Job	Truss	Truss Type	Qty	Ply	LOT 8
L264527	T03	HIP	1	1	J1921199
					Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 13:49:04 2007 Page 1

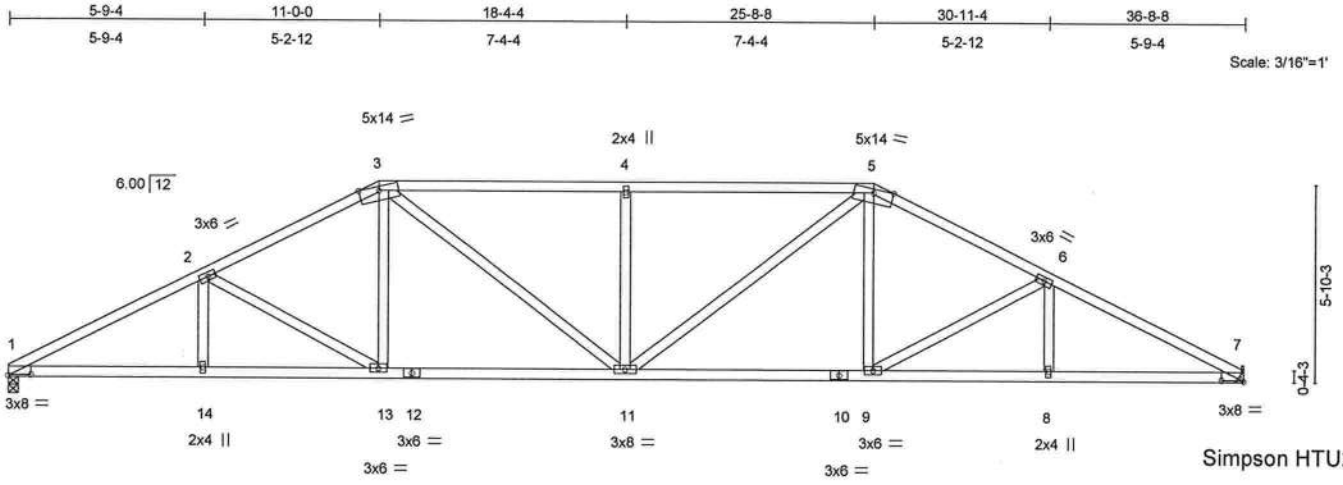


Plate Offsets (X,Y): [1:0-8-0,0-0-6], [7:0-8-0,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	0.19	11	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.47	Vert(TL)	-0.31	9-11	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.29	Horz(TL)	0.11	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 188 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-11-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-2-1 oc bracing.

REACTIONS (lb/size) 1=1165/0-3-8, 7=1165/Mechanical
Max Horz 1=-71(load case 4)
Max Uplift 1=-211(load case 6), 7=-211(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2255/1216, 2-3=-1886/1076, 3-4=-1964/1186, 4-5=-1964/1186, 5-6=-1886/1076, 6-7=-2255/1216
BOT CHORD 1-14=-998/1942, 13-14=-998/1942, 12-13=-741/1641, 11-12=-741/1641, 10-11=-741/1641, 9-10=-741/1641, 8-9=-998/1942, 7-8=-998/1942
WEBS 2-14=0/167, 2-13=-355/295, 3-13=-102/306, 3-11=-204/523, 4-11=-422/292, 5-11=-204/523, 5-9=-102/306, 6-9=-355/295, 6-8=0/167

JOINT STRESS INDEX
1 = 0.67, 2 = 0.39, 3 = 0.89, 4 = 0.33, 5 = 0.89, 6 = 0.39, 7 = 0.67, 8 = 0.33, 9 = 0.34, 10 = 0.53, 11 = 0.56, 12 = 0.53, 13 = 0.34 and 14 = 0.33

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1 Ibg Coastal Bay Blvd
Boynton Beach, FL 33435

January 4,2008

Job	Truss	Truss Type	Qty	Ply	LOT 8
L264527	T03	HIP	1	1	J1921199
					Job Reference (optional)

NOTES

3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 211 lb uplift at joint 1 and 211 lb uplift at joint 7.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

January 4,2008

Job'	Truss	Truss Type	Qty	Ply	LOT 8
L264527	T04	SPECIAL	1	1	J1921200
					Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 13:49:05 2007 Page 1

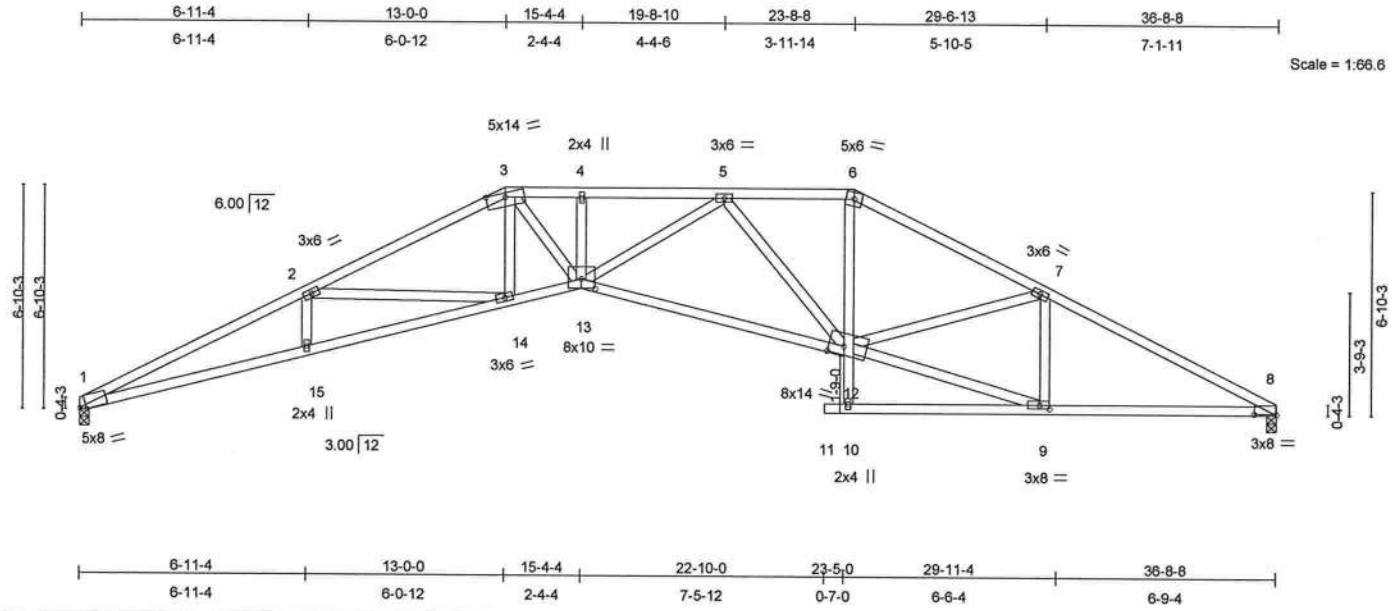


Plate Offsets (X,Y): [1:0-2-6,Edge], [8:0-8-4,0-0-6], [9:0-3-8,0-1-8], [12:0-5-8,0-3-2], [13:0-5-0,0-3-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.61	Vert(LL)	0.50	13	>878	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.83	Vert(TL)	-0.84	12-13	>523	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.73	Horz(TL)	0.50	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 189 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2 *Except* 6-10 2 X 4 SYP No.3
WEBS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 2-6-11 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 4-4-13 oc bracing.

REACTIONS (lb/size) 1=1168/0-3-8, 8=1170/0-3-8
Max Horz 1=85(load case 5)
Max Uplift 1=-224(load case 6), 8=-222(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=-4123/2169, 2-3=-3345/1715, 3-4=-3724/1929, 4-5=-3723/1929, 5-6=-2072/1210, 6-7=-2357/1282, 7-8=-2225/1197
BOT CHORD	1-15=-1891/3705, 14-15=-1889/3704, 13-14=-1306/3025, 12-13=-1247/2839, 10-12=0/102, 6-12=-374/762, 10-11=0/0, 9-10=-21/29, 8-9=-965/1906
WEBS	2-15=0/208, 2-14=-683/566, 3-14=-160/250, 3-13=-504/1222, 4-13=-156/81, 5-13=-462/1154, 5-12=-1086/527, 9-12=-983/1956, 7-9=-460/296, 7-12=-77/290

JOINT STRESS INDEX

1 = 0.77, 2 = 0.39, 3 = 0.72, 4 = 0.33, 5 = 0.65, 6 = 0.43, 7 = 0.39, 8 = 0.72, 9 = 0.75, 10 = 0.70, 12 = 0.47, 13 = 0.56, 14 = 0.37 and 15 = 0.33

NOTES

1) Unbalanced roof live loads have been considered for this design.

Julius Lee
Truss Design Engineer
Florida PE No. 34198
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

Continued on page 2

January 4,2008

Job	Truss	Truss Type	Qty	Ply	LOT 8
L264527	T04	SPECIAL	1	1	J1921200
					Job Reference (optional)

NOTES

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 224 lb uplift at joint 1 and 222 lb uplift at joint 8.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

January 4,2008

Job	Truss	Truss Type	Qty	Ply	LOT 8
L264527	T05	SPECIAL	1	1	J1921201
					Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 13:49:06 2007 Page 1

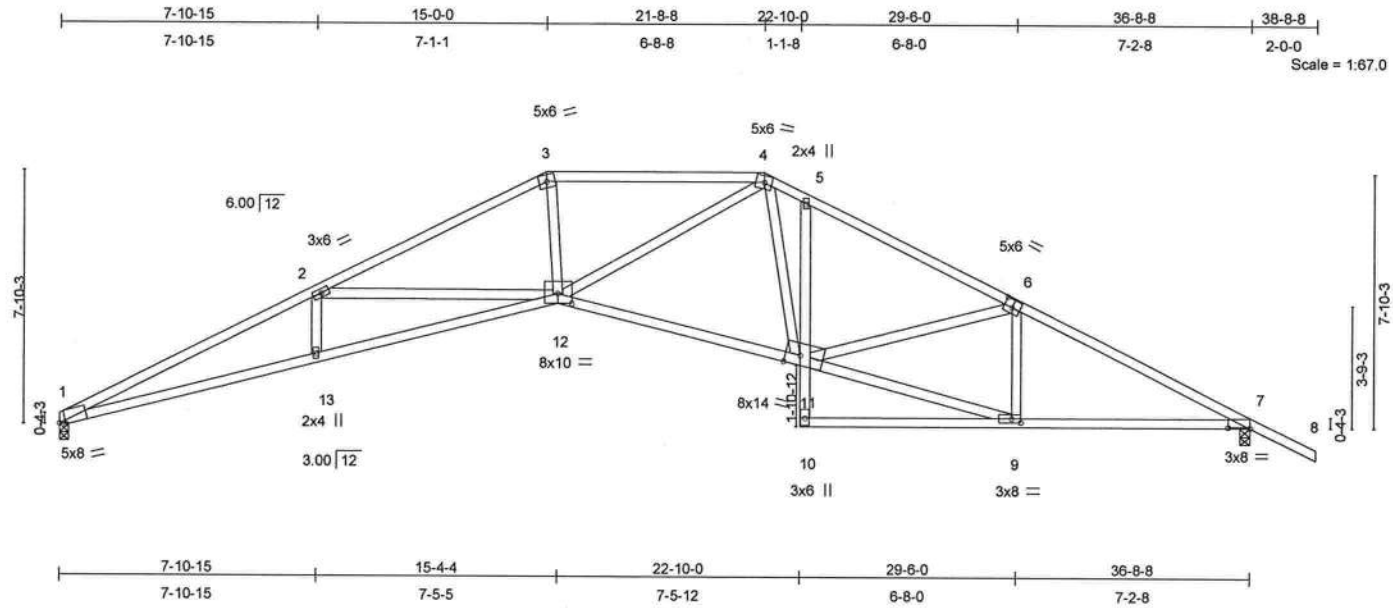


Plate Offsets (X,Y): [1:0-2-7,Edge], [6:0-3-0,0-3-0], [7:0-8-0,0-0-6], [9:0-3-8,0-1-8], [11:0-5-8,Edge], [12:0-5-0,0-3-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.85	Vert(LL)	0.42	12-13	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.88	Vert(TL)	-0.74	12-13	>592	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.86	Horz(TL)	0.44	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 191 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except* 5-10 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied or 4-6-3 oc bracing.

REACTIONS (lb/size) 1=1162/0-3-8, 7=1285/0-3-8
Max Horz 1=-137(load case 7)
Max Uplift 1=-237(load case 6), 7=-331(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-4060/2070, 2-3=-3083/1485, 3-4=-2769/1436, 4-5=-2216/1319, 5-6=-2309/1239, 6-7=-2179/1159, 7-8=0/47
BOT CHORD 1-13=-1711/3647, 12-13=-1707/3643, 11-12=-697/1998, 10-11=0/90, 5-11=-151/194, 9-10=-14/34, 7-9=-845/1862
WEBS 2-13=0/237, 2-12=-869/712, 3-12=-375/967, 4-12=-352/976, 4-11=-273/289, 9-11=-873/1915, 6-11=-57/242, 6-9=-460/292

JOINT STRESS INDEX
1 = 0.75, 2 = 0.39, 3 = 0.73, 4 = 0.51, 5 = 0.59, 6 = 0.73, 7 = 0.67, 9 = 0.73, 10 = 0.28, 11 = 0.30, 12 = 0.67 and 13 = 0.33

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

January 4,2008

Job	Truss	Truss Type	Qty	Ply	LOT 8
L264527	T05	SPECIAL	1	1	J1921201
					Job Reference (optional)

NOTES

3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 237 lb uplift at joint 1 and 331 lb uplift at joint 7.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

January 4,2008

Job	Truss	Truss Type	Qty	Ply	LOT 8
L264527	T06	SPECIAL	1	1	J1921202
					Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 13:49:07 2007 Page 1

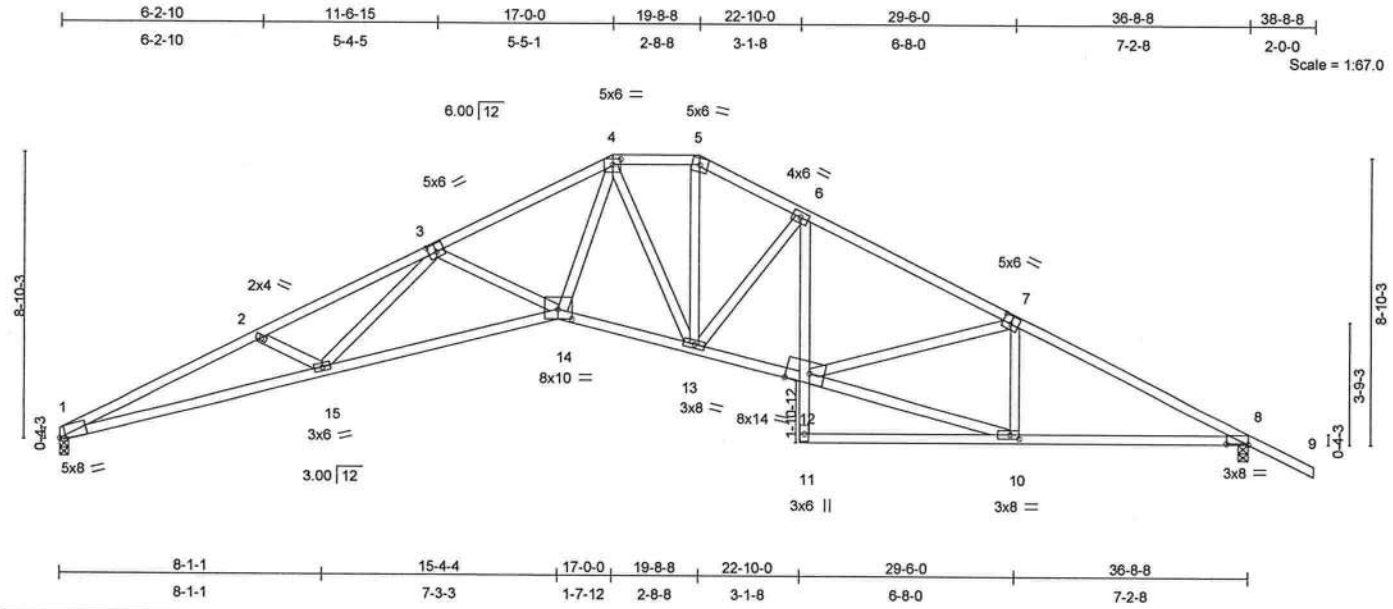


Plate Offsets (X,Y): [1:0-2-7,Edge], [3:0-3-0,0-3-0], [4:0-3-0,0-2-0], [7:0-3-0,0-3-0], [8:0-8-0,0-0-6], [10:0-3-8,0-1-8], [12:0-8-9 ,Edge], [14:0-5-0,0-3-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.46	Vert(LL)	0.43	14-15	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.81	Vert(TL)	-0.75	14-15	>586	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.71	Horz(TL)	0.43	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 203 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2 *Except* 6-11 2 X 4 SYP No.3
WEBS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 2-8-15 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 4-6-7 oc bracing.

REACTIONS (lb/size) 1=1162/0-3-8, 8=1285/0-3-8
Max Horz 1=-149(load case 7)
Max Uplift 1=-248(load case 6), 8=-342(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=-4104/2173, 2-3=-3820/2026, 3-4=-2916/1484, 4-5=-1810/1083, 5-6=-2052/1171, 6-7=-2284/1256, 7-8=-2178/1174, 8-9=0/47
BOT CHORD	1-15=-1822/3694, 14-15=-1344/3139, 13-14=-677/2106, 12-13=-780/2033, 11-12=0/93, 6-12=-49/125, 10-11=-17/77, 8-10=-859/1861
WEBS	2-15=-260/296, 3-15=-301/495, 3-14=-567/484, 4-14=-697/1641, 4-13=-613/216, 5-13=-456/781, 6-13=-320/305, 10-12=-882/1877, 7-12=-36/192, 7-10=-442/291

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

JOINT STRESS INDEX

1 = 0.76, 2 = 0.33, 3 = 0.58, 4 = 0.83, 5 = 0.36, 6 = 0.34, 7 = 0.72, 8 = 0.67, 10 = 0.71, 11 = 0.32, 12 = 0.54, 13 = 0.62, 14 = 0.58 and 15 = 0.37

NOTES

1) Unbalanced roof live loads have been considered for this design.

Continued on page 2

January 4,2008

Job	Truss	Truss Type	Qty	Ply	LOT 8
L264527	T06	SPECIAL	1	1	J1921202
					Job Reference (optional)

NOTES

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 248 lb uplift at joint 1 and 342 lb uplift at joint 8.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

January 4,2008

Job'	Truss	Truss Type	Qty	Ply	LOT 8
L264527	T07	SPECIAL	3	1	J1921203
					Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 13:49:08 2007 Page 1

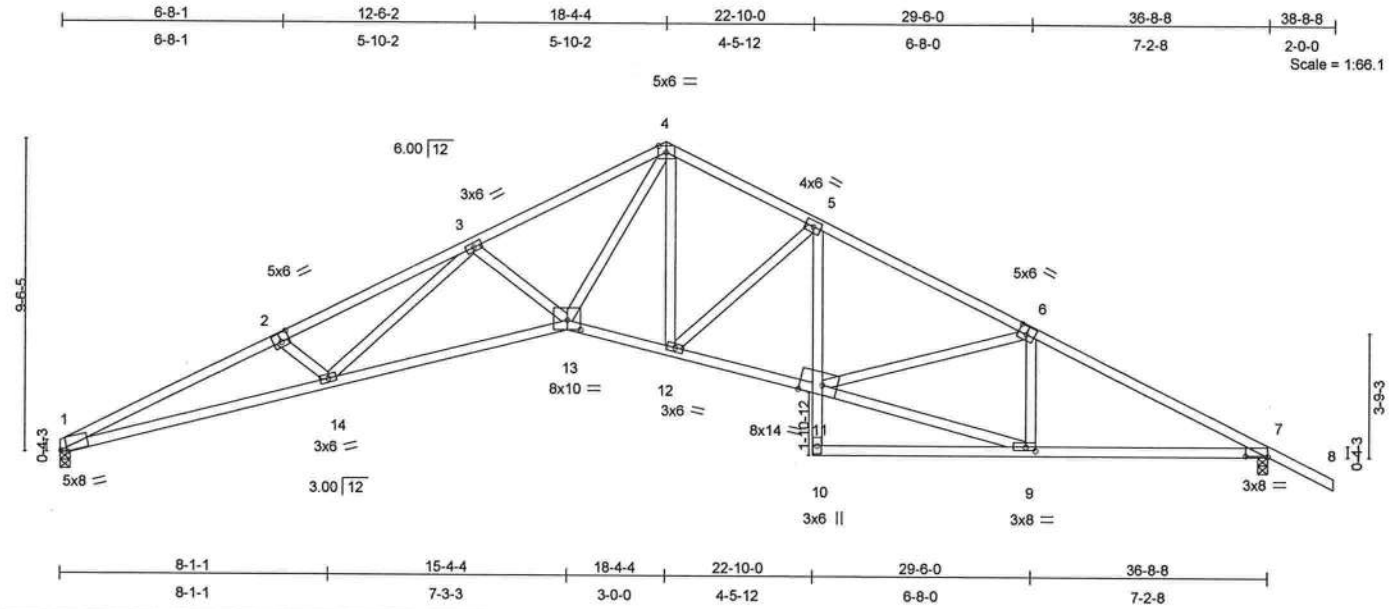


Plate Offsets (X,Y): [1:0-2-7,Edge], [2:0-3-0,0-3-0], [6:0-3-0,0-3-0], [7:0-8-0,0-0-6], [9:0-3-8,0-1-8], [11:0-8-4,Edge], [13:0-5-0 ,0-3-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.55	Vert(LL)	0.45	13-14	>976	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.82	Vert(TL)	-0.76	13-14	>575	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.71	Horz(TL)	0.44	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 198 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except* 5-10 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-7-13 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-6-0 oc bracing.

REACTIONS (lb/size) 1=1162/0-3-8, 7=1285/0-3-8
Max Horz 1=-157(load case 7)
Max Uplift 1=-254(load case 6), 7=-348(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-4095/2172, 2-3=-3848/2094, 3-4=-2957/1551, 4-5=-1994/1138, 5-6=-2286/1271, 6-7=-2178/1185, 7-8=0/47
BOT CHORD 1-14=-1816/3685, 13-14=-1285/3055, 12-13=-533/1783, 11-12=-794/2037, 10-11=0/92, 5-11=-46/151, 9-10=-18/65, 7-9=-868/1861
WEBS 2-14=-267/295, 3-14=-417/619, 3-13=-521/453, 4-13=-799/1767, 4-12=-271/385, 5-12=-390/321, 9-11=-891/1883, 6-11=-35/170, 6-9=-443/293

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

JOINT STRESS INDEX
1 = 0.76, 2 = 0.67, 3 = 0.39, 4 = 0.76, 5 = 0.29, 6 = 0.72, 7 = 0.67, 9 = 0.72, 10 = 0.31, 11 = 0.53, 12 = 0.38, 13 = 0.56 and 14 = 0.38

NOTES
1) Unbalanced roof live loads have been considered for this design.

Continued on page 2

January 4,2008

Job	Truss	Truss Type	Qty	Ply	LOT 8
L264527	T07	SPECIAL	3	1	J1921203
					Job Reference (optional)

NOTES

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 254 lb uplift at joint 1 and 348 lb uplift at joint 7.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1309 Coastal Ray Blvd
Boynton Beach, FL 33435

January 4,2008

Job	Truss	Truss Type	Qty	Ply	LOT 8
L264527	T08	SPECIAL	4	1	J1921204
					Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 13:49:09 2007 Page 1

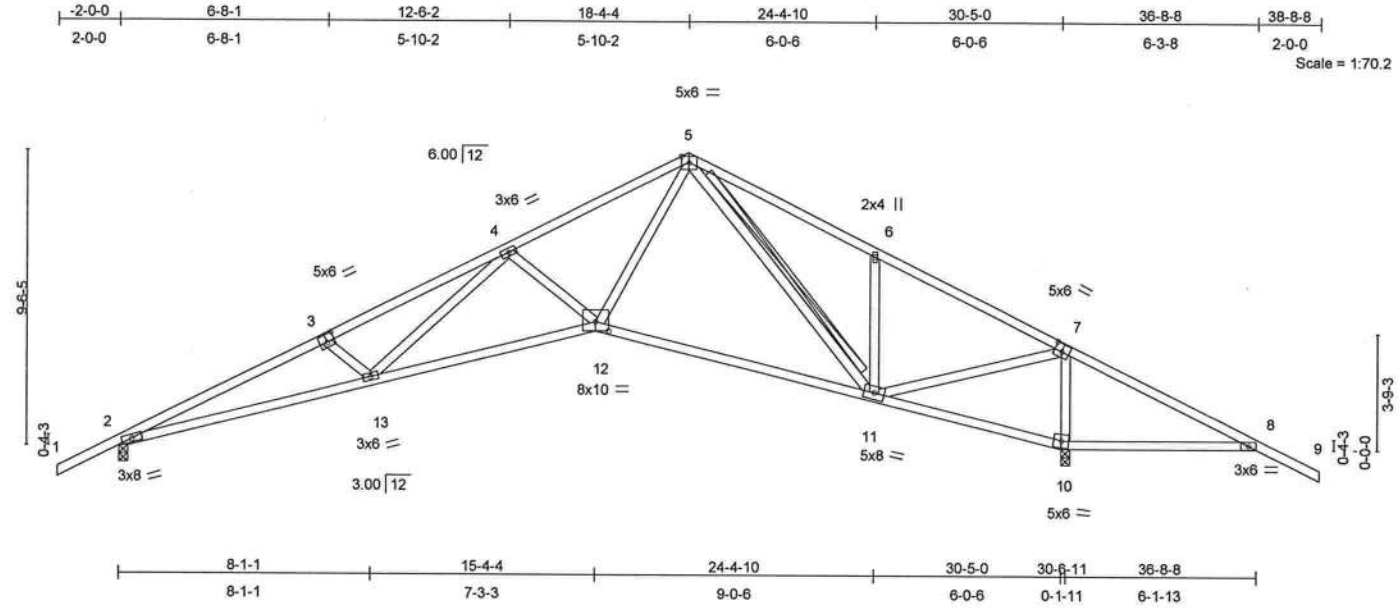


Plate Offsets (X,Y): [3:0-3-0,0-3-0], [7:0-3-0,0-3-0], [12:0-5-0,0-3-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	0.23	12-13	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.59	Vert(TL)	-0.43	12-13	>836	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.54	Horz(TL)	0.26	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 184 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-4-5 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 5-11
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1021/0-3-8, 10=1543/0-3-8
Max Horz 2=-144(load case 7)
Max Uplift 2=-311(load case 6), 10=-588(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-2959/1272, 3-4=-2708/1194, 4-5=-1805/680, 5-6=-837/332, 6-7=-842/199, 7-8=-846/714, 8-9=0/47
BOT CHORD 2-13=-983/2639, 12-13=-479/1999, 11-12=-101/988, 10-11=-616/954, 8-10=-565/896
WEBS 3-13=-266/293, 4-13=-387/600, 4-12=-520/441, 5-12=-392/1281, 5-11=-427/201, 6-11=-334/326, 7-11=-758/1320, 7-10=-1321/922

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd.
Boynton Beach, FL 33436

JOINT STRESS INDEX
2 = 0.68, 3 = 0.54, 4 = 0.39, 5 = 0.56, 6 = 0.33, 7 = 0.73, 8 = 0.44, 10 = 0.63, 11 = 0.55, 12 = 0.42 and 13 = 0.37

Continued on page 2

January 4,2008

Job	Truss	Truss Type	Qty	Ply	LOT 8
L264527	T08	SPECIAL	4	1	J1921204
					Job Reference (optional)

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever right exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 311 lb uplift at joint 2 and 588 lb uplift at joint 10.

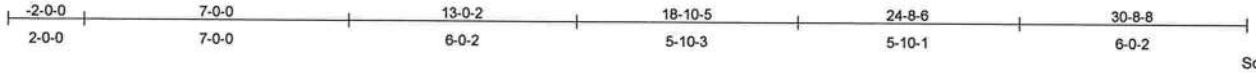
LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1103 Coastal Ray Blvd
Boynton Beach, FL 33435

January 4,2008

Job	Truss	Truss Type	Qty	Ply	LOT 8
L264527	T09	HIP	1	1	J1921205
					Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 13:49:10 2007 Page 1



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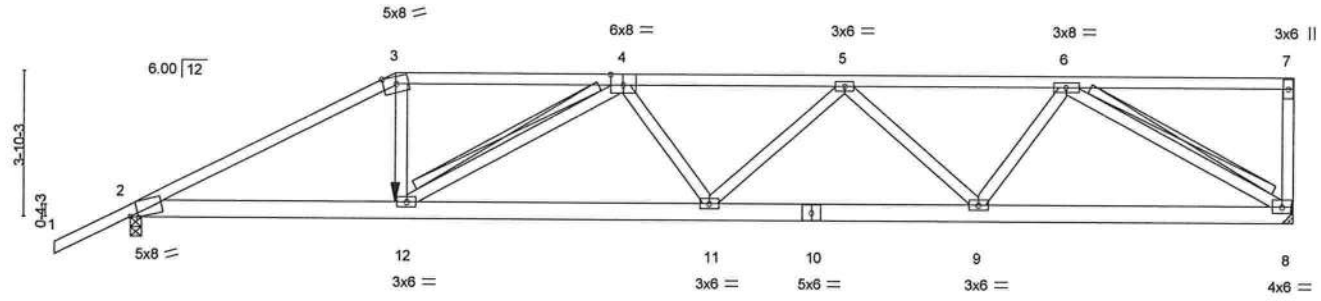

Simpson HTU26

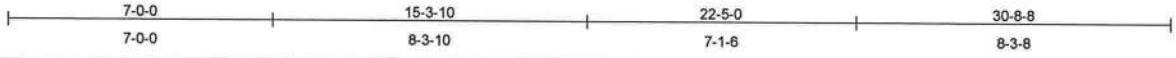

Plate Offsets (X,Y): [2:0-2-7,Edge], [4:0-3-15,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.94	Vert(LL)	-0.26	11	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.60	Vert(TL)	-0.50	11-12	>734	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	1.00	Horz(TL)	0.12	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 176 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-5-3 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-6-5 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 4-12, 6-8
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 8=2151/Mechanical, 2=2096/0-3-8
Max Horz 2=165(load case 5)
Max Uplift 8=-741(load case 4), 2=-654(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/51, 2-3=-4025/1311, 3-4=-3578/1218, 4-5=-4750/1603, 5-6=-3673/1225, 6-7=-120/42, 7-8=-330/167
BOT CHORD 2-12=-1204/3525, 11-12=-1673/4758, 10-11=-1592/4557, 9-10=-1592/4557, 8-9=-1070/3012
WEBS 3-12=-357/1234, 4-12=-1372/577, 4-11=-14/170, 5-11=-22/269, 5-9=-1236/513, 6-9=-282/1202, 6-8=-3362/1195

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33436

JOINT STRESS INDEX
2 = 0.76, 3 = 0.73, 4 = 0.46, 5 = 0.37, 6 = 0.85, 7 = 0.71, 8 = 0.85, 9 = 0.85, 10 = 0.95, 11 = 0.40 and 12 = 0.78

Continued on page 2

January 4,2008

Job	Truss	Truss Type	Qty	Ply	LOT 8
L264527	T09	HIP	1	1	J1921205
					Job Reference (optional)

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 741 lb uplift at joint 8 and 654 lb uplift at joint 2.
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-3=-54, 3-7=-118(F=-64), 2-12=-10, 8-12=-22(F=-12)
   Concentrated Loads (lb)
   Vert: 12=-411(F)

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1199 Coastal Bay Blvd
Boynton Beach, FL 33435

January 4,2008

Job	Truss	Truss Type	Qty	Ply	LOT 8
L264527	T10	HIP	1	1	J1921206
					Job Reference (optional)

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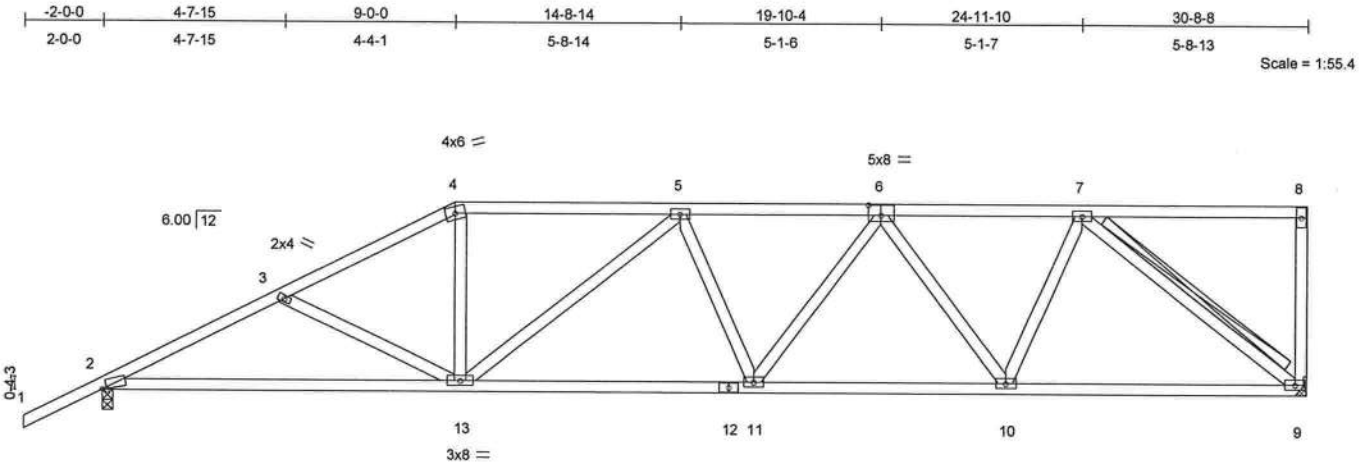

Simpson HTU26

Plate Offsets (X,Y): [2:0-1-9,0-0-7], [6:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	-0.14	2-13	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.45	Vert(TL)	-0.27	2-13	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.41	Horz(TL)	0.07	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 167 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-7-8 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-4-15 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 7-9
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 9=969/Mechanical, 2=1093/0-3-8
Max Horz 2=195(load case 6)
Max Uplift 9=-265(load case 5), 2=-262(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1801/904, 3-4=-1570/802, 4-5=-1374/777, 5-6=-1598/863, 6-7=-1198/630, 7-8=-32/12, 8-9=-142/100
BOT CHORD 2-13=-952/1544, 12-13=-906/1633, 11-12=-906/1633, 10-11=-813/1490, 9-10=-543/997
WEBS 3-13=-204/199, 4-13=-131/412, 5-13=-335/166, 5-11=-93/112, 6-11=-86/192, 6-10=-507/317, 7-10=-227/521, 7-9=-1245/685

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

JOINT STRESS INDEX
2 = 0.86, 3 = 0.33, 4 = 0.60, 5 = 0.46, 6 = 0.26, 7 = 0.46, 8 = 0.26, 9 = 0.46, 10 = 0.46, 11 = 0.46, 12 = 0.54 and 13 = 0.56

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Job	Truss	Truss Type	Qty	Ply	LOT 8
L264527	T10	HIP	1	1	J1921206
					Job Reference (optional)

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All plates are 3x6 MT20 unless otherwise indicated.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 265 lb uplift at joint 9 and 262 lb uplift at joint 2.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1393 Colonial Bay Blvd
Boynton Beach, FL 33435

January 4,2008

Job	Truss	Truss Type	Qty	Ply	LOT 8
L264527	T11	HIP	1	1	J1921207
					Job Reference (optional)

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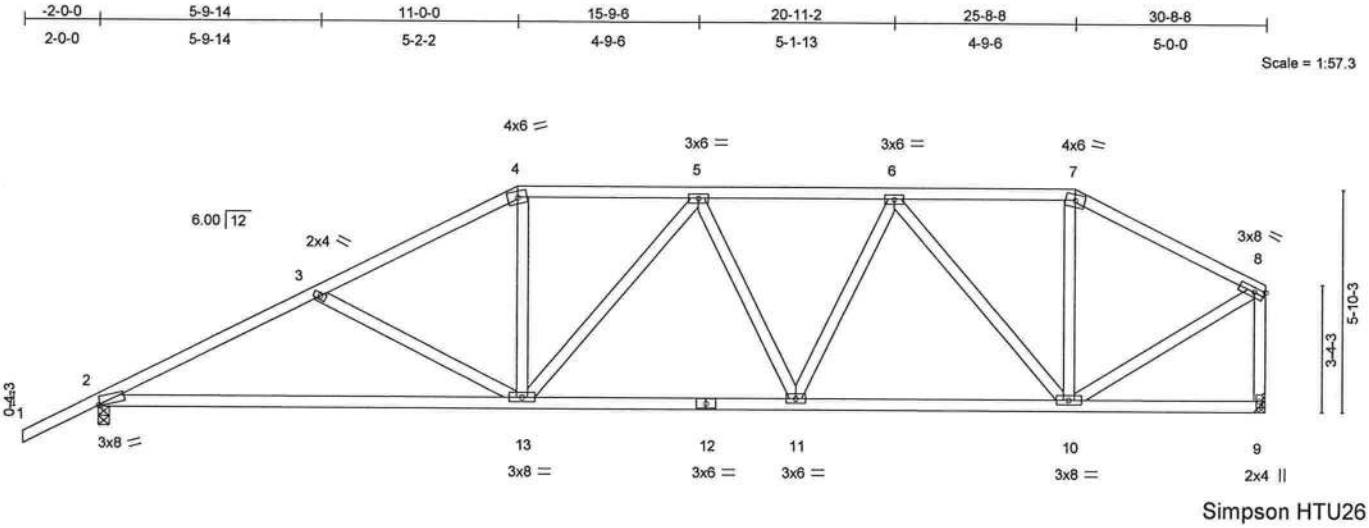

Plate Offsets (X,Y): [2:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.35	Vert(LL)	-0.30	2-13	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.65	Vert(TL)	-0.56	2-13	>650	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.62	Horz(TL)	0.06	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 173 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-6-10 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-7-7 oc bracing.

