



REVISIONS	

SOFTPLAN  
ARCHITECTURAL DESIGN SOFTWARE



FRONT ELEVATION  
SCALE: 1/4" = 1'-0"

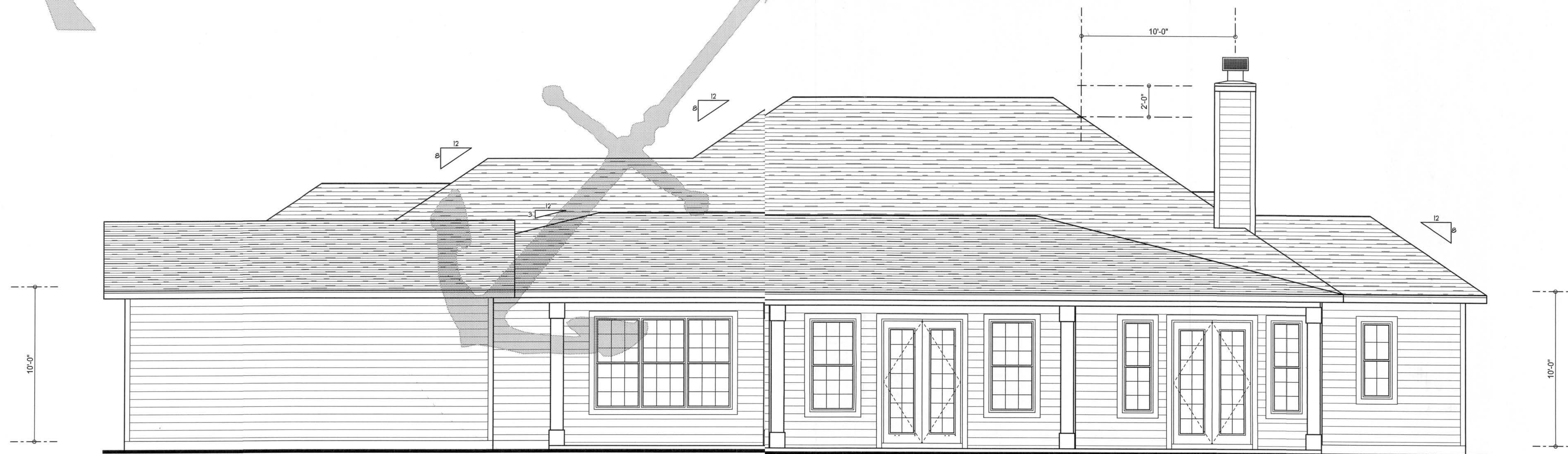
**REQUIRED ROOF VENTILATION:**  
AS PER FLORIDA BUILDING CODE 2309.7

RIDGE VENT  
MIN. 50% TOTAL VENT AREA  
LOCATED IN THE UPPER PORTION OF ATTIC (MIN. 3" ABOVE EAVE)  
4355 S.F. / 300 x 50% = 7.25 S.F. RIDGE VENT AREA REQUIRED  
65.98 FEET OF RIDGE VENT REQUIRED

SOFFIT VENT  
4355 S.F. / 300 x 50% = 7.25 S.F. SOFFIT VENT AREA REQUIRED  
241.66 FEET OF SOFFIT VENT REQUIRED

BUILDER MUST VERIFY THE FOLLOWING MINIMUM NET FREE VENT AREAS:

1. RIDGE VENTS = 16 IN2/FT (.11 FT2/FT)
2. OFF-RIDGE VENTS = .70 FT2 PER 4" UNIT
3. SOFFIT VENTS = 4.3 IN2/FT (.03 FT2/FT)



REAR ELEVATION  
SCALE: 1/4" = 1'-0"

WINDLOAD ENGINEER: Mark Disoway,  
P.E. NC53915, POB 868, Lake City, FL  
32056 386-754-5419

DIMENSIONS:  
State dimensions supersede scaled  
dimensions. Refer all questions to  
Mark Disoway, P.E. for resolution.  
Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have  
examined this plan, and that the applicable  
portion of the plan, relating to wind engineering  
comply with section R301.2.1, Florida building  
code residential 2004, to the best of my  
knowledge.

LIMITATION: This design is valid for one  
building, at specified location.

MARK DISOWAY  
P.E. 53915

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P.E. 53915  
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Richard & Xene  
Nelson Residence

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PRINTED DATE:  
July 16, 2007

DRAWN BY: Daid Disoway CHECKED BY:

FINALS DATE:  
16/ Jun / 07

JOB NUMBER:  
705081

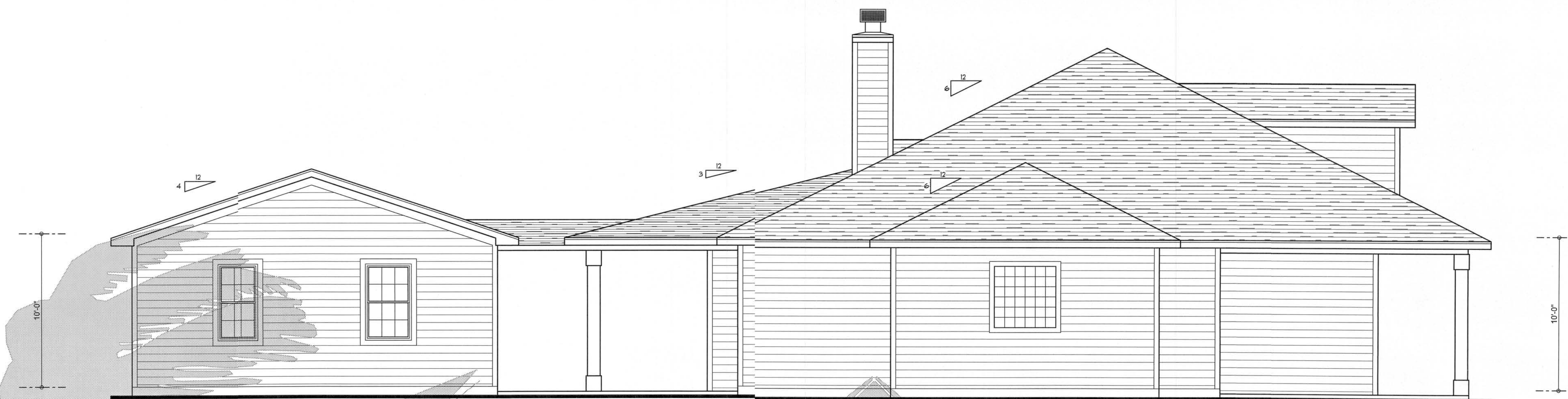
DRAWING NUMBER

**A-1**  
OF 7 SHEETS

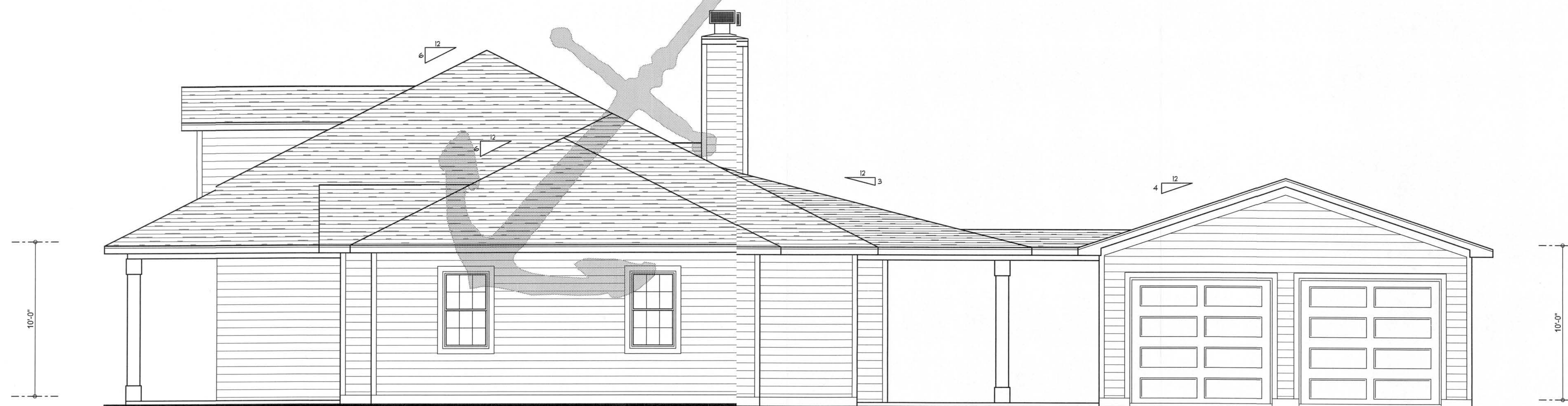


REVISIONS	

**SOFTPLAN**  
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**LEFT ELEVATION**  
SCALE: 1/4" = 1'-0"



**RIGHT ELEVATION**  
SCALE: 1/4" = 1'-0"

**WIND ANCHOR**

WINDLOAD ENGINEER: Mark Disosway,  
P.E. No. 53915, POB 868, Lake City, FL  
32056, 386-754-5419

