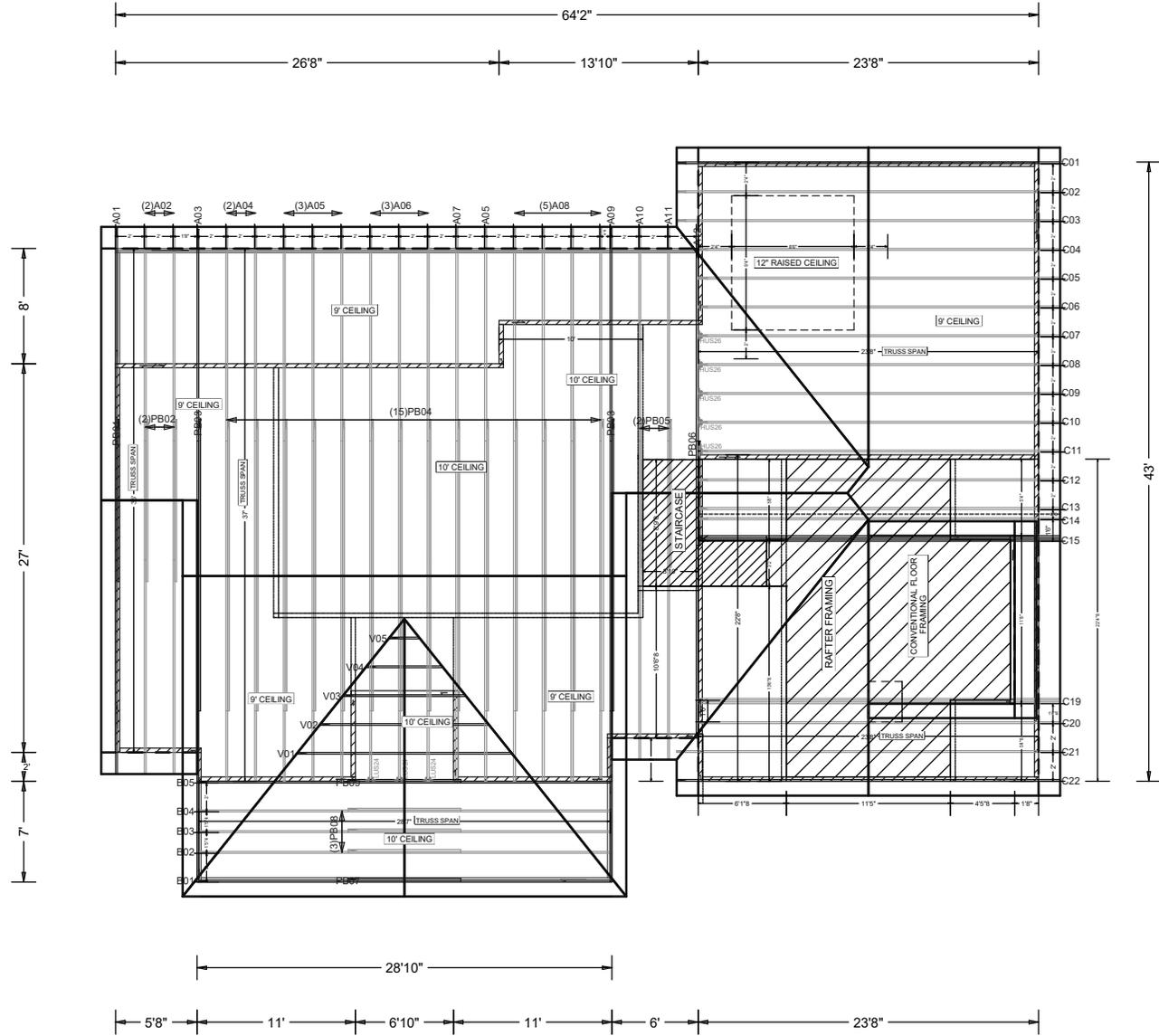


BEARING WALLS & BEAM HEIGHTS	
	09'00"00 ELEVATION
	09'00"00 ELEVATION 2 PLY
	10'00"00 ELEVATION
	10'00"00 ELEVATION 2 PLY
	18'04"00 ELEVATION
	18'04"00 ELEVATION

Total Truss Quantity = 108.



JOB #: 22-8147

Job Name: Edenfield
 Customer: Stanley Crawford Const.
 Designer: Fill in later
 ADDRESS: 3rd of sep
 SALESMAN: HOUSE
 : <Not Found>

JOB NO:
22-8147

PAGE NO:
1 OF 1

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COA #0 278
 Florida Certificate of Product Approval #FL1999
 08/24/2022

Alpine, an ITW Company
 155 Harlem Ave
 North Building, 4th Floor
 Glenview, IL 60025
 Phone: (800)755-6001
 www.alpineitw.com

Site Information:	Page 1:
Customer: W. B. Howland Company, Inc.	Job Number: 22-8147
Job Description: Edenfield	
Address: 3rd of sep	

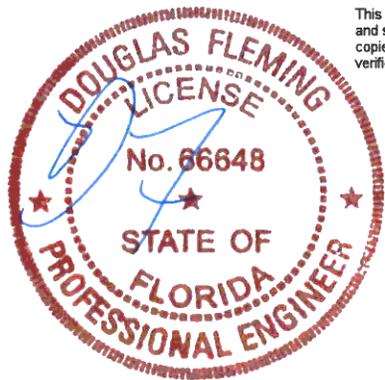
Job Engineering Criteria:	
Design Code: FBC 7th Ed. 2020 Res.	IntelliVIEW Version: 21.02.00 through 21.02.01 JRef #: 1Xlc2150005
Wind Standard: ASCE 7-16 Wind Speed (mph): 130	Design Loading (psf): 40.00
Building Type: Closed	

This package contains general notes pages, 50 truss drawing(s) and 9 detail(s).

Item	Drawing Number	Truss
1	235.22.1108.24190	A01
3	235.22.1109.08450	A03
5	235.22.1109.25000	A05
7	235.22.1109.31657	A07
9	235.22.1109.56620	A09
11	235.22.1110.12403	A11
13	235.22.1110.46783	B01
15	235.22.1111.21293	B03
17	235.22.1112.17080	B05
19	235.22.1112.22037	C02
21	235.22.1112.34110	C04
23	235.22.1112.40643	C06
25	235.22.1112.44820	C08
27	235.22.1112.55807	C10
29	235.22.1113.02720	C12
31	235.22.1113.12750	C14
33	235.22.1113.19153	C19
35	235.22.1113.24357	C21
37	235.22.1113.49600	PB01
39	235.22.1113.54017	PB03
41	235.22.1113.57120	PB05
43	235.22.1114.00550	PB07
45	235.22.1114.03333	PB09
47	235.22.1114.06913	V02
49	235.22.1114.21037	V04

Item	Drawing Number	Truss
2	235.22.1108.34100	A02
4	235.22.1109.13427	A04
6	235.22.1109.29380	A06
8	235.22.1109.34557	A08
10	235.22.1110.05803	A10
12	235.22.1110.20757	A12
14	235.22.1111.12333	B02
16	235.22.1111.25943	B04
18	235.22.1112.18903	C01
20	235.22.1112.29977	C03
22	235.22.1112.37173	C05
24	235.22.1112.43340	C07
26	235.22.1112.50373	C09
28	235.22.1112.57270	C11
30	235.22.1113.09017	C13
32	235.22.1113.16473	C15
34	235.22.1113.21843	C20
36	235.22.1113.47807	C22
38	235.22.1113.51867	PB02
40	235.22.1113.55540	PB04
42	235.22.1113.58840	PB06
44	235.22.1114.01863	PB08
46	235.22.1114.05583	V01
48	235.22.1114.10070	V03
50	235.22.1114.24400	V05

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Site Information:	Page 2:
Customer: W. B. Howland Company, Inc.	Job Number: 22-8147
Job Description: Edenfield	
Address: 3rd of sep	

Item	Drawing Number	Truss
51	A14030ENC160118	
53	GBLLETIN0118	
55	CNNAILSP1014	
57	PB160160118	
59	VALTN160118	

Item	Drawing Number	Truss
52	BRCLBSUB0119	
54	A14015ENC160118	
56	DEFLCAMB1014	
58	VAL180160118	

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed and detailed by the Building Designer.

Connector Plate Information:

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for of all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for of all load cases.

Max Web CSI = Maximum bending and axial Combined Stress Index for Webs for of all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.

W = Width of non-hanger bearing, in inches.

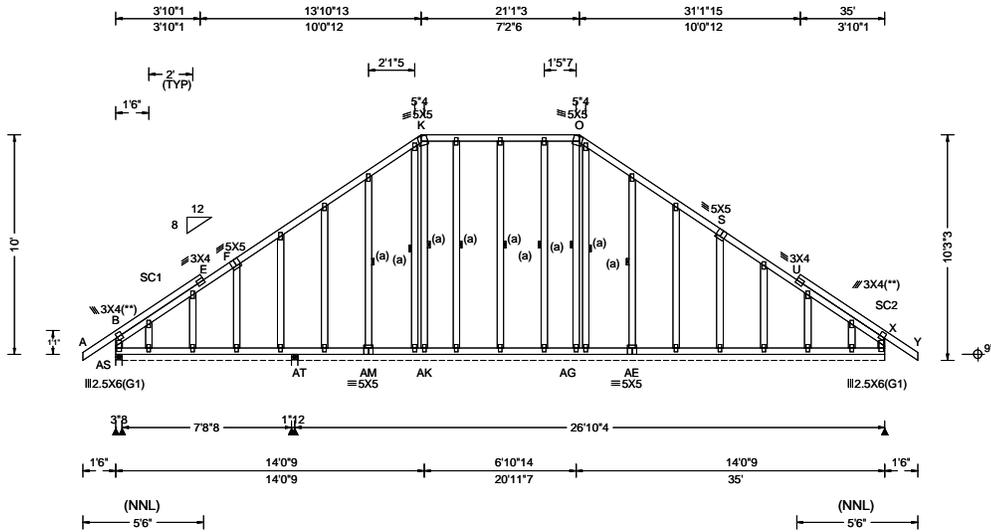
Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoclin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcacomponents.com.

SEQN: 99601 FROM:	GABL Ply: 1 Qty: 1	Job Number: 22-8147 Edenfield Truss Label: A01	Cust: R 215 JRef: 1Xlc2150005 T15 DrwNo: 235.22.1108.24190 KD / DF 08/23/2022
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Loading Criteria (psf) TCCL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 2.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.50 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.003 E 999 240 VERT(CL): 0.005 E 999 180 HORZ(LL): 0.004 U - - HORZ(TL): 0.006 U - - Creep Factor: 2.0 Max TC CSI: 0.250 Max BC CSI: 0.042 Max Web CSI: 0.114 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs), or *=PLF <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>AS 266</td> <td>-</td> <td>-</td> <td>-</td> <td>/174</td> <td>/21</td> <td>/213</td> </tr> <tr> <td>AS*69</td> <td>-</td> <td>-</td> <td>-</td> <td>/54</td> <td>/12</td> <td>-</td> </tr> <tr> <td>AT 28</td> <td>-</td> <td>-</td> <td>-</td> <td>/14</td> <td>-</td> <td>-</td> </tr> <tr> <td>X* 87</td> <td>-</td> <td>-</td> <td>-</td> <td>/48</td> <td>-</td> <td>-</td> </tr> </tbody> </table> Wind reactions based on MWFRS AS Brg Wid = 3.5 Min Req = 1.5 (Truss) AS Brg Wid = 92.5 Min Req = - AT Brg Wid = 3.5 Min Req = 1.5 (Truss) X Brg Wid = 320 Min Req = - Bearings AS, AS, AT, & AT are a rigid surface.	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	AS 266	-	-	-	/174	/21	/213	AS*69	-	-	-	/54	/12	-	AT 28	-	-	-	/14	-	-	X* 87	-	-	-	/48	-	-
Loc	Gravity			Non-Gravity																																									
	R+	/R-	/Rh	/Rw	/U	/RL																																							
AS 266	-	-	-	/174	/21	/213																																							
AS*69	-	-	-	/54	/12	-																																							
AT 28	-	-	-	/14	-	-																																							
X* 87	-	-	-	/48	-	-																																							

Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;
 Stack Chord: SC1 2x4 SP #2;
 Stack Chord: SC2 2x4 SP #2;
 Lt Stub Wedge: 2x4 SP #3;Rt Stub Wedge: 2x4 SP #3;

Bracing
 (a) Continuous lateral restraint equally spaced on member.

Plating Notes
 All plates are 2X4 except as noted.
 (**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

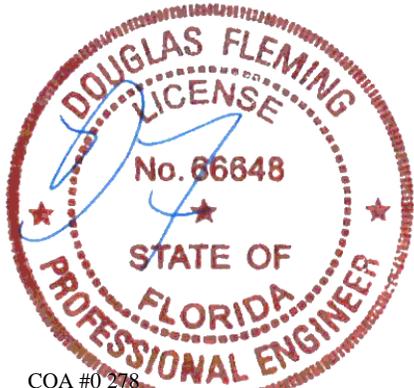
Loading
 Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Purlins
 In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" oc, all BC @ 24" oc.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes
 See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.
 Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

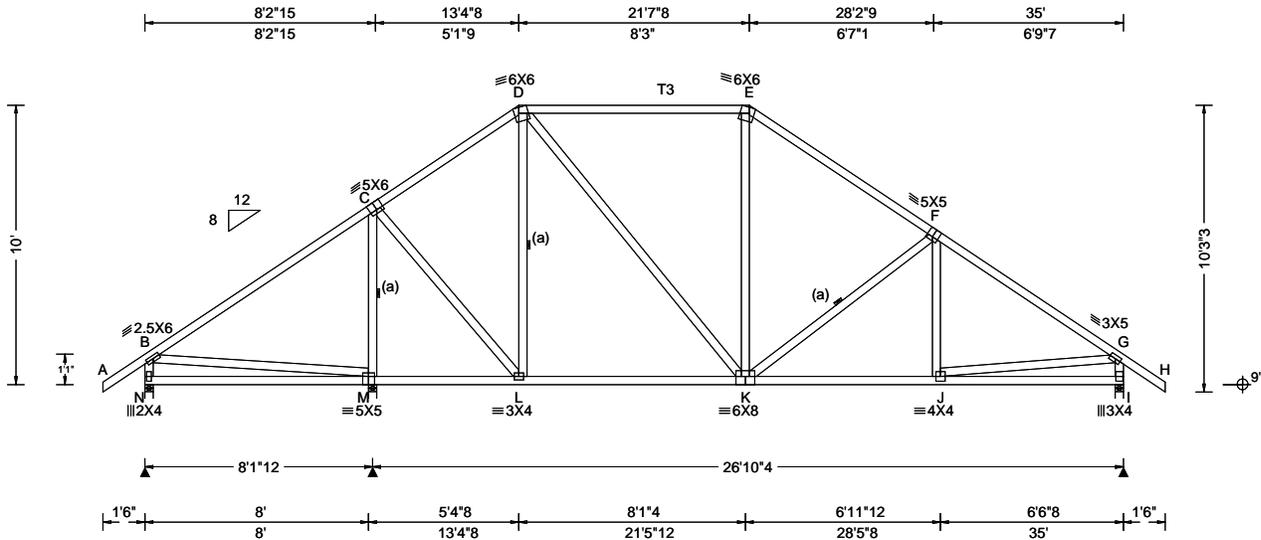
Members not listed have forces less than 375#



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****WARNING** READ AND FOLLOW ALL NOTES ON THIS DRAWING!**
****IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS**
 Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.
 Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.
 For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbccomponents.com; ICC: iccsafe.org; AWC: awc.org





Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 2.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.50 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.040 K 999 240 VERT(CL): 0.076 K 999 180 HORZ(LL): 0.014 G - - HORZ(TL): 0.028 G - - Creep Factor: 2.0 Max TC CSI: 0.842 Max BC CSI: 0.652 Max Web CSI: 0.424 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs) Gravity Loc R+ / R- / Rh / Rw / U / RL N 407 /- /- /229 /19 /209 M 1637 /- /- /885 /- /- I 1275 /- /- /740 /- /- Non-Gravity Wind reactions based on MWFRS N Brg Wid = 3.5 Min Req = 1.5 (Truss) M Brg Wid = 3.5 Min Req = 1.9 (Truss) I Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings N, M, & I are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. C - D 97 -812 E - F 70 -1181 D - E 98 -888 F - G 41 -1538 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. L - K 589 0 K - J 1189 0 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. M - C 40 -1504 K - F 165 -385 C - L 890 0 J - G 1112 0 D - L 0 -402 G - I 100 -1214 D - K 469 0
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Lumber

Top chord: 2x4 SP #2; T3 2x4 SP M-31;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Purlins

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" oc, all BC @ 24" oc.

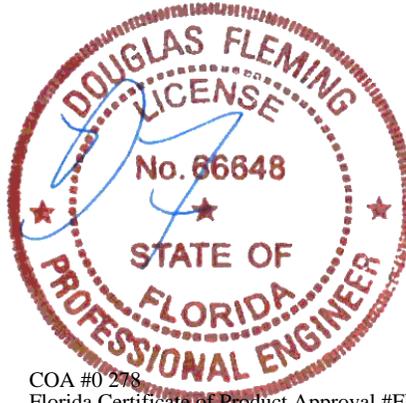
Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

THIS TRUSS MUST BE INSTALLED AS SHOWN AND NOT END FOR END.



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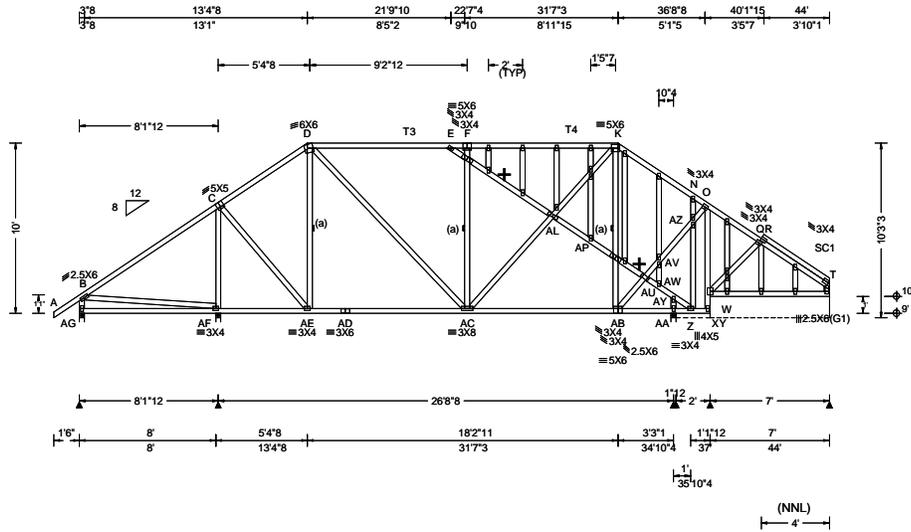
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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org



155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 112258 FROM:	GABL Ply: 1 Qty: 1	Job Number: 22-8147 Edenfield Truss Label: A03	Cust: R215 JRRef: 1Xlc2150005 T16 DrwNo: 235.22.1109.08450 KD / DF 08/23/2022
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Loading Criteria (psf) TCCL: 20.00 TCDL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 2.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 4.40 ft Loc. from endwall: not in 6.50 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.039 H 999 240 VERT(CL): 0.081 H 999 180 HORZ(LL): 0.015 Z - - HORZ(TL): 0.031 Z - - Creep Factor: 2.0 Max TC CSI: 0.797 Max BC CSI: 0.754 Max Web CSI: 0.468 VIEW Ver: 21.02.01.1216.15	▲ Maximum Reactions (lbs), or *=PLF <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>AG 523</td> <td>-</td> <td>-</td> <td>-</td> <td>/279</td> <td>/9</td> <td>/200</td> </tr> <tr> <td>AF 1394</td> <td>-</td> <td>-</td> <td>-</td> <td>/872</td> <td>-</td> <td>-</td> </tr> <tr> <td>AA 180</td> <td>-</td> <td>-</td> <td>-</td> <td>/103</td> <td>/0</td> <td>-</td> </tr> <tr> <td>AA*328</td> <td>-</td> <td>-</td> <td>-</td> <td>/160</td> <td>-</td> <td>-</td> </tr> <tr> <td>X* 150</td> <td>-</td> <td>-</td> <td>-</td> <td>/94</td> <td>-</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS AG Brg Wid = 3.5 Min Req = 1.5 (Truss) AF Brg Wid = 3.5 Min Req = 1.5 (Truss) AA Brg Wid = 3.5 Min Req = 1.5 (Truss) AA Brg Wid = 24.0 Min Req = - X Brg Wid = 84.0 Min Req = - Bearings AG, AF, AA, AA, & X are a rigid surface.</p>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	AG 523	-	-	-	/279	/9	/200	AF 1394	-	-	-	/872	-	-	AA 180	-	-	-	/103	/0	-	AA*328	-	-	-	/160	-	-	X* 150	-	-	-	/94	-	-
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Lumber
 Top chord: 2x4 SP #2; T3,T4 2x4 SP M-31;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;
 Stack Chord: SC1 2x4 SP #2;
 Rt Stub Wedge: 2x4 SP #3;

Bracing
 (a) Continuous lateral restraint equally spaced on member.

Plating Notes
 All plates are 2X4 except as noted.

Loading
 Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Purlins
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Left end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.
 + Member to be laterally braced for horizontal wind loads. bracing system to be designed and furnished by others.

Additional Notes
 See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.
 Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.



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Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
C - D	0 -835	E - F	30 -453
D - E	0 -952	F - K	25 -457

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
AE-AD	610 0	AB-AA	611 0
AD-AC	610 0	AA-Z	615 0
AC-AB	910 0		

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
C-AE	686 0	AU-AW	0 -789
D-AC	493 0	AV-AZ	477 0
E-AL	0 -711	AW-AY	0 -855
AL-AP	0 -775	AY-Z	0 -774
AP-AU	0 -811	AZ-O	447 0
AB-AU	472 0	O-X	0 -476
AU-AV	482 0		

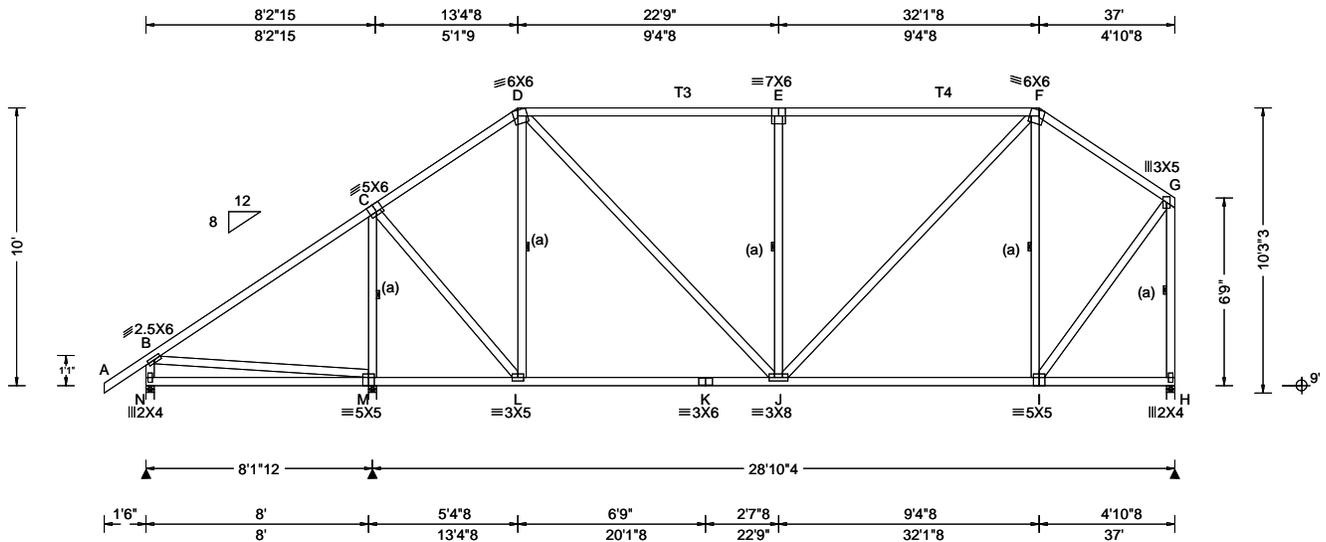
Maximum Gable Forces Per Ply (lbs)

Gables	Tens.Comp.	Gables	Tens. Comp.
AG- B	93 -488	D-AE	0 -389
AF- C	0 -1244		

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SEQN: 99604 FROM:	COMN Ply: 1 Qty: 2	Job Number: 22-8147 Edenfield Truss Label: A04	Cust: R215 JRef: 1Xlc2150005 T57 DrwNo: 235.22.1109.13427 KD / DF 08/23/2022
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Loading Criteria (psf) TCCL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 2.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.70 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.054 E 999 240 VERT(CL): 0.099 E 999 180 HORZ(LL): 0.015 D - - HORZ(TL): 0.027 D - - Creep Factor: 2.0 Max TC CSI: 0.849 Max BC CSI: 0.901 Max Web CSI: 0.428 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs) Gravity Non-Gravity <table border="1"> <thead> <tr> <th>Loc</th> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>N</td> <td>430</td> <td>-</td> <td>-</td> <td>/258</td> <td>-</td> <td>/179</td> </tr> <tr> <td>M</td> <td>1767</td> <td>-</td> <td>-</td> <td>/928</td> <td>-</td> <td>-</td> </tr> <tr> <td>H</td> <td>1434</td> <td>-</td> <td>-</td> <td>/662</td> <td>-</td> <td>-</td> </tr> </tbody> </table> Wind reactions based on MWFRS N Brg Wid = 3.5 Min Req = 1.5 (Truss) M Brg Wid = 3.5 Min Req = 2.1 (Truss) H Brg Wid = 3.5 Min Req = 1.7 (Truss) Members N, M, & H are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) <table border="1"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>C - D</td> <td>0 - 954</td> <td>E - F</td> <td>0 - 1181</td> </tr> <tr> <td>D - E</td> <td>0 - 1180</td> <td>F - G</td> <td>0 - 864</td> </tr> </tbody> </table>						Loc	R+	/R-	/Rh	/Rw	/U	/RL	N	430	-	-	/258	-	/179	M	1767	-	-	/928	-	-	H	1434	-	-	/662	-	-	Chords	Tens.Comp.	Chords	Tens. Comp.	C - D	0 - 954	E - F	0 - 1181	D - E	0 - 1180	F - G	0 - 864
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Bracing
(a) Continuous lateral restraint equally spaced on member.

Loading
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Purlins
In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" oc, all BC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.



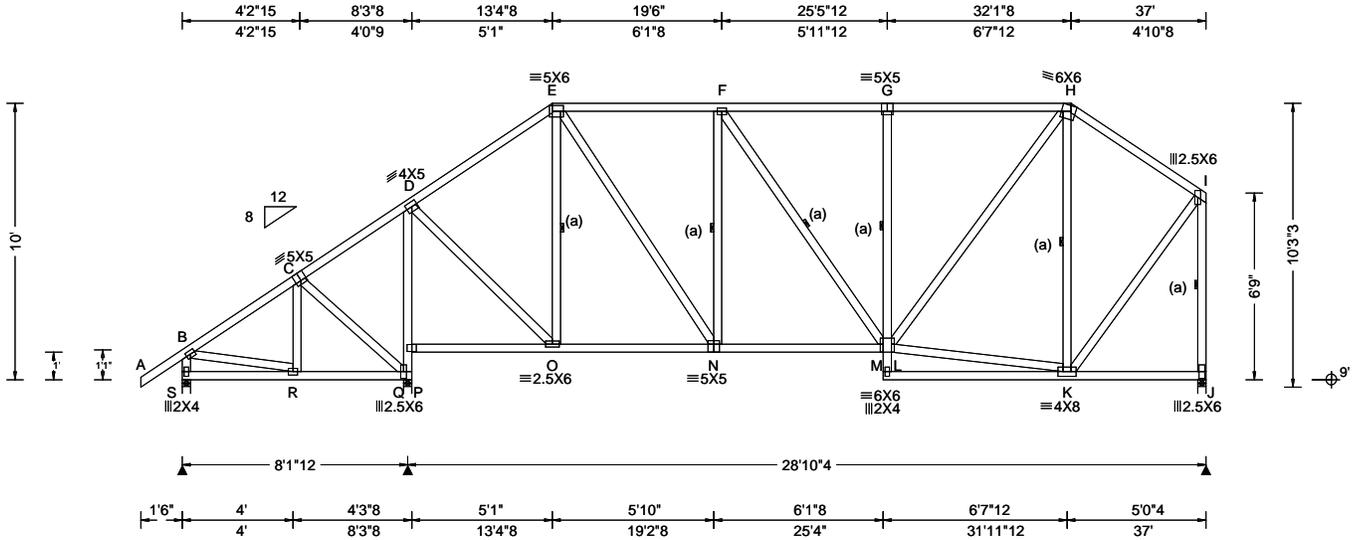
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.70 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.052 G 999 240 VERT(CL): 0.101 G 999 180 HORZ(LL): 0.016 K - - HORZ(TL): 0.032 K - - Creep Factor: 2.0 Max TC CSI: 0.462 Max BC CSI: 0.579 Max Web CSI: 0.894 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs) Gravity Loc R+ / R- / Rh / Rw / U / RL S 399 /- /- /200 /- /175 Q 1792 /- /- /988 /120 /- J 1291 /- /- /639 /45 /- Wind reactions based on MWFRS S Brg Wid = 3.5 Min Req = 1.5 (Truss) Q Brg Wid = 3.5 Min Req = 2.1 (Truss) J Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings S, Q, & J are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. D - E 206 -926 G - H 316 -1111 E - F 296 -1117 H - I 184 -768 F - G 316 -1112
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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 3X4 except as noted.

