

DATE 03/16/2007

Columbia County Building Permit

PERMIT

This Permit Expires One Year From the Date of Issue

000025627

APPLICANT DANIEL A. HINTON PHONE 352.318.3411
ADDRESS POB 2214 HIGH SPRINGS FL 32655
OWNER DANIEL A.& CAMMEY HINTON PHONE 352.318.3411
ADDRESS 1087 SW MAPLETON STREET FT. WHITE FL 32038
CONTRACTOR DANIEL A. & COURTNEY HINTON PHONE 352.318.3411
LOCATION OF PROPERTY 47-S TO US 27 TO C-138,S TO WOODLAND,TL TO GATED COMMUNITY,
(JUST BEFORE GATE, TAKE L & ADDRESS MARKED 1087.

TYPE DEVELOPMENT SFD/UTILITY ESTIMATED COST OF CONSTRUCTION 96400.00
HEATED FLOOR AREA 1928.00 TOTAL AREA 2852.00 HEIGHT 25.00 STORIES 1
FOUNDATION CONC WALLS FRAMED ROOF PITCH 9'12 FLOOR CONC
LAND USE & ZONING A-3 MAX. HEIGHT 35
Minimum Set Back Requirments: STREET-FRONT 30.00 REAR 25.00 SIDE 25.00
NO. EX.D.U. 0 FLOOD ZONE XPS DEVELOPMENT PERMIT NO.

PARCEL ID 30-7S-17-10058-653 SUBDIVISION SANTA FE RIVER PLANTATIONS
LOT 63 BLOCK PHASE UNIT TOTAL ACRES 1.39

000001352

Culvert Permit No. Culvert Waiver Contractor's License Number Applicant/Owner/Contractor
WAIVER 07-00161N BLK JTH N
Driveway Connection Septic Tank Number LU & Zoning checked by Approved for Issuance New Resident

COMMENTS: 1 FOOT ABOVE ROAD. SECTION 2.3.1 LEGAL NON-CONFORMING LOT OF RECORD.

Check # or Cash 1015

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power Foundation Monolithic
date/app. by date/app. by date/app. by
Under slab rough-in plumbing Slab Sheathing/Nailing
date/app. by date/app. by date/app. by
Framing Rough-in plumbing above slab and below wood floor
date/app. by date/app. by
Electrical rough-in Heat & Air Duct Peri. beam (Lintel)
date/app. by date/app. by date/app. by
Permanent power C.O. Final Culvert
date/app. by date/app. by date/app. by
M/H tie downs, blocking, electricity and plumbing Pool
date/app. by date/app. by
Reconnection Pump pole Utility Pole
date/app. by date/app. by date/app. by
M/H Pole Travel Trailer Re-roof
date/app. by date/app. by date/app. by

BUILDING PERMIT FEE \$ 485.00 CERTIFICATION FEE \$ 14.26 SURCHARGE FEE \$ 14.26
MISC. FEES \$ 0.00 ZONING CERT. FEE \$ 50.00 FIRE FEE \$ 0.00 WASTE FEE \$
FLOOD DEVELOPMENT FEE \$ FLOOD ZONE FEE \$ 25.00 CULVERT FEE \$ TOTAL FEE 588.52
INSPECTORS OFFICE CLERKS OFFICE

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE. PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Columbia County Building Permit Application

For Office Use Only Application # 070 3-19 Date Received 3/7 By JW Permit # 1352/25627
 Application Approved by - Zoning Official B2K Date 16.03.07 Plans Examiner OK JH Date 3-11-07
 Flood Zone X per supervisor Development Permit N/A Zoning A-3 Land Use Plan Map Category A-3
 Comments Section 2.3.1 Legal Non-conforming Lot of Record: - 1014-W/ (-1015) -

☒ NOC ☐ EH ☐ Deed or PA ☒ Site Plan ☒ State Road Info ☒ Parent Parcel # ☒ Development Permit

Name Authorized Person Signing Permit Daniel Adam Hinton CALL TO FAX 386.454.7012 HAVE TALKED ON
 Address P.O. Box 2214 High Springs, FL 32655 Phone 352-318-3411
 Owners Name Daniel Adam & Cammy B. Hinton Phone 352-318-3411 (780) 454-7012
 911 Address 1087 sw Mapleton St. H. WHITE, FL 32038
 Contractors Name Owner Builder Phone _____
 Address _____

Fee Simple Owner Name & Address _____
 Bonding Co. Name & Address Zurich Assurance Company of America 2495 Main Street Suite 209 Buffalo, NY 14214
 Architect/Engineer Name & Address _____

Mortgage Lenders Name & Address Florida Rural Rehabilitation Corporation
 Circle the correct power company - FL Power & Light Progress Energy - Clay Elec. - Suwannee Valley Elec. - Progressive Energy

Property ID Number 30-75-17-10058-653 Estimated Cost of Construction 120,000
 Subdivision Name Santa Fe River Plantations Lot 63 Block _____ Unit _____ Phase _____

Driving Directions Take Mapleton St off Hwy 27 just west of Santa Fe River. Go .9 miles down Mapleton to 1087 on R side of Road. "Alternate route" Take CR 138 south from Hwy 27 to Woodland on (D) Take woodland to the gated community. Just before gate take Left to 1087.

Type of Construction Residential - Concrete Block Number of Existing Dwellings on Property 0
 Total Acreage 1.39 Lot Size _____ Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 135 Side 60 Side 78 Rear 153
 Total Building Height 25' Number of Stories 2 Heated Floor Area 1928 Roof Pitch 9/12
TOTAL 2,852

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

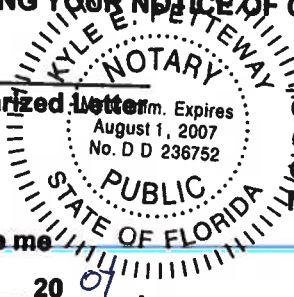
WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Daniel Adam Hinton
 Owner Builder or Authorized Person by Notarized Letter _____

STATE OF FLORIDA
 COUNTY OF COLUMBIA ALACHUA

Sworn to (or affirmed) and subscribed before me
 this 6th day of March 2007

Personally known ☒ or Produced Identification _____



Contractor Signature _____
 Contractors License Number _____
 Competency Card Number _____
 NOTARY STAMP/SEAL

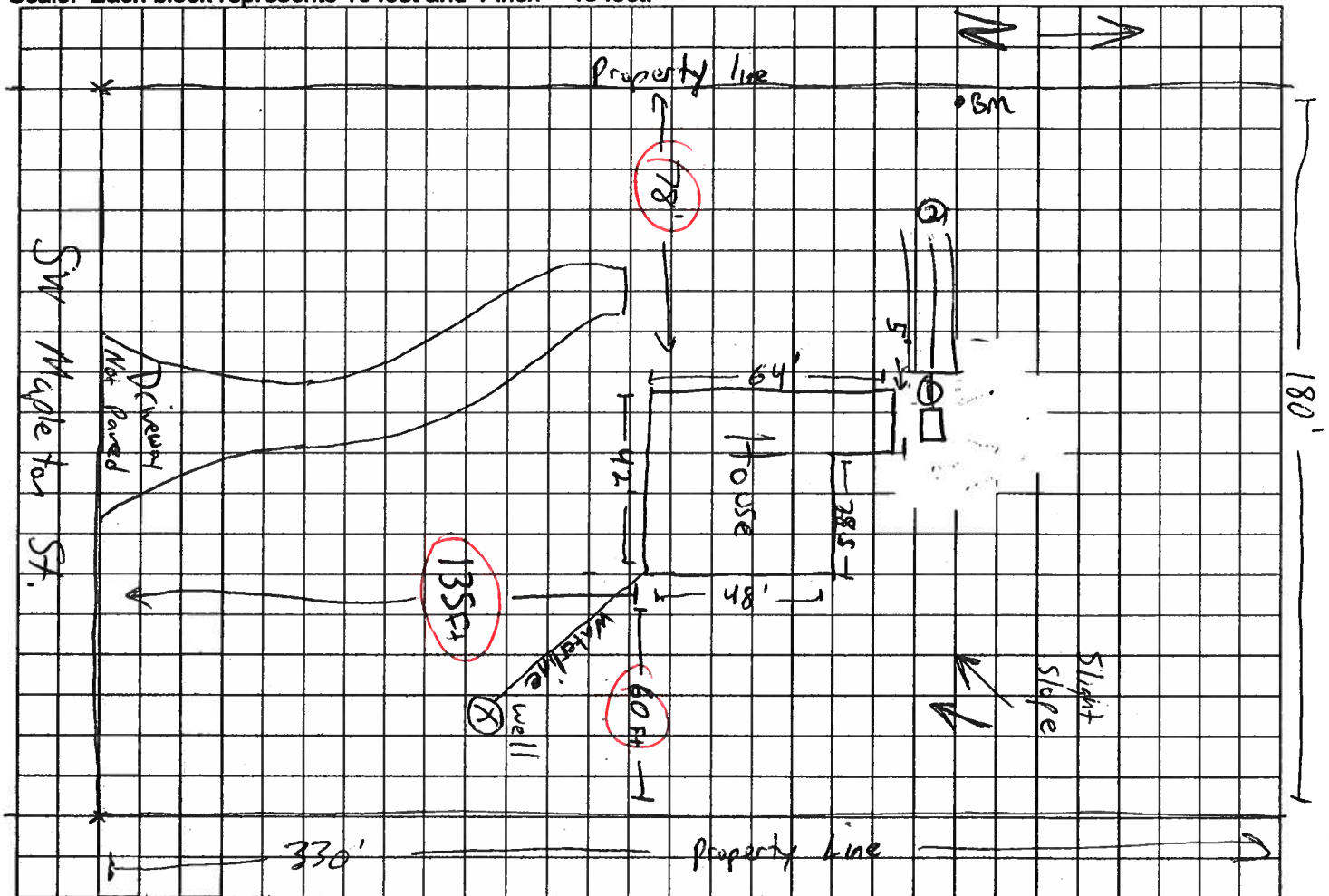
[Signature]
 Notary Signature _____
 (Revised Sept. 2006)

STATE OF FLORIDA
DEPARTMENT OF HEALTH
APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number 07-00161N

----- PART II - SITEPLAN -----

Scale: Each block represents 10 feet and 1 inch = 40 feet.



Notes:

REVISED 3/6/7
x Daniel Adam Hinton
145' SEPTIC TO WELL

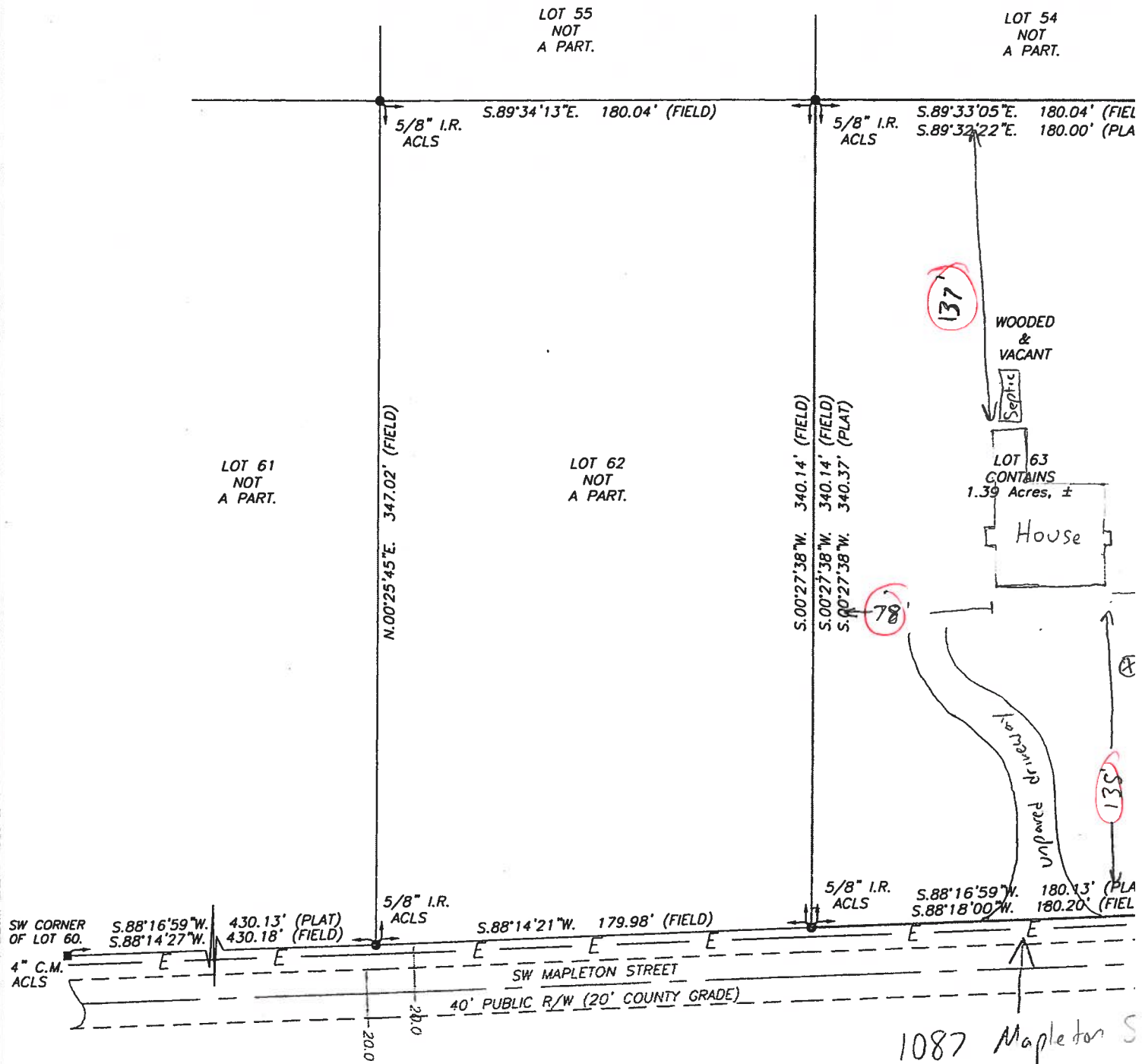
Site Plan submitted by: Daniel Adam Hinton

Plan Approved X **APPROVED** No Approved

By [Signature] **Columbia CHD** County Health Department

Date 3/7/7

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT



SIGNED:

MARK D. DUREN, LS 4708

WARRANTY DEED

(STATUTORY FORM - SECTION 689.02, F.S.)

This document prepared by and to be returned to:

Kyle E. Petteway
Grunder & Petteway, P. A.
23349 NW CR 236, Suite 10
High Springs, Florida, 32643

Inst:2007001808 Date:01/24/2007 Time:11:17

Doc Stamp-Deed : 490.00

Tax Parcel Number: R10058-653

DC, P. Dewitt Cason, Columbia County B:1108 P:1956

THIS INDENTURE made January 22, 2007,

BETWEEN Gary L. Sloan and Susan Sloan, husband and wife, whose post office address is 5460 Pine Street, Cocoa, Florida, 32927, herein called Grantor, and

Daniel A. Hinton and Cammey B. Hinton, husband and wife, whose post office address is 25647 NW 204th Avenue, High Springs, Florida, 32643, herein called Grantee,

Witnesseth that said grantor, for and in consideration of the sum of TEN AND NO/100 (\$10.00) Dollars, and other good and valuable considerations to said grantor in hand paid by said grantee, the receipt whereof is hereby acknowledged, has granted, bargained and sold to the said grantee, and grantee's heirs and assigns forever, the following described land, situate, lying and being in the county(ies) of Columbia state of Florida, to wit:

Lot 63, a replat of Lots 38, 45, and 46 of Santa Fe River Plantations, a subdivision, as per plat thereof recorded in Plat Book 5, Pages 13 through 13D, inclusive of the Public Records of Columbia County, Florida. *SSW*

AND SAID GRANTOR does hereby fully warrant the title to said land, and will defend the same against the lawful claims of all persons whomsoever.

Grantor and grantee are used for singular or plural, as context requires.

In Witness Whereof, grantor has hereunto set grantor's hand and seal the day and year first above written.

Signed, sealed and delivered in our presence:

Witness: Print Name *Allison Spang*

Karen Arnone
Witness: Print Name *Karen Arnone*

Witness: Print Name *Allison Spang*

Karen Arnone
Witness: Print Name *Karen Arnone*

Gary L. Sloan
Gary L. Sloan

Susan Sloan
Susan Sloan

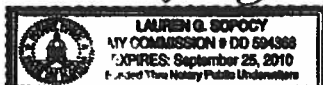
State of Florida
County of Alachua *Brevard*

The foregoing instrument was acknowledged before me this *18* day of *January* 2007 by Gary L. Sloan and Susan Sloan who

- () are personally known to me
(☒) who have produced a valid Florida driver's license as identification
() who produced _____ as identification

Lauren G. Sopoty
Notary Public at Large, State of Florida

(SEAL)



8583

@ CAM112M01 S CamaUSA Appraisal System
 3/07/2007 11:07 Legal Description Maintenance
 Year T Property Sel
 2007 R 30-7S-17-10058-653

Columbia County
 37260 Land 001 *
 AG 000
 Bldg 000
 Xfea 000
 37260 TOTAL B

HINTON DANIEL A & CAMMEY B

1	LOT 63 SANTE FE RIVER	PLANTATIONS REPLAT OF LOT 38.	2
3	ORB 441-211, 742-1258, 725-611	725-616, WD 1108-1956.	4
5			6
7			8
9			10
11			12
13			14
15			16
17			18
19			20
21			22
23			24
25			26
27			28

Mnt 2/02/2007 THRESA

F1=Task F3=Exit F4=Prompt F10=GoTo PgUp/PgDn F24=More

SCHAFFER ENGINEERING, LLC

7104 NW 42ND LANE GAINESVILLE FL 32606 PH: 386-462-1340 – 352-375-6329

March 15, 2007

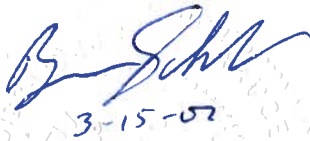
Job: lot 63 Santa Fe River Plantation

Re: Open Porch Headers

Dear Sir:

Install 2 ply 2 x 12 pt syp #2 or better headers for all porch headers. The truss span will be a maximum span of 8'-8" with the maximum header span of 10'-0"

If you have any questions or if we can be any further assistance, please feel free to contact us at your convenience.

A handwritten signature in blue ink, appearing to read "Bruce Schafer", is written over a circular professional seal. The seal contains the text "FLORIDA PROFESSIONAL ENGINEER" around the perimeter and "3-15-07" in the center.

Bruce Schafer, P.E.

48984

7104 N. W. 42nd Lane

Gainesville, Florida 32606

NOTORIZED DISCLOSURE STATEMENT

FOR OWNER/BUILDER WHEN ACTING AS THEIR OWN CONTRACTOR AND CLAIMING EXEMPTION OF CONTRACTOR LICENSING REQUIREMENTS IN ACCORDANCE WITH FLORIDA STATUTES, ss. 489.103(7).

State law requires construction to be done by licensed contractors. You have applied for a permit under an exemption to that law. The exemption allows you, as the owner of your property, to act as your own contractor with certain restrictions even though you do not have a license. You must provide direct, onsite supervision of the construction yourself. You may build or improve a one-family or two-family residence or a farm outbuilding. You may also build or improve a commercial building, provided your costs do not exceed \$75,000. The building or residence must be for your own use or occupancy. It may not be built or substantially improved for sale or lease. If you sell or lease a building you have built or substantially improved yourself within 1 year after the construction is complete, the law will presume that you built or substantially improved it for sale or lease, which is a violation of this exemption. You may not hire an unlicensed person to act as your contractor or to supervise people working on your building. It is your responsibility to make sure that people employed by you have licenses required by state law and by county or municipal licensing ordinances. You may not delegate the responsibility for supervising work to a licensed contractor who is not licensed to perform the work being done. Any person working on your building who is not licensed must work under your direct supervision and must be employed by you, which means that you must deduct F.I.C.A. and withholding tax and provide workers' compensation for that employee, all as prescribed by law. Your construction must comply with all applicable laws, ordinances, building codes, and zoning regulations.

TYPE OF CONSTRUCTION

☒ Single Family Dwelling
☐ Farm Outbuilding

☐ Two-Family Residence
☐ Other _____

NEW CONSTRUCTION OR IMPROVEMENT

☒ New Construction

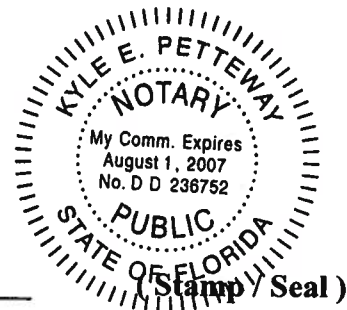
☐ Addition, Alteration, Modification or other Improvement

I Daniel Adam Hinton, have been advised of the above disclosure statement for exemption from contractor licensing as an owner/builder. I agree to comply with all requirements provided for in Florida Statutes ss.489.103(7) allowing this exception for the construction permitted by Columbia County Building Permit Number _____

Daniel Adam Hinton 3-6-07
Owner Builder Signature Date

The above signer is personally known to me or
produced identification _____

Notary Signature Kyle E. Petteway Date 3/6/07



FOR BUILDING USE ONLY

I hereby certify that the above listed owner/builder has been notified of the disclosure statement in Florida Statutes ss 489.103(7).

Date _____ Building Official/Representative _____

Jan 05 08 03:59p

Gar: ROBERT McMILLAN
(386) 454-PUMP (7867)
(386) 462-PUMP (7867)

p. 4..

NORTH FLORIDA
WATER SYSTEMS, INC.

PUMP SALES AND SERVICE

4" WELLS

11814 N.W. 202nd St., Alachua, Florida 32615

Private Well Affidavit

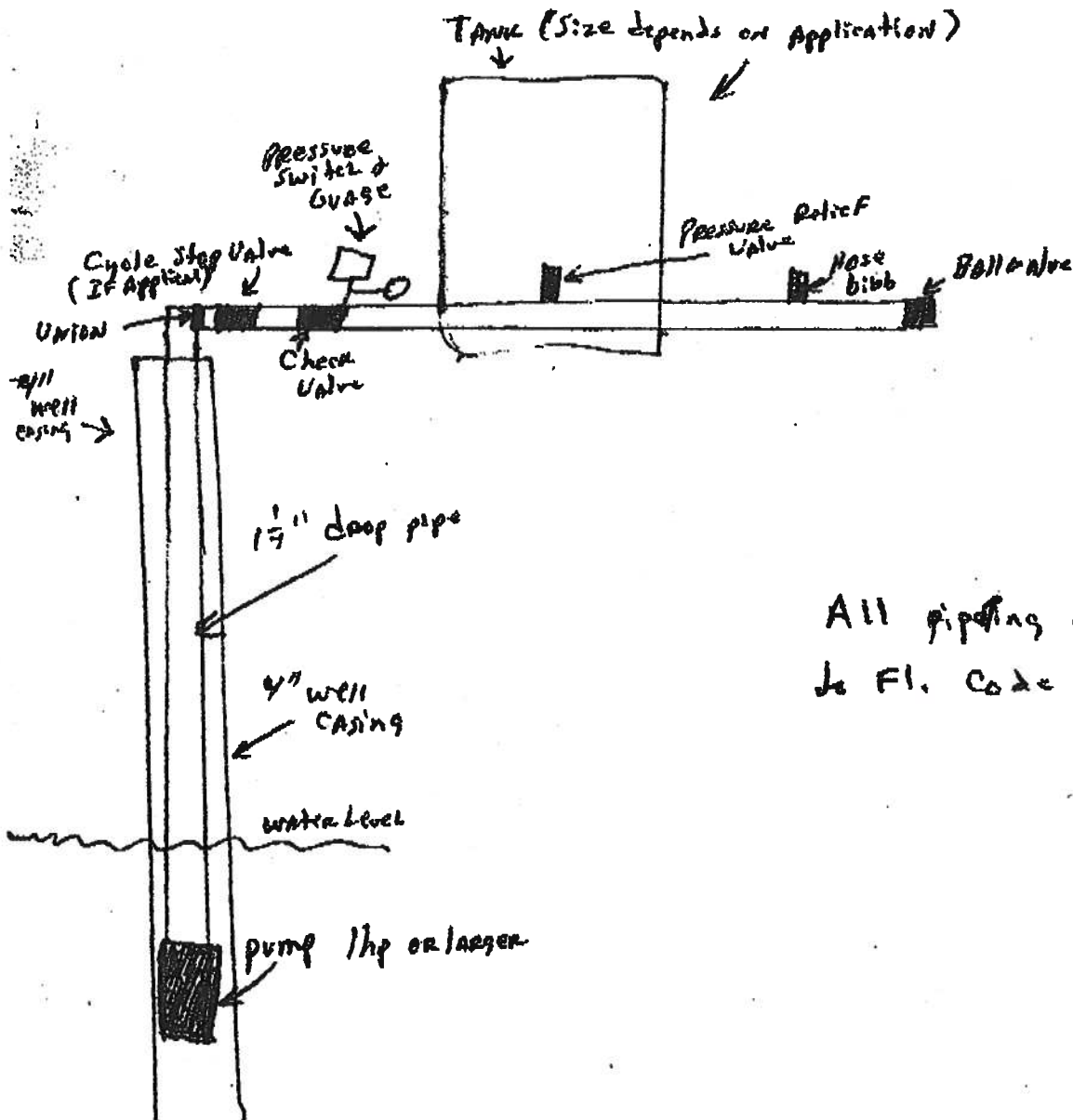
Customer: Adam Hinton.Address: _____
_____Size of Pump Motor: 1hpSize of Pressure Tank: 60 GALLONCycle Stop Value: ☒ Yes Or NoOther: _____

_____X Robert C. McMillan
Installer's Signature



NORTH FLORIDA
WATER SYSTEMS, INC.
11814 N.W. 202nd ST.
ALACHUA, FLORIDA 32615

Columbia County well diagram



All piping and wiring
to Fl. Code

Drawn by Robert
Mumma

PH # 386 462-7867

COLUMBIA COUNTY 9-1-1 ADDRESSING

P. O. Box 1787, Lake City, FL 32056-1787

PHONE: (386) 758-1125 * FAX: (386) 758-1365 * Email: ron_croft@columbiacountyfla.com

Addressing Maintenance

To maintain the Countywide Addressing Policy you must make application for a 9-1-1 Address at the time you apply for a building permit. The established standards for assigning and posting numbers to all principal buildings, dwellings, businesses and industries are contained in Columbia County Ordinance 2001-9. The addressing system is to enable Emergency Service Agencies to locate you in an emergency, and to assist the United States Postal Service and the public in the timely and efficient provision of services to residents and businesses of Columbia County.