REACTIONS (lb/size) 2=1093/0-3-8, 9=969/Mechanical
Max Horz 2=177(load case 6)
Max Uplift 2=-280(load case 6), 9=-182(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1772/936, 3-4=-1455/788, 4-5=-1250/768, 5-6=-1264/764, 6-7=-752/497, 7-8=-893/493, 8-9=-944/530
BOT CHORD 2-13=-897/1516, 12-13=-695/1337, 11-12=-695/1337, 10-11=-600/1160, 9-10=-29/33
WEBS 3-13=-311/290, 4-13=-132/385, 5-13=-252/125, 5-11=-189/132, 6-11=-93/261, 6-10=-674/354, 7-10=-19/193, 8-10=-405/843

JOINT STRESS INDEX
2 = 0.89, 3 = 0.33, 4 = 0.59, 5 = 0.45, 6 = 0.45, 7 = 0.51, 8 = 0.94, 9 = 0.42, 10 = 0.78, 11 = 0.45, 12 = 0.44 and 13 = 0.56

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

January 4,2008

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Job	Truss	Truss Type	Qty	Ply	LOT 8
L264527	T11	HIP	1	1	J1921207
					Job Reference (optional)

NOTES

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 280 lb uplift at joint 2 and 182 lb uplift at joint 9.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

January 4,2008

Job	Truss	Truss Type	Qty	Ply	LOT 8
L264527	T12	HIP	1	1	J1921208
					Job Reference (optional)

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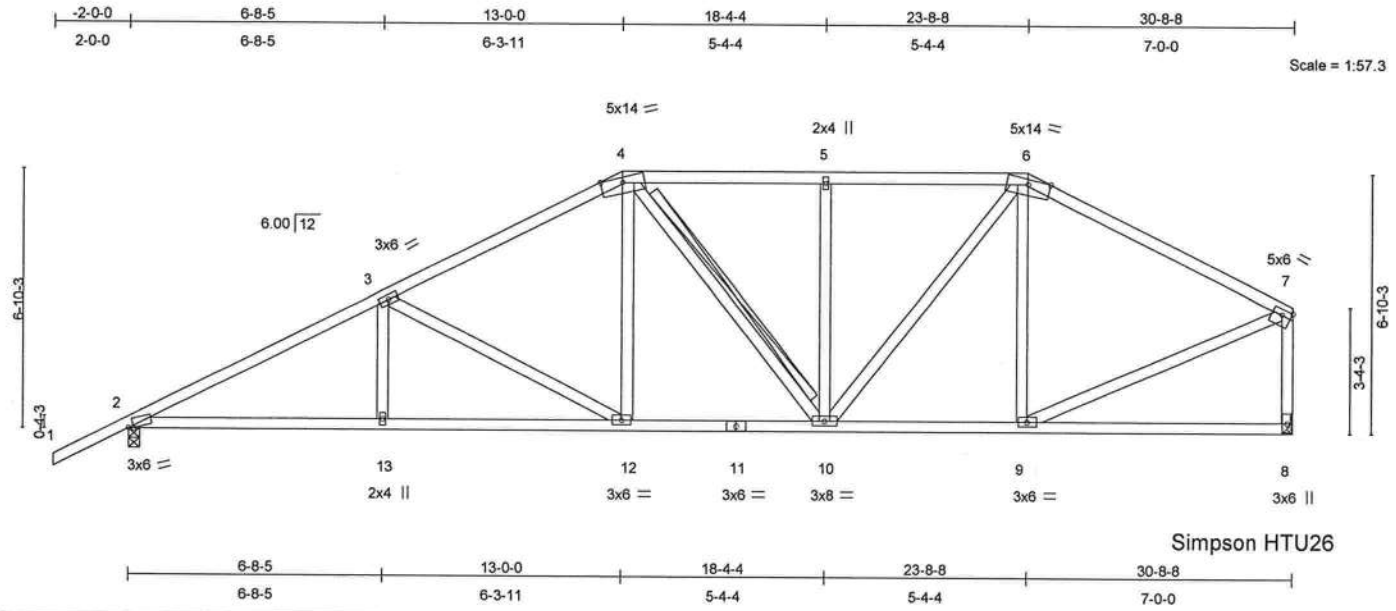

Plate Offsets (X,Y): [2:0-1-9,0-0-7], [7:Edge,0-1-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.64	Vert(LL)	0.10	12-13	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.35	Vert(TL)	-0.15	12-13	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.42	Horz(TL)	0.05	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 180 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-5-11 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-9-8 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 4-10
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1093/0-3-8, 8=969/Mechanical
Max Horz 2=189(load case 6)
Max Uplift 2=-292(load case 6), 8=-158(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1782/908, 3-4=-1326/762, 4-5=-1095/721, 5-6=-1095/721, 6-7=-1004/566, 7-8=-928/547
BOT CHORD 2-13=-866/1513, 12-13=-866/1513, 11-12=-576/1122, 10-11=-576/1122, 9-10=-404/819, 8-9=-57/74
WEBS 3-13=0/210, 3-12=-452/330, 4-12=-120/328, 4-10=-75/116, 5-10=-287/160, 6-10=-223/505, 6-9=-277/204, 7-9=-385/822

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33436

JOINT STRESS INDEX
2 = 0.77, 3 = 0.39, 4 = 0.78, 5 = 0.33, 6 = 0.94, 7 = 0.69, 8 = 0.30, 9 = 0.46, 10 = 0.56, 11 = 0.38, 12 = 0.34 and 13 = 0.33

Continued on page 2

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Job	Truss	Truss Type	Qty	Ply	LOT 8
L264527	T12	HIP	1	1	J1921208
					Job Reference (optional)

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 292 lb uplift at joint 2 and 158 lb uplift at joint 8.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1103 Coastal Bay Blvd
Boynton Beach, FL 33435

January 4,2008

Job	Truss	Truss Type	Qty	Ply	LOT 8
L264527	T13	HIP	1	1	J1921209
					Job Reference (optional)

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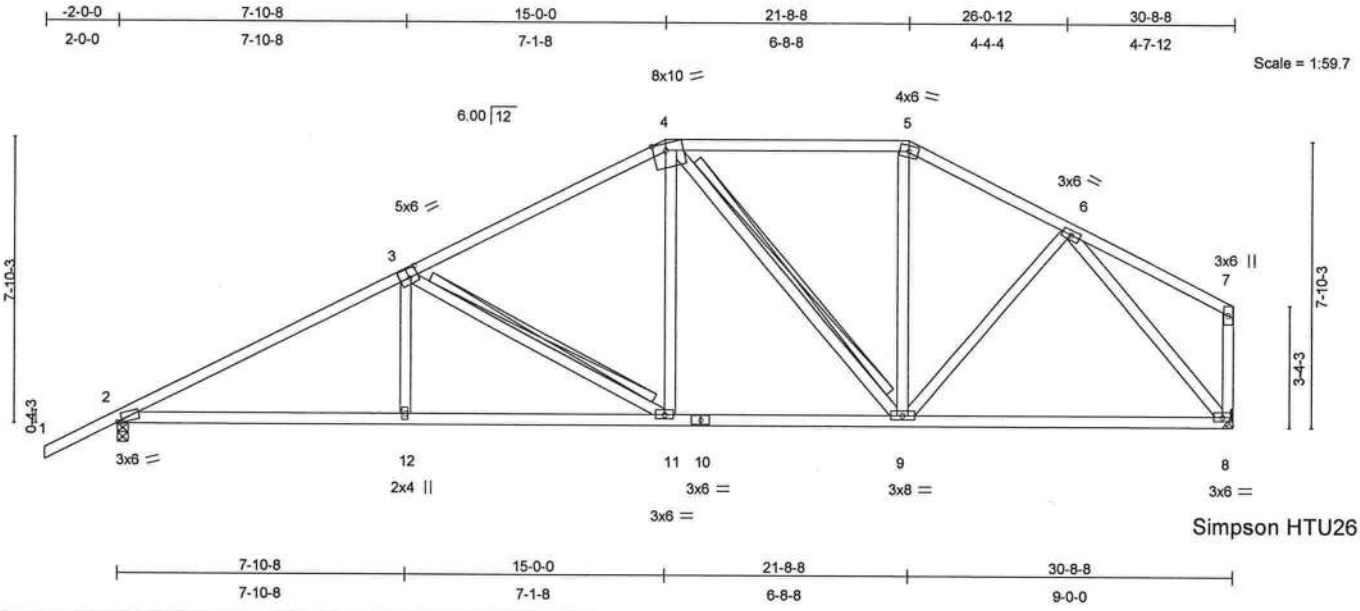

Plate Offsets (X,Y): [2:0-1-9,0-0-7], [3:0-3-0,0-3-0], [4:0-4-3,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.62	Vert(LL)	-0.13	8-9	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.44	Vert(TL)	-0.23	8-9	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.84	Horz(TL)	0.06	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 176 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 4-3-9 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 6-9-12 oc bracing.
WEBS	T-Brace: 2 X 4 SYP No.3 - 3-11, 4-9

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1093/0-3-8, 8=969/Mechanical
Max Horz 2=201(load case 6)
Max Uplift 2=-302(load case 6), 8=-172(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/47, 2-3=-1742/902, 3-4=-1205/723, 4-5=-837/617, 5-6=-979/631, 6-7=-109/79, 7-8=-147/118
BOT CHORD	2-12=-846/1468, 11-12=-846/1467, 10-11=-498/1004, 9-10=-498/1004, 8-9=-382/691
WEBS	3-12=0/250, 3-11=-537/400, 4-11=-157/369, 4-9=-329/164, 5-9=-63/207, 6-9=-51/286, 6-8=-1008/582

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

JOINT STRESS INDEX

2 = 0.76, 3 = 0.82, 4 = 0.60, 5 = 0.66, 6 = 0.37, 7 = 0.23, 8 = 0.77, 9 = 0.56, 10 = 0.33, 11 = 0.34 and 12 = 0.33

Continued on page 2

January 4,2008

Job	Truss	Truss Type	Qty	Ply	LOT 8
L264527	T13	HIP	1	1	J1921209
					Job Reference (optional)

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 302 lb uplift at joint 2 and 172 lb uplift at joint 8.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1103 Coastal Bay Blvd
Boynton Beach, FL 33435

January 4,2008

Job	Truss	Truss Type	Qty	Ply	LOT 8
L264527	T14	HIP	1	1	J1921210
					Job Reference (optional)

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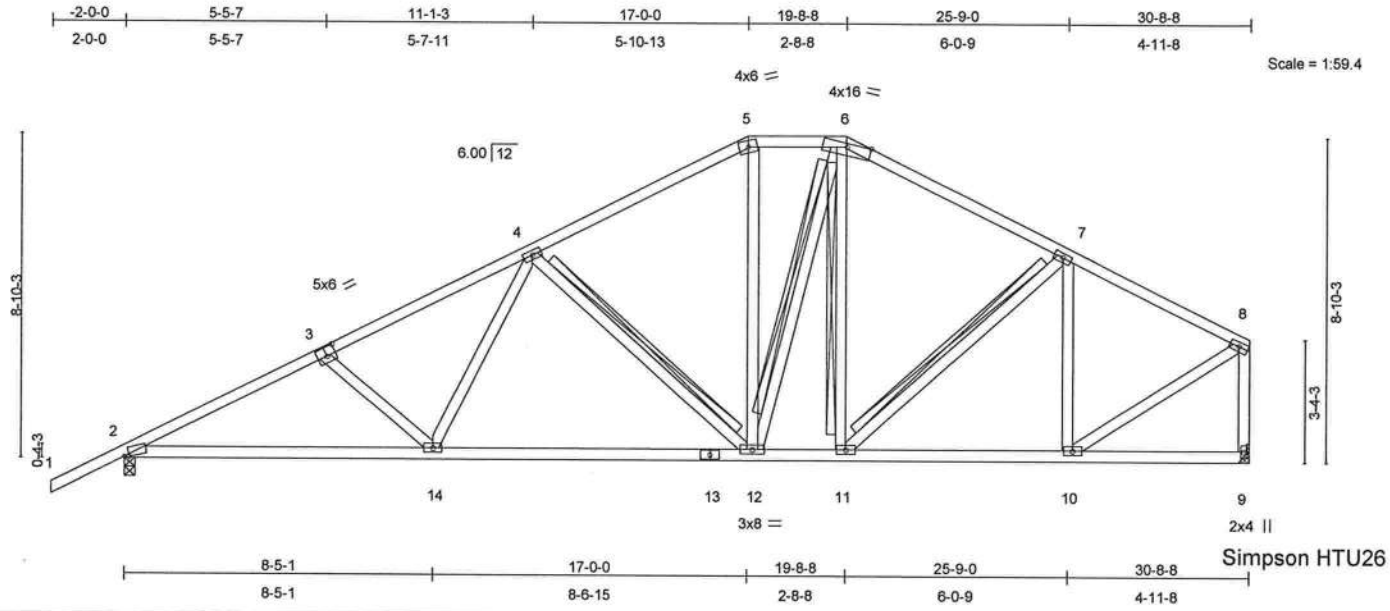

Plate Offsets (X,Y): [2:0-1-9,0-0-7], [3:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.32	Vert(LL)	-0.12	12-14	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.45	Vert(TL)	-0.24	12-14	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.27	Horz(TL)	0.05	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 195 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-7-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-6-5 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 4-12, 6-12, 6-11, 7-11
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1093/0-3-8, 9=969/Mechanical
Max Horz 2=213(load case 6)
Max Uplift 2=-310(load case 6), 9=-185(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1798/964, 3-4=-1580/900, 4-5=-1039/689, 5-6=-864/679, 6-7=-978/654, 7-8=-880/516, 8-9=-944/557
BOT CHORD 2-14=-927/1538, 13-14=-697/1231, 12-13=-697/1231, 11-12=-372/806, 10-11=-402/743, 9-10=-21/28
WEBS 3-14=-253/246, 4-14=-115/361, 4-12=-504/405, 5-12=-140/242, 6-12=-104/322, 6-11=-78/52, 7-11=-39/167, 7-10=-409/290, 8-10=-454/851

Julius Lane
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd.
Boynton Beach, FL 33436

JOINT STRESS INDEX
2 = 0.77, 3 = 0.39, 4 = 0.40, 5 = 0.53, 6 = 0.80, 7 = 0.39, 8 = 0.69, 9 = 0.41, 10 = 0.48, 11 = 0.35, 12 = 0.66, 13 = 0.40 and 14 = 0.45

Continued on page 2

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Job	Truss	Truss Type	Qty	Ply	LOT 8
L264527	T14	HIP	1	1	J1921210
					Job Reference (optional)

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 3x6 MT20 unless otherwise indicated.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 310 lb uplift at joint 2 and 185 lb uplift at joint 9.

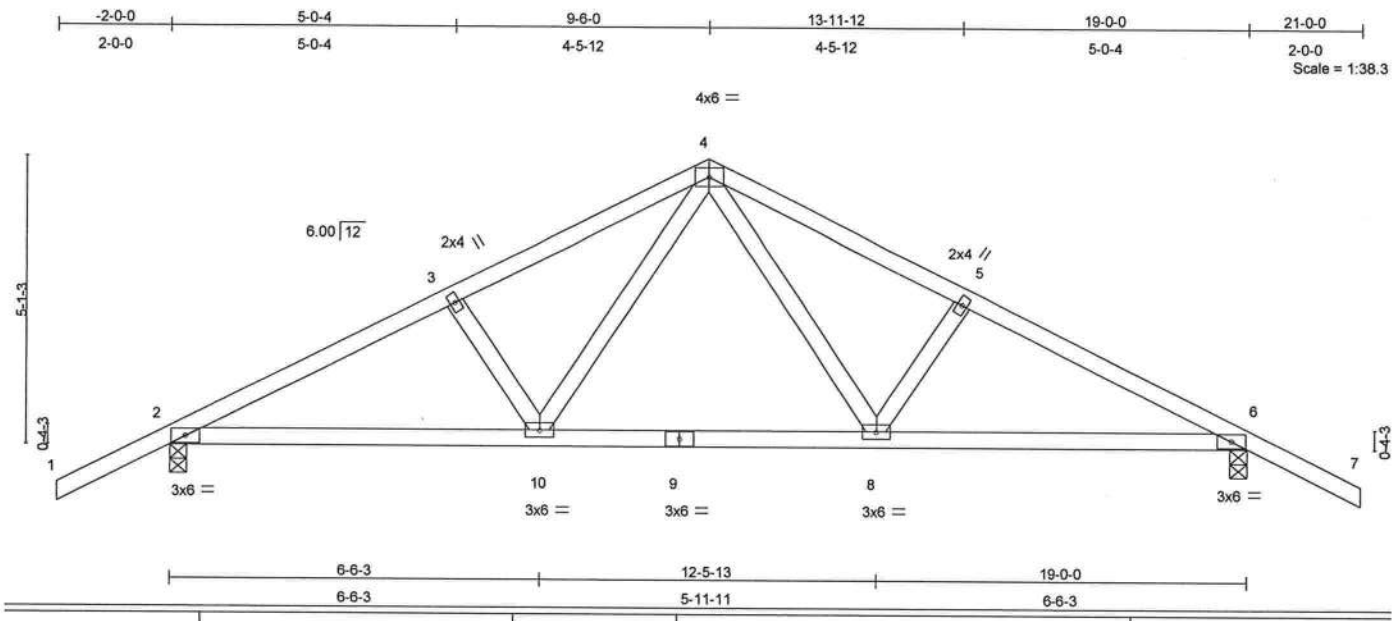
LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd.
Boynton Beach, FL 33435

January 4,2008

Job	Truss	Truss Type	Qty	Ply	LOT 8
L264527	T15	COMMON	1	1	J1921211
					Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 13:49:15 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	0.15	8-10	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.51	Vert(TL)	-0.22	8-10	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.16	Horz(TL)	0.03	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 91 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-2-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-9-0 oc bracing.

REACTIONS (lb/size) 2=894/0-3-8, 6=894/0-3-8
Max Horz 2=92(load case 6)
Max Uplift 2=-276(load case 6), 6=-276(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1416/749, 3-4=-1280/749, 4-5=-1280/749, 5-6=-1416/749, 6-7=0/47
BOT CHORD 2-10=-502/1196, 9-10=-262/838, 8-9=-262/838, 6-8=-502/1196
WEBS 3-10=-199/187, 4-10=-267/506, 4-8=-267/506, 5-8=-199/187

JOINT STRESS INDEX
2 = 0.62, 3 = 0.33, 4 = 0.46, 5 = 0.33, 6 = 0.62, 8 = 0.41, 9 = 0.64 and 10 = 0.41

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 276 lb uplift at joint 2 and 276 lb uplift at joint 6.

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

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Job	Truss	Truss Type	Qty	Ply	LOT 8
L264527	T15	COMMON	1	1	J1921211
					Job Reference (optional)

NOTES

6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-4=-54, 4-7=-54, 2-10=-10, 8-10=-70(F=-60), 6-8=-10

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

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Job	Truss	Truss Type	Qty	Ply	LOT 8
L264527	T15G	GABLE	1	1	J1921212
					Job Reference (optional)

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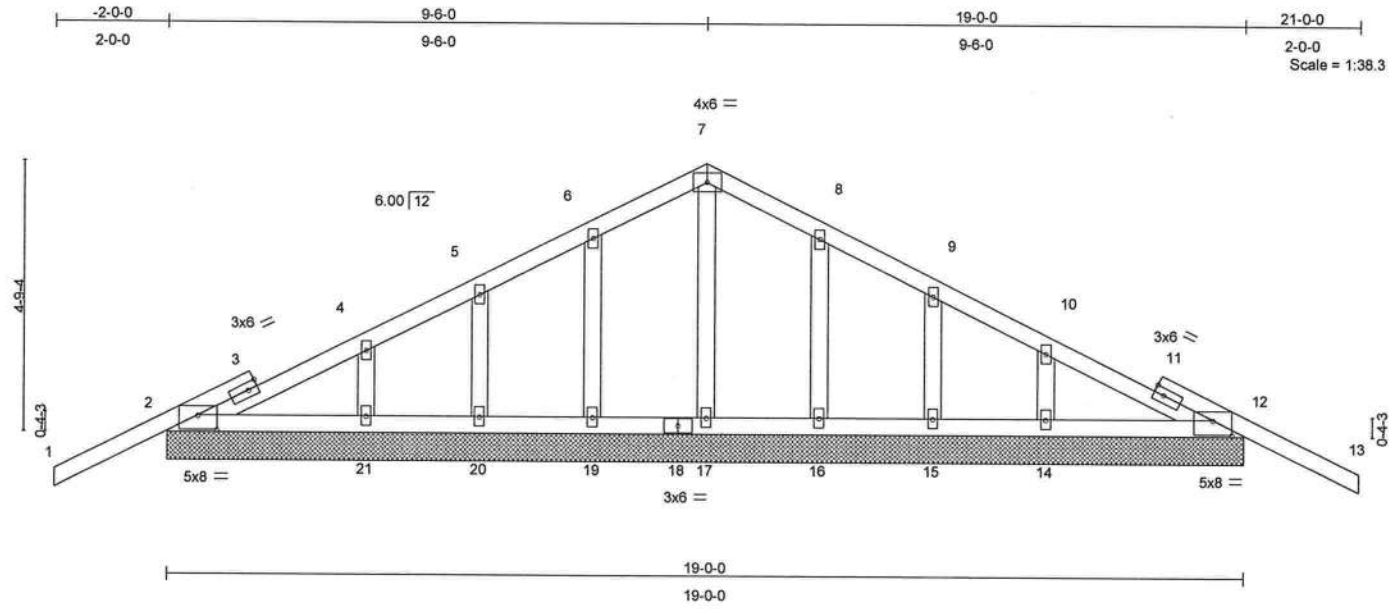

Plate Offsets (X,Y): [2:0-4-0,0-3-1], [12:0-4-0,0-3-1]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.26	Vert(LL)	-0.02	13	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.05	Vert(TL)	-0.03	13	n/r	90		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.04	Horz(TL)	0.00	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 96 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=239/19-0-0, 12=239/19-0-0, 17=129/19-0-0, 19=132/19-0-0, 20=115/19-0-0, 21=166/19-0-0, 16=132/19-0-0, 15=115/19-0-0, 14=166/19-0-0
Max Horz 2=-100(load case 7)
Max Uplift 2=-185(load case 6), 12=-200(load case 7), 19=-81(load case 6), 20=-98(load case 6), 21=-65(load case 7), 16=-80(load case 7), 15=-97(load case 7), 14=-68(load case 7)
Max Grav 2=239(load case 10), 12=239(load case 11), 17=129(load case 1), 19=136(load case 10), 20=115(load case 1), 21=166(load case 10), 16=136(load case 11), 15=115(load case 1), 14=166(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-61/37, 3-4=-52/50, 4-5=-24/88, 5-6=-20/138, 6-7=-23/191, 7-8=-23/191, 8-9=-20/138, 9-10=-24/78, 10-11=-19/38, 11-12=-32/15, 12-13=0/47
BOT CHORD 2-21=-7/114, 20-21=-7/114, 19-20=-7/114, 18-19=-7/114, 17-18=-7/114, 16-17=-7/114, 15-16=-7/114, 14-15=-7/114, 12-14=-7/114
WEBS 7-17=-110/0, 6-19=-115/102, 5-20=-99/117, 4-21=-137/102, 8-16=-115/102, 9-15=-99/117, 10-14=-137/102

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

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Job	Truss	Truss Type	Qty	Ply	LOT 8
L264527	T15G	GABLE	1	1	J1921212
					Job Reference (optional)

JOINT STRESS INDEX

2 = 0.83, 3 = 0.00, 3 = 0.18, 4 = 0.33, 5 = 0.33, 6 = 0.33, 7 = 0.24, 8 = 0.33, 9 = 0.33, 10 = 0.33, 11 = 0.00, 11 = 0.18, 12 = 0.83, 14 = 0.33, 15 = 0.33, 16 = 0.33, 17 = 0.33, 18 = 0.15, 19 = 0.33, 20 = 0.33 and 21 = 0.33

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 2x4 MT20 unless otherwise indicated.
6) Gable requires continuous bottom chord bearing.
7) Gable studs spaced at 2-0-0 oc.
8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 185 lb uplift at joint 2, 200 lb uplift at joint 12, 81 lb uplift at joint 19, 98 lb uplift at joint 20, 65 lb uplift at joint 21, 80 lb uplift at joint 16, 97 lb uplift at joint 15 and 68 lb uplift at joint 14.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1199 Coastal Bay Blvd
Boynton Beach, FL 33435

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Job	Truss	Truss Type	Qty	Ply	LOT 8
L264527	T16	COMMON	3	1	J1921213
					Job Reference (optional)

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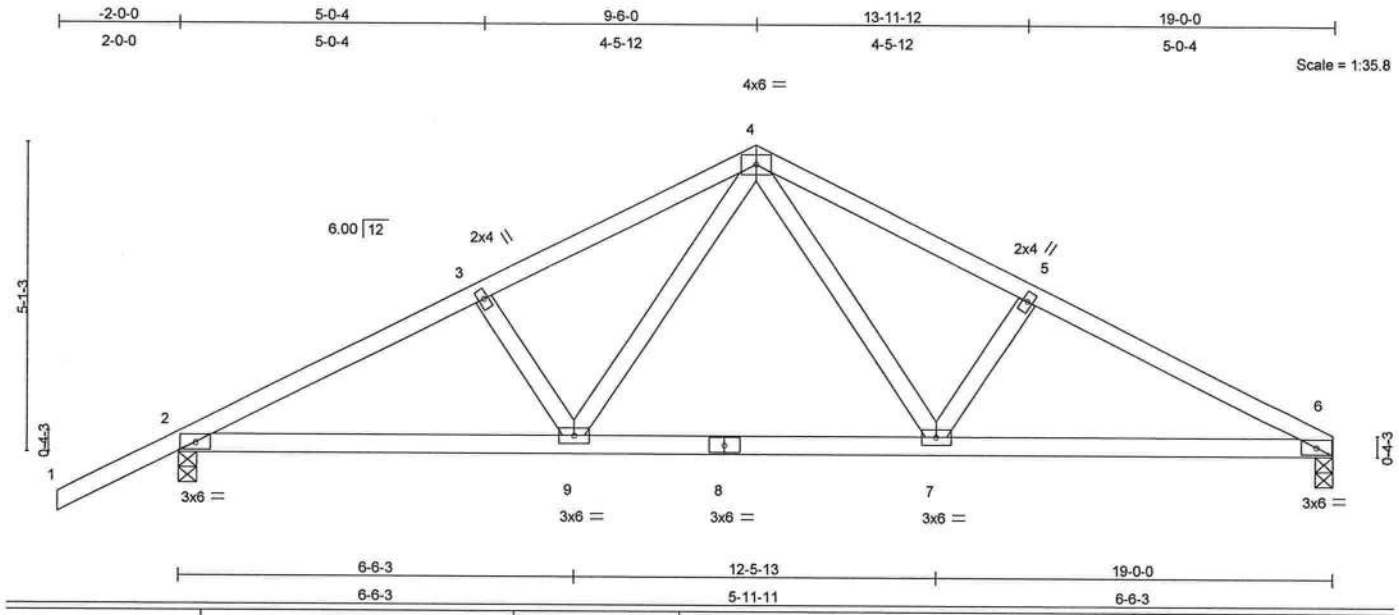

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	0.14	7-9	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.54	Vert(TL)	-0.22	7-9	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.19	Horz(TL)	0.04	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 88 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-1-5 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-8-0 oc bracing.

REACTIONS (lb/size) 6=771/0-3-8, 2=900/0-3-8
Max Horz 2=105(load case 6)
Max Uplift 6=-179(load case 7), 2=-277(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1429/771, 3-4=-1293/771, 4-5=-1318/813, 5-6=-1456/815
BOT CHORD 2-9=-600/1208, 8-9=-361/851, 7-8=-361/851, 6-7=-651/1239
WEBS 3-9=-199/189, 4-9=-261/506, 4-7=-327/543, 5-7=-218/219

JOINT STRESS INDEX
2 = 0.72, 3 = 0.33, 4 = 0.44, 5 = 0.33, 6 = 0.72, 7 = 0.41, 8 = 0.64 and 9 = 0.41

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 179 lb uplift at joint 6 and 277 lb uplift at joint 2.

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1479 Crandall Bay Blvd
Boynton Beach, FL 33436

January 4,2008

Job	Truss	Truss Type	Qty	Ply	LOT 18
L264517	CJ1	JACK	16	1	Job Reference (optional)

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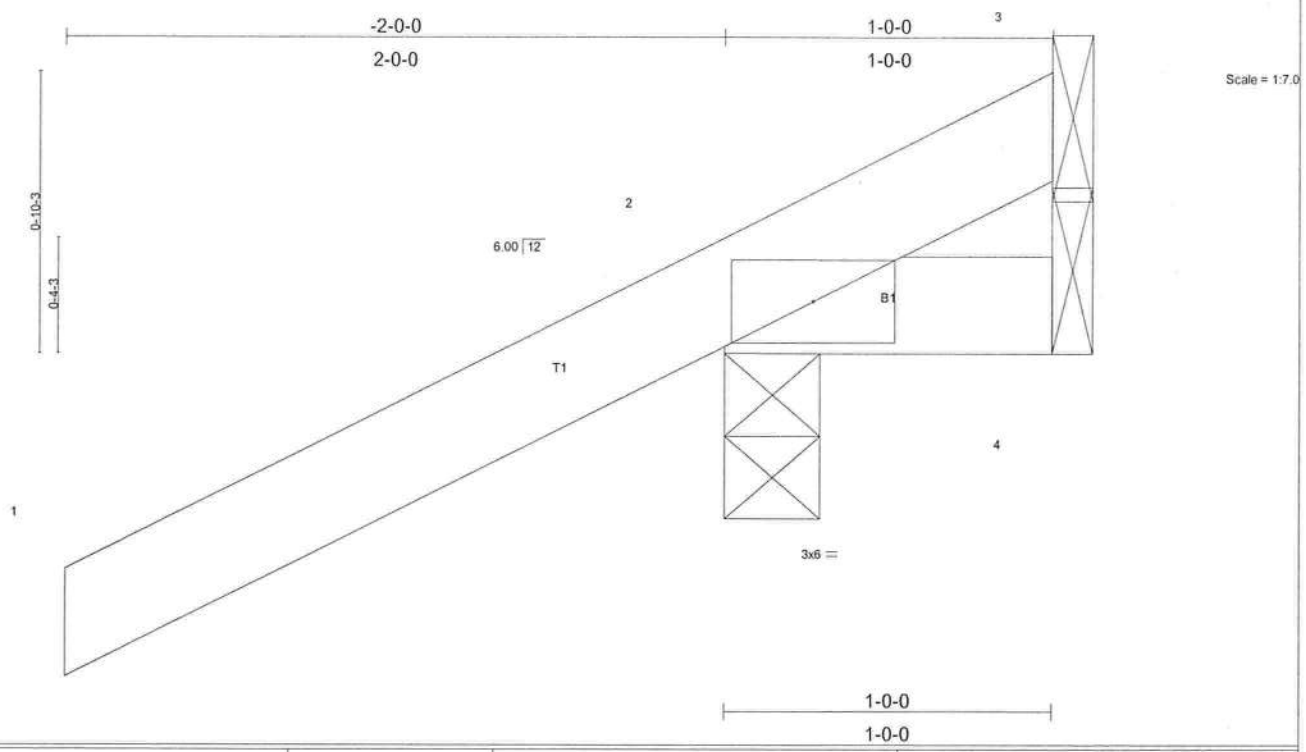

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0		Plates Increase	1.25	TC	0.28	Vert(LL)	-0.00	2	>999	360	MT20	244/190
TCDL 7.0		Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	240		
BCLL 10.0	*	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL 5.0		Code FBC2004/TPI2002		(Matrix)							Weight: 7 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=256/0-3-8, 4=5/Mechanical, 3=-90/Mechanical
Max Horz 2=87(load case 6)
Max Uplift2=-286(load case 6), 4=-9(load case 4), 3=-90(load case 1)
Max Grav2=256(load case 1), 4=14(load case 2), 3=127(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-69/75
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 286 lb uplift at joint 2, 9 lb uplift at joint 4 and 90 lb uplift at joint 3.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	LOT 18
L264517	CJ3	JACK	14	1	Job Reference (optional)

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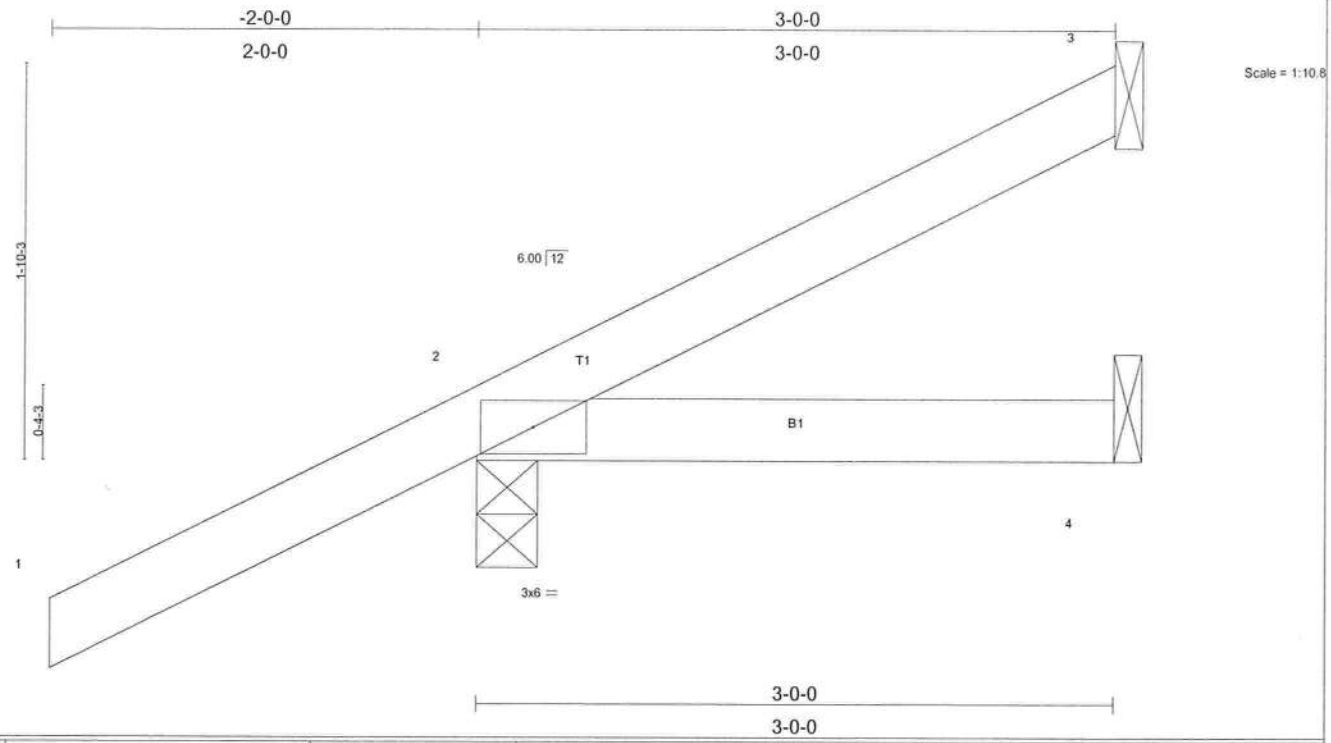

LOADING (psf)			SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.29	Vert(LL)	0.01	2-4	>999	360	MT20	244/190
TCDL	7.0		Lumber Increase	1.25	BC	0.08	Vert(TL)	-0.01	2-4	>999	240		
BCLL	10.0	*	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0		Code FBC2004/TPI2002		(Matrix)							Weight: 13 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=31/Mechanical, 2=250/0-3-8, 4=14/Mechanical
Max Horz 2=132(load case 6)
Max Uplift3=-28(load case 7), 2=-238(load case 6), 4=-27(load case 4)
Max Grav3=31(load case 1), 2=250(load case 1), 4=42(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-57/7
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 3, 238 lb uplift at joint 2 and 27 lb uplift at joint 4.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	LOT 18
L264517	CJ5	JACK	14	1	Job Reference (optional)

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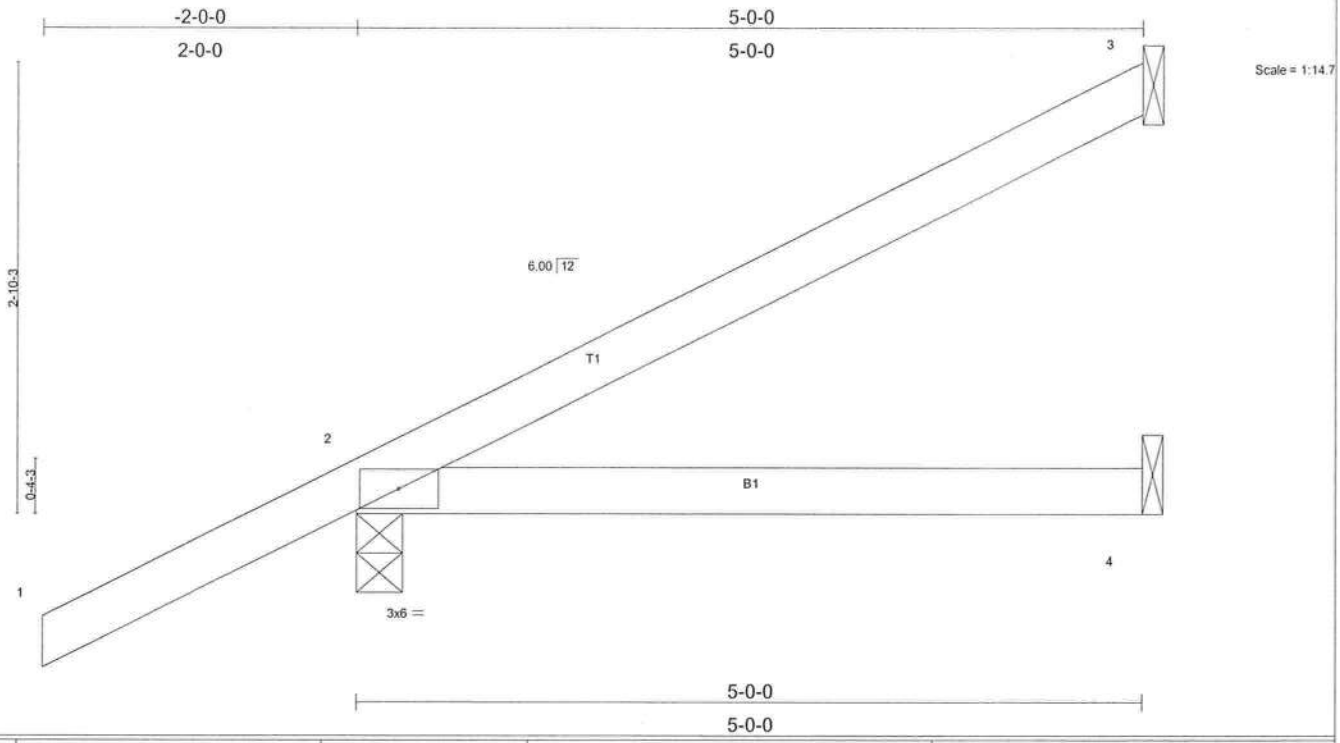

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	0.09	2-4	>663	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.24	Vert(TL)	-0.05	2-4	>999	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 19 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=103/Mechanical, 2=295/0-3-8, 4=24/Mechanical
Max Horz 2=178(load case 6)
Max Uplift3=-87(load case 6), 2=-260(load case 6), 4=-46(load case 4)
Max Grav3=103(load case 1), 2=295(load case 1), 4=72(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-88/36
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 87 lb uplift at joint 3, 260 lb uplift at joint 2 and 46 lb uplift at joint 4.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	LOT 18
L264517	EJ7	JACK	30	1	Job Reference (optional)

