DATE 06/12/2006

Columbia County Building Permit

This Permit Expires One Year From the Date of Issue

PERMIT
000024614

APPLICANT B. TRENT GIEBEIG PHONE 386.397.0545

ADDRESS 462 SW FAIRLINGTON COURT LAKE CITY FL 32025

OWNER B. TRENT GIEBIEG CONSTR.,INC. PHONE 386.397.0545

ADDRESS 314 SW ARROWBEND DRIVE LAKE CITY FL 32025

CONTRACTOR B. TRENT GIEBEIG PHONE 386.397.0545

LOCATION OF PROPERTY 90-W TO C-341-S,TL TO KICKLIGHTER RD TO CANNON CREEK PLACE T ARROWBEND DR,TL LOT ON R @ TH END.

TYPE DEVELOPMENT SFD/UTILITY ESTIMATED COST OF CONSTRUCTION 94850.00

HEATED FLOOR AREA 1897.00 TOTAL AREA 2650.00 HEIGHT 16.00 STORIES 1

FOUNDATION CONC WALLS FRAMED ROOF PITCH 6'12 FLOOR CONC

LAND USE & ZONING RSF-2 MAX. HEIGHT 35

Minimum Set Back Requirments: STREET-FRONT 25.00 REAR 15.00 SIDE 10.00

NO. EX.D.U. 0 FLOOD ZONE XPP DEVELOPMENT PERMIT NO.

PARCEL ID 24-4S-16-03114-132 SUBDIVISION CANNON CREEK PLACE

LOT 32 BLOCK PHASE UNIT TOTAL ACRES 0.50

000001110		R282811523		
Culvert Permit No.	Culvert Waiver	Contractor's License Number	Applicant/Owner/Contractor	
18"X32'MITERED	06-0456-N	BLK	JTH	N
Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance	New Resident

COMMENTS: M.F.E. @ 98.00'. ELEVATION LETTER REQUIRED.

Check # or Cash 1579

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power _____ date/app. by

Foundation _____ date/app. by

Monolithic _____ date/app. by

Under slab rough-in plumbing _____ date/app. by

Slab _____ date/app. by

Sheathing/Nailing _____ date/app. by

Framing _____ date/app. by

Rough-in plumbing above slab and below wood floor _____ date/app. by

Electrical rough-in _____ date/app. by

Heat & Air Duct _____ date/app. by

Peri. beam (Lintel) _____ date/app. by

Permanent power _____ date/app. by

C.O. Final _____ date/app. by

Culvert _____ date/app. by

M/H tie downs, blocking, electricity and plumbing _____ date/app. by

Pool _____ date/app. by

Reconnection _____ date/app. by

Pump pole _____ date/app. by

Utility Pole _____ date/app. by

M/H Pole _____ date/app. by

Travel Trailer _____ date/app. by

Re-roof _____ date/app. by

BUILDING PERMIT FEE $ 475.00 CERTIFICATION FEE $ 13.25 SURCHARGE FEE $ 13.25

MISC. FEES $ 0.00 ZONING CERT. FEE $ 50.00 FIRE FEE $ 0.00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ 25.00 **TOTAL FEE** 601.50

INSPECTORS OFFICE CLERKS OFFICE CH

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

NOTICE OF COMMENCEMENT

STATE OF: Florida
COUNTY OF: Columbia

The undersigned hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statues, the following information is provided in this Notice of Commencement:

1. Description of Property: Lot #32 Cannon Creek Place
314 SW Arrowbend Drive
Lake City, FL. 32025

2. General Description of Improvement: Construction of Single Family Residence

3. Owner Information:
   a. Name and Address: Trent Giebeig Construction, Inc.
   462 SW Fairlington Court Lake City, Fl. 32025
   b. Interest in Property: Fee Simple
   c. Name and Address of Fee Simple titleholder (if other than Owner):

4. Contractor (Name and Address): Trent Giebeig Construction, Inc.
462 SW Fairlington Court Lake City, Fl. 32025

5. Surety:
   a. Name and Address: N/A
   b. Amount of Bond:

6. Lender (Name and Address): N/A

7. Persons within the State of Florida designated by Owner upon notices or other documents may be Served as provided by 713,13 (l)(a)(7), Florida Statues.
N/A

8. In addition to himself, the Owner designates the following person to recieve a copy of the Lienor's Notice as provided in 713.13 (l)(b), Florida Statues (Name and Address):
N/A

9. Expiration date of Notice of Commencement (the expiration date is 1 year from the date of Recording unless a different date is specified):

Type Owner Name:

Trent Giebeig
Type Owner Name: Trent Giebeig

Elaine K. Tolar
Witness #1 Elaine K. Tolar

Lori G. Simpson
Witness #2 LORI G. SIMPSON

Sworn to and subscribed before me by the Owner (s) on this 8th day of May 2006

Elaine K. Tolar
Type Name: Elaine K. Tolar
Notary Public, State of Florida
COMMISSION EXPIRY / NUMBER:

Personally Known Trent Giebeig
Produced Identification
Did Take an Oath / Did Not Take an Oath

ELAINE K. TOLAR
MY COMMISSION # DD 436381
EXPIRES: October 2, 2009
Bonded Thru Notary Public Underwriters

Inst:2006011425 Date:05/10/2006 Time:13:24
DC,P.DeWitt Cason,Columbia County B:1083 P:776

LYNCH WELL DRILLING, INC.

173 SW Tustenuggee Ave
Lake City, FL. 32025
Phone 386-752-6677
Fax 386-752-1477

Cannon Creek Pl. Lot 32

Building Permit #_____ Owner's Name Trent Hiebert Const.

Well Depth_____ Ft. Casing Depth _____Ft. Water Level _____Ft.

Casing Size 4 inch Steel Pump Installation: Deep Well Submersible

Pump Make Aermotor Pump Model S20-100 HP 1

System Pressure (PSI) _____On 30 Off 50 Average Pressure 40

Pumping System GPM at average pressure and pumping level 20 (GPM)

Tank Installation: Bladder/Galvanized Make Challenger
Model PC244 Size 81

Tank Draw-down per cycle at system pressure 25.1 gallons

I HEREBY VERTIFY THAT THIS WATER WELL SYSTEM HAS BEEN INSTALLED AS PER THE ABOVE INFORMATION.

Linda Newcomb
Signature

Linda Newcomb
Print Name

2609
License Number

5-15-06
Date

For Office Use Only Application # 0605-54 Date Received 5-15-06 By LH Permit # 1110/24611

Application Approved by - Zoning Official BLK Date 22.06 Plans Examiner OK JTH Date 5-16-06

Flood Zone X Per PLAT Development Permit NA Zoning RSF2 Land Use Plan Map Category Res.-1 Den.

Comments M.F.E 98.00 SITE PLAN ON PLANS

Elevation Letter Required ~ 1579 -

Applicants Name Trent Gieberg Construction Inc Phone 397-0545

Address 462 SW Fairlington Ct Lake City FL 32025

Owners Name Trent Gieberg Phone 397-0545

911 Address 314 SW Arrowbend Drive Lake City FL 32025

Contractors Name Trent Gieberg Construction Inc Phone 752-0791

Address 462 SW Fairlington Ct Lake City FL 32025

Fee Simple Owner Name & Address —

Bonding Co. Name & Address —

Architect/Engineer Name & Address Freeman Design Group

Mortgage Lenders Name & Address —

Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progressive Energy

Property ID Number 03114-000 132 (24-4S-16) Estimated Cost of Construction 100,000.00

Subdivision Name Cannon Creek Place Lot 32 Block ___ Unit ___ Phase 1

Driving Directions Sisters Welcome South Left on Kicklighter right into Cannon Creek Place left SW Arrowbend Drive house on right at end

Type of Construction Frame / concrete stem wall Number of Existing Dwellings on Property 0

Total Acreage 1/2 acre Lot Size .55 Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 30 Side 40.2 Side 17.5 Rear 110

Total Building Height 16'11" Number of Stories 1 Heated Floor Area 1897 Roof Pitch 6/12

Porches 291 GARAGE 462 TOTAL 2,650

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Trent Gieberg Construction Inc
Owner Builder or Agent (including Contractor)

Trent Gieberg
Contractor Signature
Contractors License Number RR 0282811523
Competency Card Number 5754
NOTARY STAMP/SEAL

STATE OF FLORIDA
COUNTY OF COLUMBIA

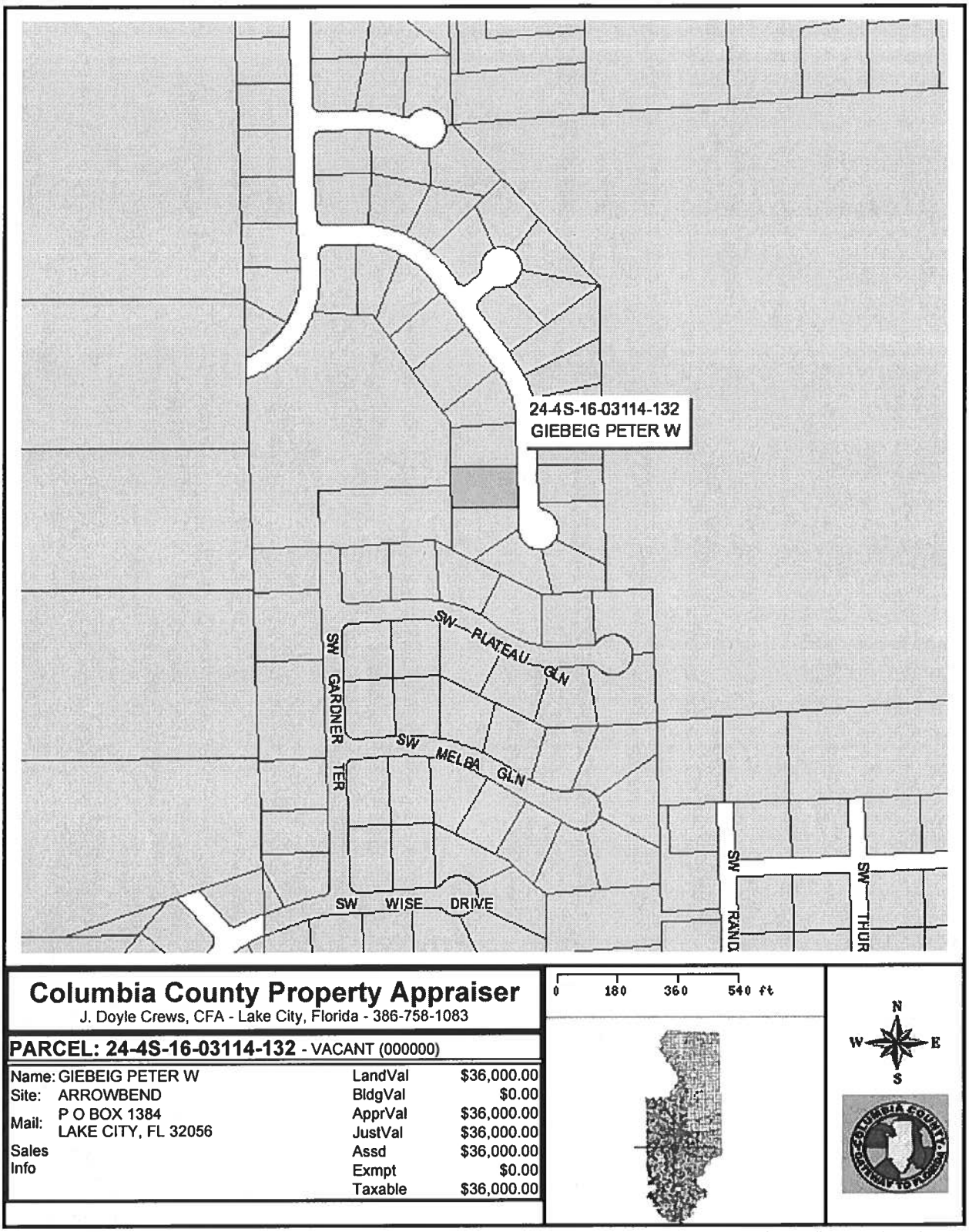
Sworn to (or affirmed) and subscribed before me this 15th day of May 2006.

Personally known X or Produced Identification ___

ELAINE K. TOLAR
MY COMMISSION # DD 430381
EXPIRES: August 2, 2009
Bonded Thru Notary Public Underwriters

Notary Signature ELAINE K. TOLAR

JW ADVISED TRENT 5.23.06



Columbia County Property Appraiser

J. Doyle Crews, CFA - Lake City, Florida - 386-758-1083

PARCEL: 24-4S-16-03114-132 - VACANT (000000)

Name:	GIEBEIG PETER W	LandVal	$36,000.00
Site:	ARROWBEND	BldgVal	$0.00
Mail:	P O BOX 1384 LAKE CITY, FL 32056	ApprVal	$36,000.00
		JustVal	$36,000.00
Sales Info		Assd	$36,000.00
		Exmpt	$0.00
		Taxable	$36,000.00

Columbia County Property Appraiser

DB Last Updated: 5/5/2006

2006 Proposed Values

Parcel: 24-4S-16-03114-132

Tax Record | Property Card | Interactive GIS Map | Print

Owner & Property Info

Search Result: 1 of 1

Owner's Name	GIEBEIG PETER W
Site Address	ARROWBEND
Mailing Address	P O BOX 1384 LAKE CITY, FL 32056
Description	LOT 32 CANNON CREEK PLACE S/D.

Use Desc. (code)	VACANT (000000)
Neighborhood	24416.00
Tax District	2
UD Codes	MKTA06
Market Area	06
Total Land Area	0.550 ACRES

Property & Assessment Values

Mkt Land Value	cnt: (1)	$36,000.00
Ag Land Value	cnt: (0)	$0.00
Building Value	cnt: (0)	$0.00
XFOB Value	cnt: (0)	$0.00
Total Appraised Value		$36,000.00

Just Value	$36,000.00
Class Value	$0.00
Assessed Value	$36,000.00
Exempt Value	$0.00
Total Taxable Value	$36,000.00

Sales History

Sale Date	Book/Page	Inst. Type	Sale Vlmp	Sale Qual	Sale RCode	Sale Price
NONE						

Building Characteristics

Bldg Item	Bldg Desc	Year Blt	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value
NONE						

Extra Features & Out Buildings

Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
NONE						

Land Breakdown

Lnd Code	Desc	Units	Adjustments	Eff Rate	Lnd Value
000000	VAC RES (MKT)	1.000 LT - (.550AC)	1.00/1.00/1.00/1.00	$36,000.00	$36,000.00

Columbia County Property Appraiser

DB Last Updated: 5/5/2006

1 of 1

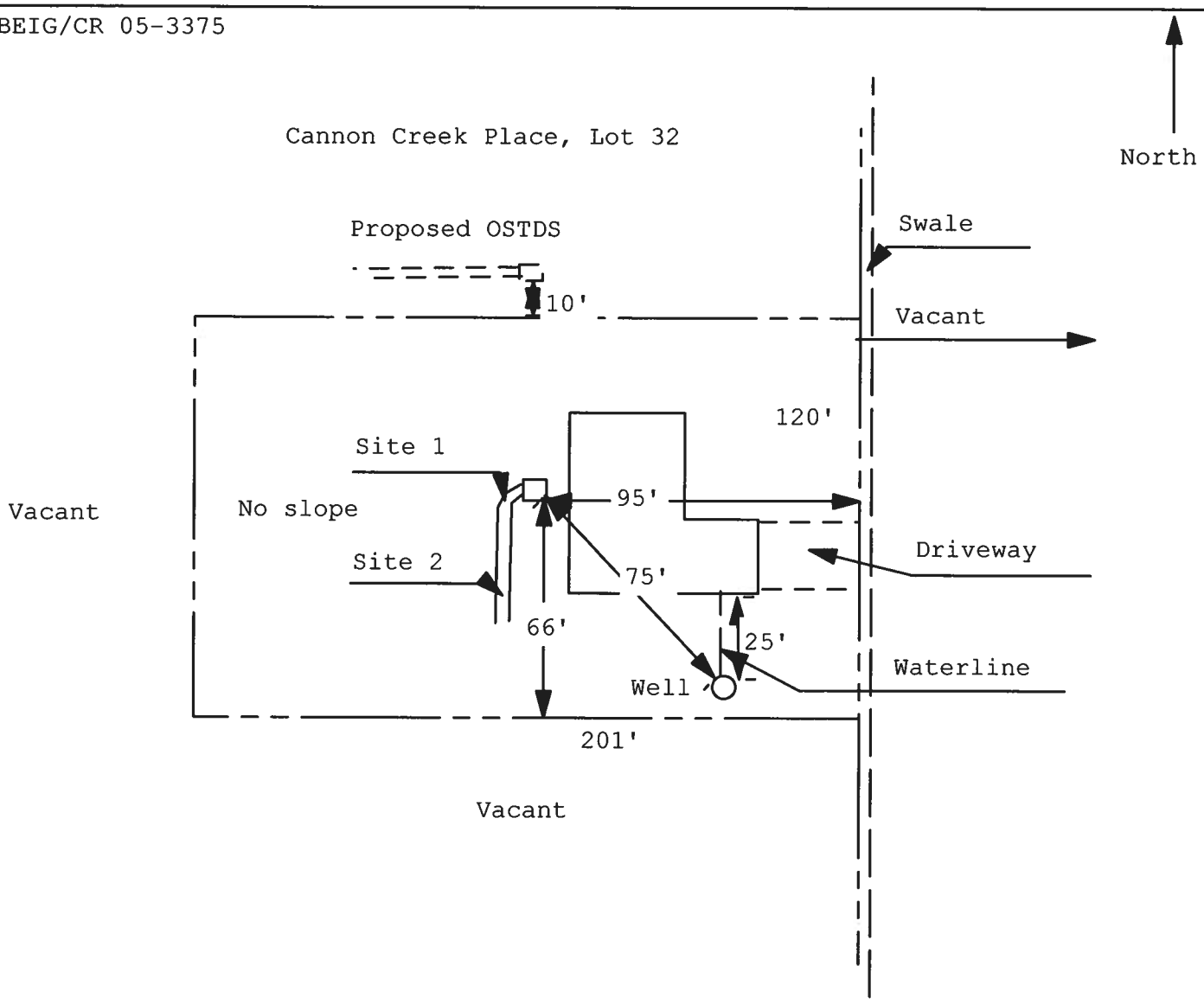
Disclaimer

Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan

Permit Application Number: 06-0456N

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT

GIEBEIG/CR 05-3375



1 inch = 50 feet

Site Plan Submitted By Paul Lloyd **Date** 5/8/06

Plan Approved ✓ **Not Approved** ____ **Date** 5/8/06

By [signature] Columbia **CPHU**

Notes: ____

Columbia County Building Department
Culvert Permit

Culvert Permit No.
000001110

DATE 06/12/2006 PARCEL ID # 24-4S-16-03114-132

APPLICANT B. TRENT GIEBEIG PHONE 386.397.0545

ADDRESS 462 SW FAIRLINGTON COURT LAKE CITY FL 32025

OWNER B. TRENT GIEBIEG CONSTR.,INC. PHONE 386.397.0545

ADDRESS 314 SW ARROWBEND DRIVE LAKE CITY FL 32025

CONTRACTOR B. TENT GIEBEIG PHONE 386.397.0545

LOCATION OF PROPERTY 90-W TO C-341-S,TL TO KICKLIGHTER RD TO CANNON CREEK PLACE TO ARROWBEND DR,TL LOT ON R @ TH END.

SUBDIVISION/LOT/BLOCK/PHASE/UNIT CANNON CREEK PLACE 32

SIGNATURE Brian Trent Giebeig

INSTALLATION REQUIREMENTS

[X] Culvert size will be 18 inches in diameter with a total lenght of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4 : 1 slope and poured with a 4 inch thick reinforced concrete slab.

INSTALLATION NOTE: Turnouts will be required as follows:
a) a majority of the current and existing driveway turnouts are paved, or;
b) the driveway to be served will be paved or formed with concrete.
Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concreted turnouts.

[ ] Culvert installation shall conform to the approved site plan standards.

[ ] Department of Transportation Permit installation approved standards.

[ ] Other _____

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED DURING THE INSTALATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00

COLUMBIA COUNTY GATEWAY TO FLORIDA

FORM 600A-2001

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs

Residential Whole Building Performance Method A

Project Name:	**St Johns Cannon Creek Lot 32**	Builder:	
Address:		Permitting Office:	Columbia 24614
City, State:	,	Permit Number:	
Owner:	**Trent Giebeig**	Jurisdiction Number:	221000
Climate Zone:	**South**		

1.	New construction or existing		New	
2.	Single family or multi-family		Single family	
3.	Number of units, if multi-family		1	
4.	Number of Bedrooms		4	
5.	Is this a worst case?		Yes	
6.	Conditioned floor area (ft²)		1897 ft²	
7.	Glass area & type	Single Pane	Double Pane	
a.	Clear glass, default U-factor	0.0 ft²	187.0 ft²	
b.	Default tint	0.0 ft²	0.0 ft²	
c.	Labeled U or SHGC	0.0 ft²	0.0 ft²	
8.	Floor types			
a.	Slab-On-Grade Edge Insulation		R=0.0, 230.2(p) ft	
b.	N/A			
c.	N/A			
9.	Wall types			
a.	Frame, Wood, Exterior		R=13.0, 1841.6 ft²	
b.	N/A			
c.	N/A			
d.	N/A			
e.	N/A			
10.	Ceiling types			
a.	Under Attic		R=30.0, 2086.7 ft²	
b.	N/A			
c.	N/A			
11.	Ducts			
a.	Sup: Con. Ret: Con. AH: Interior		Sup. R=6.0, 61.7 ft	
b.	N/A			

12.	Cooling systems		
a.	Central Unit	Cap: 36.0 kBtu/hr	
		SEER: 10.00	
b.	N/A		
c.	N/A		
13.	Heating systems		
a.	Electric Heat Pump	Cap: 36.0 kBtu/hr	
		HSPF: 7.00	
b.	N/A		
c.	N/A		
14.	Hot water systems		
a.	Electric Resistance	Cap: 50.0 gallons	
		EF: 0.90	
b.	N/A		
c.	Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)		
15.	HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)	MZ-C, PT, CF,	

Glass/Floor Area: 0.10

Total as-built points: 26010

Total base points: 32914

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: _____

DATE: 5/17/06

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____

DATE: _____

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL: _____

DATE: _____

FORM 600A-2001

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,	PERMIT #:

BASE				AS-BUILT							
GLASS TYPES .18 X Conditioned Floor Area	X BSPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	SPM X	SOF	= Points
.18	1897.0	32.50	11097.5	Double, Clear	W	1.5	6.0	30.0	61.59	0.92	1696.3
				Double, Clear	W	1.5	6.0	30.0	61.59	0.92	1696.3
				Double, Clear	W	1.5	6.0	20.0	61.59	0.92	1130.9
				Double, Clear	N	1.5	4.0	6.0	31.93	0.89	169.6
				Double, Clear	E	1.5	7.0	30.0	68.60	0.94	1937.1
				Double, Clear	E	1.5	6.0	60.0	68.60	0.92	3776.2
				Double, Clear	S	1.5	6.0	6.0	58.45	0.87	306.6
				Double, Clear	S	1.5	2.0	5.0	58.45	0.57	166.7
				As-Built Total:				**187.0**			**10879.6**
WALL TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points
Adjacent	0.0	0.00	0.0	Frame, Wood, Exterior			13.0	1841.6	2.40		4419.8
Exterior	1841.6	2.70	4972.3								
Base Total:	**1841.6**		**4972.3**	**As-Built Total:**				**1841.6**			**4419.8**
DOOR TYPES	Area X	BSPM =	Points	Type				Area X	SPM =		Points
Adjacent	0.0	0.00	0.0	Exterior Insulated				34.0	6.40		217.6
Exterior	34.0	6.40	217.6								
Base Total:	**34.0**		**217.6**	**As-Built Total:**				**34.0**			**217.6**
CEILING TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM X SCM =		Points
Under Attic	1897.0	2.80	5311.6	Under Attic			30.0	2086.7	2.77 X 1.00		5780.2
Base Total:	**1897.0**		**5311.6**	**As-Built Total:**				**2086.7**			**5780.2**
FLOOR TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points
Slab	230.2(p)	-20.0	-4604.0	Slab-On-Grade Edge Insulation			0.0	230.2(p	-20.00		-4604.0
Raised	0.0	0.00	0.0								
Base Total:			**-4604.0**	**As-Built Total:**				**230.2**			**-4604.0**
INFILTRATION	Area X	BSPM =	Points					Area X	SPM =		Points
	1897.0	18.79	35644.6					1897.0	18.79		35644.6

FORM 600A-2001

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,	PERMIT #:

BASE				AS-BUILT						
Summer Base Points:			**52639.6**	**Summer As-Built Points:**						**52337.8**
Total Summer Points	X System Multiplier	=	Cooling Points	Total Component	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	=	Cooling Points
52639.6	**0.4266**		**22456.1**	52337.8 **52337.8**	1.000 **1.00**	(1.000 x 1.165 x 0.90) **1.048**	0.341 **0.341**	0.857 **0.857**		16043.9 **16043.9**

FORM 600A-2001

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,	PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BWPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	WPM X	WOF	= Points	
.18	1897.0	2.36	805.8	Double, Clear	W	1.5	6.0	30.0	3.98	1.00	119.1	
				Double, Clear	W	1.5	6.0	30.0	3.98	1.00	119.1	
				Double, Clear	W	1.5	6.0	20.0	3.98	1.00	79.4	
				Double, Clear	N	1.5	4.0	6.0	4.38	0.99	25.9	
				Double, Clear	E	1.5	7.0	30.0	3.30	1.02	100.8	
				Double, Clear	E	1.5	6.0	60.0	3.30	1.02	202.2	
				Double, Clear	S	1.5	6.0	6.0	3.12	1.02	19.1	
				Double, Clear	S	1.5	2.0	5.0	3.12	1.25	19.5	
				As-Built Total:				**187.0**			**685.1**	
WALL TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points	
Adjacent	0.0	0.00	0.0	Frame, Wood, Exterior			13.0	1841.6	0.60		1105.0	
Exterior	1841.6	0.60	1105.0									
Base Total:	**1841.6**		**1105.0**	**As-Built Total:**				**1841.6**			**1105.0**	
DOOR TYPES	Area X	BWPM =	Points	Type				Area X	WPM =		Points	
Adjacent	0.0	0.00	0.0	Exterior Insulated				34.0	1.80		61.2	
Exterior	34.0	1.80	61.2									
Base Total:	**34.0**		**61.2**	**As-Built Total:**				**34.0**			**61.2**	
CEILING TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM X	WCM =	Points	
Under Attic	1897.0	0.10	189.7	Under Attic			30.0	2086.7	0.10 X	1.00	208.7	
Base Total:	**1897.0**		**189.7**	**As-Built Total:**				**2086.7**			**208.7**	
FLOOR TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points	
Slab	230.2(p)	-2.1	-483.4	Slab-On-Grade Edge Insulation			0.0	230.2(p	-2.10		-483.4	
Raised	0.0	0.00	0.0									
Base Total:			**-483.4**	**As-Built Total:**				**230.2**			**-483.4**	
INFILTRATION	Area X	BWPM =	Points					Area X	WPM =		Points	
	1897.0	-0.06	-113.8					1897.0	-0.06		-113.8	

FORM 600A-2001

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,	PERMIT #:

BASE			AS-BUILT						
Winter Base Points:		**1564.5**	**Winter As-Built Points:**						**1462.7**
Total Winter Points	X System Multiplier	= Heating Points	Total Component	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points	
1564.5	**0.6274**	**981.5**	1462.7 **1462.7**	1.000 **1.00**	(1.000 x 1.137 x 0.91) **1.035**	0.487 **0.487**	0.950 **0.950**	700.4 **700.4**	

FORM 600A-2001

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,	PERMIT #:

BASE				AS-BUILT						
WATER HEATING										
Number of Bedrooms	X Multiplier	= Total		Tank Volume	EF	Number of Bedrooms	X Tank Ratio	X Multiplier	X Credit Multiplier	= Total
4	2369.00	9476.0		50.0	0.90	4	1.00	2316.36	1.00	9265.4
				As-Built Total:						**9265.4**

CODE COMPLIANCE STATUS

BASE				AS-BUILT			
Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points	Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points
22456	**982**	**9476**	**32914**	**16044**	**700**	**9265**	**26010**

PASS

FORM 600A-2001

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,	PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 6-12. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 87.1

The higher the score, the more efficient the home.

Trent Giebeig, , , ,

1. New construction or existing		New	
2. Single family or multi-family		Single family	
3. Number of units, if multi-family		1	
4. Number of Bedrooms		4	
5. Is this a worst case?		Yes	
6. Conditioned floor area (ft²)		1897 ft²	
7. Glass area & type	Single Pane	Double Pane	
a. Clear - single pane	0.0 ft²	187.0 ft²	
b. Clear - double pane	0.0 ft²	0.0 ft²	
c. Tint/other SHGC - single pane	0.0 ft²	0.0 ft²	
d. Tint/other SHGC - double pane			
8. Floor types			
a. Slab-On-Grade Edge Insulation		R=0.0, 230.2(p) ft	
b. N/A			
c. N/A			
9. Wall types			
a. Frame, Wood, Exterior		R=13.0, 1841.6 ft²	
b. N/A			
c. N/A			
d. N/A			
e. N/A			
10. Ceiling types			
a. Under Attic		R=30.0, 2086.7 ft²	
b. N/A			
c. N/A			
11. Ducts			
a. Sup: Con. Ret: Con. AH: Interior		Sup. R=6.0, 61.7 ft	
b. N/A			

12. Cooling systems		
a. Central Unit		Cap: 36.0 kBtu/hr
		SEER: 10.00
b. N/A		
c. N/A		
13. Heating systems		
a. Electric Heat Pump		Cap: 36.0 kBtu/hr
		HSPF: 7.00
b. N/A		
c. N/A		
14. Hot water systems		
a. Electric Resistance		Cap: 50.0 gallons
		EF: 0.90
b. N/A		
c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)		
15. HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)		MZ-C, PT, CF,

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: _____ Date: _____

Address of New Home: _____ City/FL Zip: _____

**NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar™ designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/488-1824.*

Residential System Sizing Calculation

Summary

Trent Giebeig

Project Title:
St Johns Cannon Creek Lot 32

Code Only
Professional Version
Climate: South

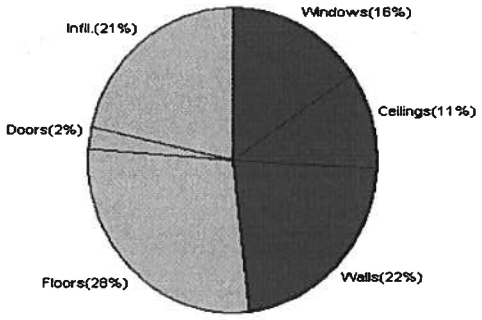
5/17/2006

Location for weather data: Gainesville - User customized: Latitude(29) Temp Range(M)					
Humidity data: Interior RH (50%) Outdoor wet bulb (78F) Humidity difference(51gr.)					
Winter design temperature	31	F	Summer design temperature	98	F
Winter setpoint	70	F	Summer setpoint	75	F
Winter temperature difference	39	F	Summer temperature difference	23	F
Total heating load calculation	**25776**	**Btuh**	**Total cooling load calculation**	**24894**	**Btuh**
Submitted heating capacity	% of calc	Btuh	Submitted cooling capacity	% of calc	Btuh
Total (Electric Heat Pump)	139.7	36000	Sensible (SHR = 0.5)	93.7	18000
Heat Pump + Auxiliary(0.0kW)	139.7	36000	Latent	316.6	18000
			Total (Electric Heat Pump)	144.6	36000

WINTER CALCULATIONS

Winter Heating Load (for 1897 sqft)

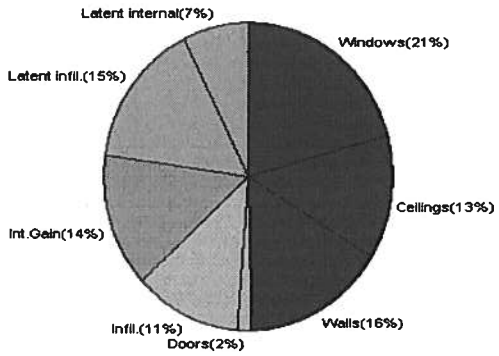
Load component			Load	
Window total	187	sqft	4021	Btuh
Wall total	1842	sqft	5709	Btuh
Door total	34	sqft	623	Btuh
Ceiling total	2087	sqft	2713	Btuh
Floor total	230	ft	7274	Btuh
Infiltration	127	cfm	5436	Btuh
Subtotal			**25776**	**Btuh**
Duct loss			0	Btuh
TOTAL HEAT LOSS			**25776**	**Btuh**



SUMMER CALCULATIONS

Summer Cooling Load (for 1897 sqft)

Load component			Load	
Window total	187	sqft	5182	Btuh
Wall total	1842	sqft	3941	Btuh
Door total	34	sqft	424	Btuh
Ceiling total	2087	sqft	3255	Btuh
Floor total			0	Btuh
Infiltration	111	cfm	2805	Btuh
Internal gain			3600	Btuh
Subtotal(sensible)			**19208**	**Btuh**
Duct gain			0	Btuh
Total sensible gain			**19208**	**Btuh**
Latent gain(infiltration)			3845	Btuh
Latent gain(internal)			1840	Btuh
Total latent gain			**5685**	**Btuh**
TOTAL HEAT GAIN			**24894**	**Btuh**



EnergyGauge® System Sizing based on ACCA Manual J.
PREPARED BY: Trent Giebeig
DATE: 5/17/06

System Sizing Calculations - Winter

Residential Load - Component Details

Trent Giebeig

Project Title:
St Johns Cannon Creek Lot 32

Code Only
Professional Version
Climate: South

Reference City: Gainesville (User customized) Winter Temperature Difference: 39.0 F

5/17/2006

Window	Panes/SHGC/Frame/U	Orientation	Area X	HTM=	Load
1	2, Clear, Wood, DEF	N	30.0	21.5	645 Btuh
2	2, Clear, Wood, DEF	N	30.0	21.5	645 Btuh
3	2, Clear, Wood, DEF	N	20.0	21.5	430 Btuh
4	2, Clear, Wood, DEF	E	6.0	21.5	129 Btuh
5	2, Clear, Wood, DEF	S	30.0	21.5	645 Btuh
6	2, Clear, Wood, DEF	S	60.0	21.5	1290 Btuh
7	2, Clear, Wood, DEF	W	6.0	21.5	129 Btuh
8	2, Clear, Wood, DEF	W	5.0	21.5	108 Btuh
	Window Total		187		4021 Btuh
Walls	Type	R-Value	Area X	HTM=	Load
1	Frame - Exterior	13.0	1842	3.1	5709 Btuh
	Wall Total		1842		5709 Btuh
Doors	Type		Area X	HTM=	Load
1	Insulated - Exter		34	18.3	623 Btuh
	Door Total		34		623Btuh
Ceilings	Type	R-Value	Area X	HTM=	Load
1	Under Attic	30.0	2087	1.3	2713 Btuh
	Ceiling Total		2087		2713Btuh
Floors	Type	R-Value	Size X	HTM=	Load
1	Slab-On-Grade Edge Insul	0	230.2 ft(p)	31.6	7274 Btuh
	Floor Total		230		7274 Btuh
Infiltration	Type	ACH X	Building Volume	CFM=	Load
	Natural	0.40	18970(sqft)	127	5436 Btuh
	Mechanical			0	0 Btuh
	Infiltration Total			127	5436 Btuh

Totals for Heating	**Subtotal**	**25776 Btuh**
	Duct Loss(using duct multiplier of 0.00)	**0 Btuh**
	Total Btuh Loss	**25776 Btuh**

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

System Sizing Calculations - Summer

Residential Load - Component Details

Trent Giebeig

Project Title:
St Johns Cannon Creek Lot 32

Code Only
Professional Version
Climate: South

Reference City: Gainesville (User customized) Summer Temperature Difference: 23.0 F 5/17/2006

Window	Type Panes/SHGC/U/InSh/ExSh	Ornt	Overhang Len	Hgt	Window Area(sqft) Gross	Shaded	Unshaded	HTM Shaded	Unshaded	Load	
1	2, Clear, DEF, N, N	N	1.5	6	30.0	0.0	30.0	24	24	720	Btuh
2	2, Clear, DEF, N, N	N	1.5	6	30.0	0.0	30.0	24	24	720	Btuh
3	2, Clear, DEF, N, N	N	1.5	6	20.0	0.0	20.0	24	24	480	Btuh
4	2, Clear, DEF, N, N	E	1.5	4	6.0	0.0	6.0	24	74	444	Btuh
5	2, Clear, DEF, N, N	S	1.5	7	30.0	30.0	0.0	24	39	720	Btuh
6	2, Clear, DEF, N, N	S	1.5	6	60.0	60.0	0.0	24	39	1440	Btuh
7	2, Clear, DEF, N, N	W	1.5	6	6.0	0.0	6.0	24	74	444	Btuh
8	2, Clear, DEF, N, N	W	1.5	2	5.0	3.1	1.9	24	74	214	Btuh
	Window Total				187					5182	Btuh

Walls	Type	R-Value	Area	HTM	Load	
1	Frame - Exterior	13.0	1841.6	2.1	3941	Btuh
	Wall Total		1841.6		3941	Btuh

Doors	Type	Area	HTM	Load	
1	Insulated - Exter	34.0	12.5	424	Btuh
	Door Total	34.0		424	Btuh

Ceilings	Type/Color	R-Value	Area	HTM	Load	
1	Under Attic/Dark	30.0	2086.7	1.6	3255	Btuh
	Ceiling Total		2086.7		3255	Btuh

Floors	Type	R-Value	Size	HTM	Load	
1	Slab-On-Grade Edge Insulation	0.0	230.2 ft(p)	0.0	0	Btuh
	Floor Total		230.2		0	Btuh

Infiltration	Type	ACH	Volume	CFM=	Load	
	Natural	0.35	18970	110.9	2805	Btuh
	Mechanical			0	0	Btuh
	Infiltration Total			111	2805	Btuh

Internal gain	Occupants	Btuh/occupant	Appliance	Load	
	8	X 300 +	1200	3600	Btuh

Totals for Cooling			
	Subtotal	**19208**	**Btuh**
	Duct gain(using duct multiplier of 0.00)	**0**	**Btuh**
	Total sensible gain	**19208**	**Btuh**
	Latent infiltration gain (for 51 gr. humidity difference)	**3845**	**Btuh**
	Latent occupant gain (8 people @ 230 Btuh per person)	**1840**	**Btuh**
	Latent other gain	**0**	**Btuh**
	TOTAL GAIN	**24894**	**Btuh**

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds/Drapes/Curtains(B), Roller Shades(R))
(ExSh - Exterior shading device: none(N) or numerical value)
(Ornt - compass orientation)

BRITT SURVEYING
830 West Duval Street • Lake City, FL 32055
Phone (386) 752-7163 • Fax (386) 752-5573

Land Surveyors and Mappers

11/15/06

L-17932

To Whom It May Concern:

C/o: Trent Giebeig

Re: Lot 32 cannon Creek Place

The elevation of the slab is found to be 99.00 feet. The minimum finished floor elevation is 98.00 feet according to the plat of record. The center line of the road is 96.99 feet. The highest adjacent grade is 98.5 feet and the lowest adjacent grade is 97.3 feet. The elevations shown hereon are based on NGVD 29 datum.

