



Lumber design values are in accordance with ANSI/TPI 1 section 6.3
These truss designs rely on lumber values established by others.

RE: 1223-036 -

Site Information:

Customer Info: KENNETH CLYATT. Project Name: . Model: .
Lot/Block: . Subdivision: .
Address: ., .
City: FT. WHITE State: FL.

MiTek, Inc.

16023 Swingley Ridge Rd.
Chesterfield, MO 63017
314.434.1200

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name: License #:
Address:
City: State:

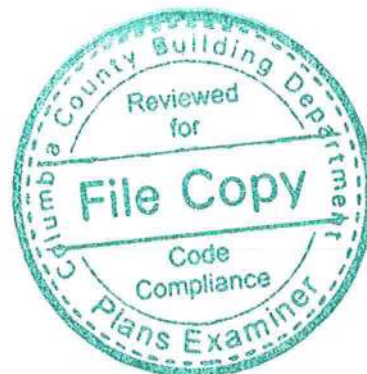
General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2023/TPI2014 Design Program: MiTek 20/20 8.7
Wind Code: ASCE 7-22 Wind Speed: 130 mph
Roof Load: 40.0 psf Floor Load: N/A psf

This package includes 16 individual, Truss Design Drawings and 0 Additional Drawings.

With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No.	Seal#	Truss Name	Date
1	T32673329	A01	1/19/24
2	T32673330	A02	1/19/24
3	T32673331	A03	1/19/24
4	T32673332	A04	1/19/24
5	T32673333	A05	1/19/24
6	T32673334	A06	1/19/24
7	T32673335	A07	1/19/24
8	T32673336	A08	1/19/24
9	T32673337	A09	1/19/24
10	T32673338	A10	1/19/24
11	T32673339	F01	1/19/24
12	T32673340	F02	1/19/24
13	T32673341	PB01	1/19/24
14	T32673342	PB02	1/19/24
15	T32673343	PB03	1/19/24
16	T32673344	PB04	1/19/24



The truss drawing(s) referenced above have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Mayo Truss Company, Inc..

Truss Design Engineer's Name: Velez, Joaquin

My license renewal date for the state of Florida is February 28, 2025.

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Joaquin Velez PE No.68182
MiTek Inc. DBA MiTek USA FL Cert 6634
16023 Swingley Ridge Rd. Chesterfield, MO 63017
Date:

January 19, 2024

Velez, Joaquin

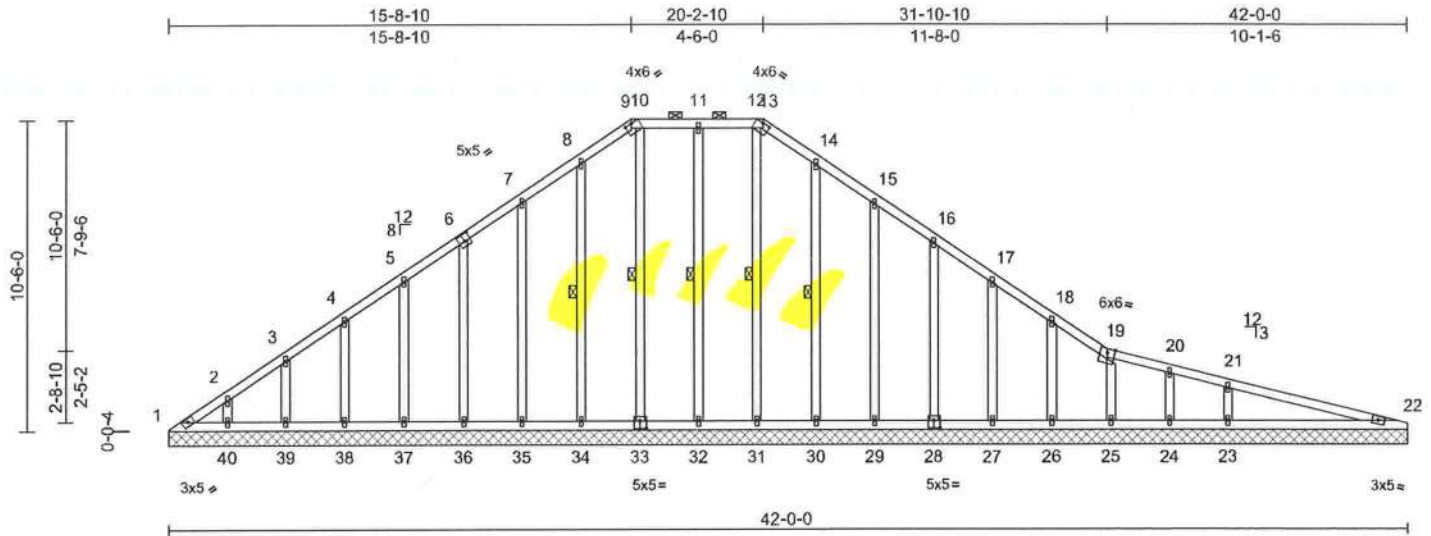
1 of 1

Job	Truss	Truss Type	Qty	Ply	Job Reference (optional)
1223-036	A01	Piggyback Base Supported Gable	1	1	T32673329

Mayo Truss Company, Inc., Mayo, FL - 32066,

Run: 8.73 S Jan 4 2024 Print: 8.730 S Jan 4 2024 MiTek Industries, Inc. Thu Jan 18 13:02:33
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Page: 1



Scale = 1:73.8

Plate Offsets (X, Y): [6:0-2-8,0-3-0], [9:0-3-0,0-0-2], [13:0-3-0,0-0-2], [28:0-2-8,0-3-0], [33:0-2-8,0-3-0]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.37	Vert(LL)	n/a	-	n/a	999	244/190
TCDL	10.0	Lumber DOL	1.25	BC	0.35	Vert(TL)	n/a	-	n/a	999	
BCLL	0.0*	Rep Stress Incr	NO	WB	0.13	Horiz(TL)	0.01	22	n/a	n/a	
BCDL	10.0	Code	FBC2023/TPI2014	Matrix-AS							
										Weight: 279 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied, except
2-0-0 oc purlins (10-0-0 max.): 9-13.
BOT CHORD Rigid ceiling directly applied.
WEBS 1 Row at midpt 8-34, 10-33, 11-32, 12-31, 14-30

REACTIONS (size)

1=42-0-0, 22=42-0-0, 23=42-0-0, 24=42-0-0, 25=42-0-0, 26=42-0-0, 27=42-0-0, 28=42-0-0, 29=42-0-0, 30=42-0-0, 31=42-0-0, 32=42-0-0, 33=42-0-0, 34=42-0-0, 35=42-0-0, 36=42-0-0, 37=42-0-0, 38=42-0-0, 39=42-0-0, 40=42-0-0
Max Horiz 1=200 (LC 10)
Max Uplift 1=49 (LC 10), 23=4 (LC 12), 24=23 (LC 1), 26=22 (LC 12), 27=21 (LC 12), 28=20 (LC 12), 29=26 (LC 12), 30=8 (LC 12), 32=2 (LC 12), 34=5 (LC 12), 35=27 (LC 12), 36=20 (LC 12), 37=21 (LC 12), 38=20 (LC 12), 39=25 (LC 12), 40=3 (LC 12)
Max Grav 1=100 (LC 18), 22=189 (LC 1), 23=518 (LC 1), 24=5 (LC 12), 25=194 (LC 1), 26=154 (LC 18), 27=168 (LC 18), 28=165 (LC 18), 29=166 (LC 18), 30=165 (LC 18), 31=171 (LC 24), 32=165 (LC 23), 33=171 (LC 17), 34=170 (LC 17), 35=165 (LC 17), 36=165 (LC 17), 37=164 (LC 17), 38=166 (LC 17), 39=164 (LC 17), 40=178 (LC 23)

FORCES

(lb) - Maximum Compression/Maximum Tension
TOP CHORD 9-10=-97/325, 10-11=-97/325, 11-12=-97/325, 12-13=-97/325, 13-14=-110/353, 14-15=-87/288, 15-16=-60/203, 16-17=-34/124, 17-18=-30/73, 18-19=-48/84, 1-2=-189/189, 2-3=-133/174, 3-4=-119/154, 4-5=-106/136, 5-7=-92/205, 7-8=-89/290, 8-9=-110/352, 19-20=-88/77, 20-21=-104/62, 21-22=-386/102

BOT CHORD

1-40=-71/191, 39-40=-71/191, 38-39=-71/191, 37-38=-71/191, 36-37=-71/191, 35-36=-71/191, 34-35=-71/191, 32-34=-71/191, 31-32=-71/191, 30-31=-71/191, 29-30=-71/191, 27-29=-71/191, 26-27=-71/191, 25-26=-71/191, 24-25=-74/195, 23-24=-74/195, 22-23=-74/359
WEBS 2-40=-120/93, 3-39=-126/127, 4-38=-125/119, 5-37=-124/120, 6-36=-125/118, 7-35=-125/132, 8-34=-130/88, 10-33=-131/0, 11-32=-125/82, 12-31=-131/0, 14-30=-125/95, 15-29=-126/130, 16-28=-125/119, 17-27=-127/121, 18-26=-120/119, 19-25=-132/78, 20-24=-21/39, 21-23=-333/186

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCCL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=42ft; eave=2ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Zone3 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 5) Provide adequate drainage to prevent water ponding.
- 6) All plates are 1.5x4 MT20 unless otherwise indicated.
- 7) Gable requires continuous bottom chord bearing.
- 8) Gable studs spaced at 2'-0" on center.
- 9) This truss has been designed for a 10.0 psf bottom chord live load in accordance with any other live loads.



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Date:

January 19, 2024

Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinstitute.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcsc.com).

MiTek®
16023 Swingley Ridge Rd.
Chesterfield, MO 63017
314.434.1200 / MiTek-US.com

Job	Truss	Truss Type	Qty	Ply	
1223-036	A01	Piggyback Base Supported Gable	1	1	T32673329 Job Reference (optional)

Mayo Truss Company, Inc., Mayo, FL - 32066,

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Page: 2

- 10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 11) All bearings are assumed to be SP No.2 .
- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 49 lb uplift at joint 1, 3 lb uplift at joint 40, 25 lb uplift at joint 39, 20 lb uplift at joint 38, 21 lb uplift at joint 37, 20 lb uplift at joint 36, 27 lb uplift at joint 35, 5 lb uplift at joint 34, 2 lb uplift at joint 32, 8 lb uplift at joint 30, 26 lb uplift at joint 29, 20 lb uplift at joint 28, 21 lb uplift at joint 27, 22 lb uplift at joint 26, 23 lb uplift at joint 24 and 4 lb uplift at joint 23.
- 13) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 14) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

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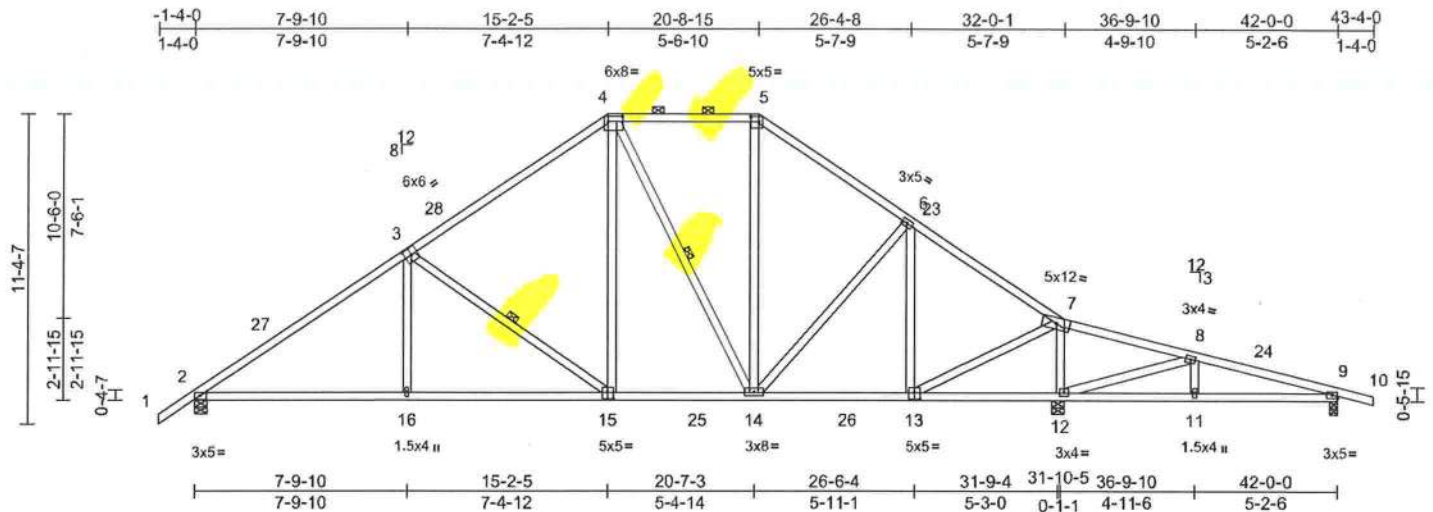
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Chesterfield, MO 63017
314.434.1200 / MiTek-US.com

Job	Truss	Truss Type	Qty	Ply	Job Reference (optional)	T32673330
1223-036	A02	Piggyback Base	17	1		

Mayo Truss Company, Inc., Mayo, FL - 32066,

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Page: 1



Scale = 1:79.9

Plate Offsets (X, Y): [2:0-2-9,0-1-8], [3:0-3-0,0-3-4], [4:0-6-4,0-2-4], [5:0-3-4,0-2-4], [13:0-2-8,0-3-0], [15:0-2-8,0-3-0]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.55	Vert(LL)	-0.12	16-19	>999	240	244/190
TCDL	10.0	Lumber DOL	1.25	BC	0.73	Vert(CT)	-0.24	16-19	>999	180	
BCLL	0.0*	Rep Stress Incr	YES	WB	0.38	Horz(CT)	0.05	12	n/a	n/a	
BCDL	10.0	Code	FBC2023/TPI2014	Matrix-AS							
										Weight: 248 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied, except
2-0-0 oc purlins (5-10-4 max.): 4-5.
BOT CHORD Rigid ceiling directly applied.
WEBS 1 Row at midpt 3-15, 4-14

REACTIONS

(size) 2=0-5-8, 9=0-3-8, 12=0-5-8
Max Horiz 2=205 (LC 10)
Max Uplift 2=32 (LC 12), 9=34 (LC 12)
Max Grav 2=1475 (LC 17), 9=360 (LC 24), 12=2099 (LC 18)

FORCES

(lb) - Maximum Compression/Maximum Tension
TOP CHORD 4-5=951/289, 5-6=1210/284, 6-7=1073/186, 7-8=-19/577, 8-9=-365/37, 9-10=0/19, 1-2=0/44, 2-4=-2026/291
BOT CHORD 2-16=-77/1762, 14-16=-78/1759, 12-14=-622/854, 11-12=0/323, 9-11=0/323
WEBS 3-16=0/330, 3-15=-748/139, 4-15=-3/686, 4-14=-343/65, 5-14=-29/381, 6-14=-21/197, 6-13=-526/123, 7-13=-119/1617, 7-12=-1671/205, 8-12=-852/56, 8-11=0/205

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-22; Vult=130mph (3-second gust)
Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft;
B=45ft; L=42ft; eave=5ft; Cat. II; Exp B; Enclosed;
MWFRS (directional) and C-C Zone3 -1-4-0 to 2-10-6, Zone1 2-10-6 to 15-2-5, Zone3 15-2-5 to 20-8-15, Zone2 20-8-15 to 26-8-3, Zone1 26-8-3 to 43-4-0 zone;
cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) Provide adequate drainage to prevent water ponding.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) All bearings are assumed to be SP No.2.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 32 lb uplift at joint 2 and 34 lb uplift at joint 9.
- 9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard



Joaquin Velez PE No.68182
MiTek Inc. DBA MiTek USA FL Cert 6634
16023 Swingley Ridge Rd. Chesterfield, MO 63017
Date:

January 19,2024



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MJ-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpins.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcsccomponents.com)

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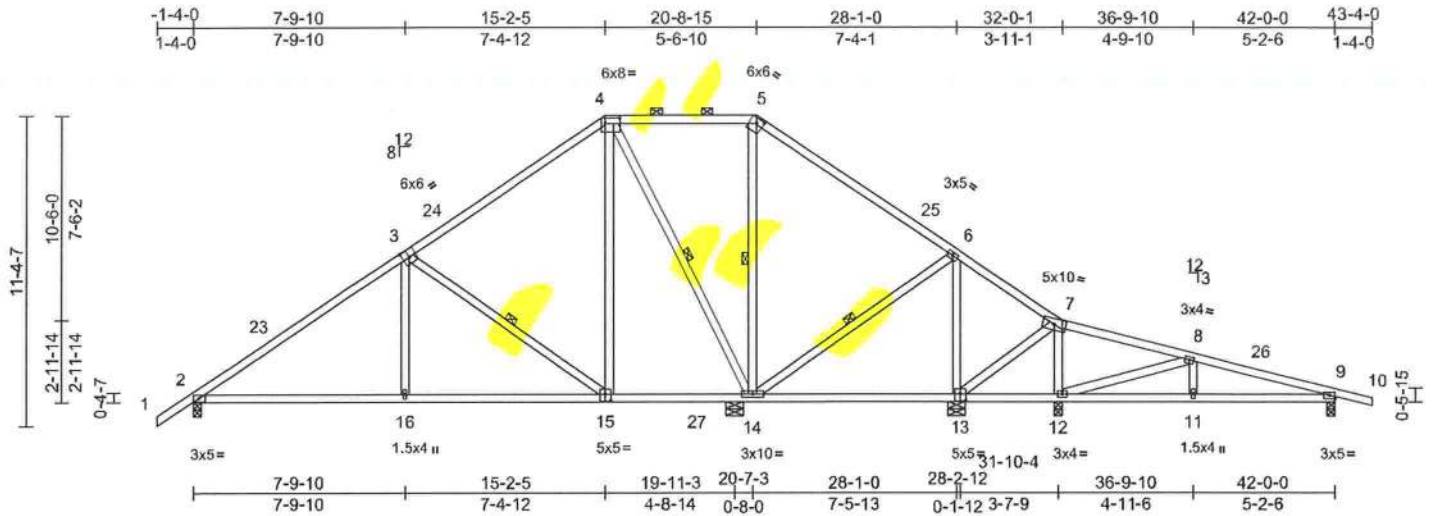
16023 Swingley Ridge Rd.
Chesterfield, MO 63017
314.434.1200 / MiTek-US.com

Job	Truss	Truss Type	Qty	Ply	Job Reference (optional)
1223-036	A03	Piggyback Base	1	1	T32673331

Mayo Truss Company, Inc., Mayo, FL - 32066,

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Page: 1



Scale = 1:79.9

Plate Offsets (X, Y): [2:0-2-9,0-1-8], [3:0-3-0,0-3-4], [4:0-6-4,0-2-4], [5:0-3-0,0-2-3], [13:0-2-8,0-3-0], [15:0-2-8,0-3-0]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.56	Vert(LL)	-0.11	16-19	>999	240	244/190
TCDL	10.0	Lumber DOL	1.25	BC	0.59	Vert(CT)	-0.21	16-19	>999	180	
CLL	0.0*	Rep Stress Incr	YES	WB	0.61	Horz(CT)	0.02	14	n/a	n/a	
BCDL	10.0	Code	FBC2023/TPI2014	Matrix-AS							
Weight: 245 lb FT = 20%											

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied, except 2-0-0 oc purlins (10-0-0 max.): 4-5.
BOT CHORD Rigid ceiling directly applied.
WEBS 1 Row at midpt 3-15, 4-14, 5-14, 6-14

REACTIONS

(size) 2=0-3-8, 9=0-3-8, 12=0-3-8, 13=0-7-12, 14=0-7-12
Max Horiz 2=-205 (LC 10)
Max Uplift 2=-31 (LC 12), 9=-35 (LC 12), 13=-6 (LC 12), 14=-1 (LC 12)
Max Grav 2=850 (LC 17), 9=408 (LC 18), 12=677 (LC 26), 13=333 (LC 18), 14=1732 (LC 17)

FORCES

(lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/44, 2-4=-914/167, 4-5=0/323, 5-6=0/462, 6-7=0/292, 7-8=0/370, 8-9=-547/58, 9-10=0/19
BOT CHORD 2-16=-5/839, 14-16=-21/835, 12-14=-353/93, 11-12=0/505, 9-11=0/505
WEBS 3-16=0/338, 3-15=-761/140, 4-15=-8/677, 4-14=-1028/125, 5-14=-550/162, 6-14=-151/109, 6-13=-326/98, 7-13=0/259, 7-12=-361/24, 8-12=-880/59, 8-11=0/214

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=42ft; eave=5ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Zone3 -1-4-0 to 2-10-6, Zone1 2-10-6 to 15-2-5, Zone3 15-2-5 to 20-8-15, Zone2 20-8-15 to 26-8-3, Zone1 26-8-3 to 43-4-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) Provide adequate drainage to prevent water ponding.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) All bearings are assumed to be SP No.2 .
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 31 lb uplift at joint 2, 1 lb uplift at joint 14, 6 lb uplift at joint 13 and 35 lb uplift at joint 9.
- 9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard



Joaquin Velez PE No.68182
MiTek Inc. DBA MiTek USA FL Cert 6634
16023 Swingley Ridge Rd. Chesterfield, MO 63017
Date:

January 19, 2024



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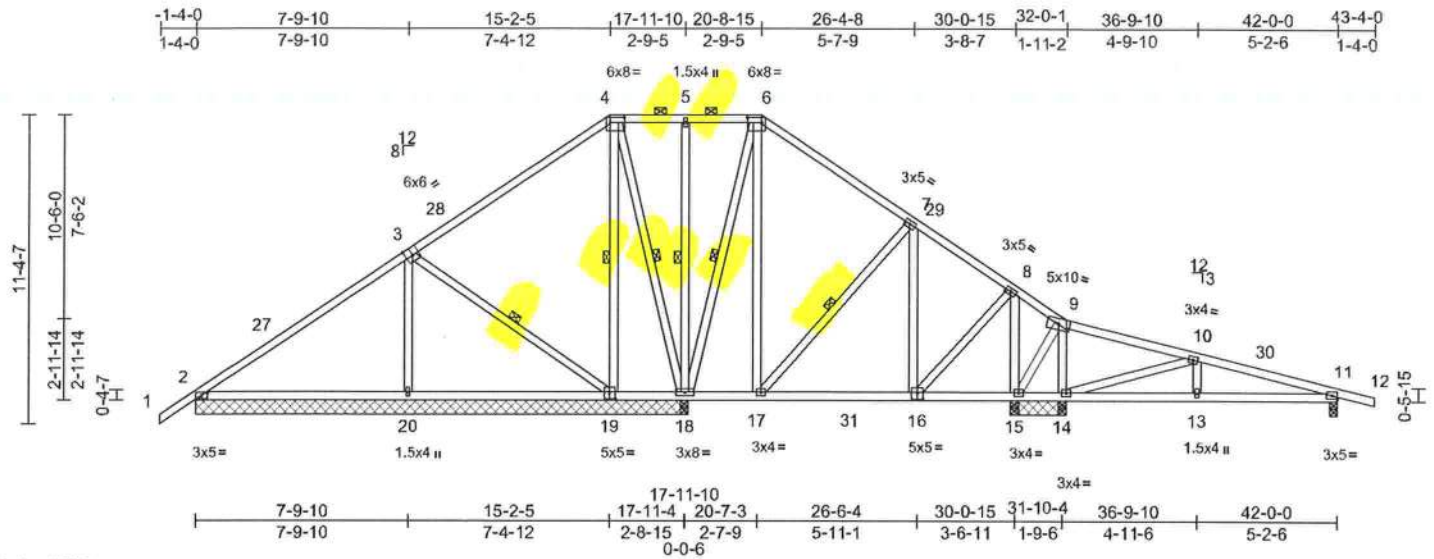
MiTek
16023 Swingley Ridge Rd.
Chesterfield, MO 63017
314.434.1200 / MiTek-US.com

Job	Truss	Truss Type	Qty	Ply	Job Reference (optional)
1223-036	A04	Piggyback Base	1	1	T32673332

Mayo Truss Company, Inc., Mayo, FL - 32066,

Run: 8.73 S Jan 4 2024 Print: 8.730 S Jan 4 2024 MiTek Industries, Inc. Thu Jan 18 13:02:37
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Page: 1



Scale = 1:79.9

Plate Offsets (X, Y): [2:0-2-9,0-1-8], [3:0-3-0,Edge], [4:0-6-4,0-2-4], [6:0-6-4,0-2-4], [16:0-2-8,0-3-0], [19:0-2-8,0-3-0]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.57	Vert(LL)	-0.09	20-23	>989	240	244/190
TCDL	10.0	Lumber DOL	1.25	BC	0.51	Vert(CT)	-0.19	20-23	>488	180	
BCLL	0.0*	Rep Stress Incr	YES	WB	0.39	Horz(CT)	0.01	11	n/a	n/a	
BCDL	10.0	Code	FBC2023/TPI2014	Matrix-AS							
Weight: 285 lb FT = 20%											

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied, except
2-0-0 oc purlins (10-0-0 max.): 4-6.
BOT CHORD Rigid ceiling directly applied.
WEBS 1 Row at midpt 3-19, 4-19, 7-17, 5-18, 4-18, 6-18

REACTIONS

(size) 2=18-1-0, 11=0-3-8, 14=2-0-14, 15=2-0-14, 18=18-1-0, 19=18-1-0, 20=18-1-0, 21=18-1-0
Max Horiz 2=205 (LC 10), 21=205 (LC 10)
Max Uplift 2=42 (LC 12), 11=34 (LC 12), 15=16 (LC 12), 18=23 (LC 12), 21=42 (LC 12)
Max Grav 2=421 (LC 23), 11=407 (LC 18), 14=691 (LC 18), 15=622 (LC 18), 18=705 (LC 18), 19=463 (LC 17), 20=661 (LC 17), 21=421 (LC 23)

FORCES

(lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/44, 2-4=253/136, 4-5=59/141, 5-6=59/141, 6-7=192/135, 7-8=217/76, 8-9=0/275, 9-10=0/359, 10-11=538/59, 11-12=0/19
BOT CHORD 2-20=43/195, 18-20=92/200, 17-18=54/152, 15-17=229/154, 14-15=341/62, 13-14=0/496, 11-13=0/496
WEBS 3-20=352/103, 3-19=191/127, 4-19=162/55, 6-17=13/295, 7-17=143/109, 7-16=222/70, 9-14=335/27, 10-14=858/60, 5-18=137/66, 4-18=164/67, 6-18=508/96, 8-15=750/41, 9-15=0/206, 8-16=7/534, 10-13=0/207

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=42ft; eave=5ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Zone3 -1-4-0 to 2-10-6, Zone1 2-10-6 to 15-2-5, Zone3 15-2-5 to 20-8-15, Zone2 20-8-15 to 26-8-3, Zone1 26-8-3 to 43-4-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- All bearings are assumed to be SP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 42 lb uplift at joint 2, 23 lb uplift at joint 18, 16 lb uplift at joint 15, 34 lb uplift at joint 11 and 42 lb uplift at joint 2.
- This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard



Joaquin Velez PE No.68182
MiTek Inc. DBA MiTek USA FL Cert 6634
16023 Swingley Ridge Rd. Chesterfield, MO 63017
Date:

January 19,2024

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcsccomponents.com)

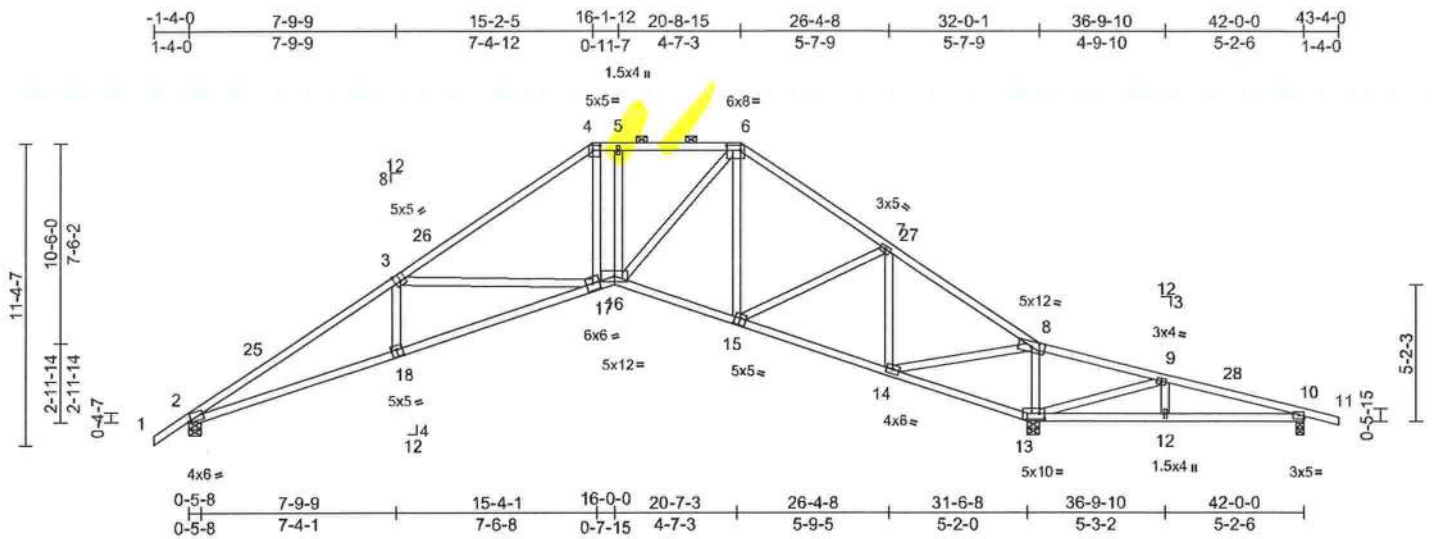
MiTek®
16023 Swingley Ridge Rd.
Chesterfield, MO 63017
314.434.1200 / MiTek-US.com

Job	Truss	Truss Type	Qty	Ply	Job Reference (optional)	T32673333
1223-036	A05	Piggyback Base	8	1		

Mayo Truss Company, Inc., Mayo, FL - 32066.

Run: 8.73 S Jan 4 2024 Print: 8.730 S Jan 4 2024 MiTek Industries, Inc. Thu Jan 18 13:02:38
ID:08eFRgYDQ3y48URTJkHwHy7b7-RfC7P5B70Hq3NSgPqnL8w3uITXbGKwCDoI7J4JC7f

Page: 1



Scale = 1:81.9

Plate Offsets (X, Y): [2:0-1-2,Edge], [3:0-2-8,0-3-4], [4:0-3-4,0-2-4], [6:0-6-4,0-2-4], [13:0-2-8,0-2-8], [15:0-2-8,0-3-0], [18:0-2-8,0-3-4]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.53	Vert(LL)	-0.17	17-18	>999	240	244/190
TCDL	10.0	Lumber DOL	1.25	BC	0.72	Vert(CT)	-0.38	17-18	>999	180	
BCLL	0.0*	Rep Stress Incr	YES	WB	0.87	Horz(CT)	0.27	13	n/a	n/a	
BCDL	10.0	Code	FBC2023/TPI2014	Matrix-AS							
Weight: 226 lb FT = 20%											

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied, except
2-0-0 oc purlins (4-2-13 max.): 4-6.
BOT CHORD Rigid ceiling directly applied.

REACTIONS

(size) 2=0-5-8, 10=0-3-8, 13=0-5-8
Max Horiz 2=-205 (LC 10)
Max Uplift 2=-38 (LC 12), 10=-66 (LC 23)
Max Grav 2=1215 (LC 1), 10=183 (LC 24),
13=2187 (LC 1)

FORCES

(lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/44, 2-4=-2983/275, 4-5=-1571/235,
5-6=-1570/236, 6-7=-1283/249,
7-8=-671/158, 8-9=-74/1430, 9-10=-97/624,
10-11=0/19
BOT CHORD 2-17=-160/2577, 16-17=0/1600,
14-16=-19/1037, 13-14=-1683/168,
12-13=-587/96, 10-12=-587/96
WEBS 3-18=0/294, 6-15=-300/30, 7-15=0/568,
7-14=-942/149, 9-12=0/239, 4-17=-15/631,
3-17=-869/214, 5-16=-29/47, 6-16=-46/905,
8-13=-1323/179, 8-14=-159/2097,
9-13=-929/51

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-22; Vult=130mph (3-second gust)
Vasd=101mph; TCCL=6.0psf; BCDL=6.0psf; h=15ft;
B=45ft; L=42ft; eave=5ft; Cat. II; Exp B; Enclosed;
MWFRS (directional) and C-C Zone3 1-4-0 to 2-10-6,
Zone1 2-10-6 to 15-2-5, Zone3 15-2-5 to 20-8-15, Zone2
20-8-15 to 26-8-3, Zone1 26-8-3 to 43-4-0 zone;
cantilever left and right exposed; end vertical left and
right exposed; C-C for members and forces & MWFRS
for reactions shown; Lumber DOL=1.60 plate grip
DOL=1.60
- 3) Building Designer / Project engineer responsible for
verifying applied roof live load shown covers rain loading
requirements specific to the use of this truss component.
- 4) Provide adequate drainage to prevent water ponding.
- 5) This truss has been designed for a 10.0 psf bottom
chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf
on the bottom chord in all areas where a rectangle
3-06-00 tall by 2-00-00 wide will fit between the bottom
chord and any other members.
- 7) All bearings are assumed to be SP No.2.
- 8) Bearing at joint(s) 2 considers parallel to grain value
using ANSI/TPI 1 angle to grain formula. Building
designer should verify capacity of bearing surface.
- 9) Provide mechanical connection (by others) of truss to
bearing plate capable of withstanding 66 lb uplift at joint
10 and 38 lb uplift at joint 2.
- 10) This truss design requires that a minimum of 7/16"
structural wood sheathing be applied directly to the top
chord and 1/2" gypsum sheetrock be applied directly to
the bottom chord.
- 11) Graphical purlin representation does not depict the size
or the orientation of the purlin along the top and/or
bottom chord.

