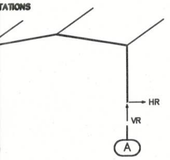
g Systems | FRAME DESCRIPTION: ca 48_/20_/14.167 20_/110_/ | USER NAME: jfrench JOB NAME: 53538A | DATE: 4/ 7/11 FILE: fl_1_4.fra | PAGE: 1-3
ONS FOR EACH LOAD GROUP ons are in kips and kip-ft. | FRAME ID #01 LOCATION: frame lines 1,4 | TIME: 09: 28: 03

GRID LINES 1 & 4

TATIONS



REACTION TABLE

	LEFT COLUMN			RIGHT COLUMN		
	8.0X11.0X0.375			8.0X11.0X0.375		
	(4)-3/4			(4)-3/4		
	HL	VL	LNL	HR	VR	LNR
	0.5	1.9	0.0	−0.5	1.9	0.0
	0.2	0.7	0.0	−0.2	0.7	0.0
	0.0	0.6	0.0	0.0	0.0	0.0
	1.4	4.1	0.0	−1.4	4.1	0.0
	0.0	0.0	0.0	0.0	0.6	0.0
	1.3	4.6	0.0	−1.3	4.6	0.0
	0.0	0.0	0.0	0.0	0.0	0.0
	−5.2	−14.4	0.0	−0.7	−9.4	0.0
	−5.4	−5.6	0.0	−0.5	−0.5	0.0
	−0.4	−11.7	0.0	0.6	−10.6	0.0
	−0.6	−10.6	0.0	0.4	−11.7	0.0
	0.4	−8.2	0.0	−0.3	−7.7	0.0
	0.3	−7.7	0.0	−0.4	−8.2	0.0
	0.7	−9.4	0.0	5.2	−14.4	0.0
	0.5	−0.5	0.0	5.4	−5.6	0.0

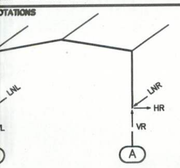
DESCRIPTION

: Roof Dead Load
: Roof Collateral Load
: Pattern Live Load Left Leanto/Canopy [PLLLxx]
: Pattern Live Load [PLLxx]
: Pattern Live Load Right Leanto/Canopy [PLLRxx]
: Roof Live Load
: Lateral Seismic Load [parallel to plane of frame]
: Lateral Primary Wind Load
: Lateral Primary Wind Load
: Longitudinal Primary Wind Load
: Longitudinal Primary Wind Load
: Longitudinal Primary Wind Load
: Lateral Primary Wind Load
: Lateral Primary Wind Load

g Systems | FRAME DESCRIPTION: ca 46_/20_/22.833 20_/110_/ | USER NAME: jfrench JOB NAME: 53538A | DATE: 4/ 7/11 FILE: fl_2_3.fra | PAGE: 2-3
TIONS FOR EACH LOAD GROUP reactions are in kips and kip-ft. seismic overstrength factor (Omega) is not included in the "LEQ" Load Group reactions. mic "BASE-ONLY" combination reactions include an overstrength factor of: 2.500 | FRAME ID #02 LOCATION: frame lines 2,3 | TIME: 09: 01: 06