L. Scott Britt
PLS #5757

24614

From: The Columbia County Building & Zoning Department
Plan Review
135 NE Hernando Av.
P.O. Box 1529
Lake City Florida 32056-1529

Reference to a building permit application Number: 0605-53

Contractor: Trent Giebeig Owners Trent Giebeig Lot 7 Phase 1 Cannon Creek Place

On the date of May 16, 2006 application 0605-53 and plans for construction of a single family dwelling were reviewed and the following information or alteration to the plans will be required to continue processing this application. If you should have any question please contact the above address, or contact phone number (386) 758-1163 or fax any information to (386) 754-7088.

Please include application number 0605-53 when making reference to this application.

1. Line 6, conditioned area square footage on form 600A-2004 of the Florida Energy Efficiency Code for Building Construction doesn't concur with the conditioned floor area on the submitted plans. The total conditioned areas on the plans are 1897 (square feet). Line 6 currently reads that the conditioned floor area equals1972 (square feet). *Please resubmit the corrected form to reflect on line 6 the actual total conditioned area to this department.*

Thank you,

Joe Haltiwanger
Plan Examiner
Columbia County Building Department

IF 52

COLUMBIA COUNTY BUILDING DEPARTMENT

Revised 10-01-05

RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2004 and FLORIDA RESIDENTIAL CODE 2004 WITH AMENDMENTS ONE (1) AND TWO (2) FAMILY DWELLINGS

ALL REQUIREMENTS ARE SUBJECT TO CHANGE
EFFECTIVE OCTOBER 1, 2005

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 16 OF THE FLORIDA BUILDING CODE 2004 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1609 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ------- 100 MPH
2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE -------110 MPH
3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

GENERAL REQUIREMENTS: Two (2) complete sets of plans containing the following:

Applicant	Plans Examiner	
☑	☐	All drawings must be clear, concise and drawn to scale ("Optional " details that are not used shall be marked void or crossed off). Square footage of different areas shall be shown on plans.
☑	☐	Designers name and signature on document (FBC 106.1). If licensed architect or engineer, official seal shall be affixed.
☑	☐	**Site Plan including:** a) Dimensions of lot b) Dimensions of building set backs c) Location of all other buildings on lot, well and septic tank if applicable, and all utility easements. d) Provide a full legal description of property.
☑	☐	**Wind-load Engineering Summary, calculations and any details required** Plans or specifications must state compliance with FBC Section 1609. The following information must be shown as per section 1603.1.4 FBC a. Basic wind speed (3-second gust), miles per hour (km/hr). b. Wind importance factor, Iw, and building classification from Table 1604.5 or Table 6-1, ASCE 7 and building classification in Table 1-1, ASCE 7. c. Wind exposure, if more than one wind exposure is utilized, the wind exposure and applicable wind direction shall be indicated. d. The applicable enclosure classifications and, if designed with ASCE 7, internal pressure coefficient. e. Components and Cladding. The design wind pressures in terms of psf (kN/m²) to be used for the design of exterior component and cladding materials not specifally designed by the registered design professional.
		Elevations including:
☑	☐	a) All sides
☑	☐	b) Roof pitch
☑	☐	c) Overhang dimensions and detail with attic ventilation

☑ ☐ d) Location, size and height above roof of chimneys.
☑ ☐ e) Location and size of skylights
☑ ☐ f) Building height
☑ ☐ e) Number of stories

Floor Plan including:

☑ ☐ a) Rooms labeled and dimensioned.
☑ ☐ b) Shear walls identified.
☑ ☐ c) Show product approval specification as required by Fla. Statute 553.842 and Fla. Administrative Code 9B-72 **(see attach forms).**
☑ ☐ d) Show safety glazing of glass, where required by code.
☑ ☐ e) Identify egress windows in bedrooms, and size.
☑ ☐ f) Fireplace (gas vented), (gas non-vented) or wood burning with hearth, **(Please circle applicable type).**
☑ ☐ g) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails.
☑ ☐ h) Must show and identify accessibility requirements (accessible bathroom)

Foundation Plan including:

☑ ☐ a) Location of all load-bearing wall with required footings indicated as standard or monolithic and dimensions and reinforcing.
☑ ☐ b) All posts and/or column footing including size and reinforcing
☑ ☐ c) Any special support required by soil analysis such as piling
☑ ☐ d) Location of any vertical steel.

Roof System:

☑ ☐ a) Truss package including:
1. Truss layout and truss details signed and sealed by Fl. Pro. Eng.
2. Roof assembly (FBC 106.1.1.2)Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

☑ ☐ b) Conventional Framing Layout including:
1. Rafter size, species and spacing
2. Attachment to wall and uplift
3. Ridge beam sized and valley framing and support details
4. Roof assembly (FBC 106.1.1.2)Roofing systems, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

Wall Sections including:

☑ ☐ a) Masonry wall
1. All materials making up wall
2. Block size and mortar type with size and spacing of reinforcement
3. Lintel, tie-beam sizes and reinforcement
4. Gable ends with rake beams showing reinforcement or gable truss and wall bracing details
5. All required connectors with uplift rating and required number and size of fasteners for continuous tie from roof to foundation shall be designed by a Windload engineer using the engineered roof truss plans.
6. Roof assembly shown here or on roof system detail (FBC 106.1.1.2) Roofing system, materials, manufacturer, fastening requirements and product evaluation with resistance rating)
7. Fire resistant construction (if required)
8. Fireproofing requirements
9. Shoe type of termite treatment (termiticide or alternative method)
10. Slab on grade
    a. Vapor retarder (6mil. Polyethylene with joints lapped 6 inches and sealed)
    b. Must show control joints, synthetic fiber reinforcement or Welded fire fabric reinforcement and supports
11. Indicate where pressure treated wood will be placed
12. Provide insulation R value for the following:

a. Attic space
b. Exterior wall cavity
c. Crawl space (if applicable)

☑ ☐ b) Wood frame wall

1. All materials making up wall
2. Size and species of studs
3. Sheathing size, type and nailing schedule
4. Headers sized
5. Gable end showing balloon framing detail or gable truss and wall hinge bracing detail
6. All required fasteners for continuous tie from roof to foundation (truss anchors, straps, anchor bolts and washers) shall be designed by a Windload engineer using the engineered roof truss plans.
7. Roof assembly shown here or on roof system detail (FBC 106.1.1.2) Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
8. Fire resistant construction (if applicable)
9. Fireproofing requirements
10. Show type of termite treatment (termiticide or alternative method)
11. Slab on grade
    a. Vapor retarder (6Mil. Polyethylene with joints lapped 6 inches and sealed
    b. Must show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and supports
12. Indicate where pressure treated wood will be placed
13. Provide insulation R value for the following:
    a. Attic space
    b. Exterior wall cavity
    c. Crawl space (if applicable)

☑ ☐ c) Metal frame wall and roof (designed, signed and sealed by Florida Prof. Engineer or Architect)

Floor Framing System:

☐ ☐ a) Floor truss package including layout and details, signed and sealed by Florida Registered Professional Engineer
☐ ☐ b) Floor joist size and spacing
☐ ☐ c) Girder size and spacing
☐ ☐ d) Attachment of joist to girder
☐ ☐ e) Wind load requirements where applicable

Plumbing Fixture layout

Electrical layout including:

☑ ☐ a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified
☑ ☐ b) Ceiling fans
☑ ☐ c) Smoke detectors
☑ ☐ d) Service panel and sub-panel size and location(s)
☑ ☐ e) Meter location with type of service entrance (overhead or underground)
☑ ☐ f) Appliances and HVAC equipment
☑ ☐ g) Arc Fault Circuits (AFCI) in bedrooms
☑ ☐ h) Exhaust fans in bathroom

HVAC information

☑ ☐ a) Energy Calculations (dimensions shall match plans)
☑ ☐ b) Manual J sizing equipment or equivalent computation
☐ ☐ c)Gas System Type (LP or Natural) Location and BTU demand of equipment
☑ ☐ **Disclosure Statement for Owner Builders**
☑ ☐ ***Notice Of Commencement Required Before Any Inspections Will Be Done**
☐ ☐ **Private Potable Water**

a) Size of pump motor
b) Size of pressure tank
c) Cycle stop valve if used

THE FOLLOWING ITEMS MUST BE SUBMITTED WITH BUILDING PLANS

1. **Building Permit Application:** A current Building Permit Application form is to be completed and submitted for all residential projects.

2. **Parcel Number:** The parcel number (Tax ID number) from the Property Appraiser (386) 758-1084 is required. A copy of property deed is also requested.

3. **Environmental Health Permit or Sewer Tap Approval:** A copy of the Environmental Health permit, existing septic approval or sewer tap approval is required before a building permit can be issued. (386) 758-1058 **(Toilet facilities shall be provided for construction workers)**

4. **City Approval: If the project is to be located within the city limits of the Town of Fort White**, prior approval is required. The Town of Fort White approval letter is required to be submitted by the owner or contractor to this office when applying for a Building Permit. **(386) 497-2321**

5. **Flood Information:** All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting application to this office. Any project located within a flood zone where the base flood elevation (100 year flood) has been established shall meet the requirements of Section 8.8 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.7 of the Columbia County Land Development Regulations. **CERTIFIED FINISHED FLOOR ELEVATIONS WILL BE REQUIRED ON ANY PROJECT WHERE THE BASE FLOOD ELEVATION (100 YEAR FLOOD) HAS BEEN ESTABLISHED.**
A development permit will also be required. Development permit cost is **$50.00**

6. **Driveway Connection:** If the property does not have an existing access to a public road, then an application for a culvert permit **($25.00)** must be made. If the applicant feels that a culvert is not needed, they may apply for a culvert waiver **($50.00)**. All culvert waivers are sent to the Columbia County Public Works Department for approval or denial. **If the project is to be located on a F.D.O.T. maintained road, than an F.D.O.T. access permit is required.**

7. **911 Address:** If the project is located in an area where the 911 address has been issued, then the proper paperwork from the 911 Addressing Department must be submitted. (386) 752-8787

ALL REQUIRED INFORMATION IS TO BE SUBMITTED FOR REVIEW. YOU WILL BE NOTIFIED WHEN YOUR APPLICATION AND PLANS ARE APPROVED AND READY TO PERMIT. PLEASE DO NOT EXPECT OR REQUEST THAT PERMIT APPLICATIONS BE REVIEWED OR APPROVED WHILE YOU ARE HERE – TIME WILL NOT ALLOW THIS –PLEASE DO NOT ASK

Architectural Testing

ANSI/AAMA/NWWDA 101/I.S.2-97
TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 480/680/880 Drop-in
PRODUCT TYPE: Aluminum Horizontal Sliding Window (XO-Fin)

	Results	
Title	**Test Specimen #1**	**Test Specimen #2**
Rating	HS-C30 71 x 71	HS-C40 71 x 59
Operating Force	11 lbf max.	14 lbf max.
Air Infiltration	0.11 cfm/ft^2	0.09 cfm/ft^2
Water Resistance Test Pressure	5.3 psf	6.0 psf
Uniform Load Deflection Test Pressure	± 30.0 psf	+ 45.0 psf -47.2 psf
Uniform Structural Load Test Pressure	± 45.0 psf	+ 67.5 psf -70.8 psf
Forced Entry Resistance	Grade 10	Grade 10

Reference should be made to ATI Report Identification No. 01-47320.03 for complete test specimen description and data

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.archtest.com

Architectural Testing

ANSI/AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.
P.O. Box 370
650 West Market Street
Gratz, Pennsylvania 17030-0370

ATI Report Identification No.: 01-47320.03
Test Dates: 10/07/03
Through: 10/08/03
And: 12/01/03
And: 12/15/03
And: 03/17/04
Report Date: 04/16/04
Expiration Date: 10/07/07

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to witness testing on two Series/Model 480/680/880 Drop-in, aluminum horizontal sliding windows at MI Home Products, Inc. test facility in Elizabethville, Pennsylvania. The samples tested successfully met the performance requirements for the following ratings: Test Specimen #1: HS-C30 71 x 71; Test Specimen #2: HS-C40 71 x 59. Test specimen description and results are reported herein.

Test Specification: The test specimens were evaluated in accordance with ANSI/AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 480/680/880 Drop-in

Product Type: Aluminum Horizontal Sliding Window (XO Fin)

Test Specimen #1: HS-C30 71 x 71

Overall Size: 5' 11-7/16" wide by 5' 11" high

Active Sash Size: 2' 11-5/8" wide by 5' 8-3/8" high

Fixed Daylight Opening Size: 2' 8-3/16" wide by 5' 5-5/8" high

Screen Size: 2' 10" wide by 5' 6-1/2" high

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.archtest.com

Test Specimen Description: (Continued)

Weatherstripping:

Description	Quantity	Location
0.250" high by 0.187" backed polypile with center fin	1 Row	Active sash top and bottom rails and fixed meeting rail interlock
0.250" high by 0.187" backed polypile with center fin	2 Rows	Jamb stile

Test Specimen #2: HS-C40 71 x 59

Overall Size: 5' 11-3/8" wide by 4' 11-1/8" high

Active Sash Size: 2' 11-5/8" wide by 4' 8-1/4" high

Fixed Daylight Opening Size: 2' 8-1/4" wide by 4' 5-7/8" high

Screen Size: 2' 10-1/4" wide by 4' 7-1/8" high

Weatherstripping:

Description	Quantity	Location
0.310" high by 0.187" backed polypile with center fin	1 Row	Active sash top and bottom rails
0.250" high by 0.187" backed polypile with center fin	1 Rows	Fixed meeting rail interlock
0.310" high by 0.187" backed polypile with center fin	2 Rows	Jamb stile
0.550" high by 1" by 1" backed polypile pad	1 Pad	Corner of bottom rail and locking stile

Test Specimen Description: (Continued)

The following descriptions apply to all specimens.

Finish: All aluminum was white.

Glazing Details: The window utilized 5/8" thick sealed insulating glass constructed from two sheets of 1/8" thick clear annealed glass and a Swiggle spacer system. The lites were interior glazed onto double-sided adhesive foam tape and secured with PVC snap-in glazing beads.

Frame Construction: The frame was constructed of thermally broken extruded aluminum. The corners were secured utilizing three #8 x 1" screws per corner through the jambs into the head and sill screw bosses. End caps were utilized on the ends of the fixed meeting rails and secured with two #8 x 3/4" screws per cap. The meeting rails were then secured to the frame with two #8 x 3/4" screws.

Sash Construction: The sash was constructed of thermally broken extruded aluminum. The corners were secured utilizing one #8 x 1" screw per corner through the head and sill into the jambs screw boss.

Screen Construction: The screen was constructed from roll-formed aluminum with keyed corners. The fiberglass mesh was secured with a flexible vinyl spline.

Hardware:

Description	Quantity	Location
Cam lock	1	One midspan of active panel with integral lock keeper on fixed meeting stile
Roller assembly	2	One each end of bottom rail
Screen constant force spring	2	5" from rails on screen stiles
Screen lift handles	2	5" from rails on screen stiles

Drainage:

Description	Quantity	Location
1-1/4" long by 1/4" wide weepslot with cover	2	3-1/2" from jambs on sill face
1/2" long by 1/8" wide weepslot	2	2" from jambs on sill track

Reinforcement: No reinforcement was utilized.

Installation: The window was installed into a #2 Spruce-Pine-Fir wood buck. The window was secured utilizing #8 x 1-5/8" drywall screws located in corners and 12" on center around nail-fin perimeter. Silicone was utilized around the exterior perimeter.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
Test Specimen #1: HS-C30 71 x 71			
2.2.2.5.1	Operating Force	11 lbf	25 lbf max.
2.1.2	Air Infiltration per ASTM E 283 1.57 psf (25 mph)	0.11 cfm/ft^2	0.3 cfm/ft^2 max.

***Note #1**: The tested specimen meets the performance levels specified in ANSI/AAMA/NWWDA 101/I.S. 2-97 for air infiltration.*

2.1.3	Water Resistance per ASTM E 547-00 (with and without screen) 4.50 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection per ASTM E 330 (Deflections reported were taken on the meeting stile) (Loads were held for 52 seconds)		
	30.0 psf (positive)	0.75"	See Note #2
	30.0 psf (negative)	0.71"	See Note #2

***Note #2**: The Uniform Load Deflection test is not requirement of ANSI/AAMA/NWWDA 101/I.S.2-97 for this product designation. The deflection data is recorded in this report for special code compliance and information only.*

2.1.4.2	Uniform Load Structural per ASTM E 330 (Permanent sets reported were taken on the meeting stile) (Loads were held for 10 seconds)		
	45.0 psf (positive)	0.13"	0.26" max.
	45.0 psf (negative)	<0.01"	0.26" max.
2.2.2.5.2	Deglazing Test per ASTM E 987 In operating direction - 70 lbs		
	Handle stile	0.13"/25%	0.50"/100%
	Lock stile	0.19"/38%	0.50"/100%
	In remaining direction - 50 lbs		
	Top rail	0.09"/19%	0.50"/100%
	Bottom rail	0.06"/13%	0.50"/100%

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
Test Specimen #1: HS-C30 71 x 71 (Continued)			
2.1.8	Forced Entry Resistance per ASTM F 588		
Type: A	Grade: 10		
	Lock Manipulation Test	No entry	No entry
	Test A1 thru A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry
Optional Performance			
4.3	Water Resistance per ASTM E 547-00 (with and without screen) 5.3 psf	No leakage	No leakage
Test Specimen #2: HS-C40 71 x 59			
2.2.2.5.1	Operating Force	14 lbf	25 lbf max.
2.1.2	Air Infiltration per ASTM E 283 1.57 psf (25 mph)	0.09 cfm/ft^2	0.3 cfm/ft^2 max.

***Note #1**: The tested specimen meets the performance levels specified in ANSI/AAMA/NWWDA 101/I.S. 2-97 for air infiltration.*

2.1.3	Water Resistance per ASTM E 547-00 (with and without screen) 4.50 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection per ASTM E 330 (Deflections reported were taken on the meeting stile) (Loads were held for 52 seconds)		
	30.0 psf (positive)	0.62"	See Note #2
	30.0 psf (negative)	0.51"	See Note #2
2.1.4.2	Uniform Load Structural per ASTM E 330 (Permanent sets reported were taken on the meeting stile) (Loads were held for 10 seconds)		
	45.0 psf (positive)	0.03"	0.21" max.
	45.0 psf (negative)	0.04"	0.21" max.

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
Test Specimen #2: HS-C40 71 x 59 (Continued)			
2.2.2.5.2	Deglazing Test per ASTM E 987 In operating direction - 70 lbs		
	Handle stile	0.13"/25%	0.50"/100%
	Lock stile	0.13"/25%	0.50"/100%
	In remaining direction - 50 lbs		
	Top rail	0.03"/6%	0.50"/100%
	Bottom rail	0.03"/6%	0.50"/100%
2.1.8	Forced Entry Resistance per ASTM F 588		
	Type: A	Grade: 10	
	Lock Manipulation Test	No entry	No entry
	Test A1 thru A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry
Optional Performance			
4.3	Water Resistance per ASTM E 547-00 (with and without screen) 6.0 psf	No leakage	No leakage
4.4.1	Uniform Load Deflection per ASTM E 330 (Deflections reported were taken on the meeting stile) (Loads were held for 52 seconds)		
	45.0 psf (positive)	0.62"	See Note #2
	47.2 psf (negative)	0.54"	See Note #2
4.4.2	Uniform Load Structural per ASTM E 330 (Permanent sets reported were taken on the meeting stile) (Loads were held for 10 seconds)		
	67.5 psf (positive)	0.04"	0.21" max.
	70.8 psf (negative)	0.08"	0.21" max.

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years from the original test date. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator. This report may not be reproduced except in full without approval of Architectural Testing.

For ARCHITECTURAL TESTING, INC

Digitally Signed by: Eric Westphal

Eric Westphal
Technician

Digitally Signed by: Steven M. Urich

Steven M. Urich, P. E.
Senior Project Engineer

EW:dme
01-47320.03

APRIL 20, 2004

Architectural Testing

ANSI/AAMA/NWWDA 101/I.S.2-97
TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 480/680/880 Drop-in
PRODUCT TYPE: Aluminum Horizontal Sliding Window (XO-Fin)

	Results	
Title	**Test Specimen #1**	**Test Specimen #2**
Rating	HS-C30 71 x 71	HS-C40 71 x 59
Operating Force	11 lbf max.	14 lbf max.
Air Infiltration	0.11 cfm/ft^2	0.09 cfm/ft^2
Water Resistance Test Pressure	5.3 psf	6.0 psf
Uniform Load Deflection Test Pressure	± 30.0 psf	+ 45.0 psf -47.2 psf
Uniform Structural Load Test Pressure	± 45.0 psf	+ 67.5 psf -70.8 psf
Forced Entry Resistance	Grade 10	Grade 10

Reference should be made to ATI Report Identification No. 01-47320.03 for complete test specimen description and data

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.archtest.com

Architectural Testing

ANSI/AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.
P.O. Box 370
650 West Market Street
Gratz, Pennsylvania 17030-0370

ATI Report Identification No.: 01-47320.03
Test Dates: 10/07/03
Through: 10/08/03
And: 12/01/03
And: 12/15/03
And: 03/17/04
Report Date: 04/16/04
Expiration Date: 10/07/07

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to witness testing on two Series/Model 480/680/880 Drop-in, aluminum horizontal sliding windows at MI Home Products, Inc. test facility in Elizabethville, Pennsylvania. The samples tested successfully met the performance requirements for the following ratings: Test Specimen #1: HS-C30 71 x 71; Test Specimen #2: HS-C40 71 x 59. Test specimen description and results are reported herein.

Test Specification: The test specimens were evaluated in accordance with ANSI/AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 480/680/880 Drop-in

Product Type: Aluminum Horizontal Sliding Window (XO Fin)

Test Specimen #1: HS-C30 71 x 71

Overall Size: 5' 11-7/16" wide by 5' 11" high

Active Sash Size: 2' 11-5/8" wide by 5' 8-3/8" high

Fixed Daylight Opening Size: 2' 8-3/16" wide by 5' 5-5/8" high

Screen Size: 2' 10" wide by 5' 6-1/2" high

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.archtest.com

Test Specimen Description: (Continued)

Weatherstripping:

Description	Quantity	Location
0.250" high by 0.187" backed polypile with center fin	1 Row	Active sash top and bottom rails and fixed meeting rail interlock
0.250" high by 0.187" backed polypile with center fin	2 Rows	Jamb stile

Test Specimen #2: HS-C40 71 x 59

Overall Size: 5' 11-3/8" wide by 4' 11-1/8" high

Active Sash Size: 2' 11-5/8" wide by 4' 8-1/4" high

Fixed Daylight Opening Size: 2' 8-1/4" wide by 4' 5-7/8" high

Screen Size: 2' 10-1/4" wide by 4' 7-1/8" high

Weatherstripping:

Description	Quantity	Location
0.310" high by 0.187" backed polypile with center fin	1 Row	Active sash top and bottom rails
0.250" high by 0.187" backed polypile with center fin	1 Rows	Fixed meeting rail interlock
0.310" high by 0.187" backed polypile with center fin	2 Rows	Jamb stile
0.550" high by 1" by 1" backed polypile pad	1 Pad	Corner of bottom rail and locking stile

Test Specimen Description: (Continued)

The following descriptions apply to all specimens.

Finish: All aluminum was white.

Glazing Details: The window utilized 5/8" thick sealed insulating glass constructed from two sheets of 1/8" thick clear annealed glass and a Swiggle spacer system. The lites were interior glazed onto double-sided adhesive foam tape and secured with PVC snap-in glazing beads.

Frame Construction: The frame was constructed of thermally broken extruded aluminum. The corners were secured utilizing three #8 x 1" screws per corner through the jambs into the head and sill screw bosses. End caps were utilized on the ends of the fixed meeting rails and secured with two #8 x 3/4" screws per cap. The meeting rails were then secured to the frame with two #8 x 3/4" screws.

Sash Construction: The sash was constructed of thermally broken extruded aluminum. The corners were secured utilizing one #8 x 1" screw per corner through the head and sill into the jambs screw boss.

Screen Construction: The screen was constructed from roll-formed aluminum with keyed corners. The fiberglass mesh was secured with a flexible vinyl spline.

Hardware:

Description	Quantity	Location
Cam lock	1	One midspan of active panel with integral lock keeper on fixed meeting stile
Roller assembly	2	One each end of bottom rail
Screen constant force spring	2	5" from rails on screen stiles
Screen lift handles	2	5" from rails on screen stiles

Drainage:

Description	Quantity	Location
1-1/4" long by 1/4" wide weepslot with cover	2	3-1/2" from jambs on sill face
1/2" long by 1/8" wide weepslot	2	2" from jambs on sill track

Reinforcement: No reinforcement was utilized.

Installation: The window was installed into a #2 Spruce-Pine-Fir wood buck. The window was secured utilizing #8 x 1-5/8" drywall screws located in corners and 12" on center around nail-fin perimeter. Silicone was utilized around the exterior perimeter.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
Test Specimen #1: HS-C30 71 x 71			
2.2.2.5.1	Operating Force	11 lbf	25 lbf max.
2.1.2	Air Infiltration per ASTM E 283 1.57 psf (25 mph)	0.11 cfm/ft^2	0.3 cfm/ft^2 max.

Note #1: *The tested specimen meets the performance levels specified in ANSI/AAMA/NWWDA 101/I.S. 2-97 for air infiltration.*

2.1.3	Water Resistance per ASTM E 547-00 (with and without screen) 4.50 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection per ASTM E 330 (Deflections reported were taken on the meeting stile) (Loads were held for 52 seconds)		
	30.0 psf (positive)	0.75"	See Note #2
	30.0 psf (negative)	0.71"	See Note #2

Note #2: *The Uniform Load Deflection test is not requirement of ANSI/AAMA/NWWDA 101/I.S.2-97 for this product designation. The deflection data is recorded in this report for special code compliance and information only.*

2.1.4.2	Uniform Load Structural per ASTM E 330 (Permanent sets reported were taken on the meeting stile) (Loads were held for 10 seconds)		
	45.0 psf (positive)	0.13"	0.26" max.
	45.0 psf (negative)	<0.01"	0.26" max.
2.2.2.5.2	Deglazing Test per ASTM E 987 In operating direction - 70 lbs		
	Handle stile	0.13"/25%	0.50"/100%
	Lock stile	0.19"/38%	0.50"/100%
	In remaining direction - 50 lbs		
	Top rail	0.09"/19%	0.50"/100%
	Bottom rail	0.06"/13%	0.50"/100%

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
Test Specimen #1: HS-C30 71 x 71 (Continued)			
2.1.8	Forced Entry Resistance per ASTM F 588		
Type: A	Grade: 10		
	Lock Manipulation Test	No entry	No entry
	Test A1 thru A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry
Optional Performance			
4.3	Water Resistance per ASTM E 547-00 (with and without screen) 5.3 psf	No leakage	No leakage
Test Specimen #2: HS-C40 71 x 59			
2.2.2.5.1	Operating Force	14 lbf	25 lbf max.
2.1.2	Air Infiltration per ASTM E 283 1.57 psf (25 mph)	0.09 cfm/ft^2	0.3 cfm/ft^2 max.

***Note #1**: The tested specimen meets the performance levels specified in ANSI/AAMA/NWWDA 101/I.S. 2-97 for air infiltration.*

2.1.3	Water Resistance per ASTM E 547-00 (with and without screen) 4.50 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection per ASTM E 330 (Deflections reported were taken on the meeting stile) (Loads were held for 52 seconds)		
	30.0 psf (positive)	0.62"	See Note #2
	30.0 psf (negative)	0.51"	See Note #2
2.1.4.2	Uniform Load Structural per ASTM E 330 (Permanent sets reported were taken on the meeting stile) (Loads were held for 10 seconds)		
	45.0 psf (positive)	0.03"	0.21" max.
	45.0 psf (negative)	0.04"	0.21" max.

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
Test Specimen #2: HS-C40 71 x 59 (Continued)			
2.2.2.5.2	Deglazing Test per ASTM E 987 In operating direction - 70 lbs		
	Handle stile	0.13"/25%	0.50"/100%
	Lock stile	0.13"/25%	0.50"/100%
	In remaining direction - 50 lbs		
	Top rail	0.03"/6%	0.50"/100%
	Bottom rail	0.03"/6%	0.50"/100%
2.1.8	Forced Entry Resistance per ASTM F 588		
	Type: A	Grade: 10	
	Lock Manipulation Test	No entry	No entry
	Test A1 thru A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry
Optional Performance			
4.3	Water Resistance per ASTM E 547-00 (with and without screen)		
	6.0 psf	No leakage	No leakage
4.4.1	Uniform Load Deflection per ASTM E 330 (Deflections reported were taken on the meeting stile) (Loads were held for 52 seconds)		
	45.0 psf (positive)	0.62"	See Note #2
	47.2 psf (negative)	0.54"	See Note #2
4.4.2	Uniform Load Structural per ASTM E 330 (Permanent sets reported were taken on the meeting stile) (Loads were held for 10 seconds)		
	67.5 psf (positive)	0.04"	0.21" max.
	70.8 psf (negative)	0.08"	0.21" max.

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years from the original test date. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator. This report may not be reproduced except in full without approval of Architectural Testing.

For ARCHITECTURAL TESTING, INC

Digitally Signed by: Eric Westphal

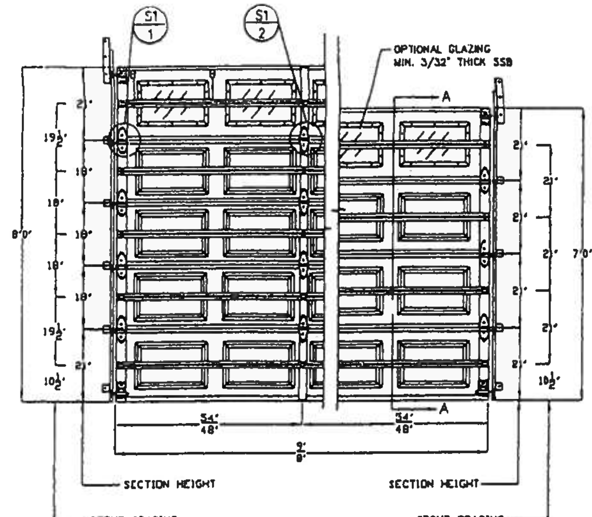
Eric Westphal
Technician

Digitally Signed by: Steven M. Urich

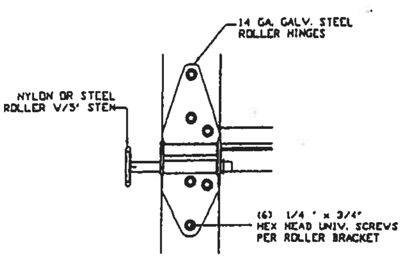
Steven M. Urich, P. E.
Senior Project Engineer

EW:dme
01-47320.03

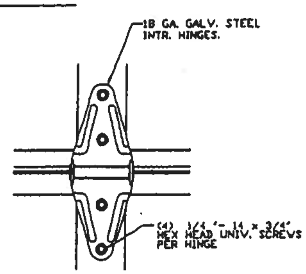
APRIL 20, 2004



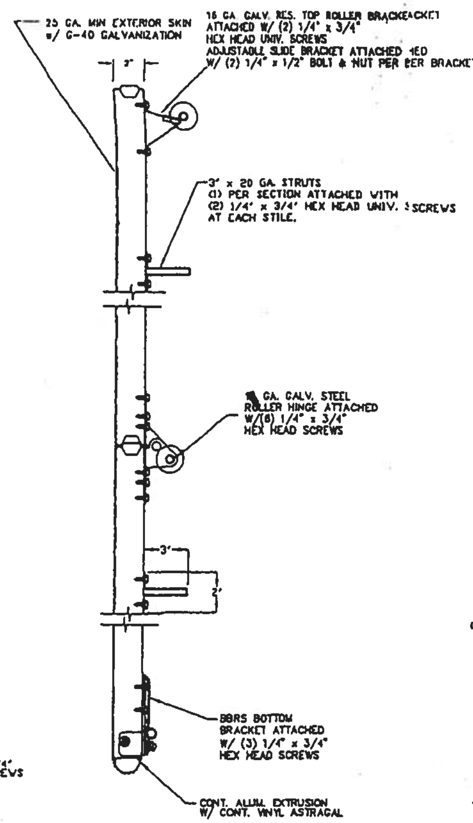
INSIDE ELEVATION



TYP. ROLLER BRACKET (S1 / 1)
N.T.S.



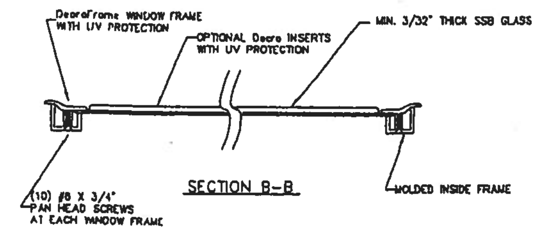
TYP. HINGE CONNECTION (S1 / 2)
N.T.S.



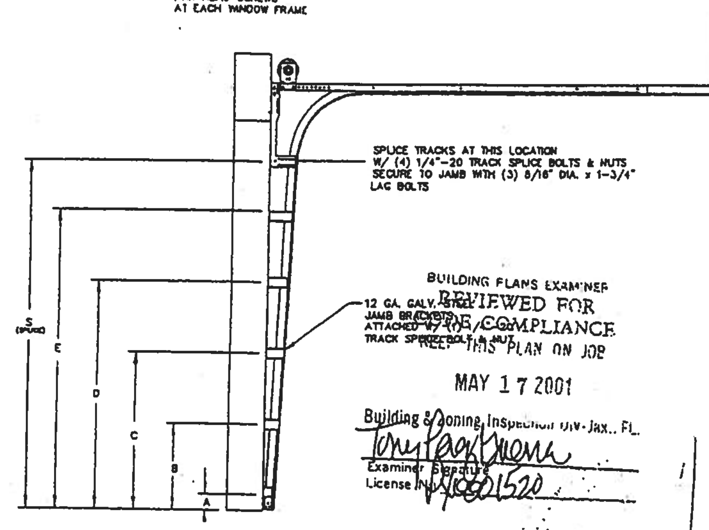
SECTION A-A (SIDE VIEW)

GLAZING OPTION CROSS SECTION

TEST No. SBC-580-011 ON OCTOBER 12, 1995 INCLUDED GLASS WINDOWS IN THE DOOR BEING USED. THE TEST PRESSURES WERE +48.4 PSF AND -54.7 PSF. BY COMPARISION, FOUR (4) WINDOWS MAY BE INSTALLED IN (1) ONE SECTION OF THE 9' X 7' AND 9' X 8' MODEL 600 AND 850 DOORS.



SECTION B-B



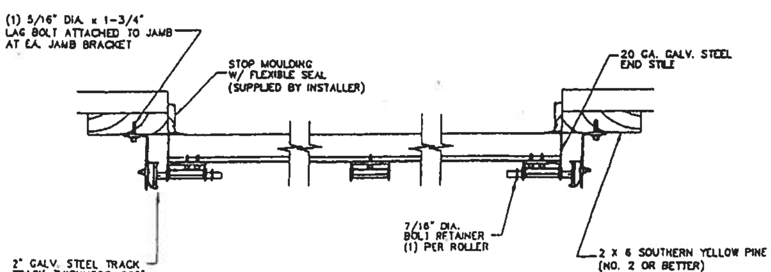
TRACK CONFIGURATION FOR 6'6" UP TO 8' TALL DOORS

JAMB BRACKET LOCATIONS

	A	B	C	D	E	S
6'-6'	4'	21-1/2'	39'	57'		70'
7'-0'	4'	21-1/2'	42'	63'		76'
7'-6'	4'	18'	36'	54'	72'	82'
8'-0'	4'	21-1/2'	39'	57'	75'	88'

SPECIFICATIONS AND NOTES

1. DOORS AND HARDWARE WILL BE DESIGNED, MANUFACTURED AND INSTALLED WITH STANDARDS AS SET FORTH BY DASMA.
2. DOOR SECTIONS SHALL BE 25 GA. (.018") MIN. ROLL FORMED LIGHT COMMERCIAL QUALITY. G-40 GALVANIZATION.
3. DOORS UPTO 7'0" HIGH CONSIST OF (4) SECTIONS AS SHOWN.
4. DOORS UPTO 8'0" HIGH CONSIST OF (5) SECTIONS AS SHOWN.
5. SUPPORTING STRUCTURAL ELEMENTS SHALL BE DESIGNED BY A REGISTRED PROFESSIONAL ENGINEER FOR WIND LOADS INDICATED ON THIS DRAWING IN ADDITION TO OTHER LOADINGS.
6. THE METHOD OF TESTING WAS IN SUBSTANTIAL CONFORMANCE WITH THE PROCEDURE DESCRIBED IN ASTM E330-90, AND THE SOUTHERN BUILDING CODE SECTION 1606 WIND LOAD DESIGN CRITERIA THE PRESSURES SHOWN ON THE DRAWINGS WERE CALCULATED USING THE FOLLOWING PARAMETERS:
   - A. BASIC WIND SPEED OF 110 MPH
   - B. DOOR CAN BE INSTALLED WITH 6 FEET OF DOORS WIDTH INSIDE THE EDGE STRIP.
   - C. 15' MEAN ROOF HEIGHT AT ANY SLOPE
   - D. USE FACTOR OF 1.0
   - E. EXPOSURE RATING OF C



TRACK MOUNTING DETAIL

WOOD JAMB ATTACHMENT TO STRUCTURE

RATED FOR 110 MPH FASTEST-MILE BASIC WIND SPEEDS

VERTICAL JAMB ATTACHMENT TO WOOD FRAME STRUCTURE
5/16" X 3" LAG SCREWS STARTING 8" FROM ENDS THEN 24" O.C.

VERTICAL JAMB ATTACHMENT TO 2,000 PSI CONCRETE
HILTI KWIK BOLT 3/8" X 4" STARTING 6" FROM ENDS THEN 24" O.C.
HILTI SLEEVE ANCHOR 3/8" X 2-3/4" STARTING 6" FROM ENDS THEN 24" O.C.
ITW/RAMSET RED HEAD 3/8" X 3" STARTING 6" FROM ENDS THEN 24" O.C.

VERTICAL JAMB ATTACHMENT TO C-90 BLOCK
HILTI SLEEVE ANCHOR 3/8" X 2-3/4" STARTING 8" FROM ENDS THEN 24" O.C.
ITW/RAMSET TAPCON 1/4" X 2-3/4" STARTING 6" FROM ENDS. USE PAIRS OF FASTENERS (3" APART) AT 24" O.C.

*LAGS AND BOLTS CAN BE COUNTERSUNK TO PROVIDE A FLUSH MOUNTING SURFACE.
*PREPARATION OF WOOD JAMBS BY OTHERS

4/19/01

MAX SIZE 8' x 8'	
DESIGN LOADS +28.7 PSF -31.3 PSF	
TEST LOADS +43.0 PSF -48.0 PSF	

Amarr

MODEL #600 STRATFORD
MODEL #950 HERITAGE III

SIZE B | DRAWING NUMBER SBC-580-007-J | SHEET 1 OF 1

TAMKO®
ROOFING PRODUCTS

January 31, 2002

TO: OUR FLORIDA CUSTOMERS:

Effective February 1, 2002, the following TAMKO shingles, as manufactured at TAMK[illegible]s Tuscaloosa, Alabama, facility, comply with ASTM D-3161, Type I modified to 110 mph. [illegible]esting was conducted using four nails per shingle. These shingles also comply with Florida Buil[illegible]ng Code TAS 100 for wind driven rain.