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Purlins

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" oc, all BC @ 24" oc.

Wind

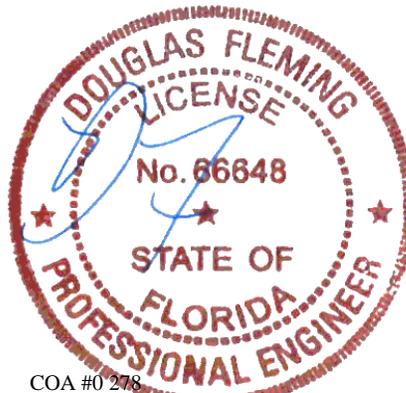
Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
O - N	698 -126	N - L	1131 -225

Maximum Web Forces Per Ply (lbs)

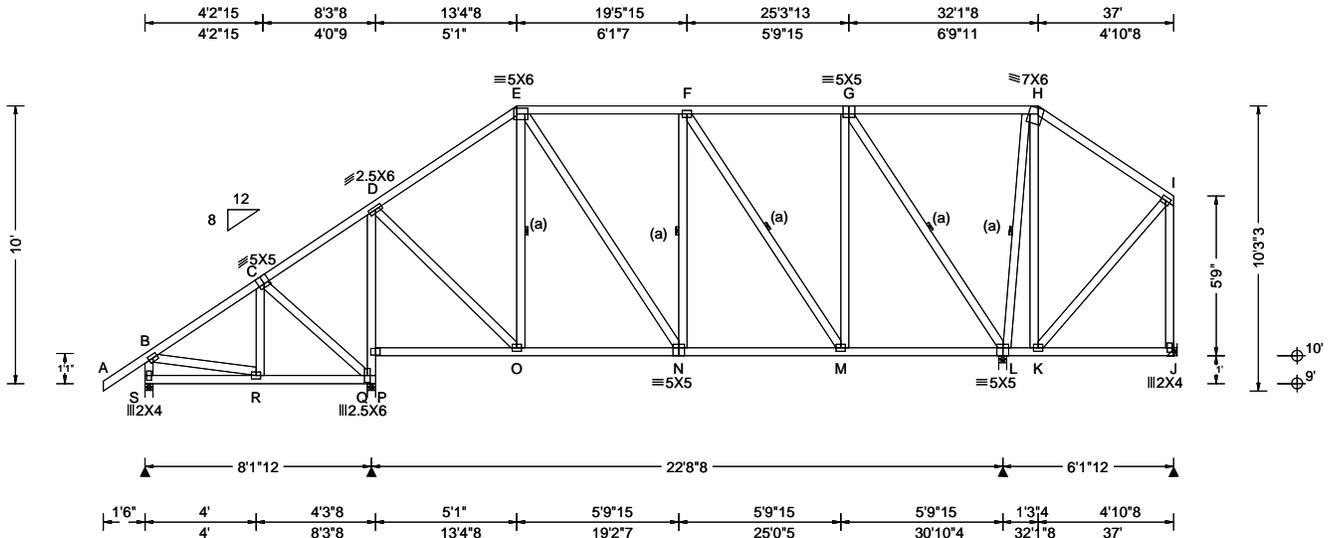
Webs	Tens.Comp.	Webs	Tens. Comp.
Q - P	290 -1570	L - K	570 -102
P - D	312 -1531	L - H	875 -214
D - O	1018 -173	K - H	226 -706
E - O	183 -526	K - I	949 -174
E - N	754 -181	I - J	250 -1257
G - L	211 -408		



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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.70 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.029 F 999 240 VERT(CL): 0.054 F 999 180 HORZ(LL): 0.010 L - - HORZ(TL): 0.019 L - - Creep Factor: 2.0 Max TC CSI: 0.643 Max BC CSI: 0.397 Max Web CSI: 0.671 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs) Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity S 409 /- /- /209 /- /175 Q 1462 /- /- /838 /17 /- L 1628 /- /- /673 /21 /- J 162 /-37 /- /111 /- /- Wind reactions based on MWFRS S Brg Wid = 3.5 Min Req = 1.5 (Truss) Q Brg Wid = 3.5 Min Req = 1.7 (Truss) L Brg Wid = 3.5 Min Req = 1.9 J Brg Wid = - Min Req = - Bearings S, Q, & L are a rigid surface. Members not listed have forces less than 375#
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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 3X4 except as noted.

Hangers / Ties

(J) Hanger Support Required, by others

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Purlins

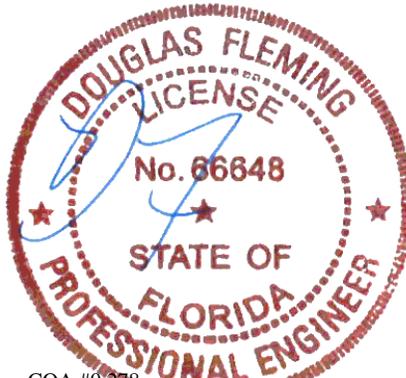
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Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

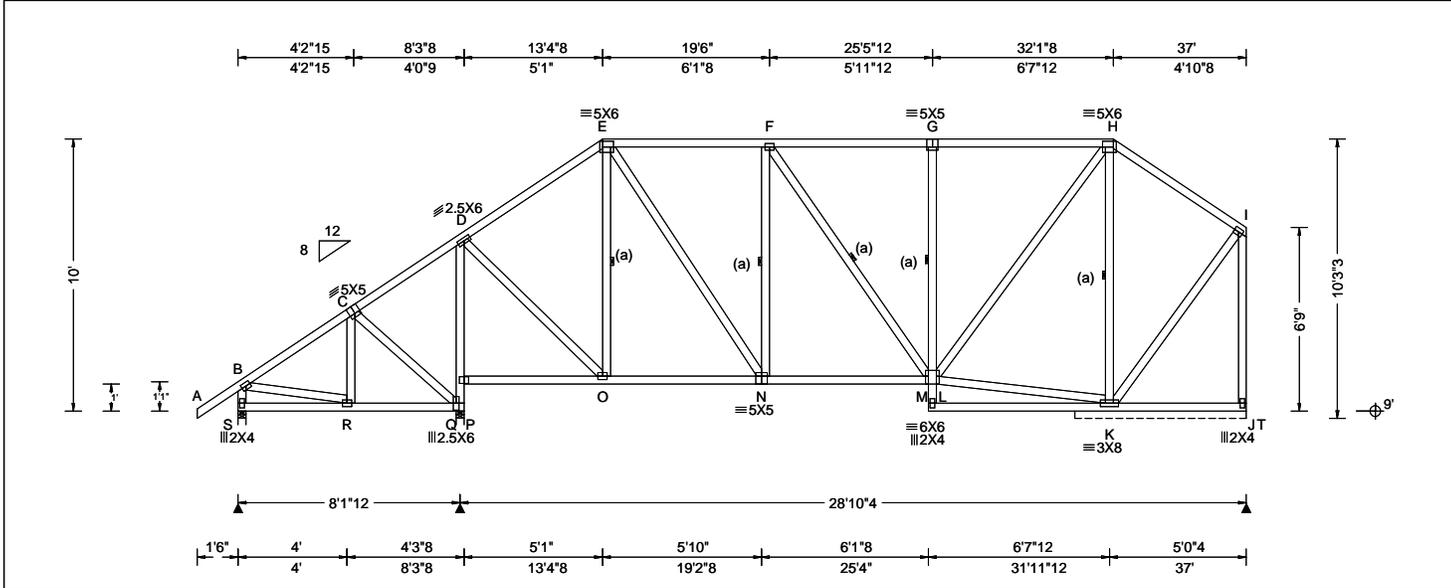
Wind loading based on both gable and hip roof types.



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Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCLL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCLL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.70 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.027 G 999 240 VERT(CL): 0.057 G 999 180 HORZ(LL): 0.008 K - - HORZ(TL): 0.018 K - - Creep Factor: 2.0 Max TC CSI: 0.598 Max BC CSI: 0.408 Max Web CSI: 0.620 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs), or *=PLF <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>S</td> <td>410</td> <td>-</td> <td>-</td> <td>/206</td> <td>-</td> <td>/175</td> </tr> <tr> <td>Q</td> <td>1361</td> <td>-</td> <td>-</td> <td>/858</td> <td>/24</td> <td>-</td> </tr> <tr> <td>T*</td> <td>231</td> <td>-</td> <td>-</td> <td>/122</td> <td>-</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS S Brg Wid = 3.5 Min Req = 1.5 (Truss) Q Brg Wid = 3.5 Min Req = 1.6 (Truss) T Brg Wid = 75.5 Min Req = - Bearings S, Q, & K are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>D - E</td> <td>166 -628</td> <td>F - G</td> <td>204 -497</td> </tr> <tr> <td>E - F</td> <td>225 -640</td> <td>G - H</td> <td>204 -497</td> </tr> </tbody> </table> Maximum Bot Chord Forces Per Ply (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>O - N</td> <td>451 -94</td> <td>N - L</td> <td>647 -152</td> </tr> </tbody> </table> Maximum Web Forces Per Ply (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Webs</th> <th>Tens.Comp.</th> <th>Webs</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>B - S</td> <td>26 -377</td> <td>G - L</td> <td>232 -455</td> </tr> <tr> <td>Q - P</td> <td>231 -1141</td> <td>L - H</td> <td>964 -277</td> </tr> <tr> <td>P - D</td> <td>253 -1095</td> <td>K - H</td> <td>342 -1147</td> </tr> <tr> <td>D - O</td> <td>657 -125</td> <td></td> <td></td> </tr> </tbody> </table> </p>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	S	410	-	-	/206	-	/175	Q	1361	-	-	/858	/24	-	T*	231	-	-	/122	-	-	Chords	Tens.Comp.	Chords	Tens. Comp.	D - E	166 -628	F - G	204 -497	E - F	225 -640	G - H	204 -497	Chords	Tens.Comp.	Chords	Tens. Comp.	O - N	451 -94	N - L	647 -152	Webs	Tens.Comp.	Webs	Tens. Comp.	B - S	26 -377	G - L	232 -455	Q - P	231 -1141	L - H	964 -277	P - D	253 -1095	K - H	342 -1147	D - O	657 -125		
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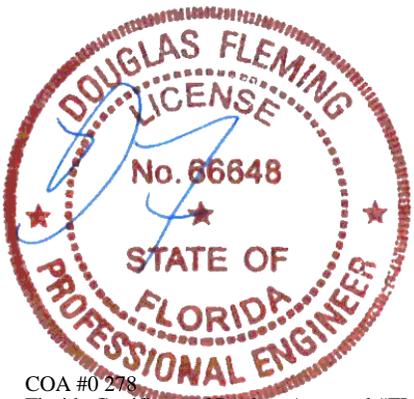
Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Bracing
 (a) Continuous lateral restraint equally spaced on member.

Plating Notes
 All plates are 3X4 except as noted.

Purlins
 In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" oc, all BC @ 24" oc.

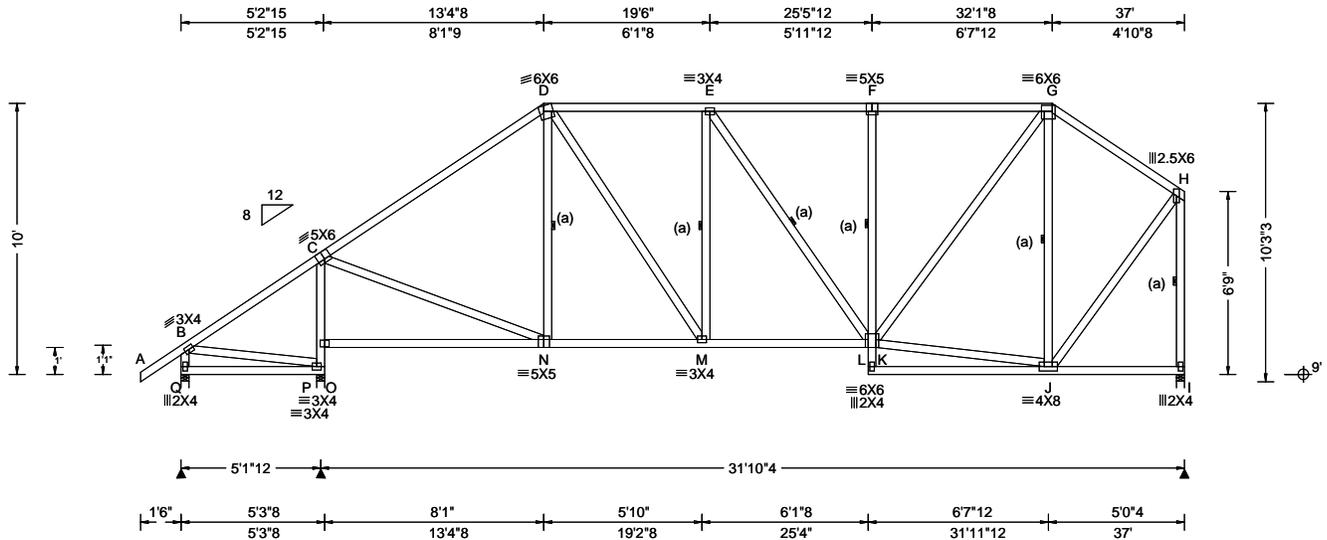
Wind
 Wind loads based on MWFRS with additional C&C member design.
 End verticals not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.



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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 2.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.70 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.063 E 999 240 VERT(CL): 0.122 E 999 180 HORZ(LL): 0.024 J - - HORZ(TL): 0.046 J - - Creep Factor: 2.0 Max TC CSI: 0.844 Max BC CSI: 0.633 Max Web CSI: 0.454 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs) Gravity Loc R+ / R- / Rh / Rw / U / RL Q 241 - / - / - /165 /15 /176 P 1845 - / - / - /984 - / - I 1429 - / - / - /543 - / - Wind reactions based on MWFRS Q Brg Wid = 3.5 Min Req = 1.5 (Truss) P Brg Wid = 3.5 Min Req = 2.2 (Truss) I Brg Wid = 3.5 Min Req = 1.7 (Truss) Bearings Q, P, & I are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.
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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Purlins

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" oc, all BC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

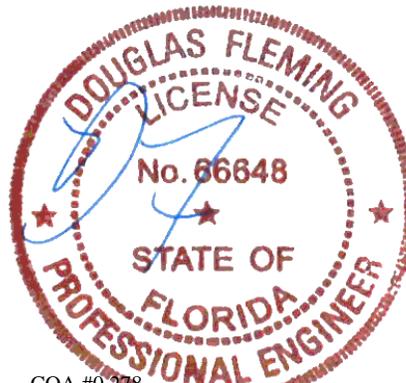
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
N - M	1093 0	M - K	1403 0

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
P - O	0 -1781	K - J	638 0
O - C	28 -1709	K - G	1065 0
C - N	1191 0	J - G	0 -808
D - M	546 0	J - H	1063 0
F - K	21 -416	H - I	0 -1394



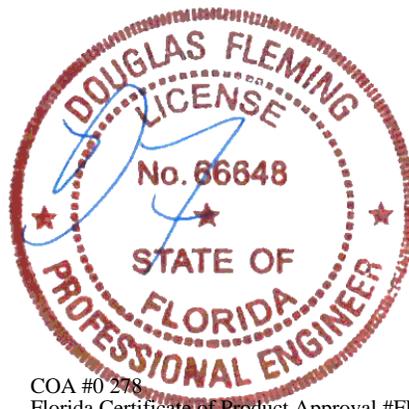
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SEQN: 112261	GABL	Ply: 1	Job Number: 22-8147	Cust: R 215	JRef: 1Xlc2150005	T7
FROM:		Qty: 1	Edenfield	DrwNo: 235.22.1109.56620		
Page 2 of 2			Truss Label: A09	KD / DF	08/23/2022	

Gables	Tens.Comp.	Gables	Tens. Comp.
AU-AD	75 -550	AU- L	63 -479



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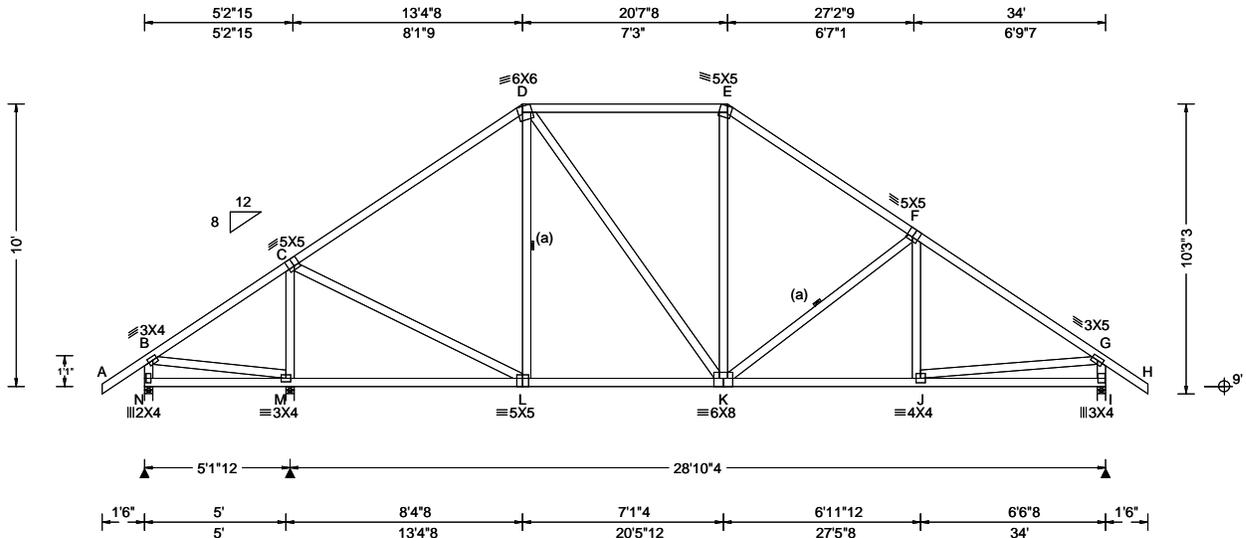
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155 Harlem Ave
 North Building, 4th Floor
 Glenview, IL 60025



Loading Criteria (psf) TCLL: 20.00 TC DL: 10.00 BCLL: 0.00 BC DL: 10.00 Des Ld: 40.00 NC BCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TC DL: 5.0 psf BC DL: 2.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.40 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.040 K 999 240 VERT(CL): 0.082 K 999 180 HORZ(LL): 0.013 G - - HORZ(TL): 0.027 G - - Creep Factor: 2.0 Max TC CSI: 0.710 Max BC CSI: 0.538 Max Web CSI: 0.527 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>N</td> <td>237</td> <td>-</td> <td>-</td> <td>/83</td> <td>/24</td> <td>/208</td> </tr> <tr> <td>M</td> <td>1567</td> <td>-</td> <td>-</td> <td>/954</td> <td>-</td> <td>-</td> </tr> <tr> <td>I</td> <td>1293</td> <td>-</td> <td>-</td> <td>/777</td> <td>-</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS N Brg Wid = 3.5 Min Req = 1.5 (Truss) M Brg Wid = 3.5 Min Req = 1.5 (Truss) I Brg Wid = 3.5 Min Req = 1.5 (Truss)</p> <p>Bearings N, M, & I are a rigid surface. Members not listed have forces less than 375#</p> Maximum Top Chord Forces Per Ply (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>C - D</td> <td>17 - 1034</td> <td>E - F</td> <td>19 - 1197</td> </tr> <tr> <td>D - E</td> <td>44 - 896</td> <td>F - G</td> <td>0 - 1574</td> </tr> </tbody> </table> Maximum Bot Chord Forces Per Ply (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>L - K</td> <td>741 0</td> <td>K - J</td> <td>1220 0</td> </tr> </tbody> </table> Maximum Web Forces Per Ply (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Webs</th> <th>Tens.Comp.</th> <th>Webs</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>M - C</td> <td>12 - 1401</td> <td>J - G</td> <td>1149 0</td> </tr> <tr> <td>C - L</td> <td>853 0</td> <td>G - I</td> <td>37 - 1237</td> </tr> <tr> <td>K - F</td> <td>118 - 414</td> <td></td> <td></td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	N	237	-	-	/83	/24	/208	M	1567	-	-	/954	-	-	I	1293	-	-	/777	-	-	Chords	Tens.Comp.	Chords	Tens. Comp.	C - D	17 - 1034	E - F	19 - 1197	D - E	44 - 896	F - G	0 - 1574	Chords	Tens.Comp.	Chords	Tens. Comp.	L - K	741 0	K - J	1220 0	Webs	Tens.Comp.	Webs	Tens. Comp.	M - C	12 - 1401	J - G	1149 0	C - L	853 0	G - I	37 - 1237	K - F	118 - 414		
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Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
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Bracing

(a) Continuous lateral restraint equally spaced on member.

Purlins

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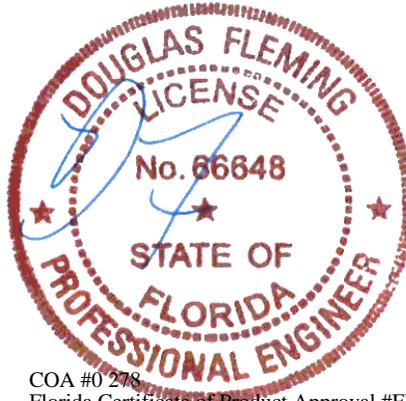
Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

THIS TRUSS MUST BE INSTALLED AS SHOWN AND NOT END FOR END.



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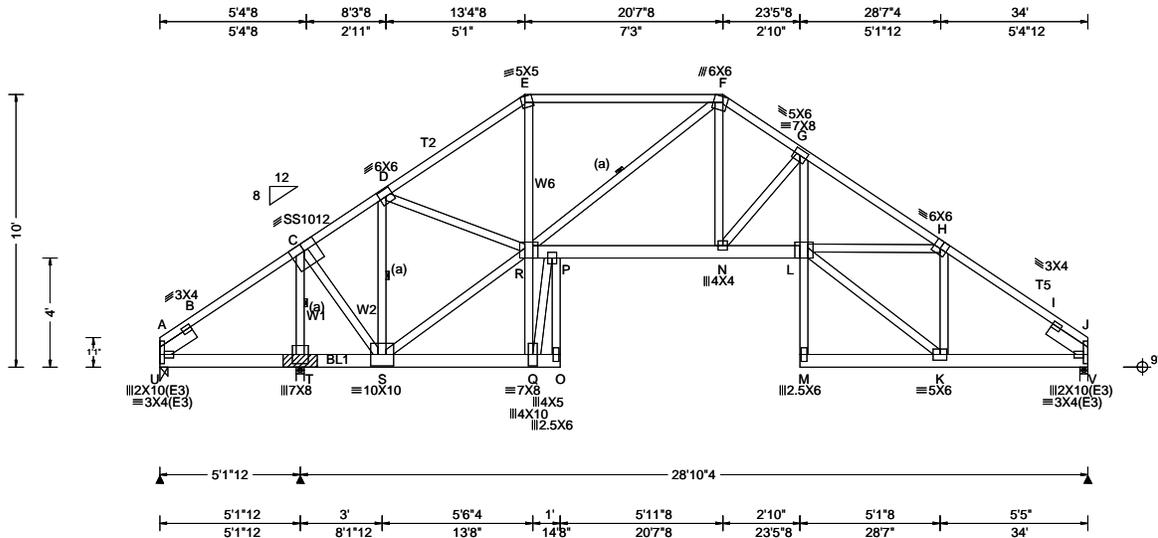
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 North Building, 4th Floor
 Glenview, IL 60025



Loading Criteria (psf) TCLL: 20.00 TC DL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 0.00 ft TC DL: 5.0 psf BCDL: 2.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE, 18SS	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.186 L 999 240 VERT(CL): 0.375 L 922 180 HORZ(LL): 0.159 J - - HORZ(TL): 0.324 J - - Creep Factor: 2.0 Max TC CSI: 0.706 Max BC CSI: 0.495 Max Web CSI: 0.933 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>U</td> <td>-</td> <td>-1355</td> <td>-</td> <td>/3</td> <td>-</td> <td>-</td> </tr> <tr> <td>T</td> <td>6965</td> <td>-</td> <td>-</td> <td>-</td> <td>/111</td> <td>-</td> </tr> <tr> <td>V</td> <td>1768</td> <td>-</td> <td>-</td> <td>-</td> <td>/52</td> <td>-</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	U	-	-1355	-	/3	-	-	T	6965	-	-	-	/111	-	V	1768	-	-	-	/52	-
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E - F	101 -3310																																					

Lumber
 Top chord: 2x4 SP M-31; T2,T5 2x4 SP #2;
 Bot chord: 2x6 SP 2400f-2.0E;
 Webs: 2x4 SP #3; W1,W6 2x4 SP #2;
 W2 2x4 SP M-31;
 Lt Slider: 2x6 SP 2400f-2.0E; block length = 1.500'
 Rt Slider: 2x6 SP 2400f-2.0E; block length = 1.692'

Bearing Block(s)
 Brg blocks:0.131"x3", min. nails
 brg x-loc #blocks length/blk #nails/blk wall plate
 2 5.000' 1 15" 18 Rigid Surface
 Brg block to be same size and species as chord.
 Refer to drawing CNNAILSP1014 for more information.

Bracing
 (a) Continuous lateral restraint equally spaced on member.

Blocking
 Blocking reinforcement required to prevent buckling of members over the bearings:
 Bearing 3 located at 33.7' (blocking >= 3.50" if used)

Special Loads
 -----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
 TC: From 64 plf at 0.00 to 64 plf at 5.13
 TC: From 32 plf at 5.13 to 32 plf at 14.06
 TC: From 64 plf at 14.06 to 64 plf at 34.00
 BC: From 20 plf at 0.00 to 20 plf at 6.06
 BC: From 10 plf at 6.06 to 10 plf at 14.67
 BC: From 20 plf at 14.67 to 20 plf at 34.00
 BC: 1015 lb Conc. Load at 6.06, 8.06,10.06,12.06
 14.06

Additional Notes
 Negative reaction(s) of -1355# MAX. from a non-wind load case requires uplift connection. See Maximum Reactions.

Purlins
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
 Wind loads and reactions based on MWFRS.
 Wind loading based on both gable and hip roof types.
 Laterally brace top chord below filler and bottom chord above filler at 24" o.c., including a lateral brace at chord ends (If no rigid diaphragm exists at that point).

