

DATE 03/28/2007

Columbia County Building Permit

This Permit Expires One Year From the Date of Issue

PERMIT

000025674

APPLICANT GERALD HARVEY PHONE 352-258-9051
ADDRESS PO BOX 526 NEWBERRY FL 32669
OWNER JAMES MCGLYNN PHONE 352-338-3839
ADDRESS 124 SW LANKTREE GLEN FORT WHITE FL 32038
CONTRACTOR HARVEY BLDG & CONSTRUCTION PHONE 352-338-3839
LOCATION OF PROPERTY 47 S, L ON 27, L ON BRIGHTON COURT (HOLLINGSWORTH EST)
AT STOP SIGN RIGHT CORNER LOT (LANKTREE GLEN)

TYPE DEVELOPMENT SFD,UTILITY ESTIMATED COST OF CONSTRUCTION 90150.00
HEATED FLOOR AREA 1803.00 TOTAL AREA 2691.00 HEIGHT 22.00 STORIES 1
FOUNDATION CONCRETE WALLS FRAMED ROOF PITCH 6/12 FLOOR SLAB
LAND USE & ZONING FORT WHITE MAX. HEIGHT _____
Minimum Set Back Requirments: STREET-FRONT _____ REAR _____ SIDE _____
NO. EX.D.U. 0 FLOOD ZONE FW DEVELOPMENT PERMIT NO. _____

PARCEL ID 34-6S-16-04059-262 SUBDIVISION HOLLINGSWORTH ESTATES
LOT 12 BLOCK D PHASE _____ UNIT _____ TOTAL ACRES 0.50

CRC058134
Culvert Permit No. _____ Culvert Waiver _____ Contractor's License Number _____ Applicant/Owner/Contractor _____
FORT WHITE 07-234 BK JH N
Driveway Connection _____ Septic Tank Number _____ LU & Zoning checked by _____ Approved for Issuance _____ New Resident _____

COMMENTS: FT WHITE LETTER INCLUDED

Check # or Cash 3436

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power _____ Foundation _____ Monolithic _____
date/app. by _____ date/app. by _____ date/app. by _____
Under slab rough-in plumbing _____ Slab _____ Sheathing/Nailing _____
date/app. by _____ date/app. by _____ date/app. by _____
Framing _____ Rough-in plumbing above slab and below wood floor _____
date/app. by _____ date/app. by _____
Electrical rough-in _____ Heat & Air Duct _____ Peri. beam (Lintel) _____
date/app. by _____ date/app. by _____ date/app. by _____
Permanent power _____ C.O. Final _____ Culvert _____
date/app. by _____ date/app. by _____ date/app. by _____
M/H tie downs, blocking, electricity and plumbing _____ Pool _____
date/app. by _____ date/app. by _____
Reconnection _____ Pump pole _____ Utility Pole _____
date/app. by _____ date/app. by _____ date/app. by _____
M/H Pole _____ Travel Trailer _____ Re-roof _____
date/app. by _____ date/app. by _____ date/app. by _____

BUILDING PERMIT FEE \$ 455.00 CERTIFICATION FEE \$ 13.46 SURCHARGE FEE \$ 13.46
MISC. FEES \$ 0.00 ZONING CERT. FEE \$ _____ FIRE FEE \$ 0.00 WASTE FEE \$ _____
FLOOD DEVELOPMENT FEE \$ _____ FLOOD ZONE FEE \$ _____ CULVERT FEE \$ _____ TOTAL FEE 481.92
INSPECTORS OFFICE Lari Locken CLERKS OFFICE CN

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVENIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Columbia County Building Permit Application

43436

For Office Use Only Application # 0703-50 Date Received 3/20/07 By G Permit # 25674

Application Approved by - Zoning Official _____ Date _____ Plans Examiner OK JH Date 3-22-07

Flood Zone _____ Development Permit _____ Zoning _____ Land Use Plan Map Category _____

Comments City Water Ft White letter included

☐ NOC ☒ EH ☐ Deed or PA ☐ Site Plan ☐ State Road Info ☐ Parent Parcel # ☐ Development Permit

Name Authorized Person Signing Permit Gerald Harvey Fax 352-472-4476

Address P.O. Box 526 Newberry, FL 32669 Phone 352-258-9051

Owners Name James A. McGlynn Phone 338-3839

911 Address 124 S.W. Oak Tree Glen Fort White 32038

Contractors Name Harvey Bldg + Const. Inc. Phone 352-472-4468

Address P.O. Box 526 Newberry, FL 32669

Fee Simple Owner Name & Address N/A

Bonding Co. Name & Address N/A

Architect/Engineer Name & Address Don Yansky - 2421 N.W. 49th Ave Gainesville 3260

Mortgage Lenders Name & Address James A. McGlynn

Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progressive Energy

Property ID Number 3465-116-04059-262 Estimated Cost of Construction \$170,000

Subdivision Name Hollingsworth Estates Lot 12 Block D Unit _____ Phase _____

Driving Directions 75 South to 47 South go to stop light in Fort White turn left on to 27 South go to Hollingsworth Estates on left Brighton Ct. go to stop sign corner lot on right - Oak Tree bl.

Type of Construction wood frame + concrete slab Number of Existing Dwellings on Property 0

Total Acreage 1/2 ac. Lot Size _____ Do you need a Culvert Permit or Culvert Waiver or Have an Existing Driv

Actual Distance of Structure from Property Lines - Front 31 ft Side 67 ft Side 81 ft Rear 21 ft

Total Building Height 22' Number of Stories 1 Heated Floor Area 1803 Roof Pitch 6 12

TOTAL 2691

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Owner Builder or Authorized Person by Gerald Harvey Harvey Bldg + Const. Inc. Notary Public - State of Florida Contractor Signature Susan Frazier

STATE OF FLORIDA
COUNTY OF COLUMBIA

My Commission Expires Jan 2011 Contractors License Number CRS 058134

Commission # DD 62648 Agency Card Number _____

Bonded Through National Notary Assn. **NOTARY STAMP/SEAL**

Sworn to (or affirmed) and subscribed before me this 19th day of March 20 07.

Susan Frazier

Tim called. 3.22.07



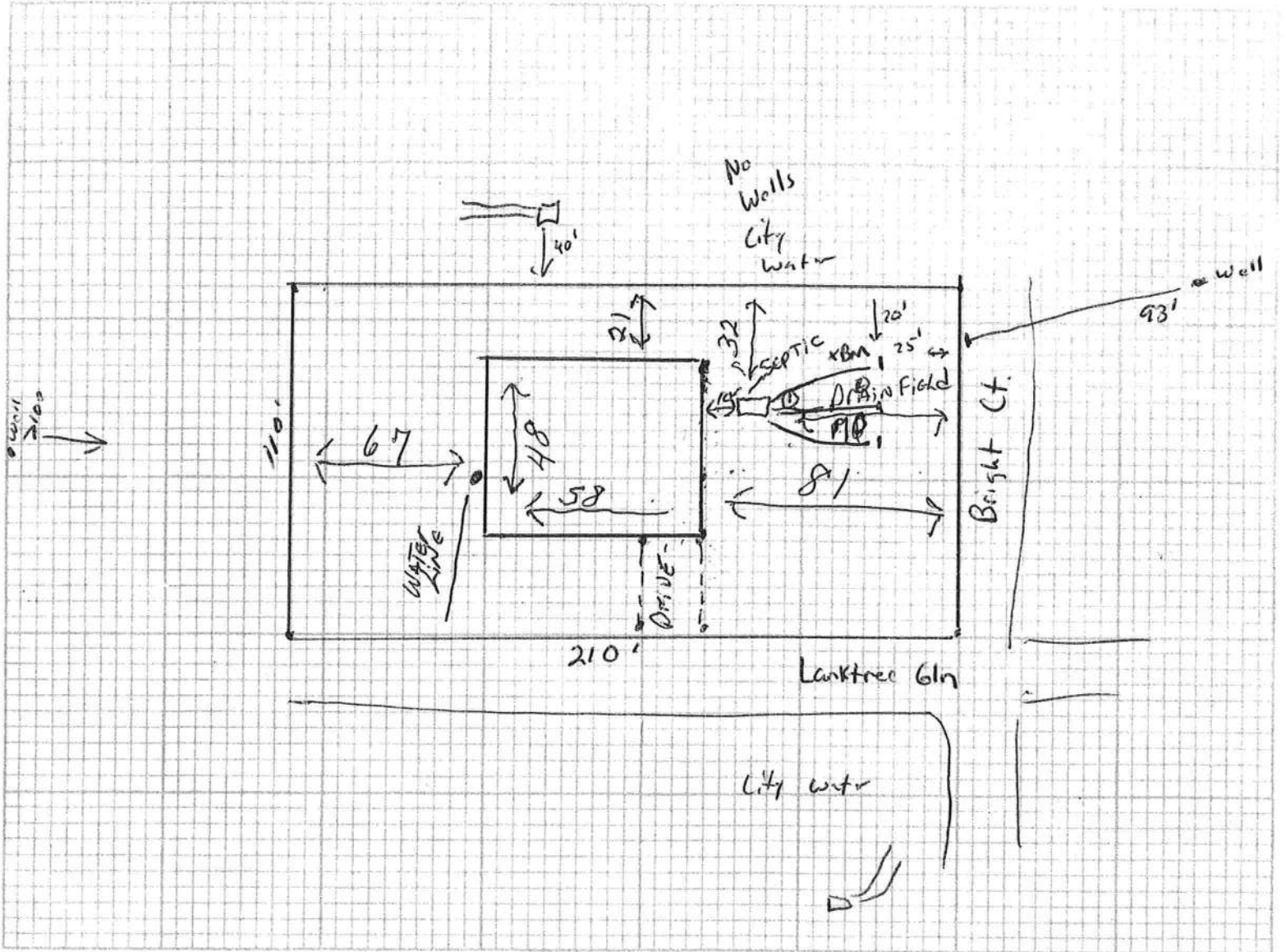
STATE OF FLORIDA
DEPARTMENT OF HEALTH

APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number 07-234

PART II - SITE PLAN

Scale: Each block represents 5 feet and 1 inch = 50 feet.



Notes: _____

Site Plan submitted by: X Gerald E. Hawley

Signature

Plan Approved ☒

Not Approved ☐

Title

Date 3/24/07

By M J N

Columbia

County Health Department

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

This instrument Prepared by & return to:
Name: **KIM WATSON, an employee of**
TITLE OFFICES, LLC
Address: **343 NW COLE TERRACE, SUITE 105**
LAKE CITY, FLORIDA 32055
File No. **07K-81049KIV**

Parcel I.D. #: **04059-243 & 262**

SPACE ABOVE THIS LINE FOR PROCESSING DATA

SPACE ABOVE THIS LINE FOR RECORDING DATA

THIS WARRANTY DEED Made the 8th day of February, A.D. 2007, by **COOK REAL ESTATE INVESTMENTS, INC., A FLORIDA CORPORATION**, having its principal place of business at **P.O. BOX 938, FORT WHITE, FL 32038**, hereinafter called the grantor, to **MCGLYNN AND SOUTH PROPERTIES, LLC**, having its principal place of business at **4309 NW 23RD AVE., SUITE 3, GAINESVILLE, FLORIDA 32606**, hereinafter called the grantee:

(Wherever used herein the terms "grantor" and "grantee" include all the parties to this instrument, singular and plural, the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporations, wherever the context so admits or requires.)

Witnesseth: That the grantor, for and in consideration of the sum of \$10.00 and other valuable consideration, receipt whereof is hereby acknowledged, does hereby grant, bargain, sell, alien, remise, release, convey and confirm unto the grantee all that certain land situate in **Columbia County, State of Florida**, viz:

Lot 3, Block C, HOLLINGSWORTH ESTATES, according to the map or plat thereof as recorded in **Plat Book 5, Page 122**, of the Public Records of **Columbia County, Florida**.

AND

Lot 12, Block D, HOLLINGSWORTH ESTATES, according to the map or plat thereof as recorded in **Plat Book 5, Page 122**, of the Public Records of **Columbia County, Florida**.

Together with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

To Have and to Hold the same in fee simple forever.

And the grantor hereby covenants with said grantee that it is lawfully seized of said land in fee simple; that it has good right and lawful authority to sell and convey said land, and hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever, and that said land is free of all encumbrances, except taxes accruing subsequent to **December 31, 2007**.

In Witness Whereof, the said grantor has caused these presents to be executed in its name and its corporate seal to be hereunto affixed by its proper officers thereunto duly authorized, the day and year first above written.

Signed, sealed and delivered in the presence of:

COOK REAL ESTATE INVESTMENTS, INC.

By: Donald W. Cook, Jr. L.S.
Name: **DONALD W. COOK, JR.**
Title: **DIRECTOR**

Lisa E. Davis
Witness Signature **Lisa E. Davis**

Donald W. Cook, Jr.
Printed Name
Witness Signature
Donald W. Cook, Jr.
Printed Name

STATE OF FLORIDA
COUNTY OF Alachua

The foregoing instrument was acknowledged before me this 8th day of February, 2007, by **DONALD W. COOK, JR.** as **DIRECTOR** of **COOK REAL ESTATE INVESTMENTS, INC., A FLORIDA CORPORATION**. He (she) is personally known to me or has produced o.c. d.c. as identification.

Lisa E. Davis
Notary Public
Commission # **DD536588**
Expires **June 2, 2010**
Small Notary Public Insurance Inc. 850-365-7019

Lisa E. Davis
Notary Public
My commission expires

TOWN OF FORT WHITE

Home of the Ichetucknee River

Post Office Box 129 Fort White, FL 32038
Email: townofftwhite@alltel.net Web: townoffortwhitefl.com
Tel: (386) 497-2321/(386) 497-3345 Fax: (386) 497-4946
Office Hours: Monday through Friday 9:00 a.m. to 1:00 p.m

CERTIFICATE OF COMPLIANCE & REQUEST FOR ISSUANCE OF BUILDING PERMIT

The undersigned hereby certify the following property is in compliance with the Town of Fort White's Comprehensive Plan and Land Development Regulations for the stated development purposes:

OWNER'S NAME: McGlynn, James A.

ADDRESS: 14112 NW 30th Ave. Gainesville, FL 32606

PROPERTY DESCRIPTION: Block D parcel #4059-262 .50ac
w/ parcel number 124 SW Lanktree Glen Fort White, FL 32038

DEVELOPMENT: Single Family Dwelling

You are hereby authorized to issue the appropriate permits

3/20/07
DATE

Janice E. Revels
LDR ADMINISTRATOR (Ka)
Town of Fort White

Mayor
Truett George
497-4741

District 1
Donald Cook
497-1086

District 2
Henry Maini
497-2992

District 3
Warren Barnes
497-3112

District 4
Demetric Jackson
497-2078

Town of Fort White
P.O. Box 129
Fort White, FL 32028

Property Owner Affidavit

Office Use Only

Application No. _____

McGlynn South Properties LLC

Owner

James A McGlynn

Additional Owners

James A McGlynn

Appointed Agent(s)

3465-16-4059-262

Parcel Number(s)

Section Township Range

New residential construction

Type of Request

I (we), the property owner(s) of the subject property, being duly sworn, depose and say the following:

1. That I am (we are) the owner(s) and record title holder(s) of the property described in the attached legal description;
2. That this property constitutes the property for which the above noted land use request is being made to the Town of Fort White;
3. That I (we), the undersigned, have appointed, and do appoint, the above noted person(s) as my (our) agent(s) to execute any agreement(s), and other documents necessary to effectuate such agreement(s) in the process of pursuing the aforementioned land use request;
4. That this affidavit has been executed to induce the Town of Fort White to consider and act on the subject request; and,
5. That I (we), the undersigned authority, hereby certify that the foregoing statements are true and correct.

[Signature]
Owner (signature)

Owner (signature)

STATE OF FLORIDA
COUNTY OF Alachua

SWORN AND SUBSCRIBED BEFORE ME
THIS 12 DAY OF March, 2007
BY Eve Stairs
WHO IS/ARE PERSONALLY KNOWN TO ME OR HAS/HAVE
PRODUCED _____
AS IDENTIFICATION.

(SEAL ABOVE)

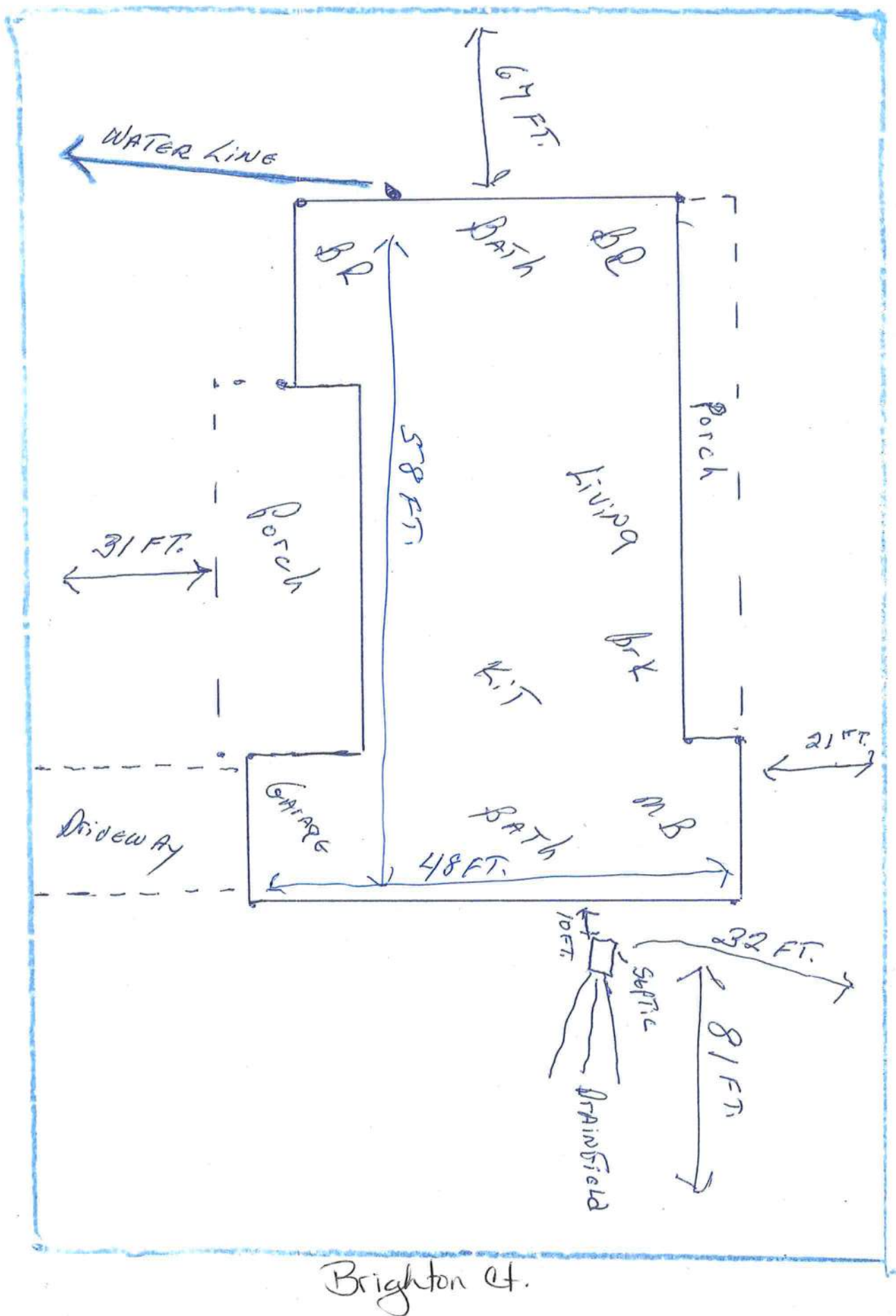
Eve Stairs
Eve Stairs

Notary Public, Commission No. DD635139

(Name of Notary typed, printed, or stamped)



LANTREE Glen



March 7th, 2007

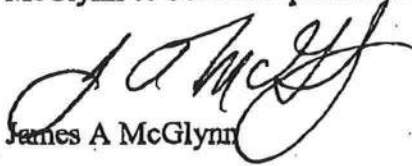
Columbia County Permitting

This letter is to authorize Harvey building and Construction to obtain a building permit and a septic tank permit for the following address.

124 SW Lanktree Glen
~~177 SW Brighton Ct~~
Ft. White, FL 32038

Also know as Lot 12 Block D Hollingsworth Estates S/D

McGlynn & South Properties LLC


James A McGlynn

~~Ernest W South~~

IC# 2608558

STATE OF FLORIDA

DEPARTMENT OF BUSINESS AND PROFESSIONAL REGULATION
CONSTRUCTION INDUSTRY LICENSING BOARD

SEQ# L06060701234

DATE	BATCH NUMBER	LICENSE NBR
06/07/2006	050806903	CRC058134

The RESIDENTIAL CONTRACTOR
Named below IS CERTIFIED
Under the provisions of Chapter 489 FS.
Expiration date: AUG 31, 2008

HARVEY, GERALD E
HARVEY BUILDING & CONSTRUCTION INC
PO BOX 526
NEWBERRY FL 32669

JEB BUSH
GOVERNOR

DISPLAY AS REQUIRED BY LAW

SIMONE MARSTILLER
SECRETARY

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs Residential Whole Building Performance Method A

Project Name: FT. WHT. SPEC Address: City, State: , Owner: Climate Zone: North	Builder: HARVEY BLDG & CONST INC Permitting Office: Columbia Co. Permit Number: 25674 Jurisdiction Number: 221000
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1. New construction or existing New ☐ 2. Single family or multi-family Single family ☐ 3. Number of units, if multi-family 1 ☐ 4. Number of Bedrooms 3 ☐ 5. Is this a worst case? Yes ☐ 6. Conditioned floor area (ft²) 1803 ft² ☐ 7. Glass type¹ and area: (Label reqd. by 13-104.4.5 if not default) <table style="width: 100%;"> <tr> <td style="width: 30%;">a. U-factor:</td> <td style="width: 30%;">Description</td> <td style="width: 40%;">Area</td> </tr> <tr> <td>(or Single or Double DEFAULT)</td> <td>7a. (Dble Default)</td> <td>213.0 ft²</td> </tr> <tr> <td>b. SHGC:</td> <td>7b. (Clear)</td> <td>213.0 ft²</td> </tr> <tr> <td>(or Clear or Tint DEFAULT)</td> <td></td> <td></td> </tr> </table> 8. Floor types <table style="width: 100%;"> <tr> <td style="width: 30%;">a. Slab-On-Grade Edge Insulation</td> <td style="width: 30%;">R=0.0, 201.0(p) ft</td> <td style="width: 40%;"></td> </tr> <tr> <td>b. N/A</td> <td></td> <td></td> </tr> <tr> <td>c. N/A</td> <td></td> <td></td> </tr> </table> 9. Wall types <table style="width: 100%;"> <tr> <td style="width: 30%;">a. Frame, Wood, Exterior</td> <td style="width: 30%;">R=13.0, 1147.0 ft²</td> <td style="width: 40%;"></td> </tr> <tr> <td>b. Frame, Wood, Adjacent</td> <td>R=13.0, 230.0 ft²</td> <td></td> </tr> <tr> <td>c. N/A</td> <td></td> <td></td> </tr> <tr> <td>d. N/A</td> <td></td> <td></td> </tr> <tr> <td>e. N/A</td> <td></td> <td></td> </tr> </table> 10. Ceiling types <table style="width: 100%;"> <tr> <td style="width: 30%;">a. Under Attic</td> <td style="width: 30%;">R=30.0, 1803.0 ft²</td> <td style="width: 40%;"></td> </tr> <tr> <td>b. Under Attic</td> <td>R=19.0, 122.0 ft²</td> <td></td> </tr> <tr> <td>c. N/A</td> <td></td> <td></td> </tr> </table> 11. Ducts <table style="width: 100%;"> <tr> <td style="width: 30%;">a. Sup: Unc. Ret: Unc. AH: Interior</td> <td style="width: 30%;">Sup. R=6.0, 189.0 ft</td> <td style="width: 40%;"></td> </tr> <tr> <td>b. N/A</td> <td></td> <td></td> </tr> </table>	a. U-factor:	Description	Area	(or Single or Double DEFAULT)	7a. (Dble Default)	213.0 ft ²	b. SHGC:	7b. (Clear)	213.0 ft ²	(or Clear or Tint DEFAULT)			a. Slab-On-Grade Edge Insulation	R=0.0, 201.0(p) ft		b. N/A			c. N/A			a. Frame, Wood, Exterior	R=13.0, 1147.0 ft ²		b. Frame, Wood, Adjacent	R=13.0, 230.0 ft ²		c. N/A			d. N/A			e. N/A			a. Under Attic	R=30.0, 1803.0 ft ²		b. Under Attic	R=19.0, 122.0 ft ²		c. N/A			a. Sup: Unc. Ret: Unc. AH: Interior	Sup. R=6.0, 189.0 ft		b. N/A			12. Cooling systems <table style="width: 100%;"> <tr> <td style="width: 30%;">a. Central Unit</td> <td style="width: 30%;">Cap: 33.0 kBtu/hr</td> <td style="width: 40%;"></td> </tr> <tr> <td></td> <td>SEER: 13.00</td> <td></td> </tr> <tr> <td>b. N/A</td> <td></td> <td></td> </tr> <tr> <td>c. N/A</td> <td></td> <td></td> </tr> </table> 13. Heating systems <table style="width: 100%;"> <tr> <td style="width: 30%;">a. Electric Heat Pump</td> <td style="width: 30%;">Cap: 35.2 kBtu/hr</td> <td style="width: 40%;"></td> </tr> <tr> <td></td> <td>HSPF: 7.70</td> <td></td> </tr> <tr> <td>b. N/A</td> <td></td> <td></td> </tr> <tr> <td>c. N/A</td> <td></td> <td></td> </tr> </table> 14. Hot water systems <table style="width: 100%;"> <tr> <td style="width: 30%;">a. Electric Resistance</td> <td style="width: 30%;">Cap: 50.0 gallons</td> <td style="width: 40%;"></td> </tr> <tr> <td></td> <td>EF: 0.92</td> <td></td> </tr> <tr> <td>b. N/A</td> <td></td> <td></td> </tr> <tr> <td>c. Conservation credits</td> <td></td> <td></td> </tr> <tr> <td colspan="3">(HR-Heat recovery, Solar DHP-Dedicated heat pump)</td> </tr> </table> 15. HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)	a. Central Unit	Cap: 33.0 kBtu/hr			SEER: 13.00		b. N/A			c. N/A			a. Electric Heat Pump	Cap: 35.2 kBtu/hr			HSPF: 7.70		b. N/A			c. N/A			a. Electric Resistance	Cap: 50.0 gallons			EF: 0.92		b. N/A			c. Conservation credits			(HR-Heat recovery, Solar DHP-Dedicated heat pump)		
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Glass/Floor Area: 0.12 Total as-built points: 23034
 Total base points: 24477

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: SUNCOAST INSULATORS
825 NW 253rd Terrace
Newberry, FL 32849
(352) 472-8995
Fax (352) 472-2633

DATE: 2/15/07

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____
DATE: _____

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.



BUILDING OFFICIAL: _____
DATE: _____

¹ Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum: .3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT					
WATER HEATING				Tank	EF	Number of	X	Tank X	Multiplier X
Number of		Multiplier	=	Volume		Bedrooms		Ratio	Credit =
Bedrooms			Total						Multiplier
3		2635.00	7905.0	50.0	0.92	3		1.00	2635.00
									1.00
									7905.0
				As-Built Total:					7905.0

CODE COMPLIANCE STATUS

BASE				AS-BUILT			
Cooling	+	Heating	+	Cooling	+	Heating	+
Points		Points		Points		Points	
Hot Water	=	Total		Hot Water	=	Total	
Points	Points	Points	Points	Points	Points	Points	Points
7313		9258		6732		8397	
		7905				7905	
		24477				23034	

PASS

WINTER CALCULATIONS**Residential Whole Building Performance Method A - Details**

ADDRESS: , , ,

PERMIT #:

BASE			AS-BUILT						
Winter Base Points: 16712.0			Winter As-Built Points: 16315.8						
Total Winter Points	X System Multiplier	= Heating Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points	
16712.0	0.5540	9258.4	(sys 1: Electric Heat Pump 35200 btuh ,EFF(7.7) Ducts:Unc(S),Unc(R),Int(AH),R6.0 16315.8 1.000 (1.069 x 1.169 x 0.93) 0.443 1.000 8397.4 16315.8 1.00 1.162 0.443 1.000 8397.4						

WINTER CALCULATIONS**Residential Whole Building Performance Method A - Details**

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT							
GLASS TYPES											
.18 X Conditioned X BWPM = Points Floor Area				Type/SC	Overhang Omt Len Hgt			Area X WPM X WOF = Points			
.18	1803.0	20.17	6546.0	1.Double, Clear	N	2.0	6.0	4.0	24.58	1.00	98.0
				2.Double, Clear	E	2.0	6.0	95.0	18.79	1.06	1893.0
				3.Double, Clear	W	2.0	6.0	114.0	20.73	1.04	2463.0
				As-Built Total:			213.0		4454.0		
WALL TYPES Area X BWPM = Points				Type	R-Value			Area X WPM = Points			
Adjacent	230.0	3.60	828.0	1. Frame, Wood, Exterior	13.0			1147.0	3.40	3899.8	
Exterior	1147.0	3.70	4243.9	2. Frame, Wood, Adjacent	13.0			230.0	3.30	759.0	
Base Total:				As-Built Total:			1377.0		4658.8		
DOOR TYPES Area X BWPM = Points				Type				Area X WPM = Points			
Adjacent	20.0	11.50	230.0	1.Exterior Insulated				36.0	8.40	302.4	
Exterior	36.0	12.30	442.8	2.Adjacent Insulated				20.0	8.00	160.0	
Base Total:				As-Built Total:			56.0		462.4		
CEILING TYPES Area X BWPM = Points				Type	R-Value			Area X WPM X WCM = Points			
Under Attic	1803.0	2.05	3696.1	1. Under Attic	30.0			1803.0	2.05 X 1.00	3696.1	
				2. Under Attic	19.0			122.0	2.70 X 1.00	329.4	
Base Total:				As-Built Total:			1925.0		4025.5		
FLOOR TYPES Area X BWPM = Points				Type	R-Value			Area X WPM = Points			
Slab	201.0(p)	8.9	1788.9	1. Slab-On-Grade Edge Insulation	0.0			201.0(p)	18.80	3778.8	
Raised	0.0	0.00	0.0								
Base Total:				As-Built Total:			201.0		3778.8		
INFILTRATION Area X BWPM = Points							Area X WPM = Points				
1803.0 -0.59 -1063.8							1803.0 -0.59 -1063.8				

SUMMER CALCULATIONS**Residential Whole Building Performance Method A - Details**

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT						
Summer Base Points: 22502.3				Summer As-Built Points: 22757.2						
Total Summer Points	X Multiplier	=	Cooling Points	Total Component (System - Points)	X Ratio	Cap X (DM x DSM x AHU)	Duct X Multiplier	System X Multiplier	Credit X Multiplier	= Cooling Points
22502.3	0.3250		7313.3	(sys 1: Central Unit 33000btuh ,SEER/EFF(13.0) Ducts:Unc(S),Unc(R),Int(AH),R6.0(INS) 22757	1.00	(1.09 x 1.147 x 0.91)	0.260	1.000		6731.7
				22757.2	1.00	1.138	0.260	1.000		6731.7

SUMMER CALCULATIONS**Residential Whole Building Performance Method A - Details**

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT							
GLASS TYPES											
.18 X Conditioned X BSPM = Points Floor Area				Type/SC	Overhang Ormt Len Hgt		Area X SPM X SOF = Points				
.18	1803.0	18.59	6033.0	1.Double, Clear	N	2.0	6.0	4.0	19.20	0.90	69.0
				2.Double, Clear	E	2.0	6.0	95.0	42.06	0.85	3388.0
				3.Double, Clear	W	2.0	6.0	114.0	38.52	0.85	3730.0
				As-Built Total:				213.0	7187.0		
WALL TYPES				Area X BSPM = Points		Type	R-Value	Area X SPM = Points			
Adjacent	230.0	0.70	161.0	1. Frame, Wood, Exterior			13.0	1147.0	1.50	1720.5	
Exterior	1147.0	1.70	1949.9	2. Frame, Wood, Adjacent			13.0	230.0	0.60	138.0	
Base Total:		1377.0	2110.9	As-Built Total:				1377.0	1858.5		
DOOR TYPES				Area X BSPM = Points		Type	Area X SPM = Points				
Adjacent	20.0	2.40	48.0	1.Exterior Insulated				36.0	4.10	147.6	
Exterior	36.0	6.10	219.6	2.Adjacent Insulated				20.0	1.60	32.0	
Base Total:		56.0	267.6	As-Built Total:				56.0	179.6		
CEILING TYPES				Area X BSPM = Points		Type	R-Value	Area X SPM X SCM = Points			
Under Attic	1803.0	1.73	3119.2	1. Under Attic			30.0	1803.0	1.73 X 1.00	3119.2	
				2. Under Attic			19.0	122.0	2.34 X 1.00	285.5	
Base Total:		1803.0	3119.2	As-Built Total:				1925.0	3404.7		
FLOOR TYPES				Area X BSPM = Points		Type	R-Value	Area X SPM = Points			
Slab	201.0(p)	-37.0	-7437.0	1. Slab-On-Grade Edge Insulation			0.0	201.0(p)	-41.20	-8281.2	
Raised	0.0	0.00	0.0								
Base Total:		-7437.0	-7437.0	As-Built Total:				201.0	-8281.2		
INFILTRATION				Area X BSPM = Points		Area X SPM = Points					
		1803.0	10.21	18408.6				1803.0	10.21	18408.6	

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 85.6

The higher the score, the more efficient the home.

1. New construction or existing	New	12. Cooling systems	
2. Single family or multi-family	Single family	a. Central Unit	Cap: 33.0 kBtu/hr
3. Number of units, if multi-family	1		SEER: 13.00
4. Number of Bedrooms	3	b. N/A	
5. Is this a worst case?	Yes	c. N/A	
6. Conditioned floor area (ft ²)	1803 ft ²		
7. Glass type ¹ and area: (Label reqd. by 13-104.4.5 if not default)		13. Heating systems	
a. U-factor:	Description Area	a. Electric Heat Pump	Cap: 35.2 kBtu/hr
(or Single or Double DEFAULT)	7a. (Dble Default) 213.0 ft ²		HSPF: 7.70
b. SHGC:		b. N/A	
(or Clear or Tint DEFAULT)	7b. (Clear) 213.0 ft ²	c. N/A	
8. Floor types			
a. Slab-On-Grade Edge Insulation	R=0.0, 201.0(p) ft	14. Hot water systems	
b. N/A		a. Electric Resistance	Cap: 50.0 gallons
c. N/A			EF: 0.92
9. Wall types		b. N/A	
a. Frame, Wood, Exterior	R=13.0, 1147.0 ft ²	c. Conservation credits	
b. Frame, Wood, Adjacent	R=13.0, 230.0 ft ²	(HR-Heat recovery, Solar	
c. N/A		DHP-Dedicated heat pump)	
d. N/A		15. HVAC credits	
e. N/A		(CF-Ceiling fan, CV-Cross ventilation,	
10. Ceiling types		HF-Whole house fan,	
a. Under Attic	R=30.0, 1803.0 ft ²	PT-Programmable Thermostat,	
b. Under Attic	R=19.0, 122.0 ft ²	MZ-C-Multizone cooling,	
c. N/A		MZ-H-Multizone heating)	
11. Ducts			
a. Sup. Unc. Ret: Unc. AH: Interior	Sup. R=6.0, 189.0 ft		
b. N/A			

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: _____ Date: _____

Address of New Home: _____ City/FL Zip: _____



**NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStarTM designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.*

¹ Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.
EnergyGauge® (Version: FLRCSB v4.5)

NOTICE OF COMMENCEMENT FORM
COLUMBIA COUNTY, FLORIDATHIS DOCUMENT MUST BE RECORDED AT THE COUNTY
CLERKS OFFICE BEFORE YOUR FIRST INSPECTIONTHE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and
in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of
Commencement.IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE
RECORDING YOUR NOTICE OF COMMENCEMENT.Tax Parcel ID Number 34-65-16-04059-262 Permit Number 00025674

1. Description of property: (legal description of the property and street address or 911 address)

Lot 12 Block D Hollingsworth Estates
124 S.W. Lank Tree Glen
Fort White, FL 320382. General description of improvement: construction - new home3. Owner Name & Address James McGlynn
2427 N.W. 147th St. Gainesville Interest in Property owner

4. Name & Address of Fee Simple Owner (if other than owner):

5. Contractor Name Harvey Bldg Const Inc. Phone Number 352-472-4468
Address P.O. Box 526 Newberry, FL 32669

6. Surety Holders Name _____

Address _____

Inst: 2007007386 Date: 04/02/2007 Time: 11:47

Amount of Bond _____

S.A. DC, P. Dewitt Cason, Columbia County B: 1115 P: 741

7. Lender Name _____

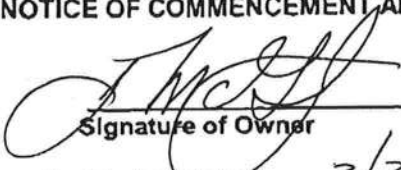
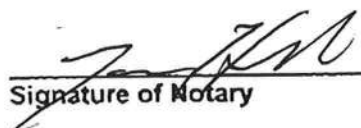
Address _____

8. Persons within the State of Florida designated by the Owner upon whom notices or other documents may be
served as provided by section 718.13 (1)(a) 7; Florida Statutes:Name Gerald Harvey or Janice Harvey Phone Number 352-472-4468Address P.O. Box 526 Newberry, FL 32669

9. In addition to himself/herself the owner designates _____ of

_____ to receive a copy of the Lien Notice as provided in Section 713.13 (1) -

(a) 7. Phone Number of the designee _____

10. Expiration date of the Notice of Commencement (the expiration date is 1 (one) year from the date of
recording, (Unless a different date is specified) _____THE OWNER MUST SIGN THE NOTICE OF COMMENCEMENT AND NO ONE ELSE MAY BE PERMITTED TO SIGN
IN HIS/HER STEAD.
Signature of OwnerSworn to (or affirmed) and subscribed before day of 3/23, 2007.
Signature of Notary NOTARY STAMP/SEALJASON P. HALL
MY COMMISSION # DD445837
EXPIRES JUNE 28, 2009

UNIVERSAL

ENGINEERING SCIENCES

Consultants In: Geotechnical Engineering •
Environmental Sciences • Construction Materials Testing

4475 S.W. 35th Terrace • Gainesville, Florida 32608 • (352) 372-3392

REPORT ON IN-PLACE DENSITY TESTS

Permit 000025674

CLIENT: Harvey Builders

PROJECT: 124 SW Lanktree Clear

AREA TESTED: F.H. & prop bldg pad

COURSE: F/G

DEPTH OF TEST: 0"-1'

TYPE OF TEST: ASTM D-2922

DATE TESTED: 4-15-07

NOTE: The below tests ~~DO/DO NOT~~ meet the minimum 95 % compaction requirements of maximum density.

REMARKS: Foundations are in N/G

[illegible]

TECH.