- Glass-Seal AR
- Elite Glass-Seal AR
- ASTM Heritage 30 AR (formerly ASTM Heritage 25 AR)
- Heritage 40 AR (formerly Heritage 30 AR)
- Heritage 50 AR (formerly Heritage 40 AR)

All testing was performed by Florida State certified independent labs.

Please direct all questions to TAMKO's Technical Services Department at 1-800-641-46[illegible]

TAMKO Roofing Products, Inc.

2300 35th STREET P.O. BOX 2149 TUSCALOOSA, AL 35403-2149 205-752-3555 FAX 205-349-2049

MIAMI-DADE
BUILDING CODE COMPLIANCE OFFICE (BCCO)
PRODUCT CONTROL DIVISION

MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901 FAX (305) 375-2908

NOTICE OF ACCEPTANCE (NOA)

Ceco Door Products
9159 Telecom Drive
Milan, TN 38358

outswing

SCOPE:
This NOA is being issued under the applicable rules and regulations governing the use of construction materials. The documentation submitted has been reviewed by Miami-Dade County Product Control Division and accepted by the Board of Rules and Appeals (BORA) to be used in Miami Dade County and other areas where allowed by the Authority Having Jurisdiction (AHJ).

This NOA shall not be valid after the expiration date stated below. The Miami-Dade County Product Control Division (In Miami Dade County) and/or the AHJ (in areas other than Miami Dade County) reserve the right to have this product or material tested for quality assurance purposes. If this product or material fails to perform in the accepted manner, the manufacturer will incur the expense of such testing and the AHJ may immediately revoke, modify, or suspend the use of such product or material within their jurisdiction. BORA reserves the right to revoke this acceptance, if it is determined by Miami-Dade County Product Control Division that this product or material fails to meet the requirements of the applicable building code.

This product is approved as described herein, and has been designed to comply with the High Velocity Hurricane Zone of the Florida Building Code.

DESCRIPTION: Series "Regent" & "Omega" 18 ga. 3^0-7^0 Outswing Commercial Steel Door

APPROVAL DOCUMENT: Drawing No. **RD0087**, titled "3-0 x 7-0 Series", sheets 1 through 7 of 7, dated 5/30/97 with revision C dated 2/24/00,prepared by the manufacturer, bearing the Miami-Dade County Product Control Renewal stamp with the Notice of Acceptance number and expiration date by the Miami-Dade County Product Control Division.

MISSILE IMPACT RATING: Large and Small Missile Impact

LABELING: Each unit shall bear a permanent label with the manufacturer's name or logo, city, state and following statement: "Miami-Dade County Product Control Approved", unless otherwise noted herein.

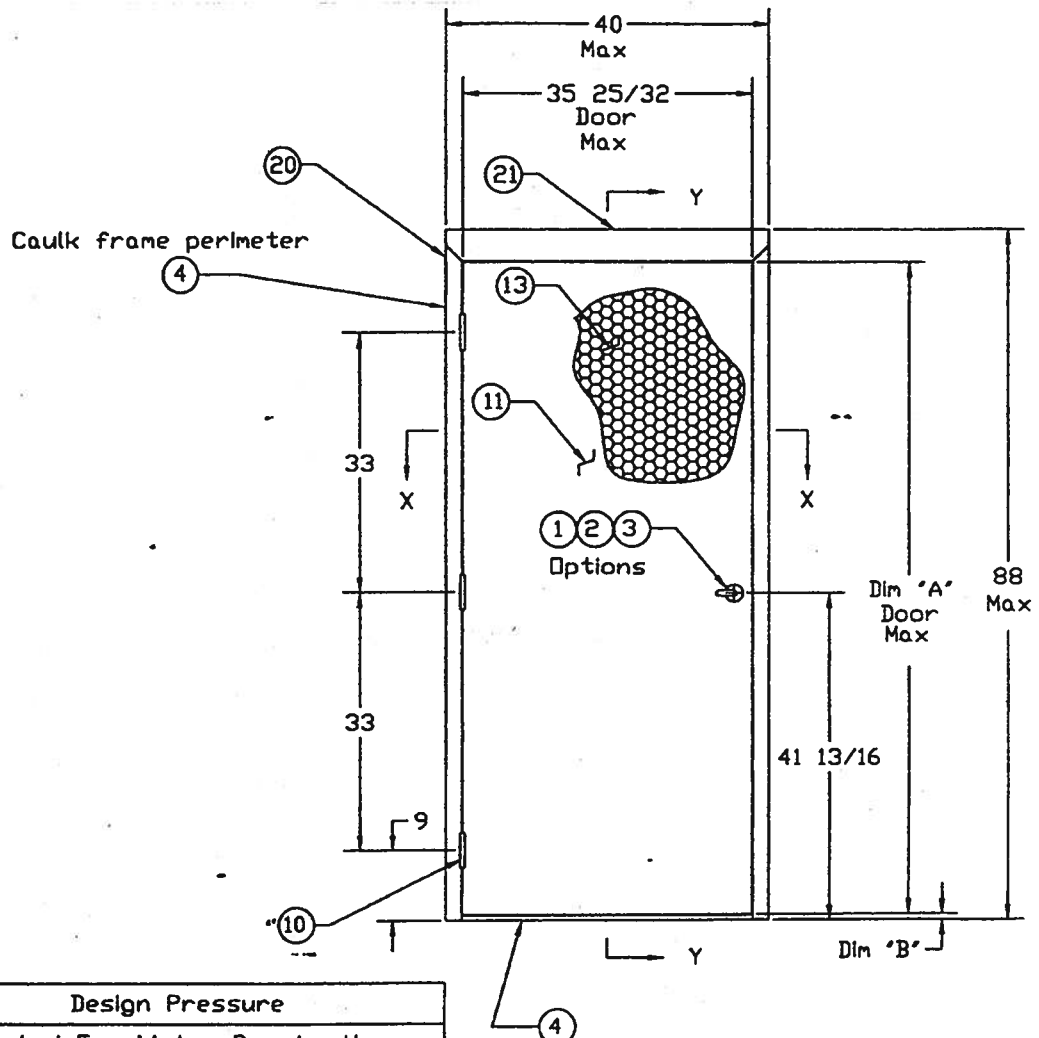
RENEWAL of this NOA shall be considered after a renewal application has been filed and there has been no change in the applicable building code negatively affecting the performance of this product.

TERMINATION of this NOA will occur after the expiration date or if there has been a revision or change in the materials, use, and/or manufacture of the product or process. Misuse of this NOA as an endorsement of any product, for sales, advertising or any other purposes shall automatically terminate this NOA. Failure to comply with any section of this NOA shall be cause for termination and removal of NOA.

ADVERTISEMENT: The NOA number preceded by the words Miami-Dade County, Florida, and followed by the expiration date may be displayed in advertising literature. If any portion of the NOA is displayed, then it shall be done in its entirety.

INSPECTION: A copy of this entire NOA shall be provided to the user by the manufacturer or its distributors and shall be available for inspection at the job site at the request of the Building Official.

This NOA **renews NOA # 00-0315.03** and consists of this page 1 as well as approval document mentioned above. The submitted documentation was reviewed by **Manuel Perez, P.E.**



Sheet 2	Frame Anchor Installation
Sheet 3	Threshold Installation
Sheet 3	Weatherstrip Installation
Sheet 4	Door Latch Reinforcement
Sheet 5-6	Cross Section View
Sheet 7	Bill Of Material

Design Pressure		
Tested For Water Penetration		
With Overhang	+85 psf	-60 psf
Without Overhang	+60 psf	-60 psf

	Dim 'A'	Dim 'B'
3/4" Undercut	83 1/8	3/4
3/8" Undercut	83 1/2	3/8

APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE June 09, 2000
BY
PRODUCT CONTROL DIV.S ON BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 00-0315.03

PRODUCT RENEWED as complying with the Florida Building Code
Acceptance No. 03-0411.01
Expiration Date Nov 14 2008
By
Miami-Dade Product Control Division

MATERIAL SPECIFICATIONS:

Finish: Rust Inhibitive Primer

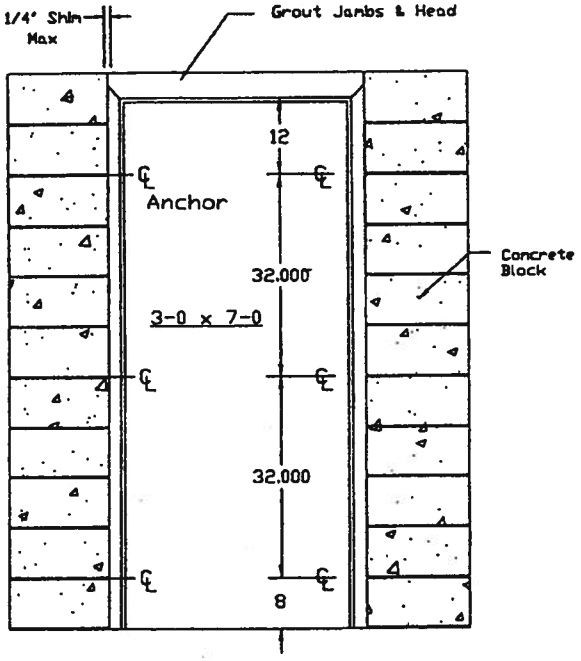
3-0 x 7-0 Series Elevation Drawing

CECO DOOR PRODUCTS
Milan, Tennessee 38358

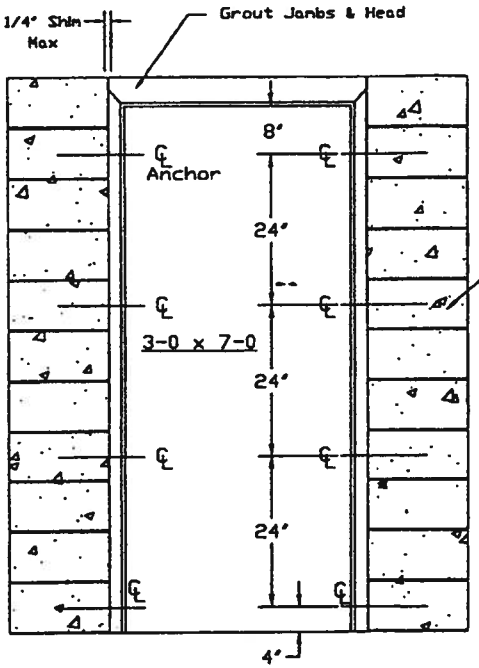
ISSUE	REVISIONS
C 2/24/00 JAR	Revised Format, Transfered Information from NOA
B 7/22/97 GWS	Deleted CD Drawings Revised Sheet Numbers

DRAWN BY: GWS
DATE: 5/30/97

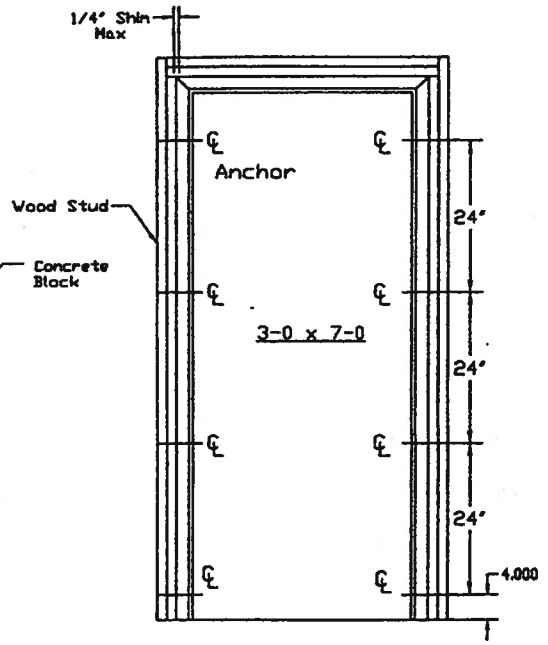
DRAWING NUMBER: RD0087
Sheet 1 Of 7



Masonry "T" Anchor

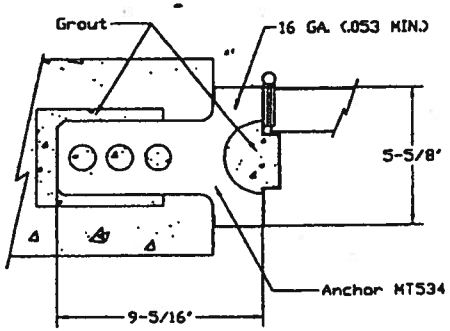


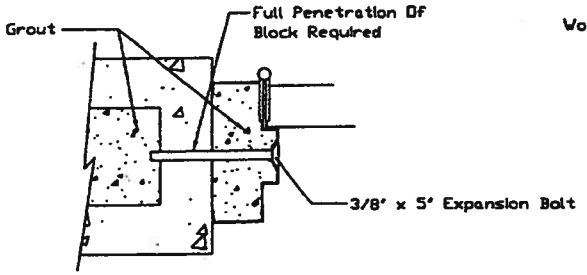
Existing Opening Anchor Into Block

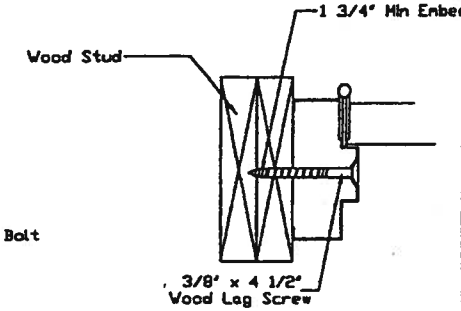


Existing Opening Anchor Into Wood Stud

PRODUCT RENEWED
as complying with the Florida Building Code
Acceptance No. 03-0411.01
Expiration Date Aug. 14, 2008
By Manuel Perez
Miami-Dade Product Control Division







APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE June 08, 2000
BY Manuel Perez
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 00-0315.03

NOTES:
1. SEE SHEET 7 FOR BILL OF MATERIALS

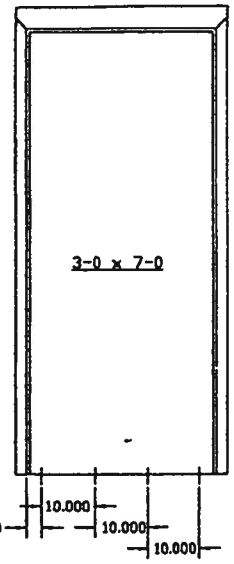
MATERIAL SPECIFICATIONS:

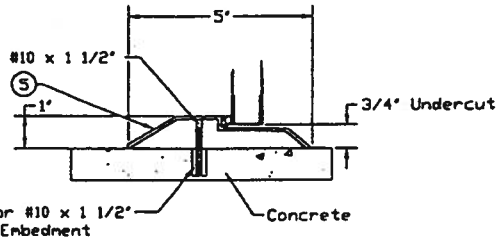
Frame Anchor Installation Details

CECO DOOR PRODUCTS
Milan, Tennessee 38358

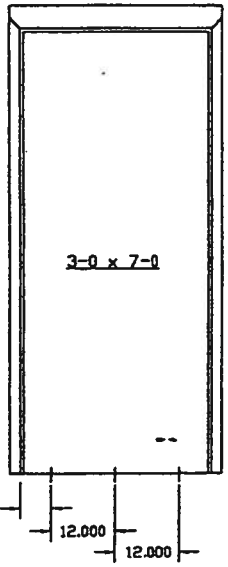
ISSUE	REVISIONS
B 2/24/00 JAR	Revised Format, Transfered Information from NOA
A 7/22/97 GWS	Revised Sheet Number
DRAWN BY: GWS	DATE: 5/30/97

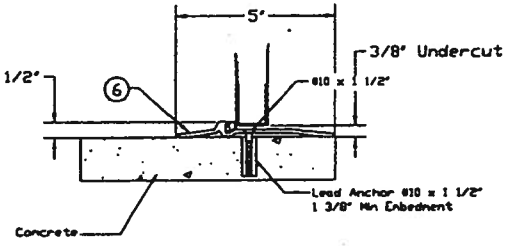
DRAWING NUMBER:
RD0087
Sheet 2 of 7



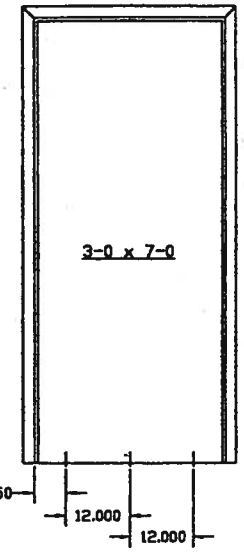


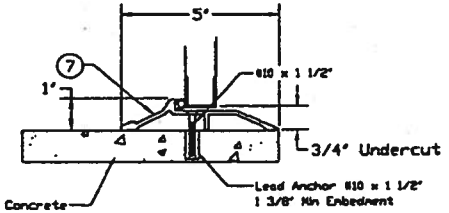
Threshold: National Guard 803S





Threshold: Penko 2005AV

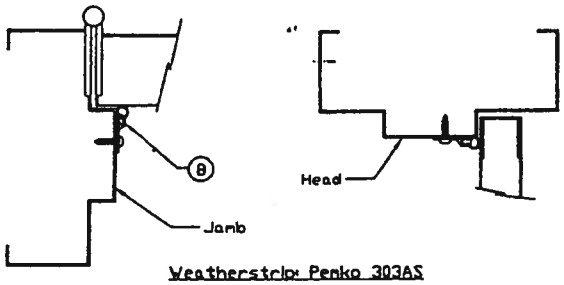




Threshold: Penko 181AV

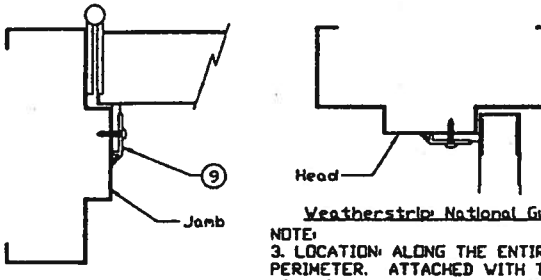
PRODUCT RENEWED
as complying with the Florida
Building Code
Acceptance No. 03-0411.01
Expiration Date Aug. 14, 2008
By
Miami-Dade Product Control
Division

NOTE: 1. All thresholds shown are made from extruded aluminum with slide-in vinyl weatherstrip insert.



Weatherstrip: Penko 303AS

NOTE:
2. LOCATION: ALONG THE ENTIRE HEAD AND JAMB PERIMETER. ATTACHED WITH THIRTY FOUR (34) #8 X 3/4" PPH SMS SPACED AT 6" O/C.



Weatherstrip: National Guard 130NA

NOTE:
3. LOCATION: ALONG THE ENTIRE HEAD AND JAMB PERIMETER. ATTACHED WITH THIRTY FOUR (34) #8 X 3/4" PPH SMS SPACED AT 6" O/C.

NOTE: 4. See Sheet 7 For Bill of Material

APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE June 08 2000
BY
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 00-0315.03

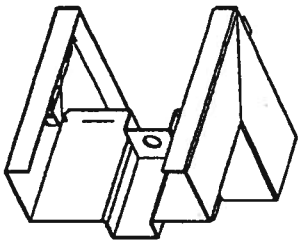
C 2/24/00 JAR	Revised Format, Transfered Information from NOA
B 7/22/97 GWS	Revised Sheet Number
ISSUE	REVISIONS
DRAWN BY: GWS	DATE: 5/30/97

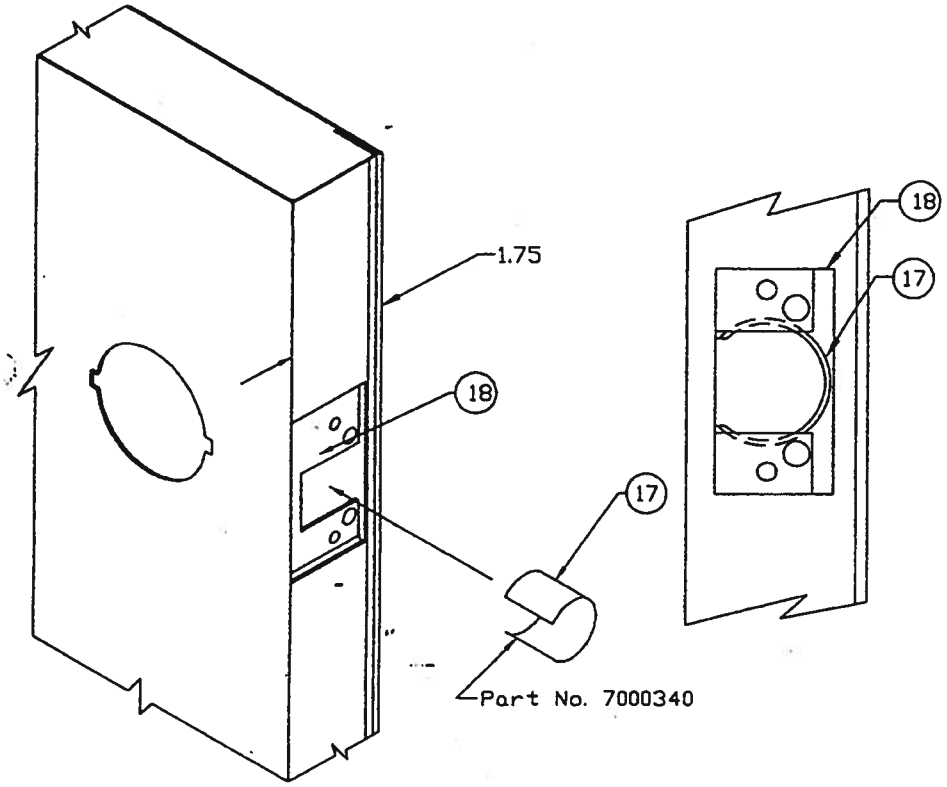
MATERIAL SPECIFICATIONS:

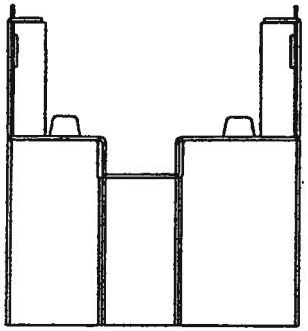
Threshold & Weatherstrip Installation details

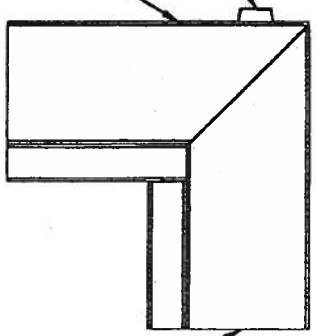
CECO DOOR PRODUCTS
Milan, Tennessee 38358

RD0087
Sheet 3 Of 7









PRODUCT RENEWED
as complying with the Florida Building Code
Acceptance No. 03-0411.01
Expiration Date Aug. 14, 2008
By: Manuel Perez
Miami Dade Product Control Division

APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE June 08 2000
BY Manuel Perez
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 00-0315.03

Note: 1. For Cylindrical Lock Only
2. See Sheet 7 For Bill Of Material

ISSUE	REVISIONS
B 2/24/00 JAR	Revised Format, Transfered Information from NOA
A 7/22/97 GWS	Revised Sheet Number

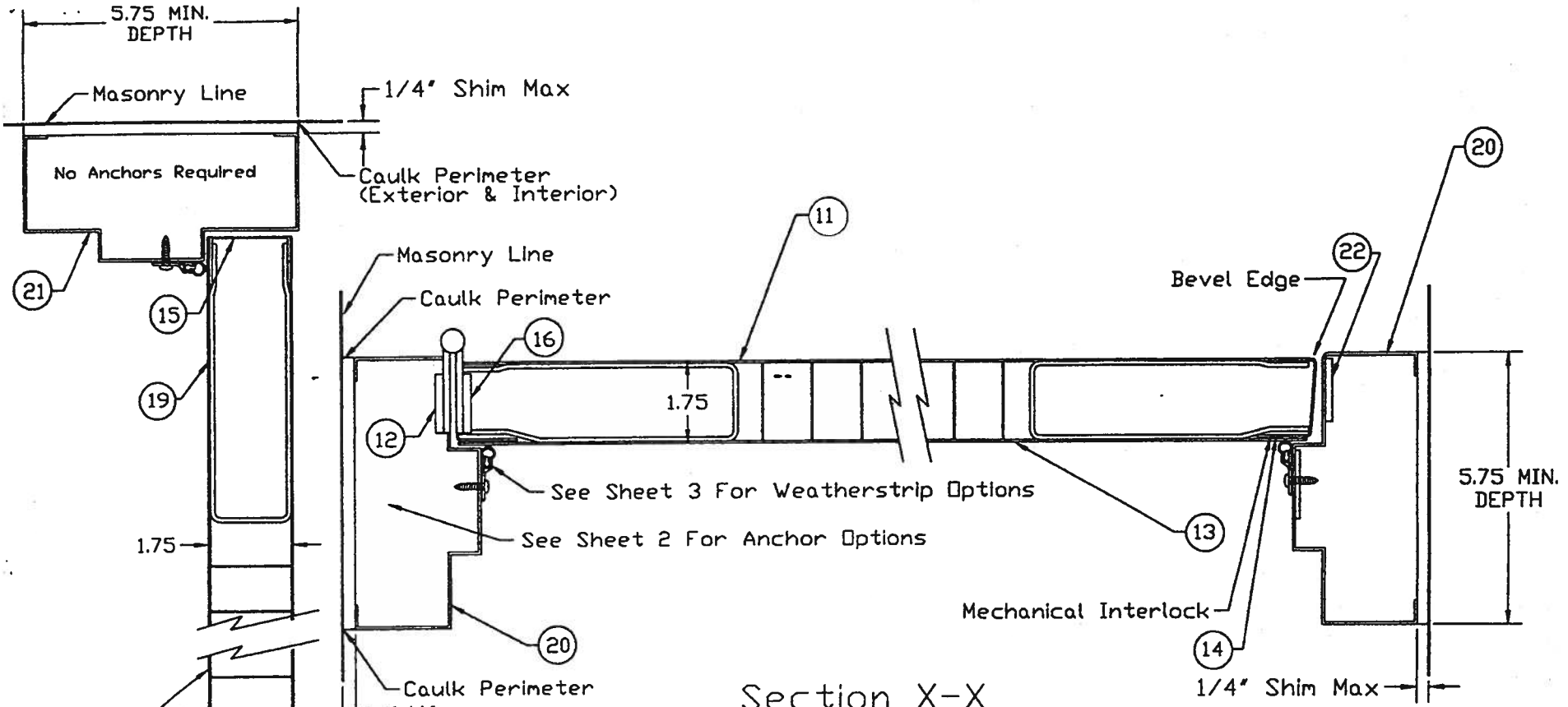
MATERIAL SPECIFICATIONS:

Cylindrical Lock Reinforcement and "SF" Seriers Frame Corner Installation Details

CECO DOOR PRODUCTS
Milan, Tennessee 38358

DRAWN BY: GWS
DATE: 6/06/97

DRAWING NUMBER:
RD0087
Sheet 4 of 7



Note: See Sheet 7 For Bill Of Material

PRODUCT RENEWED as complying with the Florida Building Code Acceptance No. 03-0411.01 Expiration Date Aug 14, 2008 By Miami Dade Product Control Division

APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE June 08 2000
BY Manuel Perez
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 00-0315.03

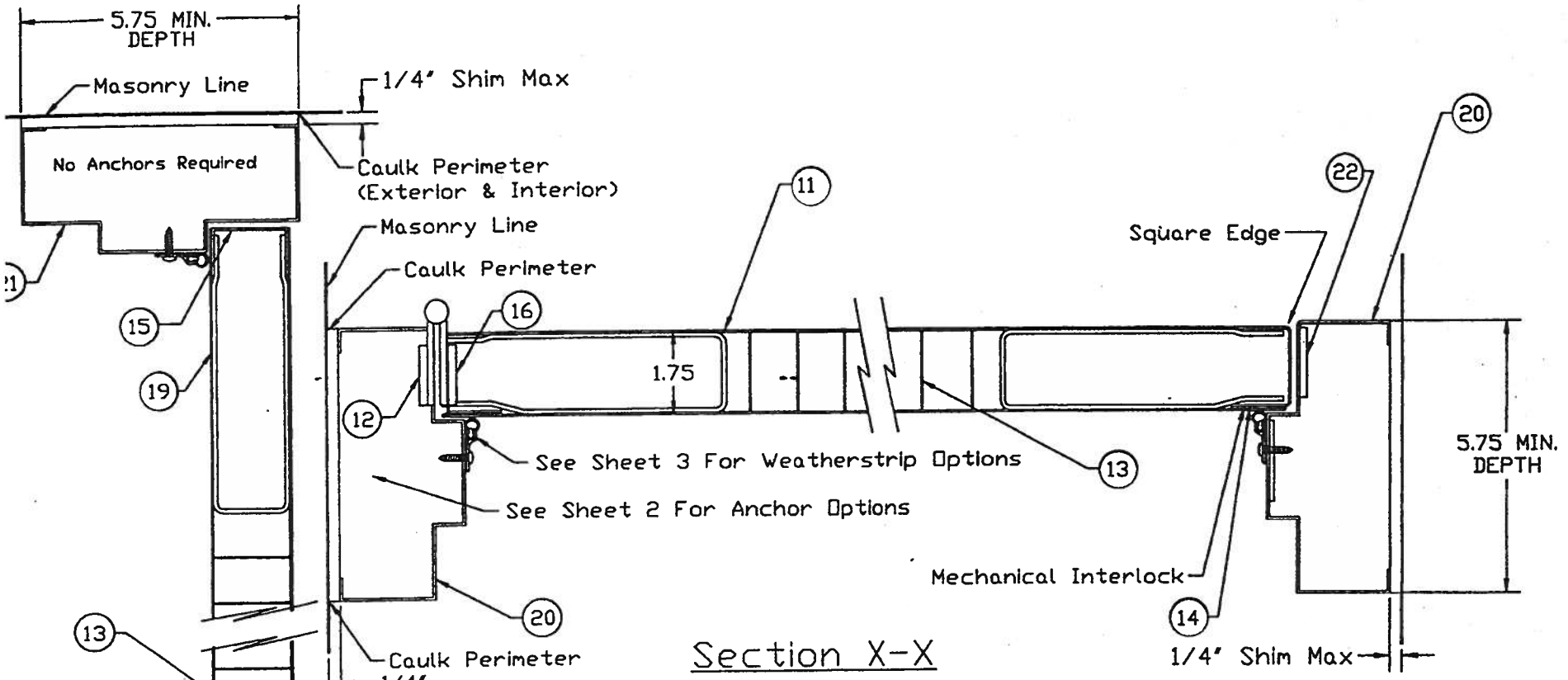
ISSUE	REVISIONS
C 2/24/00 JAR	Revised Format, Transfered Information from NOA
B 7/22/97 GWS	Revised Sheet Number

MATERIAL SPECIFICATIONS:

Cross Section View
Regent Door

CECO DOOR PRODUCTS
Milan, Tennessee 38358

DRAWN BY: GWS
DATE: 5/30/97
DRAWING NUMBER: RD0087
Sheet 5 of 7



Section X-X

Section Y-Y

Note: See Sheet 7 For Bill Of Material

PRODUCT RENEWED
as complying with the Florida Building Code
Acceptance No. 03-0411.01
Expiration Date AUG. 14, 2008
By Miami-Dade Product Control Division

APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE: June 06, 2000
BY PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 00-0315.03

ISSUE	REVISIONS
C 2/24/00 JAR	Revised Format, Transfered Information from NOA
B 7/22/97 GWS	Revised Sheet Number

MATERIAL SPECIFICATIONS:

Cross Section View
Omega Door

CECO DOOR PRODUCTS
Milan, Tennessee 38358

DRAWN BY: GWS
DATE: 5/30/97

DRAWING NUMBER: RD0087

Sheet 6 of 7

ITEM	QTY	DESCRIPTION	MATERIAL	SIZE
1	1	SCHLAGE SERIES AL53PD GRADE 2, LATCH LOCK, SINGLE LEVER OR KNOB OPERATED		
2	1	MARKS SERIES 170AB GRADE 2, LATCH LOCK, INSIDE/OUTSIDE LEVER OPERATED		
3	1	YALE SERIES AU53070 GRADE 2 LATCH LOCK, SINGLE LEVER OR KNOB OPERATED.		
4	1	CAULK FOR INSTALLATION AND WEATHERSTRIP ADAPTER SCREWS FRAME PERIMETER (INSIDE & OUT) AND FRAME SILL CORNERS	GE SILICONE HOUSEHOLD SEALANT	
5	1	NATIONAL GUARD #803S		
6	1	PEMKO #2005AV		
7	1	PEMKO #181AV		
8	1 ROW	PEMKO #303AS HIGH SURFACE APPLIED EXTRUDED ALUMINUM WEATHERSTRIP ADAPTER WITH A SILICON (TM) BULB INSERT		
9	1 ROW	NATIONAL GUARD #130NA 1-1/4" WIDE X 0.188" SURFACE APPLIED EXTRUDED ALUMINUM WEATHERSTRIP ADAPT. WITH A FOAM INSERT		
10	3	HAGAR BB1279, 4-1/2" X 4-1/2" X .0134" THICK STEEL HINGE EACH ATTACHED WITH EIGHT #12-24 X 1/2" FH MS		
11	1	FACE SHEET CONFORMING TO ASTM A366 AND ASTM-A568	COMMERCIAL QUALITY COLD ROLLED STEEL (MINIMUM YEILD STR. OF Fy=36,000 psD	18 GAUGE (.042" MIN. THICK)
12	3	HINGE REINFORCING PLATE, PLATE SPOT WELDED TO FRAME JAMB AT EACH HINGE LOCATION	STEEL	1-1/4" X 9" X 7 GA.
13	1	CORE: FULL HONEYCOMB CORE PERMANENTLY BONDED TO THE INSIDE OF EACH FACE SKIN WITH NON-FLAMMABLE ADHESIVE	PHENOLIC RESIN-IMPREGNATED KRAFT PAPER	1-1/8" CELL
14	1	DENFLEX 3500 STRUCTURAL ADHESIVE EPOXY		
15	1	ROLL FORMED STEEL CHANNEL ON THE TOP AND BOTTOM OF THE DOOR SPOT WELDED TO EXTERIOR AND GLUED TO INTERIOR SKIN		1" X 1-3/4" X 1" X 16 GA. (.053" MIN)
16	3	DOOR HINGE REINFORCEMENT		1-1/4" X 9" X 7 GA.
17	1	DOOR LATCH REINFORCEMENT, STEEL "C" RING	28 GA. GALV.	.015" THICK X 1.313 INSIDE DIAMETER
18	1	DOOR LOCK REINFORCEMENT	STEEL	16 GA.
19	1	DOOR CLOSER REINFORCEMENT, ROLLED FORM CHANNELS TACK WELDED TO DOOR END CHANNELS	STEEL	12 GA. (.093")
20	2	SERIES "SF", FRAME JAMB, DOUBLE RABBET PROFILE FACE SHEET CONFORMING TO ASTM A366 AND ASTM-A653	16 GA. (.053" MIN.) STEEL COMMERCIAL QUALITY COLD ROLLED STEEL (MINIMUM YEILD STR. OF Fy=40,000 psD	2" FACE, 5-3/4" DEPTH MIN.
21	1	SERIES "SF", FRAME HEAD, DOUBLE RABBET PROFILE FACE SHEET CONFORMING TO ASTM A366 AND ASTM-A653	16 GA. (.053" MIN.) STEEL COMMERCIAL QUALITY COLD ROLLED STEEL (MINIMUM YEILD STR. OF Fy=40,000 psD	2" FACE, 5-3/4" DEPTH MIN.
22	1	JAMB LOCK STRIKE REINFORCING PLATE	STEEL	1-1/8" X 2-1/2" X 12 GA.

PRODUCT RENEWED as complying with the Florida Building Code
Acceptance No. 03-0411.01
Expiration Date AUG 14 2008
By: Manuel Perez
Miami Dade Product Control Division

APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE June 08 2000
BY Manuel Perez
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 00-0315.03

ISSUE	REVISIONS
B 2/24/00 JAR	Revised Format, Transfered Information from NOA
A 7/22/97 GWS	Revised Sheet Number

MATERIAL SPECIFICATIONS:

3-0 x 7-0 Series Bill Of Materials

CECO DOOR PRODUCTS
Milan, Tennessee 38358

DRAWN BY: GWS
DATE: 6/02/97
DRAWING NUMBER: RD0087
Sheet 7 of 7

MIAMI-DADE

BUILDING CODE COMPLIANCE OFFICE (BCCO)
PRODUCT CONTROL DIVISION

MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901 FAX (305) 375-2908

NOTICE OF ACCEPTANCE (NOA)

Ceco Door Products
9159 Telecom Drive
Milan, TN 38358

In Swing

SCOPE:

This NOA is being issued under the applicable rules and regulations governing the use of construction materials. The documentation submitted has been reviewed by Miami-Dade County Product Control Division and accepted by the Board of Rules and Appeals (BORA) to be used in Miami Dade County and other areas where allowed by the Authority Having Jurisdiction (AHJ).

This NOA shall not be valid after the expiration date stated below. The Miami-Dade County Product Control Division (In Miami Dade County) and/or the AHJ (in areas other than Miami Dade County) reserve the right to have this product or material tested for quality assurance purposes. If this product or material fails to perform in the accepted manner, the manufacturer will incur the expense of such testing and the AHJ may immediately revoke, modify, or suspend the use of such product or material within their jurisdiction. BORA reserves the right to revoke this acceptance, if it is determined by Miami-Dade County Product Control Division that this product or material fails to meet the requirements of the applicable building code.

This product is approved as described herein, and has been designed to comply with the High Velocity Hurricane Zone of the Florida Building Code.

DESCRIPTION: The Ceco Series Single Flush / Embossed Inswing Commercial Steel Doors –Impact

APPROVAL DOCUMENT: Drawing No **RD0728**, titled "3-0 x 7-0 , Series Regent, Omega, Imperial, Versa door", prepared by manufacturer, sheets 1 through 9 of 9 dated 05/22/02 and latest revised on 10-10-02, bearing the Miami-Dade County Product Control Approval stamp with the Notice of Acceptance number and approval date by the Miami-Dade County Product Control Division.