LOAD CASE(S) Standard



Joaquin Velez PE No.68182
MiTek Inc. DBA MiTek USA FL Cert 6634
16023 Swingley Ridge Rd. Chesterfield, MO 63017
Date:

January 19,2024

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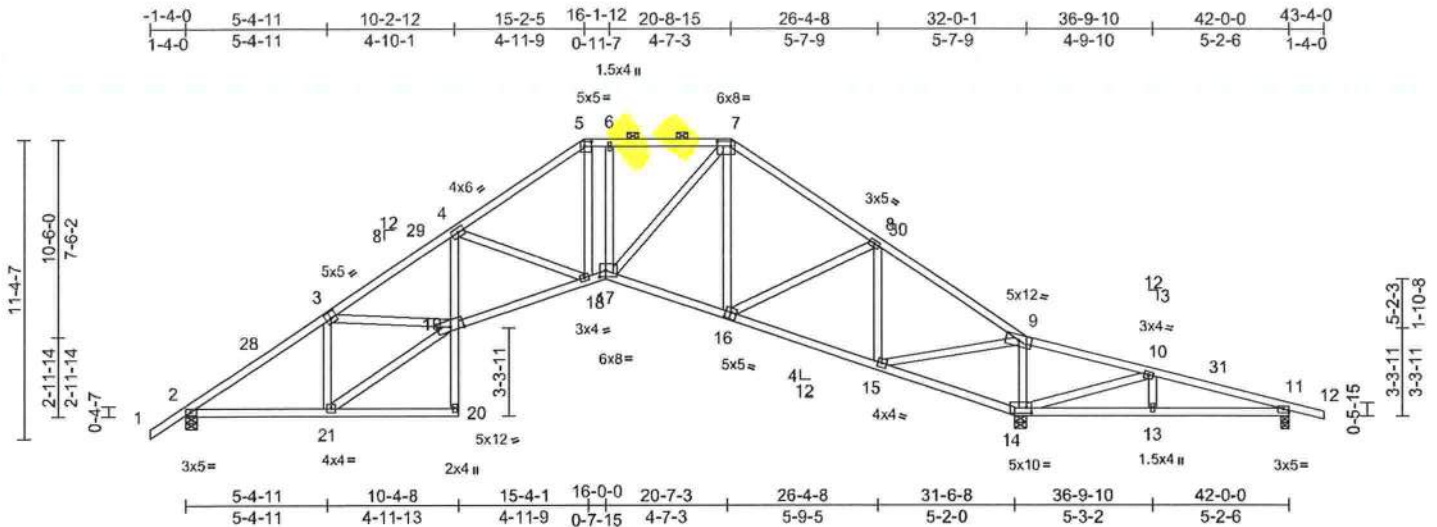
MiTek®
16023 Swingley Ridge Rd.
Chesterfield, MO 63017
314.434.1200 / MiTek-US.com

Job	Truss	Truss Type	Qty	Ply	Job Reference (optional)	T32673334
1223-036	A06	Piggyback Base	1	1		

Mayo Truss Company, Inc., Mayo, FL - 32066,

Run: 8.73 S Jan 4 2024 Print: 8.730 S Jan 4 2024 MiTek Industries, Inc. Thu Jan 18 13:02:38
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Page: 1



Scale = 1:82.8

Plate Offsets (X, Y): [2:0-2-9,0-1-8], [3:0-2-8,0-3-0], [5:0-3-4,0-2-4], [7:0-6-4,0-2-4], [14:0-2-8,0-2-8], [16:0-2-8,0-3-0], [17:0-2-12,0-3-0]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.49	Vert(LL)	-0.14	16-17	>999	240	244/190
TCDL	10.0	Lumber DOL	1.25	BC	0.68	Vert(CT)	-0.27	16-17	>999	180	
BCLL	0.0*	Rep Stress Incr	YES	WB	0.45	Horz(CT)	0.21	14	n/a	n/a	
BCDL	10.0	Code	FBC2023/TPI2014	Matrix-AS							
Weight: 248 lb FT = 20%											

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
EBS 2x4 SP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied, except
2-0-0 oc purlins (4-2-14 max.): 5-7.

BOT CHORD Rigid ceiling directly applied.

REACTIONS

(size) 2=0-5-8, 11=0-3-8, 14=0-5-8
Max Horiz 2=205 (LC 10)
Max Uplift 2=37 (LC 12), 11=48 (LC 12)
Max Grav 2=1230 (LC 1), 11=214 (LC 24), 14=2130 (LC 1)

FORCES

(lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/44, 2-4=-2672/269, 4-5=-1987/249, 5-6=-1604/236, 6-7=-1603/238, 7-8=-1338/249, 8-9=-765/159, 9-10=-73/1266, 10-11=-97/494, 11-12=0/19, 12-13=-77/1433, 13-14=-1/26, 14-15=-19/20/86, 15-16=0/353, 16-17=-94/2314, 17-18=0/1609, 18-19=-20/1084, 19-20=-1515/167, 20-21=-462/94, 21-22=-462/94
BOT CHORD 3-21=-873/115, 19-21=-95/1734, 3-19=0/729, 7-16=-263/28, 8-16=0/533, 8-15=-904/148, 10-13=0/236, 5-18=-67/740, 6-17=-55/82, 7-17=-39/882, 4-18=-651/177, 9-14=-1324/179, 9-15=-158/2015, 10-14=-908/51
WEBS

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-22; Vult=130mph (3-second gust)
Vasd=101mph; TCCL=6.0psf; BCDL=6.0psf; h=15ft;
B=45ft; L=42ft; eave=5ft; Cat. II; Exp B; Enclosed;
MWFRS (directional) and C-C Zone3 -1-4-0 to 2-10-6;
Zone1 2-10-6 to 15-2-5, Zone3 15-2-5 to 20-8-15, Zone2
20-8-15 to 26-8-3, Zone1 26-8-3 to 43-4-0 zone;
cantilever left and right exposed; end vertical left and
right exposed; C-C for members and forces & MWFRS
for reactions shown; Lumber DOL=1.60 plate grip
DOL=1.60
- 3) Building Designer / Project engineer responsible for
verifying applied roof live load shown covers rain loading
requirements specific to the use of this truss component.
- 4) Provide adequate drainage to prevent water ponding.
- 5) This truss has been designed for a 10.0 psf bottom
chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf
on the bottom chord in all areas where a rectangle
3-06-00 tall by 2-00-00 wide will fit between the bottom
chord and any other members.
- 7) All bearings are assumed to be SP No.2.
- 8) Provide mechanical connection (by others) of truss to
bearing plate capable of withstanding 37 lb uplift at joint
2 and 48 lb uplift at joint 11.
- 9) This truss design requires that a minimum of 7/16"
structural wood sheathing be applied directly to the top
chord and 1/2" gypsum sheetrock be applied directly to
the bottom chord.
- 10) Graphical purlin representation does not depict the size
or the orientation of the purlin along the top and/or
bottom chord.

LOAD CASE(S) Standard



Joaquin Velez PE No.68182
MiTek Inc. DBA MiTek USA FL Cert 6634
16023 Swingley Ridge Rd. Chesterfield, MO 63017
Date:

January 19, 2024



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 rev. 1/2/2023 BEFORE USE.

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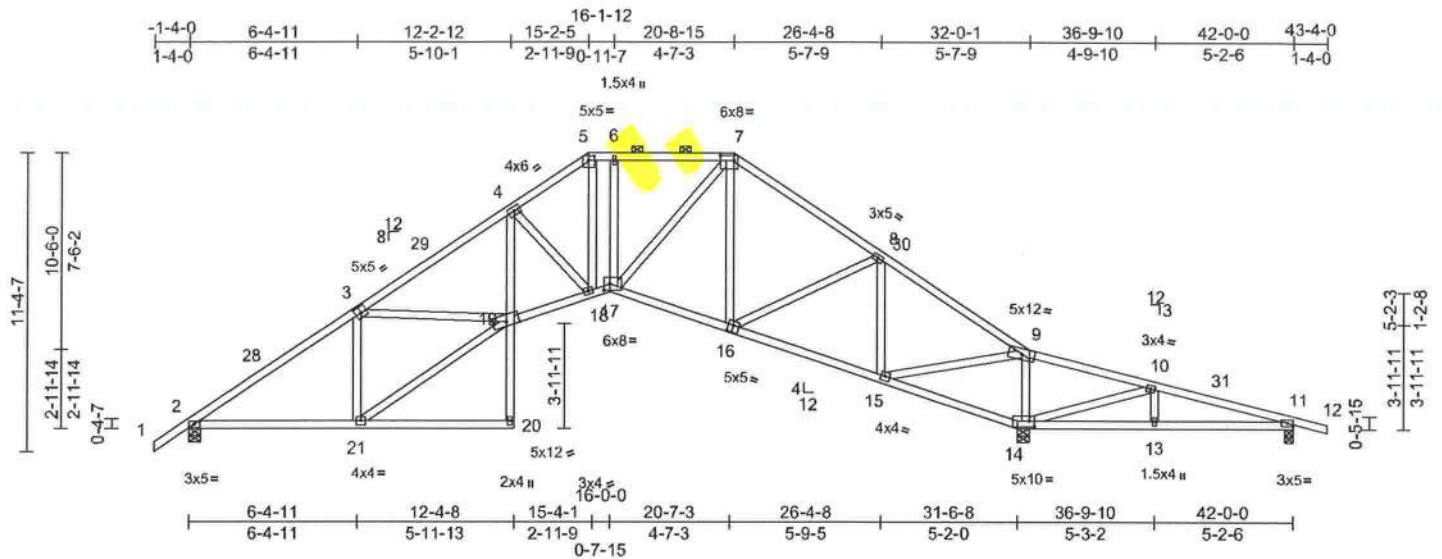
MiTek
16023 Swingley Ridge Rd.
Chesterfield, MO 63017
314.434.1200 / MiTek-US.com

Job	Truss	Truss Type	Qty	Ply	Job Reference (optional)	T32673335
1223-036	A07	Piggyback Base	1	1		

Mayo Truss Company, Inc., Mayo, FL - 32066,

Run: 8.73 S Jan 4 2024 Print: 8.730 S Jan 4 2024 MiTek Industries, Inc. Thu Jan 18 13:02:39
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Page: 1



Scale = 1:82.8

Plate Offsets (X, Y): [2:0-2-9,0-1-8], [3:0-2-8,0-3-0], [5:0-2-8,0-1-13], [7:0-6-4,0-2-4], [14:0-2-8,0-2-8], [16:0-2-8,0-3-0], [17:0-2-12,0-3-0]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.48	Vert(LL)	-0.13	16-17	>999	240	244/190
TCDL	10.0	Lumber DOL	1.25	BC	0.73	Vert(CT)	-0.26	16-17	>999	180	
BCLL	0.0*	Rep Stress Incr	YES	WB	0.45	Horz(CT)	0.19	14	n/a	n/a	
BCDL	10.0	Code	FBC2023/TPI2014	Matrix-AS							
										Weight: 252 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied, except
2-0-0 oc purlins (4-2-9 max.): 5-7.
BOT CHORD Rigid ceiling directly applied.

REACTIONS

(size) 2=0-5-8, 11=0-3-8, 14=0-5-8
Max Horiz 2=205 (LC 10)
Max Uplift 2=36 (LC 12), 11=47 (LC 12)
Max Grav 2=1234 (LC 1), 11=224 (LC 24), 14=2113 (LC 1)

FORCES

(lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/44, 2-4=-2444/262, 4-5=-1963/265, 5-6=-1615/238, 6-7=-1616/241, 7-8=-1354/250, 8-9=-794/162, 9-10=-671/1218, 10-11=-98/457, 11-12=0/19
BOT CHORD 2-21=-74/1402, 20-21=0/25, 19-20=0/101, 4-19=-6/368, 18-19=-29/2056, 17-18=0/1606, 15-17=-23/1098, 14-15=-1465/161, 13-14=-426/89, 11-13=-426/89
WEBS 3-21=-814/129, 19-21=-98/1693, 3-19=0/550, 7-16=-253/27, 8-16=0/523, 8-15=-892/147, 10-13=0/235, 5-18=-121/831, 6-17=-85/110, 7-17=-37/878, 4-18=-523/163, 9-14=-1324/179, 9-15=-155/1991, 10-14=-902/51

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=42ft; eave=5ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Zone3 -1-4-0 to 2-10-6, Zone1 2-10-6 to 15-2-5, Zone3 15-2-5 to 20-8-15, Zone2 20-8-15 to 26-8-3, Zone1 26-8-3 to 43-4-0 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) Provide adequate drainage to prevent water ponding.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 7) All bearings are assumed to be SP No.2 .
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 36 lb uplift at joint 2 and 47 lb uplift at joint 11.
- 9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard



Joaquin Velez PE No.68182
MiTek Inc. DBA MiTek USA FL Cert 6634
16023 Swingley Ridge Rd. Chesterfield, MO 63017
Date:

January 19,2024

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

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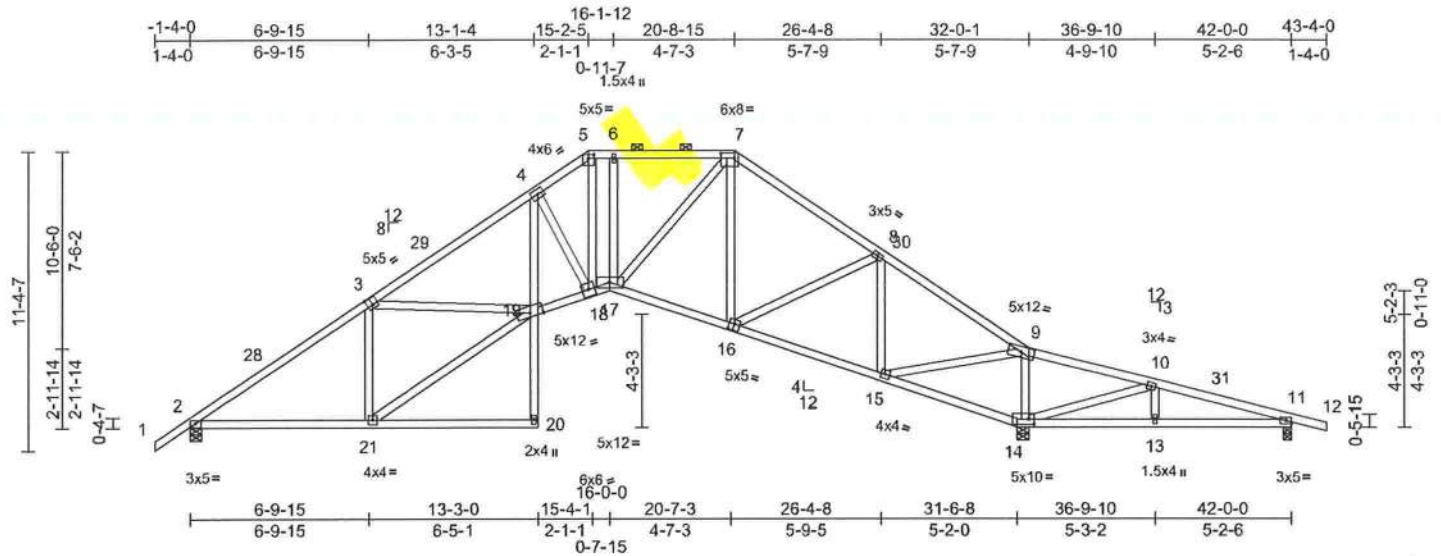
MiTek®
16023 Swingley Ridge Rd.
Chesterfield, MO 63017
314.434.1200 / MiTek-US.com

Job	Truss	Truss Type	Qty	Ply	Job Reference (optional)
1223-036	A08	Piggyback Base	1	1	T32673336

Mayo Truss Company, Inc., Mayo, FL - 32066,

Run: 8.73 S Jan 4 2024 Print: 8.730 S Jan 4 2024 MiTek Industries, Inc. Thu Jan 18 13:02:39
ID:YvyZYmKQ4d3OsnP8B_n9lty7ITg-RfC7P5B70Hq3NSgPqnL8w3ulTXbGKWrCdoi7J4zJC7f

Page: 1



Scale = 1:82.8

Plate Offsets (X, Y): [2:0-2-9,0-1-8], [3:0-2-8,0-3-4], [5:0-2-8,0-1-13], [7:0-6-4,0-2-4], [14:0-2-8,0-2-8], [16:0-2-8,0-3-0]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.47	Vert(LL)	-0.12	16-17	>999	240	MT20	244/190
CDL	10.0	Lumber DOL	1.25	BC	0.72	Vert(CT)	-0.25	16-17	>999	180		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.45	Horz(CT)	0.18	14	n/a	n/a		
BCDL	10.0	Code	FBC2023/TPI2014	Matrix-AS							Weight: 255 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied, except
2-0-0 oc purlins (4-2-11 max.): 5-7.
BOT CHORD Rigid ceiling directly applied.

REACTIONS

(size) 2=0-5-8, 11=0-3-8, 14=0-5-8
Max Horiz 2=205 (LC 10)
Max Uplift 2=36 (LC 12), 11=46 (LC 12)
Max Grav 2=1236 (LC 1), 11=231 (LC 24), 14=2103 (LC 1)

FORCES

(lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/44, 2-4=-2345/259, 4-5=-1960/274, 5-6=-1625/239, 6-7=-1626/243, 7-8=-1363/251, 8-9=-811/164, 9-10=-64/1187, 10-11=-98/433, 11-12=0/19
BOT CHORD 2-21=-73/1388, 20-21=0/27, 19-20=0/108, 4-19=-15/372, 18-19=-2/1953, 17-18=0/1612, 15-17=-24/1106, 14-15=-1434/157, 13-14=-403/87, 11-13=-403/87
WEBS 3-21=-789/135, 19-21=-98/1673, 3-19=0/475, 7-16=-250/27, 8-16=0/516, 8-15=-884/146, 10-13=0/234, 5-18=-157/871, 6-17=-92/118, 4-18=-506/179, 7-17=-36/881, 9-14=-1324/179, 9-15=-154/1977, 10-14=-898/51

NOTES

- Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=42ft; eave=5ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Zone3 -1-4-0 to 2-10-6, Zone1 2-10-6 to 15-2-5, Zone3 15-2-5 to 20-8-15, Zone2 20-8-15 to 26-8-3, Zone1 26-8-3 to 43-4-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 36 lb uplift at joint 2 and 46 lb uplift at joint 11.
- This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard



Joaquin Velez PE No.68182
MiTek Inc. DBA MiTek USA FL Cert 6634
16023 Swingley Ridge Rd. Chesterfield, MO 63017
Date:

January 19, 2024

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinet.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbccomponents.com)

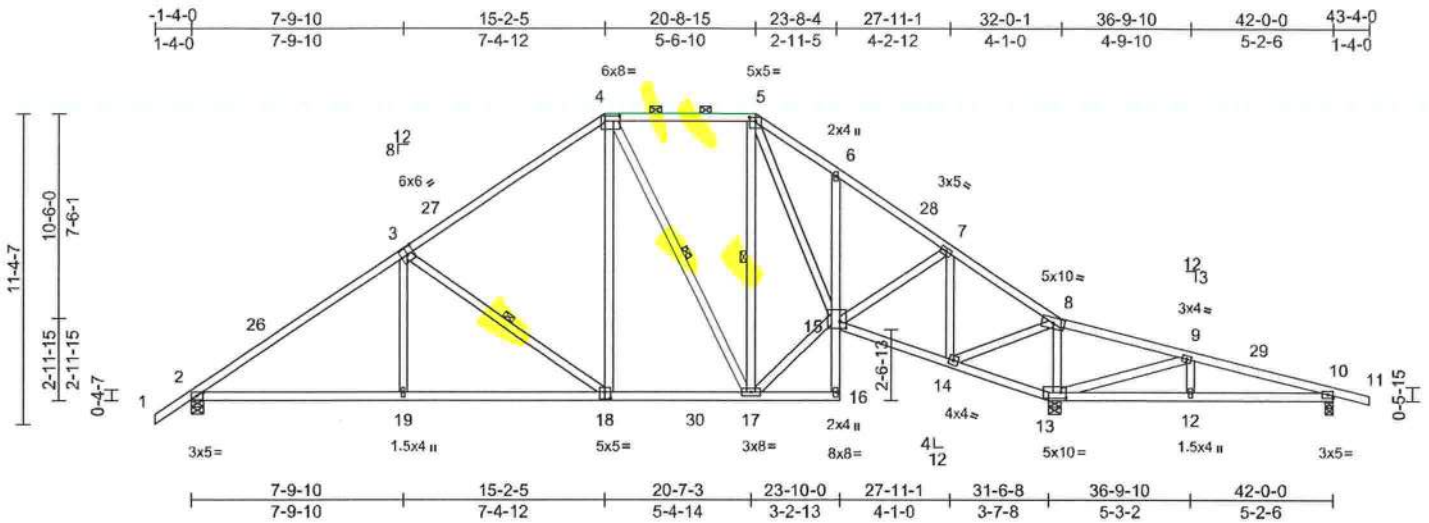
MiTek
16023 Swingley Ridge Rd.
Chesterfield, MO 63017
314.434.1200 / MiTek-US.com

Job	Truss	Truss Type	Qty	Ply	Job Reference (optional)
1223-036	A09	Piggyback Base	1	1	T32673337

Mayo Truss Company, Inc., Mayo, FL - 32066,

Run: 8.73 S Jan 4 2024 Print: 8.730 S Jan 4 2024 MiTek Industries, Inc. Thu Jan 18 13:02:40
ID:Rn9fSC6sQbYZkk1Oa6eGKNy7jNg-RfC7PsB70Hq3NSgPqnl8w3ulTXbGKWrcDol7J4zJC7f

Page: 1



Mayo Truss Company, Inc., Mayo, FL - 32066, Run: 8.73 S Jan 4 2024 Print: 8.730 S Jan 4 2024 MiTek Industries, Inc. Thu Jan 18 13:02:40 Page: 1
ID:IqKvc_HcviXJKFBL8a0l8ly7lph-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrcDoI7J4zJC?/



LUMBER		Max Grav	2=184 (LC 1), 24=275 (LC 1), 26=492 (LC 1), 27=77 (LC 17), 28=153 (LC 18), 29=155 (LC 18), 30=168 (LC 18), 31=164 (LC 18), 32=167 (LC 18), 33=163 (LC 18), 34=152 (LC 24), 35=166 (LC 24), 36=153 (LC 17), 37=167 (LC 17), 38=166 (LC 17), 39=165 (LC 17), 40=164 (LC 17), 41=164 (LC 17), 42=164 (LC 1), 43=148 (LC 17), 44=184 (LC 1), 47=275 (LC 1)	WEBS	3-43=-115/98, 4-42=-123/114, 5-41=-125/120, 6-40=-124/120, 7-39=-125/119, 8-38=-126/132, 9-37=-127/89, 11-36=-113/2, 12-35=-126/81, 13-34=-112/3, 15-33=-123/96, 16-32=-126/130, 17-31=-125/119, 18-30=-126/121, 19-29=-120/120, 20-28=-95/143, 21-27=-29/44, 22-26=-322/181, 20-27=-118/22
TOP CHORD	2x4 SP No.2				
BOT CHORD	2x4 SP No.2				
WEBS	2x4 SP No.2				
OTHERS	2x4 SP No.2				
BRACING					
TOP CHORD	Structural wood sheathing directly applied, except 2-0-0 oc purlins (6-0-0 max.): 10-14, Rigid ceiling directly applied.				
BOT CHORD					
WEBS	1.9x4 at midpt. 9.37-44.26, 12.25				
				NOTES	1) Unbalanced roof live loads have been considered for

VEBS	1 Row at Impt	9-57, 11-38; 12-33, 13-34, 15-33	FORCES	(lb) - Maximum Compression/Maximum Tension	1) Wind: ASCE 7-22; Vult=130mph (3-second gust) this design.
REACTIONS	(size)	2=42-0-0, 24=42-0-0, 26=42-0-0, 27=42-0-0, 28=42-0-0, 29=42-0-0, 30=42-0-0, 31=42-0-0, 32=42-0-0, 33=42-0-0, 34=42-0-0, 35=42-0-0, 36=42-0-0, 37=42-0-0, 38=42-0-0, 39=42-0-0, 40=42-0-0, 41=42-0-0, 42=42-0-0, 43=42-0-0, 44=42-0-0, 47=42-0-0	TOP CHORD	1-2=0/44, 2-3=-188/164, 3-4=-139/145, 4-5=-132/134, 5-6=-119/115, 6-8=-108/194, 8-9=99/280, 9-10=-119/341, 10-11=-105/316, 11-12=-105/316, 12-13=-105/316, 13-14=-105/316, 14-15=-120/342, 15-16=98/277, 16-17=-70/192, 17-18=-55/112, 18-19=-64/45, 19-20=-74/62, 20-21=-23/76, 21-22=-39/56, 22-24=-108/100, 24-25=0/19	2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDD=6.0psf; BCDD=6.0psf; h=15ft; B=45ft; L=42ft; eave=2ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Zone3 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60
	Max Horiz	2=-206 (LC 10), 44=-206 (LC 10)	BOT CHORD	2-43=-63/206, 42-43=-63/206, 41-42=-63/206, 40-41=-63/206, 39-40=-63/206, 38-39=-63/206, 37-38=-63/206, 35-37=-63/206, 34-35=-63/206, 33-34=-63/206, 32-33=-63/206, 30-32=-63/206, 29-30=-63/206, 28-29=-63/206, 27-28=-67/123, 26-27=-61/136, 24-26=-61/136	
	Max Uplift	2=-18 (LC 8), 24=-28 (LC 12), 27=-25 (LC 11), 28=-32 (LC 12), 29=-22 (LC 12), 30=-21 (LC 12), 31=-20 (LC 12), 32=-25 (LC 12), 33=-9 (LC 12), 35=-2 (LC 12), 37=-6 (LC 12), 38=-26 (LC 12), 39=-20 (LC 12), 40=-21 (LC 12), 41=-20 (LC 12), 42=-21 (LC 12), 44=-18 (LC 8), 47=-28 (LC 12)			



Joaquin Velez PE No.68182
MITek Inc. DBA MITek USA FL Cert 6634
16023 Swingley Ridge Rd. Chesterfield, MO 63017
Date:

January 19, 2024

Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Building designers are required to verify the applicability of design parameters and properly incorporate this design into the overall building design. Building designers are required to verify the applicability of design parameters and properly incorporate this design into the overall building design. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and D58-22** available from Truss Plate Institute (www.tpiinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcsccomponents.com)

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Chesterfield, MO 63017
314.434.1200 / MiTek-US.com

Job	Truss	Truss Type	Qty	Ply	
1223-036	A10	Piggyback Base Supported Gable	1	1	T32673338
			Job Reference (optional)		

Mayo Truss Company, Inc., Mayo, FL - 32066,

Run: 8.73 S Jan 4 2024 Print: 8.730 S Jan 4 2024 MiTek Industries, Inc. Thu Jan 18 13:02:40
ID:IqKvq_HcviXJKFBL8a0l8ly7lph-RfC?PsB70Hq3NSgPqnL6w3ulTXbGKWrcDoi7J4zJC?i

Page: 2

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 5) Provide adequate drainage to prevent water ponding.
- 6) All plates are 1.5x4 MT20 unless otherwise indicated.
- 7) Gable requires continuous bottom chord bearing.
- 8) Gable studs spaced at 2'-0" oc.
- 9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3'-06"-00" tall by 2'-00"-00" wide will fit between the bottom chord and any other members.
- 11) All bearings are assumed to be SP No.2 .
- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 18 lb uplift at joint 2, 28 lb uplift at joint 24, 21 lb uplift at joint 42, 20 lb uplift at joint 41, 21 lb uplift at joint 40, 20 lb uplift at joint 39, 26 lb uplift at joint 38, 6 lb uplift at joint 37, 2 lb uplift at joint 35, 9 lb uplift at joint 33, 25 lb uplift at joint 32, 20 lb uplift at joint 31, 21 lb uplift at joint 30, 22 lb uplift at joint 29, 32 lb uplift at joint 28, 25 lb uplift at joint 27, 18 lb uplift at joint 2 and 28 lb uplift at joint 24.
- 13) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 14) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

Page: 1

Plate Offsets (X, Y): [1:Edge,0-12], [14:0-1-8,Edge], [15:0-1-8,Edge]

LUMBER

TOP CHORD 2x4 SP No.2(flat)

BOT CHORD 2x4 SP No.2(flat)

WEBS 2x4 SP No.2(flat)

BRACING

TOP CHORD Structural wood sheathing directly applied on 6-0-0 oc purlins, except end verticals.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (size) 11= Mechanical, 18= Mechanical
Max Grav 11=566 (LC 1), 18=566 (LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

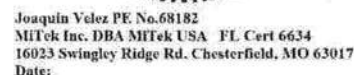
BOT CHORD 17-18=0/580, 16-17=0/1370, 15-16=0/1765,
14-15=0/1786, 13-14=0/1765, 12-13=0/1370,
11-12=0/580

WEBS
5-15=-317/135, 6-14=-317/135, 2-18=-788/0,
2-17=0/564, 3-17=-536/0, 3-16=0/284,
4-16=-290/0, 4-15=-182/396, 9-11=-788/0,
9-12=0/564, 8-12=-536/0, 8-13=0/284,
7-13=-290/0, 7-14=-182/396

NOTES

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are 3x4 MT20 unless otherwise indicated.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-00-00 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

LOAD CASE(S) Standard



January 19, 2024



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE

WARNING – Verify design parameters and READ NOTES ON THIS AND INCLUDED WIRE REFERENCE PAGE 300/301/302/303/304/305/306/307/308/309/310/311/312/313/314/315/316/317/318/319/320/321/322/323/324/325/326/327/328/329/330/331/332/333/334/335/336/337/338/339/340/341/342/343/344/345/346/347/348/349/350/351/352/353/354/355/356/357/358/359/360/361/362/363/364/365/366/367/368/369/370/371/372/373/374/375/376/377/378/379/380/381/382/383/384/385/386/387/388/389/390/391/392/393/394/395/396/397/398/399/400/401/402/403/404/405/406/407/408/409/410/411/412/413/414/415/416/417/418/419/420/421/422/423/424/425/426/427/428/429/430/431/432/433/434/435/436/437/438/439/440/441/442/443/444/445/446/447/448/449/450/451/452/453/454/455/456/457/458/459/460/461/462/463/464/465/466/467/468/469/470/471/472/473/474/475/476/477/478/479/480/481/482/483/484/485/486/487/488/489/490/491/492/493/494/495/496/497/498/499/500/501/502/503/504/505/506/507/508/509/510/511/512/513/514/515/516/517/518/519/520/521/522/523/524/525/526/527/528/529/530/531/532/533/534/535/536/537/538/539/540/541/542/543/544/545/546/547/548/549/550/551/552/553/554/555/556/557/558/559/560/561/562/563/564/565/566/567/568/569/570/571/572/573/574/575/576/577/578/579/580/581/582/583/584/585/586/587/588/589/590/591/592/593/594/595/596/597/598/599/600/601/602/603/604/605/606/607/608/609/610/611/612/613/614/615/616/617/618/619/620/621/622/623/624/625/626/627/628/629/630/631/632/633/634/635/636/637/638/639/640/641/642/643/644/645/646/647/648/649/650/651/652/653/654/655/656/657/658/659/660/661/662/663/664/665/666/667/668/669/670/671/672/673/674/675/676/677/678/679/680/681/682/683/684/685/686/687/688/689/690/691/692/693/694/695/696/697/698/699/700/701/702/703/704/705/706/707/708/709/710/711/712/713/714/715/716/717/718/719/720/721/722/723/724/725/726/727/728/729/730/731/732/733/734/735/736/737/738/739/740/741/742/743/744/745/746/747/748/749/750/751/752/753/754/755/756/757/758/759/760/761/762/763/764/765/766/767/768/769/770/771/772/773/774/775/776/777/778/779/780/781/782/783/784/785/786/787/788/789/790/791/792/793/794/795/796/797/798/799/800/801/802/803/804/805/806/807/808/809/810/811/812/813/814/815/816/817/818/819/820/821/822/823/824/825/826/827/828/829/830/831/832/833/834/835/836/837/838/839/840/841/842/843/844/845/846/847/848/849/850/851/852/853/854/855/856/857/858/859/860/861/862/863/864/865/866/867/868/869/870/871/872/873/874/875/876/877/878/879/880/881/882/883/884/885/886/887/888/889/890/891/892/893/894/895/896/897/898/899/900/901/902/903/904/905/906/907/908/909/910/911/912/913/914/915/916/917/918/919/920/921/922/923/924/925/926/927/928/929/930/931/932/933/934/935/936/937/938/939/940/941/942/943/944/945/946/947/948/949/950/951/952/953/954/955/956/957/958/959/960/961/962/963/964/965/966/967/968/969/970/971/972/973/974/975/976/977/978/979/980/981/982/983/984/985/986/987/988/989/990/991/992/993/994/995/996/997/998/999/1000/1001/1002/1003/1004/1005/1006/1007/1008/1009/1010/1011/1012/1013/1014/1015/1016/1017/1018/1019/1020/1021/1022/1023/1024/1025/1026/1027/1028/1029/1030/1031/1032/1033/1034/1035/1036/1037/1038/1039/1040/1041/1042/1043/1044/1045/1046/1047/1048/1049/1050/1051/1052/1053/1054/1055/1056/1057/1058/1059/1060/1061/1062/1063/1064/1065/1066/1067/1068/1069/1070/1071/1072/1073/1074/1075/1076/1077/1078/1079/1080/1081/1082/1083/1084/1085/1086/1087/1088/1089/1090/1091/1092/1093/1094/1095/1096/1097/1098/1099/1100/1101/1102/1103/1104/1105/1106/1107/1108/1109/1110/1111/1112/1113/1114/1115/1116/1117/1118/1119/1120/1121/1122/1123/1124/1125/1126/1127/1128/1129/1130/1131/1132/1133/1134/1135/1136/1137/1138/1139/1140/1141/1142/1143/1144/1145/1146/1147/1148/1149/1150/1151/1152/1153/1154/1155/1156/1157/1158/1159/1160/1161/1162/1163/1164/1165/1166/1167/1168/1169/1170/1171/1172/1173/1174/1175/1176/1177/1178/1179/1180/1181/1182/1183/1184/1185/1186/1187/1188/1189/1190/1191/1192/1193/1194/1195/1196/1197/1198/1199/1200/1201/1202/1203/1204/1205/1206/1207/1208/1209/1210/1211/1212/1213/1214/1215/1216/1217/1218/1219/1220/1221/1222/1223/1224/1225/1226/1227/1228/1229/1230/1231/1232/1233/1234/1235/1236/1237/1238/1239/1240/1241/1242/1243/1244/1245/1246/1247/1248/1249/1250/1251/1252/1253/1254

