREW = 3" WS3 USP OR EQUIVALENT DO NOT USE DRYWALL OR DECKING TYPE SCREW
- 3. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND SECURE PER DETAIL A
- 4. BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS.
- 5. BASE TRUSS SHALL BE DESIGNED WITH A PURLIN SPACING EQUILIVANT TO THE RAKE DIMENSION OF THE VALLEY TRUSS SPACING.
- 6. NAILING DONE PER NDS 01
- 7. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.



12

SEE DETAIL A BELOW (TYP.) GABLE END, COMMON TRUSS OR GIRDER TRUSS

SECURE VALLEY TRUSS W/ ONE ROW OF 10d NAILS 6" O.C.

ATTACH 2x4 CONTINUOUS NO.2 SP TO THE ROOF W/ TWO USP WS3 (1/4" X 3") WOOD SCREWS INTO EACH BASE TRUSS.

DETAIL A (NO SHEATHING) N.T.S.

WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH WIND DESIGN PER ASCE 7-10 160 MPH MAX MEAN ROOF HEIGHT = 30 FEET ROOF PITCH = MINIMUM 3/12 MAXIMUM 6/12 CATEGORY II BUILDING EXPOSURE C

EAFUSURE C WIND DURATION OF LOAD INCREASE: 1.60 MAX TOP CHORD TOTAL LOAD = 50 PSF MAX SPACING = 24" O.C. (BASE AND VALLEY) MINIMUM REDUCED DEAD LOAD OF 6 PSF ON THE TRUSSES



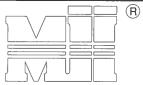
Thomas A. Albani PE No.39380 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

TRUSSED VALLEY SET DETAIL

MII-VALLEY HIGH WIND2

MiTek USA, Inc.

Page 1 of 1



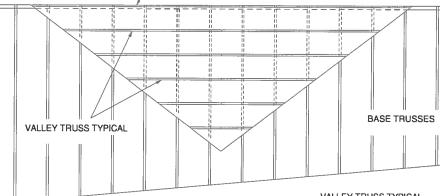
MiTek USA, Inc.

ENGINEERED BY

GABLE END, COMMON TRUSS OR GIRDER TRUSS

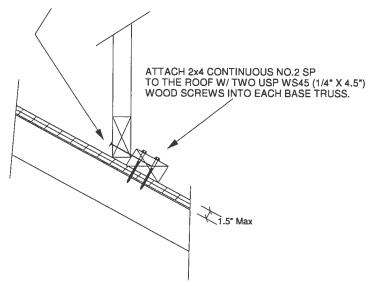
GENERAL SPECIFICATIONS

- 1. NAIL SIZE 10d (0.131" X 3") 2. WOOD SCREW = 4.5" WS45 USP OR EQUILIVANT 3. INSTALL SHEATHING TO TOP CHORD OF BASE TRUSSES.
- 4. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND SECURE TO BASE TRUSSES AS PER DETAIL A
- 5. BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS.
- 6. NAILING DONE PER NDS-01
- 7. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.



GABLE END, COMMON TRUSS VALLEY TRUSS TYPICAL 12 OR GIRDER TRUSS SEE DETAIL A BELOW (TYP.)

SECURE VALLEY TRUSS W/ ONE ROW OF 10d NAILS 6" O.C.



WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH WIND DESIGN PER ASCE 7-10 160 MPH MAX MEAN ROOF HEIGHT = 30 FEET ROOF PITCH = MINIMUM 3/12 MAXIMUM 6/12 CATEGORY II BUILDING EXPOSURE C WIND DURATION OF LOAD INCREASE: 1.60
MAX TOP CHORD TOTAL LOAD = 50 PSF
MAX SPACING = 24" O.C. (BASE AND VALLEY) MINIMUM REDUCED DEAD LOAD OF 6 PSF ON THE TRUSSES

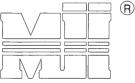
No 39380

STATE OF THE OF THE

Thomas A. Albani PE No.39380 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

MiTek USA, Inc.