COLUMBIA COUNTY BUILDING DEPARTMENT

Revised 10-01-05

RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2004 and FLORIDA RESIDENTIAL CODE 2004 WITH AMENDMENTS ONE (1) AND TWO (2) FAMILY DWELLINGS

ALL REQUIREMENTS ARE SUBJECT TO CHANGE
EFFECTIVE OCTOBER 1, 2005

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 16 OF THE FLORIDA BUILDING CODE 2004 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1609 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE _____ 100 MPH
2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE _____ 110 MPH
3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

APPLICANT - PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

GENERAL REQUIREMENTS: Two (2) complete sets of plans containing the following:

Applicant	Plans Examiner	
☒	☐	All drawings must be clear, concise and drawn to scale ("Optional " details that are not used shall be marked void or crossed off). Square footage of different areas shall be shown on plans.
☒	☐	Designers name and signature on document (FBC 106.1). If licensed architect or engineer, official seal shall be affixed.
☒	☐	<u>Site Plan including:</u> a) Dimensions of lot b) Dimensions of building set backs c) Location of all other buildings on lot, well and septic tank if applicable, and all utility easements. d) Provide a full legal description of property.
☒	☐	<u>Wind-load Engineering Summary, calculations and any details required</u> Plans or specifications must state compliance with FBC Section 1609. The following information must be shown as per section 1603.1.4 FBC a. Basic wind speed (3-second gust), miles per hour (km/hr). b. Wind importance factor, I_w , and building classification from Table 1604.5 or Table 6-1, ASCE 7 and building classification in Table 1-1, ASCE 7. c. Wind exposure, if more than one wind exposure is utilized, the wind exposure and applicable wind direction shall be indicated. d. The applicable enclosure classifications and, if designed with ASCE 7, internal pressure coefficient. e. Components and Cladding. The design wind pressures in terms of psf (kN/m ²) to be used for the design of exterior component and cladding materials not specifi ally designed by the registered design professional.
	☐	<u>Elevations including:</u> a) All sides b) Roof pitch c) Overhang dimensions and detail with attic ventilation
	☐	
	☐	

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10/10

- a. Attic space
- b. Exterior wall cavity
- c. Crawl space (if applicable)

☒ ☐

b) Wood frame wall

1. All materials making up wall
2. Size and species of studs
3. Sheathing size, type and nailing schedule
4. Headers sized
5. Gable end showing balloon framing detail or gable truss and wall hinge bracing detail
6. All required fasteners for continuous tie from roof to foundation (truss anchors, straps, anchor bolts and washers) shall be designed by a Windload engineer using the engineered roof truss plans.
7. Roof assembly shown here or on roof system detail (FBC 106.1.1.2) Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
8. Fire resistant construction (if applicable)
9. Fireproofing requirements
10. Show type of termite treatment (termiteicide or alternative method)
11. Slab on grade
 - a. Vapor retarder (6Mil. Polyethylene with joints lapped 6 inches and sealed
 - b. Must show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and supports
12. Indicate where pressure treated wood will be placed
13. Provide insulation R value for the following:
 - a. Attic space
 - b. Exterior wall cavity
 - c. Crawl space (if applicable)

☐ ☐

c) Metal frame wall and roof (designed, signed and sealed by Florida Prof. Engineer or Architect)

Floor Framing System:

- a) Floor truss package including layout and details, signed and sealed by Florida Registered Professional Engineer
- b) Floor joist size and spacing
- c) Girder size and spacing
- d) Attachment of joist to girder
- e) Wind load requirements where applicable

Plumbing Fixture layout

Electrical layout including:

- a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified
- b) Ceiling fans
- c) Smoke detectors
- d) Service panel and sub-panel size and location(s)
- e) Meter location with type of service entrance (overhead or underground)
- f) Appliances and HVAC equipment
- g) Arc Fault Circuits (AFCI) in bedrooms
- h) Exhaust fans in bathroom

HVAC information

- a) **Energy Calculations** (dimensions shall match plans)
- b) Manual J sizing equipment or equivalent computation
- c) **Gas System** Type (LP or Natural) Location and BTU demand of equipment

Disclosure Statement for Owner Builders

*****Notice Of Commencement Required Before Any Inspections Will Be Done Private Potable Water**

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- a) Size of pump motor
- b) Size of pressure tank
- c) Cycle stop valve if used

THE FOLLOWING ITEMS MUST BE SUBMITTED WITH BUILDING PLANS

1. **Building Permit Application:** A current Building Permit Application form is to be completed and submitted for all residential projects.
2. **Parcel Number:** The parcel number (Tax ID number) from the Property Appraiser (386) 758-1084 is required. A copy of property deed is also requested.
3. **Environmental Health Permit or Sewer Tap Approval:** A copy of the Environmental Health permit, existing septic approval or sewer tap approval is required before a building permit can be issued. (386) 758-1058 (Toilet facilities shall be provided for construction workers)
4. **City Approval:** If the project is to be located within the city limits of the Town of Fort White, prior approval is required. The Town of Fort White approval letter is required to be submitted by the owner or contractor to this office when applying for a Building Permit. (386) 497-2321
5. **Flood Information:** All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting application to this office. Any project located within a flood zone where the base flood elevation (100 year flood) has been established shall meet the requirements of Section 8.8 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.7 of the Columbia County Land Development Regulations. **CERTIFIED FINISHED FLOOR ELEVATIONS WILL BE REQUIRED ON ANY PROJECT WHERE THE BASE FLOOD ELEVATION (100 YEAR FLOOD) HAS BEEN ESTABLISHED.**
A development permit will also be required. Development permit cost is \$50.00
6. **Driveway Connection:** If the property does not have an existing access to a public road, then an application for a culvert permit (\$25.00) must be made. If the applicant feels that a culvert is not needed, they may apply for a culvert waiver (\$50.00). All culvert waivers are sent to the Columbia County Public Works Department for approval or denial. **If the project is to be located on a F.D.O.T. maintained road, than an F.D.O.T. access permit is required.**
7. **911 Address:** If the project is located in an area where the 911 address has been issued, then the proper paperwork from the 911 Addressing Department must be submitted. (386) 752-8787

ALL REQUIRED INFORMATION IS TO BE SUBMITTED FOR REVIEW. YOU WILL BE NOTIFIED WHEN YOUR APPLICATION AND PLANS ARE APPROVED AND READY TO PERMIT. PLEASE DO NOT EXPECT OR REQUEST THAT PERMIT APPLICATIONS BE REVIEWED OR APPROVED WHILE YOU ARE HERE – TIME WILL NOT ALLOW THIS – PLEASE DO NOT ASK

PRODUCT APPROVAL SPECIFICATION SHEET

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and approval numbers on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. Statewide approved products are listed online @ www.floridabuilding.org

Category/Subcategory	Manufacturer	Product Description	Approval Number(s)
1. EXTERIOR DOORS			
A. SWINGING			
B. SLIDING			
C. SECTIONAL/ROLL UP			
D. OTHER			
2. WINDOWS			
A. SINGLE/DOUBLE HUNG	BETTER BLIND	Single Hung INS.	47182
B. HORIZONTAL SLIDER			
C. CASEMENT			
D. FIXED			
E. MULLION			
F. SKYLIGHTS			
G. OTHER			
3. PANEL WALL			
A. SIDING	Hardi Board		PLA. 889
B. SOFFITS			
C. STOREFRONTS			
D. GLASS BLOCK			
E. OTHER			
4. ROOFING PRODUCTS			
A. ASPHALT SHINGLES	15+30 Lb	FALT	ASTM D4869
B. NON-STRUCT METAL			
C. ROOFING TILES			
D. SINGLE PLY ROOF			
E. OTHER			
5. STRUCT COMPONENTS			
A. WOOD CONNECTORS			
B. WOOD ANCHORS			
C. TRUSS PLATES			
D. INSULATION FORMS			
E. LINTELS			
F. OTHERS			
6. NEW EXTERIOR ENVELOPE PRODUCTS			
A.			

The products listed below did not demonstrate product approval at plan review. I understand that at the time of inspection of these products, the following information must be available to the inspector on the jobsite; 1) copy of the product approval, 2) performance characteristics which the product was tested and certified to comply with, 3) copy of the applicable manufacturers installation requirements. Further, I understand these products may have to be removed if approval cannot be demonstrated during inspection.


APPLICANT SIGNATURE

3/18/07
DATE

MAR-19-2007 13:03 From:

To: 813525053246

Page: 2/2

COLUMBIA COUNTY 9-1-1 ADDRESSING

P. O. Box 1787, Lake City, FL 32016-1787

PHONE: (386) 758-1125 * FAX: (386) 758-1365 * Email: ron_croft@columbiacountyfla.com

Addressing Maintenance

To maintain the Countywide Addressing Policy you must make application for a 9-1-1 Address at the time you apply for a building permit. The established standards for assigning and posting numbers to all principal buildings, dwellings, businesses and industries are contained in Columbia County Ordinance 2001-9. The addressing system is to enable Emergency Service Agencies to locate you in an emergency, and to assist the United States Postal Service and the public in the timely and efficient provision of services to residents and businesses of Columbia County.

DATE REQUESTED: 2/20/2007 DATE ISSUED: 2/22/2007

ENHANCED 9-1-1 ADDRESS:

124 SW LANKTREE GLN
FORT WHITE FL 32038
PROPERTY APPRAISER PARCEL NUMBER:
34-6S-16-04059-262

Remarks:

WAS ASSIGNED 177 BRIGHTON, CHANGED DUE TO ACCESS

* Re-Addressed 3/19/07

Address Issued By:


Columbia County 9-1-1 Addressing / GIS Department

NOTICE: THIS ADDRESS WAS ISSUED BASED ON LOCATION INFORMATION RECEIVED FROM THE REQUESTER. SHOULD, AT A LATER DATE, THE LOCATION INFORMATION BE FOUND TO BE IN ERROR, THIS ADDRESS IS SUBJECT TO CHANGE.

COLUMBIA COUNTY
9-1-1 ADDRESSING
APPROVED

635



Load Short Form
Entire House
Bounds Heating & Air

Job: Fort White Spec.
Date: 02-14-07
By: Michael C.

Project Information

For: Harvey Build. and Const.

Design Information

	Htg	Clg		Infiltration
Outside db (°F)	31	93	Method	Simplified
Inside db (°F)	68	75	Construction quality	Tight
Design TD (°F)	37	18	Fireplaces	1 (Average)
Daily range	-	M		
Inside humidity (%)	-	50		
Moisture difference (gr/lb)	-	50		

HEATING EQUIPMENT

Make	Carrier
Trade	Base 13 Puron HP
Model	25HBA336A30
Efficiency	7.7 HSPF
Heating input	
Heating output	35200 Btuh @ 47°F
Temperature rise	29 °F
Actual air flow	1100 cfm
Air flow factor	0.035 cfm/Btuh
Static pressure	1.00 in H2O
Space thermostat	

COOLING EQUIPMENT

Make	Carrier
Trade	Base 13 Puron HP
Cond	25HBA336A30
Coil	FY4ANF036
Efficiency	13 SEER
Sensible cooling	23100 Btuh
Latent cooling	9900 Btuh
Total cooling	33000 Btuh
Actual air flow	1100 cfm
Air flow factor	0.051 cfm/Btuh
Static pressure	1.00 in H2O
Load sensible heat ratio	0.84

ROOM NAME	Area (ft²)	Htg load (Btuh)	Clg load (Btuh)	Htg AVF (cfm)	Clg AVF (cfm)
bed room 1	180	3747	2006	133	103
bath	72	969	297	34	15
bed room 2	168	4320	2091	153	107
hall	36	44	68	2	3
study	119	2106	2674	75	137
foyer	45	1169	805	41	41
dining room	110	3173	1531	112	78
kitchen	151	575	1678	20	86
living room	409	6056	4826	215	247
master suite	291	5246	2647	186	136
w.i.c	28	34	53	1	3
w.i.c.2	23	518	133	18	7
master bath	85	1984	993	70	51
utility	45	544	1496	19	77
toilet	18	22	34	1	2
w.i.c. 3	23	518	133	18	7

Bold/italic values have been manually overridden

Printout certified by ACCA to meet all requirements of Manual J 8th Ed.



wrightsoft Right-Suite Residential 6.0.95 RSR25683

2007-Feb-14 14:13:39

C:\Documents and Settings\Bryan Bounds\My Documents\Wrightsoft HVAC\Harvey con\Fort White Spec 1803

Page 1

Entire House	1803	31024	21464	1100	1100
Other equip loads		3001	1460		
Equip. @ 0.98 RSM			22466		
Latent cooling			4399		
TOTALS	1803	34025	26865	1100	1100

Bold/italic values have been manually overridden

Printout certified by ACCA to meet all requirements of Manual J 8th Ed.



Duct System Summary Entire House Bounds Heating & Air

Job: Fort White Spec.
Date: 02-14-07
By: Michael C.

Project Information

For: Harvey Build. and Const.

	Heating	Cooling
External static pressure	1.00 in H2O	1.00 in H2O
Pressure losses	0.30 in H2O	0.30 in H2O
Available static pressure	0.70 in H2O	0.70 in H2O
Supply / return available pressure	0.53 / 0.17 in H2O	0.53 / 0.17 in H2O
Lowest friction rate	0.100 in/100ft	0.100 in/100ft
Actual air flow	1100 cfm	1100 cfm
Total effective length (TEL)	638 ft	

Supply Branch Detail Table

Name	Design (Btuh)	Htg (cfm)	Clg (cfm)	Design FR	Diam (in)	Rect Size (in)	Duct Matl	Actual Ln (ft)	Ftg.Eqv Ln (ft)	Trunk
bed room 1	h 1874	66	51	0.100	6	16x2	VIFx	43.0	430.0	st2
bed room 1-A	h 1874	66	51	0.100	6	16x2	VIFx	37.0	390.0	st2
bath	h 969	34	15	0.100	4	16x1	VIFx	41.0	440.0	st2
bed room 2	h 4320	153	107	0.100	8	16x4	VIFx	41.0	390.0	st2
hall	c 68	2	3	0.100	4	16x1	VIFx	35.0	330.0	st2
study	c 2674	75	137	0.100	8	16x4	VIFx	33.0	340.0	st2
foyer	h 1169	41	41	0.100	5	16x1	VIFx	22.0	180.0	st2
dining room	h 3173	112	78	0.100	7	16x3	VIFx	14.0	180.0	st2
kitchen	c 1678	20	86	0.100	6	16x2	VIFx	9.0	180.0	st2
living room-A	c 2413	107	124	0.100	7	16x3	VIFx	25.0	290.0	st2
living room	c 2413	107	124	0.100	7	16x3	VIFx	17.0	205.0	st2
master suite	h 5246	186	136	0.100	8	16x4	VIFx	25.0	110.0	st1
w.i.c	c 53	1	3	0.100	4	16x1	VIFx	13.0	145.0	st1
w.i.c.2	h 518	18	7	0.100	4	16x1	VIFx	25.0	80.0	st1
master bath	h 1984	70	51	0.100	6	16x2	VIFx	17.0	120.0	st1
utility	c 1496	19	77	0.100	6	16x2	VIFx	7.0	95.0	
toilet	c 34	1	2	0.100	4	16x1	VIFx	14.0	125.0	st1
w.i.c. 3	h 518	18	7	0.100	4	16x1	VIFx	8.0	135.0	st1

Supply Trunk Detail Table

Name	Trunk Type	Htg (cfm)	Clg (cfm)	Design FR	Veloc (fpm)	Diam (in)	Rect Duct Size (in)	Duct Material	Trunk
st1	Peak AVF	295	205	0.100	531	9	16 x 5	RectFbg	
st2	Peak AVF	786	819	0.100	819	13	16 x 9	RectFbg	

Bold/italic values have been manually overridden

Return Branch Detail Table

Name	Grill Size (in)	Htg (cfm)	Clg (cfm)	TEL (ft)	Design FR	Veloc (fpm)	Diam (in)	RectSize (in)	Stud/Joist Opening (in)	Duct Matl	Trunk
rb1	0x0	1100	1100	157.0	0.050	381	22	16x 26		VIFx	rt1

Return Trunk Detail Table

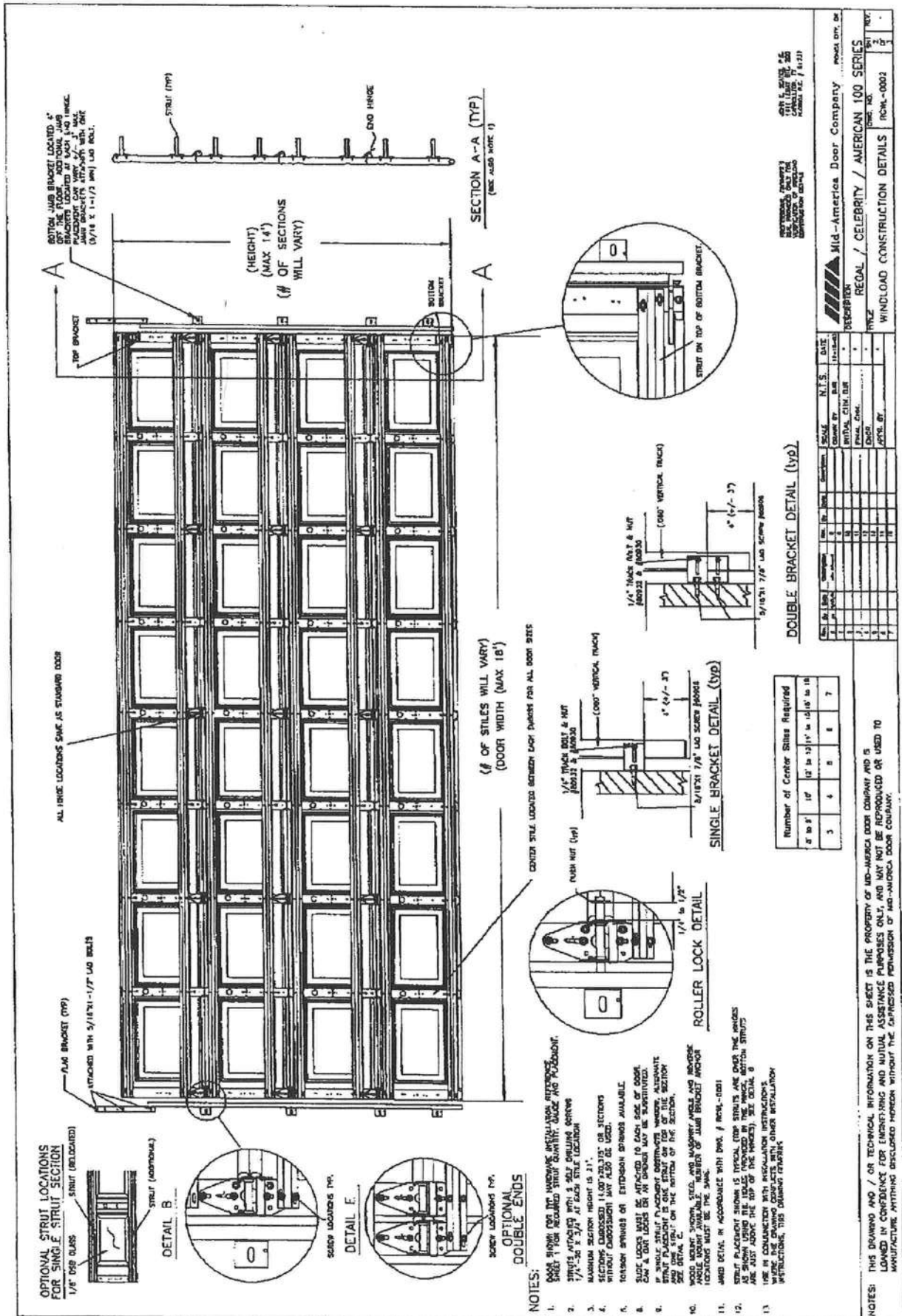
Name	Trunk Type	Htg (cfm)	Clg (cfm)	Design FR	Veloc (fpm)	Diam (in)	Rect Duct Size (in)	Duct Material	Trunk
rt1	Peak AVF	1100	1100	0.050	419	21	18 x 21	RectFbg	

General

FL 1945.3

This table is based on 4 section tall door. Doors over this height use similar construction.
WL rollers require pushnuts

This table is based on 4 section tall door. Doors over this height use similar construction.													
WL rollers require pushnuts													



DOOR FRAME PLATE CONNECTION
SCHEDULE 2.1

MASSONRY CONCRETE CONSTRUCTION	LOAD PER SQ. YD.	LOAD PER CU. YD.	THICKNESS PER INCH	QUANTITY OF MASONRY PER CU. YD.
1000	560	3	3	3
1250	623	3	3	3
1500	729	4	4	4
1750	875	4	4	4
2000	1000	4	4	4
2250	1125	5	5	5
2500	1250	5	5	5
2750	1375	6	6	6
3000	1500	6	6	6
3250	1625	6	6	6
3500	1750	7	7	7
3750	1875	7	7	7
4000	2000	8	8	8
4250	2125	8	8	8
4500	2250	9	9	9
4750	2375	9	9	9
5000	2500	10	10	10
5250	2625	10	10	10
5500	2750	11	11	11
5750	2875	11	11	11
6000	3000	12	12	12
6250	3125	12	12	12
6500	3250	12	12	12
6750	3375	13	13	13
7000	3500	13	13	13
7250	3625	14	14	14
7500	3750	14	14	14

1. CHART IS BASED ON 8'-0" MAX. DOOR HEIGHT.
2. 1/4" WASHERS REQUIRED WITH EACH TAPCON ANCHOR.
3. AND THE FLOOR.
4. ANCHORS TO BE EVENLY SPACED BETWEEN THE HIDEOUT AND EMBLEMMENT.
5. BASED ON 1/4" TAPCON ANCHOR WITH A 1 1/2"

**DOOR FRAME PLATE CONNECTION
SCHEDULE 3.1**

MASONRY/CONCRETE CONSTRUCTION	QUANTITY OF EMISSION FACTORS PER JAMB	2,000 PSI, 1,000 PSI	GRAVEL	LOAD PER	LOAD PER	TOTAL
(TSS.)	1,000 PSI	CONCRETE	1,000 PSI	1,000 PSI	1,000 PSI	1,000 PSI
1000	500	2	2	2	2	2
1500	653	2	2	2	2	2
1600	750	2	2	2	2	2
1750	875	2	2	2	2	2
2000	1000	2	2	2	2	2
2250	1125	2	2	2	2	2
2500	1250	2	2	2	2	2
2800	1500	2	2	2	2	2
3000	1625	2	2	2	2	2
3500	1750	2	2	2	2	2
3750	1875	2	2	2	2	2
4000	2000	2	2	2	2	2
4250	2125	2	2	2	2	2
4500	2250	2	2	2	2	2
4750	2375	2	2	2	2	2
5000	2500	2	2	2	2	2
5250	2625	2	2	2	2	2
5500	2750	2	2	2	2	2
5750	2875	2	2	2	2	2
6000	3000	2	2	2	2	2
6250	3125	2	2	2	2	2
6500	3250	2	2	2	2	2
7000	3500	2	2	2	2	2
7250	3625	2	2	2	2	2
7500	3750	2	2	2	2	2

2. BASED ON 3/8" HILT STUD TYPE EXPANSION ANCHORS WITH A 1/2" MIN. EMBEDMENT.
1. ANCHORS TO BE EVENLY SPACED BETWEEN THE REINFORCING AND THE FLOOR.
2. MINIMUM DISTANCE FROM CONCRETE EDGE TO ANCHOR SHALL BE 3/4".
3. CHART IS BASED ON 8'-0" MAX. DOOR HEIGHT.

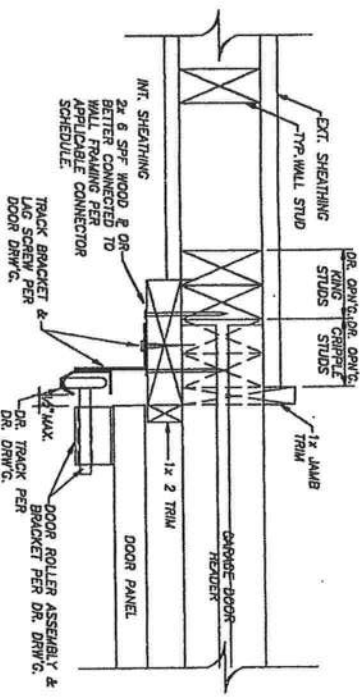
NOTES

1. HAND LOAD PERFORMANCE LABEL MUST BE AFFIXED TO THE INSIDE OF THE DOOR IN A LOCATION THAT IS EASILY SEEN.

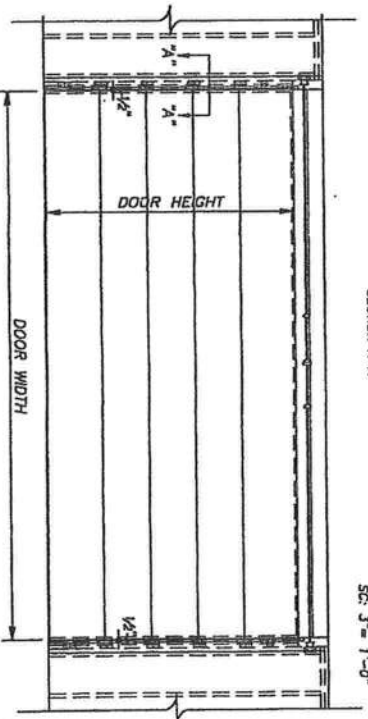
2. WHERE THE DRAWING CONFLICTS WITH OTHER INSTALLATION INSTRUCTIONS, THIS DRAWING GOVERNS.

3. THE TESTING METHODS FOR THE DOORS PERTAINING TO DRAW'S, LISTED HEREIN WERE IN ACCORDANCE WITH DASMA 108.

SBC 1998 & ASCE 7-98.



JAMB FASTENER DETAIL
SECTION A-A



ELEVATION NOTES:
1. SPACE BETWEEN INSIDE OF VERTICAL TRACK AND END OF DOOR IS 1/2" MAX.

DOOR ELEVATION

(INTERIOR ELEVATION)

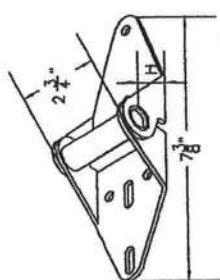
SCALE: 1/4" = 1'-0"

NOTES: THIS DRAWING AND / OR TECHNICAL INFORMATION ON THIS SHEET IS THE PROPERTY OF MID-AMERICA DOOR COMPANY AND IS LOANED IN CONFERENCE FOR ENGINEERING AND MUTUAL ASSISTANCE PURPOSES ONLY, AND MAY NOT BE REPRODUCED OR USED TO MANUFACTURE ANYTHING DISCLOSED HEREON WITHOUT THE EXPRESSED PERMISSION OF MID-AMERICA DOOR COMPANY.

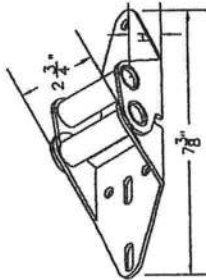
[illegible]

NOTES:

1. STRUTS ARE NOT MORE THAN 1 1/2" SHORTER THAN SECTION.
2. ROLLERS MUST HAVE STEEL BEARINGS.
3. USE IN CONJUNCTION WITH INSTALLATION INSTRUCTIONS. WHERE THE DRAWING CONFLICTS WITH OTHER INSTALLATION INSTRUCTIONS, THIS DRAWING GOVERN.

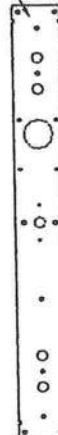


P/N 20602 (#1 HINGE)



END HINGE

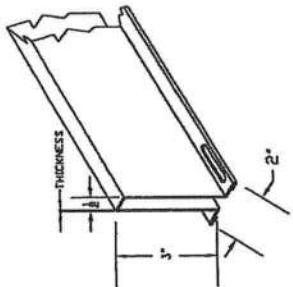
HINGE #	QAGE #	"H"	PART NUMBER
1	14	3/4"	20602
2	14	1"	20608
3	14	1 1/4"	20610
4	14	1 1/2"	20614
5	14	1 3/4"	20616
6	13	2"	20618
7	13	2 1/4"	20620
8	11	2 1/2"	20622
9	11	2 3/4"	20624



P/N 10455 & 10465 (21" & 18") CENTER STILE

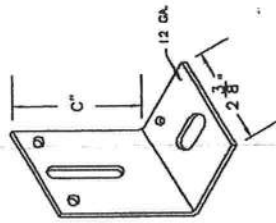


P/N 10450 & 10460 (21" & 18") END STILE



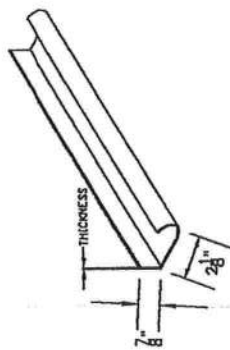
3" WINDLOAD STRUT

THICKNESS OF STRUTS ARE AS STATED BELOW	
50 GAUGE 50 KSI MINIMUM YIELD	208" MIN
18 GAUGE 50 KSI MINIMUM YIELD	247" MIN



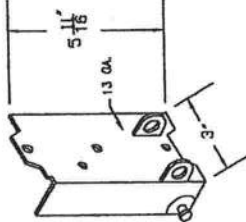
TRACK BRACKET

BRACKET #	"C"	PART NUMBER
4	3"	20368
5	3 1/4"	20370
6	3 1/2"	20372
7	3 3/4"	20374
8	4"	20376
9	4 3/4"	20378

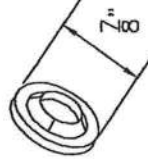


WINDLOAD TRACK

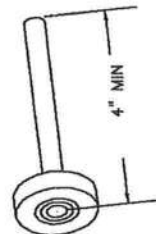
THICKNESS OF TRACK IS AS STATED BELOW	
VERTICAL TRACK IS MIN 16 GA	208" MIN
HORIZONTAL TRACK IS MIN 18 GA	2418" MIN



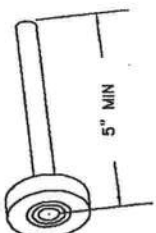
P/N 20334&5 BOTTOM BRKT.



3" P/N 20239 PUSH NUT



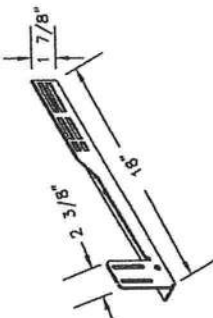
P/N 20802 SHORT STEM ROLLER



P/N 20806 LONG STEM ROLLER

.034" MIN.

20366 & 7 FLAG BRACKET



JOHN E. SCATES, P.E.
1411 LEVAY STE. 205
COROLLON, TX 75008
FLORIDA P.E. # 51737

PROFESSIONAL ENGINEER'S
SEAL PROVIDED ONLY FOR
VERIFICATION OF WINDLOAD
CONSTRUCTION DETAILS

										Mid-America Door Company										FORM DTD, OK									
DESCRIPTION										WINDLOAD SERIES HARDWARE																			
TITLE										WINDLOAD CONSTRUCTION DETAILS										DWG. NO. RCWL-0002									
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NOTES: THIS DRAWING AND / OR TECHNICAL INFORMATION ON THIS SHEET IS THE PROPERTY OF MID-AMERICA DOOR COMPANY AND IS LOANED IN CONFIDENCE FOR ENGINEERING AND MUTUAL ASSISTANCE PURPOSES ONLY, AND MAY NOT BE REPRODUCED OR USED TO MANUFACTURE ANYTHING DISCLOSED HEREON WITHOUT THE EXPRESSED PERMISSION OF MID-AMERICA DOOR COMPANY.

Shingles



The Florida Department of Community Affairs Building Code Information System

- SITE NAVIGATION
 - Home
 - Florida Building Code
 - Manufact. Buildings
 - Prototype Building
 - Surcharges
 - Training
 - Product Approval
 - Licensed Search
 - Mailing List
 - Florida Building Commission

PRODUCT APPROVAL

Product Search

- Overview
- Product Search
- Organization Search
- Product Application

User: Andrew Davis - Not Associated with Organization -

Need Help ?

Product Manufacturer:

Category:

Subcategory:

Application/Seq #:
(### or ###.##)

Application Status:

Evaluation Method:

Order by: ☒ Manufacturer ☐ Category ☐ Subcategory
☐ App / Seq # ☐ Status ☐ Evaluation Method

Organization Search

To edit an application that is NOT YET APPROVED, search for the Application/Seq # and click on the link under "Category".

Revising APPROVED (only) Applications: Click the "Revise Approved Application" button below.

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App/Seq #	Manufacturer	Category	Subcategory	Validation Entity/Validator	Status
FL586	Elk Corporation	Roofing	Asphalt Shingles		Approved
FL728	Elk Corporation	Roofing	Asphalt Shingles	Miami-Dade BCCO - VAL (305) 375-2901	Approved
FL1476	Elk Corporation	Roofing	Asphalt Shingles	Miami-Dade BCCO - VAL (305) 375-2901	Approved

12/21/2004

December 21, 2001

Dear Elk Distributor:

First, thanks for your tremendous support of our "A Whole Different Animal™" program introduced this past July. Our strategy was to introduce new products and enhancements that allowed us to be competitive and maintain our reputation for producing quality products. With the recent warranty changes in the market, we find that we need to make adjustments in order for you to maintain a competitive posture with Elk products.

Elk Warranties Effective January 02, 2002*

Prestique® Plus High Definition® 50 (previously P40)	- 50 years, 110 mph limited wind warranty**
Prestique I High Definition 40 (previously P35)	- 40 years, 90 mph limited wind warranty**
Prestique High Definition 30	- 30 years, 80 mph limited wind warranty
Raised Profile™ (previously P25)	- 30 years, 70 mph limited wind warranty

(**Requires Elk Starter and 6 nails or warranty is 80 mph.)

Due to the rapidity of these changes, it will be impossible to have proper wrappers, literature and warranties prepared to meet our January 02 effective date. The changes made for our *Whole Different Animal* program took 4-5 months to put in place. Unfortunately, there will be some confusion in the marketplace until all changes can be made. We will minimize this as quickly as possible through stickering and preparation of marketing tools that can be used by your employees and customers.

Sincerely,

Curt A. Barker
Vice President, Sales & Marketing
Elk Corporation of America

* Except as provided in this table, the limited warranty remains the same.



March 6, 2002

Subject: Elk Product Approval Information

All Prestique® and Capstone® products manufactured in Tuscaloosa, AL are certified under the Miami – Dade County Building Code Office (BCCO). These products also meet the requirements for the Florida Building Code since they are MD approved. The following test protocols must be passed by each of the products in order for MD product certification:

ASTM D3462

PA 100 (110 mph uplift and wind driven rain resistance)

PA 107 (Modified ASTM D3161 - 110 mph wind uplift resistance)

The nailing patterns that were used during the PA 100 and PA 107 wind test protocols for the Prestique and Capstone products are listed below. Also listed below are the Miami – Dade Notice of Acceptance Numbers (NOA).

Raised Profile, Prestique High Definition, Prestique 25, or Prestique 30 –

PA 100 = 4 nails

PA 107 = 5 nails

MD NOA# = 01-1226.04

Prestique I 35 or Prestique I* –

PA 100 = 4 nails

PA 107 = 5 nails

MD NOA# = 01-1226.05

Prestique Plus or Prestique Gallery Collection* –

PA 100 = 4 nails

PA 107 = 4 nails

MD NOA# = 01-1226.03

Capstone*

PA 100 = 4 Nails

PA 107 = 4 Nails

MD NOA# = 01-0523.01

* As per the Elk Limited Warranty, six nails are required for the Elk high wind warranty.

If there are any questions please contact:

Mike Reed – Technical Manager
(205) 342-0287

or

Daniel DeJarnette – QA Engineer
(205) 342-0298

FL2143	Elk Corporation	Roofing	Asphalt Shingles	PRI Asphalt Technologies, Inc. (813) 621-5777	Approved
FL3453	Elk Corporation	Roofing	Underlayments		Pending FBC Approval
FL3461	Elk Corporation	Roofing	Underlayments	PRI Asphalt Technologies, Inc. (813) 621-5777	Approved

Page:

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New Product



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my

MI HOME PRODUCTS - PRIME ALUMINUM WINDOWS - INSTALLATION INSTRUCTIONS FOR "NAIL FIN" PRODUCTS

MI Home Products appreciates your recent purchase of a maintenance free prime window, which will not rust, rot, mildew, or warp. This is a quality product that left our factory in good condition - proper handling and installation are just as important as good design and workmanship. Please follow these recommendations to allow this product to complete its function.

1. Handle units one at a time in the closed and locked position and take care not to scratch frame or glass or to bend the nailing fin.
2. Set unit plumb and square into opening and make sure that there is $3/16" \pm 1/16"$ clearance around the frame. Fasten unit into opening in the closed and locked position, making sure that fasteners are screwed in straight in order to avoid twisting or bowing of the frame. Make sure that sill is straight and level. Check operation of unit before any and all fasteners are set.
3. Use # 8 sheet metal or wood screws with a minimum of 1" penetration into the framing (stud). Place first screws (two at each corner) 3" from end of fin. For positive and negative DP's (design pressures) up to 35, do not exceed 24" spacing of additional screws. For DP's from 35.1 to 50, do not exceed 18". Install load bearing shim adjacent to each anchor. Use shim where space exceeds $1/16"$.
4. Flash over head and caulk outside perimeter in accordance with code requirements and good installation practices.
5. Fill voids between frame and construction with loose batten type insulation or non-expanding aerosol foam specifically formulated for windows and doors to eliminate drafts. The use of expanding aerosol type insulating foam, which can bow the frame, waives all stated warranties.
6. Remove plaster, mortar, paint and any other debris that may have collected on the unit and make sure that sash/vent tracks and interlocks are also clear. Do not use abrasives, solvents, ammonia, vinegar, alkaline, or acid solutions for clean-up, especially with insulated glass units as their use could cause chemical breakdown of the glass seal. Take care not to scratch glass; scratches severely weaken glass and it could eventually break from thermal expansion and contraction. Clean units with water and mild detergent as you would your automobile.

- CAUTION -

MI Home Products or its representatives are unable to control and cannot assume responsibility for the selection and placement of their products in a building or structure in a manner required by laws, statute and/or building codes. The purchaser is solely responsible for knowledge of and adherence to the same. MI Home Products window products are not provided with safety glazing unless specifically ordered with such. Many laws and codes require safety glazing near doors, bathtubs, and shower enclosures. Also be aware of emergency egress code requirements.

Corporate Headquarters:
650 West Market St.
Gratz, PA 17030-0370
(717) 365-3300

CENTRAL FLORIDA B.O.A.F.
MANUFACTURER NAME:
BETTER BUILT / MI HOMES



MI HOME PRODUCTS / BETTER-BILT**V43****VERTICAL MULLION DESIGN LOAD CAPACITIES**

CA980562-A

FOR ALUMINUM V43 TUBE MULLION (DIE # 5766; APROX. SIZE: 1 5/8" X 2 1/8" X 1/8")
WHEN USED TO MULL FIN FRAME WINDOWS

07-Feb-2002

98-1012

WDW. WTH. > MULL SPAN V	24.000	30.000	36.000	37.000	42.000	48.000	53.125
36.000	240.0	218.2	213.3	213.3	213.3	213.3	213.3
42.000	192.0	171.4	160.0	160.0	157.4	157.4	157.4
48.000	160.0	139.1	128.0	126.3	121.5	120.0	120.0
50.625	150.0	129.7	117.1	115.7	111.6	107.9	107.9
54.000	137.1	118.5	106.7	105.5	100.0	96.0	95.0
60.000	110.0	93.8	83.8	82.5	77.3	73.3	71.3
63.000	93.9	79.8	70.9	69.8	65.1	61.4	59.4
72.000	61.1	51.4	45.2	44.4	41.1	38.2	36.4

NOTES:

- * CHART APPLIES ONLY TUBE MULLION #5766 WHEN USED TO VERTICAL MULL SERIES 542/05X PW AND / OR SERIES 700/704/740/744 SH WINDOWS
- * READ WINDOW WIDTH AND MULL SPAN IN INCHES
- * WINDOW WIDTH DIMENSIONS REPRESENT THE WIDTH OF EACH WINDOW IN A SINGLE OPENING, NOT THE OVERALL WIDTH OF THE OPENING.
- * DESIGN PRESSURE VALUES ARE NOT LIMITED TO TWO WINDOWS IN A SINGLE OPENING, AND APPLY WHERE ANY NUMBER OF MULLIONS ARE EMPLOYED IN A SINGLE OPENING.
- * DESIGN PRESSURE VALUES ON THIS CHART ARE IN PSF.
- * $D_{max} = L/175$
- * MULLION MUST BE ANCHORED TO SUBSTRATE WITH CLIP NUMBER SPCT5705 USING TWO #10 X 1-1/2 SCREWS IN WOOD OR TWO 3/16 IN DIA. X 1-1/2 RAWL TAPPERS OR APPROVED EQUAL IN MASONRY OR CONCRETE.

PREPARED BY:

PRODUCT & APPLICATION ENGINEERING, INC.
 250 INTERNATIONAL PARKWAY
 SUITE 250
 HEATHROW, FLORIDA 32746
 PHONE 407 805-0365 / FAX 407 805-0366

CENTRAL FLORIDA B.O.A.E.

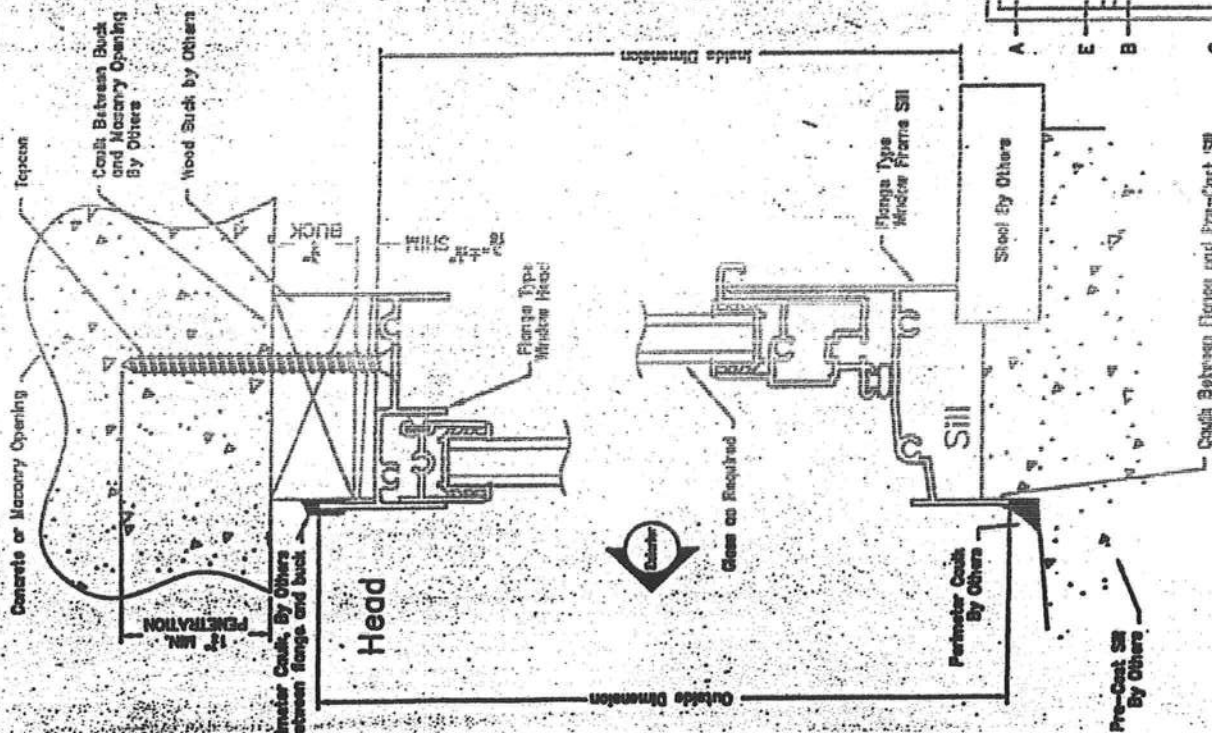
MANUFACTURER NAME

BETTER BILT / MI HOME

MASTER FILE # 7



1. Shim as required at each installation anchor as shown with load bearing shims.
2. Anchor must be of sufficient length to provide $1\frac{1}{2}$ " min. embedment into masonry or concrete.
3. Caulk between window flange and buck.
4. Caulk full perimeter of window.
5. If exact window size is not given, use trimer quantity for next larger window in chart.
6. Glass thickness will vary with window size and design load, and must comply with ASTM E330.
7. Letter designations on the trimer location chart indicate where anchors are to be installed using the elevation as a key.
8. All factory applied hardware designated for "tapcon" should be filed with #8 screws of sufficient lb. to provide $5\frac{1}{2}$ " min. embedment into wood block.



