Project Information for:	L169928		
Builder:	PRUDENTIAL	Date:	6/19/2006
Lot:	Lot 44 PRICE CREEK LANDING	Start Number:	**1479**
Subdivision:	N/A		
County or City:	COLUMBIA COUNTY		
Truss Page Count:	30		

Truss Design Load Information (UNO) Design Program: MiTek 5.2 / 6.2

Gravity		Wind		Building Code:	FBC2004
Roof (psf):	42	Wind Standard:	ASCE 7-02		
Floor (psf):	55	Wind Speed (mph):	110		

Note: See individual truss drawings for special loading conditions

Building Designer, responsible for Structural Engineering: (See attached)

FITZHUGH, JUSTIN M. CRC1328401
Address: P.O. BOX 3333
LAKE CITY, FL 32056
Designer: **39**

Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987
Company: Structural Engineering and Inspections, Inc. EB 9196
Address 16105 N. Florida Ave, Ste B, Lutz, FL 33549

Notes:

1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
4. Trusses designed for veritcal loads only, unless noted otherwise.

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	CJ1	0619061479	6/19/2006				
2	CJ3	0619061480	6/19/2006				
3	CJ5	0619061481	6/19/2006				
4	EJ3	0619061482	6/19/2006				
5	EJ4	0619061483	6/19/2006				
6	EJ7	0619061484	6/19/2006				
7	EJ7A	0619061485	6/19/2006				
8	EJ7B	0619061486	6/19/2006				
9	HJ4	0619061487	6/19/2006				
10	HJ5	0619061488	6/19/2006				
11	HJ9	0619061489	6/19/2006				
12	T01	0619061490	6/19/2006				
13	T02	0619061491	6/19/2006				
14	T03	0619061492	6/19/2006				
15	T04	0619061493	6/19/2006				
16	T05	0619061494	6/19/2006				
17	T06	0619061495	6/19/2006				
18	T07	0619061496	6/19/2006				
19	T08	0619061497	6/19/2006				
20	T09	0619061498	6/19/2006				
21	T10	0619061499	6/19/2006				
22	T11	0619061500	6/19/2006				
23	T12	0619061501	6/19/2006				
24	T13	0619061502	6/19/2006				
25	T14	0619061503	6/19/2006				
26	T15	0619061504	6/19/2006				
27	T16	0619061505	6/19/2006				
28	T17	0619061506	6/19/2006				
29	T18	0619061507	6/19/2006				
30	T19	0619061508	6/19/2006				

JUN 19 2006

Dwg.#0619061478

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Licensee Information

Name:	**FITZHUGH, JUSTIN M (Primary Name)**
	PRUDENTIAL BUILDERS INC (DBA Name)
Main Address:	**P.O. BOX 3333**
	LAKE CITY Florida 32056
County:	**COLUMBIA**
License Mailing:	
LicenseLocation:	

License Information

License Type:	**Certified Residential Contractor**
Rank:	**Cert Residental**
License Number:	**CRC1328401**
Status:	**Current,Active**
Licensure Date:	**12/23/2005**
Expires:	**08/31/2008**

Special Qualifications	Qualification Effective
Qualified Business License Required	02/06/2006

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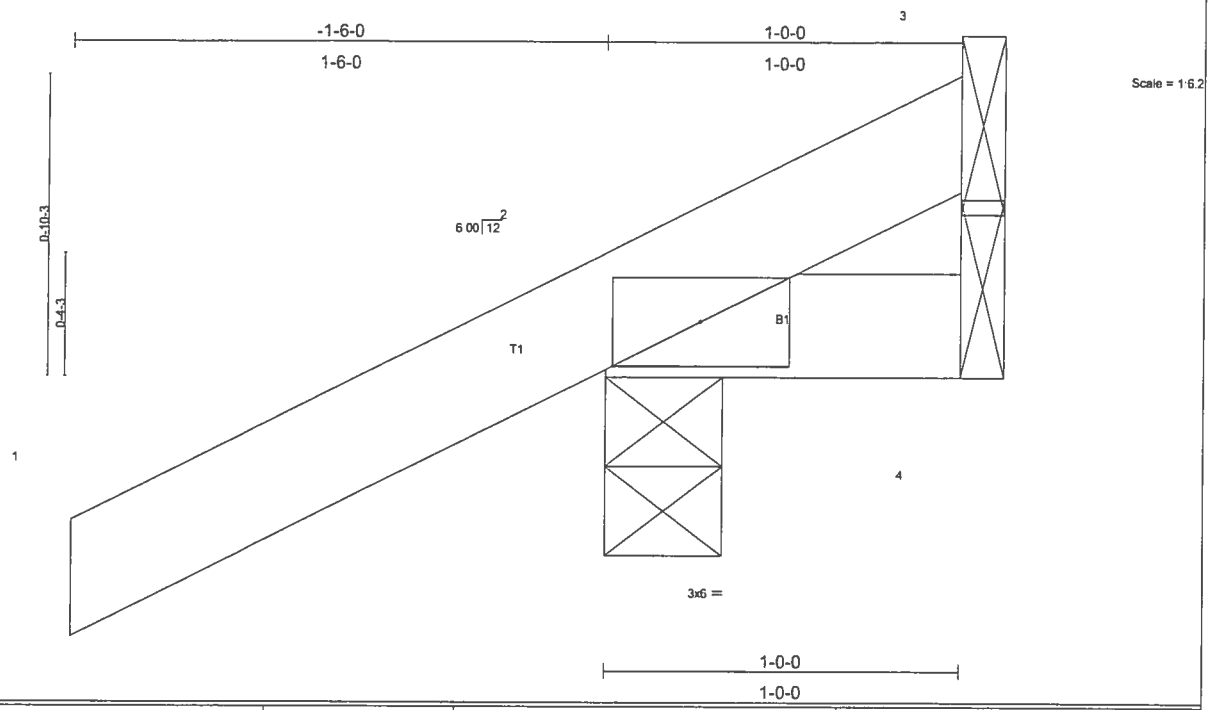
JUNE 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0619061479

Job	Truss	Truss Type	Qty	Ply	0 0
L169928	CJ1	JACK	14	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Jun 19 10:32:57 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.15	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 6 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=189/0-4-0, 4=14/Mechanical, 3=-41/Mechanical
Max Horz 2=70(load case 5)
Max Uplift2=-181(load case 5), 3=-41(load case 1)
Max Grav 2=189(load case 1), 4=14(load case 1), 3=61(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-45/35
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.10

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 181 lb uplift at joint 2 and 41 lb uplift at joint 3.

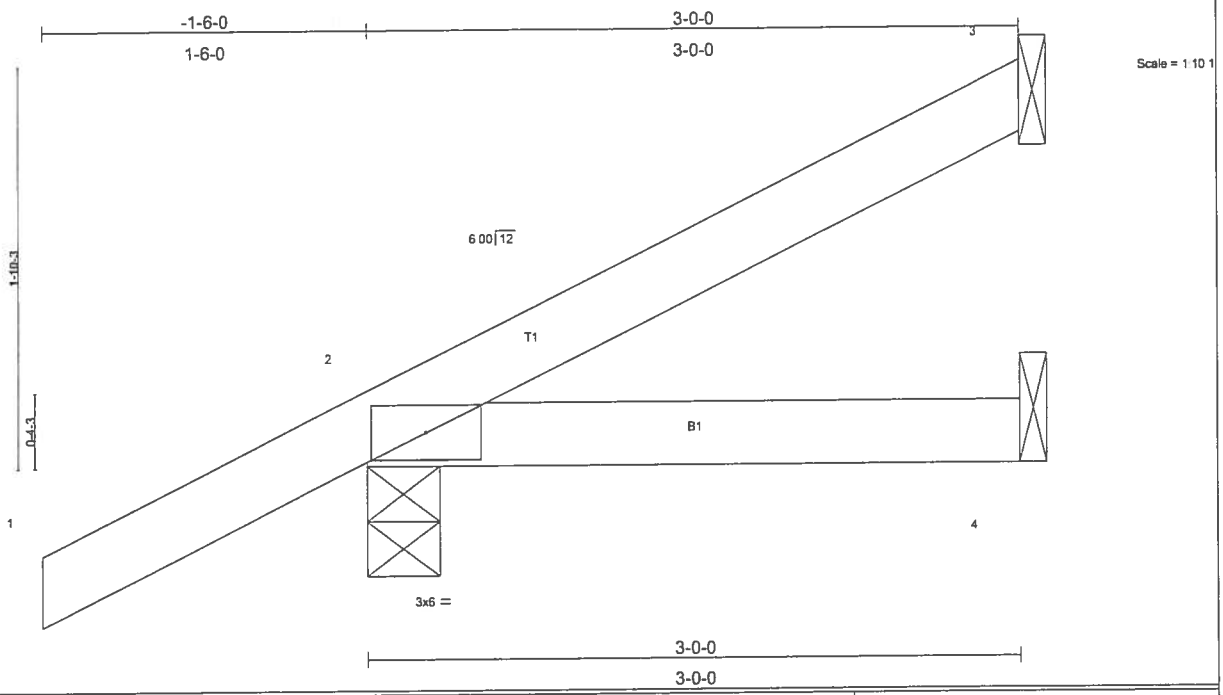
LOAD CASE(S) Standard

JUNE 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0619061480

Job	Truss	Truss Type	Qty	Ply	0 0
L169928	CJ3	JACK	10	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Jun 19 10:33:00 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.17	Vert(LL)	-0.00	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.05	Vert(TL)	-0.01	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 12 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=48/Mechanical, 2=233/0-4-0, 4=42/Mechanical
Max Horz 2=115(load case 5)
Max Uplift3=-37(load case 5), 2=-153(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-49/16
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.11

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 37 lb uplift at joint 3 and 153 lb uplift at joint 2.

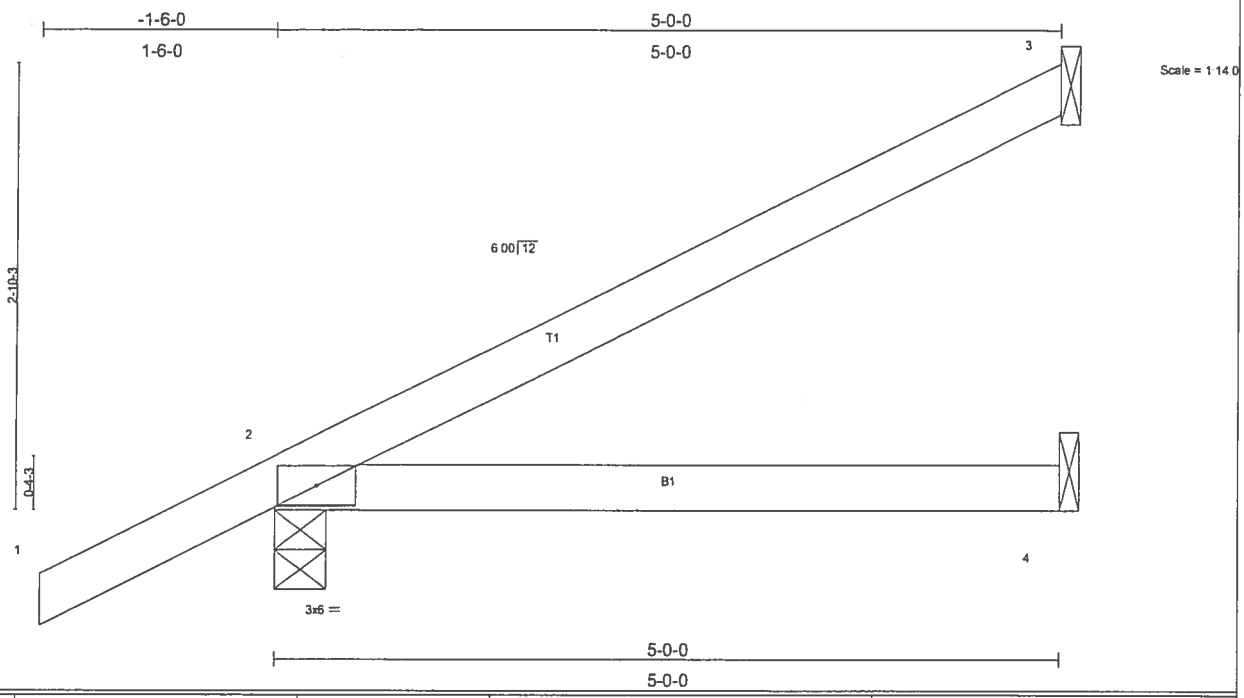
LOAD CASE(S) Standard

JUNE 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0619061481

Job	Truss	Truss Type	Qty	Ply	0 0
L169928	CJ5	JACK	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Jun 19 10:33:02 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.24	Vert(LL)	-0.03	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.16	Vert(TL)	-0.05	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 18 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=113/Mechanical, 2=306/0-4-0, 4=72/Mechanical
Max Horz 2=162(load case 5)
Max Uplift3=-101(load case 5), 2=-159(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-96/41
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.13

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 101 lb uplift at joint 3 and 159 lb uplift at joint 2.

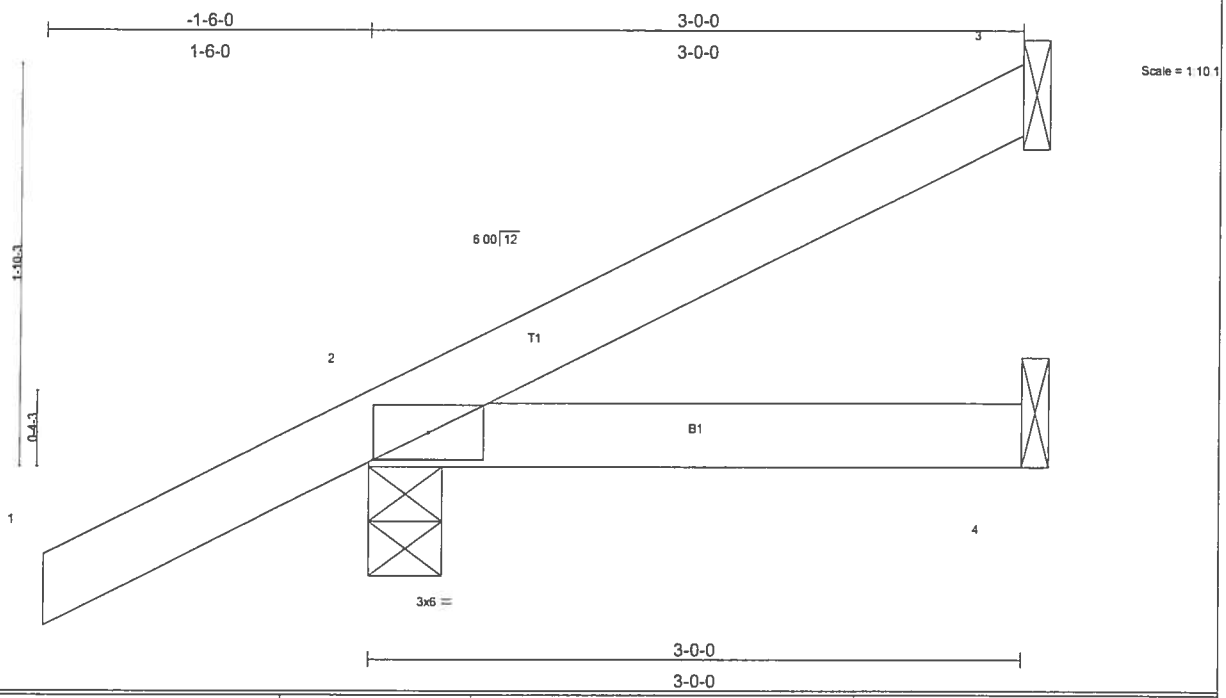
LOAD CASE(S) Standard

JUNE 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0619061482

Job	Truss	Truss Type	Qty	Ply	0 0
L169928	EJ3	MONO TRUSS	9	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Jun 19 10:33:05 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.17	Vert(LL)	-0.00	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.05	Vert(TL)	-0.01	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 12 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=48/Mechanical, 2=233/0-4-0, 4=42/Mechanical
Max Horz 2=115(load case 5)
Max Uplift3=-37(load case 5), 2=-153(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-49/16
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.11

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 37 lb uplift at joint 3 and 153 lb uplift at joint 2.