MISSILE IMPACT RATING: Large and Small Missile Impact

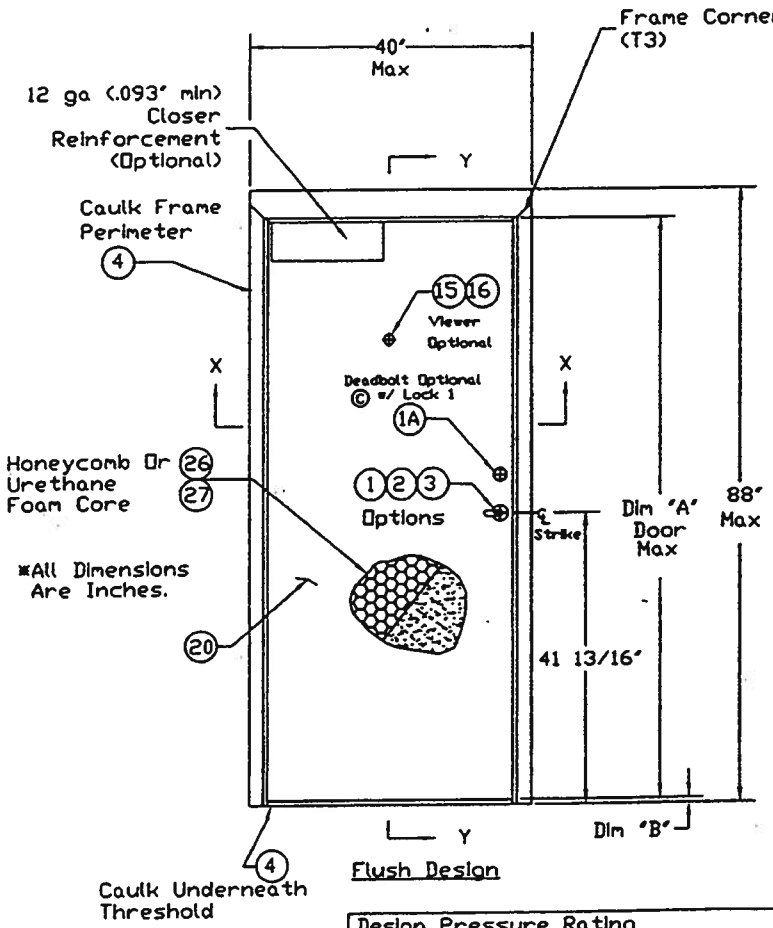
LABELING: Each unit shall bear a permanent label with the manufacturer's name or logo, city, state and following statement: "Miami-Dade County Product Control Approved", unless otherwise noted herein.

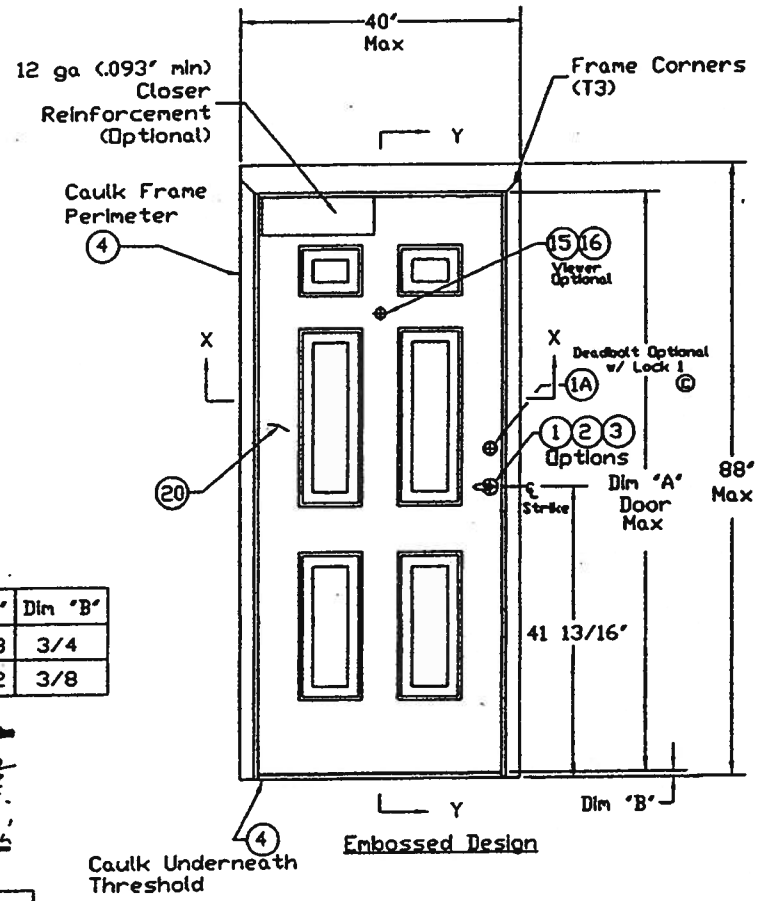
RENEWAL of this NOA shall be considered after a renewal application has been filed and there has been no change in the applicable building code negatively affecting the performance of this product.

TERMINATION of this NOA will occur after the expiration date or if there has been a revision or change in the materials, use, and/or manufacture of the product or process. Misuse of this NOA as an endorsement of any product, for sales, advertising or any other purposes shall automatically terminate this NOA. Failure to comply with any section of this NOA shall be cause for termination and removal of NOA.

ADVERTISEMENT: The NOA number preceded by the words Miami-Dade County, Florida, and followed by the expiration date may be displayed in advertising literature. If any portion of the NOA is displayed, then it shall be done in its entirety.

INSPECTION: A copy of this entire NOA shall be provided to the user by the manufacturer or its distributors and shall be available for inspection at the job site at the request of the Building Official.
This NOA consists of this page 1 as well as approval document mentioned above.
The submitted documentation was reviewed by **Ishaq I. Chanda, P.E.**





In-Swing Door (Exterior View)

	Dim "A"	Dim "B"
3/4" Undercut	83 1/8	3/4
3/8" Undercut	83 1/2	3/8

Approved as complying with the Florida Building Code
Date OCTOBER 31, 2002
NOA# 02-0807.04
Miami Dade Product Control Division
By Ishaq I. Chanda

Design Pressure Rating		
	Where Water Infiltration Requirement Is Needed	Where Water Infiltration Requirement Is Not Needed
Positive	Not Approved	+70 PSF
Negative	Not Approved	-70 PSF

Notes:
1) In-swing Not Approved For Water Infiltration
2) This Door Does Not Need A Hurricane Protection Systen
3) Hinge Spacing Is 33" O.C., 13" From Top Of Frame & 9" From The Botton.

MATERIAL SPECIFICATIONS:
Finish: Rust Inhibitive Primer

3-0 x 7-0 Series
Regent, Omega, Imperial, & Versadoor
In-Swing Elevation Drawing

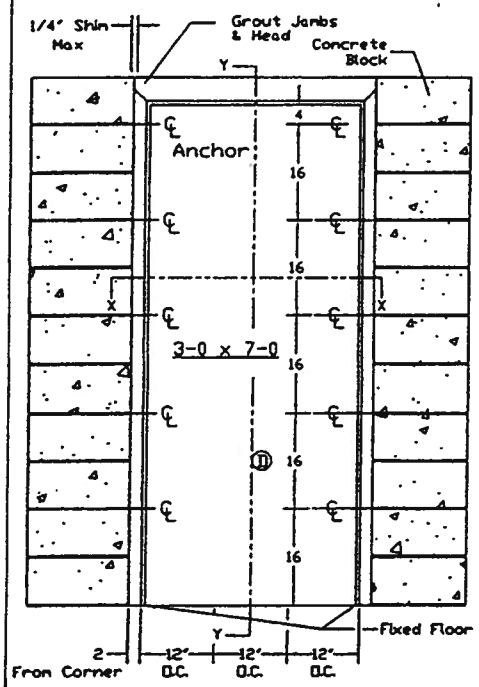
CECO DOOR PRODUCTS
Milan, Tennessee 38358

ISSUE	REVISIONS
B 10/10/02 LT	Revised Per Marked-Up Drawings From Ishaq Chanda.
A 9/26/02 LT	Revised Per Marked-Up Drawings From Ishaq Chanda.

DRAWN BY: LT
DATE: 5/22/02
DRAWING NUMBER: RD0728
Sheet 1 of 9

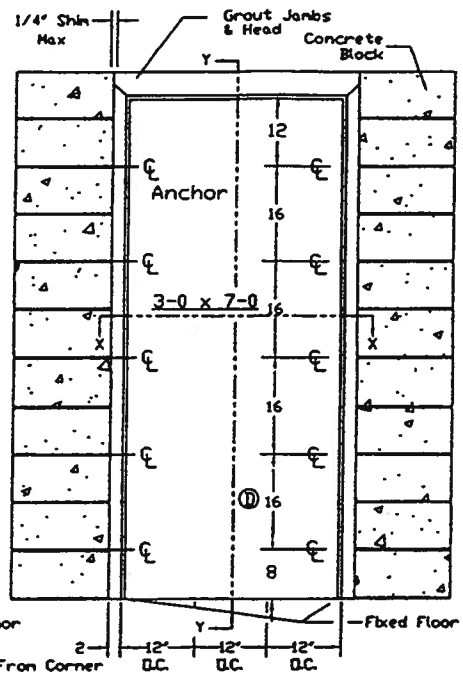
Masonry "T" Anchor

Min. 3500 PSI



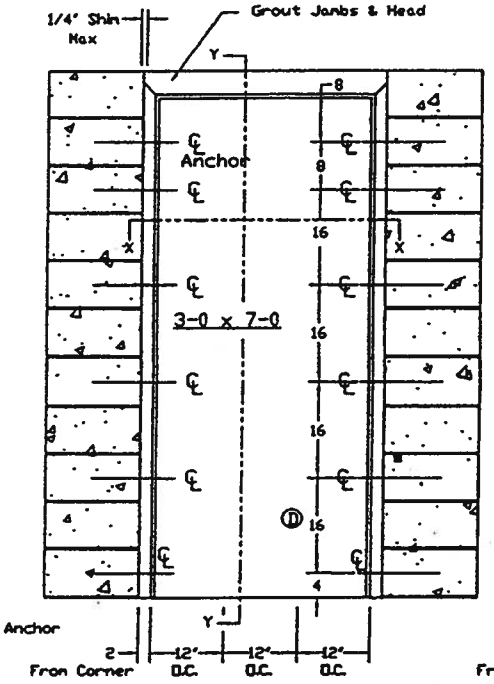
Masonry Wire Anchor

Min. 3500 PSI

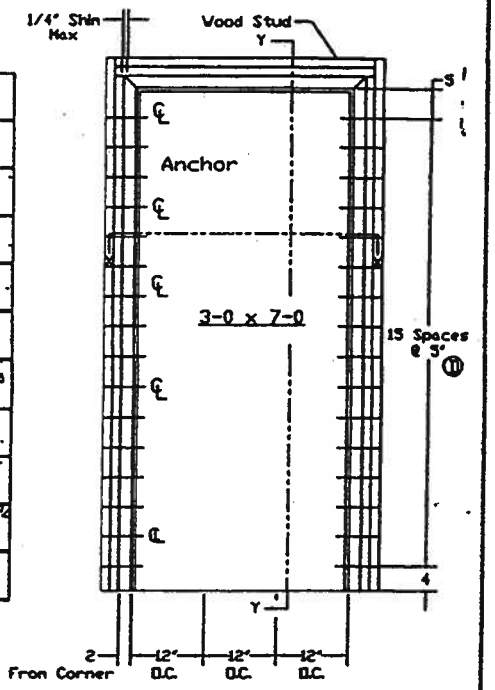


Existing Opening W/Lockbolt or Sleeve Anchor Into Block

Min. 3500 PSI

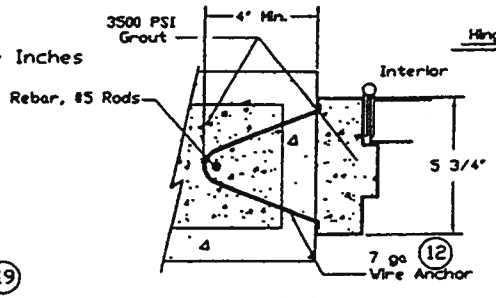


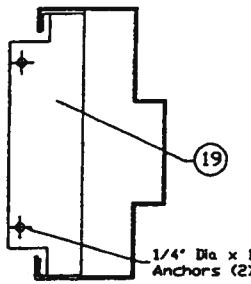
Existing Opening Anchor Into Wood Stud

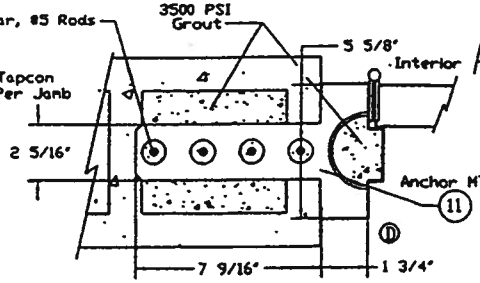


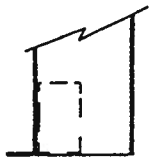
*All Dimensions Are Inches

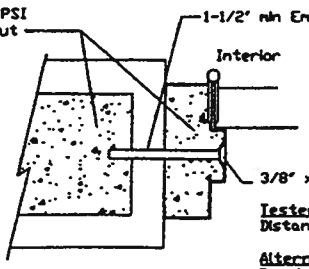
Installation Details
Hinge Jambs / Lock Jambs





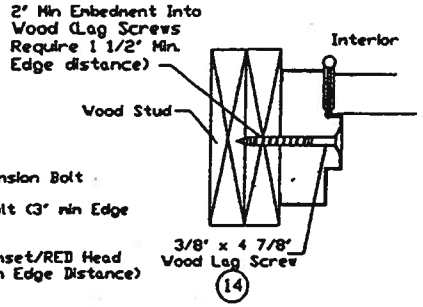






Tested Rawl Lok/Bolt (3" min Edge Distance)

Alternative ITV Ranset/RED Head Dynabolt (2-1/2" min Edge Distance)



Approved as complying with the Florida Building Code
Date OCT 31, 2002
NOA# 02-0807.04
Miami Dade Product Control Division
By Ishaq I. Chanda

ISSUE	REVISIONS
A 6/25/02 LT	Revised Per Marked-Up Drawings From Ishaq Chanda.

MATERIAL SPECIFICATIONS:

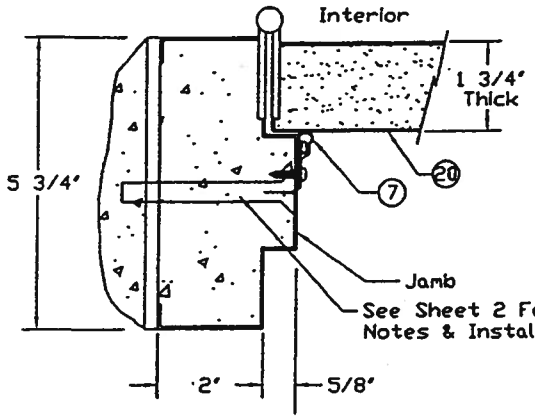
Frame Anchor (Inswing Doors)
Regent, Omega, Imperial & Versadoor
Installation Details

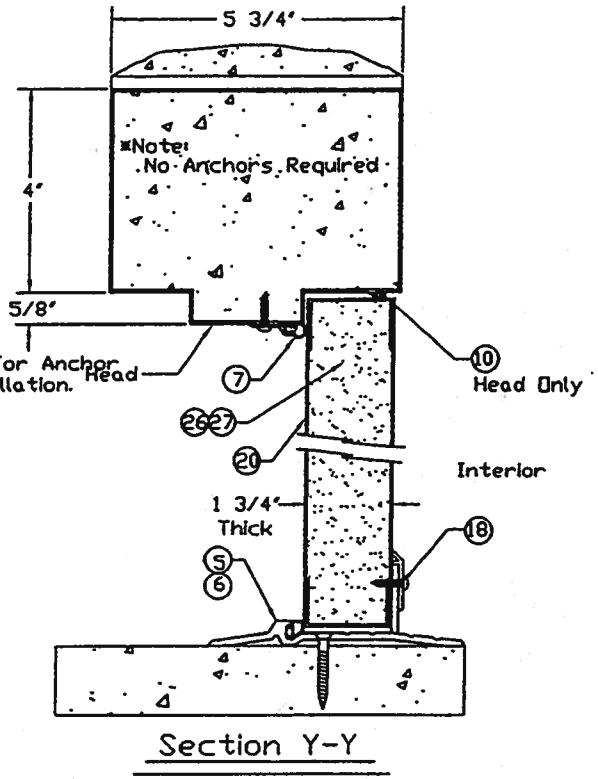
CECO DOOR PRODUCTS
Milan, Tennessee 38358

DRAWN BY:	DATE:
LT	5/22/02

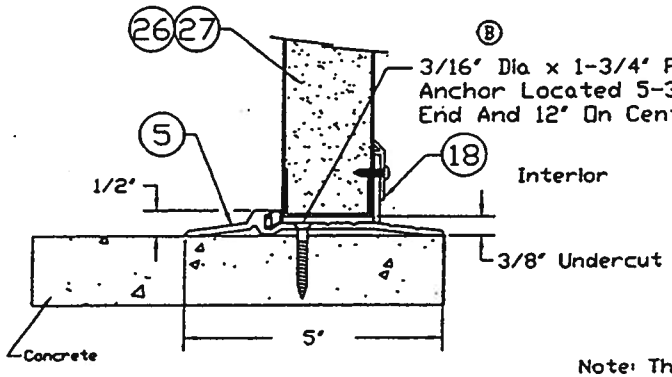
DRAWING NUMBER:
RD0728
Sheet 2 of 9

*Note: Structural Member At Header Must Be Designed To carry 58.3#/ft. load Imposed And Must Be Reviewed By Building Official.





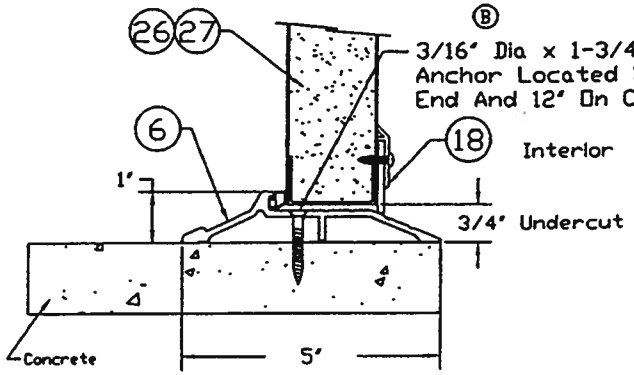
Section Y-Y



Threshold: Pemko 2005AV

Inswing
(Not Approved For Water)

Note: Thresholds Not Approved For Water.



Threshold: Pemko 181AV

Approved as complying with the Florida Building Code
Date OCT 31, 2002
NOA# 02-0807.04
Miami Dade Product Control Division
By Ishaq I. Chanda

D	10/15/02 LT	Revised Per Marked-Up Drawings From Ishaq Chanda
C	8/23/02 LT	Revised Per Marked-Up Drawings From Ishaq Chanda

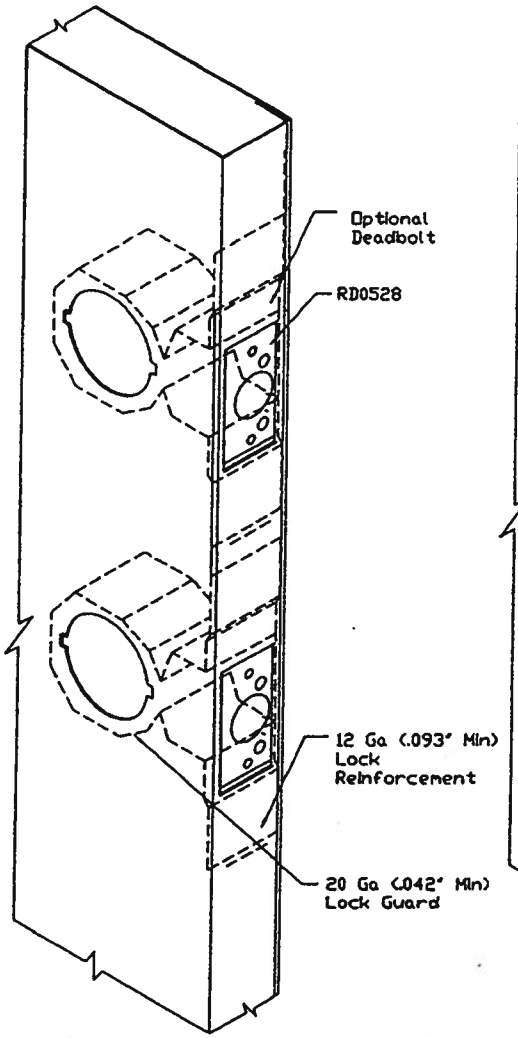
ISSUE	REVISIONS
DRAWN BY: LT	DATE: 5/22/02

MATERIAL SPECIFICATIONS:

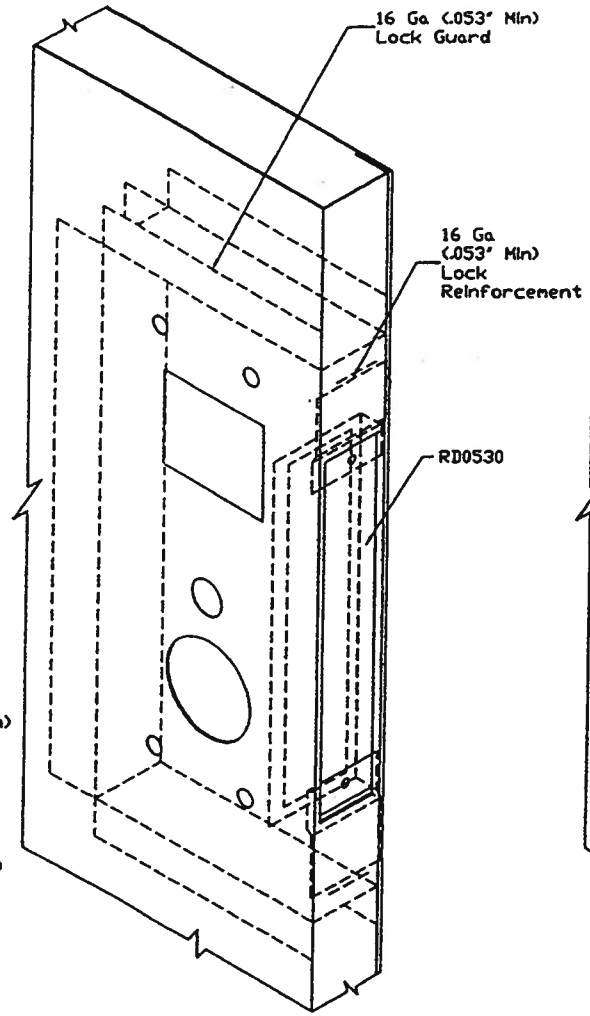
Threshold & Weatherstrip (Inswing Doors)
Regent, Omega, Imperial, Versadoor
Installation Details

CECO DOOR PRODUCTS
Milan, Tennessee 38358

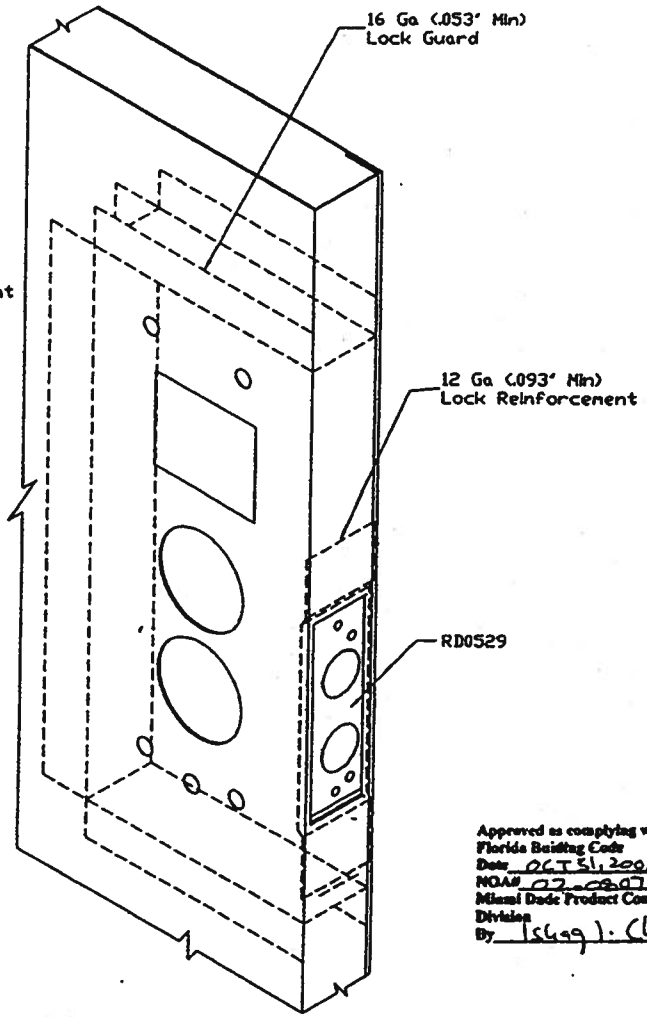
RD0728
Sheet 3 Of 9



Schlage AL53PD



Saflok MT



Saflok Premier SL2500

Approved as complying with the Florida Building Code
Date: OCT 31, 2002
NOA#: 02-0807.04
Miami Dade Product Control Division
By: Isaac J. Chanda

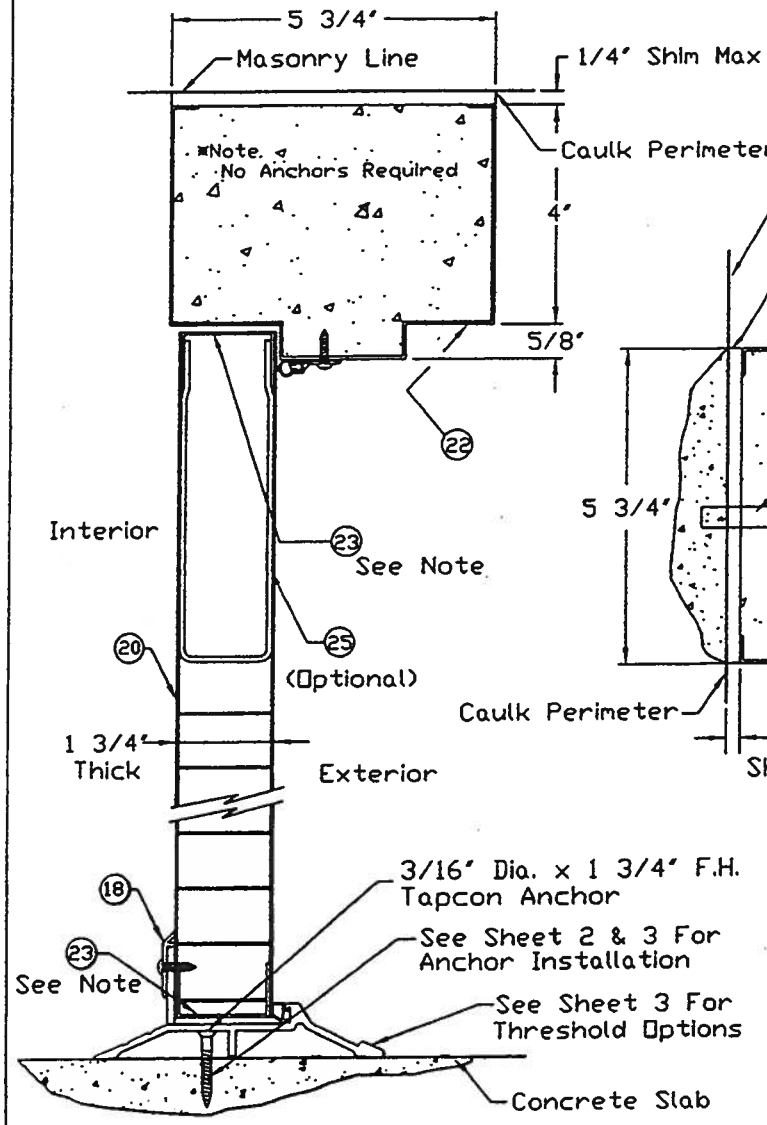
A 5/28/02 LT	Added RD0528, RD0529 & RD0530.
ISSUE	REVISIONS
DRAWN BY: LT	DATE: 5/28/02
DRAWING NUMBER: RD0728	Sheet 4 of 9

MATERIAL SPECIFICATIONS:

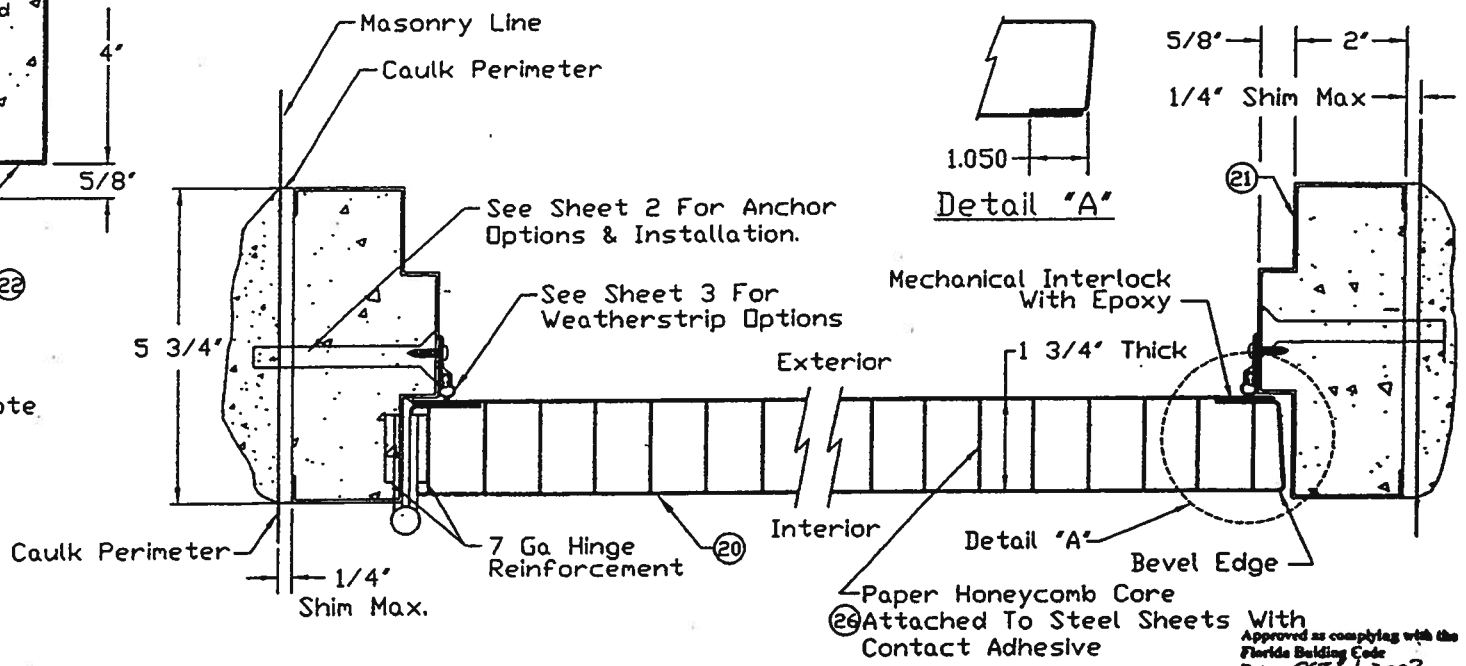
Lock Reinforcement (Inswing Doors)
Regent, Omega, Imperial, Versadoor
Reinforcement Details

CECO DOOR PRODUCTS
Milan, Tennessee 38358

*Note: Structural Member At Head Must Be Designed To Carry 58.3 #/ft. Load Imposed And Must Be Reviewed By Building Official.



Section Y-Y



Section X-X

Note 1: Top and Bottom Channel Tack Welded To Both Skins 3 Inches From Lock Edge And 6 Inches On Centers

Approved as complying with the Florida Building Code
Date OCT 31, 2002
NOA# 02-0307.04
Miami Dade Product Control Division
By Ishaq I. Chanda

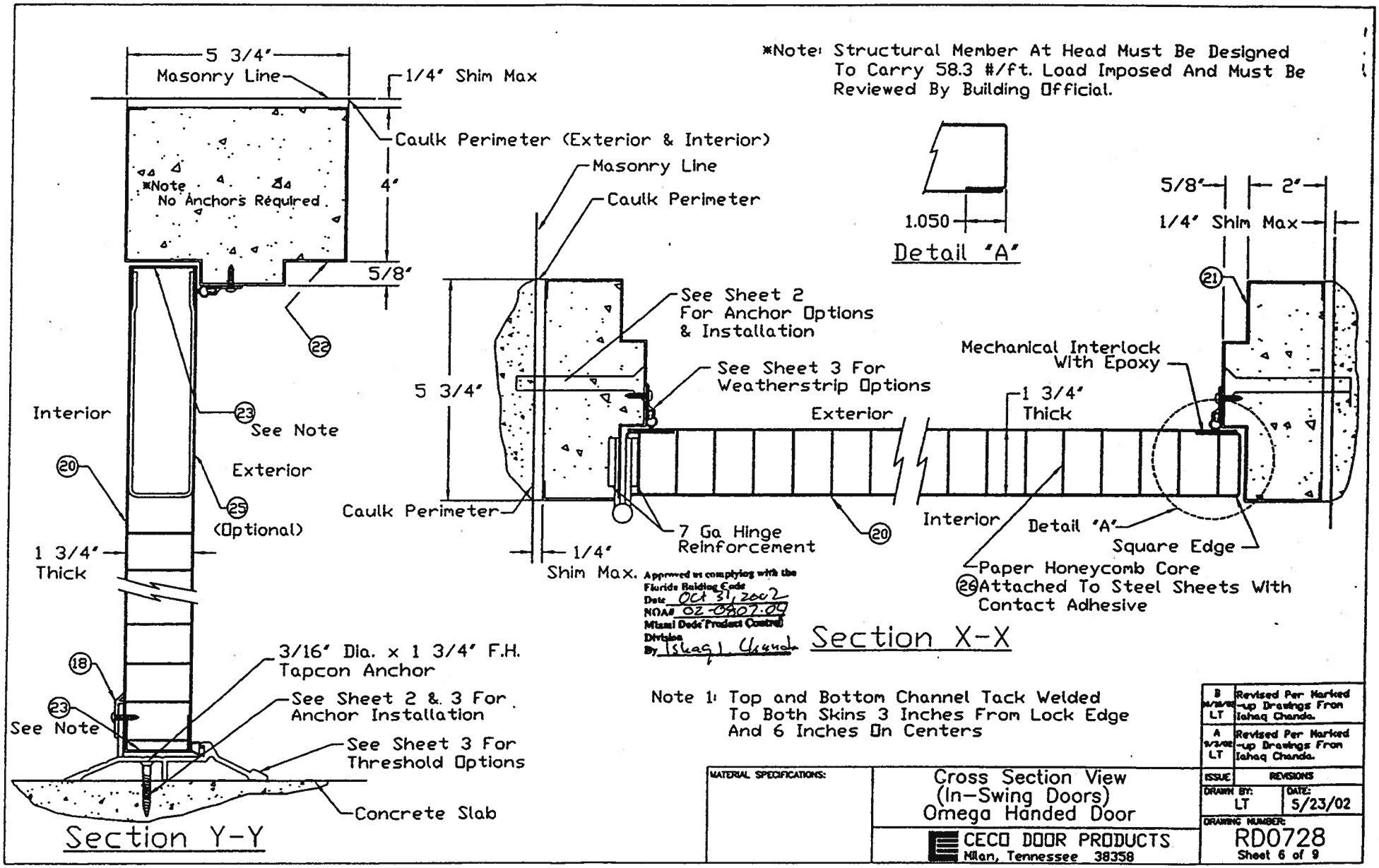
C 10/28/02 LT	Revised Per Marked -up Drawings From Ishaq Chanda.	
B 9/26/02 LT	Revised Per Marked -up Drawings From Ishaq Chanda.	
ISSUE	REVISIONS	
DRAWN BY: LT	DATE: 5/22/02	

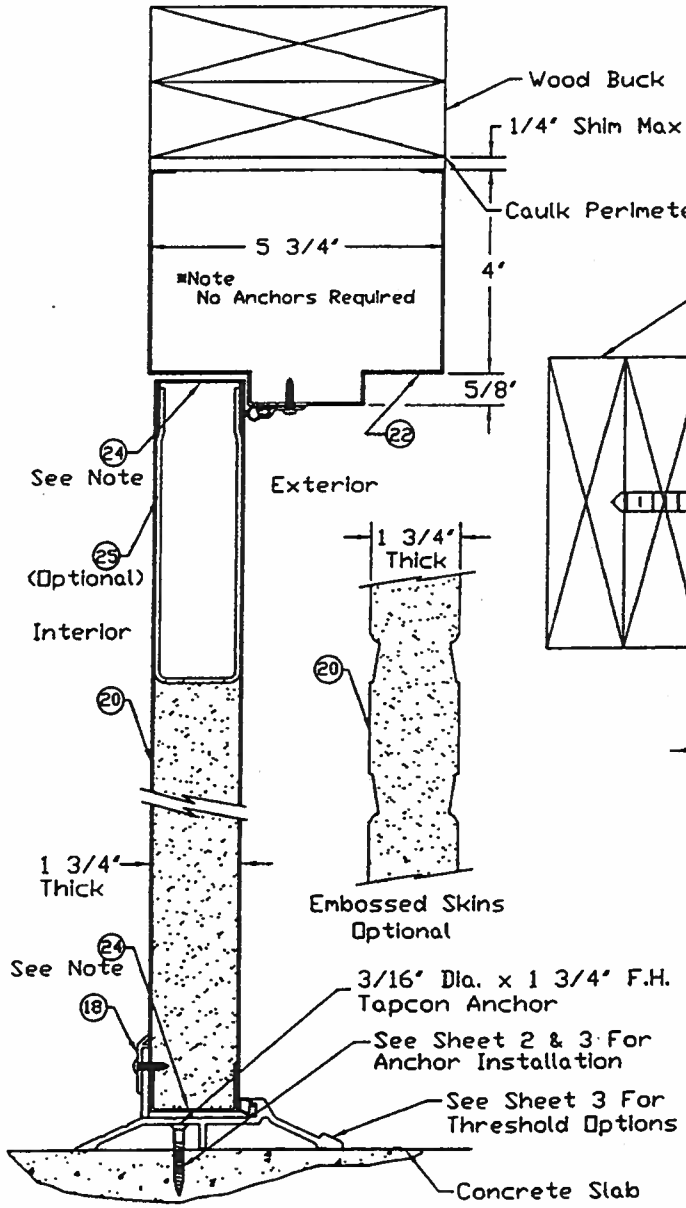
MATERIAL SPECIFICATIONS:

Cross Section View (Inswing Doors) Regent Handed Door

CECO DOOR PRODUCTS
Milan, Tennessee 38358

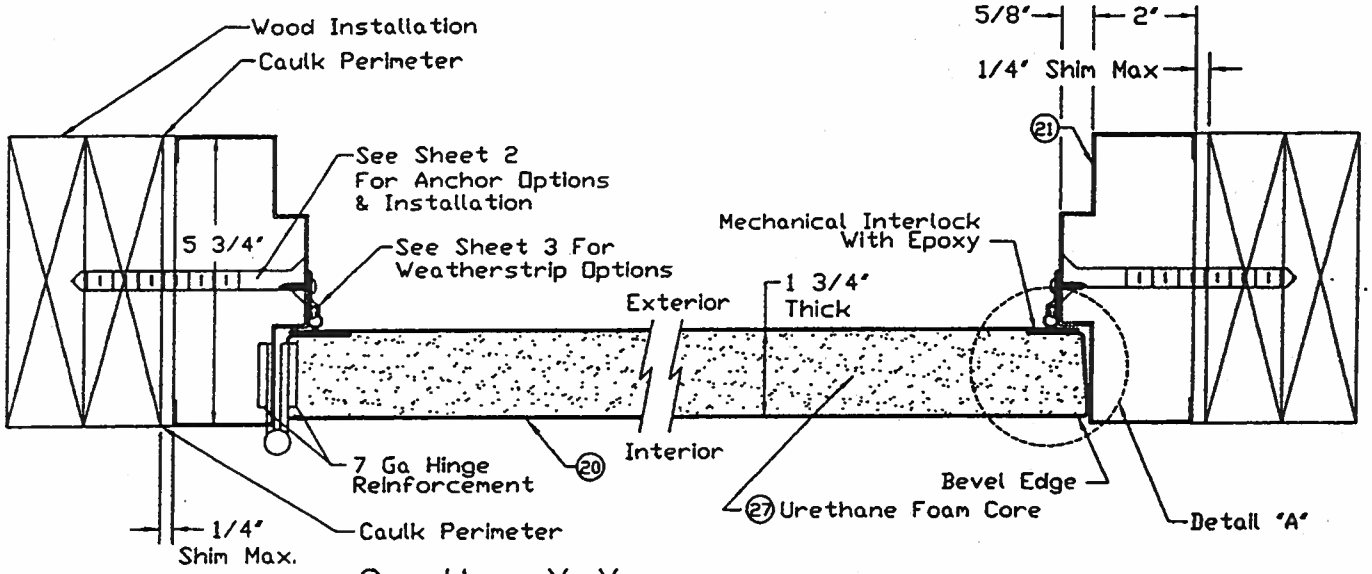
DRAWING NUMBER: RD0728
Sheet 5 of 9


*Note: Structural Member At Head Must Be Designed To Carry 58.3 #/ft. Load Imposed And Must Be Reviewed By Building Official.
5 3/4"
Masonry Line
1/4" Shim Max
Caulk Perimeter (Exterior & Interior)
*Note
No Anchors Required
4"
5/8"
22
Interior
23
See Note
Exterior
20
25
(Optional)
1 3/4"
Thick
18
23
See Note
3/16" Dia. x 1 3/4" F.H.
Tapcon Anchor
See Sheet 2 & 3 For
Anchor Installation
See Sheet 3 For
Threshold Options
Concrete Slab
Section Y-Y
Masonry Line
Caulk Perimeter
5 3/4"
See Sheet 2
For Anchor Options
& Installation
See Sheet 3 For
Weatherstrip Options
Exterior
Caulk Perimeter
7 Ga Hinge
Reinforcement
20
1/4"
Shim Max.
Approved as complying with the
Florida Building Code
Date Oct 31, 2002
NOA# 02-0807.09
Miami Dade Product Control
Division
By Ishaq Chanda
Section X-X
1.050
Detail "A"
5/8"
2"
1/4" Shim Max
21
Mechanical Interlock
With Epoxy
1 3/4"
Thick
Interior
Detail "A"
Square Edge
Paper Honeycomb Core
26
Attached To Steel Sheets With
Contact Adhesive
Note 1: Top and Bottom Channel Tack Welded To Both Skins 3 Inches From Lock Edge And 6 Inches On Centers
B 11/15/02 LT
Revised Per Marked-up Drawings From Ishaq Chanda.
A 9/3/02 LT
Revised Per Marked-up Drawings From Ishaq Chanda.
ISSUE
REVISIONS
DRAWN BY: LT
DATE: 5/23/02
DRAWING NUMBER: RD0728
Sheet 6 of 9
MATERIAL SPECIFICATIONS:
Cross Section View
(In-Swing Doors)
Omega Handed Door
CECO DOOR PRODUCTS
Milan, Tennessee 38358



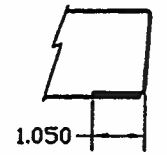
Section Y-Y

Note: Structural member at Head Must Be Designed To carry 58.3 #/ft. Load Imposed And Must Be Reviewed By Building Official.