DATE REQUESTED: 2/9/2007 DATE ISSUED: 2/12/2007

ENHANCED 9-1-1 ADDRESS:

1087 SW MAPLETON ST

FORT WHITE FL 32038

PROPERTY APPRAISER PARCEL NUMBER:

30-7S-17-10058-653

Remarks:

LOT 63 SANTE FE RIVER PLANTATIONS

Address Issued By:



Columbia County 9-1-1 Addressing / GIS Department

NOTICE: THIS ADDRESS WAS ISSUED BASED ON LOCATION INFORMATION RECEIVED FROM THE REQUESTER. SHOULD, AT A LATER DATE, THE LOCATION INFORMATION BE FOUND TO BE IN ERROR, THIS ADDRESS IS SUBJECT TO CHANGE.

Approved Address

FEB 12 2007

911Addressing/GIS Dept

Adam & Cammey Hinton
(352) 318-3411

Mailing address: P.O. Box 2214

High Springs, FL 32655

Property address: 1087 SW Mapleton Street

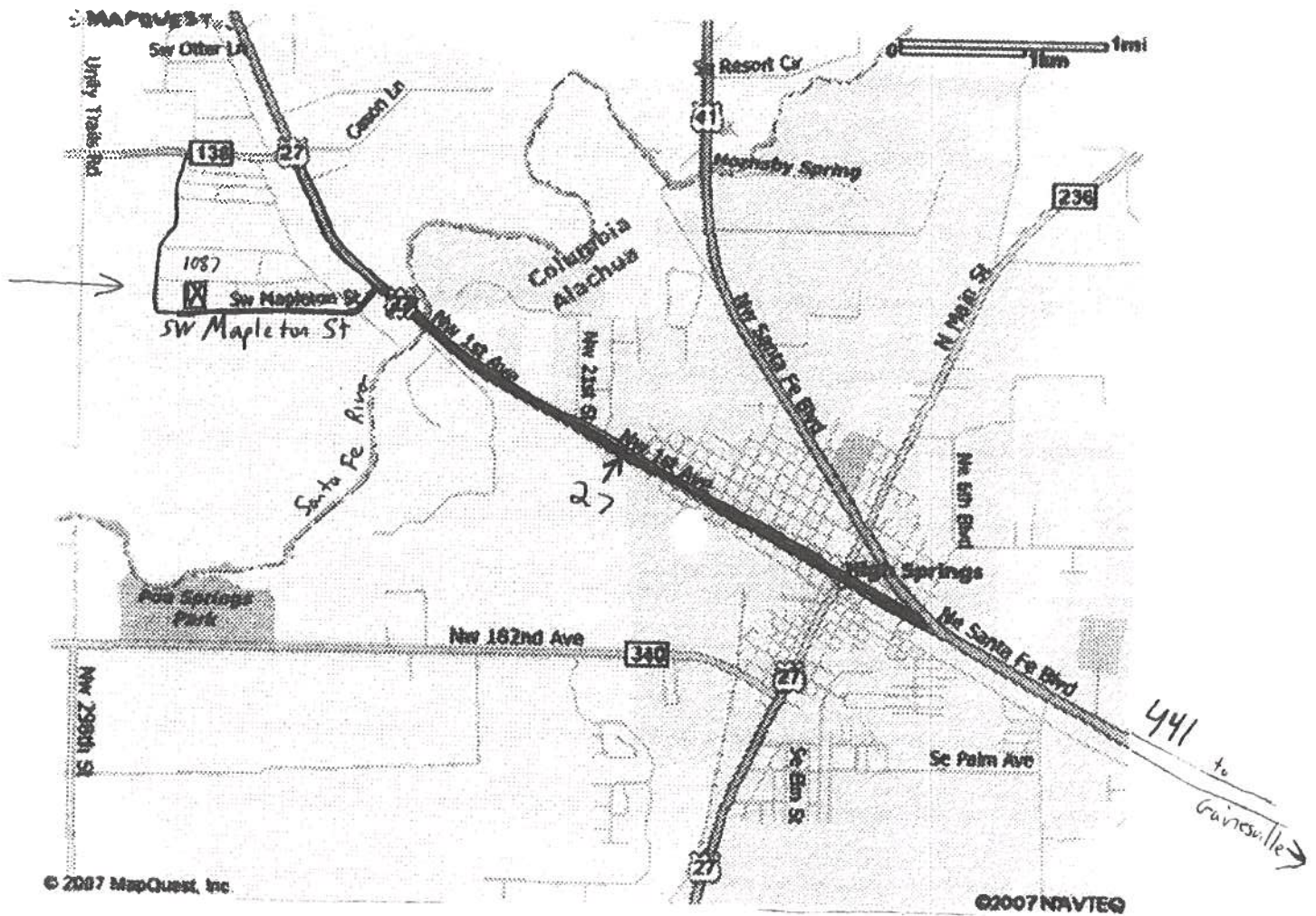
Fort White, FL 32038

Subdivision: Santa Fe River Plantations

Lot #63

Property Appraiser Parcel # 30-7S-17-10058-653

Directions: From High Springs go west on highway 27 towards Fort White, take the first left (after you cross the Santa Fe River into Columbia county) which is Mapleton Street. The road curves to the right, stay straight property is on the right 0.9 miles from highway 27.



FROM :nfwsinc

FAX NO. : 3864180738

Feb. 23 2007 06:03PM P2

Jan 05 06 03:59p

Gar: ROBERT McMILLAN
(386) 454-PUMP (7867)
(386) 462-PUMP (7867)



p. 41

**NORTH FLORIDA
WATER SYSTEMS, INC.
PUMP SALES AND SERVICE
4" WELLS**

11814 N.W. 202nd St., Alachua, Florida 32615

Private Well Affidavit

Customer: Adam Hinton.

Address: _____

Size of Pump Motor: 1/2 HP

Size of Pressure Tank: 60 GALLON

Cycle Stop Value: Yes Or No

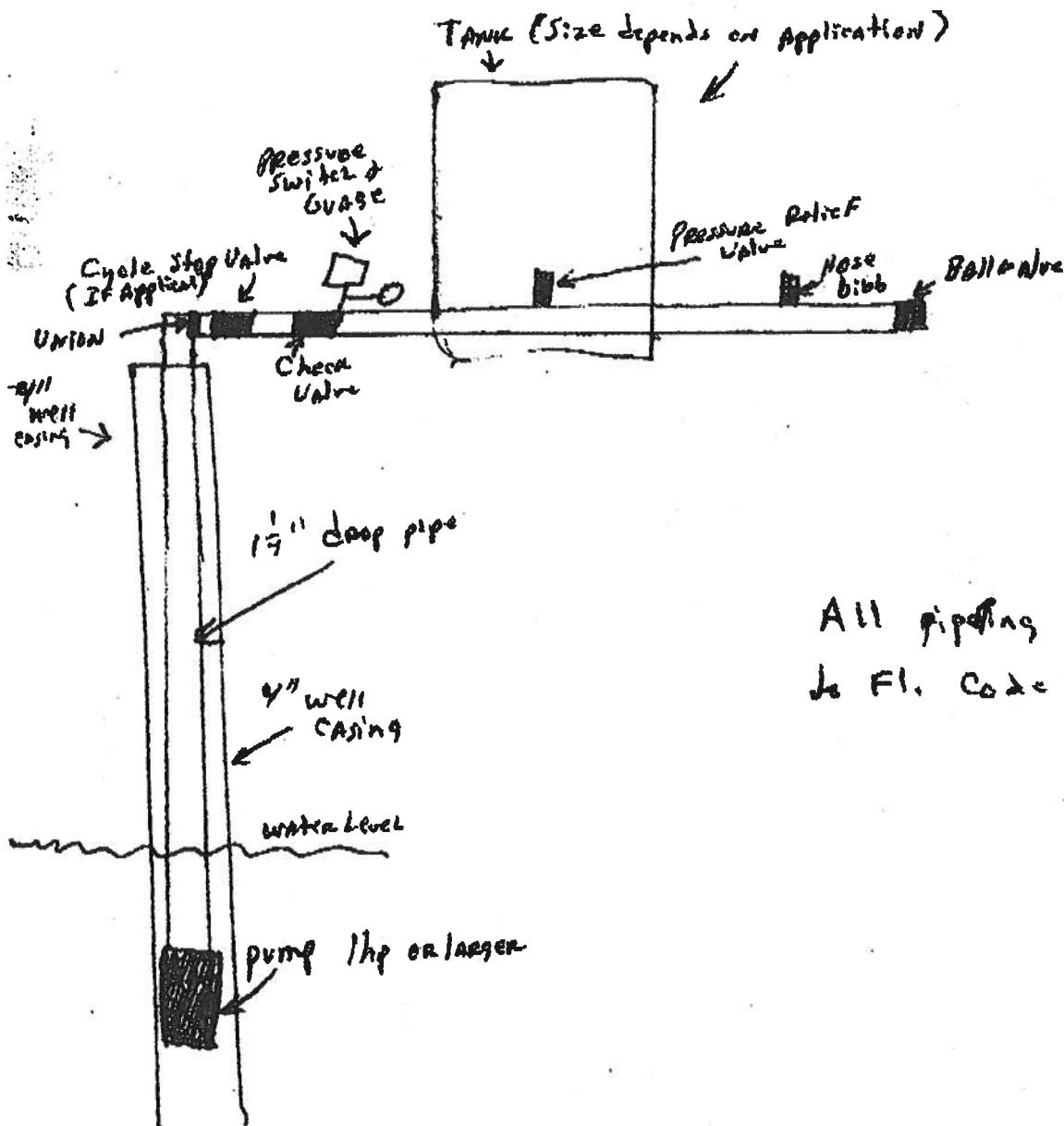
Other: _____

X Robert C. Minella
Installer's Signature



NORTH FLORIDA
WATER SYSTEMS, INC.
11814 N.W. 202nd ST.
ALACHUA, FLORIDA 32615

Columbia County well diagram



Drawn by Robert
Mumford

PA # 386 462-7867

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs

Residential Whole Building Performance Method A

Project Name: **Adam Hinton**
 Address: **Lot: 63, Sub: Sante Fe River, Plat:**
 City, State: **,**
 Owner:
 Climate Zone: **North**

Builder: **owner**
 Permitting Office:
 Permit Number:
 Jurisdiction Number:

- | | | |
|---|--|-----|
| 1. New construction or existing | New | ___ |
| 2. Single family or multi-family | Single family | ___ |
| 3. Number of units, if multi-family | 1 | ___ |
| 4. Number of Bedrooms | 3 | ___ |
| 5. Is this a worst case? | Yes | ___ |
| 6. Conditioned floor area (ft ²) | 1928 ft ² | ___ |
| 7. Glass type ¹ and area: (Label reqd. by 13-104.4.5 if not default) | | ___ |
| a. U-factor: | Description Area | |
| (or Single or Double DEFAULT) | 7a. (Dble Default) 221.0 ft ² | ___ |
| b. SHGC: | | |
| (or Clear or Tint DEFAULT) | 7b. (Clear) 221.0 ft ² | ___ |
| 8. Floor types | | |
| a. Slab-On-Grade Edge Insulation | R=0.0, 206.0(p) ft | ___ |
| b. N/A | | ___ |
| c. N/A | | ___ |
| 9. Wall types | | |
| a. Frame, Wood, Exterior | R=13.0, 150.0 ft ² | ___ |
| b. Concrete, Int Insul, Exterior | R=4.1, 1600.0 ft ² | ___ |
| c. N/A | | ___ |
| d. N/A | | ___ |
| e. N/A | | ___ |
| 10. Ceiling types | | |
| a. Single Assembly | R=19.0, 2150.0 ft ² | ___ |
| b. N/A | | ___ |
| c. N/A | | ___ |
| 11. Ducts | | |
| a. Sup: Con. Ret: Con. AH: Interior | Sup. R=6.0, 112.0 ft | ___ |
| b. N/A | | ___ |
| 12. Cooling systems | | |
| a. Central Unit | Cap: 35.0 kBtu/hr | ___ |
| | SEER: 13.00 | ___ |
| b. N/A | | ___ |
| c. N/A | | ___ |
| 13. Heating systems | | |
| a. Electric Heat Pump | Cap: 32.0 kBtu/hr | ___ |
| | HSPF: 7.70 | ___ |
| b. N/A | | ___ |
| c. N/A | | ___ |
| 14. Hot water systems | | |
| a. Electric Resistance | Cap: 40.0 gallons | ___ |
| | EF: 0.92 | ___ |
| b. N/A | | ___ |
| c. Conservation credits | | ___ |
| (HR-Heat recovery, Solar | | |
| DHP-Dedicated heat pump) | | |
| 15. HVAC credits | PT, CF, ___ | |
| (CF-Ceiling fan, CV-Cross ventilation, | | |
| HF-Whole house fan, | | |
| PT-Programmable Thermostat, | | |
| MZ-C-Multizone cooling, | | |
| MZ-H-Multizone heating) | | |

Glass/Floor Area: 0.11

Total as-built points: 25955

Total base points: 26330

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: SUNCOAST Insulators

DATE: 2-28-07

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____

DATE: _____

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.



BUILDING OFFICIAL: _____

DATE: _____

¹ Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: 63, Sub: Sante Fe River, Plat: , , ,

PERMIT #:

BASE				AS-BUILT							
GLASS TYPES .18 X Conditioned X BSPM = Points Floor Area				Type/SC	Overhang Ormt Len Hgt			Area X SPM X SOF = Points			
.18	1928.0	18.59	6451.0	1.Double, Clear	E	2.0	5.0	75.0	42.06	0.80	2514.0
				2.Double, Clear	W	2.0	5.0	51.0	38.52	0.80	1570.0
				3.Double, Clear	S	2.0	5.0	40.0	35.87	0.72	1037.0
				4.Double, Clear	N	2.0	5.0	55.0	19.20	0.87	919.0
				As-Built Total:				221.0		6040.0	
WALL TYPES Area X BSPM = Points				Type	R-Value			Area X SPM = Points			
Adjacent	0.0	0.00	0.0	1. Frame, Wood, Exterior	13.0			150.0	1.50	225.0	
Exterior	1750.0	1.70	2975.0	2. Concrete, Int Insul, Exterior	4.1			1600.0	1.13	1816.0	
Base Total: 1750.0 2975.0				As-Built Total:				1750.0		2041.0	
DOOR TYPES Area X BSPM = Points				Type				Area X SPM = Points			
Adjacent	0.0	0.00	0.0	1.Exterior Insulated				36.0	4.10	147.6	
Exterior	36.0	6.10	219.6								
Base Total: 36.0 219.6				As-Built Total:				36.0		147.6	
CEILING TYPES Area X BSPM = Points				Type	R-Value			Area X SPM X SCM = Points			
Under Attic	1928.0	1.73	3335.4	1. Single Assembly	19.0			2150.0	5.64 X 1.00	12126.0	
Base Total: 1928.0 3335.4				As-Built Total:				2150.0		12126.0	
FLOOR TYPES Area X BSPM = Points				Type	R-Value			Area X SPM = Points			
Slab	206.0(p)	-37.0	-7622.0	1. Slab-On-Grade Edge Insulation	0.0			206.0(p)	-41.20	-8487.2	
Raised	0.0	0.00	0.0								
Base Total: -7622.0				As-Built Total:				206.0		-8487.2	
INFILTRATION Area X BSPM = Points							Area X SPM = Points				
1928.0 10.21 19684.9							1928.0 10.21 19684.9				

SUMMER CALCULATIONS**Residential Whole Building Performance Method A - Details**

ADDRESS: Lot: 63, Sub: Sante Fe River, Plat: , , ,

PERMIT #:

BASE				AS-BUILT						
Summer Base Points: 25043.9				Summer As-Built Points: 31552.3						
Total Summer Points	X System Multiplier	=	Cooling Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	=	Cooling Points
				(sys 1: Central Unit 35000btuh , SEER/EFF(13.0) Ducts: Con(S), Con(R), Int(AH), R6.0(INS)						
25043.9	0.3250		8139.3	31552	1.00	(1.00 x 1.147 x 0.91)	0.260	0.902		7727.8
				31552.3	1.00	1.044	0.260	0.902		7727.8

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: 63, Sub: Sante Fe River, Plat: , , ,

PERMIT #:

BASE				AS-BUILT							
GLASS TYPES .18 X Conditioned X BWPM = Points Floor Area											
				Type/SC	Overhang Ormt Len Hgt		Area X WPM X WOF = Points				
.18	1928.0	20.17	7000.0	1.Double, Clear	E	2.0	5.0	75.0	18.79	1.08	1527.0
				2.Double, Clear	W	2.0	5.0	51.0	20.73	1.06	1119.0
				3.Double, Clear	S	2.0	5.0	40.0	13.30	1.40	744.0
				4.Double, Clear	N	2.0	5.0	55.0	24.58	1.01	1360.0
				As-Built Total:						221.0	4750.0
WALL TYPES Area X BWPM = Points				Type	R-Value		Area X WPM = Points				
Adjacent	0.0	0.00	0.0	1. Frame, Wood, Exterior	13.0		150.0	3.40	510.0		
Exterior	1750.0	3.70	6475.0	2. Concrete, Int Insul, Exterior	4.1		1600.0	6.42	10272.0		
Base Total:		1750.0	6475.0	As-Built Total:				1750.0	10782.0		
DOOR TYPES Area X BWPM = Points				Type	Area X WPM = Points						
Adjacent	0.0	0.00	0.0	1.Exterior Insulated	36.0 8.40 302.4						
Exterior	36.0	12.30	442.8								
Base Total:		36.0	442.8	As-Built Total:				36.0	302.4		
CEILING TYPES Area X BWPM = Points				Type	R-Value		Area X WPM X WCM = Points				
Under Attic	1928.0	2.05	3952.4	1. Single Assembly	19.0		2150.0	1.86 X 1.00	3999.0		
Base Total:		1928.0	3952.4	As-Built Total:				2150.0	3999.0		
FLOOR TYPES Area X BWPM = Points				Type	R-Value		Area X WPM = Points				
Slab	206.0(p)	8.9	1833.4	1. Slab-On-Grade Edge Insulation	0.0		206.0(p)	18.80	3872.8		
Raised	0.0	0.00	0.0								
Base Total:		1833.4		As-Built Total:				206.0	3872.8		
INFILTRATION Area X BWPM = Points				Area X WPM = Points							
		1928.0	-0.59					1928.0	-0.59	-1137.5	

WINTER CALCULATIONS**Residential Whole Building Performance Method A - Details**

ADDRESS: Lot: 63, Sub: Sante Fe River, Plat: , , ,

PERMIT #:

BASE			AS-BUILT						
Winter Base Points: 18566.1			Winter As-Built Points: 22568.7						
Total Winter Points	X System Multiplier	= Heating Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points	
18566.1	0.5540	10285.6	(sys 1: Electric Heat Pump 32000 btuh ,EFF(7.7) Ducts:Con(S),Con(R),Int(AH),R6.0 22568.7 1.000 (1.000 x 1.169 x 0.93)0.443 0.950 10322.6						
			22568.7	1.00	1.087	0.443	0.950	10322.6	

WATER HEATING & CODE COMPLIANCE STATUS**Residential Whole Building Performance Method A - Details**

ADDRESS: Lot: 63, Sub: Sante Fe River, Plat: , , ,

PERMIT #:

BASE				AS-BUILT					
WATER HEATING									
Number of Bedrooms	X	Multiplier	= Total	Tank Volume	EF	Number of Bedrooms	X Tank Ratio	X Multiplier	X Credit = Total Multiplier
3		2635.00	7905.0	40.0	0.92	3	1.00	2635.00	1.00 7905.0
				As-Built Total:					7905.0

CODE COMPLIANCE STATUS									
BASE					AS-BUILT				
Cooling Points	+	Heating Points	+	Hot Water Points = Total Points	Cooling Points	+	Heating Points	+	Hot Water Points = Total Points
8139		10286		7905 26330	7728		10323		7905 25955

PASS

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: 63, Sub: Sante Fe River, Plat: , , ,

PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum: .3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 84.9

The higher the score, the more efficient the home.

, Lot: 63, Sub: Sante Fe River, Plat: , , ,

1. New construction or existing	New	___	12. Cooling systems	
2. Single family or multi-family	Single family	___	a. Central Unit	Cap: 35.0 kBtu/hr
3. Number of units, if multi-family	1	___		SEER: 13.00
4. Number of Bedrooms	3	___	b. N/A	___
5. Is this a worst case?	Yes	___	c. N/A	___
6. Conditioned floor area (ft²)	1928 ft²	___		___
7. Glass type ¹ and area: (Label reqd. by 13-104.4.5 if not default)		___		___
a. U-factor:	Description Area		13. Heating systems	
(or Single or Double DEFAULT)	7a. (Dble Default) 221.0 ft²	___	a. Electric Heat Pump	Cap: 32.0 kBtu/hr
b. SHGC:		___		HSPF: 7.70
(or Clear or Tint DEFAULT)	7b. (Clear) 221.0 ft²	___	b. N/A	___
8. Floor types		___	c. N/A	___
a. Slab-On-Grade Edge Insulation	R=0.0, 206.0(p) ft	___		___
b. N/A	___	___	14. Hot water systems	
c. N/A	___	___	a. Electric Resistance	Cap: 40.0 gallons
9. Wall types		___		EF: 0.92
a. Frame, Wood, Exterior	R=13.0, 150.0 ft²	___	b. N/A	___
b. Concrete, Int Insul, Exterior	R=4.1, 1600.0 ft²	___	c. Conservation credits	___
c. N/A	___	___	(HR-Heat recovery, Solar	___
d. N/A	___	___	DHP-Dedicated heat pump)	___
e. N/A	___	___	15. HVAC credits	PT, CF, ___
10. Ceiling types		___	(CF-Ceiling fan, CV-Cross ventilation,	___
a. Single Assembly	R=19.0, 2150.0 ft²	___	HF-Whole house fan,	___
b. N/A	___	___	PT-Programmable Thermostat,	___
c. N/A	___	___	MZ-C-Multizone cooling,	___
11. Ducts		___	MZ-H-Multizone heating)	___
a. Sup: Con. Ret: Con. AH: Interior	Sup. R=6.0, 112.0 ft	___		___
b. N/A	___	___		___

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: _____

Date: _____

Address of New Home: _____

City/FL Zip: _____



***NOTE:** The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar™ designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.

Jan 05 06 03:59p

Gar:

ROBERT McMILLAN
(386) 454-PUMP (7867)
(386) 462-PUMP (7867)

P. 4.

NORTH FLORIDA
WATER SYSTEMS, INC.
PUMP SALES AND SERVICE
4" WELLS

11814 N.W. 202nd St., Alachua, Florida 32615

Private Well Affidavit

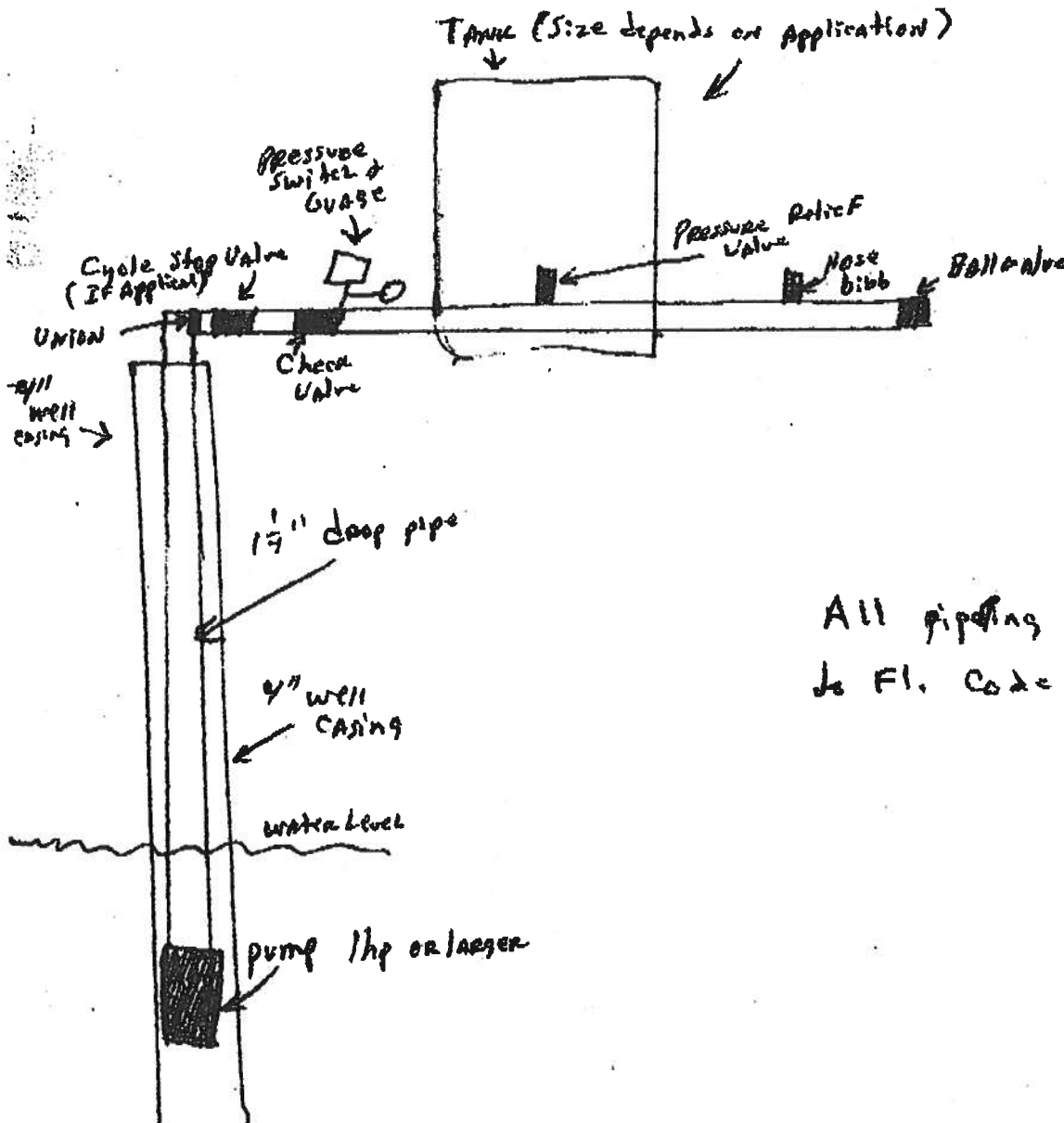
Customer: Adam Hinton.Address: _____
_____Size of Pump Motor: 1hpSize of Pressure Tank: 60 GALLONCycle Stop Value: ☒ Yes Or NoOther: _____

_____x Robert C. McMillan
Installer's Signature



**NORTH FLORIDA
WATER SYSTEMS, INC.**
11814 N.W. 202nd ST.
ALACHUA, FLORIDA 32615

Columbia County well diagram



All piping and wiring
to Fl. Code

Drawn by Robert
Mumford

PH # 386 462-7867



**From: The Columbia County Building & Zoning Department
Plan Review
135 NE Hernando Av.
P.O. Box 1529
Lake City Florida 32056-1529**

Reference to a building permit application Number: **0703-19**

Applicant: Daniel Adam Hinton, Owner/Builder. Property ID# 30-1s-10058-663

On the date of March 12, 2007 application 0703-19 and plans for construction of single family dwelling were reviewed and the following information or alteration to the plans will be required to continue processing this application. If you should have any question please contact the above address, or contact phone number (386) 758-1163 or fax any information to (386) 754-7088.