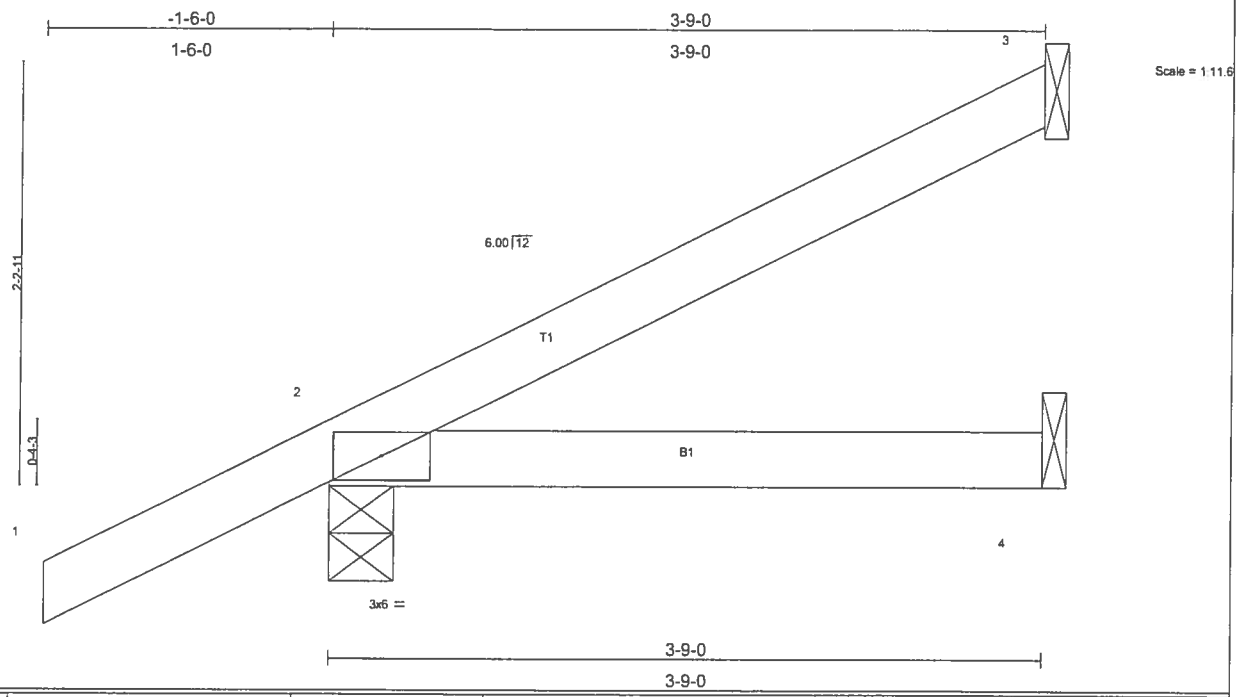
LOAD CASE(S) Standard

JUNE 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0619061483

Job	Truss	Truss Type	Qty	Ply	0 0
L169928	EJ4	JACK	7	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Jun 19 10:33:07 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.17	Vert(LL)	-0.01	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.09	Vert(TL)	-0.01	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 15 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-9-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=74/Mechanical, 2=259/0-4-0, 4=53/Mechanical
Max Horz 2=133(load case 5)
Max Uplift3=-63(load case 5), 2=-153(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-65/26
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.11

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 63 lb uplift at joint 3 and 153 lb uplift at joint 2.

LOAD CASE(S) Standard

JUNE 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0619061484

Job	Truss	Truss Type	Qty	Ply	0 0
L169928	EJ7	MONO TRUSS	19	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Jun 19 10:33:11 2006 Page 1

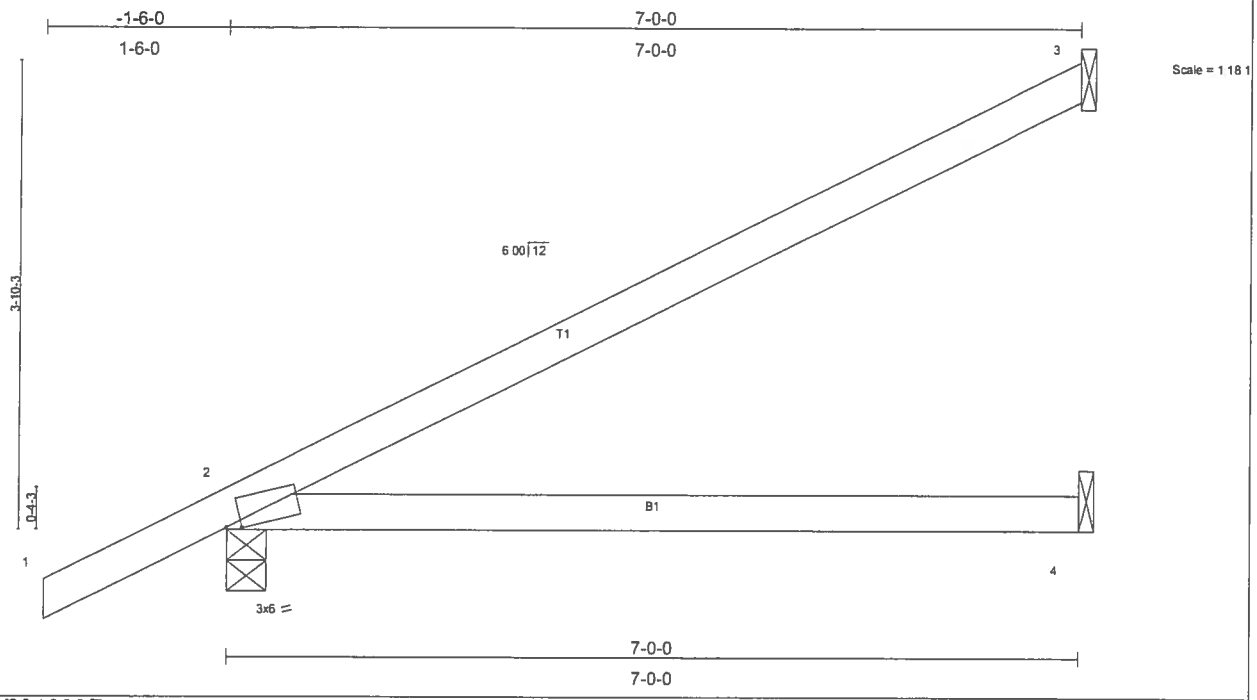


Plate Offsets (X,Y): [2:0-1-9,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.46	Vert(LL)	-0.13	2-4	>614	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.37	Vert(TL)	-0.22	2-4	>370	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 25 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=165/Mechanical, 2=385/0-4-0, 4=108/Mechanical
Max Horz 2=208(load case 5)
Max Uplift3=-138(load case 5), 2=-173(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-122/59
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.91

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 138 lb uplift at joint 3 and 173 lb uplift at joint 2.

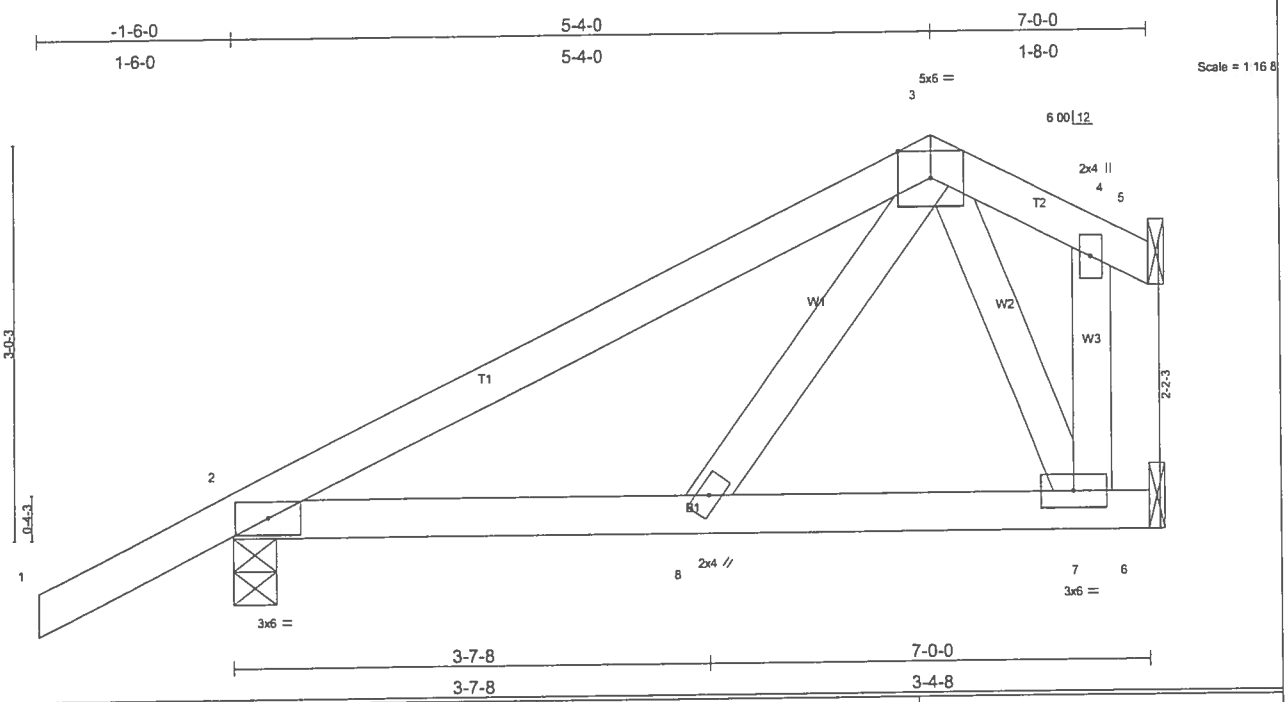
LOAD CASE(S) Standard

JUNE 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0619061485

Job	Truss	Truss Type	Qty	Ply	0 0
L169928	EJ7A	COMMON	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Jun 19 10:33:13 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.21	Vert(LL)	0.01	2-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.12	Vert(TL)	-0.01	2-8	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.09	Horz(TL)	-0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 36 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 5=198/Mechanical, 6=76/Mechanical, 2=385/0-4-0
Max Horz 6=145(load case 5)
Max Uplift5=-64(load case 5), 6=-29(load case 6), 2=-196(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 3-4=-31/79, 4-5=-87/55, 1-2=0/35, 2-3=-338/110
BOT CHORD 2-8=0/243, 7-8=0/150, 6-7=-0/182
WEBS 3-8=-4/166, 3-7=-362/291, 4-7=-191/288

JOINT STRESS INDEX
2 = 0.30, 3 = 0.39, 4 = 0.20, 7 = 0.20 and 8 = 0.11

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Refer to girder(s) for truss to truss connections.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 64 lb uplift at joint 5, 29 lb uplift at joint 6 and 196 lb uplift at joint 2.

LOAD CASE(S) Standard

JUNE 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0619061486

Job	Truss	Truss Type	Qty	Ply	0 0
L169928	EJ7B	SPECIAL	1	1	Job Reference (optional)

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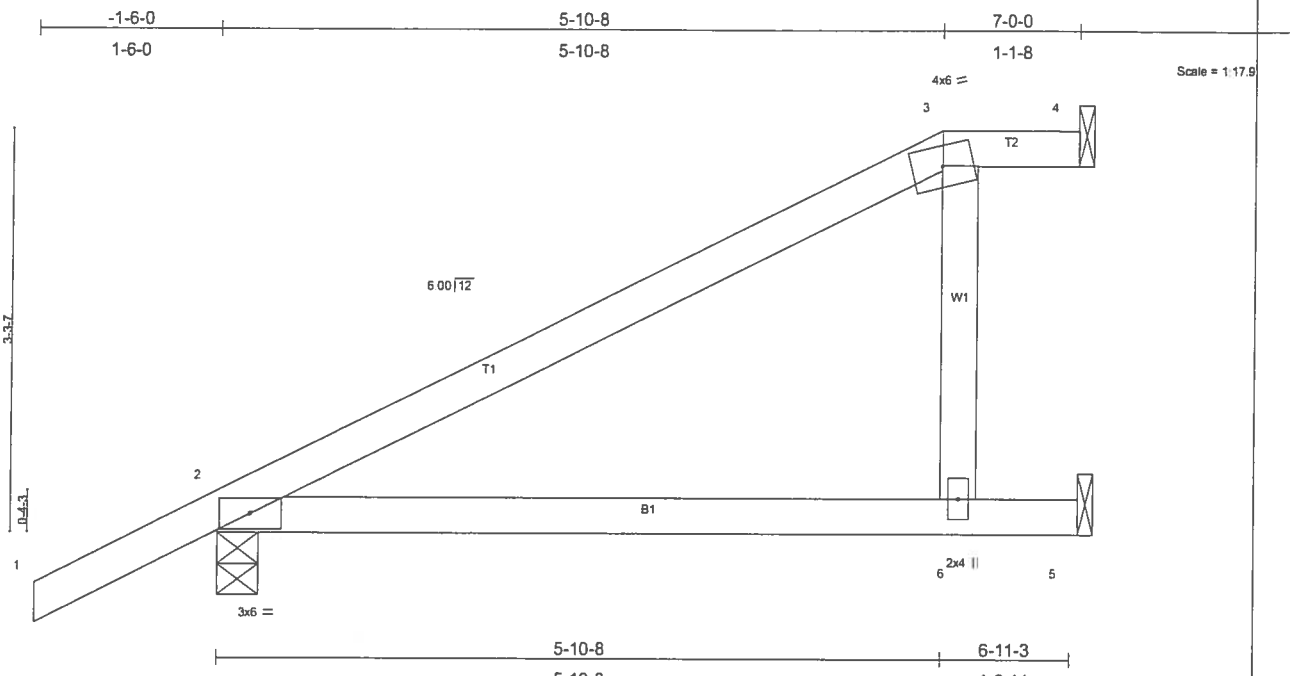

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.40	Vert(LL)	0.17	2-6	>487	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.42	Vert(TL)	-0.25	2-6	>327	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.06	Horz(TL)	0.06	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 29 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 4=77/Mechanical, 5=196/Mechanical, 2=385/0-4-0
Max Horz 5=184(load case 5)
Max Uplift4=-9(load case 3), 5=-128(load case 5), 2=-183(load case 5)
Max Grav 4=94(load case 10), 5=196(load case 1), 2=385(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 3-4=-0/1, 1-2=0/35, 2-3=-97/42
BOT CHORD 2-6=-0/241, 5-6=-0/254
WEBS 3-6=-111/250

JOINT STRESS INDEX
2 = 0.36, 3 = 0.42 and 6 = 0.14

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 9 lb uplift at joint 4, 128 lb uplift at joint 5 and 183 lb uplift at joint 2.