Section X-X

Note: Top & Bottom Channels Assembled Std Method To Skins With Spot Welds & Tape. Channels Are Then Tack Welded To Both Skins 3" From Lock Edge And 6 Inches On Center.



Detail "A"

Approved as complying with the Florida Building Code
Date OCT 24 2002
NOA# 02-0507.04
Miami Dade Product Control Division
By Ishaq I. Chanda

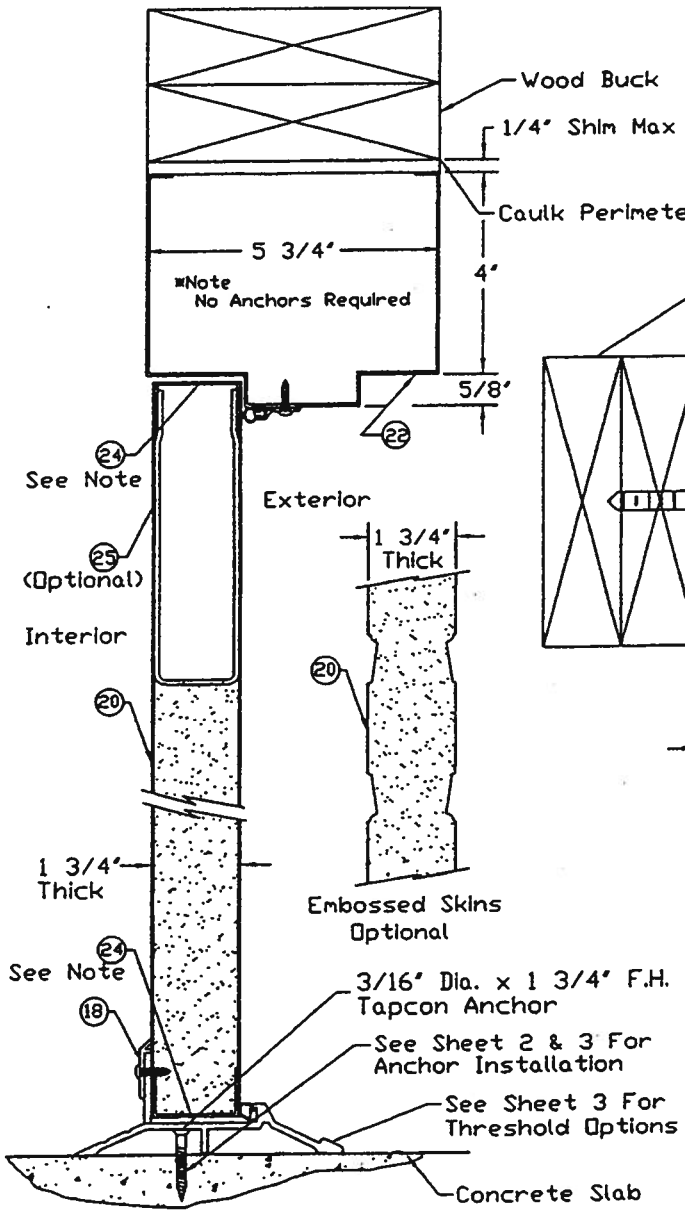
B w/rev LT	Revised Per Marked-Up Drawings From Ishaq Chanda.
ISSUE	REVISIONS

DRAWN BY: LT
DATE: 5/23/02
DRAWING NUMBER: RD0728
Sheet 7 of 9

MATERIAL SPECIFICATIONS:

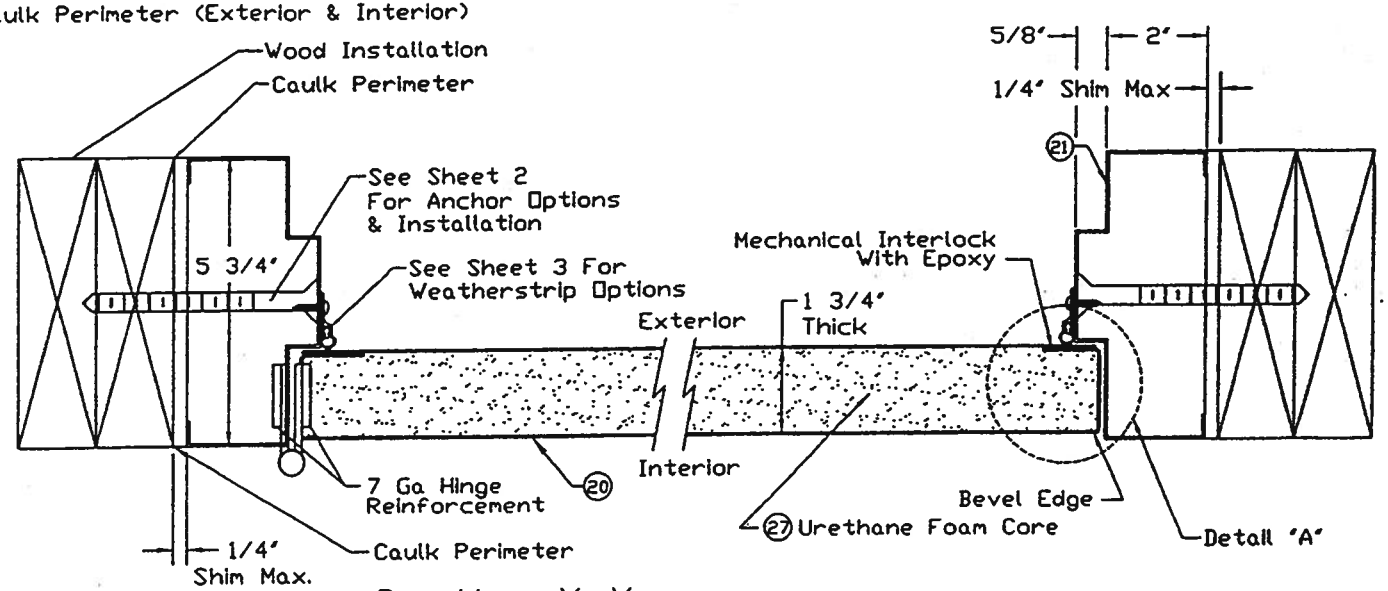
Cross Section View
(In-Swing Doors)
Imperial Handed Door

CECO DOOR PRODUCTS
Milan, Tennessee 38358



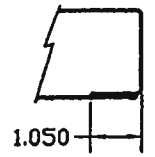
Section Y-Y

Note: Structural member at Head Must Be Designed To carry 58.3 #/ft. Load Imposed And Must Be Reviewed By Building Official.



Section X-X

Note: Top & Bottom Channels Assembled Std Method To Skins With Spot Welds & Tape. Channels Are Then Tack Welded To Both Skins 3" From Lock Edge And 6 Inches On Center.



Detail "A"

Approved as complying with the Florida Building Code
Date OCT 21, 2002
NOA# 02-0807.04
Miami Dade Product Control
Division
By Ishaq I. Chanda

B	Revised Per Marked-Up Drawings From Ishaq Chanda.
LT	

ISSUE	REVISIONS
DRAWN BY: LT	DATE: 5/23/02

MATERIAL SPECIFICATIONS:

Cross Section View
(In-Swing Doors)
Versadoor Handed Door

CECO DOOR PRODUCTS
Milan, Tennessee 38358

DRAWING NUMBER:
RD0728
Sheet 8 of 9

1	Cylindrical Lock & Lock Reinforcement (RD0528)	Schlage	AL53PD
1A	Deadbolt (Optional) ⓓ	Schlage	B100
2	Or Cylindrical Lock & Lock Reinforcement	Saflok	Premier SL2500
3	Or Mortise Lock	Saflok	MT
4	Caulk	Dow Corning	899 Silicone Glazing Sealant
5	Threshold	Pemko	2005AV36
6	Or	Pemko	181AV36
7	Weatherstrip	Pemko	303AV3684
8	Hinge (Ball Bearing)	Hager or Equal (Attached w/ (8) #12-24 x 1/2 MS Per Hinge)	4-1/2 x 4-1/2 x .134 (Std Weight)
9	Or (Spring)	Hager or Equal (Attached w/ (8) #12-24 x 1/2 MS Per Hinge)	4-1/2 x 4-1/2 x .134 (Std Weight)
10	Weatherstrip	Pemko	S88
11	Frame Anchor	Masonary Tee (RD0057)	16 ga (.053" min) Galv Steel Fymin = 30ksi
12	Or	Wire, Relaxed Dimension 9" x 8"	#7 (.167 " min) Galv Steel Wire (70,000 - 90,000 psi Tensile Strength)
13	Or	Expansion Bolt	3/8" x 5" F.H. Rawl Lok/Bolt Or 3/8" x 5" F.H. Ramset/RED Head
14	Or	Wood Lag Screw	3/8" x 4-5/8"
15	Viewer	Hager	1755
16	Or	MAG Security	8724-C
17	Drip Cap; Top	Pemko	346
18	Sweep	Pemko	315 N
19	Floor Anchor	Fixed Floor Anchor	16 ga (.053" min) galvanized Steel
20	Face Sheet A60 Galv Conforming To ASTM A653	Commercial Steel Type B (Minimum Yield Strength 30,000psi)	16 Ga (.053" min)
21	Series SF, Frame Jamb, Double Rabbet Profile, A60 Galv Conforming To ASTM A653	16 Ga (.053" min) Commercial Steel Type B (Minimum Yield Strength 30,000psi)	2" Face, 5-3/4" Depth Min. (RD0033)
22	Series SF, Frame Head, Double Rabbet, Profile A60 Galv Conforming To ASTM A653	16 Ga (.053" min) Commercial Steel Type B (Minimum Yield Strength 30,000psi)	4" Face, 5-3/4" Depth Min. (RD0033)
23	Door Channels; Spot Welded To Bottom Skin Glued To Top Skin; Tack Welded To Both	16 Ga (.053" min) A60 Galv Conforming To ASTM A653 Commercial Steel Type B (Minimum Yield Strength 30,000psi)	16 ga (.053" min) x 1" x 1-3/4" x 1"
24	Door Channels; Spot Welded To Bottom Skin Taped To Top Skin; Tack Welded To Both	16 Ga (.053" min) A60 Galv Conforming To ASTM A653 Commercial Steel Type B (Minimum Yield Strength 30,000psi)	16 ga (.053" min x 1" x 1-3/4" x 1"
25	Closer Reinforcement (Optional)	12 Ga (.093" min) CS Type B	12 ga (.093" min) x 5-3/8" x 16"
26	Honeycomb Core	Non-Impregnated Kraft Paper Ⓔ	1.2" Nominal Cell Size
27	Urethane Core	Foam Enterprises	2 lb/ft^3 Density

Approved as complying with the Florida Building Code
Date: Oct 31, 2002
NOA# 02-0807.04
Miami Dade Product Control Division
By: Ishaq I. Chanda

B 10/10/02 LT	Revised Per Marked-Up Drawings From Ishaq Chanda.
A 9/4/02 LT	Revised Per Marked-Up Drawings From Ishaq Chanda.

MATERIAL SPECIFICATIONS:

3-0 x 7-0 Series In-Swing Bill Of Materials

CECO DOOR PRODUCTS
Milan, Tennessee 38358

ISSUE	REVISIONS
DRAWN BY: LT	DATE: 5/28/02
DRAWING NUMBER: RD0728	
Sheet 9 Of 9	

From: The Columbia County Building & Zoning Department
Plan Review
135 NE Hernando Av.
P.O. Box 1529
Lake City Florida 32056-1529

Reference to a building permit application Number: 0605-54

Contractor: Trent Giebeig Owners Trent Giebeig Lot 32 Phase 1 Cannon Creek Place

On the date of May 16, 2006 application 0605-54 and plans for construction of a single family dwelling were reviewed and the following information or alteration to the plans will be required to continue processing this application. If you should have any question please contact the above address, or contact phone number (386) 758-1163 or fax any information to (386) 754-7088.

Please include application number 0605-54 when making reference to this application.

1. Line 6, conditioned area square footage on form 600A-2004 of the Florida Energy Efficiency Code for Building Construction doesn't concur with the conditioned floor area on the submitted plans. The total conditioned areas on the plans are 1897 (square feet). Line 6 currently reads that the conditioned floor area equals1972 (square feet). *Please resubmit the corrected form to reflect on line 6 the actual total conditioned area to this department.*

Thank you,

Joe Haltiwanger
Plan Examiner
Columbia County Building Department

New Construction Subterranean Termite Soil Treatment Record

OMB Approval No. 2502-0525

This form is completed by the licensed Pest Control Company.

Public reporting burden for this collection of information is estimated to average 15 minutes per response, including the time for reviewing instructions, searching existing data sources, gathering and maintaining the data needed, and completing and reviewing the collection of information. This information is mandatory and is required to obtain benefits. HUD may not collect this information, and you are not required to complete this form, unless it displays a currently valid OMB control number.
Section 24 CFR 200.926d(b)(3) requires that the sites for HUD insured structures must be free of termite hazards. This information collection requires the builder to certify that an authorized Pest Control company performed all required treatment for termites, and that the builder guarantees the treated area against infestation for one year. Builders, pest control companies, mortgage lenders, homebuyers, and HUD as a record of treatment for specific homes will use the information collected. The information is not considered confidential.

This report is submitted for informational purposes to the builder on proposed (new) construction cases when soil treatment for prevention of subterranean termite infestation is specified by the builder, architect, or required by the lender, architect, FHA, or VA.

All contracts for services are between the Pest Control Operator and builder, unless stated otherwise.

#24614

Section 1: General Information (Treating Company Information)

Company Name: Aspen Pest Control, Inc.
Company Address: 321 N.W. Cole Terrace, Suite 107 City Lake City State FL Zip 32055
Company Business License No. JB103478 Company Phone No. 386-755-3511 • 352-494-5751
FHA/VA Case No. (if any)

Section 2: Builder Information

Company Name: Trent Siebig Company Phone No.

Section 3: Property Information

Location of Structure(s) Treated (Street Address or Legal Description, City, State and Zip) 314 SW Arrowhead Rd. Lake City FL

Type of Construction (More than one box may be checked) ☒ Slab ☐ Basement ☐ Crawl ☐ Other
Approximate Depth of Footing: Outside 12 Inside 12 Type of Fill Dirt

Section 4: Treatment Information

Date(s) of Treatment(s) 11-14-06
Brand Name of Product(s) Used Termidor
EPA Registration No. 7969-210
Approximate Final Mix Solution % 0.06
Approximate Size of Treatment Area: Sq. ft. 2650 Linear ft. 243 Linear ft. of Masonry Voids 243
Approximate Total Gallons of Solution Applied 909
Was treatment completed on exterior? ☐ Yes ☒ No
Service Agreement Available? ☒ Yes ☐ No

Note: Some state laws require service agreements to be issued. This form does not preempt state law.

Attachments (List)
Comments

Name of Applicator(s) Steve Brown Certification No. (if required by State law)

The applicator has used a product in accordance with the product label and state requirements. All treatment materials and methods used comply with state and federal regulations.

Authorized Signature Date 11-14-06

Warning: HUD will prosecute false claims and statements. Conviction may result in criminal and/or civil penalties. (18 U.S.C. 1001, 1010. 1012; 31 U.S.C. 3729, 3802)

CERTIFICATE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 24-4S-16-03114-132 **Building permit No.** 000024614

Use Classification SFD/UTILITY **Fire:** 27.90

Permit Holder B. TRENT GIEBEIG **Waste:** 83.75

Owner of Building B. TRENT GIEBIEG CONSTR.,INC. **Total:** 111.65

Location: 314 SW ARROWBEND DRIVE, LAKE CITY, FL

Date: 05/03/2007

John Kence

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)

Project Information for:	L152528		
Builder:	GIEBEIG HOMES	Date:	2/23/2006
Lot:	LOT 32 CANNON CREEK PLA	Start Number:	**1730**
Subdivision:	N\A		
County or City:	COLUMBIA		
Truss Page Count:	31		

Truss Design Load Information (UNO) Design Program: MiTek 5.2 / 6.2

Gravity		**Wind**		**Building Code:**	**FBC2004**
Roof (psf):	**42**	Wind Standard:	**ASCE 7-02**		
Floor (psf):	55	Wind Speed (mph):	**110**		

Note: See individual truss drawings for special loading conditions

Building Designer, responsible for Structural Engineering: (See attached)

NORRIS, JOHN DAVID RG 0066597
Address: 351 NW CORWIN GLN
LAKE CITY, FL. 32025 Designer: **95**

Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987
Company: Structural Engineering and Inspections, Inc. EB 9196
Address 16105 N. Florida Ave, Ste B, Lutz, FL 33549

Notes:

1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
4. Trusses designed for veritcal loads only, unless noted otherwise.

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	CJ1	0223061730	2/23/2006				
2	CJ3	0223061731	2/23/2006				
3	CJ5	0223061732	2/23/2006				
4	EJ3	0223061733	2/23/2006				
5	EJ7	0223061734	2/23/2006				
6	HJ4	0223061735	2/23/2006				
7	HJ9	0223061736	2/23/2006				
8	T01	0223061737	2/23/2006				
9	T02	0223061738	2/23/2006				
10	T03	0223061739	2/23/2006				
11	T04	0223061740	2/23/2006				
12	T05	0223061741	2/23/2006				
13	T06	0223061742	2/23/2006				
14	T07	0223061743	2/23/2006				
15	T08	0223061744	2/23/2006				
16	T09	0223061745	2/23/2006				
17	T10	0223061746	2/23/2006				
18	T11	0223061747	2/23/2006				
19	T12	0223061748	2/23/2006				
20	T13	0223061749	2/23/2006				
21	T14	0223061750	2/23/2006				
22	T15	0223061751	2/23/2006				
23	T16	0223061752	2/23/2006				
24	T17	0223061753	2/23/2006				
25	T18	0223061754	2/23/2006				
26	T19	0223061755	2/23/2006				
27	T20	0223061756	2/23/2006				
28	T21	0223061757	2/23/2006				
29	T22	0223061758	2/23/2006				
30	T23	0223061759	2/23/2006				
31	T24	0223061760	2/23/2006				

FEB 23 2006

07:05:33 AM

Licensee Details

Licensee Information

Name:	**NORRIS, JOHN DAVID** (Primary Name) **INDIVIDUAL** (Alternate Name)
Main Address:	**351 NW CORWIN GLN** **LAKE CITY, Florida 32055**
Lic. Location:	**WOODGLEN DRIVE** **LAKE CITY, FL 32055** **Columbia**

License Information

License Type:	**Registered General Contractor**
Rank:	**Reg General**
License Number:	**RG0066597**
Status:	**Current, Active**
Licensure Date:	**06/20/1996**
Expires:	**08/31/2005**

Special Qualifications	Effective Date
Bldg Code Core Course Credit	
No Qualified Business License Required	02/20/2004

View Related License Information
View License Complaint

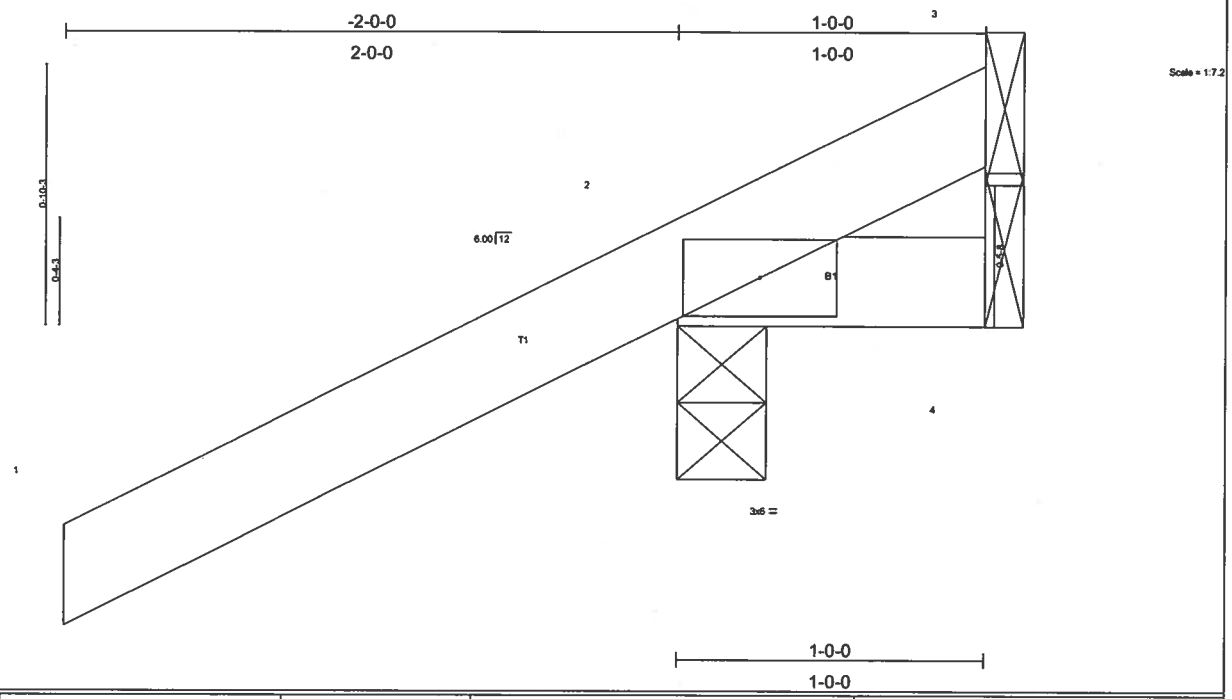
New Search

FEBRUARY 23,2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196

Dwg.#0223061730

Job	Truss	Truss Type	Qty	Ply	ST JOHNS WITH PORCH
MASTER	CJ1	ROOF TRUSS	18	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Feb 23 15:36:03 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.28	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 7 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=266/0-3-8, 4=14/Mechanical, 3=-90/Mechanical
Max Horz 2=87(load case 5)
Max Uplift2=-286(load case 5), 4=-9(load case 3), 3=-90(load case 1)
Max Grav 2=266(load case 1), 4=14(load case 1), 3=127(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-69/75
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.14

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 286 lb uplift at joint 2, 9 lb uplift at joint 4 and 90 lb uplift at joint 3.

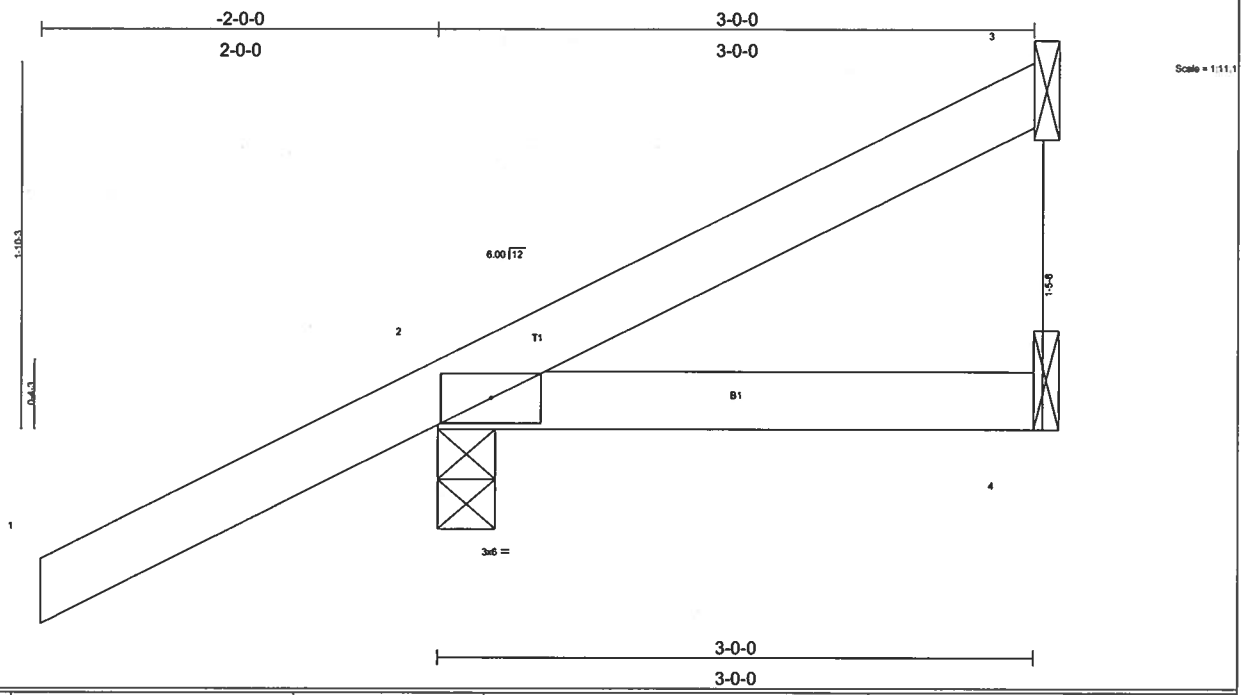
LOAD CASE(S) Standard

FEBRUARY 23,2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549

Dwg.#0223061731

Job	Truss	Truss Type	Qty	Ply	ST JOHNS WITH PORCH
MASTER	CJ3	ROOF TRUSS	14	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Feb 23 15:36:04 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	0.01	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	0.01	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 13 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=31/Mechanical, 2=278/0-3-8, 4=42/Mechanical
Max Horz 2=132(load case 5)
Max Uplift3=-28(load case 6), 2=-238(load case 5), 4=-27(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-57/7
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.13

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 3, 238 lb uplift at joint 2 and 27 lb uplift at joint 4.

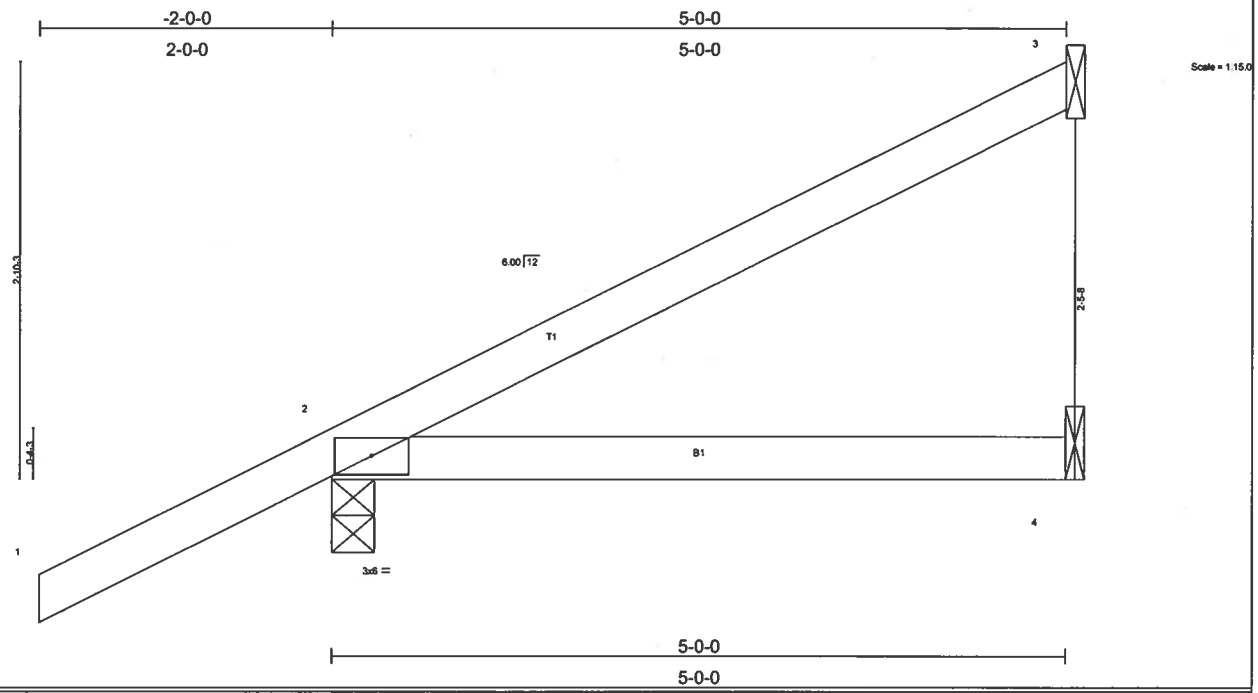
LOAD CASE(S) Standard

FEBRUARY 23,2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0223061732

Job	Truss	Truss Type	Qty	Ply	ST JOHNS WITH PORCH
MASTER	CJ5	ROOF TRUSS	14	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Feb 23 15:36:05 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	0.09	2-4	>663	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.24	Vert(TL)	0.07	2-4	>774	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 19 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=103/Mechanical, 2=343/0-3-8, 4=72/Mechanical
Max Horz 2=178(load case 5)
Max Uplift3=-87(load case 5), 2=-260(load case 5), 4=-46(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-88/36
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.15

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 87 lb uplift at joint 3, 260 lb uplift at joint 2 and 46 lb uplift at joint 4.

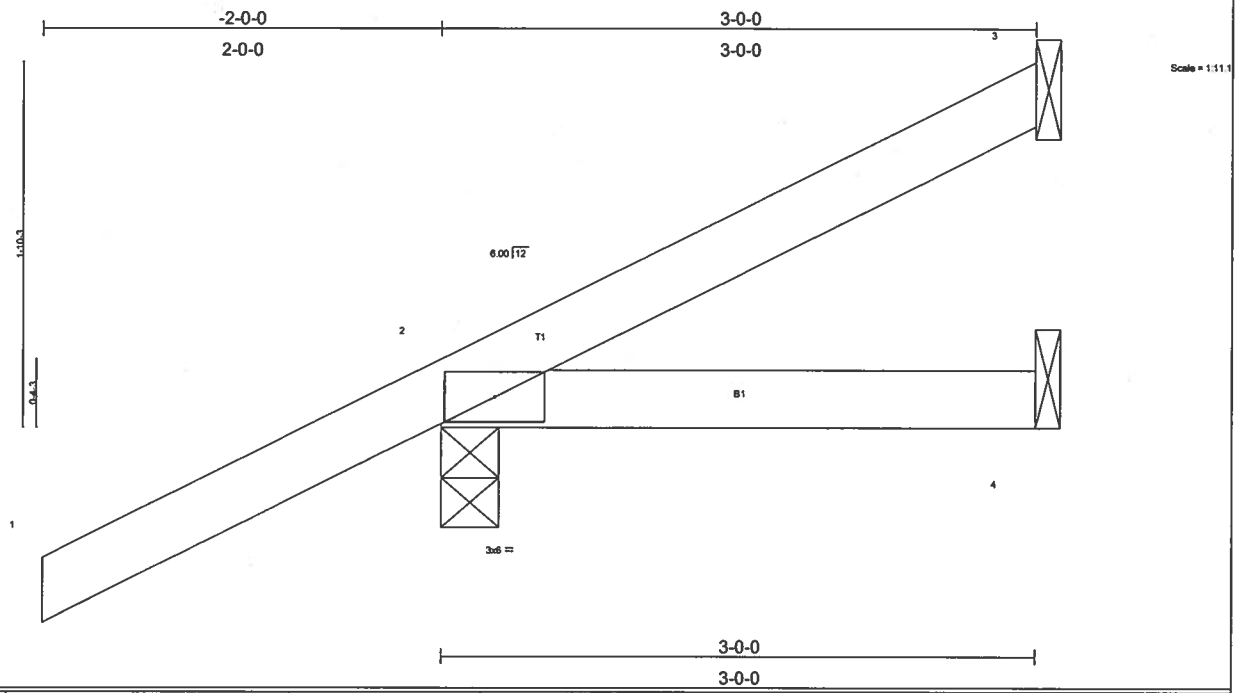
LOAD CASE(S) Standard

FEBRUARY 23,2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0223061733

Job	Truss	Truss Type	Qty	Ply	ST JOHNS WITH PORCH
MASTER	EJ3	ROOF TRUSS	3	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Feb 23 15:36:06 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	0.01	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	0.01	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 13 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=31/Mechanical, 2=278/0-3-8, 4=42/Mechanical
Max Horz 2=132(load case 5)
Max Uplift3=-28(load case 6), 2=-238(load case 5), 4=-27(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-57/7
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.13

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 3, 238 lb uplift at joint 2 and 27 lb uplift at joint 4.

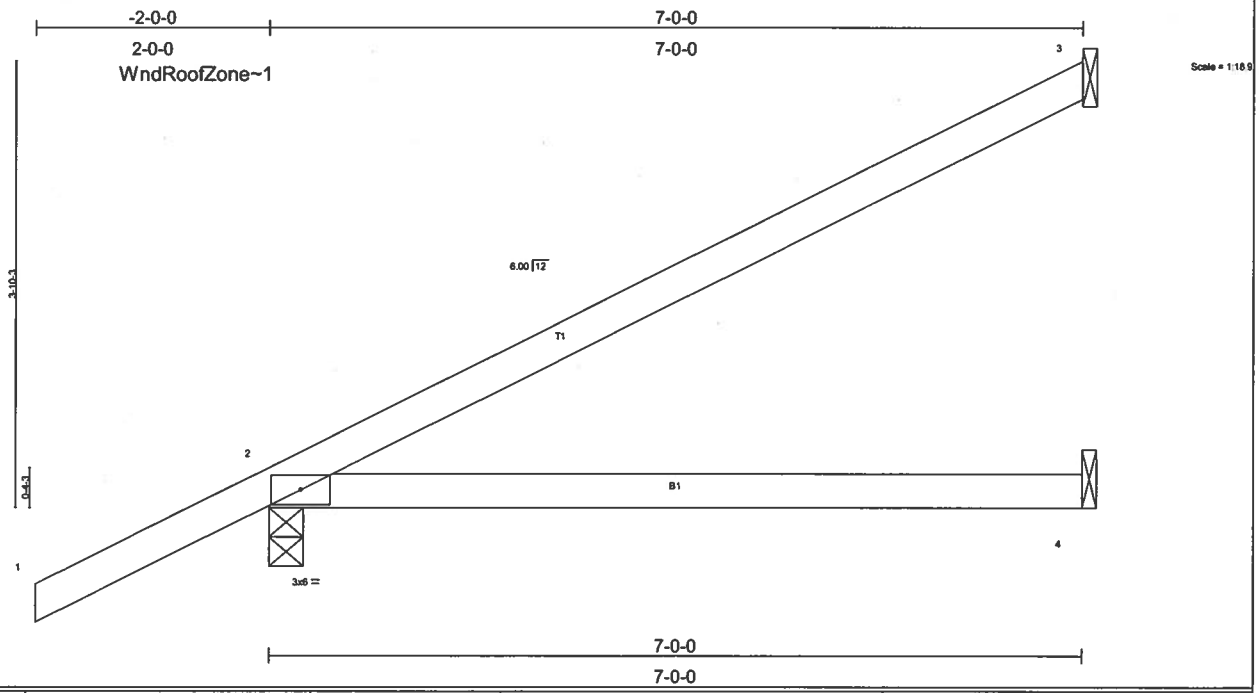
LOAD CASE(S) Standard

FEBRUARY 23,2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0223061734

Job	Truss	Truss Type	Qty	Ply	ST JOHNS WITH PORCH
MASTER	EJ7	MONO TRUSS	30	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Feb 23 15:36:06 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.50	Vert(LL)	0.33	2-4	>250	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.45	Vert(TL)	0.28	2-4	>295	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 26 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=162/Mechanical, 2=419/0-3-8, 4=104/Mechanical
Max Horz 2=224(load case 5)
Max Uplift3=-144(load case 5), 2=-295(load case 5), 4=-68(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-131/58
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.34

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 144 lb uplift at joint 3, 295 lb uplift at joint 2 and 68 lb uplift at joint 4.