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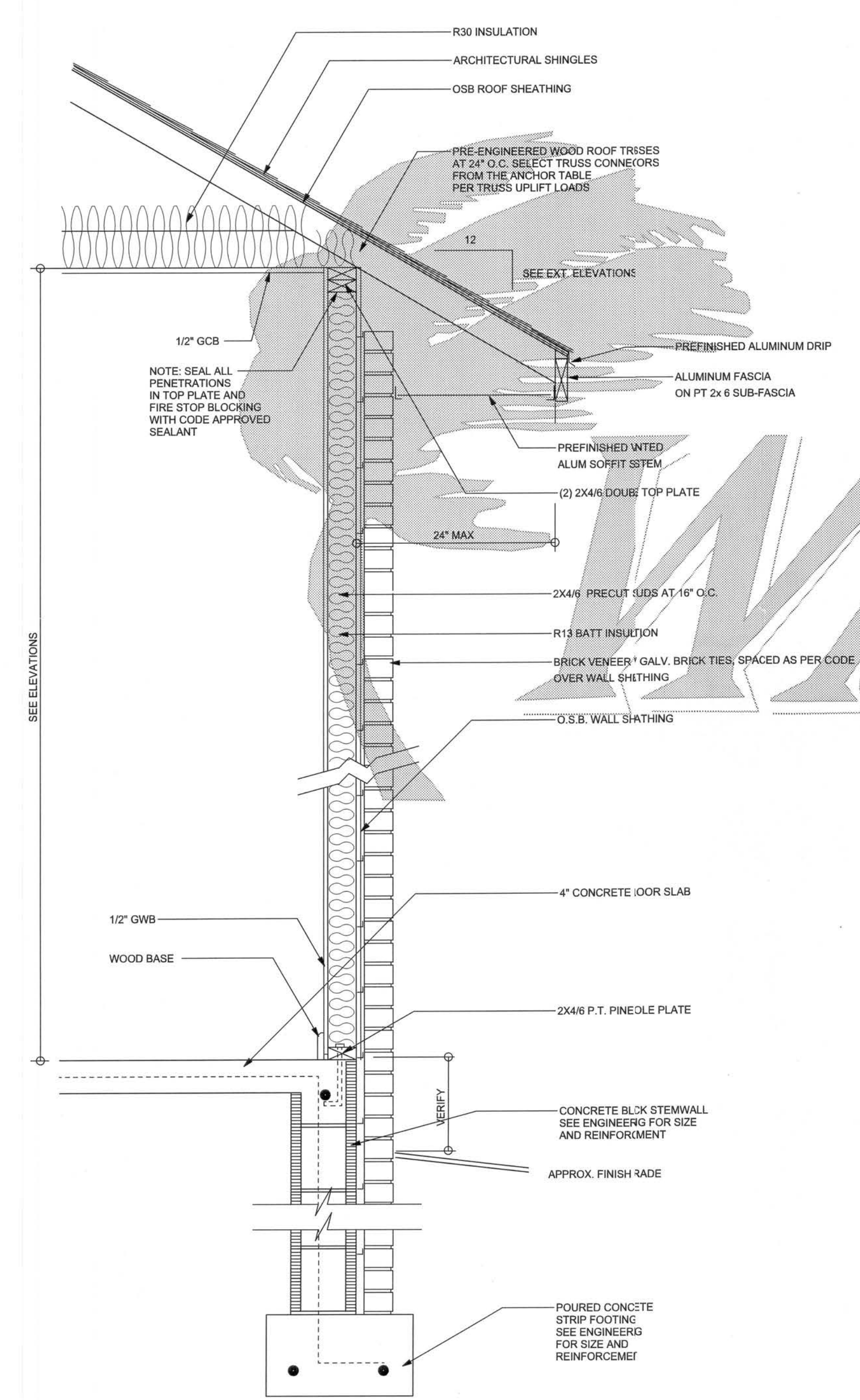
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OF 7 SHEETS

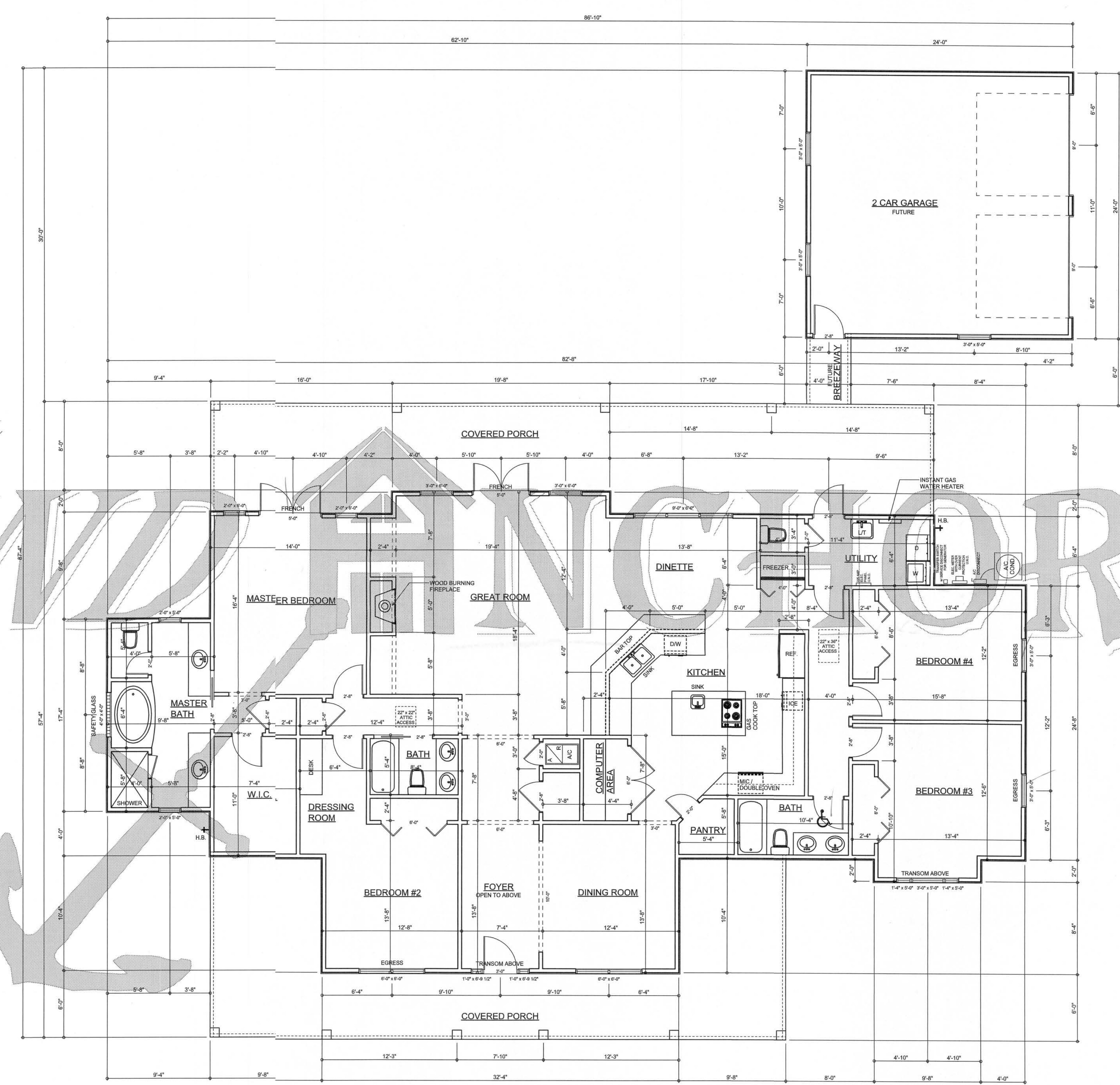


REVISIONS	

**SOFTPLAN**  
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**TYPICAL DESIGN WALL SECTION**  
**NON - STRUCTURAL DATA**  
SCALE: 1\"/>



**FLOOR PLAN**  
SCALE: 3/16\"/>

**AREA SUMMARY**

LIVING AREA	2775	S . F .
PORCH AREA	1120	S . F .
FUTURE GARAGE AREA	576	S . F .
FUTURE BREEZEWAY AREA	24	S . F .
TOTAL AREA	4495	S . F .

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7/16/07  
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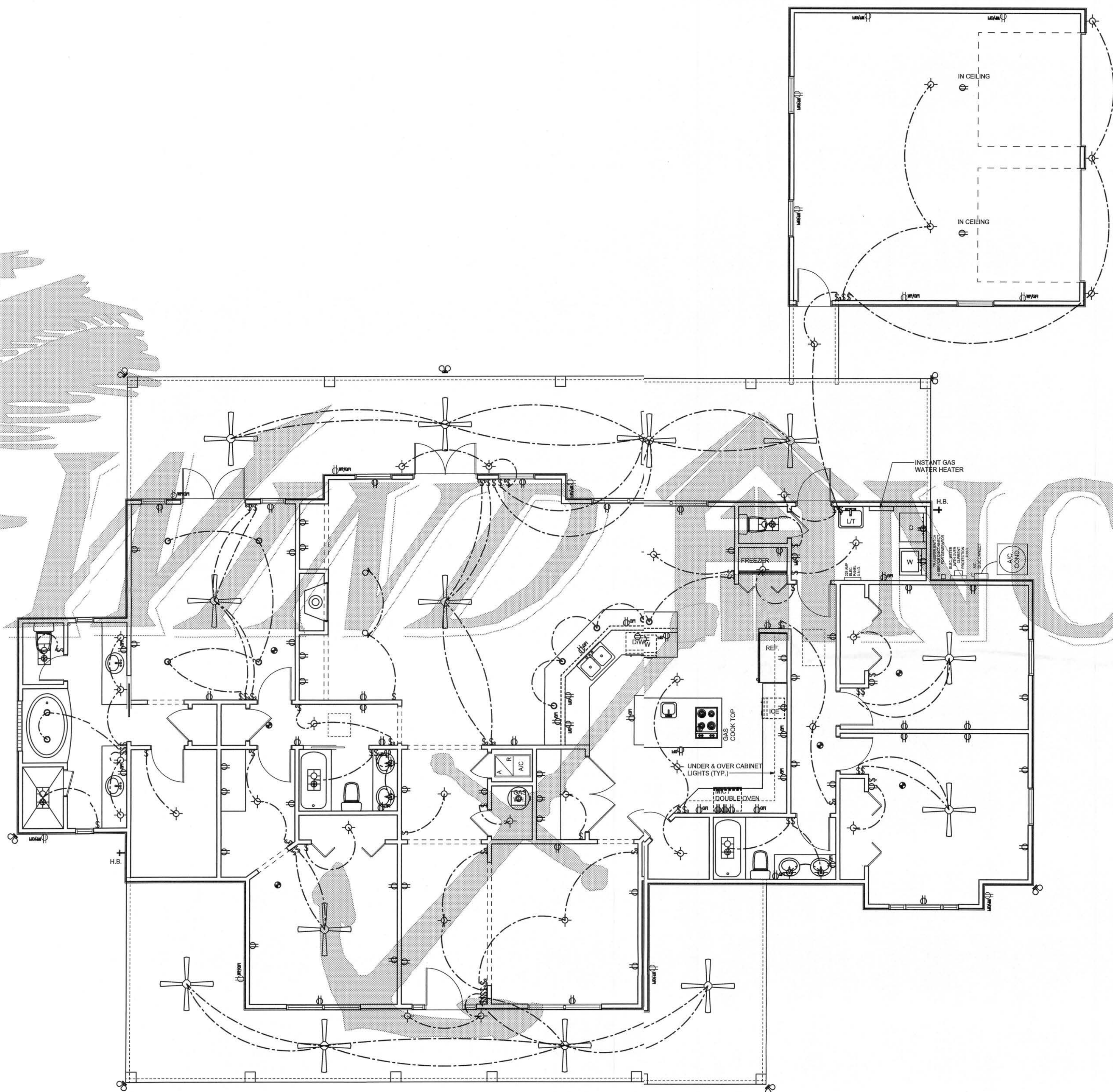
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**DRAWING NUMBER**  
**A-3**  
OF 7 SHEETS



## REVISIONS

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## ELECTRICAL PLAN NOTES

- E-1 WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.
- E-2 CONSULT THE OWNER FOR THE NUMBER OF SEPARATE TELEPHONE LINES TO BE INSTALLED.
- E-3 ALL INSTALLATIONS SHALL BE PER NAT'L. ELECTRIC CODE.
- E-4 ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.
- E-5 TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION.
- E-6 ELECTRICAL CONTR' SHALL BE RESPONSIBLE FOR THE DESIGN & SIZING OF ELECTRICAL SERVICE AND CIRCUITS.
- E-7 ENTRY OF SERVICE (UNDERGROUND OR OVERHEAD) TO BE DETERMINED BY POWER COMPANY.
- E-8 ALL BEDROOM RECEPTACLES SHALL BE AFCI (ARC FAULT CIRCUIT INTERRUPT).
- E-9 ALL OUTLETS TO BE LOCATED ABOVE BASE FLOOD ELEVATION.
- E-10 A SERVICE DISCONNECT WITH OVER CURRENT PROTECTION SHALL BE INSTALLED OUTSIDE OF THE BUILDING, ON THE LOAD SIDE OF THE METER, AT THE PLACE ELECTRIC SERVICE ENTRANCE CONDUCTORS MAY NOT BE LOCATED INSIDE OF THE OF THE BUILDING WITHOUT SPECIAL APPROVAL OF THE BUILDING OFFICIAL.