LOAD CASE(S) Standard

JUNE 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0619061487

Job	Truss	Truss Type	Qty	Ply	0 0
L169928	HJ4	JACK	2	1	Job Reference (optional)

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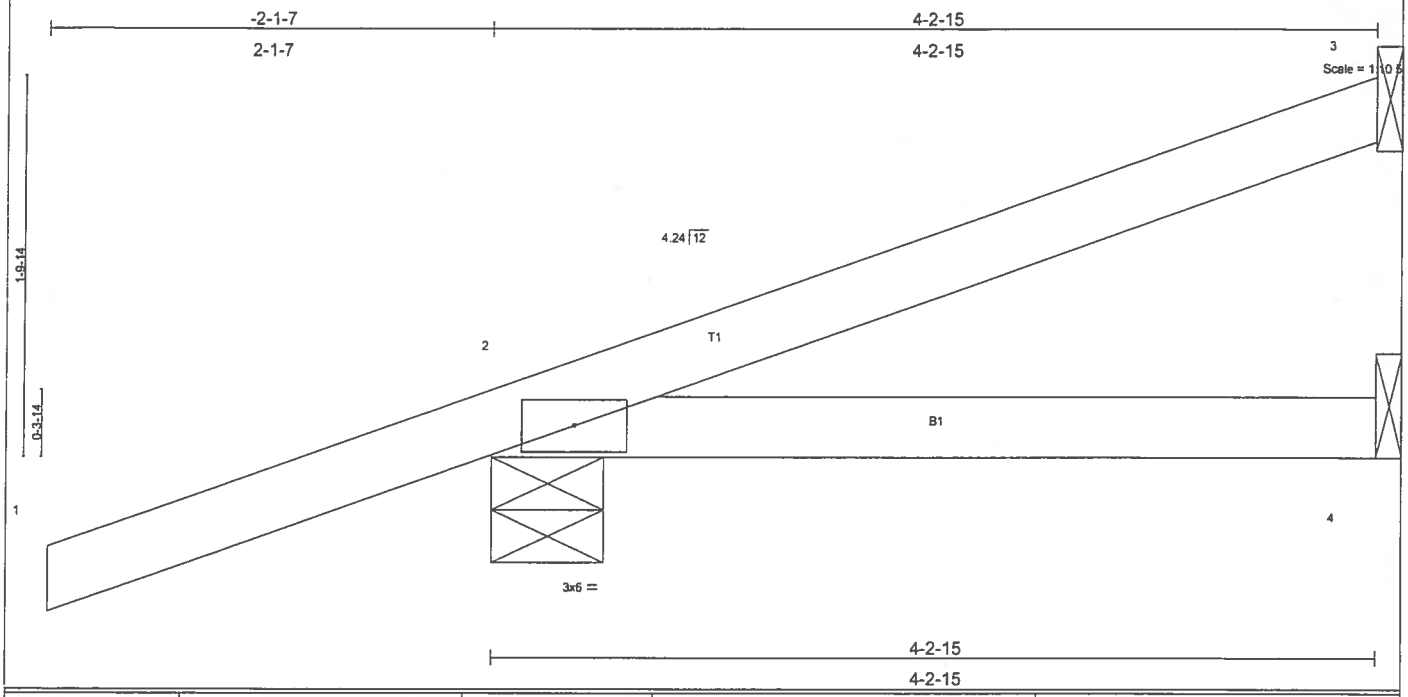

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.33	Vert(LL)	-0.01	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.07	Vert(TL)	-0.01	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 16 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-2-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=38/Mechanical, 2=231/0-6-7, 4=42/Mechanical
Max Horz 2=82(load case 2)
Max Uplift3=-15(load case 2), 2=-189(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/38, 2-3=-29/6
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.09

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 15 lb uplift at joint 3 and 189 lb uplift at joint 2.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-4(F=25, B=25)-to-3=-57(F=-2, B=-2), 2=-0(F=15, B=15)-to-4=-32(F=-1, B=-1)

JUNE 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0619061488

Job	Truss	Truss Type	Qty	Ply	0 0
L169928	HJ5	JACK	4	1	Job Reference (optional)

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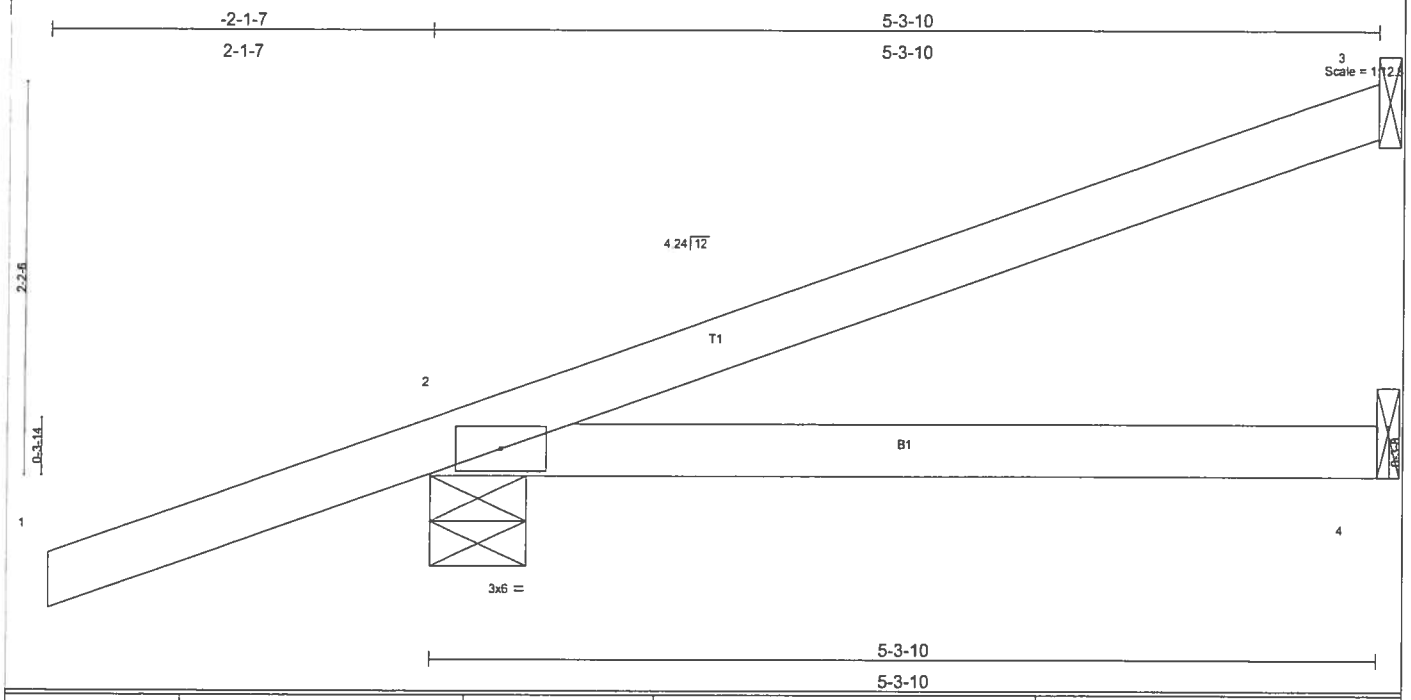

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.33	Vert(LL)	-0.02	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.13	Vert(TL)	-0.04	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 20 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-3-10 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=91/Mechanical, 2=258/0-6-7, 4=66/Mechanical
Max Horz 2=104(load case 2)
Max Uplift3=-63(load case 2), 2=-187(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/38, 2-3=-34/20
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.10

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 63 lb uplift at joint 3 and 187 lb uplift at joint 2.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-4(F=25, B=25)-to-3=-72(F=-9, B=-9), 2=0(F=15, B=15)-to-4=-40(F=-5, B=-5)

JUNE 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0619061489

Job	Truss	Truss Type	Qty	Ply	0 0
L169928	HJ9	MONO TRUSS	1	1	Job Reference (optional)

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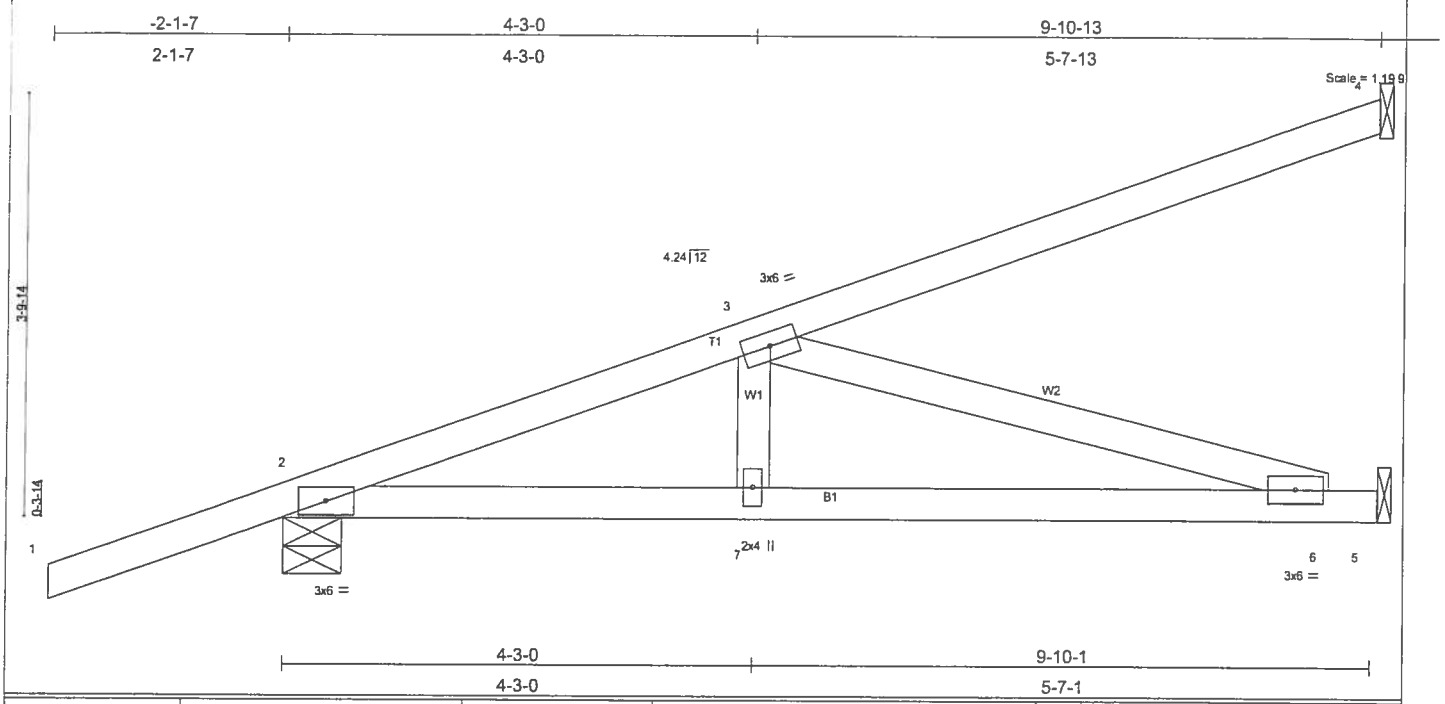

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.33	Vert(LL)	-0.07	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.40	Vert(TL)	-0.11	6-7	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.39	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 43 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=129/Mechanical, 2=547/0-6-7, 5=257/Mechanical
Max Horz 2=208(load case 2)
Max Uplift4=-102(load case 2), 2=-275(load case 2), 5=-61(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/38, 2-3=-768/150, 3-4=-58/33
BOT CHORD 2-7=-261/684, 6-7=-261/684, 5-6=0/0
WEBS 3-7=0/171, 3-6=-713/272

JOINT STRESS INDEX
2 = 0.52, 3 = 0.19, 6 = 0.20 and 7 = 0.13

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 102 lb uplift at joint 4, 275 lb uplift at joint 2 and 61 lb uplift at joint 5.

LOAD CASE(S) Standard

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THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0619061490

Job	Truss	Truss Type	Qty	Ply	0 0
L169928	T01	HIP	1	2	Job Reference (optional)

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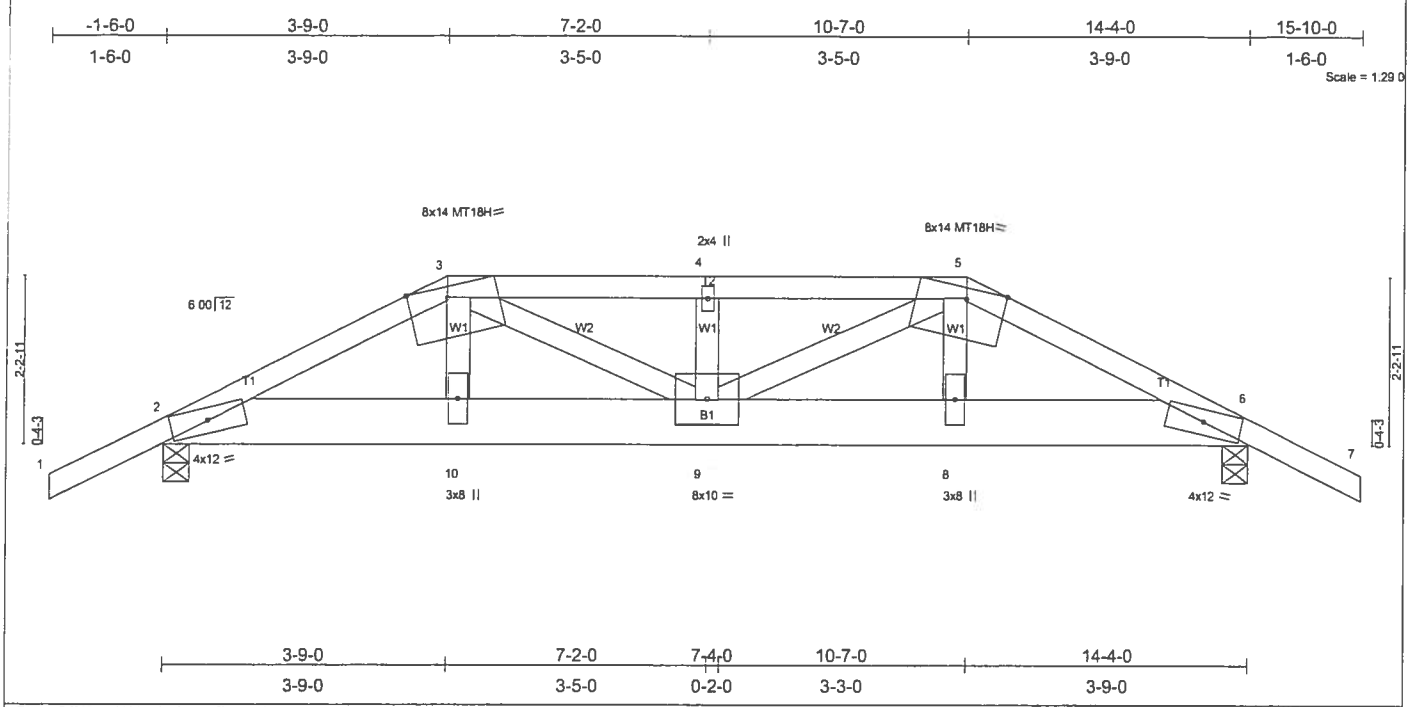

Plate Offsets (X,Y): [3:0-6-3,Edge], [5:0-6-3,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.50	Vert(LL)	-0.16	9-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.48	Vert(TL)	-0.26	9-10	>658	180	MT18H	244/190
BCLL	10.0	Rep Stress Incr	NO	WB	0.81	Horz(TL)	0.04	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 176 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-7-4 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=5910/0-4-0, 6=3607/0-4-0
Max Horz 2=-59(load case 5)
Max Uplift2=-2271(load case 4), 6=-1402(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/42, 2-3=-10198/3823, 3-4=-11297/4268, 4-5=-11297/4268, 5-6=-7620/2847, 6-7=0/42
BOT CHORD 2-10=-3402/9129, 9-10=-3497/9392, 8-9=-2526/6864, 6-8=-2508/6803
WEBS 3-10=-1152/3201, 3-9=-856/2182, 4-9=-88/137, 5-9=-1931/5027, 5-8=-226/746

JOINT STRESS INDEX
2 = 0.75, 3 = 0.76, 4 = 0.05, 5 = 0.76, 6 = 0.75, 8 = 0.52, 9 = 0.68 and 10 = 0.52

NOTES
1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-7-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc, Except member 4-9 2 X 4 - 2 rows at 0-6-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.
6) All plates are MT20 plates unless otherwise indicated.
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2271 lb uplift at joint 2 and 1402 lb uplift at joint 6.
9) Girder carries tie-in span(s): 33-8-0 from 0-0-0 to 7-4-0
10) Girder carries hip end with 3-9-0 end setback.
11) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 118 lb down and 61 lb up at 10-7-0, and 118 lb down and 61 lb up at 3-9-0, and 3118 lb down and 1177 lb up at 7-4-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-5=-73(F=-19), 5-7=-54, 2-10=-688(B=-658), 9-10=-699(F=-11, B=-658), 8-9=-41(F=-11), 6-8=-30
Concentrated Loads (lb)
Vert: 10=-118(F) 9=-3118(F) 8=-118(F)