Please include application number 0703-19 and will on making reference to this application.

This is a plan review for compliance with the Florida Residential Code 2004 only and doesn't make any consideration toward the land use and zoning requirements.

1. **Covered Porches:** Please have Schafer Engineering show the method of attachment of the 4"x4"x9" porch columns to the foundation. Also show the header beam size and the attachment method of the beams to the porch columns along with the attachment of the header beams to the 8" CMU block walls.
2. **Bonus area & unfinished storage area rooms second story:**
 - A. Although not shown as bedrooms please verify the windows which will be installed will meet the requirements of the FBC

section R310.1.1 Minimum opening area: All emergency escape and rescue openings shall have a minimum net clear opening of 5.7 square feet (0.530 m²). R310.1.2 Minimum opening height: The minimum net clear opening height shall be 24 inches (610 mm). R310.1.3 Minimum opening width. The minimum net clear opening width shall be 20 inches (508 mm).

B. Please show the thickness and type material which will be used as flooring upon the trusses which are designed to support a live load.

3. On the electrical plans identify the electrical service overcurrent protection device for the main electrical service. This device shall be installed on the exterior of structures to serve as a disconnecting means for the utility company electrical service. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground.

Thank You:



Joe Halthwaenger
Plan Examiner
Columbia County Building
Department

SCHAFFER ENGINEERING, LLC

7104 NW 42ND LANE GAINESVILLE FL 32606 PH: 386-462-1340 - 352-375-6329

March 15, 2007

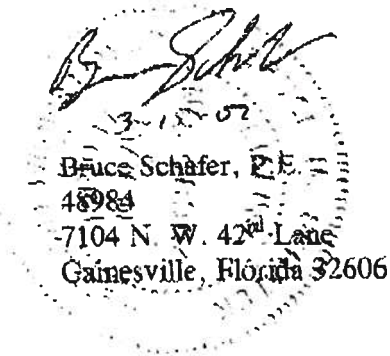
Job: lot 63 Santa Fe River Plantation

Re: Open Porch Headers

Dear Sir:

Install 2 ply 2 x 12 pt syp #2 or better headers for all porch headers. The truss span will be a maximum span of 8'-8" with the maximum header span of 10'-0"

If you have any questions or if we can be any further assistance, please feel free to contact us at your convenience.



RE: HINTA -

Trenco

818 Soundside Rd
Edenton, NC 27932

Site Information:

Project Customer: Project Name:

Lot/Block: 63

Subdivision: Santa Fe River Plantation

Address:

City: Fort White

State: Florida

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name:

License #:

Address:

City:

State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2004/TPI2002

Design Program: MiTek 20/20 6.5

Wind Code: ASCE 7-02 Wind Speed: 110 mph

Floor Load: 55.0 psf

Roof Load: 40.0 psf

This package includes 20 individual, dated Truss Design Drawings and 0 Additional Drawings.

With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No.	Seal#	Job ID#	Truss Name	Date	No.	Seal#	Job ID#	Truss Name	Date
1	E3875702	HINTA	A1	2/20/07	18	E3875719	HINTA	E1	2/20/07
2	E3875703	HINTA	A2	2/20/07	19	E3875720	HINTA	EET	2/20/07
3	E3875704	HINTA	A3	2/20/07	20	E3875721	HINTA	PIG1	2/20/07
4	E3875705	HINTA	A5	2/20/07					
5	E3875706	HINTA	A6	2/20/07					
6	E3875707	HINTA	AET1	2/20/07					
7	E3875708	HINTA	AET2	2/20/07					
8	E3875709	HINTA	AGT1	2/20/07					
9	E3875710	HINTA	AGT2	2/20/07					
10	E3875711	HINTA	B1	2/20/07					
11	E3875712	HINTA	B2	2/20/07					
12	E3875713	HINTA	BET	2/20/07					
13	E3875714	HINTA	BGT	2/20/07					
14	E3875715	HINTA	C1	2/20/07					
15	E3875716	HINTA	CET	2/20/07					
16	E3875717	HINTA	D1	2/20/07					
17	E3875718	HINTA	DET	2/20/07					

The truss drawing(s) referenced above have been prepared by TRENCO under my direct supervision based on the parameters provided by Santa Fe Truss.

Truss Design Engineer's Name: Strzyzewski, Marvin

My license renewal date for the state of is February 28, 2009.

NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-2002 Chapter 2.

Marvin A. Strzyzewski, P.E. #43144
Truss Engineering Company
818 Soundside Rd.
Edenton, NC 27932
FL Cert.#7239

February 20, 2007

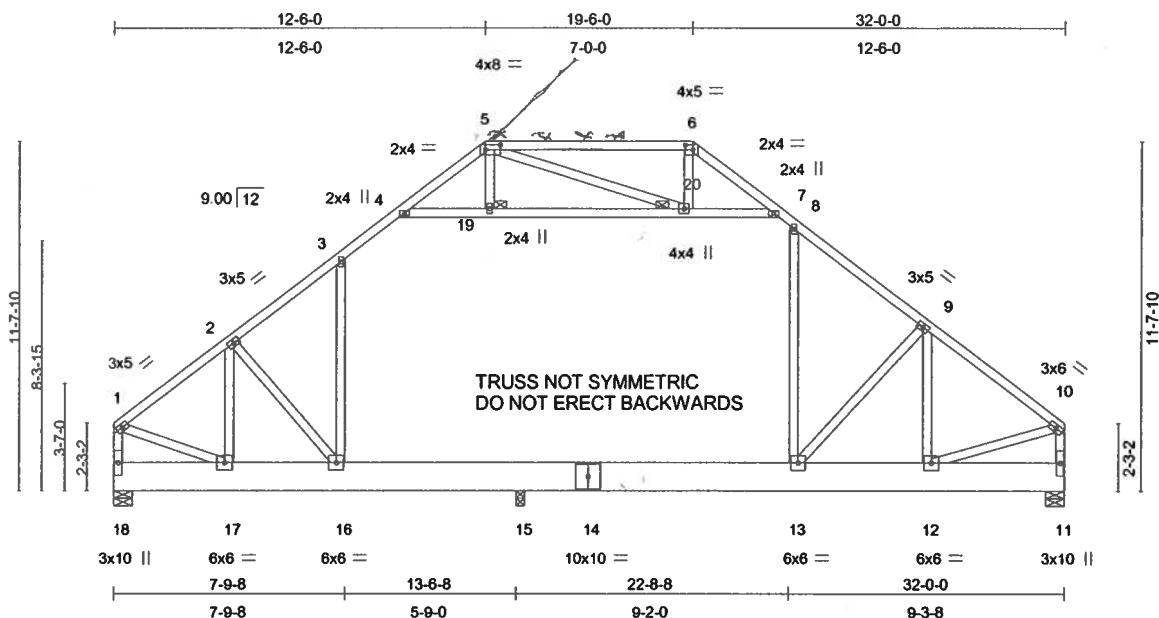
Job	Truss	Truss Type	Qty	Ply	
HINTA	A1	ROOF TRUSS	5	1	

E3875702

SANTA FE TRUSS, HIGH SPRINGS, FL.

Job Reference (optional)

6.500 s Feb 5 2007 MiTek Industries, Inc. Tue Feb 20 12:36:06 2007 Page 1



Scale = 1:77.5

Plate Offsets (X,Y): [5:0-6:0,0-2:0], [6:0-3:0,0-2:0]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.34	Vert(LL)	-0.16 13-15	>999	240	MT20	244/190
TCDL 10.0	Lumber Increase	1.25	BC 0.25	Vert(TL)	-0.23 13-15	>945	180		
BCLL 0.0	Rep Stress Incr	YES	WB 0.32	Horz(TL)	0.01 11	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)						
								Weight: 312 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2D
 BOT CHORD 2 X 12 SYP 2250F 1.9E
 WEBS 2 X 4 SYP No.3 *Except*
 4-7 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-6-1 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
 JOINTS 1 Brace at Jt(s): 19, 20

REACTIONS (lb/size) 18=1171/0-7-10, 11=1316/0-7-10, 15=1422/0-3-8
 Max Horz 18=-327(LC 3)
 Max Uplift 18=-48(LC 5), 11=-61(LC 6), 15=-7(LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-1096/83, 2-3=-1176/142, 3-4=-1049/125, 4-5=-670/190, 5-6=-529/170, 6-7=-681/163, 7-8=-976/120, 8-9=-1186/118, 9-10=-1349/101, 1-18=-1069/69, 10-11=-1243/84
 BOT CHORD 17-18=-324/319, 16-17=-288/829, 15-16=-117/865, 14-15=-117/865, 12-13=0/1021, 11-12=-13/93
 WEBS 1-17=0/836, 5-19=0/63, 5-20=-80/117, 6-20=0/70, 8-13=-38/185, 9-13=-354/251, 9-12=-150/234, 10-12=0/994, 3-16=-98/189, 4-19=-375/134, 19-20=-371/136, 7-20=-339/120, 2-17=-442/244, 2-16=-223/268

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- Ceiling dead load (5.0 psf) on member(s). 3-4, 7-8, 4-19, 19-20, 7-20
- Bottom chord live load (40.0 psf) and additional bottom chord dead load (0.0 psf) applied only to room. 15-16, 13-15
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 48 lb uplift at joint 18, 61 lb uplift at joint 11 and 7 lb uplift at joint 15.

LOAD CASE(S) Standard

Momir Stjepanovich
 February 20, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MI-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

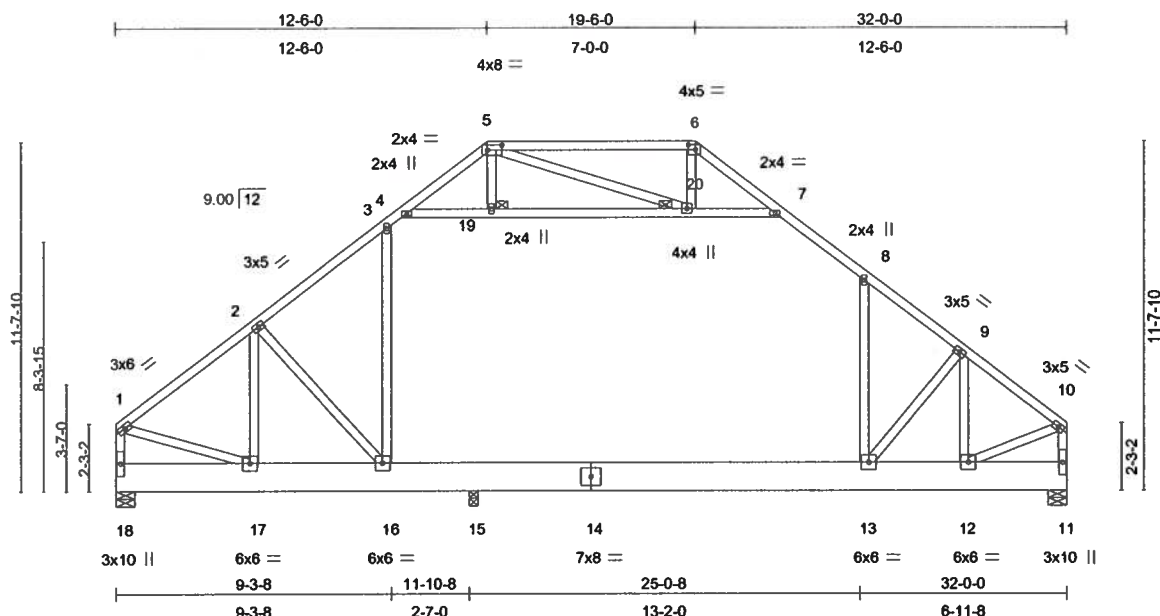
ENGINEERING BY
TRENCO
 A MiTek Affiliate

818 Soundside Road
 Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	
HINTA	A3	ROOF TRUSS	5	1	
SANTA FE TRUSS, HIGH SPRINGS, FL					Job Reference (optional)

E3875704

6.500 s Feb 5 2007 MiTek Industries, Inc. Tue Feb 20 12:36:08 2007 Page 1



Scale = 1:77.5

Plate Offsets (X,Y): [5:0-6-0,0-2-0], [6:0-3-0,0-2-0]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.59	Vert(LL)	-0.29 13-15	>808	240	MT20	244/190
TCDL 10.0	Lumber Increase	1.25	BC 0.35	Vert(TL)	-0.43 13-15	>553	180		
BCLL 0.0	Rep Stress Incr	YES	WB 0.38	Horz(TL)	0.01 11	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)						
									Weight: 310 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2D
 BOT CHORD 2 X 12 SYP 2250F 1.9E
 WEBS 2 X 4 SYP No.3 *Except*
 4-7 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-8-8 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
 JOINTS 1 Brace at Jt(s): 19, 20

REACTIONS (lb/size) 18=1093/0-7-10, 11=1538/0-7-10, 15=1349/0-3-8
 Max Horz 18=-327(LC 3)
 Max Uplift 18=-72(LC 6), 11=-87(LC 3), 15=-119(LC 4)
 Max Grav 18=1093(LC 1), 11=1538(LC 1), 15=1463(LC 8)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-1011/113, 2-3=-1328/299, 3-4=-1071/203, 4-5=-653/177, 5-6=-448/162, 6-7=-626/159, 7-8=-1180/181,
 8-9=-1365/106, 9-10=-1430/117, 1-18=-936/81, 10-11=-1447/100
 BOT CHORD 17-18=-325/318, 16-17=-195/748, 15-16=0/987, 14-15=0/987, 13-14=0/987, 12-13=-13/1110, 11-12=-11/54
 WEBS 1-17=-108/696, 5-19=0/62, 5-20=-96/42, 6-20=0/88, 8-13=0/235, 9-13=-219/181, 9-12=-101/95, 10-12=-23/1188,
 3-16=-167/354, 4-19=-544/184, 19-20=-540/186, 7-20=-544/161, 2-17=-824/234, 2-16=-227/571

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- Ceiling dead load (5.0 psf) on member(s). 3-4, 7-8, 4-19, 19-20, 7-20
- Bottom chord live load (40.0 psf) and additional bottom chord dead load (0.0 psf) applied only to room. 15-16, 13-15
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 72 lb uplift at joint 18, 87 lb uplift at joint 11 and 119 lb uplift at joint 15.

LOAD CASE(S) Standard

Maria Stapp

February 20, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MI-7473 BEFORE USE.

Design valid for use only with Mittek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oonofrio Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
 A Mittek Affiliate

818 Soundside Road
 Edenton, NC 27932

SANTA FE TRUSS, HIGH SPRINGS, FL.

6.500 s Feb 5 2007 MiTek Industries, Inc. Tue Feb 20 12:36:08 2007 Page 1



BRACING	
TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Except: 6-0-0 oc bracing: 4-5 10-0-0 oc bracing: 4-9
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

Maria Stuyvesant

February 20, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

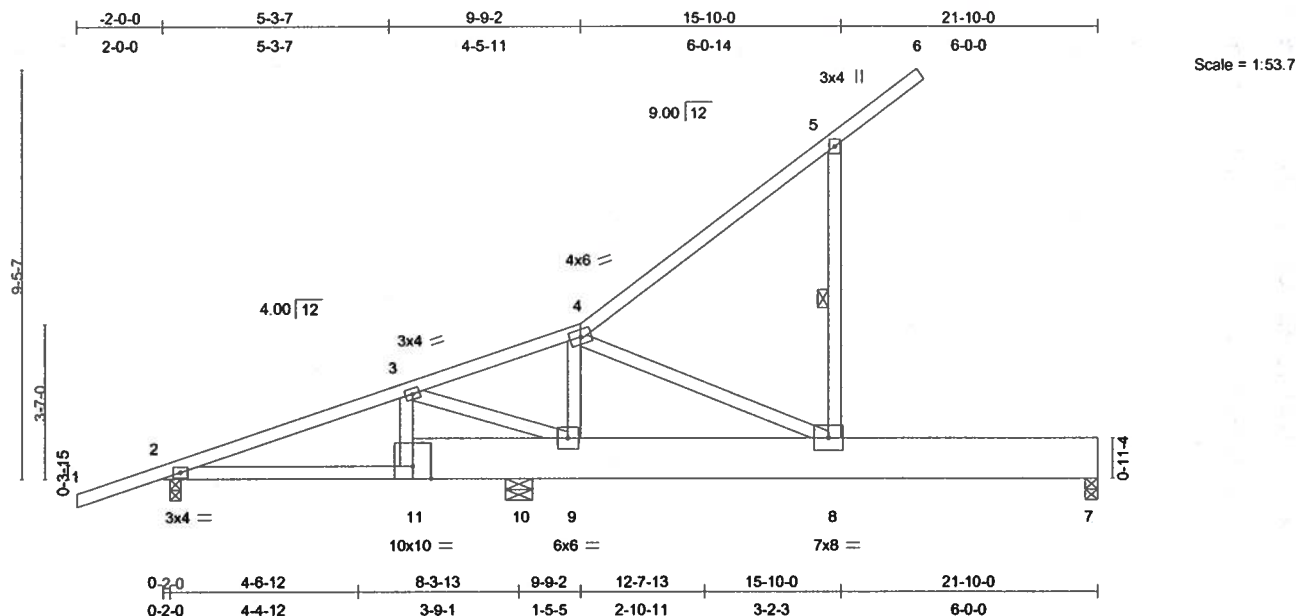
ENGINEERING BY
TRENCO
A MiTek Affiliate

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	E3875706
HINTA	A6	PORCH TRUSS	1	1	Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS, FL.

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LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.37	Vert(LL)	0.03	8	>999	240	MT20	244/190
TCDL 10.0	Lumber Increase	1.25	BC 0.32	Vert(TL)	-0.07	8	>999	180		
BCLL 0.0	Rep Stress Incr	YES	WB 0.12	Horz(TL)	-0.01	7	n/a	n/a		
BCDL 10.0	Code FBC2004/TP12002		(Matrix)							Weight: 150 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2D
 BOT CHORD 2 X 4 SYP No.2D *Except*
 7-11 2 X 12 SYP 2250F 1.9E
 WEBS 2 X 4 SYP No.3

REACTIONS (lb/size) 7=178/0-3-8, 2=224/0-3-0, 10=1210/0-7-10
 Max Horz 2=353(LC 4)
 Max Uplift 7=35(LC 4), 2=-288(LC 3), 10=-171(LC 5)
 Max Grav 7=204(LC 2), 2=224(LC 1), 10=1210(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/37, 2-3=-403/440, 3-4=-310/412, 4-5=-215/127, 5-6=-72/0, 5-8=-293/217
 BOT CHORD 2-11=-365/118, 10-11=-365/122, 9-10=-390/124, 8-9=-324/94, 7-8=0/0
 WEBS 3-9=-63/232, 4-9=-592/116, 4-8=-65/373, 3-11=-309/126

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 35 lb uplift at joint 7, 288 lb uplift at joint 2 and 171 lb uplift at joint 10.

LOAD CASE(S) Standard

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing, Except:
 10-0-0 oc bracing: 7-8.
 WEBS 1 Row at midpt 5-8

Martin A. Strassburg

February 20, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MI-7472 BEFORE USE.

Design valid for use only with Mittek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

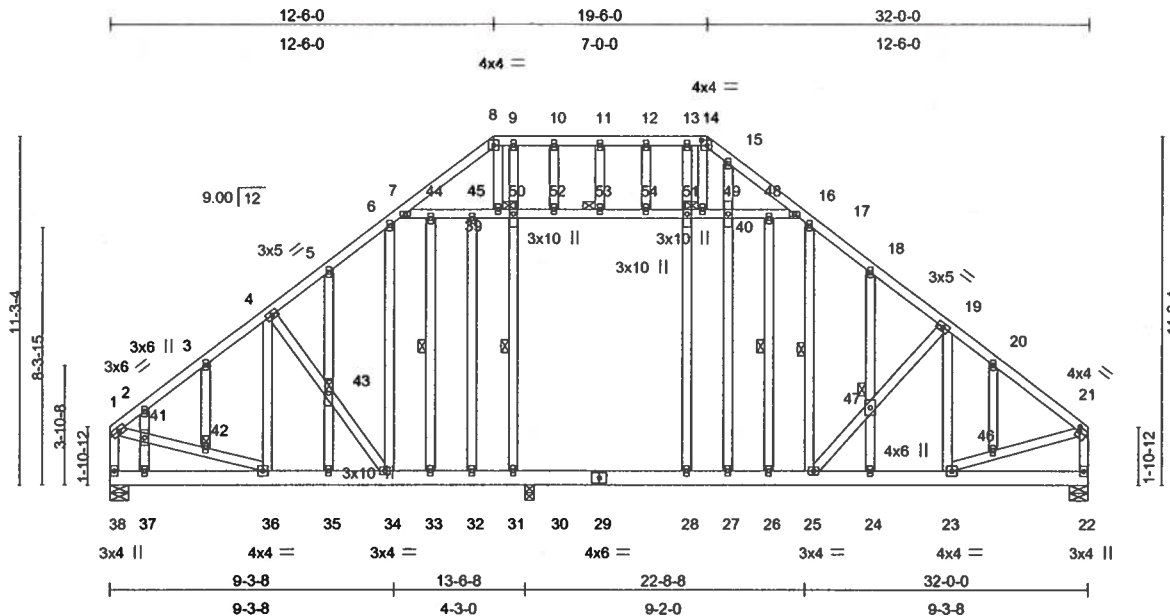
TRENCO
 A Mittek Affiliate

818 Soundside Road
 Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	
HINTA	AET1	ROOF TRUSS	1	1	
SANTA FE TRUSS, HIGH SPRINGS, FL					E3875707
Job Reference (optional)					

SANTA FE TRUSS, HIGH SPRINGS, FL

6.500 s Feb 5 2007 Mitek Industries, Inc. Tue Feb 20 12:36:11 2007 Page 1



Scale = 1:75.2

Plate Offsets (X,Y): [14:0-2-4,0-2-0], [21:0-1-0,0-1-12]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.58	Vert(LL)	-0.22	25	>979	MT20	244/190
TCDL 10.0	Lumber Increase	1.25	BC 0.54	Vert(TL)	-0.35	25	>620		
BCLL 0.0	Rep Stress Incr	YES	WB 0.74	Horz(TL)	0.03	22	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)						
									Weight: 353 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 6 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
7-16 2 X 4 SYP No.2

REACTIONS (lb/size) 38=1395/0-7-10, 22=1438/0-7-10, 30=293/0-3-8
Max Horz 38=-322(LC 3)
Max Uplift 38=-175(LC 3), 22=-205(LC 3), 30=-517(LC 4)
Max Grav 38=1395(LC 1), 22=1526(LC 9), 30=683(LC 8)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-1460/241, 2-3=-1499/242, 3-4=-1407/245, 4-5=-1519/404, 5-6=-1506/411, 6-7=-1450/432, 7-8=-1418/580,
8-9=-1165/471, 9-10=-1163/469, 10-11=-1163/469, 11-12=-1163/469, 12-13=-1163/469, 13-14=-1170/471,
14-15=-1331/530, 15-16=-1440/506, 16-17=-1540/395, 17-18=-1536/307, 18-19=-1606/272, 19-20=-1498/276,
20-21=-1690/269, 1-38=-1127/179, 21-22=-1472/237
BOT CHORD 37-38=-275/290, 36-37=-275/290, 35-36=0/1134, 34-35=0/1134, 33-34=-43/1208, 32-33=-43/1208, 31-32=-43/1208,
30-31=-43/1208, 29-30=-43/1208, 28-29=-43/1208, 27-28=-43/1208, 26-27=-43/1208, 25-26=-43/1208,
24-25=-131/1264, 23-24=-131/1264, 22-23=-18/171
WEBS 1-41=-187/986, 41-42=-191/1014, 36-42=-196/1027, 8-39=-302/726, 14-40=-209/424, 17-25=-117/146, 25-47=-112/144,
19-47=-91/129, 19-23=-312/110, 23-46=-131/1193, 21-46=-130/1145, 6-34=-50/193, 7-44=-357/237, 44-45=-357/237,
39-45=-357/237, 39-50=-342/249, 50-52=-342/249, 52-53=-342/249, 53-54=-342/249, 51-54=-342/249, 40-51=-342/249,
40-49=-346/243, 48-49=-346/243, 16-48=-346/243, 4-38=-358/227, 4-43=-236/266, 34-43=-265/296, 2-41=-126/80,
37-41=-235/99, 3-42=-14/53, 5-43=-68/53, 35-43=-73/40, 33-44=-105/76, 32-45=-108/243, 31-50=-251/399,
20-46=-6/140, 18-47=-55/62, 24-47=-29/48, 26-48=-28/38, 27-49=-76/202, 15-49=-13/90, 9-50=-204/123, 28-51=-67/255,
10-52=-27/41, 11-53=-45/30, 13-51=-4/116, 12-54=-37/29

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- All plates are 2x4 MT20 unless otherwise indicated.
- This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- Ceiling dead load (5.0 psf) on member(s). 7-44, 44-45, 39-45, 39-50, 50-52, 52-53, 53-54, 51-54, 40-51, 40-49, 48-49, 16-48
- Bottom chord live load (40.0 psf) and additional bottom chord dead load (0.0 psf) applied only to room. 30-31, 28-30
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 175 lb uplift at joint 38, 205 lb uplift at joint 22 and 517 lb uplift at joint 30.

