

(TYP.) GABLE BRACING DETAIL

WOOD FRAME

Wind Speed	Sheathing Thickness Plywood Or OSB Required Nail		Nail spacing along panel edges	Nail spacing along intermediate supports in the panel field	
120 mph Exp. B	7/16"	ASTM F1667 RSRS-01 (2 3/8" × 0.113")	6" oc	12" oc	
120 mph Exp. C	7/16"	ASTM F1667 RSRS-01 (2 3/8" × 0.113")	6" oc	6" oc	
120 mph Exp. D	19/32"	ASTM F1667 RSRS-03 (2 1/2" × 0.131") or ASTM F1667 RSRS-04 (3" × 0.120")	6" oc	6" oc	
130 mph Exp. B	7/16"	ASTM F1667 RSRS-01 (2 3/8" × 0.113")	6" oc	6" oc	
130 mph Exp. C	15/32"	ASTM F1667 RSRS-01 (2 3/8" × 0.113")	6" oc	6" oc	
130 mph Exp. D		6" oc	6" oc		
Exp. B (2 3/8" × 0.113") 140 mph 19/32" ASTM F1667 RSRS-0: Exp. C (2 1/2" × 0.131")		ASTM F1667 RSRS-01 (2 3/8" × 0.113")	6" oc	6" oc	
		or ASTM F1667 RSRS-04	6" oc	6" oc	
140 mph Exp. D		6" oc	6" oc		
150 mph Exp. C	19/32"	ASTM F1667 RSRS-03 (2 1/2" × 0.131") or ASTM F1667 RSRS-04 (3" × 0.120")	6" oc	6" oc	
150 mph Exp. D	19/32"	ASTM F1667 RSRS-03 (2 1/2" × 0.131") or ASTM F1667 RSRS-04 (3" × 0.120")	4" oc	4" oc	

ROOF SHEATHING FASTENING TABLE (RAFTER / TRUSS SG = 0.49)

on each side of ridges and hips, nail spacing is permitted to be 6 inches on center along panel edges and 6 inches on center along intermediate supports in the panel field. Note: This table specifies the code minimum thickness of roof sheathing. The thickness of the sheathing may need to be increased based in the type of roofing material being used. See manufacturer Florida product approval.

CONNECTOR TABLE				
Uplift SP	Uplift SPF	Truss Connector	To Plate	To Truss/Rafter
615	485	SDWC15600	-	-
415	290	H3	4-8dx1 1/2"	4-8dx1 1/2"
575	495	H2.5A	5-8dx1 1/2"	5-8dx1 1/2"
1340	1015	H10A	9-10d1 1/2"	9-10d1 1/2"
720	620	LTS12-20	6-10d1 1/2"	6-10d1 1/2"
1000	860	MTS12-30	7-10d1 1/2"	7-10d1 1/2"
1450	1245	HTS20-30	12-10d1 1/2"	12-10d1 1/2"
Uplift SP	Uplift SPF	Strap Ties	To One Member	To Other Member
1235	1235	LSTA21	8-10d	8-10d
1640	1455	MSTA24	9-10d	9-10d
1030	1030	CS20	7-10d	7-10d
Uplift SP	Uplift SPF	Stud Plate Ties	To Stud	To Plate
585	535	SP1	6-10d	4-10d
1065	605	SP2	6-10d	6-10d
771	771	LSTA24	10-10d	wrap under or over pla
1235	1235	LSTA24	14-10d	wrap under or over pla
Uplift SP	Uplift SPF	Holdowns @ Stemwall	To Stud / Post	Anchor
1825	1800	DTT2Z	8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD
4235	3640	HTT4	18-16dx2 1/2"	1/2"x12" Titen HD
Uplift SP	Uplift SPF	Holdowns @ Mono	To Stud / Post	Anchor
1825	1800	DTT2Z	8-SDS 1/4"x1 1/2"	1/2"x6" Titen HD
4235	3640	HTT4	18-16dx2 1/2"	1/2"x12" Titen HD
Uplift SP	Uplift SPF	Post Bases @ Stemwall	To Post	Anchor
1900		ABU44Z	12-16d	5/8"x12" Drill & Epoxy
2475		ABU66Z	12-16d	5/8"x12" Drill & Epoxy
Uplift SP	Uplift SPF	Post Bases @ Mono	To Post	Anchor
1900		ABU44Z	12-16d	5/8"x7" Drill & Epoxy
2475		ABU66Z	12-16d	5/8"x7" Drill & Epoxy



