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COA #0 278  
02/11/2022

Alpine, an ITW Company  
6750 Forum Drive, Suite 305  
Orlando, FL 32821  
Phone: (800)755-6001  
www.alpineitw.com

Site Information:	Page 1:
Customer: W. B. Howland Company, Inc.	Job Number: 21-6456
Job Description: Brian Dicks Residence	
Address:	

Job Engineering Criteria:	
Design Code: FBC 7th Ed. 2020 Res.	IntelliVIEW Version: 21.02.00 JRef #: 1XcZ2150017
Wind Standard: ASCE 7-16      Wind Speed (mph): 120 Building Type: Closed	Design Loading (psf): 40.00

This package contains general notes pages, 60 truss drawing(s) and 7 detail(s).

Item	Drawing Number	Truss	Item	Drawing Number	Truss
1	042.22.1430.01630	A01	2	042.22.1429.57980	A02
3	042.22.1429.55987	A03	4	042.22.1429.54127	A04
5	042.22.1429.51447	B01	6	042.22.1429.47997	B02
7	042.22.1429.45270	B03	8	042.22.1429.42130	B04
9	042.22.1429.22207	C01	10	042.22.1429.05487	C02
11	042.22.1429.03670	C03	12	042.22.1428.35273	E01
13	042.22.1428.33030	E02	14	042.22.1428.30370	E03
15	042.22.1428.25603	E04	16	042.22.1428.08310	E05
17	042.22.1428.05407	E06	18	042.22.1428.02350	E07
19	042.22.1427.06103	E08	20	042.22.1427.04100	E09
21	042.22.1427.02123	E10	22	042.22.1426.59243	F01
23	042.22.1426.56610	F02	24	042.22.1426.54037	F03
25	042.22.1426.51853	G01	26	042.22.1426.49823	G02
27	042.22.1426.48350	G03	28	042.22.1426.46783	G04
29	042.22.1426.44850	G05	30	042.22.1426.01603	H01
31	042.22.1425.15520	H02	32	042.22.1425.12610	H03
33	042.22.1424.54763	PB01	34	042.22.1424.52433	PB02
35	042.22.1424.50813	PB03	36	042.22.1424.47253	PB04
37	042.22.1424.45410	PB05	38	042.22.1424.43727	PB06
39	042.22.1424.42083	PB07	40	042.22.1424.39390	V01
41	042.22.1424.37293	V02	42	042.22.1424.35577	V03
43	042.22.1424.33783	V04	44	042.22.1424.31500	V05
45	042.22.1424.29333	V06	46	042.22.1424.27597	V07
47	042.22.1424.26190	V08	48	042.22.1424.24730	V09
49	042.22.1424.20573	V10	50	042.22.1424.19290	V11
51	042.22.1424.17937	V12	52	042.22.1424.16630	V13



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Site Information:	Page 2:
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Job Description: Brian Dicks Residence	
Address:	

Item	Drawing Number	Truss
53	042.22.1424.15153	V14
55	042.22.1424.12807	V16
57	042.22.1424.10227	V18
59	042.22.1424.07403	V20
61	BRCLBSUB0119	
63	PB160160118	
65	GABRST160118	
67	A12015ENC160118	

Item	Drawing Number	Truss
54	042.22.1424.13967	V15
56	042.22.1424.11533	V17
58	042.22.1424.08923	V19
60	042.22.1424.05710	V21
62	VALTN160118	
64	A12030ENC160118	
66	GBLLETIN0118	

## **General Notes**

### **Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:**

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

### **Temporary Lateral Restraint and Bracing:**

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCE), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

### **Permanent Lateral Restraint and Bracing:**

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed and detailed by the Building Designer.

### **Connector Plate Information:**

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at [www.icc-es.org](http://www.icc-es.org).

### **Fire Retardant Treated Lumber:**

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.



## **General Notes** (continued)

### **Key to Terms:**

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for of all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for of all load cases.