Continued on page 2

Momir Stjepanovic

February 20, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MT-7473 BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BC511 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mitek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	
HINTA	AET1	ROOF TRUSS	1	1	E3875707

SANTA FE TRUSS, HIGH SPRINGS, FL

6.500 s Feb 5 2007 MiTek Industries, Inc. Tue Feb 20 12:36:11 2007 Page 2

LOAD CASE(S) Standard

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIT473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

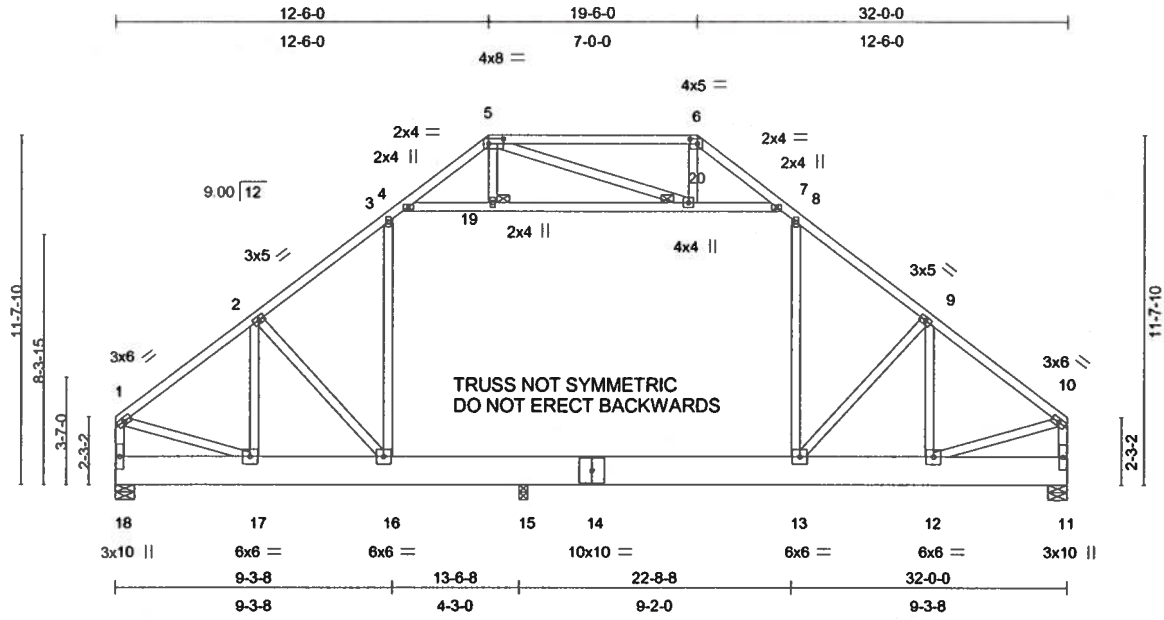
ENGINEERING BY
TRENCO
 A MiTek Affiliate
 818 Soundside Road
 Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	
HINTA	A2	ROOF TRUSS	7	1	E3875703

SANTA FE TRUSS, HIGH SPRINGS, FL

Job Reference (optional)

6.500 s Feb 5 2007 MiTek Industries, Inc. Tue Feb 20 12:36:07 2007 Page 1



Scale = 1:77.5

Plate Offsets (X,Y): [5:0-6-0,0-2-0], [6:0-3-0,0-2-0]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.33	Vert(LL)	-0.13 13-15	>999	240	MT20	244/190
TCDL 10.0	Lumber Increase	1.25	BC 0.20	Vert(TL)	-0.19 13-15	>999	180		
BCLL 0.0	Rep Stress Incr	YES	WB 0.34	Horz(TL)	0.01 11	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)						
								Weight: 317 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 12 SYP 2250F 1.9E
WEBS 2 X 4 SYP No.3 *Except*
4-7 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-3-8 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
JOINTS 1 Brace at Jt(s): 19, 20

REACTIONS (lb/size) 18=1230/0-7-10, 11=1397/0-7-10, 15=1143/0-3-8
Max Horz 18=-327(LC 3)
Max Uplift 18=-61(LC 6), 11=-94(LC 6), 15=-105(LC 4)
Max Grav 18=1230(LC 1), 11=1397(LC 1), 15=1200(LC 8)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-1200/96, 2-3=-1328/243, 3-4=-1078/168, 4-5=-673/173, 5-6=-520/161, 6-7=-676/158, 7-8=-1074/157,
8-9=-1338/135, 9-10=-1428/130, 1-18=-1107/68, 10-11=-1312/110
BOT CHORD 17-18=-325/319, 16-17=-212/899, 15-16=-2/987, 14-15=-2/987, 13-14=-2/987, 12-13=-7/1083, 11-12=-15/97
WEBS 1-17=-45/855, 5-19=0/63, 5-20=-58/66, 6-20=0/69, 8-13=0/249, 9-13=-196/177, 9-12=-141/112, 10-12=-3/1055,
3-16=-131/318, 4-19=-505/181, 19-20=-500/184, 7-20=-470/159, 2-17=-565/261, 2-16=-252/387

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- Ceiling dead load (5.0 psf) on member(s). 3-4, 7-8, 4-19, 19-20, 7-20
- Bottom chord live load (40.0 psf) and additional bottom chord dead load (0.0 psf) applied only to room. 15-16, 13-15
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 61 lb uplift at joint 18, 94 lb uplift at joint 11 and 105 lb uplift at joint 15.

LOAD CASE(S) Standard

Martin Stapp

February 20, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

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ENGINEERING BY
TRENCO
A MiTek Affiliate

818 Soundside Road
Edenton, NC 27932

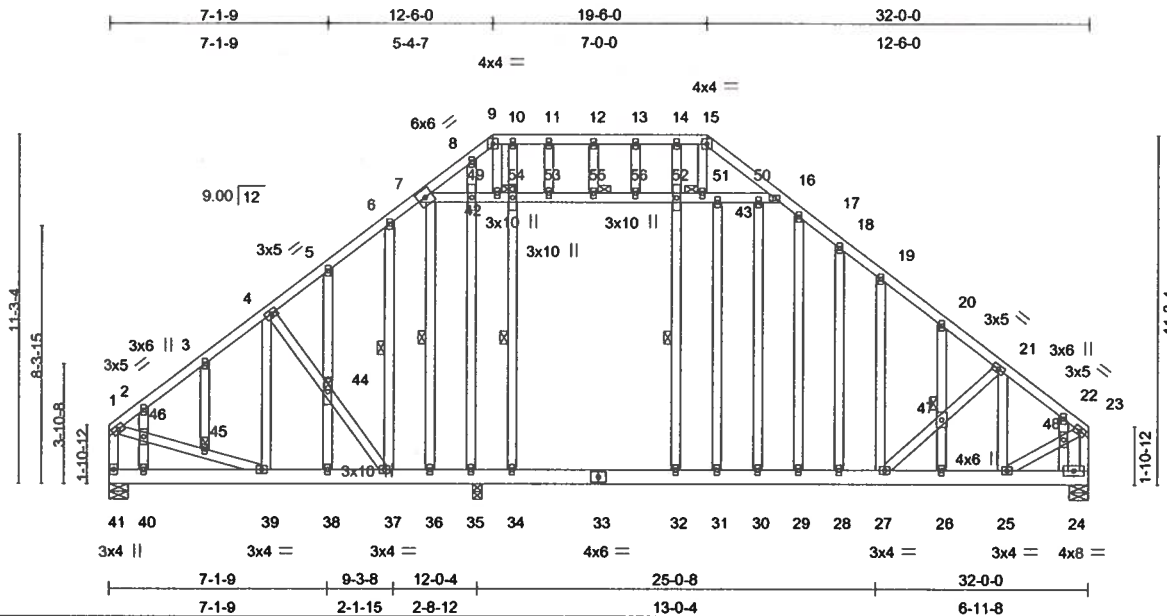
Job	Truss	Truss Type	Qty	Ply	
HINTA	AET2	GABLE	1	1	

E3875708

Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS, FL

6.500 s Feb 5 2007 MiTek Industries, Inc. Tue Feb 20 12:36:13 2007 Page 1



Scale = 1:75.2

LOADING (psf)	SPACING	CS ⁱ	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.42	Vert(LL) 0.11	27	>999	240		MT20	244/190
TCDL 10.0	Lumber Increase 1.25	BC 0.39	Vert(TL) -0.15	27	>999	180			
BCLL 0.0	Rep Stress Incr YES	WB 0.53	Horz(TL) 0.02	24	n/a	n/a			
BCDL 10.0	Code FBC2004/TPI2002	(Matrix)							
								Weight: 360 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2D
 BOT CHORD 2 X 6 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-8-6 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. Except:
 6-0-0 oc bracing: 24-25.
 WEBS 1 Row at midpt 6-37, 7-36, 34-54, 32-52
 JOINTS 1 Brace at Jt(s): 42, 43, 44, 45, 47, 55

REACTIONS (lb/size) 41=1197/0-7-10, 24=1241/0-7-10, 35=93/0-3-8
 Max Horz 41=403(LC 4)
 Max Uplift 41=-385(LC 6), 24=-385(LC 6), 35=-538(LC 4)
 Max Grav 41=1197(LC 1), 24=1271(LC 10), 35=524(LC 3)

Truss designed for wind loads in the plane of the truss only.
 For studs exposed to wind (normal to the face), see
 Standard Industry Gable End Details as applicable, or
 consult qualified building designer as per ANSI/TPI 1-2002.

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-1228/396, 2-3=-1263/457, 3-4=-1196/484, 4-5=-1181/575, 5-6=-1152/591, 6-7=-1162/650, 7-8=-757/489,
 8-9=-691/438, 9-10=-617/400, 10-11=-615/399, 11-12=-615/399, 12-13=-615/399, 13-14=-615/399, 14-15=-616/399,
 15-16=-783/439, 16-17=-1087/493, 17-18=-1181/568, 18-19=-1182/509, 19-20=-1195/455, 20-21=-1207/376,
 21-22=-1022/379, 22-23=-925/306, 1-41=-988/314, 23-24=-993/325
BOT CHORD 40-41=-371/369, 39-40=-371/369, 38-39=-164/958, 37-38=-164/958, 36-37=-117/919, 35-36=-126/930, 34-35=-126/930,
 33-34=-126/930, 32-33=-126/930, 31-32=-126/930, 30-31=-126/930, 29-30=-126/930, 28-29=-126/930, 27-28=-126/930,
 26-27=-210/780, 25-26=-210/780, 24-25=-115/57
WEBS 1-46=-326/866, 45-46=-330/885, 39-45=-335/884, 9-42=-133/263, 15-43=-190/315, 19-27=-95/77, 27-47=-70/194,
 21-47=-55/207, 21-25=-374/62, 25-48=-317/1061, 23-48=-334/1083, 6-37=-69/134, 7-49=-432/451, 42-49=-432/451,
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 3-45=-17/44, 2-46=-117/129, 40-46=-185/174, 20-47=-111/111, 26-47=-92/74, 22-48=-201/101, 24-48=-310/130,
 7-36=-201/232, 35-49=-171/174, 34-54=-93/195, 8-49=-90/72, 17-29=-133/147, 18-28=-85/85, 30-50=-33/58,
 31-51=-126/184, 32-52=-85/141, 11-53=-28/29, 10-54=-166/180, 14-52=-118/103, 12-55=-42/30, 13-56=-33/30

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCCL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- All plates are 2x4 MT20 unless otherwise indicated.
- This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 385 lb uplift at joint 41, 385 lb uplift at joint 24 and 538 lb uplift at joint 35.

LOAD CASE(S) Standard

Mommi Stuyg

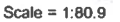
February 20, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIT-7473 BEFORE USE

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ENGINEERING BY
TRENCO
 A MiTek Affiliate
 818 Soundside Road
 Edenton, NC 27932

SANTA FE TRUSS, HIGH SPRINGS, FL 6.500 s Feb 5 2007 MiTek Industries, Inc. Tue Feb 20 12:36:14 2007 Page 1



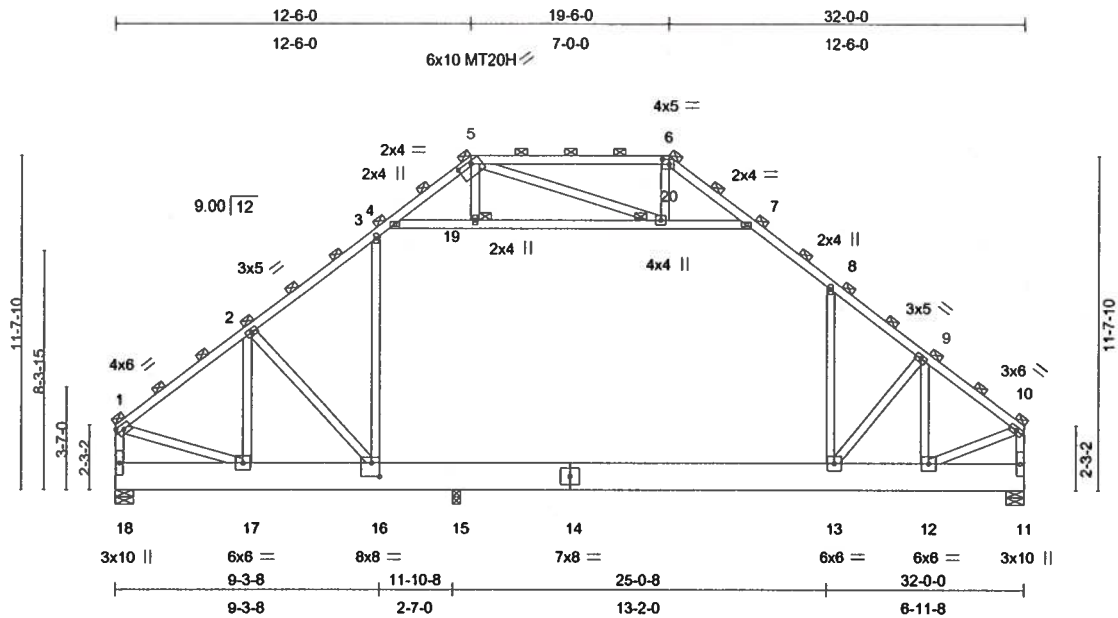
TRENCO ENGINEERING BY
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	E3875710
HINTA	AGT2	ROOF TRUSS	1	1	

SANTA FE TRUSS, HIGH SPRINGS, FL

Job Reference (optional)

6 500 s Feb 5 2007 MiTek Industries, Inc. Tue Feb 20 12:36:15 2007 Page 1



Scale = 1:81.0

Plate Offsets (X,Y): [5:0-6:0,0-1-4], [6:0-3:0,0-2-0], [16:0-3:8,0-5-12]

LOADING (psf)	SPACING	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.82	Vert(LL)	-0.43 13-15	>556	240	MT20	244/190
TCDL 10.0	Lumber Increase 1.25	BC 0.57	Vert(TL)	-0.62 13-15	>382	180	MT20H	187/143
BCLL 0.0	Rep Stress Incr NO	WB 0.58	Horz(TL)	0.02 11	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002	(Matrix)						
							Weight: 310 lb	

LUMBER

TOP CHORD 2 X 4 SYP 2400F 2.0E *Except*
5-6 2 X 4 SYP No.2D
BOT CHORD 2 X 12 SYP 2250F 1.9E
WEBS 2 X 4 SYP No.3 *Except*
4-7 2 X 4 SYP No.2

BRACING

TOP CHORD 2-0-0 oc purlins (4-6-1 max.), except end verticals
(Switched from sheeted: Spacing > 2-0-0).
BOT CHORD Rigid ceiling directly applied or 7-7-7 oc bracing.
JOINTS 1 Brace at Jt(s): 1, 5, 6, 10, 19, 20

REACTIONS (lb/size) 18=1683/0-7-10, 11=2333/0-7-10, 15=1954/0-3-8
Max Horz 18=-491(LC 3)
Max Uplift 18=-113(LC 6), 11=-132(LC 3), 15=-190(LC 4)
Max Grav 18=1683(LC 1), 11=2333(LC 1), 15=2133(LC 8)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-1569/173, 2-3=-2051/452, 3-4=-1642/307, 4-5=-977/269, 5-6=-662/244, 6-7=-933/242, 7-8=-1809/274,
8-9=-2118/163, 9-10=-2176/177, 1-18=-1451/127, 10-11=-2200/151
BOT CHORD 17-18=-485/476, 16-17=-286/1164, 15-16=0/1527, 14-15=0/1527, 13-14=0/1527, 12-13=-22/1692, 11-12=-16/82
WEBS 1-17=-163/1088, 5-19=0/94, 5-20=-150/62, 6-20=0/133, 8-13=0/400, 9-13=-297/268, 9-12=-215/131, 10-12=-38/1812,
3-16=-252/567, 4-19=-865/281, 19-20=-858/284, 7-20=-871/249, 2-17=-1246/351, 2-16=-341/865

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- All plates are MT20 plates unless otherwise indicated.
- This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- Ceiling dead load (5.0 psf) on member(s). 3-4, 7-8, 4-19, 19-20, 7-20
- Bottom chord live load (40.0 psf) and additional bottom chord dead load (0.0 psf) applied only to room. 15-16, 13-15
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 113 lb uplift at joint 18, 132 lb uplift at joint 11 and 190 lb uplift at joint 15.
- Design requires purlins at oc spacing indicated.

LOAD CASE(S) Standard

Mona A. Stapp
February 20, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITK REFERENCE PAGE MTL7473 BEFORE USE

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BC311 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

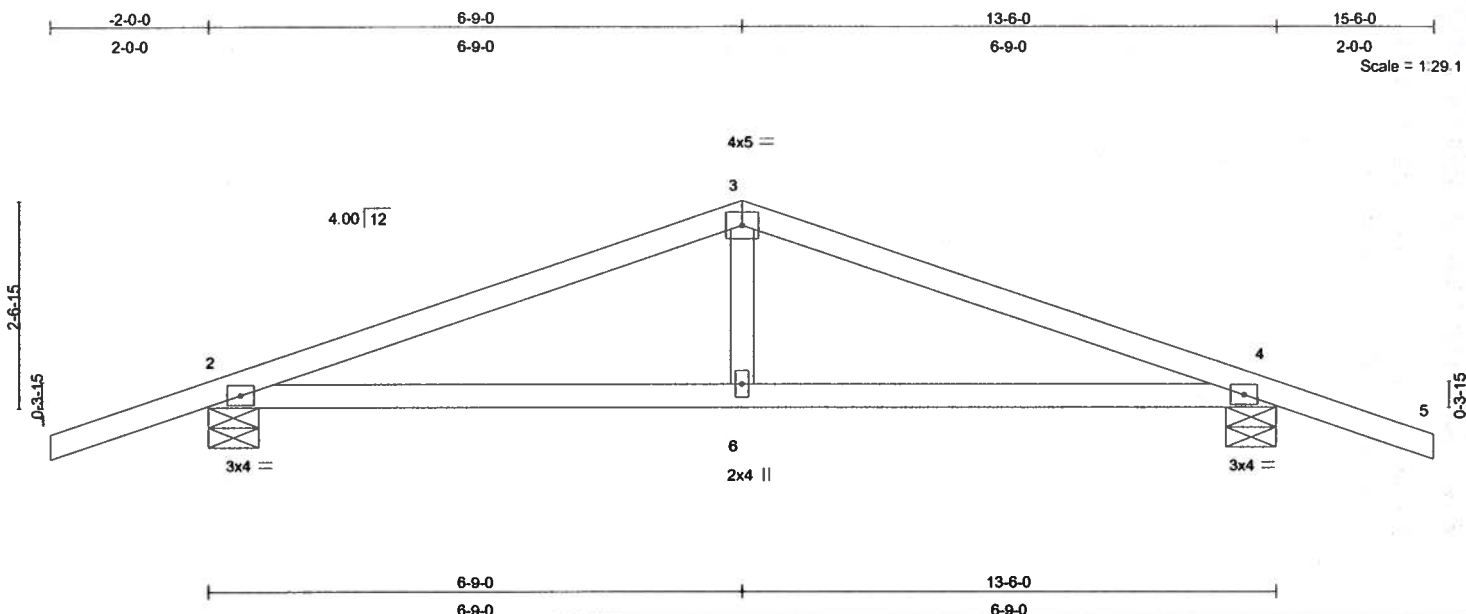
ENGINEERING BY
TRENCO
A MiTek Affiliate

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	E3875711
HINTA	B1	COMMON	7	1	Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS, FL

6.500 s Feb 5 2007 MITek Industries, Inc. Tue Feb 20 12:36:16 2007 Page 1



LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.26	Vert(LL)	-0.03	2-6	>999	240	MT20	244/190
TCDL 10.0	Lumber Increase	1.25	BC 0.30	Vert(TL)	-0.10	4-6	>999	180		
BCLL 0.0	Rep Stress Incr	YES	WB 0.09	Horz(TL)	0.02	4	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)							
									Weight: 51 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 2=654/0-7-10, 4=654/0-7-10
Max Horz 2=-50(LC 6)
Max Uplift 2=-164(LC 3), 4=-164(LC 4)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/37, 2-3=-897/70, 3-4=-897/70, 4-5=0/37
BOT CHORD 2-6=-18/788, 4-6=-18/788
WEBS 3-6=0/289

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 164 lb uplift at joint 2 and 164 lb uplift at joint 4.

LOAD CASE(S) Standard

Maria Stapp

February 20, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MD-7473 BEFORE USE.

Design valid for use only with Mittek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroff Drive, Madison, WI 53719.

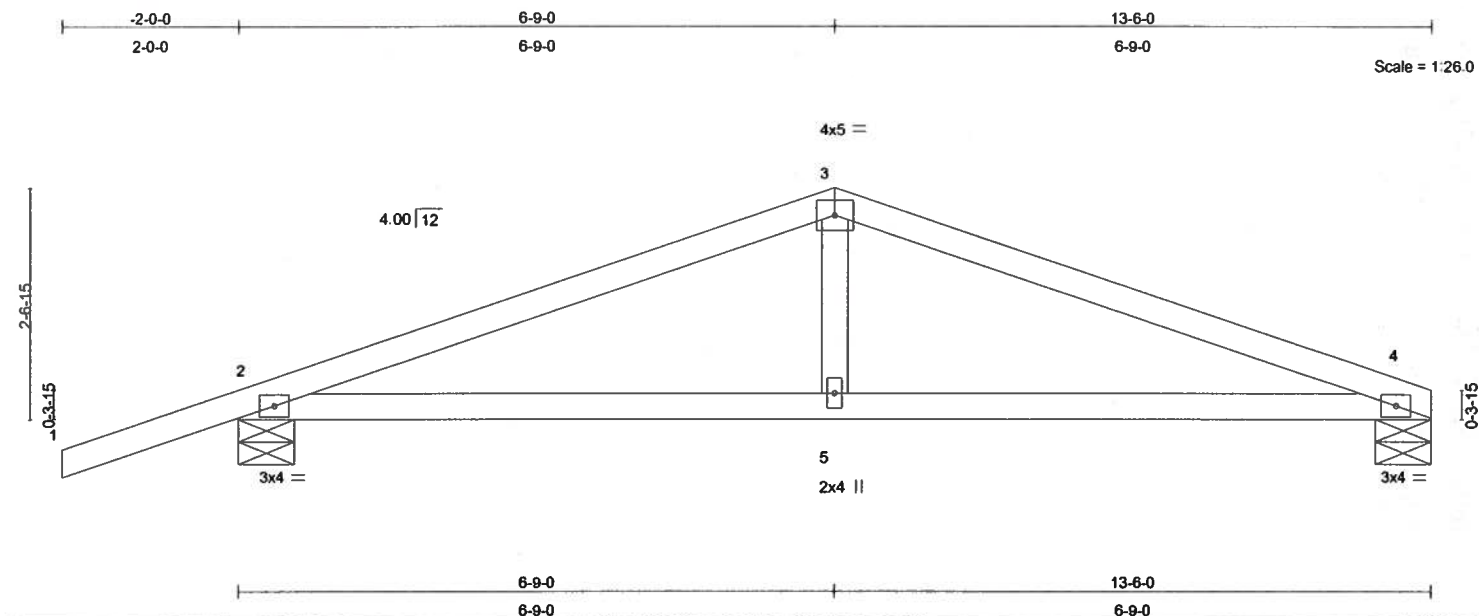
ENGINEERING BY
TRENCO
A Mittek Affiliate

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	E3875712
HINTA	B2	COMMON	4	1	Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS, FL

6.500 s Feb 5 2007 MiTek Industries, Inc. Tue Feb 20 12:36:17 2007 Page 1



LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.28	Vert(LL) -0.04	4-5	>999	240		MT20	244/190
TCDL 10.0	Lumber Increase 1.25	BC 0.31	Vert(TL) -0.12	4-5	>999	180			
BCLL 0.0	Rep Stress Incr YES	WB 0.09	Horz(TL) 0.02	4	n/a	n/a			
BCDL 10.0	Code FBC2004/TPI2002	(Matrix)							
								Weight: 48 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 4=502/0-7-10, 2=666/0-7-10
Max Horz 2=59(LC 5)
Max Uplift 4=61(LC 4), 2=166(LC 3)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/37, 2-3=-951/110, 3-4=-947/101
BOT CHORD 2-5=-49/841, 4-5=-49/841
WEBS 3-5=0/296

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 61 lb uplift at joint 4 and 166 lb uplift at joint 2.