MiTek®

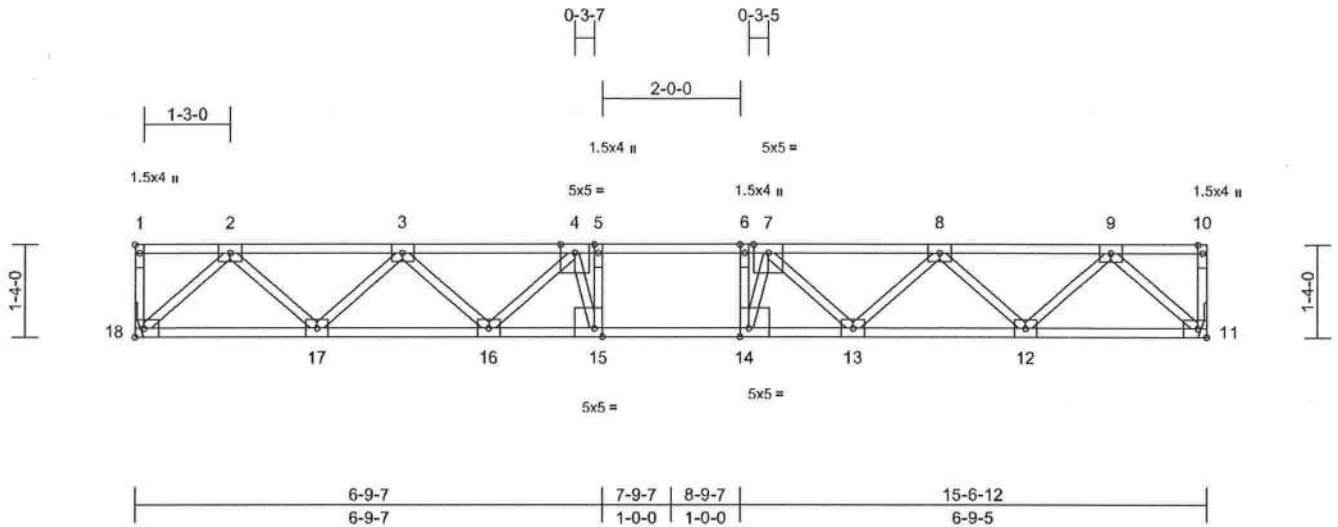
16023 Swingley Ridge Rd.
Chesterfield, MO 63017
314.434.1200 / MiTek-US.com

Job	Truss	Truss Type	Qty	Ply	Job Reference (optional)
1223-036	F02	Floor	39	1	T32673340

Mayo Truss Company, Inc., Mayo, FL - 32066,

Run: 8.73 S Jan 4 2024 Print: 8.730 S Jan 4 2024 MiTek Industries, Inc. Thu Jan 18 13:02:42
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Page: 1



Scale = 1:31.6

Plate Offsets (X, Y): [1:Edge,0-0-12], [14:0-1-8,Edge], [15:0-1-8,Edge]

ading	(psf)	Spacing	1-4-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP	
TCLL	40.0	Plate Grip DOL	1.00	TC	0.37	Vert(LL)	-0.10	14-15	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.00	BC	0.55	Vert(CT)	-0.13	14-15	>999	240		
BCLL	0.0	Rep Stress Incr	YES	WB	0.16	Horz(CT)	0.03	11	n/a	n/a		
BCDL	5.0	Code	FBC2023/TPI2014	Matrix-S								
											Weight: 81 lb	FT = 20%F, 11%E

LUMBER

TOP CHORD 2x4 SP No.2(flat)
BOT CHORD 2x4 SP No.2(flat)
WEBS 2x4 SP No.2(flat)

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (size) 11= Mechanical, 18= Mechanical
Max Grav 11=566 (LC 1), 18=566 (LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-18=-25/0, 10-11=-25/0, 1-2=0/0,
2-3=-985/0, 3-4=-1574/0, 4-5=-1786/0,
5-6=-1786/0, 6-7=-1786/0, 7-8=-1574/0,
8-9=-985/0, 9-10=0/0
BOT CHORD 17-18=0/579, 16-17=0/1370, 15-16=0/1765,
14-15=0/1786, 13-14=0/1765, 12-13=0/1370,
11-12=0/580
WEBS 5-15=-315/134, 6-14=-319/137, 2-18=-788/0,
2-17=0/564, 3-17=-536/0, 3-16=0/284,
4-16=-289/0, 4-15=-181/394, 9-11=-788/0,
9-12=0/564, 8-12=-536/0, 8-13=0/284,
7-13=-290/0, 7-14=-183/398

NOTES

- Unbalanced floor live loads have been considered for this design.
All plates are 3x4 MT20 unless otherwise indicated.
- Refer to girder(s) for truss to truss connections.
- Recommend 2x6 strongbacks, on edge, spaced at 10-00-00 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

LOAD CASE(S) Standard



Joaquin Velez PE No.68182
MiTek Inc. DBA MiTek USA FL Cert 6634
16023 Swingley Ridge Rd. Chesterfield, MO 63017
Date:

January 19,2024

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcsccomponents.com)

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Page: 1

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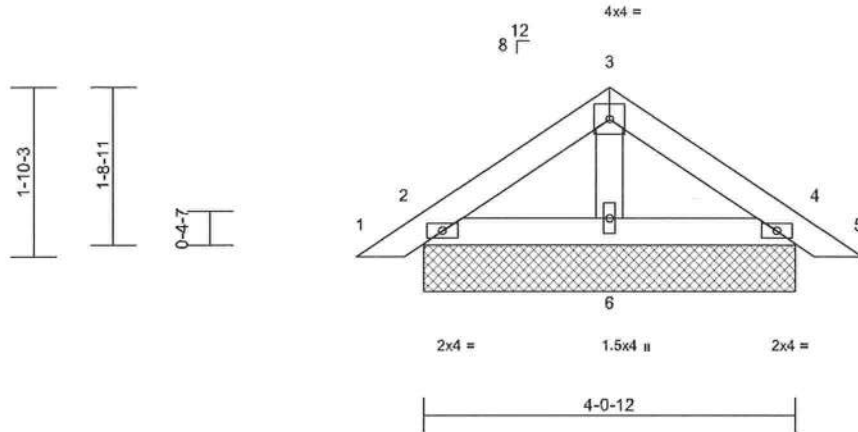
Job	Truss	Truss Type	Qty	Ply	Job Reference (optional)
1223-036	PB02	Piggyback	30	1	T32673342

Mayo Truss Company, Inc., Mayo, FL - 32066,

Run: 8.73 S Jan 4 2024 Print: 8.730 S Jan 4 2024 MiTek Industries, Inc. Thu Jan 18 13:02:42
ID:h7ZdyidMgpr03qbtFuit9y7IIM-RfC7PsB70Hq3NSgPqnl8w3uITXbGKWrdoi7J4zJC7f

Page: 1

-0-8-15	2-0-6	4-0-12	4-9-11
0-8-15	2-0-6	2-0-6	0-8-15



Scale = 1:23.7

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.04	Vert(LL)	n/a	-	n/a	999	244/190
TCDL	10.0	Lumber DOL	1.25	BC	0.05	Vert(CT)	n/a	-	n/a	999	
BCLL	0.0*	Rep Stress Incr	YES	WB	0.01	Horz(CT)	0.00	4	n/a	n/a	
BCDL	10.0	Code	FBC2023/TPI2014	Matrix-AS							
										Weight: 18 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied.

REACTIONS

(size) 2=4-0-12, 4=4-0-12, 6=4-0-12,
7=4-0-12, 11=4-0-12
Max Horiz 2=31 (LC 11), 7=31 (LC 11)
Max Uplift 2=-20 (LC 12), 4=-20 (LC 12),
7=-20 (LC 12), 11=-20 (LC 12)
Max Grav 2=118 (LC 1), 4=118 (LC 1), 6=147
(LC 1), 7=118 (LC 1), 11=118 (LC 1)

FORCES

(lb) - Maximum Compression/Maximum
Tension
TOP CHORD 1-2=0/16, 2-3=-53/62, 3-4=-53/57, 4-5=0/16
BOT CHORD 2-6=-2/45, 4-6=-2/45
WEBS 3-6=-65/22

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-22; Vult=130mph (3-second gust)
Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft;
B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed;
MWFRS (directional) and C-C Zone3 zone; cantilever
left and right exposed; end vertical left and right
exposed; C-C for members and forces & MWFRS for
reactions shown; Lumber DOL=1.60 plate grip
DOL=1.60
- Truss designed for wind loads in the plane of the truss
only. For studs exposed to wind (normal to the face),
see Standard Industry Gable End Details as applicable,
or consult qualified building designer as per ANSI/TPI 1.
- Building Designer / Project engineer responsible for
verifying applied roof live load shown covers rain loading
requirements specific to the use of this truss component.
- Gable requires continuous bottom chord bearing.

- Gable studs spaced at 4-0-0 oc.
- This truss has been designed for a 10.0 psf bottom
chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf
on the bottom chord in all areas where a rectangle
3-06-00 tall by 2-00-00 wide will fit between the bottom
chord and any other members.
- All bearings are assumed to be SP No.2.
- Provide mechanical connection (by others) of truss to
bearing plate capable of withstanding 20 lb uplift at joint
2, 20 lb uplift at joint 4, 20 lb uplift at joint 2 and 20 lb
uplift at joint 4.
- This truss design requires that a minimum of 7/16"
structural wood sheathing be applied directly to the top
chord and 1/2" gypsum sheetrock be applied directly to
the bottom chord.
- See Standard Industry Piggyback Truss Connection
Detail for Connection to base truss as applicable, or
consult qualified building designer.

LOAD CASE(S) Standard



Joaquin Velez PE No.68182
MiTek Inc. DBA MiTek USA FL Cert 6634
16023 Swingley Ridge Rd. Chesterfield, MO 63017
Date:

January 19, 2024



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITK REFERENCE PAGE MI-7473 rev. 1/2/2023 BEFORE USE.

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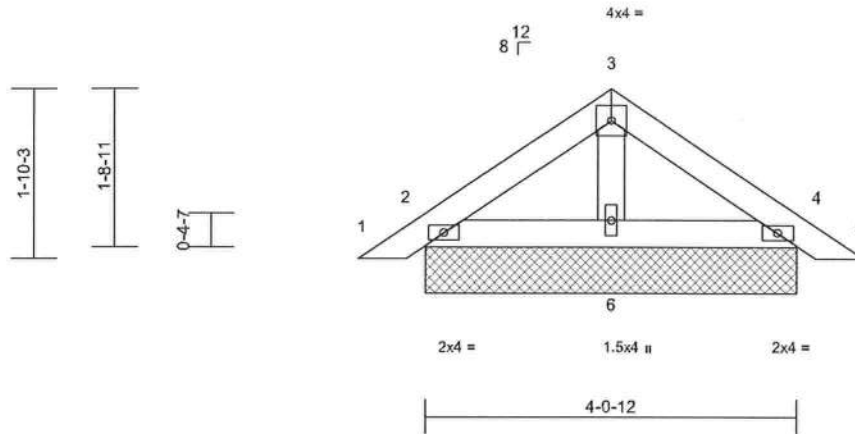
Job 1223-036	Truss PB03	Truss Type Piggyback	Qty 1	Ply 1	Job Reference (optional) T32673343
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Mayo Truss Company, Inc., Mayo, FL - 32066,

Run: 8.73 S Jan 4 2024 Print: 8.730 S Jan 4 2024 MiTek Industries, Inc. Thu Jan 18 13:02:43
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Page: 1

-0-8-15	2-0-6	4-0-12	4-9-11
0-8-15	2-0-6	2-0-6	0-8-15



Scale = 1:23.7

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.04	Vert(LL)	n/a	-	n/a	999	244/190
TCDL	10.0	Lumber DOL	1.25	BC	0.05	Vert(CT)	n/a	-	n/a	999	
BCLL	0.0*	Rep Stress Incr	YES	WB	0.01	Horz(CT)	0.00	4	n/a	n/a	
BCDL	10.0	Code	FBC2023/TPI2014	Matrix-AS							
										Weight: 18 lb	FT = 20%

1 UMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied.

REACTIONS

(size) 2=4-0-12, 4=4-0-12, 6=4-0-12,
7=4-0-12, 11=4-0-12
Max Horiz 2=31 (LC 11), 7=31 (LC 11)
Max Uplift 2=-20 (LC 12), 4=-20 (LC 12),
7=-20 (LC 12), 11=-20 (LC 12)
Max Grav 2=118 (LC 1), 4=118 (LC 1), 6=147
(LC 1), 7=118 (LC 1), 11=118 (LC 1)

FORCES

(lb) - Maximum Compression/Maximum
Tension
TOP CHORD 1-2=0/16, 2-3=-53/62, 3-4=-53/57, 4-5=0/16
BOT CHORD 2-6=-2/45, 4-6=-2/45
WEBS 3-6=-65/22

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-22; Vult=130mph (3-second gust)
Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft;
B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed;
MWFRS (directional) and C-C Zone3 zone; cantilever
left and right exposed; end vertical left and right
exposed; C-C for members and forces & MWFRS for
reactions shown; Lumber DOL=1.60 plate grip
DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1. Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- Gable requires continuous bottom chord bearing.

- Gable studs spaced at 2-0-0 oc.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SP No.2.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 20 lb uplift at joint 2, 20 lb uplift at joint 4, 20 lb uplift at joint 2 and 20 lb uplift at joint 4.
 - This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
 - See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.
- LOAD CASE(S) Standard



Joaquin Velez PE No.68182
MiTek Inc. DBA MiTek USA FL Cert 6634
16023 Swingley Ridge Rd. Chesterfield, MO 63017
Date:

January 19,2024



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinet.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcsccomponents.com)

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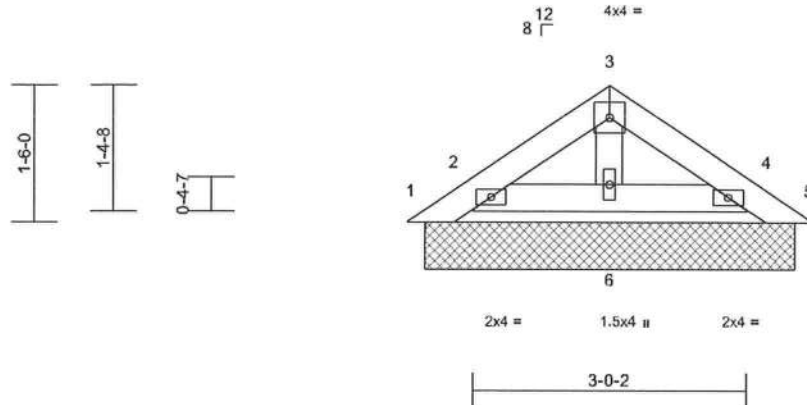
Job 1223-036	Truss PB04	Truss Type Piggyback	Qty 1	Ply 1	Job Reference (optional) T32673344
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Mayo Truss Company, Inc., Mayo, FL - 32066,

Run: 8.73 S Jan 4 2024 Print: 8.730 S Jan 4 2024 MiTek Industries, Inc. Thu Jan 18 13:02:43
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Page: 1

-0-8-9	1-6-1	3-0-2	3-8-11
0-8-9	1-6-1	1-6-1	0-8-9



Scale = 1:23.8

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.03	Vert(LL)	n/a	-	n/a	999	MT20
TCDL	10.0	Lumber DOL	1.25	BC	0.03	Vert(TL)	n/a	-	n/a	999	244/190
BCLL	0.0*	Rep Stress Incr	NO	WB	0.01	Horiz(TL)	0.00	4	n/a	n/a	
BCDL	10.0	Code	FBC2023/TPI2014	Matrix-AS							Weight: 14 lb FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied.

REACTIONS

(size) 1=4-0-12, 2=4-0-12, 4=4-0-12,
5=4-0-12, 6=4-0-12, 7=4-0-12,
10=4-0-12
Max Horiz 1=24 (LC 10)
Max Uplift 1=-19 (LC 10), 4=-6 (LC 12), 5=-5
(LC 24), 10=-6 (LC 12)
Max Grav 1=14 (LC 11), 2=127 (LC 17),
4=113 (LC 24), 5=2 (LC 12), 6=103
(LC 1), 7=127 (LC 17), 10=113 (LC
24)

FORCES

(lb) - Maximum Compression/Maximum
Tension

TOP CHORD 1-2=-27/44, 2-3=-31/43, 3-4=-33/41,
4-5=-4/25
BOT CHORD 2-6=-8/37, 4-6=-8/37
WEBS 3-6=-47/13

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-22; Vult=130mph (3-second gust)
Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft;
B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed;
MWFRS (directional) and C-C Zone3 zone; cantilever
left and right exposed; end vertical left and right
exposed; C-C for members and forces & MWFRS for
reactions shown; Lumber DOL=1.60 plate grip
DOL=1.60
- Truss designed for wind loads in the plane of the truss
only. For studs exposed to wind (normal to the face),
see Standard Industry Gable End Details as applicable,
or consult qualified building designer as per ANSI/TPI 1.

- Building Designer / Project engineer responsible for
verifying applied roof live load shown covers rain loading
requirements specific to the use of this truss component.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom
chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf
on the bottom chord in all areas where a rectangle
3-06-00 tall by 2-00-00 wide will fit between the bottom
chord and any other members.
- All bearings are assumed to be SP No.2.
- Provide mechanical connection (by others) of truss to
bearing plate capable of withstanding 6 lb uplift at joint 4,
19 lb uplift at joint 1, 5 lb uplift at joint 5 and 6 lb uplift at
joint 4.
- This truss design requires that a minimum of 7/16"
structural wood sheathing be applied directly to the top
chord and 1/2" gypsum sheetrock be applied directly to
the bottom chord.
- See Standard Industry Piggyback Truss Connection
Detail for Connection to base truss as applicable, or
consult qualified building designer.

LOAD CASE(S) Standard



Joaquin Velez PE No.68182
MiTek Inc. DBA MiTek USA FL Cert 6634
16023 Swingley Ridge Rd. Chesterfield, MO 63017
Date:

January 19,2024

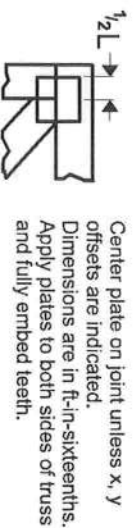
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

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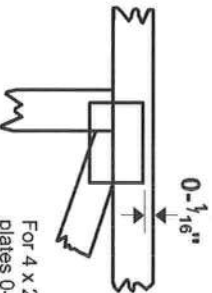
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Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- $\frac{1}{16}$ \" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

* Plate location details available in MiTek software or upon request.

PLATE SIZE

4 X 4

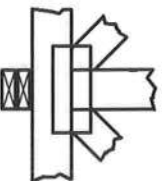
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number/letter where bearings occur. Min size shown is for crushing only.

Industry Standards:

ANSI/ITP1: National Design Specification for Metal

Plate Connected Wood Truss Construction.

DSB-22: Design Standard for Bracing.

BCSI: Building Component Safety Information.

Guide to Good Practice for Handling,

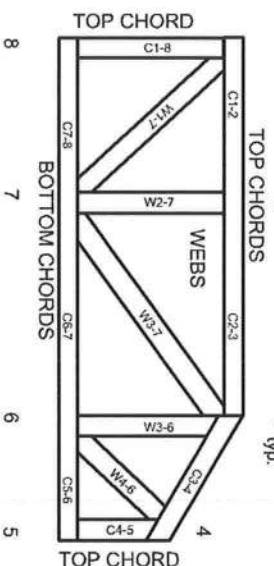
Installing, Restraining & Bracing of Metal

Plate Connected Wood Trusses.

Numbering System



1 2 3 Joint ID typ.



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

Product Code Approvals

ICC-ES Reports:

ESR-1988, ESR-2362, ESR-2685, ESR-3282

ESR-4722, ESL-1388

Design General Notes

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TP1 section 6.3 These truss designs rely on lumber values established by others.

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MiTek®

MiTek Engineering Reference Sheet: MII-7473 rev. 1/2/2023

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative for I bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and warps at joint locations are regulated by ANSI/TP1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TP1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TP1 Quality Criteria.
21. The design does not take into account any dynamic or other loads other than those expressly stated.



MIAMI-DADE COUNTY, FLORIDA
PRODUCT CONTROL SECTION

11805 SW 26 Street, Room 208

Miami, Florida 33175-2474

www.miamidade.gov/building

DEPARTMENT OF REGULATORY AND ECONOMIC RESOURCES (RER)

NOTICE OF ACCEPTANCE (NOA)

PGT Industries, Inc.
1070 Technology Drive
North Venice, FL 34275

SCOPE:

This NOA is being issued under the applicable rules and regulations governing the use of construction materials. The documentation submitted has been reviewed and accepted by Miami-Dade County RER - Product Control Section to be used in Miami Dade County and other areas where allowed by the Authority Having Jurisdiction (AHJ).

This NOA shall not be valid after the expiration date stated below. The Miami-Dade County Product Control Section (In Miami-Dade County) and/or the AHJ (in areas other than Miami-Dade County) reserve the right to have this product or material tested for quality assurance purposes. If this product or material fails to perform in the accepted manner, the manufacturer will incur the expense of such testing and the AHJ may immediately revoke, modify, or suspend the use of such product or material within their jurisdiction. RER reserves the right to revoke this acceptance, if it is determined by Miami-Dade County Product Control Section that this product or material fails to meet the requirements of the applicable building code.

This product is approved as described herein, and has been designed to comply with the Florida Building Code, including the High Velocity Hurricane Zone.-

DESCRIPTION: Series "PW-5420 Vinyl" PVC Fixed Window – N.I.

APPROVAL DOCUMENT: Drawing No. **MD-5420.0** titled "Vinyl Fixed Window NOA (NI)", sheets 1 through 9 of 9, dated 09/09/14, with revision C dated 03/16/2020, prepared by manufacturer, signed and sealed by Anthony Lynn Miller, P.E., bearing the Miami-Dade County Product Control Revision stamp with the Notice of Acceptance number and expiration date by the Miami-Dade County Product Control Section.

MISSILE IMPACT RATING: None

LABELING: Each unit shall bear a permanent label with the manufacturer's name or logo, city, state, model/series, and following statement: "Miami-Dade County Product Control Approved", unless otherwise noted herein.

RENEWAL of this NOA shall be considered after a renewal application has been filed and there has been no change in the applicable building code negatively affecting the performance of this product.

TERMINATION of this NOA will occur after the expiration date or if there has been a revision or change in the materials, use, and/or manufacture of the product or process. Misuse of this NOA as an endorsement of any product, for sales, advertising or any other purposes shall automatically terminate this NOA. Failure to comply with any section of this NOA shall be cause for termination and removal of NOA.

ADVERTISEMENT: The NOA number preceded by the words Miami-Dade County, Florida, and followed by the expiration date may be displayed in advertising literature. If any portion of the NOA is displayed, then it shall be done in its entirety.

INSPECTION: A copy of this entire NOA shall be provided to the user by the manufacturer or its distributors and shall be available for inspection at the job site at the request of the Building Official.

This NOA **revises NOA No. 19-1126.09** and consists of this page 1 and evidence pages E-1, E-2 and E-3, as well as approval document mentioned above.

The submitted documentation was reviewed by **Sifang Zhao, P.E.**



S.Z.
08/06/2020

NOA No. 20-0401.15
Expiration Date: April 30, 2025
Approval Date: August 06, 2020
Page 1

PGT Industries, Inc.

NOTICE OF ACCEPTANCE: EVIDENCE SUBMITTED

1. EVIDENCE SUBMITTED UNDER PREVIOUS NOA's

A. DRAWINGS

1. Manufacturer's die drawings and sections. *(Submitted under NOA No. 14-0930.24)*
2. Drawing No. **MD-5420.0** titled "Vinyl Fixed Window NOA (NI)", sheets 1 through 9 of 9, dated 09/09/14, with revision B dated 06/06/17, prepared by manufacturer, signed, sealed and dated 6/9/17, by Anthony Lynn Miller, P.E.
(Submitted under NOA No. 17-0614.08)

B. TESTS

1. Test reports on: 1) Uniform Static Air Pressure Test, Loading per FBC, TAS 202-94
2) Large Missile Impact Test per FBC, TAS 201-94
3) Cyclic Wind Pressure Loading per FBC, TAS 203-94
along with marked-up drawings and installation diagram of a PVC sliding glass door, a PVC fixed window and an aluminum sliding glass door, using: Kodispace 4SG TPS spacer system, Duraseal® spacer system, Super Spacer® NXT™ spacer system and XL Edge™ spacer system at insulated glass, prepared by Fenestration Testing Laboratory, Inc., Test Reports No. **FTL-8717**, **FTL-8968** and **FTL-8970**, dated 11/16/15, 06/07/16 and 06/02/16 respectively, all signed and sealed by Idalmis Ortega, P.E.
(Submitted under NOA No. 16-0629.11)
2. Test reports on: 1) Air Infiltration Test, per FBC, TAS 202-94
2) Uniform Static Air Pressure Test, Loading per FBC, TAS 202-94
3) Water Resistance Test, per FBC, TAS 202-94
4) Large Missile Impact Test per FBC, TAS 201-94
5) Cyclic Wind Pressure Loading per FBC, TAS 203-94
6) Forced Entry Test, per FBC 2411.3.2.1, and TAS 202-94
along with marked-up drawings and installation diagram of a PVC fixed window, prepared by Fenestration Testing Laboratory, Inc., Test Report No. **FTL-7897**, dated 08/01/14, signed and sealed by Idalmis Ortega, P.E.
(Submitted under NOA No. 14-0930.24)

C. CALCULATIONS

1. Anchor verification calculations and structural analysis, complying with **FBC 5th Edition (2014)**, dated 09/18/14 and 04/07/15, prepared by manufacturer, signed and sealed by Anthony Lynn Miller, P.E. *(Submitted under NOA No. 14-0930.24)*
2. Glazing complies with **ASTM E1300-09**

D. QUALITY ASSURANCE

1. Miami-Dade Department of Regulatory and Economic Resources (RER).



Sifang Zhao, P.E.
Product Control Examiner
NOA No. 20-0401.15
Expiration Date: April 30, 2025
Approval Date: August 06, 2020

PGT Industries, Inc.

NOTICE OF ACCEPTANCE: EVIDENCE SUBMITTED

E. MATERIAL CERTIFICATIONS

1. Notice of Acceptance No. **16-0712.03** issued to ENERGI Fenestration Solutions USA for their “**White Rigid PVC Exterior Extrusions for Windows and Doors**” dated 08/10/17, expiring on 02/28/18.
2. Notice of Acceptance No. **16-0712.04** issued to ENERGI Fenestration Solutions USA, Inc. for their “**Bronze and Lighter Shades of Cap Coated White Rigid PVC Exterior Extrusions for Windows and Doors**” dated 09/15/16, expiring on 04/16/20.
3. Notice of Acceptance No. **16-0712.05** issued to ENERGI Fenestration Solutions USA, Inc. for their “**Performance Core Rigid PVC Exterior Extrusions for Windows and Doors**” dated 09/15/16, expiring on 04/16/20.

F. STATEMENTS

1. Statement letter of conformance, complying with **FBC 5th Edition (2014)** and **FBC 6th Edition (2017)**, dated August 29, 2017, issued by manufacturer, signed and sealed by Anthony Lynn Miller, P.E. *(Submitted under NOA No. 17-0614.08)*
2. Statement letter of no financial interest, dated June 9, 2017, issued by manufacturer, signed and sealed by Anthony Lynn Miller, P.E. *(Submitted under NOA No. 17-0614.08)*
3. Proposal issued by Product Control, dated 6/26/14, signed by Jaime Gascon, P.E. Supervisor, Product Control Section. *(Submitted under NOA No. 14-0930.24)*
4. Proposal No. **16-0125** issued by the Product Control Section, dated March 09, 2016, signed by Ishaq Chanda, P.E. *(Submitted under NOA No. 16-0629.11)*

G. OTHERS

1. NOA No. **16-0629.11**, issued to PGT Industries, Inc. for their Series “PW-5420 Vinyl” PVC Fixed Window – N.I. approved on 08/04/16 and expiring on 04/30/20.

2. NEW EVIDENCE SUBMITTED

A. DRAWINGS

1. Drawing No. **MD-5420.0** titled “Vinyl Fixed Window NOA (NI)”, sheets 1 through 9 of 9, dated 09/09/14, with revision C dated 03/16/2020, prepared by manufacturer, signed and sealed by Anthony Lynn Miller, P.E.

B. TESTS

1. Test reports on:
 - 1) Air Infiltration Test, per FBC, TAS 202-94
 - 2) Uniform Static Air Pressure Test, Loading per FBC, TAS 202-94
 - 3) Water Resistance Test, per FBC, TAS 202-94
 - 4) Large Missile Impact Test per FBC, TAS 201-94
 - 5) Cyclic Wind Pressure Loading per FBC, TAS 203-94
 - 6) Forced Entry Test, per ASTM F588 and TAS 202-94

along with marked-up drawings and installation diagram of all PGT Industries, Inc. representative units listed below and tested to qualify **Dowsil 791** and **Dowsil 983** silicones, prepared by Fenestration Testing Laboratory, Inc., Test Reports No.: 

Sifang Zhao, P.E.
Product Control Examiner
NOA No. 20-0401.15

Expiration Date: April 30, 2025
Approval Date: August 06, 2020

PGT Industries, Inc.

NOTICE OF ACCEPTANCE: EVIDENCE SUBMITTED

B. TESTS (CONTINUED)

FTL-7897, PGT PW5520 PVC Fixed Window (unit 6 in proposal), dated 09/03/14
FTL-20-2107.1, PGT SGD780 Aluminum Sliding Glass Door (unit 7 in proposal)
FTL-20-2107.2, PGT CA740 Alum. Outswing Casement Window (unit 8 in proposal)
FTL-20-2107.3, PGT PW7620A Aluminum Fixed Window (unit 9 in proposal) and
FTL-20-2107.4, PGT PW7620A Aluminum Fixed Window (unit 10 in proposal)
dated 07/13/20, all signed and sealed by Idalmis Ortega, P.E.

C. CALCULATIONS

1. Anchor verification calculations and structural analysis, complying with **FBC-6th Edition (2017)** and **FBC-7th (2020)** dated 03/13/20, prepared by manufacturer, signed and sealed by Anthony Lynn Miller, P.E.

D. QUALITY ASSURANCE

1. Miami-Dade Department of Regulatory and Economic Resources (RER)

E. MATERIAL CERTIFICATIONS

1. NOA No. **18-0122.02** issued to ENERGI Fenestration Solutions USA, for their **"White Rigid PVC Exterior Extrusions for Windows and Doors"** dated 03/08/18, expiring on 02/28/23.
2. NOA No. **20-0203.03** issued to ENERGI Fenestration Solutions USA, Inc. for their **"Bronze and Lighter Shades of Cap Coated Rigid PVC Exterior Extrusions for Windows and Doors"** dated 02/27/20, expiring on 04/16/25.
3. NOA No. **20-0203.04** issued to ENERGI Fenestration Solutions USA, Inc. for their **"Performance Core Rigid PVC Exterior Extrusions for Windows and Doors"** dated 02/27/20, expiring on 04/16/25.

F. STATEMENTS

1. Statement letter of conformance, complying with **FBC-6th Edition (2017)** and **FBC-7th Edition (2020)**, dated 03/16/20, issued by manufacturer, signed and sealed by Anthony Lynn Miller, P.E.
2. Statement letter of no financial interest, dated 03/16/20, issued by manufacturer, signed and sealed by Anthony Lynn Miller, P.E.
3. Proposal No. **19-1155 TP** issued by the Product Control Section, dated January 10, 2020, signed by Ishaq Chanda, P.E.

G. OTHERS

1. NOA No. **19-1126.09**, issued to PGT Industries, Inc. for their Series "PW-5420 Vinyl" PVC Fixed Window – N.I., approved on 01/09/20 and expiring on 04/30/25.