## ELECTRICAL LEGEND

	CEILING FAN (PREWIRE FOR LIGHT KIT)
	DOUBLE SECURITY LIGHT
	2X4 FLUORESCENT LIGHT FIXTURE
	RECESSED CAN LIGHT
	BATH EXHAUST FAN WITH LIGHT
	BATH EXHAUST FAN
	LIGHT FIXTURE
	DUPLEX OUTLET
	220v OUTLET
	GFI DUPLEX OUTLET
	SMOKE DETECTOR
	WALL SWITCH
	3 WAY WALL SWITCH
	4 WAY WALL SWITCH
	WATER PROOF GFI OUTLET
	PHONE JACK
	TELEVISION JACK
	GARAGE DOOR OPENER
	WALL HEATER

ELECTRICAL PAN  
SCALE: 3/16" = 1'-0"

WINDLOAD ENGINEER: Mark Discoway,  
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32056, 386-754-5419

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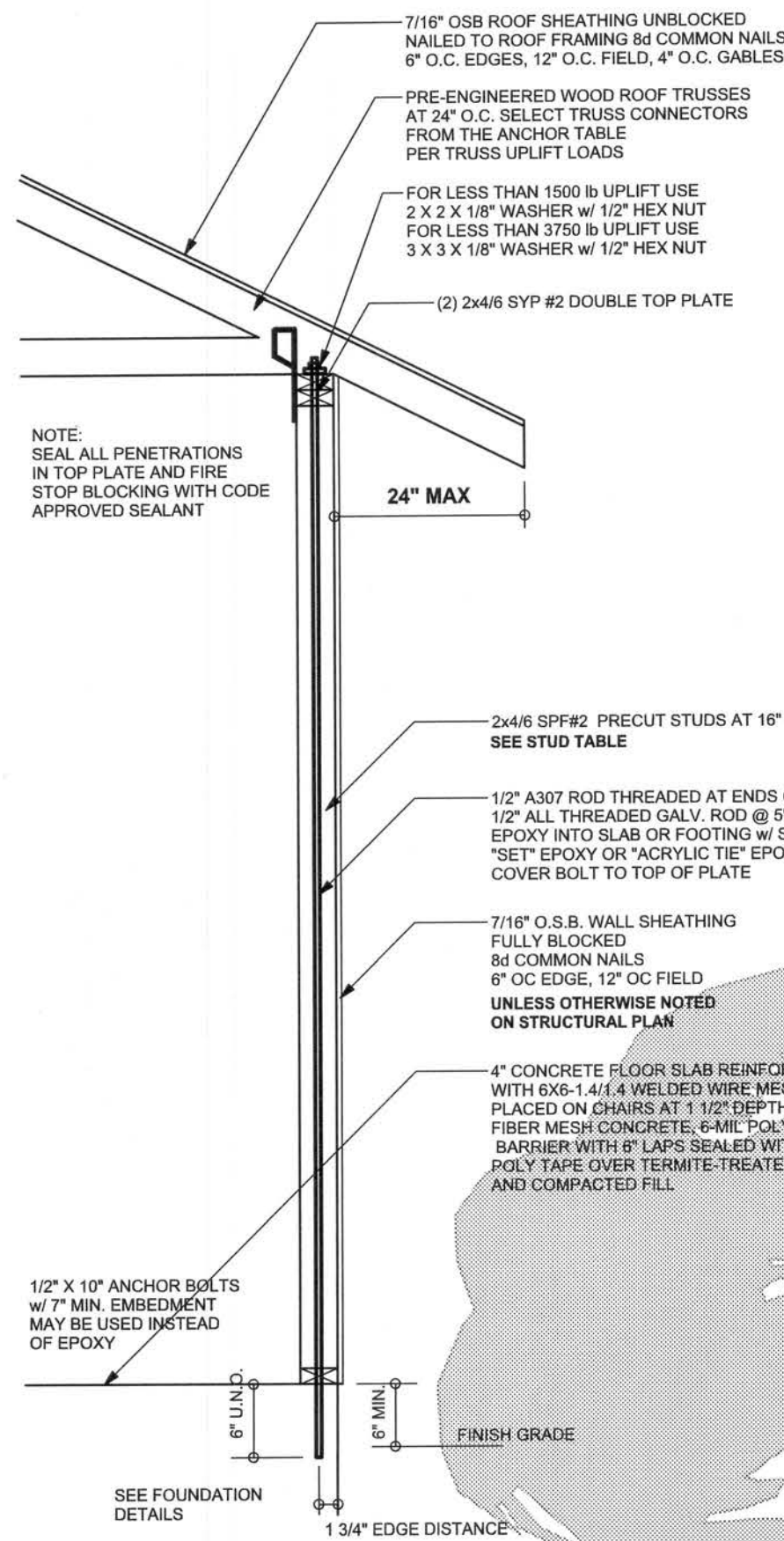
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A-4

OF 7 SHEETS



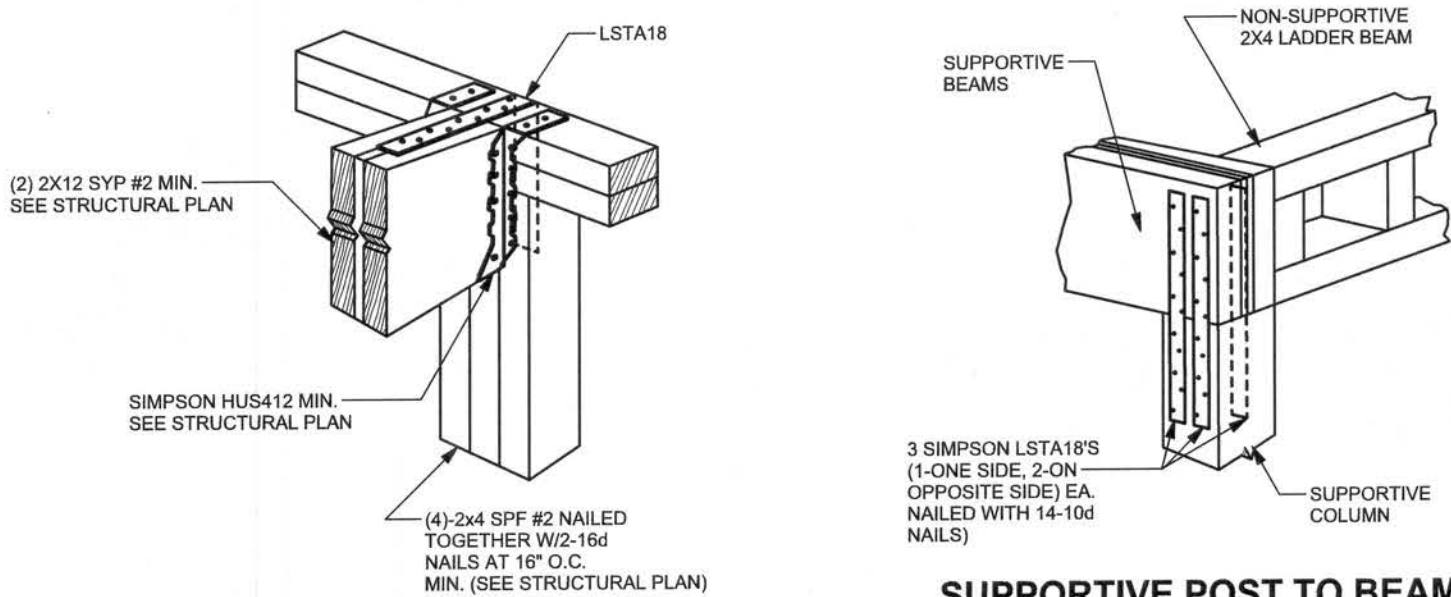


**ONE STORY WALL SECTION**  
SCALE: 3/4\"/>

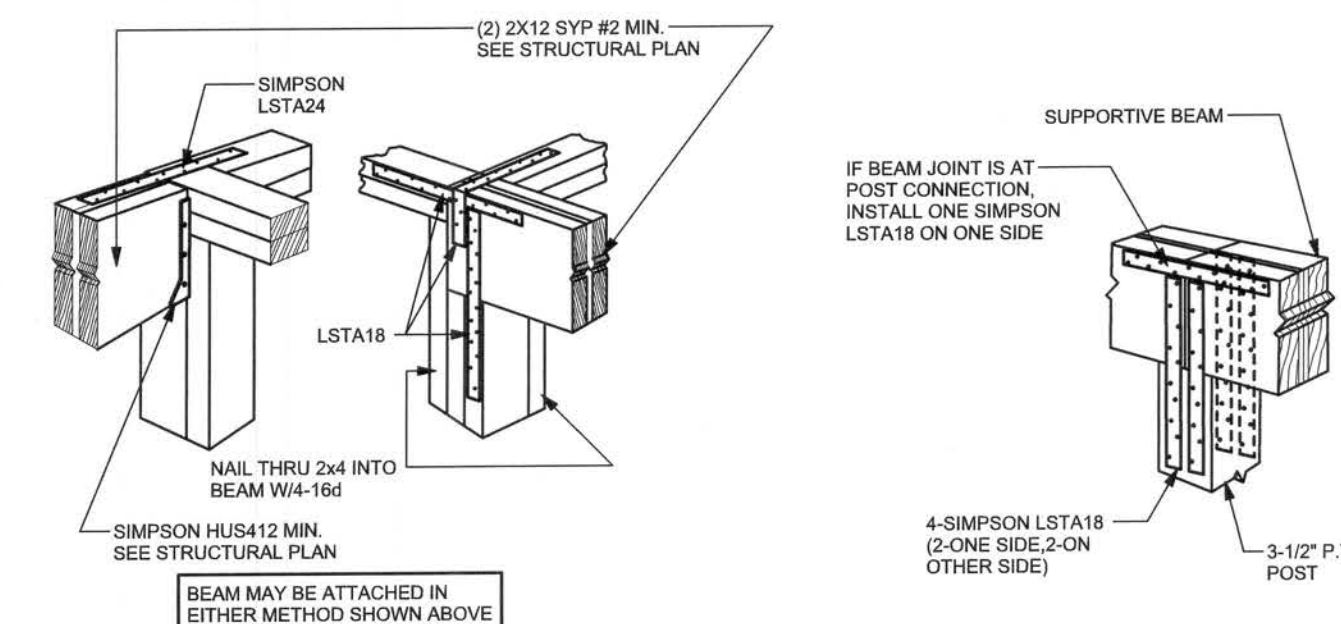
**EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS**