Max Web CSI = Maximum bending and axial Combined Stress Index for Webs for of all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.

W = Width of non-hanger bearing, in inches.

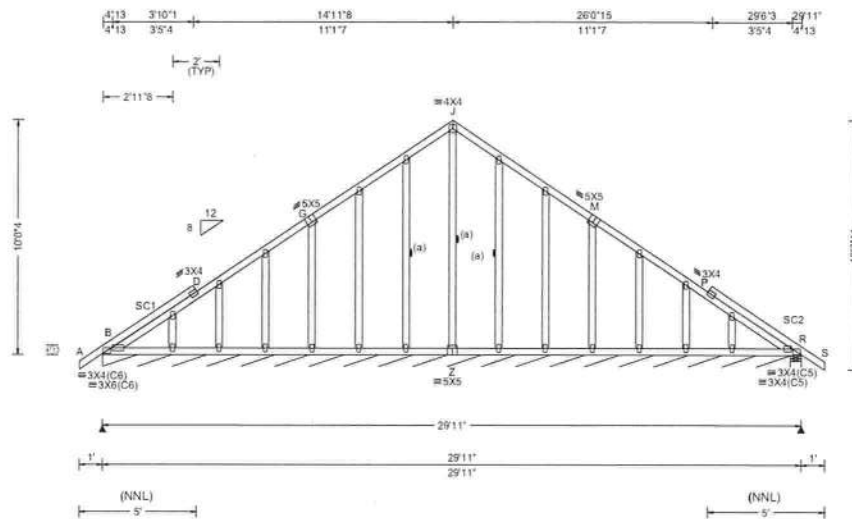
Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

**References:**

1. AWC: American Wood Council; 222 Catoclin Circle SE, Suite 201; Leesburg, VA 20175; [www.awc.org](http://www.awc.org).
2. ICC: International Code Council; [www.iccsafe.org](http://www.iccsafe.org).
3. Alpine, a division of ITW Building Components Group Inc.: 514 Earth City Expressway, Suite 242, Earth City, MO 63045; [www.alpineitw.com](http://www.alpineitw.com).
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; [www.tpinst.org](http://www.tpinst.org).
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; [www.sbcacomponents.com](http://www.sbcacomponents.com).

SEQN: 69788	GABL	Ply: 1	Job Number: 21-6456	Cust: R 215 JRef: 1XcZ2150017 T5
FROM:		Qty: 1	Brian Dicks Residence	DrwNo: 042.22.1430.01630
			Truss Label: A01	KD / DF 02/11/2022



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity Non-Gravity
TCDL: 10.00	Speed: 120 mph	Pf: NA Ce: NA	VERT(LL): 0.003 D 999 240	Loc R+ / R- / Rh / Rw / U / RL
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.005 D 999 180	B* 81 /- /- /44 /- /6
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.003 P - -	R 254 /- /- /125 /- /-
Des Ld: 40.00	EXP: B Kzt: NA		HORZ(TL): 0.004 P - -	Wind reactions based on MWFRS
NCBCLL: 10.00	Mean Height: 0.00 ft	Building Code:	Creep Factor: 2.0	B Brg Wid = 353 Min Req = -
Soffit: 2.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.317	R Brg Wid = 5.5 Min Req = 1.5 (Truss)
Load Duration: 1.00	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.087	Bearings B & R are a rigid surface.
Spacing: 24.0 "	MWFRS Parallel Dist: > 2h	Rep Fac: Yes	Max Web CSI: 0.124	Members not listed have forces less than 375#
	C&C Dist a: 3.00 ft	FT/RT: 20(0)/10(0)		
	Loc. from endwall: Any	Plate Type(s):	VIEW Ver: 21.02.00.1005.17	
	GCpi: 0.18	WAVE		
	Wind Duration: 1.60			

#### Lumber

Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;  
Stack Chord: SC1 2x4 SP #2;  
Stack Chord: SC2 2x4 SP #2;

#### Bracing

(a) Continuous lateral restraint equally spaced on member.