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THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0619061491

Job	Truss	Truss Type	Qty	Ply	0 0
L169928	T02	HIP	1	1	Job Reference (optional)

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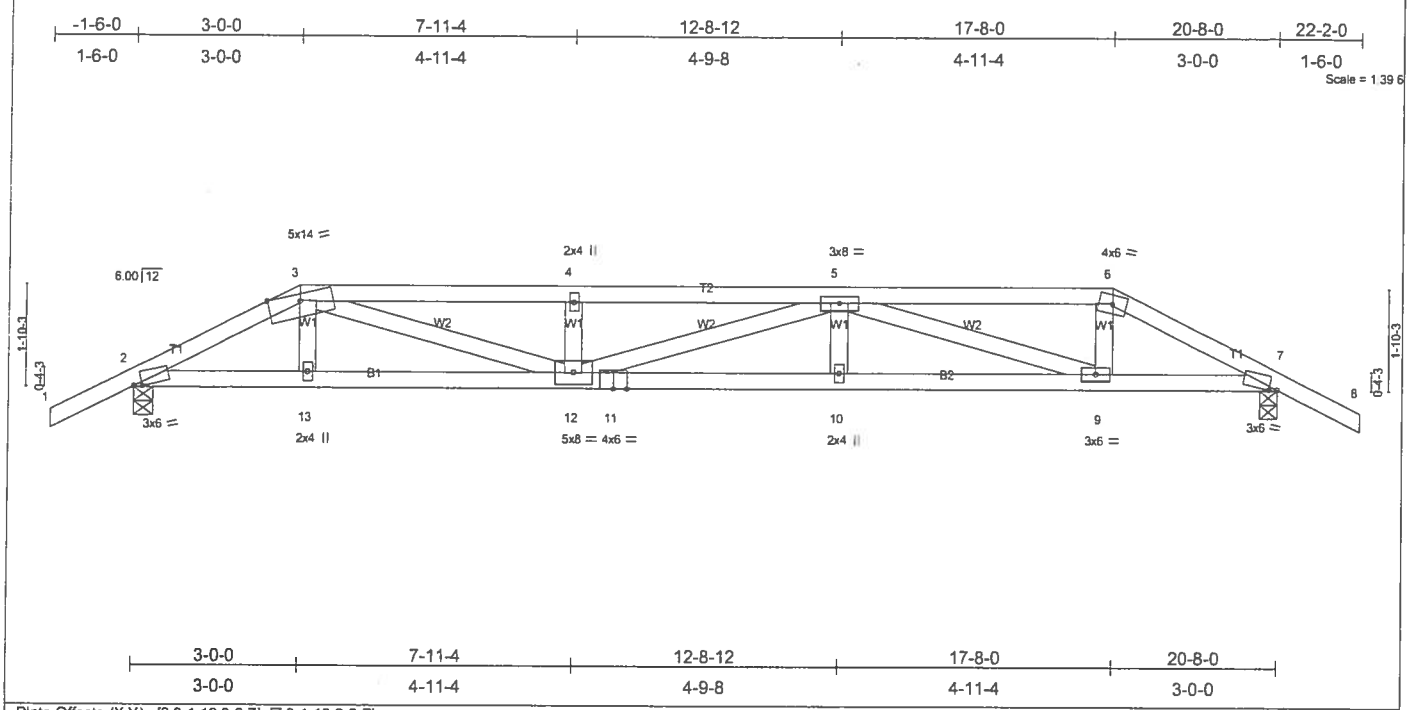

Plate Offsets (X,Y): [2:0-1-13,0-0-7], [7:0-1-13,0-0-7]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.29	Vert(LL) -0.25 10-12 >980 240	MT20 244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.70	Vert(TL) -0.40 10-12 >611 180	
BCLL 10.0	Rep Stress Incr NO	WB 0.69	Horz(TL) 0.08 7 n/a n/a	
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)		Weight: 96 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-4-5 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-10-0 oc bracing.

REACTIONS (lb/size) 2=1109/0-4-0, 7=1109/0-4-0
Max Horz 2=-50(load case 5)
Max Uplift2=-398(load case 4), 7=-398(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1942/714, 3-4=-3182/1232, 4-5=-3182/1232, 5-6=-1741/667, 6-7=-1939/712, 7-8=0/35
BOT CHORD 2-13=-608/1681, 12-13=-608/1699, 11-12=-1178/3184, 10-11=-1178/3184, 9-10=-1178/3184, 7-9=-591/1678
WEBS 3-13=-4/200, 3-12=-646/1573, 4-12=-301/225, 5-12=-19/16, 5-10=0/166, 5-9=-1538/635, 6-9=-188/675

JOINT STRESS INDEX
2 = 0.81, 3 = 0.49, 4 = 0.34, 5 = 0.70, 6 = 0.50, 7 = 0.81, 9 = 0.44, 10 = 0.34, 11 = 0.93, 12 = 0.74 and 13 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 398 lb uplift at joint 2 and 398 lb uplift at joint 7.
6) Girder carries hip end with 3-0-0 end setback.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 63 lb down and 32 lb up at 17-8-0, and 63 lb down and 32 lb up at 3-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-6=-63(F=-9), 6-8=-54, 2-13=-30, 9-13=-35(F=-5), 7-9=-30
Concentrated Loads (lb)
Vert: 13=-63(F) 9=-63(F)

JUNE 19, 2006 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0619061492

Job	Truss	Truss Type	Qty	Ply	0 0
L169928	T03	HIP	1	2	Job Reference (optional)

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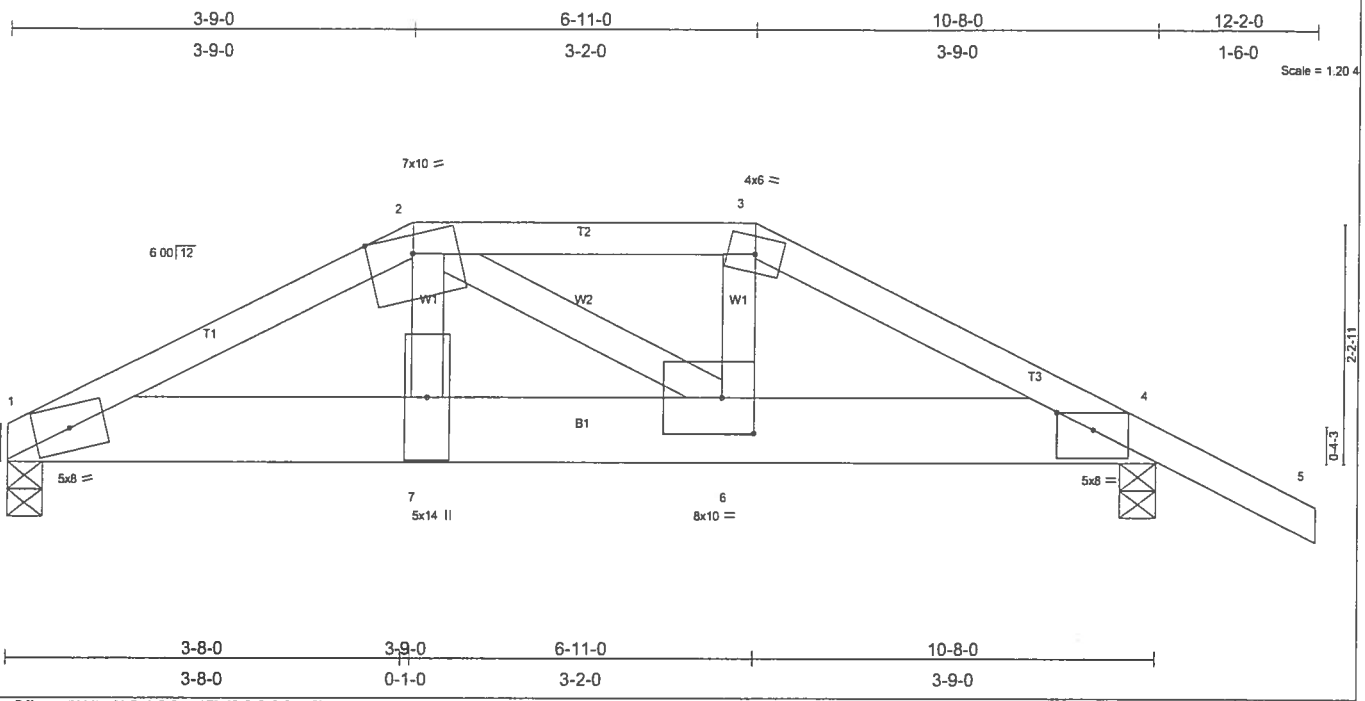

Plate Offsets (X,Y): [4:0-4-0,0-1-15], [6:0-3-8,0-4-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.25	Vert(LL)	-0.07	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.33	Vert(TL)	-0.11	6-7	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.58	Horz(TL)	0.02	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 122 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-8-11 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=3808/0-4-0, 4=4613/0-4-0
Max Horz 1=-81(load case 5)
Max Uplift1=-1404(load case 4), 4=-1776(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-7746/2851, 2-3=-6614/2449, 3-4=-7132/2605, 4-5=0/42
BOT CHORD 1-7=-2510/6905, 6-7=-2618/7201, 4-6=-2282/6372
WEBS 2-7=-1313/3597, 2-6=-720/291, 3-6=-1072/2980

JOINT STRESS INDEX
1 = 0.80, 2 = 0.84, 3 = 0.80, 4 = 0.80, 6 = 0.31 and 7 = 0.62

NOTES
1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 3 rows at 0-4-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1404 lb uplift at joint 1 and 1776 lb uplift at joint 4.
8) Girder carries tie-in span(s): 33-8-0 from 3-8-0 to 10-8-0
9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 3118 lb down and 1177 lb up at 3-8-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 2-3=-54, 3-5=-54, 1-7=-30, 4-7=-688(F=-658)
Concentrated Loads (lb)
Vert: 7=-3118(F)

JUNE 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0619061493

Job	Truss	Truss Type	Qty	Ply	0 0
L169928	T04	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Jun 19 10:33:34 2006 Page 1

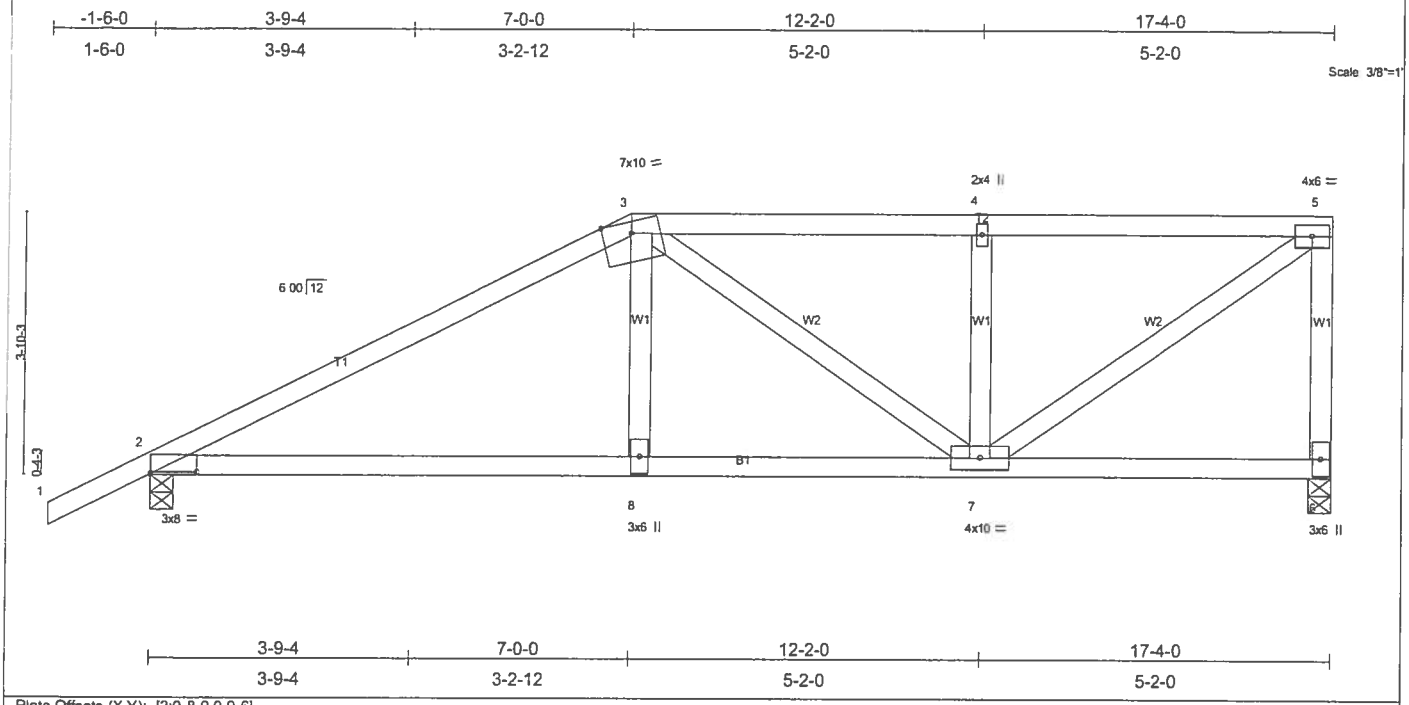

Plate Offsets (X,Y): [2:0-8-0,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.93	Vert(LL)	-0.11	2-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.69	Vert(TL)	-0.19	2-8	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.66	Horz(TL)	0.04	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 87 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-8-5 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-5-15 oc bracing.