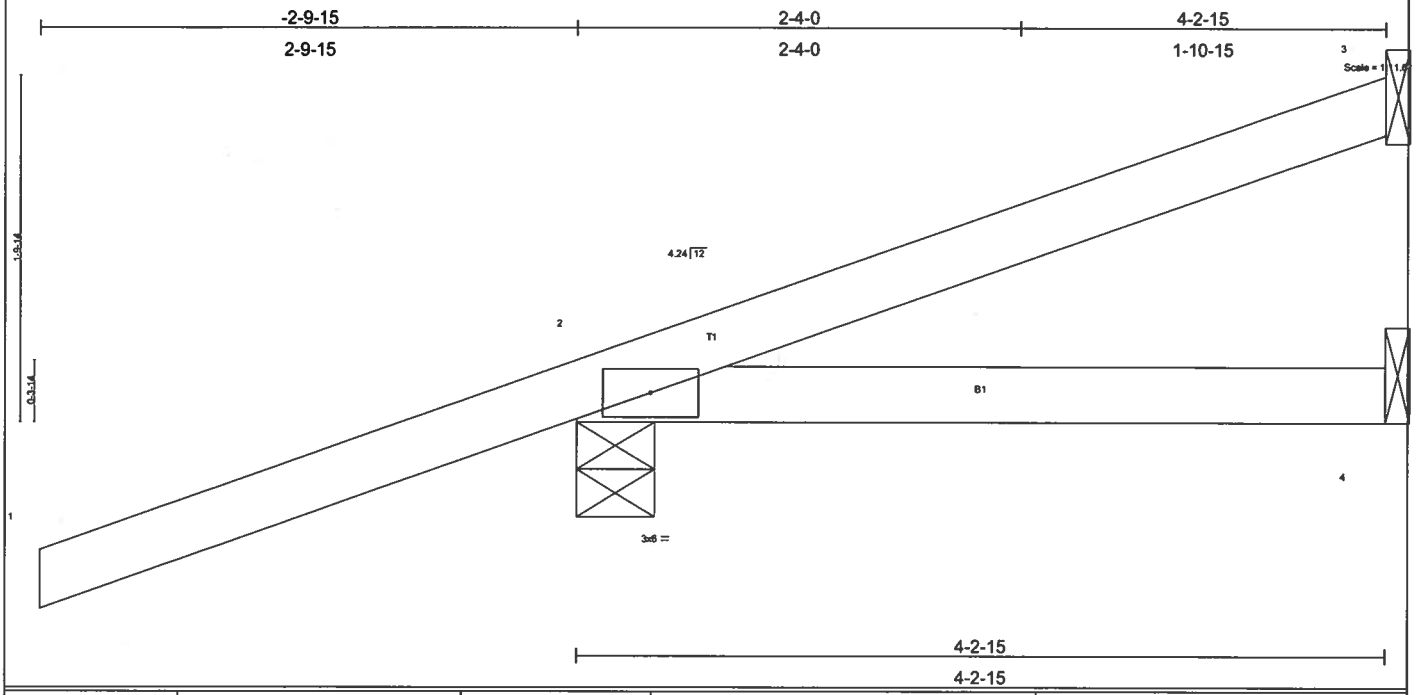
LOAD CASE(S) Standard

FEBRUARY 23,2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B LUTZ, FL 33549

Dwg.#0223061735

Job	Truss	Truss Type	Qty	Ply	ST JOHNS WITH PORCH
MASTER	HJ4	ROOF TRUSS	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Feb 23 15:36:07 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.53	Vert(LL)	0.02	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.07	Vert(TL)	0.01	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 18 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-2-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=15/Mechanical, 2=289/0-4-15, 4=42/Mechanical
Max Horz 2=98(load case 2)
Max Uplift3=-6(load case 5), 2=-302(load case 2), 4=-41(load case 2)
Max Grav 3=32(load case 6), 2=289(load case 1), 4=42(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/50, 2-3=-37/10
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.11

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 6 lb uplift at joint 3, 302 lb uplift at joint 2 and 41 lb uplift at joint 4.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

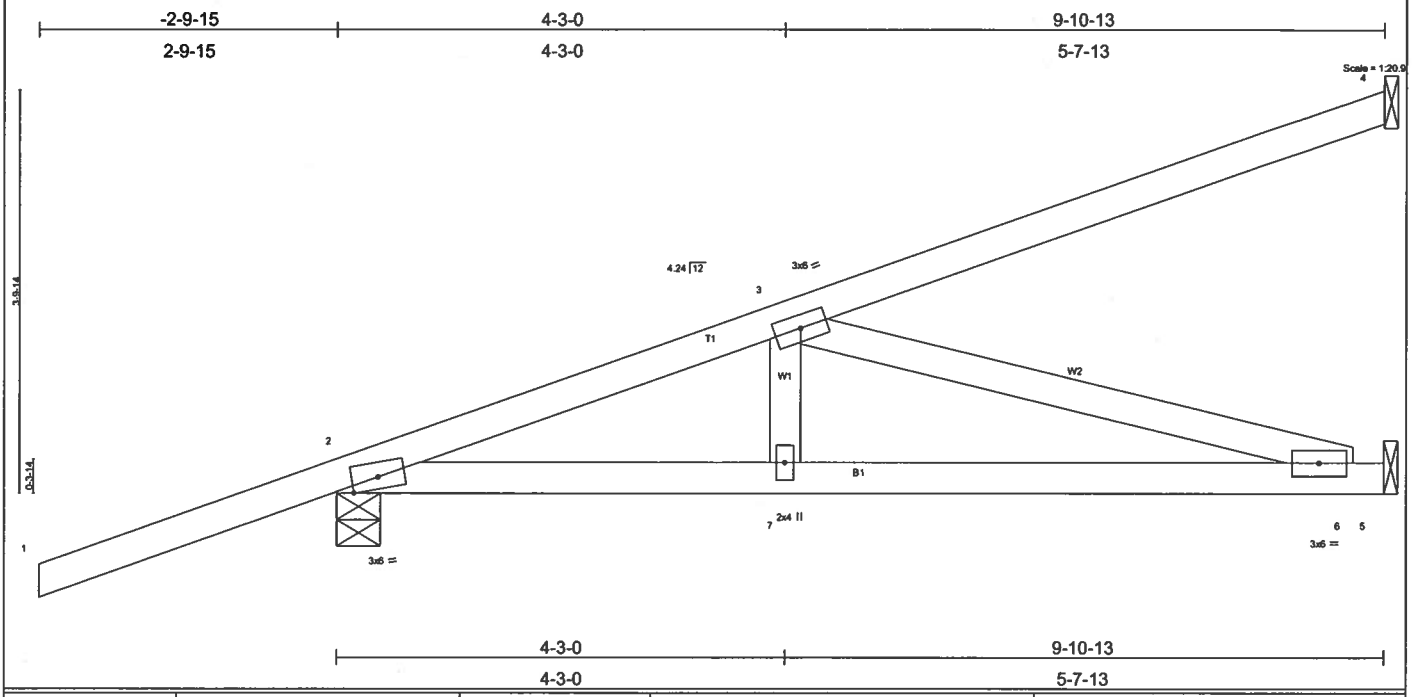
LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=26, B=26)-to-3=-57(F=-2, B=-2), 2=-0(F=15, B=15)-to-4=-32(F=-1, B=-1)

FEBRUARY 23,2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0223061736

Job	Truss	Truss Type	Qty	Ply	ST JOHNS WITH PORCH
MASTER	HJ9	ROOF TRUSS	7	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Feb 23 15:36:08 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.61	Vert(LL)	-0.10	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.57	Vert(TL)	-0.17	6-7	>685	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.49	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 45 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-11-7 oc bracing.

REACTIONS (lb/size) 4=269/Mechanical, 2=532/0-4-15, 5=377/Mechanical
Max Horz 2=269(load case 2)
Max Uplift4=-233(load case 2), 2=-399(load case 2), 5=-183(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/50, 2-3=-889/365, 3-4=-105/66
BOT CHORD 2-7=-538/824, 6-7=-538/824, 5-6=0/0
WEBS 3-7=-89/180, 3-6=-857/559

JOINT STRESS INDEX
2 = 0.77, 3 = 0.23, 6 = 0.24 and 7 = 0.13

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 233 lb uplift at joint 4, 399 lb uplift at joint 2 and 183 lb uplift at joint 5.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=26, B=26)-to-4=-134(F=-40, B=-40), 2=-0(F=15, B=15)-to-5=-74(F=-22, B=-22)

FEBRUARY 23,2006 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0223061737

Job	Truss	Truss Type	Qty	Ply	ST JOHNS WITH PORCH
MASTER	T01	ROOF TRUSS	1	1	Job Reference (optional)

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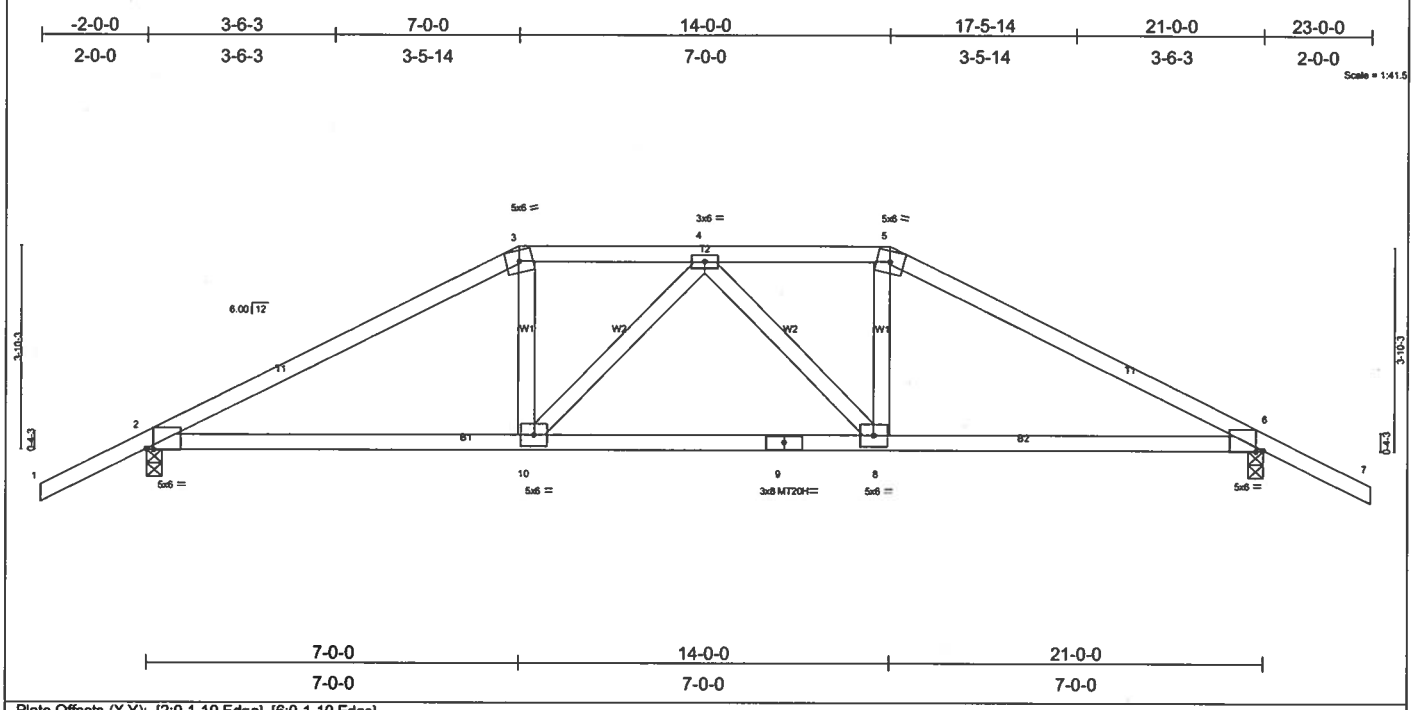

Plate Offsets (X,Y): [2:0-1-10,Edge], [6:0-1-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.52	Vert(LL)	-0.22	8-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.94	Vert(TL)	-0.37	8-10	>669	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.36	Horz(TL)	0.10	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 95 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-13 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-7-8 oc bracing.

REACTIONS (lb/size) 2=1866/0-3-8, 6=1866/0-3-8
Max Horz 2=-87(load case 5)
Max Uplift2=-842(load case 4), 6=-842(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-3339/1333, 3-4=-2937/1265, 4-5=-2937/1265, 5-6=-3339/1333, 6-7=0/47
BOT CHORD 2-10=-1122/2894, 9-10=-1267/3085, 8-9=-1267/3085, 6-8=-1081/2894
WEBS 3-10=-405/1125, 4-8=-334/283, 5-8=-405/1125, 4-10=-334/283

JOINT STRESS INDEX
2 = 0.84, 3 = 0.74, 4 = 0.37, 5 = 0.74, 6 = 0.84, 8 = 0.40, 9 = 0.91 and 10 = 0.40

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 842 lb uplift at joint 2 and 842 lb uplift at joint 6.
6) Girder carries hip end with 7-0-0 end setback.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 14-0-0, and 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-5=-118(F=-64), 5-7=-54, 2-10=-30, 8-10=-65(F=-35), 6-8=-30
Concentrated Loads (lb)
Vert: 10=-539(F) 8=-539(F)

FEBRUARY 23,2006 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0223061738

Job	Truss	Truss Type	Qty	Ply	ST JOHNS WITH PORCH
MASTER	T02	ROOF TRUSS	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Feb 23 15:36:11 2006 Page 1

-2-0-0	4-7-15	9-0-0	12-0-0	16-4-1	21-0-0	23-0-0
2-0-0	4-7-15	4-4-1	3-0-0	4-4-1	4-7-15	2-0-0

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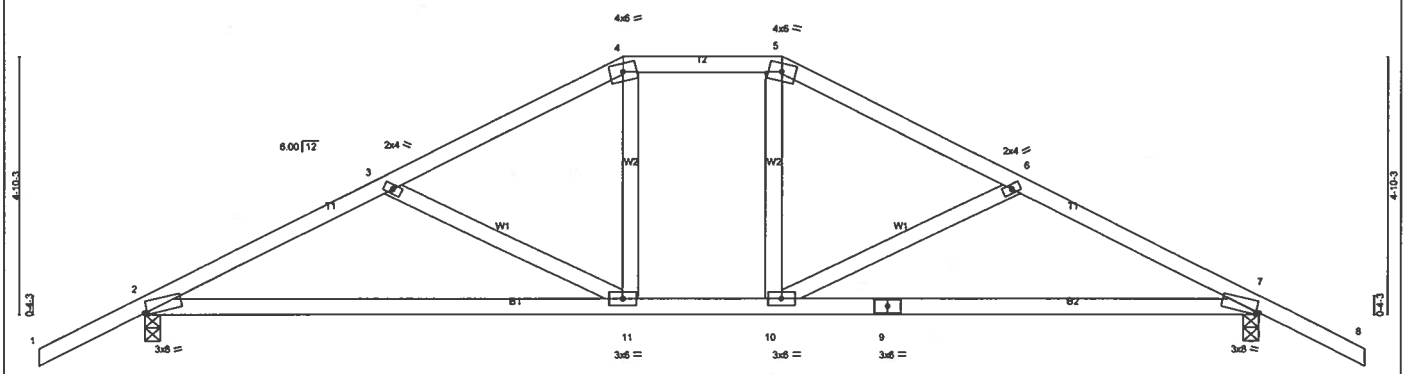

9-0-0	12-0-0	21-0-0
9-0-0	3-0-0	9-0-0

Plate Offsets (X,Y): [2:0-0-10,Edge], [7:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.21	7-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.54	Vert(TL)	-0.33	7-10	>761	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.15	Horz(TL)	0.04	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 100 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=986/0-3-8, 7=986/0-3-8
Max Horz 2=-101(load case 6)
Max Uplift2=-400(load case 5), 7=-400(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1422/603, 3-4=-1156/484, 4-5=-990/485, 5-6=-1156/484, 6-7=-1422/603, 7-8=0/47
BOT CHORD 2-11=-378/1233, 10-11=-176/990, 9-10=-378/1233, 7-9=-378/1233
WEBS 3-11=-319/227, 4-11=-58/299, 5-10=-58/299, 6-10=-319/227

JOINT STRESS INDEX
2 = 0.87, 3 = 0.34, 4 = 0.39, 5 = 0.39, 6 = 0.34, 7 = 0.87, 9 = 0.59, 10 = 0.35 and 11 = 0.35

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 400 lb uplift at joint 2 and 400 lb uplift at joint 7.

LOAD CASE(S) Standard

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THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0223061739

Job	Truss	Truss Type	Qty	Ply	ST JOHNS WITH PORCH
MASTER	T03	ROOF TRUSS	6	1	Job Reference (optional)

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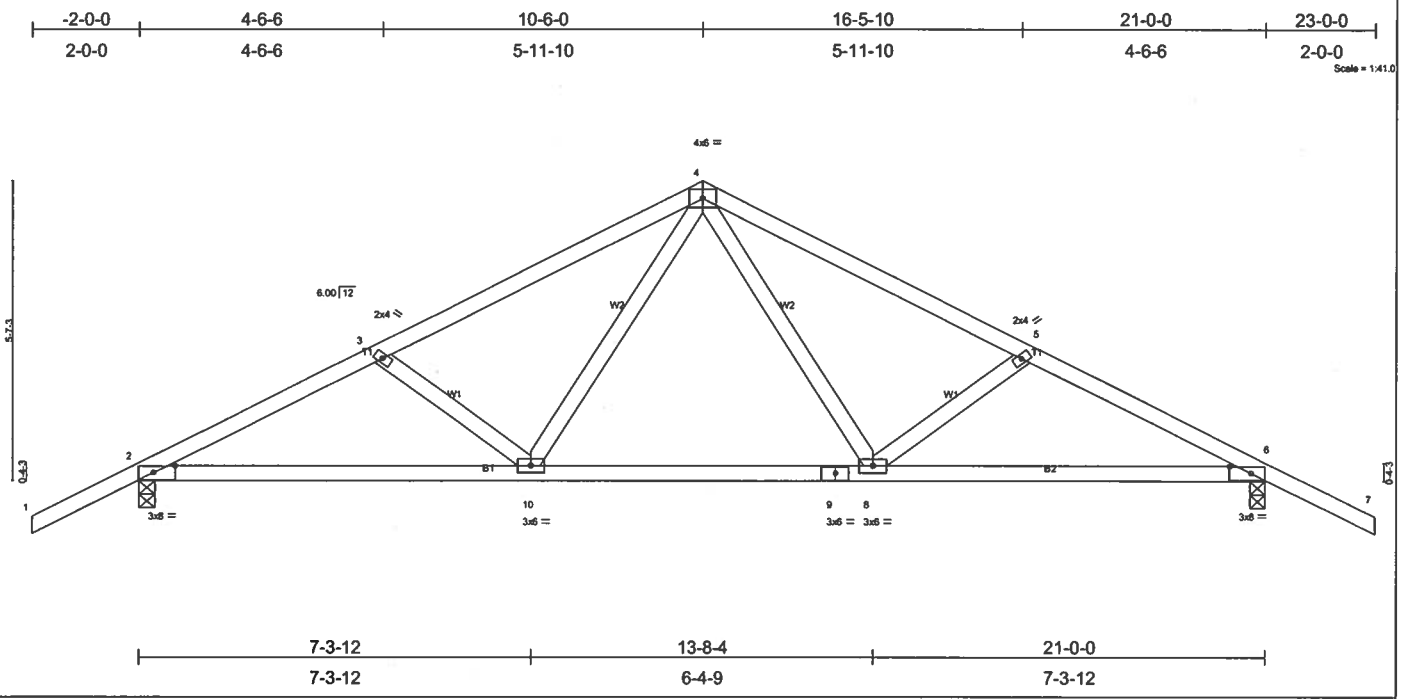

Plate Offsets (X,Y): [2:0-4-12,0-1-8], [6:0-4-12,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.36	Vert(LL)	-0.21	8-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.84	Vert(TL)	-0.33	8-10	>747	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.23	Horz(TL)	0.05	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 101 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-9-0 oc bracing.

REACTIONS (lb/size) 2=1209/0-3-8, 6=1209/0-3-8
Max Horz 2=112(load case 5)
Max Uplift2=-493(load case 5), 6=-493(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1993/889, 3-4=-1790/816, 4-5=-1790/816, 5-6=-1993/889, 6-7=0/47
BOT CHORD 2-10=-635/1719, 9-10=-329/1186, 8-9=-329/1186, 6-8=-635/1719
WEBS 3-10=-252/223, 4-10=-251/705, 4-8=-251/705, 5-8=-252/223

JOINT STRESS INDEX
2 = 0.75, 3 = 0.34, 4 = 0.85, 5 = 0.34, 6 = 0.75, 8 = 0.55, 9 = 0.51 and 10 = 0.55

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 493 lb uplift at joint 2 and 493 lb uplift at joint 6.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (pif)
Vert: 1-4=-54, 4-7=-54, 2-10=-30, 8-10=-100(F=-70), 6-8=-30

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THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0223061740

Job •	Truss	Truss Type	Qty	Ply	ST JOHNS WITH PORCH
MASTER	T04	ROOF TRUSS	1	1	Job Reference (optional)

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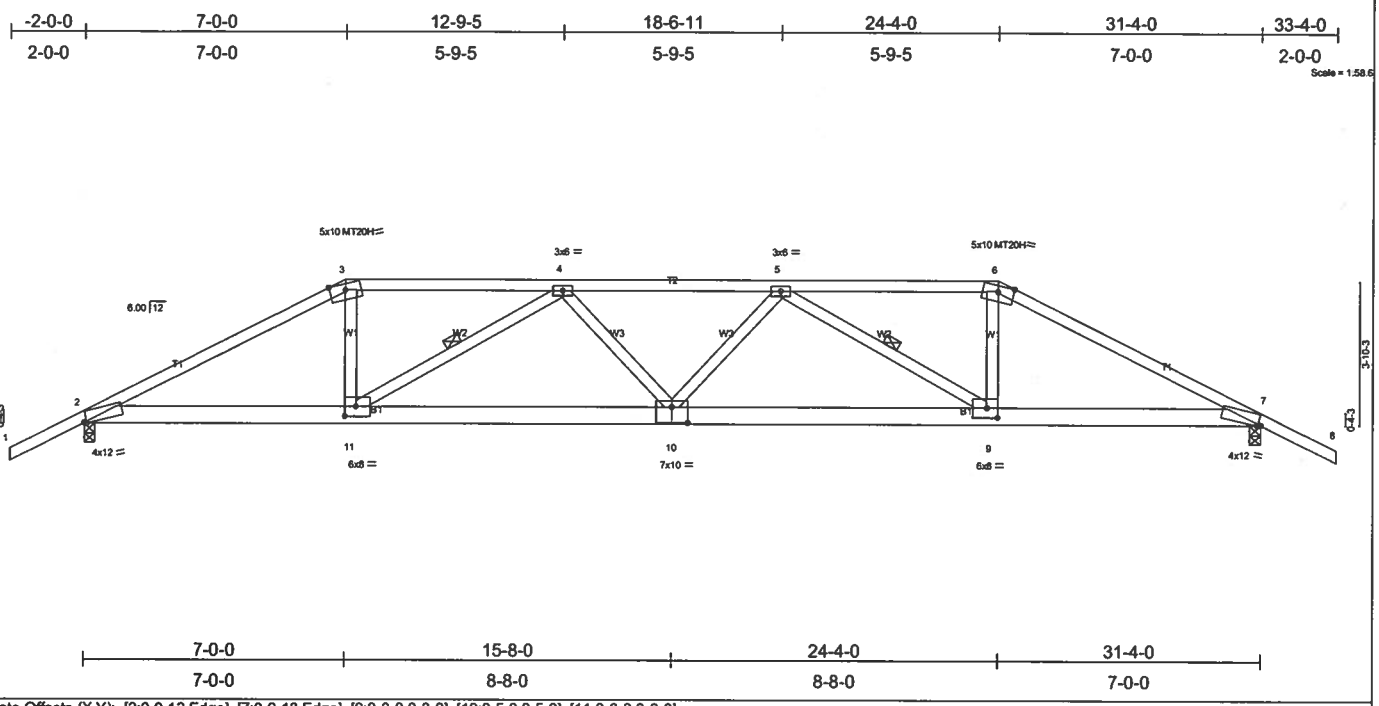

Plate Offsets (X,Y): [2:0-0-13,Edge], [7:0-0-13,Edge], [9:0-3-8,0-3-0], [10:0-5-0,0-5-0], [11:0-3-8,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.82	Vert(LL)	-0.43	9-10	>863	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.85	Vert(TL)	-0.70	9-10	>536	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.62	Horz(TL)	0.17	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 170 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2 *Except*
T2 2 X 4 SYP No.1D
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-1-14 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-11-13 oc bracing.
WEBS 1 Row at midpt 4-11, 5-9

REACTIONS (lb/size) 2=2810/0-3-8, 7=2810/0-3-8
Max Horz 2=-89(load case 5)
Max Uplift2=-1201(load case 4), 7=-1201(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/51, 2-3=-5499/2292, 3-4=-4920/2118, 4-5=-6766/2861, 5-6=-4920/2119, 6-7=-5499/2292, 7-8=0/51
BOT CHORD 2-11=-2008/4839, 10-11=-2837/6515, 9-10=-2815/6515, 7-9=-1969/4839
WEBS 3-11=-709/1946, 4-11=-1969/1021, 4-10=0/423, 5-10=0/423, 5-9=-1969/1021, 6-9=-709/1946

JOINT STRESS INDEX
2 = 0.82, 3 = 0.91, 4 = 0.57, 5 = 0.57, 6 = 0.91, 7 = 0.82, 9 = 0.54, 10 = 0.98 and 11 = 0.54

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1201 lb uplift at joint 2 and 1201 lb uplift at joint 7.
6) Girder carries hip end with 7-0-0 end setback.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 24-4-0, and 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-6=-118(F=-64), 6-8=-54, 2-11=-30, 9-11=-65(F=-35), 7-9=-30
Concentrated Loads (lb)
Vert: 11=-539(F) 9=-539(F)

FEBRUARY 23,2006 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0223061741

Job	Truss	Truss Type	Qty	Ply	ST JOHNS WITH PORCH
MASTER	T05	ROOF TRUSS	1	1	Job Reference (optional)

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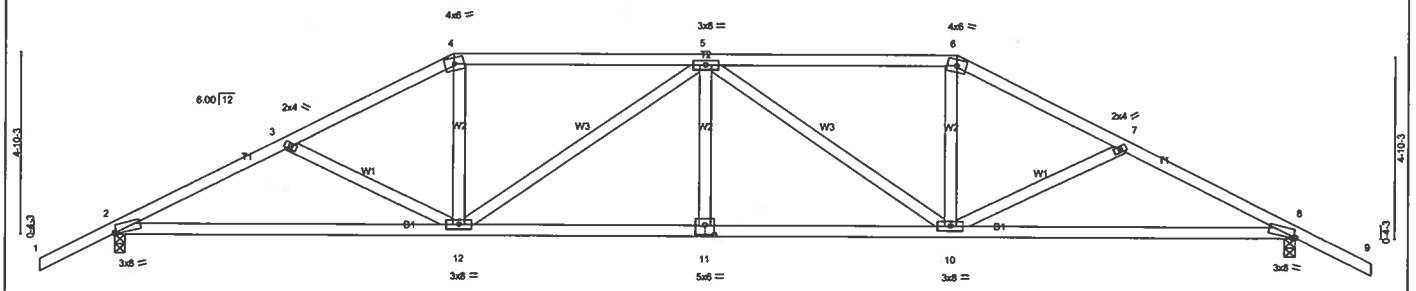
-2-0-0	4-7-15	9-0-0	15-8-0	22-4-0	26-8-1	31-4-0	33-4-0
2-0-0	4-7-15	4-4-1	6-8-0	6-8-0	4-4-1	4-7-15	2-0-0

Scale = 1:58.6



9-0-0	15-8-0	22-4-0	31-4-0
9-0-0	6-8-0	6-8-0	9-0-0

Plate Offsets (X,Y): [2:0-0-10,Edge], [8:0-0-10,Edge], [11:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.32	Vert(LL)	-0.21	8-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.71	Vert(TL)	-0.35	8-10	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.61	Horz(TL)	0.11	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 160 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-10-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-6-3 oc bracing.

REACTIONS (lb/size) 2=1420/0-3-8, 8=1420/0-3-8
Max Horz 2=101(load case 5)
Max Uplift2=-500(load case 5), 8=-500(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2344/959, 3-4=-2118/860, 4-5=-1872/832, 5-6=-1872/832, 6-7=-2118/860, 7-8=-2344/959, 8-9=0/47
BOT CHORD 2-12=-690/2044, 11-12=-690/2252, 10-11=-690/2252, 8-10=-690/2044
WEBS 3-12=-219/193, 4-12=-142/615, 5-12=-556/252, 5-11=0/159, 5-10=-556/252, 6-10=-142/615, 7-10=-219/193

JOINT STRESS INDEX
2 = 0.85, 3 = 0.34, 4 = 0.73, 5 = 0.57, 6 = 0.73, 7 = 0.34, 8 = 0.85, 10 = 0.57, 11 = 0.54 and 12 = 0.57

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 500 lb uplift at joint 2 and 500 lb uplift at joint 8.

LOAD CASE(S) Standard

FEBRUARY 23,2006 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

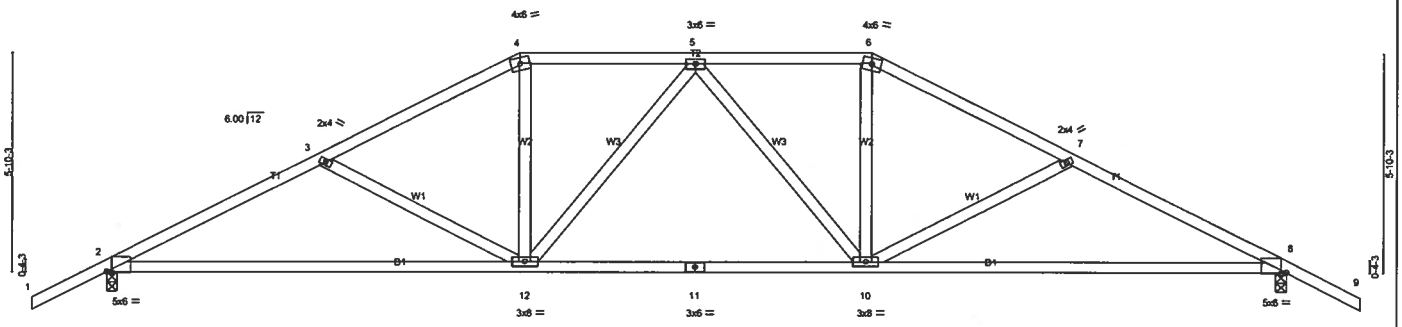
Dwg.#0223061742

Job	Truss	Truss Type	Qty	Ply	ST JOHNS WITH PORCH
MASTER	T06	ROOF TRUSS	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Feb 23 15:36:16 2006 Page 1

-2-0-0	5-9-15	11-0-0	15-8-0	20-4-0	25-6-1	31-4-0	33-4-0
2-0-0	5-9-15	5-2-2	4-8-0	4-8-0	5-2-2	5-9-15	2-0-0

Scale = 1:58.6



11-0-0	20-4-0	31-4-0
11-0-0	9-4-0	11-0-0

Plate Offsets (X,Y): [2:0-1-11,Edge], [8:0-1-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.43	Vert(LL)	-0.37	8-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.80	Vert(TL)	-0.63	8-10	>587	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.24	Horz(TL)	0.10	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 158 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-7-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-6-2 oc bracing.

REACTIONS (lb/size) 2=1420/0-3-8, 8=1420/0-3-8
Max Horz 2=-115(load case 6)
Max Uplift2=-517(load case 5), 8=-517(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2287/974, 3-4=-1965/827, 4-5=-1710/803, 5-6=-1710/803, 6-7=-1965/827, 7-8=-2287/974, 8-9=0/47
BOT CHORD 2-12=-694/1998, 11-12=-502/1805, 10-11=-502/1805, 8-10=-694/1998
WEBS 3-12=-348/289, 4-12=-144/583, 5-12=-265/166, 5-10=-265/166, 6-10=-144/583, 7-10=-348/289

JOINT STRESS INDEX
2 = 0.77, 3 = 0.34, 4 = 0.63, 5 = 0.40, 6 = 0.63, 7 = 0.34, 8 = 0.77, 10 = 0.57, 11 = 0.69 and 12 = 0.57

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 517 lb uplift at joint 2 and 517 lb uplift at joint 8.

LOAD CASE(S) Standard

FEBRUARY 23,2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0223061743

Job	Truss	Truss Type	Qty	Ply	ST JOHNS WITH PORCH
MASTER	T07	ROOF TRUSS	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Feb 23 15:36:17 2006 Page 1

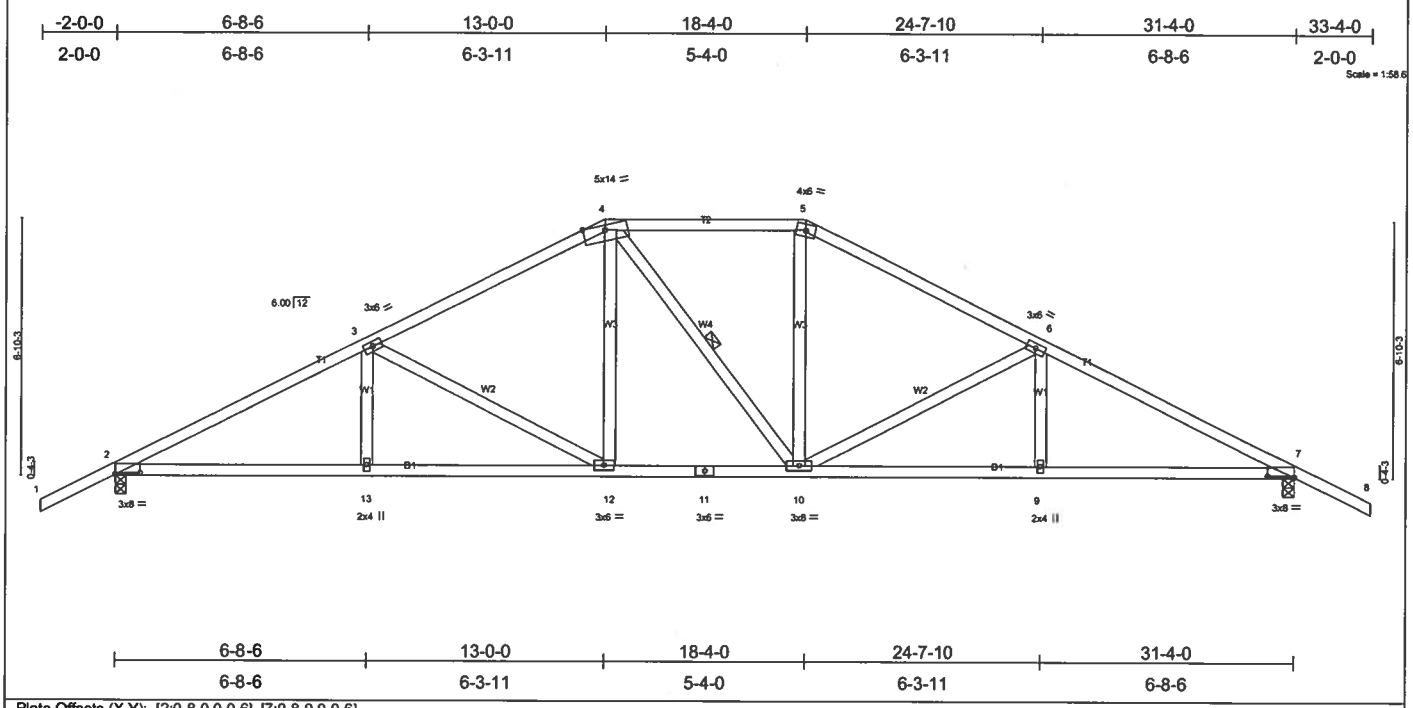


Plate Offsets (X,Y): [2:0-8-0,0-0-6], [7:0-8-0,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.33	Vert(LL)	-0.16	12-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.49	Vert(TL)	-0.25	12-13	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.56	Horz(TL)	0.10	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 165 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-9-10 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-8-11 oc bracing.
WEBS 1 Row at midpt 4-10

REACTIONS (lb/size) 2=1420/0-3-8, 7=1420/0-3-8
Max Horz 2=129(load case 5)
Max Uplift2=-532(load case 5), 7=-532(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2375/950, 3-4=-1765/802, 4-5=-1517/786, 5-6=-1766/802, 6-7=-2375/950, 7-8=0/47
BOT CHORD 2-13=-668/2043, 12-13=-668/2043, 11-12=-374/1516, 10-11=-374/1516, 9-10=-668/2043, 7-9=-668/2043
WEBS 3-13=0/216, 3-12=-607/335, 4-12=-124/448, 4-10=-151/154, 5-10=-124/448, 6-10=-606/335, 6-9=0/216

JOINT STRESS INDEX
2 = 0.75, 3 = 0.41, 4 = 0.78, 5 = 0.70, 6 = 0.41, 7 = 0.75, 9 = 0.34, 10 = 0.58, 11 = 0.53, 12 = 0.35 and 13 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 532 lb uplift at joint 2 and 532 lb uplift at joint 7.

LOAD CASE(S) Standard

FEBRUARY 23,2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0223061744

Job	Truss	Truss Type	Qty	Ply	ST JOHNS WITH PORCH
MASTER	T08	ROOF TRUSS	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Feb 23 15:36:19 2006 Page 1

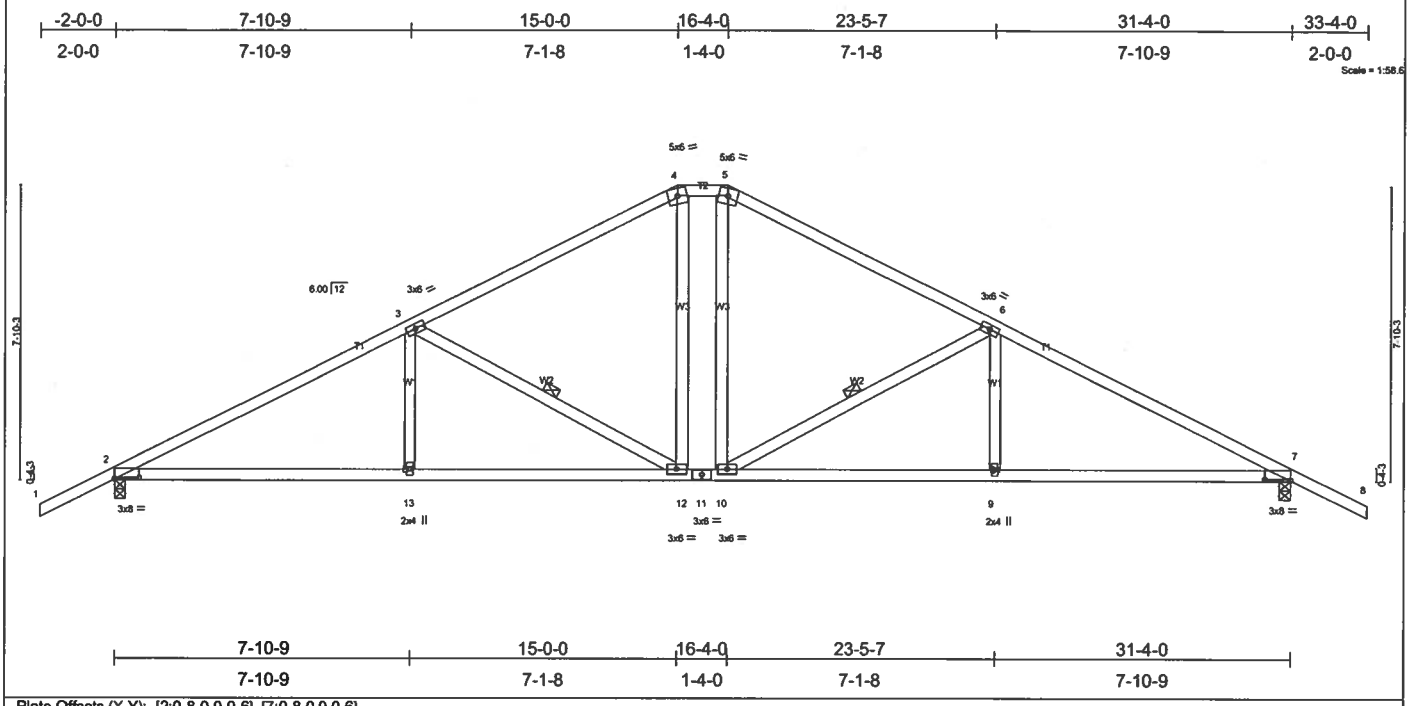


Plate Offsets (X,Y): [2:0-8-0,0-0-6], [7:0-8-0,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.46	Vert(LL)	-0.18	9-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.66	Vert(TL)	-0.29	2-13	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.26	Horz(TL)	0.10	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 162 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-7-1 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-9-2 oc bracing.
WEBS 1 Row at midpt 3-12, 6-10

REACTIONS (lb/size) 2=1420/0-3-8, 7=1420/0-3-8
Max Horz 2=-143(load case 6)
Max Uplift2=-544(load case 5), 7=-544(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2319/949, 3-4=-1600/766, 4-5=-1356/762, 5-6=-1600/766, 6-7=-2319/949, 7-8=0/47
BOT CHORD 2-13=-651/1986, 12-13=-651/1986, 11-12=-300/1356, 10-11=-300/1356, 9-10=-651/1986, 7-9=-651/1986
WEBS 3-13=0/276, 3-12=-762/405, 4-12=-171/464, 5-10=-171/464, 6-10=-762/405, 6-9=0/276

JOINT STRESS INDEX
2 = 0.74, 3 = 0.41, 4 = 0.60, 5 = 0.60, 6 = 0.41, 7 = 0.74, 9 = 0.34, 10 = 0.35, 11 = 0.63, 12 = 0.35 and 13 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 544 lb uplift at joint 2 and 544 lb uplift at joint 7.