CODE	WINDOW SIZE	ANCHOR	LOCATION
12	12 1/4 x 12 1/4	12 1/4 x 12 1/4	A, C
13	12 1/4 x 17 3/8	12 1/4 x 17 3/8	A, C
14	12 1/4 x 18 5/8	12 1/4 x 18 5/8	A, C
15	12 1/4 x 21 1/4	12 1/4 x 21 1/4	A, C
16	12 1/4 x 21 1/4	12 1/4 x 21 1/4	A, C
17	12 1/4 x 25 1/4	12 1/4 x 25 1/4	A, C
18	12 1/4 x 25 1/4	12 1/4 x 25 1/4	A, C
19	12 1/4 x 25 1/4	12 1/4 x 25 1/4	A, C
20	12 1/4 x 25 1/4	12 1/4 x 25 1/4	A, C
21	12 1/4 x 25 1/4	12 1/4 x 25 1/4	A, C
22	12 1/4 x 25 1/4	12 1/4 x 25 1/4	A, C
23	12 1/4 x 25 1/4	12 1/4 x 25 1/4	A, C
24	12 1/4 x 25 1/4	12 1/4 x 25 1/4	A, C
25	12 1/4 x 25 1/4	12 1/4 x 25 1/4	A, C
26	12 1/4 x 25 1/4	12 1/4 x 25 1/4	A, C
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41	12 1/4 x 25 1/4	12 1/4 x 25 1/4	A, C
42	12 1/4 x 25 1/4	12 1/4 x 25 1/4	A, C
43	12 1/4 x 25 1/4	12 1/4 x 25 1/4	A, C
44	12 1/4 x 25 1/4	12 1/4 x 25 1/4	A, C
45	12 1/4 x 25 1/4	12 1/4 x 25 1/4	A, C
46	12 1/4 x 25 1/4	12 1/4 x 25 1/4	A, C
47	12 1/4 x 25 1/4	12 1/4 x 25 1/4	A, C
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67	12 1/4 x 25 1/4	12 1/4 x 25 1/4	A, C
68	12 1/4 x 25 1/4	12 1/4 x 25 1/4	A, C
69	12 1/4 x 25 1/4	12 1/4 x 25 1/4	A, C
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71	12 1/4 x 25 1/4	12 1/4 x 25 1/4	A, C
72	12 1/4 x 25 1/4	12 1/4 x 25 1/4	A, C
73	12 1/4 x 25 1/4	12 1/4 x 25 1/4	A, C
74	12 1/4 x 25 1/4	12 1/4 x 25 1/4	A, C
75	12 1/4 x 25 1/4	12 1/4 x 25 1/4	A, C
76	12 1/4 x 25 1/4	12 1/4 x 25 1/4	A, C
77	12 1/4 x 25 1/4	12 1/4 x 25 1/4	A, C
78	12 1/4 x 25 1/4	12 1/4 x 25 1/4	A, C
79	12 1/4 x 25 1/4	12 1/4 x 25 1/4	A, C
80	12 1/4 x 25 1/4	12 1/4 x 25 1/4	A, C
81	12 1/4 x 25 1/4	12 1/4 x 25 1/4	A, C
82	12 1/4 x 25 1/4	12 1/4 x 25 1/4	A, C
83	12 1/4 x 25 1/4	12 1/4 x 25 1/4	A, C
84	12 1/4 x 25 1/4	12 1/4 x 25 1/4	A, C
85	12 1/4 x 25 1/4	12 1/4 x 25 1/4	A, C
86	12 1/4 x 25 1/4	12 1/4 x 25 1/4	A, C
87	12 1/4 x 25 1/4	12 1/4 x 25 1/4	A, C
88	12 1/4 x 25 1/4	12 1/4 x 25 1/4	A, C
89	12 1/4 x 25 1/4	12 1/4 x 25 1/4	A, C
90	12 1/4 x 25 1/4	12 1/4 x 25 1/4	A, C
91	12 1/4 x 25 1/4	12 1/4 x 25 1/4	A, C
92	12 1/4 x 25 1/4	12 1/4 x 25 1/4	A, C
93	12 1/4 x 25 1/4	12 1/4 x 25 1/4	A, C
94	12 1/4 x 25 1/4	12 1/4 x 25 1/4	A, C
95	12 1/4 x 25 1/4	12 1/4 x 25 1/4	A, C
96	12 1/4 x 25 1/4	12 1/4 x 25 1/4	A, C
97	12 1/4 x 25 1/4	12 1/4 x 25 1/4	A, C
98	12 1/4 x 25 1/4	12 1/4 x 25 1/4	A, C
99	12 1/4 x 25 1/4	12 1/4 x 25 1/4	A, C
100	12 1/4 x 25 1/4	12 1/4 x 25 1/4	A, C

"TAPCON" TYPE ANCHORED ALUMINUM SIF
#8 STEEL ANCHOR, 1/4" DIA. x 1/2" LONG
Design Pressure Values Refer to Table 1.1

CENTRAL WORKS, INC.
MANUFACTURED BY
1500 S. 10th St., Phoenix, AZ 85004
(602) 944-1111

INSTALLATION INSTRUCTIONS
& FASTENER SCHEDULE

740/744 SINGLE HUNG

EXTERIOR ELEVATION

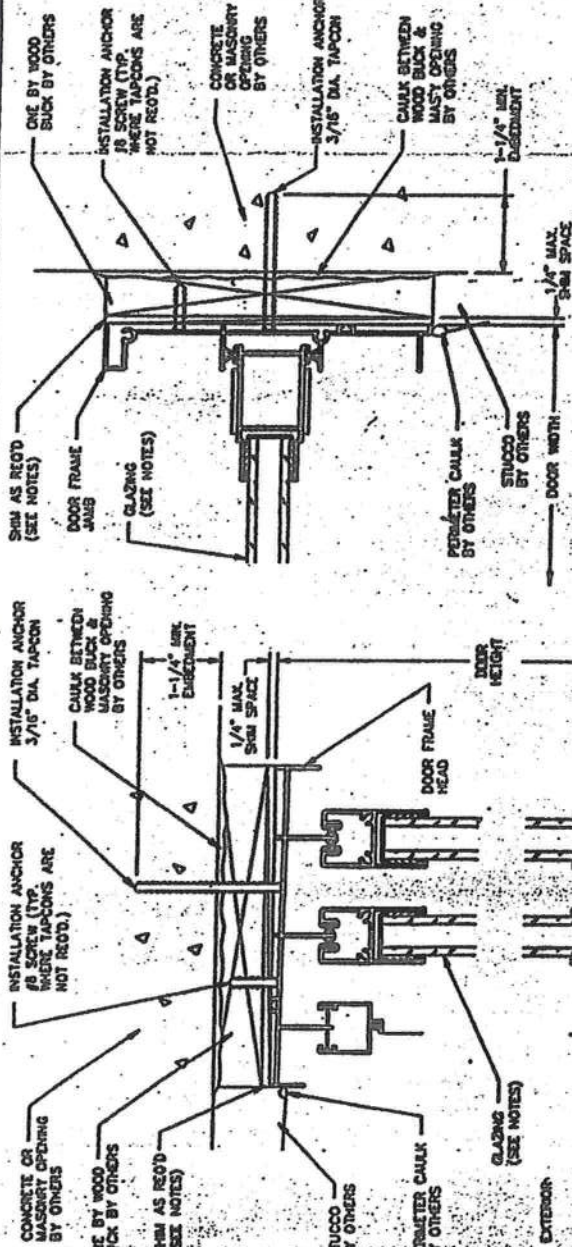
Caulk Between Flange and Pre-Cast Sill

Pre-Cast Sill By Others

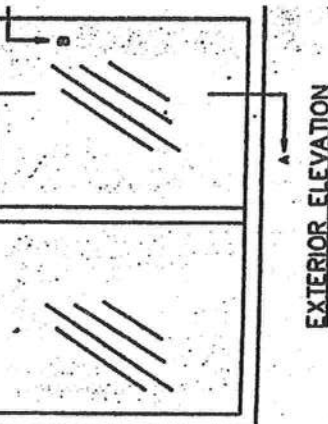
INST 7

TAPCON INSTALLATION CHART

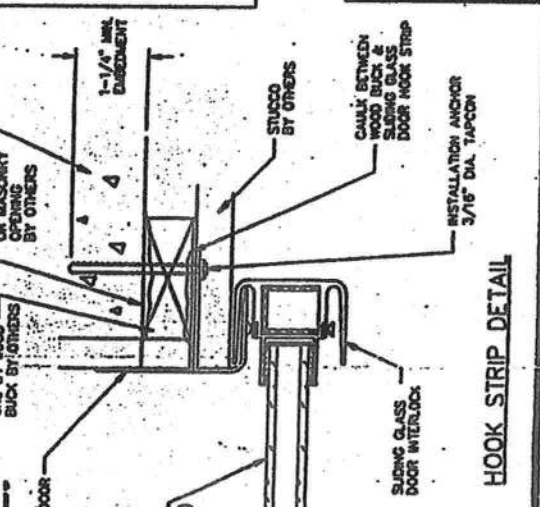
CALL SIZE	DOOR SIZE	LOCATION IN HEAD & SILL											
		UP TO 45 PSF DP	45 PSF TO 60 PSF DP	60 PSF TO 70 PSF DP	70 PSF TO 80 PSF DP	80 PSF TO 90 PSF DP	90 PSF TO 100 PSF DP	100 PSF TO 110 PSF DP	110 PSF TO 120 PSF DP	120 PSF TO 130 PSF DP	130 PSF TO 140 PSF DP	140 PSF TO 150 PSF DP	150 PSF TO 160 PSF DP
5088 X 80, 84, 88, 92, 96, 100, 104, 108, 112, 116, 120, 124, 128, 132, 136, 140, 144, 148, 152, 156, 160	60 X 80, 72 X 80, 84 X 80, 96 X 80, 108 X 80, 120 X 80, 132 X 80, 144 X 80, 156 X 80, 168 X 80	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓
5088 X 120, 124, 128, 132, 136, 140, 144, 148, 152, 156, 160	72 X 80, 84 X 80, 96 X 80, 108 X 80, 120 X 80, 132 X 80, 144 X 80, 156 X 80, 168 X 80	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓
5088 X 160, 164, 168, 172, 176, 180, 184, 188, 192, 196, 200	120 X 80, 132 X 80, 144 X 80, 156 X 80, 168 X 80, 180 X 80, 192 X 80, 204 X 80	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓
5088 X 200, 204, 208, 212, 216, 220, 224, 228, 232, 236, 240	160 X 80, 172 X 80, 184 X 80, 196 X 80, 208 X 80, 220 X 80, 232 X 80, 244 X 80	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓
5088 X 240, 244, 248, 252, 256, 260, 264, 268, 272, 276, 280	200 X 80, 212 X 80, 224 X 80, 236 X 80, 248 X 80, 260 X 80, 272 X 80, 284 X 80	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓
5088 X 280, 284, 288, 292, 296, 300, 304, 308, 312, 316, 320	240 X 80, 252 X 80, 264 X 80, 276 X 80, 288 X 80, 300 X 80, 312 X 80, 324 X 80	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓
5088 X 320, 324, 328, 332, 336, 340, 344, 348, 352, 356, 360	280 X 80, 292 X 80, 304 X 80, 316 X 80, 328 X 80, 340 X 80, 352 X 80, 364 X 80	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓
5088 X 360, 364, 368, 372, 376, 380, 384, 388, 392, 396, 400	320 X 80, 332 X 80, 344 X 80, 356 X 80, 368 X 80, 380 X 80, 392 X 80, 404 X 80	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓
5088 X 400, 404, 408, 412, 416, 420, 424, 428, 432, 436, 440	360 X 80, 372 X 80, 384 X 80, 396 X 80, 408 X 80, 420 X 80, 432 X 80, 444 X 80	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓
5088 X 440, 444, 448, 452, 456, 460, 464, 468, 472, 476, 480	400 X 80, 412 X 80, 424 X 80, 436 X 80, 448 X 80, 460 X 80, 472 X 80, 484 X 80	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓
5088 X 480, 484, 488, 492, 496, 500, 504, 508, 512, 516, 520	440 X 80, 452 X 80, 464 X 80, 476 X 80, 488 X 80, 500 X 80, 512 X 80, 524 X 80	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓
5088 X 520, 524, 528, 532, 536, 540, 544, 548, 552, 556, 560	480 X 80, 492 X 80, 504 X 80, 516 X 80, 528 X 80, 540 X 80, 552 X 80, 564 X 80	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓
5088 X 560, 564, 568, 572, 576, 580, 584, 588, 592, 596, 600	520 X 80, 532 X 80, 544 X 80, 556 X 80, 568 X 80, 580 X 80, 592 X 80, 604 X 80	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓
5088 X 600, 604, 608, 612, 616, 620, 624, 628, 632, 636, 640	560 X 80, 572 X 80, 584 X 80, 596 X 80, 608 X 80, 620 X 80, 632 X 80, 644 X 80	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓
5088 X 640, 644, 648, 652, 656, 660, 664, 668, 672, 676, 680	600 X 80, 612 X 80, 624 X 80, 636 X 80, 648 X 80, 660 X 80, 672 X 80, 684 X 80	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓
5088 X 680, 684, 688, 692, 696, 700, 704, 708, 712, 716, 720	640 X 80, 652 X 80, 664 X 80, 676 X 80, 688 X 80, 700 X 80, 712 X 80, 724 X 80	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓
5088 X 720, 724, 728, 732, 736, 740, 744, 748, 752, 756, 760	680 X 80, 692 X 80, 704 X 80, 716 X 80, 728 X 80, 740 X 80, 752 X 80, 764 X 80	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓
5088 X 760, 764, 768, 772, 776, 780, 784, 788, 792, 796, 800	720 X 80, 732 X 80, 744 X 80, 756 X 80, 768 X 80, 780 X 80, 792 X 80, 804 X 80	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓
5088 X 800, 804, 808, 812, 816, 820, 824, 828, 832, 836, 840	760 X 80, 772 X 80, 784 X 80, 796 X 80, 808 X 80, 820 X 80, 832 X 80, 844 X 80	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓
5088 X 840, 844, 848, 852, 856, 860, 864, 868, 872, 876, 880	800 X 80, 812 X 80, 824 X 80, 836 X 80, 848 X 80, 860 X 80, 872 X 80, 884 X 80	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓
5088 X 880, 884, 888, 892, 896, 900, 904, 908, 912, 916, 920	840 X 80, 852 X 80, 864 X 80, 876 X 80, 888 X 80, 900 X 80, 912 X 80, 924 X 80	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓
5088 X 920, 924, 928, 932, 936, 940, 944, 948, 952, 956, 960	880 X 80, 892 X 80, 904 X 80, 916 X 80, 928 X 80, 940 X 80, 952 X 80, 964 X 80	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓
5088 X 960, 964, 968, 972, 976, 980, 984, 988, 992, 996, 1000	920 X 80, 932 X 80, 944 X 80, 956 X 80, 968 X 80, 980 X 80, 992 X 80, 1004 X 80	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓



CENTRAL FLORIDA B.O.A.F.
MANUFACTURED NAME:
BETTER BILT / MI HOMES
MASTER FILE #

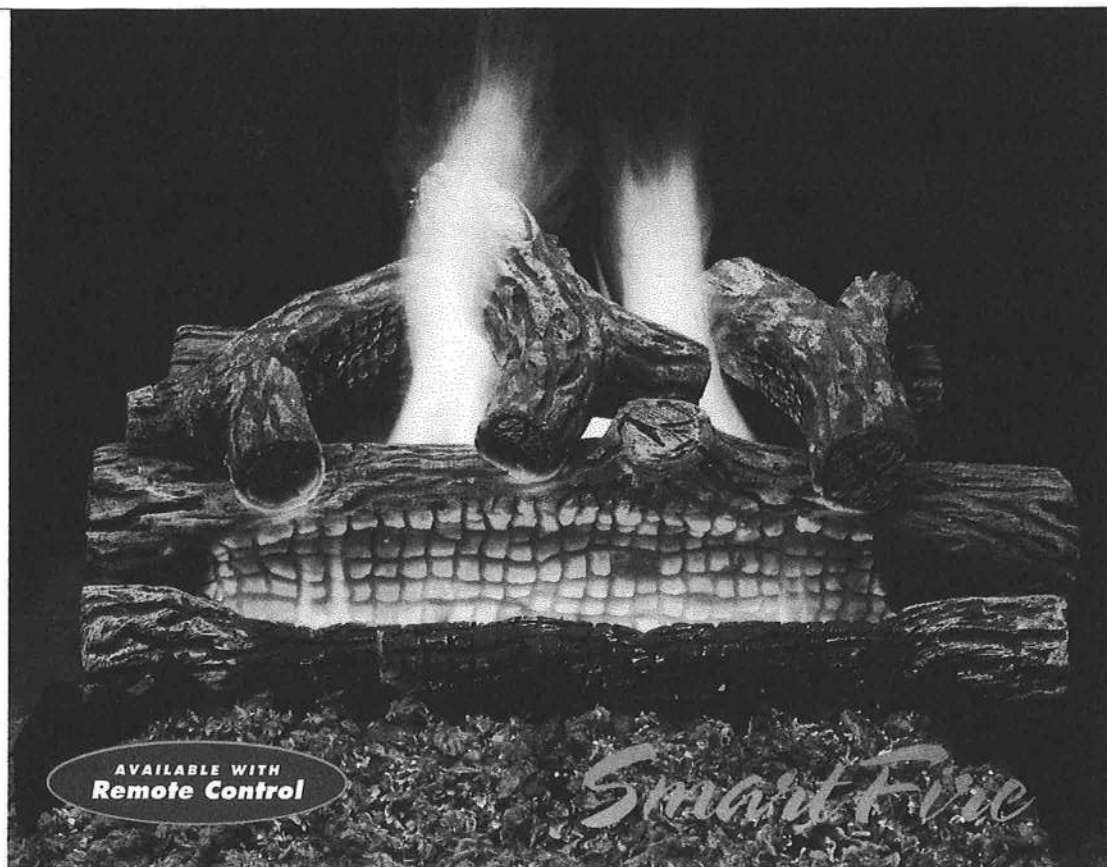


MI HOME PRODUCTS
GRATZ, PA.
SLIDING GLASS D
INSTALLATION WITH TAPCONS
DOOR: MANUAL MARTINEZ
DOOR: CIVIL
FL REC NO: 47182



- 1) READ MATERIALS ALUMINUM MUST BE USED
- 2) TAPCON TYPE INSTALLATION ANCHORS MUST BE OF SUFFICIENT LENGTH TO ACHIEVE MIN. EMBEDMENT OF 1-1/4" INTO MASONRY OR CONCRETE
- 3) ALTERNATE TAPCON INSTALLATION ANCHORS FRONT TO BACK IN ALL FRAME MEMBERS
- 4) USE LATER CAULK FOR PERIMETER SEAL AROUND EXTERIOR OF DOOR
- 5) APPROXIMATE DOOR SIZE IS NOT LISTED IN ANCHOR CHART, USE ANCHOR QUANTITY LISTED WITH NEXT LARGER SIZE FOR THE DOOR
- 6) FOR POCKET DOOR SIZES USE GLASS BYPASS DOOR SIZE & CONFIGURATION TO PROCEED OPENING
- 7) REGULAR GLASS SIZES USE GLASS BYPASS DOOR SIZE & CONFIGURATION TO PROCEED OPENING
- 8) ALL FACTORY APPLIED HOLES NOT DESIGNED FOR TAPCON ANCHORS SHOULD BE FILLED WITH #8 STEEL REINFORCING
- 9) ALL FACTORY APPLIED HOLES NOT DESIGNED FOR TAPCON ANCHORS SHOULD BE FILLED WITH #8 STEEL REINFORCING
- 10) ALL FACTORY APPLIED HOLES NOT DESIGNED FOR TAPCON ANCHORS SHOULD BE FILLED WITH #8 STEEL REINFORCING
- 11) ALL FACTORY APPLIED HOLES NOT DESIGNED FOR TAPCON ANCHORS SHOULD BE FILLED WITH #8 STEEL REINFORCING
- 12) ALL FACTORY APPLIED HOLES NOT DESIGNED FOR TAPCON ANCHORS SHOULD BE FILLED WITH #8 STEEL REINFORCING
- 13) ALL FACTORY APPLIED HOLES NOT DESIGNED FOR TAPCON ANCHORS SHOULD BE FILLED WITH #8 STEEL REINFORCING
- 14) ALL FACTORY APPLIED HOLES NOT DESIGNED FOR TAPCON ANCHORS SHOULD BE FILLED WITH #8 STEEL REINFORCING
- 15) ALL FACTORY APPLIED HOLES NOT DESIGNED FOR TAPCON ANCHORS SHOULD BE FILLED WITH #8 STEEL REINFORCING
- 16) ALL FACTORY APPLIED HOLES NOT DESIGNED FOR TAPCON ANCHORS SHOULD BE FILLED WITH #8 STEEL REINFORCING
- 17) ALL FACTORY APPLIED HOLES NOT DESIGNED FOR TAPCON ANCHORS SHOULD BE FILLED WITH #8 STEEL REINFORCING
- 18) ALL FACTORY APPLIED HOLES NOT DESIGNED FOR TAPCON ANCHORS SHOULD BE FILLED WITH #8 STEEL REINFORCING
- 19) ALL FACTORY APPLIED HOLES NOT DESIGNED FOR TAPCON ANCHORS SHOULD BE FILLED WITH #8 STEEL REINFORCING
- 20) ALL FACTORY APPLIED HOLES NOT DESIGNED FOR TAPCON ANCHORS SHOULD BE FILLED WITH #8 STEEL REINFORCING

MONESSEN



Vented Gas Logs That Heat.

A bold, new concept in vented log design combining radiant heat technology with true-to-life fiber ceramic logs to enhance both beauty and heat producing capability.

12,000 BTU Radiant Heat

Unlike conventional vented gas logs, where the heat goes up the chimney, **SmartFire's** unique design produces a high temperature glowing log surface...up to 1600° F...which radiates heat into the room (up to 12,000 BTU), making **SmartFire** not only an aesthetic addition to your home, but also a practical one.

3 burners for Realistic Wood Fire Look.

SmartFire's three burner system produces three types of flames (burning embers, glowing log, and dancing yellow flames) for the look and feel of a real woodburning fire. The standard four-position valve allows you to choose three different settings to suit your mood and budget — while still maintaining a constant heat output.

Low cost operation.

Due to its high efficiency burner arrangement, **SmartFire** allows you to minimize the damper opening (in adherence with local building codes), thus reducing heat loss up the chimney. In addition, **SmartFire's** cost of operation is 1/2 to 2/3 of conventional vented logs. (**SmartFire** can be operated for as little as 12¢ per hour.)

Safety & Convenience

Monessen's **SmartFire** enhances both safety and convenience by providing as standard a piezo ignition, safety pilot, four-position valve, and gas regulator — all factory installed and tested — and approved by AGA. **SmartFire** is available with optional remote control or a wall switch.

BTU Input

Cost of Operation, per hour

Radiant Heat Output BTU per Hour

Gas Fuel Type	Radiant Log Only (Low)	Radiant Log + Front Embers (Medium)	Radiant Log + Front Embers + Rear Burner (High)	Radiant Log Only (Low)	Radiant Log + Front Embers (Medium)	Radiant Log + Front Embers + Rear Burner (High)	Radiant Log Only (Low)	Radiant Log + Front Embers (Medium)	Radiant Log + Front Embers + Rear Burner (High)
18" Natural	21,000	32,000	48,000	12¢	18¢	28¢	7,000	9,000	9,000
18" Propane	19,000	29,000	43,000	21¢	32¢	48¢	7,000	9,000	9,000
24" Natural	24,000	37,000	55,000	14¢	32¢	33¢	9,000	12,000	12,000
24" Propane	22,000	33,000	50,000	25¢	37¢	56¢	9,000	12,000	12,000

Natural Gas based on National Average of \$0.59 per 100,000 BTU

Propane Gas based on National Average of \$1.12 per 100,000 BTU

Model Availability

Gas Fuel Type	Manual Control	Millivolt Control
18" Natural	DV18NMP	DV18NVP
18" Propane	DV18PMP	DV18PVP
24" Natural	DV24NMP	DV24NVP
24" Propane	DV24PMP	DV24PVP

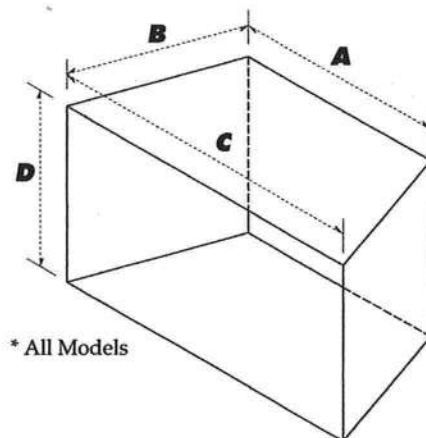
Features

- Natural Gas Units come with 6 logs
- Propane Gas Units come with 5 logs
- Available with 4 Position Manual Control
- Remote Operation with Millivolt Control for On/Off Only

Use with Optional Hand-held Remote with Receiver or Optional Wall Switch with twenty feet of Wiring.

Minimum Firebox Size

	A	B	C	D
DV18*	19"	14"	29"	16"
DV24*	23"	14"	33"	16"



* All Models

Warning:

This unit complies with ANSI Z21.60-1991 "Decorative Gas Appliances" for solid-fuel burning fireplaces. This gas log set is to be installed only in solid-fuel burning fireplace with a working flue constructed of non combustible material.

Monessen recommends that all installations be performed by qualified service technicians.

Always follow installation and operation instructions to ensure safety at all times.

The damper stop provided with this unit must be installed to prevent full closure of the fireplace damper blade and provide a minimum flue opening.

Check local codes before installation to ensure compliance.

Monessen recommends that a fireplace screen be used during operation of this log set. Always ensure the screen has adequate spaces to allow for passage of combustion air to the gas log set.

Keep small children and animals away from the open flame of the gas log set at all times.

There may be a "warm-up" smell associated with the heating of the logs, and the flame surrounding the radiant log may be orange in color. The odor should diminish in 2-3 hours of operation with the burner at its highest setting. The orange flame should turn to blue at the burner, with the radiant log glowing a reddish orange.

Distributed by:

MONESSEN
HEARTH SYSTEMS

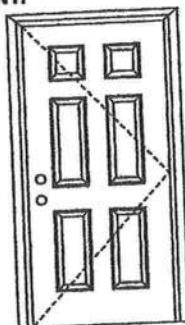
793 Lemons Mill Road • Georgetown, Kentucky 40324



X

Opaque Inswing Unit

COP-WL-EN4101-02

WOOD-EDGE STEEL DOORS**APPROVED ARRANGEMENT:**

Note:
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".

Single Door
Maximum unit size = 3'0" x 6'8"

Design Pressure
+76.0/-76.0

Limited water unless special threshold design is used.

Large Missile Impact Resistance

Hurricane protective system (shutters) is NOT REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.



Test Data Review Certificate #3026447C and COP/Test Report Validation Matrix #3026447C-001 provides additional information - available from the ITS/WH website (www.itswh.com), the Masonite website (www.masonite.com) or the Masonite technical center.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed - see MAD-WL-MA0001-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed - see MID-WL-MA0001-02.

APPROVED DOOR STYLES:

Flush



Arch Top 3-panel



6-panel



New England 4-panel



Eyebrow 4-panel



8-panel



9-panel



5-panel



5-panel with scroll



Eyebrow 5-panel



Eyebrow 5-panel with scroll

Entergy
Entry Systems

June 17, 2002

Our continuing program of product improvement makes specifications, design and product

PREMDOR Collection
Premium Quality Doors



Exclusively from

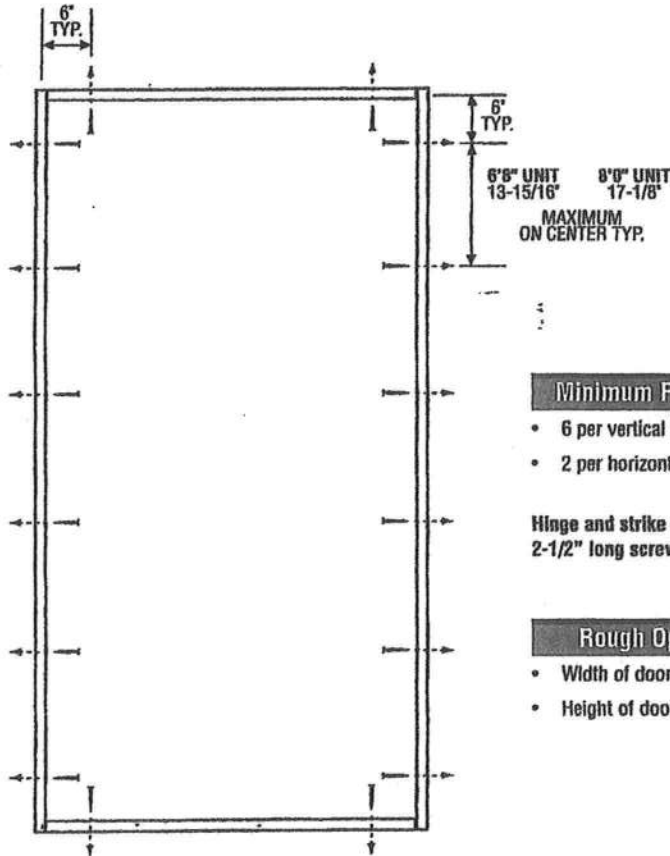
Masonite

Masonite International Corporation

X
Unit

MID-WL-MA0001-02

SINGLE DOOR



Minimum Fastener Count

- 6 per vertical framing member
- 2 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Rough Opening (RO)

- Width of door unit plus 1/2"
- Height of door unit plus 1/4"

Wernick Hervey Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITW/WH website (www.itwsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- **UNITS COVERED BY COP DOCUMENT 3146, 3166, 3241*, 3246, 3261* or 3266**
Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel - (1) at top and (1) at bottom.

*Based on required Design Pressure - see COP sheet for details.

Notes:

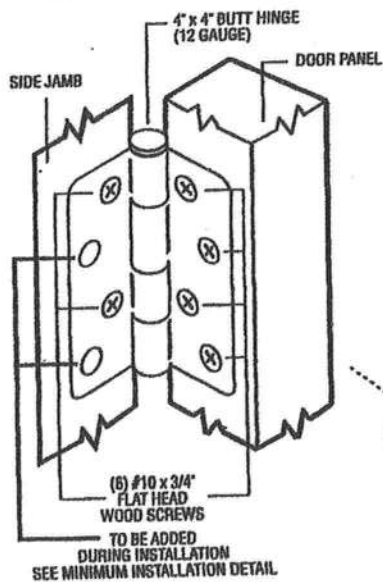
1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons.
2. The wood screw single shear design values come from Table 11.3A of ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Dade County approvals respectively, each with minimum 1-1/4" embedment.
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

X
Unit

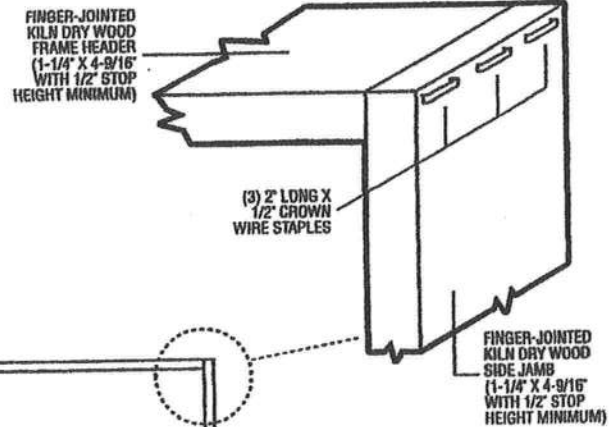
MAD-WL-MA0001-02

INSWING UNIT WITH SINGLE DOOR

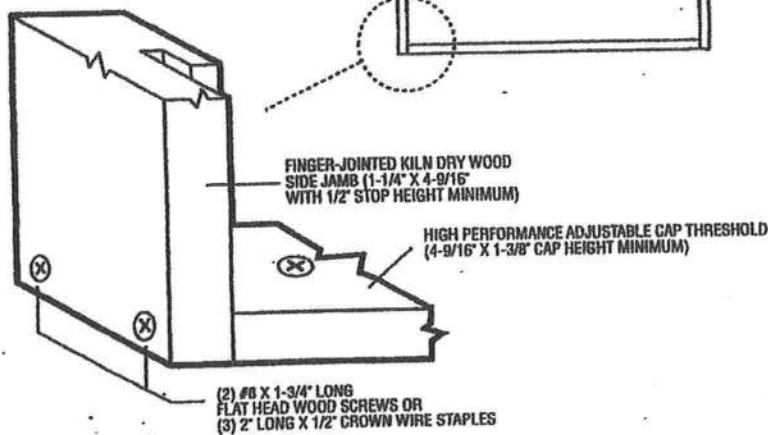
TYPICAL HINGE ATTACHMENT



TYPICAL HEADER & SIDE JAMB ATTACHMENT



TYPICAL THRESHOLD & SIDE JAMB ATTACHMENT



Warnock Hersey
W-H

Test Data Review Certificate
#3026447A; #3026447B;
#3026447C and COP/Text Report
Validation Matrix #3026447A-001,
002, 003; #3026447B-001, 002,
003; #3026447C-001, 002, 003
provides additional information -
available from the ITSAWI website
(www.itsawin.com), the Masonite
website (www.masonite.com) or
the Masonite technical center.

X

Opaque Inswing Unit

COP-WL-EN4101-02

WOOD-EDGE STEEL DOORS

CERTIFIED TEST REPORTS:

NCTL 210-1864-5, 6

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested In Accordance with Miami-Dade BCCO PA201, PA202 and PA203.

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.032" steel. Bottom end rails constructed of 0.032" steel. Interior cavity of slab filled with rigid polyurethane foam core.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH
MIAMI-DADE BCCO
PA201, PA202 & PA203

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).



State of Florida, Professional Engineer
Kurt Balthazor, P.E. - License Number 56533



Test Data Review Certificate #3025447C and COP/Test Report Validation Matrix #3026447C-001 provides additional information - available from the ITG/WH website (www.itsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

2

Entergy
Entry Systems

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

PREMIER Collection
Premium Quality Doors

Exclusively from
Masonite
Masonite International Corporation

Florida Building codes

2/01/05

Simpson strongtie

ABU44 FL474.21
H2.5A FL503.5
SPH4 FL538
H10 FL474.109
MSTA24 FL1901.57

Primed Hardi Board & Unprimed Cemplank

FL889

#15 & #30 Felt

FL

#15 ASTM D4869

#30 ASTM D4869

STRUCTURAL WIND DESIGN REPORT

Project (#07009)

1. **Name:** New House for Harvey Building and Construction, Inc. (Contractor)
2. **Location:** Fort White, FL
3. **Description:** New wood frame structure (1 story) with truss roof framing as provided by plans from Contractor.

Roof Structure

1. **Trusses:** Engineered by Truss manufacturer per ANSI/TPI 1-2002 and FBC 2004. Spacing @24"oc. Top Chords to be SYP. All trusses to be toenailed with (3) 8d at plate if used (min. 1" penetration into plate). Provide permanent bracing of trusses per Detail(s). Facenail 2x4 bracing with (2) 16d each chord location. The Contractor shall provide all temporary and permanent bracing as required for safe erection and performance of the trusses. The guidelines set forth by the TPI & WTCA Publication BCSI 1-03 "Guide to Good Practice for Handling, Installing and Bracing of Metal Plate Connected Wood Trusses" shall be a Minimum Requirement. See L-bracing requirements.
2. **Truss Connection (Hurricane Clips):** See "Roof Framing Layout" drawing(s).
3. **Conventional Roof Framing:** Use 2x8 #2 SYP @24"oc for spans greater than 12 feet up to 17 feet. Spans less than 12' use 2x6 #2 SYP. Toenail rafters with (3) 8d at each end.
4. **Roof Sheathing:** Sheathing to be **minimum 15/32" CDX** to adequately resist exterior shear and uplift forces due to nailing. Panels to be installed & facenailed according to Roof Sheathing Attachment and Nail Zone Pattern Drawing Details. Galvanized steel or aluminum metal edging (FHA drip – 26 gauge) to be nailed @4"oc. All flashing (valleys, etc.) to be same gauge and nailed at same spacing.
5. **Shingles:** Asphalt strip shingles shall have a minimum of 6 fasteners per shingle per FBC 2004. Staples not allowed

Wood Frame Exterior – 1st Story

1. **Shearwalls:** Shearwalls and required fasteners are graphically depicted in the "Shearwall Framing Layout" drawing(s) for each floor. This illustration along with the "Schedules" sheet defines: (1) the sheathing thickness & type; (2) nail size and spacing; (3) anchor bolt size and spacing; and (4) the type of holdown device at each end of the shear wall if required.
2. **Shearwall Framing:** Studs to be 2x4 SYP #2 (SPF #2) @16"oc for given ceiling height of 8 feet. For 10 foot walls use only SYP #2 studs @16"oc. See "Shearwall Framing Layout" drawing(s) and "Typical Section" detail for given size, type and grade for given wall height(s). **Top plates to be Southern Pine (SYP) only and to be overlapped minimum of 48"**. Use minimum of (22) 16d nails per overlap. Nailed plates together with 16d @18"oc with alternate pattern. **Note:** Top plates to be continuous around exterior wall framing to maintain shear transfer capacity. Contact engineer if openings break top plates. Plates will be adjusted to go around openings and be designed to resist shear loads.
3. **Shear Wall Sheathing:** 1/2" CDX sheathing will adequately resist exterior shear and uplift forces due to nailing. Panels to be installed vertical (48") and field nailed (interior studs) with 8d @12"oc.
4. **Walls Not Defined As Shearwalls:** Framing and sheathing requirements are identical to "shearwalls." Panels to be installed vertical (48") and facenailed with 8d common nails @6"oc along the edges and 12"oc along interior (field nailing) studs. Bottom plates are to be facenailed to rim joists with 16d @8"oc.
5. **Header Anchorage:** See "Shearwall Framing Layout" drawing(s).

Ron Barlow
3-7-07

Interior Shear and Bearing Walls

1. Shearwalls: Shearwalls and required fasteners are graphically depicted in the "Shearwall Framing Layout" drawing(s) for each floor. This illustration along with the "Schedules" sheet defines: (1) the sheathing thickness & type; (2) sheathing nail size and spacing; (3) facenail and toenail size and spacing for fastening bottom plate to rim joist; and (4) the type of holdown device at each end of the shear wall if required.
2. Shearwall Framing: Studs to be 2x4 #2 SYP (SPF#2) @16"oc. Check with Engineer for type, spacing & heights over maximum design loads. Top Plates (SYP) to be overlapped min. 48". Use min. of (9)16d nails per overlap. Nailed plates together with 16d @12"oc with alternate pattern. Studs to have (4)10d toenails per stud/plate connection.

Top plates are to be Southern Pine (SYP) only and to be overlapped min. 48". Use minimum of (22) 16d nails per overlap. Nailed plates together with 16d @18"oc with alternate pattern. Note: Top plates to be continuous around exterior wall framing to maintain shear transfer capacity. Contact engineer if openings break top plates. Plates will be adjusted to go around openings and be designed to resist shear loads.
3. Wall Sheathing: Panels to be installed vertical (48"). Gypsum wallboard sheathing to be used unless otherwise indicated.
4. Bottom Plate Anchorage: Use A307 anchor bolts with 3" washers at a maximum spacing of 48"oc. Install the first and last anchor bolts at least 6" from each end if a holdown device is not specified.

Gable End Framing (Gable end walls are to be constructed by following option)

1. Continuous Balloon Framing: See Attached Detail drawing(s). This option is **required for scissor or cathedral trusses** that are to be installed immediately behind the gable end wall.
2. Step Down Framing: See Attached Detail drawings for given truss type. See L-bracing requirements.

Special Wall Framing

1. Garage Door Framing: See Attached Detail drawing(s) for framing requirements for this wall section.

Fireplace Framing

1. Clearance: Maintain minimum fire separations from chimney per FBC 2004. Avoid any contact with trusses.

Porches

1. Post Columns: Use PT (SYP #2) columns at locations per "Porch Layout." Fastened as specified per Detail drawings. Use 1x trim to hide fasteners.
2. Beams: Use Multi-ply PT headers as shown per "Framing Layout" if full-length members are not available. Fastened to each post or column with fasteners specified per drawings.

Foundation

1. Concrete: Concrete to have a min. 28 day compressive strength of 3000 psi. Steel reinforcement to be #5 rebar (continuous) Grade 60 having a minimum splice lap of 25" at all locations.