LOAD CASE(S) Standard

Martin J. Stuyvesant

February 20, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

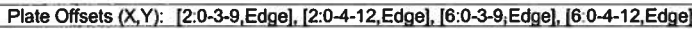
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D38-89 and BC311 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A MiTek Affiliate

818 Soundside Road
Edenton, NC 27932

SANTA FE TRUSS, HIGH SPRINGS, FL

6.500 s Feb 5 2007 MiTek Industries, Inc. Tue Feb 20 12:36:18 2007 Page 1

LOAD CASE(S) Standard

February 20, 2007

WARNING: Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MTL7473 BEFORE USE

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A MITek Affiliate

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Edenton, NC 27932

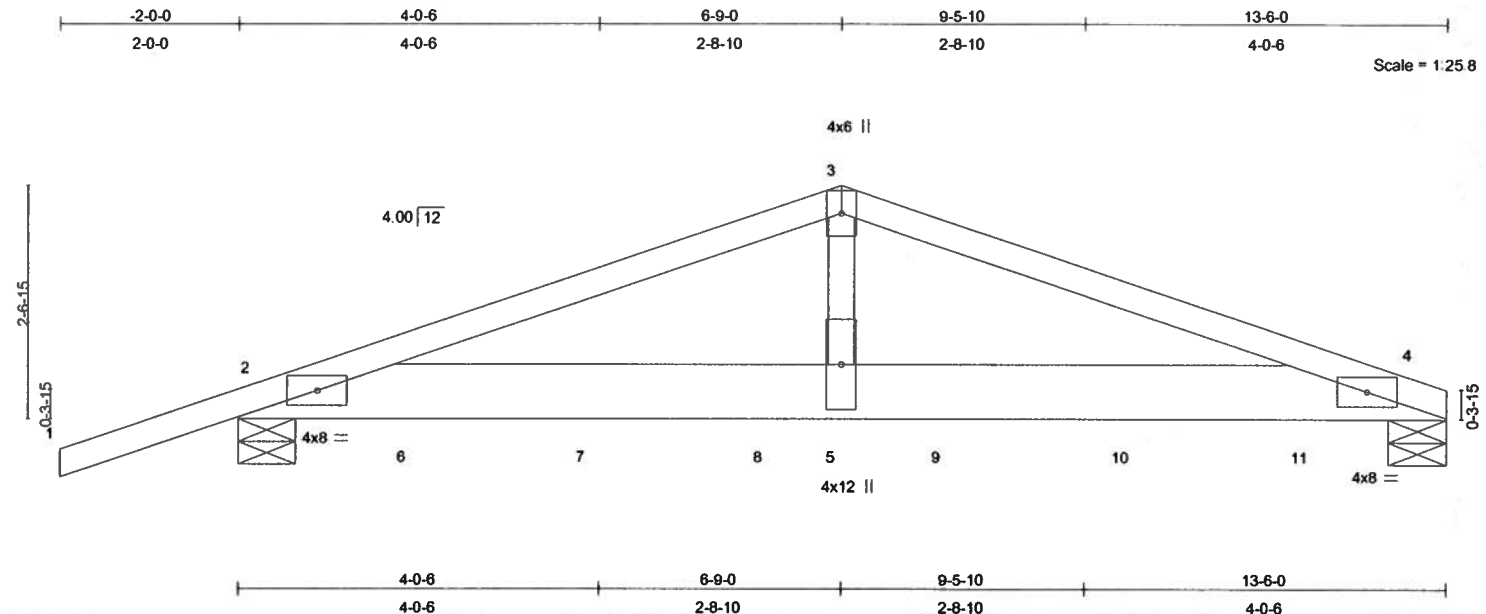
Job	Truss	Truss Type	Qty	Ply	
HINTA	BGT	COMMON	1	2	

E3875714

Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS, FL

6.500 s Feb 5 2007 MiTek Industries, Inc. Tue Feb 20 12:36:19 2007 Page 1



LOADING (psf)	SPACING	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.48	Vert(LL) -0.12	4-5	>999	240	MT20	244/190
TCDL 10.0	Lumber Increase 1.25	BC 0.53	Vert(TL) -0.24	4-5	>640	180		
BCLL 0.0	Rep Stress Incr NO	WB 0.83	Horz(TL) 0.04	4	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002	(Matrix)						
							Weight: 137 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2D
 BOT CHORD 2 X 8 SYP 2400F 2.0E
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-2-2 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=4753/0-7-10, 2=6183/0-7-10
 Max Horz 2=65(LC 5)
 Max Uplift 4=-430(LC 4), 2=-730(LC 3)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/45, 2-3=-9067/813, 3-4=-9066/806
 BOT CHORD 2-6=-722/8575, 6-7=-722/8575, 7-8=-722/8575, 8-9=-722/8575, 9-10=-722/8575, 10-11=-722/8575,
 4-11=-722/8575
 WEBS 3-5=-379/5199

NOTES

- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
 Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
 Bottom chords connected as follows: 2 X 8 - 2 rows at 0-3-0 oc.
 Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
- All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 430 lb uplift at joint 4 and 730 lb uplift at joint 2.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1506 lb down and 215 lb up at 0-3-13, 1377 lb down and 119 lb up at 1-11-4, 1377 lb down and 119 lb up at 3-11-4, 1377 lb down and 119 lb up at 5-11-4, 1377 lb down and 119 lb up at 7-11-4, and 1377 lb down and 119 lb up at 9-11-4, and 1377 lb down and 119 lb up at 11-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-3=-60, 3-4=-60, 2-4=-20

Continued on page 2

February 20, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE M1-7473 BEFORE USE.

Design valid for use only with Mittek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
 A Mittek Affiliate
 818 Soundside Road
 Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	E3875714
HINTA	BGT	COMMON	1	2	Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS, FL

6.500 s Feb 5 2007 MiTek Industries, Inc. Tue Feb 20 12:36:19 2007 Page 2

LOAD CASE(S) Standard

Concentrated Loads (lb)

Vert: 2=-1506(F) 6=-1377(F) 7=-1377(F) 8=-1377(F) 9=-1377(F) 10=-1377(F) 11=-1377(F)

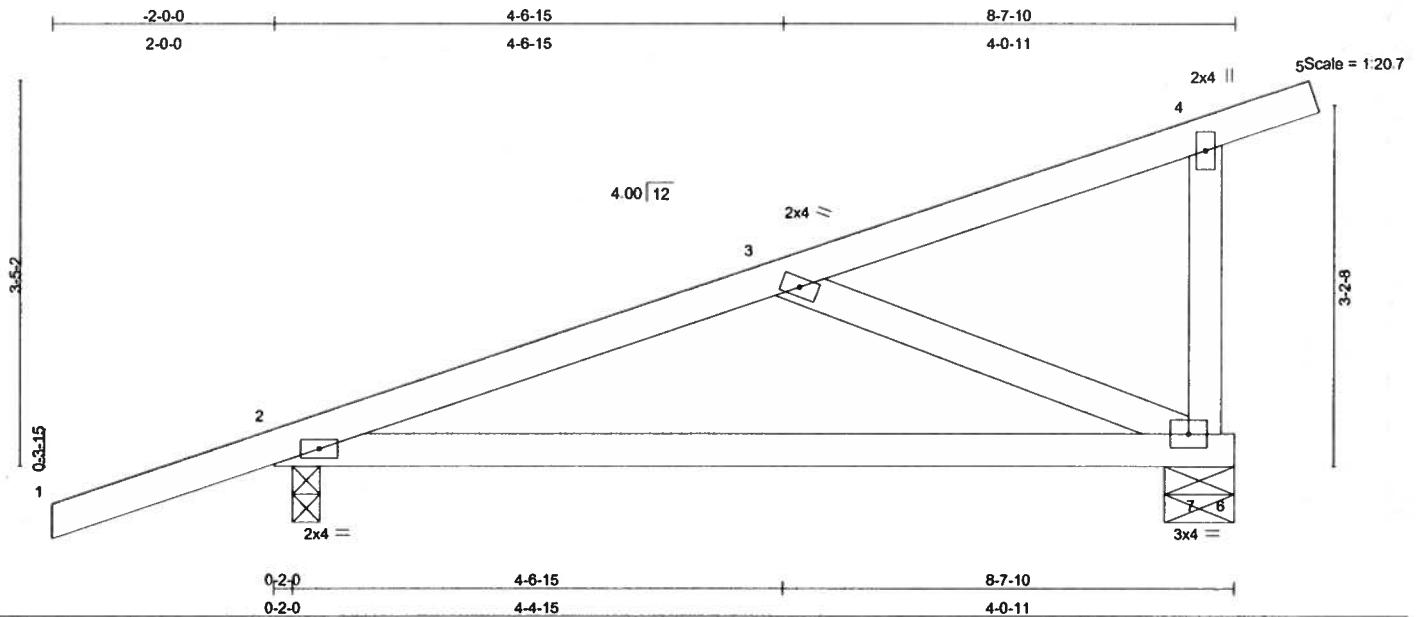
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-89 and BC311 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
 A MiTek Affiliate

818 Soundside Road
 Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply		E3875715
HINTA	C1	MONO TRUSS	32	1	Job Reference (optional)	
SANTA FE TRUSS, HIGH SPRINGS, FL			6.500 s Feb 5 2007 MiTek Industries, Inc. Tue Feb 20 12:36:19 2007 Page 1			



LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.22	Vert(LL)	-0.19	2-7	>512	240	MT20	244/190
TCDL 10.0	Lumber Increase	1.25	BC 0.41	Vert(TL)	-0.48	2-7	>205	180		
BCLL 0.0	Rep Stress Incr	YES	WB 0.11	Horz(TL)	0.00	7	n/a	n/a		
BCDL 10.0	Code FBC2004/TP12002		(Matrix)							
									Weight: 40 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3

REACTIONS (lb/size) 7=381/0-7-10, 2=470/0-3-0
Max Horz 2=132(LC 4)
Max Uplift 7=-68(LC 4), 2=-140(LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/37, 2-3=-398/101, 3-4=-57/33, 4-5=-18/0, 4-7=-162/73
BOT CHORD 2-7=-117/330, 6-7=0/0
WEBS 3-7=-355/134

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 68 lb uplift at joint 7 and 140 lb uplift at joint 2.

LOAD CASE(S) Standard

Martin A. Stapp

February 20, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

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ENGINEERING BY
TRENCO
A MiTek Affiliate

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	E3875716
HINTA	CET	GABLE	3	1	Job Reference (optional)
SANTA FE TRUSS, HIGH SPRINGS, FL					
6,500 s Feb 5 2007 MITek Industries, Inc. Tue Feb 20 12:36:20 2007 Page 1					

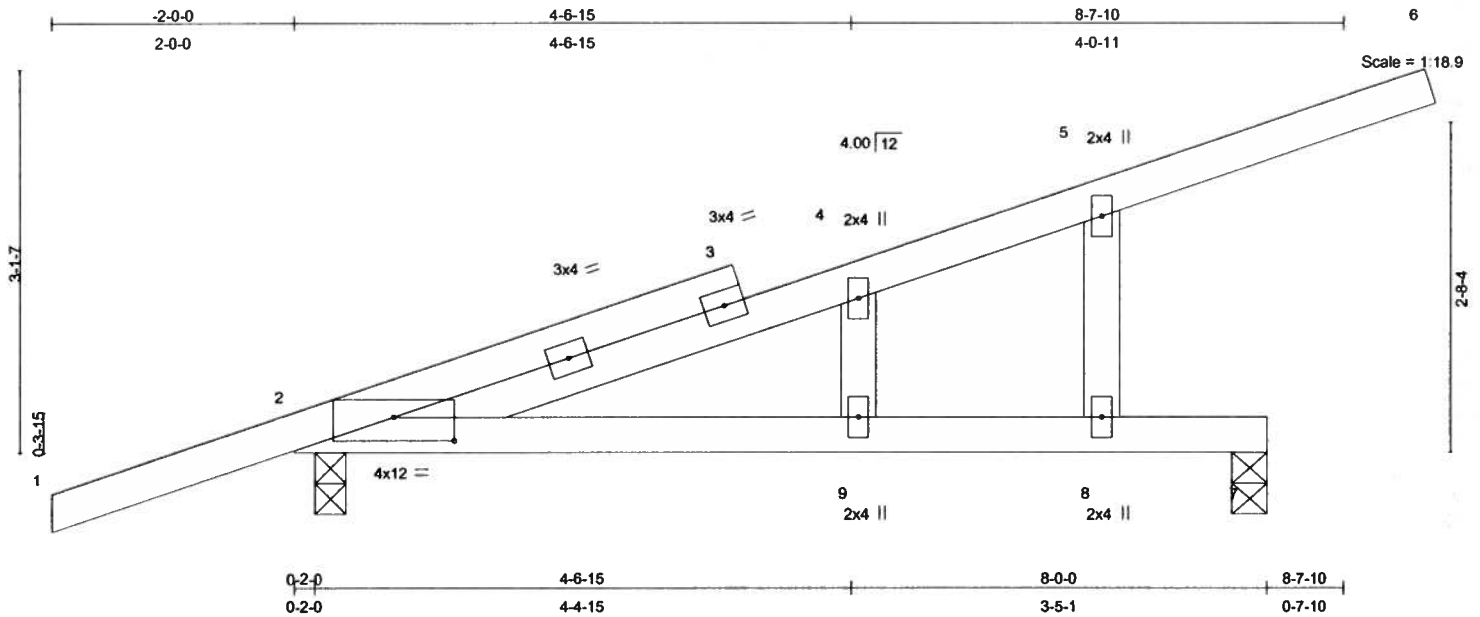


Plate Offsets (X,Y): [2:0-6-0,0-2-5]

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCCL 20.0	2-0-0	TC 0.52	Vert(LL)	-0.12	9	>791	240	MT20	244/190
TCDL 10.0	Plates Increase 1.25	BC 0.55	Vert(TL)	-0.31	9	>300	180		
BCCL 0.0	Lumber Increase 1.25	WB 0.08	Horz(TL)	0.00		n/a	n/a		
BCDL 10.0	Rep Stress Incr YES	(Matrix)							
	Code FBC2004/TPI2002								
								Weight: 38 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP 2400F 2.0E
OTHERS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 7=389/0-3-8, 2=446/0-3-0
Max Horz 2=117(LC 3)
Max Uplift 7=-103(LC 4), 2=-128(LC 3)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/37, 2-3=-74/0, 3-4=-65/9, 4-5=-77/101, 5-6=-51/0
BOT CHORD 2-9=0/0, 8-9=0/0, 7-8=0/0
WEBS 5-8=-480/196, 4-9=-47/144

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 103 lb uplift at joint 7 and 128 lb uplift at joint 2.
- 7) Non Standard bearing condition. Review required.

LOAD CASE(S) Standard

Monica A. Strassburger

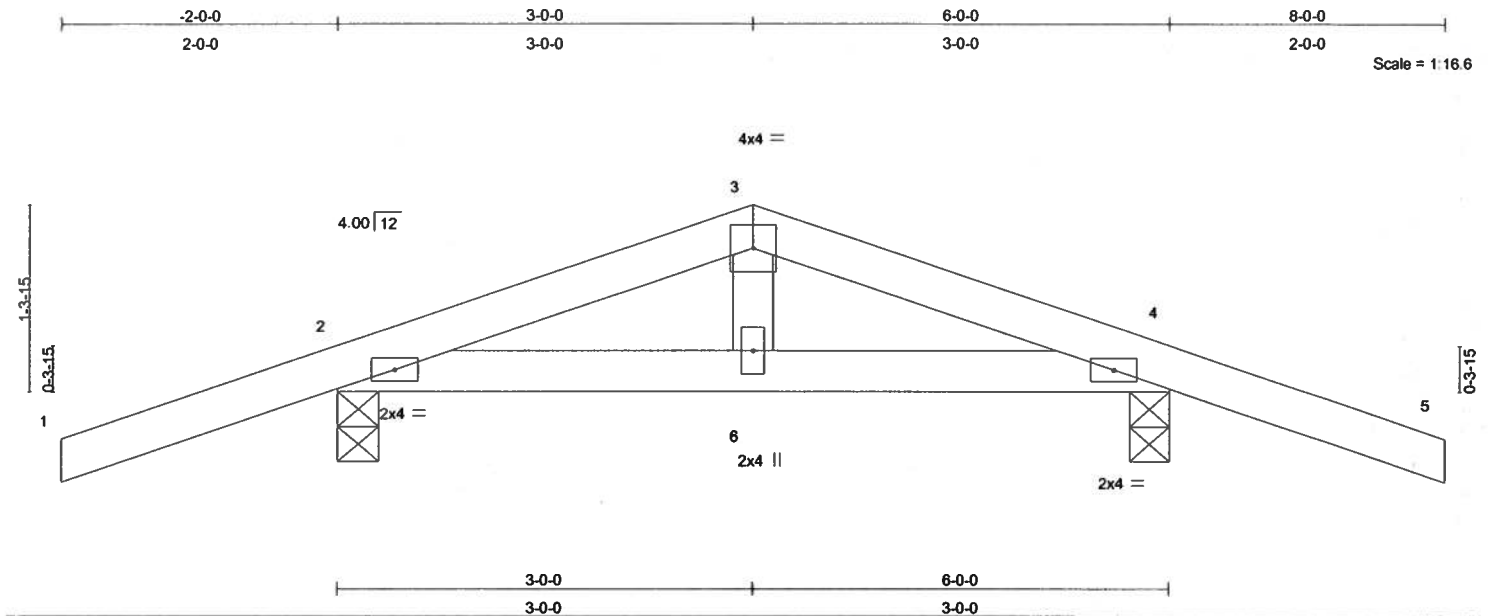
February 20, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MH-7473 BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D38-87 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mitek Affiliate
818 Soundside Road
Edenton, NC 27932

Job HINTA	Truss D1	Truss Type COMMON	Qty 1	Ply 1	E3875717
SANTA FE TRUSS, HIGH SPRINGS, FL					Job Reference (optional) 6.500 s Feb 5 2007 MiTek Industries, Inc. Tue Feb 20 12:36:20 2007 Page 1



LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.23	Vert(LL)	-0.00	6	>999	240	MT20	244/190
TCDL 10.0	Lumber Increase 1.25	BC 0.09	Vert(TL)	-0.01	2-6	>999	180		
BCLL 0.0	Rep Stress Incr YES	WB 0.04	Horz(TL)	0.00	4	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002	(Matrix)							
									Weight: 26 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6'-0" oc purlins.
BOT CHORD Rigid ceiling directly applied or 10'-0" oc bracing.

REACTIONS (lb/size) 2=357/0-3-8, 4=357/0-3-8
Max Horz 2=36(LC 5)
Max Uplift 2=-130(LC 3), 4=-130(LC 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/37, 2-3=-251/0, 3-4=-251/0, 4-5=0/37
BOT CHORD 2-6=0/222, 4-6=0/222
WEBS 3-6=0/135

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCCL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 130 lb uplift at joint 2 and 130 lb uplift at joint 4.

LOAD CASE(S) Standard

Mario A. Struppi

February 20, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with Mittek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mittek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Job Reference (optional)	E3875718
HINTA	DET	COMMON	1	1		
SANTA FE TRUSS, HIGH SPRINGS, FL						6.500 s Feb 5 2007 MiTek Industries, Inc. Tue Feb 20 12:36:21 2007 Page 1

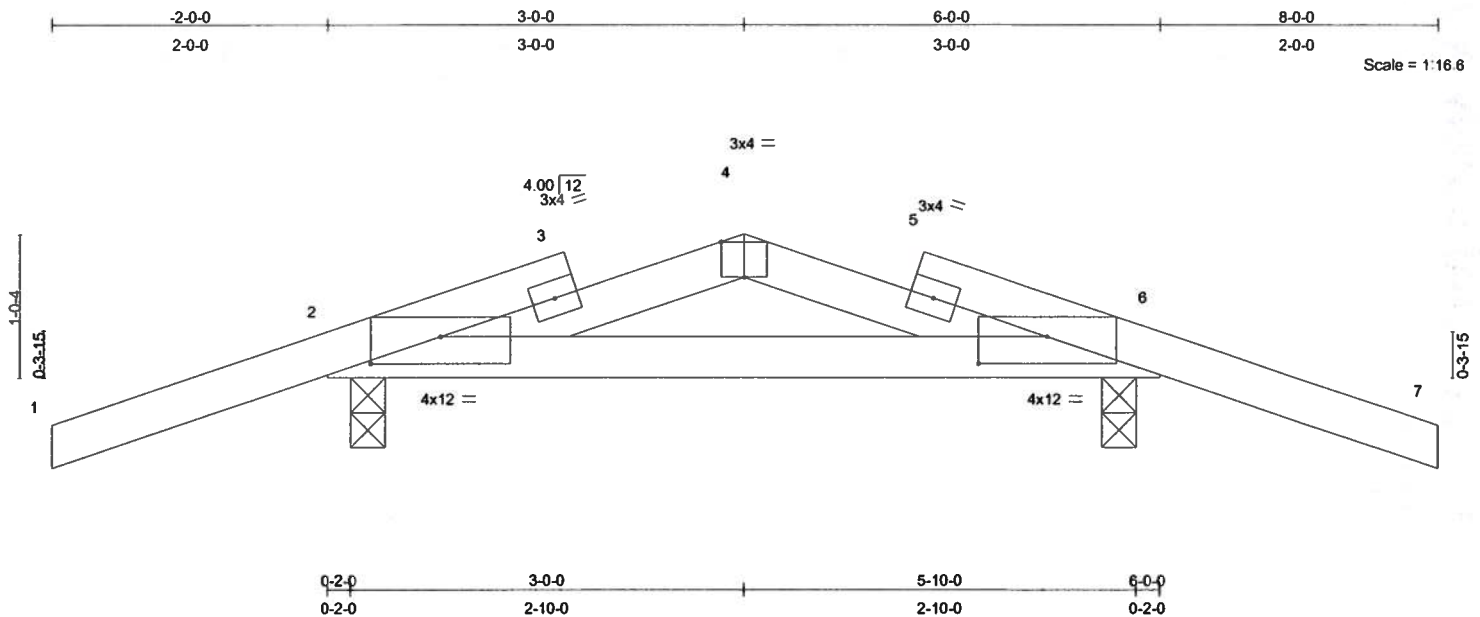


Plate Offsets (X,Y): [2-0-6-0,0-2-5], [4-0-2-0,Edge], [6-0-6-0,0-2-5]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.22	Vert(LL)	-0.05	2-6	>999	240	MT20	244/190
TCDL 10.0	Lumber Increase	1.25	BC 0.29	Vert(TL)	-0.13	2-6	>511	180		
BCLL 0.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	0.00	6	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)							
										Weight: 28 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 2=358/0-3-0, 6=358/0-3-0
Max Horz 2=-32(LC 6)
Max Uplift 2=-129(LC 3), 6=-129(LC 4)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/37, 2-3=-186/20, 3-4=-164/29, 4-5=-164/29, 5-6=-186/20, 6-7=0/37
BOT CHORD 2-6=-1/156

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 129 lb uplift at joint 2 and 129 lb uplift at joint 6.