	4'-0"	16'-0"
Ф Ф		STRUCTURAL GABLE END TRUSS (NO HEADER)
	11	6x6 SP#2 PT EMBEDDED POST (SEE POST DETAIL THIS SHEET)
10,-0"		
	O POST	ENGINEERED SES @ 24" OCHED PER ENGINEERIN
200"	.25" LVL TCH INTC LTS PER	RE-ENGINEERE RUSSES @ 24" C ATTACHED PER USS ENGINEERI SP#2
20,	75" x 11 30ER NO 1HRU BO	PRE-ENGINEERE TRUSSES @ 24" O TRUSSES @ 24" O TRUSSES @ 24" O ATTACHED PER ATTACHED PER ATTACHED PER ATTACHED PER ATTACHED PER (2) 2x12 SP#2 PORCH HEADER NOTCH INTO POST
	PORCH HEA w/(3) 1/2"	6x6 SP#2 PT EMBEDDED POST H (SEE POST DETAIL THIS SHEET)
10-0"	Od	POA
		6x6 SP#2 PT EMBEDDED POST (SEE POST DETAIL THIS SHEET)
0 		STRUCTURAL CARLE END TRUCS (NO HEADER)

STRUCTURAL PLAN

DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

	4'-0"		16'-0"	
φ φ		STRUCTURAL	GABLE END TRUSS (NO	HEADER)
	111			
	11		PT EMBEDDED POST — T DETAIL THIS SHEET)	11
100"				
	25" LVL FCH INTO POST TS PER POST		RE-ENGINEERED KUSSES @ 24" OC ATTACHED PER JSS ENGINEERIN	POST
	, , , <u>, , , , , , , , , , , , , , , , </u>			SP#2—OTCH INTO
	(2) 1.75" x 11. PORCH HEADER NO w/ (3) 1/2" THRU BOU	•		PORCH HEADER NO w/ (3) 1/2" THRU BOI
	(2) 7 RCH HEA (3) 1/2" -	6x6 SP#2 (SEE POS	PT EMBEDDED POST— T DETAIL THIS SHEET)	3) 1/2"]
10-0"				
	11	6x6 SP#2 (SEE POS	PT EMBEDDED POST — T DETAIL THIS SHEET)	
		STRUCTURAL	GABLE END TRUSS (NO	HEADER)

STRUCTURAL PLAN NOTES

GENERAL NOTES:
TPUSSES TRUSSES SHALL BE DESIGN

S SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS. TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE)

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 2500 PSI. WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE

REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185: LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT, THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 40, DEFORMED BARS, FY = 40 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT.
AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED
FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

BUILDER'S RESPONSIBILITY:

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK. CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND

BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE. PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL

ROOF SYSTEM DESIGN:

TRUSS SHEETS.

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR, IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED

GRADE & SPECIES TABLE				
		Fb	Е	
2x8	SP #2	925	1.4	
2x10	SP #2	800	1.4	
2x12	SP #2	750	1.4	
GLB	24F-V3 SP	2600	1.9	
LSL	TIMBERSTRAND	1700	1.7	
LVL	MICROLAM	2950	2.0	
PSL	PARALAM	2900	2.0	