REACTIONS (lb/size) 6=1629/0-4-0, 2=1423/0-4-0
Max Horz 2=210(load case 4)
Max Uplift6=-729(load case 3), 2=-627(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-2439/982, 3-4=-1759/789, 4-5=-1759/790, 5-6=-1462/747
BOT CHORD 2-8=-924/2100, 7-8=-934/2134, 6-7=-38/88
WEBS 3-8=-236/834, 3-7=-455/257, 4-7=-608/542, 5-7=-921/2048

JOINT STRESS INDEX
2 = 0.76, 3 = 0.79, 4 = 0.34, 5 = 0.85, 6 = 0.46, 7 = 0.97 and 8 = 0.27

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 729 lb uplift at joint 6 and 627 lb uplift at joint 2.
5) Girder carries hip end with 0-0-0 right side setback, 7-0-0 left side setback, and 7-0-0 end setback.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-5=-117(F=-63), 2-8=-30, 6-8=-65(F=-35)
Concentrated Loads (lb)
Vert: 8=-539(F)

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THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0619061494

Job	Truss	Truss Type	Qty	Ply	0 0
L169928	T05	MONO HIP	1	1	Job Reference (optional)

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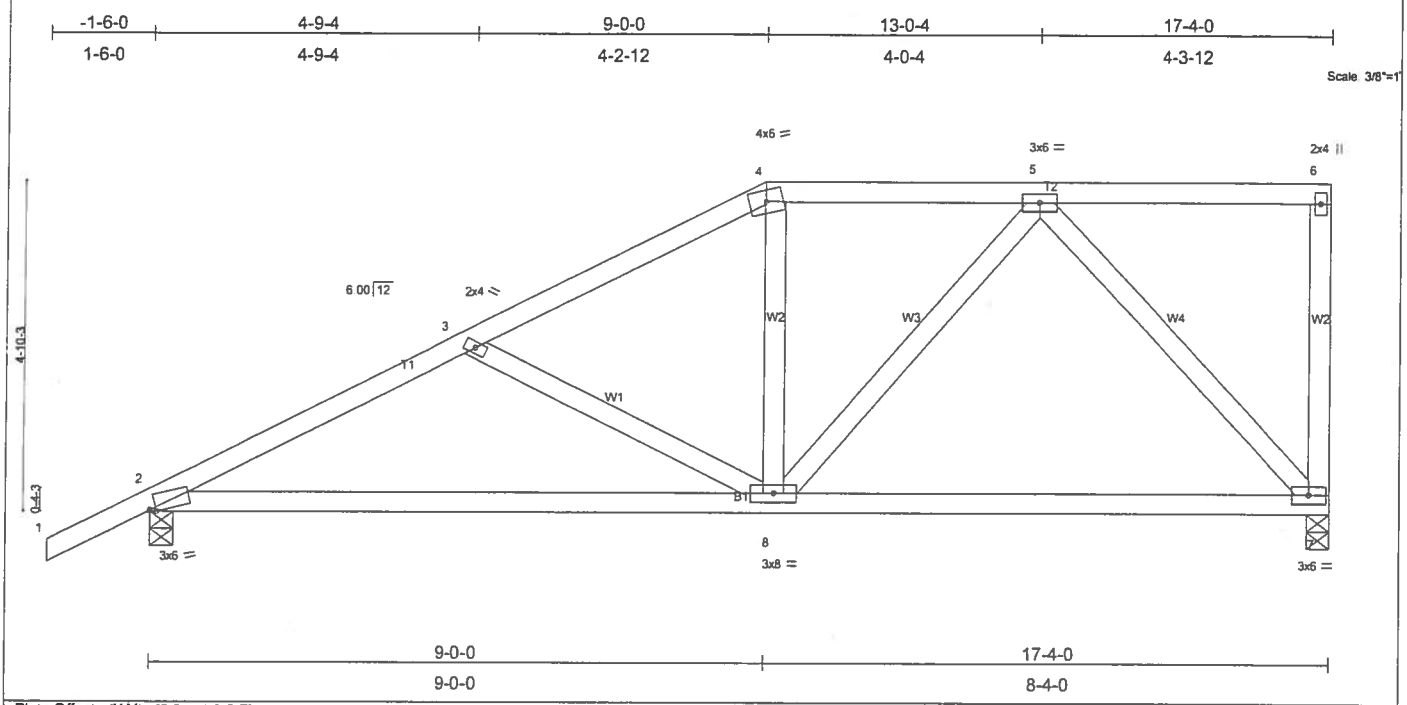

Plate Offsets (X,Y): [2:0-1-1,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	-0.13	2-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.45	Vert(TL)	-0.23	2-8	>905	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.45	Horz(TL)	0.02	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 93 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-7-3 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-7-1 oc bracing.

REACTIONS (lb/size) 7=710/0-4-0, 2=809/0-4-0
Max Horz 2=256(load case 5)
Max Uplift7=-245(load case 4), 2=-313(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1122/431, 3-4=-855/305, 4-5=-718/320, 5-6=-36/4, 6-7=-110/74
BOT CHORD 2-8=-540/967, 7-8=-238/487
WEBS 3-8=-286/246, 4-8=0/140, 5-8=-126/356, 5-7=-669/348

JOINT STRESS INDEX
2 = 0.80, 3 = 0.34, 4 = 0.39, 5 = 0.39, 6 = 0.66, 7 = 0.48 and 8 = 0.57

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 245 lb uplift at joint 7 and 313 lb uplift at joint 2.

LOAD CASE(S) Standard

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THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0619061495

Job	Truss	Truss Type	Qty	Ply	0 0
L169928	T06	MONO HIP	1	1	Job Reference (optional)

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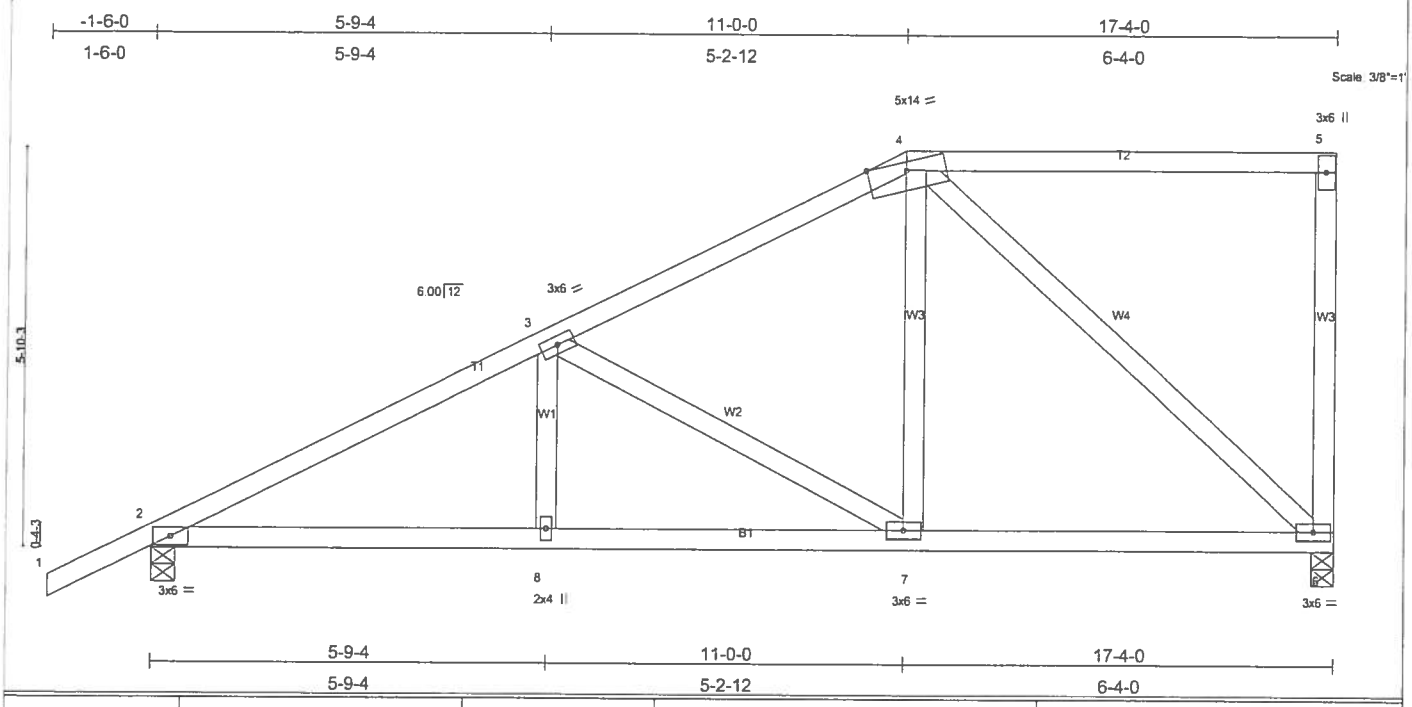

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.42	Vert(LL)	-0.05	2-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.32	Vert(TL)	-0.08	2-8	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.80	Horz(TL)	0.02	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 96 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-7-8 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-9-0 oc bracing.

REACTIONS (lb/size) 6=710/0-4-0, 2=809/0-4-0
Max Horz 2=302(load case 5)
Max Uplift6=-235(load case 5), 2=-312(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1162/366, 3-4=-662/240, 4-5=-34/14, 5-6=-159/114
BOT CHORD 2-8=-522/972, 7-8=-522/972, 6-7=-276/548
WEBS 3-8=0/179, 3-7=-495/282, 4-7=-93/408, 4-6=-690/353

JOINT STRESS INDEX
2 = 0.53, 3 = 0.41, 4 = 0.78, 5 = 0.32, 6 = 0.40, 7 = 0.35 and 8 = 0.34

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 235 lb uplift at joint 6 and 312 lb uplift at joint 2.

LOAD CASE(S) Standard

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THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0619061496

Job	Truss	Truss Type	Qty	Ply	0 0
L169928	T07	MONO HIP	1	1	Job Reference (optional)

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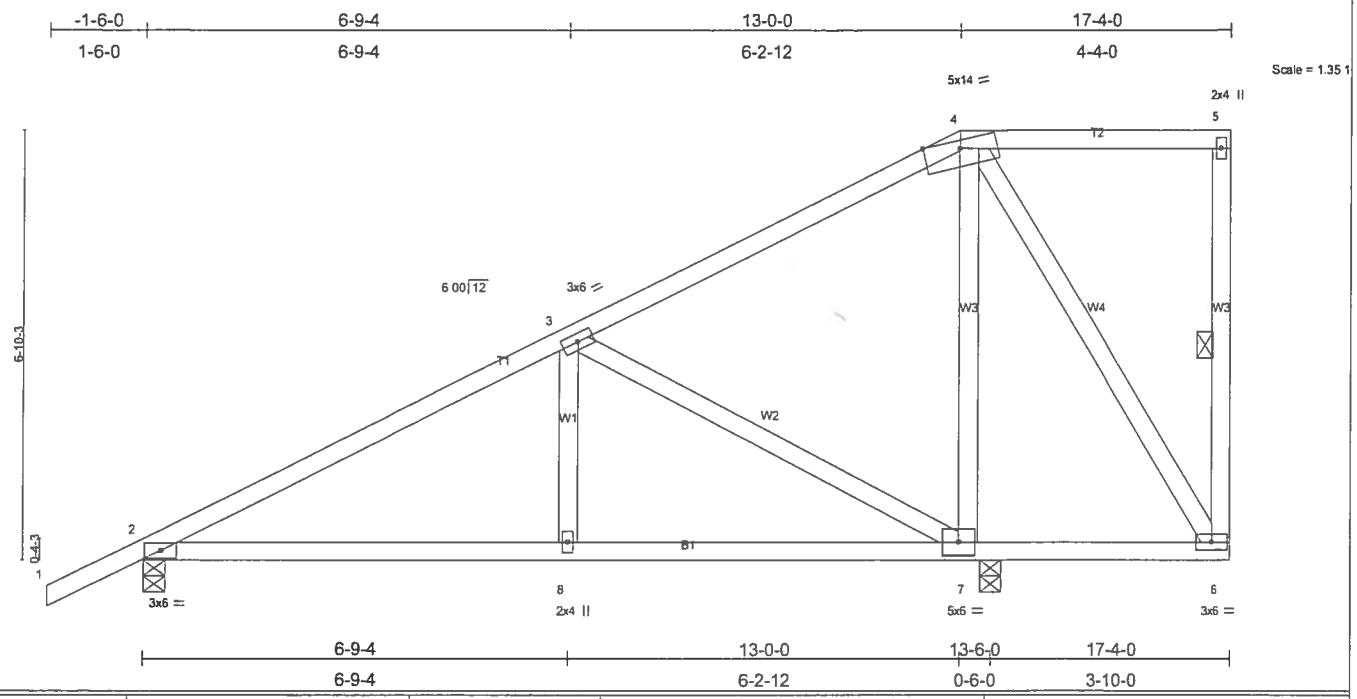


LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.28	Vert(LL)	-0.07	2-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.34	Vert(TL)	-0.11	2-8	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.59	Horz(TL)	0.01	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 101 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 5-6

REACTIONS (lb/size) 2=588/0-4-0, 7=932/0-4-0
Max Horz 2=348(load case 5)
Max Uplift2=-219(load case 5), 7=-403(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-643/81, 3-4=-191/145, 4-5=-5/8, 5-6=-85/73
BOT CHORD 2-8=-286/506, 7-8=-286/506, 6-7=-77/96
WEBS 3-8=0/225, 3-7=-650/360, 4-7=-460/474, 4-6=-193/153

JOINT STRESS INDEX
2 = 0.36, 3 = 0.41, 4 = 0.69, 5 = 0.34, 6 = 0.44, 7 = 0.19 and 8 = 0.34

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever right exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 219 lb uplift at joint 2 and 403 lb uplift at joint 7.

LOAD CASE(S) Standard

JUNE 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0619061497

Job	Truss	Truss Type	Qty	Ply	0 0
L169928	T08	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Jun 19 10:33:44 2006 Page 1

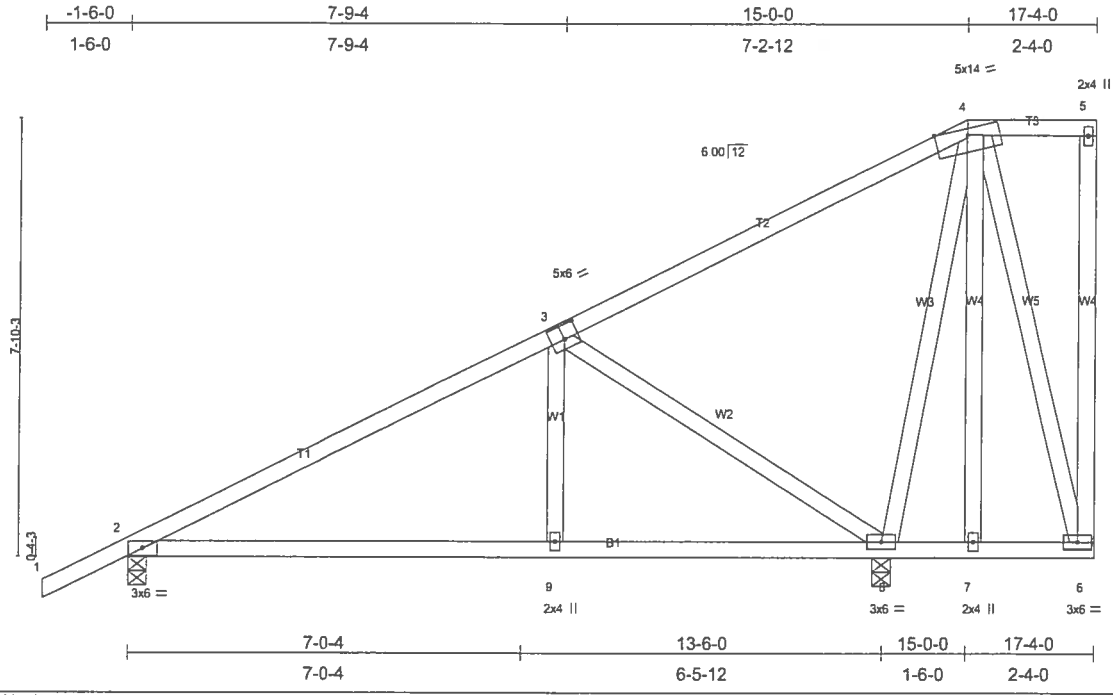


Plate Offsets (X,Y): [3:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	-0.11	2-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.39	Vert(TL)	-0.19	2-9	>869	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.62	Horz(TL)	0.01	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 116 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=621/0-4-0, 8=899/0-4-0
Max Horz 2=394(load case 5)
Max Uplift2=-207(load case 5), 8=-396(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-663/39, 3-4=-214/162, 4-5=-3/1, 5-6=-55/26
BOT CHORD 2-9=-284/517, 8-9=-285/512, 7-8=-15/6, 6-7=-15/6
WEBS 3-9=0/244, 3-8=-700/416, 4-8=-380/476, 4-7=-95/10, 4-6=-21/63

JOINT STRESS INDEX
2 = 0.53, 3 = 0.74, 4 = 0.46, 5 = 0.34, 6 = 0.52, 7 = 0.34, 8 = 0.37 and 9 = 0.34

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever right exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 207 lb uplift at joint 2 and 396 lb uplift at joint 8.