(1) 2x4 @ 16\"/>
------------------

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B, EXTERIOR LOAD BEARING & NON-LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16\"/>



**BEAM MID-WALL CONNECTION DETAIL**  
SCALE: N.T.S.



**BEAM CORNER CONNECTION DETAIL**  
SCALE: N.T.S.

**SUPPORTIVE POST TO BEAM DETAIL FOR SINGLE BEAM**  
SCALE: N.T.S.

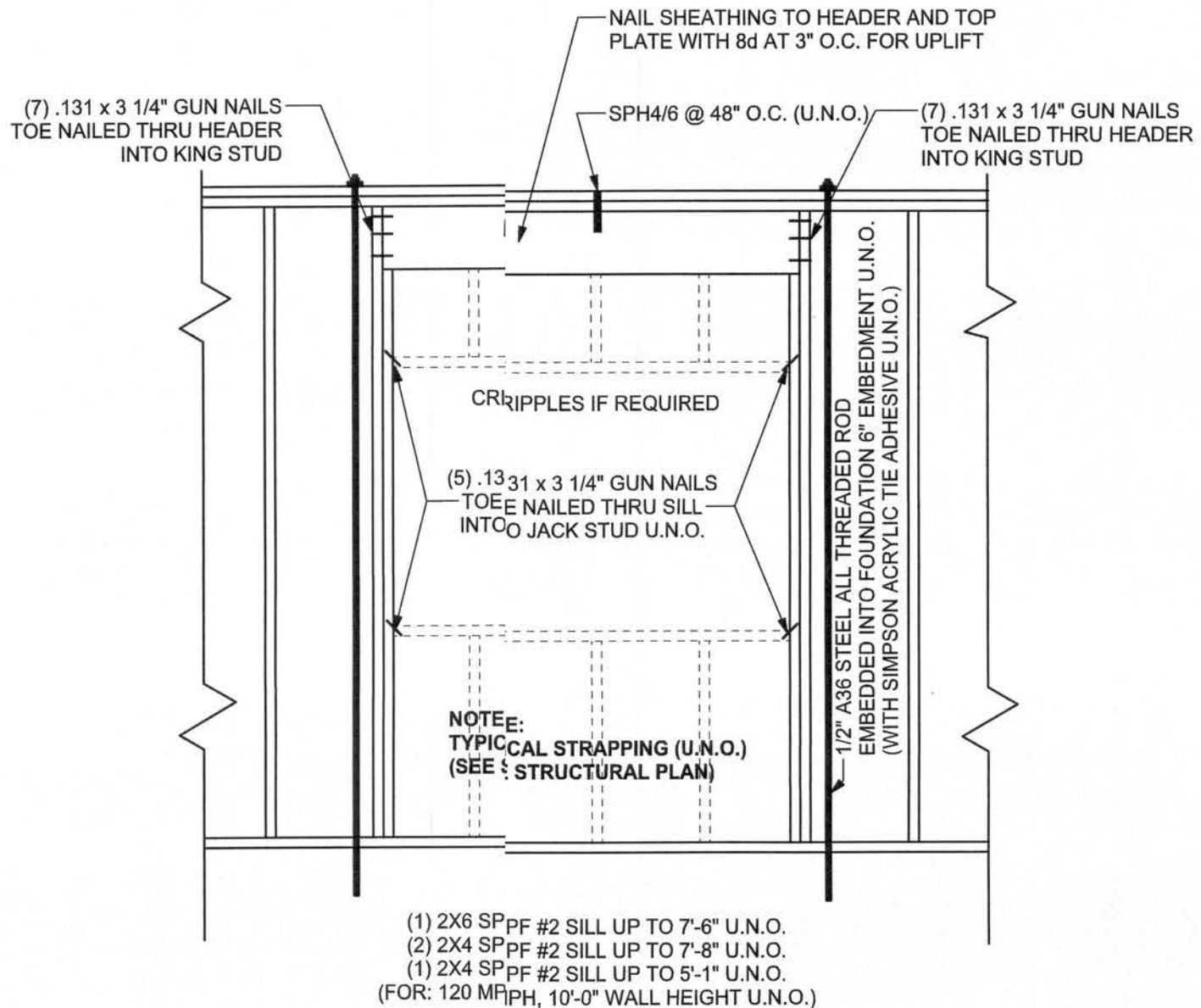
**SUPPORTIVE CENTER POST TO BEAM DETAIL**  
SCALE: N.T.S.

**4X4 / 6X6 PORCH POST DETAIL**  
SCALE: 1/2\"/>

**OPTIONAL ALUMINUM PORCH POST & HEADER ANCHORS**  
SCALE: N.T.S.

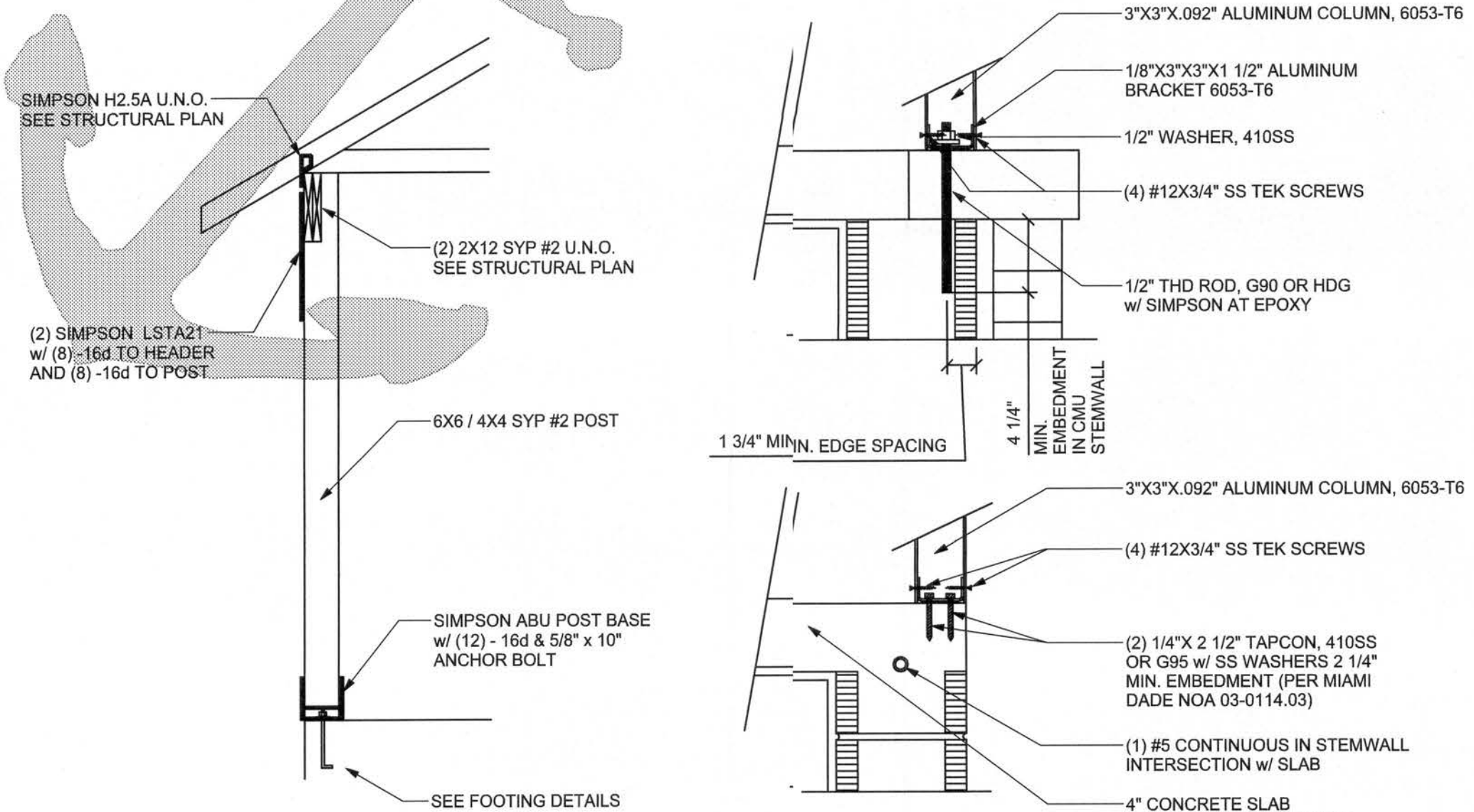
**NOTE:**  
IF TRUSS TO WALL STRAPS ARE NAILED TO THE HEADER THE SPH4/6 @ 48\"/>