Sifang Zhao, P.E.
Product Control Examiner
NOA No. 20-0401.15
Expiration Date: April 30, 2025
Approval Date: August 06, 2020

GENERAL NOTES: SERIES 5420 NON-IMPACT RESISTANT, VINYL FIXED WINDOW

- THIS PRODUCT HAS BEEN DESIGNED & TESTED TO COMPLY WITH THE REQUIREMENTS OF THE FLORIDA BUILDING CODE, INCLUDING THE HIGH VELOCITY HURRICANE ZONE (HVHZ).
- SHUTTERS ARE REQUIRED WHEN USED IN WIND-BORNE DEBRIS REGIONS.
- FOR MASONRY APPLICATIONS IN MIAMI-DADE COUNTY, USE ONLY MIAMI-DADE COUNTY APPROVED MASONRY ANCHORS. MATERIALS USED FOR ANCHOR EVALUATIONS WERE SOUTHERN PINE ASTM C90 CONCRETE MASONRY UNITS AND CONCRETE WITH MIN. KSI PER ANCHOR TYPE.
- ALL WOOD BUCKS LESS THAN 1-1/2" THICK ARE TO BE CONSIDERED 1X INSTALLATIONS. 1X WOOD BUCKS ARE OPTIONAL. IF UNIT IS INSTALLED DIRECTLY TO SUBSTRATE, WOOD BUCKS DEPICTED AS 2X ARE 1-1/2" THICK OR GREATER. 1X AND 2X BUCKS (WHEN USED) SHALL BE DESIGNED TO PROPERLY TRANSFER LOADS TO THE STRUCTURE. WOOD BUCK DESIGN AND INSTALLATION IS THE RESPONSIBILITY OF THE ENGINEER, (EOR) OR ARCHITECT OF RECORD, (AOR).
- ANCHOR EMBEDMENT TO BASE MATERIAL SHALL BE BEYOND WALL DRESSING OR STUCCO. USE ANCHORS OF SUFFICIENT LENGTH TO ACHIEVE EMBEDMENT. INSTALLATION ANCHORS SHOULD BE SEALED. OVERALL SEALING/FLASHING STRATEGY FOR WATER RESISTANCE OF INSTALLATION SHALL BE DONE BY OTHERS AND IS BEYOND THE SCOPE OF THESE INSTRUCTIONS.
- MAX. 1/4" SHIMS ARE REQUIRED AT EACH ANCHOR LOCATION WHERE THE PRODUCT IS NOT FLUSH TO THE SUBSTRATE. USE SHIMS CAPABLE OF TRANSFERRING APPLIED LOADS. WOOD BUCKS, BY OTHERS, MUST BE SUFFICIENTLY ANCHORED TO RESIST LOADS IMPOSED ON THEM BY THE WINDOW.

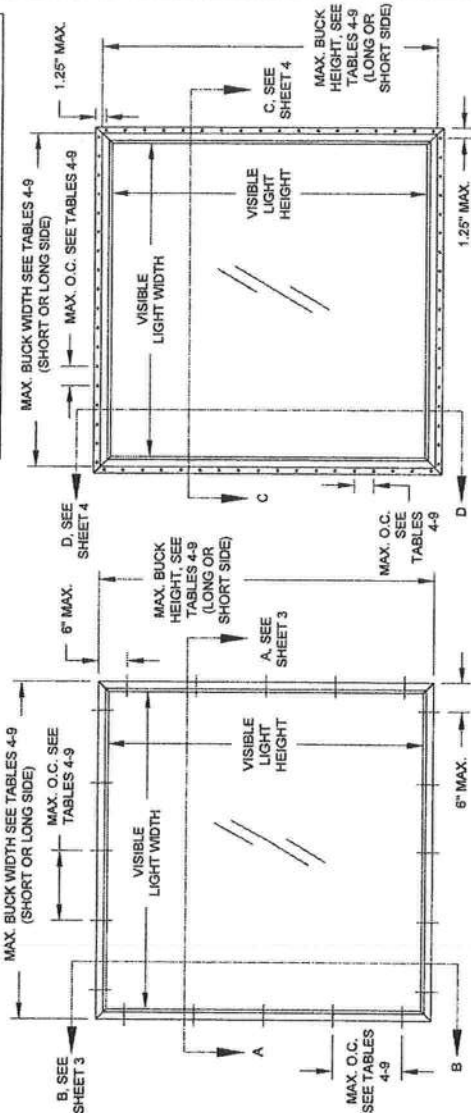
- DESIGN PRESSURES:
A. NEGATIVE DESIGN LOADS BASED ON STRUCTURAL TEST PRESSURE, FRAME ANALYSIS AND GLASS PER ASTM E1300.
B. POSITIVE DESIGN LOADS BASED ON WATER TEST PRESSURE, STRUCTURAL TEST PRESSURE, FRAME ANALYSIS AND GLASS PER ASTM E1300.
C. DESIGN LOADS ARE BASED ON ALLOWABLE STRESS DESIGN, ASD.
- THE ANCHORAGE METHODS SHOWN HAVE BEEN DESIGNED TO RESIST THE WINDLOADS CORRESPONDING TO THE REQUIRED DESIGN PRESSURE. THE 33-1/3% STRESS INCREASE HAS NOT BEEN USED IN THE DESIGN OF THIS PRODUCT. THE 1.6 LOAD DURATION FACTOR WAS USED FOR THE EVALUATION OF ANCHORS INTO WOOD. ANCHORS THAT COME INTO CONTACT WITH OTHER DISSIMILAR MATERIALS SHALL MEET THE REQUIREMENTS OF THE FLORIDA BUILDING CODE FOR CORROSION RESISTANCE.
- METAL SUBSTRATE TO MEET MIN. STRENGTH AND THICKNESS REQUIREMENTS PER CURRENT FLORIDA BUILDING CODE AND TO BE REVIEWED BY THE AUTHORITY HAVING JURISDICTION.
- REFERENCES: TEST REPORTS FTL-7897, ELCO ULTRACON NOA; DEWALT ULTRACON+ NOA; ELCO/DEWALT CRETE/FLEX NOA; ELCO/DEWALT AGGREGATOR NOA; ENERGI WINDOW AND DOOR PROFILES, LTD WHITE & BRONZE/LIGHTER SHADES OF CAP COATED PVC EXTRUSION NOA'S; NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION, ANSIA/F&PA NDS & ALUMINUM DESIGN MANUAL.

VISIBLE LIGHT FORMULAS
WIDTH: BUCK WIDTH - 4-3/16"
HEIGHT: BUCK HEIGHT - 4-3/16"

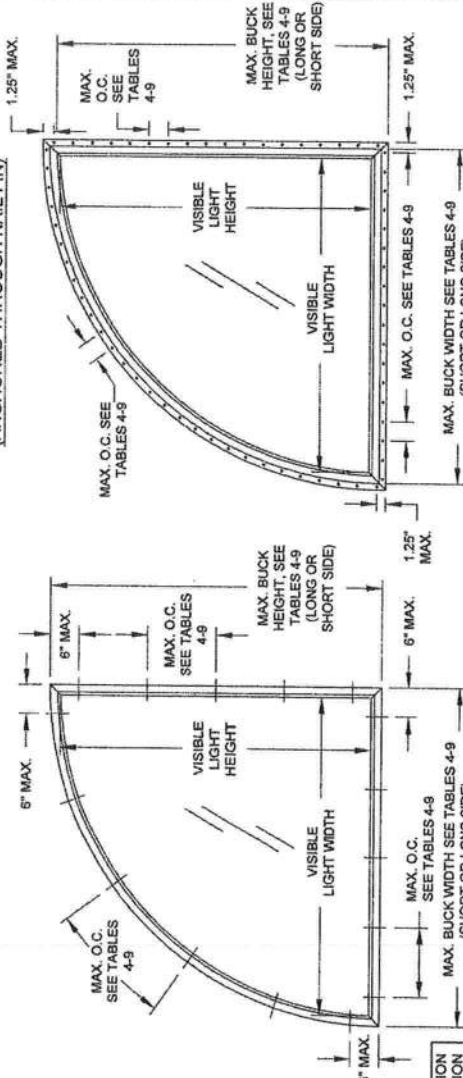
- CODES / STANDARDS USED:
- 2020 FLORIDA BUILDING CODE (FBC), 7TH EDITION
 - 2017 FLORIDA BUILDING CODE (FBC), 6TH EDITION
 - ASTM E1300-09
 - ANSIA/F&PA NDS-2018 FOR WOOD CONSTRUCTION
 - ALUMINUM DESIGN MANUAL, ADM-2015
 - AISI S100-16
 - AISC 360-16

GENERAL NOTES	1
ELEVATIONS	1
FRAME, GLASS & ANCHOR	2
OPTIONS	2
INSTALLATION, FLANGE & EQUAL LEG	3
INSTALLATION, INTEGRAL FIN & J-CHANNEL	4
GLAZING DETAILS	5
DESIGN PRESSURES	6-9
BOM & ASSEMBLY	9

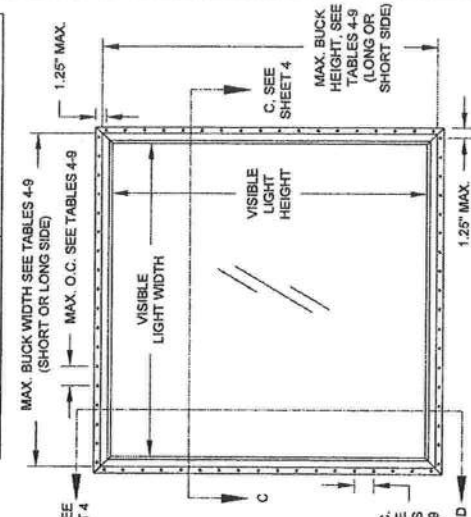
IMPACT RATING	DESIGN PRESSURE RATING
NOT RATED FOR IMPACT RESISTANCE	VARIABLE PER GLASS TYPE, SEE TABLES 4-9, SHEETS 6-8



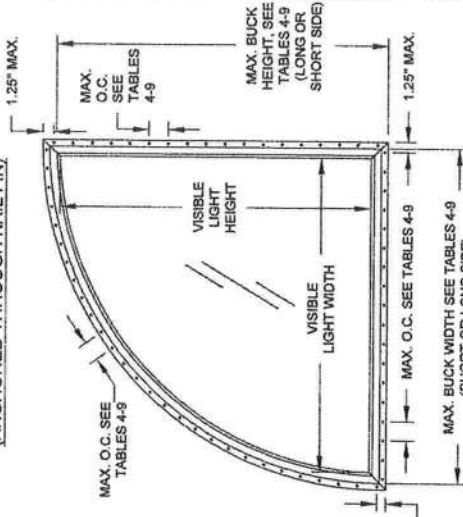
TYP. EQUAL-LEG/BOX & FLANGE FRAME (90° CORNERS)



TYP. INTEGRAL FIN & J-CHANNEL FRAME (90° CORNERS) (ANCHORED THROUGH NAIL FIN)



TYP. INTEGRAL FIN & J-CHANNEL FRAME (90° CORNERS) (ANCHORED THROUGH NAIL FIN)



TYP. EQUAL-LEG/BOX & FLANGE FRAME (90° CORNERS) (ANCHORED THROUGH NAIL FIN)

PRODUCT REVISED
Building Code Compliance with the Florida Building Code
NOA-No. 20-0401.15
Expiration Date 04/30/2025

By
Miami-Dade Product Control

C) UPDATED TO FBC 2020,
REVISED ANCHOR TYPE
TABLE.
AK - 03/16/20

1070 TECHNOLOGY DRIVE
N. VENICE, FL 34275
(941) 480-1600

REGISTRATION #2296

VINYL FIXED WINDOW NOA (NI)

GENERAL NOTES & ELEVATION

MD-5420.0

1 OF 9

9/9/14

ROSOWSKI

ANTHONY LYNN MILLER
LICENSE
No. 58705
STATE OF FLORIDA
PROFESSIONAL ENGINEER
A. LYNN MILLER, P.E.
P.E.# 58705

TABLE 1:

Glass Type	Description	Table #
1	7/8" I.G.: 1/8" A Exterior Cap + 5/8" Air Space + 1/8" A	4
2	7/8" I.G.: 1/8" T Exterior Cap + 5/8" Air Space + 1/8" T	5
3	7/8" I.G.: 3/16" A Exterior Cap + 1/2" Air Space + 3/16" A	6
4	7/8" I.G.: 3/16" T Exterior Cap + 1/2" Air Space + 3/16" T	7
5	1" I.G.: 1/4" A Exterior Cap + 1/2" Air Space + 1/4" A	8
6	1" I.G.: 1/4" T Exterior Cap + 1/2" Air Space + 1/4" T	9

"A" = ANNEALED
"T" = TEMPERED

TABLE 2: ANCHORS INSTALLED THROUGH FRAME

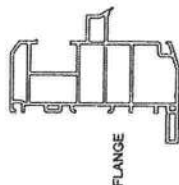
Group	Anchor	Substrate	Min. Edge Distance	Min. Embedment*
A	#10 SMS (steel, 18-8 S.S. or 410 S.S.)	P.T. Southern Pine (SG=0.55)	7/16"	1-3/8"
		Steel Stud, A36*	3/8"	0.050"
	3/16" steel Ultracon or Ultracon+	Aluminum, 6063-T5*	3/8"	0.0451" (18 Ga.)
		P.T. Southern Pine (SG=0.55)	7/16"	0.050"
		Concrete (min. 3 ksi)	1"	1-3/8"
B	#12 SMS (steel, 18-8 S.S. or 410 S.S.)	Ungrouted CMU, (ASTM C-90)	2-1/2"	1-1/4"
		Ungrouted CMU, (ASTM C-90)	1"	1-1/4"
	1/4" steel Ultracon or Ultracon+	P.T. Southern Pine (SG=0.55)	9/16"	1-3/8"
		Steel Stud, A36*	3/8"	0.050"
		Aluminum, 6063-T5*	3/8"	0.0451" (18 Ga.)
C	1/4" steel Ultracon	P.T. Southern Pine (SG=0.55)	1"	1-3/8"
		P.T. Southern Pine (SG=0.55)	1"	1-3/8"
	1/4" steel Ultracon	Concrete (min. 2.85 ksi)	1"	1-3/4"
		Ungrouted CMU, (ASTM C-90)	2-1/2"	1-1/4"
		Concrete (min. 3 ksi)	1-3/16"	1-3/4"
D	1/4" steel Ultracon	Ungrouted CMU, (ASTM C-90)	1"	1-1/4"
		Concrete (min. 3.35 ksi)	1"	1-3/4"
	1/4" steel Ultracon	Concrete (min. 2.85 ksi)	2-1/2"	1-3/4"
		Ungrouted CMU, (ASTM C-90)	2-1/2"	1-3/4"
		Concrete (min. 3.35 ksi)	2-1/2"	1-3/4"
E	1/4" steel Aggre-Gator	Ungrouted CMU, (ASTM C-90)	2-1/2"	1-1/4"
		Concrete (min. 3.275 ksi)	1-1/2"	1-3/8"
F		Grouted CMU, (ASTM C-90)	2"	2"

TABLE 3: ANCHORS INSTALLED THROUGH INTEGRAL FIN

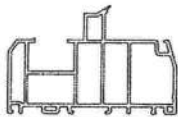
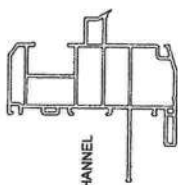
Group	Anchor	Substrate	Min. Edge Distance	Min. Embedment*
E	2-1/2" x .131" Common Nail	P.T. Southern Pine (SG=55)	3/8"	2-7/16"
		P.T. Southern Pine (SG=55)	3/8"	2-7/16"
	2-1/2" Ring-shank Roofing Nail	P.T. Southern Pine (SG=55)	1/2"	1-3/8"
F	#10 Trusshead SMS (steel, 18-8 S.S. or 410 S.S.)	Aluminum, 6063-T5*	3/8"	0.050"
		Steel Stud, Gr. 33*	3/8"	0.0451" (18 Ga.)
	#12 SMS (steel, 18-8 S.S. or 410 S.S.)	Steel, A36*	3/8"	0.050"
		P.T. Southern Pine (SG=55)	9/16"	1-3/8"
		Aluminum, 6063-T5*	3/8"	0.063"
		Steel Stud, Gr. 33*	3/8"	0.050"
		Steel, A36*	3/8"	0.050"

* MIN. OF 3
THREADS
BEYOND
THE
METAL
SUBSTRATE.

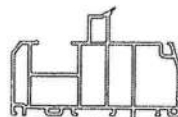
WINDOW FRAMES MAY BE ANY OF THOSE SHOWN BELOW:



FLANGE

BOX OR
EQUAL
LEG

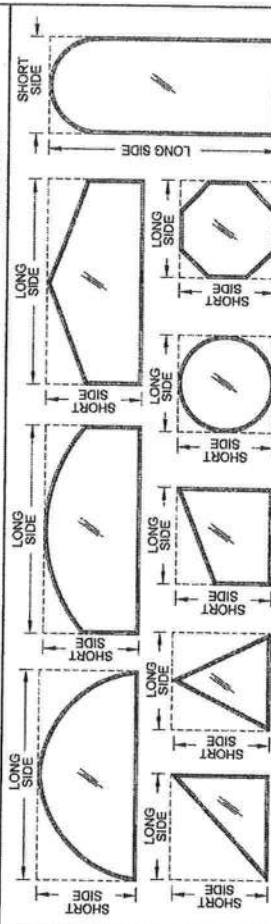
J-CHANNEL

INTEGRAL
FIN

* MIN. OF 3 THREADS
BEYOND THE METAL
SUBSTRATE.

* UNROUTED CMU
VALUES MAY BE
USED FOR GROUTED
CMU APPLICATIONS.

ALL ARCHITECTURAL WINDOW SHAPES QUALIFIED. COMMON EXAMPLES SHOWN. INSCRIBE THE SHAPE IN A BLOCK (SEE EXAMPLES BELOW), AND OBTAIN DESIGN PRESSURES FOR THAT BLOCK SIZE FROM DESIGN PRESSURE TABLES 4-9, SHEETS 6-8.



PRODUCT REVISED
as complying with the Florida
Building Code
NOA-No. 20-0401.15
Expiration Date 04/30/2025

By *[Signature]*
Miami-Dade Product Control

TABLE
AK - 03/16/20

REVISION



1070 TECHNOLOGY DRIVE
N. VENICE, FL 34275
(941)-480-1600



REGISTRATION #23296

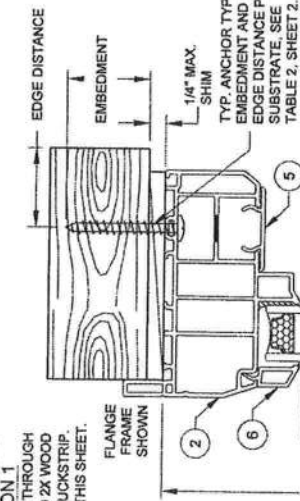
VINYL FIXED WINDOW NOA (NI)
CLASS/ANCHORS/FINISH OPTIONS
J ROSOWSKI
Date 9/9/14
MD-5420.0
Rev. C

Sheet 2 OF 9

INSTALLATION DETAILS FOR FLANGE & EQUAL-LEG/BOX FRAMES

INSTALLATION OPTION 1

ANCHORED THROUGH
FRAME INTO 2X WOOD
FRAME OR BUCKSTRIP.
SEE NOTE 2, THIS SHEET.

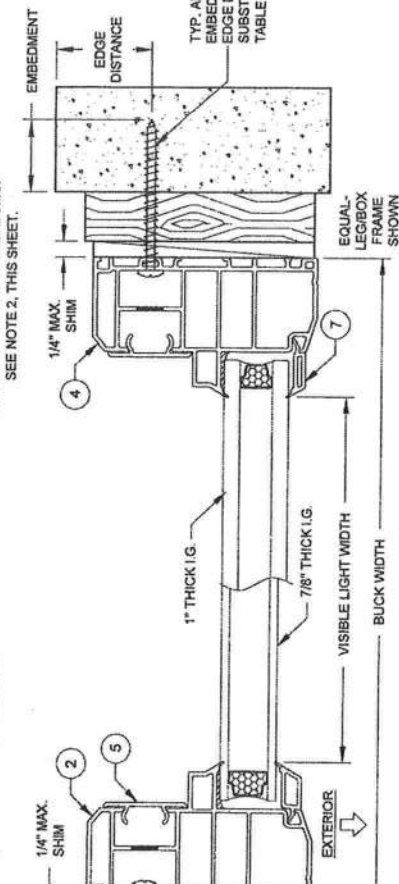


NOTES:

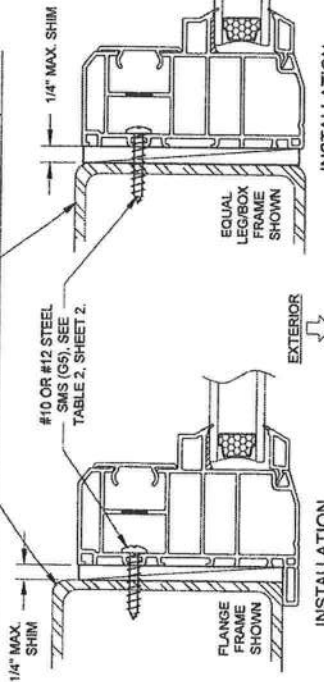
- 1) USE ONLY SUBSTRATE-APPROPRIATE ANCHORS LISTED ON TABLE 2, SHEET 2. FOLLOW EMBEDMENT AND EDGE DISTANCE LIMITS. ANY INSTALLATION OPTION SHOWN MAY BE USED ON ANY SIDE OF THE WINDOW.
- 2) MASONRY ANCHORS MAY BE USED INTO WOOD AS PER TABLE 2, SHEET 2. ALL WOOD BUCKS LESS THAN 1-1/2\"
- 3) VISIBLE LIGHT WIDTH OR HEIGHT (ALSO REFERRED TO AS DAYLIGHT OPENING) IS MEASURED FROM BEADING TO BEADING.

HORIZONTAL SECTION A-A

INSTALLATION OPTION 3 ANCHORED THROUGH FRAME AND 1X BUCKSTRIP INTO CONCRETE/CMU. SEE NOTE 2, THIS SHEET.



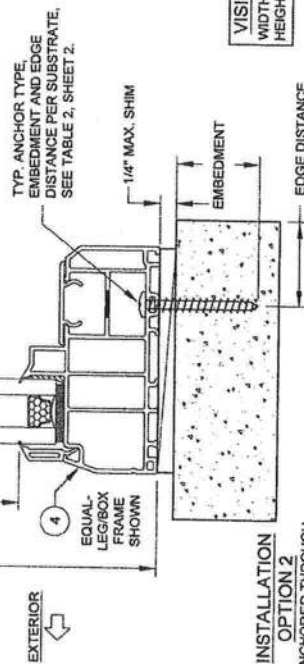
DADE APPROVED MULLION, FBC COMPLIANT ALUMINUM/STEEL FRAMING OR STEEL STUD.
MAY BE VERTICAL OR HORIZONTAL. SEE SUBSTRATE PROPERTIES, TABLE 2, SHEET 2.



INSTALLATION OPTION 4 ANCHORED THROUGH FRAME INTO METAL

VISIBLE LIGHT FORMULAS
WIDTH: BUCK WIDTH - 4-3/16"
HEIGHT: BUCK HEIGHT - 4-3/16"

INSTALLATION OPTION 2 ANCHORED THROUGH FRAME DIRECTLY INTO CONCRETE/CMU.



VERTICAL SECTION B-B

PRODUCT REVISED
as complying with the Florida
Building Code
NOA-No. 20-0401.15
Expiration Date 04/30/2025

By
Miami-Dade Product Control
C) NO CHANGES THIS SHEET.
AK - 03/18/20

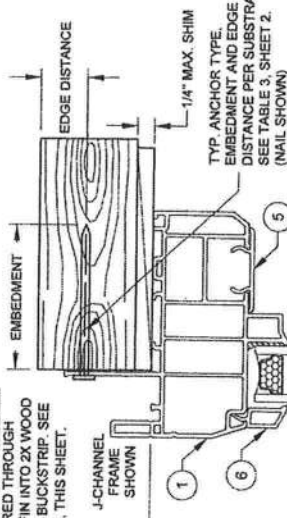


PGT		REGISTRATION #22298	
VINYL FIXED WINDOW NOA (NI)			
1070 TECHNOLOGY DRIVE N. VENICE, FL 34275 (941)-480-1600		Date	9/9/14
Scale	NTS	Sheet	3 OF 9
Series	PW-5420	Desc	FLANGE & EQUAL-LEG/BOX FRAMES
Rev	C	Rev	MD-5420.0

INSTALLATION DETAILS FOR INTEGRAL FIN & J-CHANNEL FRAMES

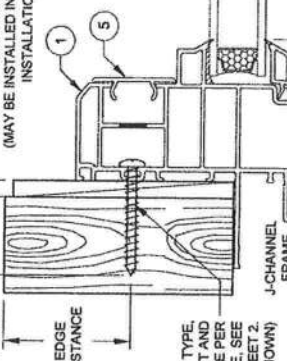
INSTALLATION OPTION 5

ANCHORED THROUGH INTEGRAL FIN INTO 2X WOOD FRAME OR BUCKSTRIP. SEE NOTE 2, THIS SHEET.



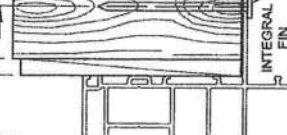
INSTALLATION OPTION 6

ANCHORED THROUGH FRAME INTO 2X WOOD FRAME OR BUCKSTRIP. SEE NOTE 2, THIS SHEET. (MAY BE INSTALLED INTO CONCRETE, SIMILAR TO INSTALLATION OPTIONS 2 & 3)



INSTALLATION OPTION 7

ANCHORED THROUGH INTEGRAL FIN INTO 2X WOOD FRAME OR BUCKSTRIP. SEE NOTE 2, THIS SHEET.



INSTALLATION OPTION 8

ANCHORED THROUGH FRAME INTO 2X WOOD FRAME OR BUCKSTRIP. SEE NOTE 2, THIS SHEET.

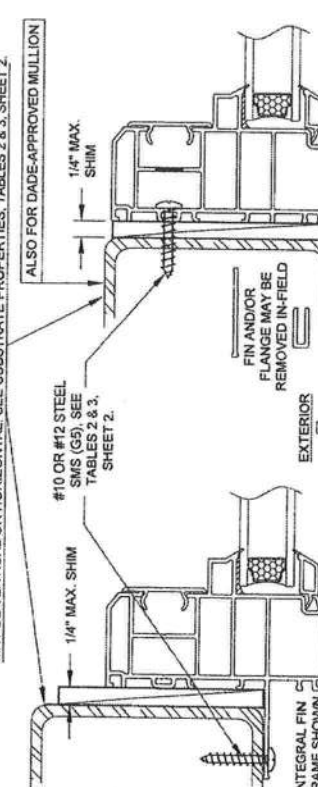


NOTES:

- 1) USE ONLY SUBSTRATE APPROPRIATE ANCHORS LISTED ON TABLES 2 & 3, SHEET 2. FOLLOW EMBEDMENT AND EDGE DISTANCE LIMITS. ANY INSTALLATION OPTION SHOWN MAY BE USED ON ANY SIDE OF THE WINDOW.
- 2) MASONRY ANCHORS MAY BE USED INTO WOOD AS PER TABLE 2, SHEET 2. ALL WOOD BUCKS LESS THAN 1-1/2\"
- 3) VISIBLE LIGHT WIDTH OR HEIGHT (ALSO REFERRED TO AS DAYLIGHT OPENING) IS MEASURED FROM BEADING TO BEADING.

HORIZONTAL SECTION C-C

FBC COMPLIANT ALUMINUM/STEEL FRAMING OR STEEL STUD. MAY BE VERTICAL OR HORIZONTAL. SEE SUBSTRATE PROPERTIES, TABLES 2 & 3, SHEET 2.



INSTALLATION OPTION 7

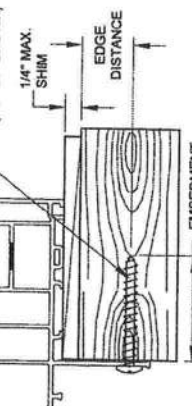
INSTALLATION THROUGH THE INTEGRAL FIN. INTO METAL. SEE TABLE 3, SHEET 2.

INSTALLATION OPTION 8

INSTALLATION THROUGH THE FRAME. INTO METAL. SEE TABLE 2, SHEET 2.

INSTALLATION OPTION 5

ANCHORED THROUGH INTEGRAL FIN INTO 2X WOOD FRAME OR BUCKSTRIP. SEE NOTE 2, THIS SHEET.