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Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
A - T	8 -1660	P - N	2944 -90
T - S	5 -1556	N - L	3957 -124
R - P	2765 -88	K - J	1943 -60

Maximum Web Forces Per Ply (lbs)

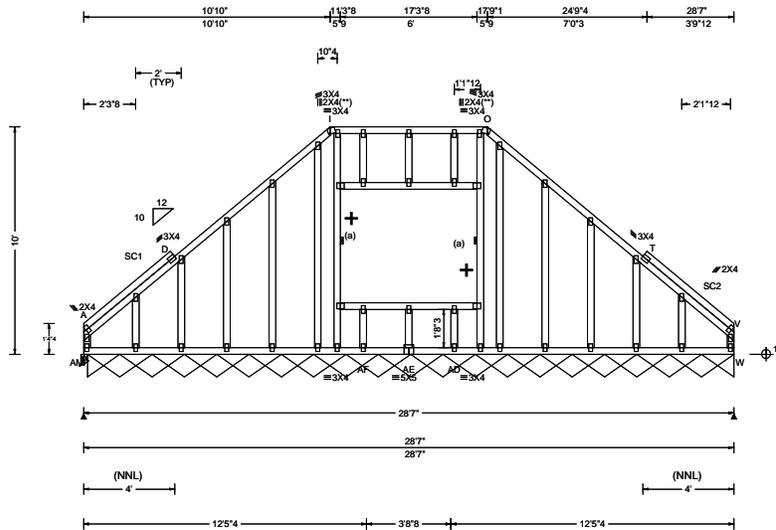
Webs	Tens.Comp.	Webs	Tens. Comp.
T - C	125 -5996	Q - P	1484 -69
C - S	4658 -61	P - O	0 -588
S - D	85 -2161	F - N	1319 0
S - R	1207 -73	N - G	53 -1621
D - R	2180 -24	G - L	1726 -25
E - R	1845 -5	L - K	2450 -73
R - Q	1396 0	L - H	2058 -67
R - F	746 -373	K - H	111 -1593

It is the responsibility of the Building Designer and Truss Fabricator to review this drawing prior to cutting lumber to verify that all data, including dimensions and loads, conform to the architectural plans/specifications and fabricators truss layout.

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SEQN: 99504 FROM:	GABL Ply: 1 Qty: 1	Job Number: 22-8147 Edenfield Truss Label: B01	Cust: R 215 JRef: 1Xlc2150005 T50 DrwNo: 235.22.1110.46783 KD / DF 08/23/2022
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Loading Criteria (psf) TCLL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 17.31 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.046 L 999 240 VERT(CL): 0.098 L 999 180 HORZ(LL): 0.004 V - - HORZ(TL): 0.011 V - - Creep Factor: 2.0 Max TC CSI: 0.353 Max BC CSI: 0.037 Max Web CSI: 0.523 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs), or * = PLF <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>AM 151</td> <td>/-</td> <td>/-</td> <td>/85</td> <td>/31</td> <td>/65</td> <td></td> </tr> <tr> <td>W* 81</td> <td>/-</td> <td>/-</td> <td>/44</td> <td>/-</td> <td>/-</td> <td></td> </tr> </tbody> </table> Wind reactions based on MWFRS AM Brg Wid = 2.0 Min Req = 1.5 (Truss) W Brg Wid = 340 Min Req = - Bearings AM & AM are a rigid surface. Members not listed have forces less than 375#	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	AM 151	/-	/-	/85	/31	/65		W* 81	/-	/-	/44	/-	/-	
Loc	Gravity			Non-Gravity																											
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W* 81	/-	/-	/44	/-	/-																										

Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;
 Stack Chord: SC1 2x4 SP #2;
 Stack Chord: SC2 2x4 SP #2;

Bracing
 (a) Continuous lateral restraint equally spaced on member.

Plating Notes
 All plates are 2X4 except as noted.
 (**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Purlins
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 End verticals not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.
 + Member to be laterally braced for horizontal wind loads. bracing system to be designed and furnished by others.

Additional Notes
 See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.
 Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

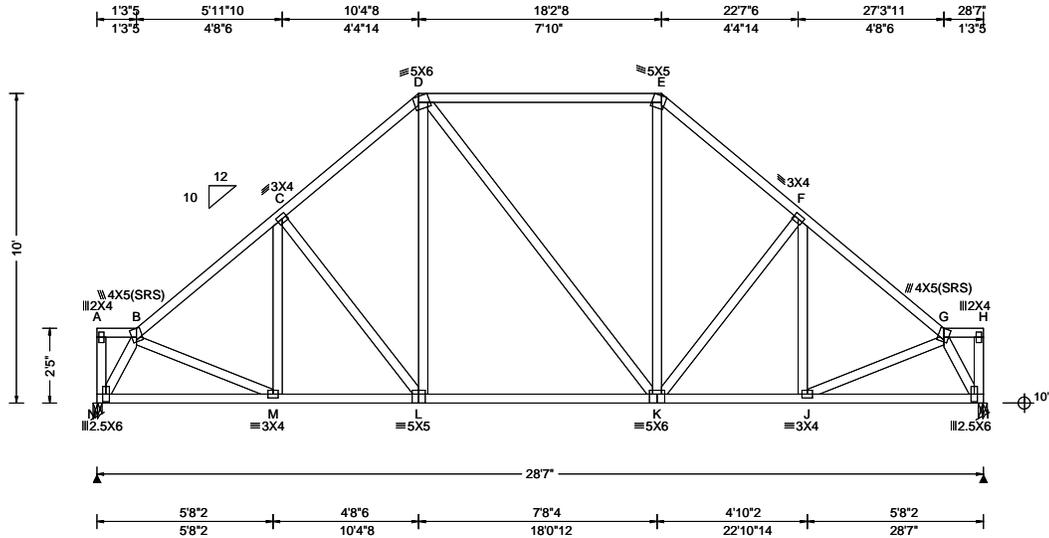
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Loading Criteria (psf) TCCL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 16.21 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.032 L 999 240 VERT(CL): 0.069 L 999 180 HORZ(LL): 0.019 I - - HORZ(TL): 0.040 I - - Creep Factor: 2.0 Max TC CSI: 0.843 Max BC CSI: 0.513 Max Web CSI: 0.286 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>N</td> <td>1230</td> <td>-</td> <td>-</td> <td>/669</td> <td>/45</td> <td>/156</td> </tr> <tr> <td>I</td> <td>1230</td> <td>-</td> <td>-</td> <td>/669</td> <td>/45</td> <td>-</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	N	1230	-	-	/669	/45	/156	I	1230	-	-	/669	/45	-
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Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

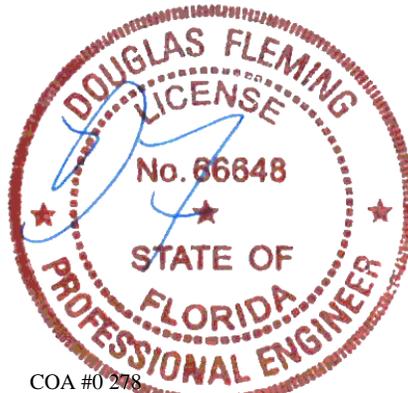
Wind loads based on MWFRS with additional C&C member design.
 End verticals not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
N - M	727 - 161	K - J	991 - 150
M - L	991 - 153	J - I	727 - 158
L - K	871 - 95		

Maximum Web Forces Per Ply (lbs)

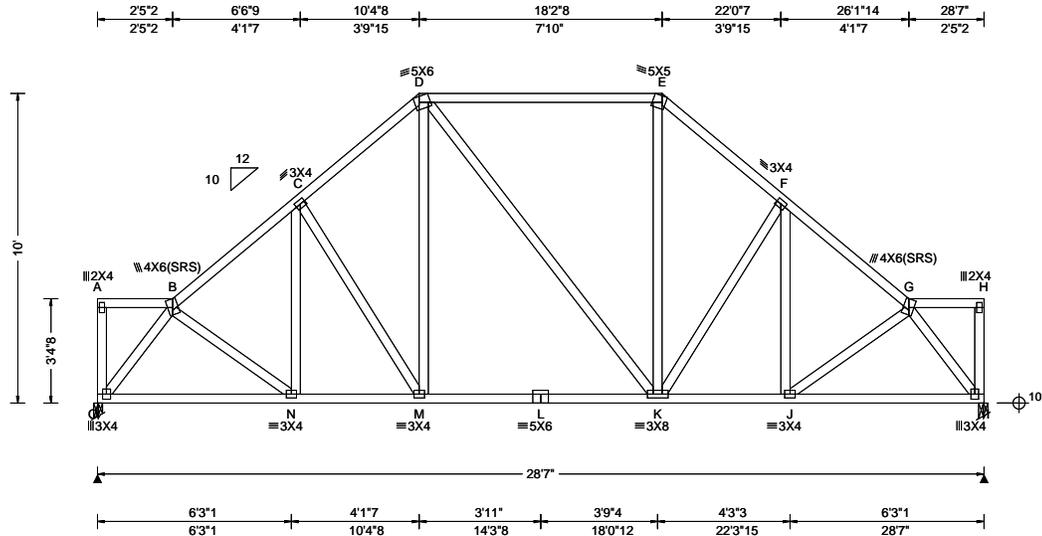
Webs	Tens.Comp.	Webs	Tens. Comp.
N - B	310 - 1391	K - E	384 - 11
D - L	383 - 11	G - I	309 - 1391



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Loading Criteria (psf) TCLL: 20.00 TC DL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 18.32 ft TC DL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg, Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.033 M 999 240 VERT(CL): 0.072 M 999 180 HORZ(LL): 0.020 I - - HORZ(TL): 0.042 I - - Creep Factor: 2.0 Max TC CSI: 0.840 Max BC CSI: 0.512 Max Web CSI: 0.448 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>O</td> <td>1230</td> <td>-</td> <td>-</td> <td>/673</td> <td>-</td> <td>/139</td> </tr> <tr> <td>I</td> <td>1230</td> <td>-</td> <td>-</td> <td>/673</td> <td>-</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS O Brg Wid = 2.0 Min Req = 1.5 (Truss) I Brg Wid = 2.0 Min Req = 1.5 (Truss) Bearings O & I are a rigid surface. Members not listed have forces less than 375#</p> Maximum Top Chord Forces Per Ply (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>B - C</td> <td>29 - 1353</td> <td>E - F</td> <td>81 - 1186</td> </tr> <tr> <td>C - D</td> <td>83 - 1190</td> <td>F - G</td> <td>29 - 1353</td> </tr> <tr> <td>D - E</td> <td>91 - 873</td> <td></td> <td></td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	O	1230	-	-	/673	-	/139	I	1230	-	-	/673	-	-	Chords	Tens.Comp.	Chords	Tens. Comp.	B - C	29 - 1353	E - F	81 - 1186	C - D	83 - 1190	F - G	29 - 1353	D - E	91 - 873		
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Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

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Wind

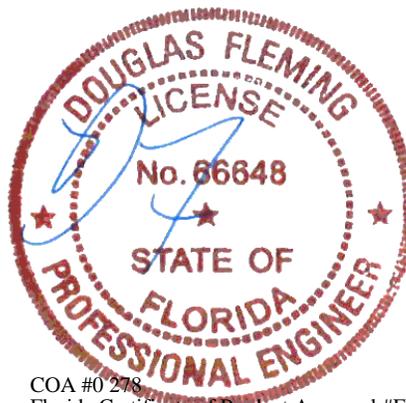
Wind loads based on MWFRS with additional C&C member design.
 End verticals not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
O - N	888 - 34	L - K	871 0
N - M	975 0	K - J	975 0
M - L	871 0	J - I	888 - 31

Maximum Web Forces Per Ply (lbs)

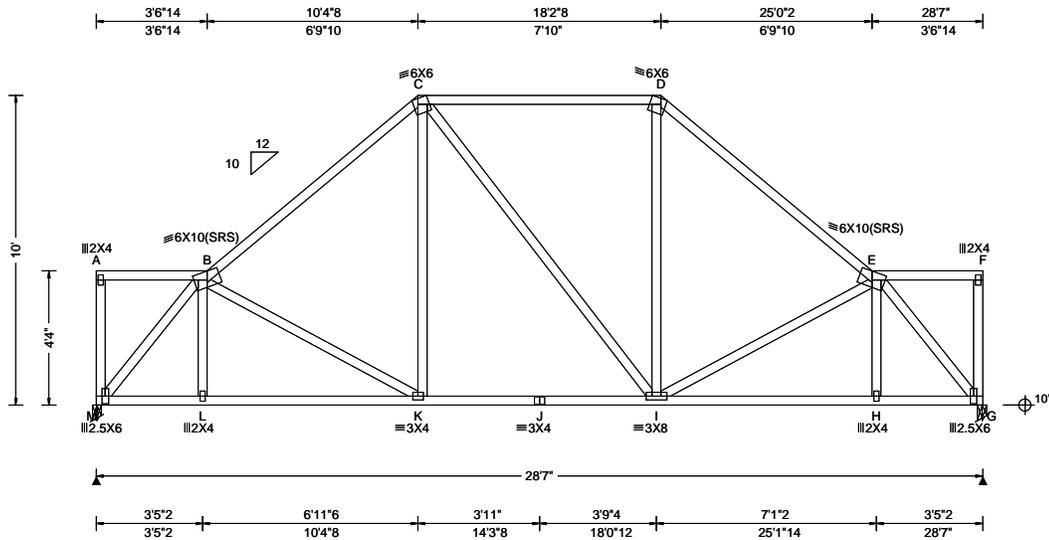
Webs	Tens.Comp.	Webs	Tens. Comp.
O - B	53 - 1421	K - E	386 - 72
D - M	386 - 74	G - I	53 - 1421



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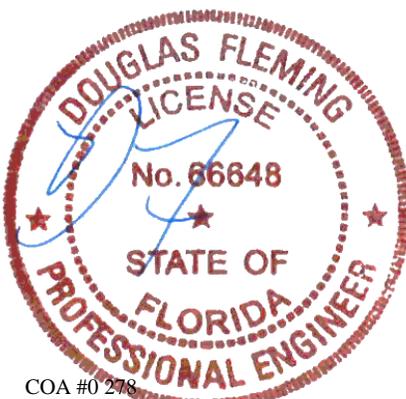


Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 17.17 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.034 K 999 240 VERT(CL): 0.074 K 999 180 HORZ(LL): 0.021 G - - HORZ(TL): 0.045 G - - Creep Factor: 2.0 Max TC CSI: 0.631 Max BC CSI: 0.551 Max Web CSI: 0.782 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>M</td> <td>1230</td> <td>-</td> <td>-</td> <td>/637</td> <td>/66</td> <td>/117</td> </tr> <tr> <td>G</td> <td>1230</td> <td>-</td> <td>-</td> <td>/637</td> <td>/66</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS M Brg Wid = 2.0 Min Req = 1.5 (Truss) G Brg Wid = 2.0 Min Req = 1.5 (Truss) Bearings M & G are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>B - C</td> <td>288 - 1275</td> <td>D - E</td> <td>288 - 1273</td> </tr> <tr> <td>C - D</td> <td>280 - 881</td> <td></td> <td></td> </tr> </tbody> </table> Maximum Bot Chord Forces Per Ply (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>M - L</td> <td>1009 - 183</td> <td>J - I</td> <td>879 - 142</td> </tr> <tr> <td>L - K</td> <td>1005 - 185</td> <td>I - H</td> <td>1006 - 185</td> </tr> <tr> <td>K - J</td> <td>879 - 142</td> <td>H - G</td> <td>1010 - 183</td> </tr> </tbody> </table> Maximum Web Forces Per Ply (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Webs</th> <th>Tens.Comp.</th> <th>Webs</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>M - B</td> <td>275 - 1518</td> <td>E - G</td> <td>275 - 1519</td> </tr> </tbody> </table> </p>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	M	1230	-	-	/637	/66	/117	G	1230	-	-	/637	/66	-	Chords	Tens.Comp.	Chords	Tens. Comp.	B - C	288 - 1275	D - E	288 - 1273	C - D	280 - 881			Chords	Tens.Comp.	Chords	Tens. Comp.	M - L	1009 - 183	J - I	879 - 142	L - K	1005 - 185	I - H	1006 - 185	K - J	879 - 142	H - G	1010 - 183	Webs	Tens.Comp.	Webs	Tens. Comp.	M - B	275 - 1518	E - G	275 - 1519
Loc	Gravity			Non-Gravity																																																															
	R+	/R-	/Rh	/Rw	/U	/RL																																																													
M	1230	-	-	/637	/66	/117																																																													
G	1230	-	-	/637	/66	-																																																													
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Webs	Tens.Comp.	Webs	Tens. Comp.																																																																
M - B	275 - 1518	E - G	275 - 1519																																																																

Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Purlins
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 End verticals not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.



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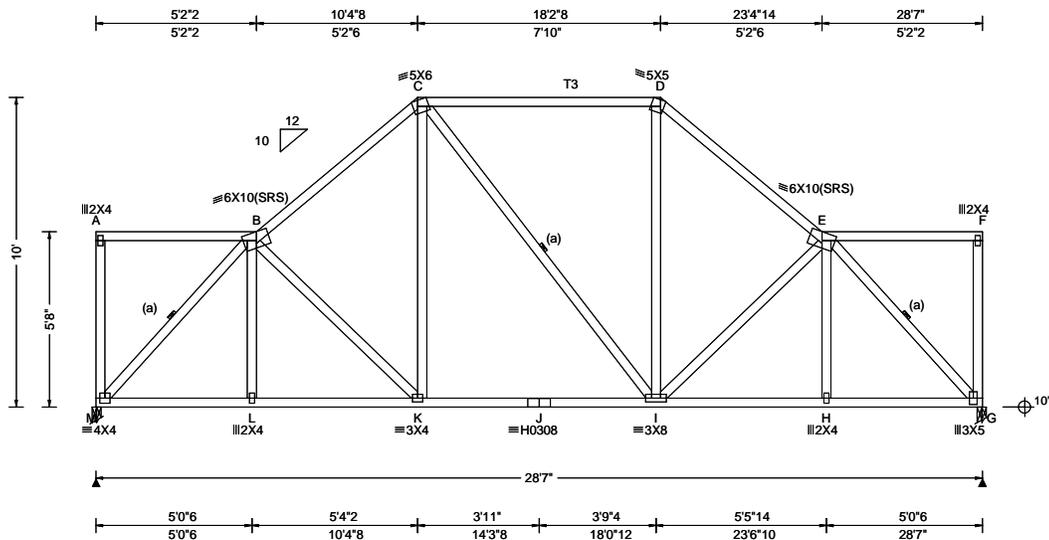
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SEQN: 99411 FROM:	SPEC Ply: 1 Qty: 1	Job Number: 22-8147 Edenfield Truss Label: B05	Cust: R 215 JRef: 1Xlc2150005 T23 DrwNo: 235.22.1112.17080 KD / DF 08/23/2022
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Loading Criteria (psf) TCLL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 19.47 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.044 K 999 240 VERT(CL): 0.091 K 999 180 HORZ(LL): 0.020 G - - HORZ(TL): 0.041 G - - Creep Factor: 2.0 Max TC CSI: 0.533 Max BC CSI: 0.511 Max Web CSI: 0.531 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL M 1458 - / - / - / 102 - / - G 1447 - / - / - / 101 - / - Wind reactions based on MWFRS M Brg Wid = 2.0 Min Req = 1.5 (Truss) G Brg Wid = 2.0 Min Req = 1.5 (Truss) Bearings M & G are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.
				B - C 130 - 1583 D - E 128 - 1562 C - D 80 - 1134

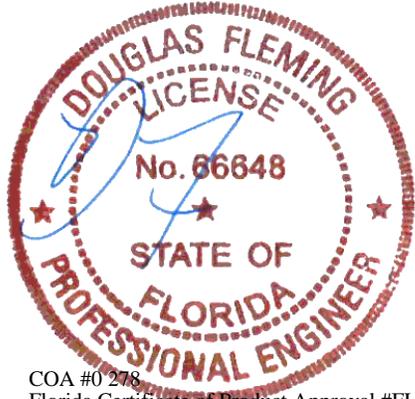
Lumber
 Top chord: 2x4 SP #2; T3 2x4 SP M-31;
 Bot chord: 2x4 SP M-31;
 Webs: 2x4 SP #3;

Bracing
 (a) Continuous lateral restraint equally spaced on member.

Special Loads
 -----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
 TC: From 66 plf at 0.00 to 66 plf at 28.58
 BC: From 20 plf at 0.00 to 20 plf at 11.94
 BC: From 10 plf at 11.94 to 10 plf at 15.94
 BC: From 20 plf at 15.94 to 20 plf at 28.58
 BC: 162 lb Conc. Load at 11.94,13.94,15.94

Purlins
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.
 The TC of this truss shall be braced with attached spans at 24" oc in lieu of structural sheathing.

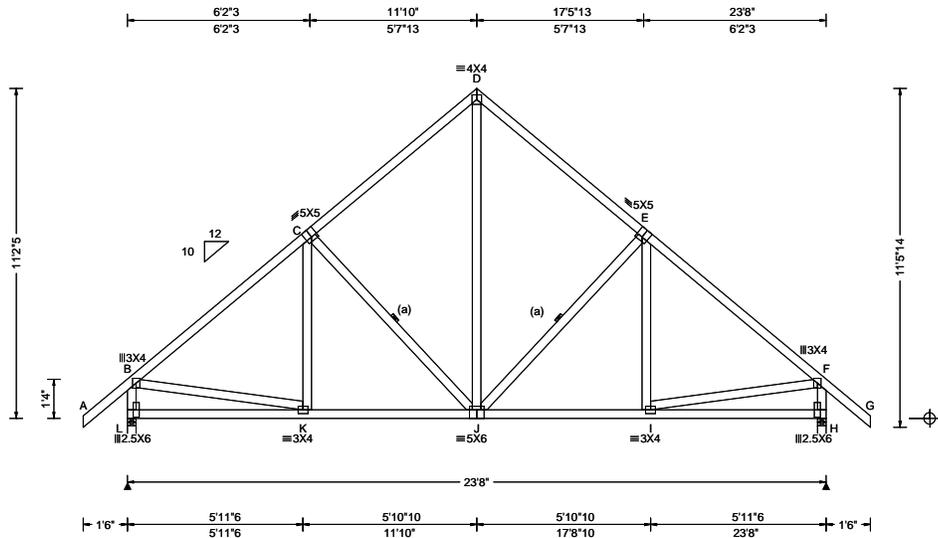
Wind
 Wind loads and reactions based on MWFRS.
 End verticals not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.



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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.027 J 999 240 VERT(CL): 0.057 J 999 180 HORZ(LL): 0.011 C - - HORZ(TL): 0.023 C - - Creep Factor: 2.0 Max TC CSI: 0.448 Max BC CSI: 0.399 Max Web CSI: 0.361 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL L 1117 /- /- /652 /19 /247 H 1117 /- /- /652 /19 /- Wind reactions based on MWFRS L Brg Wid = 3.5 Min Req = 1.5 (Truss) H Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings L & H are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 153 -1145 D - E 232 -865 C - D 232 -865 E - F 153 -1145
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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Wind

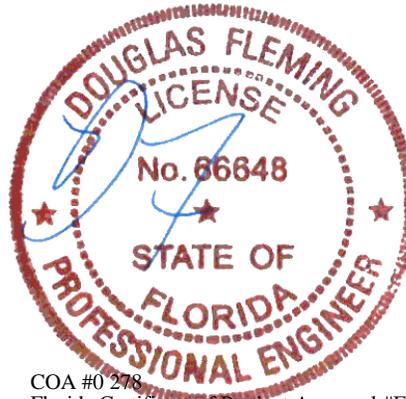
Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
K - J	789 -43	J - I	789 0

Maximum Web Forces Per Ply (lbs)

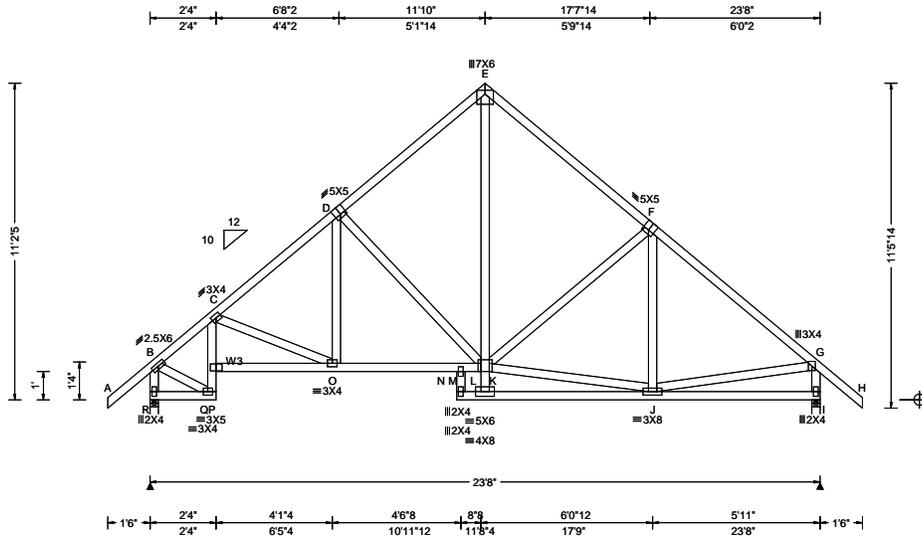
Webs	Tens.Comp.	Webs	Tens. Comp.
B - L	178 -1066	I - F	755 0
B - K	755 0	F - H	178 -1066
D - J	592 -159		



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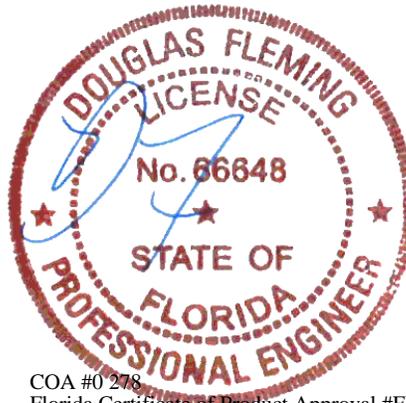
Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.065 O 999 240 VERT(CL): 0.135 O 999 180 HORZ(LL): 0.080 I - - HORZ(TL): 0.167 I - - Creep Factor: 2.0 Max TC CSI: 0.442 Max BC CSI: 0.351 Max Web CSI: 0.624 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL R 1117 /- /- /652 /19 /247 I 1117 /- /- /652 /19 /- Wind reactions based on MWFRS R Brg Wid = 3.5 Min Req = 1.5 (Truss) I Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings R & I are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 56 -966 E - F 125 -946 C - D 69 -1242 F - G 67 -1150 D - E 127 -926 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. P - O 1040 -141 M - K 871 -45 O - M 896 -44 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. B - R 70 -1121 K - J 781 0 B - Q 764 0 J - G 768 0 D - K 89 -400 G - I 85 -1068 E - K 690 -69
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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3; W3 2x4 SP M-31;

Wind

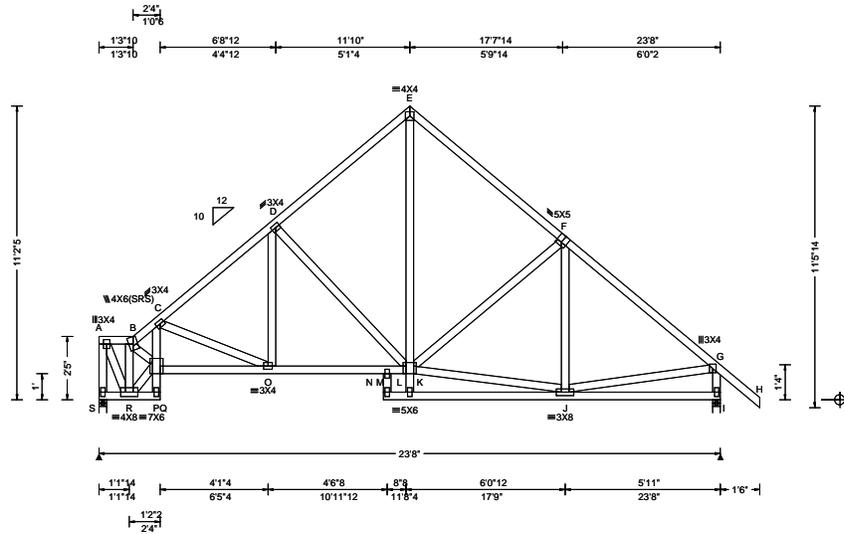
Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.
Note: Laterally brace bottom chord above filler at 20" O.C.Max. including a lateral brace at chord ends.