FOR LESS THAN 1500 lb UPLIFT USE 2 X 2 X 1/8\"/>



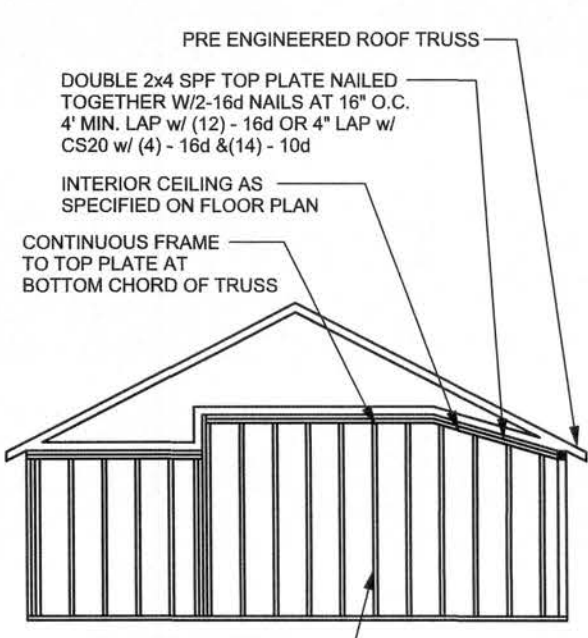
**TYPICAL 1 STORY/1 HEADER STRAPPING DETAIL**  
SCALE: 1/2\"/>

**TYPICAL GABLE END (X-BRACING)**  
ALL MEMBERS SHALL BE SYP



**GRADE & SPECIES TABLE**

		Fb (psi)	E (10 <sup>6</sup> psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2900	2.0
PSL	PARALAM	2900	2.0



**CONTINUOUS FRAME TO CEILING DIAPHRAGM DETAIL**  
SCALE: N.T.S.

**ANCHOR TABLE**

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 285	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 385	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H8	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2- HTS24			
< 2050	< 1785	LG2	14-16d	14-16d	
<b>HEAVY GIRDER TIEDOWNS*</b>					<b>TO FOUNDATION</b>
< 3965	< 3330	MG7		22-10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
<b>STUD STRAP CONNECTOR*</b>					<b>TO STUDS</b>
< 435	< 435	SSP DOUBLE TOP PLATE	3-10d		4-10d
< 455	< 420	SSP SINGLE SILL PLATE	1-10d		4-10d
< 825	< 825	DSP DOUBLE TOP PLATE	6-10d		8-10d
< 825	< 600	DSP SINGLE SILL PLATE	2-10d		8-10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
<b>STUD ANCHORS*</b>					<b>TO FOUNDATION</b>
< 1350	< 1305	LT119	8-16d		12" AB
< 2310	< 2310	LT131	18-10d, 1 1/2"		12" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3655	HTT16	18-16d		5/8" AB
< 1400	< 1400	PAH42	16-16d		
< 3335	< 3335	HPAH22	16-16d		
< 2220	< 2200	ABU14	12-16d		1/2" AB
< 2300	< 2300	ABU6	12-16d		1/2" AB
< 2320	< 2320	ABU8	18-16d		2-5/8" AB

**GENERAL NOTES:**

**TRUSSES:** TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS TO TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2x6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END, 2x8 RAFTERS 700 LB EACH END.

**SITE PREPARATION:** SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN.

**FOUNDATION:** CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE).

**CONCRETE:** MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F<sub>c</sub> = 3000 PSI.

**WELED WIRE REINFORCED SLAB:** 6" x 6" W14 x W14, FB = 85ksi, WELED WIRE REINFORCEMENT FABRIC (W.W.R.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 7'.

**FIBER CONCRETE SLAB:** CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1118. SUPPLIER TO PROVIDE ASTM C 1118 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

**CONTROL JOINTS:** WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT W/M OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE).

**REBAR:** ASTM A 615, GRADE 60, DEFORMED BARS, F<sub>y</sub> = 60 KSI, ALL LAP SPLICES 40" DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

**GLULAM BEAMS:** GLULAM BEAM, GLB, 24F-V3SP, F<sub>b</sub> = 2.4kl, E = 1800ksi. UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCULATIONS. **ROOF SHEATHING:** ALL ROOFS ARE HORIZONTAL DIAPHRAGMS, 7/16" OSB SHEATHING, UNLOCKED, STAGGERED, FASTENED WITH 8d COMMON NAILS @ 1311, 6" OC PANEL EDGES, 12" OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY, 4" OC, UNO.

**STRUCTURAL CONNECTORS:** MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE. NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

**ANCHOR BOLTS:** 3/4" ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

**WASHERS:** WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

**NAILS:** ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

**BUILDER'S RESPONSIBILITY**

**THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.**

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMTS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS TO TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

**ROOF SYSTEM DESIGN**

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2004 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

**DESIGN DATA**

**WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1**

(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60 FT IN EXP. B, 30 FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

- 1.) BASIC WIND SPEED = 110 MPH
- 2.) WIND EXPOSURE = B
- 3.) WIND IMPORTANCE FACTOR = 1.0
- 4.) BUILDING CATEGORY = II
- 5.) ROOF ANGLE = 10-45 DEGREES
- 6.) MEAN ROOF HEIGHT = <30 FT
- 7.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)
- 8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))

Zone	Effective Wind Area (ft <sup>2</sup> )	
	10	100
1	19.9 - 21.8	18.1 - 21.8
2	19.9 - 25.5	18.1 - 21.8
2 Other	-40.6	-40.6
3	19.9 - 25.5	18.1 - 21.8
3 Other	-68.3	-42.4
4	21.8 - 23.6	18.5 - 20.4
5	21.8 - 29.1	18.5 - 22.6
Doors & Windows	21.8	-29.1
Wind Caps (Zone 5, 10 ft <sup>2</sup> )		
8x7 Garage Door	19.5	-22.9
16x7 Garage Door	18.5	-21.0

**DESIGN LOADS**

FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)
	30 PSF (SLEEPING ROOMS)
	30 PSF (ATTICS WITH STORAGE)
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)
ROOF	20 PSF (FLAT OR <4:12)
	16 PSF (4:12 TO <12:12)
	12 PSF (12:12 AND GREATER)
STAIRS	40 PSF (ONE & TWO FAMILY DWELLINGS)
	SOIL BEARING CAPACITY 1000PSF
	NOT IN FLOOD ZONE (BUILDER TO VERIFY)

**REVISIONS**

NO.	DESCRIPTION

**SOFTPLAN**  
ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Disoway, P.E. No. 53515, P.O. Box 868, Lake City, FL 32066, 386-754-5419

**DIMENSIONS**  
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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**CERTIFICATION:** I hereby certify that I have examined this plan, and that the applicable portions of Section R301.2.1, Florida Building Code Residential 2004, to the best of my knowledge.

**LIMITATION:** This design is valid for one building, at specified location.

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P.E. 53515  
*Mark Disoway*  
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**Richard & Xene Nelson Residence**

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**PRINTED DATE:**  
July 16, 2007  
**DRAWN BY:** David Disoway  
**CHECKED BY:**

**FINALS DATE:**  
16 / Jun / 07

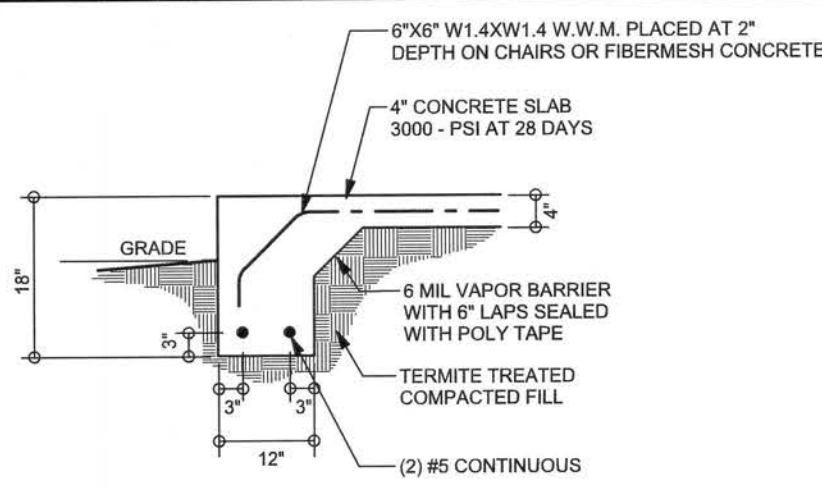
**JCB NUMBER:**  
705081

**DRAWING NUMBER**  
**S-1**  
OF 7 SHEETS

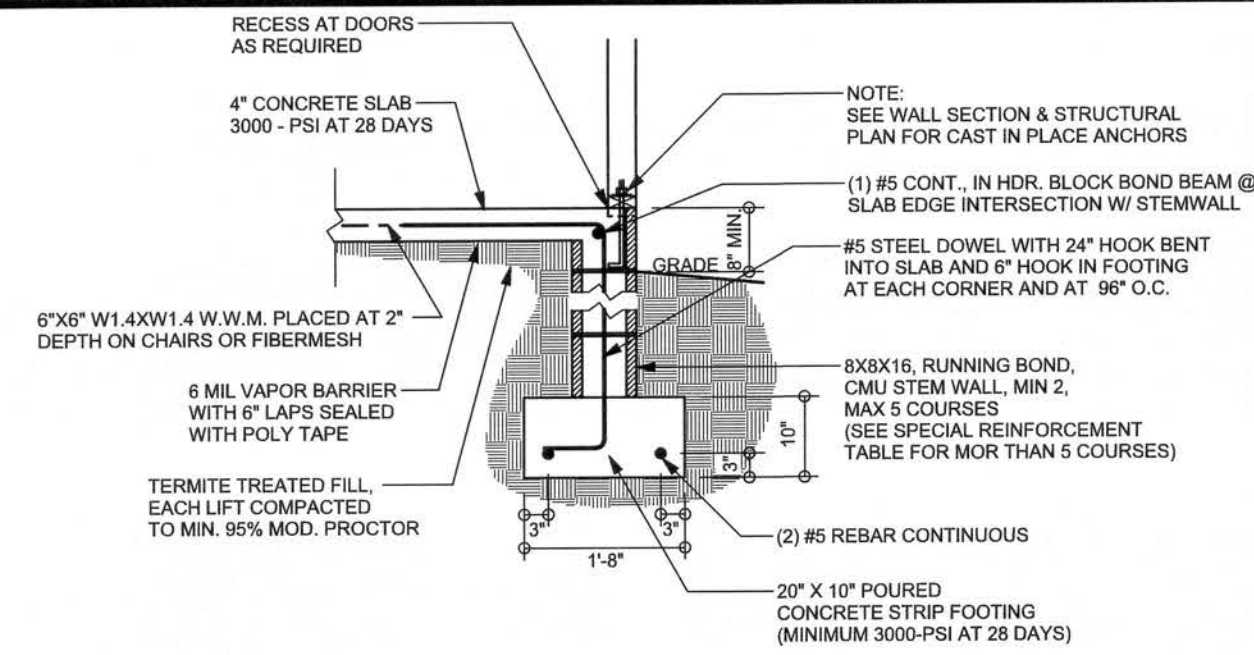


# REVISIONS

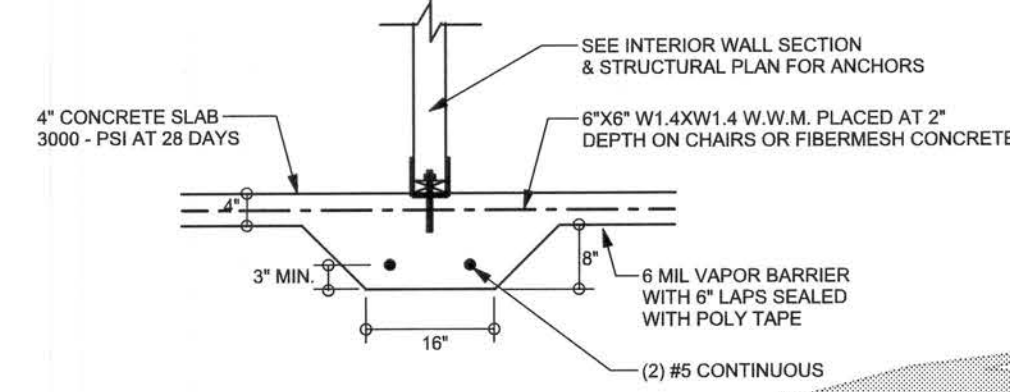
SOFTPLAN  
ARCHITECTURAL DESIGN SOFTWARE