Page 1 of 1

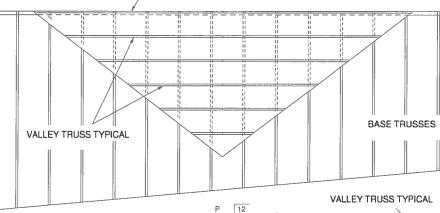


MiTek USA, Inc. ENGINEERED BY

GABLE END, COMMON TRUSS OR GIRDER TRUSS

GENERAL SPECIFICATIONS

- 1. NAIL SIZE 16d (0.131" X 3.5") 2. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND SECURE PER DETAIL A
- 3. BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS.
- 4. BASE TRUSS SHALL BE DESIGNED WITH A PURLIN SPACING EQUILIVANT TO THE RAKE DIMENSION OF THE VALLEY TRUSS SPACING.
- 5. NAILING DONE PER NDS 01
- 6. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.
- 7. ALL LUMBER SPECIES TO BE SP.

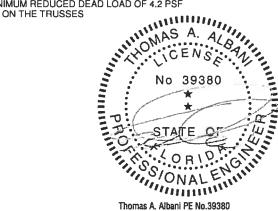


GABLE END, COMMON TRUSS OR GIRDER TRUSS SEE DETAIL A BELOW (TYP.)

SECURE VALLEY TRUSS W/ ONE ROW OF 16d NAILS 6" O.C. ATTACH 2x4 CONTINUOUS NO.2 SP TO THE ROOF W/TWO 16d NAILS INTO EACH BASE TRUSS.

DETAIL A (MAXIMUM 1" SHEATHING) N.T.S.

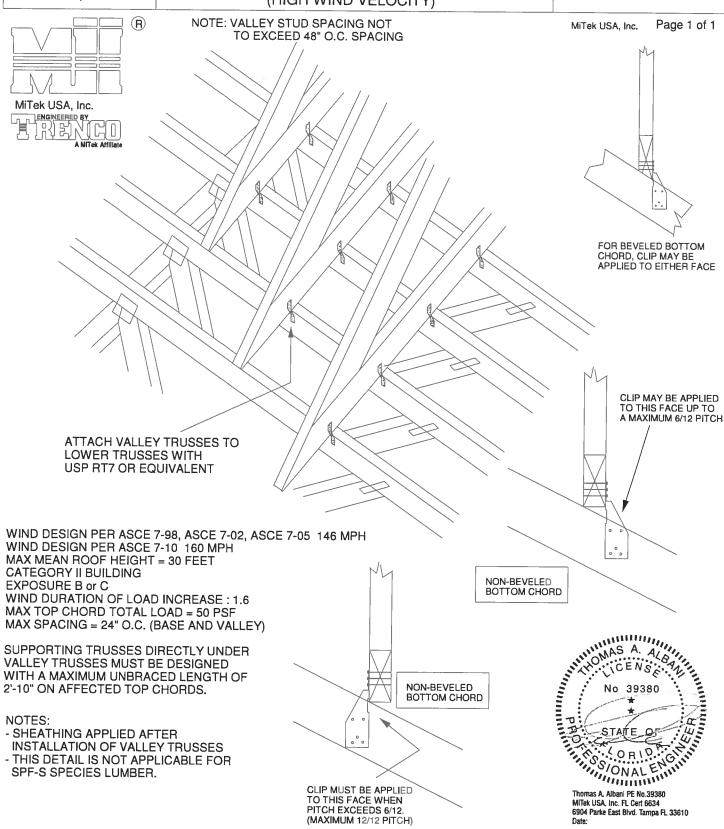
WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 120 MPH WIND DESIGN PER ASCE 7-10 150 MPH MAX MEAN ROOF HEIGHT = 30 FEET ROOF PITCH = MINIMUM 3/12 MAXIMUM 10/12 CATEGORY II BUILDING EXPOSURE C OR B
WIND DURATION OF LOAD INCREASE: 1.60
MAX TOP CHORD TOTAL LOAD = 60 PSF
MAX SPACING = 24" O.C. (BASE AND VALLEY)
MINIMUM REDUCED DEAD LOAD OF 4.2 PSF