LOAD CASE(S) Standard

FEBRUARY 23,2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0223061745

Job	Truss	Truss Type	Qty	Ply	ST JOHNS WITH PORCH
MASTER	T09	ROOF TRUSS	3	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Feb 23 15:36:20 2006 Page 1

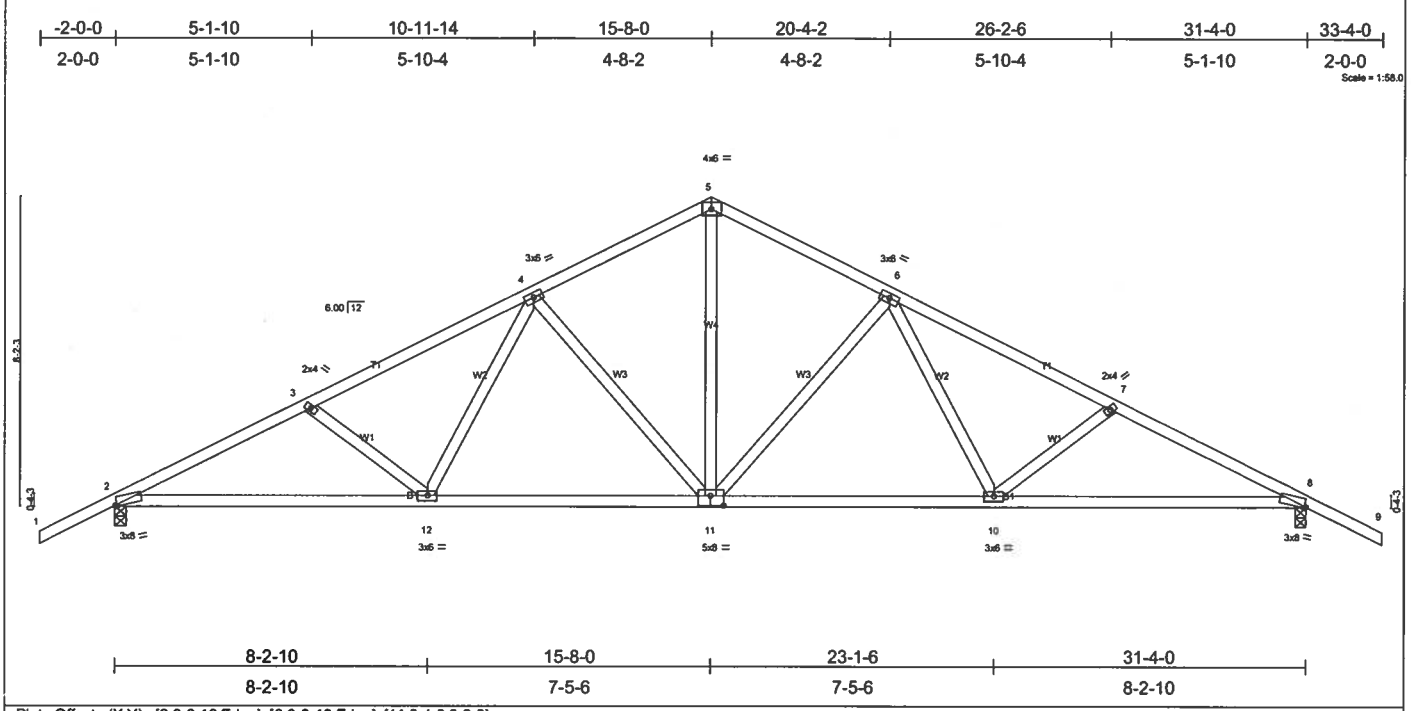


Plate Offsets (X,Y): [2:0-0-10,Edge], [8:0-0-10,Edge], [11:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.31	Vert(LL)	-0.17	11-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.55	Vert(TL)	-0.28	11-12	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.56	Horz(TL)	0.09	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 167 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-11-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-2-13 oc bracing.

REACTIONS (lb/size) 2=1420/0-3-8, 8=1420/0-3-8
Max Horz 2=-148(load case 6)
Max Uplift2=-548(load case 5), 8=-548(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2372/1019, 3-4=-2150/933, 4-5=-1511/778, 5-6=-1511/778, 6-7=-2150/933, 7-8=-2372/1019, 8-9=0/47
BOT CHORD 2-12=-746/2069, 11-12=-493/1654, 10-11=-493/1654, 8-10=-746/2069
WEBS 3-12=-275/260, 4-12=-111/461, 4-11=-564/357, 5-11=-512/1062, 6-11=-564/357, 6-10=-111/461, 7-10=-275/260

JOINT STRESS INDEX
2 = 0.80, 3 = 0.34, 4 = 0.42, 5 = 0.51, 6 = 0.42, 7 = 0.34, 8 = 0.80, 10 = 0.46, 11 = 0.47 and 12 = 0.46

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 548 lb uplift at joint 2 and 548 lb uplift at joint 8.

LOAD CASE(S) Standard

FEBRUARY 23,2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B LUTZ, FL 33549

Dwg.#0223061746

Job	Truss	Truss Type	Qty	Ply	ST JOHNS WITH PORCH
MASTER	T10	ROOF TRUSS	4	1	Job Reference (optional)

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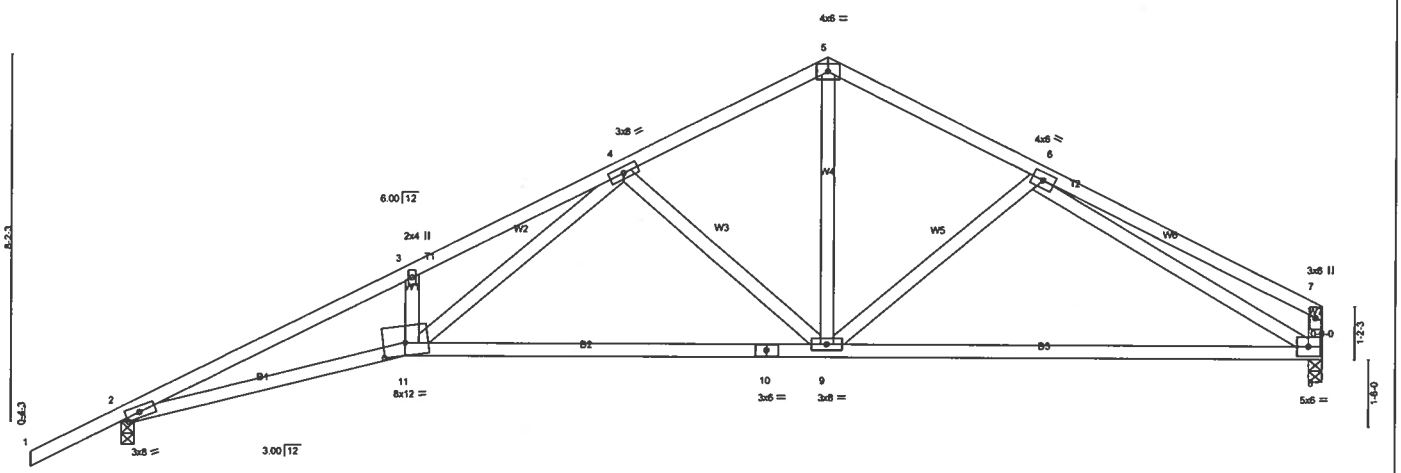
-2-0-0	6-3-8	6-5-4	11-1-8	15-8-0	20-3-13	26-4-0
2-0-0	6-3-8	0-1-12	4-8-4	4-6-8	4-7-13	6-0-3

Scale = 1:48.9



6-3-8	15-8-0	26-4-0	26-6-4
6-3-8	9-4-8	10-8-0	0-2-4

Plate Offsets (X,Y): [2:0-3-12,Edge], [11:0-6-0,0-3-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.55	Vert(LL)	-0.36	9-11	>877	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.74	Vert(TL)	-0.59	9-11	>536	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.87	Horz(TL)	0.19	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 137 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W7 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-11-2 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-4-0 oc bracing.

REACTIONS (lb/size) 2=1228/0-3-8, 8=1103/0-3-8
Max Horz 2=250(load case 5)
Max Uplift2=-493(load case 5), 8=-341(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-3625/1476, 3-4=-3582/1638, 4-5=-1355/665, 5-6=-1359/666, 6-7=-654/246, 7-8=-434/238
BOT CHORD 2-11=-1376/3261, 10-11=-709/1706, 9-10=-709/1706, 8-9=-547/1285
WEBS 3-11=-225/272, 4-11=-846/1902, 4-9=-733/460, 5-9=-397/916, 6-9=-214/233, 6-8=-922/499

JOINT STRESS INDEX
2 = 0.83, 3 = 0.34, 4 = 0.97, 5 = 0.44, 6 = 0.29, 7 = 0.67, 8 = 0.68, 9 = 0.57, 10 = 0.77 and 11 = 0.83

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 493 lb uplift at joint 2 and 341 lb uplift at joint 8.

LOAD CASE(S) Standard

FEBRUARY 23,2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

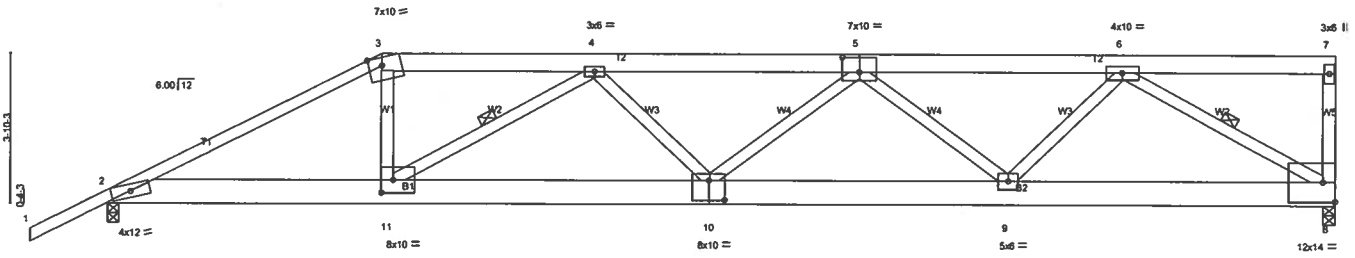
Dwg.#0223061747

Job	Truss	Truss Type	Qty	Ply	ST JOHNS WITH PORCH
MASTER	T11	ROOF TRUSS	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Feb 23 15:36:22 2006 Page 1

-2-0-0	3-5-6	7-0-0	12-5-3	19-2-0	25-10-13	31-4-0
2-0-0	3-5-6	3-6-10	5-5-3	6-8-13	6-8-13	5-5-3

Scale = 1:56.3



7-0-0	15-4-2	22-11-14	31-4-0
7-0-0	8-4-2	7-7-11	8-4-2

Plate Offsets (X,Y): [3:0-4-2,Edge], [5:0-5-0,0-4-8], [10:0-5-0,0-6-0], [11:0-3-8,0-4-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.74	Vert(LL)	-0.32	10-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.39	Vert(TL)	-0.51	10-11	>727	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.99	Horz(TL)	0.11	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 220 lb	

LUMBER
TOP CHORD 2 X 6 SYP No.1D *Except* T1 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3 *Except* W5 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-3-1 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-10-5 oc bracing.
WEBS 1 Row at midpt 4-11, 6-8

REACTIONS (lb/size) 8=2880/0-3-8, 2=2769/0-3-8
Max Horz 2=223(load case 4)
Max Uplift8=-1300(load case 5), 2=-1174(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=-5492/2346, 3-4=-4948/2181, 4-5=-6793/2969, 5-6=-5222/2250, 6-7=-121/45, 7-8=-293/218
BOT CHORD 2-11=-2148/4863, 10-11=-3033/6554, 9-10=-2995/6487, 8-9=-1806/3809
WEBS 3-11=-726/1960, 4-11=-2003/1021, 4-10=0/390, 5-10=0/446, 5-9=-1675/986, 6-9=-673/2140, 6-8=-4353/2079

JOINT STRESS INDEX
2 = 0.83, 3 = 0.99, 4 = 0.58, 5 = 0.51, 6 = 0.98, 7 = 0.58, 8 = 0.72, 9 = 0.78, 10 = 0.86 and 11 = 0.41

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1300 lb uplift at joint 8 and 1174 lb uplift at joint 2.
5) Girder carries hip end with 0-0-0 right side setback, 7-0-0 left side setback, and 7-0-0 end setback.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-7=-118(F=-64), 2-11=-30, 8-11=-65(F=-35)
Concentrated Loads (lb)
Vert: 11=-539(F)

FEBRUARY 23,2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16196 N. FLORIDA AVE. STE B, LUTZ, FL 33548

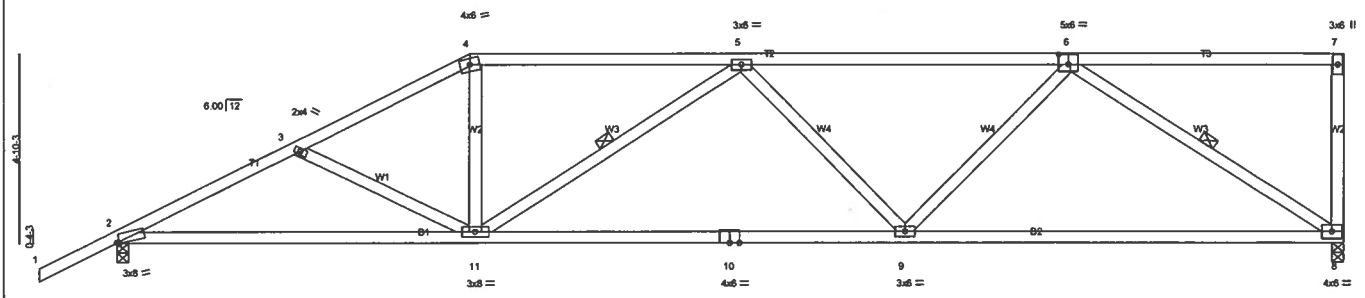
Dwg.#0223061748

Job	Truss	Truss Type	Qty	Ply	ST JOHNS WITH PORCH
MASTER	T12	ROOF TRUSS	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Feb 23 15:36:23 2006 Page 1

-2-0-0	4-7-15	9-0-0	15-11-9	24-4-7	31-4-0
2-0-0	4-7-15	4-4-1	6-11-9	8-4-14	6-11-9

Scale = 1:56.3



9-0-0	20-2-1	31-4-0
9-0-0	11-2-1	11-1-15

Plate Offsets (X,Y): [2:0-0-10,Edge], [6:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.85	Vert(LL)	-0.32	9-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.81	Vert(TL)	-0.54	9-11	>687	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.60	Horz(TL)	0.10	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 160 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-10-8 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-4-5 oc bracing.
WEBS 1 Row at midpt 5-11, 6-8

REACTIONS (lb/size) 8=1300/0-3-8, 2=1424/0-3-8
Max Horz 2=272(load case 5)
Max Uplift8=-478(load case 4), 2=-491(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2366/924, 3-4=-2146/824, 4-5=-1897/798, 5-6=-2052/785, 6-7=-84/9, 7-8=-166/110
BOT CHORD 2-11=-969/2061, 10-11=-965/2240, 9-10=-965/2240, 8-9=-671/1546
WEBS 3-11=-205/195, 4-11=-130/631, 5-11=-413/301, 5-9=-278/265, 6-9=-168/747, 6-8=-1758/797

JOINT STRESS INDEX
2 = 0.80, 3 = 0.34, 4 = 0.71, 5 = 0.37, 6 = 0.70, 7 = 0.45, 8 = 0.74, 9 = 0.48, 10 = 0.90 and 11 = 0.57

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 478 lb uplift at joint 8 and 491 lb uplift at joint 2.

LOAD CASE(S) Standard

FEBRUARY 23,2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B LUTZ, FL 33549

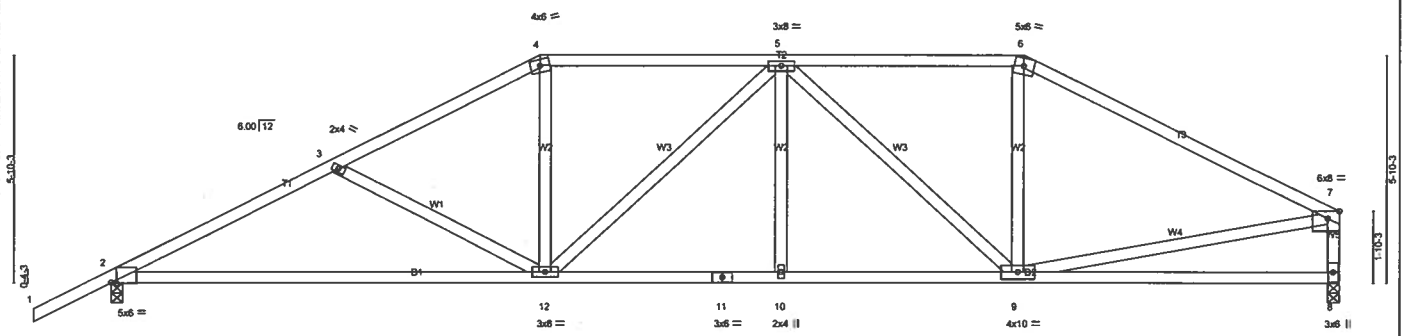
Dwg.#0223061749

Job	Truss	Truss Type	Qty	Ply	ST JOHNS WITH PORCH
MASTER	T13	ROOF TRUSS	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Feb 23 15:36:25 2006 Page 1

-2-0-0	5-9-15	11-0-0	17-2-0	23-4-0	28-6-1	31-4-0
2-0-0	5-9-15	5-2-2	6-2-0	6-2-0	5-2-1	2-9-15

Scale = 1:56.3



11-0-0	17-2-0	23-4-0	31-4-0
11-0-0	6-2-0	6-2-0	8-0-0

Plate Offsets (X,Y): [2:0-1-11,Edge], [7:0-3-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.54	Vert(LL)	-0.37	2-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.80	Vert(TL)	-0.63	2-12	>592	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.70	Horz(TL)	0.08	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 169 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W5 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-7-2 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-9-7 oc bracing.

REACTIONS (lb/size) 2=1424/0-3-8, 8=1300/0-3-8
Max Horz 2=192(load case 5)
Max Uplift2=-518(load case 5), 8=-364(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2292/973, 3-4=-1978/831, 4-5=-1724/809, 5-6=-1451/723, 6-7=-1710/714, 7-8=-1173/556
BOT CHORD 2-12=-853/2002, 11-12=-667/1827, 10-11=-667/1827, 9-10=-667/1827, 8-9=-167/285
WEBS 3-12=-336/280, 4-12=-119/548, 5-12=-263/174, 5-10=0/126, 5-9=-607/241, 6-9=-48/413, 7-9=-362/1180

JOINT STRESS INDEX
2 = 0.76, 3 = 0.34, 4 = 0.75, 5 = 0.57, 6 = 0.76, 7 = 0.66, 8 = 0.61, 9 = 0.52, 10 = 0.34, 11 = 0.63 and 12 = 0.57

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 518 lb uplift at joint 2 and 364 lb uplift at joint 8.

LOAD CASE(S) Standard

FEBRUARY 23,2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

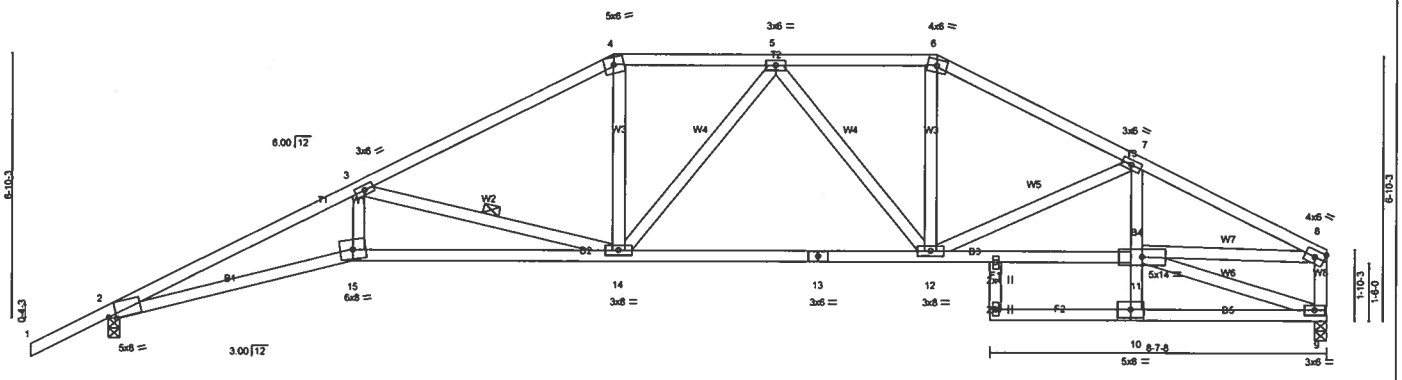
Dwg.#0223061750

Job •	Truss	Truss Type	Qty	Ply	ST JOHNS WITH PORCH
MASTER	T14	ROOF TRUSS	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Feb 23 15:36:26 2006 Page 1

-2-0-0	6-5-4	13-0-0	17-2-0	21-4-0	26-4-0	31-4-0
2-0-0	6-5-4	6-6-12	4-2-0	4-2-0	5-0-0	5-0-0

Scale = 1:56.7



6-3-8	13-0-0	21-4-0	26-4-0	31-4-0
6-3-8	6-8-8	8-4-0	5-0-0	5-0-0

Plate Offsets (X,Y): [2:0-2-7,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.53	Vert(LL)	-0.38	14-15	>979	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.90	Vert(TL)	-0.61	14-15	>609	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.67	Horz(TL)	0.35	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 182 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B4 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3 *Except*
W8 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-7-2 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-11-4 oc bracing.
WEBS 1 Row at midpt 3-14
JOINTS 1 Brace at Jt(s): 11

REACTIONS (lb/size) 2=1424/0-3-8, 9=1300/0-3-8
Max Horz 2=205(load case 5)
Max Uplift2=-532(load case 5), 9=-381(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-4404/1768, 3-4=-2294/977, 4-5=-2003/951, 5-6=-1790/860, 6-7=-2042/893, 7-8=-2553/1079, 8-9=-1238/570
BOT CHORD 2-15=-1602/3970, 14-15=-1526/3742, 13-14=-693/1959, 12-13=-693/1959, 11-12=-913/2273, 10-11=0/64, 7-11=-15/213, 9-10=-18/75
WEBS 3-15=-331/1132, 3-14=-1817/860, 4-14=-172/654, 5-14=-104/144, 5-12=-373/178, 6-12=-182/605, 7-12=-551/341, 8-11=-837/2099, 9-11=-46/56

JOINT STRESS INDEX
2 = 0.85, 3 = 0.84, 4 = 0.62, 5 = 0.40, 6 = 0.61, 7 = 0.59, 8 = 0.74, 9 = 0.65, 10 = 0.18, 11 = 0.51, 12 = 0.57, 13 = 0.81, 14 = 0.83, 15 = 0.96, 16 = 0.34 and 17 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 532 lb uplift at joint 2 and 381 lb uplift at joint 9.

LOAD CASE(S) Standard

FEBRUARY 23,2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16125 N. FLORIDA AVE. STE B, LUTZ, FL 33549

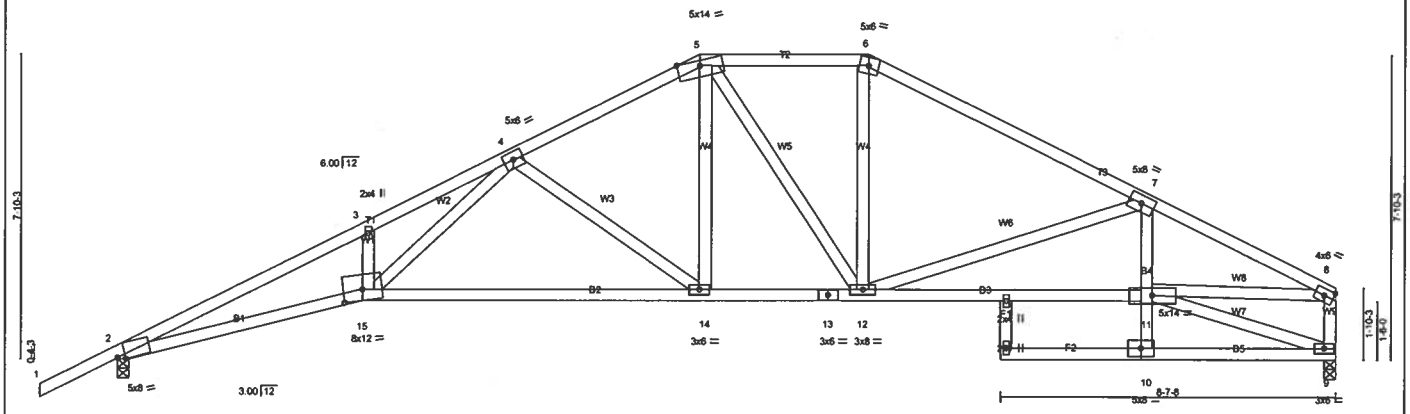
Dwg.#0223061751

Job	Truss	Truss Type	Qty	Ply	ST JOHNS WITH PORCH
MASTER	T15	ROOF TRUSS	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Feb 23 15:36:28 2006 Page 1

-2-0-0 | 6-5-4 | 10-2-6 | 15-0-0 | 19-4-0 | 26-4-0 | 31-4-0
2-0-0 | 6-5-4 | 3-9-2 | 4-9-10 | 4-4-0 | 7-0-0 | 5-0-0

Scale = 1:56.7



6-3-8 | 15-0-0 | 19-4-0 | 26-4-0 | 31-4-0
6-3-8 | 8-8-8 | 4-4-0 | 7-0-0 | 5-0-0

Plate Offsets (X,Y): [2:0-2-7,Edge], [15:0-6-0,0-3-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.72	Vert(LL)	-0.47	14-15	>791	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.87	Vert(TL)	-0.77	14-15	>483	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.83	Horz(TL)	0.34	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 187 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B4 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3 *Except*
W9 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-2-10 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-11-8 oc bracing. Except:
1 Row at midpt 11-12
JOINTS 1 Brace at Jt(s): 11

REACTIONS (lb/size) 2=1424/0-3-8, 9=1300/0-3-8
Max Horz 2=219(load case 5)
Max Uplift2=-544(load case 5), 9=-397(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-4415/1747, 3-4=-4341/1883, 4-5=-1943/895, 5-6=-1622/842, 6-7=-1889/853, 7-8=-2583/1096, 8-9=-1244/568
BOT CHORD 2-15=-1578/3981, 14-15=-956/2396, 13-14=-555/1700, 12-13=-555/1700, 11-12=-970/2372, 10-11=0/62, 7-11=0/262, 9-10=-41/48
WEBS 3-15=-171/230, 4-15=-809/1984, 4-14=-886/505, 5-14=-265/744, 5-12=-288/91, 6-12=-120/487, 7-12=-801/457, 8-11=-868/2135, 9-11=-9/79

JOINT STRESS INDEX
2 = 0.85, 3 = 0.34, 4 = 0.78, 5 = 0.47, 6 = 0.61, 7 = 0.62, 8 = 0.75, 9 = 0.65, 10 = 0.18, 11 = 0.65, 12 = 0.59, 13 = 0.69, 14 = 0.49, 15 = 0.99, 16 = 0.34 and 17 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 544 lb uplift at joint 2 and 397 lb uplift at joint 9.

LOAD CASE(S) Standard

FEBRUARY 23,2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196

Dwg.#0223061752

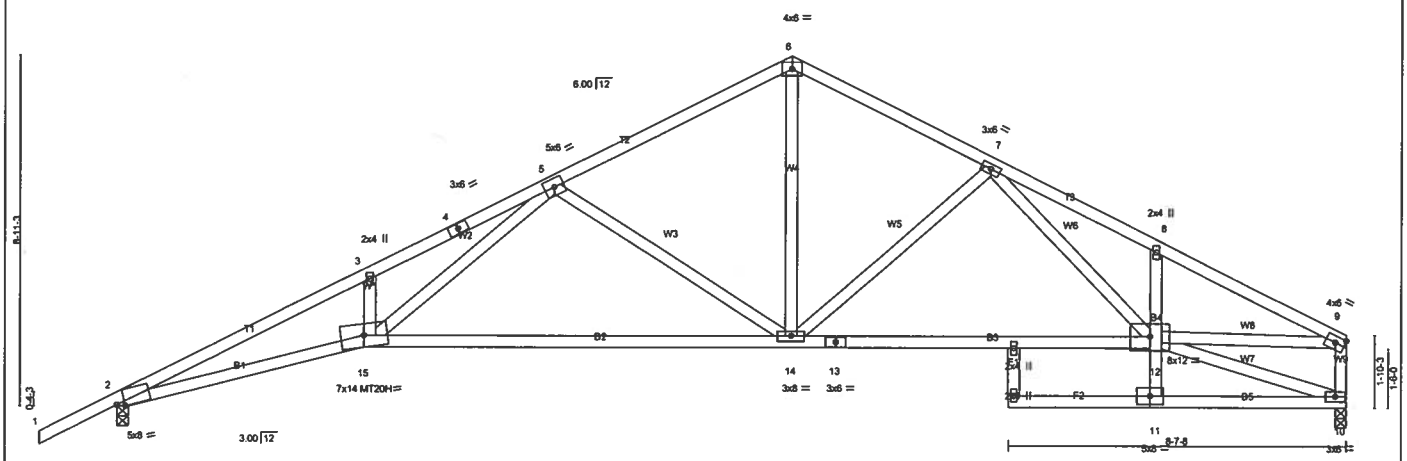
Job	Truss	Truss Type	Qty	Ply	ST JOHNS WITH PORCH
MASTER	T16	ROOF TRUSS	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Feb 23 15:36:29 2006 Page 1

-2-0-0 | 6-3-8 | 6-5-4 | 11-1-8 | 17-2-0 | 22-2-14 | 26-4-0 | 31-4-0

2-0-0 | 6-3-8 | 0-1-12 | 4-8-4 | 6-0-8 | 5-0-14 | 4-1-2 | 5-0-0

Scale = 1:58.1



6-3-8 | 17-2-0 | 26-4-0 | 31-4-0

6-3-8 | 10-10-8 | 9-2-0 | 5-0-0

Plate Offsets (X,Y): [2:0-2-7,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.71	Vert(LL)	-0.60	14-15	>623	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.97	Vert(TL)	-0.99	14-15	>376	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	YES	WB	0.87	Horz(TL)	0.34	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 181 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except* B4 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3 *Except* W9 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-2-12 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.

REACTIONS (lb/size) 2=1424/0-3-8, 10=1300/0-3-8
Max Horz 2=234(load case 5)
Max Uplift2=-555(load case 5), 10=-410(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-4453/1772, 3-4=-4386/1907, 4-5=-4284/1923, 5-6=-1711/819, 6-7=-1694/826, 7-8=-2480/1160, 8-9=-2439/1047, 9-10=-1255/585
BOT CHORD 2-15=-1601/4017, 14-15=-900/2222, 13-14=-691/1796, 12-13=-691/1796, 11-12=0/62, 8-12=-242/235, 10-11=-24/13
WEBS 3-15=-183/251, 5-15=-881/2177, 5-14=-914/552, 6-14=-476/1150, 7-14=-472/332, 7-12=-247/564, 9-12=-804/1990, 10-12=-42/131

JOINT STRESS INDEX
2 = 0.86, 3 = 0.34, 4 = 0.70, 5 = 0.88, 6 = 0.67, 7 = 0.41, 8 = 0.38, 9 = 0.78, 10 = 0.67, 11 = 0.18, 12 = 0.47, 13 = 0.92, 14 = 0.58, 15 = 0.69, 16 = 0.34 and 17 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All plates are MT20 plates unless otherwise indicated.
4) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 555 lb uplift at joint 2 and 410 lb uplift at joint 10.

LOAD CASE(S) Standard

FEBRUARY 23,2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196

Dwg.#0223061753

Job	Truss	Truss Type	Qty	Ply	ST JOHNS WITH PORCH
MASTER	T17	ROOF TRUSS	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Feb 23 15:36:31 2006 Page 1

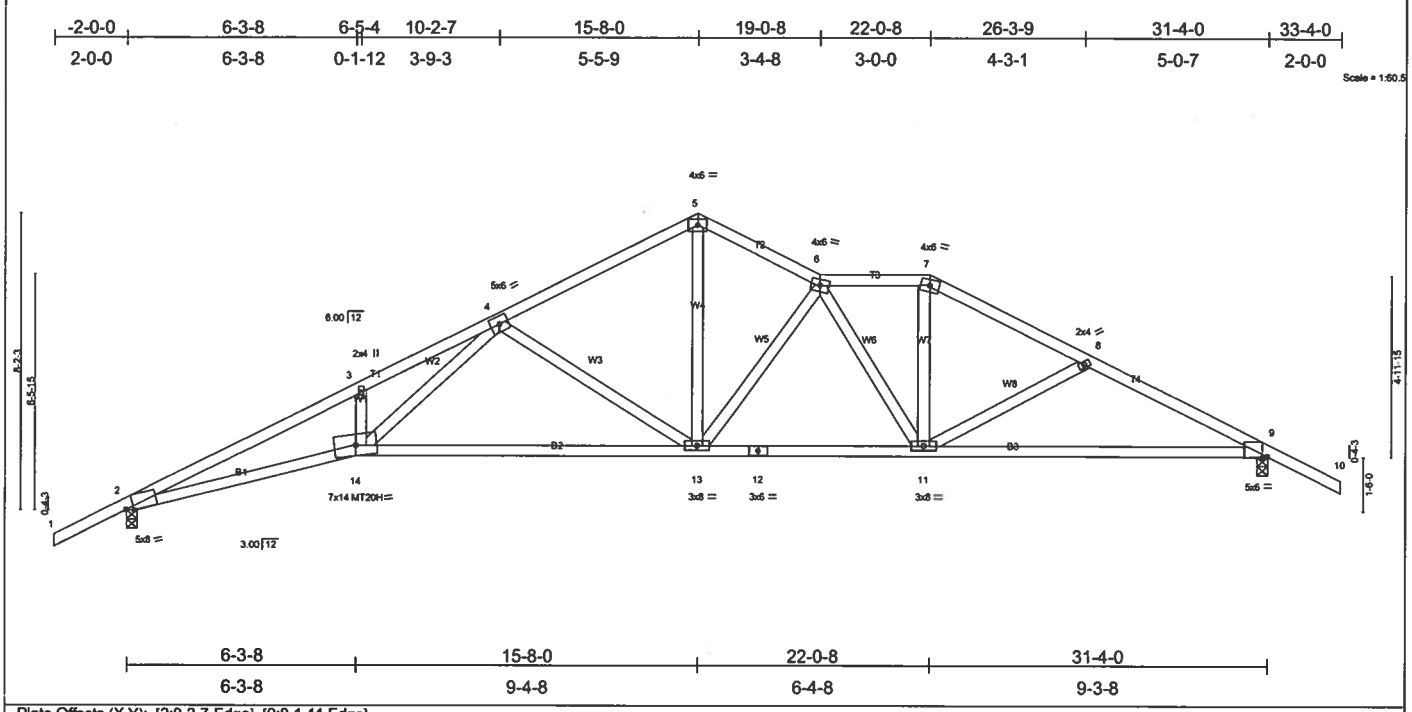


Plate Offsets (X,Y): [2:0-2-7,Edge], [9:0-1-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.71	Vert(LL)	-0.51	13-14	>735	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.90	Vert(TL)	-0.83	13-14	>447	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	YES	WB	0.71	Horz(TL)	0.27	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 163 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-2-11 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-2-13 oc bracing.

REACTIONS (lb/size) 2=1420/0-3-8, 9=1420/0-3-8
Max Horz 2=196(load case 5)
Max Uplift2=-548(load case 5), 9=-546(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-4411/1665, 3-4=-4334/1801, 4-5=-1873/851, 5-6=-1820/861, 6-7=-1828/848, 7-8=-2077/884, 8-9=-2330/991, 9-10=0/47
BOT CHORD 2-14=-1416/3977, 13-14=-843/2384, 12-13=-628/2028, 11-12=-628/2028, 9-11=-717/2034
WEBS 3-14=-169/230, 4-14=-749/1994, 4-13=-920/506, 5-13=-532/1325, 6-13=-703/355, 6-11=-386/188, 7-11=-208/681, 8-11=-271/219

JOINT STRESS INDEX
2 = 0.85, 3 = 0.34, 4 = 0.78, 5 = 0.64, 6 = 0.44, 7 = 0.48, 8 = 0.34, 9 = 0.67, 11 = 0.59, 12 = 0.79, 13 = 0.66 and 14 = 0.65

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 548 lb uplift at joint 2 and 546 lb uplift at joint 9.