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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.040 O 999 240 VERT(CL): 0.085 O 999 180 HORZ(LL): 0.033 I - - HORZ(TL): 0.070 I - - Creep Factor: 2.0 Max TC CSI: 0.441 Max BC CSI: 0.371 Max Web CSI: 0.444 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL S 1015 /- /- /559 /15 /222 I 1120 /- /- /654 /17 /- Wind reactions based on MWFRS S Brg Wid = 3.5 Min Req = 1.5 (Truss) I Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings S & I are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.																			
				<table border="1"> <tr> <td>A - B</td><td>21</td><td>-497</td><td>D - E</td><td>125</td><td>-929</td></tr> <tr> <td>B - C</td><td>8</td><td>-1681</td><td>E - F</td><td>123</td><td>-951</td></tr> <tr> <td>C - D</td><td>67</td><td>-1264</td><td>F - G</td><td>66</td><td>-1154</td></tr> </table> Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.						A - B	21	-497	D - E	125	-929	B - C	8	-1681	E - F	123	-951	C - D	67
A - B	21	-497	D - E	125	-929																		
B - C	8	-1681	E - F	123	-951																		
C - D	67	-1264	F - G	66	-1154																		

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Plating Notes

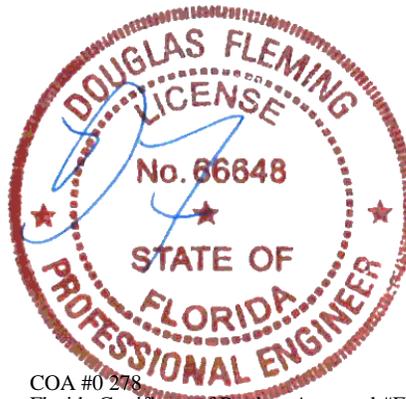
All plates are 2X4 except as noted.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

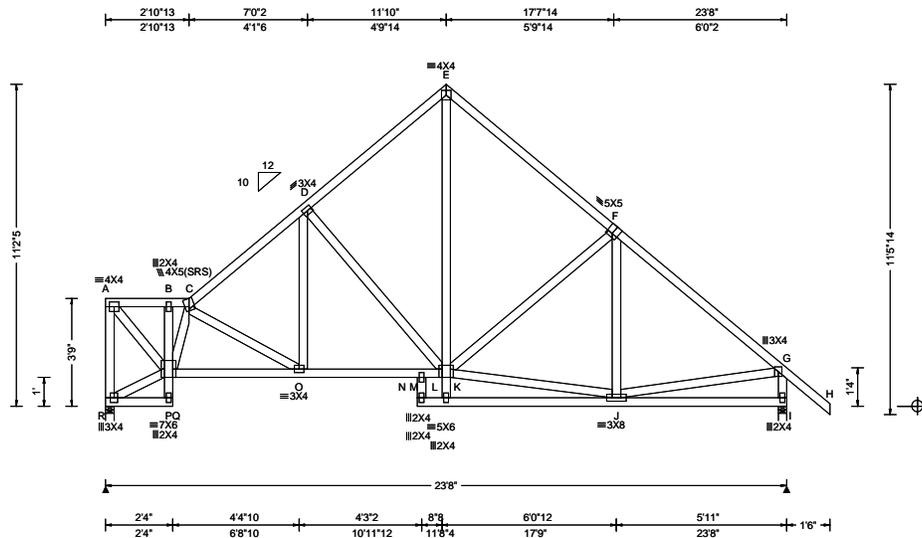
Wind loads based on MWFRS with additional C&C member design.
 End verticals not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.
 Note: Laterally brace bottom chord above filler at 20' O.C.Max. including a lateral brace at chord ends.



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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.035 L 999 240 VERT(CL): 0.075 L 999 180 HORZ(LL): 0.024 I - - HORZ(TL): 0.052 I - - Creep Factor: 2.0 Max TC CSI: 0.441 Max BC CSI: 0.336 Max Web CSI: 0.482 VIEW Ver: 21.02.00.1005.17	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity R 1015 /- /- /562 /23 /211 I 1120 /- /- /656 /13 /- Wind reactions based on MWFRS R Brg Wid = 3.5 Min Req = 1.5 (Truss) I Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings R & I are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 29 -875 D - E 127 -921 B - C 29 -889 E - F 124 -951 C - D 72 -1226 F - G 66 -1154

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

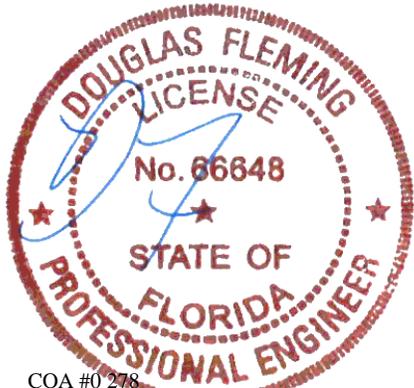
Wind
Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.
Note: Laterally brace bottom chord above filler at 2'0" O.C.Max. including a lateral brace at chord ends.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
P - O	1128 -73	M - K	859 -26
O - M	883 -23		

Maximum Web Forces Per Ply (lbs)

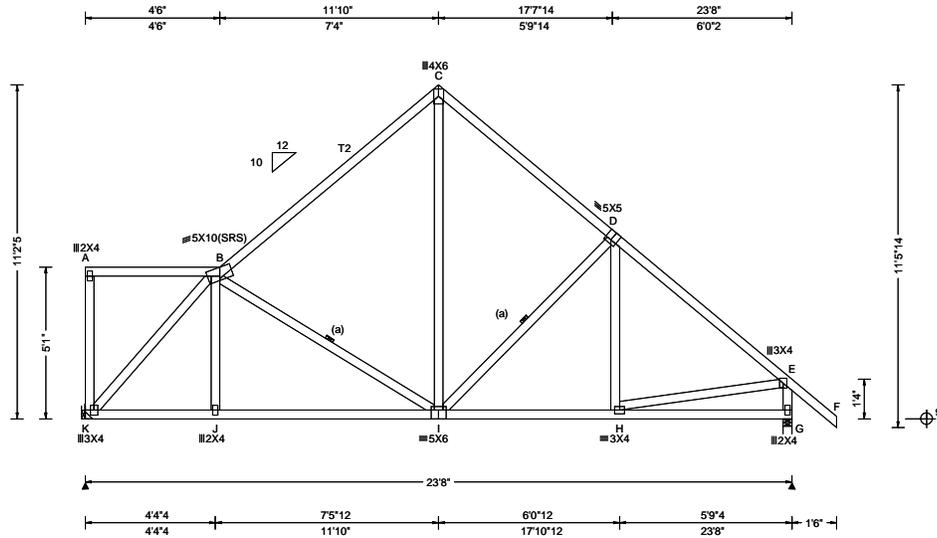
Webs	Tens.Comp.	Webs	Tens. Comp.
A - R	69 -982	E - K	701 -71
A - P	1266 -41	K - J	785 0
P - C	54 -800	J - G	771 0
D - K	86 -398	G - I	84 -1071



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Lumber
Top chord: 2x4 SP #2; T2 2x4 SP M-31;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing
(a) Continuous lateral restraint equally spaced on member.

Hangers / Ties
(J) Hanger Support Required, by others

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

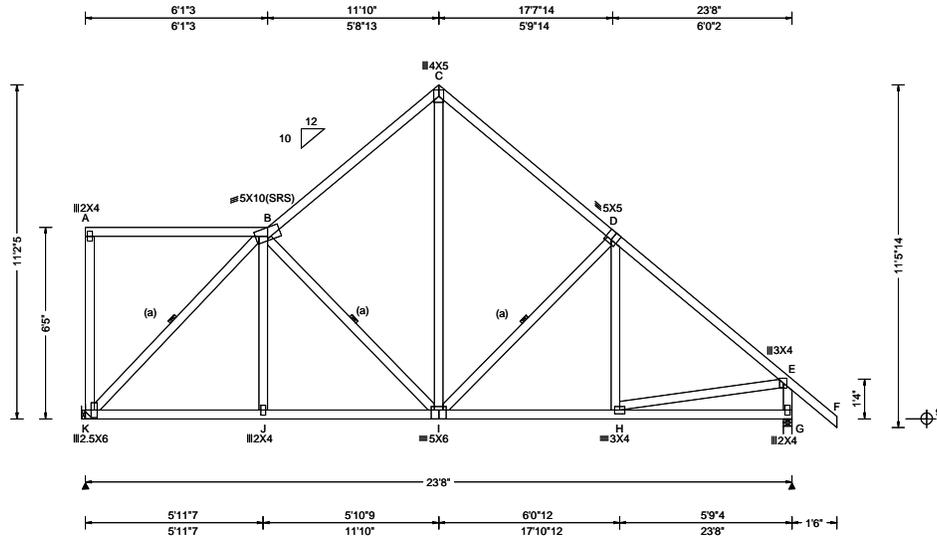
Wind
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End verticals not exposed to wind pressure.
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(a) Continuous lateral restraint equally spaced on member.

Hangers / Ties

(J) Hanger Support Required, by others

Purlins

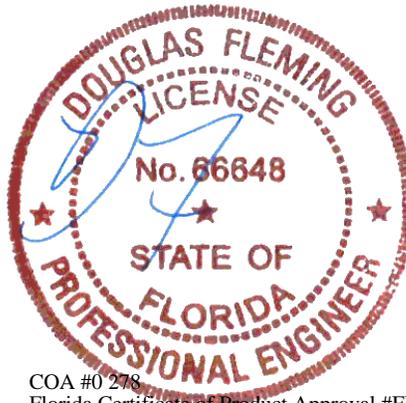
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Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

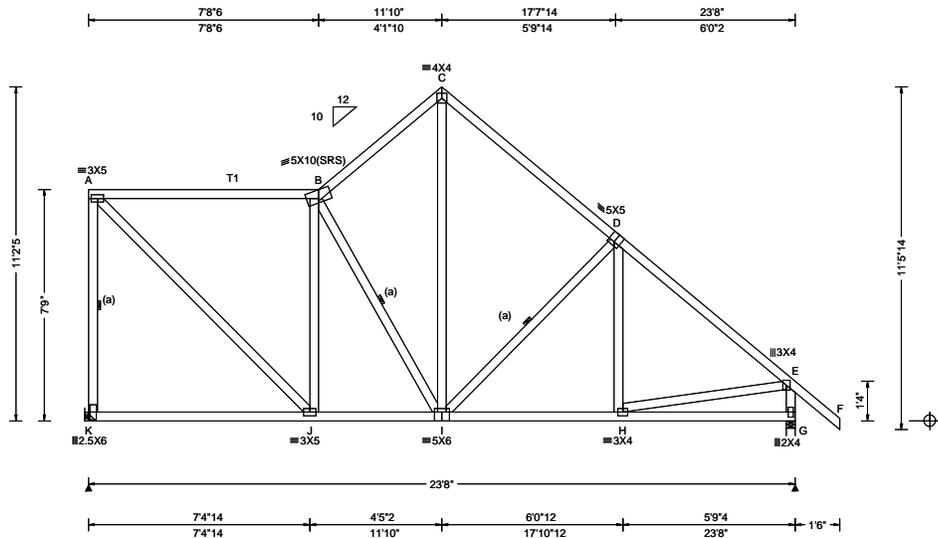
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--	---	---	--	---

Lumber
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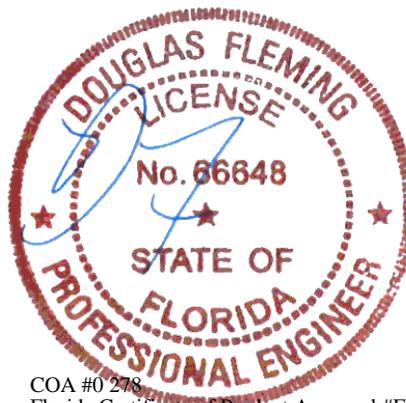
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Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
J - I	750 0	I - H	799 0

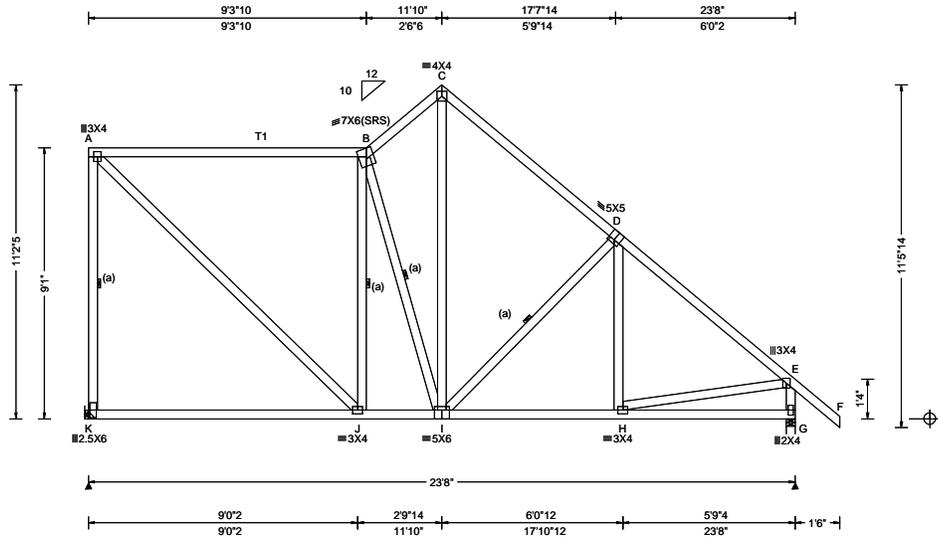
Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
A - K	271 -953	C - I	661 -154
A - J	1032 -185	H - E	773 0
J - B	205 -588	E - G	91 -1073
B - I	143 -385		



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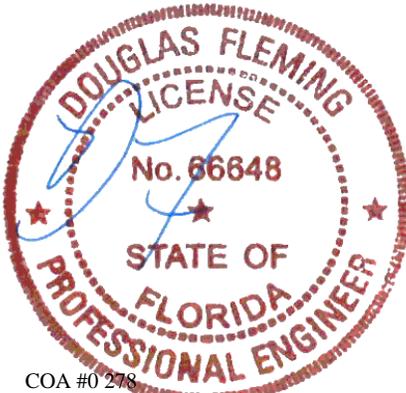
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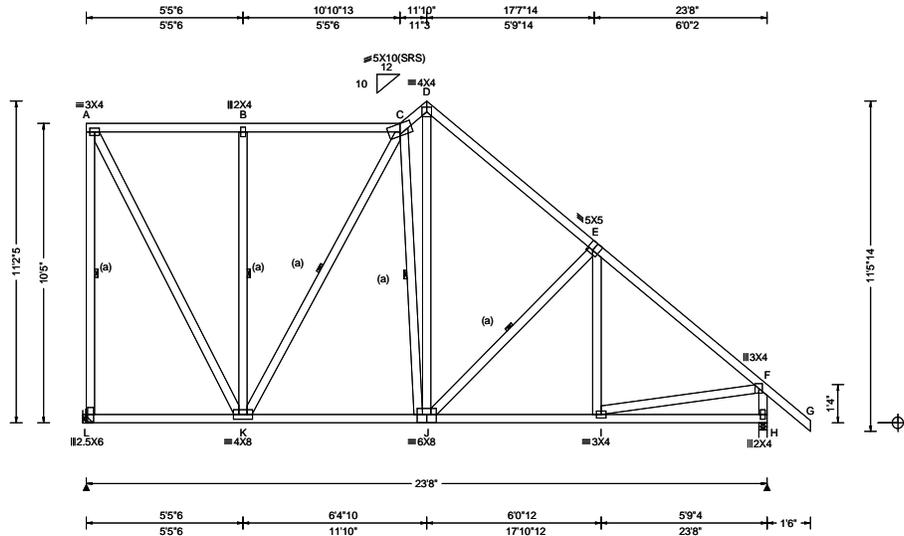
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg, Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.032 C 999 240 VERT(CL): 0.068 C 999 180 HORZ(LL): 0.011 A - - HORZ(TL): 0.024 A - - Creep Factor: 2.0 Max TC CSI: 0.436 Max BC CSI: 0.390 Max Web CSI: 0.717 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs) Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity L 1015 /- /- /563 /50 /189 H 1120 /- /- /679 /- /- Wind reactions based on MWFRS L Brg Wid = - Min Req = - H Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearing H is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 137 -448 D - E 170 -874 B - C 137 -449 E - F 92 -1150 C - D 198 -691
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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Hangers / Ties

(J) Hanger Support Required, by others

Purlins

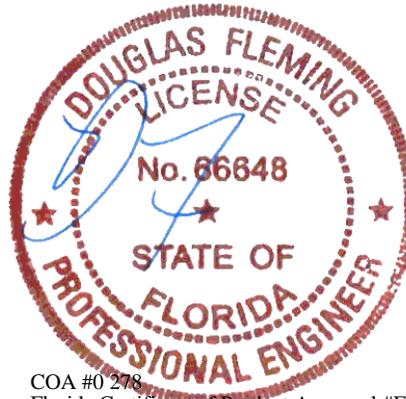
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

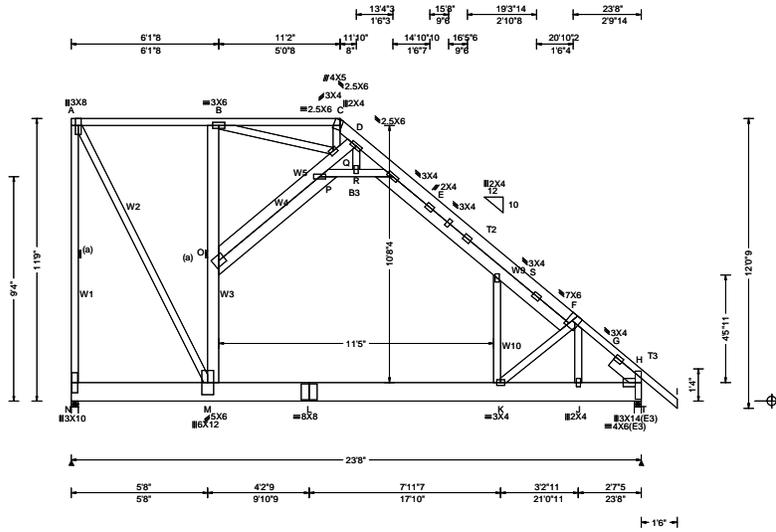
Wind loading based on both gable and hip roof types.



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Lumber
 Top chord: 2x4 SP #2; T2 2x6 SP 2400f-2.0E;
 T3 2x4 SP M-31;
 Bot chord: 2x10 SP 2400f-2.0E; B3 2x4 SP #2;
 Webs: 2x4 SP #3; W1,W2 2x4 SP M-31; W3,W4,W5,
 W9 2x6 SP 2400f-2.0E; W10 2x4 SP #2;
 Rt Slider: 2x6 SP 2400f-2.0E; block length = 1.500'

Blocking
 Blocking reinforcement required to prevent buckling of members over the bearings:
 Bearing 2 located at 23.4' (blocking >= 5.50" if used)

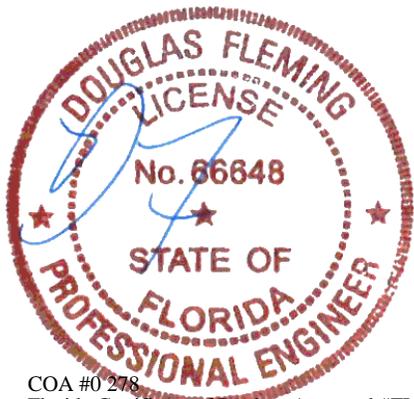
Bracing
 (a) Continuous lateral restraint equally spaced on member.

Loading
 Attic room loading from 6-1-8 to 17-6-8: Live Load: 40 PSF. Dead Load: 10 PSF Ceiling: 10 PSF, Kneewalls: 10 PSF

Purlins
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.
 Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Left end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

Deflection
 Max JT VERT DEFL: LL: 0.22" DL: 0.28". See detail DEFLCAMB1014 for camber recommendations.
 Provide for adequate drainage of roof.



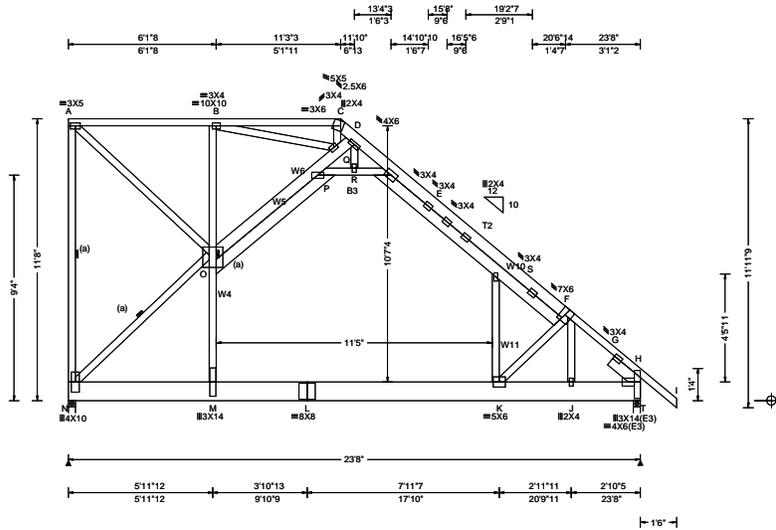
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.182 K 999 240 VERT(CL): 0.358 K 793 180 HORZ(LL): -0.133 F - - HORZ(TL): 0.263 F - - Creep Factor: 2.0 Max TC CSI: 0.622 Max BC CSI: 0.387 Max Web CSI: 0.686 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL N 1939 - / - / 583 / 53 / 195 T 2155 - / - / 735 - / - Wind reactions based on MWFRS N Brg Wid = 3.5 Min Req = 1.6 (Truss) T Brg Wid = 3.5 Min Req = 1.8 (Truss) Bearings N & T are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. S - F 1076 -3224 E - R 531 -1733 A - B 81 -800 F - G 37 -2210 B - C 436 -48 G - H 72 -2262 C - D 402 -65 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. N - M 1424 0 K - J 1603 0 M - L 1404 0 J - H 1631 0 L - K 1404 0 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. A - N 175 -901 P - Q 455 0 A - O 1088 -110 P - R 96 -1362 N - O 250 -1933 R - E 96 -1357 O - M 1536 0 E - S 1591 -34 O - P 129 -767 K - S 788 0 B - Q 64 -1264 K - F 85 -386
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Lumber
Top chord: 2x4 SP #2; T2 2x6 SP 2400f-2.0E;
Bot chord: 2x10 SP 2400f-2.0E; B3 2x4 SP #2;
Webs: 2x4 SP #3; W4, W11 2x4 SP M-31; W5, W6, W10 2x6 SP 2400f-2.0E;
Rt Slider: 2x6 SP 2400f-2.0E; block length = 1.500'

Blocking
Blocking reinforcement required to prevent buckling of members over the bearings:
Bearing 2 located at 23.4' (blocking >= 5.50" if used)

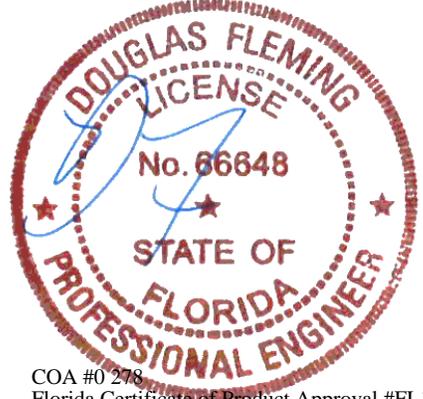
Bracing
(a) Continuous lateral restraint equally spaced on member.

Loading
Attic room loading from 6-1-8 to 17-6-8: Live Load: 40 PSF. Dead Load: 10 PSF Ceiling: 10 PSF, Kneewalls: 10 PSF

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In lieu of structural panels use purlins to brace all flat TC @ 24" oc.
Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Wind
Wind loads based on MWFRS with additional C&C member design.
Left end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

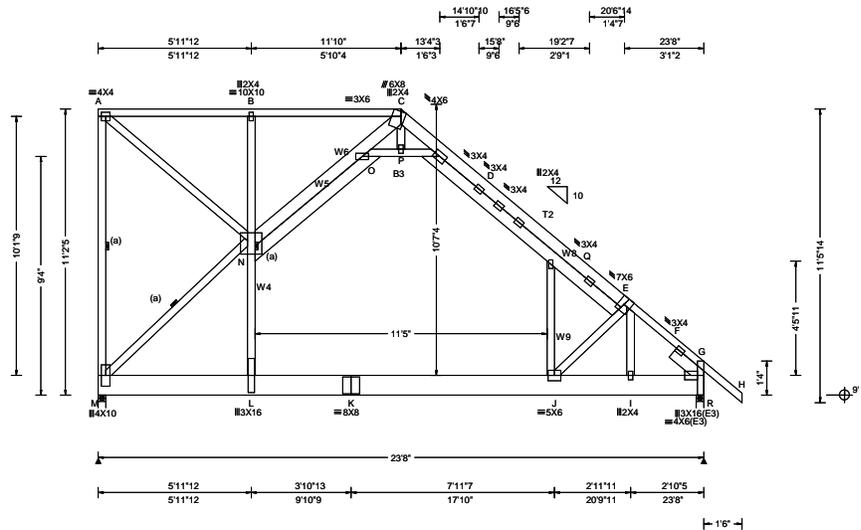
Deflection
Max JT VERT DEFL: LL: 0.18" DL: 0.20". See detail DEFLCMB1014 for camber recommendations.
Provide for adequate drainage of roof.



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Lumber
Top chord: 2x4 SP #2; T2 2x6 SP 2400f-2.0E;
Bot chord: 2x10 SP 2400f-2.0E; B3 2x4 SP #2;
Webs: 2x4 SP #3; W4,W9 2x4 SP M-31; W5,W6,
W8 2x6 SP 2400f-2.0E;
Rt Slider: 2x6 SP 2400f-2.0E; block length = 1.500'

Bracing
(a) Continuous lateral restraint equally spaced on member.

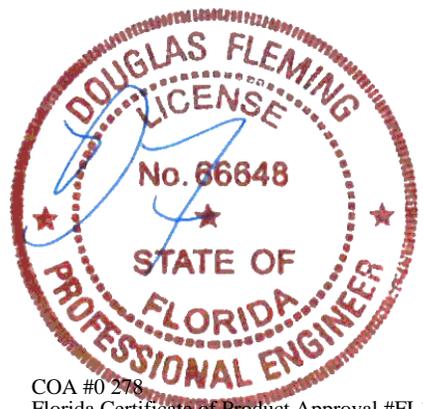
Loading
Attic room loading from 6-1-8 to 17-6-8: Live Load: 40 PSF. Dead Load: 10 PSF Ceiling: 10 PSF, Kneewalls: 10 PSF

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.
Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Wind
Wind loads based on MWFRS with additional C&C member design.
Left end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Blocking
Blocking reinforcement required to prevent buckling of members over the bearings:
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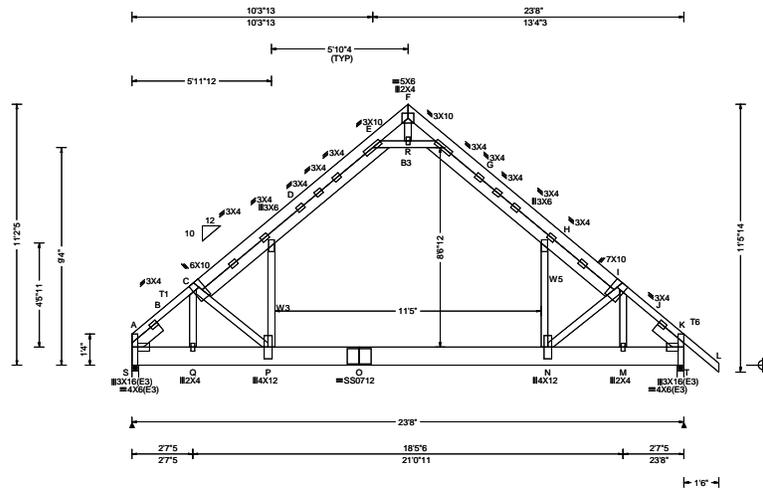
Additional Notes
The overall height of this truss excluding overhang is 11-2.5.