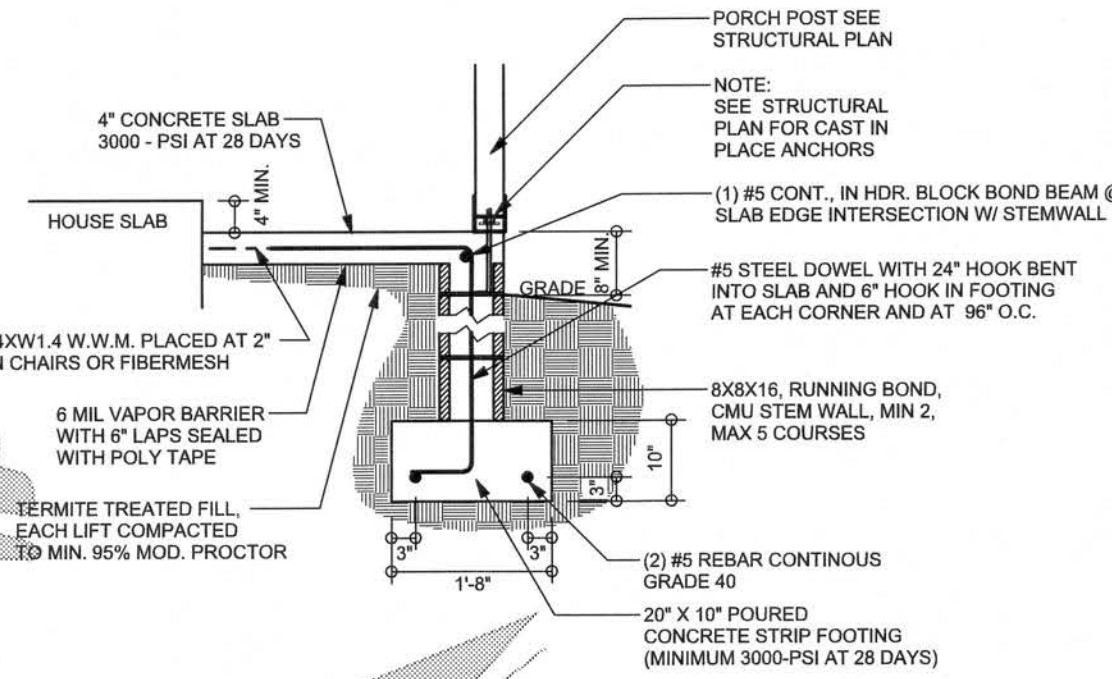
**F1 MONOLITHIC FOOTING**  
SCALE: 1/2" = 1'-0"



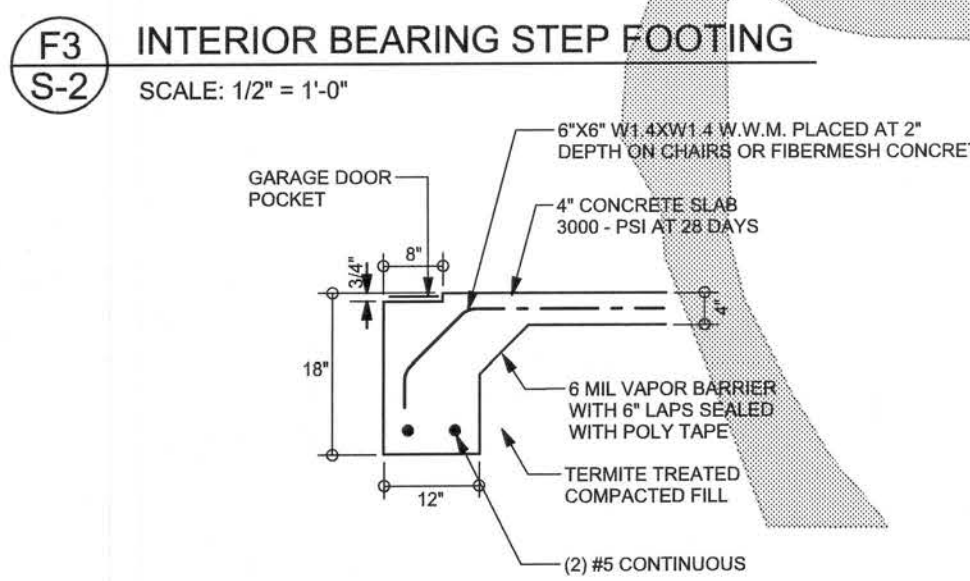
**F9 STEM WALL FOOTING**  
SCALE: 1/2" = 1'-0"



**F2 INTERIOR BEARING FOOTING**  
SCALE: 1/2" = 1'-0"



**F12 ALT. STEM WALL PORCH FOOTING**  
SCALE: 1/2" = 1'-0"

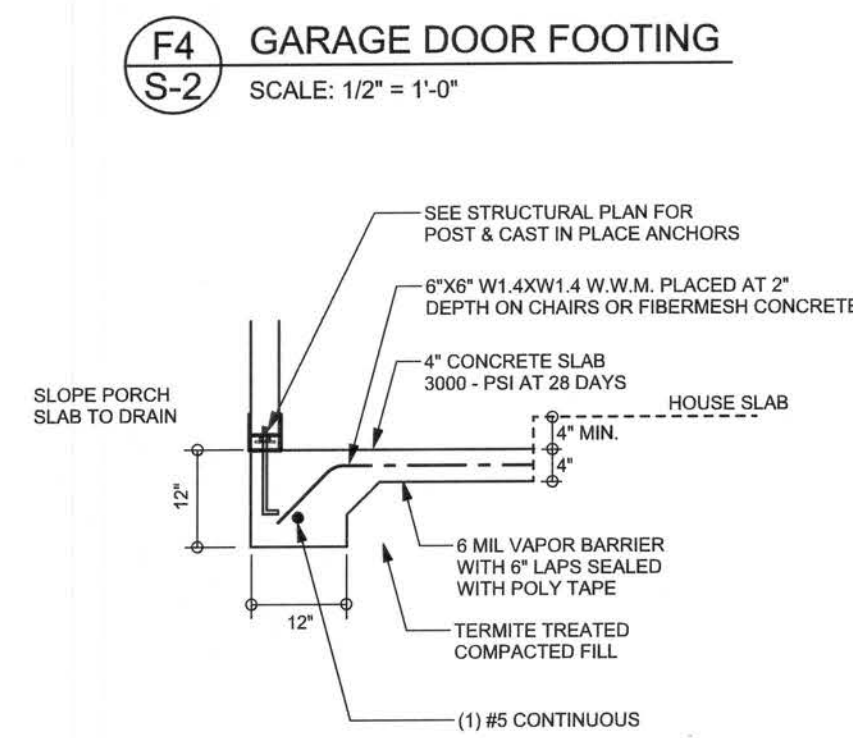


**F3 INTERIOR BEARING STEP FOOTING**  
SCALE: 1/2" = 1'-0"

## TALL STEM WALL TABLE

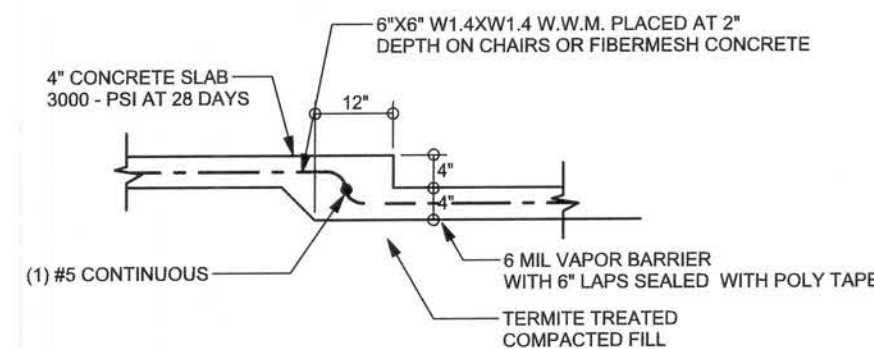
The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16" O.C. vertically or a horizontal bond beam with #5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48