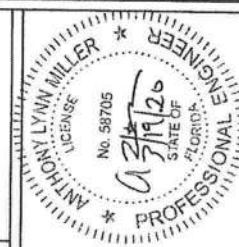


VERTICAL SECTION D-D

VISIBLE LIGHT FORMULAS
WIDTH: BUCK WIDTH - 4-3/16\"

PRODUCT REVISED
as complying with the Florida Building Code
NOA-No. 20-0401.15
Expiration Date 04/30/2025
By: [Signature]
Miami-Dade Product Control

(C) DADE MILLION NOTE,
INSTALL. OPTION 6 NOTE.
AK - 03/18/20



A LYNN MILLER, P.E.
PE# 58705

PGF REGISTRATION #22296		1070 TECHNOLOGY DRIVE N. VENICE, FL 34275 (941)-480-1600	
Series	Sheet	Date	Rev.
PW-5420	4 OF 9	9/9/14	C
VINYL FIXED WINDOW NOA (NI)		J. ROSOWSKI	
J-CHANNEL & INTEGRAL FIN FRAMES		MD-5420.0	

PRODUCT REVISED
 in compliance with the Florida
 Building Code
 NOA-No. **20-0401.15**
 Expiration Date **04/30/2025**
 By *[Signature]*
 Miami-Dade Product Control

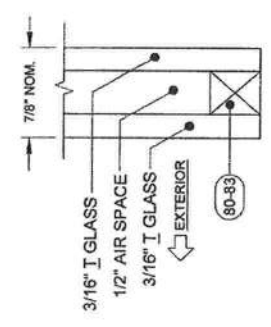
C) NO CHANGES THIS SHEET.
 AK - 03/16/20

1070 TECHNOLOGY DRIVE
 N. VENICE, FL 34275
 (941) 480-1600

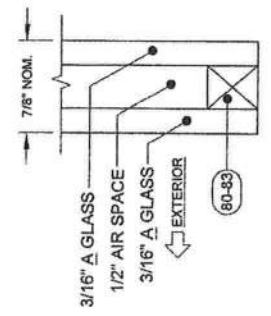
REGISTRATION #29296

VINYL FIXED WINDOW NOA (NI)
 Date 9/9/14
 J ROSOWSKI
 MD-5420.0
 5 OF 9
 9/9/20

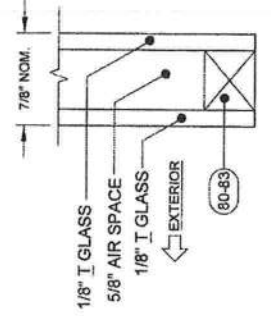
ANTHONY LYNN MILLER
 No. 58705
 03/19/20
 STATE OF FLORIDA
 PROFESSIONAL ENGINEER
 A LYNN MILLER, P.E.
 P.E.# 58705



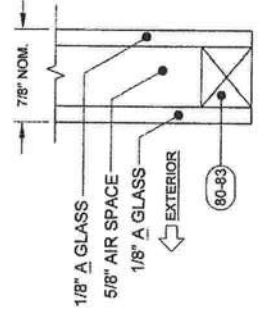
GLASS TYPE 4



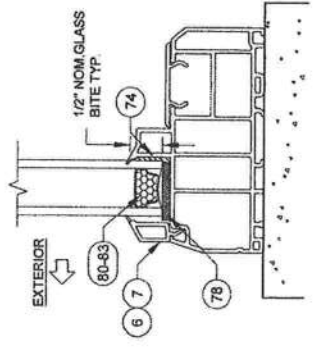
GLASS TYPE 3



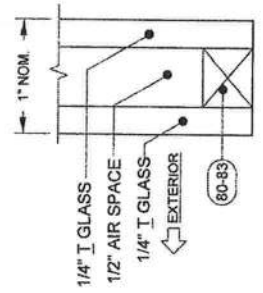
GLASS TYPE 2



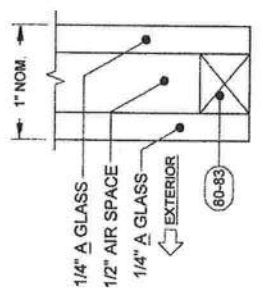
GLASS TYPE 1



TYP. GLAZING DETAIL



GLASS TYPE 6



GLASS TYPE 5

GLAZING NOTES:
 "A" = ANNEALED
 "T" = TEMPERED

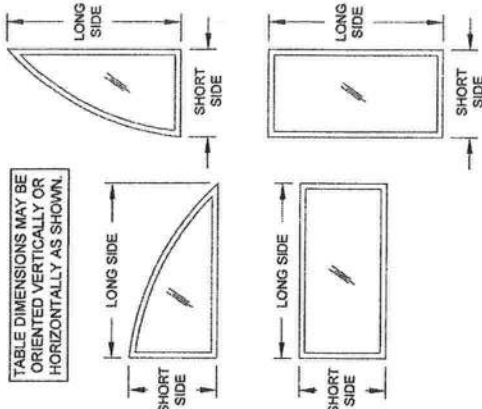
TABLE 4:

Window Design Pressure, (+/- psf)															Use this table for Glass Type: 1		
1/8" A Cap - Airspace - 1/8" A																	
Window Dimensions	Long Side (in)																
	51.05	54	56	58	62	64	68	72	76	80	84	87	89	91			
18	+80/-93.7	+80/-92.6	+80/-91.9	+80/-91.3	+80/-90.3	+80/-89.8	+80/-88.9	+80/-88.2	+80/-87.5	+80/-86.9	+80/-86.4	+80/-86.1	+80/-85.8	+80/-85.5			
20	+80/-96.4	+80/-95.2	+80/-94.5	+80/-93.5	+80/-90.6	+80/-90.1	+80/-89.2	+80/-88.5	+80/-87.8	+80/-87.1	+80/-86.6	+80/-86.3	+80/-86.0	+80/-85.7			
22	+80/-98.3	+80/-97.2	+80/-96.4	+80/-95.4	+80/-92.5	+80/-92.0	+80/-91.1	+80/-90.4	+80/-89.7	+80/-89.0	+80/-88.5	+80/-88.2	+80/-87.9	+80/-87.6			
24	+80/-100.3	+80/-99.2	+80/-98.4	+80/-97.4	+80/-94.5	+80/-94.0	+80/-93.1	+80/-92.4	+80/-91.7	+80/-91.0	+80/-90.5	+80/-90.2	+80/-89.9	+80/-89.6			
26	+80/-102.3	+80/-101.2	+80/-100.4	+80/-99.4	+80/-96.5	+80/-96.0	+80/-95.1	+80/-94.4	+80/-93.7	+80/-93.0	+80/-92.5	+80/-92.2	+80/-91.9	+80/-91.6			
28	+80/-104.3	+80/-103.2	+80/-102.4	+80/-101.4	+80/-98.5	+80/-98.0	+80/-97.1	+80/-96.4	+80/-95.7	+80/-95.0	+80/-94.5	+80/-94.2	+80/-93.9	+80/-93.6			
30	+80/-106.3	+80/-105.2	+80/-104.4	+80/-103.4	+80/-100.5	+80/-100.0	+80/-99.1	+80/-98.4	+80/-97.7	+80/-97.0	+80/-96.5	+80/-96.2	+80/-95.9	+80/-95.6			
32	+80/-108.3	+80/-107.2	+80/-106.4	+80/-105.4	+80/-102.5	+80/-102.0	+80/-101.1	+80/-100.4	+80/-99.7	+80/-99.0	+80/-98.5	+80/-98.2	+80/-97.9	+80/-97.6			
34	+80/-110.3	+80/-109.2	+80/-108.4	+80/-107.4	+80/-104.5	+80/-104.0	+80/-103.1	+80/-102.4	+80/-101.7	+80/-101.0	+80/-100.5	+80/-100.2	+80/-99.9	+80/-99.6			
36	+80/-112.3	+80/-111.2	+80/-110.4	+80/-109.4	+80/-106.5	+80/-106.0	+80/-105.1	+80/-104.4	+80/-103.7	+80/-103.0	+80/-102.5	+80/-102.2	+80/-101.9	+80/-101.6			
38	+80/-114.3	+80/-113.2	+80/-112.4	+80/-111.4	+80/-108.5	+80/-108.0	+80/-107.1	+80/-106.4	+80/-105.7	+80/-105.0	+80/-104.5	+80/-104.2	+80/-103.9	+80/-103.6			
40	+80/-116.3	+80/-115.2	+80/-114.4	+80/-113.4	+80/-110.5	+80/-110.0	+80/-109.1	+80/-108.4	+80/-107.7	+80/-107.0	+80/-106.5	+80/-106.2	+80/-105.9	+80/-105.6			
42	+80/-118.3	+80/-117.2	+80/-116.4	+80/-115.4	+80/-112.5	+80/-112.0	+80/-111.1	+80/-110.4	+80/-109.7	+80/-109.0	+80/-108.5	+80/-108.2	+80/-107.9	+80/-107.6			
44	+80/-120.3	+80/-119.2	+80/-118.4	+80/-117.4	+80/-114.5	+80/-114.0	+80/-113.1	+80/-112.4	+80/-111.7	+80/-111.0	+80/-110.5	+80/-110.2	+80/-109.9	+80/-109.6			
46	+80/-122.3	+80/-121.2	+80/-120.4	+80/-119.4	+80/-116.5	+80/-116.0	+80/-115.1	+80/-114.4	+80/-113.7	+80/-113.0	+80/-112.5	+80/-112.2	+80/-111.9	+80/-111.6			
48	+80/-124.3	+80/-123.2	+80/-122.4	+80/-121.4	+80/-118.5	+80/-118.0	+80/-117.1	+80/-116.4	+80/-115.7	+80/-115.0	+80/-114.5	+80/-114.2	+80/-113.9	+80/-113.6			
51.05	+80/-126.3	+80/-125.2	+80/-124.4	+80/-123.4	+80/-120.5	+80/-120.0	+80/-119.1	+80/-118.4	+80/-117.7	+80/-117.0	+80/-116.5	+80/-116.2	+80/-115.9	+80/-115.6			
Short Side (in)															MAX. O.C. SPACING IF ANCHORING THROUGH THE INTEGRAL FIN PER SHEET 4 APPLIES TO A, B, C OR D ANCHORS (SEE TABLE 2)	MAX. O.C. SPACING IF ANCHORING THROUGH THE INTEGRAL FIN PER SHEET 4 APPLIES TO E OR F ANCHORS (SEE TABLE 3)	4"
15"																	

TABLE 5:

Window Design Pressure, (+/- psf)															Use this table for Glass Type:	2
1/8" T Cap - Airspace - 1/8" T																
Window Dimensions	Long Side (in)															
	60.926	64	66	68	70	74	77	80	84	87	92	97	99			
32	+80/-98.2	+80/-96.6	+80/-95.6	+80/-94.7	+80/-93.9	+80/-92.4	+80/-91.4	+80/-90.5	+80/-89.5	+80/-88.7	+80/-87.7	+80/-86.7	+80/-86.4	+80/-86.4		
34	+80/-94.5	+80/-92.8	+80/-91.8	+80/-90.9	+80/-90	+80/-88.5	+80/-87.5	+80/-86.6	+80/-85.5	+80/-84.7	+80/-83.6	+80/-82.6	+80/-82.6	+80/-82.3		
36	+80/-91.4	+80/-89.6	+80/-88.5	+80/-87.6	+80/-86.7	+80/-85.1	+80/-84	+80/-83.1	+80/-81.9	+80/-81.2	+80/-80	+80/-79	+80/-79	+80/-78.7		
38	+80/-88.6	+80/-86.6	+80/-85.6	+80/-84.6	+80/-83.7	+80/-82.1	+80/-81	+80/-80	+80/-78.8	+80/-78	+80/-76.9	+80/-75.8	+80/-75.8			
40	+80/-86.3	+80/-84.3	+80/-83.1	+80/-82.1	+80/-81.1	+80/-79.4	+80/-78.3	+80/-77.3	+80/-76	+80/-75.2	+80/-74					
42	+80/-84.2	+80/-82.1	+80/-80.8	+80/-79.8	+80/-78.8	+80/-77	+80/-75.9	+80/-74.8	+80/-73.6	+80/-72.7						
44	+80/-82.4	+80/-80.3	+80/-79	+80/-77.9	+80/-76.8	+80/-75	+80/-73.7	+80/-72.7	+80/-71.4							
46	+80/-80.9	+80/-78.6	+80/-77.3	+80/-76.1	+80/-75	+80/-73.1	+80/-71.8	+80/-70.7								
48	+80/-79.7	+80/-77.3	+80/-75.9	+80/-74.6	+80/-73.5	+80/-71.5										
50	+80/-78.6	+80/-76.1	+80/-74.6	+80/-73.2	+80/-72.1	+80/-70.1										
52	+80/-77.7	+80/-75.1	+80/-73.6	+80/-72.2	+80/-70.9											
54	+80/-77.1	+80/-74.2	+80/-72.6	+80/-71.2												
56	+80/-76.6	+80/-73.6	+80/-71.9													
58	+80/-76.3	+80/-73.1														
60.926	+80/-76.1															
										MAX. O.C. SPACING IF ANCHORING THROUGH THE FRAME PER SHEETS 3 & 4		MAX. O.C. SPACING IF ANCHORING THROUGH THE INTEGRAL FIN PER SHEET 4				
										APPLIES TO 8, C OR D ANCHORS (SEE TABLE 2)		APPLIES TO FANCHORS (SEE TABLE 3)				
										15.5"		4"				

TABLE DIMENSIONS MAY BE ORIENTED VERTICALLY OR HORIZONTALLY AS SHOWN.



NOTES:

- 1) BUCK DIMENSIONS SHOWN.
- 2) FOR SIZES NOT SHOWN, ROUND UP TO THE NEXT AVAILABLE SHORT OR LONG DIMENSION.
- 3) FOR ARCHITECTURAL WINDOWS (SEE SHEET 2), FIND THE SMALLEST SQUARE WINDOW SIZE IN THE TABLE(S) ABOVE WHICH THE ARCHITECTURAL WINDOW WILL COMPLETELY FIT WITHIN.

1070 TECHNOLOGY DRIVE
N. VENICE, FL 34275
(941) 480-1600

VINYL FIXED WINDOW NOA (N)



DESIGN PRESSURE TABLE A

Sheet

6 OF 9

MD-5420.0

9/9/14

AK - 03/16/20

NO CHANGES THIS SHEET.

REVISION:

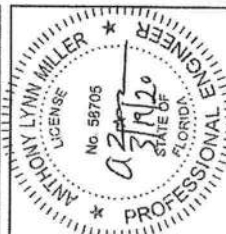
By Miami-Dade Product Control

Expiration Date 04/30/2025

NOA-No. 20-0401.15

as complying with the Florida Building Code

PRODUCT REVISED



A. LYNN MILLER, P.E.
P.E.# 58705

TABLE 6:

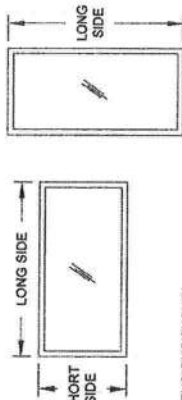
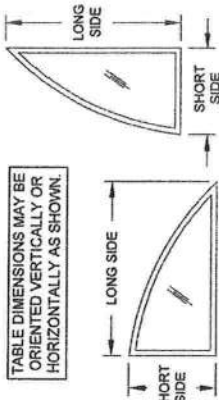
Window Design Pressure, (+/- psf)

3/16" A Cap - Airspace - 3/16" A

Window Dimensions	Long Side (in)														15"	MAX. O.C. SPACING IF ANCHORING THROUGH THE FRAME PER SHEETS 3 & 4 APPLIES TO A, B, C OR D ANCHORS (SEE TABLE 2)		MAX. O.C. SPACING IF ANCHORING THROUGH THE INTEGRAL FIN PER SHEET 4 APPLIES TO E OR F ANCHORS (SEE TABLE 3)	
	69.65	71	73	75	78	80	85	86	89	92	96	99							
32	+/-76.9	+/-76.4	+/-74.3	+/-72.5	+/-70.6	+/-69.3	+/-66.2	+/-63.7	+/-64.1	+/-62.6	+/-60.8	+/-59.8							
34	+/-73.7	+/-73.3	+/-71.3	+/-69.7	+/-68.8	+/-66.8	+/-64.1	+/-60.3	+/-58.1	+/-56.3	+/-54.7	+/-53.8							
36	+/-71	+/-70.5	+/-68.8	+/-67.9	+/-64.4	+/-62.5	+/-57.2	+/-56.3	+/-54.7	+/-52.7	+/-50.9	+/-49.7							
38	+/-68.6	+/-68.1	+/-67.4	+/-66.8	+/-63.5	+/-61	+/-55.6	+/-54.9	+/-52.7	+/-50.6	+/-48.2	+/-46.4							
40	+/-66.4	+/-65.9	+/-65.2	+/-64.6	+/-63.2	+/-60.7	+/-55.1	+/-54.2	+/-52	+/-49.7	+/-46.4	+/-45.2							
42	+/-64.6	+/-64.3	+/-63.3	+/-62.6	+/-61.7	+/-60.5	+/-55.1	+/-54.2	+/-51.8	+/-49.1	+/-46.1	+/-44.5							
44	+/-62.9	+/-62.4	+/-61.6	+/-60.9	+/-60	+/-59.4	+/-55.1	+/-54.2	+/-51.8	+/-49.1	+/-46.1	+/-44.5							
46	+/-61.5	+/-60.9	+/-60.1	+/-59.4	+/-58.4	+/-57.8	+/-55.1	+/-54.2	+/-51.8	+/-49.3	+/-46.3	+/-44.5							
48	+/-60.2	+/-59.6	+/-58.8	+/-58	+/-57	+/-56.4	+/-55	+/-54.2	+/-51.8	+/-49.5	+/-46.4	+/-44.6							
50	+/-59.1	+/-58.5	+/-57.6	+/-56.8	+/-55.8	+/-55.1	+/-53.7	+/-53.4	+/-51.8	+/-49.5	+/-46.6								
52	+/-58.1	+/-57.5	+/-56.6	+/-55.8	+/-54.6	+/-54	+/-52.5	+/-52.2	+/-51.5	+/-49.3									
54	+/-57.3	+/-56.6	+/-55.7	+/-54.8	+/-53.7	+/-53	+/-51.4	+/-51.1	+/-50.4										
56	+/-56.6	+/-55.9	+/-54.9	+/-54	+/-52.8	+/-52	+/-50.4	+/-50.2											
57	+/-56.3	+/-55.5	+/-54.5	+/-53.6	+/-52.4	+/-51.6	+/-50												
60	+/-55.5	+/-54.7	+/-53.6	+/-52.6	+/-51.3														
62	+/-55.1	+/-54.2	+/-53.1	+/-52.1	+/-50.7														
64	+/-54.8	+/-53.9	+/-52.7	+/-51.6															
66	+/-54.5	+/-53.6	+/-52.4																
68	+/-53.6	+/-53.1																	
69.65	+/-52.6																		

Short Side (in)

TABLE DIMENSIONS MAY BE ORIENTED VERTICALLY OR HORIZONTALLY AS SHOWN.

NOTES:
1) BUCK DIMENSIONS SHOWN.

2) FOR SIZES NOT SHOWN, ROUND UP TO THE NEXT AVAILABLE SHORT OR LONG DIMENSION.

3) FOR ARCHITECTURAL WINDOWS (SEE SHEET 2), FIND THE SMALLEST SQUARE WINDOW SIZE IN THE TABLE(S) ABOVE WHICH THE ARCHITECTURAL WINDOW WILL COMPLETELY FIT WITHIN.

TABLE 7:

Window Design Pressure, (+/- psf)

3/16" T Cap - Airspace - 3/16" T

Window Dimensions	Long Side (in)																		Type			
	81.52	83	85	87	89	92	94	97	99	103	107	110	113	118	122	126	132	138		144		
46	+80/-94	+80/-93.3	+80/-92.5	+80/-91.7	+80/-91	+80/-89.9	+80/-89.3	+80/-88.4	+80/-87.9	+80/-86.8	+80/-85.9	+80/-85.3	+80/-84.7	+80/-84.1	+80/-83.8	+80/-83.1	+80/-82.5	+80/-81.7	+80/-80.9	+80/-80.3		
48	+80/-91.6	+80/-90.9	+80/-90.1	+80/-89.3	+80/-88.5	+80/-87.5	+80/-86.8	+80/-85.9	+80/-85.3	+80/-84.3	+80/-83.3	+80/-82.7	+80/-82.1	+80/-81.2	+80/-80.5	+80/-80.1	+80/-79.9	+80/-79.3	+80/-78.6	+80/-78.3		
50	+80/-88.5	+80/-88.8	+80/-87.9	+80/-87.1	+80/-86.3	+80/-85.2	+80/-84.5	+80/-83.6	+80/-83	+80/-82	+80/-81	+80/-80.3	+80/-79.7	+80/-79.1	+80/-78.4	+80/-77.7	+80/-77.4	+80/-76.6	+80/-76.3	+80/-76.3		
52	+80/-87.6	+80/-86.9	+80/-86	+80/-85.1	+80/-84.3	+80/-83.2	+80/-82.5	+80/-81.5	+80/-80.9	+80/-79.8	+80/-78.8	+80/-78.1	+80/-77.5	+80/-76.5	+80/-75.8	+80/-75.2	+80/-74.5	+80/-73.8	+80/-73.8	+80/-73.8		
54	+80/-85.9	+80/-85.2	+80/-84.2	+80/-83.3	+80/-82.5	+80/-81.3	+80/-80.6	+80/-79.6	+80/-79	+80/-77.9	+80/-76.9	+80/-76.2	+80/-75.5	+80/-74.5	+80/-73.8	+80/-73.2	+80/-72.6	+80/-72.2	+80/-71.6	+80/-71.1		
56	+80/-84.4	+80/-83.6	+80/-82.6	+80/-81.7	+80/-80.8	+80/-79.7	+80/-78.9	+80/-77.9	+80/-77.3	+80/-76.1	+80/-75.9	+80/-75.2	+80/-74.3	+80/-73.7	+80/-72.6	+80/-72.2	+80/-71.6	+80/-71.1	+80/-70.5	+80/-70.3		
58	+80/-83	+80/-82.2	+80/-81.2	+80/-80.2	+80/-79.4	+80/-78.1	+80/-77.4	+80/-76.3	+80/-75.7	+80/-74.5	+80/-73.4	+80/-72.7	+80/-72.2	+80/-71.6	+80/-71.1	+80/-70.5	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3		
60	+80/-81.8	+80/-81	+80/-79.9	+80/-78.9	+80/-78	+80/-76.7	+80/-76	+80/-74.9	+80/-74.2	+80/-73	+80/-71.9	+80/-71.1	+80/-70.5	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3		
62	+80/-80.8	+80/-79.9	+80/-78.8	+80/-77.8	+80/-76.8	+80/-75.5	+80/-74.7	+80/-73.5	+80/-72.9	+80/-71.6	+80/-70.5	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3		
64	+80/-79.8	+80/-78.9	+80/-77.8	+80/-76.7	+80/-75.7	+80/-74.3	+80/-73.5	+80/-72.4	+80/-71.6	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3		
67	+80/-78.6	+80/-77.7	+80/-76.4	+80/-75.3	+80/-74.3	+80/-72.8	+80/-72	+80/-70.7	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3		
68	+80/-78.3	+80/-77.3	+80/-76.1	+80/-74.9	+80/-73.8	+80/-72.4	+80/-71.5	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3		
70	+80/-77.7	+80/-76.7	+80/-75.4	+80/-74.2	+80/-73.1	+80/-71.5	+80/-70.6	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3		
72	+80/-77.2	+80/-76.1	+80/-74.8	+80/-73.5	+80/-72.4	+80/-70.8	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3		
74	+80/-76.8	+80/-75.7	+80/-74.3	+80/-73	+80/-71.8	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3		
76	+80/-76.5	+80/-75.3	+80/-73.8	+80/-72.5	+80/-71.8	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3		
78	+80/-76.3	+80/-75	+80/-73.5	+80/-72.5	+80/-71.8	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3	+80/-70.3		
80	+80/-76.2	+80/-74.9																				
81.52	+80/-76.1																					
																			MAX. D.C. SPACING IF ANCHORING THROUGH THE FRAME PER SHEETS 3 & 4		MAX. D.C. SPACING IF ANCHORING THROUGH THE INTEGRAL FIN PER SHEET 4	
																			APPLIES TO B, C OR D ANCHORS (SEE TABLE 2)		APPLIES TO F ANCHORS (SEE TABLE 3)	
																			15.5"		3.3"	

Short Side (in)

MAX. O.C. SPACING IF ANCHORING THROUGH THE FRAME PER SHEETS 3 & 4
APPLIES TO B, C OR D ANCHORS (SEE TABLE 2)

MAX. O.C. SPACING IF ANCHORING THROUGH THE INTEGRAL FIN PER SHEET 4
APPLIES TO F ANCHORS (SEE TABLE 3)

15.5"

3.3"

PRODUCT REVISED
as complying with the Florida Building Code
NOA-No. 20-0401.15
Expiration Date 04/30/2025

By *[Signature]*
Miami-Dade Product Control

C) NO CHANGES THIS SHEET.
AK - 03/16/20

Revision:

1070 TECHNOLOGY DRIVE
N. VENICE, FL 34275
(941) 480-1600

DESIGN PRESSURE TABLES B

J ROSOWSKI

9/9/14

MD-5420.0

7 OF 9

PW-5420

NTS

REGISTRATION #29286

VINYL FIXED WINDOW NOA (N)

9/9/14

PW-5420

NTS

7 OF 9

MD-5420.0

J ROSOWSKI

9/9/14

DESIGN PRESSURE TABLES B

PW-5420

NTS

7 OF 9

MD-5420.0

J ROSOWSKI

9/9/14

DESIGN PRESSURE TABLES B

PW-5420

NTS

7 OF 9

MD-5420.0

J ROSOWSKI

9/9/14

DESIGN PRESSURE TABLES B

PW-5420

NTS

7 OF 9

MD-5420.0

J ROSOWSKI

9/9/14

DESIGN PRESSURE TABLES B

PW-5420

NTS

7 OF 9

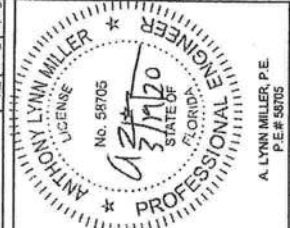
MD-5420.0

J ROSOWSKI

9/9/14

DESIGN PRESSURE TABLES B

PW-5420



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Building Code
NOA-No. 20-0401.15
Expiration Date 04/30/2025

By
Miami-Dade Product Control

AK - 03/16/20

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1070 TECHNOLOGY DRIVE
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(941) 480-1600

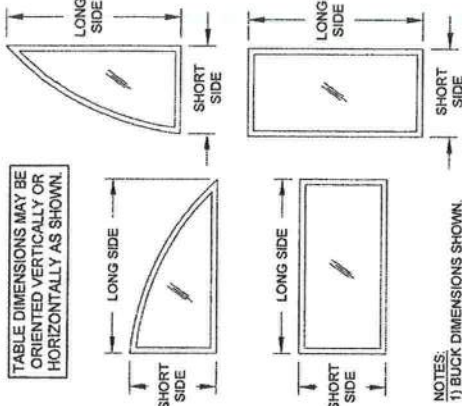
DESIGN PRESSURE TABLE C
VINYL FIXED WINDOW NOA (N)

REGISTRATION #29296

DATE 9/9/14
BY J ROSOWSKI
MD-5420.0

ANTHONY LYNN MILLER
No. 55705
3/19/20
FLORIDA
PROFESSIONAL ENGINEER

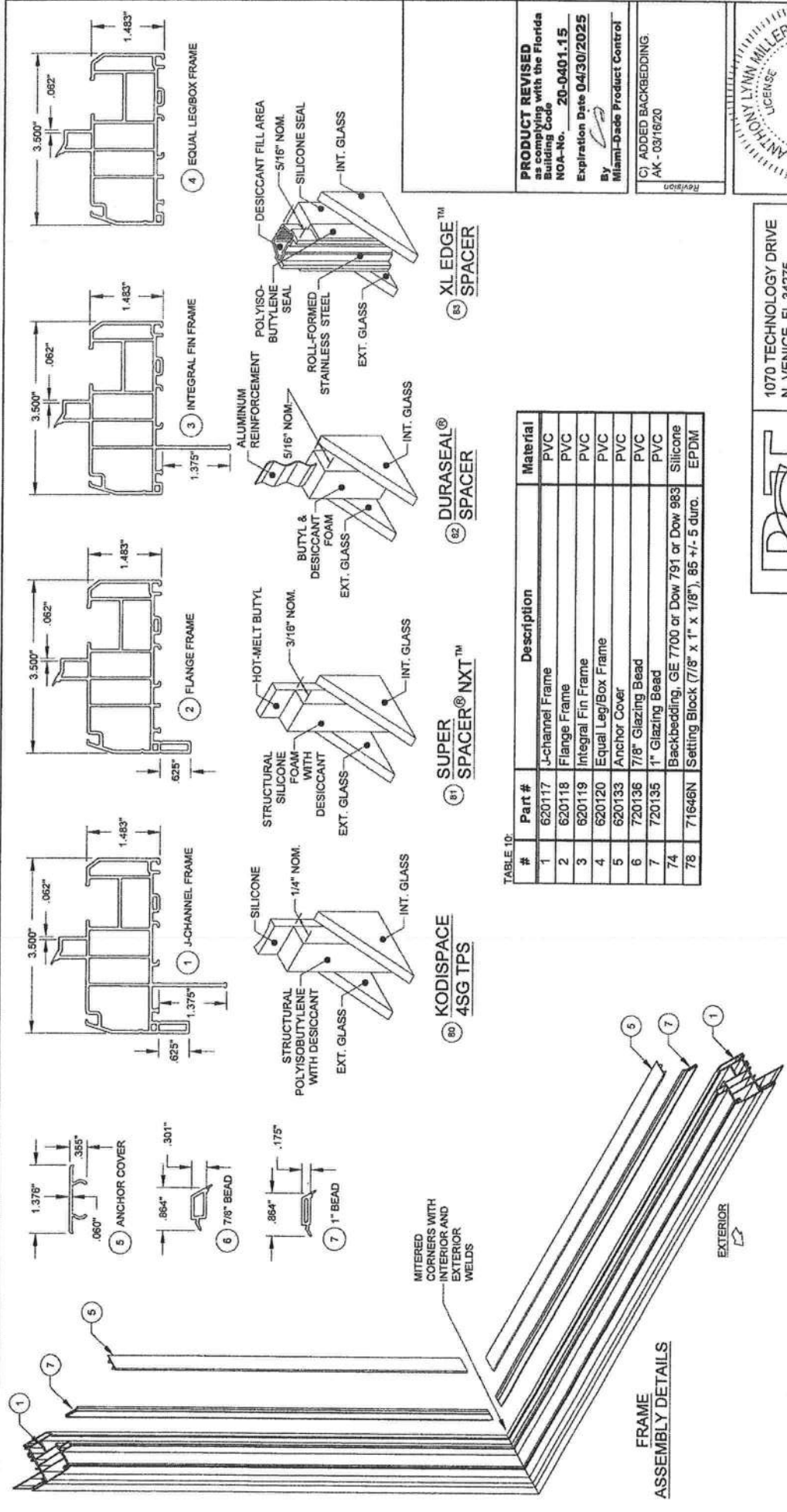
A. LYNN MILLER, P.E.
P.E.# 55705



NOTES:
1) BUCK DIMENSIONS SHOWN.
2) FOR SIZES NOT SHOWN, ROUND UP TO THE NEXT AVAILABLE SHORT OR LONG DIMENSION.
3) FOR ARCHITECTURAL WINDOWS (SEE SHEET 2), FIND THE SMALLEST SQUARE WINDOW SIZE IN THE TABLE(S) ABOVE WHICH THE ARCHITECTURAL WINDOW WILL COMPLETELY FIT WITHIN.

Window Design Pressure, (+/- psf)														
1/4" A Cap - Airspace - 1/4" A														
Window Dimensions	Long Side (in)													
	81.52	83	85	87	89	92	94	97	99	103	107	110	111	
46	+/-67.1	+/-66.7	+/-66.1	+/-65.3	+/-63.2	+/-60.1	+/-58	+/-55.1	+/-53.6	+/-51.3	+/-48.8	+/-46.8	+/-46.4	
48	+/-69.4	+/-68.9	+/-68.3	+/-67.5	+/-65.4	+/-62.3	+/-59.2	+/-56.3	+/-54.8	+/-52.5	+/-49.9	+/-47.9	+/-47.5	
50	+/-71.6	+/-71.2	+/-70.5	+/-69.7	+/-67.6	+/-64.5	+/-61.4	+/-58.5	+/-57.2	+/-54.9	+/-50.6	+/-47.3	+/-45.9	
52	+/-73.8	+/-73.4	+/-72.7	+/-71.9	+/-69.8	+/-66.7	+/-63.6	+/-60.7	+/-59.2	+/-57.1	+/-54.7	+/-50.6	+/-45.4	
54	+/-76.1	+/-75.7	+/-75	+/-74.2	+/-72.1	+/-69	+/-65.9	+/-63	+/-61.4	+/-59.1	+/-56.9	+/-54.7	+/-45.4	
56	+/-78.3	+/-77.9	+/-77.2	+/-76.4	+/-74.3	+/-71.2	+/-68.1	+/-65.2	+/-63.6	+/-61.3	+/-59.1	+/-56.9	+/-45.4	
58	+/-80.5	+/-80.1	+/-79.4	+/-78.6	+/-76.5	+/-73.4	+/-70.3	+/-67.4	+/-65.8	+/-63.5	+/-61.3	+/-59.1	+/-45.9	
60	+/-82.7	+/-82.3	+/-81.6	+/-80.8	+/-78.7	+/-75.6	+/-72.5	+/-69.6	+/-68	+/-65.7	+/-63.1	+/-60.9	+/-45.5	
62	+/-84.9	+/-84.5	+/-83.8	+/-83	+/-80.9	+/-77.8	+/-74.7	+/-71.8	+/-70.2	+/-67.9	+/-65.3	+/-63.1	+/-45.9	
64	+/-87.1	+/-86.7	+/-86	+/-85.2	+/-83.1	+/-80	+/-76.9	+/-74	+/-72.4	+/-70.1	+/-67.5	+/-65.3	+/-45.9	
67	+/-89.3	+/-88.9	+/-88.2	+/-87.4	+/-85.3	+/-82.2	+/-79.1	+/-76.2	+/-74.6	+/-72.3	+/-69.7	+/-67.5		
68	+/-91.5	+/-91.1	+/-90.4	+/-89.6	+/-87.5	+/-84.4	+/-81.3	+/-78.4	+/-76.8	+/-74.5	+/-71.9	+/-69.7		
70	+/-93.7	+/-93.3	+/-92.6	+/-91.8	+/-89.7	+/-86.6	+/-83.5	+/-80.6	+/-79	+/-76.7	+/-74.1	+/-71.9		
72	+/-95.9	+/-95.5	+/-94.8	+/-94	+/-91.9	+/-88.8	+/-85.7	+/-82.8	+/-81.2	+/-78.9	+/-76.3	+/-74.1		
74	+/-98.1	+/-97.7	+/-97	+/-96.2	+/-94.1	+/-91	+/-87.9	+/-85	+/-83.3	+/-81	+/-78.5	+/-76.3		
76	+/-100.3	+/-100	+/-99.3	+/-98.5	+/-96.4	+/-93.3	+/-90.2	+/-87.3	+/-85.7	+/-83.4	+/-80.8	+/-78.6		
78	+/-102.5	+/-102.1	+/-101.4	+/-100.6	+/-98.5	+/-95.4	+/-92.3	+/-89.4	+/-87.8	+/-85.5	+/-82.9	+/-80.7		
80	+/-104.7	+/-104.3	+/-103.6	+/-102.8	+/-100.7	+/-97.6	+/-94.5	+/-91.6	+/-90	+/-87.7	+/-85.1	+/-82.9		
81.52	+/-106.9	+/-106.5	+/-105.8	+/-105	+/-102.9	+/-99.8	+/-96.7	+/-93.8	+/-92.2	+/-89.9	+/-87.3	+/-85.1		
MAX. O.C. SPACING IF ANCHORING THROUGH THE FRAME PER SHEETS 3 & 4 THROUGH THE INTEGRAL FIN PER SHEET 4 APPLIES TO A, B, C OR D ANCHORS (SEE TABLE 2)														
MAX. O.C. SPACING IF ANCHORING THROUGH THE INTEGRAL FIN PER SHEET 4 APPLIES TO E OR F ANCHORS (SEE TABLE 3)														
2.6" FOR E ANCHORS, 4" FOR F ANCHORS														