See Typical Section drawing for footing type and size. Footings for interior load bearing walls are to be a minimum 16" wide by 8" deep. Interior shear walls have minimum 12" wide footers (8" depth). Both interior footings to have (2) #5 rebar continuous.

2. Slab Reinforcement Option: Owner/contractor can use fiber-reinforced slabs in lieu of welded wire fabric. The synthetic fiber reinforcement shall: (1) have lengths between 0.5 to 2 inches; (2) have a dosage amount between 0.75 to 1.5 pounds per cubic yard; and (3) comply with ASTM C1116.

Additional Notes

Design Data

1. Contractor/Owner is responsible for fabricating structure(s) according to the requirements and specifications of the Florida Building Code (FBC 2004), and local codes and regulations of the jurisdictional Building Department. Structure has been designed to satisfy the wind load requirements of FBC 2004-Chapter 16 and ASCE7-02. Contractor is responsible for verifying that modification of the existing structure(s) will pose no detriment to such structure(s) originally designed to resist wind forces per requirements of past building codes and jurisdictional Building Department(s). Information required by FBC 2004 for drawings is listed below:

BASIC WIND SPEED = 110 MPH for EXPOSURE B
IMPORTANCE FACTOR, $I=1.0$ (CATEGORY II, ASCE 7-02)
INTERNAL PRESSURE COEFFICIENT = +/- 0.18 (ENCLOSED)
WIND COMPONENT & CLADDING (CC) LOADS – 32 sf (4x8' CDX Sheet)
(+20.1, -25.83 psf) Wall, (+10.67, -35.5 psf) Roof, (-60.34 psf) Roof Overhang

2. Wood framing and fasteners to meet ANSI/AF&PA NDS-2005 requirements. Nail & screw fasteners shall be installed flush with surface of sheathing.
3. Fastener requirements: (1) All nails are Common galvanized steel; (2) all bolts (except interior anchor bolts) are to be galvanized steel including nuts and washers; and (3) all other hardware (Simpson, etc.) is to be installed according to manufacturer's specifications and recommendations. Nailing (size and number) shall satisfy Table 2304.9.1 of FBC 2004 unless otherwise indicated. **NOTE** fasteners directly exposed to the weather are to be hot-dipped galvanized steel (nails, bolts, nuts and washers). Simpson fasteners are to be installed according to the manufacturer's recommendations and specifications. Fasteners for the new CA & AQC preservative-treated and fire-retardant-treated wood shall be of hot-dipped zinc-coated galvanized steel (HDG), or stainless steel (SS). The coating weights for zinc-coated fasteners shall be in accordance with ASTM A 153M or ASTM A 641. It is recommended to use Stainless Steel (AISI types 410, 302, 304 and 305) since it has proven to be the most stable and offers the best corrosion protection for the new treated wood chemicals. **Coastal environments are required: to use 316 Stainless Steel and HDG fasteners and related hardware at all exposed areas and specified locations; and to use 316 Stainless Steel only at preservative-treated wood locations.**
4. Foundation requirements of structure may vary due to design loading and existing soil conditions. Foundations should be cast against well compacted soil having a minimum density of 90%; and is clean, free of organics and well graded (A2-A3-A4) up to a depth of at least 4 feet below the footings. Notify Engineer or Geotechnical Engineering Services if existing soil conditions fail to meet this criteria and/or if wet soil conditions or unsuitable material (clay, muck, etc., - after 4 feet below the footings) are encountered. Also, Contractor/Owner to verify design and construction loading requirements on foundation as required by Truss Manufacturer.
5. The same-thickness substitution of OSB sheathing for CDX plywood may be permitted if requested by the Owner and Contractor. This material has the same shear strength capacity and nail schedule requirements as CDX. **The installation of OSB to conform to the Manufacturer's recommendations and specifications.** CDX plywood is recommended (over other sheathing materials) because its physical properties remain stable over time, and due to the material's superior performance record during Hurricane Andrew (1992).
6. The Texture 1-11 (T1-11) siding (5/8") may be used in lieu of 1/2" CDX plywood for exterior sheathing of walls. The T1-11 siding shall satisfy the following requirements: 5-ply having a min. nominal thickness of 19/32"; grooves no wider than 1 1/2" and spaced no closer than 12"; and texturing that does not penetrate through the face veneer. **The installation of T1-11 to be conform to Manufacturer's recommendations and specifications.**
7. The use of Red Head 5/8 x 6" concrete wedge anchors can be substituted in lieu of 1/2 and 5/8" anchor bolts. A 3/4 x 6 1/4" anchor can be used for larger anchor bolts but can not exceed 6470 lbs of pullout.

Concrete Construction Notes

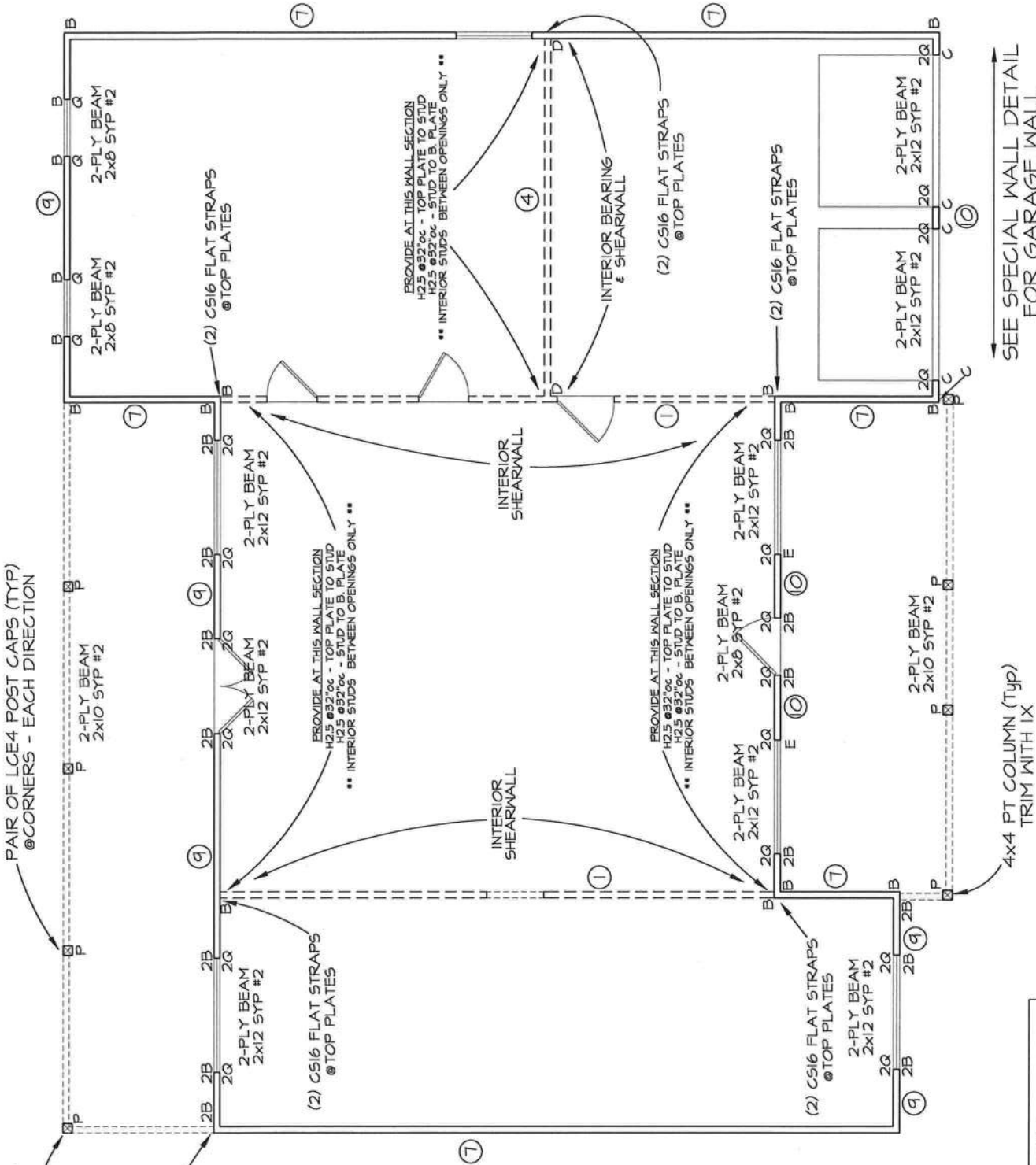
1. Concrete work shall conform to "Building Code Requirements for Reinforced Concrete" (ACI-318) and "Specifications for Structural Concrete" (ACI-301), Latest Edition.
2. All concrete shall be cured for a minimum of 7 days. If forms for vertical surfaces are removed prior to the end of the curing period, spray surfaces with liquid membrane curing compound.
3. Reinforcing steel shall conform to ASTM A615, Grade 60 ($F_y=60$ ksi). Lap continuous bars for tension lap splice per ACI-318, unless otherwise noted. Provide corner bars of same size and spacing as horizontal wall reinforcement. Cover for concrete reinforcing steel shall be in accordance with ACI-318, Paragraph 7.7.
4. Welded wire fabric (WWF) shall conform to ASTM A185. Lap sheets on mesh space and wire tie adjacent sheets together securely. Cut alternate reinforcement at control joints.
5. All slabs on grade shall have construction or control joints not to exceed 20'-0" spacing, unless otherwise noted.
6. Electrical conduit and other pipes to be embedded in structural concrete floor slabs or walls shall be placed in accordance with the requirements of ACI-318, Paragraph 6.3.

Masonry Construction Notes

1. Concrete and brick masonry work shall conform to "Building Code Requirements for Masonry Structures" (ACI 530-05/ASCE5-05) and "Specifications for Masonry Structures: (ACI 530.1-05/ASCE6-05).
2. Concrete masonry units shall be Type I and comply with "Standard Specifications for Hollow Load-Bearing Concrete Masonry Units" (ASTM C90-90).
3. The minimum net area compressive strength of Masonry (f'_m), as determined by the unit strength method, shall be 1500 psi.
4. Mortar shall conform to ASTM C270. Type N Mortar shall be used unless otherwise noted. Type S mortar shall be used with masonry in contact with earth.
5. Masonry column reinforcement shall have #2 ties in the bed joints at 8"oc, unless otherwise noted. Not required for piers. Grout for filling block cores and bond beams shall have a minimum compressive strength (f'_c) of 4000 psi at the age of 28 days.
6. Brick solid units shall comply with "Specifications for Facing Brick" (ASTM C216). Brick hollow units shall comply with "Specifications for Hollow Brick" (ASTM C652).

4x4 PT COLUMN (Typ)
TRIM WITH 1X

HUC410 (Typ)



SEE SCHEDULES

- ⑤ - Shearwall Type (Shearwall Schedule)
 - A - Fastener Type (Fastener Schedule)
- All Fasteners are Simpson unless otherwise noted.
Note Header Size @ Windows, etc.

SEE SPECIAL WALL DETAIL
FOR GARAGE WALL

SHEARWALL FRAMING LAYOUT

Nt's

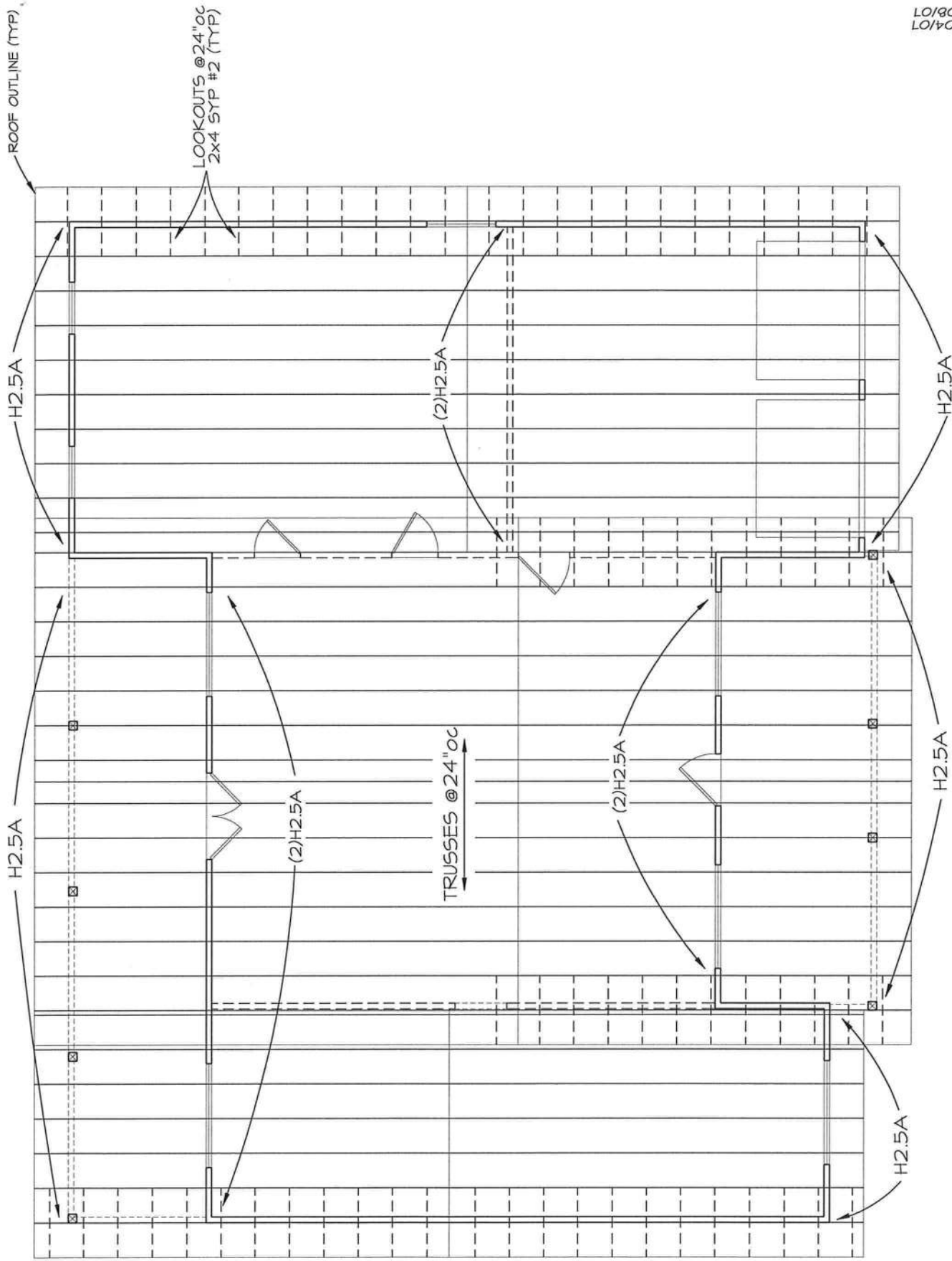
USE MINIMUM HEADER SIZE OF 2-PLY 2x6 SYP #2

JOB #07009

RON BARLOW, PE

Created: 3/10/07
Revised: 3/10/07

PROVIDE GABLE END FRAMING:
STEP-DOWN TRUSSES OR BALLOON FRAMING



- NOTES: 1) All Fasteners are Simpson.
 2) Provide Clip each Truss/Rafter End between Leaders.
 3) Install Double (2) Clips Diagonally across from Each Other to Avoid Splitting Truss.

ROOF FRAMING LAYOUT

JOB #070009

Nts

RON BARLOW, PE

Created: 3/04/01
 Revised: 3/08/01

SCHEDULES FOR SINGLE STORY CONSTRUCTION

***** SHEARWALL SCHEDULE *****
VALUES FOR SOUTHERN PINE (SYP) FRAMING

* REDUCED VALUES FOR SUBSTITUTION WITH SPRUCE-PINE-FIR (SPF).

SW#	SHEATHING (Sides)	SHEAR CAPACITY (plf)	EDGE NAILING	ANCHOR BOLTS
1	1/2" GYP. BD. (1) - INDOOR ONLY	100 (80)*	5d @ 1" oc	1/2" @ 12" oc
2	1/2" GYP. BD. (2) - INDOOR ONLY	200 (160)*	5d @ 1" oc	1/2" @ 32" oc or 5/8" @ 64" oc
3	1/2" GYP. BD. (1) - INDOOR ONLY	125 (100)*	5d @ 4" oc	1/2" @ 64" oc
4	1/2" GYP. BD. (2) - INDOOR ONLY	250 (200)*	5d @ 4" oc	1/2" @ 32" oc or 5/8" @ 56" oc
5	3/8" CDX (1) - INDOOR ONLY	220 (180)*	8d @ 6" oc	1/2" @ 32" oc or 5/8" @ 48" oc
6	3/8" CDX (2) - INDOOR ONLY	440 (360)*	8d @ 6" oc	5/8" @ 24" oc
7	1/2" CDX (1)	175 (140)*	8d @ 6" oc	1/2" @ 48" oc or 5/8" @ 64" oc
8	1/2" CDX (1)	260 (210)*	8d @ 6" oc	1/2" @ 32" oc or 5/8" @ 48" oc
9	1/2" CDX (1)	380 (310)*	8d @ 4" oc	1/2" @ 24" oc or 5/8" @ 32" oc
10	1/2" CDX (1)	490 (400)*	8d @ 3" oc	5/8" @ 24" oc
11	1/2" CDX (1)	600 (440)*	10d @ 3" oc	5/8" @ 24" oc
12	1/2" CDX (1)	770 (630)*	10d @ 2" oc	5/8" @ 16" oc
13	1/2" CDX (2)	920 (750)*	10d @ 4" oc	5/8" @ 16" oc

NOTE: FOR SW 11 & 12 - PROVIDE 3X's AT ADJOINING PANEL EDGES WITH NAILS STAGGERED.
FOR SW 13 - OFFSET PANEL JOINTS ON EACH SIDE OF WALL MINIMUM OF ONE STUD BAY.

***** FASTENER SCHEDULE *****
VALUES FOR SOUTHERN PINE (SYP) FRAMING

* REDUCED VALUES FOR SUBSTITUTION WITH SPRUCE-PINE-FIR (SPF).

#	TYPE	DESCRIPTION	UPLIFT	ADDITIONAL INFORMATION
A	STUD TIE	H2.5	415 (365)*	FASTEN TO BOTTOM PLATE & STUD WITH (5) 8D EACH
B	STUD TIE	SP4	735 (632)*	WRAP UNDER BOTTOM PLATE & NAIL (3) 10D EACH SIDE OF STUD
C	HOLDOWN	LTT20B	1750 (1542)*	1/2" ANCHOR BOLT & (10) 16d NAILED TO (2) 2X OR 4X STUDS(S)
D	HOLDOWN	HD2A	2775 (2525)*	5/8" ANCHOR BOLT & (2) 5/8" STUD BOLTS THRU (2) 2X STUDS
E	HOLDOWN	HTT16	3480 (3167)*	5/8" ANCHOR BOLT & (18) 16d NAILED TO (2) 2X OR 4X STUDS(S)
F	HOLDOWN	HD5A	3705 (3372)*	5/8" ANCHOR BOLT & (2) 3/4" STUD BOLTS THRU (2) 2X STUDS
G	HOLDOWN	HTT22	5250 (4778)*	5/8" ANCHOR BOLT & (32) 16d NAILED TO (2) 2X OR 4X STUD
H	HOLDOWN	HD6A	5510 (5014)*	1/2" ANCHOR BOLT & (2) 1/2" STUD BOLTS THRU (3) 2X STUDS
J	HOLDOWN	HD8A	7910 (7248)*	1/2" ANCHOR BOLT & (2) 1/2" STUD BOLTS THRU (3) 2X STUDS
K	HOLDOWN	HD10A	9900 (9009)*	1/2" ANCHOR BOLT & (4) 1/2" STUD BOLTS THRU (3) 2X STUDS
P	POST ANCHOR	ABU44/46/66	2200	5/8" ANCHOR BOLT & (12) 16d NAILED TO 4x4/4x6/6x6 POST
Q	FLAT STRAP**	MSTA12	760 (654)*	(10) 10d - TOTAL
R	FLAT STRAP**	MSTA18	1065 (916)*	(14) 10d - TOTAL
S	FLAT STRAP**	CS18	1270 (1156)*	(18) 10d - TOTAL
T	FLAT STRAP**	CS16	1650 (1414)*	(22) 10d - TOTAL
V	TWIST STRAP**	LTS12-20	775 (665)*	(12) 10d - TOTAL
W	TWIST STRAP**	MTS12-20	1000 (860)*	(14) 10d - TOTAL

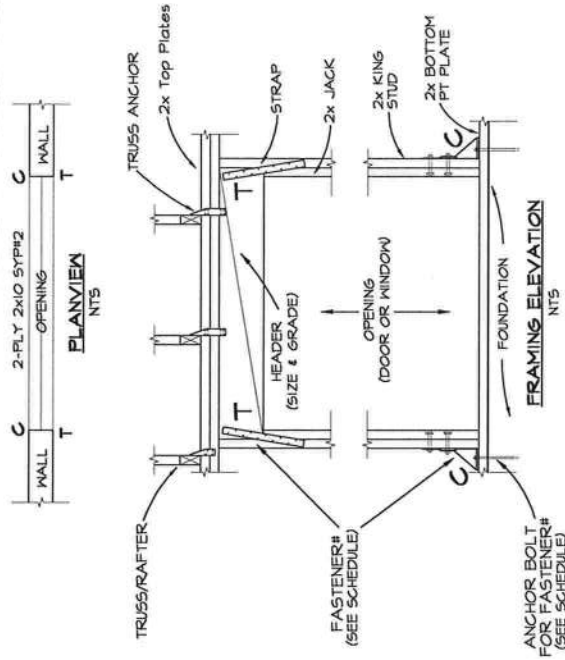
** STRAP(S) FOR FASTENING HEADERS & BEAMS TO STUD(S), COLUMNS & POSTS. USE ONLY (1) STRAP EACH SIDE OF STUD. FASTEN STRAP(S) AT EACH END USING THE GIVEN TOTAL OF NAILS DIVIDED BY TWO.

HOLDOWNS - INSTALLER CAN SUBSTITUTE HOLDOWNS USING ANY HIGHER CAPACITY ITEM IN SCHEDULE (A-K).
STRAPS - INSTALLER CAN SUBSTITUTE STRAP COMBINATIONS AT SAME OR HIGHER CAPACITY IN SCHEDULE (Q-W).

Note: All Fasteners are SIMPSON and shall be installed According to Manufacturer's Specifications and Recommendations. All Bolts are equal to or better than ASTM A307-Grade A. All Nails are Common Galvanized. Drywall Screws (1/4" Type W) can be substituted for 5d cooler nails.

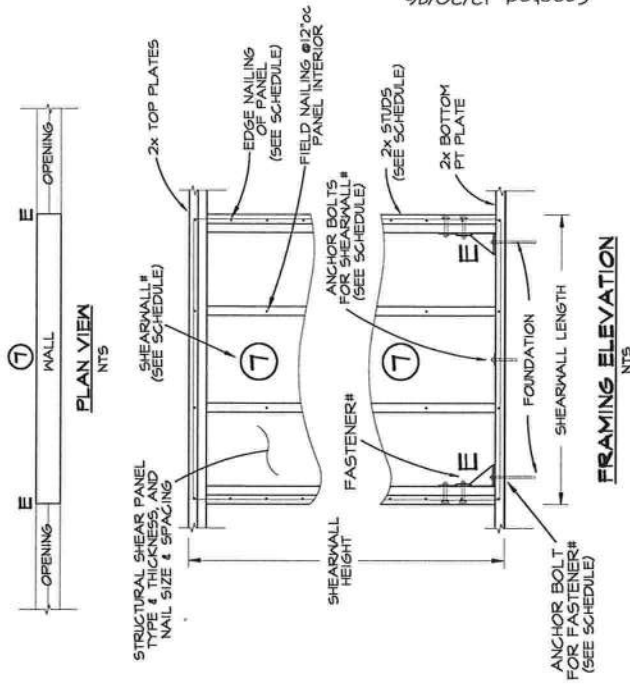
EXAMPLE 1: HEADER

- INSTRUCTIONS:
1. READ THE SAMPLE SHEARWALL LAYOUT BELOW. CHECK FASTENER SCHEDULE.
 2. SELECT FASTENERS FROM SCHEDULE BASED ON THE CORRESPONDING LETTERS C & T.
 3. CONTRACTOR TO THEN INSTALL HD2A HOLDOWN AND CS16 STRAP AS PER ELEVATION BELOW.



EXAMPLE 2: SHEARWALL

- INSTRUCTIONS:
1. READ THE SAMPLE SHEARWALL LAYOUT BELOW. CHECK SHEARWALL AND FASTENER SCHEDULE.
 2. SELECT SHEARWALL & FASTENERS FROM SCHEDULES BASED ON DESIGNATED LETTERS/SYMBOLS.
 3. CONTRACTOR TO THEN INSTALL HD5A HOLDOWN AS PER ELEVATION BELOW.

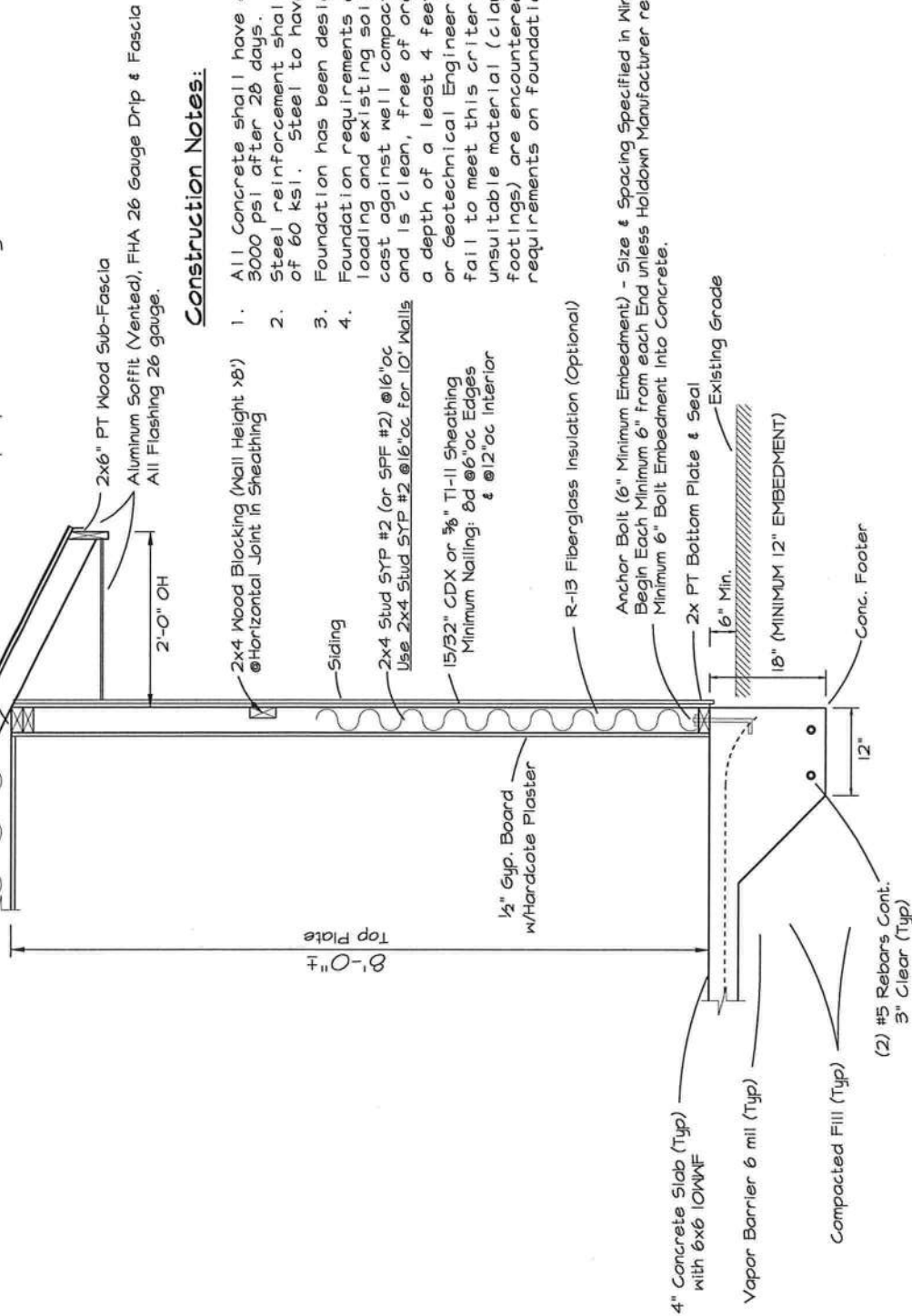


Created: 12/20/96
Revised: 09/19/99

Fabricated Wood Trusses @24"oc
Designed & Manufactured per TPI-2002

Fiberglass Shingles on 30# Felt
CDX Roof Sheathing - Thickness Per Wind Design

R-30 Fiberglass Insulation
(2) 2x Top Plates (Southern Pine SYP Only)
Hurricane Clips per Wind Design



Construction Notes:

1. All Concrete shall have a minimum compressive strength of 3000 psi after 28 days.
2. Steel reinforcement shall have a minimum yield strength of 60 ksi. Steel to have a minimum overlap of 25".
3. Foundation has been designed for a bearing capacity of 1500 psi.
4. Foundation requirements of structure may vary due to design loading and existing soil conditions. Foundations should be cast against well compacted soil having a minimum density of 90% and is clean, free of organics and well graded (A2-A3-A4) up to a depth of a least 4 feet below the footings. Notify Engineer or Geotechnical Engineering Services if existing soil conditions fail to meet this criteria and/or if wet soil conditions or unsuitable material (clay, muck, etc., - after 4 feet below the footings) are encountered. Also, verify design and construction requirements on foundation as required by Truss Manufacturer.

WOOD FRAME WALL SECTION w/MONOLITHIC SLAB

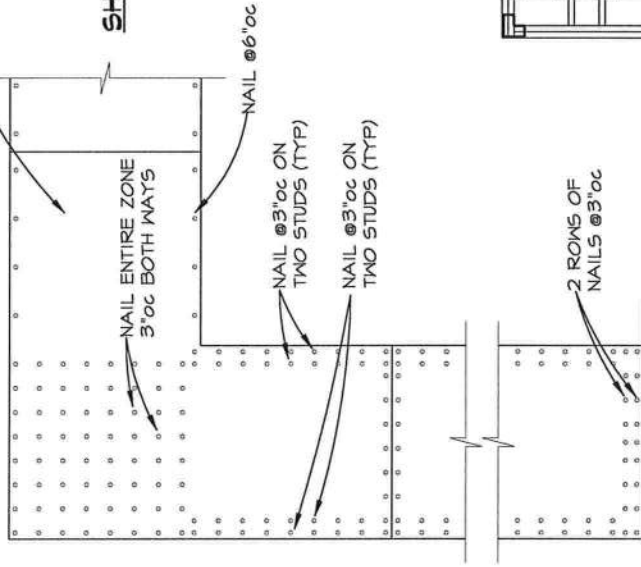
Nts

Created: 7/1/98
Revised: 11/22/05

RON BARLOW, PE

PROVIDE L-SHAPED SHEATHING

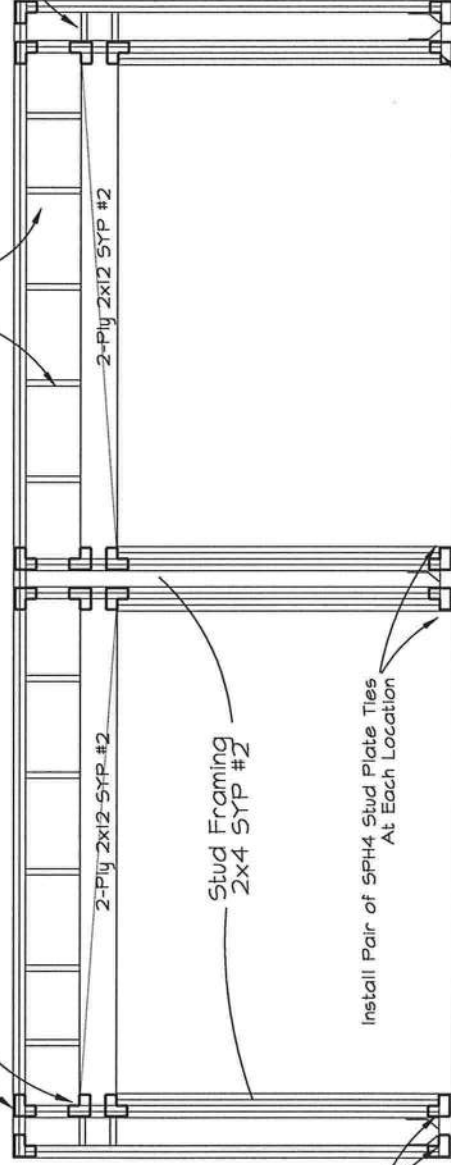
SHEATHING NAILING @ PANELS



FASTEN CRIPPLE STUDS WITH
SIMPSON H2.5 AT EACH END (TYP)

Pair of LCE4 Post Caps (Typ)
Install on Each Side

PROVIDE BLOCKING FOR
3"oc NAILING (TYP)



Install Pair of SPH4 Stud Plate Ties
At Each Location

Install Pair of SPH4 Stud Plate Ties
At Each Location

Install SPH4 Stud Plate Tie
At Each Location

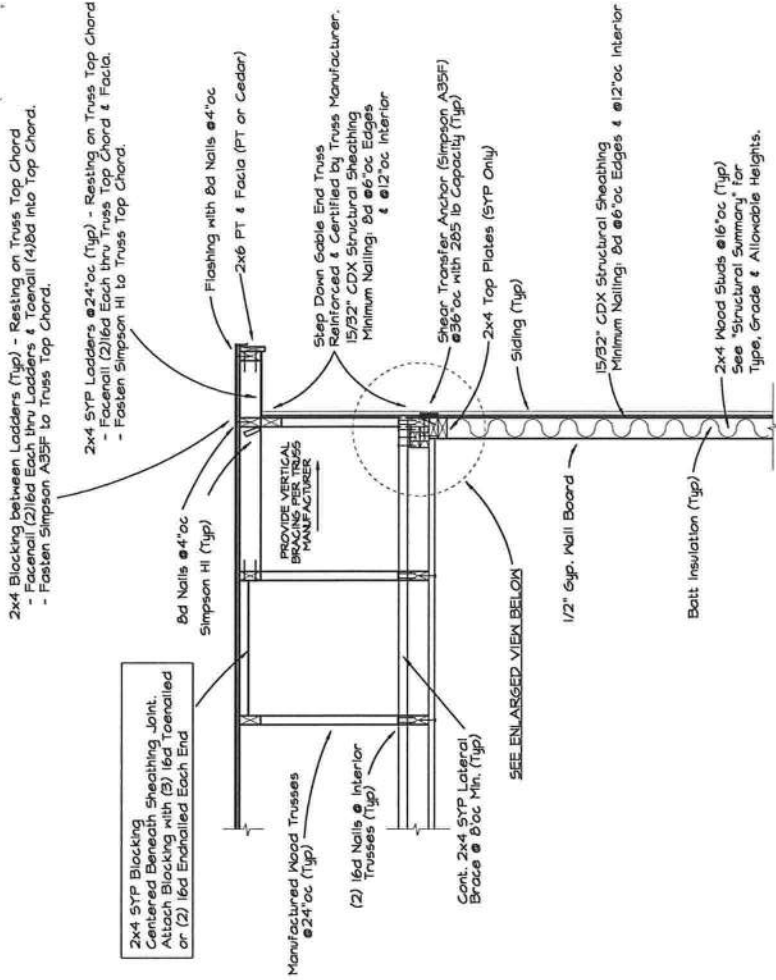
PT 4x4 SYP #2 Or 2-Ply
2x4 SYP #2 Bottom Plate.
Fasten to Rim Joists per
Nail Schedule for SW #10
Provide (2) 5/8" Anchor Bolts (Minimum)

2x4 SYP #2 Studs
Pair of LCE4 Post Caps (Typ)
Top & Bottom

FRAMING DETAIL - 1ST STORY ENDWALL (GARAGE DOORS)

Nts

Created: 7/24/05
Revised: 3/2/07



STEP DOWN GABLE

Nts

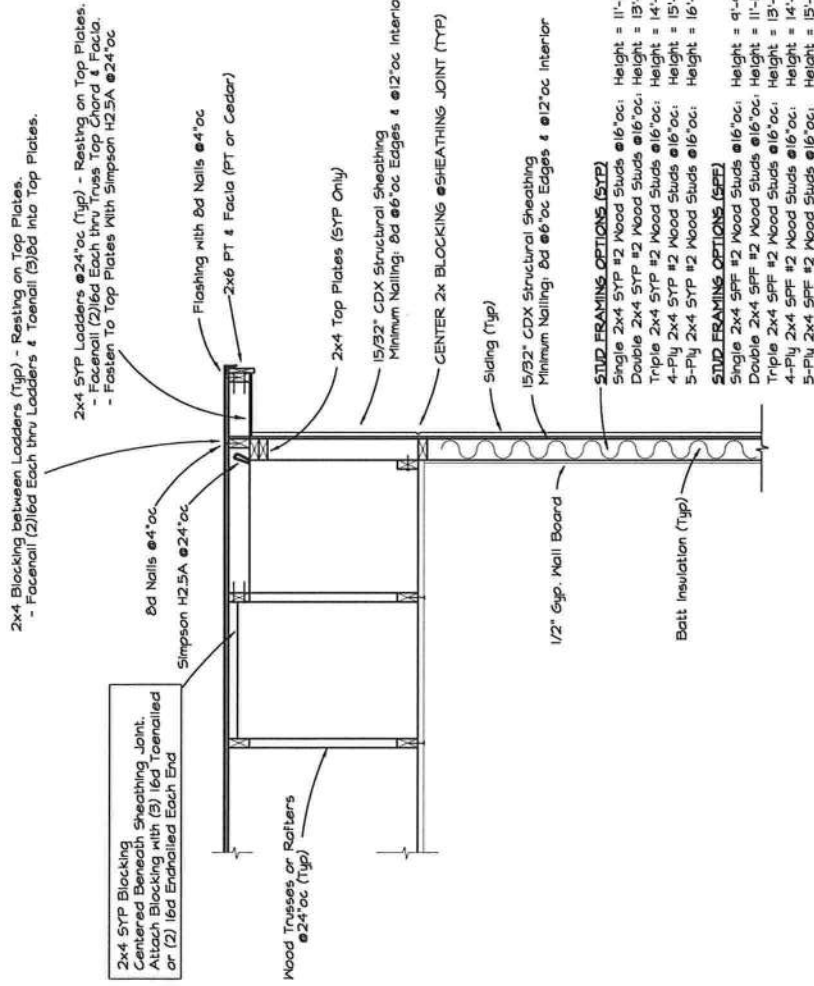
All Nails to be Common Nails (Galvanized)

NOTE: IF MAXIMUM HEIGHT EXCEEDS THESE VALUES THEN 2x6 STUD FRAMING WILL BE REQUIRED. CONTACT ENGINEER

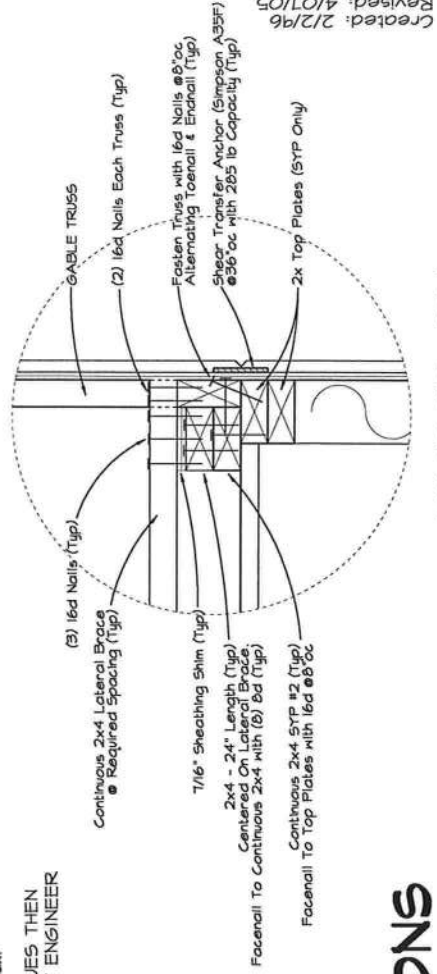
BALLOON FRAMING

Nts

All Nails to be Common Nails (Galvanized)



NOTE: IF MAXIMUM HEIGHT EXCEEDS THESE VALUES THEN 2x6 STUD FRAMING WILL BE REQUIRED. CONTACT ENGINEER



ENLARGED VIEW

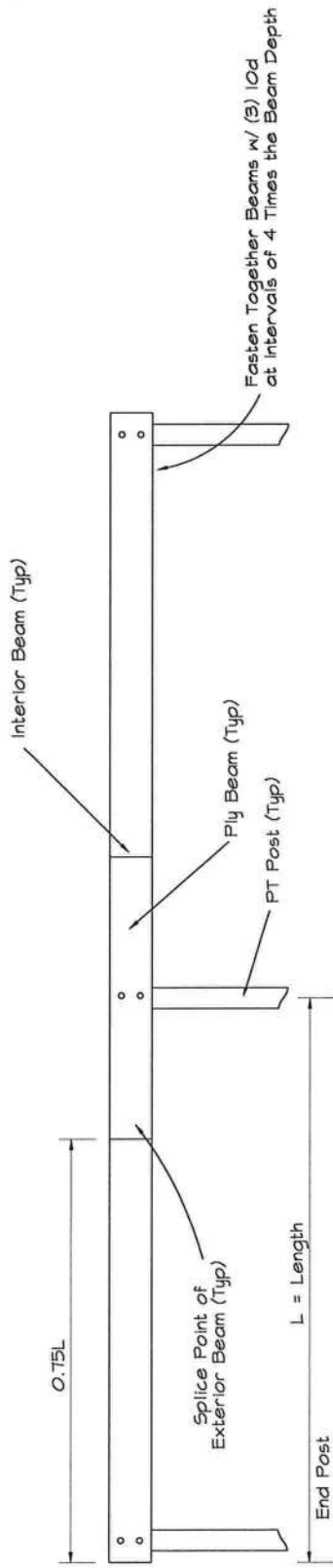
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RON BARLOW, PE