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3 Complete Trusses Required



Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 79.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE, 18SS	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.114 P 999 240 VERT(CL): 0.232 P 999 180 HORZ(LL): -0.082 H - - HORZ(TL): 0.169 H - - Creep Factor: 2.0 Max TC CSI: 0.444 Max BC CSI: 0.330 Max Web CSI: 0.393 VIEW Ver: 21.02.01.1214.12	▲ Maximum Reactions (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>S</td> <td>5646</td> <td>-</td> <td>-</td> <td>/1910</td> <td>/24</td> <td>/757</td> </tr> <tr> <td>T</td> <td>5974</td> <td>-</td> <td>-</td> <td>/2171</td> <td>/58</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS S Brg Wid = 3.5 Min Req = 1.6 (Truss) T Brg Wid = 3.5 Min Req = 1.6 (Truss) Bearings S & T are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs)</p> <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>C - D</td> <td>2903 -257</td> <td>B - C</td> <td>139 -2023</td> </tr> <tr> <td>G - H</td> <td>3821 -310</td> <td>G - R</td> <td>947 -1850</td> </tr> <tr> <td>H - I</td> <td>1513 -3654</td> <td>I - J</td> <td>138 -1989</td> </tr> <tr> <td>A - B</td> <td>126 -2071</td> <td>J - K</td> <td>154 -2035</td> </tr> </tbody> </table> <p>Maximum Bot Chord Forces Per Ply (lbs)</p> <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>A - Q</td> <td>1530 -58</td> <td>O - N</td> <td>1336 0</td> </tr> <tr> <td>Q - P</td> <td>1536 -53</td> <td>N - M</td> <td>1496 -9</td> </tr> <tr> <td>P - O</td> <td>1336 0</td> <td>M - K</td> <td>1489 -12</td> </tr> </tbody> </table> <p>Maximum Web Forces Per Ply (lbs)</p> <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Webs</th> <th>Tens.Comp.</th> <th>Webs</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>Q - C</td> <td>13 -718</td> <td>R - G</td> <td>287 -1719</td> </tr> <tr> <td>D - P</td> <td>1468 0</td> <td>N - H</td> <td>1450 0</td> </tr> <tr> <td>E - R</td> <td>287 -1719</td> <td>I - M</td> <td>24 -741</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	S	5646	-	-	/1910	/24	/757	T	5974	-	-	/2171	/58	-	Chords	Tens.Comp.	Chords	Tens. Comp.	C - D	2903 -257	B - C	139 -2023	G - H	3821 -310	G - R	947 -1850	H - I	1513 -3654	I - J	138 -1989	A - B	126 -2071	J - K	154 -2035	Chords	Tens.Comp.	Chords	Tens. Comp.	A - Q	1530 -58	O - N	1336 0	Q - P	1536 -53	N - M	1496 -9	P - O	1336 0	M - K	1489 -12	Webs	Tens.Comp.	Webs	Tens. Comp.	Q - C	13 -718	R - G	287 -1719	D - P	1468 0	N - H	1450 0	E - R	287 -1719	I - M	24 -741
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Lumber
 Top chord: 2x6 SP 2400F-2.0E; T1 2x4 SP M-31; T6 2x4 SP #2;
 Bot chord: 2x10 SP 2400F-2.0E; B3 2x4 SP #2;
 Webs: 2x4 SP #3; W3,W5 2x4 SP #2;
 Lt Slider: 2x6 SP 2400F-2.0E; block length = 1.500'
 Rt Slider: 2x6 SP 2400F-2.0E; block length = 1.500'

Nailnote
 Nail Schedule: 0.131"x3", min. nails
 Top Chord: 1 Row @ 3.25" o.c.
 Bot Chord: 1 Row @ 12.00" o.c.
 Webs : 1 Row @ 4" o.c.
 Repeat nailing as each layer is applied. Use equal spacing between rows and stagger nails in each row to avoid splitting.

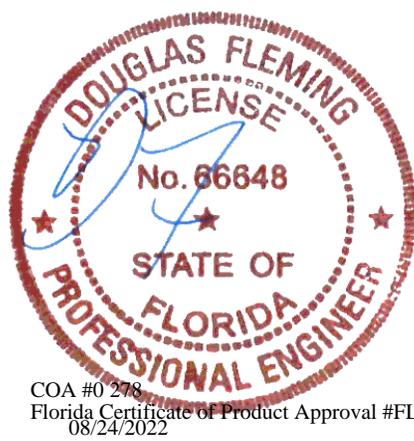
Loading
 Attic room loading from 6-1-8 to 17-6-8: Live Load: 40 PSF. Dead Load: 10 PSF Ceiling: 10 PSF, Kneewalls: 10 PSF

Purlins
 In lieu of structural panels use purlins to brace TC @ 24" oc.
 Collar-tie braced with continuous lateral bracing at 24" oc.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Blocking
 Apply additional nailing over the following bearings with fasteners at 4" oc both perpendicular and parallel to grain. In lieu of additional nailing, apply blocking reinforcement to prevent buckling of members over the bearings:
 Bearing 1 located at 0.0' (blocking >= 3.50" if used)
 Bearing 2 located at 23.4' (blocking >= 3.50" if used)

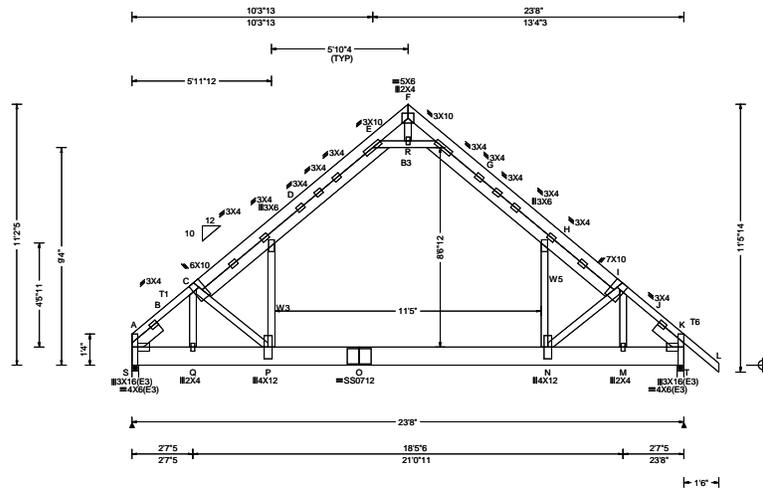
Additional Notes
 The overall height of this truss excluding overhang is 11-2-5.



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3 Complete Trusses Required



Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 79.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, 18SS	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.114 P 999 240 VERT(CL): 0.232 P 999 180 HORZ(LL): -0.082 H - - HORZ(TL): 0.169 H - - Creep Factor: 2.0 Max TC CSI: 0.444 Max BC CSI: 0.330 Max Web CSI: 0.393 VIEW Ver: 21.02.01.1214.12	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL S 5646 /- /- /1910 /24 /757 T 5974 /- /- /2171 /58 /- Wind reactions based on MWFRS S Brg Wid = 3.5 Min Req = 1.6 (Truss) T Brg Wid = 3.5 Min Req = 1.6 (Truss) Bearings S & T are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. C - D 2903 -257 B - C 139 -2023 G - H 3821 -310 G - R 947 -1850 H - I 1513 -3654 I - J 138 -1989 A - B 126 -2071 J - K 154 -2035 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - Q 1530 -58 O - N 1336 0 Q - P 1536 -53 N - M 1496 -9 P - O 1336 0 M - K 1489 -12 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. Q - C 13 -718 R - G 287 -1719 D - P 1468 0 N - H 1450 0 E - R 287 -1719 I - M 24 -741
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Lumber
Top chord: 2x6 SP 2400F-2.0E; T1 2x4 SP M-31; T6 2x4 SP #2;
Bot chord: 2x10 SP 2400F-2.0E; B3 2x4 SP #2;
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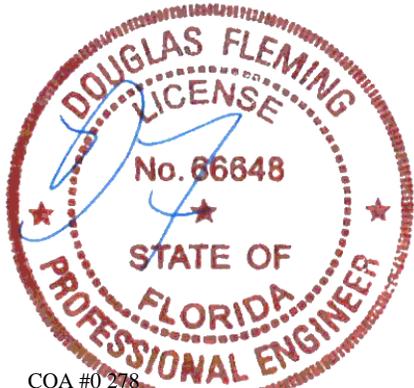
Loading
Attic room loading from 6-1-8 to 17-6-8: Live Load: 40 PSF. Dead Load: 10 PSF Ceiling: 10 PSF, Kneewalls: 10 PSF

Purlins
In lieu of structural panels use purlins to brace TC/BC @24" oc.
Collar-tie braced with continuous lateral bracing at 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

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Apply additional nailing over the following bearings with fasteners at 4" oc both perpendicular and parallel to grain. In lieu of additional nailing, apply blocking reinforcement to prevent buckling of members over the bearings:
Bearing 1 located at 0.0' (blocking >= 3.50" if used)
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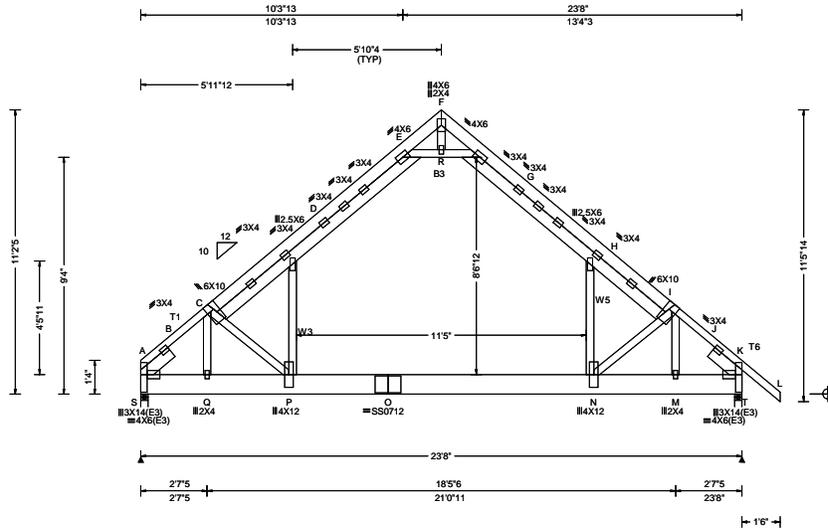
Additional Notes
The overall height of this truss excluding overhang is 11-2-5.



COA #0 278
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08/24/2022

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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, 18SS	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.104 P 999 240 VERT(CL): 0.211 P 999 180 HORZ(LL): -0.075 H - - HORZ(TL): 0.154 H - - Creep Factor: 2.0 Max TC CSI: 0.395 Max BC CSI: 0.300 Max Web CSI: 0.358 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs) Gravity Loc R+ / R- / Rh / Rw / U / RL S 1715 - / - / - / 580 / 7 / 230 T 1815 - / - / - / 659 / 18 / - Non-Gravity Wind reactions based on MWFRS S Brg Wid = 3.5 Min Req = 1.5 (Truss) T Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings S & T are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. C - D 2646 -131 B - C 45 -1844 G - H 3482 -150 G - R 764 -1686 H - I 1248 -3285 I - J 44 -1813 A - B 32 -1888 J - K 69 -1854
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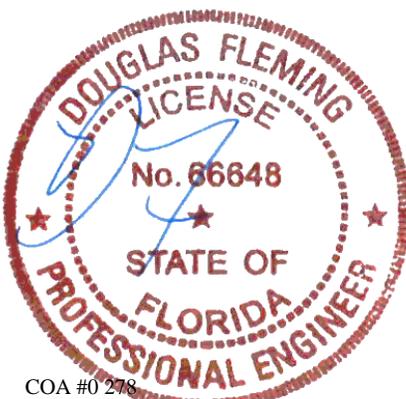
Lumber
Top chord: 2x6 SP 2400f-2.0E; T1 2x4 SP M-31; T6 2x4 SP #2;
Bot chord: 2x10 SP 2400f-2.0E; B3 2x4 SP #2;
Webs: 2x4 SP #3; W3,W5 2x4 SP #2;
Lt Slider: 2x6 SP 2400f-2.0E; block length = 1.500'
Rt Slider: 2x6 SP 2400f-2.0E; block length = 1.500'

Loading
Attic room loading from 6-1-8 to 17-6-8: Live Load: 40 PSF. Dead Load: 10 PSF Ceiling: 10 PSF, Kneewalls: 10 PSF

Purlins
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Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

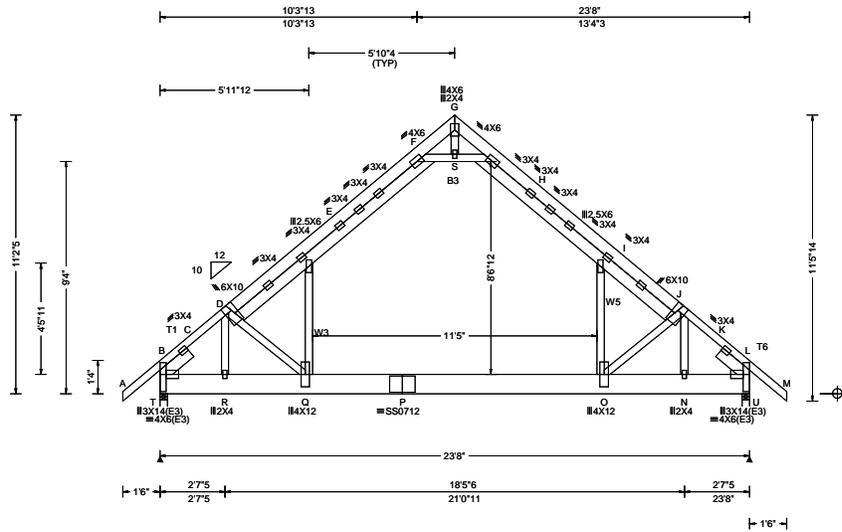
Blocking
Blocking reinforcement required to prevent buckling of members over the bearings:
Bearing 1 located at 0.0' (blocking >= 3.50" if used)
Bearing 2 located at 23.4' (blocking >= 3.50" if used)



COA #0 278
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, 18SS	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.103 Q 999 240 VERT(CL): 0.209 Q 999 180 HORZ(LL): -0.076 I - - HORZ(TL): 0.155 I - - Creep Factor: 2.0 Max TC CSI: 0.393 Max BC CSI: 0.300 Max Web CSI: 0.356 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL T 1818 /- /- /659 /17 /246 U 1812 /- /- /659 /17 /- Wind reactions based on MWFRS T Brg Wid = 3.5 Min Req = 1.5 (Truss) U Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings T & U are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. D - E 2634 -231 C - D 123 -1830 H - I 3481 -281 H - S 866 -1683 I - J 1377 -3323 J - K 126 -1824 B - C 138 -1872 K - L 137 -1865
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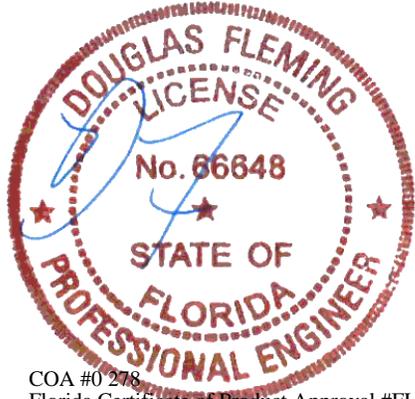
Lumber
Top chord: 2x6 SP 2400f-2.0E; T1, T6 2x4 SP M-31;
Bot chord: 2x10 SP 2400f-2.0E; B3 2x4 SP #2;
Webs: 2x4 SP #3; W3, W5 2x4 SP #2;
Lt Slider: 2x6 SP 2400f-2.0E; block length = 1.500'
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Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

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Wind loads based on MWFRS with additional C&C member design.
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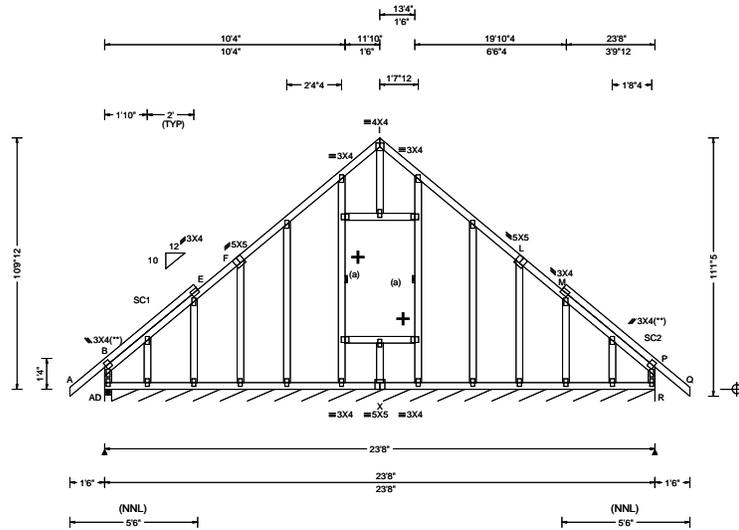
Blocking
Blocking reinforcement required to prevent buckling of members over the bearings:
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF																								
TCLL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.017 I 999 240 VERT(CL): 0.030 I 999 180 HORZ(LL): 0.026 P - - HORZ(TL): 0.046 P - - Creep Factor: 2.0 Max TC CSI: 0.260 Max BC CSI: 0.089 Max Web CSI: 0.466 VIEW Ver: 21.02.00.1005.17	<table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th colspan="2">Gravity</th> <th colspan="4">Non-Gravity</th> </tr> <tr> <th>Loc</th> <th>R+</th> <th>/ R-</th> <th>/ Rh</th> <th>/ Rw</th> <th>/ U / RL</th> </tr> </thead> <tbody> <tr> <td>AD</td> <td>289</td> <td>-</td> <td>-</td> <td>/207</td> <td>/36 /242</td> </tr> <tr> <td>R*</td> <td>83</td> <td>-</td> <td>-</td> <td>/49</td> <td>/1 -</td> </tr> </tbody> </table> Wind reactions based on MWFRS AD Brg Wid = 3.5 Min Req = 1.5 (Truss) R Brg Wid = 280 Min Req = - Bearings AD & AR are a rigid surface. Members not listed have forces less than 375#	Gravity		Non-Gravity				Loc	R+	/ R-	/ Rh	/ Rw	/ U / RL	AD	289	-	-	/207	/36 /242	R*	83	-	-	/49	/1 -
Gravity		Non-Gravity																										
Loc	R+	/ R-	/ Rh	/ Rw	/ U / RL																							
AD	289	-	-	/207	/36 /242																							
R*	83	-	-	/49	/1 -																							

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;
 Stack Chord: SC1 2x4 SP #2;
 Stack Chord: SC2 2x4 SP #2;

+ Member to be laterally braced for horizontal wind loads. bracing system to be designed and furnished by others.

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

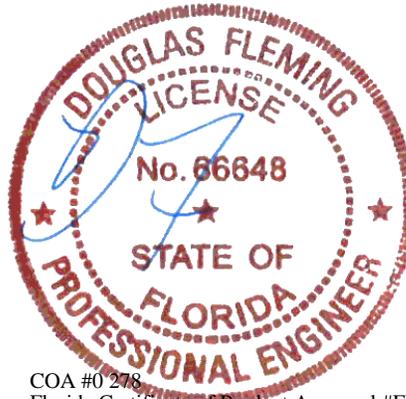
All plates are 2X4 except as noted.
 (**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Wind

Wind loads based on MWFRS with additional C&C member design.
 End verticals not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.
 Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

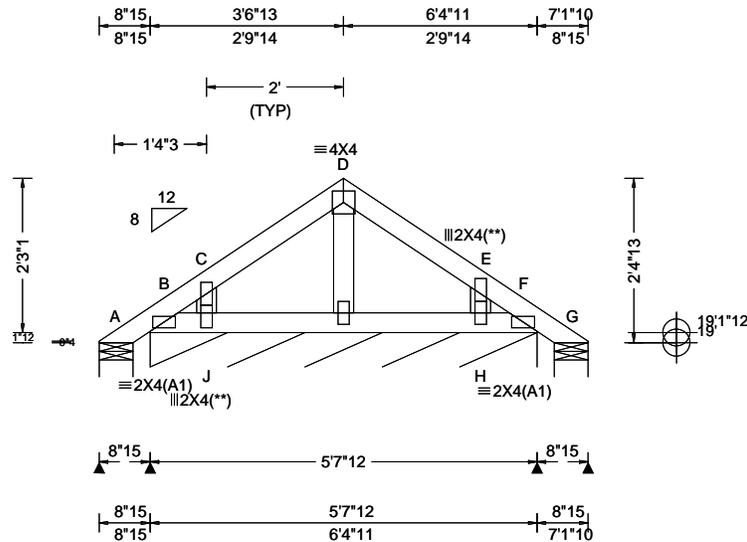


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SEQN: 99628 FROM:	GABL Ply: 1 Qty: 1	Job Number: 22-8147 Edenfield Truss Label: PB01	Cust: R 215 JRef: 1Xlc2150005 T36 DrwNo: 235.22.1113.49600 KD / DF 08/23/2022
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 20.21 ft TCDL: 5.0 psf BCDL: 2.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.000 D 999 240 VERT(CL): 0.000 D 999 180 HORZ(LL): 0.000 F - - HORZ(TL): 0.000 E - - Creep Factor: 2.0 Max TC CSI: 0.051 Max BC CSI: 0.016 Max Web CSI: 0.021 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 18 /- /- /35 /20 /43 B* 74 /- /- /54 /9 /- G 18 /- /- /15 /3 /- Wind reactions based on MWFRS A Brg Wid = 5.9 Min Req = 1.5 (Truss) B Brg Wid = 67.8 Min Req = - G Brg Wid = 5.9 Min Req = 1.5 (Truss) Bearings A, B, & G are a rigid surface. Members not listed have forces less than 375#
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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Purlins

In lieu of rigid ceiling use purlins to brace BC @ 24" oc.

Wind

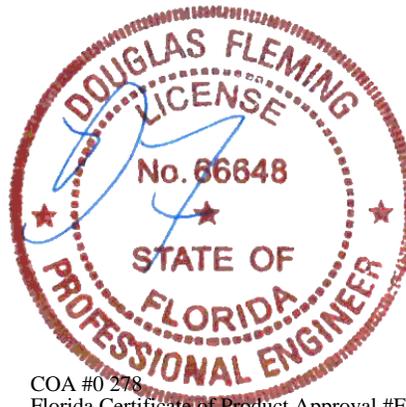
Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

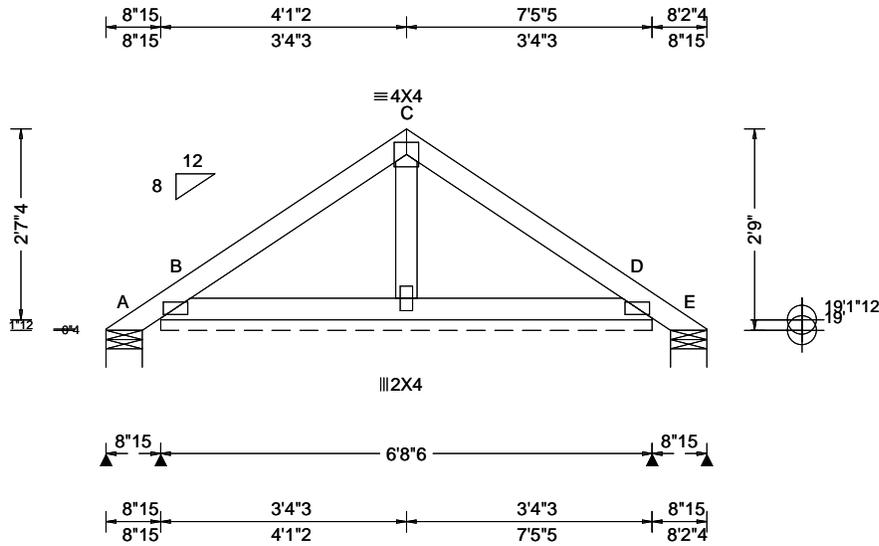
Refer to DWG PB160160118 for piggyback details.



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Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 20.39 ft TCCL: 5.0 psf BCDL: 2.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.001 B 999 240 VERT(CL): 0.001 B 999 180 HORZ(LL): 0.001 B - - HORZ(TL): 0.001 B - - Creep Factor: 2.0 Max TC CSI: 0.118 Max BC CSI: 0.050 Max Web CSI: 0.019 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs), or *=PLF <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>A</td> <td>-</td> <td>/-44</td> <td>/-</td> <td>/41</td> <td>/60</td> <td>/50</td> </tr> <tr> <td>B*</td> <td>92</td> <td>/-</td> <td>/-</td> <td>/62</td> <td>/15</td> <td>/-</td> </tr> <tr> <td>E</td> <td>-</td> <td>/-44</td> <td>/-</td> <td>/18</td> <td>/32</td> <td>/-</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	A	-	/-44	/-	/41	/60	/50	B*	92	/-	/-	/62	/15	/-	E	-	/-44	/-	/18	/32	/-
				Loc		Gravity			Non-Gravity																													
R+	/R-	/Rh	/Rw		/U	/RL																																
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				Wind reactions based on MWFRS A Brg Wid = 5.9 Min Req = 1.5 (Truss) B Brg Wid = 80.4 Min Req = - E Brg Wid = 5.9 Min Req = 1.5 (Truss) Bearings A, B, & E are a rigid surface. Members not listed have forces less than 375#																																		

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4(A1) except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Purlins

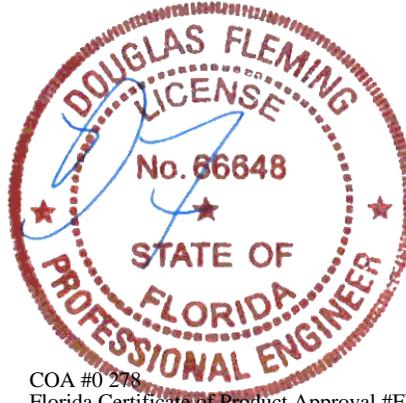
In lieu of rigid ceiling use purlins to brace BC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes

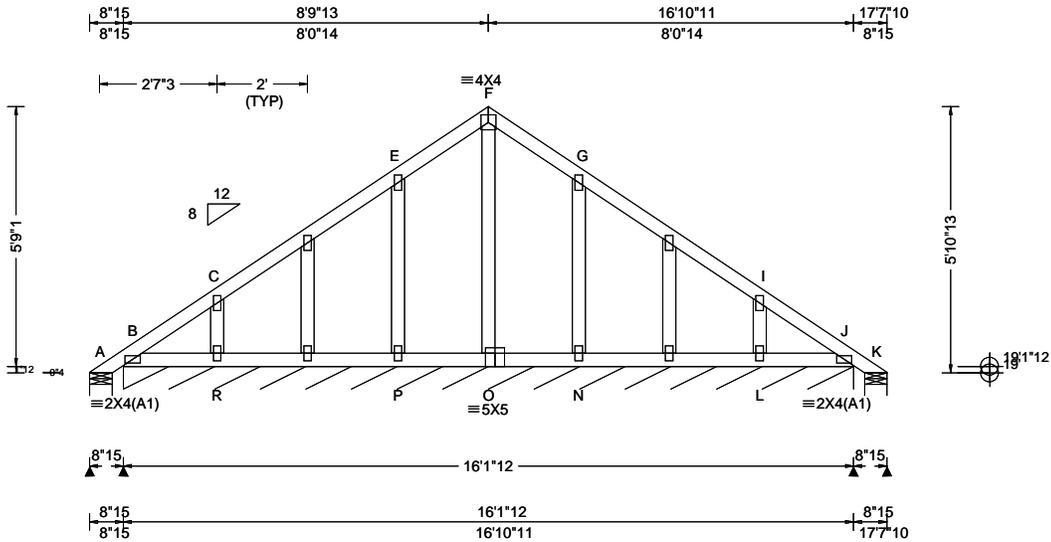
See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 21.96 ft TCDL: 5.0 psf BCDL: 2.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.13 ft Loc. from endwall: not in 6.50 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.001 G 999 240 VERT(CL): 0.001 G 999 180 HORZ(LL): 0.001 H - - HORZ(TL): 0.002 G - - Creep Factor: 2.0 Max TC CSI: 0.050 Max BC CSI: 0.023 Max Web CSI: 0.052 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 10 /- /- /75 /65 /118 B* 71 /- /- /52 /10 /- K 10 /- /- /9 /- /- Wind reactions based on MWFRS A Brg Wid = 5.9 Min Req = 1.5 (Truss) B Brg Wid = 193 Min Req = - K Brg Wid = 5.9 Min Req = 1.5 (Truss) Bearings A, B, & K are a rigid surface. Members not listed have forces less than 375#
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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Purlins