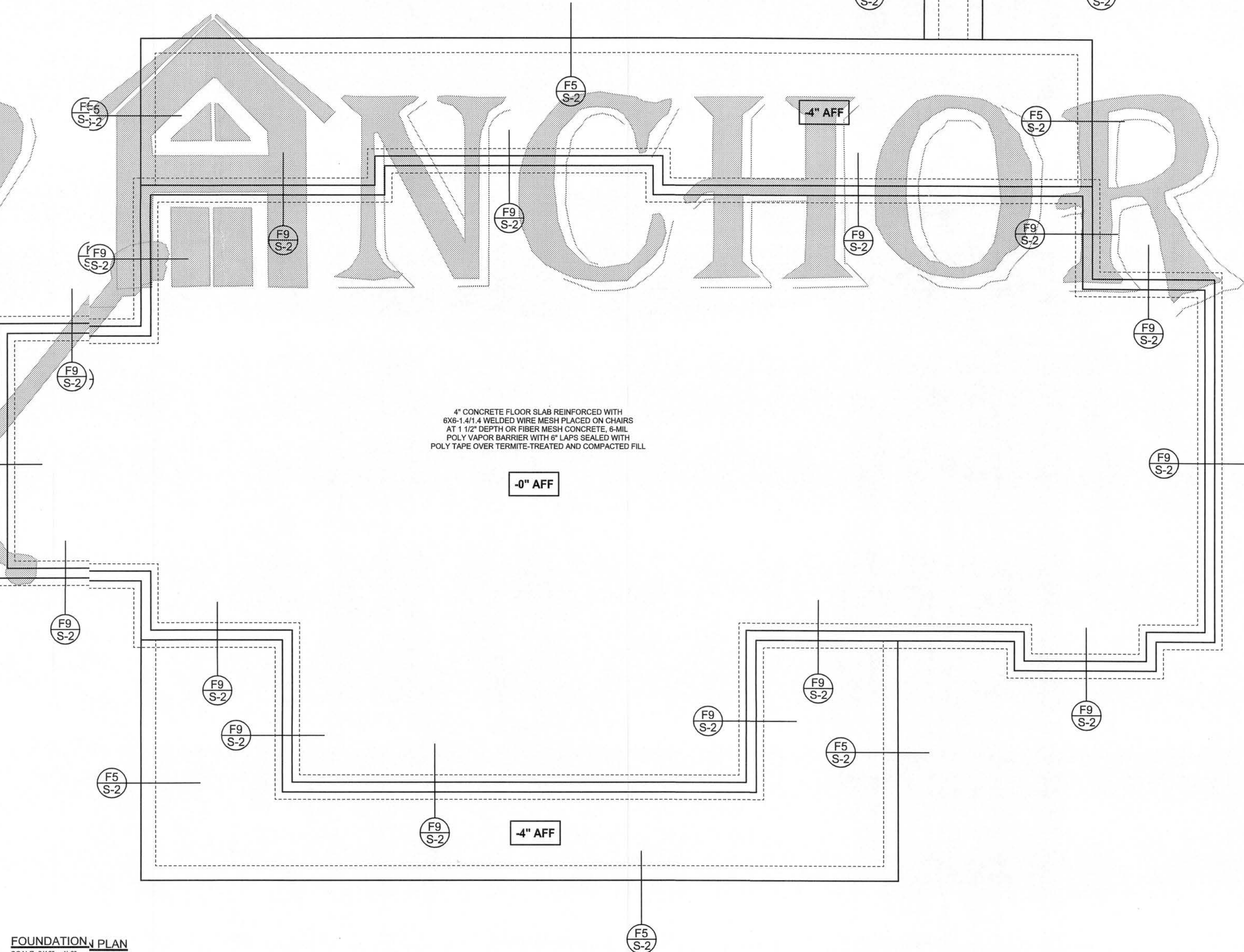


**F4 GARAGE DOOR FOOTING**  
SCALE: 1/2" = 1'-0"

**F5 PORCH FOOTING**  
SCALE: 1/2" = 1'-0"



**F6 TYPICAL NON-BEARING STEP FOOTING**  
SCALE: 1/2" = 1'-0"



**FOUNDATION PLAN**  
SCALE: 3/16" = 1'-0"  
DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

WINDLOAD ENGINEER: Mark Disoway,  
E.E. No. 53915, P.O. Box 868, Lake City, FL  
32056, 386-754-5419

REVISIONS:  
Noted dimensions supersede scaled  
dimensions. Refer all questions to  
Mark Disoway, P.E. for resolution.  
Do not proceed without clarification.

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permission and consent of Mark Disoway.

CERTIFICATION: I hereby certify that I have  
examined this plan, and that the applicable  
provisions of the plan, relating to wind engineering  
comply with section R301.2.1, Florida Building  
Code Residential 2004, to the best of my  
knowledge.

NOTATION: This design is valid for one  
building, at specified location.

MARK DISOWAY  
P.E. 53915

SEAL

Richard & Xene  
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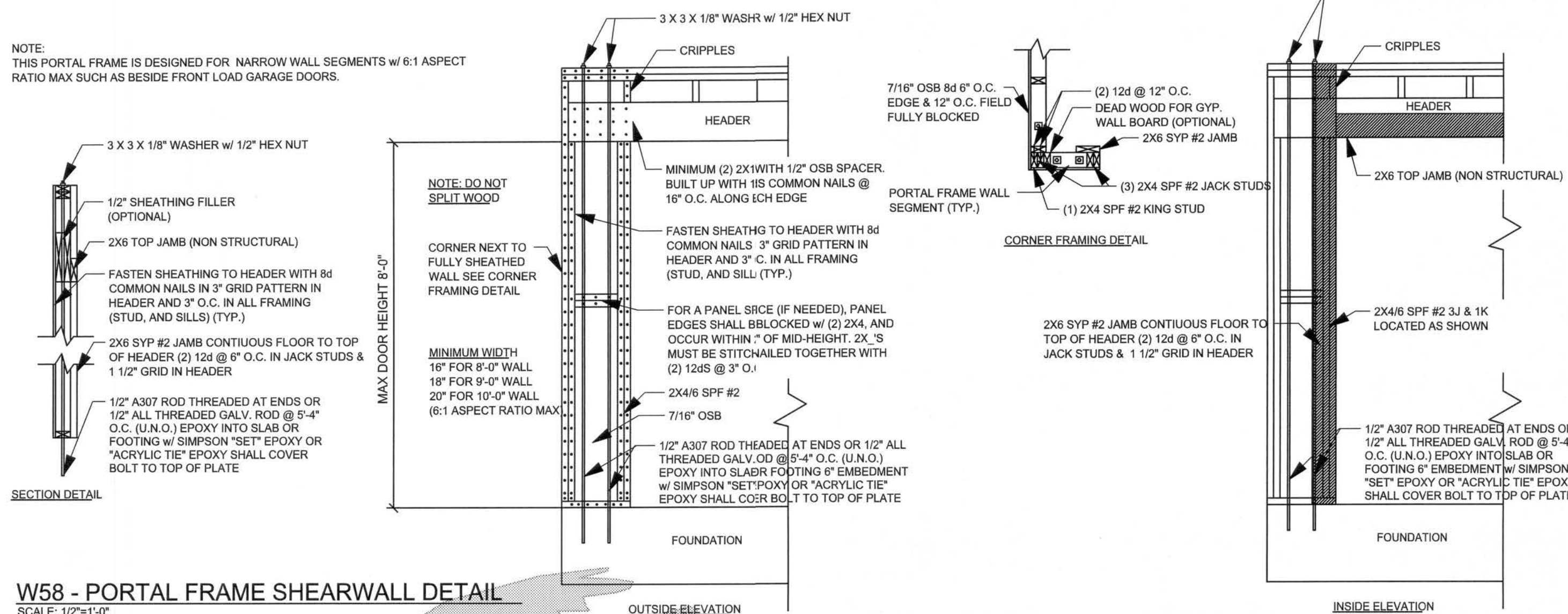
JOB NUMBER:  
705081

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**S-2**

OF 7 SHEETS

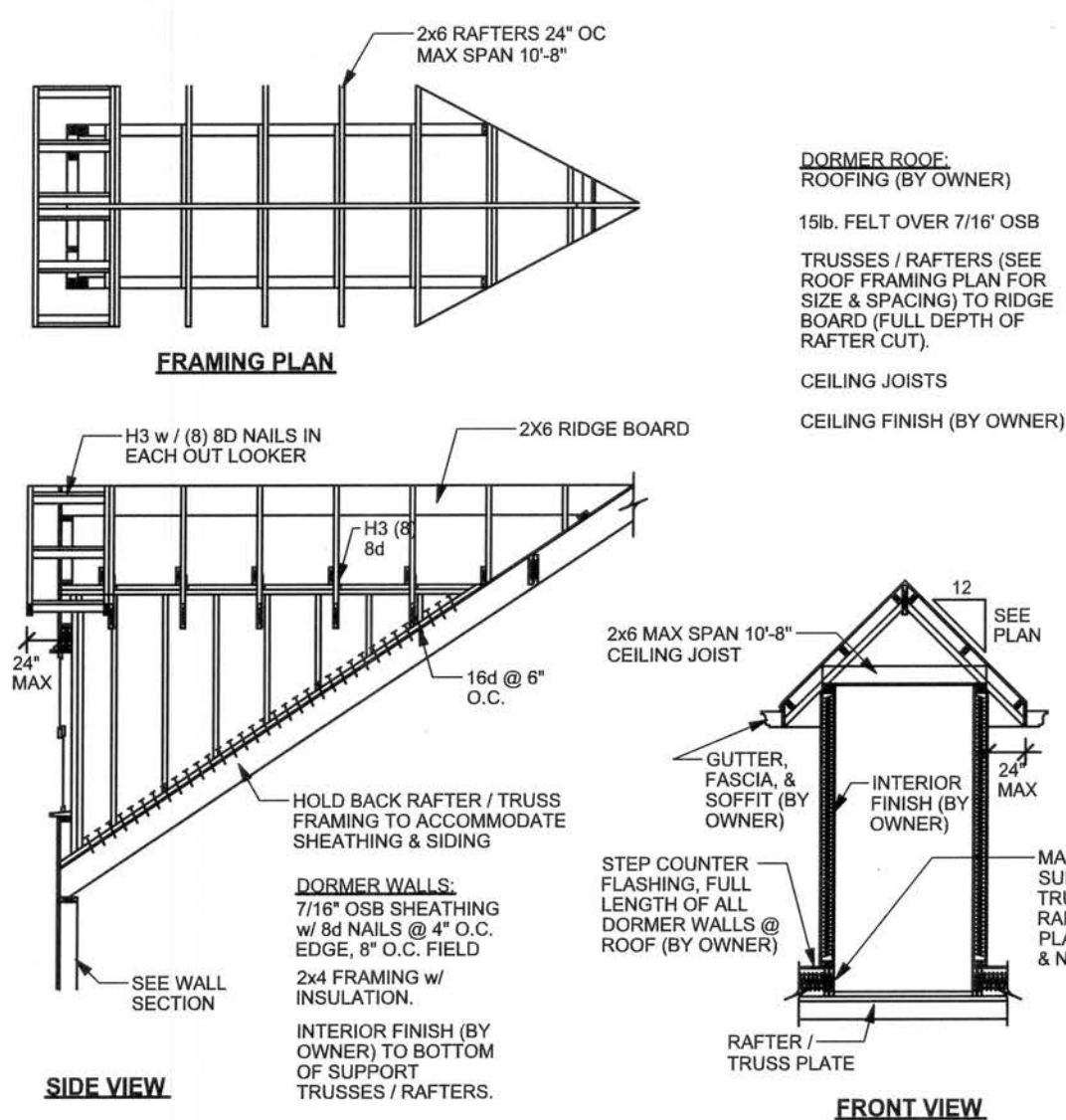


NOTE:  
THIS PORTAL FRAME IS DESIGNED FOR NARROW WALL SEGMENTS w/ 6:1 ASPECT  
RATIO MAX SUCH AS BESIDE FRONT LOAD GARAGE DOORS.