GRID LINES 2 & 3

OTATIONS

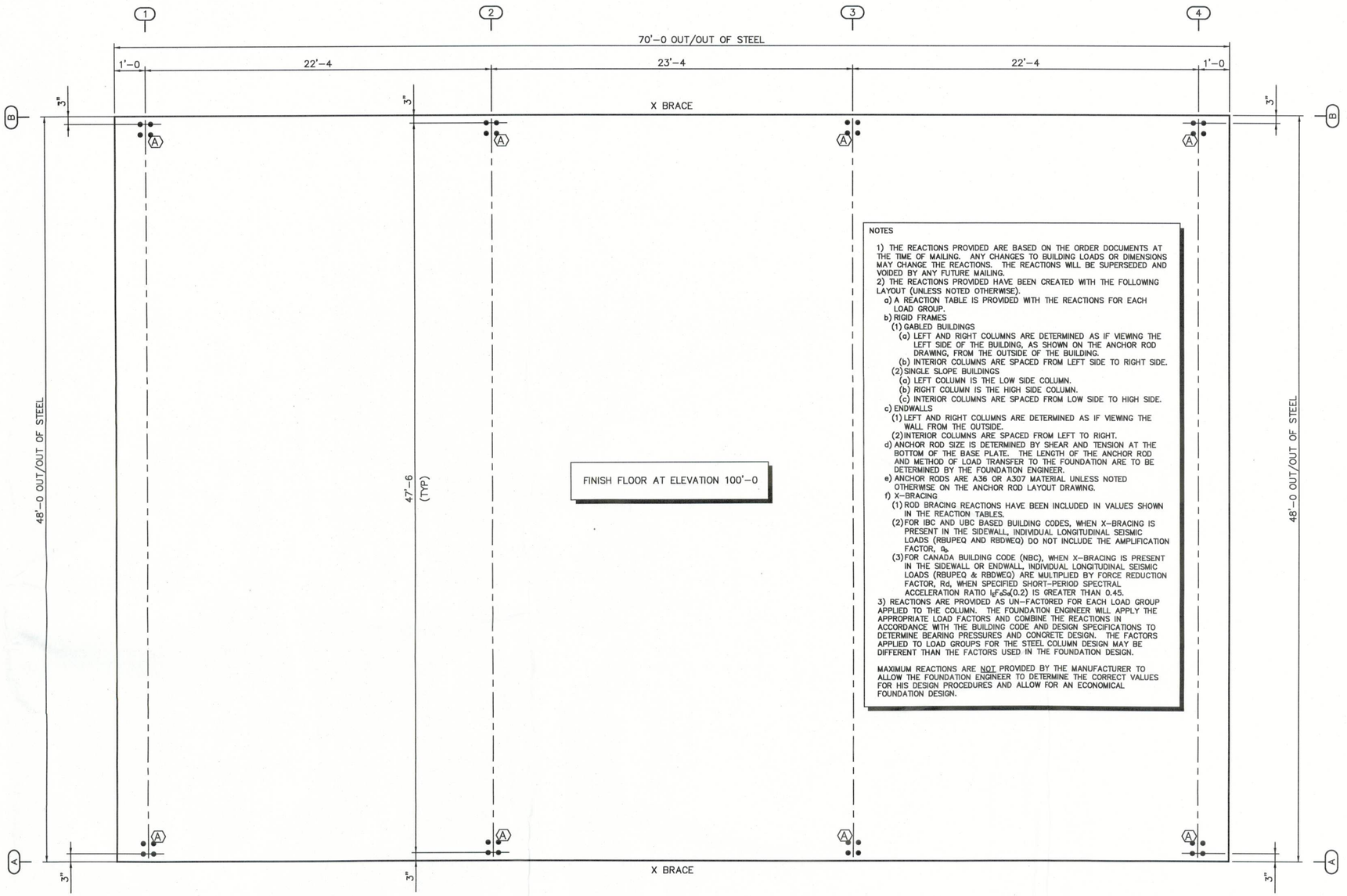


REACTION TABLE

	LEFT COLUMN			RIGHT COLUMN		
	8.0X11.0X0.375			8.0X11.0X0.375		
	(4)-3/4			(4)-3/4		
	HL	VL	LNL	HR	VR	LNR
	0.7	2.7	0.0	−0.7	2.7	0.0
	0.4	1.1	0.0	−0.4	1.1	0.0
	0.0	0.9	0.0	0.0	0.0	0.0
	2.3	6.6	0.0	−2.3	6.6	0.0
	0.0	0.0	0.0	0.0	0.9	0.0
	2.2	7.5	0.0	−2.2	7.5	0.0
	0.0	0.2	0.0	0.0	0.2	0.0
	0.0	0.0	0.0	0.0	0.0	0.0
	0.0	−0.2	−0.2	0.0	−0.2	−0.2
	−5.9	−18.2	0.0	−1.6	−12.2	0.0
	−6.9	−4.0	0.0	−0.6	2.0	0.0
	1.6	−12.2	0.0	5.9	−18.2	0.0
	0.6	2.0	0.0	6.9	−4.0	0.0
	0.0	−18.8	0.0	0.4	−17.1	0.0
	0.0	−4.4	−5.1	0.0	−4.4	−5.1
	−0.4	−17.1	0.0	0.0	−18.8	0.0
	1.3	−13.2	0.0	−1.2	−12.5	0.0
	1.2	−12.5	0.0	−1.3	−13.2	0.0
	0.0	4.4	0.0	0.0	4.4	0.0