Created: 2/12/96
 Revised: 4/10/05

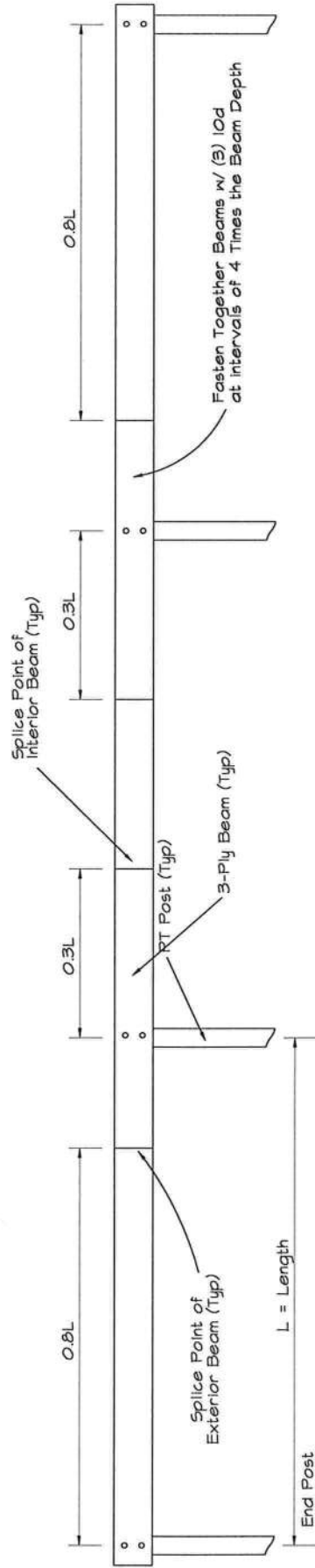
GABLE END WALL OPTIONS

***** DETAIL INFORMATION IS FOR THIS PROJECT ONLY *****



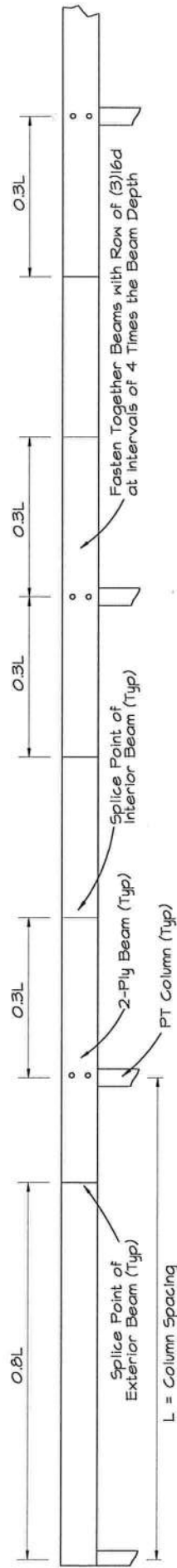
BEAM SPLICE DETAIL - 3 Columns

Nts



BEAM SPLICE DETAIL - 4 Columns

Nts



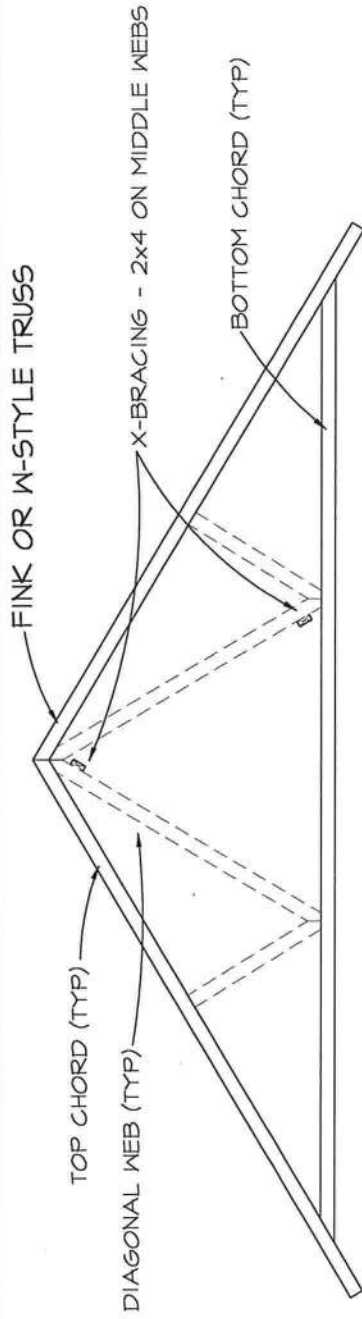
Created: 1/04/98
Revised: 7/27/00

MULTI-PLY BEAM SPLICE - 4 or More Columns

Nts

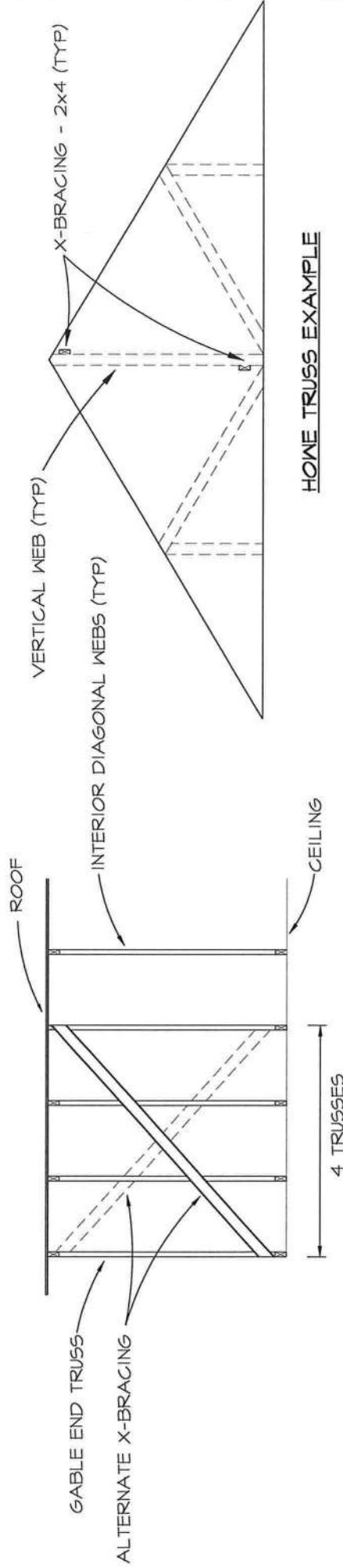
*** FOR THIS PROJECT ONLY ***

THESE FRAMING METHODS WILL REDUCE SIZE OF BEAMS. IF NOT USED, THEN NUMBER OF PLIES WILL INCREASE DUE TO SIMPLE SUPPORT. CONTACT ENGINEER IF DIFFERENT.



X-BRACING - 2x4 (TYP)
DIAGONALS TO BE POSITIONED ON THE INSIDE FACE OF MIDDLE WEBS
(4 TRUSSES) AND THEN FACENAILED WITH (2)16d AT EACH WEB LOCATION.

FRONT VIEW - FINK TRUSS



SIDE VIEW - FINK TRUSS

PERMANENT X-BRACING OF FINK TRUSSES @ GABLE END

Nts

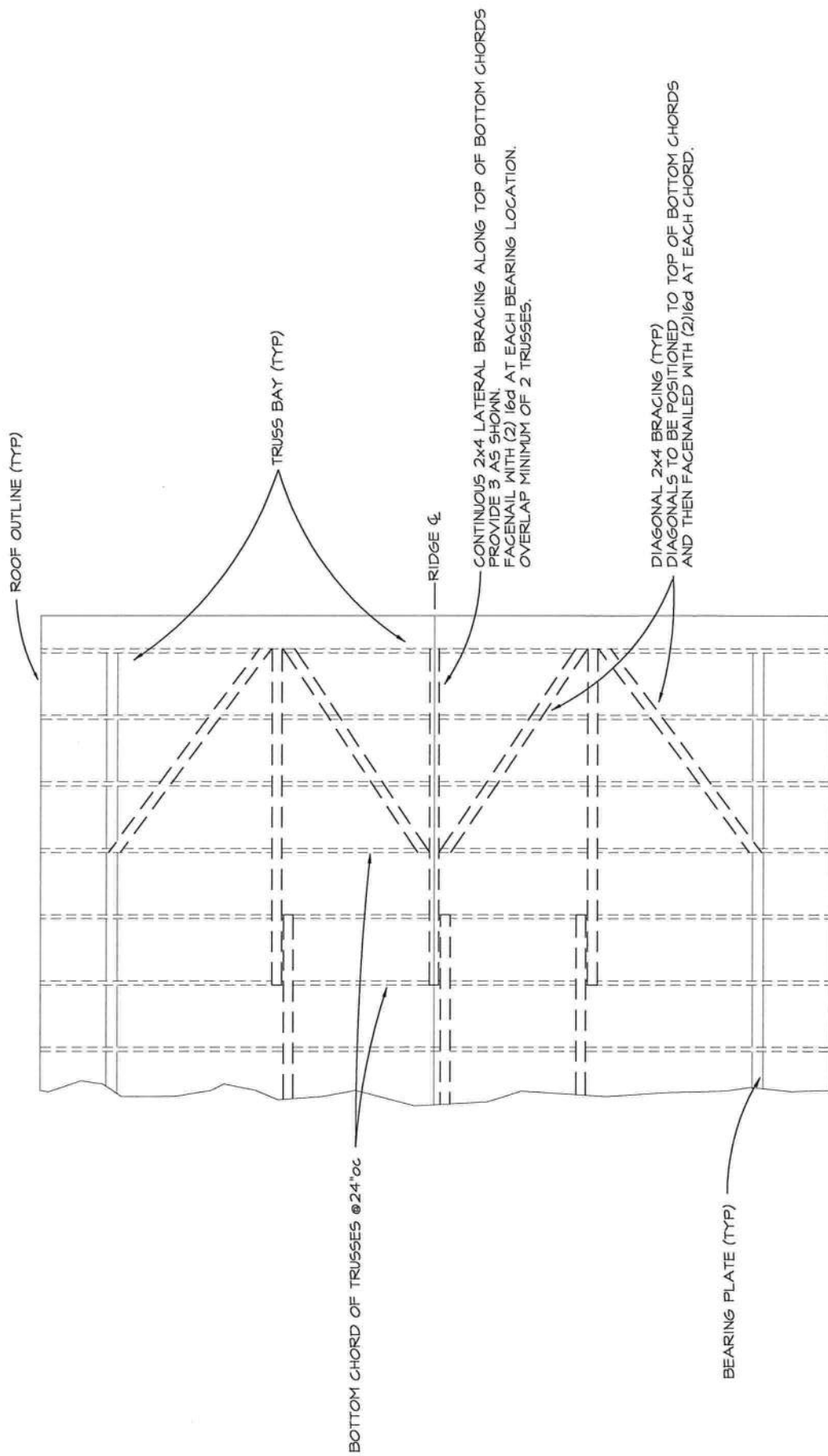
***** DETAIL INFORMATION IS FOR THIS PROJECT ONLY *****

IF TRUSS IS HOWE STYLE, THEN NAIL X-BRACING ON OPPOSITE SIDE OF MIDDLE VERTICAL WEB AS SHOWN ABOVE.

The Contractor Shall Provide all Temporary and Permanent Bracing as Required for Safe Erection and Performance of the Trusses. The Guidelines Set Forth by the Truss Plate Institute Publication "HIB-q1, Commentary and Recommendations for Handling, Installing and Bracing Metal Plate Connected Wood Trusses" Shall be a Minimum Requirement.

Created: 5/15/03
Revised: 5/18/03

RON BARLOW, PE



PERMANENT TRUSS BRACING OF BOTTOM CHORDS

Nts

***** DETAIL INFORMATION IS FOR THIS PROJECT ONLY *****

**** PROVIDE DIAGONAL AND LATERAL BRACING OF BOTTOM CHORDS FOR EACH TRUSS BAY AS SHOWN ****

**** CONTINUOUS LATERAL BRACING NOT TO EXCEED 10'oc SPACING ****

The Contractor Shall Provide all Temporary and Permanent Bracing as Required for Safe Erection and Performance of the Trusses. The Guidelines Set Forth by the Truss Plate Institute Publication "HIB-q1, Commentary and Recommendations for Handling, Installing and Bracing Metal Plate Connected Wood Trusses" Shall be a Minimum Requirement.

Created: 5/11/96
Revised: 5/18/03

RON BARLOW, PE

INDIVIDUAL SHEATHING ATTACHED
TO A MINIMUM OF 3 TRUSSES.

EDGE NAILING OF PANELS
SEE ZONE SCHEDULE

FIELD (INTERIOR)
NAILING OF PANELS
SEE ZONE SCHEDULE

8d NAILS @ 4" oc
ALONG GABLE END TRUSS

RIDGE ϕ

2x4 Cont. (Typ)

OVERHANG - SEE DETAILS
NOT TO EXCEED 24"

GABLE ROOF SHEATHING ATTACHMENT

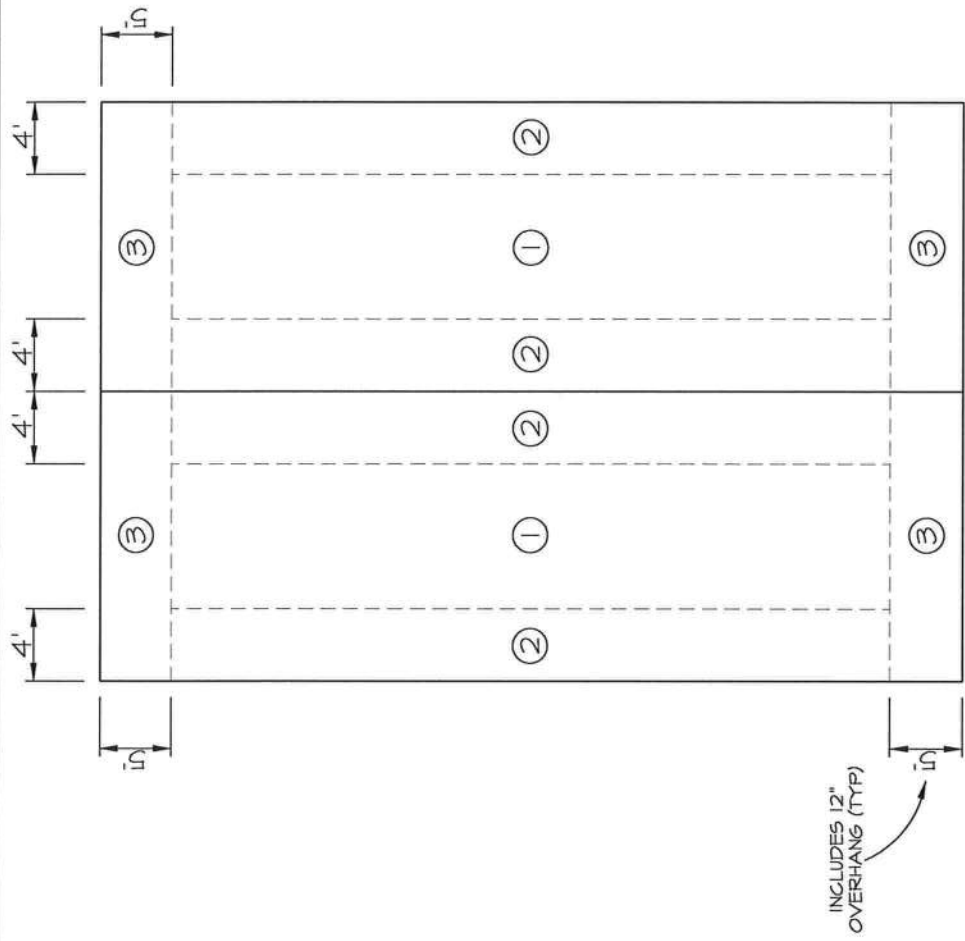
Nts

NOTES:

1. SEE DETAIL DRAWING OF "ROOF FASTENING ZONES FOR WIND UPLIFT"
FOR REQUIRED NAIL SIZE & SPACING REQUIREMENTS FOR GABLE & HIP
ROOF STYLES.

Created: 7/11/98
Revised: 4/08/05

RON BARLOW, PE



GABLE OR FLAT ROOF STYLE

ROOF FASTENING SCHEDULES FOR WIND UPLIFT

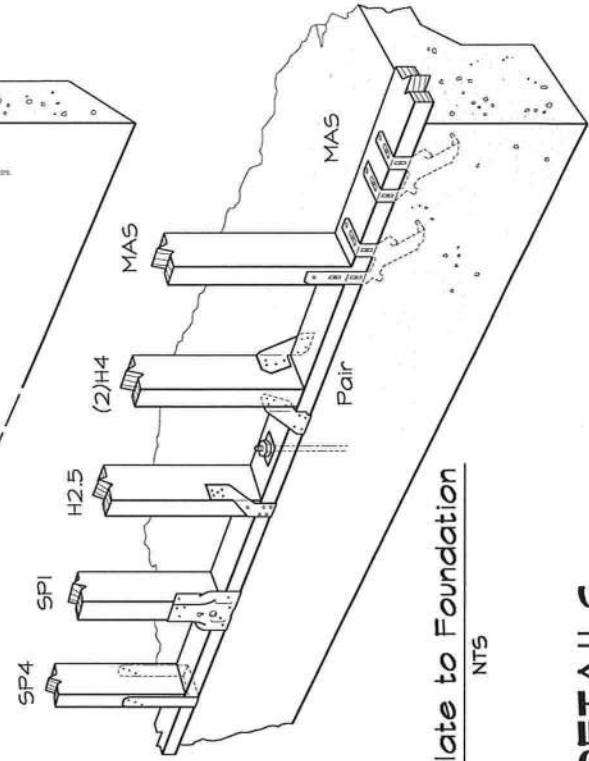
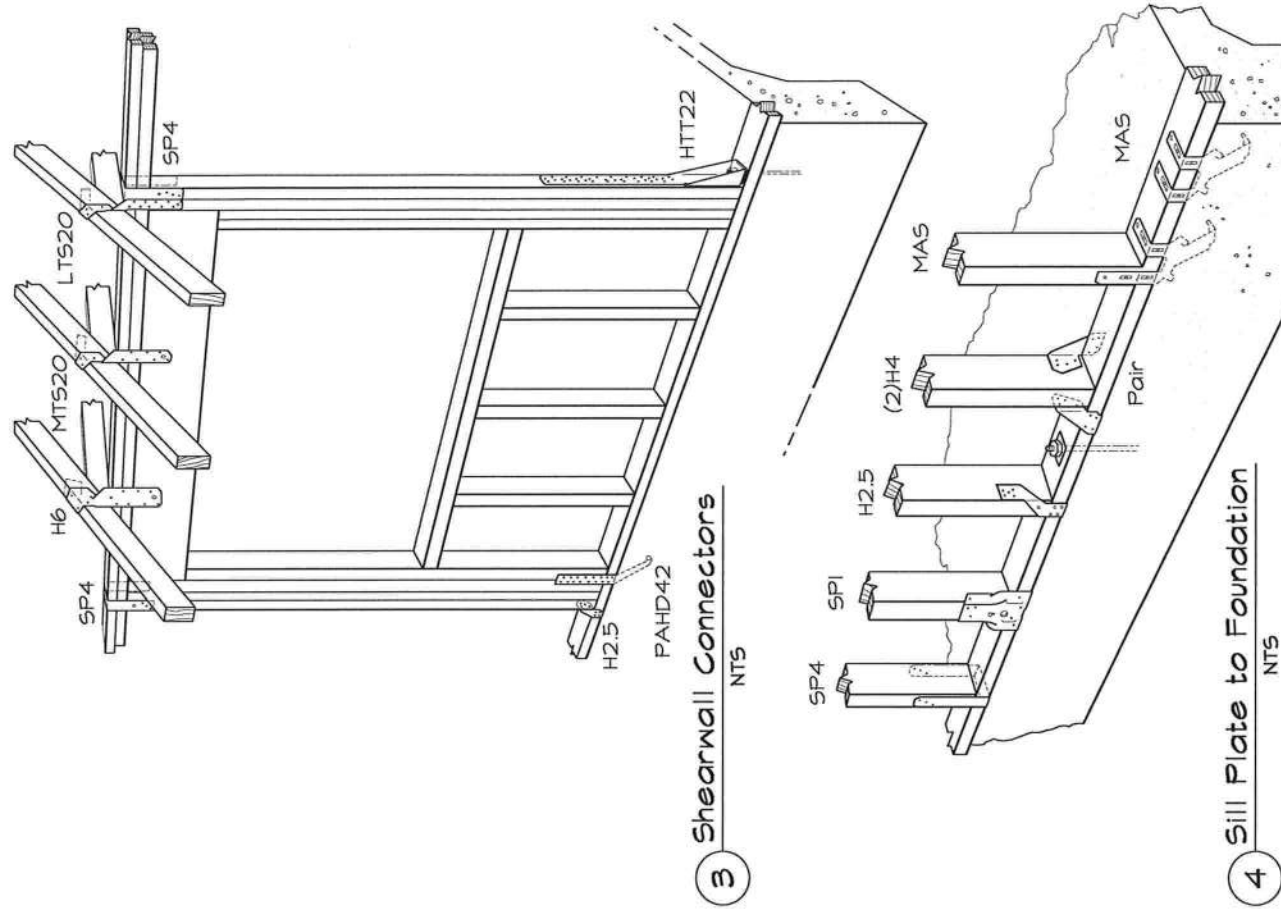
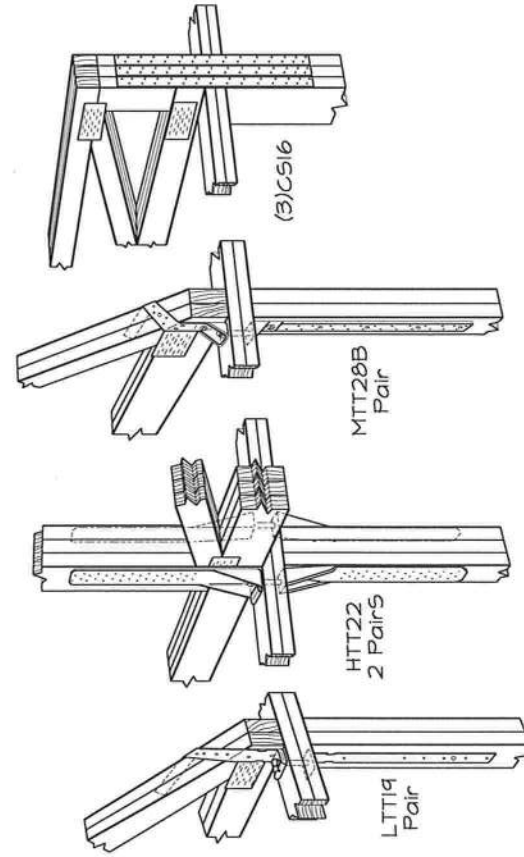
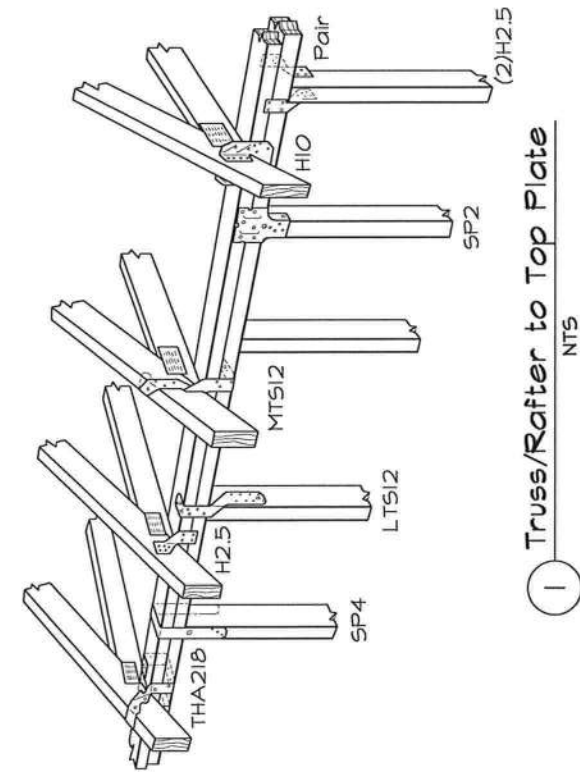
Nts

NOTES:

1. ALL NAILS TO BE COMMON.
2. USE 12d NAILS FOR THE ATTACHMENT OF 3/4" CDX ROOF SHEATHING TO 48"oc TRUSSES.
3. ZONES SHOWN ABOVE INDICATE AREAS OF THE ROOF WITH DIFFERENT FASTENING REQUIREMENTS, AND SHOULD NOT BE CONFUSED WITH ASCE 7 PRESSURE COEFFICIENT ZONES.

WIND SPEED	NAIL/PANEL LOCATION	ROOF FASTENING ZONE		
		①	②	③
130 MPH OR MORE	10d EDGES	6	6	4a
	10d FIELD	6	6	6a
120 MPH	8d EDGES	6	6	4b
	8d FIELD	6	6	6b
110 MPH OR LESS	8d EDGES	8	6	4
	8d FIELD	8	6	4

(a) USE 12d RING SHANK NAILS FOR MEAN ROOF HEIGHTS GREATER THAN 25'.



SIMPSON FRAMING DETAILS

Note: These Details are quick aid to help the Owner/Contractor recognize fasteners and their applications. Please refer to the current Simpson catalog "Wood Construction Connectors" for connector load values, installation, fastener schedulers and other important information.

07009.wsr

WoodWorks® Shearwalls 2002a

Mar. 9, 2007 01:38:35

COMPANY
 Ron Barlow, P.E.
 4321 NW 18th Place
 Gainesville, FL 32605
 (352) 335-6724

PROJECT
 New House
 Fort White Spec House
 Harvey Building

STRUCTURAL DATA

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## STORY INFORMATION

|            | Elevation[ft] | Floor/Ceiling<br>Depth[in] | Wall<br>Height[ft] | Building<br>Mass[lbs] | Story<br>Shear[lbs] |
|------------|---------------|----------------------------|--------------------|-----------------------|---------------------|
| Ceiling    | 11.67         | 10.0"                      |                    |                       |                     |
| Level 1    | 2.83          | 10.0"                      | 8.00               | 0                     | 0                   |
| Foundation | 2.00          |                            | Totals:            | 0                     | 0                   |

## BLOCK INFORMATION

| Block:Block 1   | 1 Stories | ROOF PANELS<br>Type | Slope | Overhang<br>[ft] | Mass<br>[lbs] |
|-----------------|-----------|---------------------|-------|------------------|---------------|
| Location[ft]:   | 2.00      | North               | Side  | 26.6             | 2.00          |
| Extent[ft]:     | 12.67     | South               | Side  | 26.6             | 2.00          |
| Ridge Loc[ft]:  | 2.00      | East                | Gable | 90.0             | 2.00          |
| Ridge Elev[ft]: | 9.10      | West                | Gable | 90.0             | 2.00          |

## BLOCK INFORMATION

| Block:Block 2   | 1 Stories | ROOF PANELS<br>Type | Slope | Overhang<br>[ft] | Mass<br>[lbs] |
|-----------------|-----------|---------------------|-------|------------------|---------------|
| Location[ft]:   | 8.33      | North               | Side  | 6.6              | 2.00          |
| Extent[ft]:     | 25.75     | South               | Side  | 6.6              | 2.00          |
| Ridge Loc[ft]:  | 8.33      | East                | Gable | 90.0             | 2.00          |
| Ridge Elev[ft]: | 1.73      | West                | Gable | 90.0             | 2.00          |

## BLOCK INFORMATION

| Block:Block 3   | 1 Stories | ROOF PANELS<br>Type | Slope | Overhang<br>[ft] | Mass<br>[lbs] |
|-----------------|-----------|---------------------|-------|------------------|---------------|
| Location[ft]:   | 0.00      | North               | Side  | 26.6             | 2.00          |
| Extent[ft]:     | 19.50     | South               | Side  | 26.6             | 2.00          |
| Ridge Loc[ft]:  | 0.00      | East                | Gable | 90.0             | 2.00          |
| Ridge Elev[ft]: | 11.60     | West                | Gable | 90.0             | 2.00          |

## DESIGN SETTINGS

Design Code: FBC 2004

Wind Capacity Increase: Shear = 1.40; C&amp;C Panels = 1.50; Withdrawal = 1.60

Wind: Sheathing capacities are added together

Shearwall Relative Rigidities based on Flexible Design

Holddown Forces based on Applied Force

Max Shearwall Offset: Plan = 4.00'; Height to Width Ratio = 3.50

## SITE INFORMATION

SBC Occupancy: All others; ASCE Equivalent - Category II

WIND: ASCE-7 02 General analytic method for all buildings

Design wind speed = 110mph; ; Enclosed building

## MATERIALS by WALL GROUP

| Wall                           | Sheathing [in]                    | Fasteners: Spacing[in] | Framing Members [in] | Apply |
|--------------------------------|-----------------------------------|------------------------|----------------------|-------|
| Grp Surf Material Thickness Or | Size Type Edge Field Blkg Species | G Spacing Notes        |                      |       |
| 1 Ext Structural I 5/16 Vert   | 8d Nail 6 12 yes                  | D.Fir-L 0.50 16        |                      |       |
| Int Gyp WB 1-ply 1/2 Horz      | 5d Nail 7 10 yes                  | D.Fir-L 0.50 16        |                      |       |

## SHEARLINE DIMENSIONS

| Name Level | Wall Design<br>Group(s) | Location [ft]<br>X = Y = | Extent [ft]<br>Start End | Full Height<br>Sheathing [ft] |
|------------|-------------------------|--------------------------|--------------------------|-------------------------------|
| 1          |                         | 0.00                     | 2.00 38.33               |                               |
|            | 1 1                     | 0.00                     | 2.00 38.33               | 36.33                         |
| 2          |                         | 12.67                    | 2.00 38.33               |                               |
|            | 1 1                     | 12.67                    | 2.00 38.33               | 33.33                         |
| 3          |                         | 38.42                    | 0.00 46.33               |                               |
|            | 1 1                     | 38.42                    | 0.00 46.33               | 38.00                         |
| 4          |                         | 57.92                    | 0.00 46.33               |                               |
|            | 1 1                     | 57.92                    | 0.00 46.33               | 42.33                         |
| A          |                         |                          | 2.00 0.00 57.92          |                               |
|            | 1 1                     |                          | 2.00 0.00 12.67          | 6.67                          |
| B          |                         |                          | 8.33 12.67 38.42         |                               |
|            | 1 1                     |                          | 8.33 0.00 57.92          | 6.75                          |
| C          |                         |                          | 20.67 38.42 57.92        |                               |
|            | 1 1                     |                          | 20.67 0.00 57.92         | 19.50                         |
| D          |                         |                          | 38.33 0.00 38.42         |                               |
|            | 1 1                     |                          | 38.33 0.00 57.92         | 19.42                         |
| E          |                         |                          | 46.33 38.42 57.92        |                               |
|            | 1 1                     |                          | 46.33 38.42 57.92        | 13.50                         |

# LOADS

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WIND SHEAR LOADS

Name	Building	Lev	Profile	Tributary Width[ft]	Location[ft]		Magnitude [lbs,plf,psf]	Wind Direction
	Face				From	To		
Block 1 N Wall	North	1	Line		0.00	12.67	49.7	Windward
Block 1 N Wall	North	1	Line		0.00	12.67	16.3	Leeward
Block 1 N Roof	North	1	Line		-2.00	14.67	79.7	Leeward
Block 1 N Roof	North	1	Line		-2.00	14.67	29.4	Windward
Block 1 E Wall	East	1	Line		2.00	8.33	49.7	Windward
Block 1 E Wall	East	1	Line		2.00	8.33	31.8	Leeward
Block 1 E Wall	East	1	Line		2.00	20.17	0.0-59.8	Leeward
Block 1 E Wall	East	1	Line		2.00	20.17	0.0-95.8	Windward
Block 1 E Wall	East	1	Line		20.17	38.33	59.8-0.0	Leeward
Block 1 E Wall	East	1	Line		20.17	38.33	95.8-0.0	Windward
Block 1 S Wall	South	1	Line		0.00	12.67	49.7	Windward
Block 1 S Wall	South	1	Line		0.00	12.67	16.3	Leeward
Block 1 S Roof	South	1	Line		-2.00	14.67	79.7	Leeward
Block 1 S Roof	South	1	Line		-2.00	14.67	29.4	Windward
Block 1 W Wall	West	1	Line		2.00	38.33	49.7	Windward
Block 1 W Wall	West	1	Line		2.00	38.33	31.8	Leeward
Block 1 W Wall	West	1	Line		2.00	20.17	0.0-59.8	Leeward
Block 1 W Wall	West	1	Line		2.00	20.17	0.0-95.8	Windward
Block 1 W Wall	West	1	Line		20.17	38.33	59.8-0.0	Leeward
Block 1 W Wall	West	1	Line		20.17	38.33	95.8-0.0	Windward
Block 2 N Wall	North	1	Line		12.67	38.42	49.7	Windward
Block 2 N Wall	North	1	Line		12.67	38.42	29.0	Leeward
Block 2 N Roof	North	1	Line		10.67	40.42	10.0	Leeward
Block 2 N Roof	North	1	Line		10.67	40.42	-20.1	Windward
Block 2 E Wall	East	1	Line		8.33	23.33	0.0-11.2	Leeward
Block 2 E Wall	East	1	Line		8.33	23.33	0.0-17.8	Windward
Block 2 E Wall	East	1	Line		23.33	38.33	11.2-0.0	Leeward
Block 2 E Wall	East	1	Line		23.33	38.33	17.8-0.0	Windward
Block 2 S Wall	South	1	Line		12.67	38.42	49.7	Windward
Block 2 S Wall	South	1	Line		12.67	38.42	29.0	Leeward
Block 2 S Roof	South	1	Line		10.67	40.42	10.0	Leeward
Block 2 S Roof	South	1	Line		10.67	40.42	-20.1	Windward
Block 2 W Wall	West	1	Line		8.33	23.33	0.0-11.2	Leeward
Block 2 W Wall	West	1	Line		8.33	23.33	0.0-17.8	Windward
Block 2 W Wall	West	1	Line		23.33	38.33	11.2-0.0	Leeward
Block 2 W Wall	West	1	Line		23.33	38.33	17.8-0.0	Windward
Block 3 N Wall	North	1	Line		38.42	57.92	49.7	Windward
Block 3 N Wall	North	1	Line		38.42	57.92	18.3	Leeward
Block 3 N Roof	North	1	Line		36.42	59.92	101.6	Leeward
Block 3 N Roof	North	1	Line		36.42	59.92	42.2	Windward
Block 3 E Wall	East	1	Line		0.00	46.33	49.7	Windward
Block 3 E Wall	East	1	Line		0.00	46.33	32.5	Leeward
Block 3 E Wall	East	1	Line		0.00	23.17	0.0-77.9	Leeward
Block 3 E Wall	East	1	Line		0.00	23.17	0.0-124.3	Windward
Block 3 E Wall	East	1	Line		23.17	46.33	77.9-0.0	Leeward
Block 3 E Wall	East	1	Line		23.17	46.33	124.3-0.0	Windward
Block 3 S Wall	South	1	Line		38.42	57.92	49.7	Windward
Block 3 S Wall	South	1	Line		38.42	57.92	18.3	Leeward
Block 3 S Roof	South	1	Line		36.42	59.92	101.6	Leeward
Block 3 S Roof	South	1	Line		36.42	59.92	42.2	Windward
Block 3 W Wall	West	1	Line		0.00	8.33	49.7	Windward
Block 3 W Wall	West	1	Line		38.33	46.33	49.7	Windward
Block 3 W Wall	West	1	Line		0.00	8.33	32.5	Leeward
Block 3 W Wall	West	1	Line		38.33	46.33	32.5	Leeward
Block 3 W Wall	West	1	Line		0.00	23.17	0.0-77.9	Leeward
Block 3 W Wall	West	1	Line		0.00	23.17	0.0-124.3	Windward
Block 3 W Wall	West	1	Line		23.17	46.33	77.9-0.0	Leeward
Block 3 W Wall	West	1	Line		23.17	46.33	124.3-0.0	Windward

WIND C&C LOADS

Name	Building	Level	Magnitude [psf]	
	Face		Interior	End Zone
Block 1 N Wall	North	1	23.6	29.1
Block 1 E Wall	East	1	23.6	29.1
Block 1 S Wall	South	1	23.6	29.1
Block 1 W Wall	West	1	23.6	29.1
Block 2 N Wall	North	1	21.6	26.6
Block 2 S Wall	South	1	21.6	26.6
Block 3 N Wall	North	1	23.6	29.1
Block 3 E Wall	East	1	23.6	29.1
Block 3 S Wall	South	1	23.6	29.1
Block 3 W Wall	West	1	23.6	29.1

WIND DESIGN

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FLEXIBLE DIAPHRAGM DESIGN

SHEAR RESULTS by SHEARLINE

| Line | Lev | Wall | Wind | Total | Cumulative Shear Force |      |            | Shear Capacity [plf] |       |      | Response |           |
|------|-----|------|------|-------|------------------------|------|------------|----------------------|-------|------|----------|-----------|
|      |     | Grp. | Dir. | FHS   | V[lbs]                 | Unit | V/FHS[plf] | Inter                | Exter | Perf | Total    | V/FHS/Tot |
| 1    | 1   | 1    | Both | 36.33 | 1326                   | 36.5 | 36.5       |                      | 280.0 | 1.00 | 280.0    | 0.13      |
| 2    | 1   | 1    | Both | 33.33 | 2196                   | 60.4 | 65.9       |                      | 280.0 | 0.87 | 243.5    | 0.27      |
| 3    | 1   | 1    | Both | 38.00 | 3214                   | 69.4 | 84.6       |                      | 280.0 | 0.71 | 197.6    | 0.43      |
| 4    | 1   | 1    | Both | 42.33 | 2351                   | 50.7 | 55.5       |                      | 280.0 | 1.00 | 280.0    | 0.20      |
| A    | 1   | 1    | Both | 6.67  | 871                    | 68.8 | 130.7      |                      | 280.0 | 0.71 | 198.0    | 0.66      |
| B    | 1   | 1    | W->E | 6.75  | 2582                   | 44.6 | 382.6      |                      | 280.0 | 0.68 | 191.5    | 2.00*     |
| C    | 1   | 1    | W->E | 19.50 | 5541                   | 95.7 | 284.2      |                      | 280.0 | 1.00 | 280.0    | 1.01*     |
| D    | 1   | 1    | W->E | 19.42 | 2854                   | 49.3 | 147.0      |                      | 280.0 | 0.70 | 196.8    | 0.75      |
| E    | 1   | 1    | Both | 13.50 | 422                    | 21.6 | 31.2       |                      | 280.0 | 0.79 | 220.6    | 0.14      |

\* WARNING design capacity has been exceeded.