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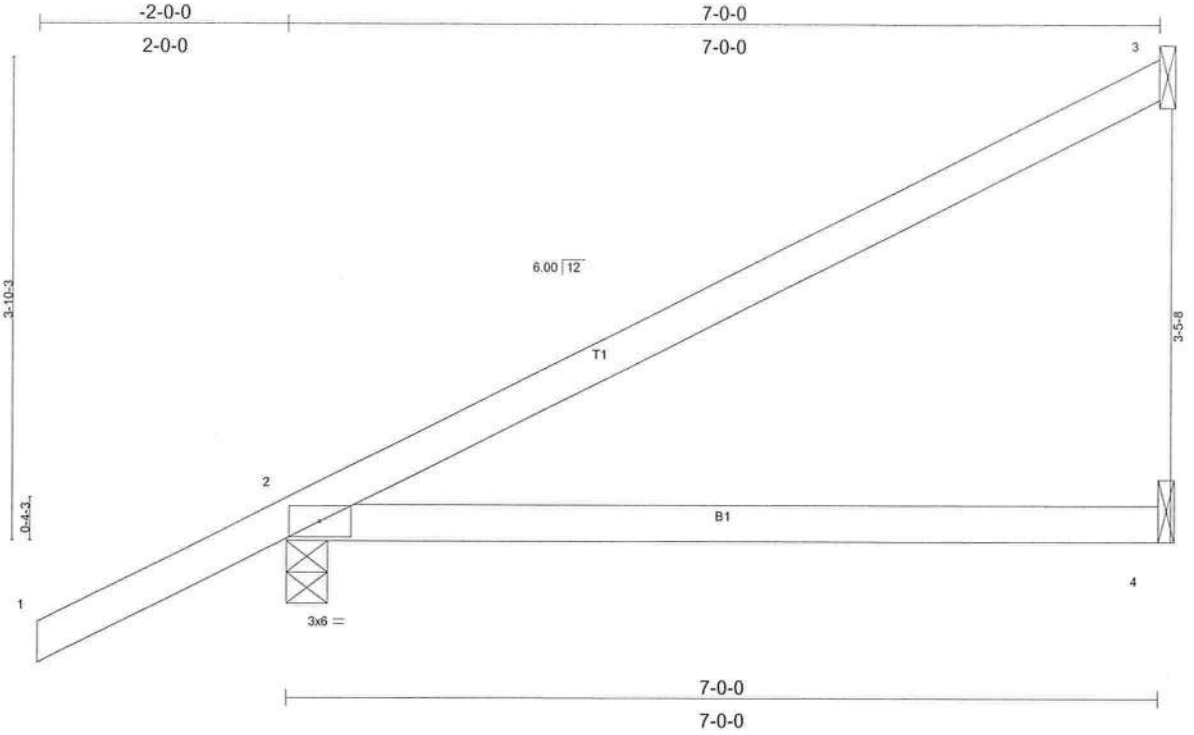

*** Design Problems ***
REVIEW REQUIRED

Max Deflection In Panel Exceeded: 2-3, 2-4
Max Vertical Deflection Exceeded In Span: 2-4

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.50	Vert(LL)	0.32	2-4	>253	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.45	Vert(TL)	-0.16	2-4	>506	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 26 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=154/Mechanical, 2=352/0-4-0, 4=44/Mechanical
Max Horz 2=161(load case 6)
Max Uplift3=-94(load case 6), 2=-225(load case 6), 4=-64(load case 5)
Max Grav3=154(load case 1), 2=352(load case 1), 4=93(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-131/54
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 94 lb uplift at joint 3, 225 lb uplift at joint 2 and 64 lb uplift at joint 4.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	LOT 8
L264527	T16	COMMON	3	1	J1921213
					Job Reference (optional)

NOTES

6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-6=-54, 2-9=-10, 7-9=-70(F=-60), 6-7=-10

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

January 4,2008

Job	Truss	Truss Type	Qty	Ply	LOT 8
L264527	T17	HOWE	1	2	J1921214

Job Reference (optional)

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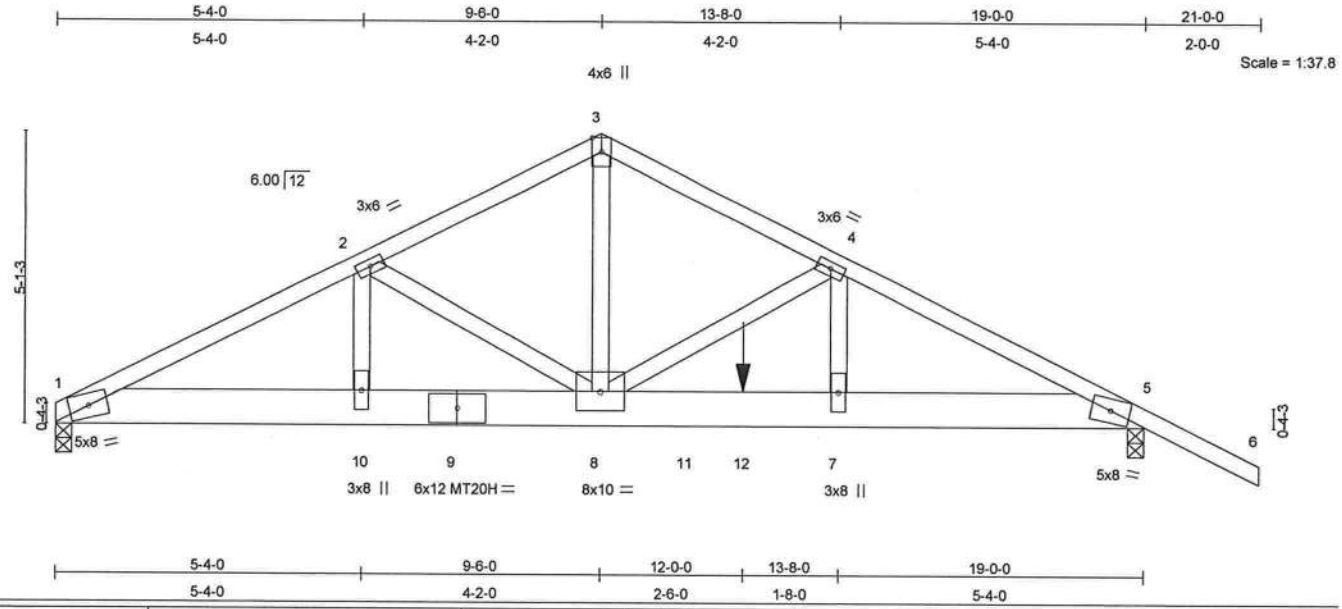

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.31	Vert(LL)	-0.11	7-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.43	Vert(TL)	-0.21	7-8	>999	240	MT20H	187/143
BCLL	10.0	* Rep Stress Incr	NO	WB	0.46	Horz(TL)	0.04	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 239 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-6-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=4884/0-3-8, 5=3516/0-3-8
Max Horz 1=-111(load case 6)
Max Uplift 1=-1317(load case 5), 5=-1001(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-8132/2180, 2-3=-5888/1594, 3-4=-5876/1603, 4-5=-7289/1908, 5-6=0/53
BOT CHORD 1-10=-1947/7255, 9-10=-1947/7255, 8-9=-1947/7255, 8-11=-1661/6476, 11-12=-1661/6476, 7-12=-1661/6476, 5-7=-1661/6476
WEBS 2-10=-545/1961, 2-8=-2393/709, 3-8=-1342/4963, 4-8=-1479/428, 4-7=-278/1151

JOINT STRESS INDEX
1 = 0.82, 2 = 0.71, 3 = 0.55, 4 = 0.71, 5 = 0.82, 7 = 0.31, 8 = 0.45, 9 = 0.71 and 10 = 0.31

NOTES
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-7-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

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NOTES

4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
5) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) All plates are MT20 plates unless otherwise indicated.
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1317 lb uplift at joint 1 and 1001 lb uplift at joint 5.
9) Girder carries tie-in span(s): 30-8-8 from 0-0-0 to 11-0-0

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-3=-54, 3-6=-54, 1-11=-465(B=-455), 5-11=-10
   Concentrated Loads (lb)
   Vert: 12=-2151(F)

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Conrad Say Blvd
Boynton Beach, FL 33435

January 4,2008

Job	Truss	Truss Type	Qty	Ply	LOT 8
L264527	T18	HIP	1	1	J1921215
					Job Reference (optional)

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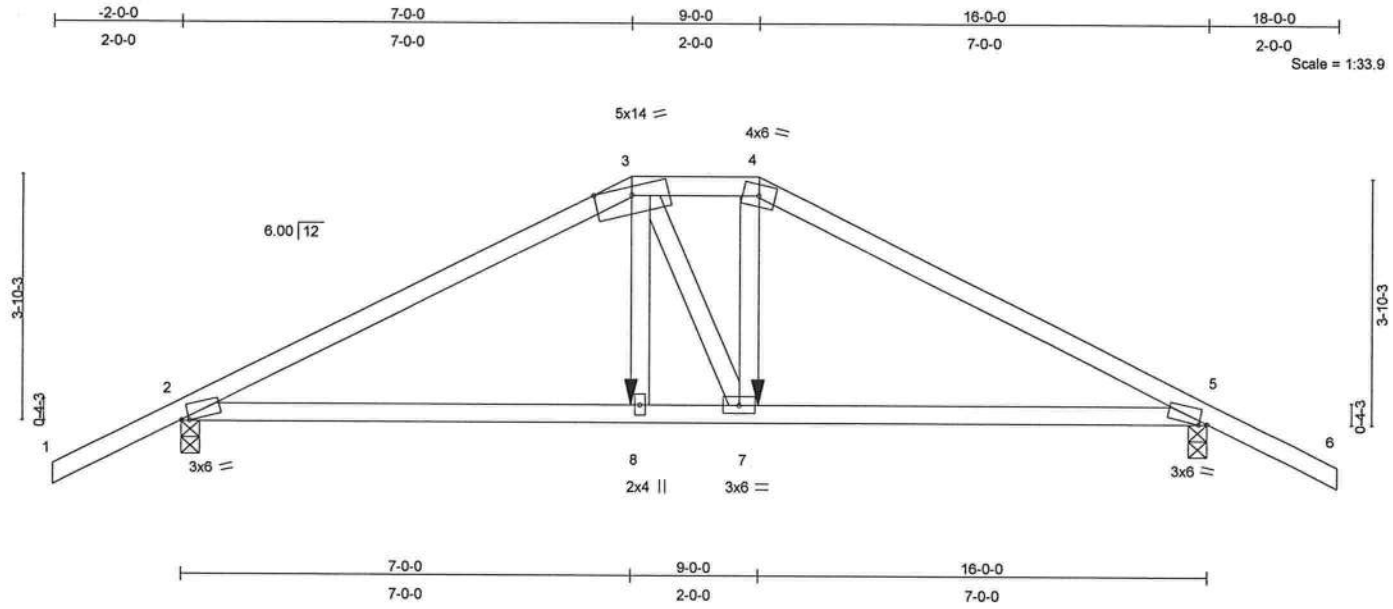

Plate Offsets (X,Y): [2:0-1-8,0-0-7], [5:0-1-8,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.40	Vert(LL)	0.12	2-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.42	Vert(TL)	-0.14	2-8	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.19	Horz(TL)	0.04	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 72 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-10 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-5-6 oc bracing.

REACTIONS (lb/size) 2=1103/0-3-8, 5=1103/0-3-8
Max Horz 2=77(load case 5)
Max Uplift 2=-595(load case 5), 5=-595(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1778/804, 3-4=-1526/770, 4-5=-1781/805, 5-6=0/47
BOT CHORD 2-8=-674/1503, 7-8=-684/1523, 5-7=-658/1506
WEBS 3-8=-262/480, 3-7=-146/159, 4-7=-303/592

JOINT STRESS INDEX
2 = 0.77, 3 = 0.87, 4 = 0.76, 5 = 0.77, 7 = 0.38 and 8 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Julius Lee
Truss Design Engineer
Florida PE# No. 34169
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

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Job	Truss	Truss Type	Qty	Ply	LOT 8 J1921215
L264527	T18	HIP	1	1	Job Reference (optional)

NOTES

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 595 lb uplift at joint 2 and 595 lb uplift at joint 5.
7) Girder carries hip end with 7-0-0 end setback.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-3=-54, 3-4=-118(F=-64), 4-6=-54, 2-8=-10, 7-8=-22(F=-12), 5-7=-10
   Concentrated Loads (lb)
   Vert: 8=-411(F) 7=-411(F)

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Central Bay Blvd
Boynton Beach, FL 33435

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Job	Truss	Truss Type	Qty	Ply	LOT 8
L264527	T19	QUEENPOST	2	1	J1921216
					Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 13:49:19 2007 Page 1

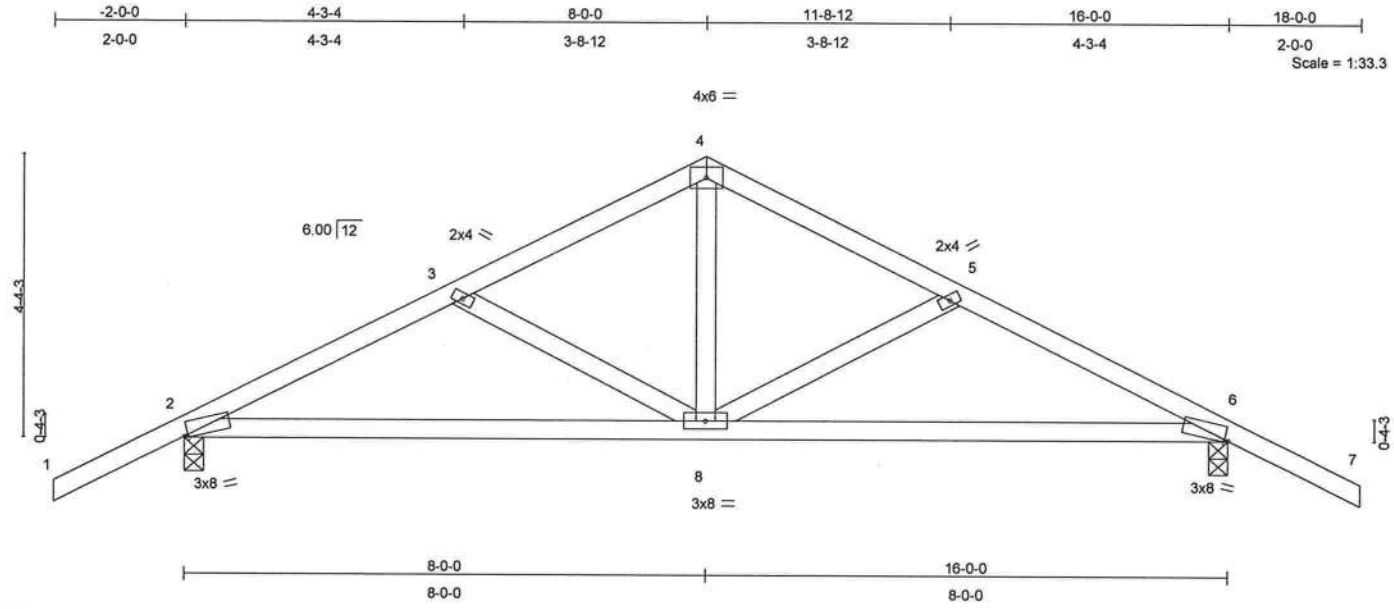

Plate Offsets (X,Y): [2:0-0-10,Edge], [6:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	0.19	2-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.32	Vert(TL)	-0.12	2-8	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.20	Horz(TL)	-0.02	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 75 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-7-15 oc bracing.

REACTIONS (lb/size) 2=619/0-3-8, 6=619/0-3-8
Max Horz 2=83(load case 6)
Max Uplift 2=-404(load case 6), 6=-404(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-802/1064, 3-4=-590/942, 4-5=-590/942, 5-6=-802/1064, 6-7=0/47
BOT CHORD 2-8=-810/663, 6-8=-810/663
WEBS 3-8=-217/243, 4-8=-670/331, 5-8=-217/243

JOINT STRESS INDEX
2 = 0.74, 3 = 0.13, 4 = 0.32, 5 = 0.13, 6 = 0.74 and 8 = 0.17

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1105 Coastal Bay Blvd
Boynton Beach, FL 33435

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Job	Truss	Truss Type	Qty	Ply	LOT 8 J1921216
L264527	T19	QUEENPOST	2	1	Job Reference (optional)

NOTES

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 404 lb uplift at joint 2 and 404 lb uplift at joint 6.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

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Job	Truss	Truss Type	Qty	Ply	LOT 8
L264527	T20	COMMON	1	2	J1921217

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Jan 04 16:35:53 2008 Page 1

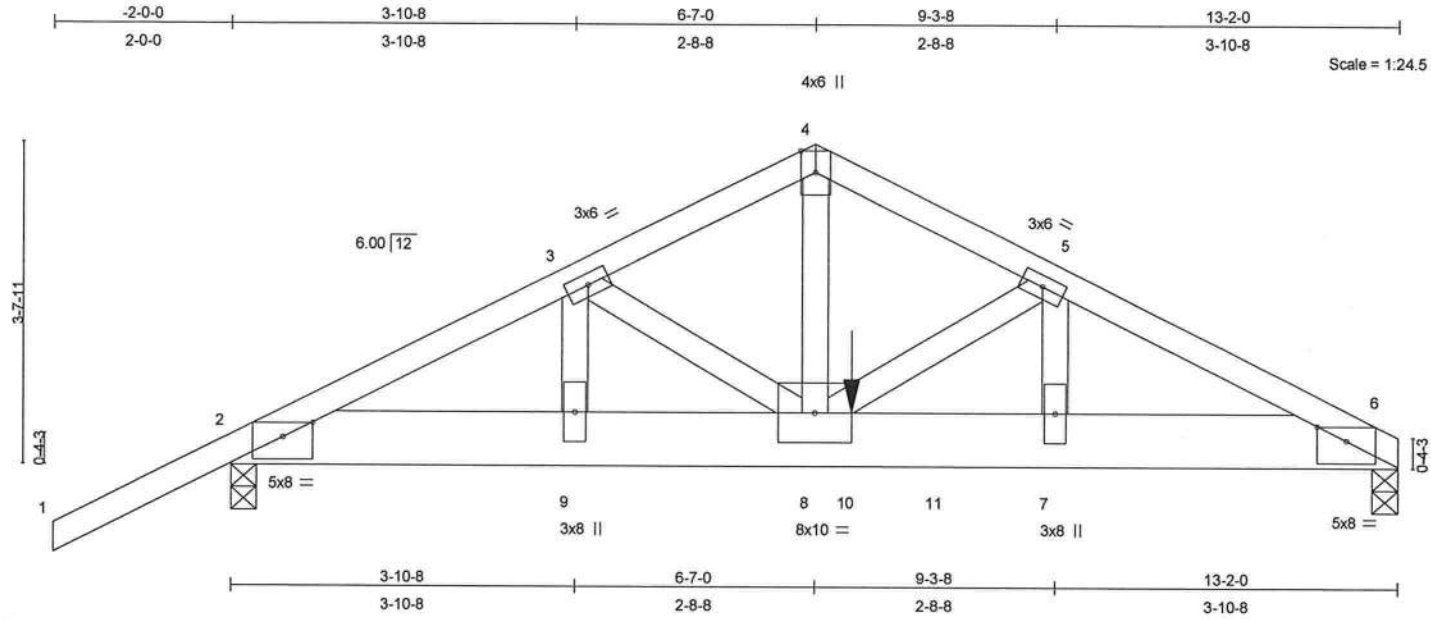

Plate Offsets (X,Y): [2:0-4-0,0-1-15], [6:0-4-0,0-1-15]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.22	Vert(LL)	-0.06	7-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.28	Vert(TL)	-0.11	7-8	>999	240		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.35	Horz(TL)	0.02	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 166 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-2-10 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 6=3999/0-3-8, 2=2281/0-3-8
Max Horz 2=94(load case 5)
Max Uplift 6=-1083(load case 6), 2=-673(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=-4266/1115, 3-4=-4536/1241, 4-5=-4533/1234, 5-6=-6364/1715
BOT CHORD 2-9=-969/3785, 8-9=-969/3785, 8-10=-1506/5684, 10-11=-1506/5684, 7-11=-1506/5684, 6-7=-1506/5684
WEBS 3-9=-469/184, 3-8=-113/411, 4-8=-1041/3829, 5-8=-1989/583, 5-7=-457/1734

JOINT STRESS INDEX
2 = 0.71, 3 = 0.64, 4 = 0.44, 5 = 0.64, 6 = 0.71, 7 = 0.28, 8 = 0.36 and 9 = 0.28

NOTES
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-4-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.

Julius Lee
Truss Design Engineer
Florida PE. No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

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Job	Truss	Truss Type	Qty	Ply	LOT 8
L264527	T20	COMMON	1	2	J1921217
					Job Reference (optional)

NOTES

4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
5) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1083 lb uplift at joint 6 and 673 lb uplift at joint 2.
8) Girder carries tie-in span(s): 36-8-8 from 8-0-0 to 12-0-0

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-4=-54, 4-6=-54, 2-11=-10, 6-11=-561(F=-551)
   Concentrated Loads (lb)
   Vert: 10=-2575(F)

Julius Lee
Truss Design Engineer
Florida PE# No. 34183
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

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Job	Truss	Truss Type	Qty	Ply	LOT 8
L264527	T20G	GABLE	1	1	J1921218
					Job Reference (optional)

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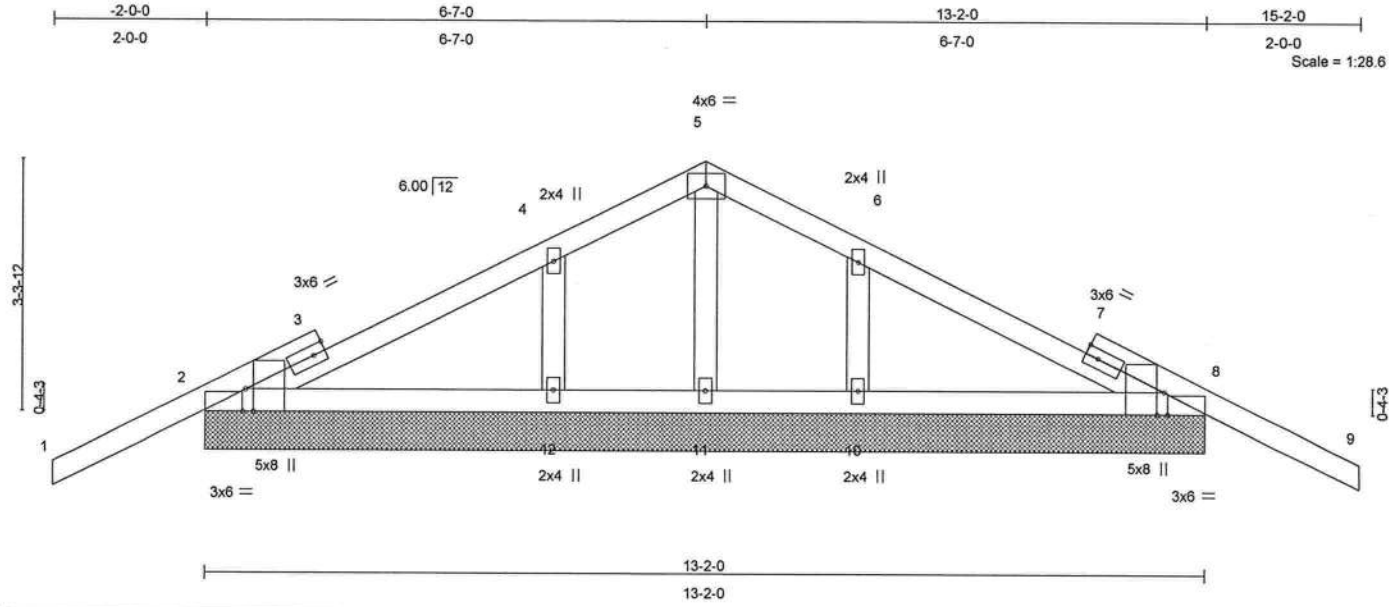

Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-0-8,Edge], [8:0-3-8,Edge], [8:0-0-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.26	Vert(LL)	-0.01	9	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.09	Vert(TL)	-0.01	9	n/r	90		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.05	Horz(TL)	0.00	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 61 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 10-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=223/13-2-0, 8=223/13-2-0, 11=117/13-2-0, 12=247/13-2-0, 10=247/13-2-0
Max Horz 2=80(load case 6)
Max Uplift 2=-188(load case 6), 8=-199(load case 7), 12=-125(load case 6), 10=-129(load case 7)
Max Grav 2=235(load case 10), 8=235(load case 11), 11=117(load case 1), 12=249(load case 10), 10=249(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-64/59, 3-4=-47/142, 4-5=0/97, 5-6=0/97, 6-7=-23/142, 7-8=-35/59, 8-9=0/47
BOT CHORD 2-12=-84/143, 11-12=-84/143, 10-11=-84/143, 8-10=-84/143
WEBS 5-11=-123/5, 4-12=-198/186, 6-10=-198/186

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1105 Coastal Bay Blvd.
Boynton Beach, FL 33435

JOINT STRESS INDEX
2 = 0.39, 2 = 0.00, 3 = 0.00, 3 = 0.21, 4 = 0.10, 5 = 0.04, 6 = 0.10, 7 = 0.00, 7 = 0.21, 8 = 0.39, 8 = 0.00, 10 = 0.10, 11 = 0.04 and 12 = 0.10

NOTES
1) Unbalanced roof live loads have been considered for this design.

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January 4,2008

Job	Truss	Truss Type	Qty	Ply	LOT 8
L264527	T20G	GABLE	1	1	J1921218
					Job Reference (optional)

NOTES

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) Gable requires continuous bottom chord bearing.
6) Gable studs spaced at 2-0-0 oc.
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 188 lb uplift at joint 2, 199 lb uplift at joint 8, 125 lb uplift at joint 12 and 129 lb uplift at joint 10.

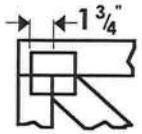
LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

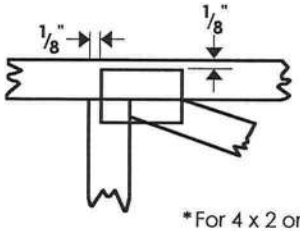
January 4,2008

Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seat.



*For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.

*This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

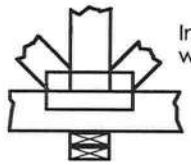
4 x 4

The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

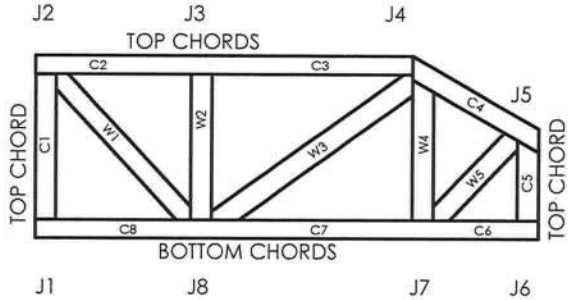
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DILHR	960022-W, 970036-N
NER	561

MiTek®

TEE-LOK®

MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

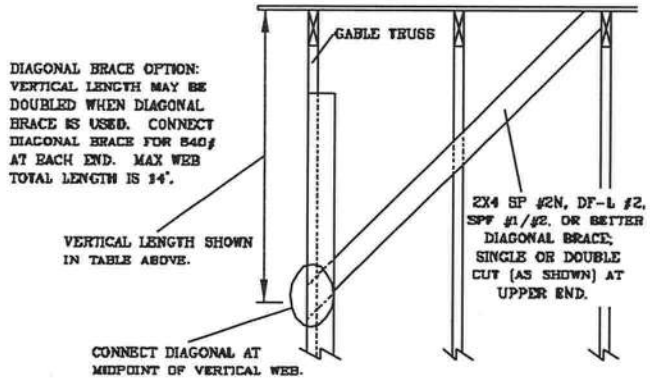
Failure to Follow Could Cause Property Damage or Personal Injury

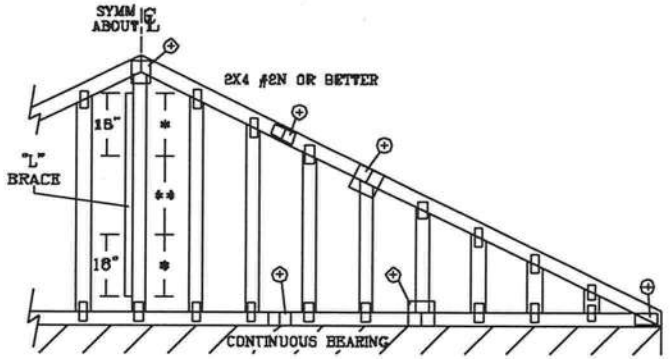
1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
4. Unless otherwise noted, locate chord splices at ¼ panel length (± 6" from adjacent joint.)
5. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
6. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
8. Plate type, size and location dimensions shown indicate minimum plating requirements.
9. Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
10. Top chords must be sheathed or purlins provided at spacing shown on design.
11. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
12. Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
13. Do not overload roof or floor trusses with stacks of construction materials.
14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
15. Care should be exercised in handling, erection and installation of trusses.

ASCE 7-02: 130 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE * GROUP A	(1) 1X4 "L" BRACE * GROUP B	(1) 2X4 "L" BRACE * GROUP A	(1) 2X4 "L" BRACE * GROUP B	(2) 2X4 "L" BRACE ** GROUP A	(2) 2X4 "L" BRACE ** GROUP B	(1) 2X6 "L" BRACE * GROUP A	(1) 2X6 "L" BRACE * GROUP B	(2) 2X6 "L" BRACE ** GROUP A	(2) 2X6 "L" BRACE ** GROUP B
24" O.C.	SPF HF	#1 / #2	3' 4"	5' 10"	6' 0"	6' 11"	7' 1"	8' 3"	8' 6"	10' 10"	11' 2"	12' 11"	13' 3"
		#3	3' 3"	4' 11"	4' 11"	6' 6"	6' 6"	8' 3"	8' 3"	10' 1"	10' 1"	12' 11"	12' 11"
		STUD	3' 3"	4' 11"	4' 11"	6' 5"	6' 5"	8' 3"	8' 3"	10' 0"	10' 0"	12' 11"	12' 11"
		STANDARD	3' 3"	4' 2"	4' 2"	5' 6"	5' 6"	7' 5"	7' 5"	8' 8"	8' 8"	11' 8"	11' 8"
	SP DFL	#1	3' 8"	5' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 11"	10' 10"	11' 8"	12' 11"	13' 11"
		#2	3' 7"	5' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 11"	10' 10"	11' 8"	12' 11"	13' 11"
		#3	3' 6"	5' 0"	5' 0"	6' 8"	6' 8"	8' 3"	8' 8"	10' 4"	10' 4"	12' 11"	13' 7"
		STUD	3' 6"	5' 0"	5' 0"	6' 7"	6' 7"	8' 3"	8' 8"	10' 3"	10' 3"	12' 11"	13' 7"
		STANDARD	3' 4"	4' 3"	4' 3"	5' 8"	5' 8"	7' 8"	7' 8"	8' 10"	8' 10"	12' 0"	12' 0"
16" O.C.	SPF HF	#1 / #2	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 5"	9' 8"	12' 5"	12' 9"	14' 0"	14' 0"
		#3	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STUD	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STANDARD	3' 9"	5' 2"	5' 2"	6' 10"	6' 10"	9' 2"	9' 2"	10' 7"	10' 7"	14' 0"	14' 0"
	SP DFL	#1	4' 3"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#2	4' 2"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#3	4' 0"	6' 2"	6' 2"	7' 11"	8' 2"	9' 5"	9' 11"	12' 5"	12' 8"	14' 0"	14' 0"
		STUD	4' 0"	6' 1"	6' 1"	7' 11"	8' 1"	9' 5"	9' 11"	12' 5"	12' 5"	14' 0"	14' 0"
		STANDARD	3' 10"	5' 3"	5' 3"	6' 11"	6' 11"	9' 4"	9' 4"	10' 10"	10' 10"	14' 0"	14' 0"
12" O.C.	SPF HF	#1 / #2	4' 3"	7' 4"	7' 7"	8' 9"	8' 11"	10' 5"	10' 8"	13' 8"	14' 0"	14' 0"	14' 0"
		#3	4' 2"	6' 11"	6' 11"	8' 9"	8' 9"	10' 5"	10' 5"	13' 8"	13' 8"	14' 0"	14' 0"
		STUD	4' 2"	6' 11"	6' 11"	8' 9"	8' 9"	10' 5"	10' 5"	13' 8"	13' 8"	14' 0"	14' 0"
		STANDARD	4' 2"	5' 11"	5' 11"	7' 10"	7' 10"	10' 5"	10' 5"	12' 3"	12' 3"	14' 0"	14' 0"
	SP DFL	#1	4' 8"	7' 4"	7' 11"	8' 9"	9' 5"	10' 5"	11' 2"	13' 8"	14' 0"	14' 0"	14' 0"
		#2	4' 7"	7' 4"	7' 11"	8' 9"	9' 5"	10' 5"	11' 2"	13' 8"	14' 0"	14' 0"	14' 0"
		#3	4' 4"	7' 2"	7' 2"	8' 9"	9' 2"	10' 5"	10' 11"	13' 8"	14' 0"	14' 0"	14' 0"
		STUD	4' 4"	7' 1"	7' 1"	8' 9"	9' 2"	10' 5"	10' 11"	13' 8"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 3"	6' 1"	6' 1"	8' 0"	8' 0"	10' 5"	10' 8"	12' 6"	12' 6"	14' 0"	14' 0"





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

BRACING GROUP SPECIES AND GRADES:

GROUP A:

SPRUCE-PINE-FIR		HEM-FIR	
#1 / #2	STANDARD	#2	STUD
#3	STUD	#3	STANDARD

DOUGLAS FIR-LARCH	SOUTHERN PINE
#3	#3
STUD	STUD
STANDARD	STANDARD

GROUP B:

HEM-FIR
#1 & BTR
#1

SOUTHERN PINE	DOUGLAS FIR-LARCH
#2	#1
#2	#2

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 136 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.
* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 8"	2X4
GREATER THAN 11' 8"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR, SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161
No: 34869
STATE OF FLORIDA

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF	ASCE7-02-GAB13015
DATE	11/26/03
DRWG	MITEK STD GABLE 15 E HT
-ENG	

ASCE 7-02: 130 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE * GROUP A	(1) 1X4 "L" BRACE * GROUP B	(1) 2X4 "L" BRACE * GROUP A	(1) 2X4 "L" BRACE * GROUP B	(2) 2X4 "L" BRACE ** GROUP A	(2) 2X4 "L" BRACE ** GROUP B	(1) 2X6 "L" BRACE * GROUP A	(1) 2X6 "L" BRACE * GROUP B	(2) 2X6 "L" BRACE ** GROUP A	(2) 2X6 "L" BRACE ** GROUP B
24" O.C.	SPF HF	#1 / #2	3' 2"	5' 6"	5' 8"	6' 6"	6' 9"	7' 10"	8' 0"	10' 3"	10' 7"	12' 3"	12' 7"
		#3	3' 1"	4' 5"	4' 5"	5' 10"	5' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"
		STUD	3' 1"	4' 5"	4' 5"	5' 10"	5' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"
		STANDARD	2' 11"	3' 9"	3' 9"	5' 0"	5' 0"	6' 9"	6' 9"	7' 10"	7' 10"	10' 7"	10' 7"
	SP DFL	#1	3' 6"	5' 6"	5' 11"	6' 8"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"
		#2	3' 5"	5' 6"	5' 11"	6' 6"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"
		#3	3' 3"	4' 6"	4' 6"	6' 0"	6' 0"	7' 10"	8' 1"	9' 4"	9' 4"	12' 3"	12' 5"
		STUD	3' 3"	4' 6"	4' 6"	5' 11"	5' 11"	7' 10"	8' 0"	9' 3"	9' 3"	12' 3"	12' 6"
		STANDARD	3' 0"	3' 10"	3' 10"	5' 1"	5' 1"	6' 11"	6' 11"	8' 0"	8' 0"	10' 10"	10' 10"
16" O.C.	SPF HF	#1 / #2	3' 8"	6' 4"	6' 6"	7' 6"	7' 8"	8' 11"	9' 2"	11' 9"	12' 1"	14' 0"	14' 0"
		#3	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 2"	11' 2"	14' 0"	14' 0"
		STUD	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 1"	11' 1"	14' 0"	14' 0"
		STANDARD	3' 7"	4' 8"	4' 8"	6' 2"	6' 2"	8' 3"	8' 3"	9' 7"	9' 7"	12' 11"	12' 11"
	SP DFL	#1	4' 0"	6' 4"	6' 10"	7' 6"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"
		#2	3' 11"	6' 4"	6' 10"	7' 6"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"
		#3	3' 9"	5' 7"	5' 7"	7' 4"	7' 4"	8' 11"	9' 5"	11' 5"	11' 5"	14' 0"	14' 0"
		STUD	3' 8"	5' 6"	5' 6"	7' 3"	7' 3"	8' 11"	9' 5"	11' 4"	11' 4"	14' 0"	14' 0"
		STANDARD	3' 8"	4' 9"	4' 9"	6' 3"	6' 3"	8' 5"	8' 5"	9' 9"	9' 9"	13' 3"	13' 3"
12" O.C.	SPF HF	#1 / #2	4' 0"	6' 11"	7' 2"	8' 3"	8' 6"	9' 10"	10' 1"	12' 11"	13' 4"	14' 0"	14' 0"
		#3	3' 11"	6' 3"	6' 3"	8' 3"	8' 3"	9' 10"	9' 10"	12' 11"	12' 11"	14' 0"	14' 0"
		STUD	3' 11"	6' 3"	6' 3"	8' 3"	8' 3"	9' 10"	9' 10"	12' 10"	12' 10"	14' 0"	14' 0"
		STANDARD	3' 11"	5' 4"	5' 4"	7' 1"	7' 1"	9' 6"	9' 6"	11' 1"	11' 1"	14' 0"	14' 0"
	SP DFL	#1	4' 5"	6' 11"	7' 6"	8' 3"	8' 11"	9' 10"	10' 7"	12' 11"	13' 11"	14' 0"	14' 0"
		#2	4' 4"	6' 11"	7' 6"	8' 3"	8' 11"	9' 10"	10' 7"	12' 11"	13' 11"	14' 0"	14' 0"
		#3	4' 2"	6' 5"	6' 5"	8' 3"	8' 6"	9' 10"	10' 4"	12' 11"	13' 3"	14' 0"	14' 0"
		STUD	4' 2"	6' 4"	6' 4"	8' 3"	8' 5"	9' 10"	10' 4"	12' 11"	13' 1"	14' 0"	14' 0"
		STANDARD	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"

BRACING GROUP SPECIES AND GRADES:

GROUP A:

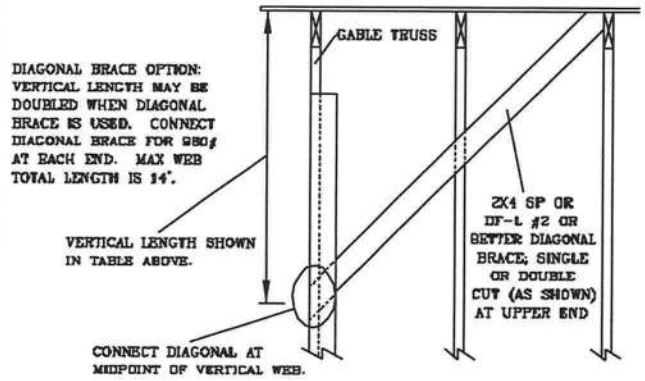
SPRUCE-PINE-FIR		HEM-FIR	
#1 / #2	STANDARD	#2	STUD
#3	STUD	#3	STANDARD

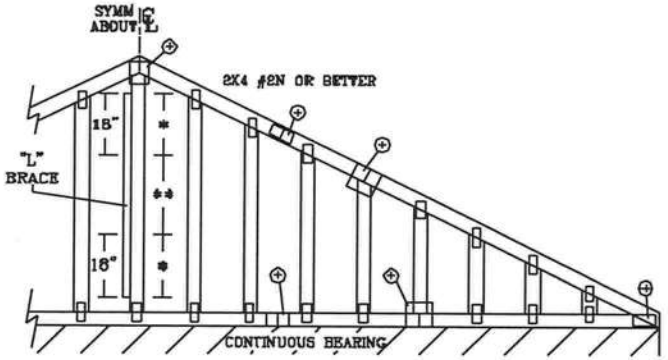
DOUGLAS FIR-LARCH	SOUTHERN PINE
#3	#3
STUD	STUD
STANDARD	STANDARD

GROUP B:

HEM-FIR
#1 & BTR
#1

SOUTHERN PINE	DOUGLAS FIR-LARCH
#3	#1
#2	#2





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 180 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.
* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4
GREATER THAN 11' 6"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1456 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2161

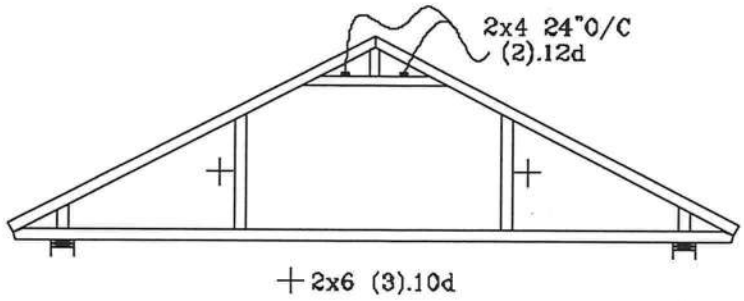
No: 34869
STATE OF FLORIDA

MAX. TOT. LD. 60 PSF

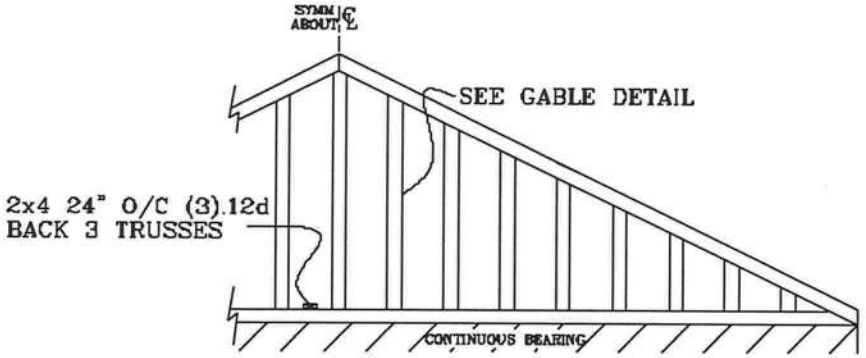
MAX. SPACING 24.0"

REF ASCE7-02-GAB13030
DATE 11/26/03
DWG MITEK STD GABLE 90' E H7
-ENG

TYPICAL ATTIC TRUSS BRACING

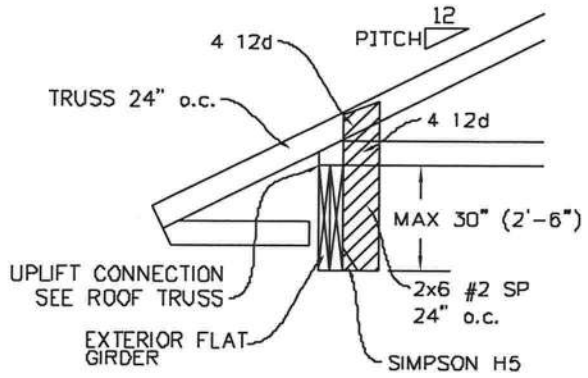


GABLE END TRUSS DETAIL

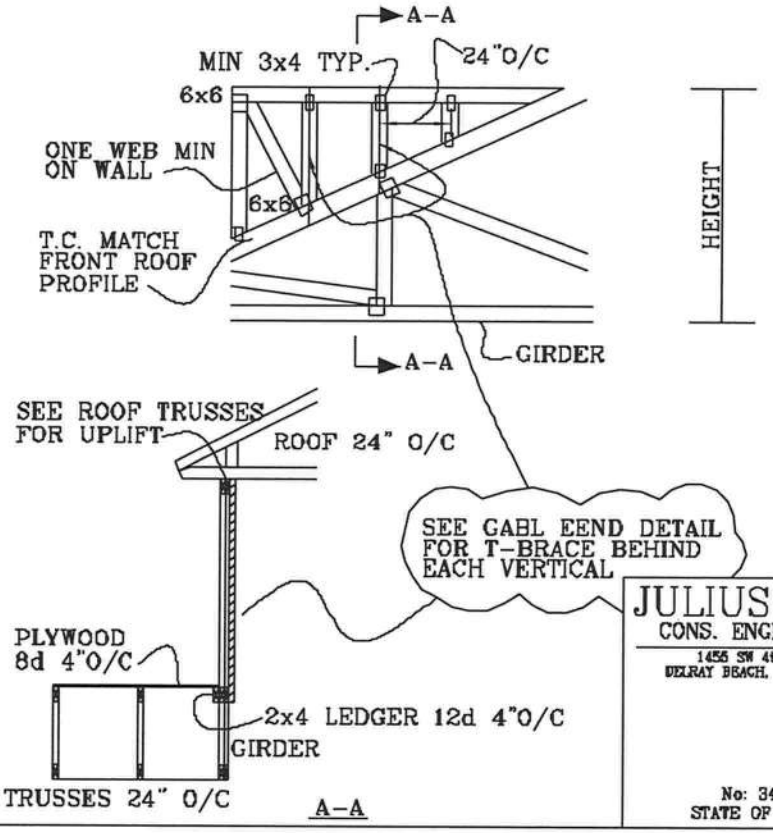


MINIMUM BC BRACING ON GABLE TRUSS. OTHER PERMANENT BRACING DESIGNS BY ARCHITECT OR EOR

TYPICAL ALTERNATE BRACING DETAIL FOR EXTERIOR FLAT GIRDER TRUSS



TYPICAL WALL GIRDER VERTICAL WEB BRACING DETAIL



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DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

PIGGYBACK DETAIL

TOP CHORD 2X4 #2 OR BETTER
BOT CHORD 2X4 #2 OR BETTER
WEBS 2X4 #3 OR BETTER

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

110 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST CAT I, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF

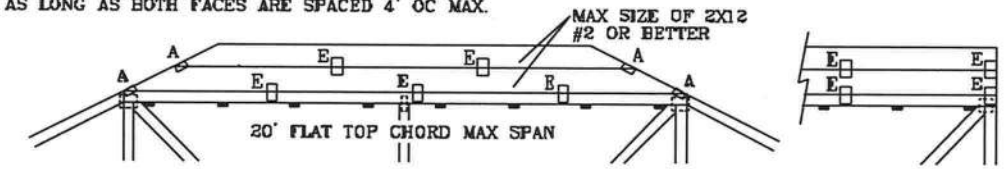
110 MPH WIND, 30' MEAN HGT, FBC ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TC DL=5 PSF, WIND BC DL=5 PSF

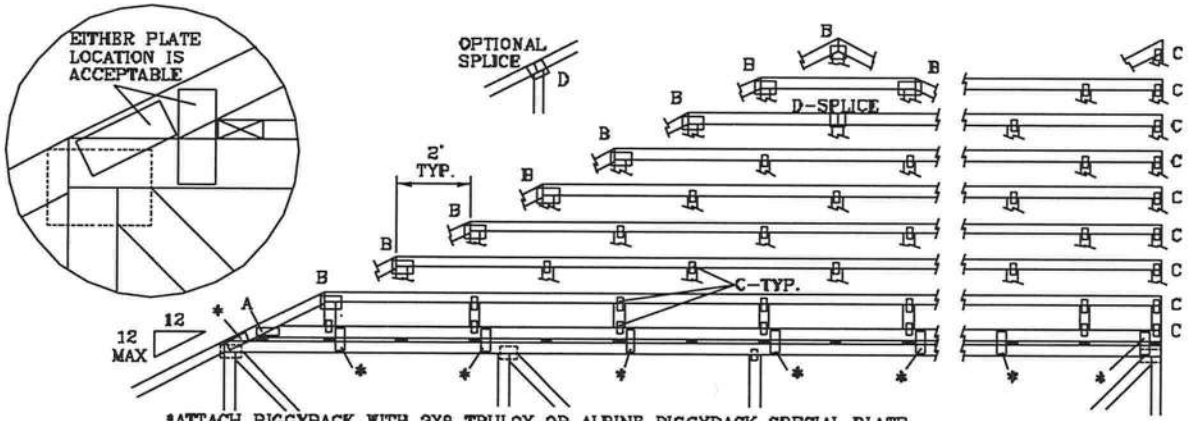
130 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=5 PSF, WIND BC DL=5 PSF

FRONT FACE (E,*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX.

JOINT TYPE	SPANS UP TO			
	30'	34'	38'	52'
A	2X4	2.5X4	2.5X4	3X5
B	4X6	5X6	5X6	5X6
C	1.5X3	1.5X4	1.5X4	1.5X4
D	5X4	5X5	5X5	5X6
E	4X6 OR 3X6 TRULOX AT 4' OC, ROTATED VERTICALLY			

ATTACH TRULOX PLATES WITH (8) 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRULOX INFORMATION.



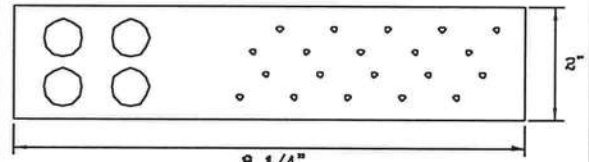


*ATTACH PIGGYBACK WITH 3X8 TRULOX OR ALPINE PIGGYBACK SPECIAL PLATE.

WEB BRACING CHART	
WEB LENGTH	REQUIRED BRACING
0' TO 7'9"	NO BRACING
7'9" TO 10'	1x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 8d NAILS AT 4" OC.
10' TO 14'	2x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d NAILS AT 4" OC.

* PIGGYBACK SPECIAL PLATE

ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS.



THIS DRAWING REPLACES DRAWINGS 634,016 634,017 & 847,045

xxWARNINGxx TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 537199 AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 537199 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2161

No: 34869
STATE OF FLORIDA

MAX LOADING
55 PSF AT 1.33 DUR. FAC.
50 PSF AT 1.25 DUR. FAC.
47 PSF AT 1.15 DUR. FAC.
SPACING 24.0"

REF PIGGYBACK
DATE 09/12/07
DRWG MITEK STD PIGGY
-ENG JL

VALLEY TRUSS DETAIL

TOP CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER.
BOT CHORD 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER.
WEBS 2X4 SP #3 OR BETTER.

* 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).

** ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH: (2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR FBC 2004 110 MPH, ASCE 7-02 110 MPH WIND OR (3) 16d FOR ASCE 7-02 130 MPH WIND. 15' MEAN HEIGHT, ENCLOSED BUILDING, EXP. C, RESIDENTIAL, WIND TC DL=5 PSF.

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".

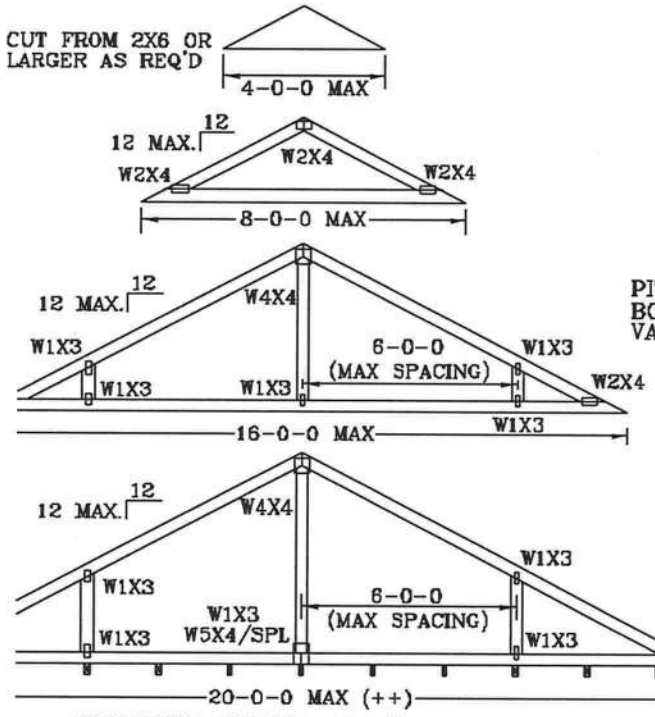
MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0".

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH:
PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS INSTALLATION
OR
PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN
OR
BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON ENGINEERS' SEALED DESIGN.

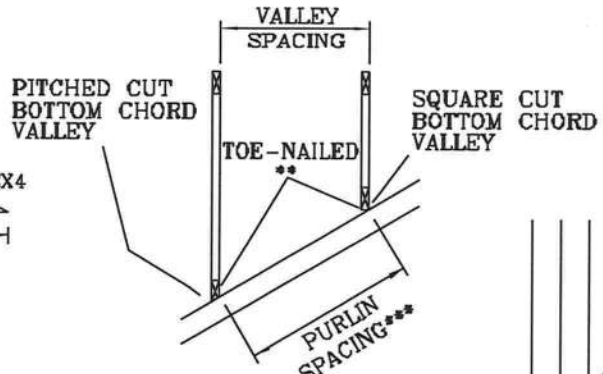
*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

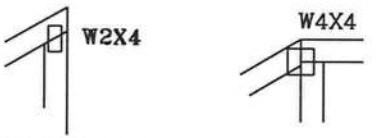
++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0".

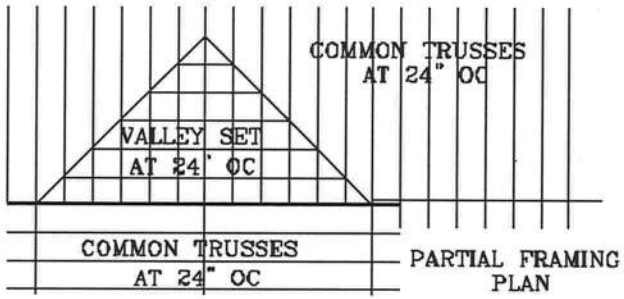
BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN.



SUPPORTING TRUSSES AT 24" OC MAXIMUM SPACING.







THIS DRAWING REPLACES DRAWING A105

xxWARNINGxx TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

TC LL	20	20	PSF	REF VALLEY DETAIL
TC DL	7	15	PSF	DATE 11/26/03
BC DL	5	5	PSF	DRWG VALTRUSS1103
BC LL	0	0	PSF	-ENG JL
TOT. LD.	32	40	PSF	
DUR.FAC.	1.25	1.25		
SPACING		24"		

TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AF&PA NDS-2001 SECTION 12.4.1 - EDGE DISTANCE, END DISTANCE, SPACING: "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

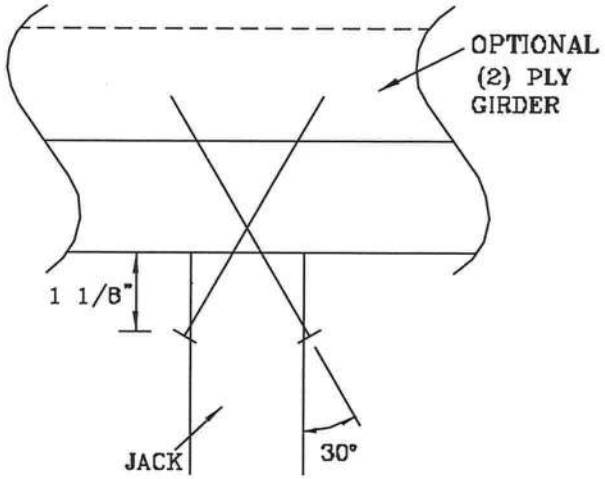
THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

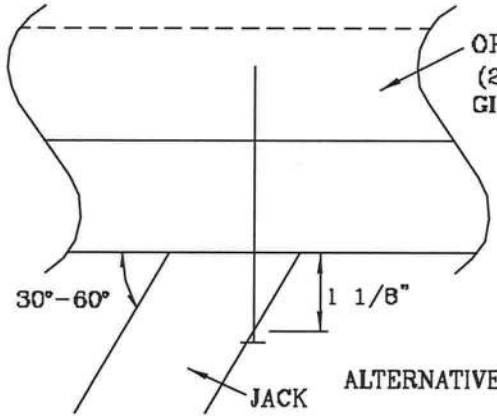
THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

MAXIMUM VERTICAL RESISTANCE OF 16d (0.162"X3.5") COMMON TOE-NAILS

NUMBER OF TOE-NAILS	SOUTHERN PINE		DOUGLAS FIR-LARCH		HEM-FIR		SPRUCE PINE FIR	
	1 PLY	2 PLIES	1 PLY	2 PLIES	1 PLY	2 PLIES	1 PLY	2 PLIES
2	197#	256#	181#	234#	156#	203#	154#	199#
3	296#	383#	271#	351#	234#	304#	230#	298#
4	394#	511#	361#	468#	312#	406#	307#	397#
5	493#	639#	452#	585#	390#	507#	384#	496#

ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR.





ALTERNATIVE CONDITION

THIS DRAWING REPLACES DRAWING 784040

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY (INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

TC LL	PSF	REF	TOE-NAIL
TC DL	PSF	DATE	09/12/07
BC DL	PSF	DRWG	CNTONAIL1103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		
DUR. FAC.	1.00		
SPACING			

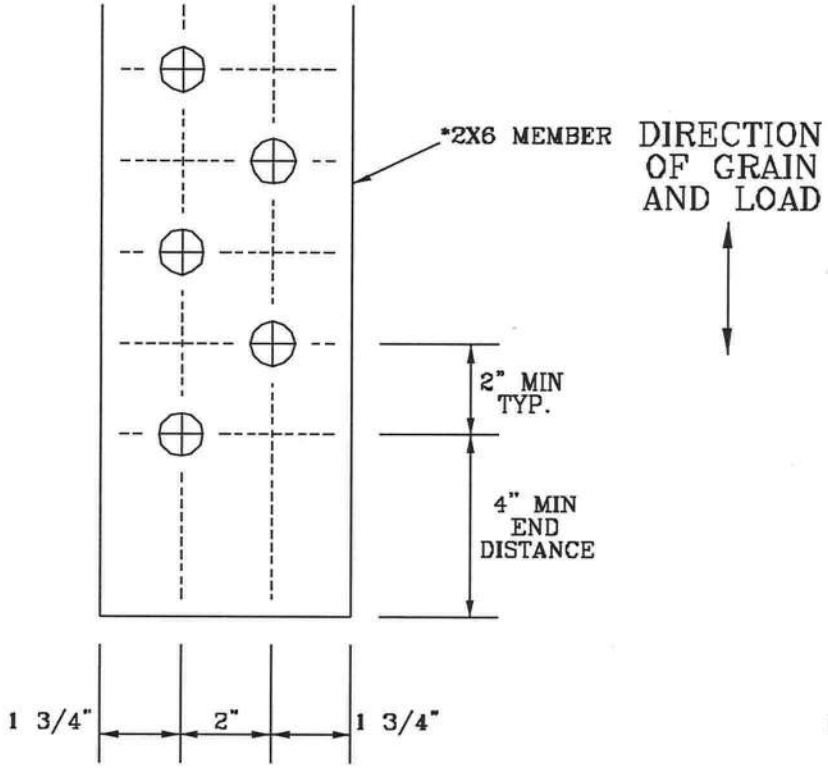
1/2" DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN.

* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN.

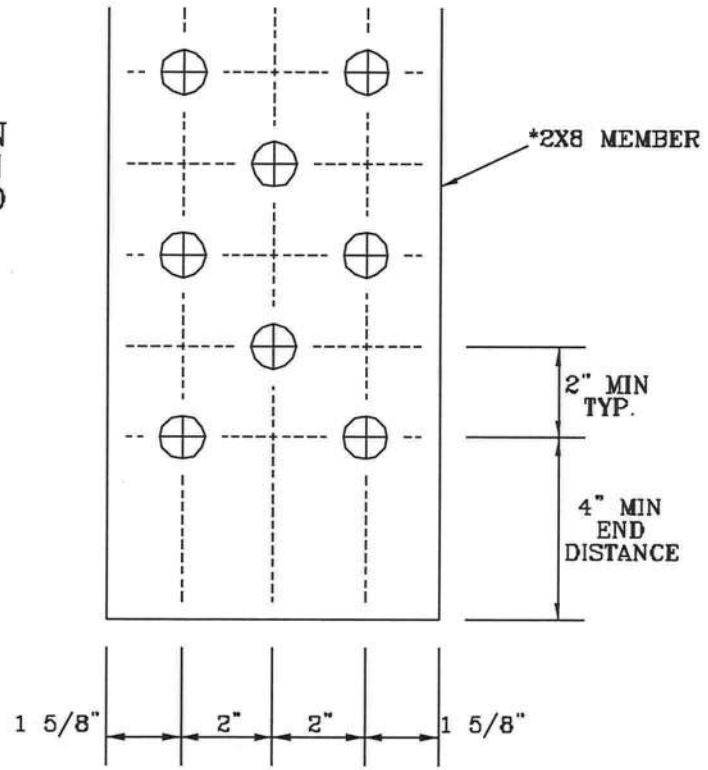
BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. BOLT QUANTITIES AS NOTED ON SEALED DESIGN MUST BE APPLIED IN ONE OF THE PATTERNS SHOWN BELOW.

WASHERS REQUIRED UNDER BOLT HEAD AND NUT



2X6 DETAIL



2X8 DETAIL

THIS DRAWING REPLACES DRAWING A628.016

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

TC LL	PSF	REF BOLT SPACING
TC DL	PSF	DATE 11/26/03
BC DL	PSF	DRWG CNBOLTSP1103
BC LL	PSF	-ENG JL
TOT. LD.	PSF	
DUR. FAC.		
SPACING		

TRULOX CONNECTION DETAIL

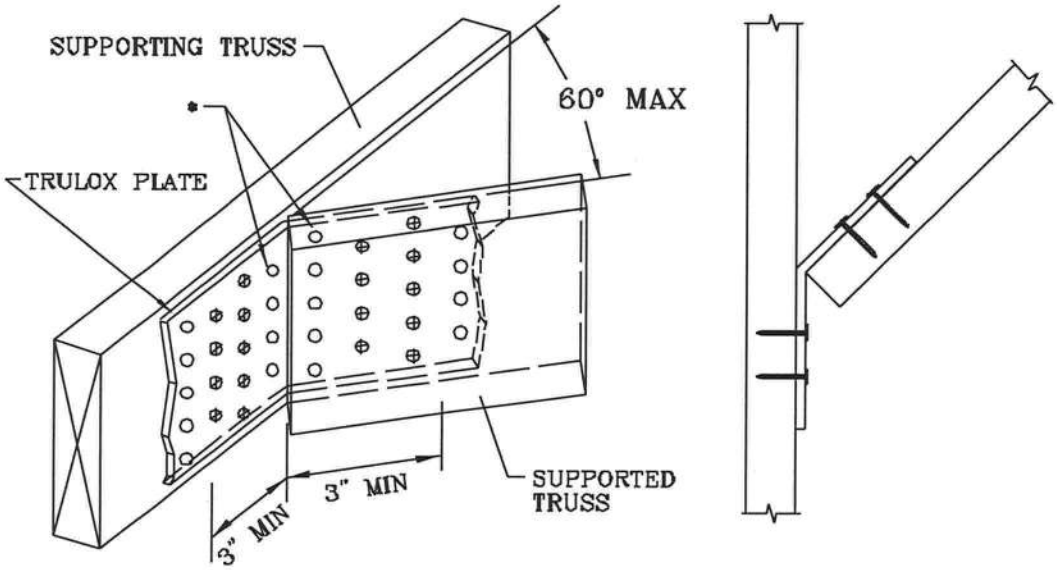
11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (⊕).

* NAILS MAY BE OMITTED FROM THESE ROWS.

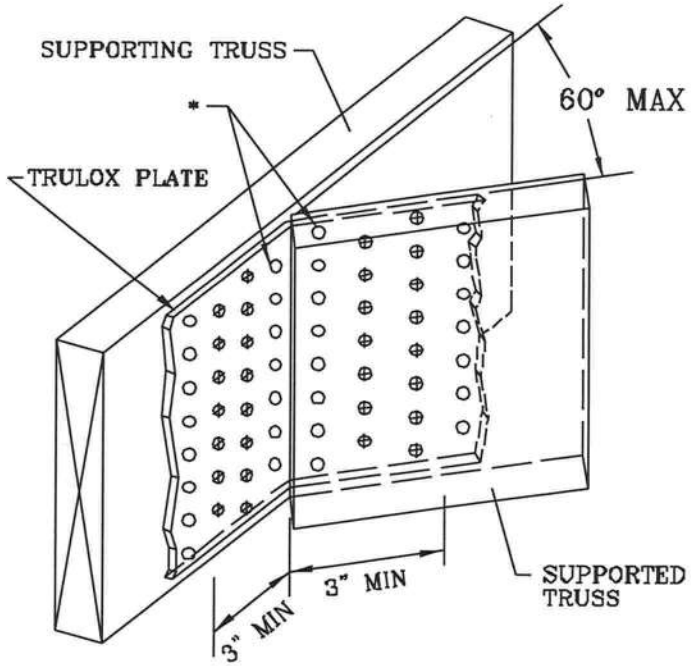
THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.



MINIMUM 3X6 TRULOX PLATE



MINIMUM 5X6 TRULOX PLATE

TRULOX PLATE SIZE	REQUIRED NAILS PER TRUSS	MAXIMUM LOAD UP OR DOWN
3X6	9	350#
5X6	15	990#

THIS DRAWING REPLACES DRAWINGS 1,158,989 1,158,989/R 1,154,944 1,152,217 1,152,017 1,159,154 & 1,151,524

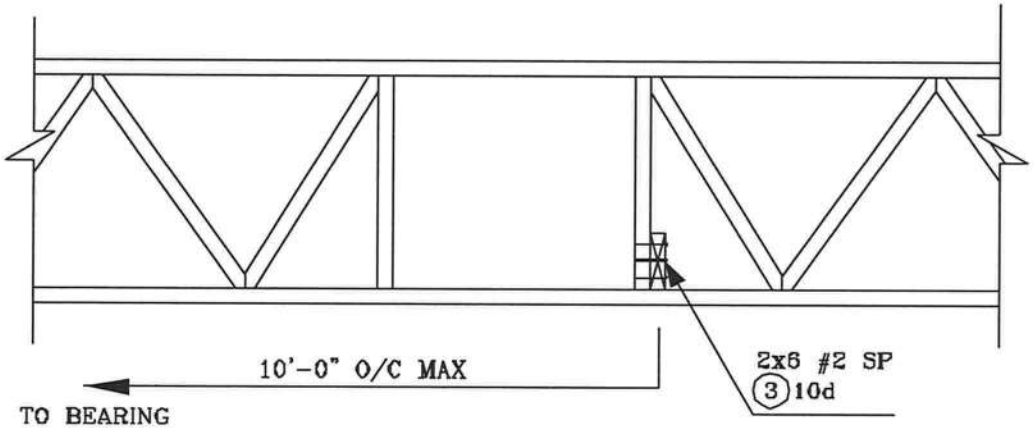
WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2351

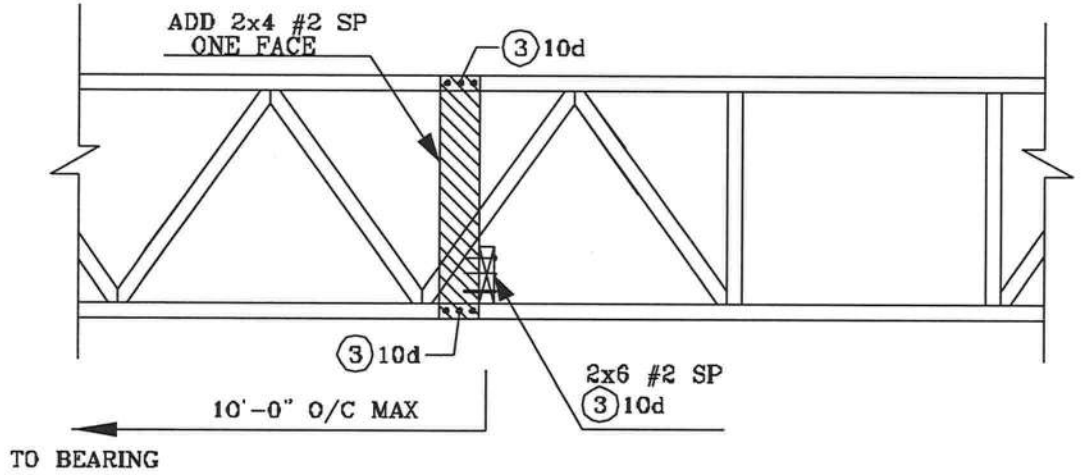
No: 34869
STATE OF FLORIDA

REF	TRULOX
DATE	11/26/03
DRWG	CNTRULOX1103
-ENG	JL

STRONG BACK DETAIL
SYSTEM–42 OR FLAT TRUSS



ALTERNATE DETAIL FOR
STRONG BACK WITH VERTICAL
NOT LINING UP



JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2161

No: 34869
STATE OF FLORIDA

Job	Truss	Truss Type	Qty	Ply	LOT 18
L264517	EJ7A	COMMON	1	1	Job Reference (optional)

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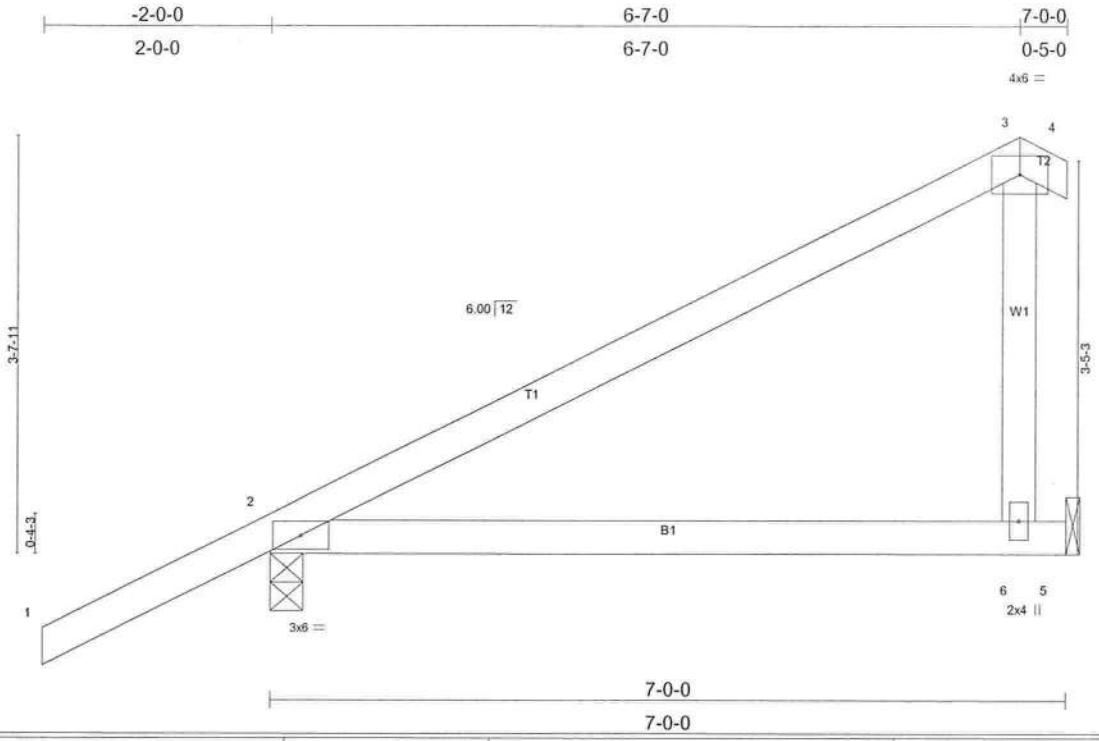


LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0		Plates Increase	1.25	TC	0.37	Vert(LL)	0.10	2-6	>812	360	MT20	244/190
TCDL 7.0		Lumber Increase	1.25	BC	0.29	Vert(TL)	-0.17	2-6	>473	240		
BCLL 10.0	*	Rep Stress Incr	YES	WB	0.05	Horz(TL)	0.00		n/a	n/a		
BCDL 5.0		Code FBC2004/TPI2002		(Matrix)							Weight: 30 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=351/0-3-8, 5=202/Mechanical
Max Horz 2=147(load case 6)
Max Uplift2=-146(load case 6), 5=-69(load case 6)

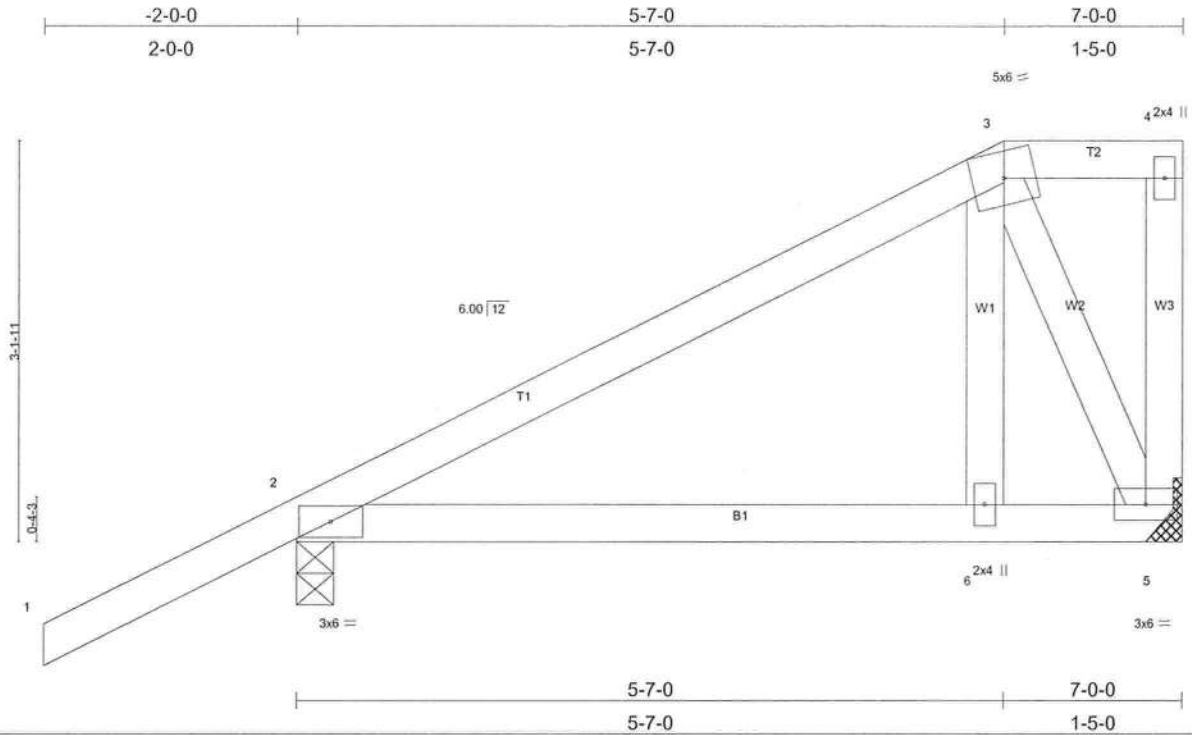
FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-95/52, 3-4=0/10
BOT CHORD 2-6=0/0, 5-6=0/0
WEBS 3-6=-171/217

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 146 lb uplift at joint 2 and 69 lb uplift at joint 5.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	LOT 18
L264517	EJ7B	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Dec 26 11:20:55 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.27	Vert(LL)	-0.02	2-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.16	Vert(TL)	-0.03	2-6	>999	240		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.11	Horz(TL)	0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 37 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=407/0-3-8, 5=450/Mechanical
Max Horz 2=140(load case 5)
Max Uplift2=-170(load case 5), 5=-151(load case 4)

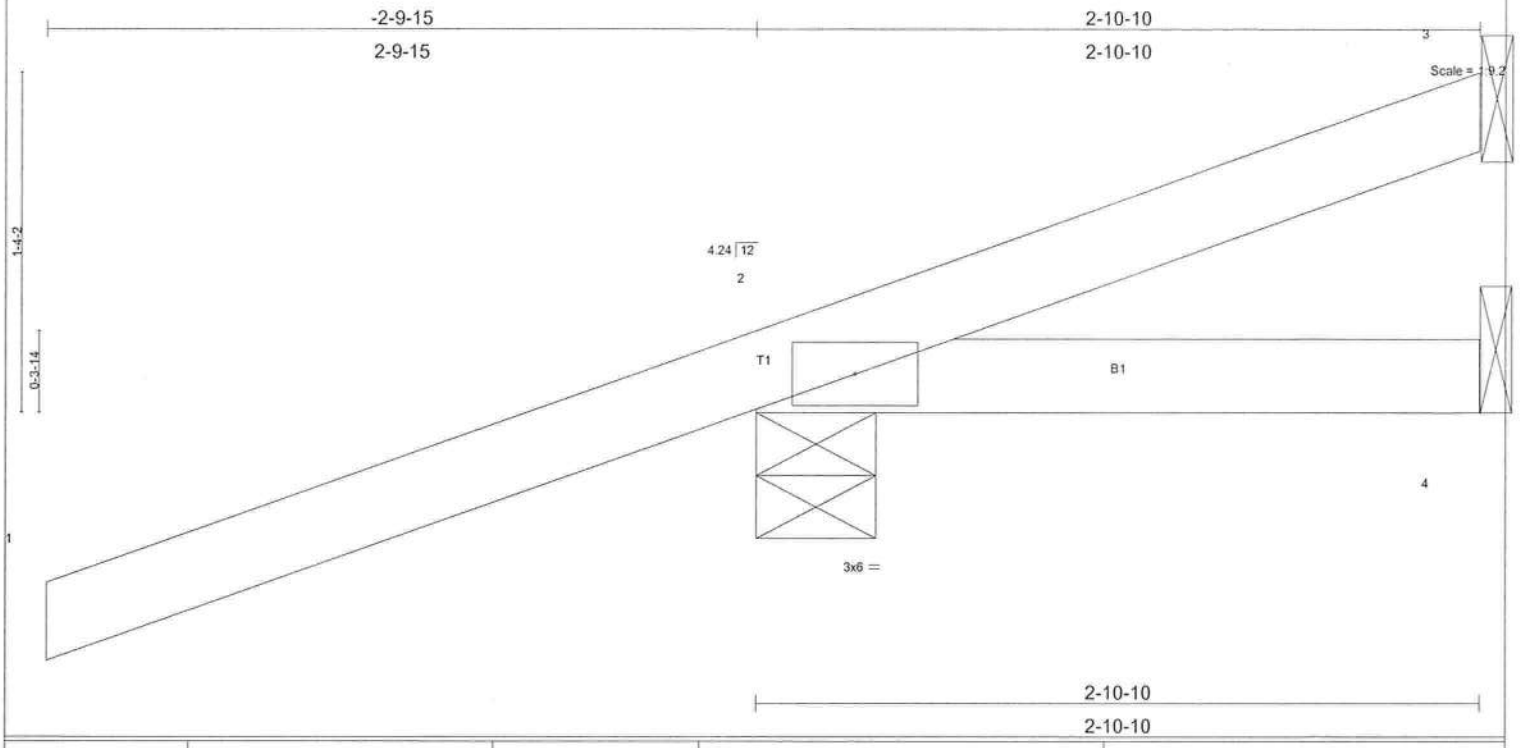
FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-325/55, 3-4=-1/0
BOT CHORD 2-6=-73/227, 5-6=-70/212
WEBS 3-6=-69/297, 3-5=-518/171, 4-5=-37/33