LOAD CASE(S) Standard

FEBRUARY 23,2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196

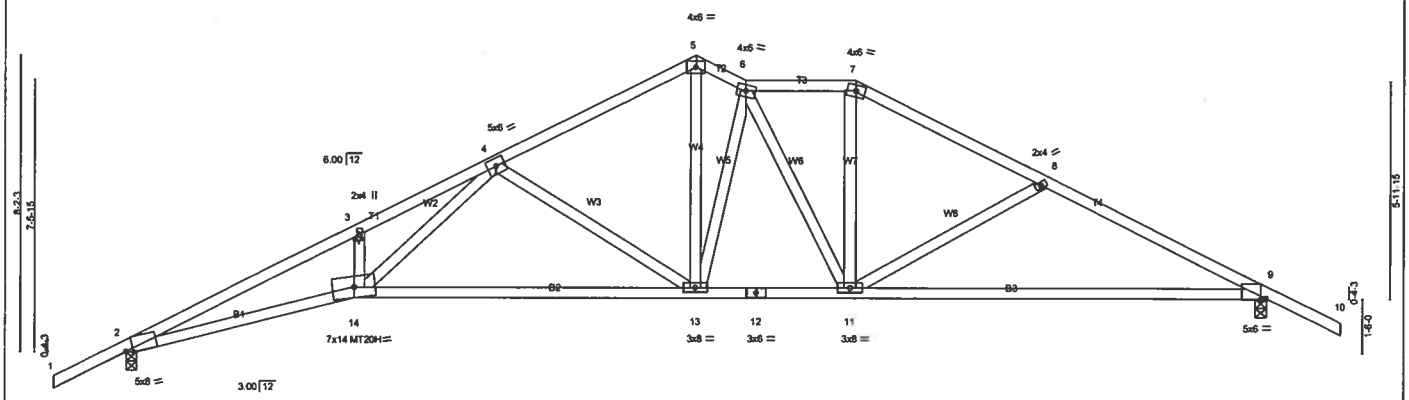
Dwg.#0223061754

Job	Truss	Truss Type	Qty	Ply	ST JOHNS WITH PORCH
MASTER	T18	ROOF TRUSS	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Feb 23 15:36:32 2006 Page 1

-2-0-0	6-3-8	6-5-4	10-2-7	15-8-0	17-0-8	20-0-8	25-1-7	31-4-0	33-4-0
2-0-0	6-3-8	0-1-12	3-9-3	5-5-9	1-4-8	3-0-0	5-0-15	6-2-9	2-0-0

Scale = 1:60.5



6-3-8	15-8-0	20-0-8	31-4-0
6-3-8	9-4-8	4-4-8	11-3-8

Plate Offsets (X,Y): [2:0-2-7,Edge], [9:0-1-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.71	Vert(LL)	-0.51	13-14	>725	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.91	Vert(TL)	-0.85	13-14	>440	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	YES	WB	0.70	Horz(TL)	0.26	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 168 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-2-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.

REACTIONS (lb/size) 2=1420/0-3-8, 9=1420/0-3-8
Max Horz 2=196(load case 5)
Max Uplift2=-548(load case 5), 9=-546(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-4416/1661, 3-4=-4340/1798, 4-5=-1870/851, 5-6=-1745/840, 6-7=-1681/813, 7-8=-1936/841, 8-9=-2263/984, 9-10=0/47
BOT CHORD 2-14=-1413/3982, 13-14=-840/2380, 12-13=-477/1751, 11-12=-477/1751, 9-11=-699/1975
WEBS 3-14=-172/232, 4-14=-749/2006, 4-13=-915/502, 5-13=-481/1241, 6-13=-620/239, 6-11=-227/105, 7-11=-151/575, 8-11=-365/297

JOINT STRESS INDEX
2 = 0.85, 3 = 0.34, 4 = 0.79, 5 = 0.61, 6 = 0.47, 7 = 0.60, 8 = 0.34, 9 = 0.78, 11 = 0.62, 12 = 0.86, 13 = 0.69 and 14 = 0.66

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 548 lb uplift at joint 2 and 546 lb uplift at joint 9.

LOAD CASE(S) Standard

FEBRUARY 23,2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0223061755

Job	Truss	Truss Type	Qty	Ply	ST JOHNS WITH PORCH
MASTER	T19	ROOF TRUSS	1	1	Job Reference (optional)

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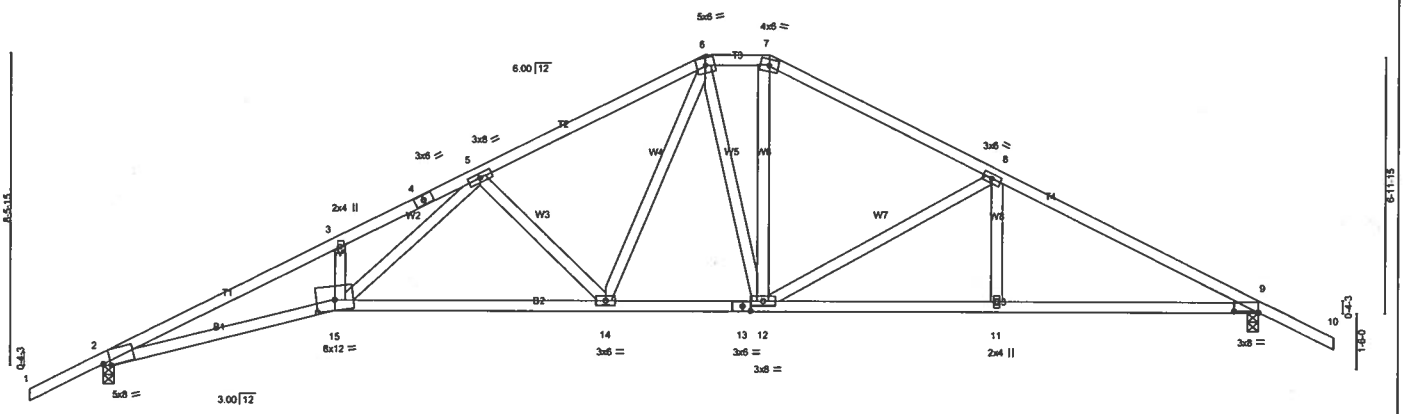
-2-0-0	6-3-8	6-5-4	10-2-7	16-3-8	18-0-8	24-2-14	31-4-0	33-4-0
2-0-0	6-3-8	0-1-12	3-9-3	6-1-1	1-9-0	6-2-6	7-1-2	2-0-0

Scale = 1:59.9



6-3-8	13-7-5	18-0-8	24-2-14	31-4-0
6-3-8	7-3-13	4-5-3	6-2-6	7-1-2

Plate Offsets (X,Y): [2:0-2-7,Edge], [9:0-8-0,0-0-6], [13:0-2-12,0-1-8], [15:0-6-0,0-3-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.72	Vert(LL)	-0.40	14-15	>940	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.89	Vert(TL)	-0.64	14-15	>580	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.61	Horz(TL)	0.26	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 167 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-2-5 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-2-15 oc bracing.

REACTIONS (lb/size) 2=1420/0-3-8, 9=1420/0-3-8
Max Horz 2=200(load case 5)
Max Uplift2=-551(load case 5), 9=-527(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-4363/1671, 3-4=-4283/1788, 4-5=-4201/1800, 5-6=-2128/972, 6-7=-1487/789, 7-8=-1732/806, 8-9=-2354/952, 9-10=0/47
BOT CHORD 2-15=-1422/3931, 14-15=-852/2410, 13-14=-387/1512, 12-13=-387/1512, 11-12=-664/2022, 9-11=-664/2022
WEBS 3-15=-161/217, 5-15=-742/1902, 5-14=-840/488, 6-14=-353/870, 6-12=-273/145, 7-12=-211/565, 8-12=-633/343, 8-11=0/229

JOINT STRESS INDEX
2 = 0.84, 3 = 0.34, 4 = 0.62, 5 = 0.98, 6 = 0.50, 7 = 0.64, 8 = 0.41, 9 = 0.75, 11 = 0.34, 12 = 0.69, 13 = 0.57, 14 = 0.77 and 15 = 0.91

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 551 lb uplift at joint 2 and 527 lb uplift at joint 9.

LOAD CASE(S) Standard

FEBRUARY 23,2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0223061756

Job	Truss	Truss Type	Qty	Ply	ST JOHNS WITH PORCH
MASTER	T20	ROOF TRUSS	3	1	Job Reference (optional)

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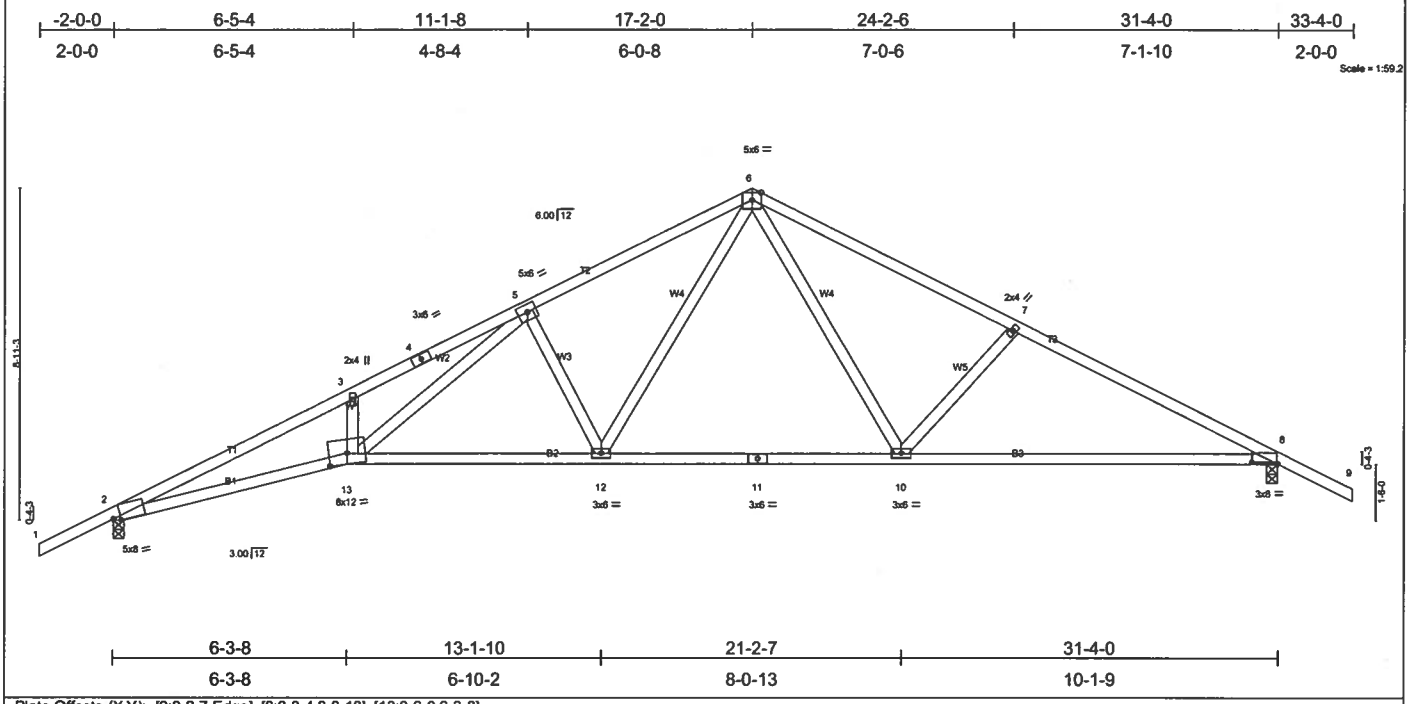

Plate Offsets (X,Y): [2:0-2-7,Edge], [8:0-8-4,0-0-10], [13:0-6-0,0-3-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.57	Vert(LL)	-0.39	12-13	>949	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.90	Vert(TL)	-0.63	12-13	>589	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.65	Horz(TL)	0.26	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 154 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-6-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-2-7 oc bracing.

REACTIONS (lb/size) 2=1420/0-3-8, 8=1420/0-3-8
Max Horz 2=206(load case 5)
Max Uplift2=-555(load case 5), 8=-533(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-4352/1692, 3-4=-4295/1830, 4-5=-4193/1845, 5-6=-2213/1045, 6-7=-2027/924, 7-8=-2268/981, 8-9=0/47
BOT CHORD 2-13=-1442/3917, 12-13=-769/2248, 11-12=-353/1410, 10-11=-353/1410, 8-10=-690/1970
WEBS 3-13=-196/258, 5-13=-851/2033, 5-12=-748/459, 6-12=-436/1032, 6-10=-227/671, 7-10=-380/343

JOINT STRESS INDEX
2 = 0.84, 3 = 0.34, 4 = 0.68, 5 = 0.82, 6 = 0.64, 7 = 0.34, 8 = 0.82, 10 = 0.54, 11 = 0.58, 12 = 0.82 and 13 = 0.86

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 555 lb uplift at joint 2 and 533 lb uplift at joint 8.

LOAD CASE(S) Standard

FEBRUARY 23,2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0223061757

Job	Truss	Truss Type	Qty	Ply	ST JOHNS WITH PORCH
MASTER	T21	ROOF TRUSS	1	1	Job Reference (optional)

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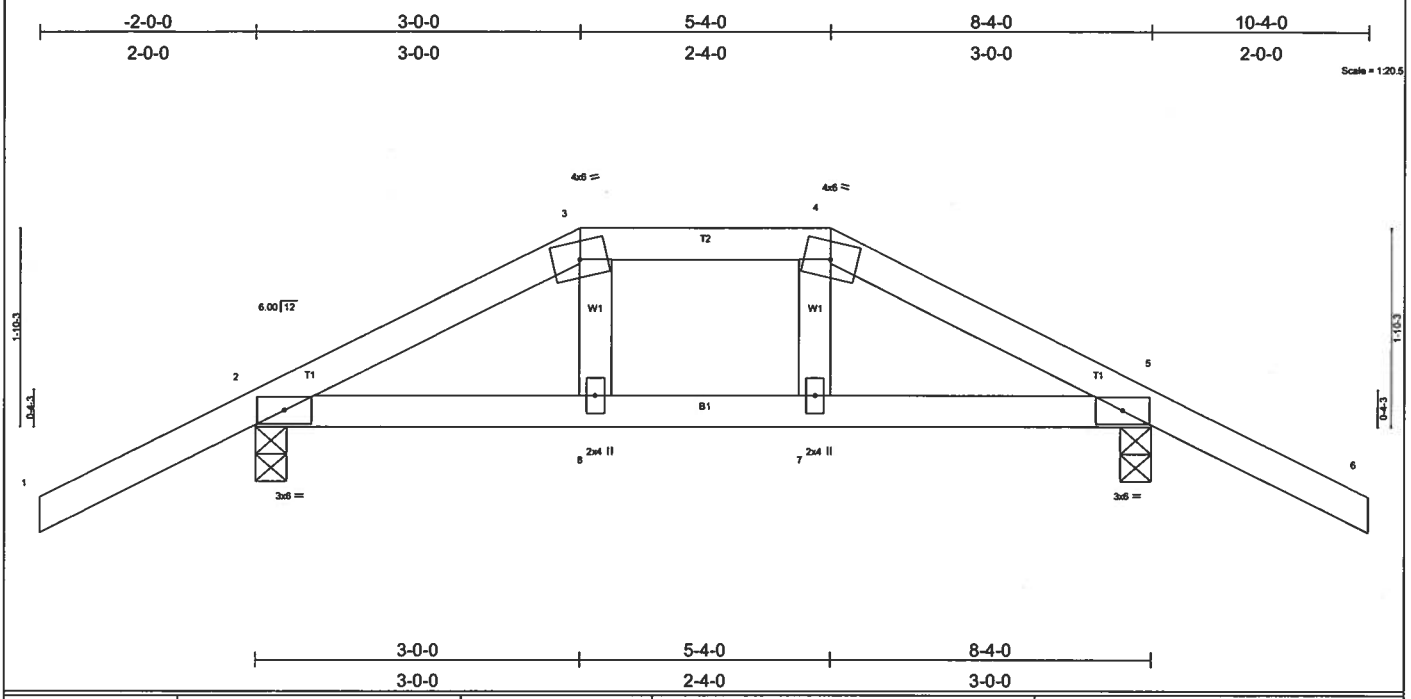

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.27	Vert(LL)	0.01	8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.17	Vert(TL)	-0.02	7-8	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.05	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 37 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=533/0-3-8, 5=533/0-3-8
Max Horz 2=59(load case 4)
Max Uplift2=-378(load case 4), 5=-378(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-587/300, 3-4=-488/275, 4-5=-587/300, 5-6=0/47
BOT CHORD 2-8=-217/476, 7-8=-223/488, 5-7=-216/476
WEBS 3-8=-80/149, 4-7=-80/149

JOINT STRESS INDEX
2 = 0.52, 3 = 0.11, 4 = 0.11, 5 = 0.52, 7 = 0.11 and 8 = 0.11

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 378 lb uplift at joint 2 and 378 lb uplift at joint 5.
5) Girder carries hip end with 3-0-0 end setback.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 63 lb down and 32 lb up at 5-4-0, and 63 lb down and 32 lb up at 3-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-4=-64(F=-10), 4-6=-54, 2-8=-30, 7-8=-35(F=-5), 5-7=-30
Concentrated Loads (lb)
Vert: 8=-63(F) 7=-63(F)

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THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0223061758

Job	Truss	Truss Type	Qty	Ply	ST JOHNS WITH PORCH
MASTER	T22	ROOF TRUSS	3	1	Job Reference (optional)

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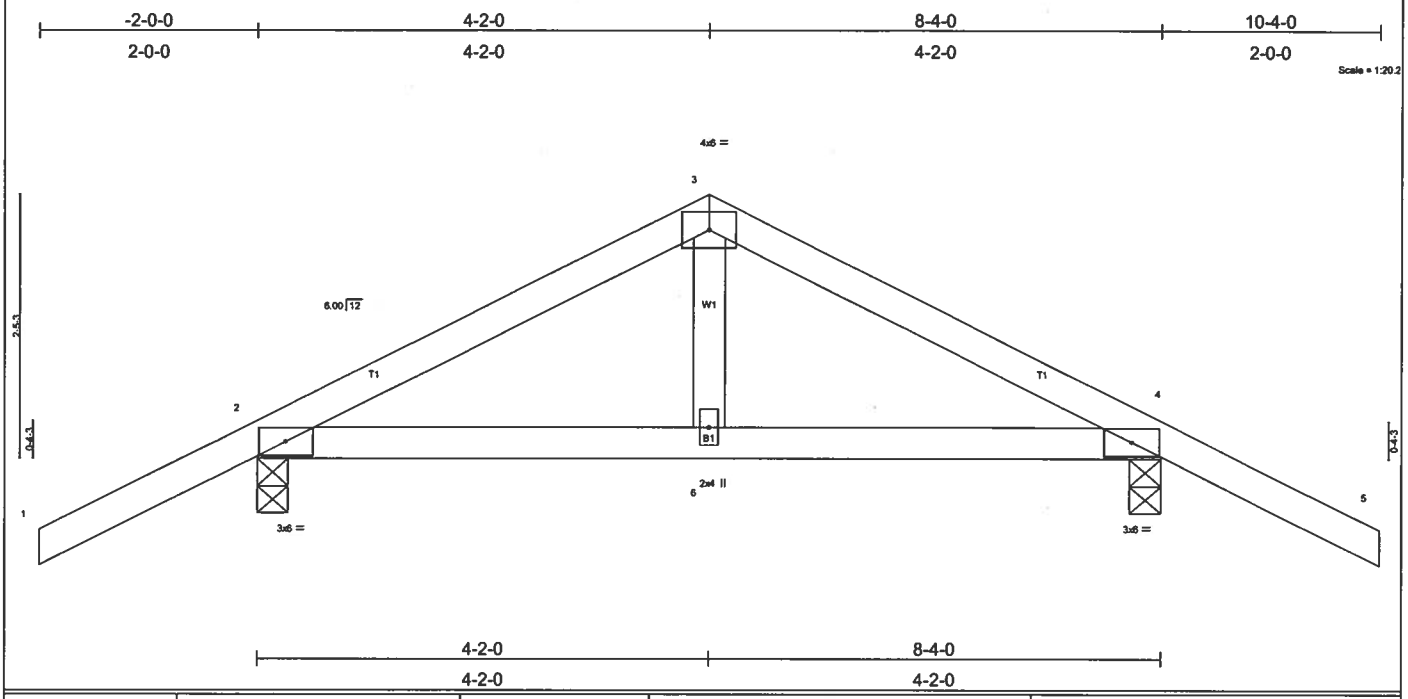

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	0.01	2-6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.10	Vert(TL)	0.01	2-6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.04	Horz(TL)	0.00	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 36 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=454/0-3-8, 4=454/0-3-8
Max Horz 2=-67(load case 6)
Max Uplift2=-344(load case 5), 4=-344(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-398/448, 3-4=-398/448, 4-5=0/47
BOT CHORD 2-6=-242/305, 4-6=-242/305
WEBS 3-6=-204/120

JOINT STRESS INDEX
2 = 0.59, 3 = 0.36, 4 = 0.59 and 6 = 0.09

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 344 lb uplift at joint 2 and 344 lb uplift at joint 4.

LOAD CASE(S) Standard

FEBRUARY 23,2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0223061759

Job	Truss	Truss Type	Qty	Ply	ST JOHNS WITH PORCH
MASTER	T23	ROOF TRUSS	1	1	Job Reference (optional)

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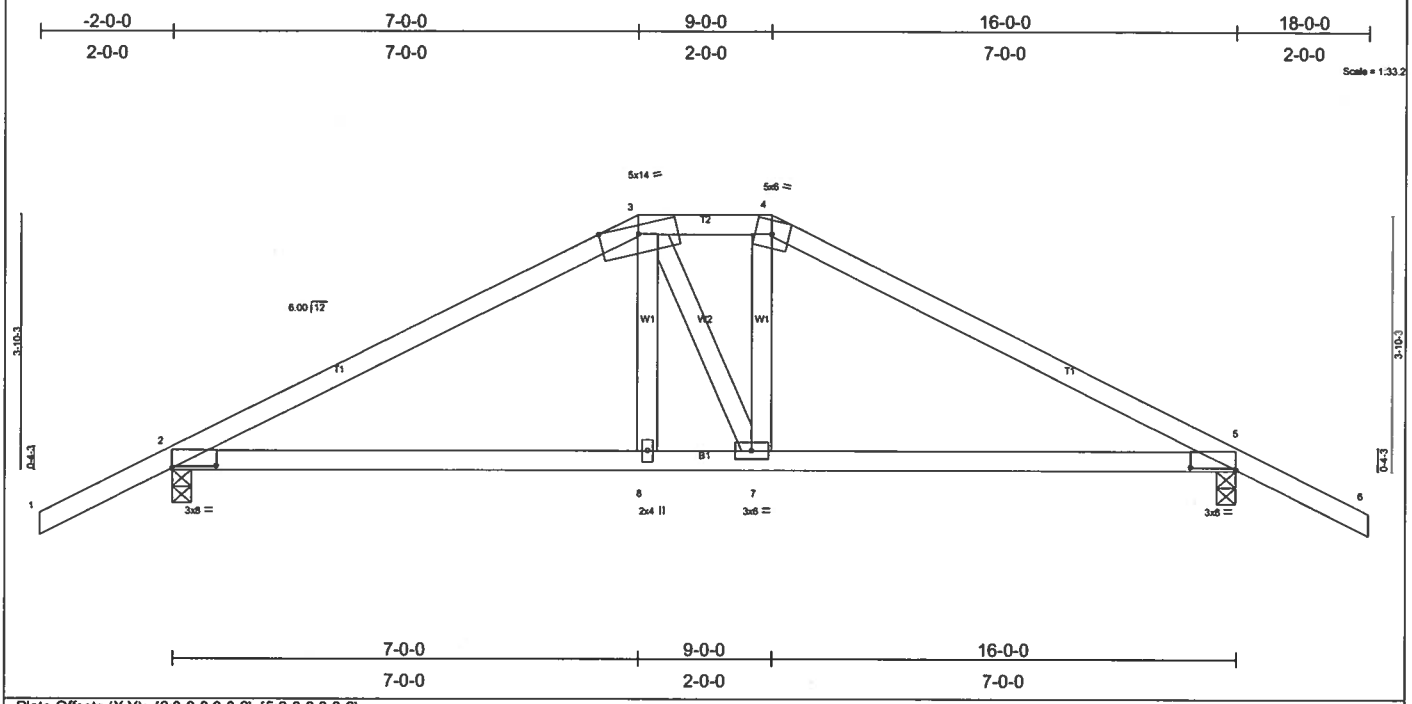

Plate Offsets (X,Y): [2:0-8-0,0-0-6], [5:0-8-0,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	0.14	2-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.61	Vert(TL)	-0.20	2-8	>945	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.27	Horz(TL)	0.05	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 72 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-9-1 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-15 oc bracing.

REACTIONS (lb/size) 2=1408/0-3-8, 5=1408/0-3-8
Max Horz 2=87(load case 4)
Max Uplift2=-862(load case 4), 5=-862(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2312/1221, 3-4=-2015/1170, 4-5=-2317/1223, 5-6=0/47
BOT CHORD 2-8=-1017/1981, 7-8=-1031/2011, 5-7=-995/1986
WEBS 3-8=-375/717, 3-7=-143/162, 4-7=-423/831

JOINT STRESS INDEX
2 = 0.73, 3 = 0.92, 4 = 0.65, 5 = 0.74, 7 = 0.54 and 8 = 0.52

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 862 lb uplift at joint 2 and 862 lb uplift at joint 5.
5) Girder carries hip end with 7-0-0 end setback.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 9-0-0, and 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-4=-118(F=-64), 4-6=-54, 2-8=-30, 7-8=-65(F=-35), 5-7=-30
Concentrated Loads (lb)
Vert: 8=-539(F) 7=-539(F)

FEBRUARY 23,2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B LUTZ, FL 33549

Dwg.#0223061760

Job	Truss	Truss Type	Qty	Ply	ST JOHNS WITH PORCH
MASTER	T24	ROOF TRUSS	3	1	Job Reference (optional)

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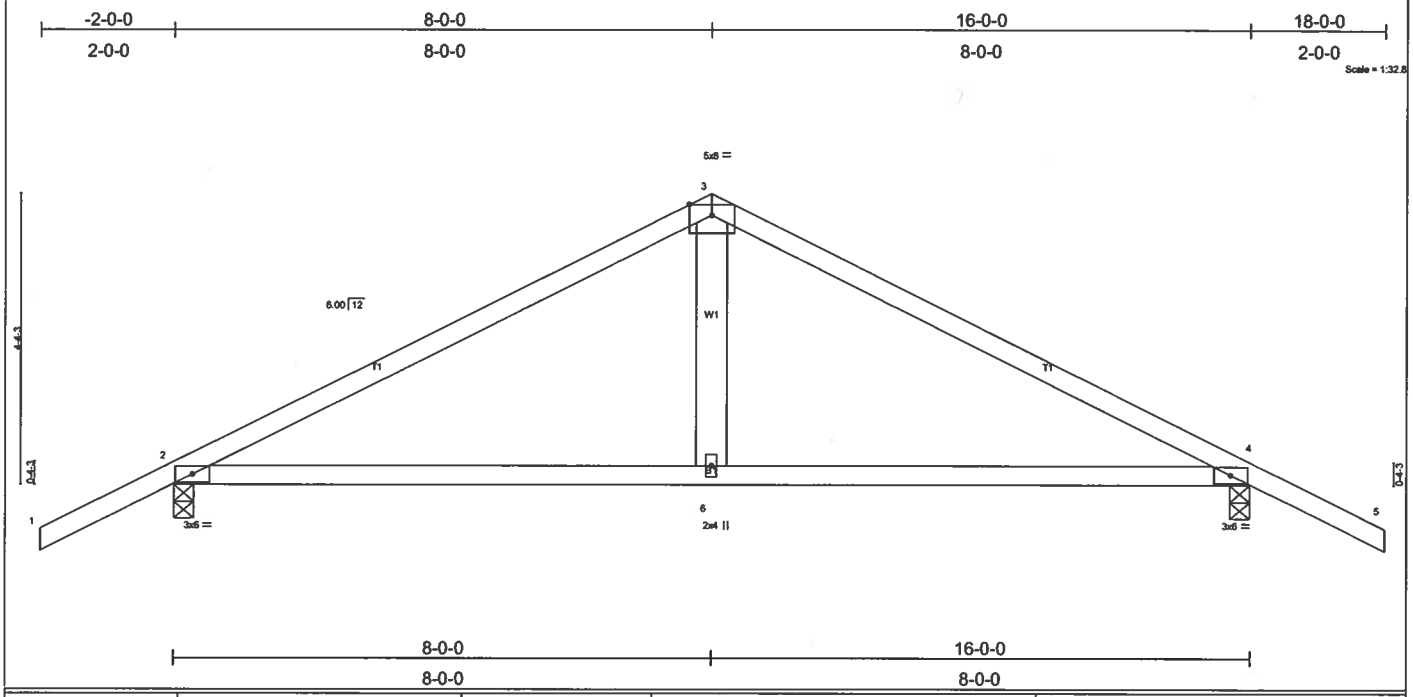


LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.51	Vert(LL)	0.24	2-6	>782	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.55	Vert(TL)	-0.20	2-6	>928	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.07	Horz(TL)	0.02	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 66 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 6 SYP No.1D

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-11-13 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-1-0 oc bracing.

REACTIONS (lb/size) 2=776/0-3-8, 4=776/0-3-8
Max Horz 2=94(load case 5)
Max Uplift2=-539(load case 5), 4=-539(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-930/981, 3-4=-930/981, 4-5=0/47
BOT CHORD 2-6=-689/752, 4-6=-689/752
WEBS 3-6=-489/295

JOINT STRESS INDEX
2 = 0.62, 3 = 0.78, 4 = 0.62 and 6 = 0.22

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 539 lb uplift at joint 2 and 539 lb uplift at joint 4.

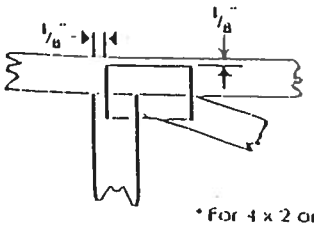
LOAD CASE(S) Standard

FEBRUARY 23,2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B LUTZ, FL 33549

Symbols

PLATE LOCATION AND ORIENTATION

* Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seal.



* For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.

* This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

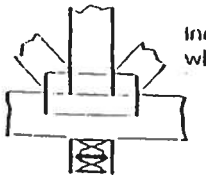
4 X 4

The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

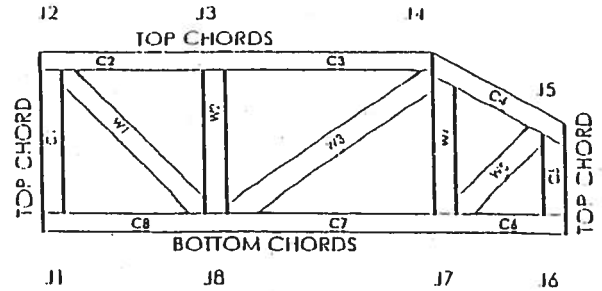
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DILHR	960022-W, 970036-H
HER	561

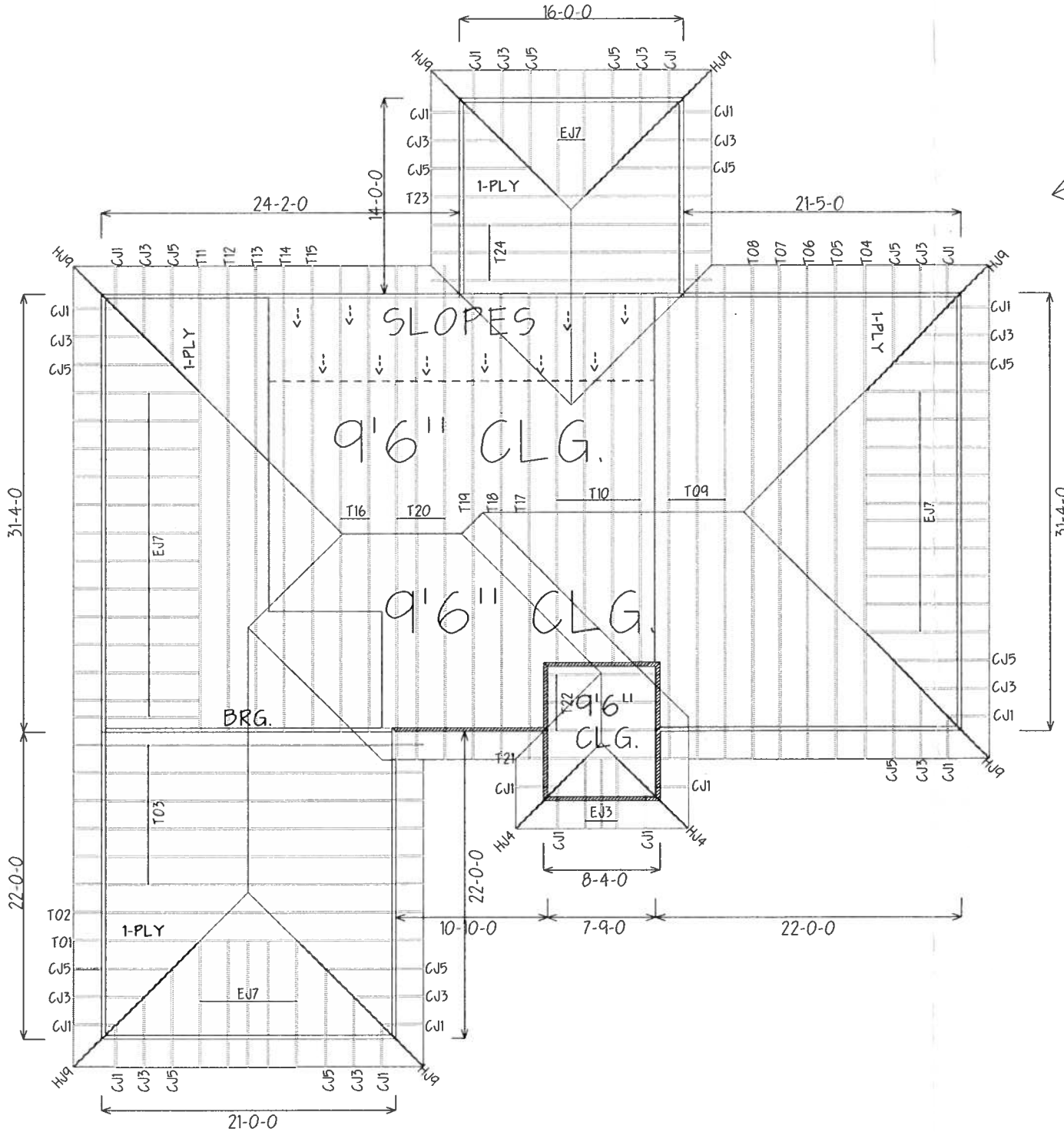
MiTek Engineering Reference Sheet: MII-7473

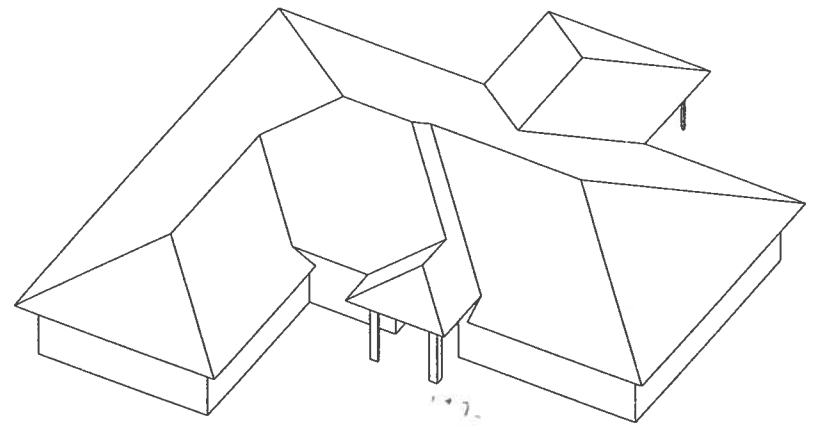
General Safety Notes

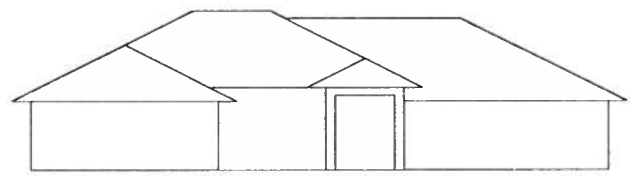
Failure to Follow Could Cause Property Damage or Personal Injury

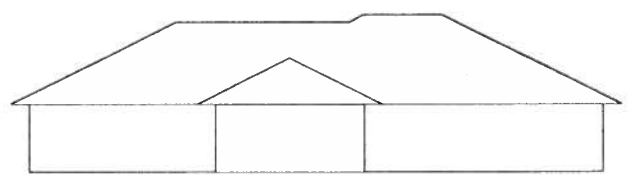
1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
4. Unless otherwise noted, locate chord splices at ¼ panel length (± 6" from adjacent joint.)
5. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
6. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
8. Plate type, size and location dimensions shown indicate minimum plating requirements.
9. Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
10. Top chords must be sheathed or purlins provided at spacing shown on design.
11. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
12. Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
13. Do not overload roof or floor trusses with stacks of construction materials.
14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
15. Care should be exercised in handling, erection and installation of trusses.

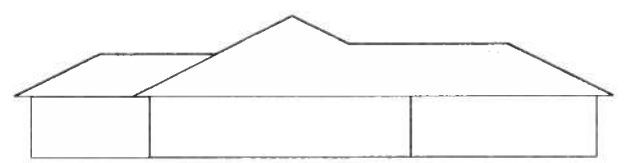
6/12 PITCH - 2'0" O/H

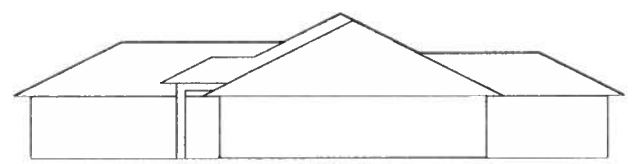












BEARING HEIGHT SCHEDULE

8'-0"

9'-6"

NOTES:

1) REFER TO HIB 91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BRACING.) REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED.

2) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL V105 FOR ALTERNATE BRACING REQUIREMENTS.

3) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.

4) ALL TRUSSES ARE DESIGNED FOR 2' o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.

5) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.

6) ST42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.

7) ALL ROOF TRUSS HANGERS TO BE SIMPSON HUS26 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSS HANGERS TO BE SIMPSON THA422 UNLESS OTHERWISE NOTED.

8) BEAM/HEADER/LINTEL (HDR) TO BE FURNISHED BY BUILDER.

SHOP DRAWING APPROVAL

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION OF TRUSSES AND VOIDS ALL PREVIOUS ARCHITECTURAL OR OTHER TRUSS LAYOUTS. REVIEW AND APPROVAL OF THIS LAYOUT MUST BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VERIFY ALL CONDITIONS TO INSURE AGAINST CHANGES THAT WILL RESULT IN EXTRA CHARGES TO YOU.

Requested Delivery Date: _____

Approved By: _____ Date: _____

Builders FirstSource

Bunnell
PHONE: 904-437-3349 FAX: 904-437-3994

Jacksonville
PHONE: 904-772-6100 FAX: 904-772-1973

Lake City
PHONE: 904-755-6894 FAX: 904-755-7973

Sanford
PHONE: 407-322-0059 FAX: 407-322-5553

BUILDER: GIEBEIG HOMES

LEGAL ADDRESS:

MODEL:	REVISION:
ST.JOHNS 4-BDRM	SCALE: NTS
DATE: 2/23/06	DRAWN BY: K.L.H
	JOB #: L152528