#### Plating Notes

All plates are 2X4 except as noted.

#### Wind

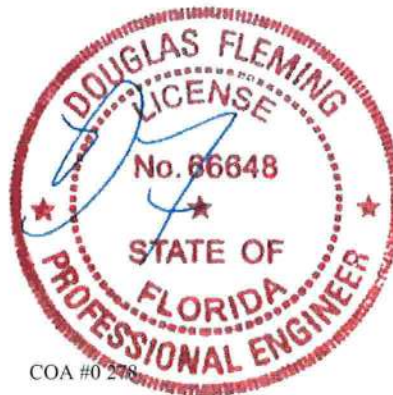
Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

#### Additional Notes

See DWGS A12015ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.



COA #0 218

02/11/2022

**\*\*WARNING\*\*** READ AND FOLLOW ALL NOTES ON THIS DRAWING!  
**\*\*IMPORTANT\*\*** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCE) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.

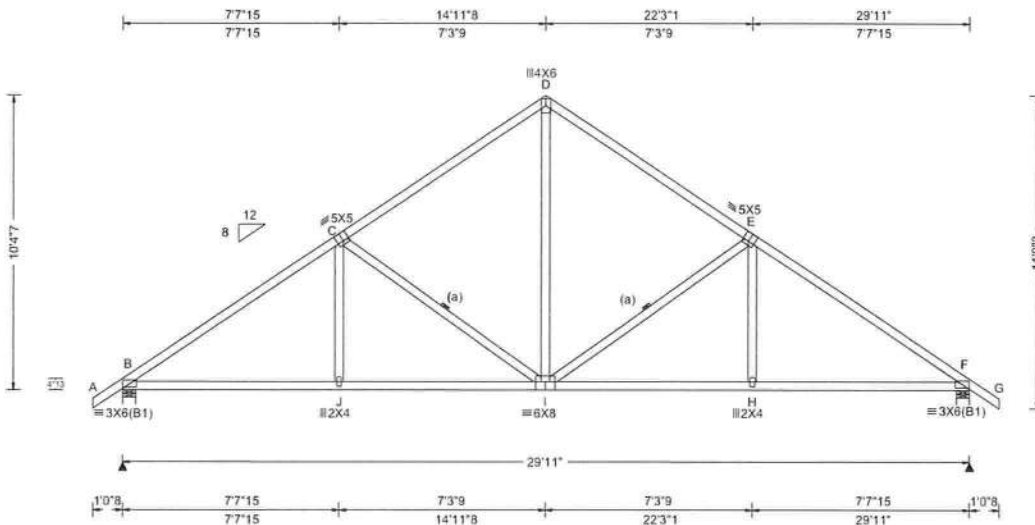
Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see these web sites: Alpine: [alpineitw.com](http://alpineitw.com); TPI: [tpinst.org](http://tpinst.org); SBCE: [sbcecomponents.com](http://sbcecomponents.com); ICC: [iccsafe.org](http://iccsafe.org); AWC: [awc.org](http://awc.org)

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SEQN: 69824 FROM:	COMN Qty: 7	Ply: 1 Job Number: 21-6456 Brian Dicks Residence Truss Label: A02	Cust: R 215 JRef: 1XcZ2150017 T4 DrwNo: 042.22.1429.57980 KD / DF 02/11/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.00 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 0.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: > 2h C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.057 I 999 240 VERT(CL): 0.118 I 999 180 HORZ(LL): 0.023 F - - HORZ(TL): 0.048 F - -  Creep Factor: 2.0 Max TC CSI: 0.781 Max BC CSI: 0.252 Max Web CSI: 0.387  VIEW Ver: 21.02.00.1005.17	<b>Maximum Reactions (lbs)</b> Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1329 - / - / - / 751 - / 181 F 1329 - / - / - / 751 - / - Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.5 (Truss) F Brg Wid = 5.5 Min Req = 1.5 (Truss) Bearings B & F are a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp. B - C 108 -1820 D - E 156 -1268 C - D 156 -1268 E - F 108 -1820

#### Lumber

Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP M-31;  
Webs: 2x4 SP #3;

#### Bracing

(a) Continuous lateral restraint equally spaced on member.

#### Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

#### Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - J	1409	0	I - H	1407	0
J - I	1407	0	H - F	1409	0

#### Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - I	112	-571	I - E	112	-571
D - I	812	-55			



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**\*\*WARNING\*\*** READ AND FOLLOW ALL NOTES ON THIS DRAWING!  
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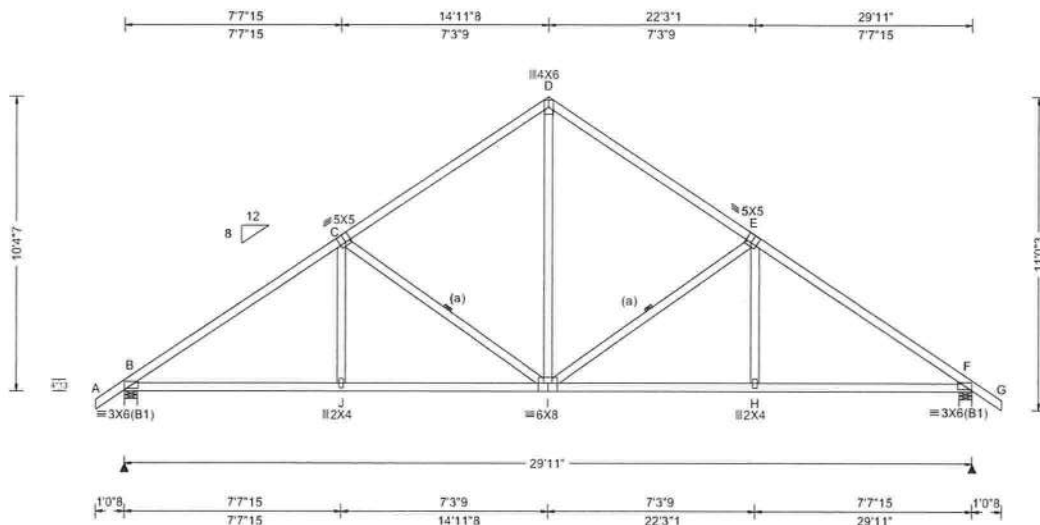
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCE) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.

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SEQN: 49734 FROM:	COMN Qty: 15	Ply: 1 Job Number: 21-6456 Brian Dicks Residence Truss Label: A03	Cust: R 215 JRef: 1Xc22150017 T2 DrwNo: 042.22.1429.55987 KD / DF 02/11/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.00 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.04 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.057 I 999 240 VERT(CL): 0.118 I 999 180 HORZ(LL): 0.023 F - - HORZ(TL): 0.048 F - - Creep Factor: 2.0 Max TC CSI: 0.781 Max BC CSI: 0.252 Max Web CSI: 0.387  VIEW Ver: 21.02.00.1005.17	Gravity Loc R+ / R- / Rh / Rw / U / RL B 1329 /- /- /751 /- /181 F 1329 /- /- /751 /- /- Non-Gravity Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.5 (Truss) F Brg Wid = 5.5 Min Req = 1.5 (Truss) Bearings B & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 108 -1820 D - E 156 -1268 C - D 156 -1268 E - F 108 -1820

#### Lumber

Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP M-31;  
Webs: 2x4 SP #3;

#### Bracing

(a) Continuous lateral restraint equally spaced on member.