Window Dimensions		Window Design Pressure, (+/- psf)																		Use this table for Glass Type:	
		1/4" T Cap - Airspace - 1/4" T																		6	
Long Side (in)																					
46	84.85	86	90	92	94	97	100	102	105	109	112	116	120	124	128	133	138	144			
	+80/-96.2	+80/-95.7	+80/-94.2	+80/-93.5	+80/-92.8	+80/-91.9	+80/-91.1	+80/-90.5	+80/-89.8	+80/-88.9	+80/-88.2	+80/-87.5	+80/-86.7	+80/-86.1	+80/-85.5	+80/-84.8	+80/-84.1	+80/-83.5			
48	+80/-93.7	+80/-93.2	+80/-91.6	+80/-90.9	+80/-90.2	+80/-89.3	+80/-88.4	+80/-87.8	+80/-87.1	+80/-86.2	+80/-85.5	+80/-84.7	+80/-84	+80/-83.3	+80/-82.7	+80/-82	+80/-81.3	+80/-80.6			
50	+80/-91.5	+80/-91	+80/-89.3	+80/-88.6	+80/-87.9	+80/-86.9	+80/-86	+80/-85.5	+80/-84.7	+80/-83.7	+80/-83	+80/-82.2	+80/-81.5	+80/-80.8	+80/-80.2	+/-79.4	+/-78.8	+80/-80.9			
52	+80/-89.4	+80/-88.9	+80/-87.2	+80/-86.5	+80/-85.7	+80/-84.7	+80/-83.8	+80/-83.3	+80/-82.4	+80/-81.5	+80/-80.8	+/-79.9	+/-78.2	+/-77.8	+/-77.1	+/-76.4	+/-75.7	+80/-77.1			
54	+80/-87.6	+80/-87.1	+80/-85.3	+80/-84.5	+80/-83.8	+80/-82.8	+80/-81.8	+80/-81.2	+80/-80.4	+/-79.4	+/-78.7	+/-77.9	+/-76.1	+/-75.7	+/-75.1	+/-74.4	+/-73.7	+80/-74.9			
56	+80/-84.8	+80/-84.3	+80/-83.6	+80/-82.8	+80/-82	+80/-81	+80	+/-79.4	+/-78.5	+/-77.5	+/-76.8	+/-75.9	+/-75.1	+/-74.4	+/-73.7	+/-73.1	+/-72.4	+80/-73.7			
58	+80/-82.6	+80/-82.1	+80/-81.2	+80/-80.4	+80/-80	+/-79.3	+/-78.3	+/-77.7	+/-76.8	+/-75.8	+/-75	+/-74.2	+/-73.3	+/-72.6	+/-71.9	+/-71.2	+/-70.5	+80/-71.7			
60	+80/-80.2	+80/-79.7	+80/-78.8	+80/-78	+80/-77.1	+80/-76.1	+80/-75.1	+80/-74.4	+80/-73.5	+80/-72.6	+80/-71.9	+80/-71.1	+80/-70.2	+80/-69.4	+80/-68.7	+80/-67.9	+80/-67.1	+80/-66.3			
62	+80/-77.8	+80/-77.3	+80/-76.4	+80/-75.5	+80/-74.6	+80/-73.6	+80/-72.6	+80/-71.9	+80/-71.2	+80/-70.4	+80/-69.6	+80/-68.8	+80/-68	+80/-67.2	+80/-66.4	+80/-65.6	+80/-64.8	+80/-64			
64	+80/-75.4	+80/-74.9	+80/-74	+80/-73.1	+80/-72.2	+80/-71.2	+80/-70.3	+80/-69.4	+80/-68.5	+80/-67.6	+80/-66.7	+80/-65.8	+80/-64.9	+80/-64	+80/-63.1	+80/-62.2	+80/-61.3	+80/-60.4			
66	+80/-73.0	+80/-72.5	+80/-71.6	+80/-70.7	+80/-69.8	+80/-68.8	+80/-67.9	+80/-67	+80/-66.1	+80/-65.2	+80/-64.3	+80/-63.4	+80/-62.5	+80/-61.6	+80/-60.7	+80/-59.8	+80/-58.9	+80/-58			
68	+80/-70.6	+80/-70.1	+80/-69.2	+80/-68.3	+80/-67.4	+80/-66.4	+80/-65.5	+80/-64.6	+80/-63.7	+80/-62.8	+80/-61.9	+80/-61	+80/-60.1	+80/-59.2	+80/-58.3	+80/-57.4	+80/-56.5	+80/-55.6			
70	+80/-68.2	+80/-67.7	+80/-66.8	+80/-65.9	+80/-65	+80/-64.1	+80/-63.2	+80/-62.3	+80/-61.4	+80/-60.5	+80/-59.6	+80/-58.7	+80/-57.8	+80/-56.9	+80/-56	+80/-55.1	+80/-54.2	+80/-53.3			
72	+80/-65.8	+80/-65.3	+80/-64.4	+80/-63.5	+80/-62.6	+80/-61.7	+80/-60.8	+80/-59.9	+80/-59	+80/-58.1	+80/-57.2	+80/-56.3	+80/-55.4	+80/-54.5	+80/-53.6	+80/-52.7	+80/-51.8	+80/-50.9			
74	+80/-63.4	+80/-62.9	+80/-62	+80/-61.1	+80/-60.2	+80/-59.3	+80/-58.4	+80/-57.5	+80/-56.6	+80/-55.7	+80/-54.8	+80/-53.9	+80/-53	+80/-52.1	+80/-51.2	+80/-50.3	+80/-49.4	+80/-48.5			
76	+80/-61.0	+80/-60.5	+80/-59.6	+80/-58.7	+80/-57.8	+80/-56.9	+80/-56	+80/-55.1	+80/-54.2	+80/-53.3	+80/-52.4	+80/-51.5	+80/-50.6	+80/-49.7	+80/-48.8	+80/-47.9	+80/-47	+80/-46.1			
78	+80/-58.6	+80/-58.1	+80/-57.2	+80/-56.3	+80/-55.4	+80/-54.5	+80/-53.6	+80/-52.7	+80/-51.8	+80/-50.9	+80/-50	+80/-49.1	+80/-48.2	+80/-47.3	+80/-46.4	+80/-45.5	+80/-44.6	+80/-43.7			
80	+80/-56.2	+80/-55.7	+80/-54.8	+80/-53.9	+80/-53	+80/-52.1	+80/-51.2	+80/-50.3	+80/-49.4	+80/-48.5	+80/-47.6	+80/-46.7	+80/-45.8	+80/-44.9	+80/-44	+80/-43.1	+80/-42.2	+80/-41.3			
83	+80/-53.8	+80/-53.3	+80/-52.4	+80/-51.5	+80/-50.6	+80/-49.7	+80/-48.8	+80/-47.9	+80/-47	+80/-46.1	+80/-45.2	+80/-44.3	+80/-43.4	+80/-42.5	+80/-41.6	+80/-40.7	+80/-39.8	+80/-38.9			
84.85	+80/-51.4	+80/-50.9	+80/-50	+80/-49.1	+80/-48.2	+80/-47.3	+80/-46.4	+80/-45.5	+80/-44.6	+80/-43.7	+80/-42.8	+80/-41.9	+80/-41	+80/-40.1	+80/-39.2	+80/-38.3	+80/-37.4	+80/-36.5			
MAX. O.C. SPACING IF ANCHORING THROUGH THE FRAME PER SHEETS 3 & 4 THROUGH THE INTEGRAL FIN PER SHEET 4 APPLIES TO B, C OR D ANCHORS (SEE TABLE 2)																					
MAX. O.C. SPACING IF ANCHORING THROUGH THE FRAME PER SHEETS 3 & 4 THROUGH THE INTEGRAL FIN PER SHEET 4 APPLIES TO E ANCHORS (SEE TABLE 3)																					
15.5"																					
3.2"																					



Part #	Description	Material
80	Kommerling 4SG TPS Spacer System	See this Sheet for Materials
81	Quanex Super Spacer nXT with Hot Melt Butyl	
82	Quanex Duraseal Spacer	
83	Cardinal XL Edge Spacer	

REFERENCE TEST REPORTS: FTL-8717, 8668 & 8870

- NOTES:
- 1) SOME PARTS NOT SHOWN FOR CLARITY.
 - 2) J-CHANNEL FRAME SHOWN, PARTS #1.
 - 3) OTHER FRAME TYPES APPLY.
 - 4) ITEMS # 8-73, 75-77, 79 & 84-89 ARE NOT USED AND ARE NOT PART OF THIS APPROVAL.
 - 5) ENERGI PVC TO BE LABELED FOR AAMA EXTRUDER CODE.

TABLE 10:

#	Part #	Description	Material
1	620117	J-channel Frame	PVC
2	620118	Flange Frame	PVC
3	620119	Integral Fin Frame	PVC
4	620120	Equal Leg/Box Frame	PVC
5	620133	Anchor Cover	PVC
6	720136	7/8" Glazing Bead	PVC
7	720135	1" Glazing Bead	PVC
74		Backbedding, GE 7700 or Dow 983	Silicone
78	71646N	Setting Block (7/8" x 1" x 1/8"), 85 +/- 5 duro.	EPDM

- 80 KODISPACE 4SG TPS
- 81 SUPER SPACER[®] NXT[™]
- 82 DURASEAL[®]
- 83 XL EDGE[™] SPACER

PRODUCT REVISED
as complying with the Florida Building Code
NOA-No. 20-0401.15
Expiration Date 04/30/2025
By Miami-Dade Product Control

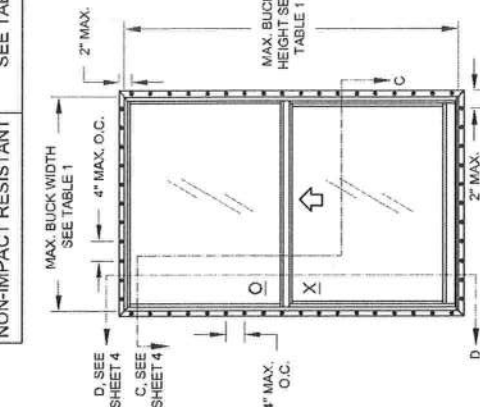
C) ADDED BACKBEDDING.
AK - 03/18/20

ANTHONY LYNN MILLER
LICENSE
No. 58705
03/19/20
FLORIDA
PROFESSIONAL ENGINEER
STATE OF
A LYNN MILLER, P.E.
P.E. # 58705

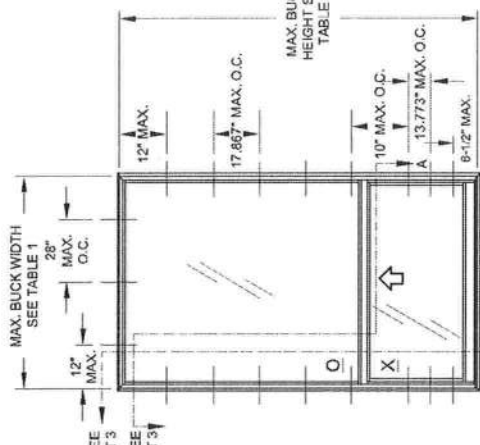
1070 TECHNOLOGY DRIVE
N. VENICE, FL 34275
(941)-480-1600

REGISTRATION #29296
VINYL FIXED WINDOW NOA (NI)
Date 9/9/14
J ROSOWSKI
BOM & ASSEMBLY
PW-5420 NTS 9 OF 9 MD-5420.0 C

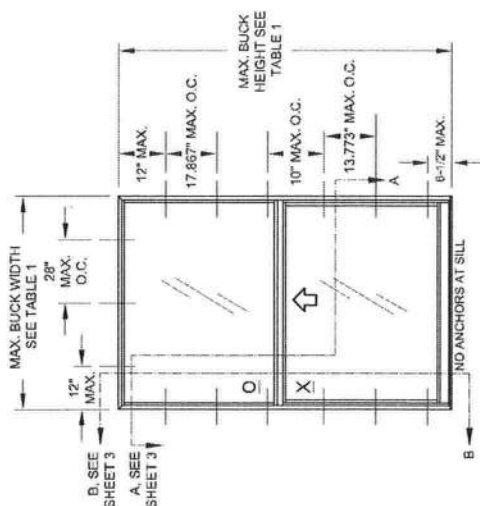
IMPACT RATING	DESIGN PRESSURE RATING
NON-IMPACT RESISTANT	SEE TABLE 1



ELEVATION FOR TYP. FIN OR J-CHANNEL FRAME,
EQUAL-LITE CONFIGURATION
(SIMILAR ANCHOR DIMENSIONS FOR OTHER CONFIGURATIONS)



ELEVATION FOR TYP. FLANGE FRAME,
PROVIEW/ORIEL CONFIGURATION
(COTTAGE SIMILAR)



ELEVATION FOR TYP. EQUAL LEG FRAME,
EQUAL-LITE CONFIGURATION

GENERAL NOTES: SERIES 5400 NON-IMPACT RESISTANT,
VINYL SINGLE HUNG WINDOW

- THIS PRODUCT HAS BEEN DESIGNED & TESTED TO COMPLY WITH THE REQUIREMENTS OF THE FLORIDA BUILDING CODE.
- ALL WOOD BUCKS LESS THAN 1-1/2\"
- ANCHOR EMBEDMENT TO BASE MATERIAL SHALL BE BEYOND WALL DRESSING OR STUCCO. USE ANCHORS OF SUFFICIENT EMBEDMENT. INSTALLATION ANCHORS SHOULD BE SEALED. OVERALL SEALING/FLASHING STRATEGY FOR WATER RESISTANCE OF INSTALLATION SHALL BE DONE BY OTHERS AND IS BEYOND THE SCOPE OF THESE INSTRUCTIONS.
- MAX. 1/4\"
- THE ANCHORAGE METHODS SHOWN HAVE BEEN DESIGNED TO RESIST THE WINDLOADS CORRESPONDING TO THE REQUIRED DESIGN PRESSURE. THE 33-1/3% STRESS INCREASE HAS NOT BEEN USED IN THE DESIGN OF THIS PRODUCT. THE 1.8 LOAD DURATION FACTOR WAS USED FOR THE EVALUATION OF ANCHORS INTO WOOD. ANCHORS THAT COME INTO CONTACT WITH OTHER DISSIMILAR MATERIALS SHALL MEET THE REQUIREMENTS OF THE FLORIDA BUILDING CODE FOR CORROSION RESISTANCE.

TABLE 1:

Window Buck Size	Reinf. Level	Design Pressure	Certification (CAR) Number
Width		(+) psf	(-) psf
40"	R4	50.0	70.0
36"	R4	50.0	50.0
36"	R1	50.0	50.0
52-1/8"	R2	65.0	70.0
52-1/8"			
52-1/8"			
52-1/8"			
52-1/8"			
52-1/8"			

MIN. SASH HEIGHT = WINDOW BUCK HEIGHT - 50.136 (APPLIES TO ANY HEIGHT 91.78" OR LESS).

SHAPES MAY BE USED BY INSCRIBING THE SHAPE IN A BLOCK AND OBTAINING DESIGN PRESSURES FOR THAT BLOCK SIZE FROM THE TABLE ON THIS SHEET.

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1070 TECHNOLOGY DRIVE
N. VENICE, FL 34275 (941) 480-1600



A. LYNN MILLER, P.E.
P.E.# 88705

Rev	09/30/11
By	J. ROSOWSKI
Check	SH5400-FPA
Date	1 OF 4
Sheet	SH 5400
General Notes & Elevations	VINYL SINGLE HUNG INSTALLATION, NI
Added Anchors to Table 2	210/2/2R20

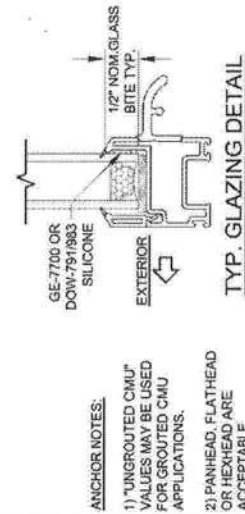


TABLE 2: ANCHORS INSTALLED THROUGH FRAME

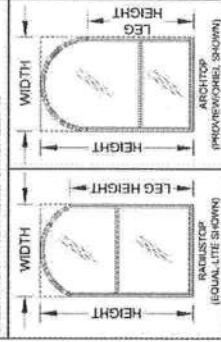
Anchor	Substrate	Min. Edge Distance	Min. Embedment
#10 SMS (steel, 18-8 S.S., or 410 S.S.) Max. DP of 50.0 psf	P.T. Southern Pine (SG=0.55)	7/16"	1-3/8"
	Steel Stud, A36	3/8"	0.050"
	Steel Stud, A653 Gr. 33 Aluminum, 6063-T5	3/8"	0.0346" (20 Ga.) 0.0713" (14 Ga.)
#12 SMS (steel, 18-8 S.S., or 410 S.S.)	P.T. Southern Pine (SG=0.55)	9/16"	1-3/8"
	Steel Stud, A36	3/8"	0.050"
	Steel Stud, A653 Gr. 33 Aluminum, 6063-T5	3/8"	0.0346" (20 Ga.) 0.0713" (14 Ga.)
3/16" Ultracon Max. DP of 50.0 psf	P.T. Southern Pine (SG=0.55)	7/16"	1-3/8"
	Concrete (min. 3,000 ksi)	1"	1-3/8"
	Ungrouted CMU, (ASTM C-90)	2-1/2"	1-1/4"
3/16" Ultracon+ Max. DP of 50.0 psf	P.T. Southern Pine (SG=0.55)	7/16"	1-3/8"
	Concrete (min. 3 ksi)	1"	1-3/8"
	Ungrouted CMU, (ASTM C-90)	1"	1-1/4"
1/4" Ultracon	P.T. Southern Pine (SG=0.55)	1"	1-3/8"
	Concrete (min. 2.85 ksi)	1"	1-3/4"
	Ungrouted CMU, (ASTM C-90)	2-1/2"	1-1/4"
1/4" Ultracon+	P.T. Southern Pine (SG=0.55)	1"	1-3/8"
	Concrete (min. 3 ksi)	1-3/16"	1-3/4"
	Ungrouted CMU, (ASTM C-90)	1"	1-1/4"
1/4" Crete-Flex (410 S.S.)	P.T. Southern Pine (SG=0.55)	1"	1-3/8"
	Concrete (min. 3,35 ksi)	1"	1-3/4"
	Ungrouted CMU, (ASTM C-90)	2-1/2"	1-1/4"
1/4" Aggre-Gator (18-8 S.S.)	P.T. Southern Pine (SG=0.55)	1-1/2"	1-3/8"
	Concrete (min. 3,275 ksi)	1"	1-3/8"
	Ungrouted CMU, (ASTM C-90)	2"	1-1/4"

TABLE 3: ANCHORS INSTALLED THROUGH INTEGRAL FIN

Anchor	Substrate	Min. Edge Distance	Min. Embedment
2-1/2" x .131" Common Nail Max. DP of 50.0	P.T. Southern Pine (SG=0.55)	9/16"	2-7/16"
	P.T. Southern Pine (SG=0.55)	9/16"	2-7/16"
	P.T. Southern Pine (SG=0.55)	9/16"	2-7/16"
2-1/2" x .131" Ring-shank Nail Max. DP of 50.0	P.T. Southern Pine (SG=0.55)	3/4"	1-3/8"
	Aluminum, 6063-T5	3/8"	0.0713" (14 Ga.)
	Steel Stud, Gr. 33	3/8"	0.0346" (20 Ga.)
#10 SMS (steel, 18-8 S.S., or 410 S.S.)	Steel Stud, Gr. 33	3/8"	0.050"
	Steel, A36	3/8"	0.050"
	Steel, A36	3/8"	0.050"



WINDOW SHAPES AS SHOWN BELOW OR SIMILAR MAY BE USED BY INSURING THE SHAPE IN A BLOCK AND OBTAINING DESIGN PRESSURES AND ANCHORAGE FOR THAT BLOCK SIZE FROM THE TABLE ON SHEET 1.



TYP. GLAZING DETAIL

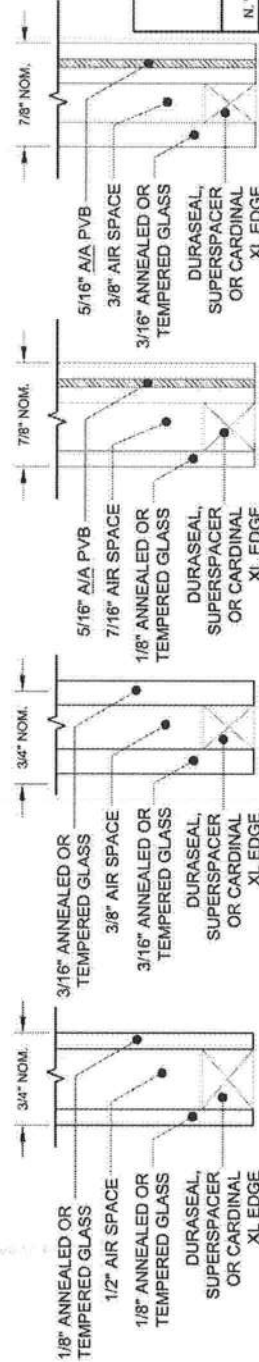
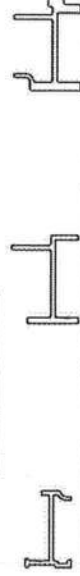


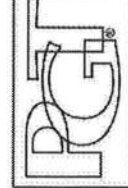
TABLE 4: REINFORCEMENT TYPES

Level	Reinforcement			
	Upper Lite Bottom Rail	Top Rail	Bottom Rail	Side Rails
R1	B	A	A	A
R2	C	A	A	A
R4	B	A	A	N/A

GLAZING TYPES



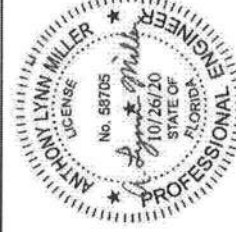
REINFORCEMENT TYPE A REINFORCEMENT TYPE B REINFORCEMENT TYPE C



VINYL SINGLE HUNG INSTALLATION, NI	Date	09/30/11
GLASS/ANCHORS/FRACTION OPTIONS	By	J ROSOWSKI
SH 5400	Rev.	2 OF 4
SH5400-FPA	Rev.	A

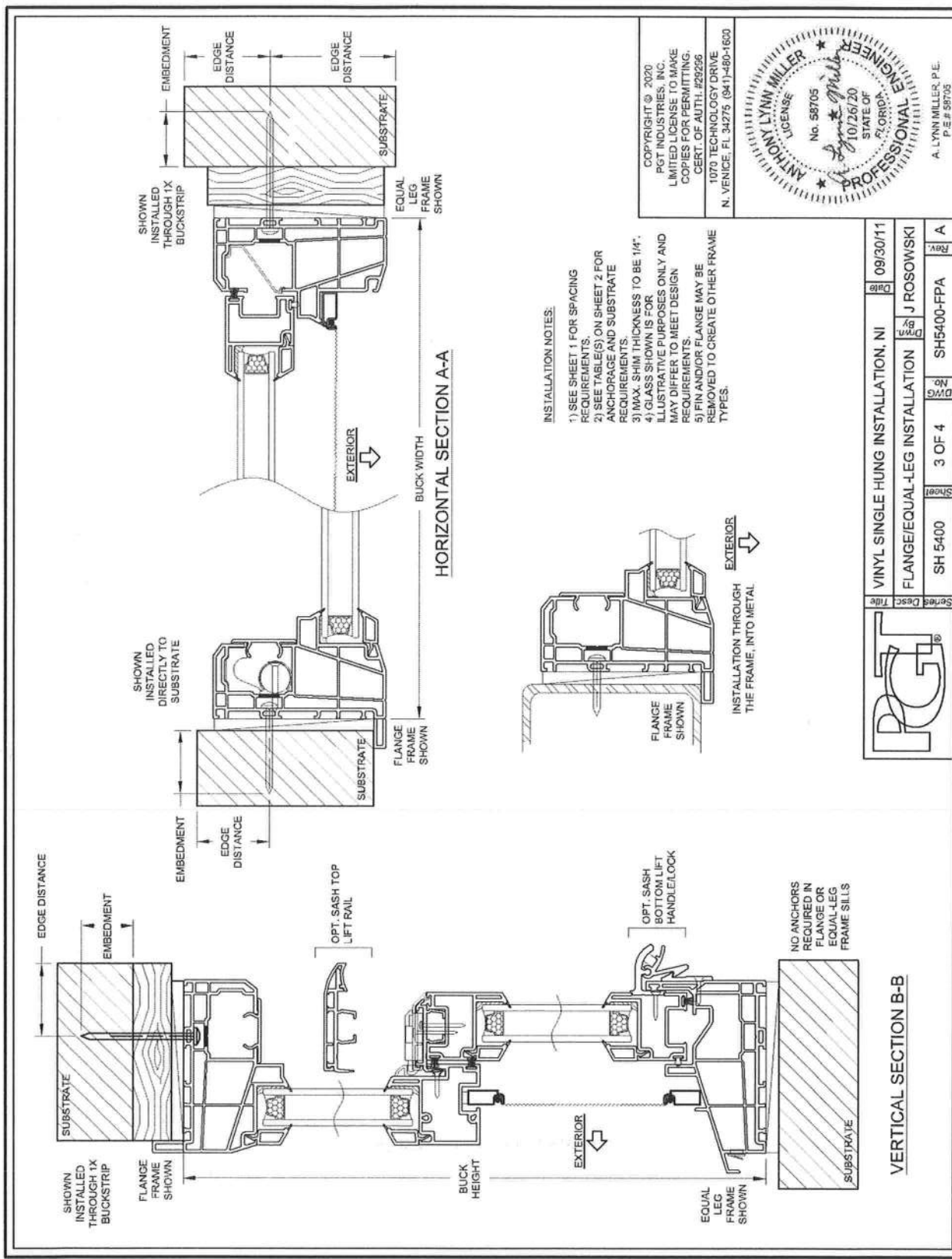
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1073 TECHNOLOGY DRIVE
N. VENICE, FL 34275 (941) 460-1600



A. LYNN MILLER, P.E.
P.E. # 58705

PVB INTERLAYER MANUFACTURED BY KURARAY AMERICA, INC.

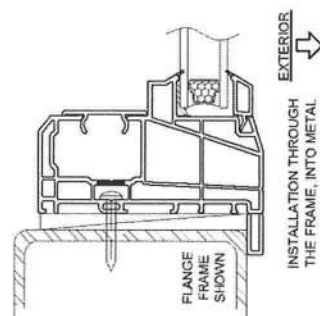


HORIZONTAL SECTION A-A

VERTICAL SECTION B-B

INSTALLATION NOTES:

- 1) SEE SHEET 1 FOR SPACING REQUIREMENTS.
- 2) SEE TABLE(S) ON SHEET 2 FOR ANCHORAGE AND SUBSTRATE REQUIREMENTS.
- 3) MAX. SHIM THICKNESS TO BE 1/4".
- 4) GLASS SHOWN IS FOR ILLUSTRATIVE PURPOSES ONLY AND MAY DIFFER TO MEET DESIGN REQUIREMENTS.
- 5) FIN AND/OR FLANGE MAY BE REMOVED TO CREATE OTHER FRAME TYPES.

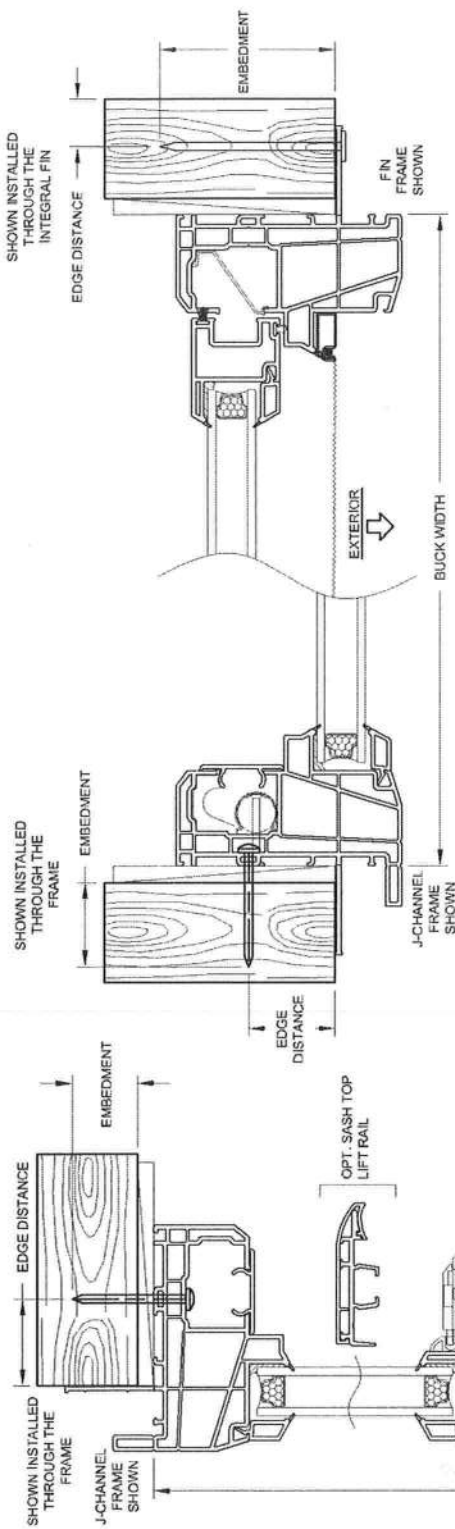


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1070 TECHNOLOGY DRIVE
N. VENICE, FL 34275 (941) 460-1600

ANTHONY LYNN MILLER
No. 58705
10/26/20
STATE OF FLORIDA
PROFESSIONAL ENGINEER
P.E. # 58705

A LYNN MILLER, P.E.
P.E. # 58705

	Title	Desc	Date	Rev
	VINYL SINGLE HUNG INSTALLATION, NI	J ROSOWSKI	09/30/11	A
SH 5400	3 OF 4	SH5400-FPA	Rev	A



HORIZONTAL SECTION C-C

VERTICAL SECTION D-D

INSTALLATION NOTES:

- 1) SEE SHEET 1 FOR SPACING REQUIREMENTS.
- 2) SEE TABLE(S) ON SHEET 2 FOR ANCHORAGE AND SUBSTRATE REQUIREMENTS.
- 3) MAX. SHIM THICKNESS TO BE 1/4".
- 4) GLASS SHOWN IS FOR ILLUSTRATIVE PURPOSES ONLY AND MAY DIFFER TO MEET DESIGN REQUIREMENTS.
- 5) FIN AND/OR FLANGE MAY BE REMOVED TO CREATE OTHER FRAME TYPES.

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1070 TECHNOLOGY DRIVE
N. VENICE, FL 34275 (941) 480-1600



A. LYNN MILLER, P.E.
P.E.# 58705

	Series	Desc	Title	VINYL SINGLE HUNG INSTALLATION, NI		Date	09/30/11
				INTEGRAL FIN INSTALLATION		Drawn By	J ROSOWSKI
				SH 5400	DWG No	SH5400-FPA	
				4 OF 4	Sheet	Rev.	
						A	



plastro

5200 W. CENTURY BLVD.
LOS ANGELES, CA 90045

Smooth / Wood Grain / White Wood Grain

Rustic / Mahogany

Series N Fiberglass Door

INSWING / OUTSWING

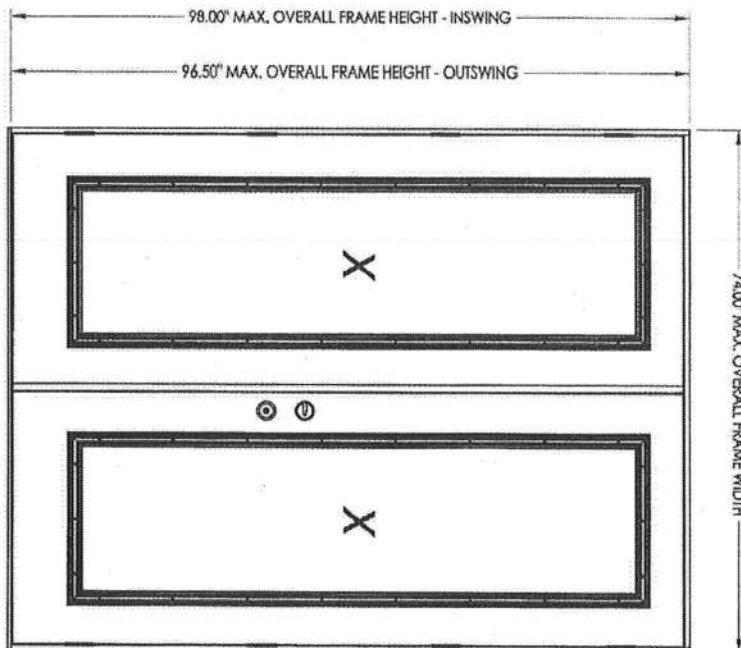
"NON-IMPACT"

GENERAL NOTES

1. This product has been evaluated and is in compliance with the 7th Edition (2020) Florida Building Code (FBC) structural requirements including the "High Velocity Hurricane Zone" (HVHZ).
2. Product anchors shall be as listed and spaced as shown on details. Anchor embedment to base material shall be beyond wall dressing or stucco.
3. When used in the "HVHZ" this product complies with Section 1626 of the Florida Building Code and does not require an impact resistant covering.
4. When used in areas requiring wind borne debris protection, this product is required to be protected with an impact resistant covering that complies with FBC Sections 1609.1.2 & R301.2.1.2.
5. For 2x stud construction, anchoring of these units shall be the same as that shown for 2x buck masonry construction.
6. Site conditions that deviate from the details of this drawing require further engineering analysis by a licensed engineer or registered architect.
7. Outswing configurations meet water infiltration requirements for "HVHZ".
8. Inswing configurations do not meet the water infiltration requirements for the "HVHZ". Inswing units shall be installed only in non-habitable areas or at habitable locations protected by an overhang or canopy such that the angle between the edge of canopy or overhang to sill is less than 45 degrees.