DESIGN CRITERIA & LOAD	S:		
BUILDING CODE	8TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2023)		
CODE FOR DESIGN LOADS	ASCE 7-22		
WINDLOADS			
BASIC WIND SPEED (ASCE 7-22, 3S GUST)	130 MPH		
WIND EXPOSURE (BUILDER MUST FIELD VERIFY)	С		
TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VERIFY)	I		
RISK CATEGORY	II		
ENCLOSURE CLASSIFICATION	ENCLOSED		
INTERNAL PRESSURE COEFFICIENT	0.18		
ROOF ANGLE	7-45 DEGREES		
MEAN ROOF HEIGHT	30 FT		
C&C DESIGN PRESSURES	SEE TABLE		
FLOOR LOADING			
ROOMS OTHER THAN SLEEPING ROOM	40 PSF LIVE LOAD		
SLEEPING ROOMS	30 PSF LIVE LOAD		
ROOF LOADING			
FLAT OR < 4:12	20 PSF LIVE LOAD		
4:12 TO < 12:12	16 PSF LIVE LOAD		
12:12 & GREATER	12 PSF LIVE LOAD		
SOIL BEARING CAPACITY	1500 PSF		
FLOOD ZONE	THIS BUILDING IS NOT IN THE FLOOD ZONE		

OMPONENT & CLADING DESIGN PRESSURES 130 MPH (EXP C)				
	INTERIOR		ZONE 5 END 4' FROM ALL OUTSIDE CORNER	
- 20	+25.6(Vasd)	-27.8(Vasd)	+25.6(Vasd)	-34.2(Vasd)
- 20	+42.6(Vult)	-46.2(Vult)	+42.6(Vult)	-57(Vult)

Mark Disosway FL PE 53915 This item has been digitally signed and sealed by Mark Disosway PE on digital signature date. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

C=US, O=Florida, dnQualifier=A01 Mark Disosway 410C0000017E Digital Signature 97DE07CA0007 46F0, CN=Mark PRINTED SIGNATURE IS NOT VALID d DISOSWAY 2024-08-27 18: 05:05

DIMENSIONS: Stated dimensions supercede scaled dimensions. Refer all questions to

Mark Disosway, P.E. for resolution. Do not proceed without clarification. COPYRIGHTS AND PROPERTY RIGHTS: Mark Disosway, P.E. hereby expressly reserves its common law copyrights and property right in