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

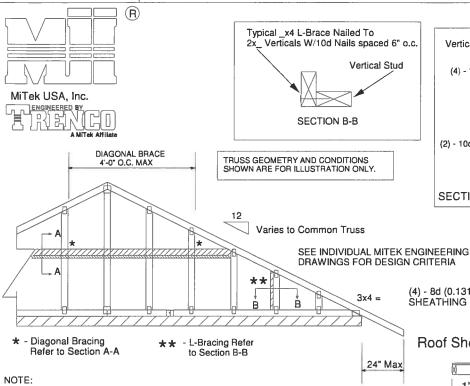
TRUSSED VALLEY SET DETAIL (HIGH WIND VELOCITY)

MII-VALLEY



Standard Gable End Detail

MII-GE146-001



MiTek USA, Inc. Page 1 of 2 Vertical Stud DIAGONAL BRACE (4) - 16d Nails 16d Nails Spaced 6" o.c. (2) - 10d Nails into 2x6 2x6 Stud or 2x4 No.2 of better Typical Horizontal Brace Nailed To 2x_ Verticals w/(4)-10d Nails SECTION A-A 2x4 Stud

> PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SP BLOCK

Roof Sheathing

1'-3"

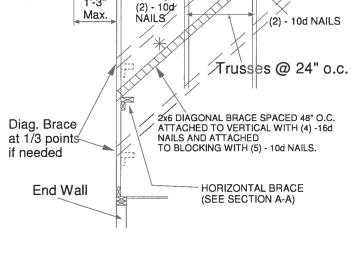
- 1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
- 2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
- 3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
- 4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES:
- 2x4 No 3/STUD SP OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
- 5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF
- 5. DIAGONAL BRACE TO BE APPHOXIMATELY 45 DEGREES TO HOOP DIAPHRAM AT 4-0" O.C.
 6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4.
- (REFER TO SECTION A-A)
- GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
 THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
 DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR
- TYPE TRUSSES.
- 10. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Minimum Stud Size	Stud Spacing	Without Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS	
Species and Grade		Maximum Stud Length				
2x4 SP No 3/Stud	12" O.C.	3-11-3	6-8-0	7-2-14	11-9-10	
2x4 SP No 3/Stud	16" O.C.	3-6-14	5-9-5	7-1-13	10-8-11	
2x4 SP No 3/Stud	24" O.C.	3-1-8	4-8-9	6-2-15	9-4-7	

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of web with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAXIMUM WIND SPEED = 146 MPH MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING EXPOSURE B or C ASCE 7-98, ASCE 7-02, ASCE 7-05 **DURATION OF LOAD INCREASE: 1.60**

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS





MiTek USA, Inc. Fl. Cert 6634 6904 Parke East Blvd. Tampa FL 33610

OCTOBER 5, 2016

REPLACE BROKEN OVERHANG

MII-REP13B

MiTek USA, Inc.

Page 1 of 1

(R)

MiTek USA, Inc. ENGINEERED BY

TRUSS CRITERIA:

LOADING: 40-10-0-10 **DURATION FACTOR: 1.15** SPACING: 24" O.C. TOP CHORD: 2x4 OR 2x6

PITCH: 4/12 - 12/12 HEEL HEIGHT: STANDARD HEEL UP TO 12" ENERGY HEEL

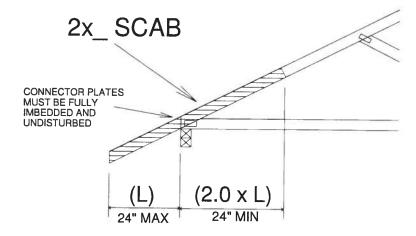
END BEARING CONDITION

NOTES:

1. ATTACH 2x_ SCAB (MINIMUM NO.2 GRADE SPF, HF, SP, DF) TO ONE FACE OF TRUSS WITH TWO ROWS OF 10d (0.131" X 3") SPACED 6" O.C.
2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH

AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.