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Refer to girder(s) for truss to truss connections.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 170 lb uplift at joint 2 and 151 lb uplift at joint 5.
8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 243 lb down and 98 lb up at 5-7-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-4=-98(F=-44), 2-6=-10, 5-6=-18(F=-8)
Concentrated Loads (lb)
Vert: 6=-243(F)

Job	Truss	Truss Type	Qty	Ply	LOT 18
L264517	HJ3	MONO TRUSS	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Dec 26 11:20:55 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.54	Vert(LL)	-0.00	2-4	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.05	Vert(TL)	-0.00	2-4	>999	240		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 13 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-10-10 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=-28/Mechanical, 2=346/0-5-11, 4=13/Mechanical
Max Horz 2=110(load case 3)
Max Uplift3=-28(load case 1), 2=-318(load case 3)
Max Grav3=56(load case 3), 2=346(load case 1), 4=39(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/50, 2-3=-62/20
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 3 and 318 lb uplift at joint 2.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	LOT 18
L264517	HJ7	MONO TRUSS	1	1	Job Reference (optional)

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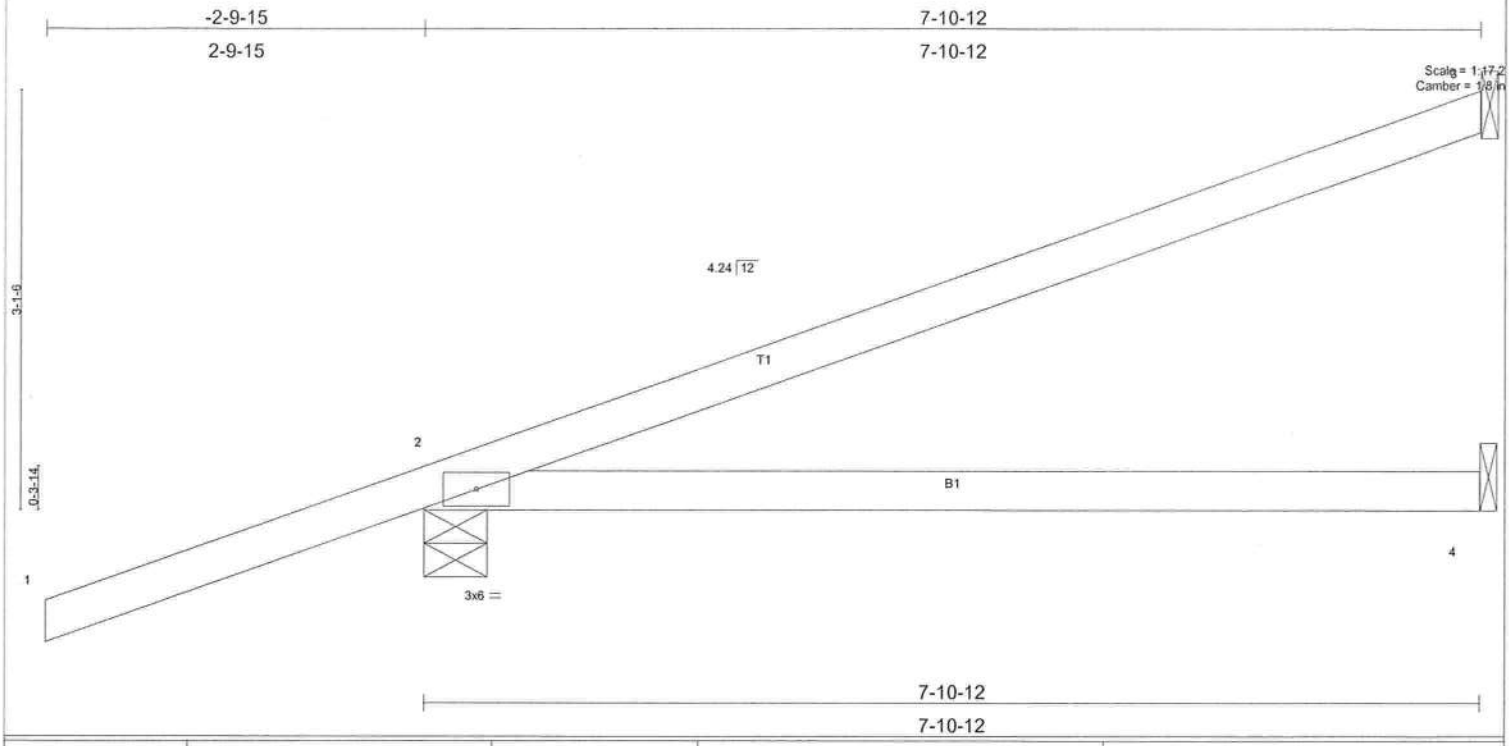

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0		Plates Increase	1.25	TC	0.62	Vert(LL)	-0.12	2-4	>733	360	MT20	244/190
TCDL 7.0		Lumber Increase	1.25	BC	0.37	Vert(TL)	-0.24	2-4	>386	240		
BCLL 10.0	*	Rep Stress Incr	NO	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL 5.0		Code FBC2004/TPI2002		(Matrix)							Weight: 29 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 7-10-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=237/Mechanical, 2=367/0-5-11, 4=54/Mechanical
Max Horz 2=194(load case 3)
Max Uplift3=-190(load case 3), 2=-257(load case 3)
Max Grav3=237(load case 1), 2=367(load case 1), 4=115(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/50, 2-3=-88/55
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 190 lb uplift at joint 3 and 257 lb uplift at joint 2.
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=25, B=25)-to-3=-107(F=-26, B=-26), 2=-0(F=5, B=5)-to-4=-20(F=-5, B=-5)

Job	Truss	Truss Type	Qty	Ply	LOT 18
L264517	HJ9	MONO TRUSS	6	1	Job Reference (optional)

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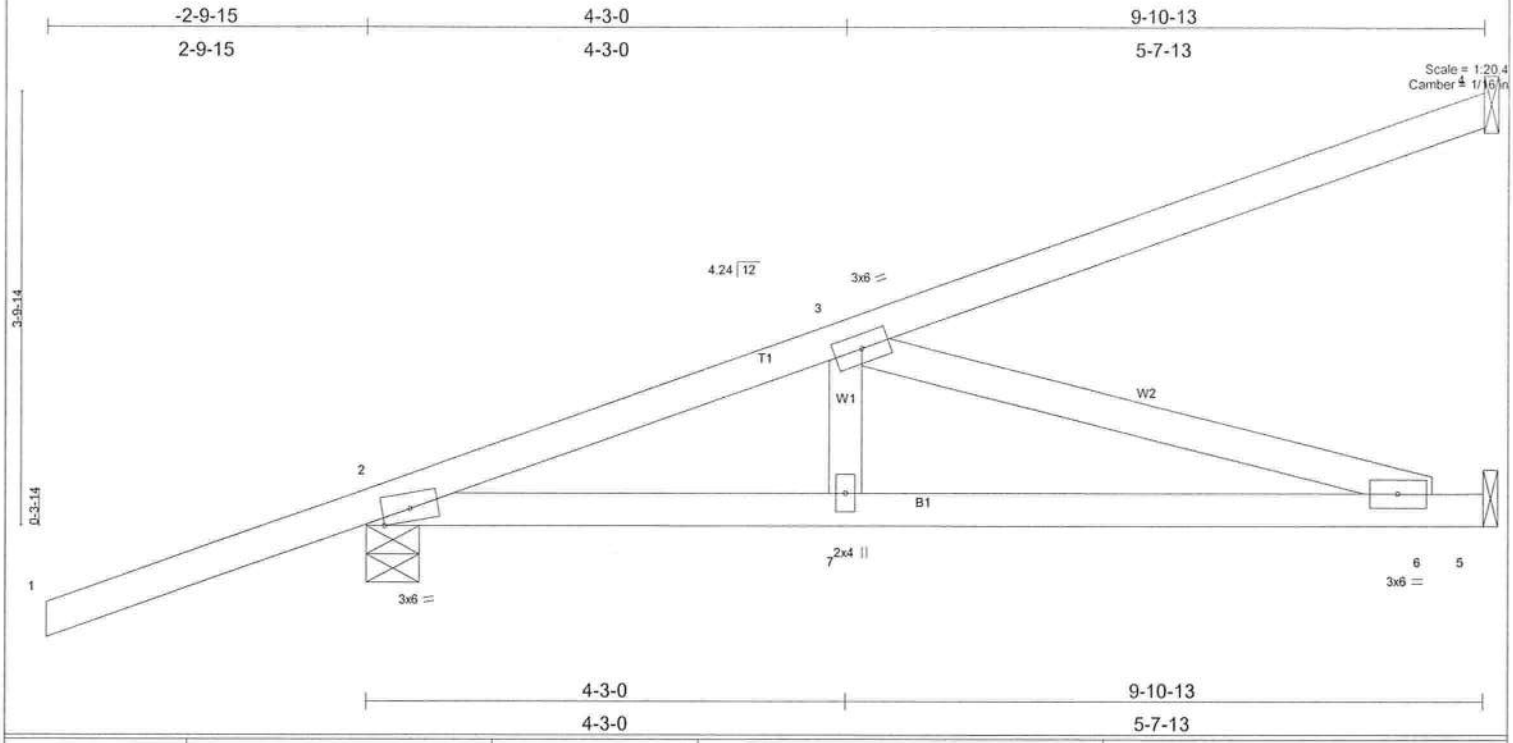

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0		Plates Increase	1.25	TC	0.61	Vert(LL)	0.10	6-7	>999	360	MT20	244/190
TCDL 7.0		Lumber Increase	1.25	BC	0.40	Vert(TL)	-0.12	6-7	>984	240		
BCLL 10.0	*	Rep Stress Incr	NO	WB	0.34	Horz(TL)	0.01	5	n/a	n/a		
BCDL 5.0		Code FBC2004/TPI2002		(Matrix)							Weight: 45 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-11-9 oc bracing.

REACTIONS (lb/size) 4=268/Mechanical, 2=456/0-5-11, 5=218/Mechanical
Max Horz 2=269(load case 3)
Max Uplift4=-233(load case 3), 2=-401(load case 3), 5=-181(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/50, 2-3=-647/363, 3-4=-105/65
BOT CHORD 2-7=-535/599, 6-7=-535/599, 5-6=0/0
WEBS 3-7=-94/190, 3-6=-624/558

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 233 lb uplift at joint 4, 401 lb uplift at joint 2 and 181 lb uplift at joint 5.
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=-0(F=5, B=5)-to-5=-25(F=-7, B=-7)

Job	Truss	Truss Type	Qty	Ply	LOT 18
L264517	T01	SPECIAL	1	2	Job Reference (optional)

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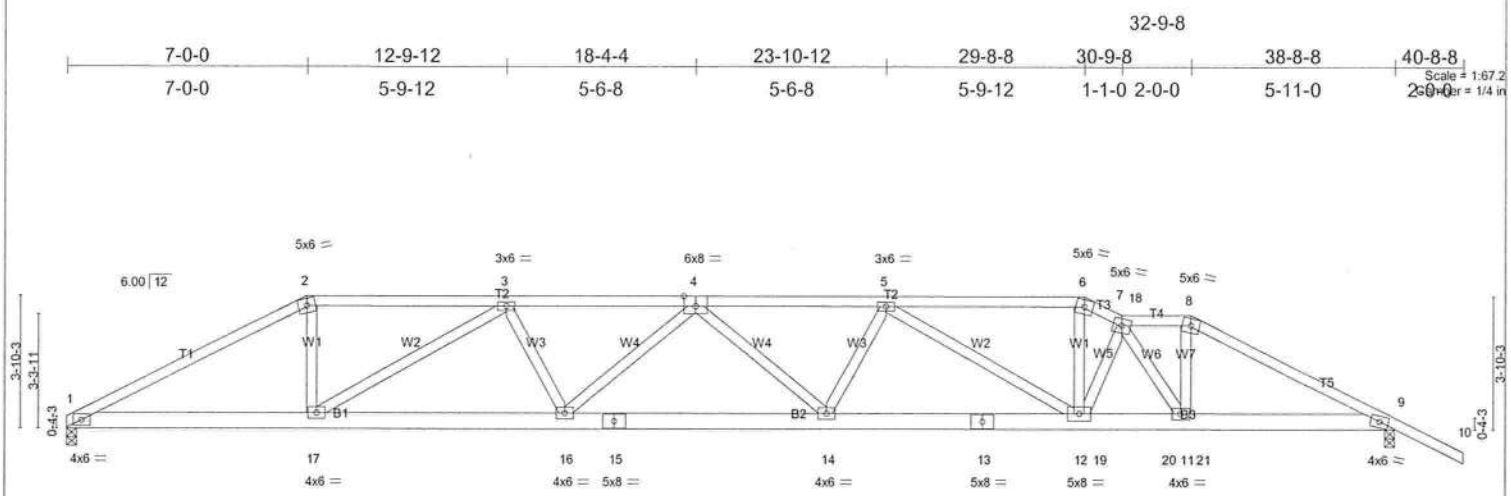

Plate Offsets (X,Y): [4:0-4-0,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.42	Vert(LL)	-0.30	14-16	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.45	Vert(TL)	-0.57	14-16	>803	240		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.46	Horz(TL)	0.12	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 440 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-8-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=2560/0-3-8, 9=2766/0-3-8
Max Horz 1=-104(load case 6)
Max Uplift1=-793(load case 4), 9=-803(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-5345/1716, 2-3=-4782/1587, 3-4=-7187/2350, 4-5=-7467/2394, 5-6=-5653/1782, 6-18=-5980/1858, 7-18=-6013/1855, 7-8=-4883/1486, 8-9=-5429/1605, 9-10=0/51
BOT CHORD 1-17=-1489/4704, 16-17=-2231/6881, 15-16=-2477/7681, 14-15=-2477/7681, 13-14=-2299/7293, 12-13=-2299/7293, 12-19=-1708/5846, 19-20=-1708/5846, 11-20=-1708/5846, 11-21=-1371/4782, 9-21=-1371/4782
WEBS 2-17=-551/1849, 3-17=-2559/897, 3-16=-114/714, 4-16=-694/297, 4-14=-345/215, 5-14=-43/435, 5-12=-1999/738, 6-12=-456/1854, 7-12=-702/174, 7-11=-1806/629, 8-11=-626/2031

NOTES
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.
6) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 793 lb uplift at joint 1 and 803 lb uplift at joint 9.
9) Girder carries tie-in span(s): 7-0-0 from 30-1-8 to 32-1-8
10) Girder carries hip end with 8-7-0 right side setback, 7-0-0 left side setback, and 7-0-0 end setback.
11) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 411 lb down and 165 lb up at 7-0-0, and 450 lb down and 124 lb up at 33-1-8 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 2-6=-118(F=-64), 6-18=-118(F=-64), 7-18=-54, 7-8=-54, 8-10=-54, 1-17=-10, 17-19=-22(F=-12), 19-20=-85(F=-75), 9-20=-10
Concentrated Loads (lb)
Vert: 17=-411(F) 21=-450(F)

Job	Truss	Truss Type	Qty	Ply	LOT 18
L264517	T02	SPECIAL	1	1	Job Reference (optional)

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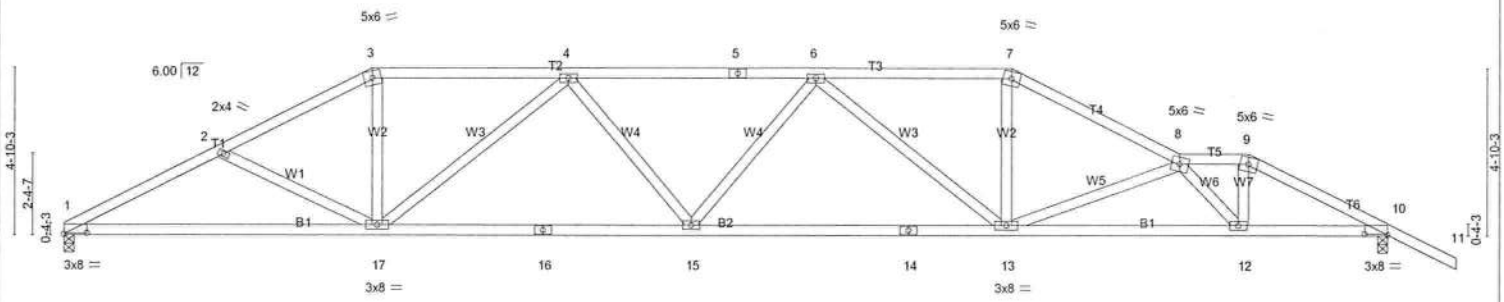
4-7-15	9-0-0	14-8-13	21-11-11	27-8-8	32-8-0	34-8-0	38-8-8	40-8-8
4-7-15	4-4-1	5-8-13	7-2-14	5-8-13	4-11-8	2-0-0	4-0-8	2-0-0

Scale = 1:67.3
Camber = 1/4 in



9-0-0	18-4-4	27-8-8	34-8-0	38-8-8
9-0-0	9-4-4	9-4-4	6-11-8	4-0-8

Plate Offsets (X,Y): [1:0-8-0,0-0-6], [10:0-8-0,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	0.28	13-15	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.60	Vert(TL)	-0.52	13-15	>888	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.76	Horz(TL)	0.16	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 194 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-7-4 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-4-2 oc bracing.

REACTIONS (lb/size) 1=1226/0-3-8, 10=1348/0-3-8
Max Horz 1=-102(load case 7)
Max Uplift1=-237(load case 5), 10=-314(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2376/1296, 2-3=-2145/1165, 3-4=-1893/1103, 4-5=-2551/1420, 5-6=-2551/1420, 6-7=-2142/1224, 7-8=-2405/1290, 8-9=-2095/1096, 9-10=-2354/1166, 10-11=0/47
BOT CHORD 1-17=-1005/2071, 16-17=-1111/2468, 15-16=-1111/2468, 14-15=-1155/2561, 13-14=-1155/2561, 12-13=-1392/2907, 10-12=-880/2029
WEBS 2-17=-227/248, 3-17=-318/668, 4-17=-819/410, 4-15=0/229, 6-15=-68/126, 6-13=-652/311, 7-13=-350/754, 8-13=-851/527, 8-12=-1215/717, 9-12=-473/937

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 3x6 MT20 unless otherwise indicated.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 237 lb uplift at joint 1 and 314 lb uplift at joint 10.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	LOT 18
L264517	T03	SPECIAL	1	1	Job Reference (optional)

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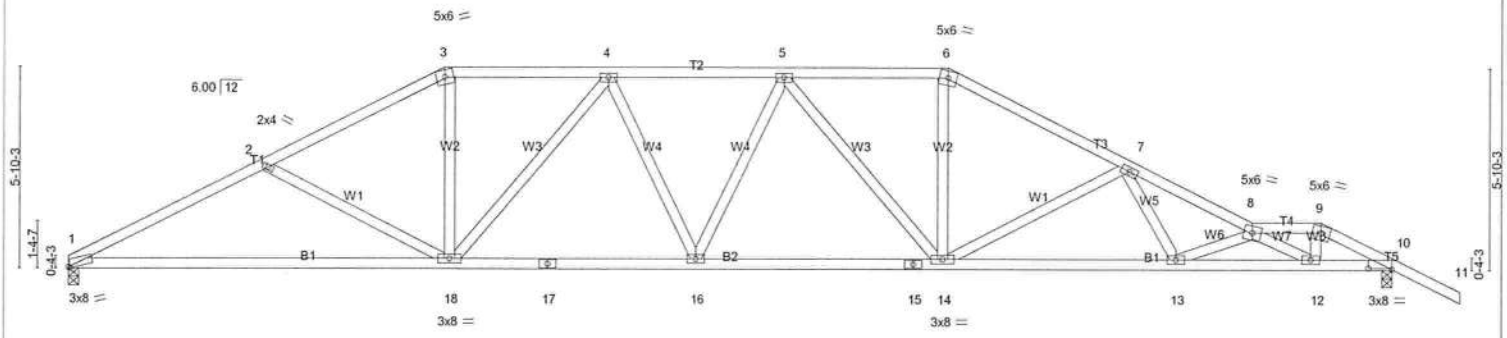
5-9-14	11-0-0	15-9-6	20-11-2	25-8-8	30-10-10	34-8-0	36-8-0	38-8-8	40-8-8
5-9-14	5-2-2	4-9-6	5-1-13	4-9-6	5-2-2	3-9-6	2-0-0	2-0-8	2-0-0

Scale = 1:67.3
Camber = 5/16 in



11-0-0	18-4-4	25-8-8	32-5-0	36-8-0	38-8-8
11-0-0	7-4-4	7-4-4	6-8-8	4-3-0	2-0-8

Plate Offsets (X,Y): [1:0-0-10,Edge], [10:0-8-0,0-0-6]

LOADING (psf)		SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES GRIP
TCLL 20.0		Plates Increase 1.25	TC 0.46	Vert(LL) -0.33 1-18 >999 360	MT20 244/190
TCDL 7.0		Lumber Increase 1.25	BC 0.85	Vert(TL) -0.66 1-18 >703 240	
BCLL 10.0	*	Rep Stress Incr NO	WB 0.51	Horz(TL) 0.15 10 n/a n/a	
BCDL 5.0		Code FBC2004/TPI2002	(Matrix)		Weight: 206 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-3-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-7-0 oc bracing.

REACTIONS (lb/size) 1=1227/0-3-8, 10=1360/0-3-8
Max Horz 1=-114(load case 7)
Max Uplift1=-218(load case 6), 10=-337(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2338/1298, 2-3=-2029/1131, 3-4=-1769/1075, 4-5=-2100/1242, 5-6=-1923/1157, 6-7=-2194/1218, 7-8=-3189/1691, 8-9=-2065/1010, 9-10=-2254/1083, 10-11=0/47
BOT CHORD 1-18=-994/2037, 17-18=-877/2061, 16-17=-877/2061, 15-16=-905/2114, 14-15=-905/2114, 13-14=-1237/2587, 12-13=-1880/3832, 10-12=-810/1931
WEBS 2-18=-325/324, 3-18=-293/609, 4-18=-555/253, 4-16=-20/160, 5-16=-95/64, 5-14=-429/170, 6-14=-324/669, 7-14=-770/507, 7-13=-249/590, 8-13=-1096/593, 8-12=-2039/1158, 9-12=-494/984

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 3x6 MT20 unless otherwise indicated.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 218 lb uplift at joint 1 and 337 lb uplift at joint 10.
8) Girder carries hip end with 2-0-8 right side setback, 34-8-0 left side setback, and 2-0-8 end setback.
9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 12 lb down and 9 lb up at 36-8-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-6=-54, 6-8=-54, 8-9=-54, 9-11=-54, 1-10=-10
Concentrated Loads (lb)
Vert: 12=-12(B)

Job	Truss	Truss Type	Qty	Ply	LOT 18
L264517	T04	SPECIAL	1	1	Job Reference (optional)

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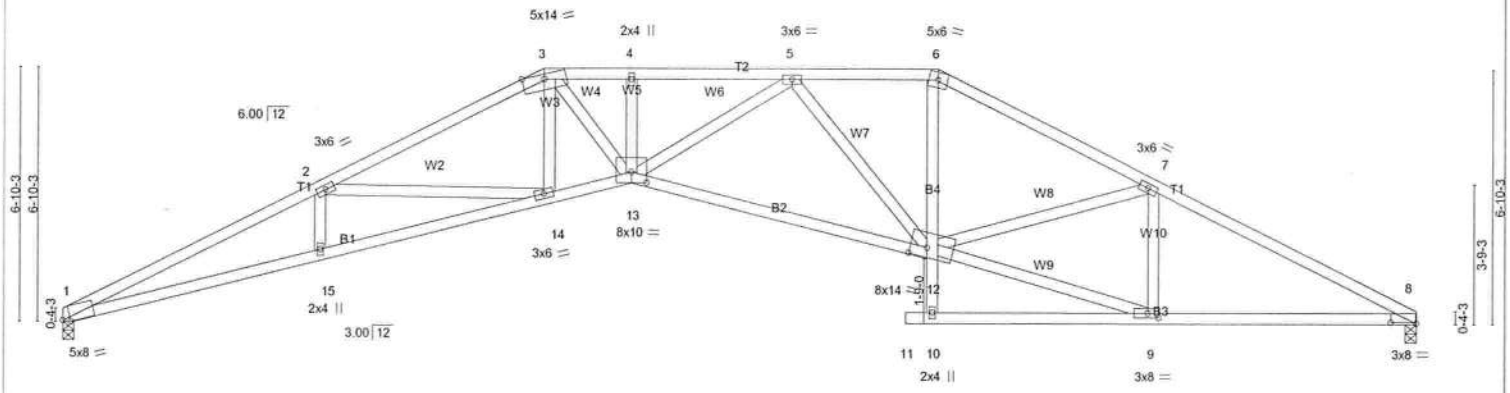
6-11-4	13-0-0	15-4-4	19-8-10	23-8-8	29-6-13	36-8-8
6-11-4	6-0-12	2-4-4	4-4-6	3-11-14	5-10-5	7-1-11

Scale = 1:62.4
Camber = 7/16 in



6-11-4	13-0-0	15-4-4	22-10-0	23-5-0	29-11-4	36-8-8
6-11-4	6-0-12	2-4-4	7-5-12	0-7-0	6-6-4	6-9-4

Plate Offsets (X,Y): [1:0-2-6,Edge], [8:0-8-4,0-0-6], [9:0-3-8,0-1-8], [12:0-5-8,0-3-2], [13:0-5-0,0-3-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.61	Vert(LL)	0.50	13	>878	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.83	Vert(TL)	-0.84	12-13	>523	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.73	Horz(TL)	0.50	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 189 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B4 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-6-11 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-4-13 oc bracing.

REACTIONS (lb/size) 1=1168/0-3-8, 8=1170/0-3-8
Max Horz 1=85(load case 5)
Max Uplift1=-224(load case 6), 8=-222(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-4123/2169, 2-3=-3345/1715, 3-4=-3724/1929, 4-5=-3723/1929, 5-6=-2072/1210, 6-7=-2357/1282, 7-8=-2225/1197
BOT CHORD 1-15=-1891/3705, 14-15=-1889/3704, 13-14=-1306/3025, 12-13=-1247/2839, 10-12=0/102, 6-12=-374/762, 10-11=0/0, 9-10=-21/29, 8-9=-965/1906
WEBS 2-15=0/208, 2-14=-683/566, 3-14=-160/250, 3-13=-504/1222, 4-13=-156/81, 5-13=-462/1154, 5-12=-1086/527, 9-12=-983/1956, 7-9=-460/296, 7-12=-77/290

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 224 lb uplift at joint 1 and 222 lb uplift at joint 8.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	LOT 18
L264517	T05	SPECIAL	1	1	Job Reference (optional)

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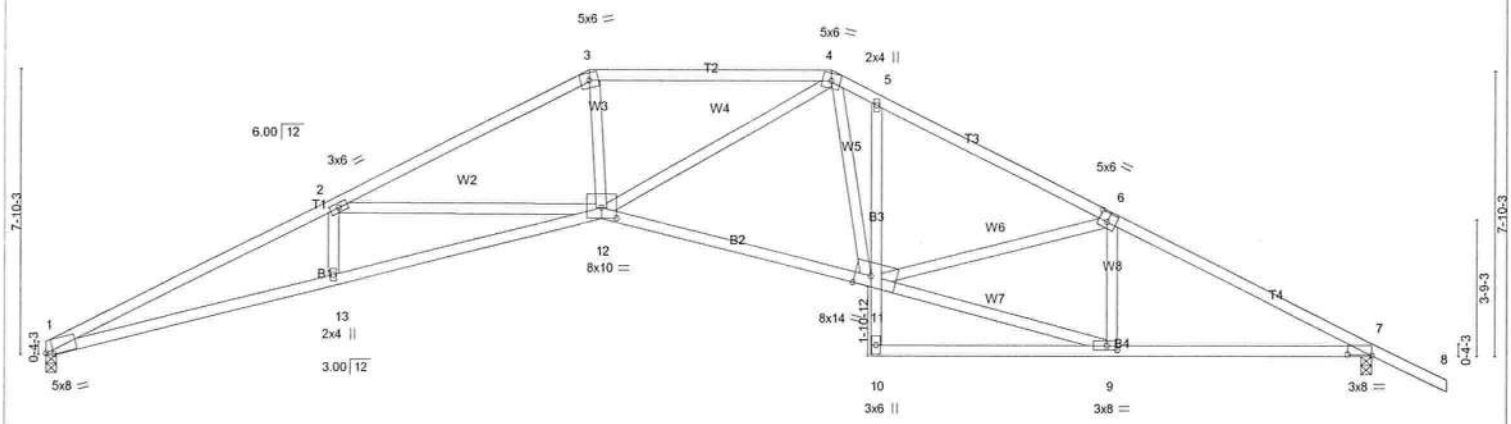
7-10-15	15-0-0	21-8-8	22-10-0	29-6-0	36-8-8	38-8-8
7-10-15	7-1-1	6-8-8	1-1-8	6-8-0	7-2-8	2-0-0

Scale: 3/16"=1'
Camber = 3/8 in



7-10-15	15-4-4	22-10-0	29-6-0	36-8-8
7-10-15	7-5-5	7-5-12	6-8-0	7-2-8

Plate Offsets (X,Y): [1:0-2-7,Edge], [6:0-3-0,0-3-0], [7:0-8-0,0-0-6], [9:0-3-8,0-1-8], [11:0-5-8,Edge], [12:0-5-0,0-3-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.85	Vert(LL)	0.42	12-13	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.88	Vert(TL)	-0.74	12-13	>592	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.86	Horz(TL)	0.44	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 191 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B3 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied or 4-6-3 oc bracing.

REACTIONS (lb/size) 1=1162/0-3-8, 7=1285/0-3-8
Max Horz 1=-137(load case 7)
Max Uplift1=-237(load case 6), 7=-331(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-4060/2070, 2-3=-3083/1485, 3-4=-2769/1436, 4-5=-2216/1319, 5-6=-2309/1239, 6-7=-2179/1159, 7-8=0/47
BOT CHORD 1-13=-1711/3647, 12-13=-1707/3643, 11-12=-697/1998, 10-11=0/90, 5-11=-151/194, 9-10=-14/34, 7-9=-845/1862
WEBS 2-13=0/237, 2-12=-869/712, 3-12=-375/967, 4-12=-352/976, 4-11=-273/289, 9-11=-873/1915, 6-11=-57/242, 6-9=-460/292

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 237 lb uplift at joint 1 and 331 lb uplift at joint 7.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	LOT 18
L264517	T06	SPECIAL	1	1	Job Reference (optional)

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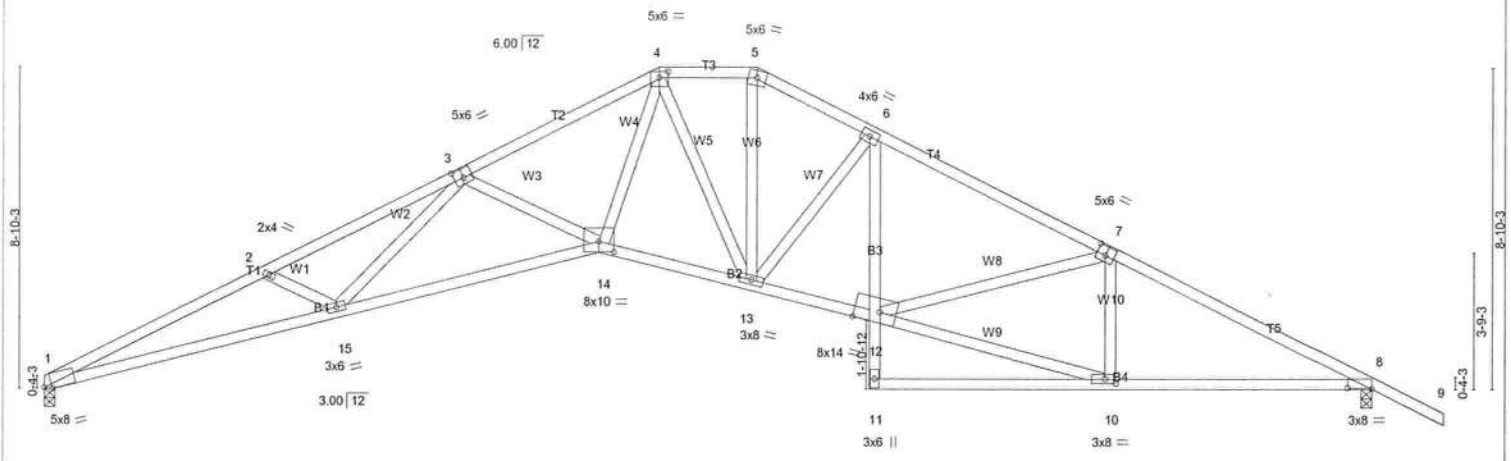
6-2-10	11-6-15	17-0-0	19-8-8	22-10-0	29-6-0	36-8-8	38-8-8
6-2-10	5-4-5	5-5-1	2-8-8	3-1-8	6-8-0	7-2-8	2-0-0

Scale: 3/16"=1'
Camber = 3/8 in



8-1-1	15-4-4	17-0-0	19-8-8	22-10-0	29-6-0	36-8-8
8-1-1	7-3-3	1-7-12	2-8-8	3-1-8	6-8-0	7-2-8

Plate Offsets (X,Y): [1:0-2-7,Edge], [3:0-3-0,0-3-0], [4:0-3-0,0-2-0], [7:0-3-0,0-3-0], [8:0-8-0,0-0-6], [10:0-3-8,0-1-8], [12:0-8-9,Edge], [14:0-5-0,0-3-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.46	Vert(LL)	0.43	14-15	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.81	Vert(TL)	-0.75	14-15	>586	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.71	Horz(TL)	0.43	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 203 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B3 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-8-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-6-7 oc bracing.

REACTIONS (lb/size) 1=1162/0-3-8, 8=1285/0-3-8
Max Horz 1=-149(load case 7)
Max Uplift1=-248(load case 6), 8=-342(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-4104/2173, 2-3=-3820/2026, 3-4=-2916/1484, 4-5=-1810/1083, 5-6=-2052/1171, 6-7=-2284/1256, 7-8=-2178/1174, 8-9=0/47
BOT CHORD 1-15=-1822/3694, 14-15=-1344/3139, 13-14=-677/2106, 12-13=-780/2033, 11-12=0/93, 6-12=-49/125, 10-11=-17/77, 8-10=-859/1861
WEBS 2-15=-260/296, 3-15=-301/495, 3-14=-567/484, 4-14=-697/1641, 4-13=-613/216, 5-13=-456/781, 6-13=-320/305, 10-12=-882/1877, 7-12=-36/192, 7-10=-442/291

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 248 lb uplift at joint 1 and 342 lb uplift at joint 8.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	LOT 18
L264517	T07	SPECIAL	3	1	Job Reference (optional)

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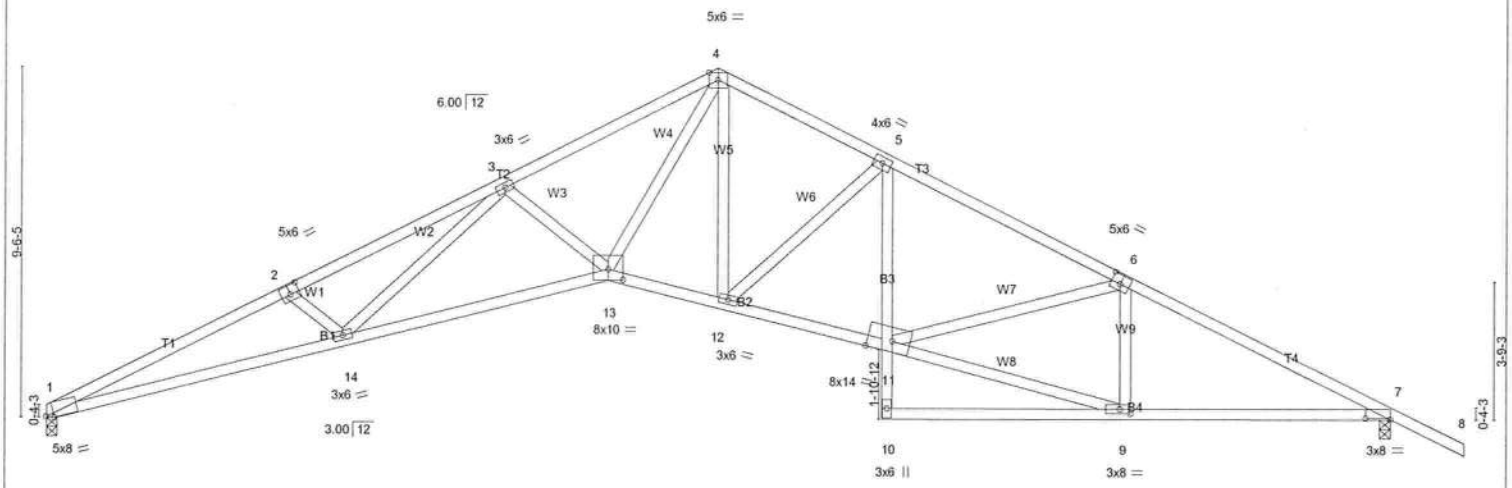
6-8-1	12-6-2	18-4-4	22-10-0	29-6-0	36-8-8	38-8-8
6-8-1	5-10-2	5-10-2	4-5-12	6-8-0	7-2-8	2-0-0

Scale = 1:62.9
Camber = 3/8 in



8-1-1	15-4-4	18-4-4	22-10-0	29-6-0	36-8-8
8-1-1	7-3-3	3-0-0	4-5-12	6-8-0	7-2-8

Plate Offsets (X,Y): [1:0-2-7,Edge], [2:0-3-0,0-3-0], [6:0-3-0,0-3-0], [7:0-8-0,0-0-6], [9:0-3-8,0-1-8], [11:0-8-4,Edge], [13:0-5-0,0-3-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.55	Vert(LL)	0.45	13-14	>976	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.82	Vert(TL)	-0.76	13-14	>575	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.71	Horz(TL)	0.44	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 198 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B3 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-7-13 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-6-0 oc bracing.