TABLE OF CONTENTS

SHEET#	DESCRIPTION
1	Typical elevations, design pressures & general notes
2	Door panel details
3	Horizontal & Vertical Cross Sections (2x Buck)
4	Horizontal & Vertical Cross Sections (1x Buck)
5	Horizontal & Vertical Cross Sections (Direct to Masonry)
6	Horizontal & Vertical Cross Sections (Direct to Masonry)
7	Vertical Cross Sections (Thresholds)
8	Buck and frame anchoring
9	Astrogal details
10	Bill of materials & components



OVERALL FRAME DIMENSION	OVERALL D.L.O. DIMENSION	GLASS TYPE	DESIGN PRESSURE (PSF)
74.0\"	21.0\"	G1	+47.0
			-47.0

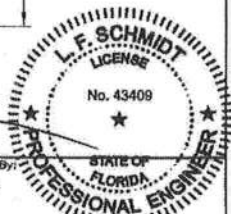
DATE: 02/20/12	SCALE: N.T.S.
DWG. BY: JK	CHK. BY: LFS
DRAWING NO.: FL-15220.14	SHEET 1 OF 10

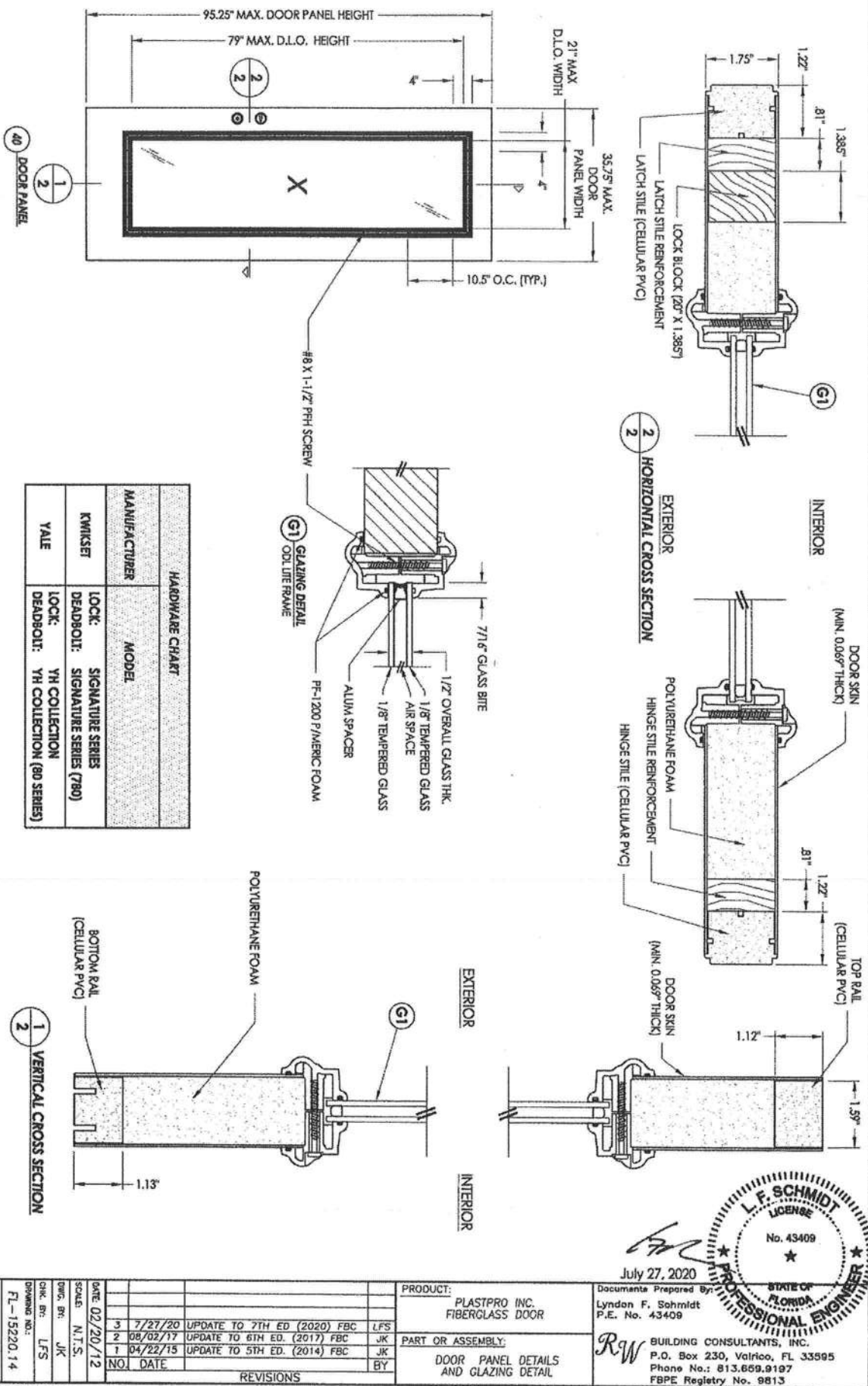
NO.	DATE	REVISIONS
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2	08/02/17	UPDATE TO 8TH ED. (2017) FBC
1	04/22/15	UPDATE TO 5TH ED. (2014) FBC

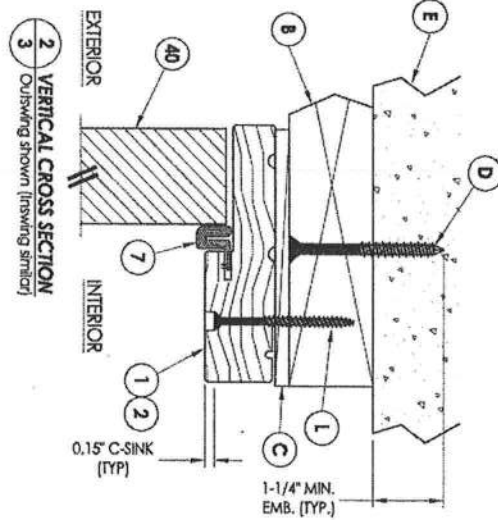
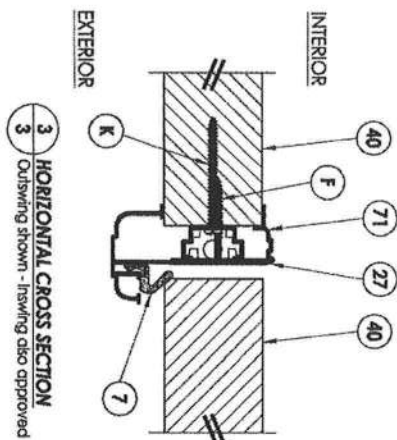
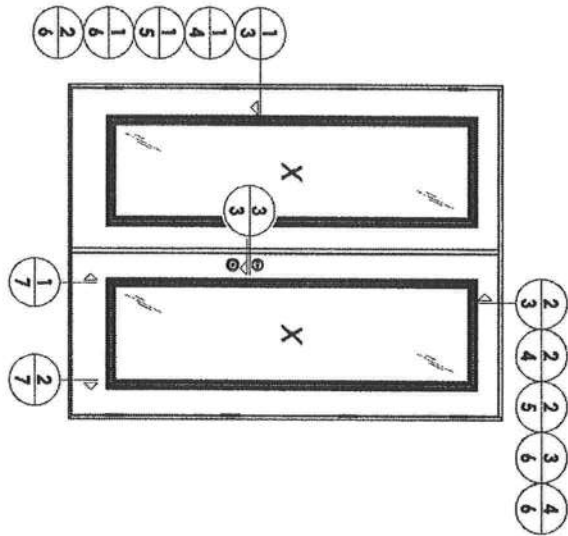
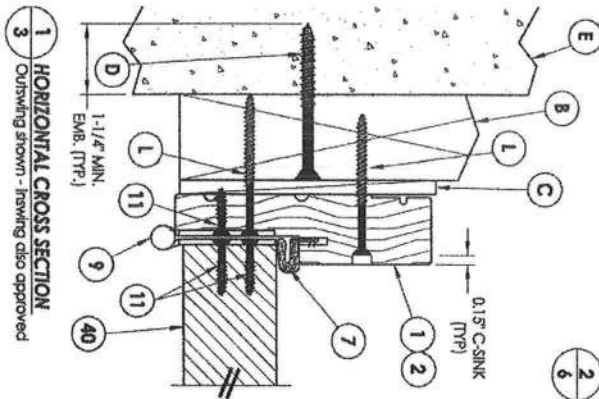
PRODUCT:	PLASTPRO INC. FIBERGLASS DOOR
PART OR ASSEMBLY:	TYPICAL ELEVATION, DESIGN PRESSURES & GENERAL NOTES

Documents Prepared By:
Lyndon F. Schmidt
P.E. No. 43409

BUILDING CONSULTANTS, INC.
P.O. Box 230, Valrico, FL 33595
Phone No.: 813.659.9197
FBPE Registry No. 9813







NO.	DATE	REVISIONS	BY
3	7/27/20	UPDATE TO 7TH ED. (2020) FBC	LFS
2	06/02/17	UPDATE TO 6TH ED. (2017) FBC	JK
1	04/22/15	UPDATE TO 5TH ED. (2014) FBC	JK

PRODUCT:
PLASTPRO INC.
FIBERGLASS DOOR

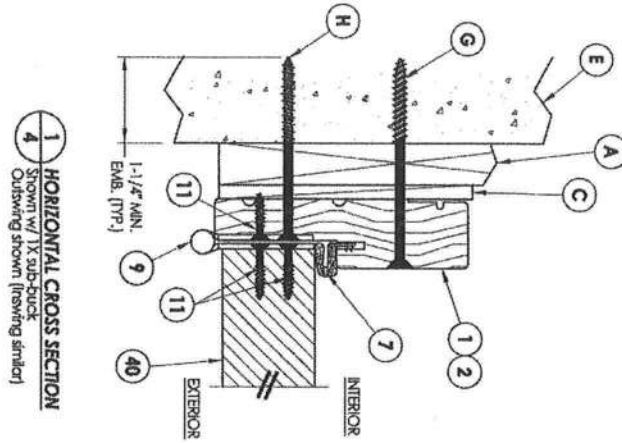
PART OR ASSEMBLY:
HORIZONTAL
CROSS SECTIONS

July 27, 2020

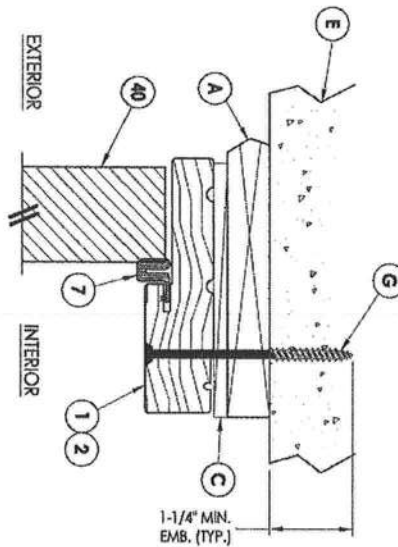
Documents Prepared By:
Lyndon F. Schmidt
P.E. No. 43409

BUILDING CONSULTANTS, INC.
P.O. Box 230, Valrico, FL 33595
Phone No.: 813.659.9197
FBPE Registry No. 9813





1 HORIZONTAL CROSS SECTION
4 Shown w/ 1x sub-buck
Outswing shown (inswing similar)

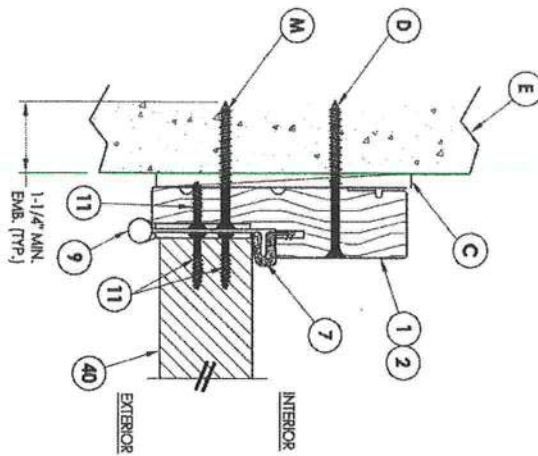


2 VERTICAL CROSS SECTION
4 Shown w/ 1x sub-buck
Outswing shown (inswing similar)

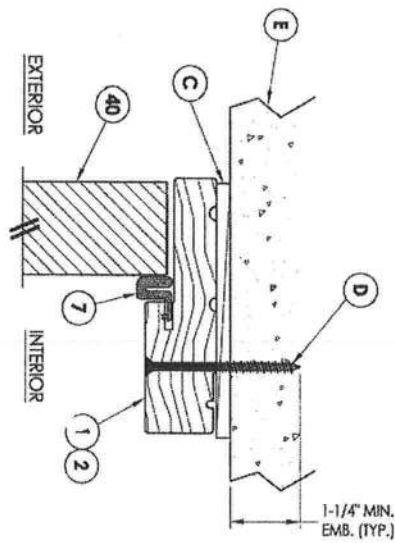
DATE: 02/20/12				PRODUCT:				Documents Prepared By:			
SCALE: N.T.S.				PLASTPRO INC.				Lyndon F. Schmidt			
CHK. BY: JK				FIBERGLASS DOOR				P.E. No. 43409			
DWG. BY: LFS				PART OR ASSEMBLY:				BUILDING CONSULTANTS, INC.			
DRAWING NO. FL-15220.14				HORIZONTAL & VERTICAL				P.O. Box 230, Valrico, FL 33595			
SHEET 4 OF 10				CROSS SECTIONS (1X BUCK)				Phone No.: 813.659.9197			
								FBPE Registry No. 9813			



1 HORIZONTAL CROSS SECTION
Shown Direct to Masonry
Outswing shown (Inswing similar)



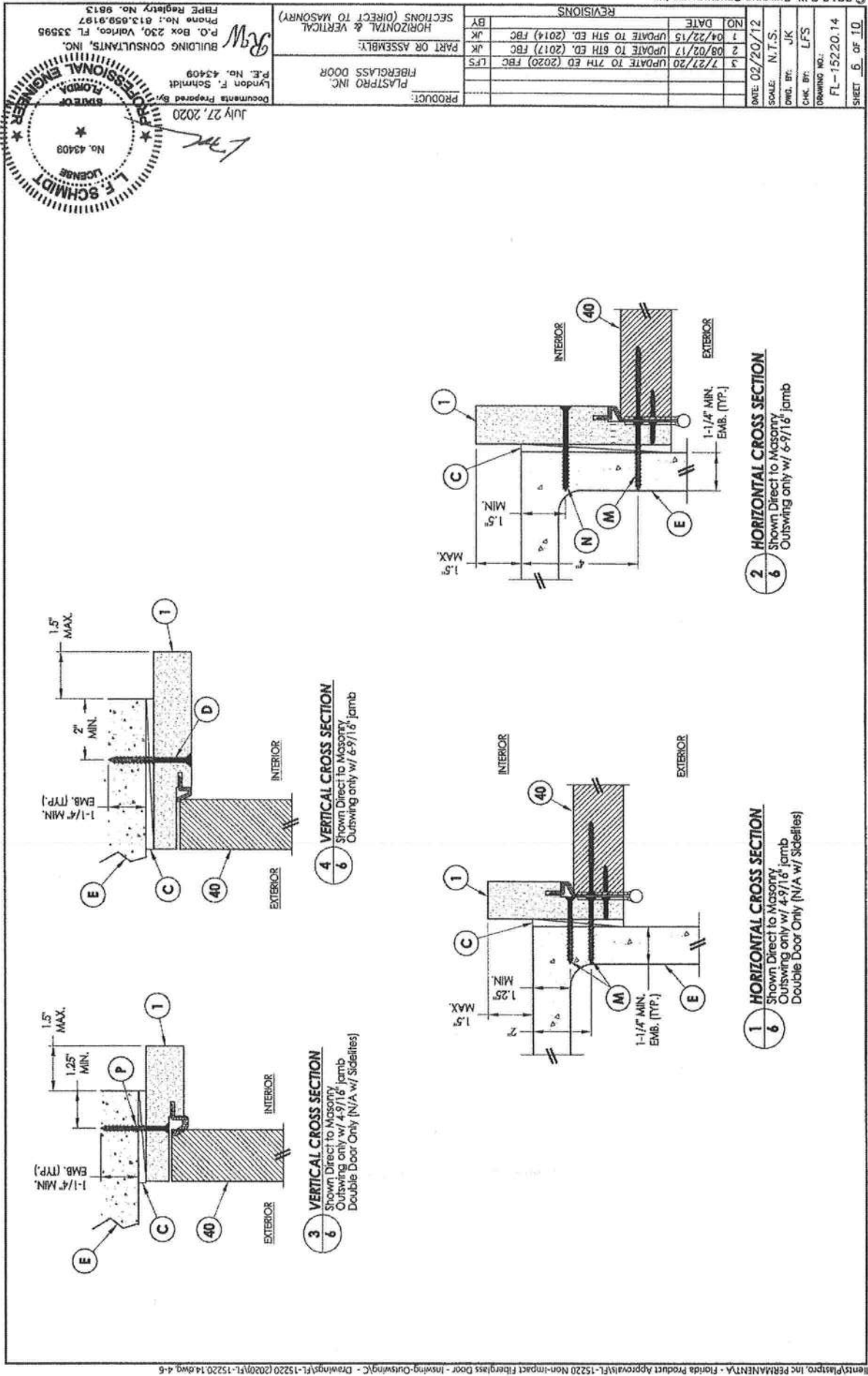
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Shown Direct to Masonry
Outswing shown (Inswing similar)

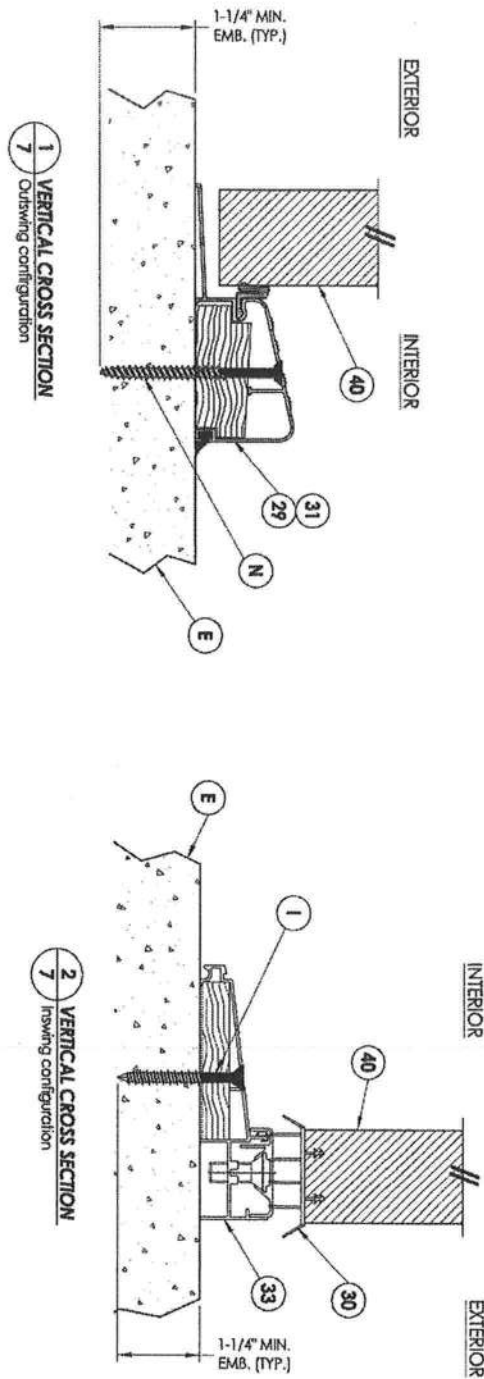


REVISIONS				PRODUCT:	
NO.	DATE	DESCRIPTION	BY	PART OR ASSEMBLY:	
3	7/27/20	UPDATE TO 7TH ED. (2020) FBC	LFS	HORIZONTAL & VERTICAL CROSS SECTIONS (DIRECT TO MASONRY)	
2	08/02/17	UPDATE TO 6TH ED. (2017) FBC	JK		
1	04/22/15	UPDATE TO 5TH ED. (2014) FBC	JK		
				PLASTPRO INC. FIBERGLASS DOOR	

July 27, 2020
 Documents Prepared By:
 Lyndon F. Schmidt
 P.E. No. 43409
 R.W. BUILDING CONSULTANTS, INC.
 P.O. Box 230, Valrico, FL 33595
 Phone No.: 813.659.9197
 FBPE Registry No. 9813







DATE: 02/20/12		SCALE: N.T.S.		DATE: 02/20/12	
CHG. BY: LFS		CHK. BY: JK		DATE: 02/20/12	
FL-15220.14		SHEET 7 of 10		DATE: 02/20/12	
NO.		DATE		REVISIONS	
3		7/27/20		UPDATE TO 7TH ED. (2020) FBC	
2		08/02/17		UPDATE TO 6TH ED. (2017) FBC	
1		04/22/15		UPDATE TO 5TH ED. (2014) FBC	
BY		JK		LFS	
PART OR ASSEMBLY:		VERTICAL CROSS SECTIONS		PRODUCT:	
PLASTPRO INC.		FIBERGLASS DOOR		DOCUMENTS PREPARED BY:	
Lyndon F. Schmidt		P.E. No. 43409		BUILDING CONSULTANTS, INC.	
P.O. Box 230, Vairco, FL 33595		Phone No.: 813.659.9197		FBPE Registry No. 9813	

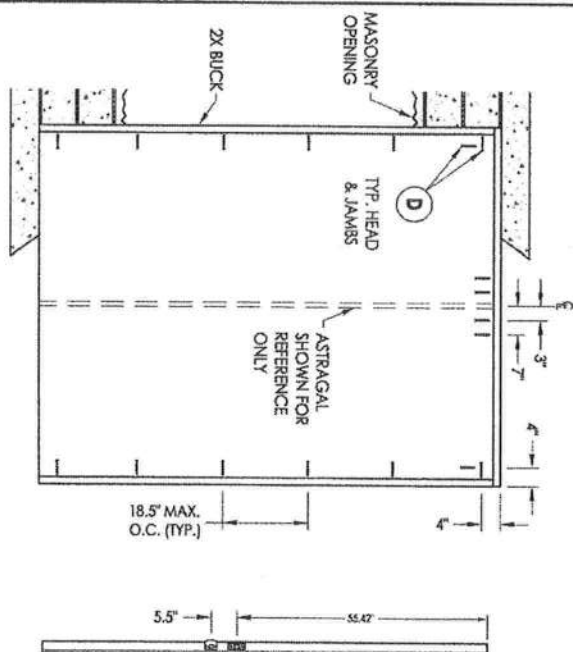
- CONCRETE ANCHOR NOTES:**
1. Concrete anchor locations at the corners may be adjusted to maintain the min. edge distance to mortar joints.
 2. Concrete anchor locations noted as "MAX. ON CENTER" must be adjusted to maintain the min. edge distance to mortar joints, additional concrete anchors may be required to ensure the "MAX. ON CENTER" dimension are not exceeded.
 3. Concrete anchor table:

ANCHOR TYPE	ANCHOR SIZE	MIN. EMBEDMENT	MIN. CLEARANCE TO MASONRY EDGE	MIN. CLEARANCE TO ADJACENT ANCHOR
ITW [®] TAPCON [®]	1/4"	1-1/4"	2"	4"
ELCO [®] ULTRACON [®]	1/4"	1-1/4"	1"	4"
ITW [®] TAPCON [®]	3/16"	1-1/4"	3"	1-1/2"

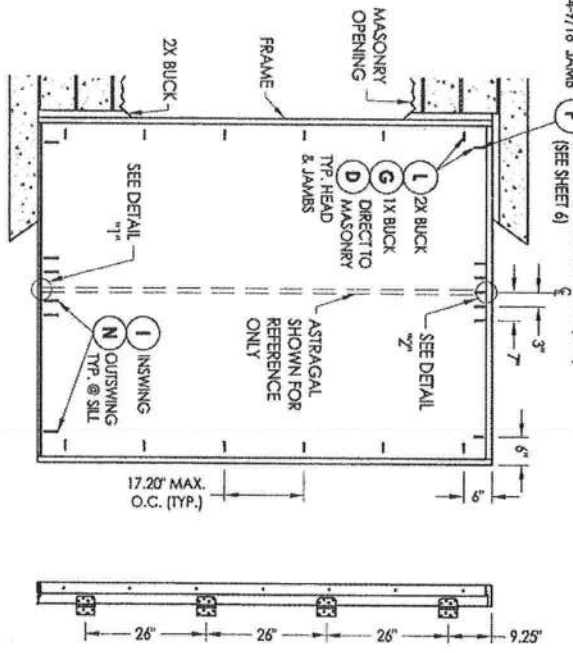
WOOD SCREW INSTALLATION NOTES:

1. Maintain a minimum 5/8" edge distance, 1" end distance, & 1" o.c. spacing of wood screws to prevent the splitting of wood.

BUCK ANCHORING

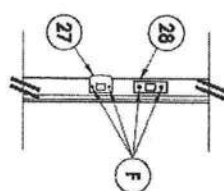


ASTRAGAL STRIKE JAMB

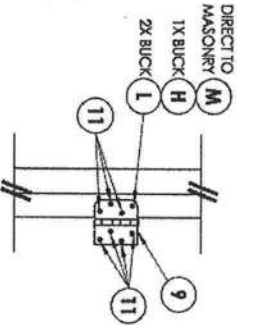
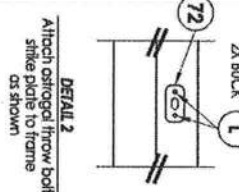
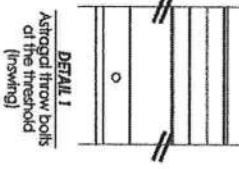
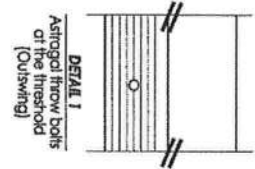


FRAME ANCHORING
Masonry 2x buck construction

HINGE JAMB



LATCH & DEAD BOLT DETAIL



PRODUCT: PLASTPRO INC. FIBERGLASS DOOR

PART OR ASSEMBLY: BUCK AND FRAME ANCHORING

DATE: 07/20/20

SCALE: N.T.S.

CHK. BY: JKS

DATE: 07/20/20

SCALE: N.T.S.

CHK. BY: JKS

DATE: 07/20/20

SCALE: N.T.S.

CHK. BY: JKS

REVISIONS

NO.	DATE	DESCRIPTION	BY
3	7/27/20	UPDATE TO 7TH ED. (2020) FBC	LFS
2	08/02/17	UPDATE TO 6TH ED. (2017) FBC	JK
1	04/22/15	UPDATE TO 5TH ED. (2014) FBC	JK

STATE OF FLORIDA

L.F. SCHMIDT

PROFESSIONAL ENGINEER

No. 43409

July 27, 2020

Documents Prepared By: Lyndon F. Schmidt

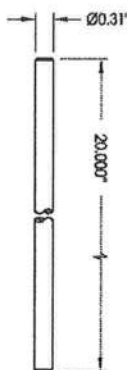
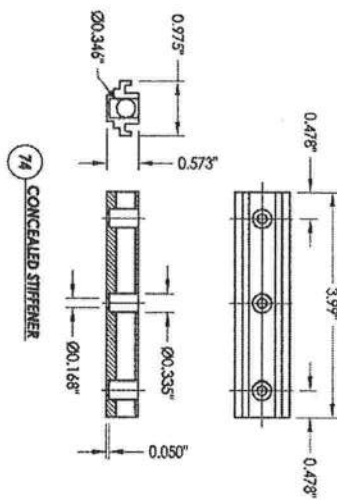
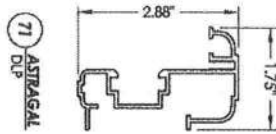
P.E. No. 43409

BUILDING CONSULTANTS, INC.

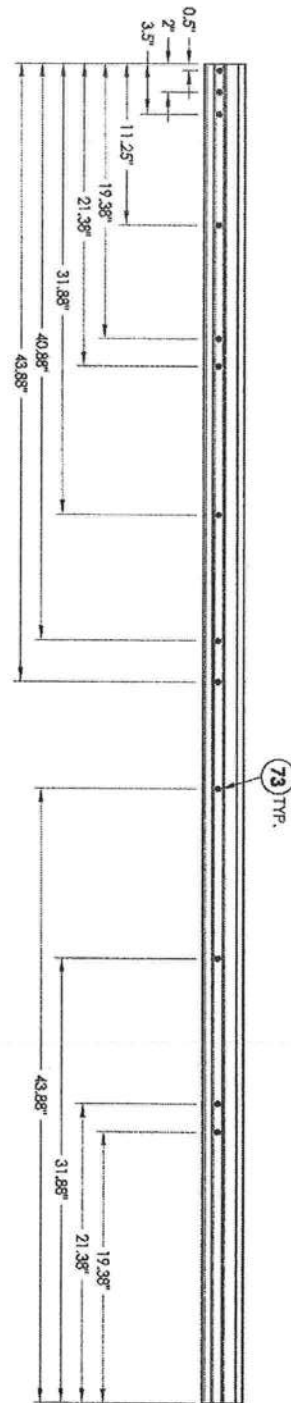
P.O. Box 230, Valrico, FL 33595

Phone No.: 813.659.9197

FBPE Registry No. 9813

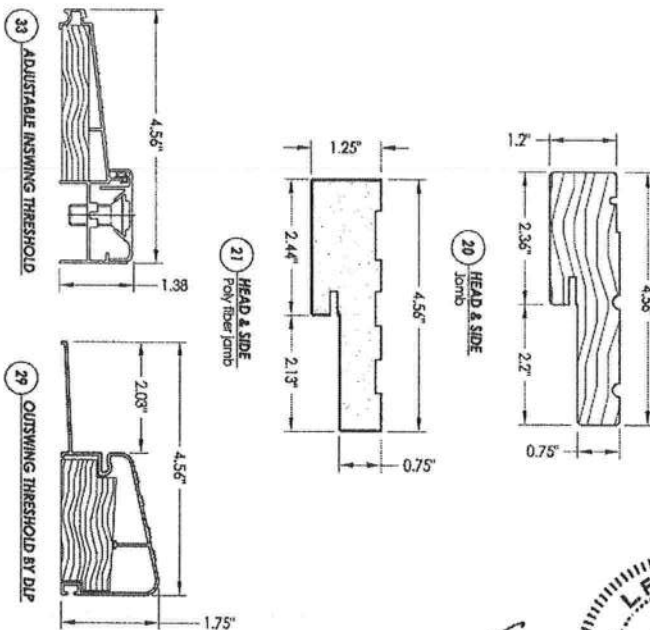


ASTRAGAL SCREW LOCATIONS



DATE: 02/20/12				PRODUCT: PLASTPRO INC. FIBERGLASS DOOR		Documents Prepared By: Lyndon F. Schmidt P.E. No. 43409		JULY 27, 2020 STATE OF FLORIDA PROFESSIONAL ENGINEER	
SCALE: N.T.S.				PART OR ASSEMBLY: ASTRAGAL DETAILS		RW BUILDING CONSULTANTS, INC.		P.O. Box 230, Valrico, FL 33595	
DWG. BY: JK								Phone No.: 813.659.9197	
CHK. BY: LFS								FBPE Registry No. 9813	
DESIGNED NO.: FL-15220.14									
SHEET 9 OF 10									
NO. DATE				BY					
3 7/27/20 UPDATE TO 7TH ED. (2020) FBC				LFS					
2 08/02/17 UPDATE TO 6TH ED. (2017) FBC				JK					
1 04/22/15 UPDATE TO 5TH ED. (2014) FBC				JK					
REVISIONS									

BILL OF MATERIALS		
ITEM #	DESCRIPTION	MATERIAL
A	1X BUCK SG >= 0.55	WOOD
B	2X BUCK SG >= 0.55	WOOD
C	1/4" MAX. SHIM SPACE	
D	1/4" X 2-3/4" ELCO OR ITW CONCRETE SCREW	STEEL
E	MASONRY - 3,000 PSI MIN. CONCRETE CONFORMING TO ACI 301 OR HOLLOW BLOCK CONFORMING TO ASTM C90	CONCRETE
G	3/16" X 3-1/4" PHH ITW CONCRETE SCREW	STEEL
H	1/4" X 3-1/4" PHH ELCO OR ITW CONCRETE SCREW	STEEL
I	1/4" X 2-1/4" PHH ELCO OR ITW CONCRETE SCREW	STEEL
J	1/4" X 3-3/4" PHH ELCO OR ITW CONCRETE SCREW	STEEL
K	#9 X 2-1/2" PHH WOOD SCREW	STEEL
L	#10 X 2-1/2" PHH WOOD SCREW (1.15" MIN. EMBEDMENT)	STEEL
M	#8 X 2" PHH WOOD SCREW	STEEL
N	1/4" X 3-1/4" PHH ITW CONCRETE SCREW	STEEL
7	FORCE 5 WEATHERSTRIPPING BY ENDURA	FOAM
8	COMPRESSION WEATHERSTRIP GLON 650 BY SCHLEGEL	FOAM
9	4" X 4" BUTT HINGE	STEEL
11	#9 X 3/4" PHH WOOD SCREW	STEEL
12	#10 X 3" PHH WOOD SCREW	STEEL
20	FINGER JOINTED PINE FRAME, HEAD & HINGE JAMBS	WOOD
21	POLY FIBER JAMB	COMP. / VINYL
27	LATCH STRIKE PLATE	STEEL
28	DEADBOLT PLATE	STEEL
29	INSWING THRESHOLD BY DLP	ALUM./WOOD
30	INSWING VINYL DOOR BOTTOM SWEEP	VINYL
33	INSWING ADJUSTABLE ALUMINUM THRESHOLD BY DLP	ALUM. / WOOD
40	DOOR PANEL - SEE DOOR PANEL DETAIL SHEET FOR CONSTRUCTION DETAILS	
71	ASTRAGAL BY DLP (6063-15 ALUMINUM)	ALUM.
72	ASTRAGAL STRIKE PLATE	STEEL
73	#8 X 2" PHH SCREW	STEEL
74	CONCEALED STIFFENER BY PLASPRO (6063-16 ALUMINUM)	ALUM.
77	SLIDE BOLT ROD	STEEL



PRODUCT: PLASTPRO INC. FIBERGLASS DOOR PART OR ASSEMBLY: BILL OF MATERIALS & COMPONENTS				Document Prepared By: Lyndon F. Schmidt P.E. No. 43409 R.W. BUILDING CONSULTANTS, INC. P.O. Box 230, Valrico, FL 33595 Phone No.: 813.659.9197 FBPE Registry No. 9813	
REVISIONS NO. DATE DESCRIPTION BY 3 7/27/20 UPDATE TO 7TH ED. (2020) FBC LFS 2 08/02/17 UPDATE TO 6TH ED. (2017) FBC JK 1 04/22/15 UPDATE TO 5TH ED. (2014) FBC JK				DATE: 02/20/12 SCALE: N.T.S. INC. BY: JK CHK. BY: LFS DRAWING NO.: FL-15220.14 SHEET 10 of 10	