DESCRIPTION

: Roof Dead Load
: Roof Collateral Load
: Pattern Live Load Left Leanto/Canopy [PLLLxx]
: Pattern Live Load [PLLxx]
: Pattern Live Load Right Leanto/Canopy [PLLRxx]
: Roof Live Load
: Downward Acting Rod Brace Load from Long. Seismic
: Lateral Seismic Load [parallel to plane of frame]
: Upward Acting Rod Brace Load from Longit. Seismic
: Lateral Primary Wind Load
: Lateral Primary Wind Load
: Lateral Primary Wind Load
: Longitudinal Primary Wind Load
: Upward Acting Rod Brace Load from Longitud. Wind
: Longitudinal Primary Wind Load
: Longitudinal Primary Wind Load
: Longitudinal Primary Wind Load
: Downward Acting Rod Brace Load from Longit. Wind

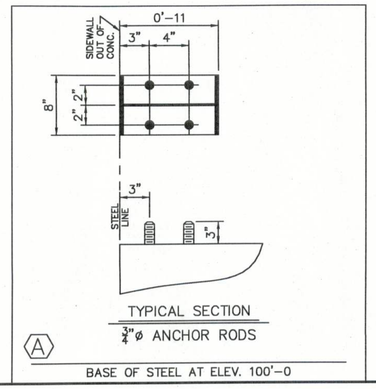


NOTES

1) THE REACTIONS PROVIDED ARE BASED ON THE ORDER DOCUMENTS AT THE TIME OF MAILING. ANY CHANGES TO BUILDING LOADS OR DIMENSIONS MAY CHANGE THE REACTIONS. THE REACTIONS WILL BE SUPERSEDED AND VOIDED BY ANY FUTURE MAILING.
2) THE REACTIONS PROVIDED HAVE BEEN CREATED WITH THE FOLLOWING LAYOUT (UNLESS NOTED OTHERWISE).
   a) A REACTION TABLE IS PROVIDED WITH THE REACTIONS FOR EACH LOAD GROUP.
   b) RIGID FRAMES
      (1) GABLED BUILDINGS
         (a) LEFT AND RIGHT COLUMNS ARE DETERMINED AS IF VIEWING THE LEFT SIDE OF THE BUILDING, AS SHOWN ON THE ANCHOR ROD DRAWING, FROM THE OUTSIDE OF THE BUILDING.
         (b) INTERIOR COLUMNS ARE SPACED FROM LEFT SIDE TO RIGHT SIDE.
      (2) SINGLE SLOPE BUILDINGS
         (a) LEFT COLUMN IS THE LOW SIDE COLUMN.
         (b) RIGHT COLUMN IS THE HIGH SIDE COLUMN.
         (c) INTERIOR COLUMNS ARE SPACED FROM LOW SIDE TO HIGH SIDE.
   c) ENDWALLS
      (1) LEFT AND RIGHT COLUMNS ARE DETERMINED AS IF VIEWING THE WALL FROM THE OUTSIDE.
      (2) INTERIOR COLUMNS ARE SPACED FROM LEFT TO RIGHT.
   d) ANCHOR ROD SIZE IS DETERMINED BY SHEAR AND TENSION AT THE BOTTOM OF THE BASE PLATE. THE LENGTH OF THE ANCHOR ROD AND METHOD OF LOAD TRANSFER TO THE FOUNDATION ARE TO BE DETERMINED BY THE FOUNDATION ENGINEER.
   e) ANCHOR RODS ARE A36 OR A307 MATERIAL UNLESS NOTED OTHERWISE ON THE ANCHOR ROD LAYOUT DRAWING.
   f) X-BRACING
      (1) ROD BRACING REACTIONS HAVE BEEN INCLUDED IN VALUES SHOWN IN THE REACTION TABLES.
      (2) FOR IBC AND UBC BASED BUILDING CODES, WHEN X-BRACING IS PRESENT IN THE SIDEWALL, INDIVIDUAL LONGITUDINAL SEISMIC LOADS (RBUPEQ AND RBDWEQ) DO NOT INCLUDE THE AMPLIFICATION FACTOR, Ω_0.
      (3) FOR CANADA BUILDING CODE (NBC), WHEN X-BRACING IS PRESENT IN THE SIDEWALL OR ENDWALL, INDIVIDUAL LONGITUDINAL SEISMIC LOADS (RBUPEQ & RBDWEQ) ARE MULTIPLIED BY FORCE REDUCTION FACTOR, Rd, WHEN SPECIFIED SHORT-PERIOD SPECTRAL ACCELERATION RATIO $I_E F_a S_a(0.2)$ IS GREATER THAN 0.45.
3) REACTIONS ARE PROVIDED AS UN-FACTORED FOR EACH LOAD GROUP APPLIED TO THE COLUMN. THE FOUNDATION ENGINEER WILL APPLY THE APPROPRIATE LOAD FACTORS AND COMBINE THE REACTIONS IN ACCORDANCE WITH THE BUILDING CODE AND DESIGN SPECIFICATIONS TO DETERMINE BEARING PRESSURES AND CONCRETE DESIGN. THE FACTORS APPLIED TO LOAD GROUPS FOR THE STEEL COLUMN DESIGN MAY BE DIFFERENT THAN THE FACTORS USED IN THE FOUNDATION DESIGN.