REACTIONS (lb/size) 1=1162/0-3-8, 7=1285/0-3-8
Max Horz 1=-157(load case 7)
Max Uplift1=-254(load case 6), 7=-348(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-4095/2172, 2-3=-3848/2094, 3-4=-2957/1551, 4-5=-1994/1138, 5-6=-2286/1271, 6-7=-2178/1185, 7-8=0/47
BOT CHORD 1-14=-1816/3685, 13-14=-1285/3055, 12-13=-533/1783, 11-12=-794/2037, 10-11=0/92, 5-11=-46/151, 9-10=-18/65, 7-9=-868/1861
WEBS 2-14=-267/295, 3-14=-417/619, 3-13=-521/453, 4-13=-799/1767, 4-12=-271/385, 5-12=-390/321, 9-11=-891/1883, 6-11=-35/170, 6-9=-443/293

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 254 lb uplift at joint 1 and 348 lb uplift at joint 7.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	LOT 18
L264517	T08	SPECIAL	4	1	Job Reference (optional)

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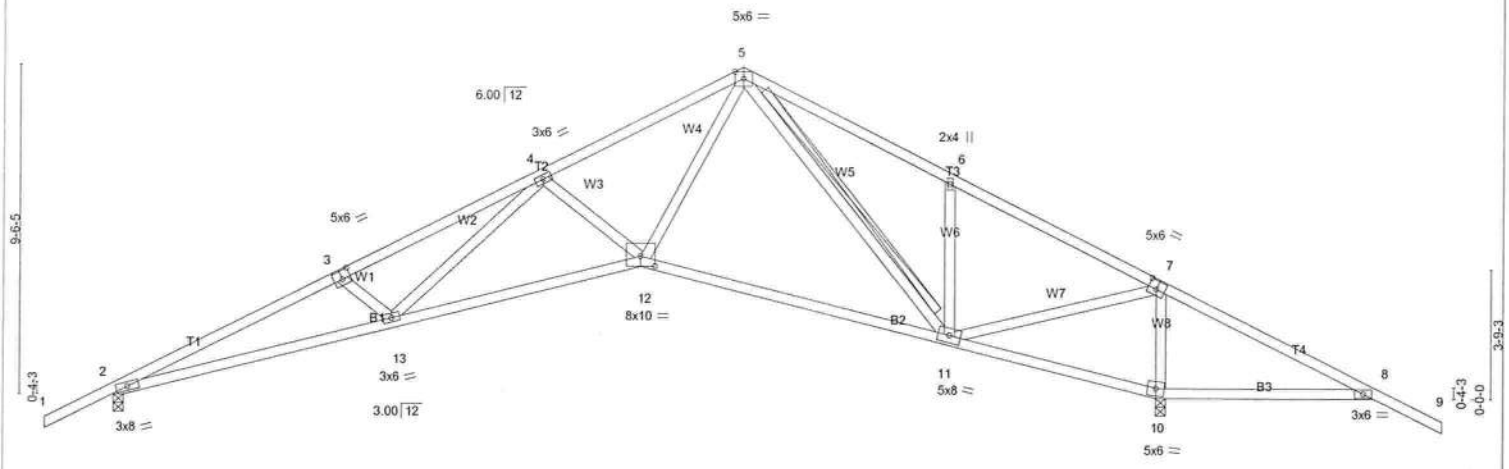
-2-0-0	6-8-1	12-6-2	18-4-4	24-4-10	30-5-0	36-8-8	38-8-8
2-0-0	6-8-1	5-10-2	5-10-2	6-0-6	6-0-6	6-3-8	2-0-0

Scale = 1:67.0
Camber = 1/4 in



8-1-1	15-4-4	24-4-10	30-5-0	36-8-8
8-1-1	7-3-3	9-0-6	6-0-6	6-3-8

Plate Offsets (X,Y): [3:0-3-0,0-3-0], [7:0-3-0,0-3-0], [12:0-5-0,0-3-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	0.23	12-13	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.59	Vert(TL)	-0.43	12-13	>836	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.54	Horz(TL)	0.26	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 184 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-4-5 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 5-11
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c. ,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1021/0-3-8, 10=1543/0-3-8
Max Horz 2=-144(load case 7)
Max Uplift2=-311(load case 6), 10=-588(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-2959/1272, 3-4=-2708/1194, 4-5=-1805/680, 5-6=-837/332, 6-7=-842/199, 7-8=-846/714, 8-9=0/47
BOT CHORD 2-13=-983/2639, 12-13=-479/1999, 11-12=-101/988, 10-11=-616/954, 8-10=-565/896
WEBS 3-13=-266/293, 4-13=-387/600, 4-12=-520/441, 5-12=-392/1281, 5-11=-427/201, 6-11=-334/326, 7-11=-758/1320, 7-10=-1321/922

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever right exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 311 lb uplift at joint 2 and 588 lb uplift at joint 10.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	LOT 18
L264517	T09	HIP	1	1	Job Reference (optional)

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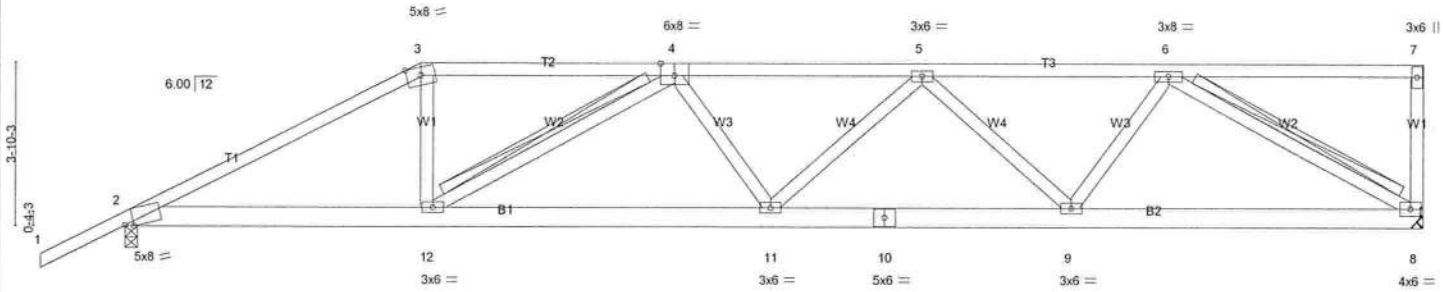
-2-0-0	7-0-0	13-0-2	18-10-5	24-8-6	30-8-8
2-0-0	7-0-0	6-0-2	5-10-3	5-10-1	6-0-2

Scale = 1:54.5
Camber = 1/4 in



7-0-0	15-3-10	22-5-0	30-8-8
7-0-0	8-3-10	7-1-6	8-3-8

Plate Offsets (X,Y): [2:0-2-7,Edge], [4:0-3-15,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.94	Vert(LL)	-0.26	11	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.60	Vert(TL)	-0.50	11-12	>734	240		
BCLL	10.0 *	Rep Stress Incr	NO	WB	1.00	Horz(TL)	0.12	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 176 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-5-3 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-6-5 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 4-12, 6-8
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c. ,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 8=2151/Mechanical, 2=2096/0-3-8
Max Horz 2=165(load case 5)
Max Uplift8=-741(load case 4), 2=-654(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/51, 2-3=-4025/1311, 3-4=-3578/1218, 4-5=-4750/1603, 5-6=-3673/1225, 6-7=-120/42, 7-8=-330/167
BOT CHORD 2-12=-1204/3525, 11-12=-1673/4758, 10-11=-1592/4557, 9-10=-1592/4557, 8-9=-1070/3012
WEBS 3-12=-357/1234, 4-12=-1372/577, 4-11=-14/170, 5-11=-22/269, 5-9=-1236/513, 6-9=-282/1202, 6-8=-3362/1195

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 741 lb uplift at joint 8 and 654 lb uplift at joint 2.
7) Girder carries hip end with 0-0-0 right side setback, 7-0-0 left side setback, and 7-0-0 end setback.
8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 411 lb down and 165 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-7=-118(F=-64), 2-12=-10, 8-12=-22(F=-12)
Concentrated Loads (lb)
Vert: 12=-411(F)

Job	Truss	Truss Type	Qty	Ply	LOT 18
L264517	T10	HIP	1	1	Job Reference (optional)

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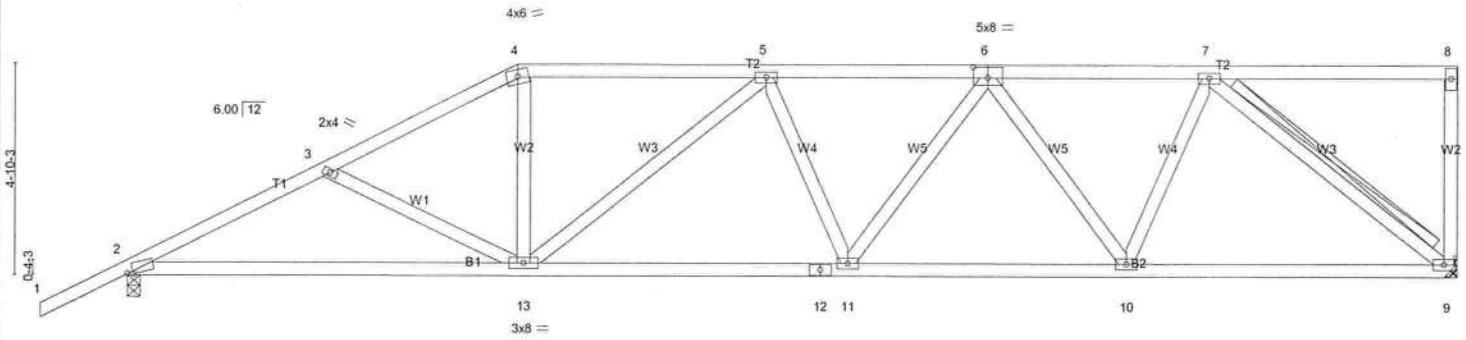
-2-0-0	4-7-15	9-0-0	14-8-14	19-10-4	24-11-10	30-8-8
2-0-0	4-7-15	4-4-1	5-8-14	5-1-6	5-1-7	5-8-13

Scale = 1:53.2
Camber = 1/8 in



9-0-0	16-7-10	23-0-13	30-8-8
9-0-0	7-7-10	6-5-4	7-7-11

Plate Offsets (X,Y): [2:0-1-9,0-0-7], [6:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	-0.14	2-13	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.45	Vert(TL)	-0.27	2-13	>999	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.41	Horz(TL)	0.07	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 167 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-7-8 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-4-15 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 7-9
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c. ,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 9=969/Mechanical, 2=1093/0-3-8
Max Horz 2=195(load case 6)
Max Uplift9=-265(load case 5), 2=-262(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1801/904, 3-4=-1570/802, 4-5=-1374/777, 5-6=-1598/863, 6-7=-1198/630, 7-8=-32/12, 8-9=-142/100
BOT CHORD 2-13=-952/1544, 12-13=-906/1633, 11-12=-906/1633, 10-11=-813/1490, 9-10=-543/997
WEBS 3-13=-204/199, 4-13=-131/412, 5-13=-335/166, 5-11=-93/112, 6-11=-86/192, 6-10=-507/317, 7-10=-227/521, 7-9=-1245/685

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All plates are 3x6 MT20 unless otherwise indicated.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Refer to girder(s) for truss to truss connections.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 265 lb uplift at joint 9 and 262 lb uplift at joint 2.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	LOT 18
L264517	T11	HIP	1	1	Job Reference (optional)

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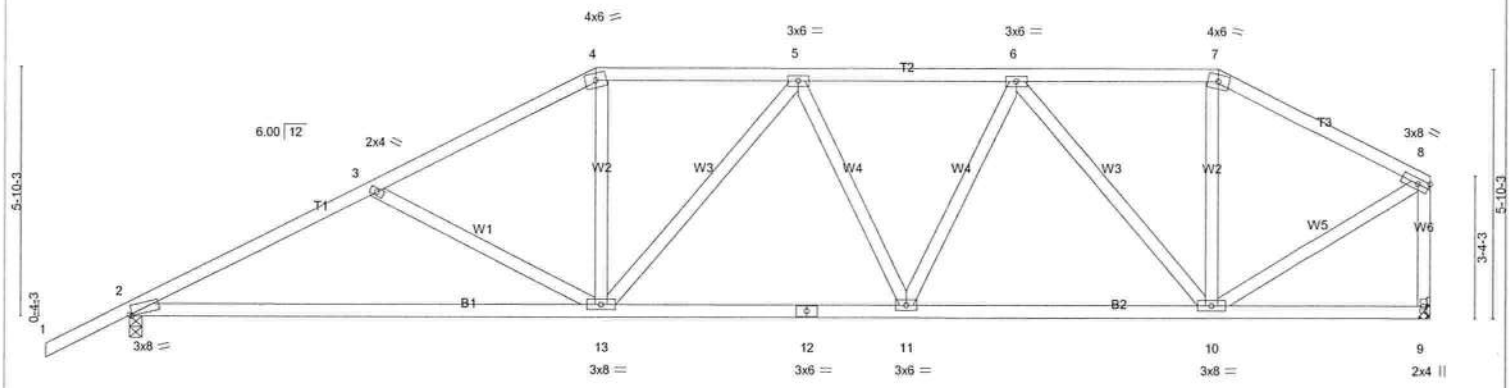
-2-0-0	5-9-14	11-0-0	15-9-6	20-11-2	25-8-8	30-8-8
2-0-0	5-9-14	5-2-2	4-9-6	5-1-13	4-9-6	5-0-0

Scale = 1:54.5
Camber = 1/4 in



11-0-0	18-4-4	25-8-8	30-8-8
11-0-0	7-4-4	7-4-4	5-0-0

Plate Offsets (X,Y): [2:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.35	Vert(LL)	-0.30	2-13	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.65	Vert(TL)	-0.56	2-13	>650	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.62	Horz(TL)	0.06	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 173 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-6-10 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-7-7 oc bracing.

REACTIONS (lb/size) 2=1093/0-3-8, 9=969/Mechanical
Max Horz 2=177(load case 6)
Max Uplift2=-280(load case 6), 9=-182(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1772/936, 3-4=-1455/788, 4-5=-1250/768, 5-6=-1264/764, 6-7=-752/497, 7-8=-893/493, 8-9=-944/530
BOT CHORD 2-13=-897/1516, 12-13=-695/1337, 11-12=-695/1337, 10-11=-600/1160, 9-10=-29/33
WEBS 3-13=-311/290, 4-13=-132/385, 5-13=-252/125, 5-11=-189/132, 6-11=-93/261, 6-10=-674/354, 7-10=-19/193, 8-10=-405/843

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Refer to girder(s) for truss to truss connections.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 280 lb uplift at joint 2 and 182 lb uplift at joint 9.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	LOT 18
L264517	T12	HIP	1	1	Job Reference (optional)

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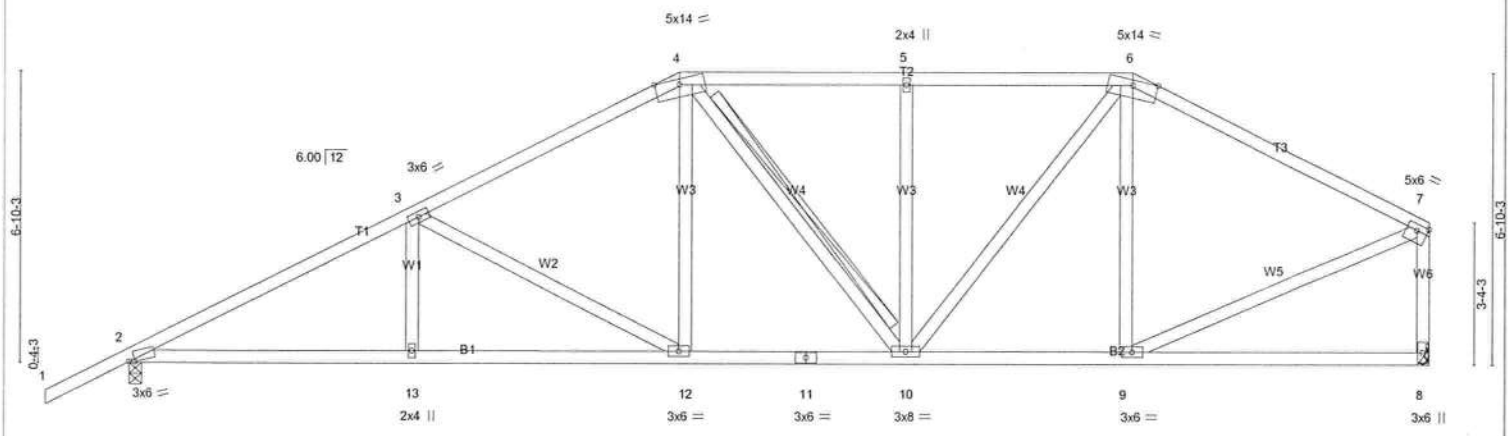
-2-0-0	6-8-5	13-0-0	18-4-4	23-8-8	30-8-8
2-0-0	6-8-5	6-3-11	5-4-4	5-4-4	7-0-0

Scale = 1:54.5
Camber = 1/16 in



6-8-5	13-0-0	18-4-4	23-8-8	30-8-8
6-8-5	6-3-11	5-4-4	5-4-4	7-0-0

Plate Offsets (X,Y): [2:0-1-9,0-0-7], [7:Edge,0-1-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.64	Vert(LL)	0.10	12-13	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.35	Vert(TL)	-0.15	12-13	>999	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.42	Horz(TL)	0.05	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 180 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-5-11 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-9-8 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 4-10
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c. ,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1093/0-3-8, 8=969/Mechanical
Max Horz 2=189(load case 6)
Max Uplift2=-292(load case 6), 8=-158(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1782/908, 3-4=-1326/762, 4-5=-1095/721, 5-6=-1095/721, 6-7=-1004/566, 7-8=-928/547
BOT CHORD 2-13=-866/1513, 12-13=-866/1513, 11-12=-576/1122, 10-11=-576/1122, 9-10=-404/819, 8-9=-57/74
WEBS 3-13=0/210, 3-12=-452/330, 4-12=-120/328, 4-10=-75/116, 5-10=-287/160, 6-10=-223/505, 6-9=-277/204, 7-9=-385/822

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Refer to girder(s) for truss to truss connections.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 292 lb uplift at joint 2 and 158 lb uplift at joint 8.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	LOT 18
L264517	T13	HIP	1	1	Job Reference (optional)

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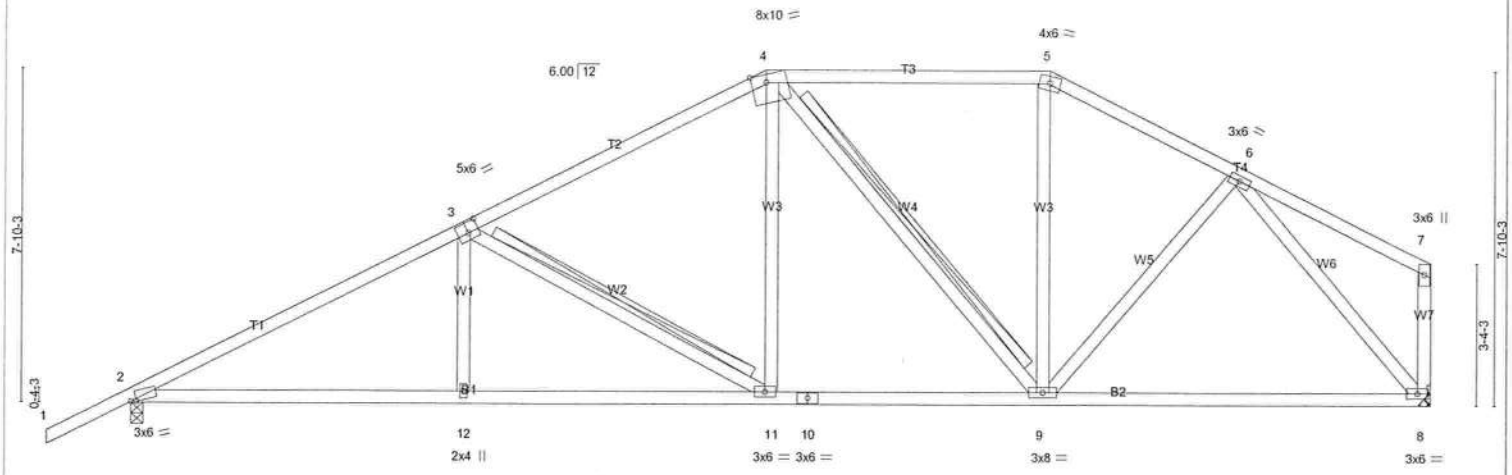
-2-0-0	7-10-8	15-0-0	21-8-8	26-0-12	30-8-8
2-0-0	7-10-8	7-1-8	6-8-8	4-4-4	4-7-12

Scale = 1:54.5
Camber = 1/8 in



7-10-8	15-0-0	21-8-8	30-8-8
7-10-8	7-1-8	6-8-8	9-0-0

Plate Offsets (X,Y): [2:0-1-9,0-0-7], [3:0-3-0,0-3-0], [4:0-4-3,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.62	Vert(LL)	-0.13	8-9	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.44	Vert(TL)	-0.23	8-9	>999	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.84	Horz(TL)	0.06	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 176 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-3-9 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-9-12 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 3-11, 4-9
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c. ,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1093/0-3-8, 8=969/Mechanical
Max Horz 2=201(load case 6)
Max Uplift2=-302(load case 6), 8=-172(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1742/902, 3-4=-1205/723, 4-5=-837/617, 5-6=-979/631, 6-7=-109/79, 7-8=-147/118
BOT CHORD 2-12=-846/1468, 11-12=-846/1467, 10-11=-498/1004, 9-10=-498/1004, 8-9=-382/691
WEBS 3-12=0/250, 3-11=-537/400, 4-11=-157/369, 4-9=-329/164, 5-9=-63/207, 6-9=-51/286, 6-8=-1008/582

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Refer to girder(s) for truss to truss connections.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 302 lb uplift at joint 2 and 172 lb uplift at joint 8.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	LOT 18
L264517	T14	HIP	1	1	Job Reference (optional)

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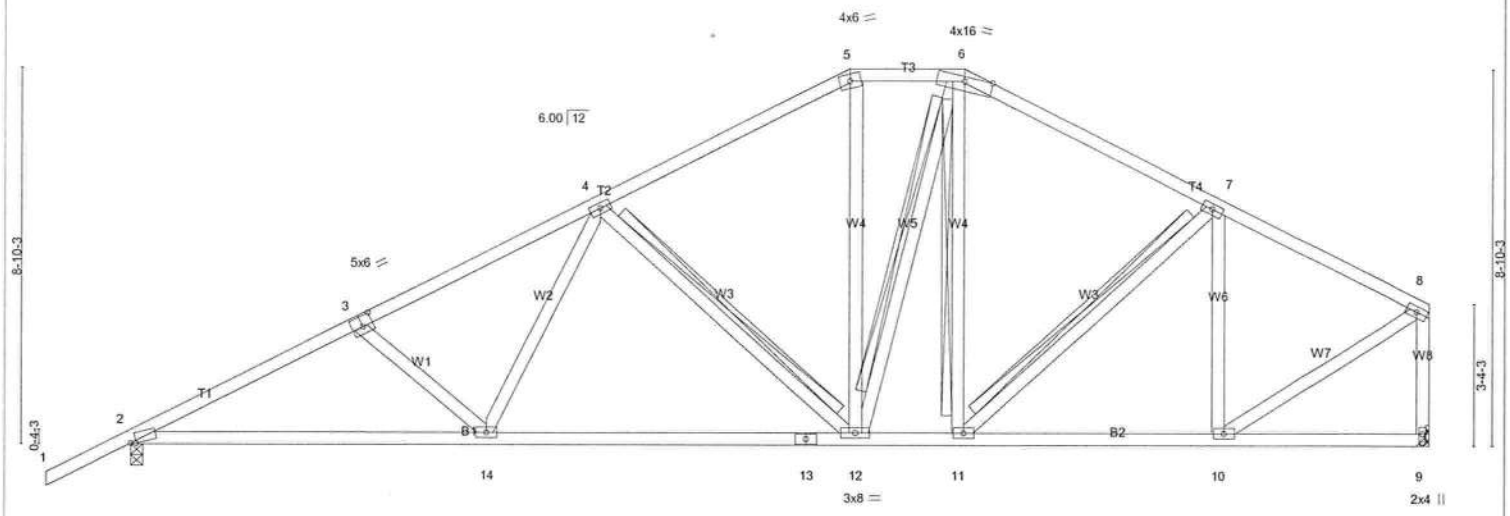
-2-0-0	5-5-7	11-1-3	17-0-0	19-8-8	25-9-0	30-8-8
2-0-0	5-5-7	5-7-11	5-10-13	2-8-8	6-0-9	4-11-8

Scale = 1:54.5
Camber = 1/8 in



8-5-1	17-0-0	19-8-8	25-9-0	30-8-8
8-5-1	8-6-15	2-8-8	6-0-9	4-11-8

Plate Offsets (X,Y): [2:0-1-9,0-0-7], [3:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.32	Vert(LL)	-0.12	12-14	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.45	Vert(TL)	-0.24	12-14	>999	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.27	Horz(TL)	0.05	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 195 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-7-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-6-5 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 4-12, 6-12, 6-11, 7-11
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c. ,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1093/0-3-8, 9=969/Mechanical
Max Horz 2=213(load case 6)
Max Uplift2=-310(load case 6), 9=-185(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1798/964, 3-4=-1580/900, 4-5=-1039/689, 5-6=-864/679, 6-7=-978/654, 7-8=-880/516, 8-9=-944/557
BOT CHORD 2-14=-927/1538, 13-14=-697/1231, 12-13=-697/1231, 11-12=-372/806, 10-11=-402/743, 9-10=-21/28
WEBS 3-14=-253/246, 4-14=-115/361, 4-12=-504/405, 5-12=-140/242, 6-12=-104/322, 6-11=-78/52, 7-11=-39/167, 7-10=-409/290, 8-10=-454/851

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 3x6 MT20 unless otherwise indicated.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Refer to girder(s) for truss to truss connections.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 310 lb uplift at joint 2 and 185 lb uplift at joint 9.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	LOT 18
L264517	T15	HIP	1	1	Job Reference (optional)

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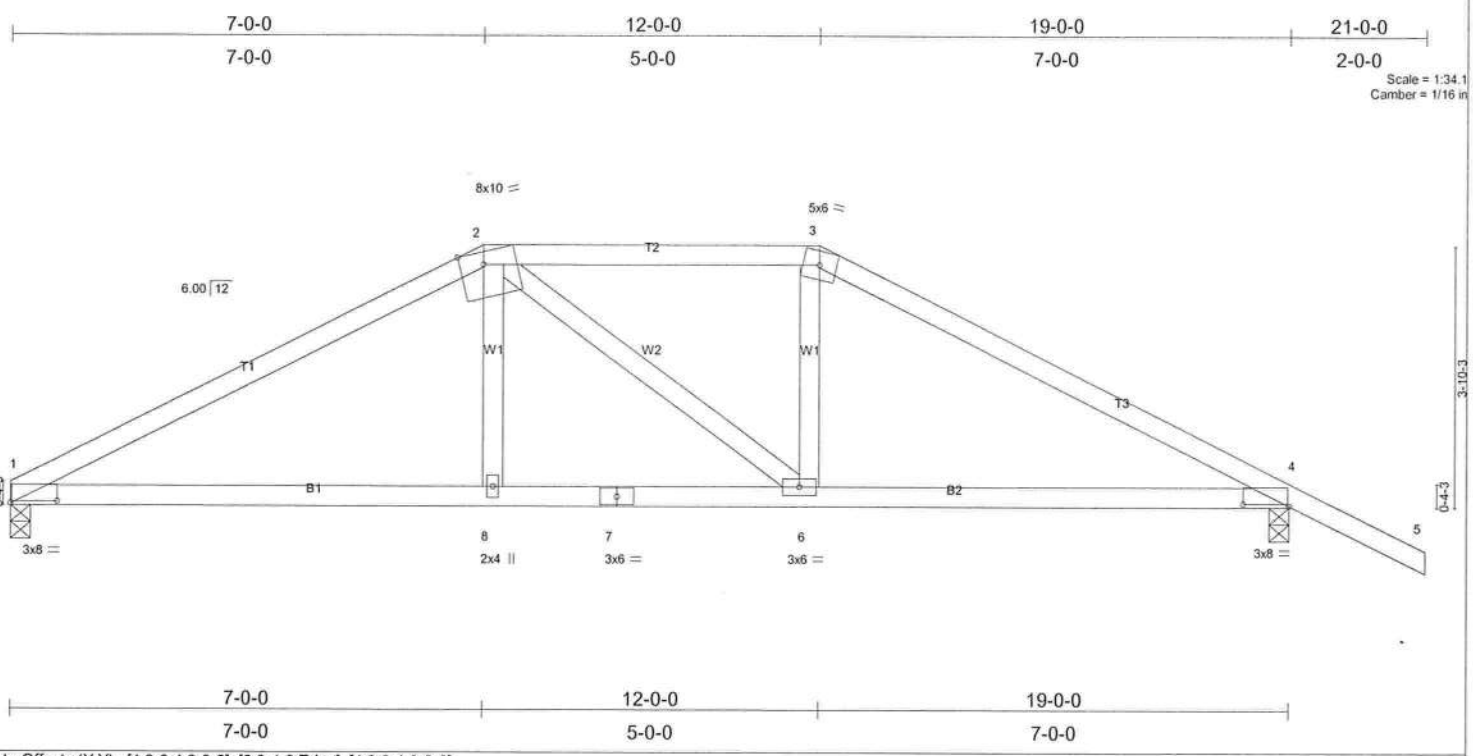

Plate Offsets (X,Y): [1:0-8-4,0-0-6], [2:0-4-3,Edge], [4:0-8-4,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	-0.08	1-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.56	Vert(TL)	-0.17	1-8	>999	240		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.18	Horz(TL)	0.06	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 81 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-9-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-1-7 oc bracing.

REACTIONS (lb/size) 1=1189/0-3-8, 4=1318/0-3-8
Max Horz 1=-90(load case 6)
Max Uplift1=-358(load case 5), 4=-456(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2278/694, 2-3=-1957/639, 3-4=-2260/676, 4-5=0/47
BOT CHORD 1-8=-580/1957, 7-8=-585/1978, 6-7=-585/1978, 4-6=-535/1936
WEBS 2-8=-126/524, 2-6=-149/118, 3-6=-145/576

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 358 lb uplift at joint 1 and 456 lb uplift at joint 4.
7) Girder carries hip end with 7-0-0 end setback.
8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 411 lb down and 165 lb up at 12-0-0, and 411 lb down and 165 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 2-3=-118(F=-64), 3-5=-54, 1-8=-10, 6-8=-22(F=-12), 4-6=-10
Concentrated Loads (lb)
Vert: 8=-411(F) 6=-411(F)

Job	Truss	Truss Type	Qty	Ply	LOT 18
L264517	T16	HIP	1	1	Job Reference (optional)

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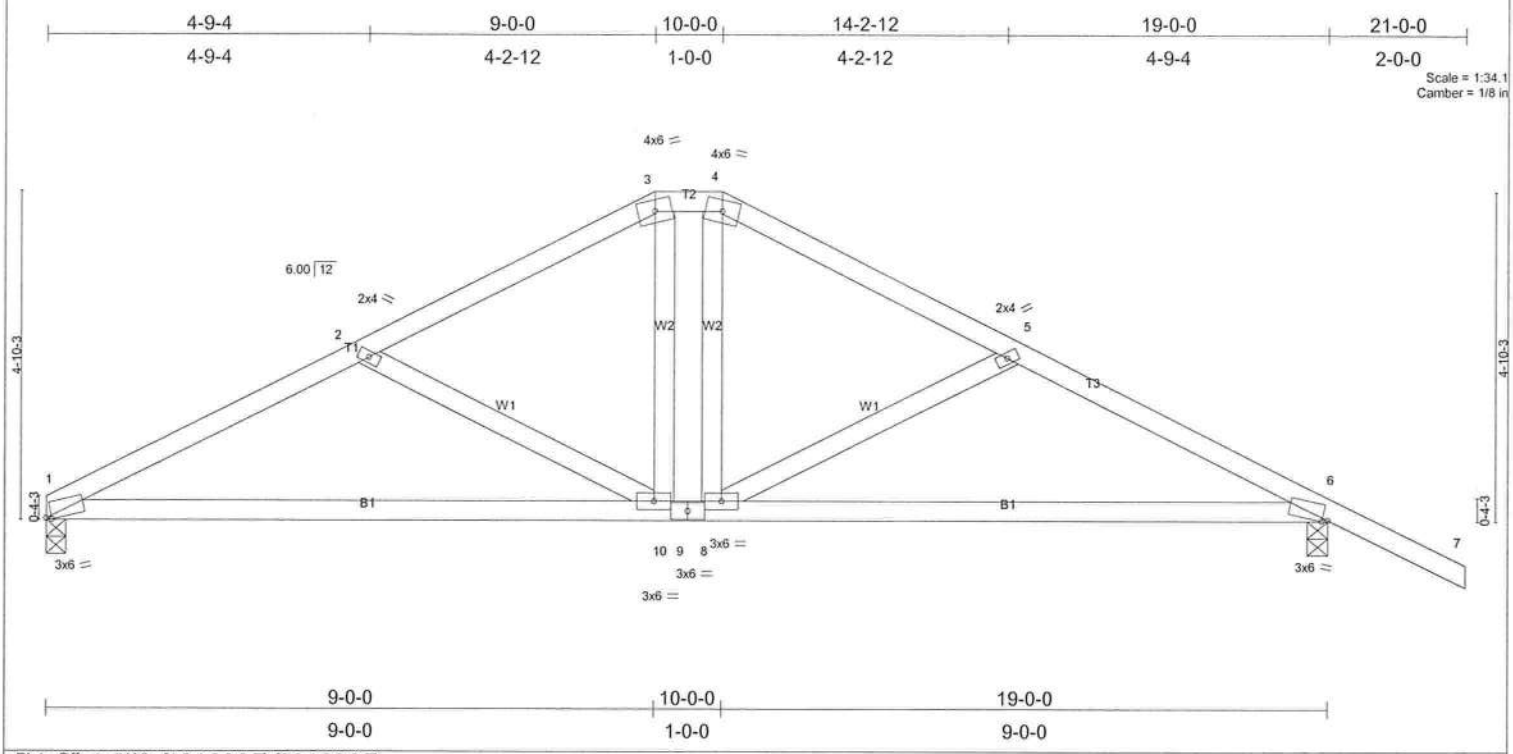

Plate Offsets (X,Y): [1:0-1-0,0-0-7], [6:0-1-0,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.12	1-10	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.48	Vert(TL)	-0.23	1-10	>972	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.14	Horz(TL)	0.03	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 90 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-11-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=592/0-3-8, 6=721/0-3-8
Max Horz 1=-102(load case 7)
Max Uplift1=-127(load case 6), 6=-225(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1021/590, 2-3=-756/444, 3-4=-630/445, 4-5=-753/439, 5-6=-1008/550, 6-7=0/47
BOT CHORD 1-10=-380/875, 9-10=-136/630, 8-9=-136/630, 6-8=-329/844
WEBS 2-10=-302/280, 3-10=-126/211, 4-8=-39/197, 5-8=-268/219

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 127 lb uplift at joint 1 and 225 lb uplift at joint 6.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	LOT 18
L264517	T17	HOWE	1	2	Job Reference (optional)

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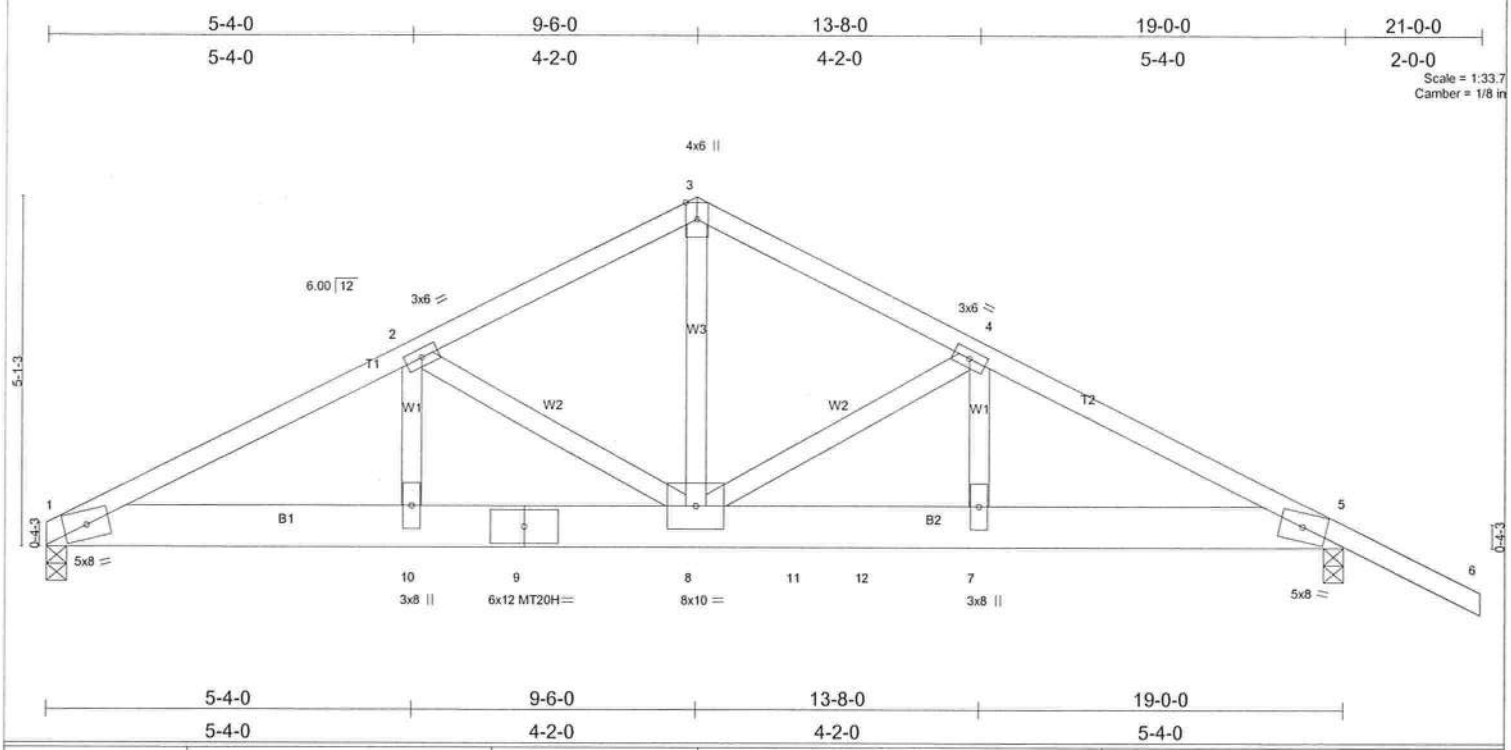


LOADING (psf)			SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.31	Vert(LL)	-0.11	7-8	>999	360	MT20	244/190
TCDL	7.0		Lumber Increase	1.25	BC	0.43	Vert(TL)	-0.21	7-8	>999	240	MT20H	187/143
BCLL	10.0	*	Rep Stress Incr	NO	WB	0.46	Horz(TL)	0.04	5	n/a	n/a		
BCDL	5.0		Code FBC2004/TPI2002		(Matrix)							Weight: 239 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-6-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=4884/0-3-8, 5=3516/0-3-8
Max Horz 1=-111(load case 6)
Max Uplift1=-1317(load case 5), 5=-1001(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-8132/2180, 2-3=-5888/1594, 3-4=-5876/1603, 4-5=-7289/1908, 5-6=0/53
BOT CHORD 1-10=-1947/7255, 9-10=-1947/7255, 8-9=-1947/7255, 8-11=-1661/6476, 11-12=-1661/6476, 7-12=-1661/6476, 5-7=-1661/6476
WEBS 2-10=-545/1961, 2-8=-2393/709, 3-8=-1342/4963, 4-8=-1479/428, 4-7=-278/1151

NOTES
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-7-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
5) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) All plates are MT20 plates unless otherwise indicated.
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1317 lb uplift at joint 1 and 1001 lb uplift at joint 5.
9) Girder carries tie-in span(s): 30-8-8 from 0-0-0 to 11-0-0
10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 2151 lb down and 595 lb up at 12-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-6=-54, 1-11=-465(B=-455), 5-11=-10
Concentrated Loads (lb)
Vert: 12=-2151(F)

Job	Truss	Truss Type	Qty	Ply	LOT 18
L264517	T18	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Dec 26 11:21:08 2007 Page 1

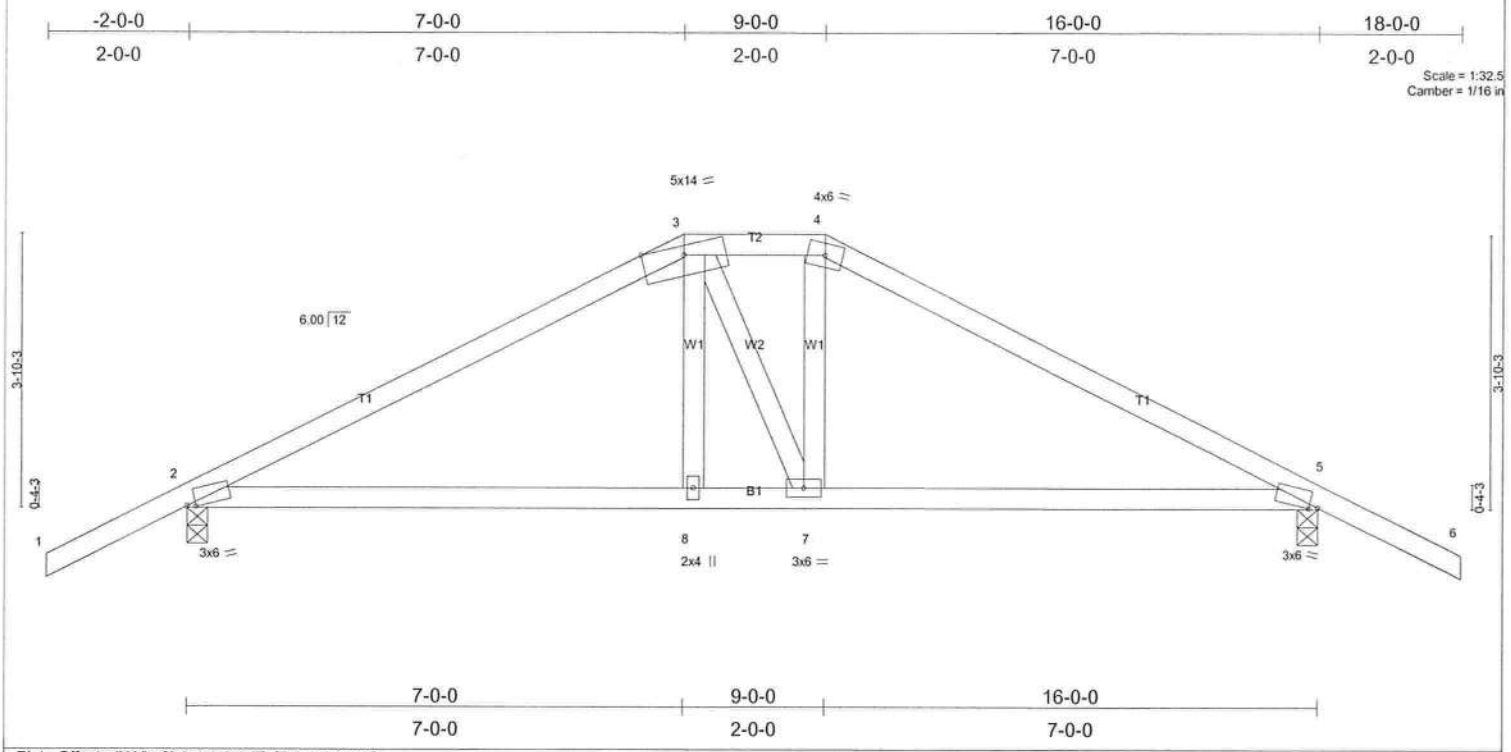


Plate Offsets (X,Y): [2:0-1-8,0-0-7], [5:0-1-8,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.40	Vert(LL)	0.12	2-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.42	Vert(TL)	-0.14	2-8	>999	240		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.19	Horz(TL)	0.04	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 72 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-10 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-5-6 oc bracing.