LOAD CASE(S) Standard

JUNE 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0619061498

Job	Truss	Truss Type	Qty	Ply	0 0
L169928	T09	COMMON	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Jun 19 10:33:46 2006 Page 1

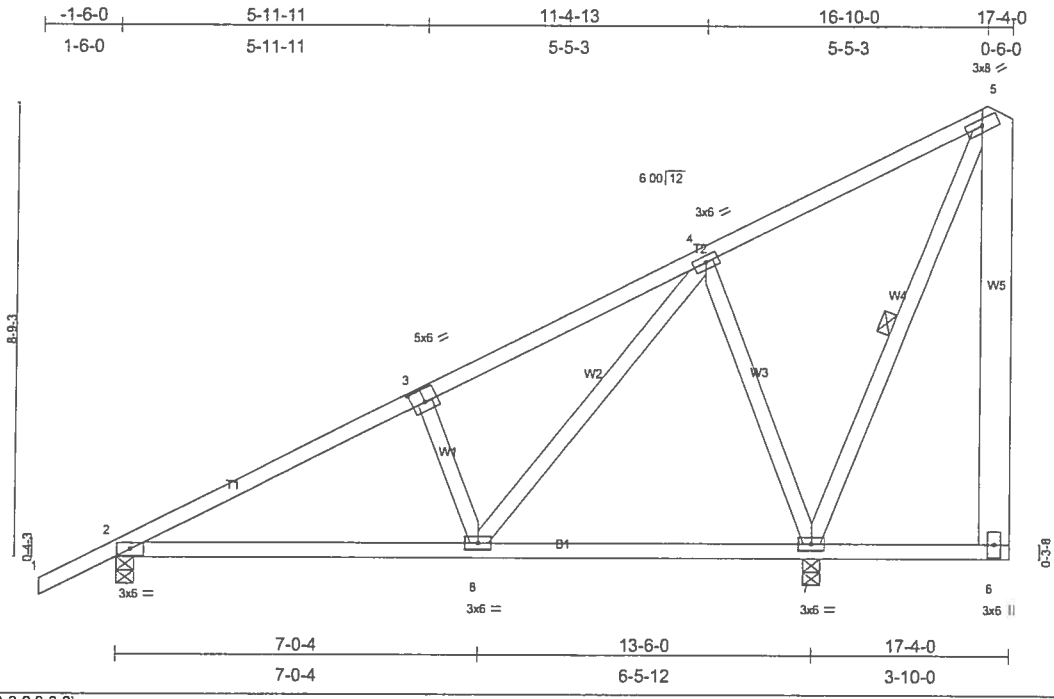


Plate Offsets (X,Y): [3:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.21	Vert(LL)	-0.07	2-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.28	Vert(TL)	-0.11	2-8	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.36	Horz(TL)	0.01	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 119 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 "Except"
W5 2 X 8 SYP 2400F 2.0E

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 5-7

REACTIONS (lb/size) 2=616/0-4-0, 7=890/0-4-0
Max Horz 2=440(load case 5)
Max Uplift2=-171(load case 5), 7=-458(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-718/0, 3-4=-608/41, 4-5=-209/151, 5-6=-110/35
BOT CHORD 2-8=-308/581, 7-8=-72/110, 6-7=-24/17
WEBS 3-8=-281/280, 4-8=-271/614, 4-7=-541/399, 5-7=-241/342

JOINT STRESS INDEX
2 = 0.35, 3 = 0.38, 4 = 0.46, 5 = 0.79, 6 = 0.16, 7 = 0.49 and 8 = 0.49

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever right exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 171 lb uplift at joint 2 and 458 lb uplift at joint 7.

LOAD CASE(S) Standard

JUNE 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0619061499

Job	Truss	Truss Type	Qty	Ply	0 0
L169928	T10	COMMON	1	1	Job Reference (optional)

Builders FirstSource, Lake City, FI 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Jun 19 10:33:49 2006 Page 1

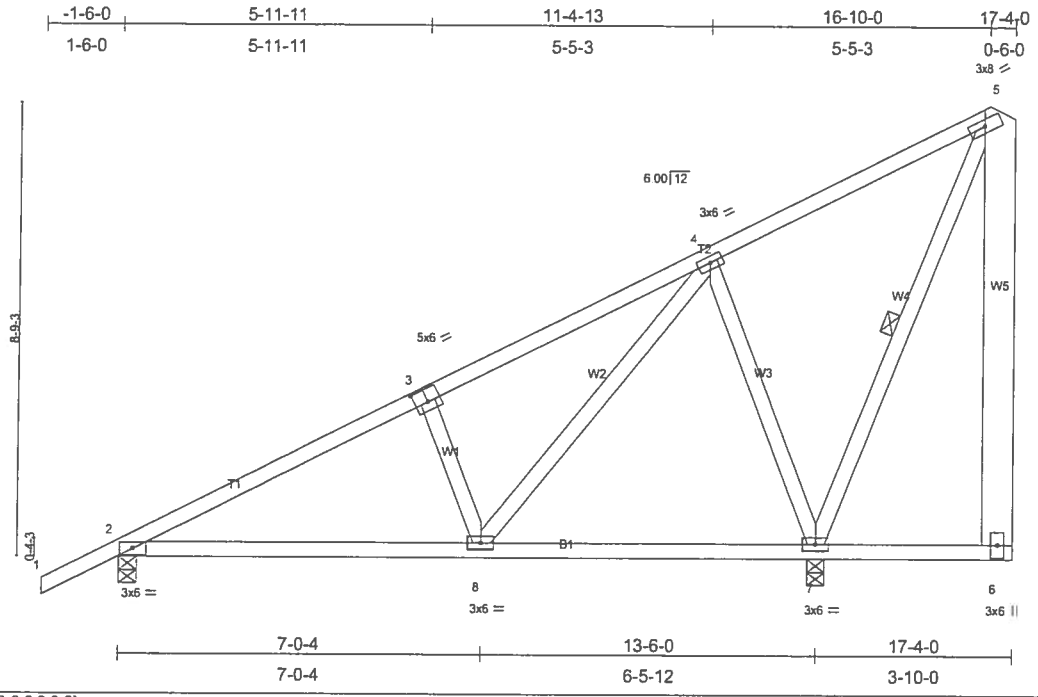


Plate Offsets (X,Y): [3:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.21	Vert(LL)	-0.07	2-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.28	Vert(TL)	-0.11	2-8	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.36	Horz(TL)	0.01	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 119 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W5 2 X 8 SYP No.1D

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 5-7

REACTIONS (lb/size) 2=616/0-4-0, 7=890/0-4-0
Max Horz 2=440(load case 5)
Max Uplift2=-171(load case 5), 7=-458(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-718/0, 3-4=-608/41, 4-5=-209/151, 5-6=-110/35
BOT CHORD 2-8=-308/581, 7-8=-72/110, 6-7=-23/17
WEBS 3-8=-281/280, 4-8=-271/614, 4-7=-541/399, 5-7=-241/342

JOINT STRESS INDEX
2 = 0.35, 3 = 0.38, 4 = 0.46, 5 = 0.78, 6 = 0.16, 7 = 0.49 and 8 = 0.49

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever right exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 171 lb uplift at joint 2 and 458 lb uplift at joint 7.

LOAD CASE(S) Standard

JUNE 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0619061500

Job	Truss	Truss Type	Qty	Ply	0 0
L169928	T11	COMMON	3	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Jun 19 10:33:52 2006 Page 1

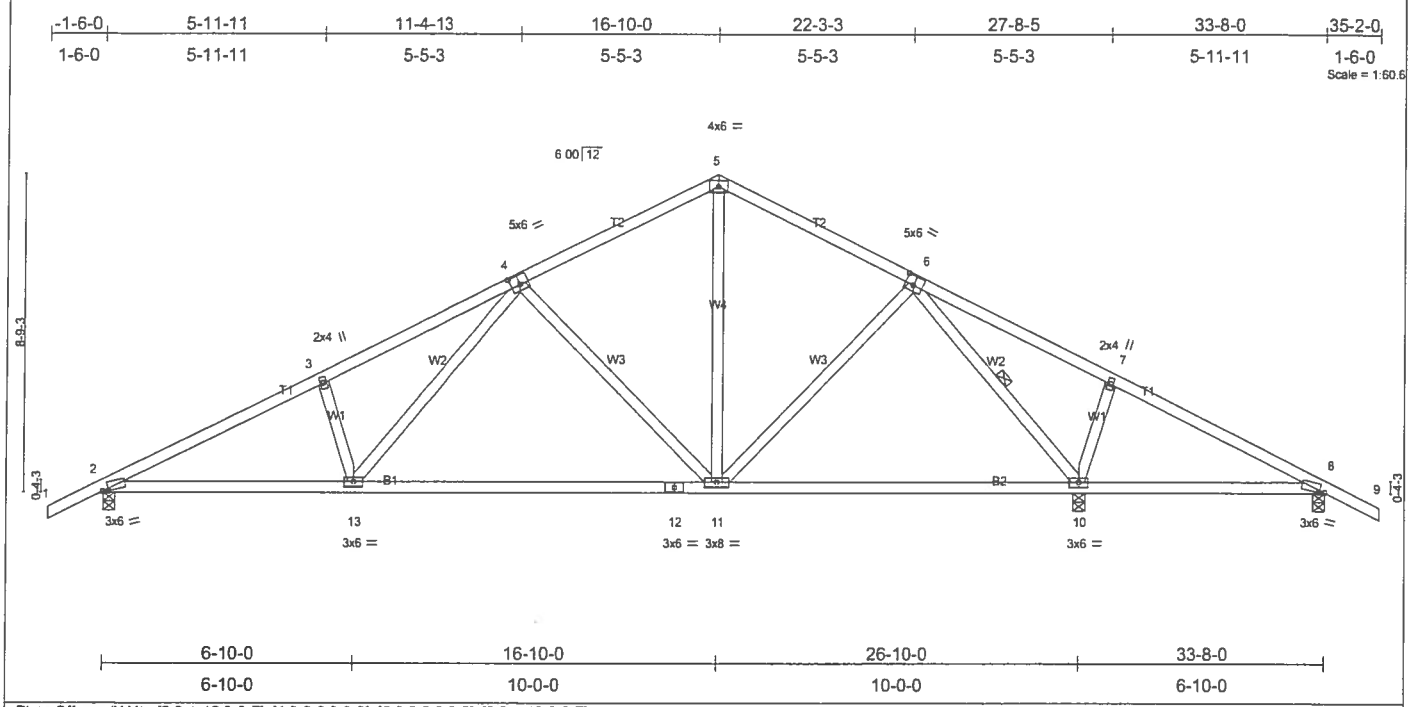


Plate Offsets (X,Y): [2:0-1-13,0-0-7], [4:0-3-0,0-3-0], [6:0-3-0,0-3-0], [8:0-1-13,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	0.13	8-10	>626	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.64	Vert(TL)	-0.35	11-13	>906	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.64	Horz(TL)	0.04	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 176 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 6-10

REACTIONS (lb/size) 2=1151/0-4-0, 10=1693/0-4-0, 8=136/0-4-0
Max Horz 2=146(load case 5)
Max Uplift2=-453(load case 5), 10=-594(load case 6), 8=-226(load case 6)
Max Grav 2=1151(load case 1), 10=1693(load case 1), 8=209(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1898/785, 3-4=-1799/853, 4-5=-906/514, 5-6=-905/513, 6-7=-84/501, 7-8=-146/425, 8-9=0/35
BOT CHORD 2-13=-551/1624, 12-13=-334/1143, 11-12=-334/1143, 10-11=-48/491, 8-10=-315/243
WEBS 3-13=-254/267, 4-13=-267/657, 4-11=-577/397, 5-11=-210/446, 6-11=-54/428, 6-10=-1430/595, 7-10=-305/317

JOINT STRESS INDEX
2 = 0.80, 3 = 0.34, 4 = 0.49, 5 = 0.60, 6 = 0.49, 7 = 0.34, 8 = 0.80, 10 = 0.50, 11 = 0.57, 12 = 0.60 and 13 = 0.50

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust), h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 453 lb uplift at joint 2, 594 lb uplift at joint 10 and 226 lb uplift at joint 8.

LOAD CASE(S) Standard

JUNE 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0619061501

Job	Truss	Truss Type	Qty	Ply	0 0
L169928	T12	SPECIAL	3	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Jun 19 10:33:56 2006 Page 1

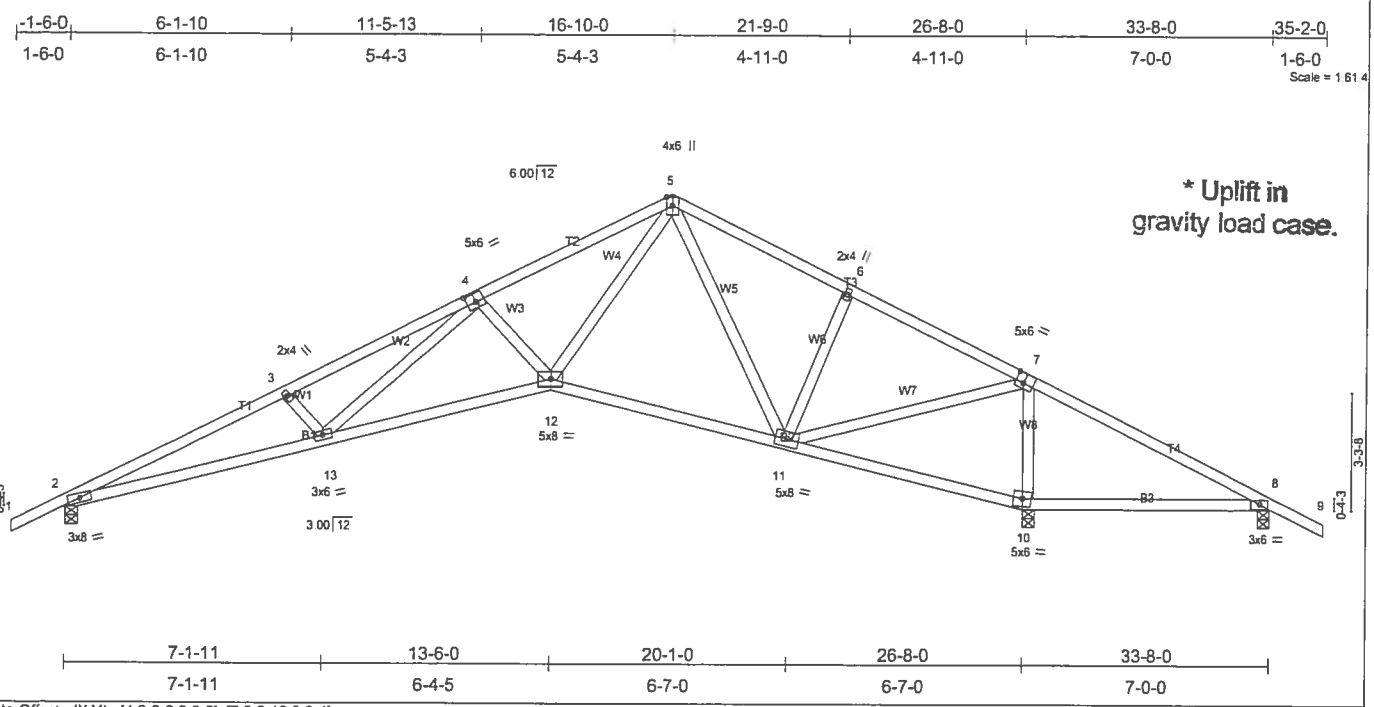


Plate Offsets (X,Y): [4:0-3-0,0-3-0], [7:0-2-12,0-3-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.57	Vert(LL)	-0.23	12-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.60	Vert(TL)	0.13	8-10	>648	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.68	Horz(TL)	0.19	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 169 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-5-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-10-9 oc bracing.

REACTIONS (lb/size) 2=1009/0-4-0, 10=2348/0-4-0, 8=-377/0-4-0
Max Horz 2=-146(load case 6)
Max Uplift2=-409(load case 5), 10=-777(load case 5), 8=-501(load case 9)
Max Grav 2=1009(load case 1), 10=2348(load case 1), 8=69(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/34, 2-3=-2826/1132, 3-4=-2629/1097, 4-5=-1583/660, 5-6=-410/317, 6-7=-514/283, 7-8=-516/1623, 8-9=0/35
BOT CHORD 2-13=-894/2542, 12-13=-483/1757, 11-12=-16/649, 10-11=-1462/630, 8-10=-1359/586
WEBS 3-13=-232/260, 4-13=-390/846, 4-12=-525/387, 5-12=-428/1387, 5-11=-687/235, 6-11=-216/209, 7-11=-554/1849, 7-10=-1756/788

JOINT STRESS INDEX
2 = 0.67, 3 = 0.34, 4 = 0.49, 5 = 0.67, 6 = 0.34, 7 = 0.79, 8 = 0.55, 10 = 0.97, 11 = 0.92, 12 = 0.69 and 13 = 0.53

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 409 lb uplift at joint 2, 777 lb uplift at joint 10 and 501 lb uplift at joint 8.