MAXIMUM REACTIONS ARE NOT PROVIDED BY THE MANUFACTURER TO ALLOW THE FOUNDATION ENGINEER TO DETERMINE THE CORRECT VALUES FOR HIS DESIGN PROCEDURES AND ALLOW FOR AN ECONOMICAL FOUNDATION DESIGN.

ANCHOR ROD SETTING PLAN



ANCHOR BOLTS TO BE DESIGNED BY FOUNDATON ENGINEER USING DIAMETERS SHOWN IN THIS TABLE.	
ANCHOR ROD DESCRIPTION	QUANTITY
¾"ø DIAMETER X	32

Anchor Rod Drawings

1) This drawing is for anchor rod placement only and is not foundation design.
2) Foundation must be square and level with all anchor rods true in size, location, and projection.
3) Projection shown must be held to keep threads clear of finished concrete.
4) This structural design data includes magnitude and location of design loads and suppose conditions, material properties, and type and size of major structural members necessary to show compliance with the Order Documents at the time of this issue. Any change to building loads or dimensions may change structural member sizes and locations shown. This structural design data will be superseded and voided by any future mailing.
5) Anchor rod size is determined by shear and tension at the bottom of the base plate. The length of the anchor rod and method of load transfer to the foundation are to be determined by the foundation engineer, and are not provided by the manufacturer.
6) Anchor rods are A36 or A307 material unless noted otherwise.

Revison	Date	Description	By	Ck'd
0	04/12/11	FOR ERECTOR INSTALLATION	EDB	MARK

A&S BUILDING SYSTEMS
1880 HWY. 116, CARYVILLE, TENNESSEE 37714
PHONE: 865-426-2141 FAX: 865-426-2011

Project Name & Location:
RING POWER CORPORATION
390 SW RING COURT
LAKE CITY FL 32025

Customer:
METAL BUILDING SPECIALIS
1266 LECHLADE ST.
JACKSONVILLE FL 32205

Drawing Status: ☐ Preliminary ☐ For Approval ☐ For Construction Permit ☒ For Erector Installation

Scale:	NOT TO SCALE
Drawn by:	EDB 4/8/11
Checked by:	MARK 4/8/11
Project Engineer:	JRF
Job Number:	22-T-53538-1
Sheet Number:	F1 of 1

The engineer whose seal appears hereon is an employee for the manufacturer for the materials described herein. Said seal or certification is limited to the products designed and manufactured by manufacturer only. The undersigned engineer is not the overall engineer of record for this project.

Aaron K. Batchellor, P.E.
Florida P.E. 67187

AARON K. BATCHELLOR LICENSE No. 67187 STATE OF FLORIDA PROFESSIONAL ENGINEER
Apr 13, 2011