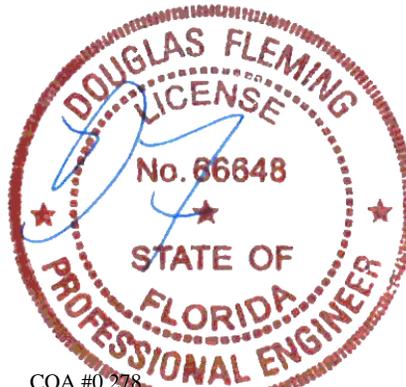
In lieu of rigid ceiling use purlins to brace BC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

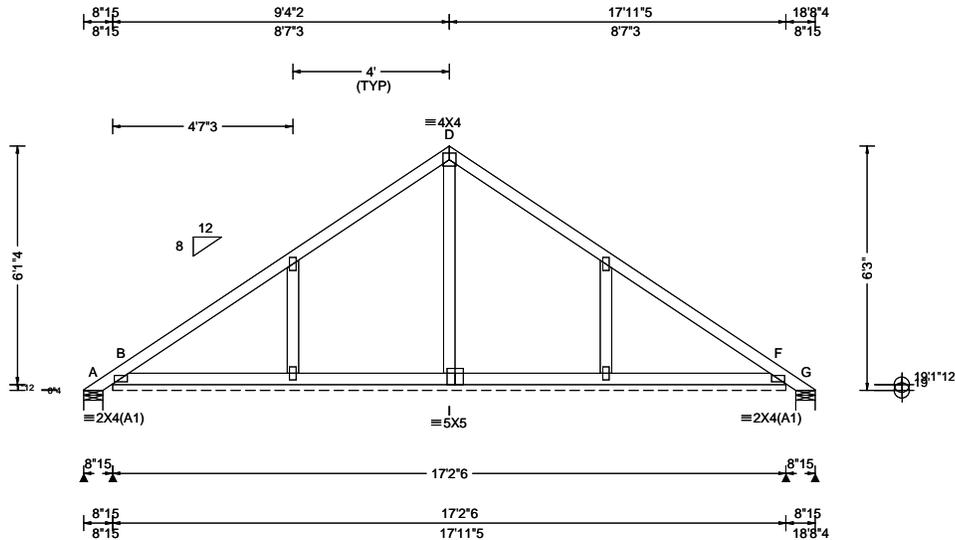
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Loading Criteria (psf) TCLL: 20.00 TC DL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 22.14 ft TC DL: 5.0 psf BCDL: 2.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.13 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.001 B 999 240 VERT(CL): 0.002 B 999 180 HORZ(LL): 0.002 E - - HORZ(TL): 0.002 E - - Creep Factor: 2.0 Max TC CSI: 0.231 Max BC CSI: 0.079 Max Web CSI: 0.094 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs), or *=PLF <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>A</td> <td>-</td> <td>/-67</td> <td>/-</td> <td>/87</td> <td>/128</td> <td>/121</td> </tr> <tr> <td>B*</td> <td>80</td> <td>/-</td> <td>/-</td> <td>/57</td> <td>/13</td> <td>/-</td> </tr> <tr> <td>G</td> <td>-</td> <td>/-67</td> <td>/-</td> <td>/30</td> <td>/61</td> <td>/-</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	A	-	/-67	/-	/87	/128	/121	B*	80	/-	/-	/57	/13	/-	G	-	/-67	/-	/30	/61	/-
				Loc		Gravity			Non-Gravity																													
R+	/R-	/Rh	/Rw		/U	/RL																																
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G	-	/-67	/-	/30	/61	/-																																
				Wind reactions based on MWFRS A Brg Wid = 5.9 Min Req = 1.5 (Truss) B Brg Wid = 206 Min Req = - G Brg Wid = 5.9 Min Req = 1.5 (Truss) Bearings A, B, & G are a rigid surface. Members not listed have forces less than 375#																																		

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Purlins

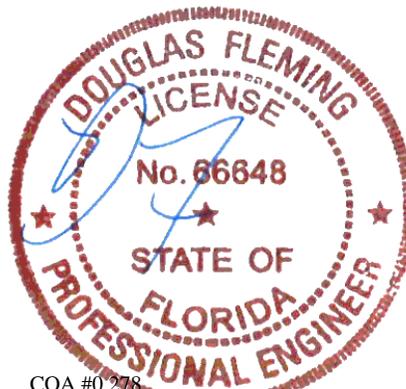
In lieu of rigid ceiling use purlins to brace BC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes

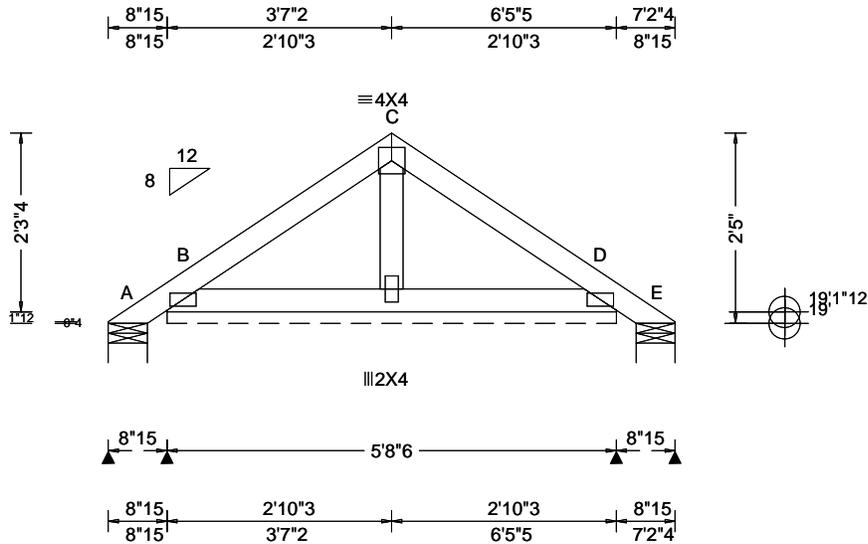
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 20.22 ft TCDL: 5.0 psf BCDL: 2.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.001 B 999 240 VERT(CL): 0.001 B 999 180 HORZ(LL): 0.000 B - - HORZ(TL): 0.001 B - - Creep Factor: 2.0 Max TC CSI: 0.082 Max BC CSI: 0.036 Max Web CSI: 0.016 VIEW Ver: 21.02.00.1005.17	Gravity Loc R+ / R- / Rh / Rw / U / RL A - /-21 /- /33 /39 /43 B* 88 /- /- /60 /1 /- E - /-21 /- /8 /15 /- Non-Gravity Wind reactions based on MWFRS A Brg Wid = 5.9 Min Req = 1.5 (Truss) B Brg Wid = 68.4 Min Req = - E Brg Wid = 5.9 Min Req = 1.5 (Truss) Bearings A, B, & E are a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4(A1) except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Purlins

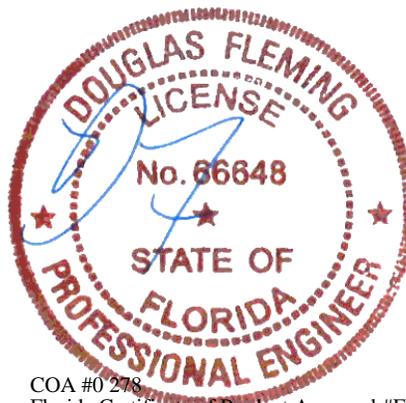
In lieu of rigid ceiling use purlins to brace BC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.
Refer to DWG PB160160118 for piggyback details.

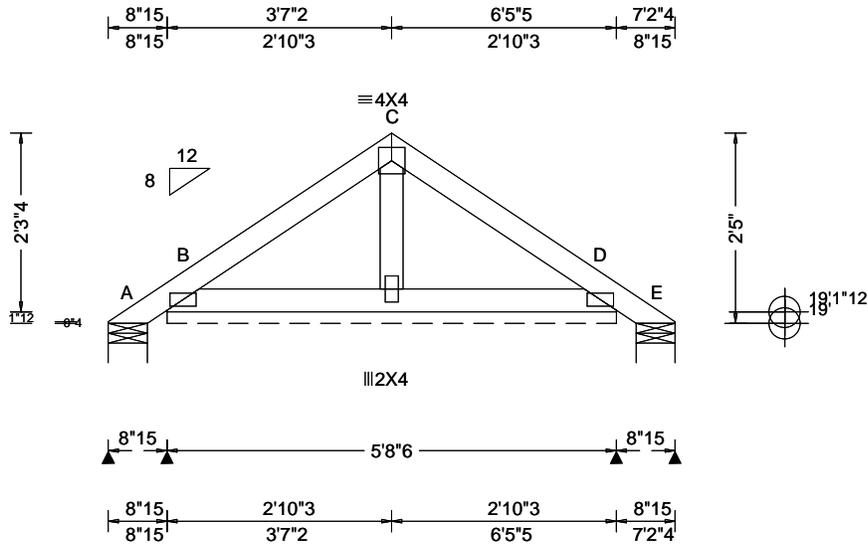


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SEQN: 99634 FROM:	SPEC Ply: 1 Qty: 1	Job Number: 22-8147 Edenfield Truss Label: PB06	Cust: R 215 JRef: 1Xlc2150005 T59 DrwNo: 235.22.1113.58840 KD / DF 08/23/2022
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Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 0.00 ft TCCL: 5.0 psf BCDL: 2.0 psf MWFRS Parallel Dist: > 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.001 B 999 240 VERT(CL): 0.001 B 999 180 HORZ(LL): 0.000 B - - HORZ(TL): 0.001 B - - Creep Factor: 2.0 Max TC CSI: 0.082 Max BC CSI: 0.036 Max Web CSI: 0.016 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL
				A - /-21 /- /33 /39 /44 B* 88 /- /- /60 /1 /- E - /-21 /- /8 /15 /- Wind reactions based on MWFRS A Brg Wid = 5.9 Min Req = 1.5 (Truss) B Brg Wid = 68.4 Min Req = - E Brg Wid = 5.9 Min Req = 1.5 (Truss) Bearings A, B, & E are a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4(A1) except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Purlins

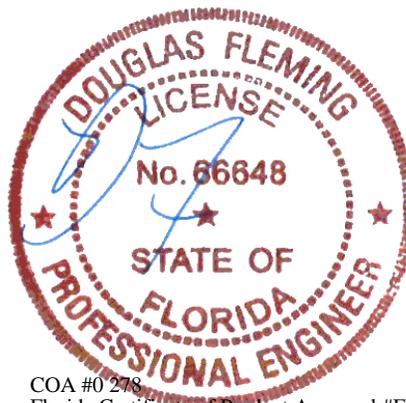
In lieu of rigid ceiling use purlins to brace BC @ 24" oc.

Wind

Wind loads based on MWFRS.
 Wind loading based on both gable and hip roof types.

Additional Notes

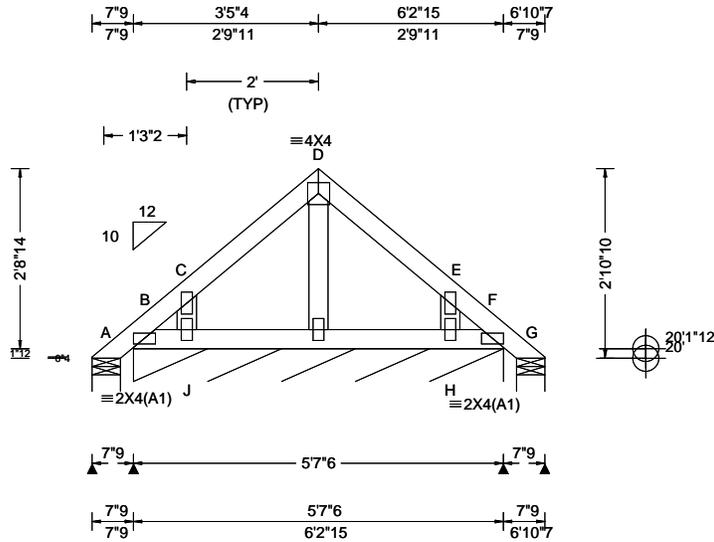
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 Refer to DWG PB160160118 for piggyback details.



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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 17.31 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.000 D 999 240 VERT(CL): 0.000 D 999 180 HORZ(LL): 0.000 F - - HORZ(TL): 0.000 E - - Creep Factor: 2.0 Max TC CSI: 0.054 Max BC CSI: 0.017 Max Web CSI: 0.023 VIEW Ver: 21.02.00.1005.17	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 16 /- /- /48 /34 /56 B* 75 /- /- /58 /10 /- G 16 /- /- /13 /2 /- Wind reactions based on MWFRS A Brg Wid = 5.2 Min Req = 1.5 (Truss) B Brg Wid = 67.4 Min Req = - G Brg Wid = 5.2 Min Req = 1.5 (Truss) Bearings A, B, & G are a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

Loading

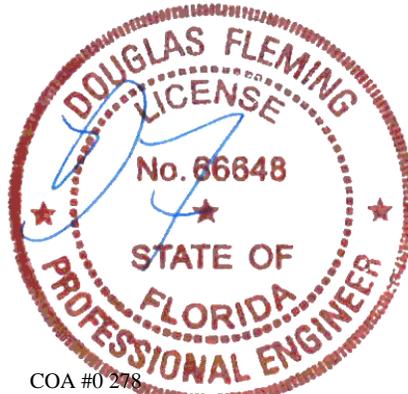
Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

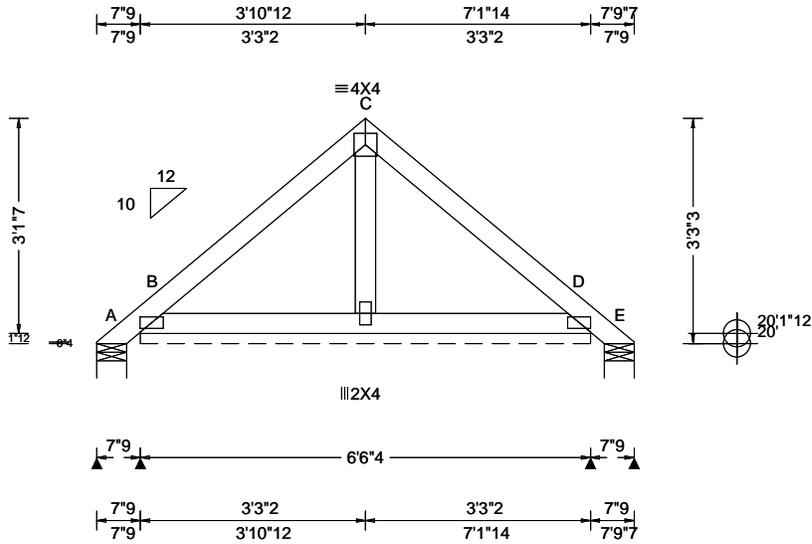
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 18.32 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.000 B 999 240 VERT(CL): 0.001 D 999 180 HORZ(LL): 0.001 D - - HORZ(TL): 0.001 D - - Creep Factor: 2.0 Max TC CSI: 0.117 Max BC CSI: 0.046 Max Web CSI: 0.018 VIEW Ver: 21.02.00.1005.17	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity A - /-57 /- /67 /91 /65 B* 97 /- /- /69 /23 /- E - /-57 /- /35 /48 /- Wind reactions based on MWFRS A Brg Wid = 5.2 Min Req = 1.5 (Truss) B Brg Wid = 78.3 Min Req = - E Brg Wid = 5.2 Min Req = 1.5 (Truss) Bearings A, B, & E are a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4(A1) except as noted.

Loading

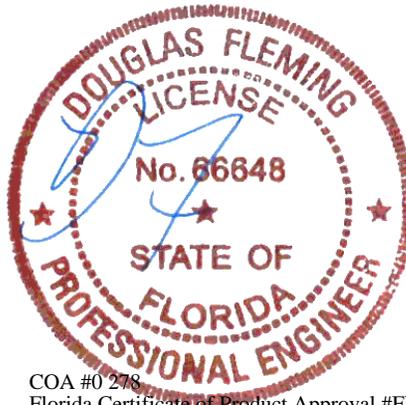
Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

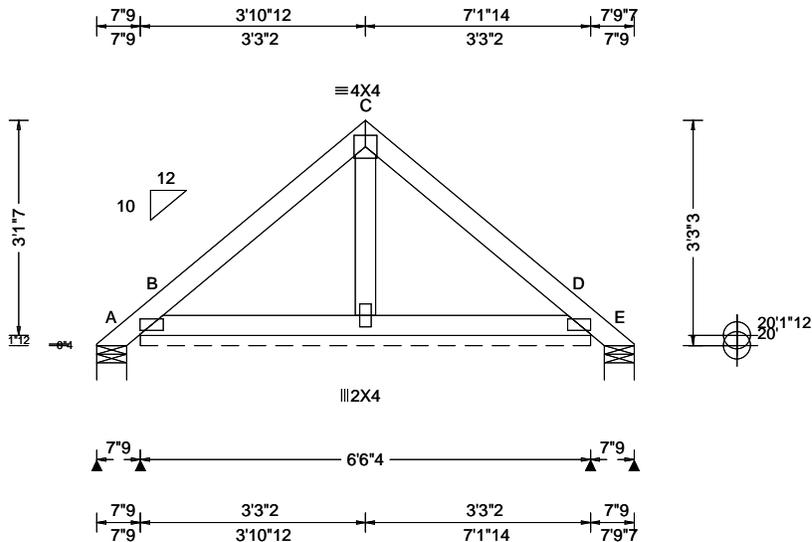
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 19.47 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.000 B 999 240 VERT(CL): 0.001 D 999 180 HORZ(LL): 0.001 D - - HORZ(TL): 0.001 D - - Creep Factor: 2.0 Max TC CSI: 0.117 Max BC CSI: 0.046 Max Web CSI: 0.018 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A - /-57 /- /68 /91 /66 B* 97 /- /- /69 /25 /- E - /-57 /- /37 /48 /- Wind reactions based on MWFRS A Brg Wid = 5.2 Min Req = 1.5 (Truss) B Brg Wid = 78.3 Min Req = - E Brg Wid = 5.2 Min Req = 1.5 (Truss) Bearings A, B, & E are a rigid surface. Members not listed have forces less than 375#
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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4(A1) except as noted.

Loading

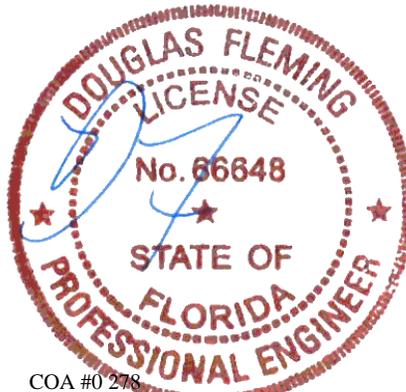
Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind

Wind loads based on MWFRS.
Wind loading based on both gable and hip roof types.

Additional Notes

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Refer to DWG PB160160118 for piggyback details.

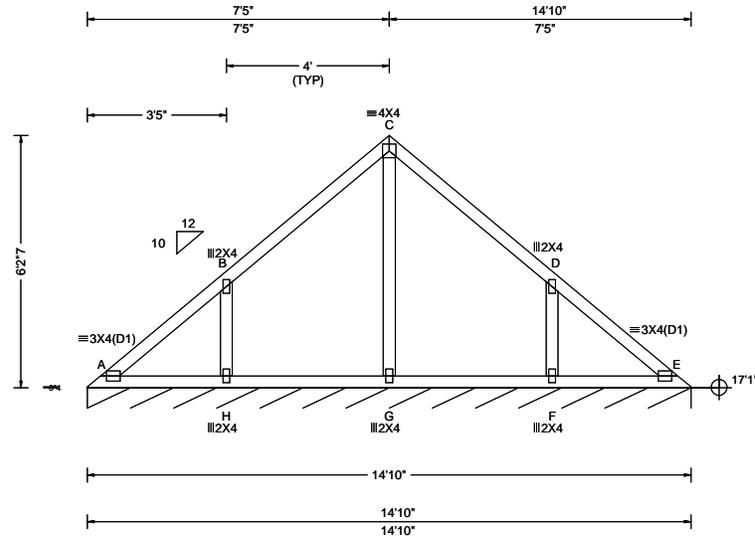


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SEQN: 99428 FROM:	VAL Ply: 1 Qty: 1	Job Number: 22-8147 Edenfield Truss Label: V01	Cust: R 215 JRef: 1Xlc2150005 T51 DrwNo: 235.22.1114.05583 KD / DF 08/23/2022
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Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 20.34 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.002 E 999 240 VERT(CL): 0.004 E 999 180 HORZ(LL): -0.002 B - - HORZ(TL): 0.002 E - - Creep Factor: 2.0 Max TC CSI: 0.268 Max BC CSI: 0.119 Max Web CSI: 0.138 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL E* 86 /- /- /46 /5 /8 Wind reactions based on MWFRS E Brg Wid = 178 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#

Lumber

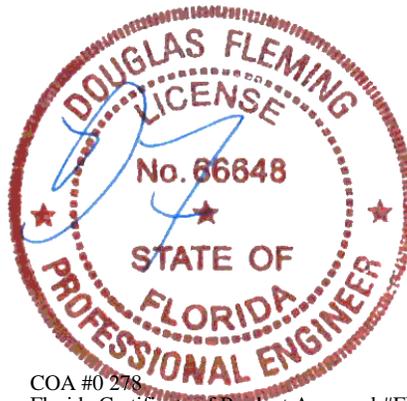
Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes

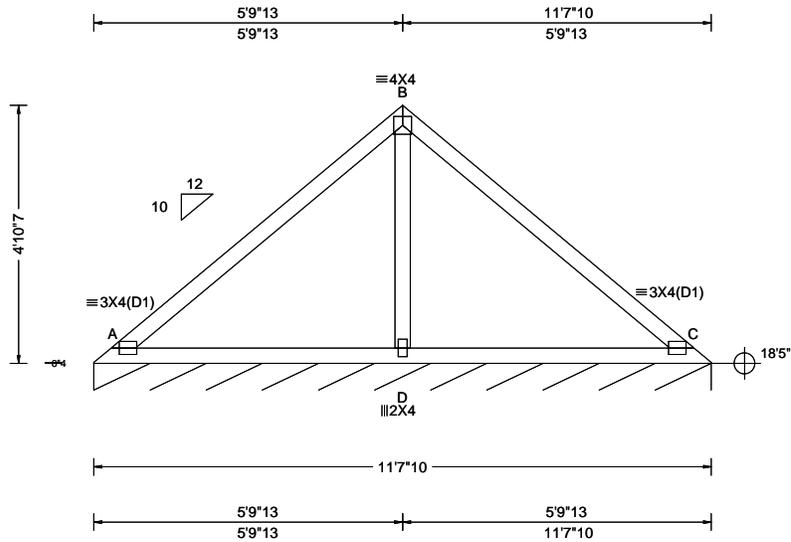
See DWGS VALTN160118 and VAL180160118 for valley details.



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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 21.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.017 C 999 240 VERT(CL): 0.037 C 999 180 HORZ(LL): -0.010 C - - HORZ(TL): 0.022 C - - Creep Factor: 2.0 Max TC CSI: 0.508 Max BC CSI: 0.425 Max Web CSI: 0.287 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL C* 86 /- /- /46 /0 /8 Wind reactions based on MWFRS C Brg Wid = 139 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 402 -141 B - C 402 -144 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. B - D 258 -698
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Lumber

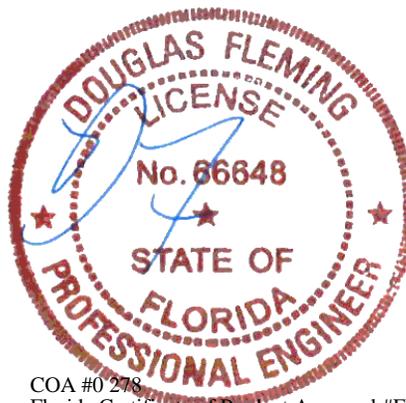
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

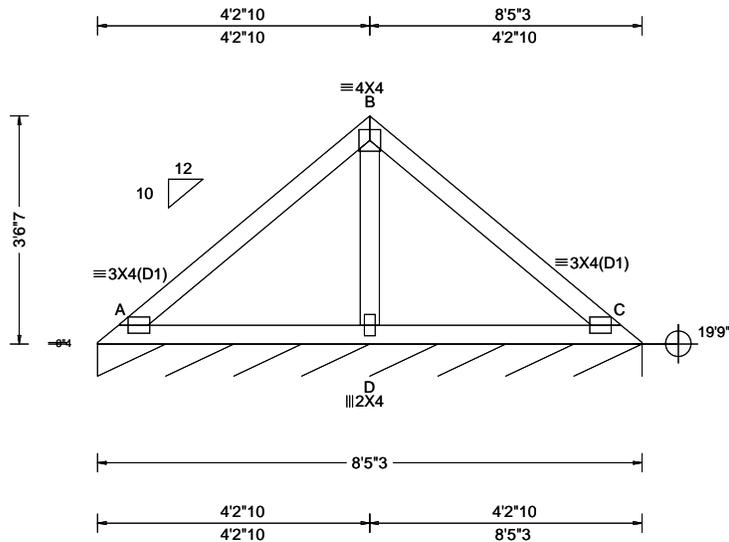
See DWGS VALTN160118 and VAL180160118 for valley details.



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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF																																																												
TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 21.67 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/ VERT(LL): 0.006 C 999 240 VERT(CL): 0.014 C 999 180 HORZ(LL): -0.004 C - - HORZ(TL): 0.008 C - - Creep Factor: 2.0 Max TC CSI: 0.245 Max BC CSI: 0.208 Max Web CSI: 0.104 VIEW Ver: 21.02.00.1005.17	<table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th colspan="2">Gravity</th> <th colspan="4">Non-Gravity</th> </tr> <tr> <th>Loc</th> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U /RL</th> </tr> </thead> <tbody> <tr> <td>C*</td> <td>86</td> <td>/-</td> <td>/-</td> <td>/45</td> <td>/-</td> </tr> <tr> <td colspan="6">Wind reactions based on MWFRS</td> </tr> <tr> <td colspan="6">C Brg Wid = 101 Min Req = -</td> </tr> <tr> <td colspan="6">Bearing A is a rigid surface.</td> </tr> <tr> <td colspan="6">Members not listed have forces less than 375#</td> </tr> <tr> <td colspan="6">Maximum Web Forces Per Ply (lbs)</td> </tr> <tr> <td colspan="2">Webs</td> <td colspan="4">Tens.Comp.</td> </tr> <tr> <td colspan="2">B - D</td> <td colspan="4">196 -416</td> </tr> </tbody> </table>	Gravity		Non-Gravity				Loc	R+	/R-	/Rh	/Rw	/U /RL	C*	86	/-	/-	/45	/-	Wind reactions based on MWFRS						C Brg Wid = 101 Min Req = -						Bearing A is a rigid surface.						Members not listed have forces less than 375#						Maximum Web Forces Per Ply (lbs)						Webs		Tens.Comp.				B - D		196 -416			
Gravity		Non-Gravity																																																														
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Lumber

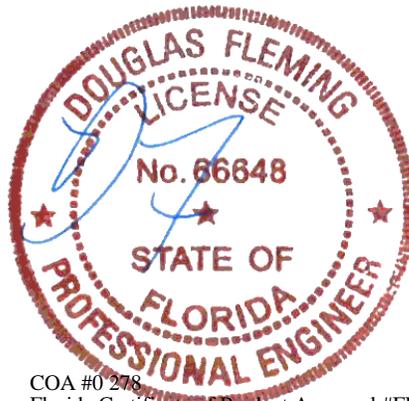
Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes

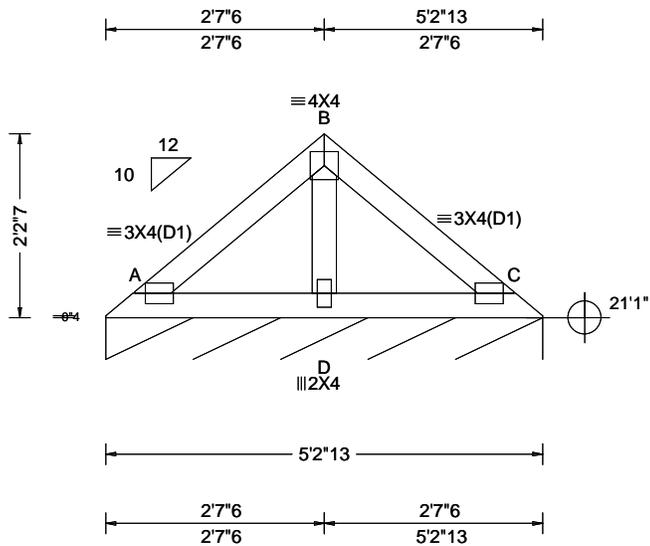
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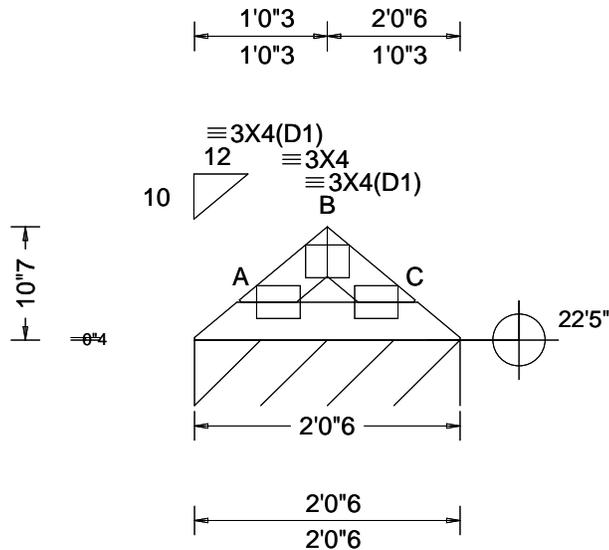
Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 22.34 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.002 C 999 240 VERT(CL): 0.003 C 999 180 HORZ(LL): -0.001 C - - HORZ(TL): 0.002 C - - Creep Factor: 2.0 Max TC CSI: 0.078 Max BC CSI: 0.067 Max Web CSI: 0.038 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs), or *=PLF <table border="1"> <thead> <tr> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>Loc</th> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>C*</td> <td>85</td> <td>/-</td> <td>/-</td> <td>/44</td> <td>/-</td> <td>/7</td> </tr> </tbody> </table> Wind reactions based on MWFRS C Brg Wid = 62.8 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#	Gravity			Non-Gravity			Loc	R+	/R-	/Rh	/Rw	/U	/RL	C*	85	/-	/-	/44	/-	/7
				Gravity			Non-Gravity																	
Loc	R+	/R-	/Rh	/Rw	/U	/RL																		
C*	85	/-	/-	/44	/-	/7																		
Lumber Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #3; Wind Wind loads based on MWFRS with additional C&C member design. Wind loading based on both gable and hip roof types. Additional Notes See DWGS VALTN160118 and VAL180160118 for valley details.																								