REACTIONS (lb/size) 2=1103/0-3-8, 5=1103/0-3-8
Max Horz 2=77(load case 5)
Max Uplift2=-595(load case 5), 5=-595(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1778/804, 3-4=-1526/770, 4-5=-1781/805, 5-6=0/47
BOT CHORD 2-8=-674/1503, 7-8=-684/1523, 5-7=-658/1506
WEBS 3-8=-262/480, 3-7=-146/159, 4-7=-303/592

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 595 lb uplift at joint 2 and 595 lb uplift at joint 5.
7) Girder carries hip end with 7-0-0 end setback.
8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 411 lb down and 165 lb up at 9-0-0, and 411 lb down and 165 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

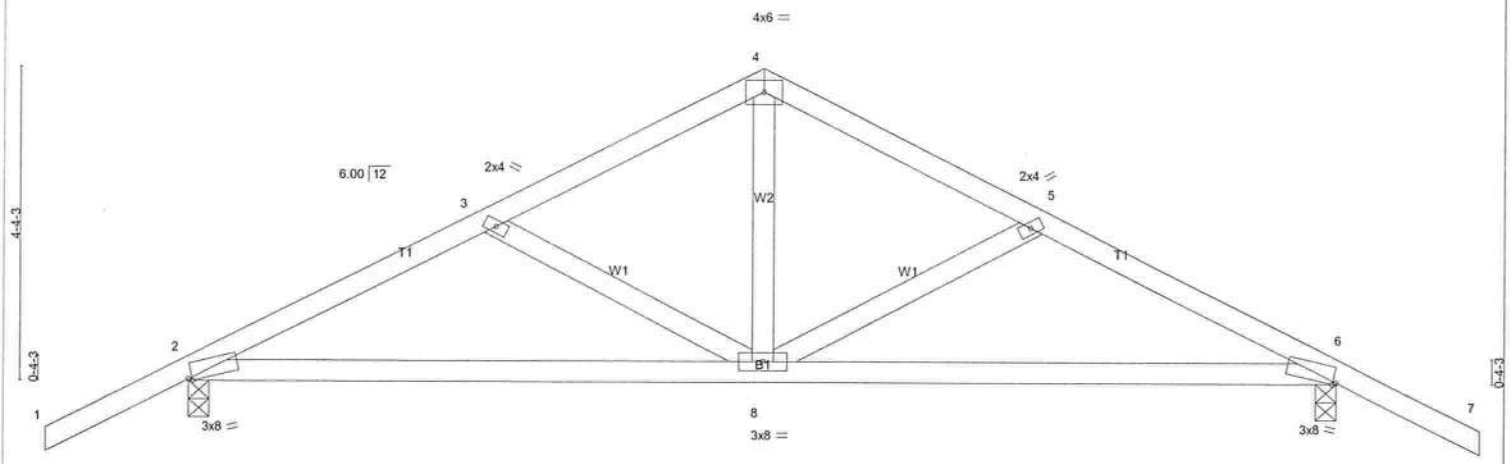
LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-4=-118(F=-64), 4-6=-54, 2-8=-10, 7-8=-22(F=-12), 5-7=-10
Concentrated Loads (lb)
Vert: 8=-411(F) 7=-411(F)

Job	Truss	Truss Type	Qty	Ply	LOT 18
L264517	T19	QUEENPOST	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Dec 26 11:21:09 2007 Page 1

-2-0-0	4-3-4	8-0-0	11-8-12	16-0-0	18-0-0
2-0-0	4-3-4	3-8-12	3-8-12	4-3-4	2-0-0

Scale: 3/8"=1'



8-0-0	16-0-0
8-0-0	8-0-0

Plate Offsets (X,Y): [2:0-0-10,Edge], [6:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	0.19	2-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.32	Vert(TL)	-0.12	2-8	>999	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.20	Horz(TL)	-0.02	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 75 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

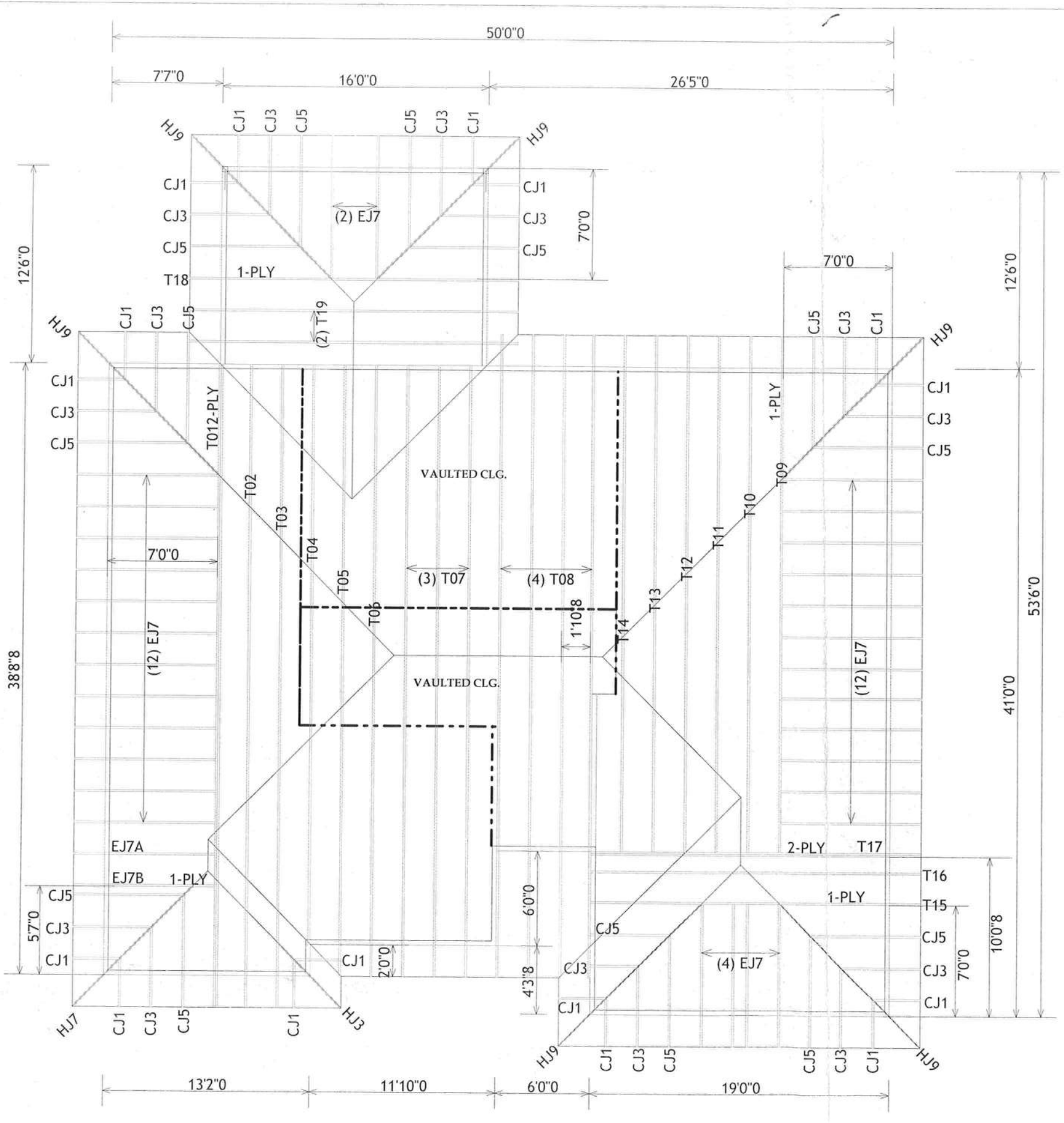
BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-7-15 oc bracing.

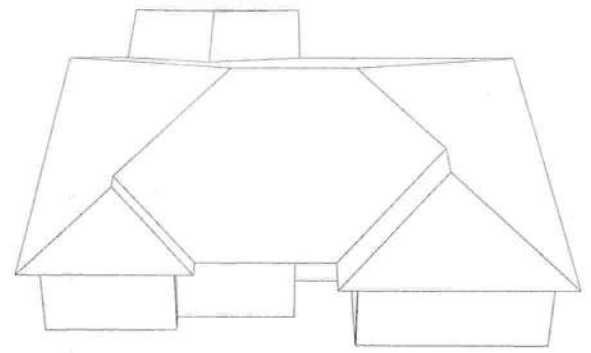
REACTIONS (lb/size) 2=619/0-3-8, 6=619/0-3-8
Max Horz 2=83(load case 6)
Max Uplift2=-404(load case 6), 6=-404(load case 7)

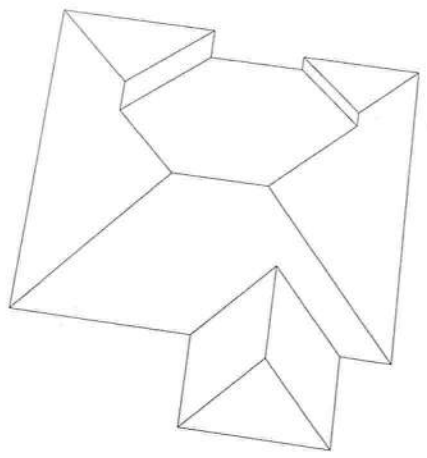
FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-802/1064, 3-4=-590/942, 4-5=-590/942, 5-6=-802/1064, 6-7=0/47
BOT CHORD 2-8=-810/663, 6-8=-810/663
WEBS 3-8=-217/243, 4-8=-670/331, 5-8=-217/243

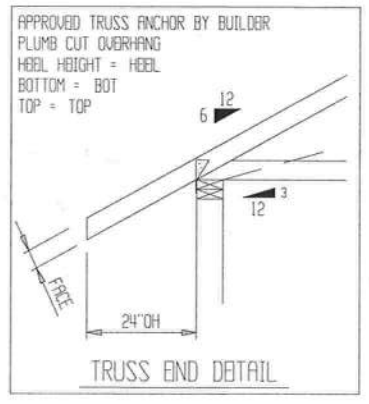
NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 404 lb uplift at joint 2 and 404 lb uplift at joint 6.

LOAD CASE(S) Standard









TRUSS END DETAIL

HANGER SCHEDULE
7-HTU26

BEARING HEIGHT SCHEDULE

9'-0"

OVERHANG
2'-0"

ROOF PITCH(S)
6/12

NOTES:

1) REFER TO HIB 91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BRACING.) REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED.
2.) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL V105 FOR ALTERNATE BRACING REQUIREMENTS.
3.) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.
4.) ALL TRUSSES ARE DESIGNED FOR 2' o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.
5.) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.
6.) SY42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.
7.) ALL ROOF TRUSS HANGERS TO BE SIMPSON HUS26 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSS HANGERS TO BE SIMPSON THA422 UNLESS OTHERWISE NOTED.
8.) BEAM/HEADER/LINTEL (HDR) TO BE FURNISHED BY BUILDER.

SHOP DRAWING APPROVAL

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION OF TRUSSES AND VOIDS ALL PREVIOUS ARCHITECTURAL OR OTHER TRUSS LAYOUTS. REVIEW AND APPROVAL OF THIS LAYOUT MUST BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VERIFY ALL CONDITIONS TO INSURE AGAINST CHANGES THAT WILL RESULT IN EXTRA CHARGES TO YOU.

Requested Delivery Date: _____

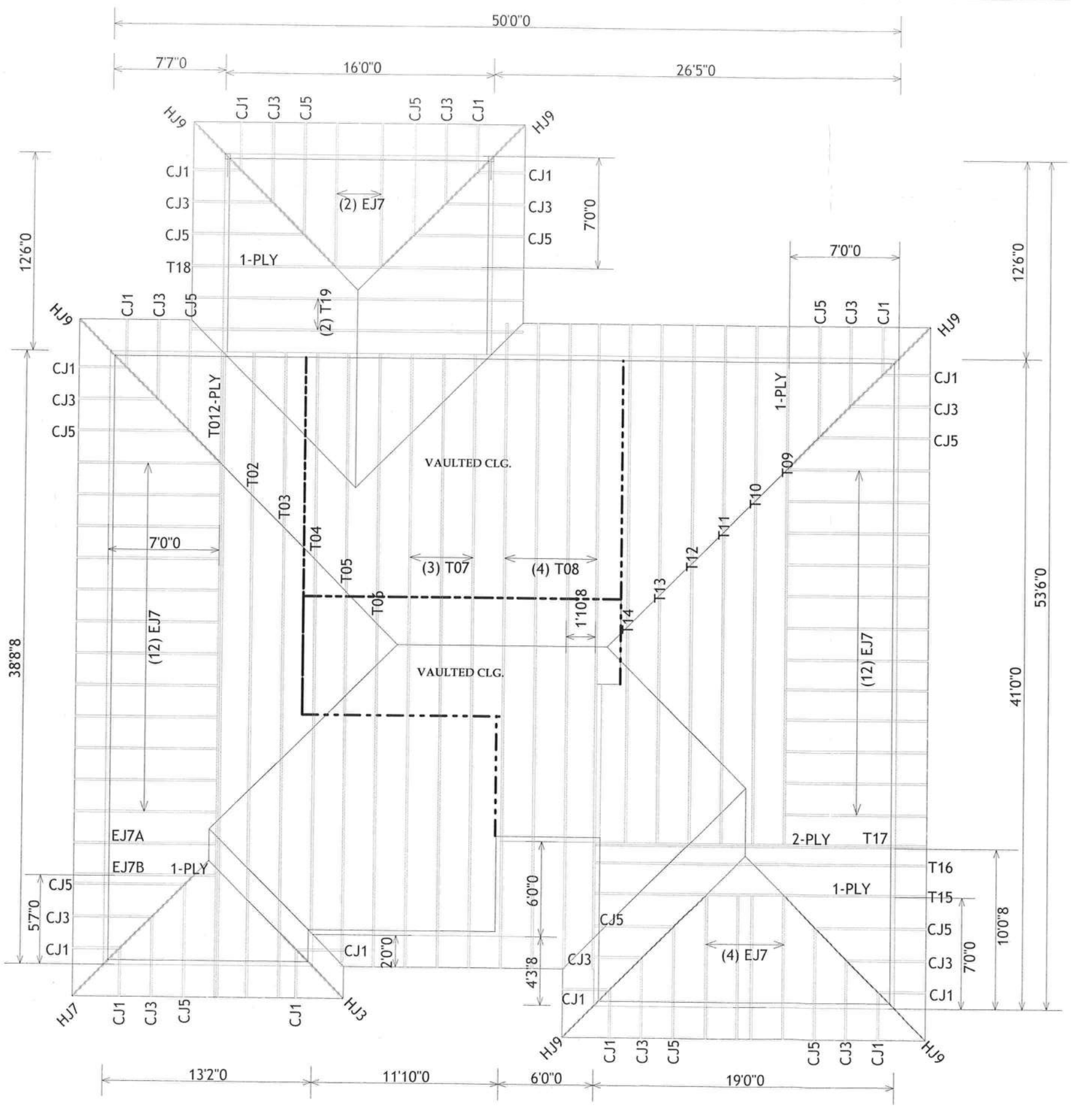
Approved by: _____ Date: _____

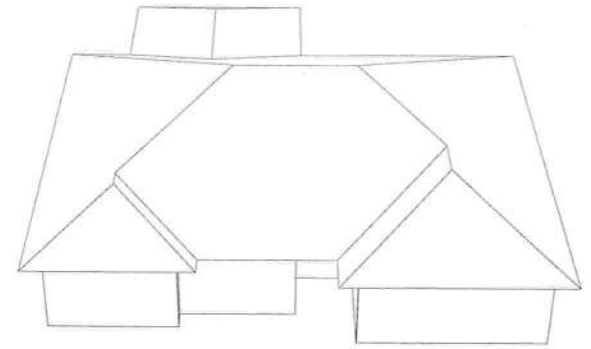
Builders FirstSource
Bunnell
PHONE: 904-437-3349 FAX: 904-437-3994
Jacksonville
PHONE: 904-772-6100 FAX: 904-772-1973
Lake City
PHONE: 904-755-6894 FAX: 904-755-7973
Sanford
PHONE: 407-322-0059 FAX: 407-322-5553

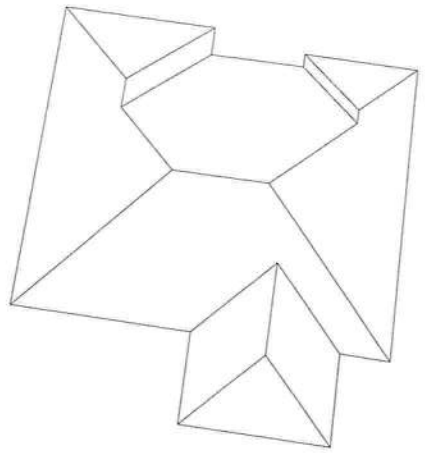
BUILDER:
CASH ACCOUNT - MIKE ROBERTS

LEGAL ADDRESS:
COLUMBIA, FL - LOT 18

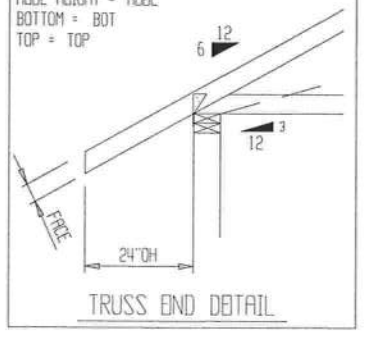
MODEL:	REVISION:
CUSTOM	B.CANNADY
	SCALE: NTS
DATE:	DRAWN BY:
12/26/07	A.MONDRAGON
JOB #:	
L264517	







APPROVED TRUSS ANCHOR BY BUILDER
PLUMB CUT OVERHANG
HEEL HEIGHT = HEEL
BOTTOM = BOT
TOP = TOP



TRUSS END DETAIL

HANGER SCHEDULE
7-HTU26

BEARING HEIGHT SCHEDULE

9'-0"

OVERHANG
2'-0"

ROOF PITCH(S)
6/12

NOTES:

1) REFER TO HIB 91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BRACING.) REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED.

2) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL V10S FOR ALTERNATE BRACING REQUIREMENTS.

3) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.

4) ALL TRUSSES ARE DESIGNED FOR 2' o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.

5) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.

6) SY42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.

7) ALL ROOF TRUSS HANGERS TO BE SIMPSON HUS26 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSS HANGERS TO BE SIMPSON THA422 UNLESS OTHERWISE NOTED.

8) BEAM/HEADER/LINTEL (HDR) TO BE FURNISHED BY BUILDER.

SHOP DRAWING APPROVAL

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Requested Delivery Date: _____

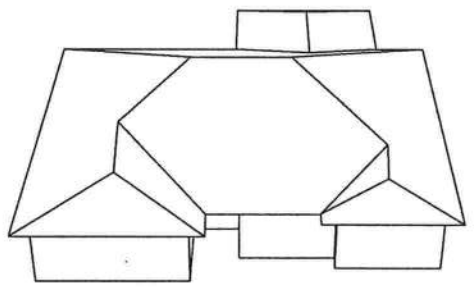
Approved by: _____ Date: _____

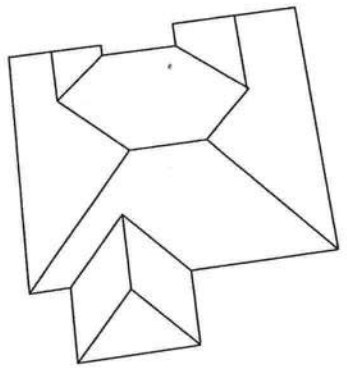
Builders FirstSource
Bunnell
PHONE: 904-437-3344 FAX: 904-437-3994
Jacksonville
PHONE: 904-772-6100 FAX: 904-772-1973
Lake City
PHONE: 904-755-6894 FAX: 904-755-7973
Sanford
PHONE: 407-322-0059 FAX: 407-322-5553

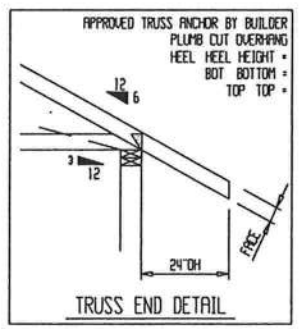
BUILDER:
CASH ACCOUNT - MIKE ROBERTS

LEGAL ADDRESS:
COLUMBIA, FL - LOT 18

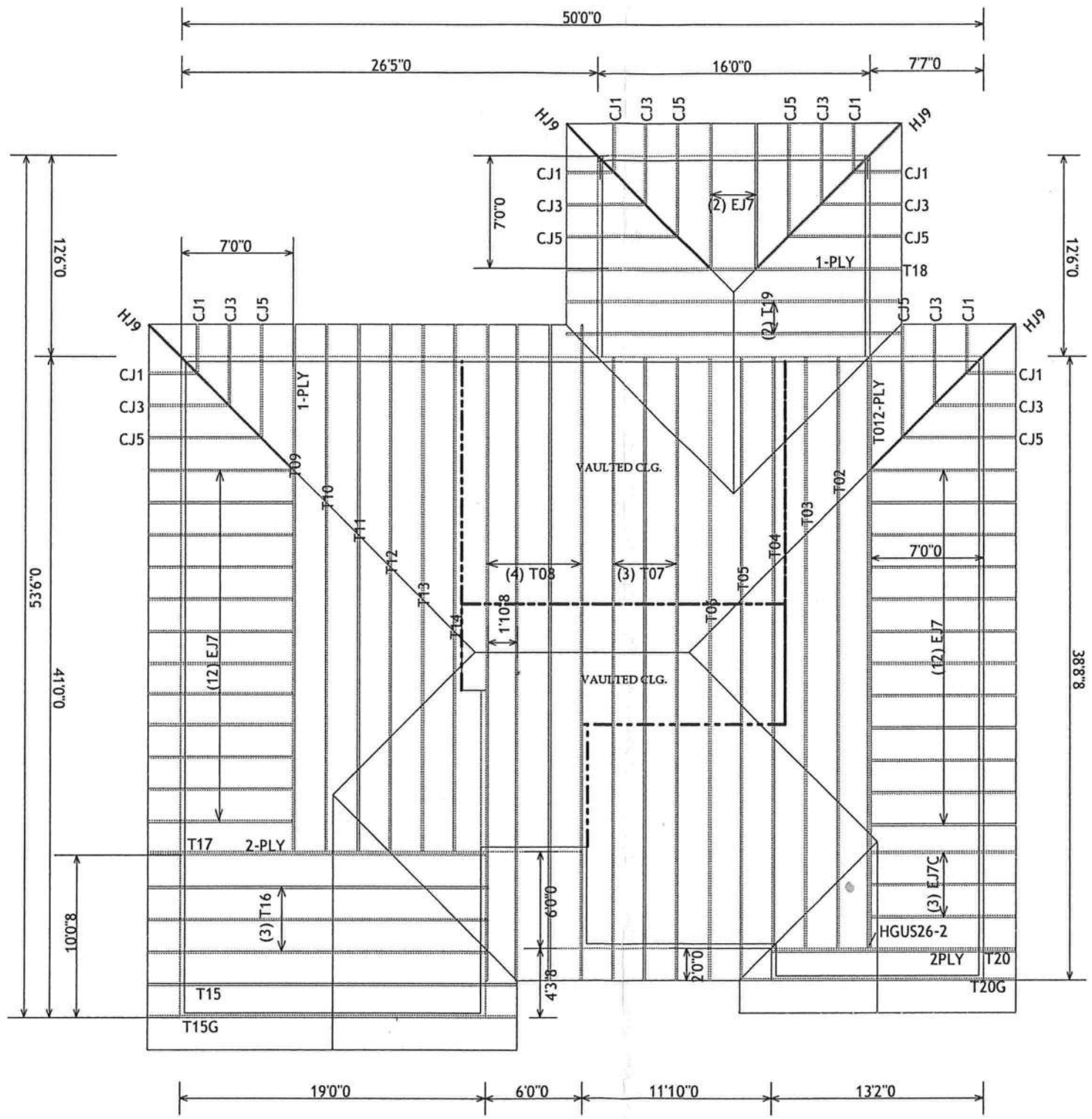
MODEL:	REVISED:
CUSTOM	B.CANNADY
	SCALE: NTS
DATE: **12/26/07**	DRAWN BY: A.MONDRAGON
	JOB #: **L264517**







HANGER SCHEDULE
8-HTU26
1-HGUS26-2



BEARING HEIGHT SCHEDULE

9'-0"

OVERHANG
2'-0"

ROOF PITCH(S)
6/12

NOTES:

1) REFER TO HIB 91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BRACING.) REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED.

2) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL V105 FOR ALTERNATE BRACING REQUIREMENTS.

3) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.

4) ALL TRUSSES ARE DESIGNED FOR 2' o.c. MAXIMUM SPACING UNLESS OTHERWISE NOTED.

5) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.

6) SY42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.

7) ALL ROOF TRUSS HANGERS TO BE SIMPSON LUS26 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSS HANGERS TO BE SIMPSON THA422 UNLESS OTHERWISE NOTED.

8) BEAM/HEADER/LINTEL (HDR) TO BE FURNISHED BY BUILDER.

SHOP DRAWING APPROVAL

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Requested Delivery Date : _____

Approved By _____ Date _____

Builders FirstSource
Bunnell
PHONE: 904-437-3349 FAX: 904-437-3994
Jacksonville
PHONE: 904-772-6100 FAX: 904-772-1975
Lake City
PHONE: 904-755-6894 FAX: 904-755-7975
Sanford
PHONE: 407-322-0099 FAX: 407-322-9993

BUILDER:
CASH ACCOUNT - MIKE ROBERTS

LEGAL ADDRESS:
COLUMBIA, FL - LOT 8

MODEL: CUSTOM
REVISION: B.CANNADY
SCALE: NTS

DATE: 12/26/07
DRAWN BY: A.MONDRAGON
JOB #: L264527

F.C.

Florida Department of Transportation
District 2 – Lake City Maintenance

Rev. 8-15-05

F.D.O.T. Permits Office, Lake City Maintenance
Post Office Box 1415
Lake City, Florida 32056-1415

Date: 1-30-2008

Re: Notice of Approved State FDOT Access Connection Permit
Access Permit Category A
Residential Connection

Permitted: Mr Michael W. Roberts.
Mailing Address: 657 SW Catherine Lane, Lake City Fl.32025
Permit No: 2008-A-292-6 / State Hwy No: 247(N) /Mile Post: 8.854 + -
Road Section No. 29090/ Col, County

Mr. Roberts

Enclosed within is your approved state access permit applied for previously. I would like to take this opportunity to thank you for your courteous assistance during this time. Cooperation between yourself and our office has allowed us to process your application in a most timely manner, and for this I thank you.

Below is information that if followed can prevent permit and construction problems down the road, please read them carefully and pay special attention to item number 4, 5 and 6.

1.) If you plan to hire a contractor to construct your new access connection (driveway), we would recommend that you make several complete copies of the enclosed connection permit packet and seek at least three bids, as with most things in this life, all contractors are different. A complete listing of all contractors for the county you have permitted too is available on request.

2.) Please take the time to review your new permit package and read all of the permit construction descriptions and requirements as well as the General and any Special Provisions attached, very closely. **State Specifications call for much greater final construction requirements and standards than called for by either city or county government agencies.** Items such as sloped shoulders, mitered end sections, extended radii returns and grass sod are many times over-looked. Be sure to point these items out to those bidding for your business.

3.) Once a contractor is selected and you are ready to activate & Commence construction of the approved connection, you **must contact** The Permits Office here at Lake City Maintenance 48 hours in advance. Be aware that failure to call and activate your approved permit according To this permit provision is legal reason to suspend or revoke the approved Permit. Please take the time to call us to legally activate your permit so all will Go well.

4.) A Final Access Connection Inspection is Mandatory before the new access Can be utilized. We would highly recommend that before making any Final payments to your contractor that you call our office and set up the required FDOT Final Inspection. Contractors who are not willing to accept this pre-Contract agreement may not be worthy of your business. Be aware that you are Legally responsible for liability of the access connection as long as you have not Received a final passing inspection through this office.

5.) **A special note in regards to access permits issued on State Roadways Where the State has future plan for construction or where the State contractor is presently working**: When this is the case, you are Required to make 48 hour advance contact both to our office and the Lake City Construction Office before starting actual construction on your approved access Permit. Please phone 961-7050 to notify them of your intentions, tell them the State highway number on which you are permitted and be specific about your Permitted location Mile Post and permit number. If you decide to activate your Permit and start construction during the on-going FDOT Project and you elect to Hire a contractor other than the on-site FDOT Project contractor, and you must Obtain legal permission from the on-site project supervisor before commencing. All contactors must complete all permitted construction, with a passing FDOT Permits Office inspection within 30 days of the first day of driveway construction. Failure to abide by this permit provision will automatically require the removal of The permitted connection by the State FDOT or On-site Contractor's forces. Neither the FDOT nor the FDOT's on-site project contractor is under any Obligation to construct or complete you're permitted connection unless prior legal Written agreements have been entered into by both parties.

6.) **Special Note about permit validation periods:** Your newly issued permit is Valid for a period of 1 year from the date of original signature from the permits Office, however, as a special provision of this permit, you only have 30 days of Total construction time once you activate the permit and start any type if Driveway construction upon the FDOT Right-of-Way.

No. 6 Continued: To explain this permit provision more clearly, let's say you Activated your new permit to start construction on the first day of the 2nd month of Your approved permit, then all work and the required final passing inspection must Be completed by the first day of the 3rd month (30 days later.) The other 10 months Are not valid after you have officially activated the permits construction Commencement starts date.

The same is true of whatever month you activate your permit. You must start construction in time to be completed within the 30 day period in which you activate the permit, (See Part 3, Permit Approval Section of Page 1 of 3 of the Driveway Connection Permit for All Categories Form No. 850-040-18). Once activated you have only 30 days in which to be completely finished and have received the required final passing FDOT inspection. In most all cases every driveway access permitted can be completed within this 30 day period. **THIS IS A VERY IMPORTANT PERMIT PROVISION, PLEASE READ CAREFULLY. IF YOU DO NOT UNDERSTAND THIS PROVISION YOU SHOULD CONTACT THE FDOT PERMITS OFFICE AND REQUEST FURTHER CLARIFICATION IMMEDIDEATELY UPON READING THESE PERMIT PROVISIONS.**

Important Notice about State and Local County Permits Offices:
If you are planning improvements to your property, please be aware that complete construction of the permitted State FDOT Access Connection with an official final passing FDOT inspection is required before we can release you back to the county government. Once the connection has passed State Inspection the County Government shall be officially notified at which time, you may make county application for property permit improvements.

Well there it is, if you follow the above suggestions both you and the Permits Office can expect all to be in order when the time comes for you to request the final driveway construction inspection. Remember that we here at the Permits Office are always available in case you have a question or problem, about your approved access permit. We also offer driveway layout assistance if requested, please call us!

Sincerely,

Neil E. Miles
Access Permits Coordinator

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

DRIVEWAY CONNECTION PERMIT FOR ALL CATEGORIES

PART 1: PERMIT INFORMATION

APPLICATION NUMBER: 2008-A-292-6

Permit Category: A Access Classification: 4

Project: RES. 14' ASP/CON DRIVEWAY WITH DOUBLE 30' T/R.

Permittee: MICHAEL W. ROBERTS

Section/Mile Post: 29090 / 8.854+- State Road: 247(N)

Section/Mile Post: N/A State Road: N/A

PART 2: PERMITTEE INFORMATION

Permittee Name: MICHAEL W. ROBERTS

Permittee Mailing Address: 657 SW CATHERINE LANE

City, State, Zip: LAKE CITY, FL.32025

Telephone: (386)755-9476

Engineer/Consultant/or Project Manager: N/A

Engineer responsible for construction inspection: N/A (NAME) (P.E. #)

Mailing Address: N/A

City, State, Zip: N/A

Telephone: N/A Mobile Phone N/A

PART 3: PERMIT APPROVAL

The above application has been reviewed and is hereby approved subject to all Provisions as attached.

Permit Number: 2008-A-292-6

Signature: Department of Transportation Title: PERMITS COORDINATOR

Department Representative's Printed Name NEIL E. MILES

Temporary Permit ○ YES ◉ NO (If temporary, this permit is only valid for 6 months)

Special provisions attached ◉ YES ○ NO

Date of Issuance: JAN 30 2008

If this is a normal (non-temporary) permit it authorizes construction for one year from the date of issuance. This can only be extended by the Department as specified in 14-96.007(6).

See following pages for General and Special Provisions

PART 4: GENERAL PROVISIONS

1. Notify the Department of Transportation Maintenance Office at least 48 hours in advance of starting proposed

   Phone: (386) 961-7180 , Attention: NEIL E. MILES, PERMITS COORDINATOR

2. A copy of the approved permit must be displayed in a prominent location in the immediate vicinity of the connection construction.

3. Comply with Rule 14-96.008(1), F.A.C., Disruption of Traffic.

4. Comply with Rule 14-96.008(7), F.A.C., on Utility Notification Requirements.

5. All work performed in the Department's right of way shall be done in accordance with the most current Department standards, specifications and the permit provisions.

6. The permittee shall not commence use of the connection prior to a final inspection and acceptance by the Department.

7. Comply with Rule 14-96.003(3)(a), F.A.C., Cost of Construction.

8. If a Significant Change of the permittee's land use, as defined in Section 335.182, Florida Statutes, occurs, the Permittee must contact the Department.

9. Medians may be added and median openings may be changed by the Department as part of a Construction Project or Safety Project. The provision for a median might change the operation of the connection to be for right turns only.