3. WHEN NAILING THE SCABS, THE USE OF A BACKUP WEIGHT IS RECOMMENDED TO AVOID LOOSENING OF THE CONNECTOR PLATES AT THE JOINTS OR SPLICES.

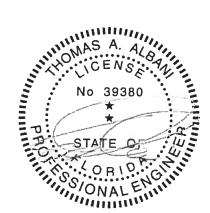


IMPORTANT

This detail to be used only with trusses (spans less than 40') spaced 24" o.c. maximum and having pitches between 4/12 and 12/12 and total top chord loads not exceeding 50 psf.

Trusses not fitting these criteria should be examined individually.

REFER TO INDIVIDUAL TRUSS DESIGN FOR PLATE SIZES AND LUMBER GRADES



Thomas A. Albani PE No.39380 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

LATERAL BRACING RECOMMENDATIONS

MII-STRGBCK

 $\Rightarrow \mathbb{R}$

MiTek USA, Inc.

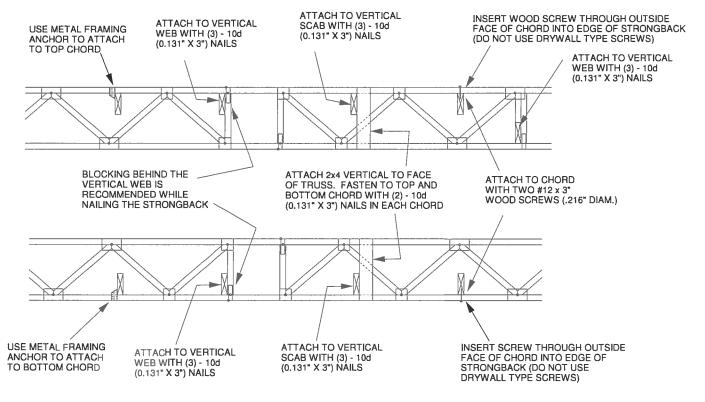
Page 1 of 1

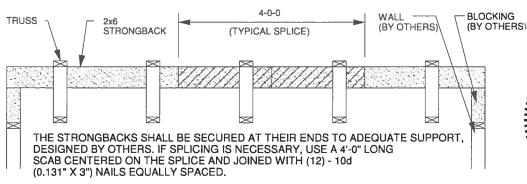


TO MINIMIZE VIBRATION COMMON TO ALL SHALLOW FRAMING SYSTEMS, 2x6 "STRONGBACK" IS RECOMMENDED, LOCATED EVERY 8 TO 10 FEET ALONG A FLOOR TRUSS.

NOTE 1: 2X6 STRONGBACK ORIENTED VERTICALLY MAY BE POSITIONED DIRECTLY UNDER THE TOP CHORD OR DIRECTLY ABOVE THE BOTTOM CHORD. SECURELY FASTENED TO THE TRUSS USING ANY OF THE METHODS ILLUSTRATED BELOW.

NOTE 2: STRONGBACK BRACING ALSO SATISFIES THE LATERAL BRACING REQUIREMENTS FOR THE BOTTOM CHORD OF THE TRUSS WHEN IT IS PLACED ON TOP OF THE BOTTOM CHORD, IS CONTINUOUS FROM END TO END, CONNECTED WITH A METHOD OTHER THAN METAL FRAMING ANCHOR, AND PROPERLY CONNECTED, BY OTHERS, AT THE ENDS.





ALTERNATE METHOD OF SPLICING:
OVERLAP STRONGBACK MEMBERS A MINIMUM OF 4'-0" AND FASTEN WITH (12) - 10d
(0.131" X 3") NAILS STAGGERED AND EQUALLY SPACED.
(TO BE USED ONLY WHEN STRONGBACK IS NOT ALIGNED WITH A VERTICAL)

No 39380

No 39380

STATE OF THE STATE OF TH

N