LOAD CASE(S) Standard

Moni Shyngul

February 20, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

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818 Soundside Road
Edenton, NC 27932



February 20, 2007

Job	Truss	Truss Type	Qty	Ply	3	E3875720
HINTA	EET	MONO TRUSS	2	1	Job Reference (optional)	
SANTA FE TRUSS, HIGH SPRINGS, FL			6 500 s Feb 5 2007 MiTek Industries, Inc. Tue Feb 20 12:36:22 2007 Page 1			

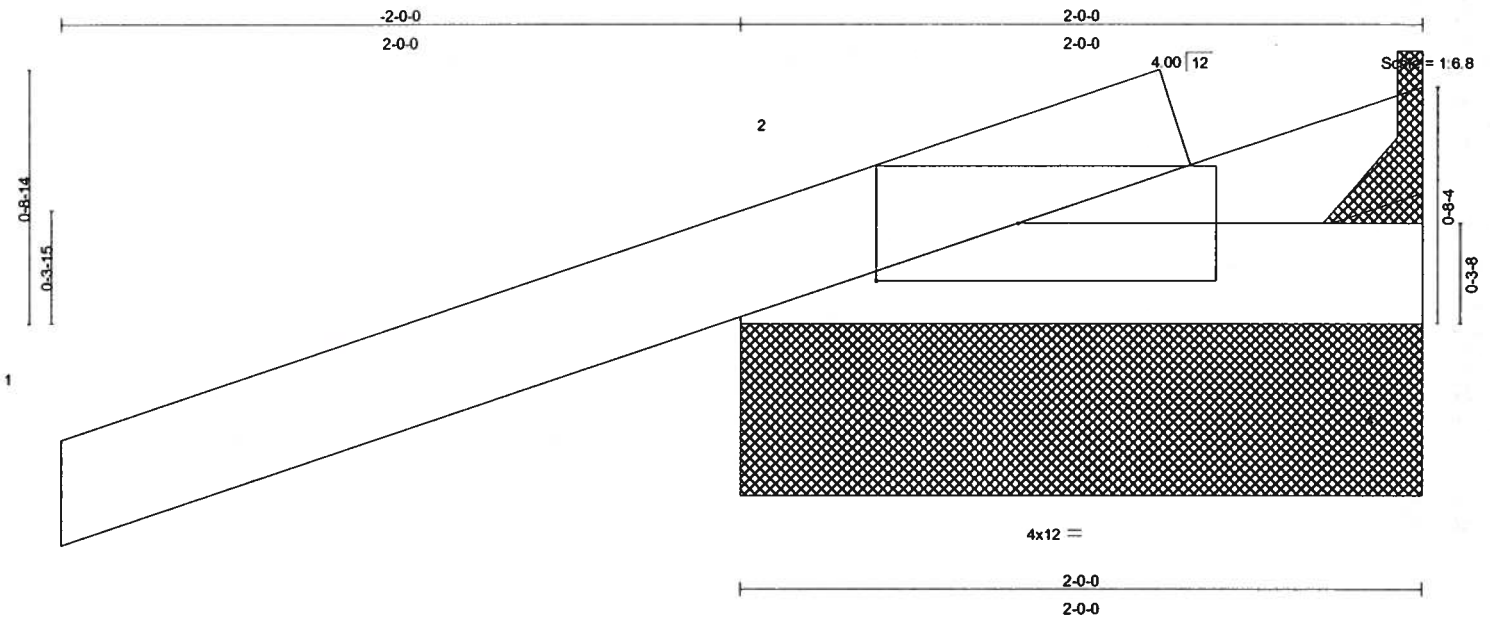


Plate Offsets (X,Y): [2:0-5:0,0-2:0]

LOADING (psf)	SPACING	2:0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.28	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL 10.0	Lumber Increase	1.25	BC 0.02	Vert(TL)	-0.00	2	>999	180		
BCLL 0.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)							
										Weight: 10 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=305/2-0-0, 3=-48/2-0-0, 3=-48/2-0-0, 4=17/2-0-0

Max Horz 2=45(LC 3)
Max Uplift 2=-176(LC 3), 3=-48(LC 1), 3=-48(LC 1)
Max Grav 2=305(LC 1), 3=53(LC 3), 4=33(LC 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/37, 2-3=-34/12
BOT CHORD 2-4=0/0

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- 4) Bearing at joint(s) 3 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 176 lb uplift at joint 2 and 48 lb uplift at joint 3.
- 6) Design requires purlins at oc spacing indicated.

LOAD CASE(S) Standard

Maria Stapp
February 20, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

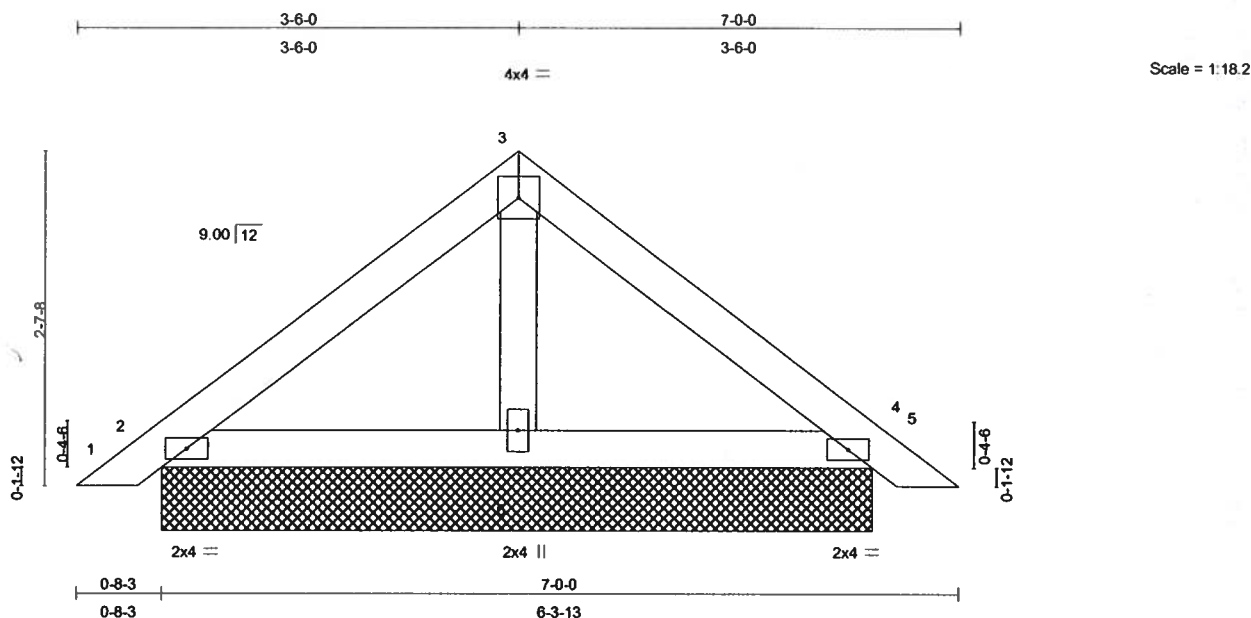
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Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	E3875721
HINTA	PIG1	ROOF TRUSS	21	1	

SANTA FE TRUSS, HIGH SPRINGS, FL.

Job Reference (optional)

6.500 s Feb 5 2007 MiTek Industries, Inc. Tue Feb 20 12:36:22 2007 Page 1



LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.09	Vert(LL)	0.00	5	n/r	120	MT20	244/190
TCDL 10.0	Plates Increase 1.25	BC 0.07	Vert(TL)	0.01	5	n/r	120		
BCLL 0.0	Lumber Increase 1.25	WB 0.02	Horz(TL)	0.00	4	n/a	n/a		
BCDL 10.0	Rep Stress Incr YES	(Matrix)							
	Code FBC2004/TPI2002								
								Weight: 24 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
OTHERS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=156/5-7-11, 4=156/5-7-11, 6=193/5-7-11
Max Horz 2=-59(LC 3)
Max Uplift 2=45(LC 5), 4=-52(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/15, 2-3=-78/42, 3-4=-78/30, 4-5=0/15
BOT CHORD 2-6=-9/36, 4-6=-9/36
WEBS 3-6=-122/25

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left and right exposed; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- Gable requires continuous bottom chord bearing.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 45 lb uplift at joint 2 and 52 lb uplift at joint 4.

LOAD CASE(S) Standard

Martin J. Stapp

February 20, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A MiTek Affiliate

818 Soundside Road
Edenton, NC 27932

PRODUCT APPROVAL SPECIFICATION SHEET

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and approval numbers on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. Statewide approved products are listed online @ www.floridabuilding.org

Category/Subcategory	Manufacturer	Product Description	Approval Number(s)
1. EXTERIOR DOORS			
A. SWINGING	Musenite International	Exterior doors	5507.1
B. SLIDING			
C. SECTIONAL/ROLL UP			
D. OTHER			
2. WINDOWS			
A. SINGLE/DOUBLE HUNG	MI Windows and doors	3'0" x 5'0" + 3'4" x 5'0" LoE vinyl windows	5438.9
B. HORIZONTAL SLIDER			
C. CASEMENT			
D. FIXED	MI Windows and doors	2'0" x 2'0" Octagon	5100.11
E. MULLION			
F. SKYLIGHTS			
G. OTHER			
3. PANEL WALL			
A. SIDING	✓ Hardie board	Hardie Panel Siding	889.5
B. SOFFITS	✓ Alcoa Home Exteriors Inc	Aluminum Soffit	5543.2
C. STOREFRONTS			
D. GLASS BLOCK			
E. OTHER			
4. ROOFING PRODUCTS			
A. ASPHALT SHINGLES			
B. NON-STRUCT METAL	Gulf Coast Metals	29 gauge Tuff-Rib	2672
C. ROOFING TILES			
D. SINGLE PLY ROOF			
E. OTHER			
5. STRUCT COMPONENTS			
A. WOOD CONNECTORS			
B. WOOD ANCHORS			
C. TRUSS PLATES			
D. INSULATION FORMS			
E. LINTELS	Crest Precast	Precast Lintels 4'-6'	4569
F. OTHERS			
6. NEW EXTERIOR ENVELOPE PRODUCTS			
A.			

The products listed below did not demonstrate product approval at plan review. I understand that at the time of inspection of these products, the following information must be available to the inspector on the jobsite; 1) copy of the product approval, 2) performance characteristics which the product was tested and certified to comply with, 3) copy of the applicable manufacturers installation requirements. Further, I understand these products may have to be removed if approval cannot be demonstrated during inspection.

David M. Best

APPLICANT SIGNATURE

3-7-07

DATE

SCHAFER ENGINEERING, LLC

**7104 N. W. 42ND LANE
GAINESVILLE, FLORIDA 32606**

THE HINTON RESIDENCE

**SCHAFER ENGINEERING, LLC
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Florida Building Code \ Latest Edition**

386-462-1340 / 352-375-6329

SCHAFER ENGINEERING, LLC

February 8, 2007

SUMMARY: Wind Load Analysis for The Hinton Residence
Wind Speed: 110 M.P.H. \ No Copies Permitted \ one use only
Florida Building Code \ Latest Edition

Foundation:

20" wide x 10" deep stemwall footing with (2) #5 rebar continuous minimum. CMU walls must have #5 dowels at 64" o.c. maximum with a standard 90 degree ACI hook in footing and a 4" slab on grade. Monolithic slab to be 12" wide x 20" deep minimum with (2) #5 rebar continuous with 12" minimum coverage on face of foundation. It is assumed that ideal soil conditions and pad preparation are provided.

Walls:

8" CMU block with vertical #5 reinforcing bar in grout filled cell at 64" o.c. maximum spacing. Wall heights are 9' maximum. Provide an 8" x 8" bond beam with 1-#5 rebar horizontal continuous at the top course. Install pre-cast, pre-engineered lintels or pre-engineered steel lintels spanning over all openings. One #5 rebar each corner. One #5 rebar each side of door and window openings. Two #5 rebar in openings wider than 12'-0". One #5 rebar where girders or girder trusses bear on masonry wall.

Shearwalls:

Transverse: 70'-0"

Longitudinal: 38'-0"

Allowable pounds per foot unit shear on shearwalls: 322

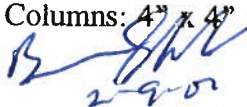
Trusses:

Pre-engineered Pre-fabricate trusses installed with the bracing system designed by the manufacturer. Trusses must be anchored according to the truss engineering requirements. Trusses must bear on exterior walls and porch headers.

Roof Sheathing:

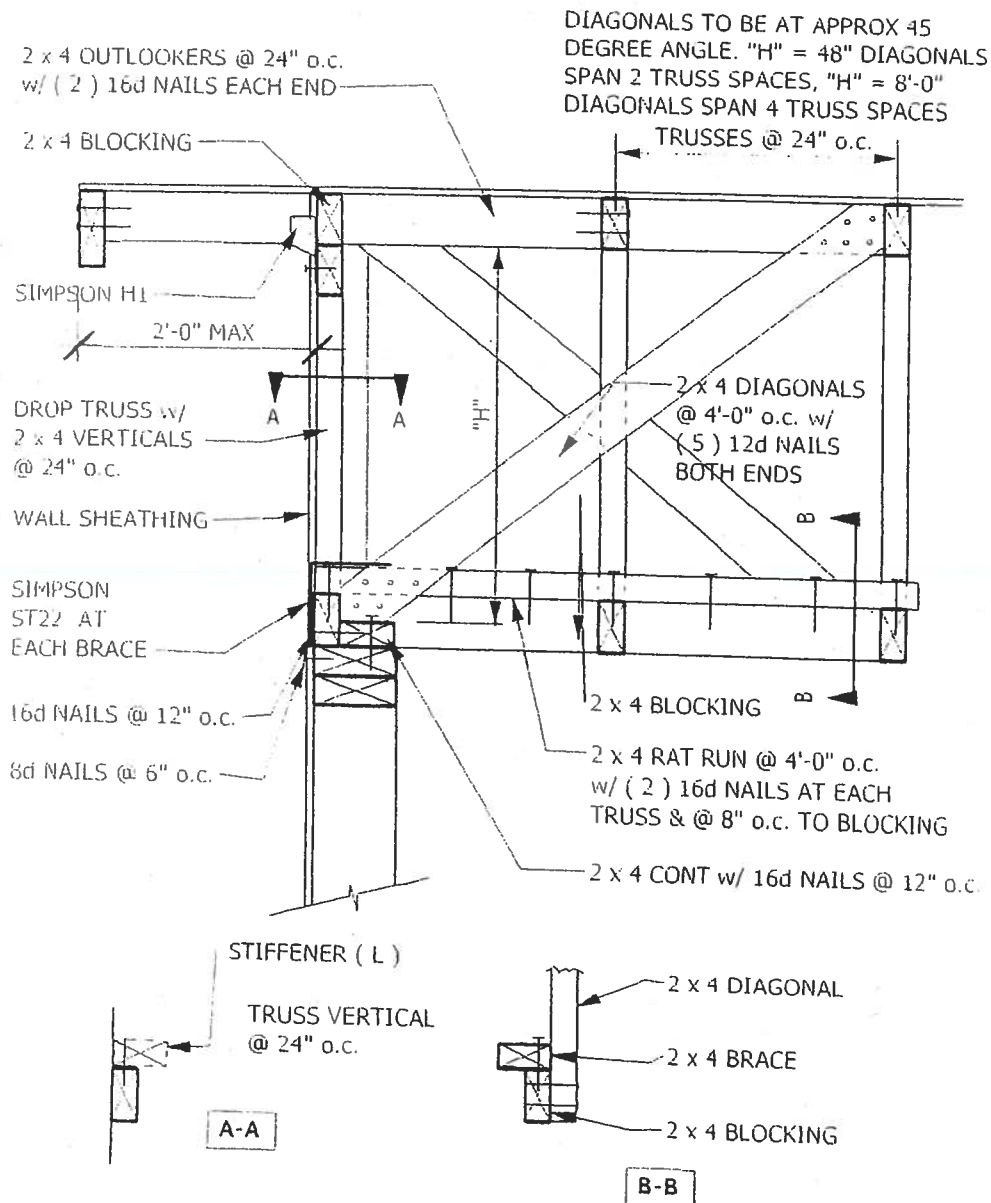
7/16" osb minimum attached to the top chords of the trusses with 8d/131 gauge nails spaced at 4" o.c. edges and 8" interior. Install a ceiling diaphragm on open porches using same the nails and nail patterns as the roof sheathing.

Columns: 4" x 4" x 9' syp #2 pt @ 144" maximum spacing.


Bruce Schafer P. E.
7104 N. W. 42nd Lane
Gainesville, Florida 32606

SCHAFFER ENGINEERING, LLC

7104 N. W. 42ND LANE
GAINESVILLE, FLORIDA 32606



TYPICAL GABLE END BRACING

Historical Rel.

[Signature]
2-9-07

48984
7104 NW 42nd Ln
Gainesville, FL

TIE-DOWN TABLES

HEADERS				
Uplift Force Lbs	Top Connector **	Rating Lbs	Bottom Connector **	Rating Lbs
to 455	LSTA9	725	H3	455
to 910	LSTA12	905	2-H3	910
to 1265	LSTA18	1265	LTT19	1350
to 1750	2-LSTA12	1810	LTT20	1750
to 2530	2-LSTA18	2530	HD2A-2.5	2565
to 2865	3-LSTA18	3255	HD2A-3.5	2865
to 3700	3-LSTA24	3880	HD5A-3	3700

Total uplift for each truss resting on the header and divide by 2 to determine the uplift force.
Use proper bolt anchors sufficient to support required load.

TRUSSES/GIRDERS		
Uplift Force Lbs	Top Connector **	Bottom Connector **
to 500	H2.5	N/A
501-1049	H10	N/A
1050-1350	TS22	LTT19
1351-1750	2-TS22	LTT20
1751-2570	2-TS22	HD2A
2571-3665	3-TS22	HD5A
3666-5260	2-MST148	HTT22
5261-8300	2-MST48	HD10A

Two 12d common toenails are required per truss/rafter per bearing point into plate.
Use proper bolt anchors.

Strap rafters to truss or at each end with minimum uplift resistance of 450# each end.
Strap ridge beam at each end with minimum uplift resistance of 1000#.

It is the contractors responsibility to provide a continuous load path from truss/rafter/ridge beam to foundation.

	Top Connector **	Rating Lbs	Bottom Connector **	Rating
BEAM SEATS	LSTA18*	1200	LTT19*	1250
POSTS (max 17' spacing)	2-LSTA18	2400	ABU44	2300

*or per truss engineering

Use proper bolt anchors

All beams to be sheathed or strapped to Double Top Plate when applicable.

CRIPPLES

Sheathing nailing alone adequate w/8d nails @ 3" O.C.

STUDS

Wall sheathing nailing Adequate exterior walls bottom w/8d nails @ 3" O.C.

Use SP1 & SP2 @ 32" O.C. on all bearing walls.

Interior anchor bolts to be 1/2" x 8" A307 or 1/2" x 7" wedge anchor or equivalent.

** Equivalent Simpson hardware, or other manufacturer, may be substituted for any of the hardware specified on this page as long as it meets the required load capacities/uplift resistance.

NOTE:

1. For nailing into SPF members, multiply table values by .86

2. See truss engineering for anchor tie-down values.

ASCE 7-02

2/8/07

Wind Load Design per ASCE 7-02

User Input Data		
Structure Type	Building	
Basic Wind Speed (V)	110	mph
Structural Category	II	
Exposure	B	
Struc Nat Frequency (n1)	1	Hz
Slope of Roof (Theta)	36.9	Deg
Type of Roof	Gabled	
Eave Height (Eht)	9.33	ft
Ridge Height (RHt)	25.38	ft
Mean Roof Height (Ht)	18.84	ft
Width Perp. to Wind (B)	64.00	ft
Width Parallel to Wind (L)	44.00	ft
Damping Ratio (beta)	0.01	

Red values should be changed only through "Main Menu"

Calculated Parameters	
Type of Structure	
Height/Least Horizontal Dim	0.43
Flexible Structure	No

Calculated Parameters	
Importance Factor	1
Hurricane Prone Region (V>100 mph)	
Table C6-4 Values	
Alpha =	7.000
zg =	1200.000
At =	0.143
Bt =	0.840
Am =	0.250
Bm =	0.450
Cc =	0.300
I =	320.00 ft
Epsilon =	0.333
Zmin =	30.00 ft

Gust Factor Category I: Rigid Structures - Simplified Method		
Gust1	For rigid structures (Nat Freq > 1 Hz) use 0.85	0.85
Gust Factor Category II: Rigid Structures - Complete Analysis		
Zm	Zmin	30.00 ft
Izm	Cc * (33/z)^0.167	0.3048
Lzm	I*(zm/33)^Epsilon	309.99 ft
Q	(1/(1+0.63*((B+Ht)/Lzm)^0.63))^0.5	0.8858
Gust2	0.925*((1+1.7*Izm*3.4*Q)/(1+1.7*3.4*Izm))	0.8576
Gust Factor Category III: Flexible or Dynamically Sensitive Structures		
Vhref	V*(5280/3600)	161.33 ft/s
Vzm	bm*(zm/33)^Am*Vhref	70.89 ft/s
NF1	NatFreq*Lzm/Vzm	4.37 Hz
Rn	(7.47*NF1)/(1+10.302*NF1)^1.667	0.0552
Nh	4.6*NatFreq*Ht/Vzm	1.22
Nb	4.6*NatFreq*B/Vzm	4.15
Nd	15.4*NatFreq*Depth/Vzm	9.56
Rh	1/Nh-(1/(2*Nh^2)*(1-Exp(-2*Nh))))	0.5125
Rb	1/Nb-(1/(2*Nb^2)*(1-Exp(-2*Nb))))	0.2118
Rd	1/Nd-(1/(2*Nd^2)*(1-Exp(-2*Nd))))	0.0991
RR	((1/Beta)*Rn*Rh*Rb*(0.53+0.47*Rd))^0.5	0.5877
gg	+(2*LN(3600*n1))^0.5+0.577/(2*LN(3600*n1))^0.5	4.19
Gust3	0.925*((1+1.7*Izm*(3.4^2*Q^2+GG^2*RR^2)^0.5)/(1+1.7*3.4*Izm))	1.01

Gust Factor Summary	
Main Wind-force resisting system:	Components and Cladding:
Gust Factor Category: I	Gust Factor Category: I
Gust Factor (G) 0.86	Gust Factor (G) 0.86

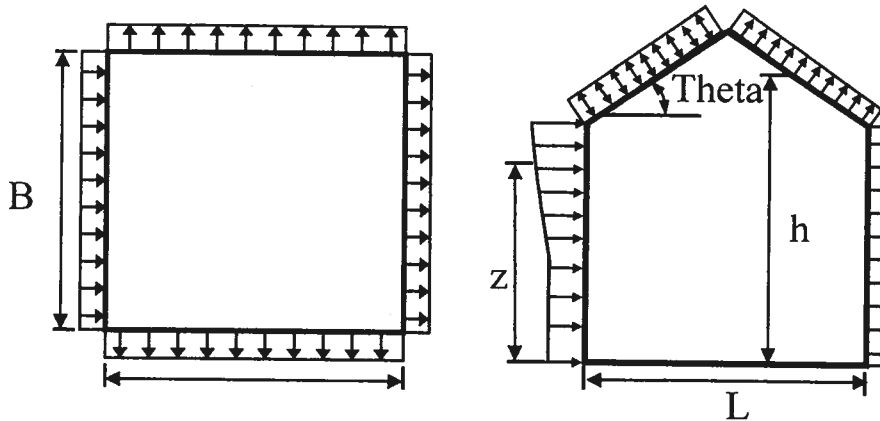
Wind Load Design per ASCE 7-02

6.5.12.2.1 Design Wind Pressure - Buildings of All Heights (Non-flexible)

Elev. ft	Kz	Kzt	Kd	qz lb/ft^2	Pressure (lb/ft^2)	
					Windward Wall*	
			1.00		+GCpi	-GCpi
25.38	0.70	1.00	1.00	21.70	11.47	18.31
20	0.70	1.00	1.00	21.70	11.47	18.31
18.84	0.70	1.00	1.00	21.70	11.47	18.31
15	0.70	1.00	1.00	21.70	11.47	18.31

Figure 6-3 - External Pressure Coefficients, Cp

Loads on Main Wind-Force Resisting Systems



Variable	Formula	Value	Units
Kh	$2.01 \cdot (Ht/zg)^{(2/\alpha)}$	0.61	
Kht	Topographic factor (Fig 6-2)	1.00	
Qh	$0.00256 \cdot (V)^2 \cdot \text{ImpFac} \cdot Kh \cdot Kht \cdot Kd$	19.00	psf

Wall Pressure Coefficients, Cp	
Surface	Cp
Windward Wall (See Figure 6.5.12.2.1 for Pressures)	0.80

Roof Pressure Coefficients, Cp	
Roof Area (sq. ft.)	-
Reduction Factor	1.00

Description	Cp	Pressure (psf)	
		+GCpi	-GCpi
Leeward Walls (Wind Dir Parallel to 64 ft wall)	-0.50	-11.57	-4.73
Leeward Walls (Wind Dir Parallel to 44 ft wall)	-0.41	-10.09	-3.25
Side Walls	-0.70	-14.83	-7.99
Roof - Normal to Ridge (Theta >= 10)			
Windward - Max Negative	0.00	0.00	0.00
Windward - Max Positive	0.00	0.00	0.00
Leeward Normal to Ridge	-0.60	-13.20	-6.36
Overhang Top	0.00	0.00	0.00
Overhang Bottom	0.80	0.69	0.69
Roof - Parallel to Ridge (All Theta)			
Dist from Windward Edge: 0 ft to 9.42 ft	-0.90	-18.09	-11.25

ASCE 7-02

2/8/07

Wind Load Design per ASCE 7-02

Dist from Windward Edge: 9.42 ft to 18.84 ft	-0.90	-18.09	-11.25
Dist from Windward Edge: 18.84 ft to 37.68 ft	-0.50	-11.57	-4.73
Dist from Windward Edge: > 37.68 ft	-0.30	-8.31	-1.47

* Horizontal distance from windward edge

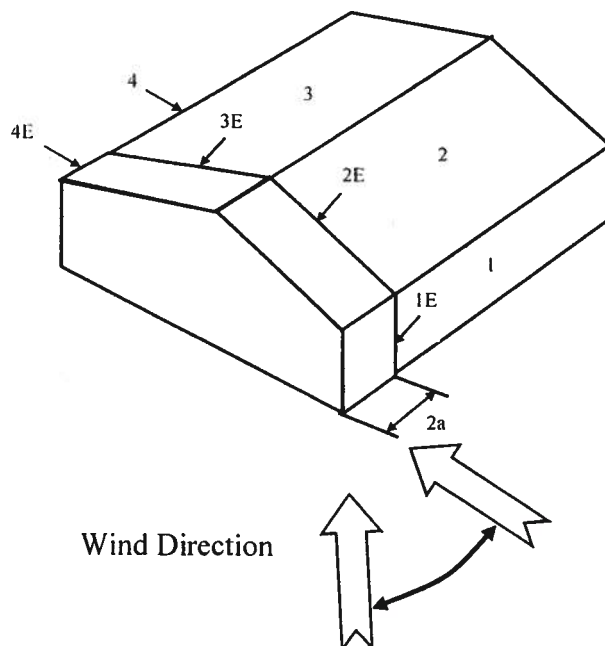
Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht ≤ 60 ft

Kh =	2.01*(Ht/zg)^(2/Alpha)	=	0.61
Kht =	Topographic factor (Fig 6-2)	=	1.00
Qh =	0.00256*(V)^2*ImpFac*Kh*Kht*Kd	=	19.00