SHEARWALL SEGMENTS

| Name      | Lev | Type | Wall  | Dimensions [ft] |       |       | Full Height Perf |        | Shear [lbs] |          | Opening [ft] |        |
|-----------|-----|------|-------|-----------------|-------|-------|------------------|--------|-------------|----------|--------------|--------|
|           |     |      | Group | Location        | Start | End   | Sheathing        | Factor | Force       | Capacity | Width        | Height |
| 1-1       | 1   | II   | 1     | 0.00            | 2.00  | 38.33 | 36.33            | 1.00   | 1326        | 10173    |              |        |
| 2-1       | 1   | II   | 1     | 12.67           | 2.00  | 8.33  | 6.33             | 1.00   | 417         | 1773     |              |        |
| 2-2       | 1   | II   | 1     | 12.67           | 8.33  | 38.33 | 27.00            | 0.87   | 1779        | 6573     |              |        |
| Opening 1 |     |      |       |                 | 20.83 | 23.83 |                  |        |             |          | 3.00         | 6.67   |
| 3-1       | 1   | II   | 1     | 38.42           | 0.00  | 8.33  | 8.33             | 1.00   | 705         | 2333     |              |        |
| 3-2       | 1   | II   | 1     | 38.42           | 8.33  | 38.33 | 21.67            | 0.71   | 1833        | 4281     |              |        |
| Opening 1 |     |      |       |                 | 17.17 | 20.17 |                  |        |             |          | 3.00         | 6.67   |
| Opening 2 |     |      |       |                 | 24.83 | 27.50 |                  |        |             |          | 2.67         | 6.67   |
| Opening 3 |     |      |       |                 | 32.83 | 35.50 |                  |        |             |          | 2.67         | 6.67   |
| 3-3       | 1   | II   | 1     | 38.42           | 38.33 | 46.33 | 8.00             | 1.00   | 676         | 2239     |              |        |
| 4-1       | 1   | II   | 1     | 57.92           | 0.00  | 46.33 | 42.33            | 1.00   | 2351        | 11852    |              |        |
| Opening 1 |     |      |       |                 | 21.50 | 25.50 |                  |        |             |          | 4.00         | 2.00   |
| A-1       | 1   | II   | 1     | 2.00            | 0.00  | 12.67 | 6.67             | 0.71   | 871         | 1320     |              |        |
| Opening 1 |     |      |       |                 | 3.33  | 9.33  |                  |        |             |          | 6.00         | 5.00   |
| B-1       | 1   | II   | 1     | 8.33            | 12.67 | 38.42 | 6.75             | 0.68   | 2582        | 1293     |              |        |
| Opening 1 |     |      |       |                 | 14.67 | 20.67 |                  |        |             |          | 6.00         | 5.00   |
| Opening 2 |     |      |       |                 | 24.09 | 27.09 |                  |        |             |          | 3.00         | 6.67   |
| Opening 3 |     |      |       |                 | 30.42 | 36.42 |                  |        |             |          | 6.00         | 5.00   |
| C-1       | 1   | II   | 1     | 20.67           | 38.42 | 57.92 | 19.50            | 1.00   | 5541        | 5460     |              |        |
| D-1       | 1   | II   | 1     | 38.33           | 0.00  | 12.67 | 6.67             | 0.71   | 980         | 1320     |              |        |
| Opening 1 |     |      |       |                 | 3.17  | 9.17  |                  |        |             |          | 6.00         | 5.00   |
| D-2       | 1   | II   | 1     | 38.33           | 12.67 | 38.42 | 12.75            | 0.70   | 1874        | 2509     |              |        |
| Opening 1 |     |      |       |                 | 21.00 | 26.00 |                  |        |             |          | 5.00         | 6.67   |
| Opening 2 |     |      |       |                 | 30.42 | 36.42 |                  |        |             |          | 6.00         | 5.00   |
| E-1       | 1   | II   | 1     | 46.33           | 38.42 | 57.92 | 13.50            | 0.79   | 422         | 2978     |              |        |
| Opening 1 |     |      |       |                 | 41.92 | 44.92 |                  |        |             |          | 3.00         | 5.00   |
| Opening 2 |     |      |       |                 | 51.42 | 54.42 |                  |        |             |          | 3.00         | 5.00   |

DRAGSTRUT AND HOLDDOWN FORCES

| Level | Location | [ft]  | Wall | Position on Wall |           | Holddown Force [lbs] |      |        |          | Dragstrut  |
|-------|----------|-------|------|------------------|-----------|----------------------|------|--------|----------|------------|
|       | X        | Y     |      | or Opening       |           | Shear                | Dead | Uplift | Combined | Force[lbs] |
| 1     | 0.00     | 2.00  | 1-1  | Left             | Wall End  | 292                  |      |        | 292      |            |
| 1     | 0.00     | 38.33 | 1-1  | Left             | Wall End  | 292                  |      |        | 292      |            |
| 1     | 12.67    | 2.00  | 2-2  | Left             | Wall End  | 527                  |      |        | 527      |            |
| 1     | 12.67    | 8.33  | 2-2  | Left             | Wall End  | 1054                 |      |        | 1054     |            |
| 1     | 12.67    | 38.33 | 2-2  | Left             | Wall End  | 527                  |      |        | 527      |            |
| 1     | 12.67    | 20.83 | 2-2  | Left             | Opening 1 |                      |      |        |          | 102        |
| 1     | 12.67    | 23.83 | 2-2  | Right            | Opening 1 |                      |      |        |          | 79         |
| 1     | 38.42    | 0.00  | 3-3  | Left             | Wall End  | 677                  |      |        | 677      |            |
| 1     | 38.42    | 8.33  | 3-3  | Left             | Wall End  | 1353                 |      |        | 1353     |            |
| 1     | 38.42    | 38.33 | 3-3  | Left             | Wall End  | 1353                 |      |        | 1353     |            |
| 1     | 38.42    | 46.33 | 3-3  | Left             | Wall End  | 677                  |      |        | 677      |            |
| 1     | 38.42    | 17.17 | 3-2  | Left             | Opening 1 |                      |      |        |          | 261        |
| 1     | 38.42    | 20.17 | 3-2  | Right            | Opening 1 |                      |      |        |          | 53         |
| 1     | 38.42    | 24.83 | 3-2  | Left             | Opening 2 |                      |      |        |          | 124        |
| 1     | 38.42    | 27.50 | 3-2  | Right            | Opening 2 |                      |      |        |          | 61         |
| 1     | 38.42    | 32.83 | 3-2  | Left             | Opening 3 |                      |      |        |          | 20         |
| 1     | 38.42    | 35.50 | 3-2  | Right            | Opening 3 |                      |      |        |          | 165        |
| 1     | 57.92    | 0.00  | 4-1  | Left             | Wall End  | 444                  |      |        | 444      |            |
| 1     | 57.92    | 46.33 | 4-1  | Left             | Wall End  | 444                  |      |        | 444      |            |
| 1     | 57.92    | 21.50 | 4-1  | Left             | Opening 1 |                      |      |        |          | 103        |
| 1     | 57.92    | 25.50 | 4-1  | Right            | Opening 1 |                      |      |        |          | 100        |
| 1     | 0.00     | 2.00  | A-1  | Left             | Wall End  | 1045                 |      |        | 1045     |            |
| 1     | 12.67    | 2.00  | A-1  | Left             | Wall End  | 1045                 |      |        | 1045     |            |
| 1     | 3.33     | 2.00  | A-1  | Left             | Opening 1 |                      |      |        |          | 206        |
| 1     | 9.33     | 2.00  | A-1  | Right            | Opening 1 |                      |      |        |          | 206        |
| 1     | 20.67    | 8.33  | B-1  | Left             | Wall End  | 3061                 |      |        | 3061     | 922        |
| 1     | 30.42    | 8.33  | B-1  | Left             | Wall End  | 3061                 |      |        | 3061     | 1226       |
| 1     | 24.09    | 8.33  | B-1  | Left             | Opening 2 |                      |      |        |          | 233        |
| 1     | 27.09    | 8.33  | B-1  | Right            | Opening 2 |                      |      |        |          | 99         |
| 1     | 38.42    | 20.67 | C-1  | Left             | Wall End  | 2273                 |      |        | 2273     | 3675       |

|   |       |       |     |       |           |      |      |      |
|---|-------|-------|-----|-------|-----------|------|------|------|
| 1 | 57.92 | 20.67 | C-1 | Left  | Wall End  | 2273 | 2273 |      |
| 1 | 0.00  | 38.33 | D-2 | Left  | Wall End  | 1176 | 1176 |      |
| 1 | 12.67 | 38.33 | D-2 | Left  | Wall End  | 2352 | 2352 |      |
| 1 | 30.42 | 38.33 | D-2 | Left  | Wall End  | 1176 | 1176 | 1355 |
| 1 | 21.00 | 38.33 | D-2 | Left  | Opening 1 |      |      | 1170 |
| 1 | 26.00 | 38.33 | D-2 | Right | Opening 1 |      |      | 924  |
| 1 | 3.17  | 38.33 | D-1 | Left  | Opening 1 |      |      | 309  |
| 1 | 9.17  | 38.33 | D-1 | Right | Opening 1 |      |      | 14   |
| 1 | 38.42 | 46.33 | E-1 | Left  | Wall End  | 250  | 250  |      |
| 1 | 57.92 | 46.33 | E-1 | Left  | Wall End  | 250  | 250  |      |
| 1 | 41.92 | 46.33 | E-1 | Left  | Opening 1 |      |      | 34   |
| 1 | 44.92 | 46.33 | E-1 | Right | Opening 1 |      |      | 31   |
| 1 | 51.42 | 46.33 | E-1 | Left  | Opening 2 |      |      | 31   |
| 1 | 54.42 | 46.33 | E-1 | Right | Opening 2 |      |      | 34   |

# RIGID DIAPHRAGM DESIGN

## SHEAR RESULTS by SHEARLINE

| Line | Lev | Wall | Wind | Total | Cumulative Shear Force |       |            | Shear Capacity [plf] |       |      |       | Response  |
|------|-----|------|------|-------|------------------------|-------|------------|----------------------|-------|------|-------|-----------|
|      |     | Grp. | Dir. | FHS   | V[lbs]                 | Unit  | V/FHS[plf] | Inter                | Exter | Perf | Total | V/FHS/Tot |
| 1    | 1   | 1    | Both | 36.33 | 2392                   | 65.8  | 65.8       |                      | 280.0 | 1.00 | 280.0 | 0.24      |
| 2    | 1   | 1    | Both | 33.33 | 1950                   | 53.7  | 58.5       |                      | 280.0 | 0.87 | 243.5 | 0.24      |
| 3    | 1   | 1    | Both | 38.00 | 2235                   | 48.2  | 58.8       |                      | 280.0 | 0.71 | 197.6 | 0.30      |
| 4    | 1   | 1    | Both | 42.33 | 3493                   | 75.4  | 82.5       |                      | 280.0 | 1.00 | 280.0 | 0.29      |
| A    | 1   | 1    | W->E | 6.67  | 1227                   | 96.9  | 184.1      |                      | 280.0 | 0.71 | 198.0 | 0.93      |
| B    | 1   | 1    | W->E | 6.75  | 1168                   | 20.2  | 173.1      |                      | 280.0 | 0.68 | 191.5 | 0.90      |
| C    | 1   | 1    | W->E | 19.50 | 4658                   | 80.4  | 238.9      |                      | 280.0 | 1.00 | 280.0 | 0.85      |
| D    | 1   | 1    | W->E | 19.42 | 3084                   | 53.3  | 158.8      |                      | 280.0 | 0.70 | 196.8 | 0.81      |
| E    | 1   | 1    | W->E | 13.50 | 2356                   | 120.8 | 174.5      |                      | 280.0 | 0.79 | 220.6 | 0.79      |

## SHEARWALL SEGMENTS

| Name      | Lev | Type | Wall  | Dimensions [ft] |       | Full Height Perf |                  | Shear [lbs] |          | Opening [ft] |        |
|-----------|-----|------|-------|-----------------|-------|------------------|------------------|-------------|----------|--------------|--------|
|           |     |      | Group | Location        | Start | End              | Sheathing Factor | Force       | Capacity | Width        | Height |
| 1-1       | 1   | II   | 1     | 0.00            | 2.00  | 38.33            | 36.33            | 1.00        | 2392     | 10173        |        |
| 2-1       | 1   | II   | 1     | 12.67           | 2.00  | 8.33             | 6.33             | 1.00        | 370      | 1773         |        |
| 2-2       | 1   | II   | 1     | 12.67           | 8.33  | 38.33            | 27.00            | 0.87        | 1579     | 6573         |        |
| Opening 1 |     |      |       |                 | 20.83 | 23.83            |                  |             |          | 3.00         | 6.67   |
| 3-1       | 1   | II   | 1     | 38.42           | 0.00  | 8.33             | 8.33             | 1.00        | 490      | 2333         |        |
| 3-2       | 1   | II   | 1     | 38.42           | 8.33  | 38.33            | 21.67            | 0.71        | 1274     | 4281         |        |
| Opening 1 |     |      |       |                 | 17.17 | 20.17            |                  |             |          | 3.00         | 6.67   |
| Opening 2 |     |      |       |                 | 24.83 | 27.50            |                  |             |          | 2.67         | 6.67   |
| Opening 3 |     |      |       |                 | 32.83 | 35.50            |                  |             |          | 2.67         | 6.67   |
| 3-3       | 1   | II   | 1     | 38.42           | 38.33 | 46.33            | 8.00             | 1.00        | 470      | 2239         |        |
| 4-1       | 1   | II   | 1     | 57.92           | 0.00  | 46.33            | 42.33            | 1.00        | 3493     | 11852        |        |
| Opening 1 |     |      |       |                 | 21.50 | 25.50            |                  |             |          | 4.00         | 2.00   |
| A-1       | 1   | II   | 1     | 2.00            | 0.00  | 12.67            | 6.67             | 0.71        | 1227     | 1320         |        |
| Opening 1 |     |      |       |                 | 3.33  | 9.33             |                  |             |          | 6.00         | 5.00   |
| B-1       | 1   | II   | 1     | 8.33            | 12.67 | 38.42            | 6.75             | 0.68        | 1168     | 1293         |        |
| Opening 1 |     |      |       |                 | 14.67 | 20.67            |                  |             |          | 6.00         | 5.00   |
| Opening 2 |     |      |       |                 | 24.09 | 27.09            |                  |             |          | 3.00         | 6.67   |
| Opening 3 |     |      |       |                 | 30.42 | 36.42            |                  |             |          | 6.00         | 5.00   |
| C-1       | 1   | II   | 1     | 20.67           | 38.42 | 57.92            | 19.50            | 1.00        | 4658     | 5460         |        |
| D-1       | 1   | II   | 1     | 38.33           | 0.00  | 12.67            | 6.67             | 0.71        | 1059     | 1320         |        |
| Opening 1 |     |      |       |                 | 3.17  | 9.17             |                  |             |          | 6.00         | 5.00   |
| D-2       | 1   | II   | 1     | 38.33           | 12.67 | 38.42            | 12.75            | 0.70        | 2025     | 2509         |        |
| Opening 1 |     |      |       |                 | 21.00 | 26.00            |                  |             |          | 5.00         | 6.67   |
| Opening 2 |     |      |       |                 | 30.42 | 36.42            |                  |             |          | 6.00         | 5.00   |
| E-1       | 1   | II   | 1     | 46.33           | 38.42 | 57.92            | 13.50            | 0.79        | 2356     | 2978         |        |
| Opening 1 |     |      |       |                 | 41.92 | 44.92            |                  |             |          | 3.00         | 5.00   |
| Opening 2 |     |      |       |                 | 51.42 | 54.42            |                  |             |          | 3.00         | 5.00   |

## DRAGSTRUT AND HOLDDOWN FORCES

| Level | Location | Y     | Wall | Position on Wall or Opening |           | Holddown Force [lbs] |      |                 | Dragstrut Force[lbs] |
|-------|----------|-------|------|-----------------------------|-----------|----------------------|------|-----------------|----------------------|
|       | X        | Y     |      |                             |           | Shear                | Dead | Uplift Combined |                      |
| 1     | 0.00     | 2.00  | 1-1  | Left                        | Wall End  | 527                  |      | 527             |                      |
| 1     | 0.00     | 38.33 | 1-1  | Left                        | Wall End  | 527                  |      | 527             |                      |
| 1     | 12.67    | 2.00  | 2-2  | Left                        | Wall End  | 468                  |      | 468             |                      |
| 1     | 12.67    | 8.33  | 2-2  | Left                        | Wall End  | 936                  |      | 936             |                      |
| 1     | 12.67    | 38.33 | 2-2  | Left                        | Wall End  | 468                  |      | 468             |                      |
| 1     | 12.67    | 20.83 | 2-2  | Left                        | Opening 1 |                      |      |                 | 91                   |
| 1     | 12.67    | 23.83 | 2-2  | Right                       | Opening 1 |                      |      |                 | 70                   |
| 1     | 38.42    | 0.00  | 3-3  | Left                        | Wall End  | 470                  |      | 470             |                      |
| 1     | 38.42    | 8.33  | 3-3  | Left                        | Wall End  | 941                  |      | 941             |                      |
| 1     | 38.42    | 38.33 | 3-3  | Left                        | Wall End  | 941                  |      | 941             |                      |
| 1     | 38.42    | 46.33 | 3-3  | Left                        | Wall End  | 470                  |      | 470             |                      |
| 1     | 38.42    | 17.17 | 3-2  | Left                        | Opening 1 |                      |      |                 | 182                  |
| 1     | 38.42    | 20.17 | 3-2  | Right                       | Opening 1 |                      |      |                 | 37                   |
| 1     | 38.42    | 24.83 | 3-2  | Left                        | Opening 2 |                      |      |                 | 86                   |
| 1     | 38.42    | 27.50 | 3-2  | Right                       | Opening 2 |                      |      |                 | 42                   |
| 1     | 38.42    | 32.83 | 3-2  | Left                        | Opening 3 |                      |      |                 | 14                   |
| 1     | 38.42    | 35.50 | 3-2  | Right                       | Opening 3 |                      |      |                 | 115                  |
| 1     | 57.92    | 0.00  | 4-1  | Left                        | Wall End  | 660                  |      | 660             |                      |

|   |       |       |     |       |           |      |  |      |      |
|---|-------|-------|-----|-------|-----------|------|--|------|------|
| 1 | 57.92 | 46.33 | 4-1 | Left  | Wall End  | 660  |  | 660  |      |
| 1 | 57.92 | 21.50 | 4-1 | Left  | Opening 1 |      |  |      | 153  |
| 1 | 57.92 | 25.50 | 4-1 | Right | Opening 1 |      |  |      | 148  |
| 1 | 0.00  | 2.00  | A-1 | Left  | Wall End  | 1472 |  | 1472 |      |
| 1 | 12.67 | 2.00  | A-1 | Left  | Wall End  | 1472 |  | 1472 | 0    |
| 1 | 3.33  | 2.00  | A-1 | Left  | Opening 1 |      |  |      | 291  |
| 1 | 9.33  | 2.00  | A-1 | Right | Opening 1 |      |  |      | 291  |
| 1 | 20.67 | 8.33  | B-1 | Left  | Wall End  | 1385 |  | 1385 | 417  |
| 1 | 30.42 | 8.33  | B-1 | Left  | Wall End  | 1385 |  | 1385 | 555  |
| 1 | 24.09 | 8.33  | B-1 | Left  | Opening 2 |      |  |      | 105  |
| 1 | 27.09 | 8.33  | B-1 | Right | Opening 2 |      |  |      | 45   |
| 1 | 38.42 | 20.67 | C-1 | Left  | Wall End  | 1911 |  | 1911 | 3090 |
| 1 | 57.92 | 20.67 | C-1 | Left  | Wall End  | 1911 |  | 1911 |      |
| 1 | 0.00  | 38.33 | D-2 | Left  | Wall End  | 1271 |  | 1271 |      |
| 1 | 12.67 | 38.33 | D-2 | Left  | Wall End  | 2541 |  | 2541 |      |
| 1 | 30.42 | 38.33 | D-2 | Left  | Wall End  | 1271 |  | 1271 | 1464 |
| 1 | 21.00 | 38.33 | D-2 | Left  | Opening 1 |      |  |      | 1264 |
| 1 | 26.00 | 38.33 | D-2 | Right | Opening 1 |      |  |      | 998  |
| 1 | 3.17  | 38.33 | D-1 | Left  | Opening 1 |      |  |      | 334  |
| 1 | 9.17  | 38.33 | D-1 | Right | Opening 1 |      |  |      | 15   |
| 1 | 38.42 | 46.33 | E-1 | Left  | Wall End  | 1396 |  | 1396 |      |
| 1 | 57.92 | 46.33 | E-1 | Left  | Wall End  | 1396 |  | 1396 |      |
| 1 | 41.92 | 46.33 | E-1 | Left  | Opening 1 |      |  |      | 188  |
| 1 | 44.92 | 46.33 | E-1 | Right | Opening 1 |      |  |      | 175  |
| 1 | 51.42 | 46.33 | E-1 | Left  | Opening 2 |      |  |      | 175  |
| 1 | 54.42 | 46.33 | E-1 | Right | Opening 2 |      |  |      | 188  |

#### COMPONENT AND CLADDING DESIGN

| COMPONENTS AND CLADDING by SHEARLINE |       |            |                 |      |      | Fasteners [lbs] |     |     |          |      |
|--------------------------------------|-------|------------|-----------------|------|------|-----------------|-----|-----|----------|------|
| Name                                 | Level | Wall Group | Sheathing [psf] |      |      | Force Capacity  |     |     | Response |      |
|                                      |       |            | Force           | Cap. | Resp | End             | Int |     | End      | Int  |
| 1                                    | 1     | 1          | 23.9            | 30.0 | 0.80 | 32              | 31  | 107 | 0.30     | 0.29 |
| 2                                    | 1     | 1          | 23.9            | 30.0 | 0.80 |                 |     |     |          |      |
| 3                                    | 1     | 1          | 23.9            | 30.0 | 0.80 |                 |     |     |          |      |
| 4                                    | 1     | 1          | 23.9            | 30.0 | 0.80 | 32              | 31  | 107 | 0.30     | 0.29 |
| A                                    | 1     | 1          | 29.1            | 30.0 | 0.97 | 39              | 31  | 107 | 0.36     | 0.29 |
| B                                    | 1     | 1          | 29.1            | 30.0 | 0.97 | 39              | 31  | 107 | 0.36     | 0.29 |
| D                                    | 1     | 1          | 29.1            | 30.0 | 0.97 | 39              | 31  | 107 | 0.36     | 0.29 |
| E                                    | 1     | 1          | 29.1            | 30.0 | 0.97 | 39              | 31  | 107 | 0.36     | 0.29 |

(BASED ON FLORIDA BUILDING CODE - FBC 2004 )

| GCp Coefficients Table |                 |                      |
|------------------------|-----------------|----------------------|
|                        | <u>End Zone</u> | <u>Interior Zone</u> |
| <u>WINDWARD</u>        | Wall (5E)       | 0.5                  |
|                        | Roof (2E)       | -1.4                 |
|                        | Overhang        | -1.5                 |
| <u>LEEWARD</u>         | Roof (3E)       | -0.8                 |
|                        | Wall (6E)       | -0.7                 |
|                        | Overhang        | -0.8                 |

|          | Building Dimensions (ft) |             |          |
|----------|--------------------------|-------------|----------|
|          | Width                    | Eave Height | Overhang |
| Windward | 23.2                     | 8           | 0        |
| Leeward  | 23.2                     | 8           | 0        |

| Design Data           |       |
|-----------------------|-------|
| FBC Wind Speed, (mph) | 110   |
| Wind Pressure (psf)   | 24.73 |
| Roof Height (ft)      | 13.8  |
| Mean Roof Height (ft) | 15.0  |
| End Zone (ft)         | 6     |
| Roof Pitch, s/12      | 3     |
| Bent Spacing (ft)     | 1.33  |
| Roof Dead Load (psf)  | 10    |
| Wall Dead Load (psf)  | 0     |
| Floor Dead Load (psf) | 0     |

| WIND PRESSURES (psf) |               |          | WIND LOADS (pf) |           |               | FRAME LOADING |                 |                  |                 |
|----------------------|---------------|----------|-----------------|-----------|---------------|---------------|-----------------|------------------|-----------------|
| End Zone             | Interior Zone | End Zone | Interior Zone   | End Zone  | Interior Zone | End Zone      |                 | Interior Zone    |                 |
|                      |               |          |                 |           |               | Forces (lb)   | Moments (ft-lb) | Forces (lb)      | Moments (ft-lb) |
| Wall (1E)            | 12.3640368    | Wall (1) | 6.18201839      | Wall (1E) | 16.4441689    | Wall (1)      | 8.22208446      | Wall (1) - X dir | 131.55          |
| Roof (2E)            | 34.619303     | Roof (2) | 24.7280736      | Roof (2E) | 45.043673     | Roof (2)      | 32.8883378      | Roof (2) - X dir | 526.21          |
| Overhang             | 0             | Overhang | 0               | Overhang  | 0             | Overhang      | 0               | Roof (2) - Y dir | 267.05          |
|                      |               |          |                 |           |               |               |                 |                  | 2910.88         |
|                      |               |          |                 |           |               |               |                 |                  | 1068.21         |
|                      |               |          |                 |           |               |               |                 |                  | 37173.82        |
|                      |               |          |                 |           |               |               |                 |                  | 763.01          |
|                      |               |          |                 |           |               |               |                 |                  | 26552.73        |
| Roof (3E)            | 19.7824588    | Roof (3) | 16.0732478      | Roof (3E) | 26.3106703    | Roof (3)      | 21.3774196      | Wall (4) - X dir | 184.17          |
| Wall (4E)            | 17.3096515    | Wall (4) | 13.6004405      | Wall (4E) | 23.0218365    | Wall (4)      | 18.0885858      | Roof (3) - X dir | 736.70          |
| Overhang             | 0             | Overhang | 0               | Overhang  | 0             | Overhang      | 0               | Roof (3) - Y dir | 144.71          |
|                      |               |          |                 |           |               |               |                 |                  | 152.60          |
|                      |               |          |                 |           |               |               |                 |                  | 1663.36         |
|                      |               |          |                 |           |               |               |                 |                  | 123.99          |
|                      |               |          |                 |           |               |               |                 |                  | 7080.73         |
|                      |               |          |                 |           |               |               |                 |                  | 495.96          |
|                      |               |          |                 |           |               |               |                 |                  | 5753.09         |

**CALCULATION RESULTS:**

1) Uplift (lb) at Truss/Rafter Connection

|                      |     |
|----------------------|-----|
| <u>End Zone</u>      | 741 |
| <u>Interior Zone</u> | 486 |

RE: HBFWSP -

**Trenco**

818 Soundside Rd  
Edenton, NC 27932

**Site Information:**

Project Customer: HARVEY BUILDERS Project Name:  
Lot/Block: Subdivision: HOLLINGSWORTH ESTATES  
Address: OFF HWY 27  
City: FT. WHITE State: FL

**Name Address and License # of Structural Engineer of Record, If there is one, for the building.**

Name: License #:  
Address:  
City: State:

**General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):**

Design Code: FBC2004/TPI2002 Design Program: MiTek 20/20 6.5  
Wind Code: ASCE 7-02 Wind Speed: 110 mph Floor Load: N/A psf  
Roof Load: 40.0 psf

This package includes 14 individual, dated Truss Design Drawings and 0 Additional Drawings.  
With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

| No. | Seal#    | Job ID# | Truss Name | Date    |
|-----|----------|---------|------------|---------|
| 1   | E3885228 | HBFWSP  | A          | 2/23/07 |
| 2   | E3885229 | HBFWSP  | AET        | 2/23/07 |
| 3   | E3885230 | HBFWSP  | AET1       | 2/23/07 |
| 4   | E3885231 | HBFWSP  | B          | 2/23/07 |
| 5   | E3885232 | HBFWSP  | BET        | 2/23/07 |
| 6   | E3885233 | HBFWSP  | C          | 2/23/07 |
| 7   | E3885234 | HBFWSP  | CET        | 2/23/07 |
| 8   | E3885235 | HBFWSP  | V1         | 2/23/07 |
| 9   | E3885236 | HBFWSP  | V2         | 2/23/07 |
| 10  | E3885237 | HBFWSP  | V3         | 2/23/07 |
| 11  | E3885238 | HBFWSP  | V4         | 2/23/07 |
| 12  | E3885239 | HBFWSP  | V5         | 2/23/07 |
| 13  | E3885240 | HBFWSP  | V6         | 2/23/07 |
| 14  | E3885241 | HBFWSP  | V7         | 2/23/07 |

The truss drawing(s) referenced above have been prepared by TRENCO under my direct supervision based on the parameters provided by Santa Fe Truss.

Truss Design Engineer's Name: Strzyzewski, Marvin  
My license renewal date for the state of is February 28, 2009.

**NOTE:** The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-2002 Chapter 2.



Marvin A. Strzyzewski, FL Lic. #743144  
Truss Engineering Company  
818 Soundside Rd.  
Edenton, NC 27932  
FL Cert. #7239

|               |            |                          |           |          |                                      |
|---------------|------------|--------------------------|-----------|----------|--------------------------------------|
| Job<br>HBFWSF | Truss<br>A | Truss Type<br>ROOF TRUSS | Qty<br>13 | Ply<br>1 | Job Reference (optional)<br>E3885228 |
|---------------|------------|--------------------------|-----------|----------|--------------------------------------|

SANTA FE TRUSS, HIGH SPRINGS, FL.

6.500 s Feb 5 2007 MiTek Industries, Inc. Fri Feb 23 11:16:12 2007 Page 1

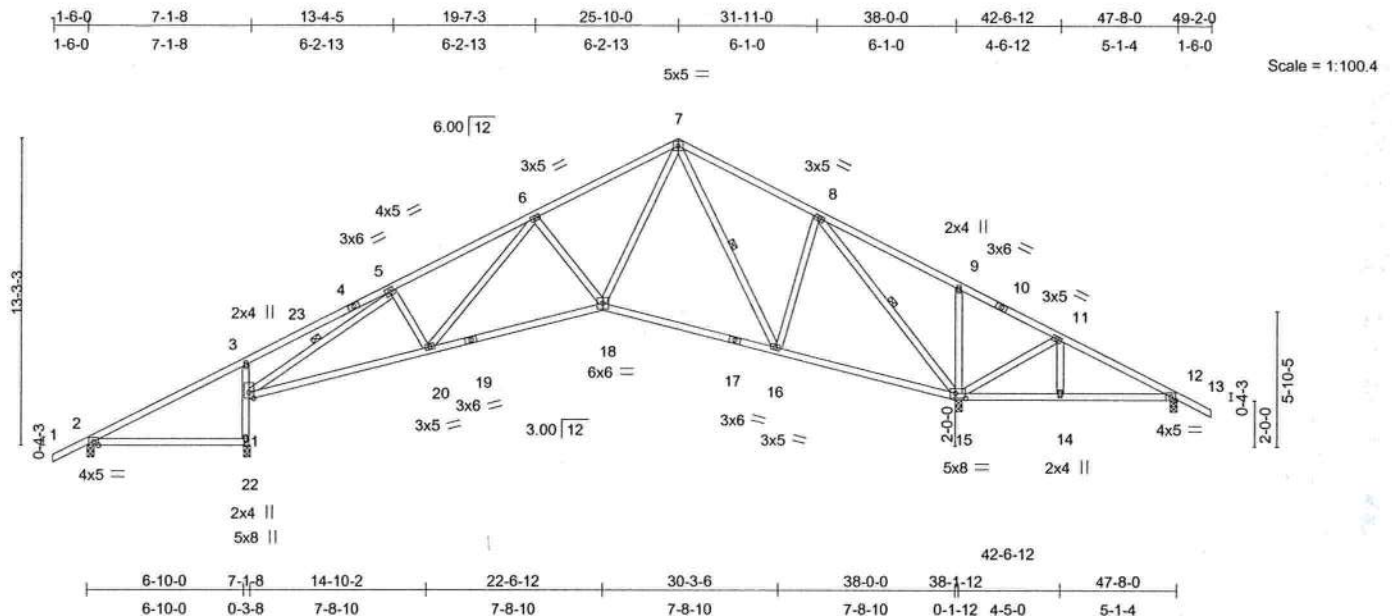


Plate Offsets (X,Y): [15:0-5-4,0-2-8]

| LOADING (psf) | SPACING              | 2-0-0 | CSI      | DEFL           | in (loc) | l/defl | L/d | PLATES         | GRIP    |
|---------------|----------------------|-------|----------|----------------|----------|--------|-----|----------------|---------|
| TCLL 20.0     | Plates Increase 1.25 |       | TC 0.43  | Vert(LL) 0.12  | 2-22     | >652   | 240 | MT20           | 244/190 |
| TCDL 10.0     | Lumber Increase 1.25 |       | BC 0.63  | Vert(TL) -0.38 | 20-21    | >984   | 180 |                |         |
| BCLL 0.0      | Rep Stress Incr YES  |       | WB 0.97  | Horz(TL) 0.18  | 15       | n/a    | n/a |                |         |
| BCDL 10.0     | Code FBC2004/TPI2002 |       | (Matrix) |                |          |        |     |                |         |
|               |                      |       |          |                |          |        |     | Weight: 261 lb |         |

#### LUMBER

TOP CHORD 2 X 4 SYP No.2D  
BOT CHORD 2 X 4 SYP No.2D  
WEBS 2 X 4 SYP No.3

#### BRACING

TOP CHORD Structural wood sheathing directly applied or 4-4-5 oc purlins.  
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.  
WEBS 1 Row at midpt 5-21, 7-16, 8-15

**REACTIONS** (lb/size) 2=332/0-3-8, 22=1436/0-3-8, 15=2177/0-3-8, 12=42/0-3-8  
Max Horz 2=208(LC 4)  
Max Uplift 2=-229(LC 5), 22=-411(LC 5), 15=-513(LC 6), 12=-264(LC 6)  
Max Grav 2=334(LC 9), 22=1436(LC 1), 15=2177(LC 1), 12=177(LC 10)

#### FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/39, 2-3=-178/197, 3-23=-70/213, 4-23=-44/224, 4-5=-33/238, 5-6=-2028/317, 6-7=-1622/247, 7-8=-783/219,  
8-9=-44/983, 9-10=-118/965, 10-11=-135/858, 11-12=0/593, 12-13=0/39  
BOT CHORD 2-22=-12/8, 20-21=-131/1785, 19-20=-49/1665, 18-19=-39/1698, 17-18=0/907, 16-17=0/873, 15-16=0/504,  
14-15=-493/27, 12-14=-493/27  
WEBS 5-21=-2164/165, 5-20=0/189, 6-20=-97/268, 6-18=-461/216, 7-18=-66/1224, 7-16=-548/75, 8-16=0/575, 8-15=-2067/210,  
9-15=-330/196, 11-15=-396/338, 11-14=-99/169, 21-22=-1367/358, 3-21=-439/322

#### NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 229 lb uplift at joint 2, 411 lb uplift at joint 22, 513 lb uplift at joint 15 and 264 lb uplift at joint 12.

LOAD CASE(S) Standard

*Marianne Stapp*

February 23, 2007

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.**

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

ENGINEERING BY  
**TRENCO**  
A MiTek Affiliate

818 Soundside Road  
Edenton, NC 27932

|        |       |            |     |     |          |
|--------|-------|------------|-----|-----|----------|
| Job    | Truss | Truss Type | Qty | Ply |          |
| HBFWSP | AET   | GABLE      | 1   | 1   |          |
|        |       |            |     |     | E3885229 |

SANTA FE TRUSS, HIGH SPRINGS, FL.

Job Reference (optional)

6.500 s Feb 5 2007 MiTek Industries, Inc. Fri Feb 23 11:16:16 2007 Page 1

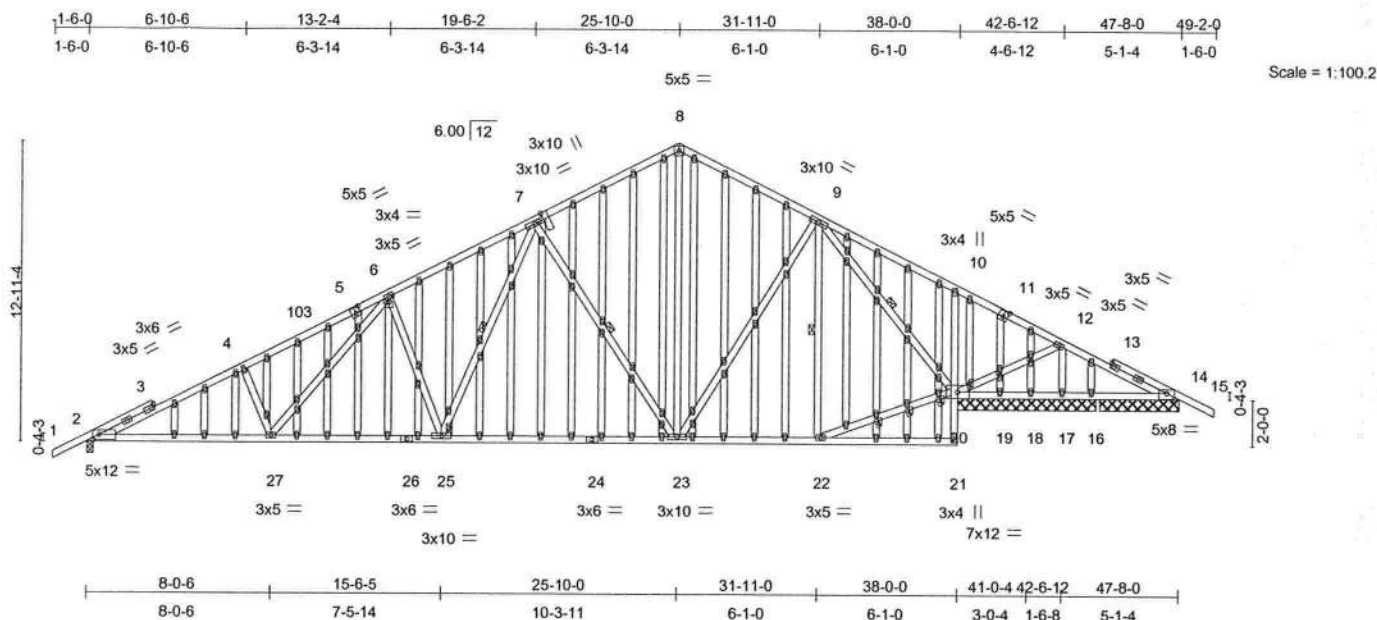


Plate Offsets (X,Y): [2:0-3-4,0-2-12], [5:0-2-8,0-3-0], [6:0-2-0,0-0-0], [10:1-0-12,0-1-0], [11:0-2-8,0-3-0], [14:0-4-0,0-3-1], [37:0-3-8,Edge], [88:0-2-0,0-1-0], [92:0-2-0,0-1-0], [96:0-2-0,0-1-0], [97:0-2-0,0-0-0], [99:0-1-12,0-1-0]

| LOADING (psf) | SPACING              | 2:0-0 | CSI      | DEFL     | in (loc)    | l/defl | L/d | PLATES | GRIP           |
|---------------|----------------------|-------|----------|----------|-------------|--------|-----|--------|----------------|
| TCLL 20.0     | Plates Increase      | 1.25  | TC 0.69  | Vert(LL) | 0.39 23-25  | >999   | 240 | MT20   | 244/190        |
| TCDL 10.0     | Lumber Increase      | 1.25  | BC 0.72  | Vert(TL) | -0.66 23-25 | >685   | 180 |        |                |
| BCLL 0.0      | Rep Stress Incr      | YES   | WB 0.97  | Horz(TL) | 0.06 20     | n/a    | n/a |        |                |
| BCDL 10.0     | Code FBC2004/TPI2002 |       | (Matrix) |          |             |        |     |        |                |
|               |                      |       |          |          |             |        |     |        | Weight: 603 lb |

#### LUMBER

TOP CHORD 2 X 4 SYP No.2D  
 BOT CHORD 2 X 4 SYP No.2D \*Except\*  
 10-21 2 X 4 SYP No.3  
 WEBS 2 X 4 SYP No.3 \*Except\*  
 8-23 2 X 4 SYP No.2  
 OTHERS 2 X 4 SYP No.3

#### BRACING

TOP CHORD Structural wood sheathing directly applied or 2-9-3 oc purlins.  
 BOT CHORD Rigid ceiling directly applied or 5-2-15 oc bracing. Except:  
 6-0-0 oc bracing: 10-20  
 1 Row at midpt 7-25, 7-23, 9-22, 9-20

#### REACTIONS

(lb/size) 2=1521/0-3-8, 20=2304/6-0-8, 14=138/3-5-8, 17=-116/6-0-8, 19=16/6-0-8, 18=39/6-0-8, 16=87/6-0-8  
 Max Horz 2=304(LC 4)  
 Max Uplift 2=-845(LC 5), 20=-1188(LC 5), 14=-92(LC 6), 17=-285(LC 9)  
 Max Grav 2=1521(LC 1), 20=2304(LC 1), 14=190(LC 10), 17=196(LC 4), 19=67(LC 2), 18=61(LC 2), 16=143(LC 2)

#### FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/39, 2-3=-2867/1402, 3-4=-2799/1420, 4-103=-2740/1452, 5-103=-2646/1464, 5-6=-2586/1477, 6-7=-1935/1048,  
 7-8=-1007/607, 8-9=-1002/607, 9-10=-358/825, 10-11=-443/827, 11-12=-462/715, 12-13=-194/389, 13-14=-205/314,  
 14-15=0/39  
 BOT CHORD 2-27=-1420/2536, 26-27=-955/1879, 25-26=-955/1879, 24-25=-607/1327, 23-24=-607/1327, 22-23=-186/516,  
 21-22=-61/43, 20-21=-47/122, 10-20=-339/210, 19-20=-291/241, 18-19=-291/241, 17-18=-291/241, 16-17=-291/241,  
 14-16=-291/241  
 WEBS 4-27=-463/325, 6-27=-547/770, 6-25=-639/390, 7-25=-548/849, 7-23=-915/533, 8-23=-390/505, 9-23=-261/616,  
 9-22=-73/84, 20-22=-137/539, 9-20=-1922/983, 12-20=-431/336, 12-17=-196/286

#### NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- All plates are 2x4 MT20 unless otherwise indicated.
- This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- Gable studs spaced at 1-4-0 oc.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 845 lb uplift at joint 2, 1188 lb uplift at joint 20, 92 lb uplift at joint 14 and 285 lb uplift at joint 17.