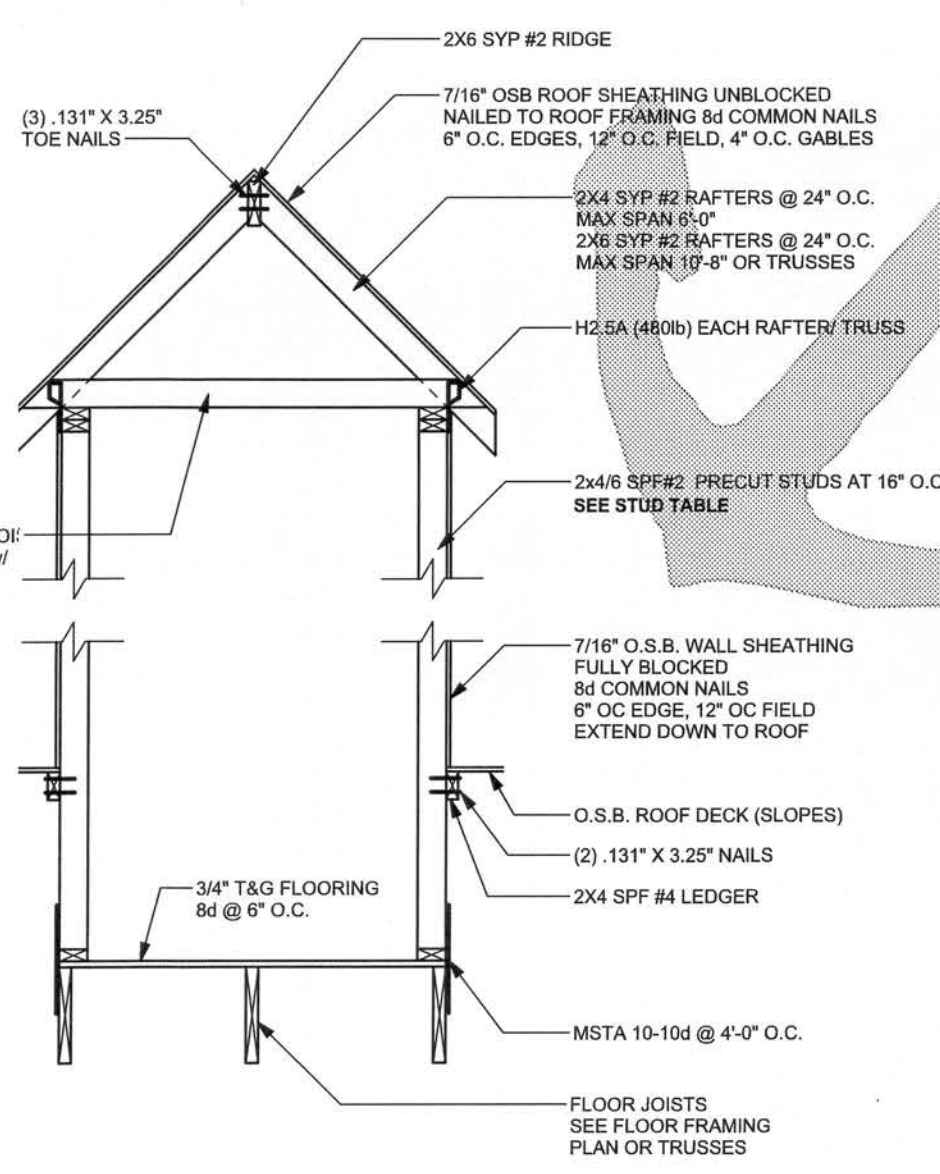


**W58 - PORTAL FRAME SHEARWALL DETAIL**  
SCALE: 1/2"=1'-0"

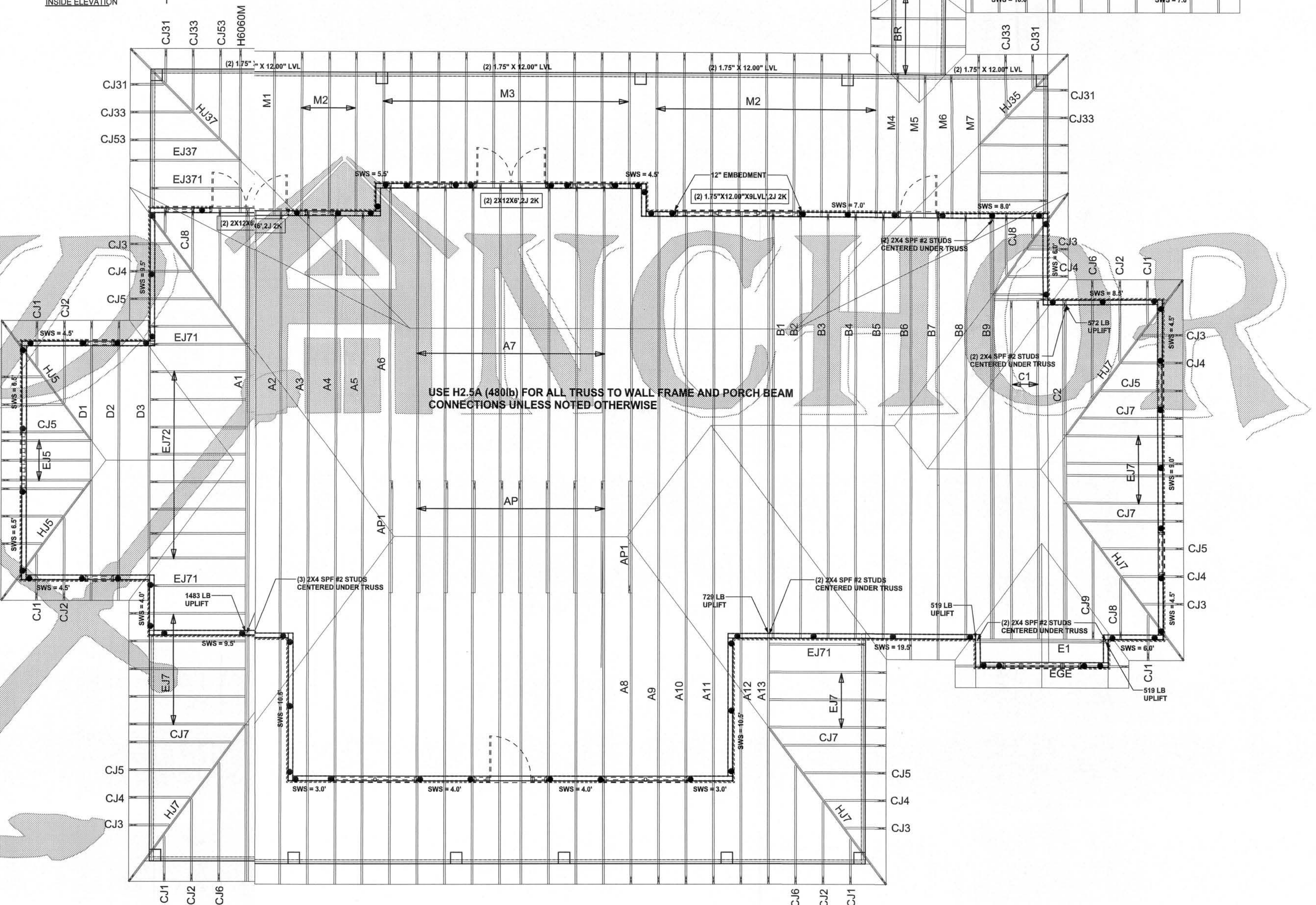
**ALTERNATE WALL TIE CONNECTION WHERE  
THREADED ROD CANNOT BE PLACED IN WALL.**  
SCALE: 1/2"=1'-0"



**DORMER ANCHORING DETAIL (ON ROOF)**  
SCALE: N.T.S.



**DORMER ANCHORING DETAIL (ON FLOOR)**  
SCALE: N.T.S.



**STRUCTURAL PLAN**  
SCALE: 3/16"=1'-0"

**STRUCTURAL PLAN NOTES**

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP#2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSS-83, BCSS-81, BCSS-82, & BCSS-81. BCSS-82, & BCSS-81, & BCSS-83 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

**THREADED ROD LEGEND**

- INDICATES LOCATION OF: 1ST FLOOR 1/2" A307 ALL THREADED ROD
- ⊙ INDICATES LOCATION OF: 2ND FLOOR 1/2" A307 ALL THREADED ROD

**HEADER LEGEND**

- (2) 2X12X10' L1 1K ○ HEADER/BEAM CALL-OUT (U.N.O.)
- NUMBER OF KING STUDS (FULL LENGTH)
- NUMBER OF JACK STUDS (UNDER HEADER)
- SPAN OF HEADER
- SIZE OF HEADER MATERIAL
- NUMBER OF PLIES IN HEADER

**TOTAL SHEAR WALL SEGMENTS (HOUSE)**

SW5 = 0.0' INDICATES SHEAR WALL SEGMENTS	
REQUIRED	ACTUAL
TRANSVERSE 45.0'	71.5'
LONGITUDINAL 33.0'	91.5'

**TOTAL SHEAR WALL SEGMENTS (GARAGE)**

SW5 = 0.0' INDICATES SHEAR WALL SEGMENTS	
REQUIRED	ACTUAL
TRANSVERSE 13.5'	24.0'
LONGITUDINAL 11.4'	41.0'

**WALL LEGEND**

SW5 = 0.0'	1ST FLOOR EXTERIOR WALL
SW5 = 0.0'	2ND FLOOR EXTERIOR
IBW	1ST FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1
IBW	2ND FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER, ANDERSON TRUSS  
JOB #7-185

REVISIONS	

**SOFTPLAN**  
ARCHITECTURAL DESIGN SOFTWARE

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**CERTIFICATION:** I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, Florida building code (residential 2004), to the best of my knowledge.

**LIMITATION:** This design is valid for one building, at specified location.

MARK DISOSWAY  
P.E. 5315

*Handwritten signature and date: 11/16/07*  
SEAL

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**S-3**  
OF 7 SHEETS