Case A						
Surface	GCpf	+GCpi	-GCpi	qh (psf)	Min P (psf)	Max P (psf)
1	0.56	0.18	-0.18	21.70	8.25	16.06
2	0.21	0.18	-0.18	21.70	0.65	8.46
3	-0.43	0.18	-0.18	21.70	-13.24	-5.43
4	-0.37	0.18	-0.18	21.70	-11.94	-4.12
5	0.00	0.18	-0.18	21.70	-3.91	3.91
6	0.00	0.18	-0.18	21.70	-3.91	3.91
1E	0.69	0.18	-0.18	21.70	11.07	18.88
2E	0.27	0.18	-0.18	21.70	1.95	9.77
3E	-0.53	0.18	-0.18	21.70	-15.41	-7.60
4E	-0.48	0.18	-0.18	21.70	-14.32	-6.51
5E	0.00	0.18	-0.18	21.70	-3.91	3.91
6E	0.00	0.18	-0.18	21.70	-3.91	3.91

* p = qh * (GCpf - GCpi)



ASCE 7-02

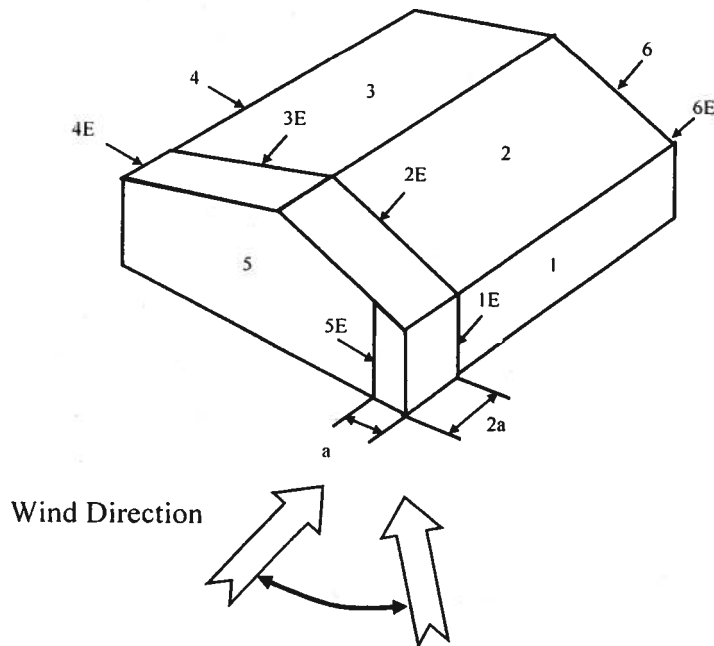
2/8/07

Wind Load Design per ASCE 7-02**Figure 6-4 - External Pressure Coefficients, GCpf**Loads on Main Wind-Force Resisting Systems w/ $H_t \leq 60$ ft

$$\begin{aligned}
 K_h &= 2.01 \cdot (H_t/z_g)^{(2/\alpha)} &= & 0.61 \\
 K_{ht} &= \text{Topographic factor (Fig 6-2)} &= & 1.00 \\
 Q_h &= 0.00256 \cdot (V)^2 \cdot \text{ImpFac} \cdot K_h \cdot K_{ht} \cdot K_d &= & 19.00
 \end{aligned}$$

Case B						
Surface	GCpf	+GCpi	-GCpi	qh (psf)	Min P (psf)	Max P (psf)
1	-0.45	0.18	-0.18	21.70	-13.67	-5.86
2	-0.69	0.18	-0.18	21.70	-18.88	-11.07
3	-0.37	0.18	-0.18	21.70	-11.94	-4.12
4	-0.45	0.18	-0.18	21.70	-13.67	-5.86
5	0.40	0.18	-0.18	21.70	4.77	12.59
6	-0.29	0.18	-0.18	21.70	-10.20	-2.39
1E	-0.48	0.18	-0.18	21.70	-14.32	-6.51
2E	-1.07	0.18	-0.18	21.70	-27.13	-19.31
3E	-0.53	0.18	-0.18	21.70	-15.41	-7.60
4E	-0.48	0.18	-0.18	21.70	-14.32	-6.51
5E	0.61	0.18	-0.18	21.70	9.33	17.14
6E	-0.43	0.18	-0.18	21.70	-13.24	-5.43

$$* p = q_h * (GCpf - GCpi)$$



2/8/07

Figure 6-5 - External Pressure Coefficients, G_{Cp}

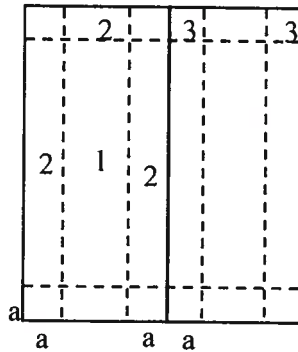
 $10 < \text{Theta} \leq 45$
$$a = 4.4 \implies \boxed{4.40 \text{ ft}}$$
[illegible]

Table 6-7 Internal Pressure Coefficients for Buildings, G_{cpi}

ASCE 7-02

2/8/07

Wind Load Design per ASCE 7-02

Condition	Gcpi	
	Max +	Max -
Open Buildings	0.00	0.00
Partially Enclosed Buildings	0.55	-0.55
Enclosed Buildings	0.18	-0.18
Enclosed Buildings	0.18	-0.18

Table 6-8 External Pressure Coefficients for Arched Roofs, Cp

r (Rise-to-Span Ratio) = 0.3

Condition	Variable	Cp		
		Windward Quarter	Center Half	Leeward Quarter
Roof on Elevated Structure	Cp	0.13	-1	-0.5
	P (+GCpi) - psf	-1.38	-19.72	-11.57
	P (-GCpi) -psf	5.46	-12.88	-4.73
Roof Springing from Ground	Cp	0.42	-1	-0.5
	P (+GCpi) - psf	3.42	-19.72	-11.57
	P (-GCpi) -psf	3.42	-19.72	-11.57

Table 6-9 Force Coefficients for Monoslope Roofs over Open Buildings, Cf

Variable	Description	Value	
L	Roof dimension normal to wind direction	44.00	ft
B	Roof dimension parallel to wind direction	64.00	ft
L/B	Ratio of L to B	0.688	
Theta	Slope of Roof	36.9	Deg
Cf	Force Coefficient	0.00	
X	Distance to center of pressure from windward edge	0.00	ft

Adam Hinton
HVAC Load Calculations

for

Adam Hinton
Lot 63 Santa Fe River Plantation
High Springs Fl



**RHVAC RESIDENTIAL
HVAC LOADS**

Prepared By:
Chuck Fischer
North Central Florida Air Conditioning
P.O Box 700
High Springs Fl 32655-0700
386-454-4767
Friday, February 09, 2007



Project Report

General Project Information

Project Filename: C:\Documents and Settings\Heat\My Documents\Projects\AutoLoad MJ8.rhv
Project Title: Adam Hinton
Designed By: Chuck Fischer
Project Date: February 9th 2007
Client Name: Adam Hinton
Client Address: Lot 63 Santa Fe River Plantation
Client City: High Springs Fl
Client Comment:
Company Name: North Central Florida Air Conditioning
Company Representative: Chuck Fischer
Company Address: P.O Box 700
Company City: High Springs Fl 32655-0700
Company Phone: 386-454-4767
Company Fax: 386-454-4854
Company Comment: Bedroom 2,3 & 4 R/A are 10x10x8 Master bedroom R/A is 12x12x9 Main R/A is 20x20x18

Design Data

Reference City: Gainesville, Florida
Daily Temperature Range: Medium
Latitude: 29 Degrees
Elevation: 152 ft.
Altitude Factor: 0.995
Elevation Sensible Adj. Factor: 1.000
Elevation Total Adj. Factor: 1.000
Elevation Heating Adj. Factor: 1.000
Elevation Heating Adj. Factor: 1.000

	Outdoor Dry Bulb	Outdoor Wet Bulb	Indoor Rel.Hum	Indoor Dry Bulb	Grains Difference
Winter:	31	0	0	68	0
Summer:	93	77	50	75	50

Check Figures

Total Building Supply CFM:	1,153	CFM Per Square ft.:	0.598
Square ft. of Room Area:	1,928	Square ft. Per Ton:	706
Volume (ft³) of Cond. Space:	17,773	Air Turnover Rate (per hour):	3.9

Building Loads

Total Heating Required With Outside Air:	42,183 Btuh	42.183 MBH
Total Sensible Gain:	25,227 Btuh	83 %
Total Latent Gain:	5,160 Btuh	17 %
Total Cooling Required With Outside Air:	30,387 Btuh	2.53 Tons (Based On Sensible + Latent)
		2.73 Tons (Based On 77% Sensible Capacity)

Notes

Calculations are based on 8th edition of ACCA Manual J.
All computed results are estimates as building use and weather may vary.
Be sure to select a unit that meets both sensible and latent loads.



Miscellaneous Report

System 1 Main Floor Input Data	Outdoor Dry Bulb	Outdoor Wet Bulb	Indoor Rel.Hum	Indoor Dry Bulb	Grains Difference
Winter:	31	0	50	68	30.84
Summer:	93	77	50	75	50.06

Duct Sizing Inputs

	Main Trunk	Runouts
Calculate:	Yes	Yes
Use Schedule:	No	No
Roughness Factor:	0.00300	0.01000
Pressure Drop:	0.1000 in.wg./100 ft.	0.1000 in.wg./100 ft.
Minimum Velocity:	650 ft./min	450 ft./min
Maximum Velocity:	900 ft./min	750 ft./min
Minimum Height:	0 in.	0 in.
Maximum Height:	0 in.	0 in.

Outside Air Data

	Winter	Summer
Infiltration:	0.900 AC/hr	0.400 AC/hr
Volume of Conditioned Space:	X 17773 Cu.ft.	X 17773 Cu.ft.
	15,996 Cu.ft./hr	7,109 Cu.ft./hr
	X 0.0167	X 0.0167
Total Building Infiltration:	267 CFM	118 CFM
Total Building Ventilation:	0 CFM	0 CFM

---System 1---

Infiltration & Ventilation Sensible Gain Multiplier:	19.69	= (1.10 X 0.995 X 18.00 Summer Temp. Difference)
Infiltration & Ventilation Latent Gain Multiplier:	33.85	= (0.68 X 0.995 X 50.06 Grains Difference)
Infiltration & Ventilation Sensible Loss Multiplier:	40.48	= (1.10 X 0.995 X 37.00 Winter Temp. Difference)



Load Preview Report

Scope	Area	Sens Gain	Lat Gain	Net Gain	Sens Loss	Win CFM	Sum CFM	Sys CFM	Duct Size
Building: 2.53 Net Tons, 2.73 Recommended Tons, 706 ft.²/Ton, 42.18 MBH Heating									
Building	1,928	25,227	5,160	30,387	42,183	551	1,153	1,153	
System 1: 2.53 Net Tons, 2.73 Recommended Tons, 706 ft.²/Ton, 42.18 MBH Heating									
System 1	1,928	25,227	5,160	30,387	42,183	551	1,153	1,153	15x15
Zone 2	449	6,342	1,531	7,873	11,229	147	290	290	
1-Bedroom 2	215	2,827	715	3,542	5,162	67	129	129	1-7
2-Bath 2	61	822	156	978	1,470	19	38	38	1-4
3-Bedroom 3	173	2,692	660	3,352	4,597	60	123	123	1-7
Zone 3	931	11,719	1,415	13,134	14,012	183	536	536	
4-Foyer	84	914	103	1,017	1,153	15	42	42	1-4
5-Dining Room	144	1,864	179	2,043	2,363	31	85	85	1-5
6-Laundry Room	92	1,861	327	2,188	3,308	43	85	85	1-5
7-Great Room	284	2,528	217	2,745	3,184	42	116	116	1-6
8-Kitchen	197	3,778	527	4,305	3,181	42	173	173	1-8
12-Stair Well	61	712	62	774	703	9	33	33	1-3
13-Hall	69	60	0	60	120	2	3	3	1-1
Zone 1	302	4,157	1,377	5,534	9,295	121	190	190	
9-Master W.I.C	46	186	83	269	778	10	9	9	1-2
10-Master Bedroom	174	3,030	877	3,907	4,525	59	139	139	1-7
11-Master Bath	82	941	417	1,358	3,992	52	43	43	1-4
Zone 4	246	3,009	837	3,846	7,647	100	138	138	
14-Bonus Room	200	2,082	524	2,606	4,969	65	95	95	1-6
15-Bonus Bath	46	926	313	1,239	2,678	35	42	42	1-4



Total Building Summary Loads

Component Description	Area Quan	Sen Loss	Lat Gain	Sen Gain	Total Gain
1D-hv-o: Glazing-Double pane, operable window, heat-absorbing, vinyl frame, ground reflectance = 0.32, outdoor insect screen with 50% coverage, light color blinds at 45° with 25% coverage, external shade screen coefficient of 0.35 and 50% coverage	94	1,980	0	1,291	1,291
1D-hv-o: Glazing-Double pane, operable window, heat-absorbing, vinyl frame, ground reflectance = 0.23, outdoor insect screen with 50% coverage, light color blinds at 45° with 25% coverage, external shade screen coefficient of 0.35 and 50% coverage	80	1,687	0	1,099	1,099
10C-b: Glazing-French door, double pane low-e glass (e = 0.40), metal frame with break, ground reflectance = 0.32	35.4	956	0	818	818
11P: Door-Polyurethane Core	40.8	438	0	344	344
13A-7.5ocs: Wall-Block, board insulation only, R-7.5 board insulation, open core, siding finish	1598	5,615	0	2,474	2,474
12E-5sw: Wall-Frame, R-19 insulation in 2 x 6 stud cavity, R-5 board insulation, siding finish, wood studs	466.2	897	0	381	381
16DR-44: Roof/Ceiling-Under attic or knee wall, Vented Attic with Radiant Barrier, White or Light Color Shingles, Any Wood Shake, Light Metal, Tar and Gravel Membrane, R-44 insulation	1927.4	1,568	0	1,400	1,400
22A-pm: Floor-Slab on grade, No edge insulation, no insulation below floor, any floor cover, passive, heavy dry or light wet soil	257	11,221	0	0	0
Subtotals for structure:		24,362	0	7,807	7,807
People:	5		1,150	1,500	2,650
Equipment:			0	1,200	1,200
Lighting:	2400			8,184	8,184
Ductwork:		7,029	0	4,204	4,204
Infiltration: Winter CFM: 267, Summer CFM: 118		10,792	4,010	2,332	6,342
Ventilation: Winter CFM: 0, Summer CFM: 0		0	0	0	0
Total Building Load Totals:		42,183	5,160	25,227	30,387

Check Figures

Total Building Supply CFM:	1,153	CFM Per Square ft.:	0.598
Square ft. of Room Area:	1,928	Square ft. Per Ton:	706
Volume (ft³) of Cond. Space:	17,773	Air Turnover Rate (per hour):	3.9

Building Loads

Total Heating Required With Outside Air:	42,183 Btuh	42.183 MBH
Total Sensible Gain:	25,227 Btuh	83 %
Total Latent Gain:	5,160 Btuh	17 %
Total Cooling Required With Outside Air:	30,387 Btuh	2.53 Tons (Based On Sensible + Latent)
		2.73 Tons (Based On 77% Sensible Capacity)

Notes

Calculations are based on 8th edition of ACCA Manual J.
All computed results are estimates as building use and weather may vary.
Be sure to select a unit that meets both sensible and latent loads.



System 1 Main Floor Summary Loads (Average Method)

Component Description	Area Quan	Sen Loss	Lat Gain	Sen Gain	Total Gain
1D-hv-o: Glazing-Double pane, operable window, heat-absorbing, vinyl frame, ground reflectance = 0.32, outdoor insect screen with 50% coverage, light color blinds at 45° with 25% coverage, external shade screen coefficient of 0.35 and 50% coverage	94	1,980	0	1,291	1,291
1D-hv-o: Glazing-Double pane, operable window, heat-absorbing, vinyl frame, ground reflectance = 0.23, outdoor insect screen with 50% coverage, light color blinds at 45° with 25% coverage, external shade screen coefficient of 0.35 and 50% coverage	80	1,687	0	1,099	1,099
10C-b: Glazing-French door, double pane low-e glass (e = 0.40), metal frame with break, ground reflectance = 0.32	35.4	956	0	818	818
11P: Door-Polyurethane Core	40.8	438	0	344	344
13A-7.5ocs: Wall-Block, board insulation only, R-7.5 board insulation, open core, siding finish	1598	5,615	0	2,474	2,474
12E-5sw: Wall-Frame, R-19 insulation in 2 x 6 stud cavity, R-5 board insulation, siding finish, wood studs	466.2	897	0	381	381
16DR-44: Roof/Ceiling-Under attic or knee wall, Vented Attic with Radiant Barrier, White or Light Color Shingles, Any Wood Shake, Light Metal, Tar and Gravel Membrane, R-44 insulation	1927.4	1,568	0	1,400	1,400
22A-pm: Floor-Slab on grade, No edge insulation, no insulation below floor, any floor cover, passive, heavy dry or light wet soil	257	11,221	0	0	0
Subtotals for structure:		24,362	0	7,807	7,807
People:	5		1,150	1,500	2,650
Equipment:			0	1,200	1,200
Lighting:	2400			8,184	8,184
Ductwork:		7,029	0	4,204	4,204
Infiltration: Winter CFM: 267, Summer CFM: 118		10,792	4,010	2,332	6,342
Ventilation: Winter CFM: 0, Summer CFM: 0		0	0	0	0
System 1 Main Floor Load Totals:		42,183	5,160	25,227	30,387

Check Figures

Supply CFM:	1,153	CFM Per Square ft.:	0.598
Square ft. of Room Area:	1,928	Square ft. Per Ton:	706
Volume (ft³) of Cond. Space:	17,773	Air Turnover Rate (per hour):	3.9

System Loads

Total Heating Required With Outside Air:	42,183 Btuh	42.183 MBH
Total Sensible Gain:	25,227 Btuh	83 %
Total Latent Gain:	5,160 Btuh	17 %
Total Cooling Required With Outside Air:	30,387 Btuh	2.53 Tons (Based On Sensible + Latent)
		2.73 Tons (Based On 77% Sensible Capacity)

Notes

Calculations are based on 8th edition of ACCA Manual J.
 All computed results are estimates as building use and weather may vary.
 Be sure to select a unit that meets both sensible and latent loads.



System 1, Zone 2 Summary Loads (Average Method)

Component Description	Area Quan	Sen Loss	Lat Gain	Sen Gain	Total Gain
1D-hv-o: Glazing-Double pane, operable window, heat-absorbing, vinyl frame, ground reflectance = 0.32, outdoor insect screen with 50% coverage, light color blinds at 45° with 25% coverage, external shade screen coefficient of 0.35 and 50% coverage	45	948	0	618	618
1D-hv-o: Glazing-Double pane, operable window, heat-absorbing, vinyl frame, ground reflectance = 0.23, outdoor insect screen with 50% coverage, light color blinds at 45° with 25% coverage, external shade screen coefficient of 0.35 and 50% coverage	15	316	0	206	206
13A-7.5ocs: Wall-Block, board insulation only, R-7.5 board insulation, open core, siding finish	558.5	1,962	0	865	865
16DR-44: Roof/Ceiling-Under attic or knee wall, Vented Attic with Radiant Barrier, White or Light Color Shingles, Any Wood Shake, Light Metal, Tar and Gravel Membrane, R-44 insulation	449.4	366	0	327	327
22A-pm: Floor-Slab on grade, No edge insulation, no insulation below floor, any floor cover, passive, heavy dry or light wet soil	66	2,882	0	0	0
Subtotals for structure:		6,474	0	2,016	2,016
People:	2		460	600	1,060
Equipment:			0	0	0
Lighting:	600			2,046	2,046
Ductwork:		1,871	0	1,057	1,057
Infiltration: Winter CFM: 71, Summer CFM: 32		2,884	1,071	623	1,694
System 1, Zone 2 Load Totals:		11,229	1,531	6,342	7,873

Check Figures

Supply CFM:	290	CFM Per Square ft.:	0.646
Square ft. of Room Area:	449	Square ft. Per Ton:	628
Volume (ft³) of Cond. Space:	4,224	Air Turnover Rate (per hour):	4.1

Zone Loads

Total Heating Required:	11,229 Btuh	11.229 MBH
Total Sensible Gain:	6,342 Btuh	81 %
Total Latent Gain:	1,531 Btuh	19 %
Total Cooling Required:	7,873 Btuh	0.66 Tons (Based On Sensible + Latent)
		0.71 Tons (Based On 77% Sensible Capacity)

Notes

Calculations are based on 8th edition of ACCA Manual J.
 All computed results are estimates as building use and weather may vary.
 Be sure to select a unit that meets both sensible and latent loads.



System 1, Zone 3 Summary Loads (Average Method)

Component Description	Area Quan	Sen Loss	Lat Gain	Sen Gain	Total Gain
1D-hv-o: Glazing-Double pane, operable window, heat-absorbing, vinyl frame, ground reflectance = 0.32, outdoor insect screen with 50% coverage, light color blinds at 45° with 25% coverage, external shade screen coefficient of 0.35 and 50% coverage	49	1,032	0	673	673
10C-b: Glazing-French door, double pane low-e glass (e = 0.40), metal frame with break, ground reflectance = 0.32	35.4	956	0	818	818
1D-hv-o: Glazing-Double pane, operable window, heat-absorbing, vinyl frame, ground reflectance = 0.23, outdoor insect screen with 50% coverage, light color blinds at 45° with 25% coverage, external shade screen coefficient of 0.35 and 50% coverage	9	190	0	124	124
11P: Door-Polyurethane Core	40.8	438	0	344	344
13A-7.5ocs: Wall-Block, board insulation only, R-7.5 board insulation, open core, siding finish	549.2	1,930	0	851	851
16DR-44: Roof/Ceiling-Under attic or knee wall, Vented Attic with Radiant Barrier, White or Light Color Shingles, Any Wood Shake, Light Metal, Tar and Gravel Membrane, R-44 insulation	929.7	756	0	676	676
22A-pm: Floor-Slab on grade, No edge insulation, no insulation below floor, any floor cover, passive, heavy dry or light wet soil	73	3,188	0	0	0
Subtotals for structure:		8,490	0	3,486	3,486
People:	1		230	300	530
Equipment:			0	1,200	1,200
Lighting:	1200			4,092	4,092
Ductwork:		2,335	0	1,953	1,953
Infiltration: Winter CFM: 79, Summer CFM: 35		3,187	1,185	688	1,873
System 1, Zone 3 Load Totals:		14,012	1,415	11,719	13,134

Check Figures

Supply CFM:	536	CFM Per Square ft.:	0.575
Square ft. of Room Area:	931	Square ft. Per Ton:	810
Volume (ft³) of Cond. Space:	8,741	Air Turnover Rate (per hour):	3.7

Zone Loads

Total Heating Required:	14,012 Btuh	14.012 MBH
Total Sensible Gain:	11,719 Btuh	89 %
Total Latent Gain:	1,415 Btuh	11 %
Total Cooling Required:	13,134 Btuh	1.09 Tons (Based On Sensible + Latent)
		1.15 Tons (Based On 77% Sensible Capacity)

Notes

Calculations are based on 8th edition of ACCA Manual J.
 All computed results are estimates as building use and weather may vary.
 Be sure to select a unit that meets both sensible and latent loads.



System 1, Zone 1 Summary Loads (Average Method)

Component Description	Area Quan	Sen Loss	Lat Gain	Sen Gain	Total Gain
1D-hv-o: Glazing-Double pane, operable window, heat-absorbing, vinyl frame, ground reflectance = 0.23, outdoor insect screen with 50% coverage, light color blinds at 45° with 25% coverage, external shade screen coefficient of 0.35 and 50% coverage	39	822	0	536	536
13A-7.5ocs: Wall-Block, board insulation only, R-7.5 board insulation, open core, siding finish	490.2	1,723	0	758	758
16DR-44: Roof/Ceiling-Under attic or knee wall, Vented Attic with Radiant Barrier, White or Light Color Shingles, Any Wood Shake, Light Metal, Tar and Gravel Membrane, R-44 insulation	301.6	245	0	218	218
22A-pm: Floor-Slab on grade, No edge insulation, no insulation below floor, any floor cover, passive, heavy dry or light wet soil	57	2,488	0	0	0
Subtotals for structure:		5,278	0	1,512	1,512
People:	2		460	600	1,060
Equipment:			0	0	0
Lighting:	240			818	818
Ductwork:		1,549	0	693	693
Infiltration: Winter CFM: 61, Summer CFM: 27		2,468	917	534	1,451
System 1, Zone 1 Load Totals:		9,295	1,377	4,157	5,534

Check Figures

Supply CFM:	190	CFM Per Square ft.:	0.629
Square ft. of Room Area:	302	Square ft. Per Ton:	588
Volume (ft³) of Cond. Space:	2,834	Air Turnover Rate (per hour):	4.0

Zone Loads

Total Heating Required:	9,295 Btuh	9.295 MBH
Total Sensible Gain:	4,157 Btuh	75 %
Total Latent Gain:	1,377 Btuh	25 %
Total Cooling Required:	5,534 Btuh	0.46 Tons (Based On Sensible + Latent)
		0.51 Tons (Based On 77% Sensible Capacity)

Notes

Calculations are based on 8th edition of ACCA Manual J.
 All computed results are estimates as building use and weather may vary.
 Be sure to select a unit that meets both sensible and latent loads.