LOAD CASE(S) Standard

*Manning*

February 23, 2007

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.**

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

ENGINEERING BY  
**TRENCO**  
 A MiTek Affiliate

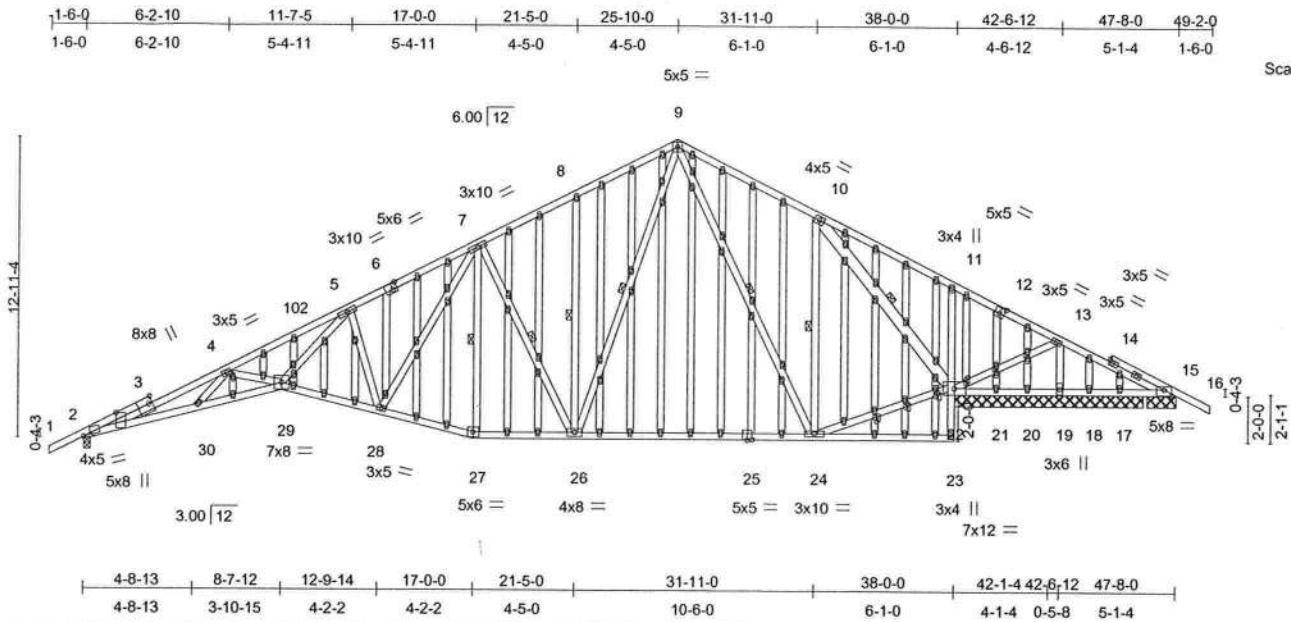
818 Soundside Road  
 Edenton, NC 27932

|        |       |            |     |     |          |
|--------|-------|------------|-----|-----|----------|
| Job    | Truss | Truss Type | Qty | Ply | E3885230 |
| HBFWSP | AET1  | GABLE      | 1   | 1   |          |

SANTA FE TRUSS, HIGH SPRINGS, FL.

Job Reference (optional)

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Scale = 1:100.2

Plate Offsets (X,Y): [2:0-3-13,0-0-1], [2:1-0-5,Edge], [3:0-3-8,Edge], [4:0-1-14,0-1-0], [6:0-2-4,0-3-4], [12:0-2-8,0-3-0], [15:0-4-0,0-3-1], [25:0-2-8,0-0-4], [84:0-2-0,0-1-0], [89:0-2-0,0-1-0], [94:0-2-0,0-1-0], [95:0-2-0,0-0-0], [97:0-1-12,0-1-0], [99:0-1-12,0-1-0]

| LOADING (psf) | SPACING              | CSI      | DEFL           | in (loc) | l/defl | L/d | PLATES         | GRIP    |
|---------------|----------------------|----------|----------------|----------|--------|-----|----------------|---------|
| TCLL 20.0     | Plates Increase 1.25 | TC 0.96  | Vert(LL) 0.54  | 29       | >842   | 240 | MT20           | 244/190 |
| TCDL 10.0     | Lumber Increase 1.25 | BC 0.99  | Vert(TL) -0.85 | 29       | >531   | 180 |                |         |
| BCLL 0.0      | Rep Stress Incr YES  | WB 0.92  | Horz(TL) 0.30  | 22       | n/a    | n/a |                |         |
| BCDL 10.0     | Code FBC2004/TPI2002 | (Matrix) |                |          |        |     |                |         |
|               |                      |          |                |          |        |     | Weight: 581 lb |         |

#### LUMBER

TOP CHORD 2 X 4 SYP No.2D  
 BOT CHORD 2 X 4 SYP No.2D \*Except\*  
 11-23 2 X 4 SYP No.3  
 WEBS 2 X 4 SYP No.3 \*Except\*  
 9-26 2 X 4 SYP No.2, 9-24 2 X 4 SYP No.2, 10-22 2 X 6 SYP No.2  
 OTHERS 2 X 4 SYP No.3

#### BRACING

TOP CHORD Structural wood sheathing directly applied.  
 BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing. Except:  
 6-0-0 oc bracing: 11-22  
 1 Row at midpt 7-27, 7-26, 8-26, 9-26, 9-24, 10-24, 10-22

#### REACTIONS

(lb/size) 2=1332/0-3-8, 22=3751/8-2-8, 15=-158/1-3-8, 19=-1008/8-2-8, 21=-55/8-2-8, 20=60/8-2-8, 18=23/8-2-8, 17=40/8-2-8  
 Max Horz 2=255(LC 4)  
 Max Uplift 2=-747(LC 5), 22=-1940(LC 5), 15=-295(LC 9), 19=-1082(LC 9), 21=-55(LC 1), 20=-10(LC 5), 17=-23(LC 3)  
 Max Grav 2=1332(LC 1), 22=3751(LC 1), 15=151(LC 5), 19=648(LC 5), 21=77(LC 5), 20=75(LC 2), 18=46(LC 2), 17=72(LC 2)

#### FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/38, 2-3=-5112/2668, 3-4=-5044/2683, 4-102=-4343/2217, 5-102=-4254/2239, 5-6=-2069/1083, 6-7=-1996/1100, 7-8=-951/580, 8-9=-934/648, 9-10=-170/266, 10-11=-1259/2538, 11-12=-1344/2541, 12-13=-1363/2471, 13-14=-561/1076, 14-15=-579/965, 15-16=0/40  
 BOT CHORD 2-30=-2583/4712, 29-30=-2488/4641, 28-29=-1041/2219, 27-28=-488/1186, 26-27=-483/1143, 25-26=-103/414, 24-25=-103/414, 23-24=-66/13, 22-23=-25/65, 11-22=-341/211, 21-22=-915/566, 20-21=-915/566, 19-20=-915/566, 18-19=-915/566, 17-18=-915/566, 15-17=-915/566  
 WEBS 4-30=-109/251, 4-29=-705/513, 5-29=-1399/2561, 5-28=-1292/705, 7-28=-723/1274, 7-27=-240/85, 7-26=-767/460, 8-26=-243/109, 9-26=-692/1163, 9-24=-1179/518, 10-24=-670/1308, 22-24=-158/192, 10-22=-3411/1779, 13-22=-1437/852, 13-19=-630/1100

#### NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- 7) Gable studs spaced at 1-4-0 oc.
- 8) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity

Continued on page 2

*Mani Shrivastava*

February 23, 2007

#### WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIT-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

ENGINEERING BY  
**TRENCO**  
 A MiTek Affiliate

818 Soundside Road  
 Edenton, NC 27932

| Job    | Truss | Truss Type | Qty | Ply |          |
|--------|-------|------------|-----|-----|----------|
| HBFWSP | AET1  | GABLE      | 1   | 1   | E3885230 |

SANTA FE TRUSS, HIGH SPRINGS, FL.

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#### NOTES

- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 747 lb uplift at joint 2, 1940 lb uplift at joint 22, 295 lb uplift at joint 15, 1082 lb uplift at joint 19, 55 lb uplift at joint 21, 10 lb uplift at joint 20 and 23 lb uplift at joint 17.
- 10) Uplift for first LC exceeds limits

**LOAD CASE(S)** Standard

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.**

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ENGINEERING BY  
**TRENCO**  
 A MiTek Affiliate

818 Soundside Road  
 Edenton, NC 27932

|        |       |            |     |     |                          |
|--------|-------|------------|-----|-----|--------------------------|
| Job    | Truss | Truss Type | Qty | Ply | Job Reference (optional) |
| HBFWSP | B     | ROOF TRUSS | 5   | 1   |                          |

E3885231

SANTA FE TRUSS, HIGH SPRINGS, FL

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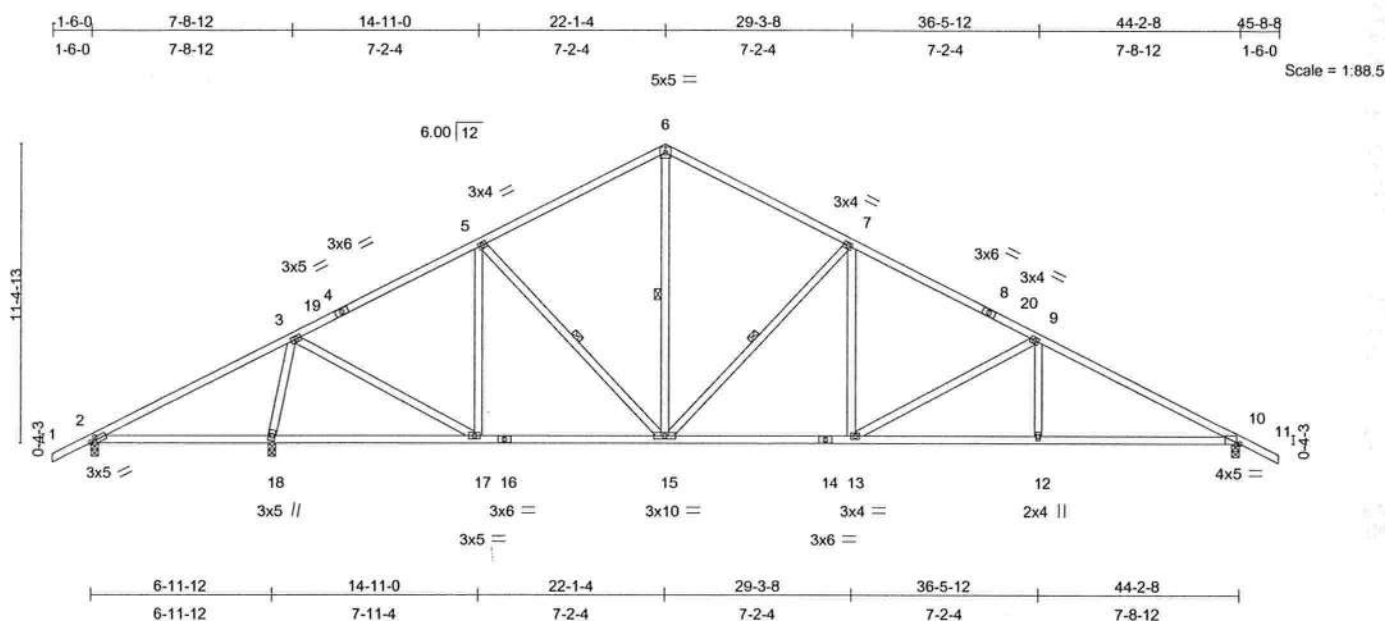


Plate Offsets (X,Y): [2:0-2-10,0-1-8], [10:0-1-4,0-0-2]

| LOADING (psf) | SPACING              | CSI      | DEFL           | in (loc) | l/defl | L/d | PLATES         | GRIP    |
|---------------|----------------------|----------|----------------|----------|--------|-----|----------------|---------|
| TCLL 20.0     | Plates Increase 1.25 | TC 0.49  | Vert(LL) 0.21  | 10-12    | >999   | 240 | MT20           | 244/190 |
| TCDL 10.0     | Lumber Increase 1.25 | BC 0.58  | Vert(TL) -0.30 | 10-12    | >999   | 180 |                |         |
| BCLL 0.0      | Rep Stress Incr YES  | WB 0.78  | Horz(TL) 0.08  | 10       | n/a    | n/a |                |         |
| BCDL 10.0     | Code FBC2004/TPI2002 | (Matrix) |                |          |        |     |                |         |
|               |                      |          |                |          |        |     | Weight: 246 lb |         |

**LUMBER**

TOP CHORD 2 X 4 SYP No.2D  
 BOT CHORD 2 X 4 SYP No.2D  
 WEBS 2 X 4 SYP No.3

**BRACING**

TOP CHORD Structural wood sheathing directly applied or 3-6-2 oc purlins.  
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.  
 WEBS 1 Row at midpt 5-15, 6-15, 7-15

**REACTIONS** (lb/size) 2=25/0-3-8, 18=2166/0-3-8, 10=1520/0-3-8

Max Horz 2=154(LC 4)

Max Uplift 2=127(LC 10), 18=1103(LC 5), 10=856(LC 6)

Max Grav 2=136(LC 9), 18=2166(LC 1), 10=1520(LC 1)

**FORCES** (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/39, 2-3=-375/803, 3-19=-1245/619, 4-19=-1216/621, 4-5=-1143/648, 5-6=-1324/751, 6-7=-1324/751,  
 7-8=-1863/1016, 8-20=-1933/989, 9-20=-1963/988, 9-10=-2631/1323, 10-11=0/39

BOT CHORD 2-18=-613/432, 17-18=-256/248, 16-17=-337/1023, 15-16=-337/1023, 14-15=-639/1666, 13-14=-639/1666,  
 12-13=-1028/2255, 10-12=-1028/2255

WEBS 3-18=-2015/979, 3-17=-591/1396, 5-17=-531/176, 5-15=-75/246, 6-15=-491/666, 7-15=-837/500, 7-13=-335/523,  
 9-13=-670/489, 9-12=-135/320

**NOTES**

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 127 lb uplift at joint 2, 1103 lb uplift at joint 18 and 856 lb uplift at joint 10.

LOAD CASE(S) Standard

February 23, 2007

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.**

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

ENGINEERING BY  
**TRENCO**  
 A MiTek Affiliate

818 Soundside Road  
 Edenon, NC 27932

|        |       |            |     |     |                          |          |
|--------|-------|------------|-----|-----|--------------------------|----------|
| Job    | Truss | Truss Type | Qty | Ply | Job Reference (optional) | E3885232 |
| HBFWSF | BET   | GABLE      | 1   | 1   |                          |          |

SANTA FE TRUSS, HIGH SPRINGS, FL

6.500 s Feb 5 2007 Mitek Industries, Inc. Fri Feb 23 11:16:24 2007 Page 1

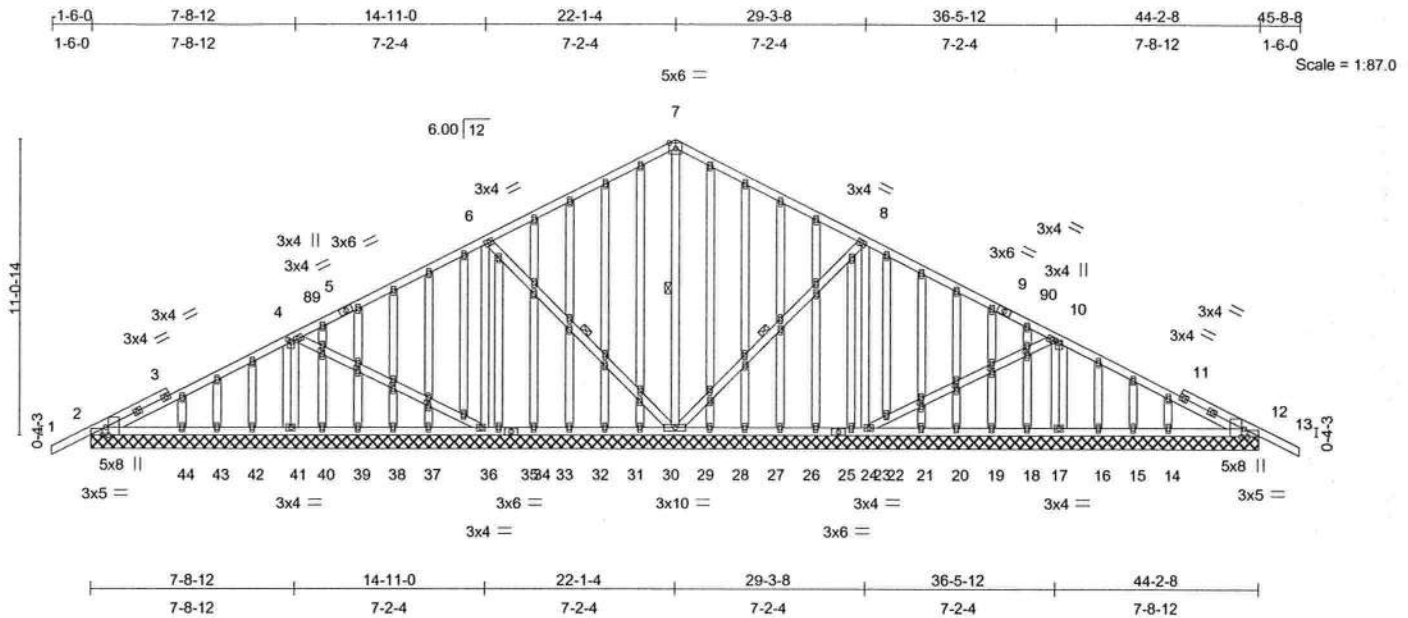


Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-1-8,Edge], [4:0-1-4,0-1-8], [10:0-1-4,0-1-8], [12:0-1-8,Edge], [12:0-3-8,Edge], [56:0-1-12,0-1-0], [58:0-1-12,0-1-0], [60:0-1-12,0-1-0], [62:0-1-12,0-1-0], [68:0-0-0,0-0-0], [68:0-0-0,0-0-0], [70:0-0-0,0-0-0], [70:0-0-0,0-0-0], [72:0-0-0,0-0-0], [72:0-0-0,0-0-0], [74:0-0-0,0-0-0], [74:0-0-0,0-0-0], [79:0-1-12,0-1-0], [81:0-1-12,0-1-0], [83:0-1-12,0-1-0], [85:0-1-12,0-1-0]

| LOADING (psf) | SPACING              | 2-0-0 | CSI      | DEFL     | in (loc) | l/defl | L/d | PLATES | GRIP           |
|---------------|----------------------|-------|----------|----------|----------|--------|-----|--------|----------------|
| TCLL 20.0     | Plates Increase      | 1.25  | TC 0.37  | Vert(LL) | 0.03     | 13     | n/r | 120    | MT20           |
| TCDL 10.0     | Lumber Increase      | 1.25  | BC 0.26  | Vert(TL) | 0.05     | 13     | n/r | 120    | 244/190        |
| BCLL 0.0      | Rep Stress Incr      | YES   | WB 0.36  | Horz(TL) | 0.01     | 12     | n/a | n/a    |                |
| BCDL 10.0     | Code FBC2004/TPI2002 |       | (Matrix) |          |          |        |     |        | Weight: 473 lb |

#### LUMBER

TOP CHORD 2 X 4 SYP No.2D  
BOT CHORD 2 X 4 SYP No.2D  
WEBS 2 X 4 SYP No.3  
OTHERS 2 X 4 SYP No.3

#### BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.  
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.  
WEBS 1 Row at midpt 6-29, 7-29, 8-29

**REACTIONS (lb/size)** 2=203/44-2-8, 41=609/44-2-8, 36=338/44-2-8, 29=598/44-2-8, 22=338/44-2-8, 17=609/44-2-8, 30=27/44-2-8, 31=27/44-2-8, 32=26/44-2-8, 33=29/44-2-8, 35=6/44-2-8, 37=38/44-2-8, 38=22/44-2-8, 39=28/44-2-8, 40=24/44-2-8, 42=56/44-2-8, 43=74/44-2-8, 44=201/44-2-8, 28=27/44-2-8, 27=27/44-2-8, 26=26/44-2-8, 25=29/44-2-8, 23=6/44-2-8, 21=38/44-2-8, 20=22/44-2-8, 19=28/44-2-8, 18=24/44-2-8, 16=56/44-2-8, 15=74/44-2-8, 14=201/44-2-8, 12=203/44-2-8

Max Horz 2=-149(LC 3)

Max Uplift 2=-81(LC 5), 41=-304(LC 5), 36=-99(LC 5), 29=-209(LC 5), 22=-98(LC 5), 17=-294(LC 6), 43=-75(LC 9), 44=-70(LC 5), 15=-75(LC 10), 14=-73(LC 6), 12=-98(LC 6)

Max Grav 2=204(LC 9), 41=617(LC 9), 36=357(LC 9), 29=598(LC 1), 22=357(LC 10), 17=617(LC 10), 30=53(LC 2), 31=54(LC 2), 32=52(LC 2), 33=58(LC 2), 35=13(LC 2), 37=76(LC 2), 38=45(LC 2), 39=57(LC 2), 40=44(LC 2), 42=78(LC 2), 43=64(LC 5), 44=203(LC 9), 28=53(LC 2), 27=54(LC 2), 26=52(LC 2), 25=58(LC 2), 23=13(LC 2), 21=76(LC 2), 20=45(LC 2), 19=57(LC 2), 18=44(LC 2), 16=78(LC 2), 15=66(LC 6), 14=203(LC 10), 12=204(LC 10)

#### FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/39, 2-3=-201/158, 3-4=-181/281, 4-89=-89/69, 5-89=-59/71, 5-6=-19/119, 6-7=-57/165, 7-8=-57/165, 8-9=0/119, 9-90=-59/27, 10-90=-89/25, 10-11=-137/281, 11-12=-157/158, 12-13=0/39  
BOT CHORD 2-44=-158/205, 43-44=-158/205, 42-43=-158/205, 41-42=-158/205, 40-41=-158/205, 39-40=-158/205, 38-39=-158/205, 37-38=-158/205, 36-37=-158/205, 35-36=-24/229, 34-35=-24/229, 33-34=-24/229, 32-33=-24/229, 31-32=-24/229, 30-31=-24/229, 29-30=-24/229, 28-29=-24/245, 27-28=-24/245, 26-27=-24/245, 25-26=-24/245, 24-25=-24/245, 23-24=-24/245, 22-23=-24/245, 21-22=-158/251, 20-21=-158/251, 19-20=-158/251, 18-19=-158/251, 17-18=-158/251, 16-17=-158/251, 15-16=-158/251, 14-15=-158/251, 12-14=-158/251  
WEBS 4-41=-596/322, 4-36=-20/168, 6-36=-394/127, 6-29=-55/111, 7-29=-494/97, 8-29=-55/100, 8-22=-394/115, 10-22=-6/168, 10-17=-596/311

#### NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Cable For Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.

*Martin Stapp*

February 23, 2007

#### WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

ENGINEERING BY  
**TRENCO**  
A Mitek Affiliate

818 Soundside Road  
Edenton, NC 27932

|        |       |            |     |     |          |
|--------|-------|------------|-----|-----|----------|
| Job    | Truss | Truss Type | Qty | Ply |          |
| HBFWSP | BET   | GABLE      | 1   | 1   | E3885232 |

SANTA FE TRUSS, HIGH SPRINGS, FL

6.500 s Feb 5 2007 MiTek Industries, Inc. Fri Feb 23 11:16:25 2007 Page 2

#### NOTES

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- 7) Gable requires continuous bottom chord bearing.
- 8) Gable studs spaced at 1-4-0 oc.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 81 lb uplift at joint 2, 304 lb uplift at joint 41, 99 lb uplift at joint 36, 209 lb uplift at joint 29, 98 lb uplift at joint 22, 294 lb uplift at joint 17, 75 lb uplift at joint 43, 70 lb uplift at joint 44, 75 lb uplift at joint 15, 73 lb uplift at joint 14 and 98 lb uplift at joint 12.

LOAD CASE(S) Standard



**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.**

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D88-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

ENGINEERING BY  
**TRENCO**  
 A MiTek Affiliate

818 Soundside Road  
 Edenton, NC 27932

|        |       |            |     |     |                          |
|--------|-------|------------|-----|-----|--------------------------|
| Job    | Truss | Truss Type | Qty | Ply | Job Reference (optional) |
| HBFWSP | C     | ROOF TRUSS | 9   | 1   |                          |

E3885233

SANTA FE TRUSS, HIGH SPRINGS, FL.

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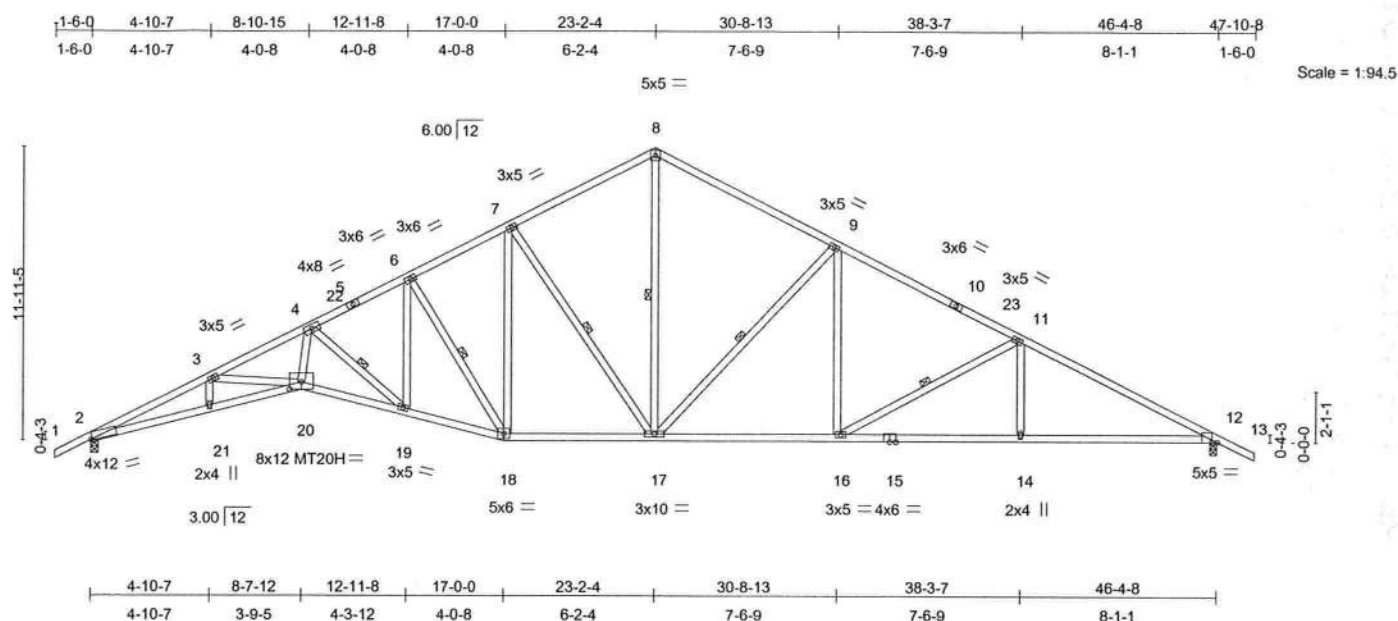


Plate Offsets (X,Y): [2:0-0-14,Edge], [12:0-2-8,0-0-2], [20:0-6-0,0-3-10]

| LOADING (psf) | SPACING              | 2-0-0 | CSI      | DEFL     | in (loc)    | l/defl | L/d | PLATES | GRIP           |
|---------------|----------------------|-------|----------|----------|-------------|--------|-----|--------|----------------|
| TCLL 20.0     | Plates Increase      | 1.25  | TC 0.69  | Vert(LL) | 0.65 19-20  | >857   | 240 | MT20   | 244/190        |
| TCDL 10.0     | Lumber Increase      | 1.25  | BC 0.96  | Vert(TL) | -1.07 19-20 | >515   | 180 | MT20H  | 187/143        |
| BCLL 0.0      | Rep Stress Incr      | YES   | WB 0.92  | Horz(TL) | 0.55 12     | n/a    | n/a |        |                |
| BCDL 10.0     | Code FBC2004/TPI2002 |       | (Matrix) |          |             |        |     |        | Weight: 278 lb |

**LUMBER**

TOP CHORD 2 X 4 SYP No.2D  
 BOT CHORD 2 X 4 SYP No.2D \*Except\*  
 2-20 2 X 4 SYP 2400F 2.0E  
 WEBS 2 X 4 SYP No.3

**BRACING**

TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins.  
 BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.  
 WEBS 1 Row at midpt 4-19, 6-18, 7-17, 8-17, 9-17, 11-16

**REACTIONS**

(lb/size) 2=1942/0-3-8, 12=1942/0-3-8  
 Max Horz 2=163(LC 4)  
 Max Uplift 2=-1060(LC 5), 12=-1060(LC 6)

**FORCES (lb) - Maximum Compression/Maximum Tension**

TOP CHORD 1-2=0/38, 2-3=-6557/3311, 3-4=-6271/3099, 4-22=-3518/1759, 5-22=-3507/1767, 5-6=-3469/1777, 6-7=-2640/1388, 7-8=-2137/1160, 8-9=-2169/1156, 9-10=-2736/1435, 10-23=-2769/1406, 11-23=-2840/1404, 11-12=-3535/1755, 12-13=0/39  
 BOT CHORD 2-21=-2981/5904, 20-21=-2958/5915, 19-20=-2520/5399, 18-19=-1421/3208, 17-18=-967/2305, 16-17=-1028/2447, 15-16=-1408/3055, 14-15=-1408/3055, 12-14=-1408/3055  
 WEBS 3-21=-60/136, 3-20=-213/254, 4-20=-1428/2863, 4-19=-2822/1448, 6-19=-751/1415, 6-18=-1494/783, 7-18=-395/592, 7-17=-820/486, 8-17=-861/1417, 9-17=-876/528, 9-16=-350/548, 11-16=-697/509, 11-14=-142/335

**NOTES**

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- All plates are MT20 plates unless otherwise indicated.
- This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1060 lb uplift at joint 2 and 1060 lb uplift at joint 12.

LOAD CASE(S) Standard

February 23, 2007

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.**

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

ENGINEERING BY  
**TRENCO**  
 A MiTek Affiliate

818 Soundside Road  
 Edenton, NC 27932

| Job    | Truss | Truss Type | Qty | Ply | Job Reference (optional) | E3885234 |
|--------|-------|------------|-----|-----|--------------------------|----------|
| HBFWSF | CET   | GABLE      | 1   | 1   |                          |          |

SANTA FE TRUSS, HIGH SPRINGS, FL.

6.500 s Feb 5 2007 Mitek Industries, Inc. Fri Feb 23 11:16:29 2007 Page 1

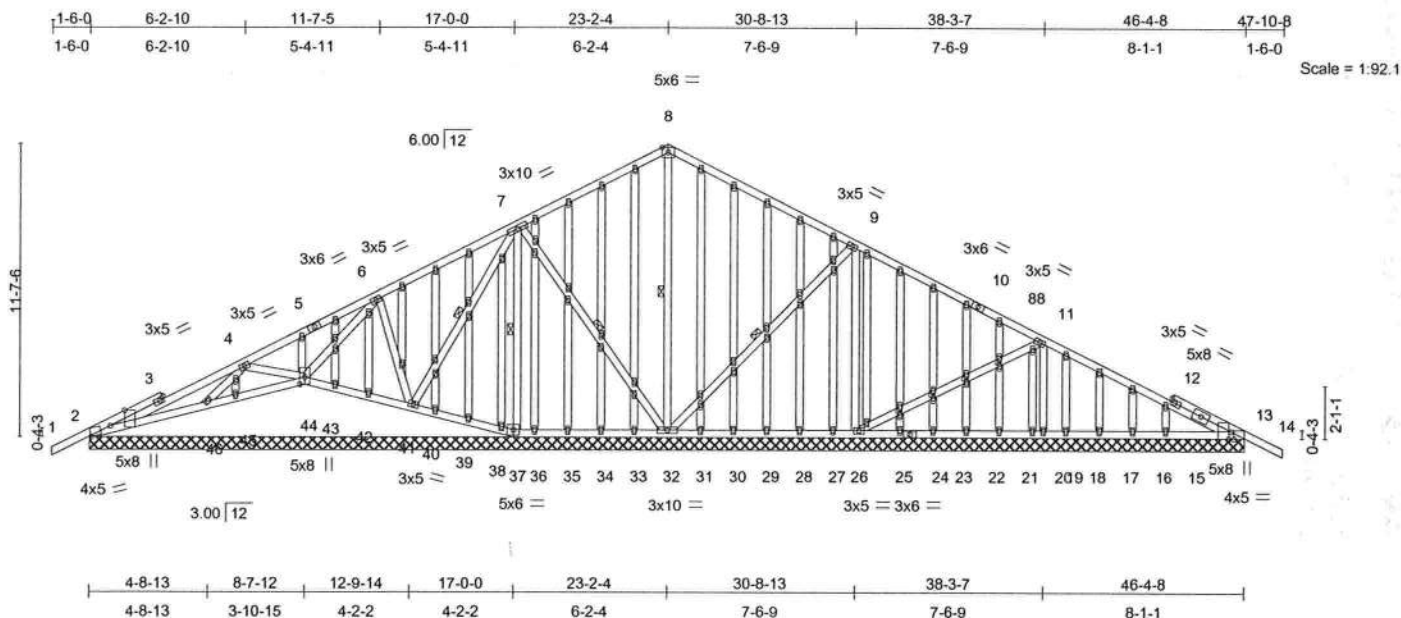


Plate Offsets (X,Y): [2:0-7-6,Edge], [2:0-9-15,Edge], [13:0-1-8,Edge], [13:0-3-8,Edge], [44:0-3-10,0-2-8], [77:0-1-12,0-1-0], [79:0-1-12,0-1-0], [81:0-1-12,0-1-0], [83:0-1-12,0-1-0]

| LOADING (psf) | SPACING              | CSI      | DEFL          | in (loc) | l/defl | L/d | PLATES         | GRIP    |
|---------------|----------------------|----------|---------------|----------|--------|-----|----------------|---------|
| TCLL 20.0     | Plates Increase 1.25 | TC 0.39  | Vert(LL) 0.03 | 14       | n/r    | 120 | MT20           | 244/190 |
| TCCL 10.0     | Lumber Increase 1.25 | BC 0.32  | Vert(TL) 0.06 | 14       | n/r    | 120 |                |         |
| BCCL 0.0      | Rep Stress Incr YES  | WB 0.41  | Horz(TL) 0.01 | 13       | n/a    | n/a |                |         |
| BCDL 10.0     | Code FBC2004/TPI2002 | (Matrix) |               |          |        |     |                |         |
|               |                      |          |               |          |        |     | Weight: 504 lb |         |

| LUMBER                    | BRACING                                                                   |
|---------------------------|---------------------------------------------------------------------------|
| TOP CHORD 2 X 4 SYP No.2D | TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. |
| BOT CHORD 2 X 4 SYP No.2D | BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing, Except:     |
| WEBS 2 X 4 SYP No.3       | 10-0-0 oc bracing: 45-46,44-45,43-44,42-43,41-42.                         |
| OTHERS 2 X 4 SYP No.3     | WEBS 1 Row at midpt 7-41, 7-37, 7-32, 8-32, 9-32                          |

**REACTIONS (lb/size)** 2=249/46-4-8, 44=307/46-4-8, 37=158/46-4-8, 46=380/46-4-8, 41=346/46-4-8, 32=556/46-4-8, 26=368/46-4-8, 19=651/46-4-8, 33=27/46-4-8, 34=26/46-4-8, 35=27/46-4-8, 36=24/46-4-8, 38=21/46-4-8, 39=29/46-4-8, 40=20/46-4-8, 42=36/46-4-8, 43=8/46-4-8, 45=-5/46-4-8, 31=27/46-4-8, 30=27/46-4-8, 29=26/46-4-8, 28=28/46-4-8, 27=18/46-4-8, 25=33/46-4-8, 23=25/46-4-8, 22=27/46-4-8, 21=28/46-4-8, 20=19/46-4-8, 18=14/46-4-8, 17=58/46-4-8, 16=-86/46-4-8, 15=230/46-4-8, 13=189/46-4-8

Max Horz 2=-158(LC 3)

Max Uplift 2=-139(LC 5), 44=-66(LC 5), 37=-68(LC 5), 46=-130(LC 5), 41=-125(LC 5), 32=-184(LC 5), 26=-123(LC 5), 19=-343(LC 6), 45=-5(LC 1), 17=-5(LC 6), 16=-87(LC 10), 15=-102(LC 6), 13=-85(LC 6)

Max Grav 2=250(LC 9), 44=315(LC 9), 37=172(LC 9), 46=382(LC 9), 41=356(LC 9), 32=556(LC 1), 26=370(LC 10), 19=651(LC 1), 33=53(LC 2), 34=53(LC 2), 35=55(LC 2), 36=48(LC 2), 38=42(LC 2), 39=58(LC 2), 40=39(LC 2), 42=71(LC 2), 43=19(LC 2), 45=33(LC 2), 31=53(LC 2), 30=54(LC 2), 29=52(LC 2), 28=57(LC 2), 27=36(LC 2), 25=66(LC 2), 23=49(LC 2), 22=54(LC 2), 21=55(LC 2), 20=42(LC 2), 18=43(LC 2), 17=72(LC 2), 16=81(LC 6), 15=232(LC 10), 13=189(LC 1)

#### FORCES (lb) - Maximum Compression/Maximum Tension

**TOP CHORD** 1-2=0/35, 2-3=-153/208, 3-4=-139/298, 4-5=-85/66, 5-6=-72/165, 6-7=-53/130, 7-8=-92/151, 8-9=-86/145, 9-10=-7/101, 10-88=-38/43, 11-88=-109/41, 11-12=-149/297, 12-13=-170/165, 13-14=0/39

**BOT CHORD** 2-46=-200/180, 45-46=-78/140, 44-45=-80/155, 43-44=0/199, 42-43=-2/197, 41-42=-1/199, 40-41=-11/228, 39-40=-13/228, 38-39=-12/228, 37-38=-10/227, 36-37=-9/219, 35-36=-8/219, 34-35=-8/219, 33-34=-8/219, 32-33=-8/219, 31-32=-1/241, 30-31=-1/241, 29-30=-1/241, 28-29=-1/241, 27-28=-1/241, 26-27=-1/241, 25-26=-170/266, 24-25=-170/266, 23-24=-170/266, 22-23=-170/266, 21-22=-170/266, 20-21=-170/266, 19-20=-170/266, 18-19=-170/266, 17-18=-170/266, 16-17=-170/266, 15-16=-170/266, 13-15=-170/266

**WEBS** 4-46=-477/254, 4-44=-234/221, 6-44=-190/69, 6-41=-219/128, 7-41=-144/26, 7-37=-178/54, 7-32=-94/129, 8-32=-431/49, 9-32=-41/87, 9-26=-426/143, 11-26=-28/192, 11-19=-639/343

#### NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

See plates on page 2

*Maria A. [Signature]*

February 23, 2007

#### WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MU-7473 BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

ENGINEERING BY  
**TRENCO**  
A Mitek Affiliate

818 Soundside Road  
Edenton, NC 27932

| Job    | Truss | Truss Type | Qty | Ply |          |
|--------|-------|------------|-----|-----|----------|
| HBFWSP | CET   | GABLE      | 1   | 1   | E3885234 |

SANTA FE TRUSS, HIGH SPRINGS, FL.

6.500 s Feb 5 2007 MiTek Industries, Inc. Fri Feb 23 11:16:30 2007 Page 2

#### NOTES

- 6) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- 7) Gable requires continuous bottom chord bearing.
- 8) Gable studs spaced at 1-4-0 oc.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 139 lb uplift at joint 2, 66 lb uplift at joint 44, 68 lb uplift at joint 37, 130 lb uplift at joint 46, 125 lb uplift at joint 41, 184 lb uplift at joint 32, 123 lb uplift at joint 26, 343 lb uplift at joint 19, 5 lb uplift at joint 45, 5 lb uplift at joint 17, 87 lb uplift at joint 16, 102 lb uplift at joint 15 and 85 lb uplift at joint 13.
- 10) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 44, 46, 41, 38, 39, 40, 42, 43, 45.