10. All conditions in NOTICE OF INTENT WILL APPLY unless specifically changed by the Department.

11. All approved connection(s) and turning movements are subject to the Department's continuing authority to modify such connection(s) or turning movements in order to protect safety and traffic operations on the state highway or State Highway System.

12. **Transportation Control Features and Devices in the State Right of Way.** Transportation control features and devices in the Department's right of way, including, but not limited to, traffic signals, medians, median openings, or any other transportation control features or devices in the state right of way, are operational and safety characteristics of the State Highway and are not means of access. The Department may install, remove or modify any present or future transportation control feature or device in the state right of way to make changes to promote safety in the right of way or efficient traffic operations on the highway.

13. The Permittee for him/herself, his/her heirs, his/her assigns and successors in interest, binds and is bound and obligated to save and hold the State of Florida, and the Department, its agents and employees harmless from any and all damages, claims, expense, or injuries arising out of any act, neglect, or omission by the applicant, his/her heirs, assigns and successors in interest that may occur by reason of this facility design, construction, maintenance, or continuing existence of the connection facility,except that the applicant shall not be liable under this provision for damages arising from the sole negligence of the Department.

14. The Permittee shall be responsible for determining and notify all other users of the right of way.

15. Starting work on the State Right of Way means that I am accepting all conditions on the Permit.

PART 5: SPECIAL PROVISIONS

NON-CONFORMING CONNECTIONS: ◉ YES ○ NO

If this is a non-conforming connection permit, as defined in Rule Chapters 14-96 and 14-97, then the following shall be a part of this permit.

1. The non-conforming connection(s) described in this permit is (are) not permitted for traffic volumes exceeding the Permit Category on page 1 of this permit, or as specified in "Other Special Provisions" below.

2. All non-conforming connections will be subject to closure or relocation when reasonable access becomes available in the future.

OTHER SPECIAL PROVISIONS:

REFER TO APPROVED ACCESS PERMIT, GENERAL AND SPECIAL PROVISION SHEET AND THE LEGAL ATTACHED COVER LETTER FOR OFFICIAL DRIVEWAY CONSTRUCTION AND SAFETY SPECIFICATION, AND FDOT APPROVED SITE-PLAN FOR ANY ADDITIONAL INFORMATION NEEDED TO COMPLETE DRIVEWAYS. ALL WORK APPROVED HEREIN UNDER THIS PLAN SHALL BE ACCORDING TO THE STATE FDOT'S MOST CURRENT ROADWAY AND CONSTRUCTION SPECIFICATION AT THE TIME OF ACTUAL CONSTRUCTION AND PERMIT ACTIVATION. UPON ACTIVATION THE PERMIT THE PERMITTEE HAVE (30 DAYS) TO COMPLETE ALL PHASES OF PERMITTED PROJECT. PERMITTEE SHALL ADHERE TO THE FINAL APPROVED SITE-PLAN DATED JAN 3 0 2008 . THIS PERMIT IS FOR: MICHAEL W. ROBERTS RES D/W.PERMITTEE SHALL NOTIFY THE FDOT PERMITS DEPT FOR PRE-CONSTRUCTION MEETING (BEFORE) ANY WORK ON THE FDOT'S R.O.W. PROJECT CONSIST OF:14' ASP/ CON D/W W/D 30' T/R. ETC. WHILE WORKING ON THE FDOT'S R.O.W. APPROPRIATE (MOT) SHALL BE IN PLACE CONES, BARACADES, SIGNS, ETC. (ALL) WORKERS WITH IN 15' FEET OF THE EDGE OF THE TRAVEL WAY SUPERVISORS, CREW MEMBERS AND ANY PERSONAL ON THE (FDOTS' R.O.W.) SHALL WEAR ANSI / ISEA CLASS 2 APPAREL (AT ALL TIMES).WORKERS OPERATING MACHINERY OR EQUIPMENT IN WHICH LOOSE CLOTHING COULD BECOME ENTANGLED, SHALL WEAR FITTED H/VISIBLE SAFETY APPAREL. OTHERS WISE COULD RESULT IN (FDOT SAFETY CODE VIOLATION).

PART 6: APPEAL PROCEDURES

You may petition for an administrative hearing pursuant to sections 120.569 and 120.57, Florida Statutes. If you dispute the facts stated in the foregoing Notice of Intended Department Action (hereinafter Notice), you may petition for a formal administrative hearing pursuant to section 120.57 (1), Florida Statutes. If you agree with the facts stated in the Notice, you may petition for an informal administrative hearing pursuant to section 120. 57(2), Florida Statutes. You must file the petition with:

Clerk of Agency Proceedings
Department of Transportation
Haydon Burns Building
605 Suwannee Street, M.S. 58
Tallahassee, Florida 32399-0458

The petition for an administrative hearing must conform to the requirements of Rule 28-106.201(2) or Rule 28-106.301(2), Florida Administrative Code, and be filed with the Clerk of Agency Proceedings by 5:00 p.m. no later than 21 days after you received the Notice. The petition must include a copy of the Notice, be legible, on 8 1/2 by 11 inch white paper, and contain:

1. Your name, address, telephone number, any Department of Transportation identifying number on the Notice, if known, the name and identification number of each agency affected, if known, and the name, address, and telephone number of your representative, if any, which shall be the address for service purposes during the course of the proceeding.
2. An explanation of how your substantial interests will be affected by the action described in the Notice;
3. A statement of when and how you received the Notice;
4. A statement of all disputed issues of material fact. If there are none, you must so indicate;
5. A concise statement of the ultimate facts alleged, including the specific facts you contend warrant reversal or modification of the agency's proposed action, as well as an explanation of how the alleged facts relate to the specific rules and statutes you contend require reversal or modification of the agency's proposed action;
6. A statement of the relief sought, stating precisely the desired action you wish the agency to take in respect to the agency's proposed action.

If there are disputed issues of material fact a formal hearing will be held, where you may present evidence and argument on all issues involved and conduct cross-examination. If there are no disputed issues of material fact an informal hearing will be held, where you may present evidence or a written statement for consideration by the Department.

Mediation, pursuant to section 120.573, Florida Statutes, may be available if agreed to by all parties, and on such terms as may be agreed upon by all parties. The right to an adminstrative hearing is not affected when mediation does not result in a settlement.

Your petition for an administrative hearing shall be dismissed if it is not in substantial compliance with the above requirements of Rule 28-106.201(2) or Rule 28-106.301(2), Florida Administrative Code. If you fail to timely file your petition in accordance with the above requirements, you will have waived your right to have the intended action reviewed pursuant to chapter 120, Florida Statutes, and the action set forth in the Notice shall be conclusive and final.

FLORIDA DEPARTMENT OF TRANSPORTATION

CHARLIE CRIST
GOVERNOR

STEPHANIE KOPELOUSOS
SECRETARY

PERMITTEE: MICHAEL W. ROBERTS. SEC NO:29090 PERMIT CAT: A

M.P. 21.391+- STATE RD: 247 (N)

PROJ. DESCIPTION: 14' ASP/ CON D/W W/D 30' T/R.

PERMIT NO: 08-A-292-6

Asst. Maintenance Engineer or Permits Coordinator Approval

NEIL E. MILES, PERMITS COORDINATOR

THE FOLLOWING ARE ADDITIONAL SPECIAL PERMIT PROVISIONS THAT ARE A LEGAL PART OF THIS PERMIT & DO APPLY TO THE ABOVE REFERENCED PERMIT, IF SO MARKED MUST BE COMPLYED WITH IN ADDITIONAL TO THE GENERAL PROVISIONS.

1. XXX All portions of the FDOT right-of-way disturbed during construction under this permit shall be mulched seeded and /or 2 feet of grass sod placed adjacent to the driving lane, or as called for under the approved permit & per FDOT specifications.
2. XXX Permitted shall restore wildflowers disturbed during permitted construction with new seed to be (amount and & method) determined by Mr. Dick Bush, District Landscaping Engineer. Seed shall be delivered to Lake City Maintenance, Permits Office before commencement of permitted placement.
3. XXX The Permitted will contact the appropriate city, county, state government agency; a minimum of forty-eight (48) hours in advance of starting excavation within the area of any signalized intersection.
4. XXX the Permitted can be required to physically relocate (move), as so indicated under this permit at a future date, due to proposed future or on-going FDOT roadway construction planned within the limits of the permitted area.
5. XXX existing utilities may be located within the construction area. Prior to permit approval, permitted shall locate and notify all utilities within the proposed limits of construction and or permitted area and obtain detailed information from the utility owners as to possible conflicts between utilities and permit tee's work. Permitted shall be responsible for pre & post permit coordination, and all adjustments and shall be solely responsible for resolving any conflicts of utilities, either before or during or after the final permitting. The Permitted shall be solely responsible for any and all damages to existing utilities and/or damage to third parties caused by interference with or damage to existing utilities. The Permitted shall show positive proof that all utility owners with existing interest in the area permitted, have been previously contacted in advance of final permit approval.
6. XXX No business is to be done on FDOT right-of-ways, if vehicles are to be serviced on roadside with pumps, Pump islands must be located at least twelve (12) feet from right-of-way line.
7. XXX Driveway permits are granted to permit access to abutting property only. Parking on right-of-way may be restricted or prohibited.
8. XXX the erection of signs on or overhanging the right-of-way of state roads is not permitted. The connection of any type of subsurface drainage to FDOT storm drains or ditches is prohibited unless by permit or as shown in the general or special provisions of the referenced permit.
9. XXX All Construction and/or Maintenance on the Department's right-of-way shall conform to Federal Manual on Uniform Traffic Control Devices (MUTCD), the Department's most current manual of the Roadway and Traffic Design Standards Specifications for Road and Bridge Construction.
10. XXX Pre and Final Inspections are required by FDOT Permits Office and the assigned inspector.
11. XXX a pre-construction review of the construction planned under the permit shall be mandatory. The Permit tee shall make contact with the Lake City, Permits Office at (904) 961-7180 or 961-7193, a minimum of 48 hours in advance of the Permit tee's planned start date so as to arrange a mutually time to meet. Failure by the Permit tee to meet this requirement can be reason for revocation of the approved permit.
12. XXX If proposed permitted work limits are within a State Roadway Construction Area that is proposed or underway then the permit tee shall schedule commencement date and all planned work under this permit with the State Foot's contract representative in charge of on-site project operational responsibilities.
13. XXX Final approved permit shall adhere to the signed and sealed plans, with no plan substitutions once approved.

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

DRIVEWAY/CONNECTION APPLICATION
CATEGORY A

(INDIVIDUAL HOMES, DUPLEXES OR OTHER USES LESS THAN 20 TRIPS/DAY TOTAL)

850-040-14
SYSTEMS PLANNING
09/02

OFFICE USE ONLY

Application Number: 08-A-292-6

Category: A

Section Road Number & Mile Post: 29090 / 8.854+-

Accepted By: Dale L.Cray
FDOT STAFF (TYPE OR PRINT)

Date: 1-29-2008

APPLICANT COMPLETE REMAINDER OF FORM

PART I: APPLICANT INFORMATION (Please type or print)

APPLICANT: Michael W. Roberts

Mailing Address: 657 SW Catherine Lane

City, State, Zip: Lake City, Fl.32025

Telephone: (386)755-9476

Physical Address of Site (if different):

Attach Map & Drawing If Necessary

PROPERTY OWNER: (if different from above) same AS Above

Mailing Address:

City, State, Zip:

Telephone:

PART 2: NOTICE TO APPLICANT

Proposed traffic control features and devices in the right of way, such as median openings and other traffic control devices, are not part of the connection(s) to be authorized by a connection permit. The Department reserves the right to change these features in the future in order to promote safety in the right of way or efficient traffic operations on the highway. Expenditure by the applicant of monies for installation or maintenance of such features shall not create any interest in the features or their maintenance.

PART 3: CERTIFICATION AND SIGNATURE

I certify that I am familiar with the information contained in this application and that to the best of my knowledge and belief such information is true, complete and accurate. I will not begin work on the connection until I receive my Permit and I understand all the conditions of the Permit. When I begin work on the connection I am accepting all conditions listed in my Permit.

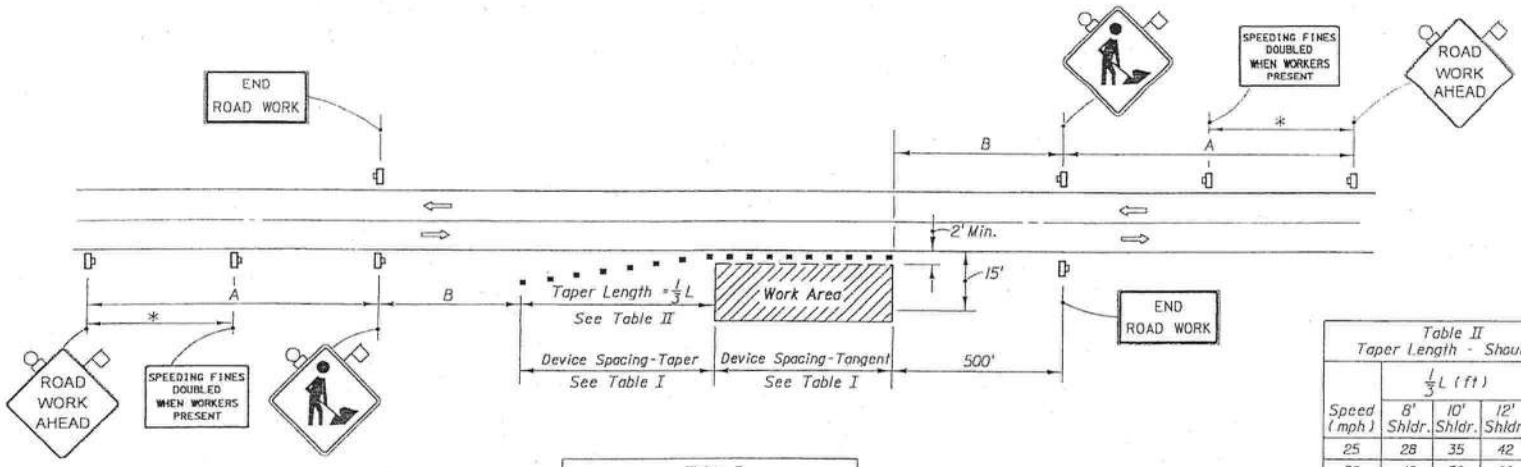
Signed: Mike Roberts
(Applicant)

Date: 1/29/08

Printed Name: Mike Roberts

ACCESS CONNECTION CONSTRUCTION DESCRIPTION

PERMITTEE: MICHAEL W. ROBERTS/ FDOT ACCESS PERMIT NO. 2008-A-292-6/ CURRENT ADDRESS: 657 SW CATHERINE LANE ,LAKE CITY FL. 32025/ SECTION NO. 29090/ M.P. 8.854 +- / PERMITTED CO: COL.03084-008 , COL COUNTY, FL/ **PROPOSED: 14' FT WIDE**, RESIDENTIAL ASPHALT CONCRETE DRIVEWAY WITH DOUBLE **30'** TURNING RADII,**. IF CONCRETE MIN OF 6" TO R.O.W LINE 2500 PSI (NO-FIBER). THE NEW DRIVEWAY SHALL REQUIRE 46' LF OF 14 X 23 (ELLIPTICAL) PIPE** (NOTE: IF EXISTING, NO PORTION OF THE ASPHALT PAVED SHOULDER CAN BE USED AS PART OF EITHER TURNOUT RADII. **THE NEW RESIDENTIAL CONNECTION CENTERLINE SHALL BE CONSTRUCTED AT OR APPROXIMATELY **91** LF S OF THE NW PROPERTY CORNER FOR THE ABOVE REFERENCED PROPERTY. ** THE DRIVEWAY SURFACE WILL REGUIRE A MINIMUM OF **6"** INCHES OF CRUSHED LIMEROCK MATERIAL TO FDOT R.O.W LINE. (NO MORE THAN 1 INCH DIA.) COVER OVER ALL TRAVEL SURFACES (INCLUDING DRIVEWAY WIDTH AND BOTH TURNING RADII.) ** THE NEW CONNECTION SHALL ALSO REQUIRE TWO (**2**) FULL **5** FOOT WIDE OR GREATER SLOPED AND STABILIZED EARTH SHOULDERS, (**1:4** GRADE REQUIRED) THOUGHOUT THE ENTIRE TURNING RADII TO THE R/W LINE (NOTE THAT THE SLOPED SHOULDER MUST BE STABILIZED IN PLACE.) ** IF LESS THAN 1:4, BOTH SLOPED AND RADIUSED EARTH SHOULDERS MAY BE SEEDED AND STRAW MULCHED WITH COPIOUS AMOUNTS OF HULLED COASTAL BERMUDA GRASS SEED & BROWN TOP MILLET SEED IN A **40/60 %** MIX. **DRIVEWAYS WITH SLOPES 1:4 OR STEEPER MUST BE GRASSING SODDED.** ** MAINTENANCE OF TRAFFIC SAFETY REQUIREMENTS: A CORRECT MAINTENANCE OF TRAFFIC PLAN SHALL BE REQUIRED TO BE IN PLACE BEFORE ANY TYPE OF WORK CAN COMMENCE UNDER THIS APPROVED PERMIT. ****PERMIT ACTIVATION AND NOTICE INFORMATION: THERE IS A MINIMUM 48-HOUR ADVANCED NOTICE OF PERMIT ACTIVATION REQUIRED FROM THE PERMITTEE TO THE LOCAL FDOT PERMITS OFFICE BEFORE ANY WORK CAN COMMENCE UPON FDOT RIGHT-OF-WAY, CALL 386-961-7180 TO COMPLETE THIS PERMIT PROVISION. A FINAL (PASSING) DRIVEWAY INSPECTION IS MANDATORY BEFORE THE CONNECTION CAN BE UTILIZED, CONTACT FDOT PERMITS A MINIMUM OF 48 HOURS IN ADVANCE OF PROPOSED UTILIZATION DATE FOR FINAL INSPECTION. FAILURE TO CONTACT THE FDOT PERMITS OFFICE CAN RESULT IN PERMIT SUSPENSION OR REMOVAL, CALL 386-961-7180 TO REQUEST THE REQUIRED FINAL INSPECTION. ONCE STARTED YOU HAVE ONLY 30 CONTINUOUS DAYS TO COMPLETE THE DRIVEWAY CONNECTION, REFER TO ITEM NUMBER 6 OF THE ATTACHED COVER LETTER.**



DISTANCE BETWEEN SIGNS		
Speed	Spacing (ft)	
	A	B
40 mph or less	200	200
45 mph	350	350
50 mph or greater	500	500

* 500' beyond the ROAD WORK AHEAD sign or midway between signs whichever is less.

Table I
Device Spacing

Speed (mph)	Max. Distance Between Devices (ft)			
	Cones or Tubular Markers		Type I or Type II Barricades or Vertical Panels or Drums	
	Taper	Tangent	Taper	Tangent
25	25	50	25	50
30 to 45	25	50	30	50
50 to 70	25	50	50	100

Table II
Taper Length - Shoulder

Speed (mph)	1/3 L (ft)			Notes
	8' Shldr.	10' Shldr.	12' Shldr.	
25	28	35	42	$L = \frac{WS^2}{60}$
30	40	50	60	
35	55	68	82	
40	72	90	107	
45	120	150	180	L = WS
50	133	167	200	
55	147	183	220	
60	160	200	240	
65	173	217	260	
70	187	233	280	

8' minimum shoulder width.

1/3 L = Length of shoulder taper in feet

W = Width of total shoulder in feet (combined paved and unpaved width)

S = Posted speed limit (mph)

SYMBOLS

- Work Area
- Sign With 18" x 18" (Min.) Orange Flag And Type B Light
- Channelizing Device (See Index No. 600)
- Work Zone Sign
- Lane Identification + Direction of Traffic

GENERAL NOTES

1. All vehicles, equipment, workers (except flaggers), and their activities are restricted to one side of the roadway.
2. When four or more work vehicles enter the through traffic lanes in a one hour period or less (excluding establishing and terminating the work area), the advanced FLAGGER sign shall be substituted for the WORKERS sign. For location of flaggers and FLAGGER signs, see Index No. 603.
3. WORKERS sign to be removed or fully covered when no work is being performed.
4. SHOULDER WORK sign may be used as an alternate to the WORKER symbol sign only on the side where the shoulder work is being performed.
5. When a side road intersects the highway within the TTC zone, additional TTC devices shall be placed in accordance with other applicable TCZ Indexes.
6. For general TCZ requirements and additional information refer to Index No. 600.

DURATION NOTES

1. Signs and channelizing devices may be omitted if all of the following conditions are met:
   a) Work operations are 60 minutes or less.
   b) Vehicles in the work area have high-intensity, rotating, flashing, oscillating, or strobe lights operating.

CONDITIONS

WHERE ANY VEHICLE, EQUIPMENT, WORKERS OR THEIR ACTIVITIES ENCROACH THE AREA CLOSER THAN 15' BUT NOT CLOSER THAN 2' TO THE EDGE OF TRAVEL WAY.

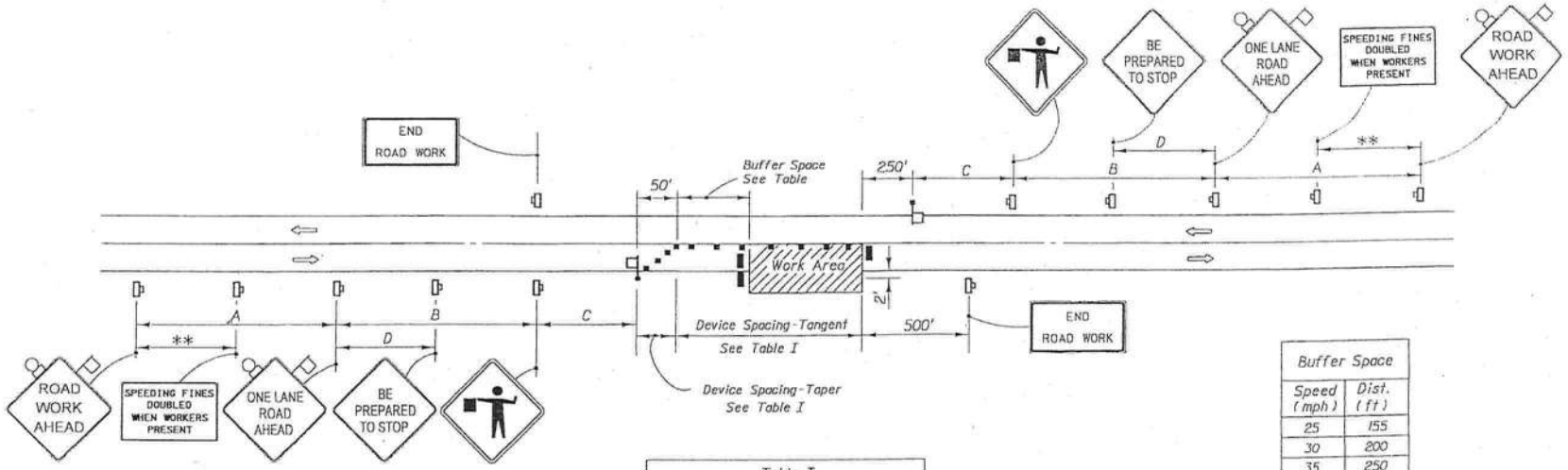
2006 FDOT Design Standards

TWO-LANE TWO-WAY, WORK ON SHOULDER

Last Revision 07/01/05

Sheet No. 1 of 1

Index No. 602



DISTANCE BETWEEN SIGNS				
Speed	Spacing (ft)			
	A	B	C	D ***
40 mph or less	200	200	200	100
45 mph	350	350	350	175
50 mph	500	500	500	250
*55 mph or greater	2640	1640	1000	500

* The ROAD WORK 1 MILE sign may be used as an alternate to the ROAD WORK AHEAD sign.

** 500' beyond the ROAD WORK AHEAD sign or midway between signs whichever is less.

*** BE PREPARED TO STOP sign may be omitted for speeds of 45 MPH or less.

Table I Device Spacing				
Speed (mph)	Max. Distance Between Devices (ft)			
	Cones or Tubular Markers		Type I or Type II Barricades or Vertical Panels or Drums	
	Taper	Tangent	Taper	Tangent
25 to 45	20	50	20	50
50 to 70	20	50	20	100

Buffer Space	
Speed (mph)	Dist. (ft)
25	155
30	200
35	250
40	305
45	360
50	425
55	495
60	570
65	645
70	730

When Buffer Space cannot be attained due to geometric constraints, the greatest attainable length shall be used, but not less than 200 ft.

SYMBOLS

- Work Area
- Sign With 18" x 18" (Min.) Orange Flag And Type B Light
- Type I, Type II Or Type III Barricade Or Vertical Panel Or Drum
- Channelizing Device (See Index No. 600)
- Work Zone Sign
- Flagger
- Lane Identification + Direction of Traffic

GENERAL NOTES

1. Work operations shall be confined to one traffic lane, leaving the opposite lane open to traffic.
2. All vehicles, equipment, workers (except flaggers), and their activities are restricted to one side of the roadway.
3. Additional one-way control may be effected by the following means:
   (1) Flag-carrying vehicle; (2) Official vehicle; (3) Pilot vehicles; (4) Traffic signals.

   When flaggers are the sole means of one-way control the flaggers shall be in sight of each other or in direct communication at all times.
4. The ONE-LANE ROAD signs are to be fully covered and the FLAGGER signs either removed or fully covered when no work is being performed and the highway is open to two-way traffic.
5. When a side road intersects the highway within the TTC zone, additional TTC devices shall be placed in accordance with other applicable TCZ Indexes.
6. The two channelizing devices directly in front of the work area and the one channelizing device directly at the end of the work area may be omitted provided vehicles in the work area have high-intensity rotating, flashing, oscillating, or strobe lights operating.
7. For general TCZ requirements and additional information, refer to Index No. 600.

DURATION NOTES

1. ROAD WORK AHEAD and the BE PREPARED TO STOP signs may be omitted if all of the following conditions are met:
   a) Work operations are 60 minutes or less.
   b) Speed limit is 45 mph or less.
   c) No sight obstructions to vehicles approaching the work area for a distance equal to the buffer space.
   d) Vehicles in the work area have high-intensity, rotating, flashing, oscillating, or strobe lights operating.
   e) Volume and complexity of the roadway has been considered.

CONDITIONS

WHERE ANY VEHICLE, EQUIPMENT, WORKERS OR THEIR ACTIVITIES ENCROACH THE AREA BETWEEN THE CENTERLINE AND A LINE 2' OUTSIDE THE EDGE OF TRAVEL WAY.

2006 FDOT Design Standards

TWO-LANE TWO-WAY, WORK WITHIN THE TRAVEL WAY

Last Revision 07/01/05 | Sheet No. 1 of 1 | Index No. 603

RECORD OF SALE OF GOODS OR SERVICE/RECEIPT TRANSMITTAL

Form 350-080-32
Comptroller
General Accounting

DISTRICT OFFICE 2/Maintenance **CUSTODIAN NO.** _____

SOLD TO:

NAME: Michael W. Roberts

ADDRESS: 657 SW Catherine Lane

Lake City, FL 32025

DELIVERY:

☐ PICK UP: _____ RECEIVED BY (SIGNATURE)

☐ SHIP TO:

SOLD TO ADDRESS: _____

CONTACT: _____

TELEPHONE NO. _____

86291

PAYMENT METHOD

☒ INDIVIDUAL SALE: AMOUNT OF CHECK $ 50.00 AND / OR AMOUNT OF CASH $ _____

☐ BATCH TRANSMITTAL: AMOUNT OF CHECKS $ _____ AND / OR AMOUNT OF CASH $ _____

☐ SALE ON ACCOUNT: ACCOUNT # _____ (Send copy to Accounts Receivable - MS 42)

DESCRIPTION OF SALE(S)

DESCRIPTION OF SALE	UNIT PRICE	SUBTOTAL	SALES TAX	DISCRET. TAX	TOTAL
Connection Fee	50.00				50.00
P# 08-A-292-6					
GRAND TOTAL					50.00

TRANSACTION AUTHORIZED BY:

Rana Crawford — PRINT NAME

Rana Crawford — SIGNATURE

386-961-7180 — TELEPHONE NO.

1-29-08 — DATE

IF SALE ON ACCOUNT PERSON AUTHORIZING SALE

_____ PRINT NAME

_____ TELEPHONE NO.

_____ SIGNATURE

_____ DATE

COST DISTRIBUTION

ORGANIZATION CODE	EO	OBJECT	AMOUNT	FINANCIAL PROJ. (11 DIGITS)	B	EOB
5-910200000	HM	004029	50.00	2139401A102	1	393

inal, Cashier's Office, or Accounts Receivable (if sale on Account) MS 42 Copies - (1) Customer (1) Financial Reporting, MS 42 (1) Retained in Custodial Office

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

RECEIPT OF CONNECTION APPLICATION AND FEE (OR WAIVER OF FEE)

850-040-16
SYSTEMS PLANNING
06/06

IMPORTANT NOTE: Even though your application has been accepted, it may not be complete. We will contact you if more information is needed.

(1) APPLICATION NUMBER: 08-A-292-6

APPLICANT:

(2) Name/Address MICHAEL W. ROBERTS
657 SW CATHERINE LANE
LAKE CITY, FL.32025

(3) Project Name: MICHAEL W. ROBERTS (RES. DRIVEWAY)

(4) Fee

		VEHICLES PER DAY	FEE
◉	Category A	1-20	$50.00
○	Category B	21-600	$250.00
○	Category C	601-1,200	$1,000.00
○	Category D	1,201-4,000	$2,000.00
○	Category E	4,001-10,000	$3,000.00
○	Category F	10,001-30,000	$4,000.00
○	Category G	30,001 +	$5,000.00
○	Temporary		$250.00
○	Safety		NO FEE
○	Government Entity		NO FEE

RECEIPT NO.: 86291

(5) Application Fee Collected $ 50.00

Payment Type:
Money Order ○
Check ◉ Check Number 2350
Cash ○

(6) Fee Collected By

Name DALE L. CRAY (PRINT)
Signature D L C
Date: 1-29-2008 District 2 Unit 292

(7) Receipt Given Back to Applicant Via

○ Hand Delivery ○ Mail ○ Courier Service ○ Other

Applicant (or Agent) Signature (if available) _____

This form bears your application number and serves as your receipt.

(8) If fee is waived, give justification below or on separate sheet.

FOR AGENCY USE ONLY - ATTACH COPY OF CHECK ON THE NEXT PAGE

Make Checks payable to: State of Florida Department of Transportation

This Instrument Prepared by & return to:
Name: Brenda Styons, an employee of TITLE OFFICES, LLC
Address: 1089 SW MAIN BLVD. LAKE CITY, FLORIDA 32025
File No. 05Y-03042BS

Parcel I.D. #: 03084-008

SPACE ABOVE THIS LINE FOR PROCESSING DATA

Inst:2005007220 Date:03/30/2005 Time:09:05
Doc Stamp-Deed : 133.00
DC,P.DeWitt Cason,Columbia County B:1041 P:2507

SPACE ABOVE THIS LINE FOR RECORDING DATA

THIS WARRANTY DEED Made the **23rd** day of **March**, A.D. **2005**, by

JOSEPH F BALL, JR. and **CHRISTINE M. BALL**, HIS WIFE, hereinafter called the grantors, to

MICHAEL W. ROBERTS, SINGLE whose post office address is

657 SW CATHERINE LN, LAKE CITY, FL 32025, hereinafter called the grantee:

(Wherever used herein the terms "grantors" and "grantee" include all the parties to this instrument, singular and plural, the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporations, wherever the context so admits or requires.)

Witnesseth: That the grantors, for and in consideration of the sum of $10.00 and other valuable consideration, receipt whereof is hereby acknowledged, do hereby grant, bargain, sell, alien, remise, release, convey and confirm unto the grantee all that certain land situate in **Columbia County, State of FLORIDA**, viz:

Lot 8, SADDLE RIDGE, according to the map or plat thereof as recorded in Plat Book 5, Page 67, of the Public Records of Columbia County, FLORIDA.

Together with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

To Have and to Hold the same in fee simple forever.

And the grantors hereby covenant with said grantee that they are lawfully seized of said land in fee simple; that they have good right and lawful authority to sell and convey said land, and hereby fully warrant the title to said land and will defend the same against the lawful claims of all persons whomsoever, and that said land is free of all encumbrances, except taxes accruing subsequent to December 31, 2004.

In Witness Whereof, the said grantors have signed and sealed these presents, the day and year first above written.

Signed, sealed and delivered in the presence of:

Barbara Rose
Witness Signature
BARBARA ROSE
Printed Name

Bonita Hadwin
Witness Signature
BONITA HADWIN
Printed Name

JOSEPH F BALL, JR. L.S.
Address:
248 SW MELON CT, LAKE CITY, FL. 32024

CHRISTINE M. BALL L.S.
Address:
248 SW MELON CT, LAKE CITY, FL. 32024

STATE OF FLORIDA
COUNTY OF COLUMBIA

Columbia County Property Appraiser

DB Last Updated: 12/29/2006

2007 Proposed Values

Parcel: 21-4S-16-03084-008

Tax Record | Property Card | Interactive GIS Map | Print

Owner & Property Info

<< Prev Search Result: 10 of 15 Next >>

Owner's Name	ROBERTS MICHAEL W		
Site Address			
Mailing Address	657 SW CATHERINE LANE LAKE CITY, FL 32025		
Use Desc. (code)	VACANT (000000)		
Neighborhood	21416.02	Tax District	3
UD Codes	MKTA06	Market Area	06
Total Land Area	0.520 ACRES		
Description	LOT 8 SADDLE RIDGE S/D. ORB 715-498, WD 1027-299, WD 1041-2507.		

GIS Aerial

Property & Assessment Values

Mkt Land Value	cnt: (1)	$22,500.00
Ag Land Value	cnt: (0)	$0.00
Building Value	cnt: (0)	$0.00
XFOB Value	cnt: (0)	$0.00
Total Appraised Value		$22,500.00

Just Value	$22,500.00
Class Value	$0.00
Assessed Value	$22,500.00
Exempt Value	$0.00
Total Taxable Value	$22,500.00

Sales History

Sale Date	Book/Page	Inst. Type	Sale Vlmp	Sale Qual	Sale RCode	Sale Price
3/23/2005	1041/2507	WD	V	Q		$19,000.00
9/29/2004	1027/299	WD	V	Q		$10,800.00
4/6/1990	715/498	WD	V	U		$5,000.00

Building Characteristics

Bldg Item	Bldg Desc	Year Blt	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value
NONE						

Extra Features & Out Buildings

Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
NONE						

Land Breakdown

Lnd Code	Desc	Units	Adjustments	Eff Rate	Lnd Value
000000	VAC RES (MKT)	1.000 LT - (.520AC)	1.00/1.00/1.00/1.00	$22,500.00	$22,500.00

CERTIFICATE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 21-4S-16-03084-008 **Building permit No.** 000027011

Use Classification SFD,UTILITY **Fire:** 6.42

Permit Holder WILLIAM WOOD **Waste:** 16.75

Owner of Building MIKE ROBERTS **Total:** 23.17

Location: 5429 SW SR 247, LAKE CITY, FL

Date: 09/17/2009

Wayne A. Russ

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)

Name:	MICHAEL W. ROBERTS
County:	COL. CO.
S.R. NO:	247
Sec. NO:	29090
Permit NO.	08-A-292-6

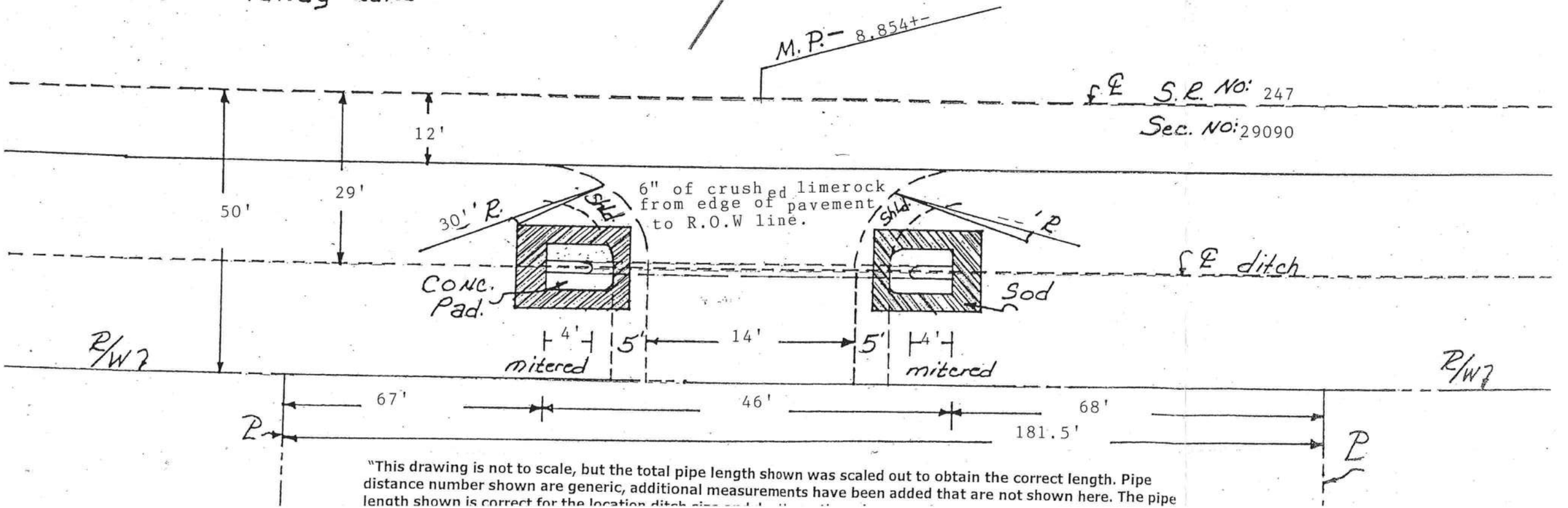
PROCESSED, LAKE CITY MAINT.
FDOT
PERMITS
PERMITS DEPT.

48 HOURS BEFORE YOU DIG
CALL SUNSHINE
1-800-432-4770
IT'S THE LAW IN FLORIDA

FDOT APPROVED
Date: JAN 3 0 2007

THIS PERMIT IS FOR A 14'EARTH LIMEROCK DRIVEWAY WITH DOUBLE 30' TURN RADII.DRIVEWAY SHALL HAVE 6" OF CRUSHED LIMEROCK FROM EDGE OF PAVEMENT TO R.O.W LINE.CONCRETE MITERED PADS ON 1:4 SLOPE WITH 1'OF SOD AROUND MITERED ENDS. REFER TO THE ATTACHED SHEET FOR CONSTRUCTION DETIALS.
IF PERMITTEE USE CONCRETE IT SHALL 6"PSI 2500(NO-FIBER) FROM EDGE OF PAVEMENT TO (R.O.W. LINE).DRIVEWAY WILL REQUIRE46' OF 14 x 23(ELLIPTICAL PIPE).

Residential Driveway Rural



"This drawing is not to scale, but the total pipe length shown was scaled out to obtain the correct length. Pipe distance number shown are generic, additional measurements have been added that are not shown here. The pipe length shown is correct for the location ditch size