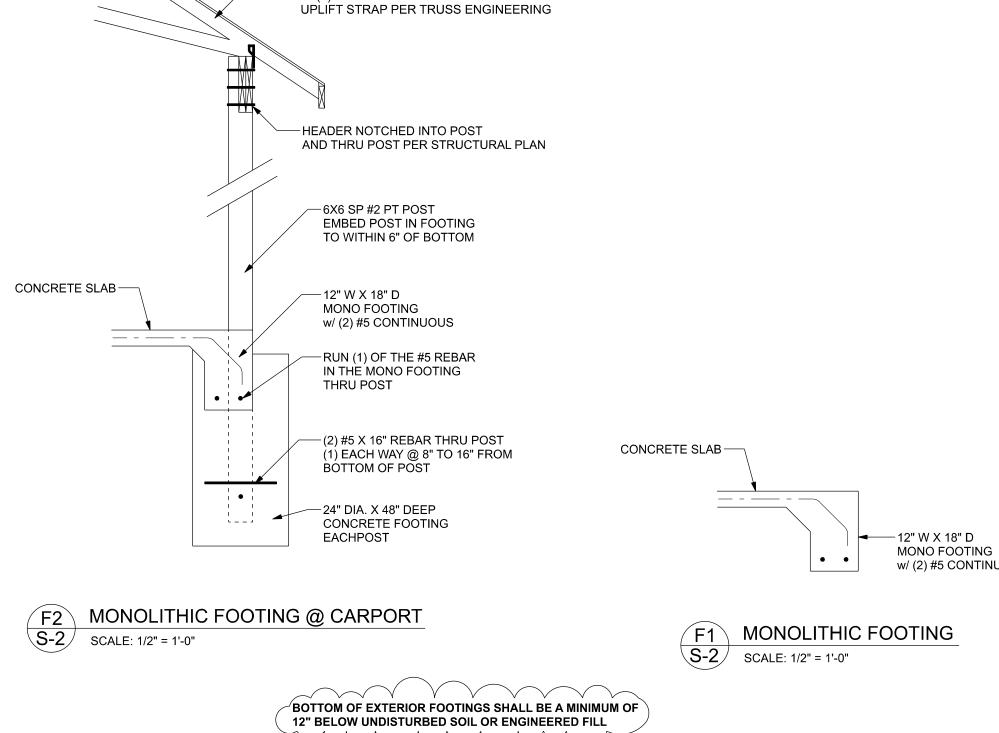
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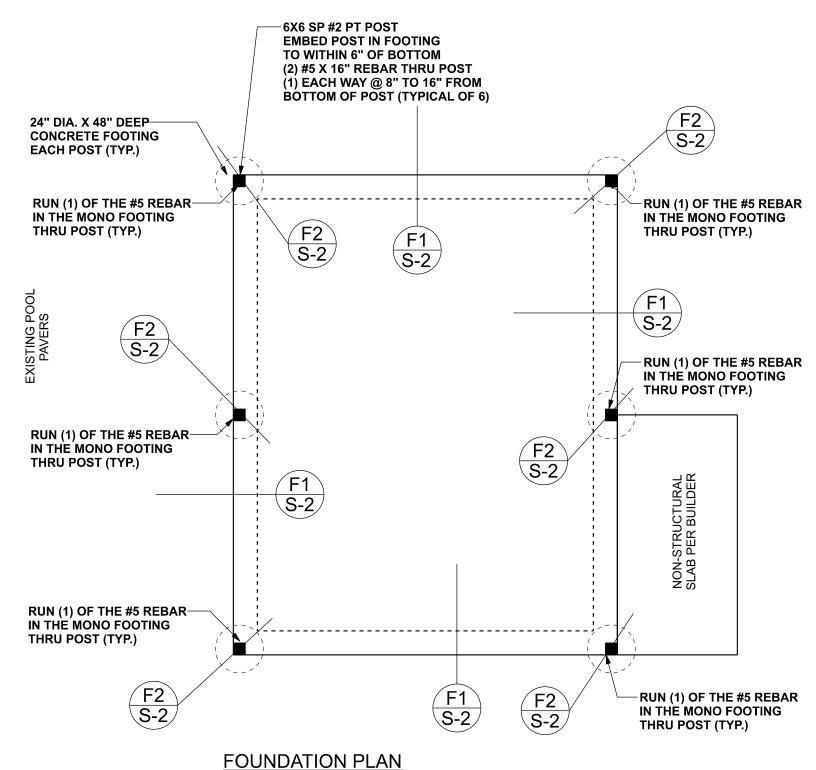
to the best of my knowledge. LIMITATION: This design is valid for one building, at specified location.

Mark Disosway P.E. **163 SW Midtown Place** Suite 103 Lake City, Florida 32025 386.754.5419 disoswaydesign@gmail.com

> JOB NUMBER: 240952

> > **S-1** OF 1 SHEET





SCALE: 1/4" = 1'-0"

FOUNDATION NOTES FN - 1 DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS, RECESSES IN SLAB, STEP DOWNS, ETC. DISOSWAY DESIGN GROUP OR MARK DISOSWAY, PE IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN. CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING

FN - 2 IN ALL AREAS BY REVIEWINGTHE ROOF TRUSS PLAN (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN N - 3 THE SLAB SHALL BE: 4" CONCRETE SLAB REINFORCED w/ 6X6-1.4/1.4 WELDED WIRE MESH PLACED ON CHAIRS @ 1 1/2" DEPTH OR FIBER MESH CONCRETE. 6-MIL POLY VAPOR BARRIER w/ 6" LAPS SEALED w/ POLY TAPE OVER TERMITE-TREATED & COMPACTED FILL (ALSO, ANY OTHER CODE APPROVED TERMITE-TREATMENT METHOD CAN BE USED INSTEAD)