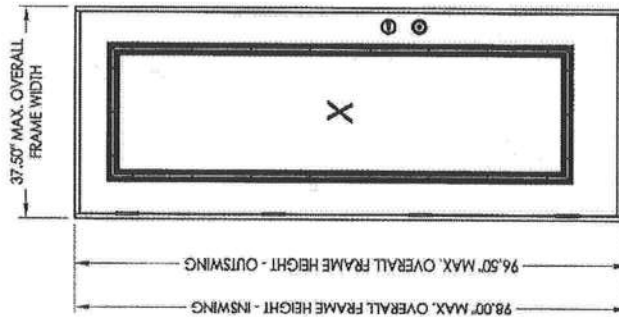
5200 W. CENTURY BLVD.
LOS ANGELES, CA 90045

**Smooth / Wood Grain / White Wood Grain
Rustic / Mahogany
Series N Fiberglass Door
INSWING / OUTSWING
"NON-IMPACT"**

GENERAL NOTES

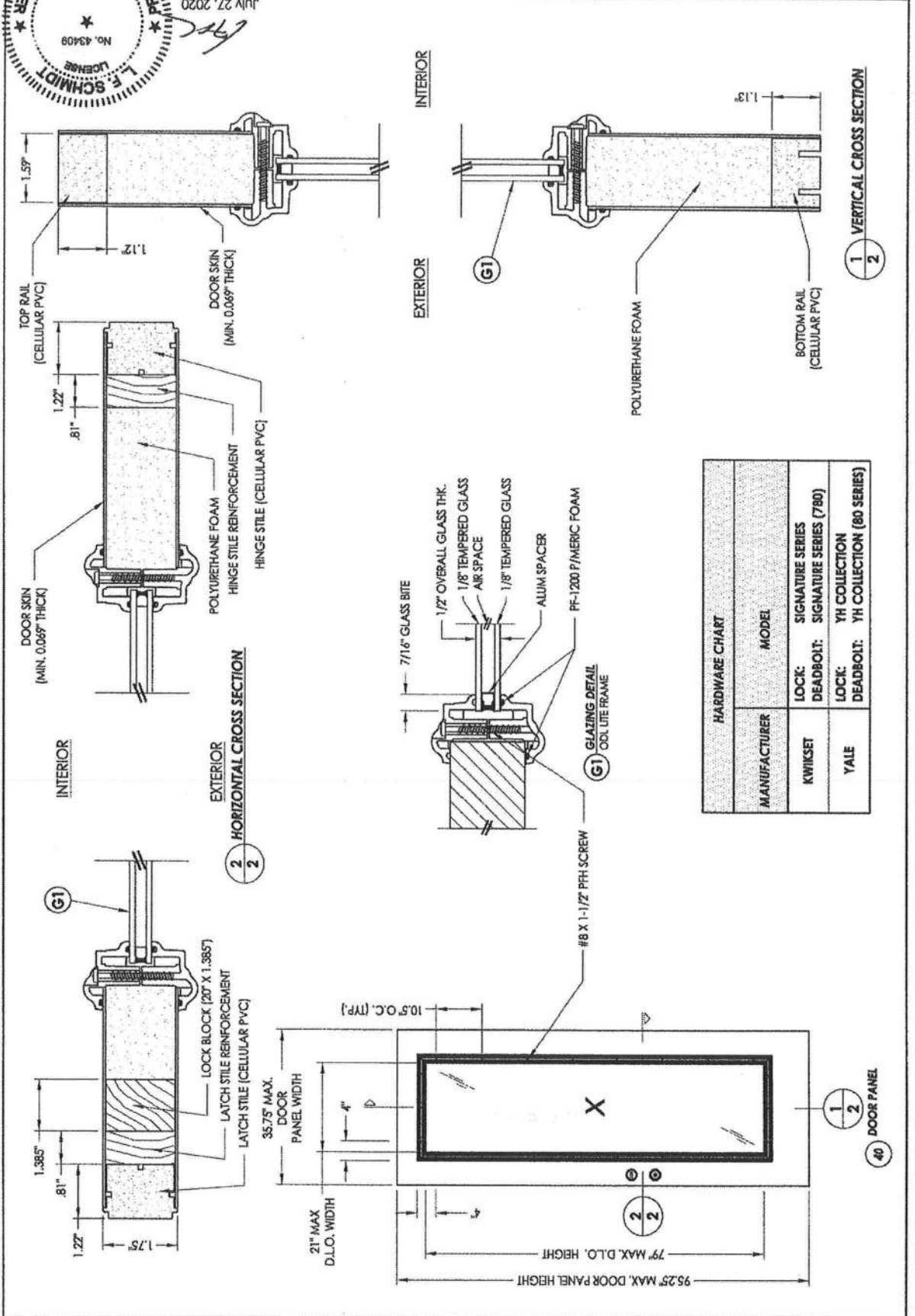
1. This product has been evaluated and is in compliance with the 7th Edition (2020) Florida Building Code (FBC) structural requirements including the "High Velocity Hurricane Zone" (HVHZ).
2. Product anchors shall be as listed and spaced as shown on details. Anchor embedment to base material shall be beyond wall dressing or stucco.
3. When used in the "HVHZ" this product is required to be protected with an impact resistant covering that complies with Section 1626 of the FBC.
4. When used in areas outside of the "HVHZ" requiring wind borne debris protection, this product is required to be protected with an impact resistant covering that complies with FBC Sections 1609.1.2 & 1620.2.1.2.
5. For 2x stud construction, anchoring of these units shall be the same as that shown for 2x buck masonry construction.
6. Site conditions that deviate from the details of this drawing require further engineering analysis by a licensed engineer or registered architect.
7. Outswing configurations meet water infiltration requirements for "HVHZ".
8. Inswing configurations do not meet the water infiltration requirements for the "HVHZ". Inswing units shall be installed only in non-habitable areas or at habitable locations protected by an overhang or canopy such that the angle between the edge of canopy or overhang to sill is less than 45 degrees.

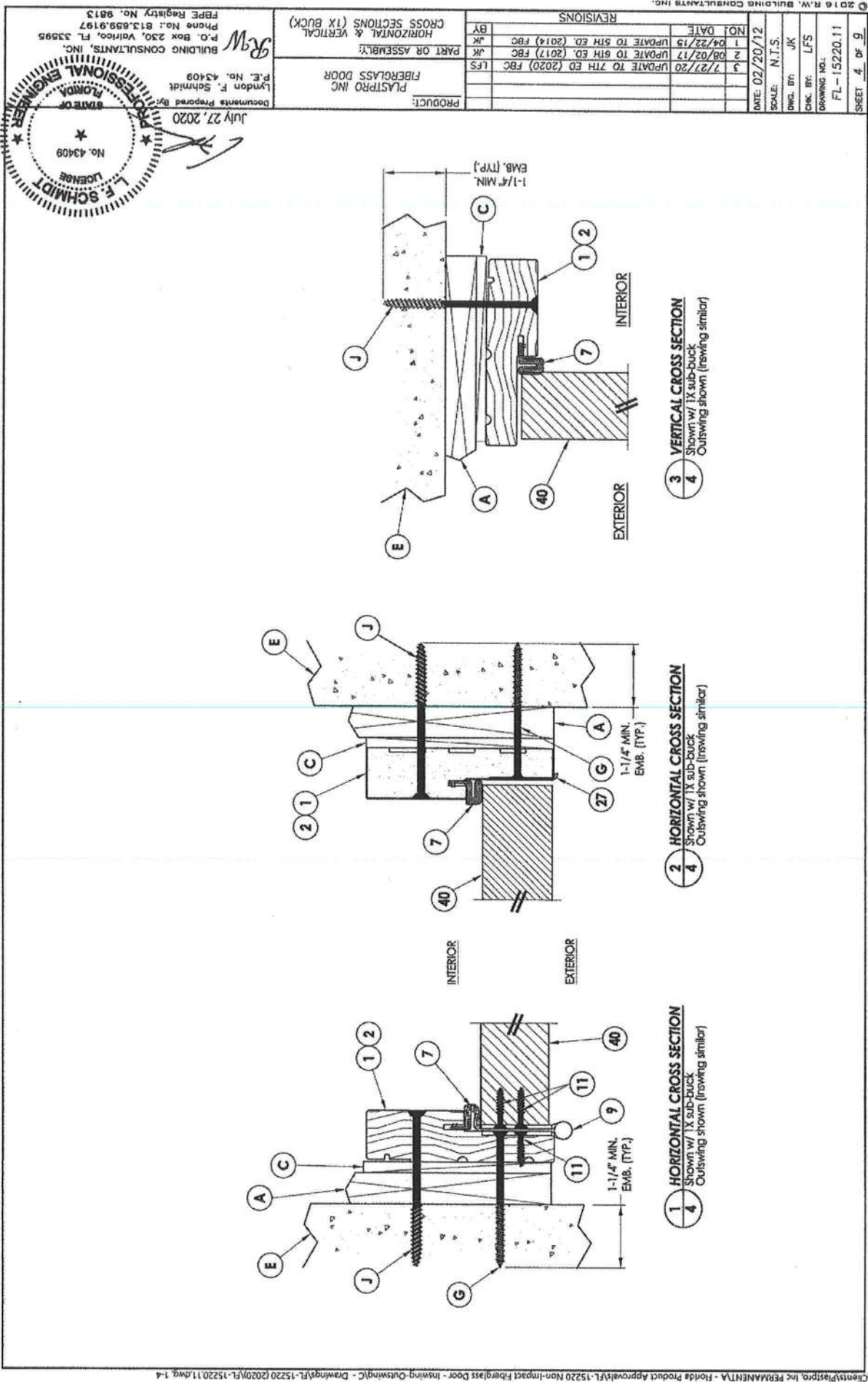
TABLE OF CONTENTS	
SHEET#	DESCRIPTION
1	Typical elevations, design pressures & general notes
2	Door panel details and glazing detail
3	Horizontal & Vertical Cross Sections (2X Buck)
4	Horizontal & Vertical Cross Sections (1X Buck)
5	Horizontal & Vertical Cross Sections (Direct to Masonry)
6	Vertical Cross Sections (Thresholds)
7	Horizontal & Vertical Cross Sections (Direct to Masonry)
8	Buck and frame anchoring
9	Bill of materials & components



SWING	OVERALL FRAME DIMENSION	DESIGN PRESSURE (PSF)	
		POSITIVE	NEGATIVE
INSWING	37.50" x 98.00"	+47.0	-47.0
OUTSWING	37.50" x 94.50"	+47.0	-47.0

PLASTPRO INC. FIBERGLASS DOOR PART OR ASSEMBLY TYPICAL ELEVATION, DESIGN PRESSURES & GENERAL NOTES		REVISIONS	
DATE: 02/20/12	NO. 1	DATE: 04/22/15	NO. 1
SCALE: N.T.S.	DATE: 08/02/17	DATE: 08/02/17	NO. 2
CHK. BY: JKS	DATE: 07/27/20	DATE: 07/27/20	NO. 3
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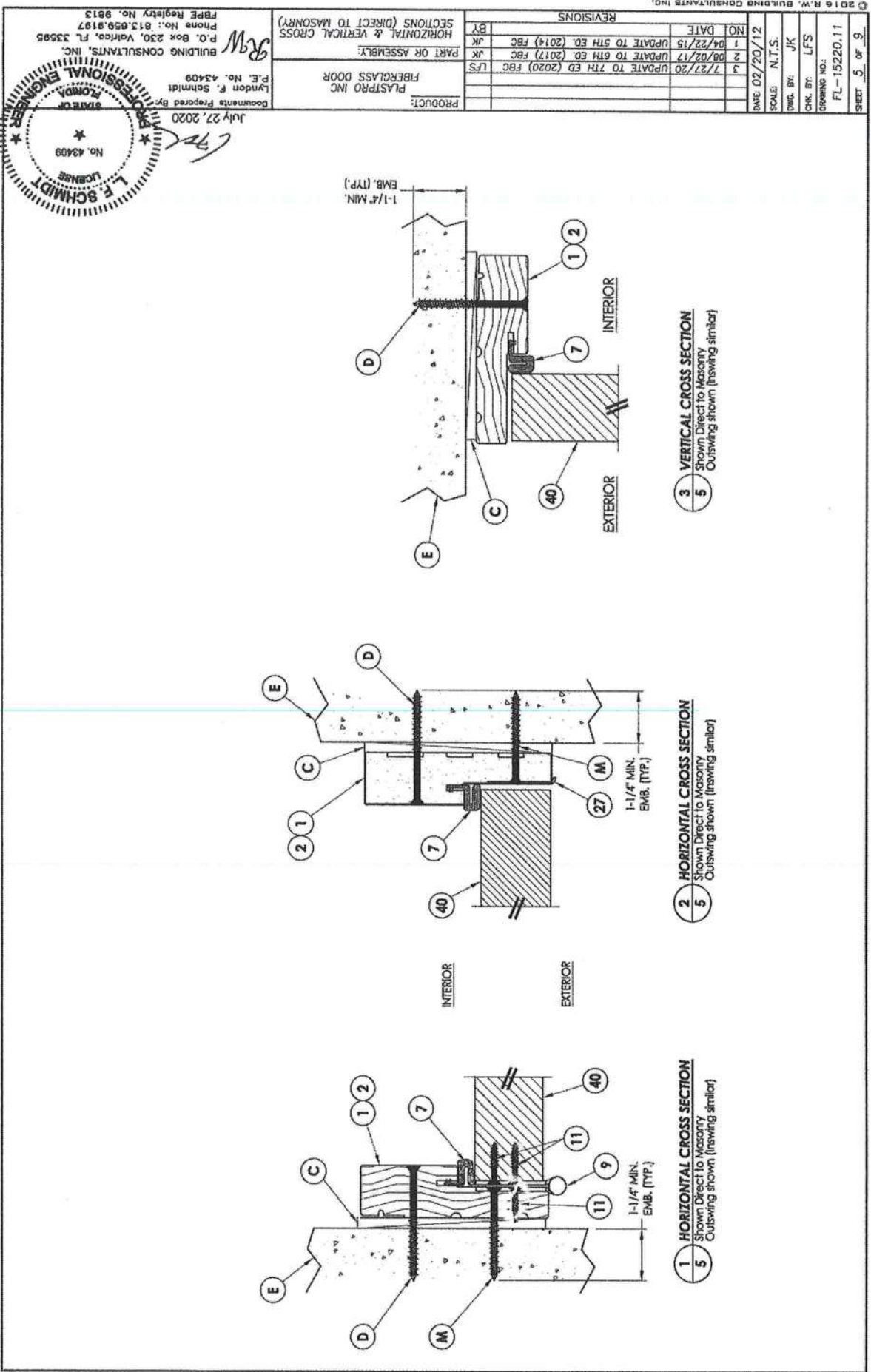


NO. DATE 1 04/22/19 2 08/02/17 3 7/27/20		REVISIONS UPDATE TO 5TH ED. (2014) FBC JK UPDATE TO 6TH ED. (2017) FBC JK UPDATE TO 7TH ED. (2020) FBC LFS
DATE: 02/20/12 SCALE: N.T.S. DWG. BY: JK CHK. BY: LFS		DRAWING NO.: FL-15220.11 SHEET 4 of 9
PRODUCT: PLASTPRO INC FIBERGLASS DOOR		
PART OR ASSEMBLY: HORIZONTAL & VERTICAL CROSS SECTIONS (1X BUCK)		

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July 27, 2020
 Documents Prepared By:
 Lyndon F. Schmidt
 P.E. No. 43409
 BUILDING CONSULTANTS, INC.
 P.O. Box 230, Vero Beach, FL 33595
 Phone No.: 813.659.9197
 FBPE Registry No. 9813

L.F. SCHMIDT
 LICENSEE
 No. 43409
 STATE OF FLORIDA
 PROFESSIONAL ENGINEER



DATE: 02/20/12	SCALE: N.T.S.	DWG. BY: JK	CHK. BY: LFS	DRAWING NO.: FL-15220.11	SHEET 5 OF 9
NO.	DATE	DESCRIPTION	BY	DATE	DESCRIPTION
1	04/22/19	UPDATE TO 5TH ED. (2014) FBC	JK		
2	08/02/17	UPDATE TO 6TH ED. (2017) FBC	JK		
3	7/27/20	UPDATE TO 7TH ED. (2020) FBC	LFS		

REVISIONS

PRODUCT: PLASTIPRO INC. FIBERGLASS DOOR

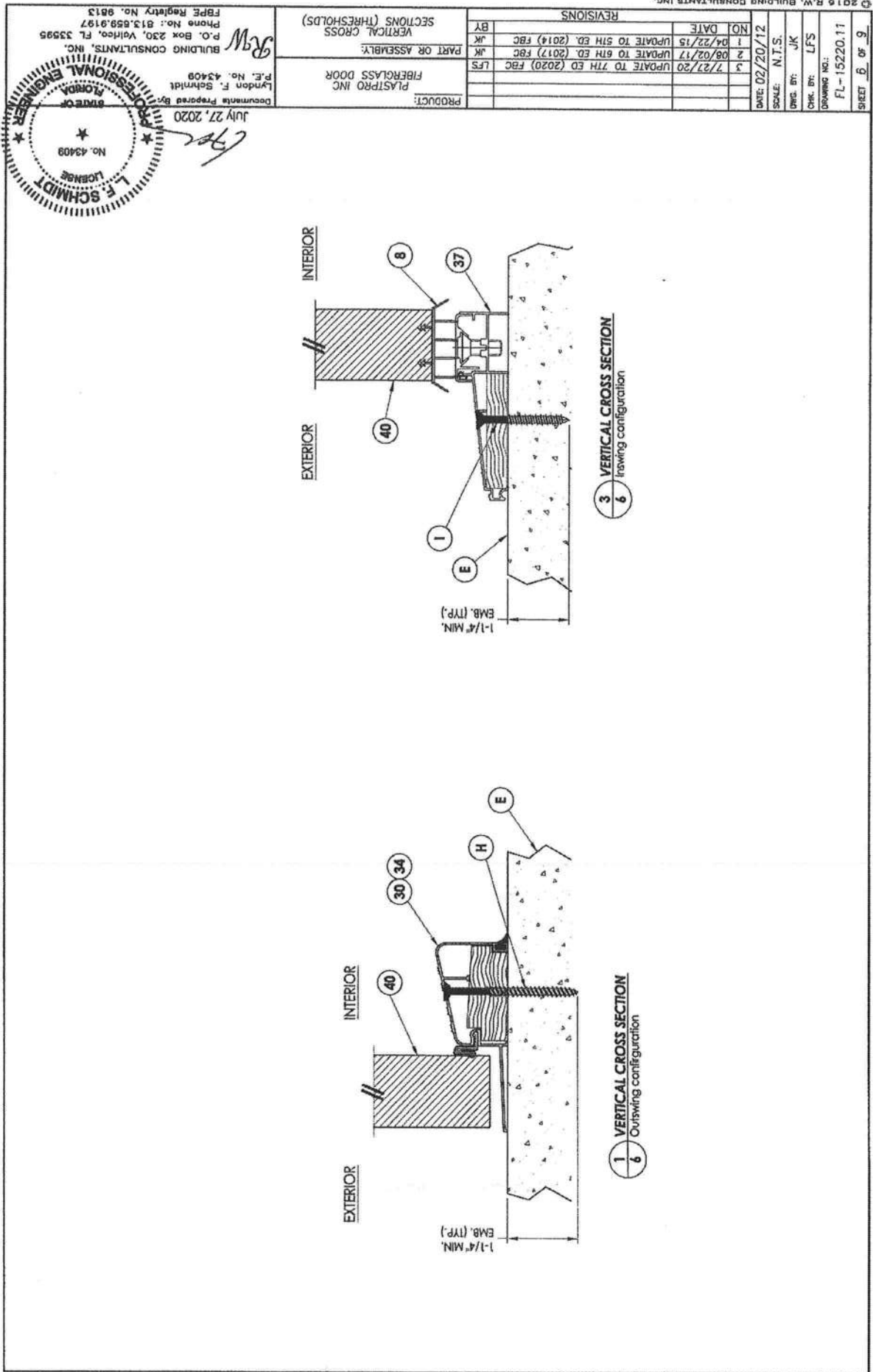
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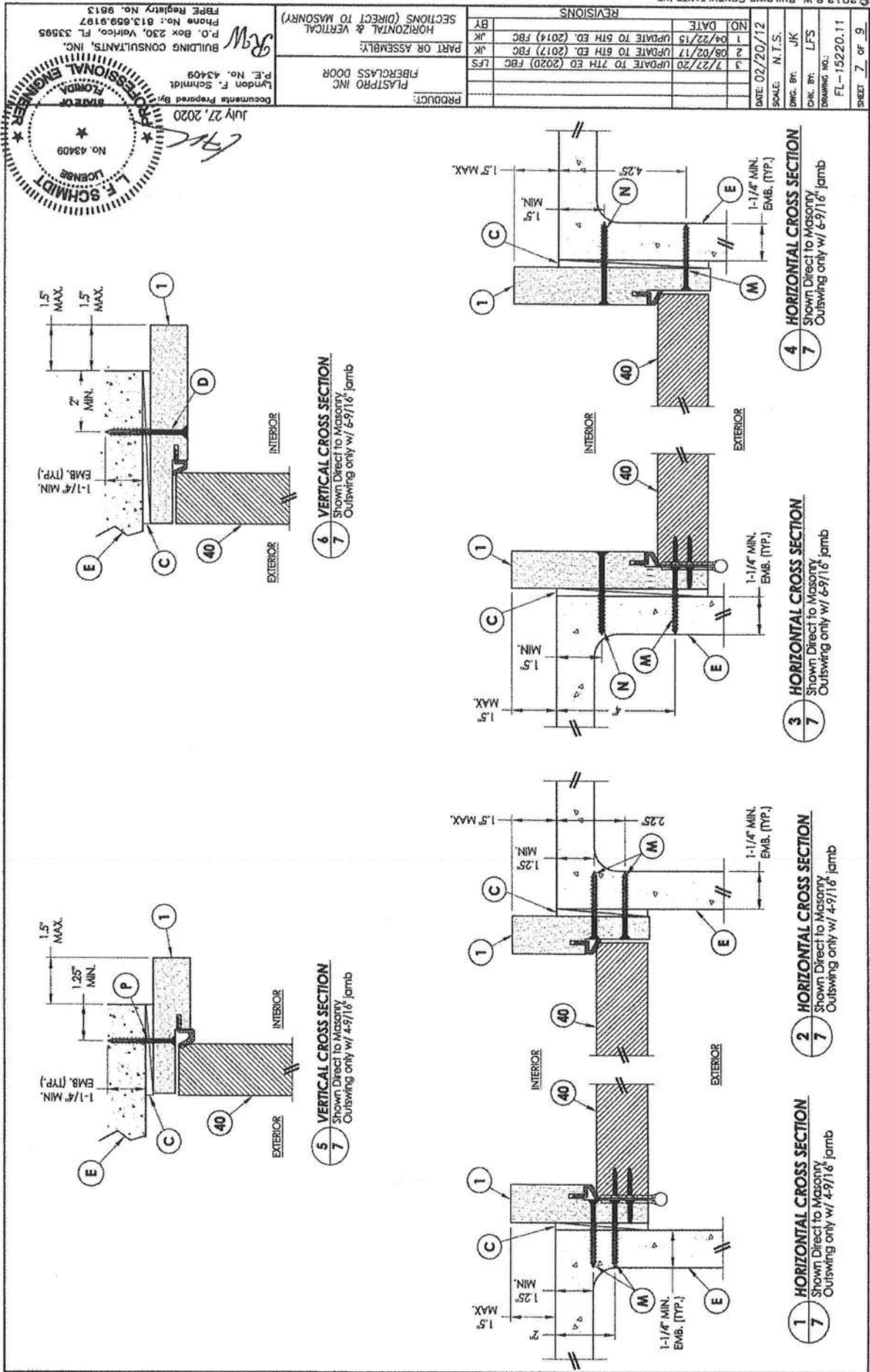
Documents Prepared By: Lyndon F. Schmidt
P.E. No. 43409

July 27, 2020

BUILDING CONSULTANTS, INC.
P.O. Box 230, Vero Beach, FL 33595
Phone No.: 813.059.9197
FBPE Registry No. 9813







DATE: 02/20/12	SCALE: N.T.S.	DWG. BY: JK	CHEK. BY: LFS	DRAWING NO.: FL-15220.11	SHEET 7 of 9
NO.	DATE	DESCRIPTION	BY	DATE	DESCRIPTION
1	04/22/15	UPDATE TO 6TH ED. (2014) FBC	JK		
2	08/02/17	UPDATE TO 6TH ED. (2017) FBC	JK		
3	7/27/20	UPDATE TO 7TH ED. (2020) FBC	LFS		

REVISIONS

PRODUCT: PLASTPRO INC FIBERGLASS DOOR

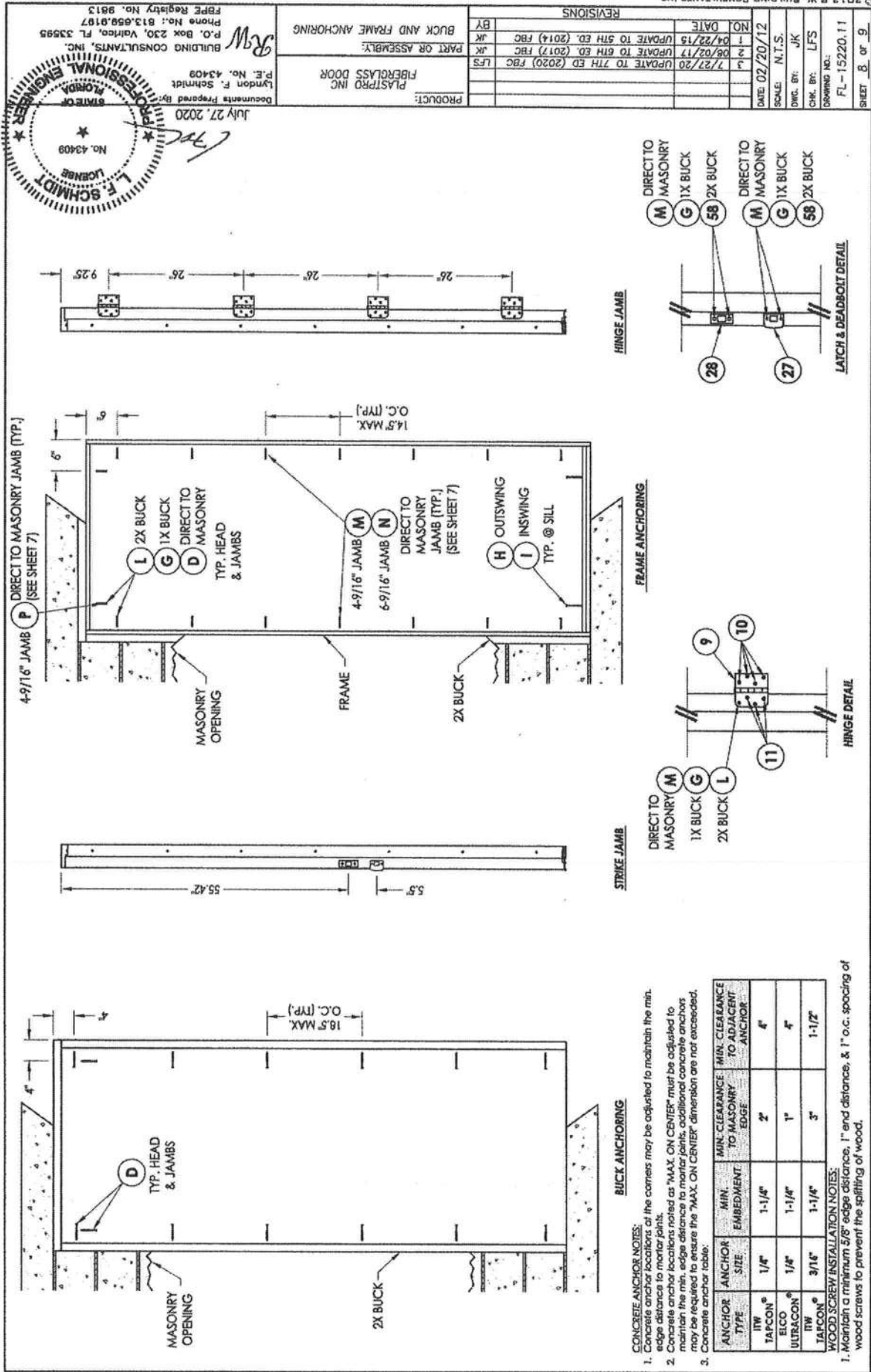
PART OR ASSEMBLY: HORIZONTAL & VERTICAL SECTIONS (DIRECT TO MASONRY)

PLASTPRO INC
P.O. Box 230, Vero Beach, FL 33595
Phone No: 813.659.9187
FPE Registry No. 9813

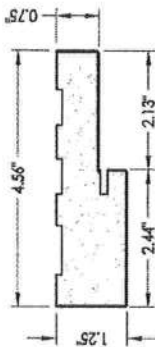
Documents Prepared By: Lyndon F. Schmidt
P.E. No. 43409

July 27, 2020

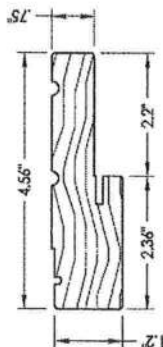
PROFESSIONAL ENGINEER
L.F. SCHMIDT
No. 43409
FLORIDA



BILL OF MATERIALS		
ITEM #	DESCRIPTION	MATERIAL
A	1X BUCK SG >= 0.55	WOOD
B	2X BUCK SG >= 0.55	WOOD
C	1/4" MAX. SHIM SPACE	
D	1/4" X 2-3/4" PPH ELCO OR ITW CONCRETE SCREW	STEEL
E	MASONRY - 3,000 PSI MIN. CONCRETE CONFORMING TO ACI 301 OR HOLLOW BLOCK CONFORMING TO ASTM C90	CONCRETE
G	3/16" X 3-1/4" PPH ITW CONCRETE SCREW	STEEL
H	1/4" X 3-1/4" PPH ELCO OR ITW CONCRETE SCREW	STEEL
I	1/4" X 1-3/4" PPH ELCO OR ITW CONCRETE SCREW	STEEL
J	1/4" X 3-3/4" PPH ELCO OR ITW CONCRETE SCREW	STEEL
K	1/4" X 2-1/4" ITW CONCRETE SCREW	STEEL
L	#10 X 2-1/2" PPH WOOD SCREW (1.15" MIN. EMBEDMENT)	STEEL
M	3/16" X 2-1/4" ITW CONCRETE SCREW	STEEL
N	3/16" X 2-3/4" ITW CONCRETE SCREW	STEEL
P	1/4" X 2-1/4" PPH ELCO CONCRETE SCREW	STEEL
1	POLY FIBER JAMB	COMP. / VINYL
2	FINGER JOINTED PINE FRAME	WOOD
7	WEATHERSTRIP	FOAM
8	DOOR BOTTOM SWEEP	VINYL
9	4" X 4" BUTT HINGE	STEEL
11	#9 X 3/4" PPH WOOD SCREW	STEEL
27	LATCH STRIKE PLATE	STEEL
28	DEADBOLT STRIKE PLATE	STEEL
30	OUTSWING THRESHOLD	ALUM/WOOD
34	OUTSWING THRESHOLD	ALUM/WOOD
37	INSWING THRESHOLD	ALUM/WOOD
40	DOOR PANEL - SEE DOOR PANEL DETAIL SHEET FOR CONSTRUCTION DETAILS	
58	#8 X 2" PPH WOOD SCREW	STEEL



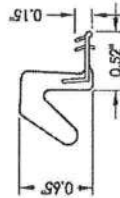
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Plastpro



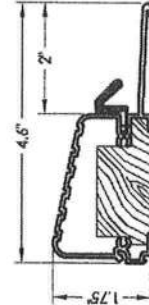
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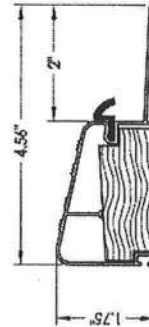
8 DOOR BOTTOM SWEEP



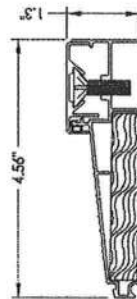
7 WEATHERSTRIP
Endura Force 5



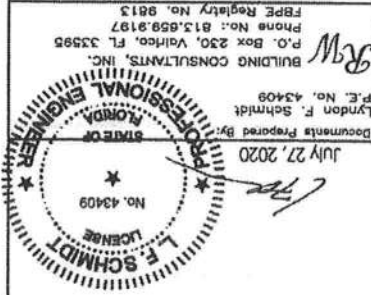
34 OUTSWING THRESHOLD
Columbia High Dam (S81)



30 OUTSWING THRESHOLD
DLP (QOSR)



37 INSWING THRESHOLD
DLP (ISC)



July 27, 2020
Documents Prepared By: Lyndon F. Schmidt
P.E. No. 43409
BUILDING CONSULTANTS, INC.
P.O. Box 250, Valrico, FL 33595
Phone No.: 813.659.9197
FBPE Registry No. 9813

PRODUCT: PLASTPRO INC
FIBERGLASS DOOR
PART OR ASSEMBLY
BILL OF MATERIALS

REVISIONS	
NO.	DATE
1	04/22/15
2	08/02/17
3	7/27/20
UPDATE TO 5TH ED. (2014) FBC	
UPDATE TO 6TH ED. (2017) FBC	
UPDATE TO 7TH ED. (2020) FBC	
DATE: 02/20/12	
SCALE: N.T.S.	
DWG. BY: JK	
CHK. BY: LFS	
DRAWING NO: FL-15220.11	
SHEET 9 OF 9	