LOAD CASE(S) Standard

#### WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.

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ENGINEERING BY  
**TRENCO**  
A MiTek Affiliate

818 Soundside Road  
Edenton, NC 27932

|                                   |       |            |     |     |          |
|-----------------------------------|-------|------------|-----|-----|----------|
| Job                               | Truss | Truss Type | Qty | Ply |          |
| HBFWSP                            | V1    | GABLE      | 1   | 1   |          |
| SANTA FE TRUSS, HIGH SPRINGS, FL. |       |            |     |     | E3885235 |

Job Reference (optional)

6.500 s Feb 5 2007 MiTek Industries, Inc. Fri Feb 23 14:16:10 2007 Page 1

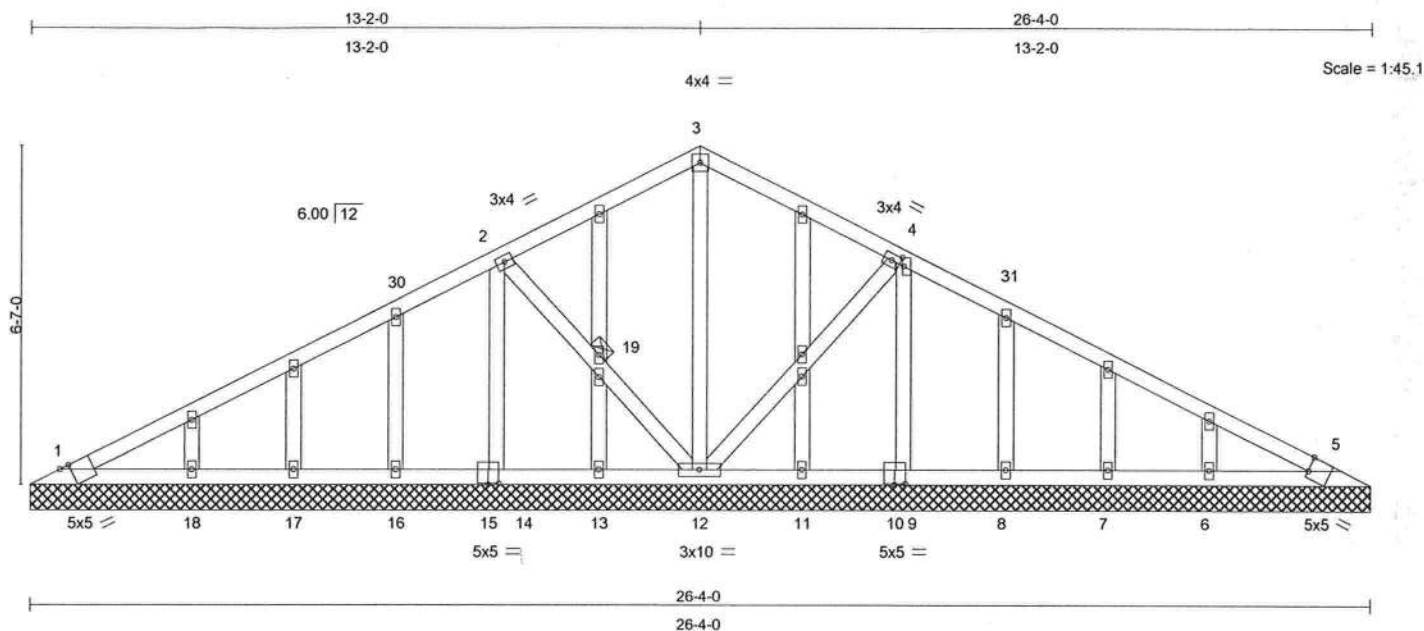


Plate Offsets (X,Y): [1:0-2-3,Edge], [5:0-0-3,Edge], [10:0-2-8,0-0-4], [15:0-2-8,0-0-4], [26:0-2-0,0-0-5]

| LOADING (psf) | SPACING              | 2-0-0 | CSI      | DEFL     | in   | (loc) | l/defl | L/d | PLATES | GRIP           |
|---------------|----------------------|-------|----------|----------|------|-------|--------|-----|--------|----------------|
| TCLL 20.0     | Plates Increase      | 1.25  | TC 0.51  | Vert(LL) | n/a  | -     | n/a    | 999 | MT20   | 244/190        |
| TCDL 10.0     | Lumber Increase      | 1.25  | BC 0.54  | Vert(TL) | n/a  | -     | n/a    | 999 |        |                |
| BCLL 0.0      | Rep Stress Incr      | YES   | WB 0.26  | Horz(TL) | 0.01 | 5     | n/a    | n/a |        |                |
| BCDL 10.0     | Code FBC2004/TPI2002 |       | (Matrix) |          |      |       |        |     |        |                |
|               |                      |       |          |          |      |       |        |     |        | Weight: 151 lb |

#### LUMBER

TOP CHORD 2 X 4 SYP No.2D  
BOT CHORD 2 X 4 SYP No.2D  
WEBS 2 X 4 SYP No.3  
OTHERS 2 X 4 SYP No.3

#### BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.  
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.  
JOINTS 1 Brace at Jt(s): 19

**REACTIONS (lb/size)** 1=279/26-4-0, 5=282/26-4-0, 12=671/26-4-0, 14=34/26-4-0, 16=62/26-4-0, 17=42/26-4-0, 18=280/26-4-0, 11=42/26-4-0, 9=34/26-4-0, 8=62/26-4-0, 7=43/26-4-0, 6=282/26-4-0, 13=65/26-4-0  
Max Horz 1=-91(LC 3)  
Max Uplift 1=-31(LC 5), 5=-32(LC 6), 12=-342(LC 6), 17=-43(LC 9), 18=-156(LC 5), 7=-43(LC 10), 6=-157(LC 6)  
Max Grav 1=279(LC 1), 5=282(LC 1), 12=671(LC 1), 14=78(LC 2), 16=89(LC 2), 17=80(LC 5), 18=282(LC 9), 11=81(LC 2), 9=78(LC 2), 8=89(LC 2), 7=81(LC 6), 6=283(LC 10), 13=88(LC 2)

#### FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-30=-484/40, 2-30=-275/59, 2-3=-142/16, 3-4=-142/16, 4-31=-282/62, 5-31=-491/43  
BOT CHORD 1-18=-48/329, 17-18=-48/329, 16-17=-48/329, 15-16=-48/329, 14-15=-48/329, 13-14=-48/329, 12-13=-48/329, 11-12=0/335, 10-11=0/335, 9-10=0/335, 8-9=0/335, 7-8=0/335, 6-7=0/335, 5-6=0/335  
WEBS 2-19=-436/276, 12-19=-420/268, 4-12=-439/276, 13-19=-23/10

#### NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02, 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- All plates are 2x4 MT20 unless otherwise indicated.
- This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- Gable requires continuous bottom chord bearing.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 31 lb uplift at joint 1, 32 lb uplift at joint 5, 342 lb uplift at joint 12, 43 lb uplift at joint 17, 156 lb uplift at joint 18, 43 lb uplift at joint 7 and 157 lb uplift at joint 6.

LOAD CASE(S) Standard

*Martin Stapp*

February 23, 2007

#### WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MH-7473 BEFORE USE.

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ENGINEERING BY  
**TRENCO**  
A MiTek Affiliate

818 Soundside Road  
Edenton, NC 27932

|                                   |       |            |     |     |                          |
|-----------------------------------|-------|------------|-----|-----|--------------------------|
| Job                               | Truss | Truss Type | Qty | Ply |                          |
| HBFWSP                            | V2    | ROOF TRUSS | 1   | 1   |                          |
|                                   |       |            |     |     | E3885236                 |
| SANTA FE TRUSS, HIGH SPRINGS, FL. |       |            |     |     | Job Reference (optional) |

6.500 s Feb 5 2007 MiTek Industries, Inc. Fri Feb 23 11:16:32 2007 Page 1

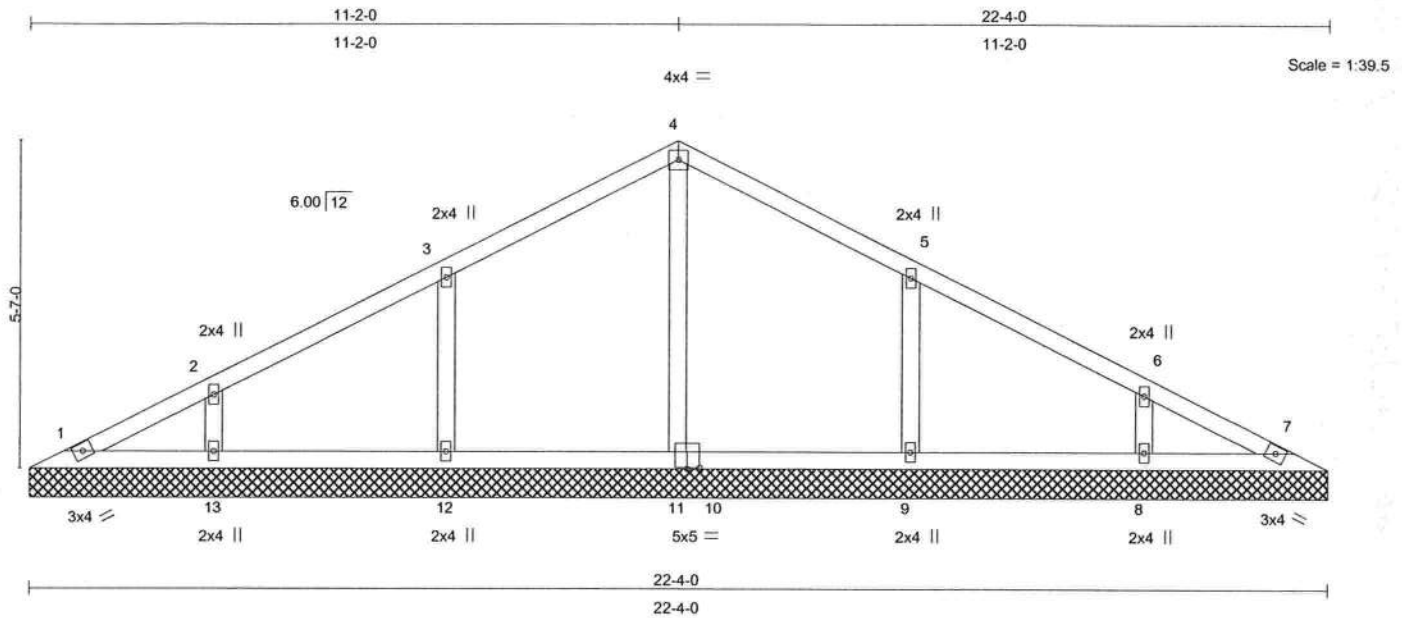


Plate Offsets (X,Y): [10:0-2-8,0-0-4]

| LOADING (psf) | SPACING              | 2-0-0 | CSI      | DEFL     | in   | (loc) | l/defl | L/d | PLATES | GRIP          |
|---------------|----------------------|-------|----------|----------|------|-------|--------|-----|--------|---------------|
| TCLL 20.0     | Plates Increase      | 1.25  | TC 0.10  | Vert(LL) | n/a  | -     | n/a    | 999 | MT20   | 244/190       |
| TCDL 10.0     | Lumber Increase      | 1.25  | BC 0.08  | Vert(TL) | n/a  | -     | n/a    | 999 |        |               |
| BCLL 0.0      | Rep Stress Incr      | YES   | WB 0.10  | Horz(TL) | 0.00 | 7     | n/a    | n/a |        |               |
| BCDL 10.0     | Code FBC2004/TPI2002 |       | (Matrix) |          |      |       |        |     |        |               |
|               |                      |       |          |          |      |       |        |     |        | Weight: 88 lb |

#### LUMBER

TOP CHORD 2 X 4 SYP No.2D  
BOT CHORD 2 X 4 SYP No.2D  
OTHERS 2 X 4 SYP No.3

#### BRACING

TOP CHORD Structural wood sheathing directly applied or 6'-0" oc purlins.  
BOT CHORD Rigid ceiling directly applied or 10'-0" oc bracing.

**REACTIONS** (lb/size) 1=81/22'-4", 7=81/22'-4", 11=288/22'-4", 12=328/22'-4", 13=291/22'-4", 9=328/22'-4", 8=291/22'-4"  
Max Horz 1=-77(LC 3)  
Max Uplift 1=-7(LC 3), 12=-118(LC 5), 13=-143(LC 5), 9=-118(LC 6), 8=-143(LC 6)  
Max Grav 1=81(LC 1), 7=81(LC 1), 11=288(LC 1), 12=338(LC 9), 13=291(LC 1), 9=338(LC 10), 8=291(LC 1)

#### FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-85/36, 2-3=-83/65, 3-4=-86/131, 4-5=-86/132, 5-6=-83/43, 6-7=-64/23  
BOT CHORD 1-13=0/75, 12-13=0/75, 11-12=0/75, 10-11=0/75, 9-10=0/75, 8-9=0/75, 7-8=0/75  
WEBS 4-11=-209/2, 3-12=-257/159, 2-13=-215/175, 5-9=-257/159, 6-8=-215/175

#### NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCCL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- Gable requires continuous bottom chord bearing.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 7 lb uplift at joint 1, 118 lb uplift at joint 12, 143 lb uplift at joint 13, 118 lb uplift at joint 9 and 143 lb uplift at joint 8.

**LOAD CASE(S)** Standard

*Maria A. Stapp*

February 23, 2007

#### WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

ENGINEERING BY  
**TRENCO**  
A MiTek Affiliate

818 Soundside Road  
Edenton, NC 27932

|               |             |                          |          |          |                          |          |
|---------------|-------------|--------------------------|----------|----------|--------------------------|----------|
| Job<br>HBFWSP | Truss<br>V3 | Truss Type<br>ROOF TRUSS | Qty<br>1 | Ply<br>1 | Job Reference (optional) | E3885237 |
|---------------|-------------|--------------------------|----------|----------|--------------------------|----------|

SANTA FE TRUSS, HIGH SPRINGS, FL.

6.500 s Feb 5 2007 MiTek Industries, Inc. Fri Feb 23 11:16:33 2007 Page 1

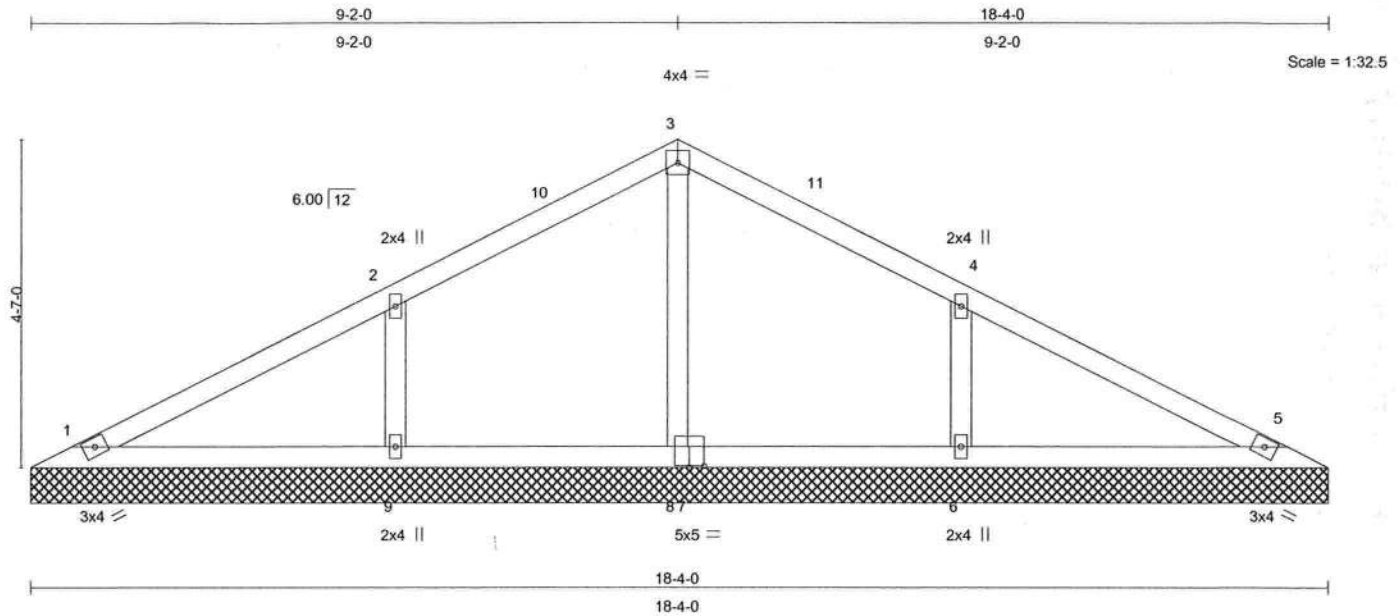


Plate Offsets (X,Y): [7:0-2-8,0-0-4]

| LOADING (psf) | SPACING              | 2-0-0 | CSI      | DEFL     | in   | (loc) | l/defl | L/d | PLATES        | GRIP    |
|---------------|----------------------|-------|----------|----------|------|-------|--------|-----|---------------|---------|
| TCLL 20.0     | Plates Increase      | 1.25  | TC 0.14  | Vert(LL) | n/a  | -     | n/a    | 999 | MT20          | 244/190 |
| TCDL 10.0     | Lumber Increase      | 1.25  | BC 0.13  | Vert(TL) | n/a  | -     | n/a    | 999 |               |         |
| BCLL 0.0      | Rep Stress Incr      | YES   | WB 0.07  | Horz(TL) | 0.00 | 5     | n/a    | n/a |               |         |
| BCDL 10.0     | Code FBC2004/TPI2002 |       | (Matrix) |          |      |       |        |     |               |         |
|               |                      |       |          |          |      |       |        |     | Weight: 68 lb |         |

#### LUMBER

TOP CHORD 2 X 4 SYP No.2D  
BOT CHORD 2 X 4 SYP No.2D  
OTHERS 2 X 4 SYP No.3

#### BRACING

TOP CHORD Structural wood sheathing directly applied or 6'-0" oc purlins.  
BOT CHORD Rigid ceiling directly applied or 10'-0" oc bracing.

**REACTIONS** (lb/size) 1=158/18-4-0, 5=158/18-4-0, 8=250/18-4-0, 9=401/18-4-0, 6=401/18-4-0  
Max Horz 1=-62(LC 3)  
Max Uplift 1=-29(LC 6), 5=-37(LC 6), 9=-185(LC 5), 6=-185(LC 6)  
Max Grav 1=158(LC 1), 5=158(LC 1), 8=250(LC 1), 9=408(LC 9), 6=408(LC 10)

#### FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-82/50, 2-10=-87/95, 3-10=-25/111, 3-11=-25/109, 4-11=-87/93, 4-5=-82/50  
BOT CHORD 1-9=0/63, 8-9=0/63, 7-8=0/63, 6-7=0/63, 5-6=0/63  
WEBS 3-8=-191/14, 2-9=-293/213, 4-6=-293/213

#### NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- Gable requires continuous bottom chord bearing.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 29 lb uplift at joint 1, 37 lb uplift at joint 5, 185 lb uplift at joint 9 and 185 lb uplift at joint 6.

**LOAD CASE(S)** Standard

*Monica Stapp*

February 23, 2007

#### WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

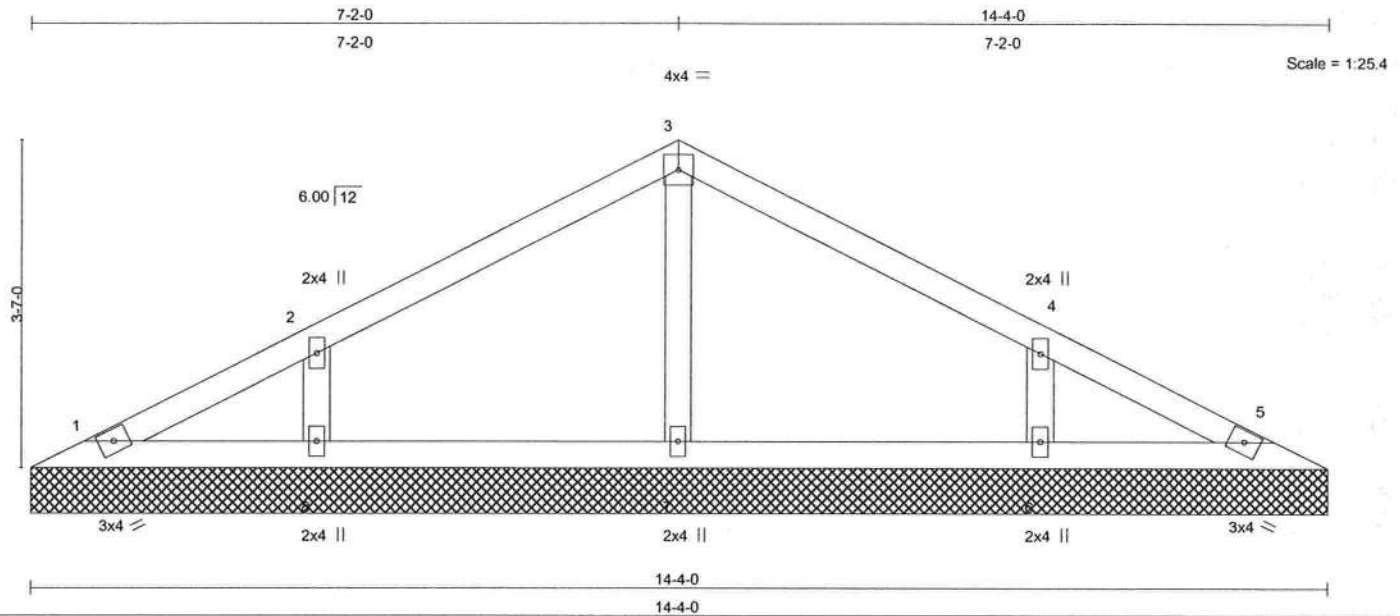
ENGINEERING BY  
**TRENCO**  
A MiTek Affiliate

818 Soundside Road  
Edenton, NC 27932

|        |       |            |     |     |          |
|--------|-------|------------|-----|-----|----------|
| Job    | Truss | Truss Type | Qty | Ply |          |
| HBFWSP | V4    | ROOF TRUSS | 1   | 1   | E3885238 |

SANTA FE TRUSS, HIGH SPRINGS, FL.

Job Reference (optional)  
6.500 s Feb 5 2007 MiTek Industries, Inc. Fri Feb 23 11:16:34 2007 Page 1



| LOADING (psf) | SPACING              | 2-0-0 | CSI      | DEFL     | in   | (loc) | l/defl | L/d | PLATES        | GRIP    |
|---------------|----------------------|-------|----------|----------|------|-------|--------|-----|---------------|---------|
| TCLL 20.0     | Plates Increase      | 1.25  | TC 0.12  | Vert(LL) | n/a  | -     | n/a    | 999 | MT20          | 244/190 |
| TCDL 10.0     | Lumber Increase      | 1.25  | BC 0.07  | Vert(TL) | n/a  | -     | n/a    | 999 |               |         |
| BCLL 0.0      | Rep Stress Incr      | YES   | WB 0.05  | Horz(TL) | 0.00 | 5     | n/a    | n/a |               |         |
| BCDL 10.0     | Code FBC2004/TPI2002 |       | (Matrix) |          |      |       |        |     |               |         |
|               |                      |       |          |          |      |       |        |     | Weight: 51 lb |         |

#### LUMBER

TOP CHORD 2 X 4 SYP No.2D  
BOT CHORD 2 X 4 SYP No.2D  
OTHERS 2 X 4 SYP No.3

#### BRACING

TOP CHORD Structural wood sheathing directly applied or 6'-0-0 oc purlins.  
BOT CHORD Rigid ceiling directly applied or 10'-0-0 oc bracing.

**REACTIONS** (lb/size) 1=83/14-4-0, 5=83/14-4-0, 7=303/14-4-0, 8=289/14-4-0, 6=289/14-4-0  
Max Horz 1=-48(LC 3)  
Max Uplift 1=-21(LC 6), 5=-12(LC 5), 7=-29(LC 5), 8=-141(LC 5), 6=-141(LC 6)  
Max Grav 1=83(LC 1), 5=83(LC 1), 7=303(LC 1), 8=302(LC 9), 6=302(LC 10)

#### FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-59/33, 2-3=-86/92, 3-4=-86/79, 4-5=-59/23  
BOT CHORD 1-8=0/42, 7-8=0/42, 6-7=0/42, 5-6=0/42  
WEBS 3-7=-221/72, 2-8=-227/173, 4-6=-227/173

#### NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- Gable requires continuous bottom chord bearing.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 21 lb uplift at joint 1, 12 lb uplift at joint 5, 29 lb uplift at joint 7, 141 lb uplift at joint 8 and 141 lb uplift at joint 6.

**LOAD CASE(S)** Standard

*Martin J. Stapp*

February 23, 2007

#### WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

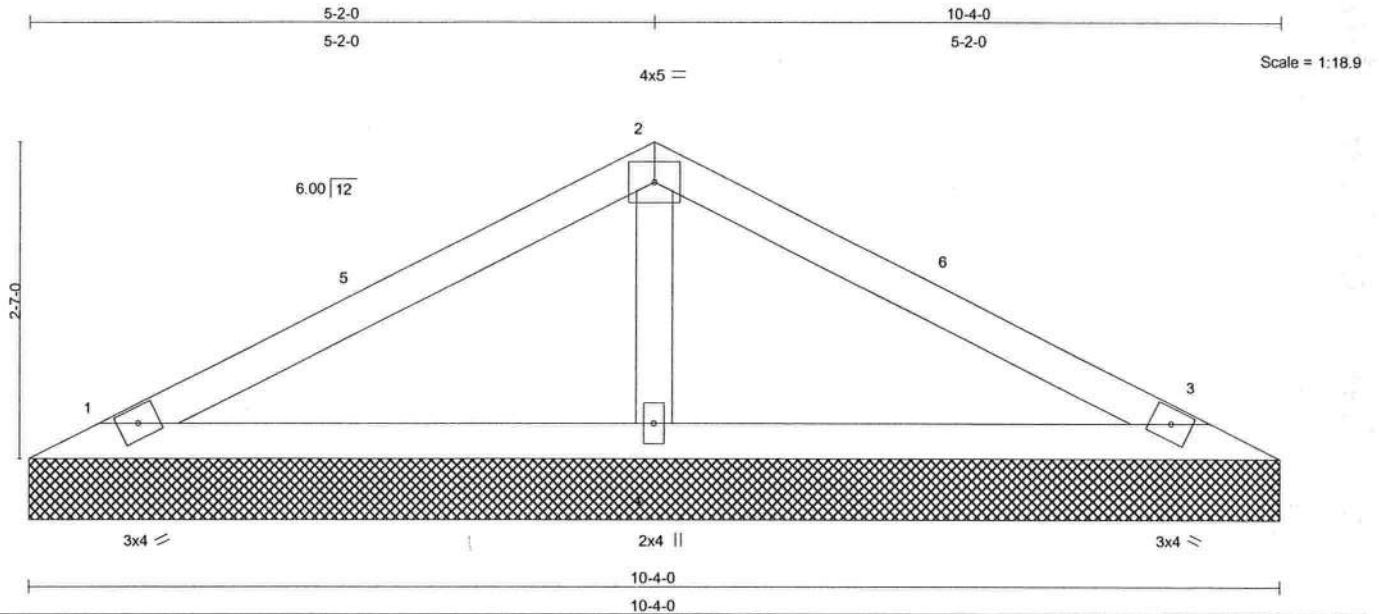
ENGINEERING BY  
**TRENCO**  
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818 Soundside Road  
Edenton, NC 27932

|               |             |                          |          |          |                          |          |
|---------------|-------------|--------------------------|----------|----------|--------------------------|----------|
| Job<br>HBFWSP | Truss<br>V5 | Truss Type<br>ROOF TRUSS | Qty<br>1 | Ply<br>1 | Job Reference (optional) | E3885239 |
|---------------|-------------|--------------------------|----------|----------|--------------------------|----------|

SANTA FE TRUSS, HIGH SPRINGS, FL.

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| LOADING (psf) | SPACING              | 2-0-0 | CSI      | DEFL     | in   | (loc) | l/defl | L/d | PLATES        | GRIP    |
|---------------|----------------------|-------|----------|----------|------|-------|--------|-----|---------------|---------|
| TCLL 20.0     | Plates Increase      | 1.25  | TC 0.15  | Vert(LL) | n/a  | -     | n/a    | 999 | MT20          | 244/190 |
| TCDL 10.0     | Lumber Increase      | 1.25  | BC 0.13  | Vert(TL) | n/a  | -     | n/a    | 999 |               |         |
| BCLL 0.0      | Rep Stress Incr      | YES   | WB 0.05  | Horz(TL) | 0.00 | 3     | n/a    | n/a |               |         |
| BCDL 10.0     | Code FBC2004/TPI2002 |       | (Matrix) |          |      |       |        |     | Weight: 34 lb |         |

#### LUMBER

TOP CHORD 2 X 4 SYP No.2D  
BOT CHORD 2 X 4 SYP No.2D  
OTHERS 2 X 4 SYP No.3

#### BRACING

TOP CHORD Structural wood sheathing directly applied or 6'-0" oc purlins.  
BOT CHORD Rigid ceiling directly applied or 10'-0" oc bracing.

#### REACTIONS (lb/size) 1=157/10-4-0, 3=157/10-4-0, 4=415/10-4-0

Max Horz 1=33(LC 4)  
Max Uplift 1=-57(LC 5), 3=-60(LC 6), 4=-113(LC 5)  
Max Grav 1=162(LC 9), 3=162(LC 10), 4=415(LC 1)

#### FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-5=-96/44, 2-5=-27/54, 2-6=-27/51, 3-6=-96/40  
BOT CHORD 1-4=-2/40, 3-4=-2/40  
WEBS 2-4=-275/140

#### NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- Gable requires continuous bottom chord bearing.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 57 lb uplift at joint 1, 60 lb uplift at joint 3 and 113 lb uplift at joint 4.

#### LOAD CASE(S) Standard

*Monica Stapp*

February 23, 2007

#### WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.

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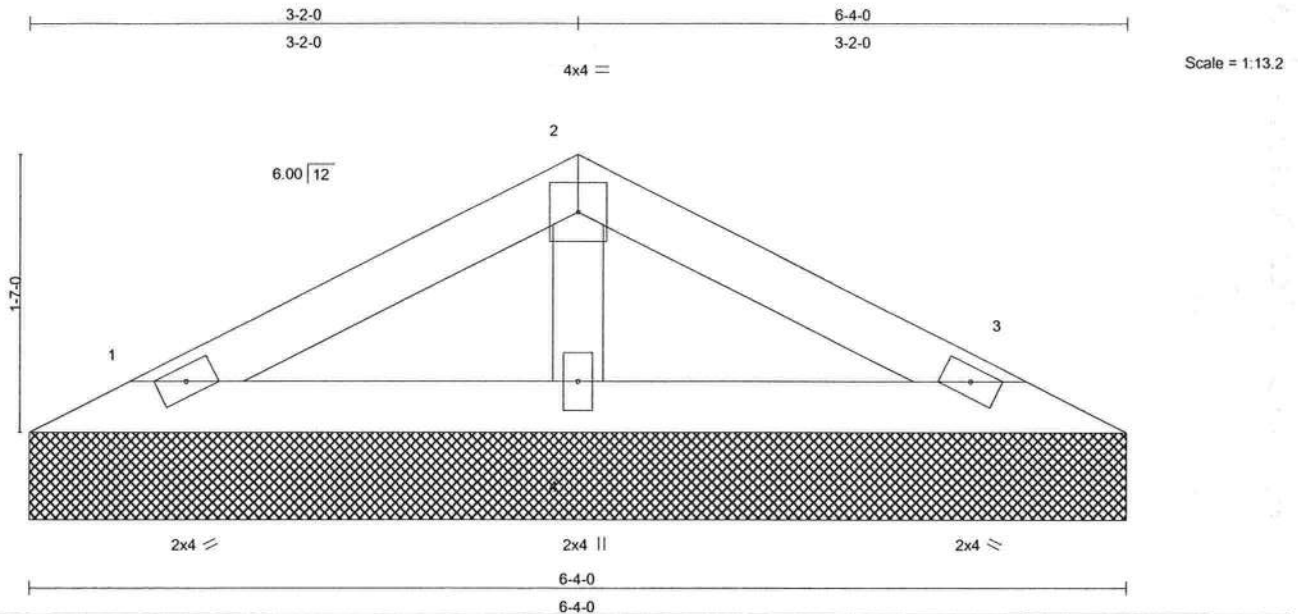
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**TRENCO**  
A MiTek Affiliate

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Edenton, NC 27932

|               |             |                          |          |          |                                      |
|---------------|-------------|--------------------------|----------|----------|--------------------------------------|
| Job<br>HBFWSP | Truss<br>V6 | Truss Type<br>ROOF TRUSS | Qty<br>1 | Ply<br>1 | Job Reference (optional)<br>E3885240 |
|---------------|-------------|--------------------------|----------|----------|--------------------------------------|

SANTA FE TRUSS, HIGH SPRINGS, FL.

6.500 s Feb 5 2007 MiTek Industries, Inc. Fri Feb 23 11:16:35 2007 Page 1



| LOADING (psf) | SPACING              | 2-0-0 | CSI      | DEFL     | in   | (loc) | l/defl | L/d | PLATES        | GRIP    |
|---------------|----------------------|-------|----------|----------|------|-------|--------|-----|---------------|---------|
| TCLL 20.0     | Plates Increase      | 1.25  | TC 0.08  | Vert(LL) | n/a  | -     | n/a    | 999 | MT20          | 244/190 |
| TCDL 10.0     | Lumber Increase      | 1.25  | BC 0.06  | Vert(TL) | n/a  | -     | n/a    | 999 |               |         |
| BCLL 0.0      | Rep Stress Incr      | YES   | WB 0.02  | Horz(TL) | 0.00 | 3     | n/a    | n/a |               |         |
| BCDL 10.0     | Code FBC2004/TP12002 |       | (Matrix) |          |      |       |        |     |               |         |
|               |                      |       |          |          |      |       |        |     | Weight: 19 lb |         |

#### LUMBER

TOP CHORD 2 X 4 SYP No.2D  
BOT CHORD 2 X 4 SYP No.2D  
OTHERS 2 X 4 SYP No.3

#### BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.  
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 1=104/6-4-0, 3=104/6-4-0, 4=199/6-4-0  
Max Horz 1=-19(LC 3)  
Max Uplift 1=-47(LC 5), 3=-47(LC 5), 4=-39(LC 5)

**FORCES** (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-47/28, 2-3=-47/28  
BOT CHORD 1-4=-1/17, 3-4=-1/17  
WEBS 2-4=-136/71

#### NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS automatic zone; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- Gable requires continuous bottom chord bearing.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 47 lb uplift at joint 1, 47 lb uplift at joint 3 and 39 lb uplift at joint 4.

**LOAD CASE(S)** Standard

*Manning*

February 23, 2007

#### WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

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ENGINEERING BY  
**TRENCO**  
A MiTek Affiliate

818 Soundside Road  
Edenton, NC 27932

|                                   |       |            |                                                                           |     |                          |          |
|-----------------------------------|-------|------------|---------------------------------------------------------------------------|-----|--------------------------|----------|
| Job                               | Truss | Truss Type | Qty                                                                       | Ply | Job Reference (optional) | E3885241 |
| HBFWSP                            | V7    | ROOF TRUSS | 1                                                                         | 1   |                          |          |
| SANTA FE TRUSS, HIGH SPRINGS, FL. |       | 2          | 6.500 s Feb 5 2007 MiTek Industries, Inc. Fri Feb 23 11:16:35 2007 Page 1 |     |                          |          |

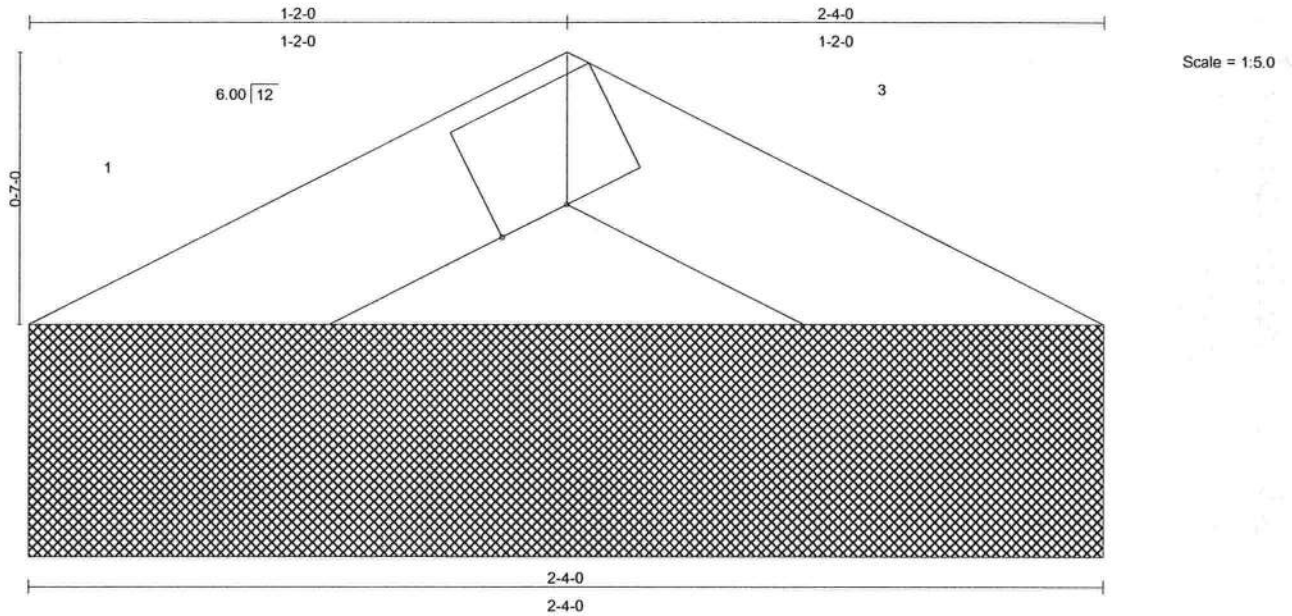


Plate Offsets (X,Y): [2:0-1-14,Edge]

| LOADING (psf) | SPACING              | 2-0-0 | CSI      | DEFL     | in   | (loc) | l/defl | L/d | PLATES | GRIP         |
|---------------|----------------------|-------|----------|----------|------|-------|--------|-----|--------|--------------|
| TCLL 20.0     | Plates Increase      | 1.25  | TC 0.01  | Vert(LL) | n/a  | -     | n/a    | 999 | MT20   | 244/190      |
| TCDL 10.0     | Lumber Increase      | 1.25  | BC 0.00  | Vert(TL) | n/a  | -     | n/a    | 999 |        |              |
| BCLL 0.0      | Rep Stress Incr      | YES   | WB 0.00  | Horz(TL) | 0.00 | 3     | n/a    | n/a |        |              |
| BCDL 10.0     | Code FBC2004/TPI2002 |       | (Matrix) |          |      |       |        |     |        |              |
|               |                      |       |          |          |      |       |        |     |        | Weight: 4 lb |

#### LUMBER

TOP CHORD 2 X 4 SYP No.2D

**REACTIONS** (lb/size) 1=25/2-4-0, 2=50/2-4-0, 3=25/2-4-0  
 Max Horz 1=-6(LC 3)  
 Max Uplift 1=-20(LC 5), 2=-21(LC 5), 3=-20(LC 5)

**FORCES** (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-9/12, 2-3=-9/12

#### NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS automatic zone; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- 3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- 4) Gable requires continuous bottom chord bearing.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 20 lb uplift at joint 1, 21 lb uplift at joint 2 and 20 lb uplift at joint 3.
- 6) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2.

**LOAD CASE(S)** Standard

#### BRACING

TOP CHORD Structural wood sheathing directly applied or 2-4-0 oc purlins.  
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

*Martin Stapp*

February 23, 2007

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.**

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ENGINEERING BY  
**TRENCO**  
 A MiTek Affiliate

818 Soundside Road  
 Edenton, NC 27932

# CERTIFICATE OF OCCUPANCY

## OCCUPANCY

COLUMBIA COUNTY, FLORIDA

### Department of Building and Zoning Inspection

*This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.*

Parcel Number 34-6S-16-04059-262

Building permit No. 000025674

Use Classification SFD, UTILITY

Fire: 16.74

Permit Holder HARVEY BLDG & CONSTRUCTION

Waste: 50.25

Owner of Building JAMES MCGLYNN

Total: 66.99

Location: 124 SW LANKTREE GLEN, FT. WHITE, FL

Date: 07/31/2007

*Harry Dickie*

Building Inspector

POST IN A CONSPICUOUS PLACE  
(Business Places Only)



## Notice of Treatment

41470

Applicator: Florida Pest Control & Chemical Co. (www.flapest.com)

Address: 116 NW 16 AVE

City: GVILLE Phone: 376-2661

Site Location: Subdivision \_\_\_\_\_

Lot # \_\_\_\_\_ Block# \_\_\_\_\_ Permit # 25674

Address: 124 SW LANKTREE CUN FT WHITE

### Product used

### Active Ingredient

### % Concentration

☒ Premise Imidacloprid 0.1%

☐ Termidor Fipronil 0.12%

☐ Bora-Care Disodium Octaborate Tetrahydrate 23.0%

Type treatment:

☒ Soil

☐ Wood

Area Treated

Square feet

Linear feet

Gallons Applied

PERIMETER

—

209.8

42

As per Florida Building Code 104.2.6 – If soil chemical barrier method for termite prevention is used, final exterior treatment shall be completed prior to final building approval.

If this notice is for the final exterior treatment, initial this line BE.

7/27/07  
Date

227  
Time

BILLE  
Print Technician's Name

Remarks: \_\_\_\_\_

Applicator - White

Permit File - Canary

Permit Holder - Pink

10/05

©

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41470

Applicator: **Florida Pest Control & Chemical Co. (www.flapest.com)**

Address: 116 NW 16 AVE

City: G-VILLE

Phone: 376-2661

Site Location: Subdivision \_\_\_\_\_

Lot # \_\_\_\_\_ Block# \_\_\_\_\_ Permit # 25674

Address 124 SW LANKTIZEE GLN FT WILLIE

| <u>Product used</u> | <u>Active Ingredient</u> | <u>% Concentration</u> |
|---------------------|--------------------------|------------------------|
|---------------------|--------------------------|------------------------|

|                                  |              |      |
|----------------------------------|--------------|------|
| <input type="checkbox"/> Premise | Imidacloprid | 0.1% |
|----------------------------------|--------------|------|

|                                   |          |       |
|-----------------------------------|----------|-------|
| <input type="checkbox"/> Termidor | Fipronil | 0.12% |
|-----------------------------------|----------|-------|

|                                    |                                  |       |
|------------------------------------|----------------------------------|-------|
| <input type="checkbox"/> Bora-Care | Disodium Octaborate Tetrahydrate | 23.0% |
|------------------------------------|----------------------------------|-------|

Type treatment:

☒ Soil

☐ Wood

Area Treated

Square feet

Linear feet

Gallons Applied

MB-GRG-PRCH

2686

211.70

135

As per Florida Building Code 104.2.6 – If soil chemical barrier method for termite prevention is used, final exterior treatment shall be completed prior to final building approval.

If this notice is for the final exterior treatment, initial this line \_\_\_\_\_.

4/18/07

Date

212

Time

BILLE

Print Technician's Name

Remarks: \_\_\_\_\_

Applicator - White

Permit File - Canary

Permit Holder - Pink

10/05

©