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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF						
				Gravity			Non-Gravity			
				Loc	R+	/R-	/Rh	/Rw	/U	/RL
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	C*	84	/-	/-	/38	/-	/5
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.000 C 999 240	Wind reactions based on MWFRS						
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.001 C 999 180	C Brg Wid = 24.4 Min Req = -						
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.000 A - -	Bearing A is a rigid surface.						
Des Ld: 40.00	EXP: B Kzt: NA		HORZ(TL): 0.000 A - -	Members not listed have forces less than 375#						
NCBCLL: 10.00	Mean Height: 23.00 ft	Building Code:	Creep Factor: 2.0							
Soffit: 0.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.015							
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.026							
Spacing: 24.0 "	MWFRS Parallel Dist: h/2 to h	Rep Fac: Yes	Max Web CSI: 0.000							
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)	VIEW Ver: 21.02.00.1005.17							
	Loc. from endwall: not in 9.00 ft	Plate Type(s):								
	GCp1: 0.18	WAVE								
	Wind Duration: 1.60									

Lumber

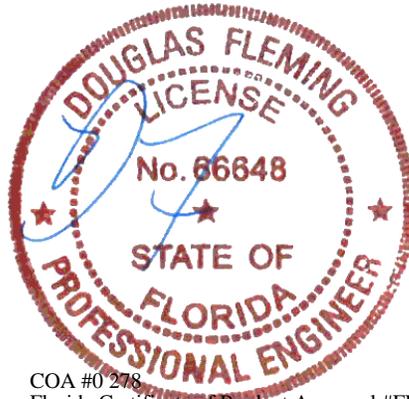
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Wind

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Wind loading based on both gable and hip roof types.

Additional Notes

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Gable Stud Reinforcement Detail

ASCE 7-16: 140 mph Wind Speed, 30' Mean Height, Enclosed, Exposure C, Kzt = 1.00

- Or: 120 mph Wind Speed, 30' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00
- Or: 120 mph Wind Speed, 30' Mean Height, Enclosed, Exposure D, Kzt = 1.00
- Or: 100 mph wind speed, 30' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

Max Gable Vertical Length	2x4 Gable Vertical Spacing		Brace Grade	No Braces	(1) 1x4 'L' Brace *		(1) 2x4 'L' Brace *		(2) 2x4 'L' Brace **		(1) 2x6 'L' Brace *		(2) 2x6 'L' Brace **		
	Species	Grade			Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	
	24" o.c.	SPF	#1 / #2	HF	#1 / #2	4' 1"	6' 11"	7' 2"	8' 2"	8' 6"	9' 9"	10' 2"	12' 10"	13' 4"	14' 0"
#3					3' 10"	6' 2"	6' 7"	8' 1"	8' 5"	9' 8"	10' 0"	12' 8"	13' 2"	14' 0"	14' 0"
Stud					3' 10"	6' 2"	6' 6"	8' 1"	8' 5"	9' 8"	10' 0"	12' 8"	13' 2"	14' 0"	14' 0"
Standard			#1	3' 10"	5' 3"	5' 7"	7' 0"	7' 6"	9' 6"	10' 0"	11' 0"	11' 10"	14' 0"	14' 0"	
			#2	4' 2"	7' 0"	7' 3"	8' 3"	8' 7"	9' 10"	10' 3"	13' 0"	13' 6"	14' 0"	14' 0"	
			#3	4' 1"	6' 11"	7' 2"	8' 2"	8' 6"	9' 9"	10' 2"	12' 10"	13' 4"	14' 0"	14' 0"	
SP		DFL	#1	4' 0"	5' 7"	5' 11"	7' 5"	7' 11"	9' 8"	10' 1"	11' 7"	12' 5"	14' 0"	14' 0"	
			#2	4' 0"	5' 7"	5' 11"	7' 5"	7' 11"	9' 8"	10' 1"	11' 7"	12' 5"	14' 0"	14' 0"	
			Stud	4' 0"	5' 7"	5' 11"	7' 5"	7' 11"	9' 8"	10' 1"	11' 7"	12' 5"	14' 0"	14' 0"	
		Standard	#1	3' 9"	4' 11"	5' 13"	6' 6"	7' 0"	8' 10"	9' 6"	10' 3"	11' 0"	13' 11"	14' 0"	
			#2	4' 8"	7' 11"	8' 3"	9' 4"	9' 9"	11' 2"	11' 7"	14' 0"	14' 0"	14' 0"	14' 0"	
			#3	4' 5"	7' 6"	8' 3"	9' 3"	9' 7"	11' 0"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"	
16" o.c.	SPF	#1 / #2	HF	#1 / #2	4' 8"	7' 11"	8' 3"	9' 4"	9' 9"	11' 2"	11' 7"	14' 0"	14' 0"	14' 0"	14' 0"
				#3	4' 5"	7' 6"	8' 3"	9' 3"	9' 7"	11' 0"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"
				Stud	4' 5"	7' 6"	8' 0"	9' 3"	9' 7"	11' 0"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	#1	4' 10"	8' 0"	8' 4"	9' 6"	9' 10"	11' 3"	11' 9"	14' 0"	14' 0"	14' 0"	14' 0"	
			#2	4' 8"	7' 11"	8' 3"	9' 4"	9' 9"	11' 2"	11' 7"	14' 0"	14' 0"	14' 0"	14' 0"	
			#3	4' 7"	6' 10"	7' 3"	9' 1"	9' 8"	11' 1"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"	
	SP	DFL	#1	4' 7"	6' 10"	7' 3"	9' 1"	9' 8"	11' 1"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"	
			#2	4' 7"	6' 10"	7' 3"	9' 1"	9' 8"	11' 1"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"	
			Stud	4' 7"	6' 10"	7' 3"	9' 1"	9' 8"	11' 1"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"	
		Standard	#1 / #2	4' 5"	6' 0"	6' 5"	8' 0"	8' 7"	10' 10"	11' 6"	12' 7"	13' 15"	14' 0"	14' 0"	
			#3	4' 5"	6' 0"	6' 5"	8' 0"	8' 7"	10' 10"	11' 6"	12' 7"	13' 15"	14' 0"	14' 0"	
			Stud	4' 5"	6' 0"	6' 5"	8' 0"	8' 7"	10' 10"	11' 6"	12' 7"	13' 15"	14' 0"	14' 0"	
12" o.c.	SPF	#1 / #2	HF	#1 / #2	5' 2"	8' 9"	9' 1"	10' 4"	10' 9"	11' 2"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"
				#3	4' 10"	8' 7"	8' 11"	10' 2"	10' 7"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
				Stud	4' 10"	8' 7"	8' 11"	10' 2"	10' 7"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	#1	4' 10"	7' 5"	7' 11"	9' 11"	10' 7"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"	
			#2	5' 4"	8' 10"	9' 2"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"	
			#3	5' 2"	8' 9"	9' 1"	10' 4"	10' 9"	11' 2"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"	
	SP	DFL	#1	5' 4"	8' 10"	9' 2"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"	
			#2	5' 2"	8' 9"	9' 1"	10' 4"	10' 9"	12' 3"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"	
			#3	5' 0"	7' 10"	8' 4"	10' 3"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"	
		Standard	#1	5' 0"	7' 10"	8' 4"	10' 3"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"	
			#2	5' 0"	7' 10"	8' 4"	10' 3"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"	
			Stud	5' 0"	7' 10"	8' 4"	10' 3"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"	

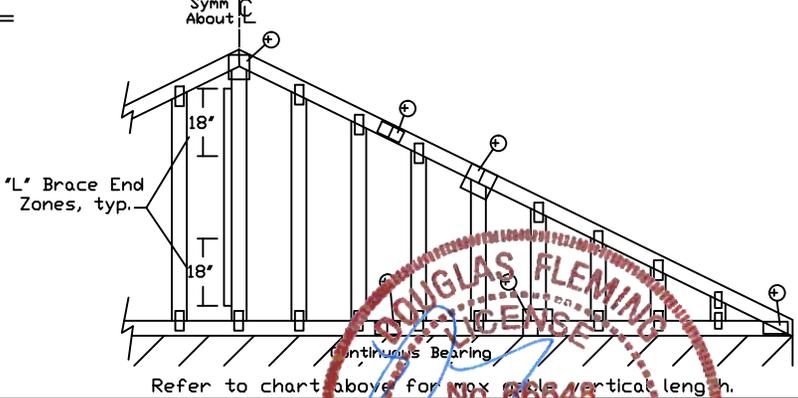
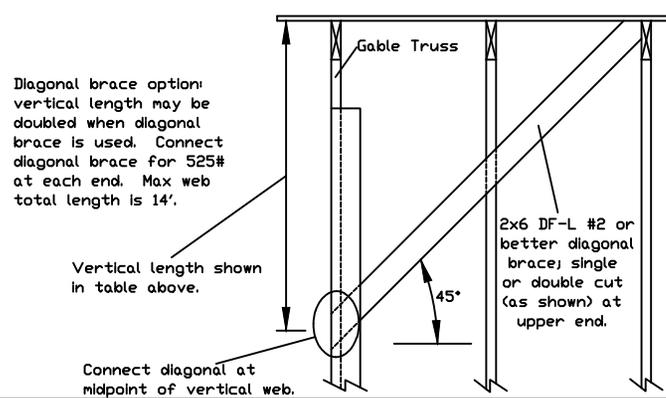
Bracing Group Species and Grades:

Group A:			
Spruce-Pine-Fir		Hem-Fir	
#1 / #2	Standard	#2	Stud
#3	Stud	#3	Standard
Douglas Fir-Larch		Southern Pine***	
#3	Stud	#3	Stud
Standard	Standard	Standard	Standard

Group B:			
Hem-Fir			
#1 & Btr		#1	
Douglas Fir-Larch			
#1	#2	#1	#2

1x4 Braces shall be SRB (Stress-Rated Board).
 ***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:
 Wind Load deflection criterion is L/240.
 Provide uplift connections for 100 plf over continuous bearing (5 psf TC Dead Load).
 Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12' plywood overhang.



Attach 'L' braces with 10d (0.128"x3.0" min) nails.
 * For (1) 'L' brace: space nails at 2' o.c. in 18" end zones and 4' o.c. between zones.
 ** For (2) 'L' braces: space nails at 3' o.c. in 18" end zones and 6' o.c. between zones.

'L' bracing must be a minimum of 80% of web member length.

Gable Vertical Plate Sizes	
Vertical Length	No Splice
Less than 4' 0"	2X4
Greater than 4' 0", but less than 11' 6"	3X4
Greater than 11' 6"	4X4

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.

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North Building, 4th Floor
Glenview, IL 60025

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08/24/2022 2:28
 MAX. TOT. LD. 60 PSF
 MAX. SPACING 24.0"

REF	ASCE7-16-GAB14030
DATE	01/26/2018
DRWG	A14030ENC160118
MAX. TOT. LD. 60 PSF	
MAX. SPACING 24.0"	

CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforcement or scab reinforcement.

Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

Use scabs instead of L- or T- reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

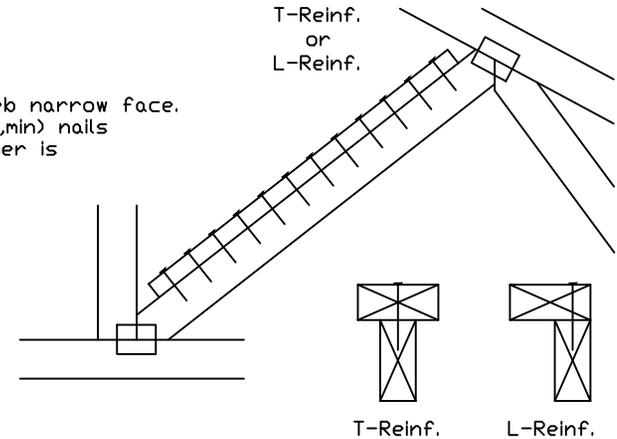
Web Member Size	Specified CLR Restraint	Alternative Reinforcement T- or L- Reinf.	Scab Reinf.
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6 or 2x4	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4(⊗)
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6(⊗)

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

(⊗) Center scab on wide face of web. Apply (1) scab to each face of web.

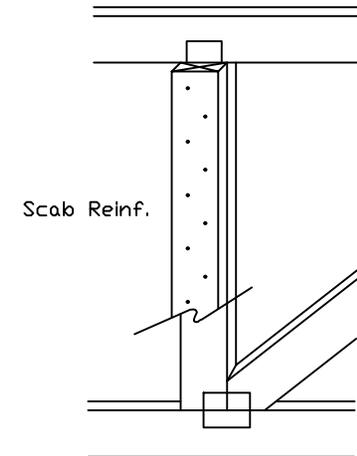
T-Reinforcement or L-Reinforcement:

Apply to either side of web narrow face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



Scab Reinforcement:

Apply scab(s) to wide face of web. No more than (1) scab per face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



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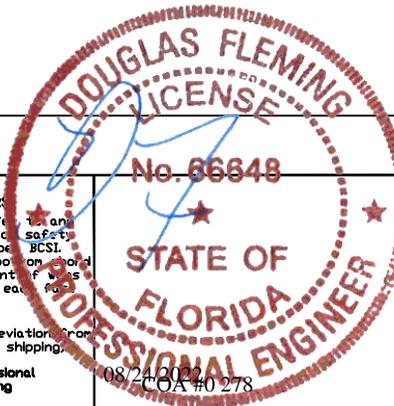
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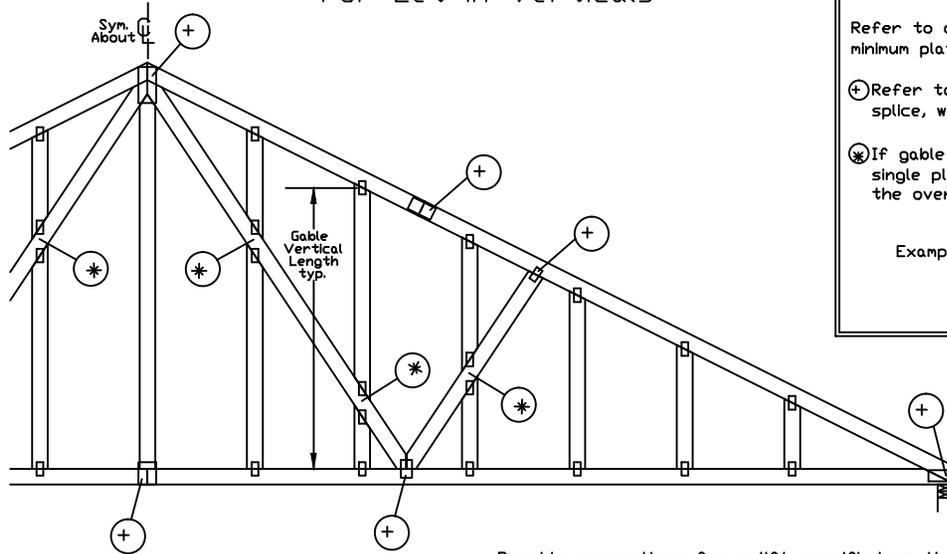
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TC LL	PSF	REF CLR Subst.
TC DL	PSF	DATE 01/02/19
BC DL	PSF	DRWG BRCLBSUB0119
BC LL	PSF	
TOT. LD.	PSF	
DUR. FAC.		
SPACING		

Gable Detail For Let-in Verticals

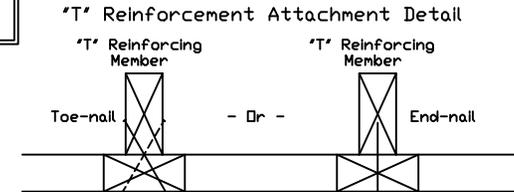


Gable Truss Plate Sizes

Refer to appropriate Alpine gable detail for minimum plate sizes for vertical studs.

- ⊕ Refer to Engineered truss design for peak, splice, web, and heel plates.
- ⊗ If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.

Example:



Provide connections for uplift specified on the engineered truss design.
Attach each 'T' reinforcing member with
End Driven Nails:
10d Common (0.148"x3",min) Nails at 4' o.c. plus
(4) nails in the top and bottom chords.

Toenailed Nails:
10d Common (0.148"x3",min) Toenails at 4' o.c. plus
(4) toenails in the top and bottom chords.

This detail to be used with the appropriate Alpine gable detail for ASCE wind load.

- ASCE 7-05 Gable Detail Drawings
A13015051014, A12015051014, A11015051014, A10015051014, A14015051014,
A13030051014, A12030051014, A11030051014, A10030051014, A14030051014
- ASCE 7-10 & ASCE 7-16 Gable Detail Drawings
A11515ENC100118, A12015ENC100118, A14015ENC100118, A16015ENC100118,
A18015ENC100118, A20015ENC100118, A20015END100118, A20015PED100118,
A11530ENC100118, A12030ENC100118, A14030ENC100118, A16030ENC100118,
A18030ENC100118, A20030ENC100118, A20030END100118, A20030PED100118,
S11515ENC100118, S12015ENC100118, S14015ENC100118, S16015ENC100118,
S18015ENC100118, S20015ENC100118, S20015END100118, S20015PED100118,
S11530ENC100118, S12030ENC100118, S14030ENC100118, S16030ENC100118,
S18030ENC100118, S20030ENC100118, S20030END100118, S20030PED100118

See appropriate Alpine gable detail for maximum unreinforced gable vertical length.

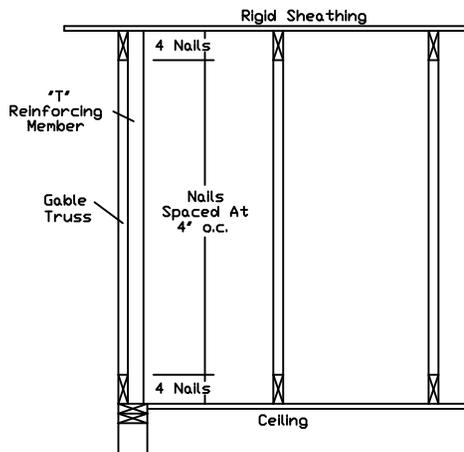
To convert from 'L' to 'T' reinforcing members, multiply 'T' increase by length (based on appropriate Alpine gable detail).

Maximum allowable 'T' reinforced gable vertical length is 14' from top to bottom chord.
'T' reinforcing member material must match size, specie, and grade of the 'L' reinforcing member.

Web Length Increase w/ 'T' Brace

'T' Reinf. Mbr. Size	'T' Increase
2x4	30 %
2x6	20 %

Example:
ASCE 7-10 Wind Speed = 120 mph
Mean Roof Height = 30 ft, Kzt = 1.00
Gable Vertical = 24' o.c. SP #3
'T' Reinforcing Member Size = 2x4
'T' Brace Increase (From Above) = 30% = 1.30
(1) 2x4 'L' Brace Length = 8' 7"
Maximum 'T' Reinforced Gable Vertical Length
1.30 x 8' 7" = 11' 2"



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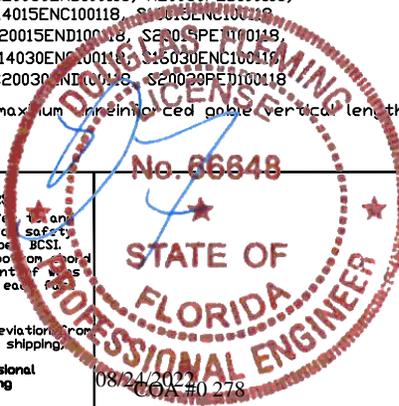
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025



REF LET-IN VERT

DATE 01/02/2018

DRWG GBLLETIN0118

MAX. TOT. LD. 60 PSF

DUR. FAC. ANY

MAX. SPACING 24'0"

Gable Stud Reinforcement Detail

ASCE 7-16: 140 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

- Or: 120 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00
- Or: 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00
- Or: 100 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

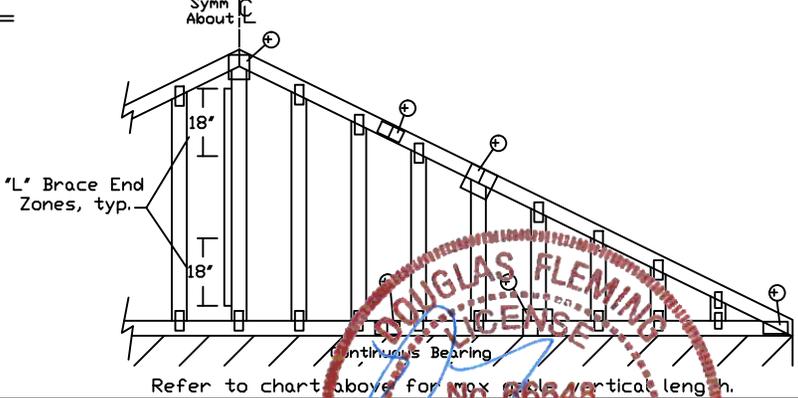
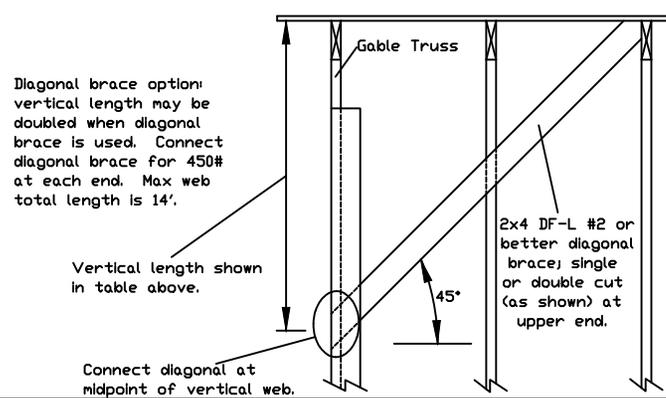
Max Gable Vertical Length	2x4 Gable Vertical Spacing		Brace Grade	No Braces	(1) 1x4 'L' Brace *		(1) 2x4 'L' Brace *		(2) 2x4 'L' Brace **		(1) 2x6 'L' Brace *		(2) 2x6 'L' Brace **		
	Species	Grade			Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	
	24" o.c.	SPF	#1 / #2	HF	#1 / #2	4' 3"	7' 3"	7' 7"	8' 7"	8' 11"	10' 3"	10' 8"	13' 6"	14' 0"	14' 0"
#3					4' 1"	6' 7"	7' 1"	8' 6"	8' 10"	10' 1"	10' 6"	13' 4"	13' 10"	14' 0"	14' 0"
Stud					4' 1"	6' 7"	7' 0"	8' 6"	8' 10"	10' 1"	10' 6"	13' 4"	13' 10"	14' 0"	14' 0"
Standard			#1	4' 1"	5' 8"	6' 0"	7' 7"	8' 1"	10' 1"	10' 6"	11' 10"	12' 8"	14' 0"	14' 0"	
			#2	4' 6"	7' 4"	7' 8"	8' 8"	9' 0"	10' 4"	10' 9"	13' 8"	14' 0"	14' 0"	14' 0"	
			#3	4' 3"	7' 3"	7' 7"	8' 7"	8' 11"	10' 3"	10' 8"	13' 6"	14' 0"	14' 0"	14' 0"	
SP		DFL	#1	4' 2"	6' 0"	6' 4"	7' 11"	8' 6"	10' 2"	10' 7"	12' 5"	13' 4"	14' 0"	14' 0"	
			#2	4' 2"	6' 0"	6' 4"	7' 11"	8' 6"	10' 2"	10' 7"	12' 5"	13' 4"	14' 0"	14' 0"	
			Standard	4' 0"	5' 3"	5' 7"	7' 0"	7' 6"	9' 6"	10' 2"	11' 0"	11' 10"	14' 0"	14' 0"	
		SPF	#1 / #2	4' 11"	8' 4"	8' 8"	9' 10"	10' 3"	11' 8"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"	
			#3	4' 8"	8' 1"	8' 8"	9' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	
			Stud	4' 8"	8' 1"	8' 6"	9' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	
SP	DFL	#1	4' 9"	7' 4"	7' 9"	9' 9"	10' 2"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"		
		#2	4' 11"	8' 4"	8' 8"	9' 10"	10' 3"	11' 8"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"		
		Standard	4' 9"	7' 4"	7' 9"	9' 9"	10' 2"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"		
	SPF	#1 / #2	4' 8"	6' 5"	6' 10"	8' 7"	9' 2"	11' 7"	12' 1"	13' 6"	14' 0"	14' 0"	14' 0"		
		#3	5' 1"	8' 5"	8' 9"	9' 11"	10' 4"	11' 10"	12' 4"	14' 0"	14' 0"	14' 0"	14' 0"		
		Stud	4' 9"	7' 4"	7' 9"	9' 9"	10' 2"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"		
12" o.c.	SPF	HF	#1 / #2	5' 5"	9' 2"	9' 6"	10' 10"	11' 3"	11' 8"	13' 5"	14' 0"	14' 0"	14' 0"		
			#3	5' 1"	9' 0"	9' 4"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"		
			Stud	5' 1"	9' 0"	9' 4"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"		
	Standard	#1	5' 1"	8' 0"	8' 6"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"		
		#2	5' 8"	9' 3"	9' 8"	10' 11"	11' 4"	13' 0"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"		
		#3	5' 5"	9' 2"	9' 6"	10' 10"	11' 3"	12' 11"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"		
SP	DFL	#1	5' 3"	8' 5"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"			
		#2	5' 3"	8' 5"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"			
		Standard	5' 1"	7' 5"	7' 11"	9' 11"	10' 7"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"			

Bracing Group Species and Grades:

Group A:			
Spruce-Pine-Fir		Hem-Fir	
#1 / #2	Standard	#2	Stud
#3	Stud	#3	Standard
Douglas Fir-Larch		Southern Pine***	
#3	Stud	#3	Stud
#3	Standard	#3	Standard
Group B:			
Hem-Fir			
#1 & Btr			
#1			
Douglas Fir-Larch		Southern Pine***	
#1	Stud	#1	Stud
#2	Stud	#2	Stud

1x4 Braces shall be SRB (Stress-Rated Board).
 ***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:
 Wind Load deflection criterion is L/240.
 Provide uplift connections for 55 plf over continuous bearing (5 psf TC Dead Load).
 Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12' plywood overhang.