LOAD CASE(S) Standard

JUNE 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

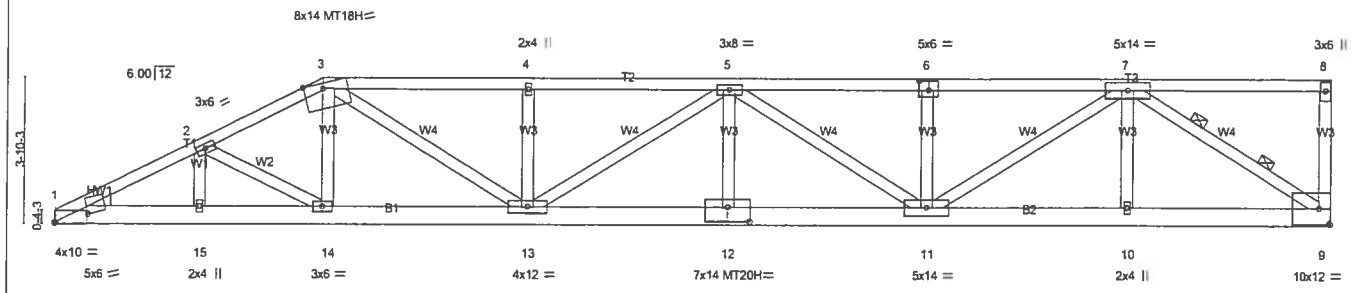
Dwg.#0619061502

Job	Truss	Truss Type	Qty	Ply	0 0
L169928	T13	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Jun 19 10:33:58 2006 Page 1

3-9-4	7-0-0	12-5-1	17-8-6	22-11-10	28-2-15	33-8-0
3-9-4	3-2-12	5-5-1	5-3-5	5-3-5	5-3-5	5-5-1

Scale = 1:58.1



3-9-4	7-0-0	12-5-1	17-8-6	22-11-10	28-2-15	33-8-0
3-9-4	3-2-12	5-5-1	5-3-5	5-3-5	5-3-5	5-5-1

Plate Offsets (X,Y): [1:0-0-0,0-0-4], [1:0-10-11,0-0-3], [3:0-6-3,Edge], [6:0-3-0,0-3-0], [12:0-7-0,0-4-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.82	Vert(LL)	-0.56	12-13	>722	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.92	Vert(TL)	-0.89	12-13	>451	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.96	Horz(TL)	0.17	9	n/a	n/a	MT18H	244/190
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 210 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-11-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-11-13 oc bracing.
WEBS 2 Rows at 1/3 pts 7-9

REACTIONS (lb/size) 1=3118/Mechanical, 9=3118/Mechanical
Max Horz 1=160(load case 4)
Max Uplift1=-1061(load case 3), 9=-1169(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-6117/2114, 2-3=-5664/2039, 3-4=-7053/2616, 4-5=-7052/2616, 5-6=-6549/2448, 6-7=-6551/2450, 7-8=-93/41, 8-9=-149/97
BOT CHORD 1-15=-1957/5442, 14-15=-1957/5442, 13-14=-1861/5094, 12-13=-2832/7593, 11-12=-2830/7587, 10-11=-1535/4099, 9-10=-1535/4099
WEBS 2-15=-64/249, 2-14=-444/240, 3-14=-319/987, 3-13=-923/2324, 4-13=-263/198, 5-13=-647/281, 5-12=-190/742, 5-11=-1243/457, 6-11=-275/194, 7-11=-1095/2936, 7-10=-183/740, 7-9=-4795/1788

JOINT STRESS INDEX
1 = 0.81, 1 = 0.85, 2 = 0.41, 3 = 0.82, 4 = 0.34, 5 = 0.59, 6 = 0.79, 7 = 0.81, 8 = 0.42, 9 = 0.82, 10 = 0.54, 11 = 0.83, 12 = 0.89, 13 = 0.96, 14 = 0.64 and 15 = 0.34

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide adequate drainage to prevent water ponding.
3) All plates are MT20 plates unless otherwise indicated.
4) Refer to girder(s) for truss to truss connections.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1061 lb uplift at joint 1 and 1169 lb uplift at joint 9.
7) Girder carries tie-in span(s): 7-0-0 from 0-0-0 to 33-8-0
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-8=-54, 1-9=-132(F=-102)

JUNE 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

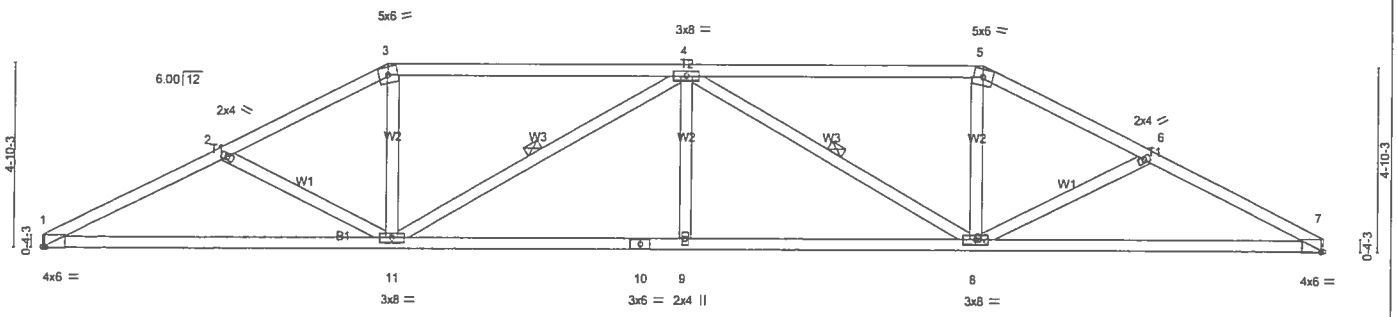
Dwg.#0619061503

Job	Truss	Truss Type	Qty	Ply	0 0
L169928	T14	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Jun 19 10:34:01 2006 Page 1

4-9-4	9-0-0	16-10-0	24-8-0	28-10-12	33-8-0
4-9-4	4-2-12	7-10-0	7-10-0	4-2-12	4-9-4

Scale = 1:57.8



9-0-0	16-10-0	24-8-0	33-8-0
9-0-0	7-10-0	7-10-0	9-0-0

Plate Offsets (X,Y): [1:0-0-12,0-0-2], [7:0-0-12,0-0-2]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.43	Vert(LL)	-0.25	1-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.73	Vert(TL)	-0.42	1-11	>965	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.28	Horz(TL)	0.14	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 162 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-5-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-5-0 oc bracing.
WEBS 1 Row at midpt 4-11, 4-8

REACTIONS (lb/size) 1=1409/Mechanical, 7=1409/Mechanical
Max Horz 1=-66(load case 3)
Max Uplift1=-401(load case 5), 7=-401(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2645/1139, 2-3=-2403/1017, 3-4=-2135/972, 4-5=-2135/972, 5-6=-2403/1017, 6-7=-2645/1139
BOT CHORD 1-11=-944/2325, 10-11=-969/2681, 9-10=-969/2681, 8-9=-969/2681, 7-8=-944/2325
WEBS 2-11=-244/236, 3-11=-198/713, 4-11=-739/313, 4-9=0/204, 4-8=-739/312, 5-8=-198/713, 6-8=-244/236

JOINT STRESS INDEX
1 = 0.82, 2 = 0.34, 3 = 0.67, 4 = 0.57, 5 = 0.67, 6 = 0.34, 7 = 0.82, 8 = 0.57, 9 = 0.34, 10 = 0.88 and 11 = 0.57

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 401 lb uplift at joint 1 and 401 lb uplift at joint 7.

LOAD CASE(S) Standard

JUNE 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

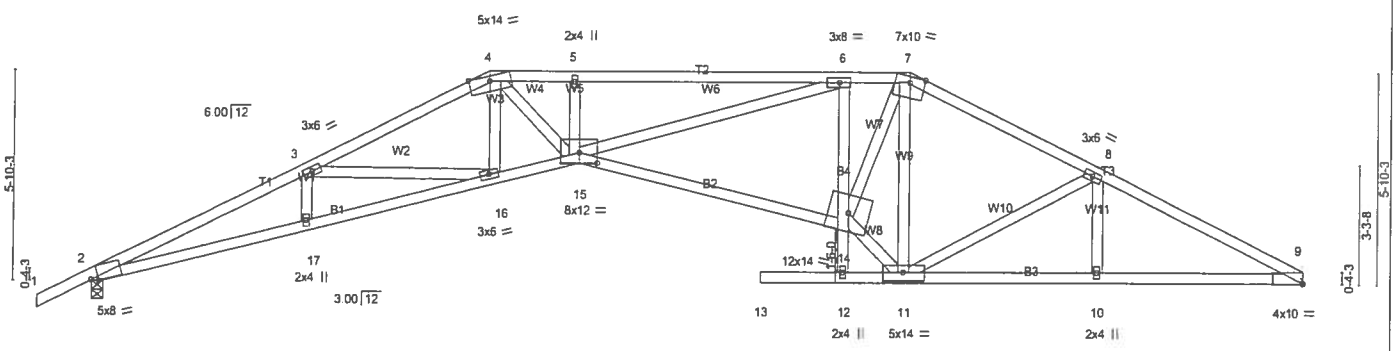
Dwg.#0619061504

Job	Truss	Truss Type	Qty	Ply	0 0
L169928	T15	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, FI 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Jun 19 10:34:04 2006 Page 1

-1-6-0	5-10-11	11-0-0	18-6-0	22-8-0	27-10-12	33-8-0
1-6-0	5-10-11	5-1-5	7-6-0	4-2-0	5-2-12	5-9-4

Scale = 1:61.0



5-10-11	11-0-0	13-6-0	18-6-0	20-8-0	22-8-0	27-10-12	33-8-0
5-10-11	5-1-5	2-6-0	5-0-0	2-2-0	2-0-0	5-2-12	5-9-4

Plate Offsets (X,Y): [2:0-2-7,Edge], [9:0-0-0,0-0-4], [15:0-6-0,0-3-10]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.96	Vert(LL)	-0.67	14-15	>599	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.98	Vert(TL)	-1.08	14-15	>372	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.87	Horz(TL)	0.61	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 180 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except* B4 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.

REACTIONS (lb/size) 9=1443/Mechanical, 2=1526/0-4-0
Max Horz 2=123(load case 5)
Max Uplift9=-409(load case 6), 2=-507(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/34, 2-3=-4924/1855, 3-4=-4203/1569, 4-5=-5193/1940, 5-6=-5177/1928, 6-7=-3059/1228, 7-8=-2259/943, 8-9=-2785/1099
BOT CHORD 2-17=-1603/4439, 16-17=-1608/4445, 15-16=-1212/3839, 14-15=-1009/3239, 12-14=0/101, 6-14=-914/403, 12-13=0/0, 11-12=-62/19, 10-11=-899/2424, 9-10=-899/2424
WEBS 3-17=0/152, 3-16=-609/381, 4-16=-128/320, 4-15=-642/1981, 6-15=-693/2119, 11-14=-768/2700, 7-14=-788/2614, 7-11=-1568/430, 8-11=-535/326, 8-10=0/188, 5-15=-276/227

JOINT STRESS INDEX
2 = 0.95, 3 = 0.41, 4 = 0.98, 5 = 0.34, 6 = 0.97, 7 = 0.86, 8 = 0.41, 9 = 0.68, 10 = 0.34, 11 = 0.97, 12 = 0.36, 14 = 0.97, 15 = 0.77, 16 = 0.38 and 17 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 409 lb uplift at joint 9 and 507 lb uplift at joint 2.

LOAD CASE(S) Standard

JUNE 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

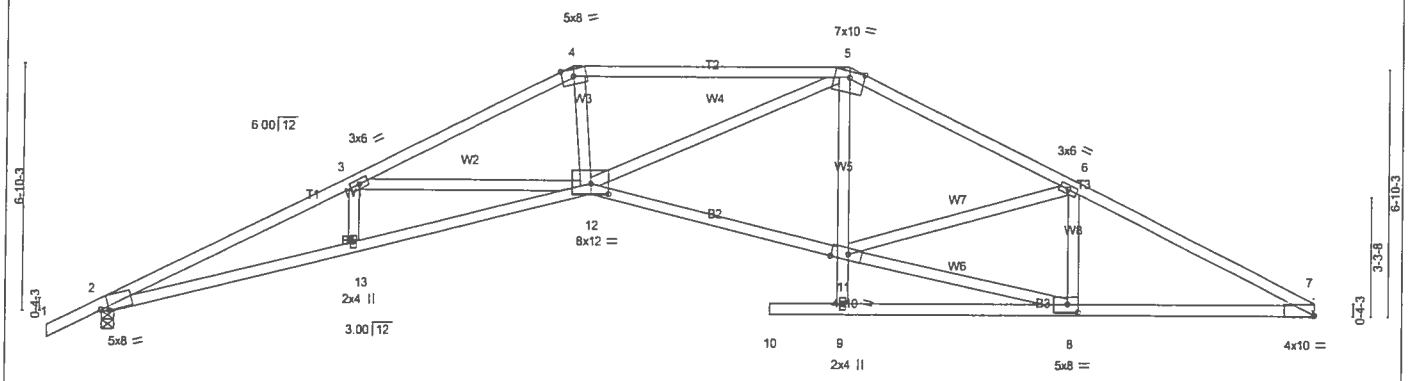
Dwg.#0619061505

Job	Truss	Truss Type	Qty	Ply	0 0
L169928	T16	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, FI 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Jun 19 10:34:06 2006 Page 1

-1-6-0	6-10-11	13-0-0	18-6-0	20-8-0	26-10-12	33-8-0
1-6-0	6-10-11	6-1-5	5-6-0	2-2-0	6-2-12	6-9-4

Scale = 1:61.0



6-10-11	13-6-0	18-6-0	20-8-0	26-10-12	33-8-0
6-10-11	6-7-5	5-0-0	2-2-0	6-2-12	6-9-4

Plate Offsets (X,Y): [2:0-2-7,Edge], [7:0-0-0,0-0-4], [8:0-3-8,0-2-8], [11:0-5-8,Edge], [12:0-6-0,0-3-10]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.76	Vert(LL)	-0.49	11-12	>811	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.78	Vert(TL)	-0.80	11-12	>500	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.77	Horz(TL)	0.45	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 172 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B1 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-3-0 oc bracing.

REACTIONS (lb/size) 7=1437/Mechanical, 2=1522/0-4-0
Max Horz 2=137(load case 5)
Max Uplift7=-425(load case 6), 2=-523(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/34, 2-3=-4881/1872, 3-4=-3901/1453, 4-5=-3632/1424, 5-6=-2728/1102, 6-7=-2722/1092
BOT CHORD 2-13=-1611/4408, 12-13=-1611/4409, 11-12=-747/2487, 9-10=0/0, 8-9=-10/28, 7-8=-879/2359
WEBS 3-13=0/198, 3-12=-839/524, 4-12=-371/1347, 5-12=-403/1383, 9-11=0/147, 5-11=-66/274, 6-11=-100/214, 6-8=-354/241, 8-11=-896/2405

JOINT STRESS INDEX
2 = 0.94, 3 = 0.41, 4 = 0.73, 5 = 0.75, 6 = 0.41, 7 = 0.66, 8 = 0.55, 9 = 0.34, 11 = 0.98, 12 = 0.81 and 13 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 425 lb uplift at joint 7 and 523 lb uplift at joint 2.