System 1, Zone 4 Summary Loads (Average Method)

Component Description	Area Quan	Sen Loss	Lat Gain	Sen Gain	Total Gain
1D-hv-o: Glazing-Double pane, operable window, heat-absorbing, vinyl frame, ground reflectance = 0.23, outdoor insect screen with 50% coverage, light color blinds at 45° with 25% coverage, external shade screen coefficient of 0.35 and 50% coverage	17	359	0	233	233
12E-5sw: Wall-Frame, R-19 insulation in 2 x 6 stud cavity, R-5 board insulation, siding finish, wood studs	466.2	897	0	381	381
16DR-44: Roof/Ceiling-Under attic or knee wall, Vented Attic with Radiant Barrier, White or Light Color Shingles, Any Wood Shake, Light Metal, Tar and Gravel Membrane, R-44 insulation	246.6	201	0	179	179
22A-pm: Floor-Slab on grade, No edge insulation, no insulation below floor, any floor cover, passive, heavy dry or light wet soil	61	2,663	0	0	0
Subtotals for structure:		4,120	0	793	793
People:	0		0	0	0
Equipment:			0	0	0
Lighting:	360			1,228	1,228
Ductwork:		1,274	0	501	501
Infiltration: Winter CFM: 56, Summer CFM: 25		2,253	837	487	1,324
System 1, Zone 4 Load Totals:		7,647	837	3,009	3,846

Check Figures

Supply CFM:	138	CFM Per Square ft.:	0.559
Square ft. of Room Area:	246	Square ft. Per Ton:	698
Volume (ft³) of Cond. Space:	1,974	Air Turnover Rate (per hour):	4.2

Zone Loads

Total Heating Required:	7,647 Btuh	7.647 MBH
Total Sensible Gain:	3,009 Btuh	78 %
Total Latent Gain:	837 Btuh	22 %
Total Cooling Required:	3,846 Btuh	0.32 Tons (Based On Sensible + Latent)
		0.35 Tons (Based On 77% Sensible Capacity)

Notes

Calculations are based on 8th edition of ACCA Manual J.
 All computed results are estimates as building use and weather may vary.
 Be sure to select a unit that meets both sensible and latent loads.



System 1 Room Load Summary

Room No	Room Name	Area SF	Htg Sens Btuh	Htg Nom CFM	Run Duct Size	Run Duct Vel	Clg Sens Btuh	Clg Lat Btuh	Clg Nom CFM	Air Sys CFM
---Zone 2---										
1	Bedroom 2	215	5,162	67	1-7	484	2,827	715	129	129
2	Bath 2	61	1,470	19	1-4	431	822	156	38	38
3	Bedroom 3	173	4,597	60	1-7	460	2,692	660	123	123
Zone 2 subtotal		449	11,229	147			6,342	1,531	290	290
---Zone 3---										
4	Foyer	84	1,153	15	1-4	479	914	103	42	42
5	Dining Room	144	2,363	31	1-5	625	1,864	179	85	85
6	Laundry Room	92	3,308	43	1-5	624	1,861	327	85	85
7	Great Room	284	3,184	42	1-6	589	2,528	217	116	116
8	Kitchen	197	3,181	42	1-8	495	3,778	527	173	173
12	Stair Well	61	703	9	1-3	663	712	62	33	33
13	Hall	69	120	2	1-1	503	60	0	3	3
Zone 3 subtotal		931	14,012	183			11,719	1,415	536	536
---Zone 1---										
9	Master W.I.C	46	778	10	1-2	390	186	83	9	9
10	Master Bedroom	174	4,525	59	1-7	518	3,030	877	139	139
11	Master Bath	82	3,992	52	1-4	493	941	417	43	43
Zone 1 subtotal		302	9,295	121			4,157	1,377	190	190
---Zone 4---										
14	Bonus Room	200	4,969	65	1-6	485	2,082	524	95	95
15	Bonus Bath	46	2,678	35	1-4	485	926	313	42	42
Zone 4 subtotal		246	7,647	100			3,009	837	138	138
System 1 total		1,928	42,183	551			25,227	5,160	1,153	1,153

System 1 Main Trunk Size: 15x15 in.
Velocity: 826 ft./min
Loss per 100 ft.: 0.083 in.wg

Cooling System Summary

	Cooling Tons	Sensible/Latent Split	Sensible Btuh	Latent Btuh	Total Btuh
Net Required:	2.53	83% / 17%	25,227	5,160	30,387
Recommended:	2.73	77% / 23%	25,227	7,535	32,762
Actual:	2.92	76% / 24%	26,500	8,500	35,000

Equipment Data

	Heating System	Cooling System
Type:	Air Cooled Condensor	Air Cooled Condensor
Model:	GSH130361+ARPF364216+HKR-10	GSH130361+ARPF364216
Brand:	Goodman	Goodman
Efficiency:	7.7	Seer 13
Sound:		
Capacity:	32.000	35.000
Sensible Capacity:	n/a	26,500 Btuh
Latent Capacity:	n/a	8,500 Btuh

**Columbia County Building Department
Culvert Waiver**

**Culvert Waiver No.
000001352**

DATE: 03/16/2007

BUILDING PERMIT NO. 25627

APPLICANT DANIEL A. HINTON

PHONE 352.318.3411

ADDRESS POB 2214 HIGH SPRINGS FL 32655

OWNER DANIEL ADAM & CAMMEY HINTON

PHONE 352.318.3411

ADDRESS 1087 SW MAPLETON STREET FT. WHITE FL 32038

CONTRACTOR DANIEL A. & CAMMEY HINTON

PHONE 352.318.3411

LOCATION OF PROPERTY 47-S TO US 27 TO C-138-S TO WOODLAND, TL TO GATED COMMUNITY (JUST
BEFORE GATE, TAKE L TO ADDRESS MARKED 1087.

SUBDIVISION/LOT/BLOCK/PHASE/UNIT SANTA FE RIVER PLANT 63

PARCEL ID # 30-7S-17-10058-653

I HEREBY CERTIFY THAT I UNDERSTAND AND WILL FULLY COMPLY WITH THE DECISION OF THE COLUMBIA
COUNTY PUBLIC WORKS DEPARTMENT IN CONNECTION WITH THE HEREIN PROPOSED APPLICATION.

SIGNATURE: 

A SEPARATE CHECK IS REQUIRED
MAKE CHECKS PAYABLE TO BCC

Amount Paid 50.00

PUBLIC WORKS DEPARTMENT USE ONLY

I HEREBY CERTIFY THAT I HAVE EXAMINED THIS APPLICATION AND DETERMINED THAT THE
CULVERT WAIVER IS:

✓ APPROVED

NOT APPROVED - NEEDS A CULVERT PERMIT

COMMENTS: _____

SIGNED: 

DATE: 3-22-07

ANY QUESTIONS PLEASE CONTACT THE PUBLIC WORKS DEPARTMENT AT 386-752-5955.

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

RECEIVED

MAR 21 2007

By: _____



NOTICE OF COMMENCEMENT

This document prepared by and to be returned to:

Kyle E. Petteway
Grunder & Petteway, P. A.
23349 NW CR 236, Suite 10
High Springs, Florida, 32643

Inst:2007005642 Date:03/09/2007 Time:12:53
✓ 9. DC, P. Dewitt Cason, Columbia County B:1113 P:4

PERMIT NO.:
TAX FOLIO NUMBER: R10058-653

The undersigned hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

1. **Description of Property:**

Lot 63, a replat of Lots 38, 45, and 46 of Santa Fe River Plantations, a subdivision, as per plat thereof recorded in Plat Book 5, Pages 13 through 13D, inclusive of the Public Records of Columbia County, Florida. ~~SSA~~

Address:

2. **General description of improvement:** Residential Home Construction

3. **Owner information:**

a. **Name and address:**

Daniel A. Hinton and Cammey B. Hinton.
25647 NW 204th Avenue
High Springs, FL 32643

b. **Phone number:** 386-454-7012

c. **Fax number:**

d. **Interest in property:** Fee simple title holder

e. **Name and address of fee simple titleholder (if other than owner):** N/A

4. **Contractor:** Daniel A. Hinton and Cammey B. Hinton.
25647 NW 204th Avenue
High Springs, FL 32643

a. **Phone number:** (386) 454-7012

5. **Surety on any payment bond:** None

6. **Lender:** Florida Rural Rehabilitation Corporation
4020 Newberry Road
Gainesville, FL 32606

a. **Phone number:** (352) 372-3100

7. **Persons within the State of Florida designated by Owner upon whom notices or other documents may be served as provided for by Section 713.13 (1) (a) 7, Florida Statutes:**

Daniel A. Hinton and Cammey B. Hinton
25647 NW 204th Avenue
High Springs, FL 32643
386-454-7012

a. **Phone number:**

8. **In addition to himself, Owner designates the following person to receive a copy of the Lienor's Notice as provided in Section 713.13 (1) (b), Florida Statutes:**

None

a. **Phone number:**

b. **Fax number:**

9. **Expiration Date of Notice of Commencement (the expiration is one year from date of recording unless a different date is specified):**


Daniel A. Hinton



1. Description of Property:

Lot 63, a replat of Lots 38, 45, and 46 of Santa Fe River Plantations, a subdivision, as per plat thereof recorded in Plat Book 5, Pages 13 through 13D, inclusive of the Public Records of Columbia County, Florida. ~~32A~~

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b. Phone number: 386-454-7012

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4. Contractor: Daniel A. Hinton and Cammey B. Hinton.
25647 NW 204th Avenue
High Springs, FL 32643

a. Phone number: (386) 454-7012

5. Surety on any payment bond: None

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Daniel A. Hinton

Daniel A. Hinton

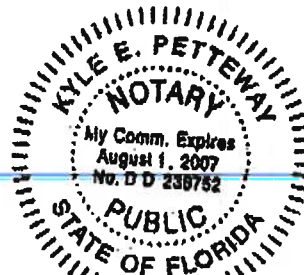
Cammey B. Hinton

Cammey B. Hinton

STATE OF FLORIDA
COUNTY OF ALACHUA

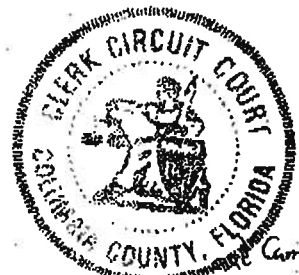
SWORN TO and subscribed before me this 6th day of March, 2007 by Daniel A. Hinton who
(☒) personally known to me () produced a valid Florida driver's license as identification () produced
as identification

[Signature]
Signature of Notary
(SEAL) 8646



STATE OF FLORIDA, COUNTY OF COLUMBIA
I HEREBY CERTIFY, that the above and foregoing
is a true copy of the original filed in this office.
P. DAWY, CLERK OF COURT

[Signature]
Deputy Clerk
Date: 3-09-07



25627

ALACHUA

PEST CONTROL inc.



"If we can't trap 'em, we'll rope 'em"

Household Pests • Termites
Fumigation



14900 NW 140th St. / P.O. Box 1132 / Alachua, Florida 32616-1132
(386) 462-2958 (352) 375-1555 (386) 462-1310 Fax

Certificate of Protective Treatment for Prevention of Subterranean Termites

- 1) Applicators Name Wade Hodge
- 2) Time and Date of Treatment 8:30AM 4-25-07
- 3) Site Location 1087 SW Mapleton ST.
Ft. White
- 4) Chemical used and % of Concentration Termidor .06%
- 5) Number of Gallons of Finish Product and Type of Slab 370gal
Stem Wall

All above information is accurate and product was used strictly by label recommendations to the best of my knowledge.

Alachua Pest Control Application Technician.

Wade Hodge
Signature

4-25-07
Date

COLUMBIA COUNTY
OFFICIAL SEAL

OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 30-7S-17-10058-653

Building permit No. 000025627

Use Classification SFD/UTILITY

Fire: 70.62

Permit Holder DANIEL A. & COURTNEY HINTON

Waste: 184.25

Owner of Building DANIEL A. & CAMMEY HINTON

Total: 254.87

Location: 1087 SW MALETON ST, FT. WHITE, FL

Date: 11/05/2007

Harry Sticks

Building Inspector



POST IN A CONSPICUOUS PLACE
(Business Places Only)

Daniel Adam & Cammy Hinton

1087 SW Mapleton St
Fort White, FL

BOUNDARY SURVEY
IN SECTION 30,
TOWNSHIP 7 SOUTH,
RANGE 12 EAST.
COLUMBIA COUNTY, FLA.

DESCRIPTION:
LOT 63 OF "A REPLAT OF LOTS 38, 45 AND
46 OF SANTA FE RIVER PLANTATION", A
SUBDIVISION AS PER PLAT THEREOF
RECORDED IN PLAT BOOK 5, PAGES 13, 13A
-13D, OF THE PUBLIC RECORDS OF
COLUMBIA COUNTY, FLORIDA.

- SURVEYOR'S NOTES:
1. BOUNDARY BASED ON MONUMENTATION FOUND IN ACCORDANCE WITH THE RETRACEMENT OF THE ORIGINAL SURVEY FOR SAID PLAT OF RECORD.
 2. BEARINGS BASED ON PLAT OF RECORD USING MONUMENTS FOUND ON THE WEST LINE OF SAID LOT 63.
 3. THIS PARCEL IS IN ZONE "X" AND IS DETERMINED TO BE OUTSIDE THE 500 YEAR FLOOD PLAIN AS PER FLOOD INSURANCE RATE MAP, DATED JANUARY 6, 1988, COMMUNITY PANEL NO. 120070 0270 B.
 4. NO EASEMENT FOR UTILITY AND/OR DRAINAGE IS SHOWN ON THIS LOT IN RECORDS IN THE POSSESSION OF THIS OFFICE.
 5. THE IMPROVEMENTS, IF ANY, INDICATED ON THIS SURVEY DRAWING ARE AS LOCATED ON DATE OF FIELD SURVEY AS SHOWN HEREON.
 6. IF THEY EXIST, NO UNDERGROUND ENCROACHMENTS AND/OR UTILITIES WERE LOCATED FOR THIS SURVEY EXCEPT AS SHOWN HEREON.
 7. NOT VALID WITHOUT THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER.
 8. CLOSURE OF FIELD SURVEY IS 1/48,790.
 9. THIS SURVEY WAS PREPARED WITHOUT THE BENEFIT OF AN ABSTRACT OR TITLE POLICY. THEREFORE EXCEPTION IS MADE HEREON REGARDING EASEMENTS, RESERVATIONS, RESTRICTIONS, AND/OR TITLE CONFLICTS OF RECORD, IF ANY, NOT PROVIDED BY THE CLIENT OR HIS AGENTS.
 10. CERTIFIED TO:

DANIEL A. AND CAMMEY B. HINTON
GRUNDER & PETTEWAY, P.A.
FIRST AMERICAN TITLE INSURANCE COMPANY

SYMBOL LEGEND

- CONCRETE MONUMENT FOUND
- CONCRETE MONUMENT SET, LS 4708
- IRON PIN OR PIPE FOUND
- 5/8" IRON ROD SET, LS 4708
- WIRE FENCE
- ELECTRIC UTILITY LINE (OVERHEAD)
- UNDERGROUND ELECTRIC SERVICE
- CABLE TV LINE (OVERHEAD)
- CHAIN LINK FENCE
- WOODEN FENCE
- CMP CORRUGATED METAL PIPE
- RCP REINFORCED CONCRETE PIPE
- LS LAND SURVEYOR
- LB LICENSED BUSINESS
- ORB OFFICIAL RECORD BOOK
- PRM PERMANENT REFERENCE MONUMENT
- PCP PERMANENT CONTROL POINT
- UTILITY POLE
- R/W RIGHT-OF-WAY
- NO ID. NO IDENTIFICATION
- FLA. D.O.T. FLA. DEPT. OF TRANSPORTATION
- C.M. CENTERLINE
- CONCRETE MONUMENT
- I.R. IRON ROD
- I.P. IRON PIPE



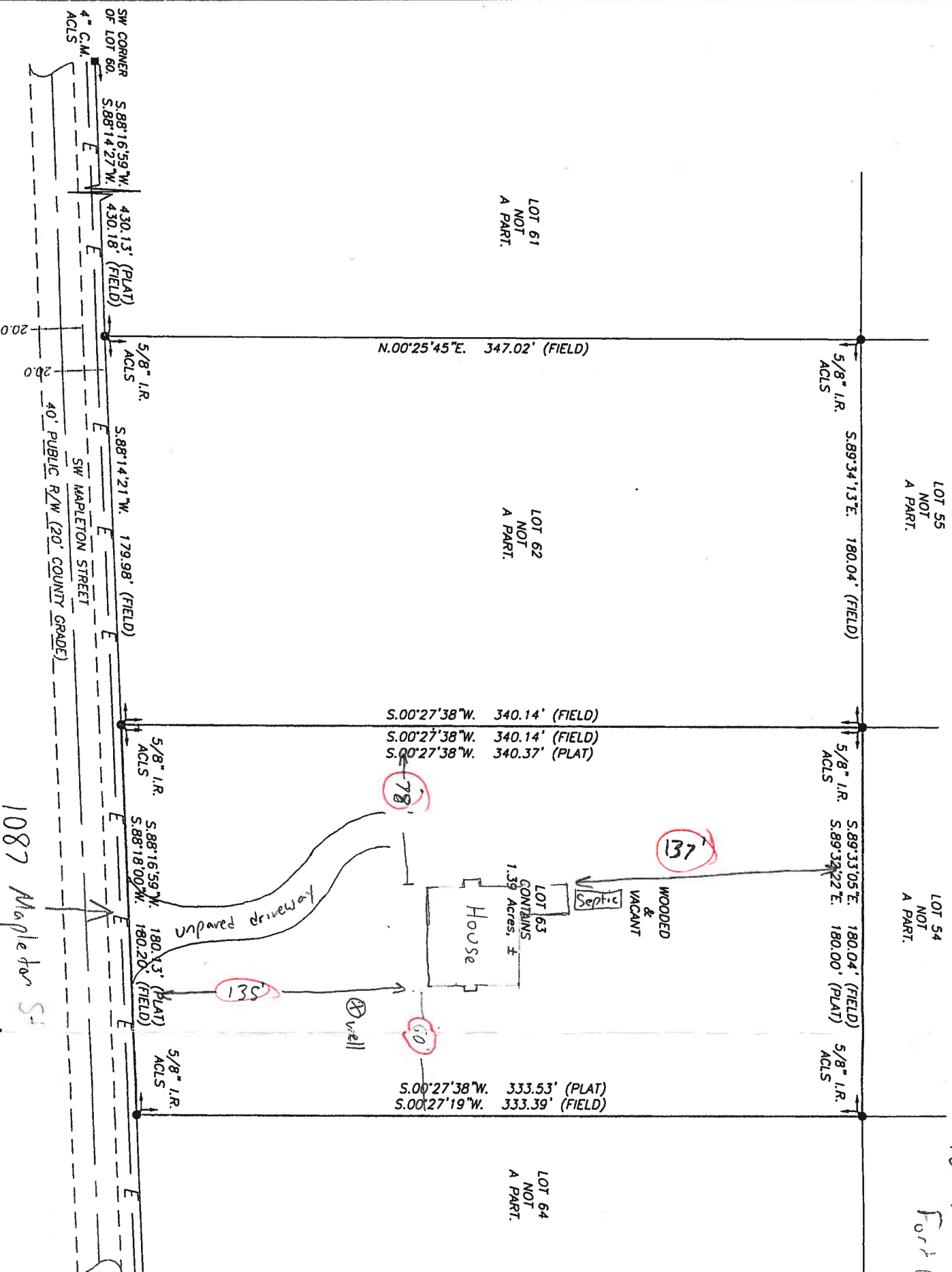
MARK D. DUREN, P.S.M.
LS 4708

1604 SW SISTERS WELCOME ROAD
LAKE CITY, FLA. 32025
(386) 758-9831 OFFICE
(386) 758-8010 FAX

FIELD SURVEY DATE, JANUARY 5, 2002
DATE DRAWN, JANUARY 8, 2002
FOR, HINTON

FIELD BOOK, 123 PAGE, 22
DRAWN BY, M. DUREN/A. DYAL

WO# 07-009



SIGNED:
MARK D. DUREN, LS 4708

Daniel Adam & Cammy Hinton

1087 SW Mapleton St
Fort White, FL

BOUNDARY SURVEY
IN SECTION 30,
TOWNSHIP 7 SOUTH,
RANGE 12 EAST.
COLUMBIA COUNTY, FLA.

DESCRIPTION:
LOT 63 OF "A REPLAT OF LOTS 38, 45 AND 46 OF SANTA FE RIVER PLANTATION", A SUBDIVISION AS PER PLAT THEREOF RECORDED IN PLAT BOOK 5, PAGES 13, 13A - 13D, OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.

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GRUNDER & PETTEWAY, P.A.
FIRST AMERICAN TITLE INSURANCE COMPANY

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- o— WOODEN FENCE
- o— CORRUGATED METAL PIPE
- o— RCP REINFORCED CONCRETE PIPE
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- ⊗ UTILITY POLE
- ⊗ RIGHT-OF-WAY
- NO ID. NO IDENTIFICATION
- FLA. D.O.T. FLA. DEPT. OF TRANSPORTATION
- ⊕ CENTERLINE
- C.M. CONCRETE MONUMENT
- I.R. IRON ROD
- I.P. IRON PIPE



MARK D. DUREN, P.S.M.
LS 4708

1604 SW SISTERS WELCOME ROAD
LAKE CITY, FLA. 32025
(386) 758-9831 OFFICE
(386) 758-8010 FAX

FIELD SURVEY DATE: JANUARY 5, 2002
DATE DRAWN: JANUARY 8, 2002
FOR: HINTON

FIELD BOOK: 123 PAGE: 22
DRAWN BY: M. DUREN/A. DYAL

WO# 07-009

LOT 61
NOT
A PART.

LOT 62
NOT
A PART.

LOT 55
NOT
A PART.

LOT 54
NOT
A PART.

LOT 64
NOT
A PART.

N.00°25'45"E. 347.02' (FIELD)

5/8" I.R. 5.89°34'13"E. 180.04' (FIELD)
ACLS

5/8" I.R. 5.89°33'05"E. 180.04' (FIELD)
ACLS

5/8" I.R. 5.89°33'22"E. 180.00' (PLAT)
ACLS

S.00°27'38"W. 340.14' (FIELD)
S.00°27'38"W. 340.14' (FIELD)
S.00°27'38"W. 340.37' (PLAT)

S.00°27'38"W. 333.53' (PLAT)
S.00°27'19"W. 333.39' (FIELD)

430.13' (PLAT)

430.18' (FIELD)

5/8" I.R. 5.88°14'21"W. 179.98' (FIELD)

5/8" I.R. 5.88°14'21"W. 179.98' (FIELD)

5/8" I.R. 5.88°16'59"W. 180.13' (PLAT)

5/8" I.R. 5.88°16'59"W. 180.20' (FIELD)

5/8" I.R. 5.88°18'00"W. 180.20' (FIELD)

SW CORNER
OF LOT 60.
4" C.M.
ACLS

430.13' (PLAT)

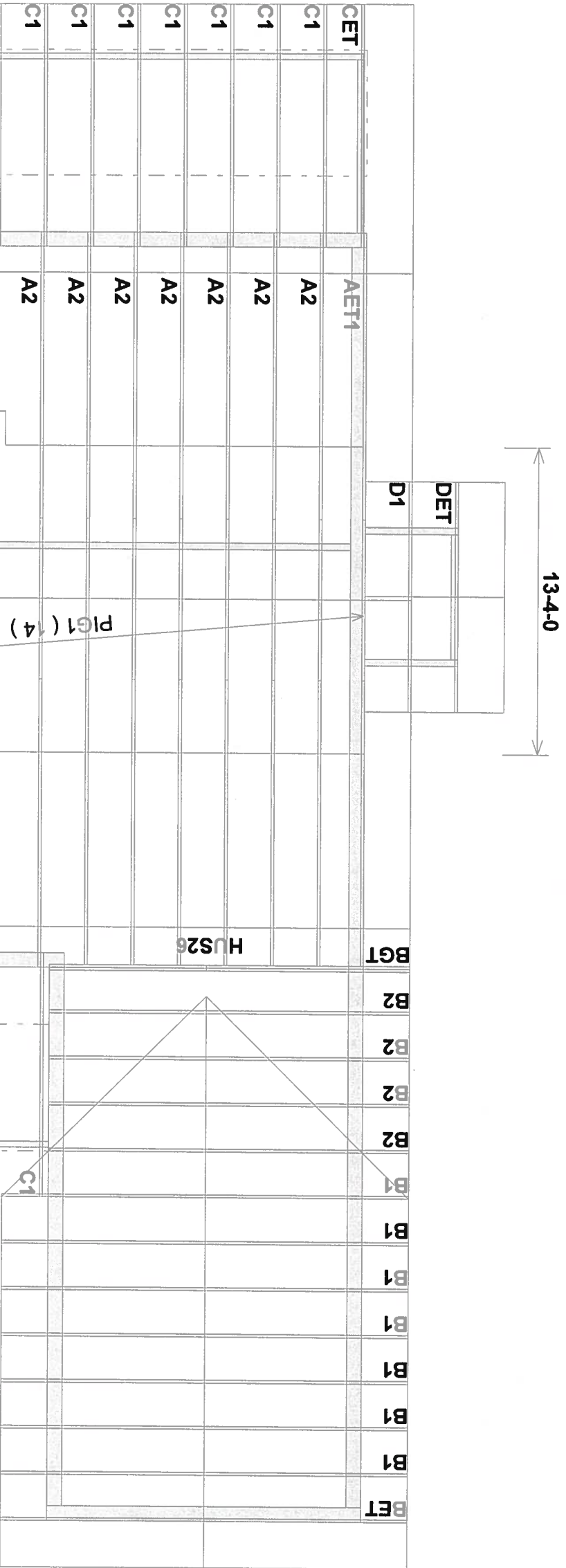
430.18' (FIELD)

40' PUBLIC R/W (20' COUNTY GRADE)

SW MAPLETON STREET

1087 Mapleton St

SIGNED:
MARK D. DUREN, LS 4708



Santa Fe Truss
410 SW POE SPRINGS RD
HIGH SPRINGS, FLORIDA
386-454-7711 / FAX 386-1055
Adam Hinton / Residence
ALL WALLS SHOWN ARE LOAD BEARING
SLAB, IN FEET-INCHES-SIXTEENTHS.
ALL HEIGHTS 10-40, EXCEPT AS NOTED
IN BOXED NUMBERS