#### Wind

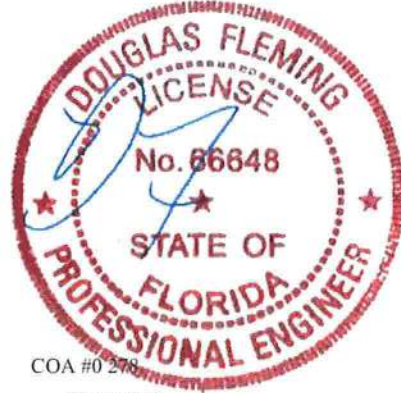
Wind loads based on MWFRS with additional C&C member design.  
Wind loading based on both gable and hip roof types.

#### Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - J	1409 0	I - H	1407 0
J - I	1407 0	H - F	1409 0

#### Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
C - I	112 -571	I - E	112 -571
D - I	812 -55		



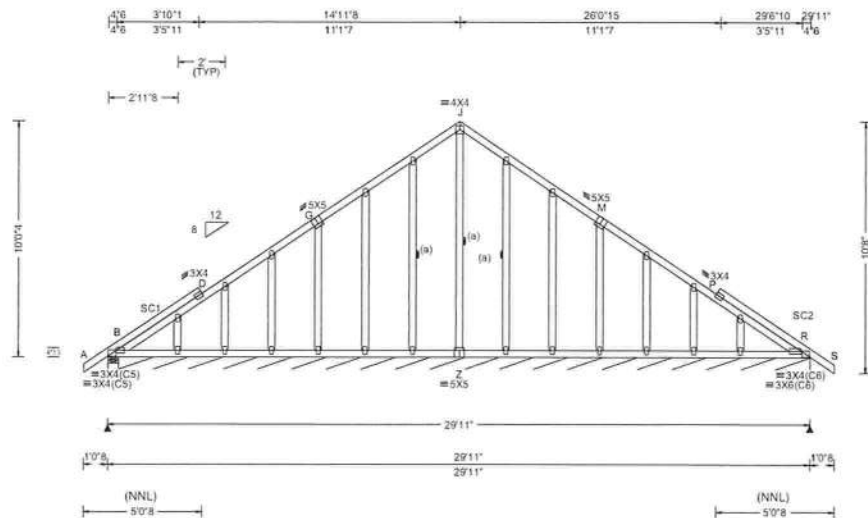
COA #0 278  
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For more information see these web sites: Alpine: [alpineitw.com](http://alpineitw.com); TPI: [tpinst.org](http://tpinst.org); SBCEA: [sbceacomponents.com](http://sbceacomponents.com); ICC: [iccsafe.org](http://iccsafe.org); AWC: [awc.org](http://awc.org)

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SEQN: 69551 FROM:	GABL Qty: 1	Ply: 1 Qty: 1	Job Number: 21-6456 Brian Dicks Residence Truss Label: A04	Cust: R 215 JRef: 1Xc22150017 T3 DrwNo: 042.22.1429.54127 KD / DF 02/11/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.00 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.003 P 999 240 VERT(CL): 0.005 P 999 180 HORZ(LL): 0.003 P - - HORZ(TL): 0.004 P - - Creep Factor: 2.0 Max TC CSI: 0.317 Max BC CSI: 0.085 Max Web CSI: 0.124  VIEW Ver: 21.02.00.1005.17	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 258 /- /- /137 /- /178 R* 81 /- /- /44 /- /- Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.5 (Truss) R Brg Wid = 353 Min Req = - Bearings B & B are a rigid surface. Members not listed have forces less than 375#

**Lumber**  
Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;  
Stack Chord: SC1 2x4 SP #2;  
Stack Chord: SC2 2x4 SP #2;

**Bracing**  
(a) Continuous lateral restraint equally spaced on member.

**Plating Notes**  
All plates are 2X4 except as noted.

**Purlins**  
In lieu of structural panels use purlins to brace TC @ 24" oc.

**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
Wind loading based on both gable and hip roof types.

**Additional Notes**  
See DWGS A12015ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.  
  
Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.



COA #0248  
02/11/2022

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