LOAD CASE(S) Standard

JUNE 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0619061506

Job	Truss	Truss Type	Qty	Ply	0 0
L169928	T17	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Jun 19 10:34:09 2006 Page 1

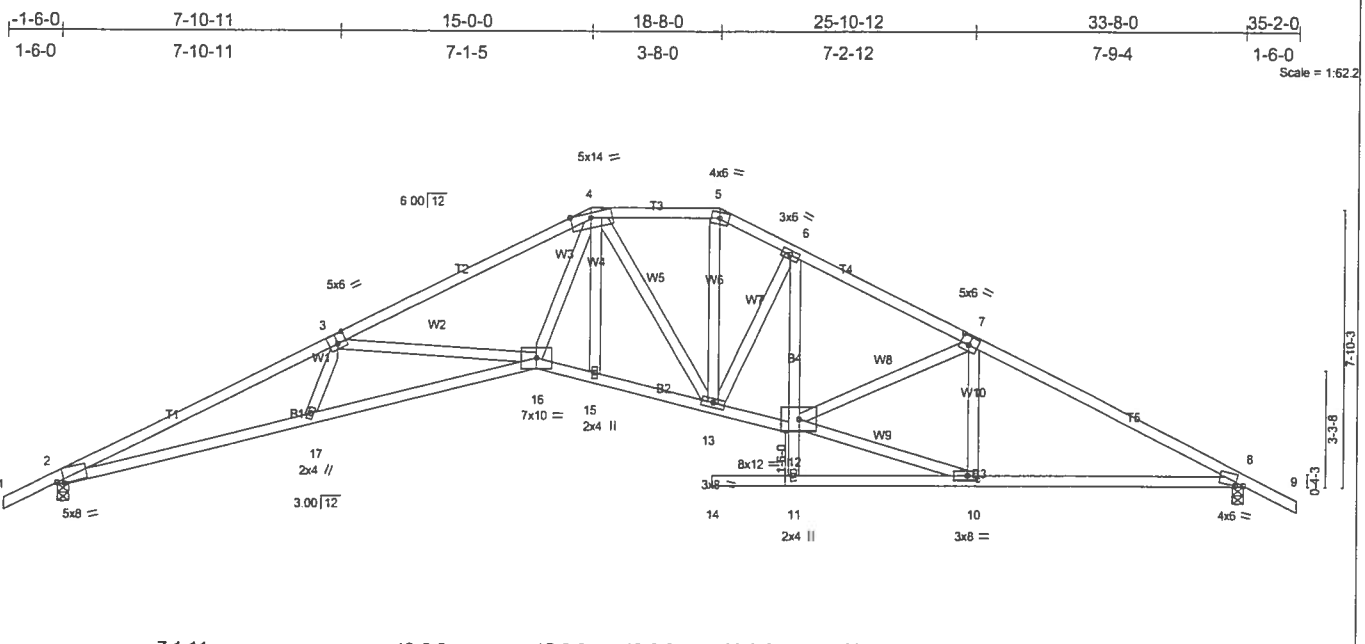


Plate Offsets (X,Y): [2:0-2-7,Edge], [3:0-3-0,0-3-4], [7:0-3-0,0-3-0], [8:0-2-13,0-0-11], [10:0-3-8,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.76	Vert(LL)	-0.46	16-17	>865	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.79	Vert(TL)	-0.75	16-17	>537	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.74	Horz(TL)	0.43	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 193 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2 *Except*
T1 2 X 4 SYP No.1D, T5 2 X 4 SYP No.1D
BOT CHORD 2 X 4 SYP No.2 *Except*
B1 2 X 4 SYP No.1D, B4 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-5-2 oc bracing.
JOINTS 1 Brace at Jt(s): 12

REACTIONS (lb/size) 2=1519/0-4-0, 8=1531/0-4-0
Max Horz 2=-133(load case 6)
Max Uplift2=-534(load case 5), 8=-532(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/34, 2-3=-4881/1800, 3-4=-3765/1350, 4-5=-2194/951, 5-6=-2448/1033, 6-7=-2726/1087, 7-8=-2616/1031, 8-9=0/35
BOT CHORD 2-17=-1479/4411, 16-17=-1490/4320, 15-16=-661/2695, 13-15=-666/2696, 12-13=-638/2420, 11-12=0/137, 6-12=-125/367, 11-14=0/0, 10-11=-31/56, 8-10=-744/2248
WEBS 3-17=0/260, 3-16=-912/605, 4-16=-586/2041, 4-15=0/50, 4-13=-802/281, 10-12=-755/2311, 7-12=-36/228, 7-10=-481/256, 5-13=-403/955, 6-13=-439/272

JOINT STRESS INDEX
2 = 0.94, 3 = 0.80, 4 = 0.83, 5 = 0.52, 6 = 0.45, 7 = 0.76, 8 = 0.87, 10 = 0.91, 11 = 0.34, 12 = 0.92, 13 = 0.64, 15 = 0.34, 16 = 0.82 and 17 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 534 lb uplift at joint 2 and 532 lb uplift at joint 8.

LOAD CASE(S) Standard

JUNE 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

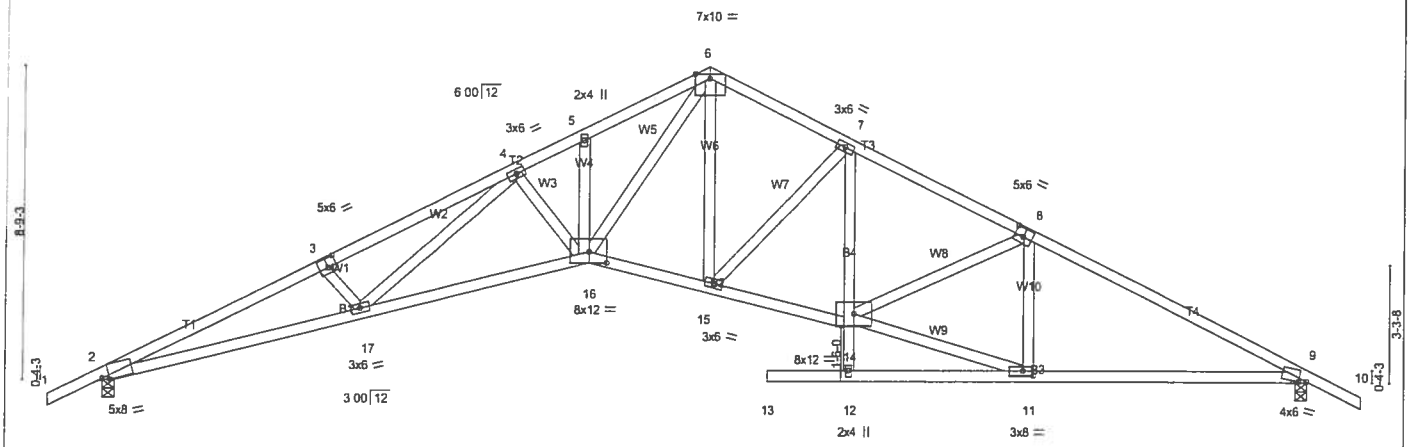
Dwg.#0619061507

Job	Truss	Truss Type	Qty	Ply	0 0
L169928	T18	SPECIAL	4	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Jun 19 10:34:13 2006 Page 1

-1-6-0 | 6-1-10 | 11-5-13 | 16-10-0 | 18-6-0 | 25-9-12 | 33-8-0 | 35-2-0
1-6-0 | 6-1-10 | 5-4-3 | 5-4-3 | 1-8-0 | 7-3-12 | 7-10-4 | 1-6-0

Scale = 1:61.4



7-1-11 | 13-6-0 | 18-6-0 | 20-8-0 | 25-9-12 | 33-8-0
7-1-11 | 6-4-5 | 5-0-0 | 2-2-0 | 5-1-12 | 7-10-4

Plate Offsets (X,Y): [2:0-2-7,Edge], [3:0-3-0,0-3-0], [8:0-3-0,0-3-0], [9:0-2-13,0-0-11], [11:0-3-8,0-1-8], [16:0-6-0,0-3-10]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.60	Vert(LL)	-0.51	16-17	>787	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.93	Vert(TL)	-0.82	16-17	>489	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.75	Horz(TL)	0.45	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 190 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B4 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-4-11 oc purlins.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
JOINTS 1 Brace at Jt(s): 14

REACTIONS (lb/size) 2=1519/0-4-0, 9=1531/0-4-0
Max Horz 2=-146(load case 6)
Max Uplift2=-545(load case 5), 9=-542(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/34, 2-3=-4870/1872, 3-4=-4696/1860, 4-5=-3739/1434, 5-6=-3696/1475, 6-7=-2323/985, 7-8=-2735/1108, 8-9=-2615/1047, 9-10=0/35
BOT CHORD 2-17=-1558/4401, 16-17=-1134/3723, 15-16=-454/2116, 14-15=-666/2453, 12-14=0/132, 7-14=-109/374, 12-13=0/0, 11-12=-32/31, 9-11=-758/2247
WEBS 3-17=-194/244, 4-17=-376/799, 4-16=-470/336, 6-15=-228/523, 7-15=-539/293, 11-14=-763/2330, 8-14=-4/215, 8-11=-493/262, 5-16=-54/70, 6-16=-810/2306

JOINT STRESS INDEX
2 = 0.94, 3 = 0.62, 4 = 0.52, 5 = 0.34, 6 = 0.81, 7 = 0.41, 8 = 0.76, 9 = 0.87, 11 = 0.92, 12 = 0.34, 14 = 0.77, 15 = 0.41, 16 = 0.69 and 17 = 0.50

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 545 lb uplift at joint 2 and 542 lb uplift at joint 9.

LOAD CASE(S) Standard

JUNE 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0619061508

Job	Truss	Truss Type	Qty	Ply	0 0
L169928	T19	COMMON	9	1	Job Reference (optional)

Builders FirstSource, Lake City, FL 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Jun 19 10:34:17 2006 Page 1

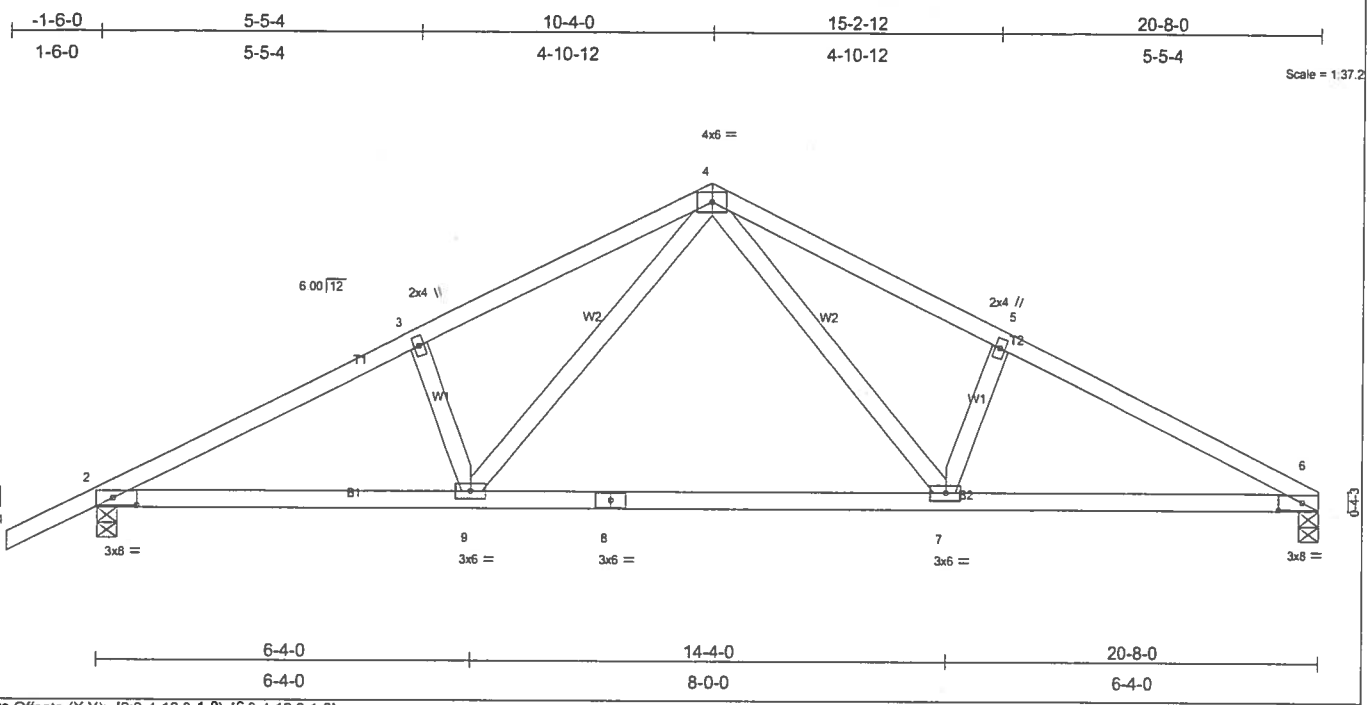


Plate Offsets (X,Y): [2:0-4-12,0-1-8], [6:0-4-12,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.31	7-9	>775	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	1.00	Vert(TL)	-0.51	7-9	>475	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.29	Horz(TL)	0.05	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 95 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-6-1 oc bracing.

REACTIONS (lb/size) 6=1050/0-4-0, 2=1148/0-4-0
Max Horz 2=119(load case 5)
Max Uplift6=-352(load case 6), 2=-451(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1961/821, 3-4=-1858/871, 4-5=-1876/899, 5-6=-1977/847
BOT CHORD 2-9=-644/1677, 8-9=-363/1104, 7-8=-363/1104, 6-7=-673/1696
WEBS 3-9=-218/227, 4-9=-340/853, 4-7=-379/877, 5-7=-228/242

JOINT STRESS INDEX
2 = 0.73, 3 = 0.34, 4 = 0.69, 5 = 0.34, 6 = 0.71, 7 = 0.61, 8 = 0.81 and 9 = 0.61

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 352 lb uplift at joint 6 and 451 lb uplift at joint 2.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-6=-54, 2-9=-30, 7-9=-80(F=-50), 6-7=-30

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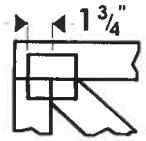
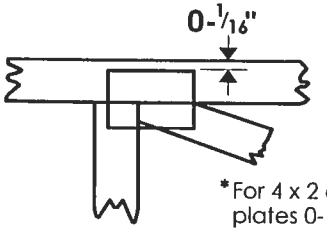
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and securely seat.



*For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

*This symbol indicates the required direction of slots in connector plates.

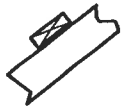
***Plate location details available in MiTek 20/20 software or upon request.**

PLATE SIZE

4 x 4

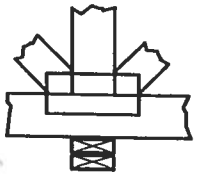
The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

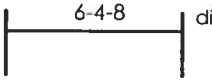


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

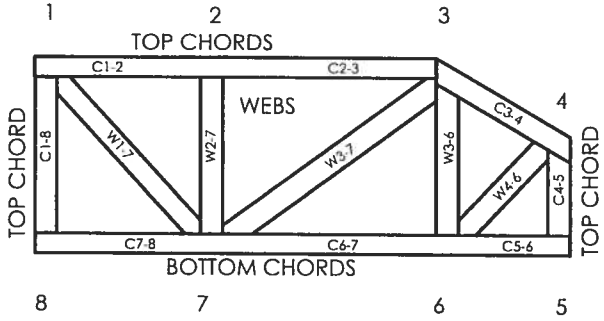
Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI1: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 95-43, 96-20-1, 96-67, 84-32
ICBO	4922, 5243, 5363, 3907
SBCCI	9667, 9730, 9604B, 9511, 9432A

MiTek®

TEE-LOK®

MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI1.
2. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
3. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
4. Cut members to bear tightly against each other.
5. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI1.
6. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI1.
7. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
8. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
9. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
10. Plate type, size, orientation and location dimensions shown indicate minimum plating requirements.
11. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
12. Top chords must be sheathed or purlins provided at spacing shown on design.
13. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
14. Connections not shown are the responsibility of others.
15. Do not cut or alter truss member or plate without prior approval of a professional engineer.
16. Install and load vertically unless indicated otherwise.