Attach 'L' braces with 10d (0.128"x3.0" min) nails.
 * For (1) 'L' brace: space nails at 2' o.c. in 18' end zones and 4' o.c. between zones.
 ** For (2) 'L' braces: space nails at 3' o.c. in 18' end zones and 6' o.c. between zones.
 'L' bracing must be a minimum of 80% of web member length.

Gable Vertical Plate Sizes	
Vertical Length	No Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4' 0"	3X4

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.

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08/24/2024 10:28

MAX. TOT. LD. 60 PSF
MAX. SPACING 24.0"

REF	ASCE7-16-GAB14015
DATE	01/26/2018
DRWG	A14015ENC160118

NAIL SPACING DETAIL

MINIMUM SPACING FOR SINGLE BLOCK IS SHOWN. DOUBLE NAIL SPACINGS AND STAGGER NAILING FOR TWO BLOCKS. GREATER SPACING MAY BE REQUIRED TO AVOID SPLITTING.

BLOCK LOCATION, SIZE, LENGTH, GRADE AND TOTAL NUMBER AND TYPE OF NAILS ARE TO BE SPECIFIED ON SEALED DESIGN REFERENCING THIS DETAIL.

LOAD PERPENDICULAR TO GRAIN

- A - EDGE DISTANCE AND SPACING BETWEEN STAGGERED ROWS OF NAILS (6 NAIL DIAMETERS)
- B - SPACING OF NAILS IN A ROW (12 NAIL DIAMETERS)
- C - END DISTANCE (15 NAIL DIAMETERS)

LOAD PARALLEL TO GRAIN

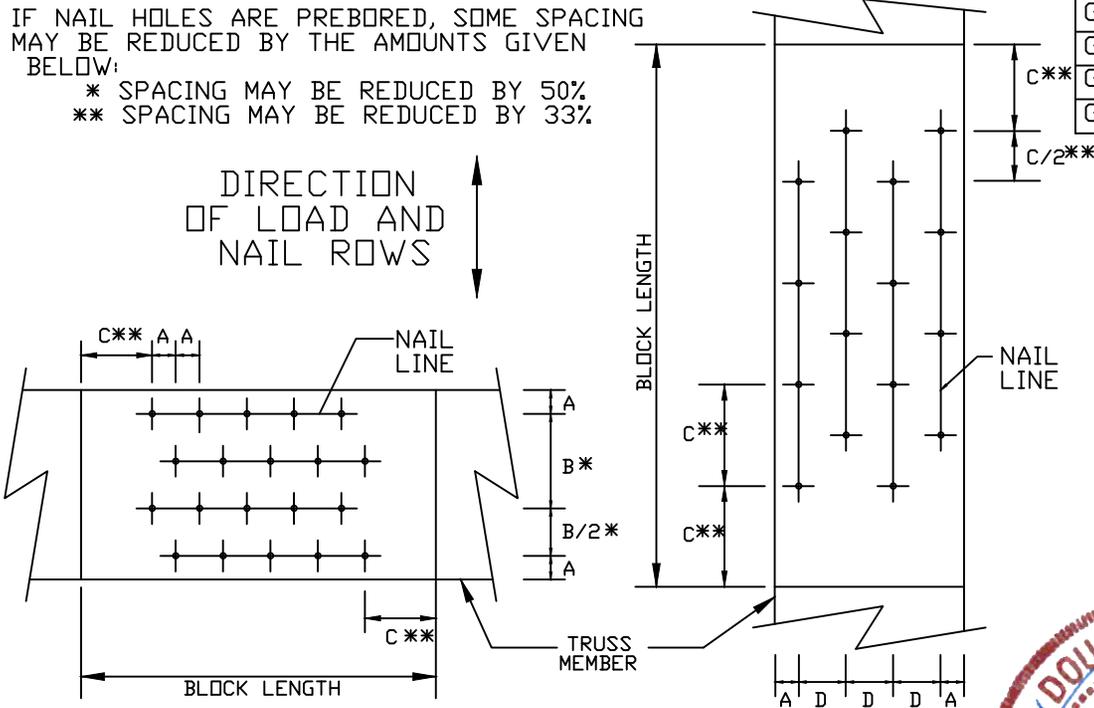
- A - EDGE DISTANCE (6 NAIL DIAMETERS)
- C - SPACING OF NAILS IN A ROW AND END DISTANCE (15 NAIL DIAMETERS)
- D - SPACING BETWEEN STAGGERED ROWS OF NAILS (7 1/2 NAIL DIAMETERS)

IF NAIL HOLES ARE PREBORED, SOME SPACING MAY BE REDUCED BY THE AMOUNTS GIVEN BELOW:

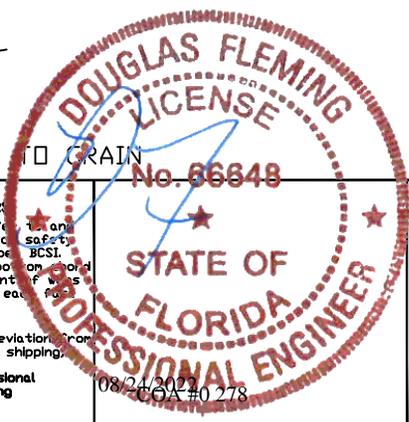
- * SPACING MAY BE REDUCED BY 50%
- ** SPACING MAY BE REDUCED BY 33%

MINIMUM NAIL SPACING DISTANCES

NAIL TYPE	DISTANCES			
	A	B*	C**	D
8d BOX (0.113"X 2.5",MIN)	3/4"	1 3/8"	1 3/4"	7/8"
10d BOX (0.128"X 3",MIN)	7/8"	1 5/8"	2"	1"
12d BOX (0.128"X 3.25",MIN)	7/8"	1 5/8"	2"	1"
16d BOX (0.135"X 3.5",MIN)	7/8"	1 5/8"	2 1/8"	1 1/8"
20d BOX (0.148"X 4",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
8d COMMON (0.131"X 2.5",MIN)	7/8"	1 5/8"	2"	1"
10d COMMON (0.148"X 3",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
12d COMMON (0.148"X 3.25",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
16d COMMON (0.162"X 3.5",MIN)	1"	2"	2 1/2"	1 1/4"
GUN (0.120"X 2.5",MIN)	3/4"	1 1/2"	1 7/8"	1"
GUN (0.131"X 2.5",MIN)	7/8"	1 5/8"	2"	1"
GUN (0.120"X 3",MIN)	3/4"	1 1/2"	1 7/8"	1"
GUN (0.131"X 3",MIN)	7/8"	1 5/8"	2"	1"



LOAD APPLIED PERPENDICULAR TO GRAIN LOAD APPLIED PARALLEL TO GRAIN



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REF	NAIL SPACE
DATE	10/01/14
DRWG	CNNAILSP1014

Commentary: Deflection and Camber

Camber may be built into trusses to compensate for the vertical deflection that results from the application of loads. Providing camber has the following advantages:

- Helps to ensure level ceilings and floors after dead loads are applied.
- Facilitates drainage to avoid ponding on flat or low slope roofs.
- Compensates for different deflection characteristics between adjacent trusses.
- Improves appearance of garage door headers and other long spans that can appear to "sag."
- Avoids "dips" in roof ridgelines at the transition from the gable to adjacent clear span trusses.

In accordance with ANSI/TPI 1 the Building Designer, through the Construction Documents, shall provide the location, direction, and magnitude of all loads attributable to ponding that may occur due to the design of the roof drainage system. The Building Designer shall also specify any dead load, live load, and in-service creep deflection criteria for flat or low-slope roofs subject to ponding loads.

The amount of camber is dependent on the truss type, span, loading, application, etceteras.

More restrictive limits for allowable deflection and slenderness ratio (L/D) may be required to help control vibration.

The following tables are provided as guidelines for limiting deflection and estimating camber. Conditions or codes may exist that require exceeding these recommendations, or past experience may warrant using more stringent limitations.

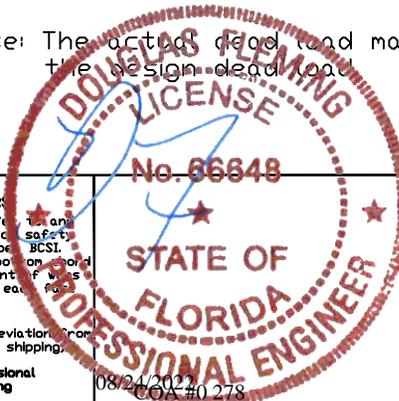
L = Span of Truss (inches)
D = Depth of Truss at Deflection Point (inches)

Recommended Truss Deflection Limits

Truss Type	L/D	Deflection Limits	
		Live Load	Total Load
Pitched Roof Trusses	24	L/240 (vertical)	L/180 (vertical)
Floor of Room-In-Attic Trusses	24	L/360 (vertical)	L/240 (vertical)
Flat or Shallow Pitched Roof Trusses	24	L/360 (vertical)	L/240 (vertical)
Residential Floor Trusses	24	L/360 (vertical)	L/240 (vertical)
Commercial Floor Trusses	20	L/480 (vertical)	L/240 (vertical)
Scissors Trusses	24	0.75" (horizontal)	1.25" (horizontal)

Truss Type	Recommended Camber
Pitched Trusses	1.00 x Deflection from Actual Dead Load
Sloping Parallel Chord Trusses	1.5 x Vertical Deflection from Actual Dead Load
Floor Trusses	(0.25 x Deflection from Live Load) + Actual Dead Load
Flat Roof Trusses	(0.25 x Deflection from Live Load) + (1.5 x Design Dead Load Deflection)

Note: The actual dead load may be considerably less than the design dead load.



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REF	DEFLEC/CAMB
DATE	10/01/14
DRWG	DEFLCAMB1014

Piggyback Detail - ASCE 7-16: 160 mph, 30' Mean Height, Enclosed, Exposure C, Kzt=1.00

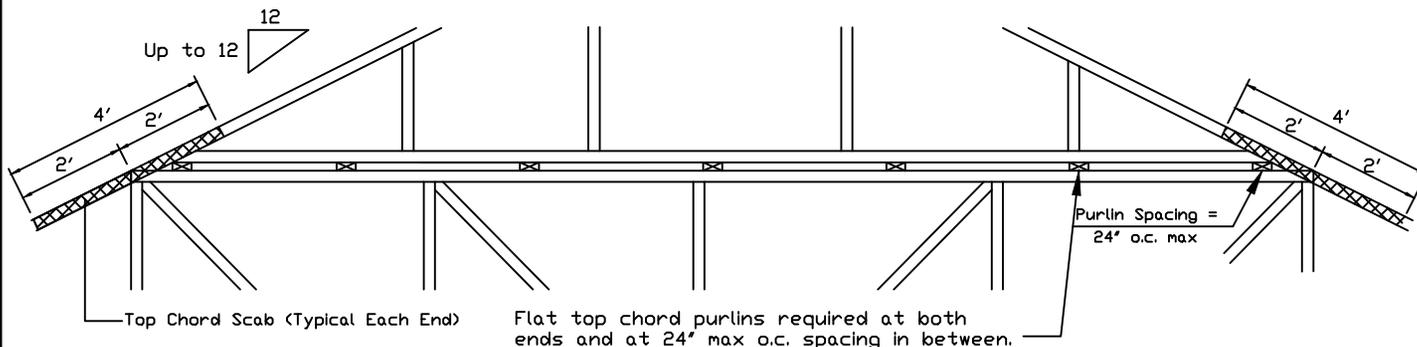
160 mph Wind, 30.00 ft Mean Hgt, ASCE 7-16, Enclosed Bldg. located anywhere in roof, Exp C, Wind DL= 5.0 psf (min), Kzt=1.0.
 Or 140 mph wind, 30.00 ft Mean Hgt, ASCE 7-16, Enclosed Bldg. located anywhere in roof, Exp D, wind DL= 5.0 psf (min), Kzt=1.0.

Note: Top chords of trusses supporting piggyback cap trusses must be adequately braced by sheathing or purlins. The building Engineer of Record shall provide diagonal bracing or any other suitable anchorage to permanently restrain purlins, and lateral bracing for out of plane loads over gable ends.

Maximum truss spacing is 24' o.c. detail is not applicable if cap supports additional loads such as cupola, steeple, chimney or drag strut loads.

** Refer to Engineer's sealed truss design drawing for piggyback and base truss specifications.

Detail A : Purlin Spacing = 24" o.c. or less

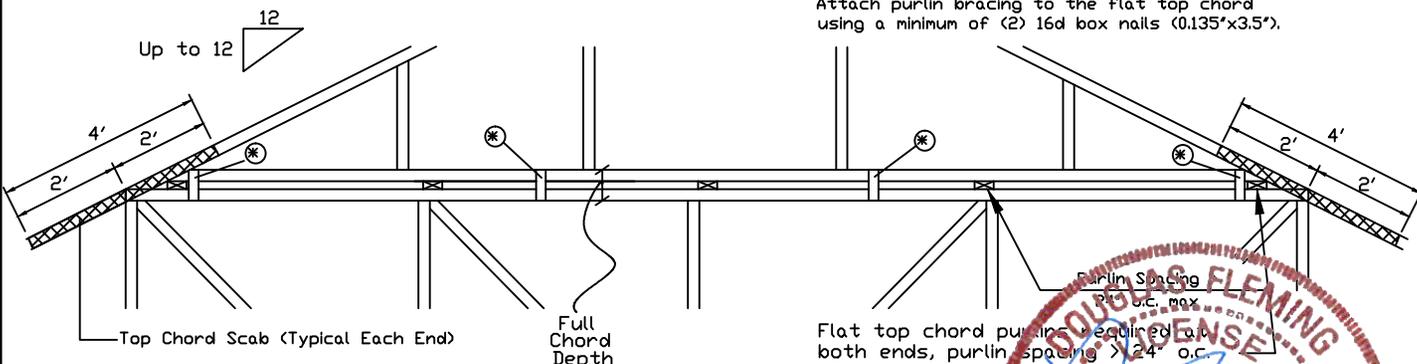


Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

Attach purlin bracing to the flat top chord using (2) 16d box nails (0.135"x3.5").

The top chord #3 grade 2x4 scab may be replaced with either of the following: (1) 3X8 Trulox plate attached with (8) 0.120"x1.375" nails, (4) into cap TC & (4) into base truss TC or (1) 28PB wave piggyback plate plated to the piggyback truss TC and attached to the base truss TC with (4) 0.120"x1.375" nails. Note: Nailing thru holes of wave plate is acceptable.

Detail B : Purlin Spacing > 24" o.c.



Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

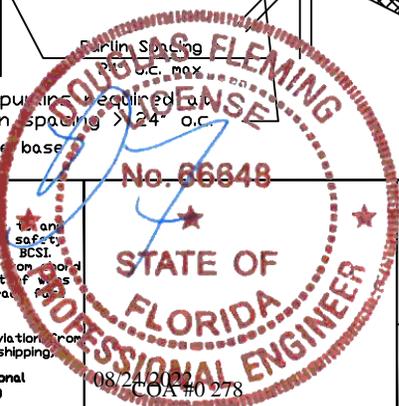
Attach purlin bracing to the flat top chord using a minimum of (2) 16d box nails (0.135"x3.5").

Note: If purlins or sheathing are not specified on the flat top of the base truss, purlins must be installed at 24' o.c. max. and use Detail A

* In addition, provide connection with one of the following methods:
Trulox Use 3X8 Trulox plates for 2x4 chord member, and 3X10 Trulox plates for 2x6 and larger chord members. Attach to each face @ 8' o.c. with (4) 0.120"x1.375" nails into cap bottom chord and (4) in base truss top chord. Trulox plates may be staggered 4' o.c. front to back faces.
APA Rated Gusset 8"x8"x7/16" (min) APA rated sheathing gussets (each face). Attach @ 8' o.c. with (8) 6d common (0.113"x2") nails per gusset, (4) in cap bottom chord and (4) in base truss top chord. Gussets may be staggered 4' o.c. front to back faces.
2x4 Vertical Scabs 2x4 SPF #2, full chord depth scabs (each face). Attach @ 8' o.c. with (6) 10d box nails (0.128"x3") per scab, (3) in cap bottom chord and (3) in base truss top chord. Scabs may be staggered 4' o.c. front to back faces.
28PB Wave Piggyback Plate One 28PB wave piggyback plate to each face @ 8' o.c. Attach teeth to piggyback at time of fabrication. Attach to supporting truss with (4) 0.120"x1.375" nails per face per ply. Piggyback plates may be staggered 4' o.c. front to back faces.

155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

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REF	PIGGYBACK
DATE	01/02/2018
DRWG	PB160160118
SPACING	24.0"

Valley Detail - ASCE 7-16: 180 mph, 30' Mean Height, Partially Enclosed, Exp. C, Kzt=1.00

Top Chord 2x4 SP #2N, SPF #1/#2, DF-L #2 or better.
 Bot Chord 2x4 SP #2N or SPF #1/#2 or better.
 Webs 2x4 SP #3, SPF #1/#2, DF-L #2 or better.

** Attach each valley to every supporting truss with:
 535# connection or with (1) Simpson H2.5A or equivalent connector for
 ASCE 7-16 180 mph. 30' Mean Height, Part. Enc. Building, Exp. C, Wind TC DL=5 psf, Kzt = 1.00
 Or
 ASCE 7-16 160 mph. 30' Mean Height, Part. Enc. Building, Exp. D, Wind TC DL=5 psf, Kzt = 1.00

Bottom chord may be square or pitched cut as shown.

Valleys short enough to be cut as solid triangular members from a single 2x6, or larger as required, shall be permitted in lieu of fabricating from separate 2x4 members.

All plates shown are Alpine Wave Plates.

Unless specified otherwise on engineer's sealed design, for vertical valley webs taller than 7-9" apply 2x4 "T" reinforcement, 80% length of web, same species and grade or better, attached with 10d box (0.128" x 3.0") nails at 6" o.c. In lieu of "T" reinforcement, 2x4 Continuous Lateral Restraint applied at mid-length of web is permitted with diagonal bracing as shown in DRWG BRCLBANC1014.

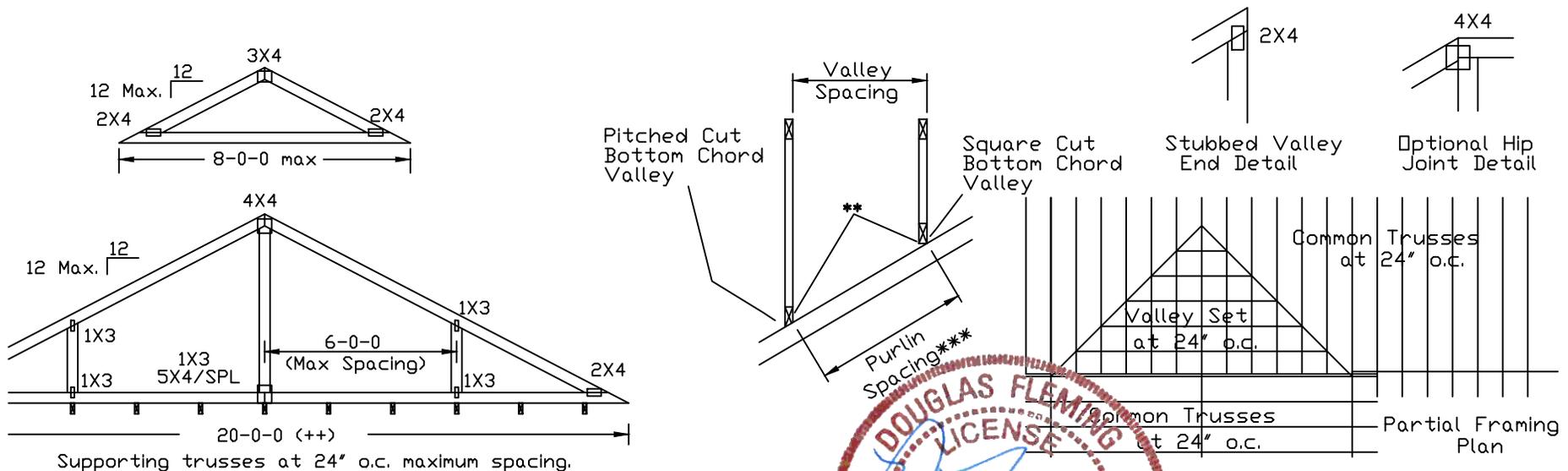
Top chord of truss beneath valley set must be braced with properly attached, rated sheathing applied prior to valley truss installation.

Or
 Purlins at 24" o.c. or as otherwise specified on engineer's sealed design

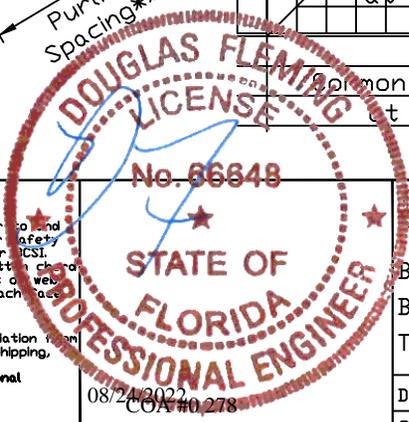
Or
 By valley trusses used in lieu of purlin spacing as specified on Engineer's sealed design.

*** Note that the purlin spacing for bracing the top chord of the truss beneath the valley is measured along the slope of the top chord.

++ Larger spans may be built as long as the vertical height does not exceed 14'-0".



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BC LL	30	30	40PSF	REF	VALLEY DETAIL
C DL	20	15	7PSF	DATE	01/26/2018
BC DL	10	10	10 PSF	DRWG	VAL180160118
BC LL	0	0	0PSF		
TOT. LD.	60	55	57PSF		
DUR.FAC.	1.25/1.33	1.15	1.15		
SPACING	24.0"				

Valley Detail - ASCE 7-16: 30' Mean Height, Enclosed, Exp. C, Kzt=1.00

Top Chord 2x4 SP #2N, SPF #1/#2, DF-L #2 or better.
 Bot Chord 2x4 SP #2N or SPF #1/#2 or better.
 Webs 2x4 SP #3, SPF #1/#2, DF-L #2 or better.

** Attach each valley to every supporting truss with:
 (2) 16d box (0.135" x 3.5") nails toe-nailed for
 ASCE 7-16, 30' Mean Height, Enclosed Building, Exp. C,
 Wind TC DL=5 psf, Kzt = 1.00, Max. Wind Speed based on
 supporting truss material at connection location:
 170 mph for SP (G = 0.55, min.),
 155 mph for DF-L (G = 0.50, min.), or
 120 mph for HF & SPF (G = 0.42, min.).

Maximum top chord pitch is 10/12 for supporting trusses
 below valley trusses.

Bottom chord of valley trusses may be square or
 pitched cut as shown.

Valleys short enough to be cut as solid triangular
 members from a single 2x6, or larger as required,
 shall be permitted in lieu of fabricating from
 separate 2x4 members.

All plates shown are Alpine Wave Plates.

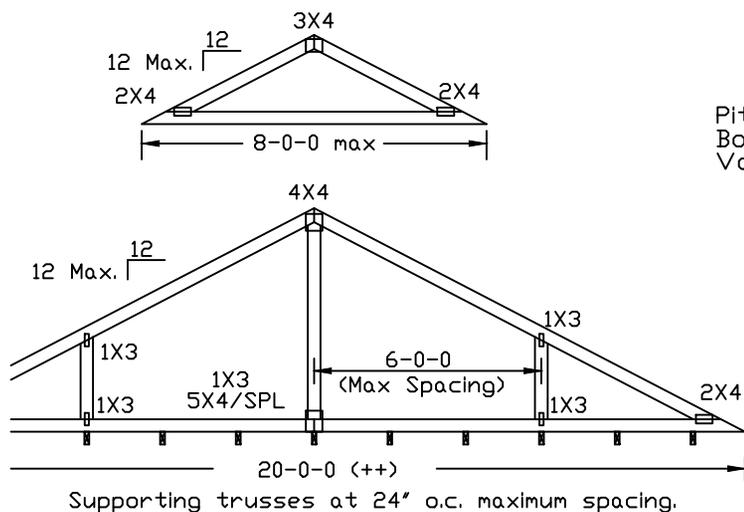
Unless specified otherwise on engineer's sealed design, for vertical
 valley webs taller than 7-9" apply 2x4 "T" reinforcement, 80% length of
 web, same species and grade or better, attached with 10d box
 (0.128" x 3.0") nails at 6" o.c. In lieu of "T" reinforcement, 2x4 Continuous
 Lateral Restraint applied at mid-length of web is permitted with diagonal
 bracing as shown in DRWG BRCLBANC1014.

Top chord of truss beneath valley set must be braced with:
 properly attached, rated sheathing applied prior to valley truss
 installation.

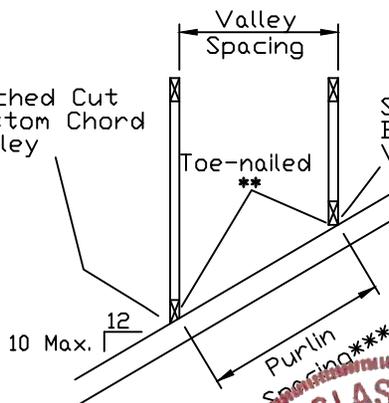
- Or
- Purlins at 24" o.c. or as otherwise specified on engineer's sealed design
- Or
- By valley trusses used in lieu of purlin spacing as specified on
 Engineer's sealed design.

*** Note that the purlin spacing for bracing the top chord of the truss
 beneath the valley is measured along the slope of the top chord.

++ Larger spans may be built as long as the vertical height does
 not exceed 14'-0".



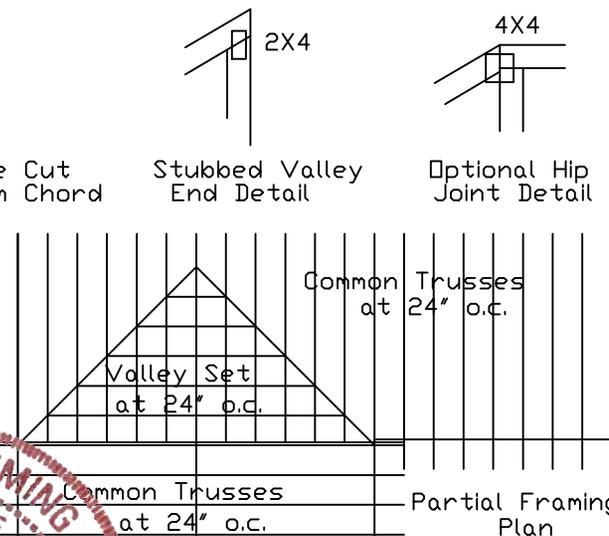
Pitched Cut
 Bottom Chord
 Valley



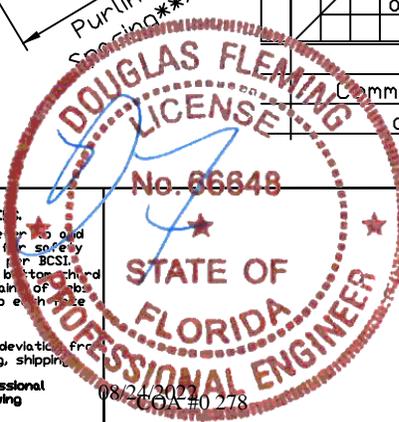
Square Cut
 Bottom Chord
 Valley

Stubbed Valley
 End Detail

Optional Hip
 Joint Detail



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TC LL	30	30	40PSF	REF	VALLEY DETAIL
TC DL	20	15	7 PSF	DATE	01/26/2018
BC DL	10	10	10 PSF	DRWG	VALTN160118
BC LL	0	0	0 PSF		
TOT. LD.	60	55	57PSF		
DUR.FAC.	1.25/1.33	1.15	1.15		
SPACING	24.0"				