Wind Load Analysis and Certification

Lominack Residence by Hartley Brothers, Inc.

2023 Florida Building Code section 1609 according to ASCE 7-22

Ultimate Design Wind Speed (Vult) = 130 MPH (3 second gust)

Nominal Design Wind Speed (Vasd)) = 101 MPH

Risk Category = II

Exposure Category = B, Enclosed Building

Applicable Internal Pressure Coefficient = .18

Design Wind Pressure for use of External Components (Components and Cladding)= +32.1psf, -43.3psf



Roof Decking

7/16" or 5/8" OSB or ½", 5/8" or 3/4" CDX Decking; 48"x96" Sheets, Perpendicular to Roof Framing Members

8d common (.131" dia) or 8d ring-shank (.113' dia.) nails at 4" O.C. on Ends, 8" O.C. in Interior

Trusses or Rafters at 2' O.C. (horizontal distance), No Intermediate Blocking Required

Rafters: 2x6 SYP #2 up to 10' horizontal span, 2x8 SYP #2 up to 14' horizontal span

Shear Wall Segments

7/16" OSB or ½" CDX plywood, 48" Wide Sheets - Sheathing Continuous from Top Plate down to Pressure Treated Sole Plate Bearing on Foundation.

8d common (.131" dia) nails at 3" O.C. on Edges and Ends, 8" O.C. in Interior

Transverse Shearwall = 30', Longitudinal Shearwall = 81'

2x4 SPF (No. 1&2) Studs at 16" O.C., up to 12'

2x6 SPF (No. 1&2) Studs at 16" O.C., up to 17'

2x6 SP#2 studs at 16" O.C. up to 18' wall height

See attached detail for stud and jack requirements for wall openings

Nail Together Double Top Plate with two (2) 12-d comm nails every 16" O.C. (SYP top plates)

Other Wall Segments - Same as Shear Walls

Gabled End Wall Framing

N/A

Special Notes: All headers and beams to be double 2x12 SP#2.

All girders and beams to have a min of two (2) 2x4 SP#2 or two (2) 2x6 SP#2 studs under each bearing point.

This structural and windload analysis is based on the attached truss layout. Any deviation from the attached layout invalidates this structural and windload analysis.

Footings and Foundations (Based on Truss Engineering)

20" deep x 14" wide monolithic with 2-#5's, Continuous, 3000 psi Concrete

or: 20" Wide x 10" Deep 3000 psi Concrete Strip Footing with 2-#5's, Continuous

8"x8"x16" Concrete Masonry Stemwall, Minimum 2 Courses, Maximum 4 Courses, Fully Grouted, except sections over 3 courses need only cells with rebar to be grouted. 1-#5 Vertical Dowel at Corners and 6'-0" O.C. (10" hook top and bottom) (min 25" lap all #5 rebar) (1) #5 continuous top course. All 4" slabs requires 6x6 WWM

Porch Footers: 16" deep x 14" wide monolithic with 2-#5's, Continuous or see above or: 8" wide by 8" deep bell footing with 1-#5, Continuous with minimum of 30"x30" x 15" pad under each post (w/ 3- #5 each way)

Note: footer design based on continuous bearing of 2000 psf. Footers for any concentrated loads greater than 10,000 lbs must be reviewed with windload engineer. Movement – The information presented in this document is not calculated or intended for the use or purpose of mitigating or addressing unsuitable soils or subsurface conditions in any way or manner, whatsoever.

Hurricane-Resistance Hardware (Based on Truss Engineering)

Truss Clips/Headers/Girders/Posts/Beams /Top and Bottom of Wall Unit - See Table

Note: regarding strapping of headers and beams, 50% of the strap shall be on the header/beam and 50% of the strap is on the supporting member below it (jacks and posts).

Anchor Bolts- 1/2"Dia. x 10" J Bolts (with min 7.5" embedment) at 48"O.C. (First bolt at 9" from Corner, then 48" O.C.) and at each end of Each Opening (2" round or square washers).

I hereby certify that the accompanying Wind Load Analysis for the Lominack Residence, demonstrates compliance with the 2023 FBC section 1609 according to ASCE 7-22, to the best of my knowledge.

> Frank J. Sapienza Jr. License Professional Engineer Florida License Number 48566

This item has been digitally signed and sealed by FRANK J SAPIENZA JR PE using Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.



Frank J

Digitally signed by Frank J Sapienza Jr Sapienza Jr Date: 2024.09.01 11:15:34 -04'00'

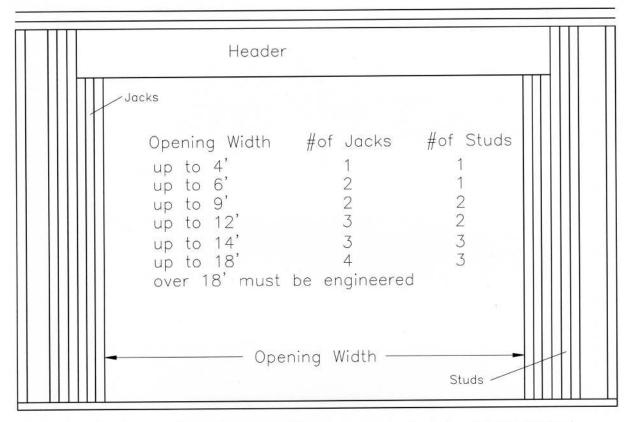
HOLD-DOWN TABLE

Wood Sections	Uplift				2 0
	Force Lbs	Top Connector Simpson **	Rating Lbs	Bottom Connector Simpson **	Rating Lbs
HEADERS	to AEE lb-	LCTAC	77.5	LIO	AFF
	up to 455 lbs	LSTA9	775	H3	455
	up to 910 lbs	LSTA12	970	2-H3	910
	up to 1235 lbs	LSTA18	1235	LTTP2	1680
	up to 1940 lbs		1940	LTTP2	2275
	up to 2470 lbs	2-LSTA18	2470	HTT4	3000
	up to 3000 lbs		3705	HTT4	3000
	up to 3705 lbs		3705	HTT5	4350
To determine uplift force on head					
(assumes uniform load)	Note: must u	ise proper bolt an	chors suffi	cient to support required	load
- 10: 1 1:0					
Trusses/Girders - Uplift	110 54 1			2425	
up to 600 lbs - use	The state of the s	the state of the s			
over 600 lbs but u				equired at bottom	
	H6 or equivalent				
	2-H6 or equivaler			m	
	2-H6 or equivaler				
up to 3690 lbs use	3-H6 or equivaler			etaan i	
		Must Use proper	A STATE OF THE PARTY OF THE PAR	7.1CC	
Note: it is the contractors re		rovide a continu	ous load	path	
from truss/rafter/ridge bean					
Strap rafters to truss or at eac	Contract to the contract to th	CONTRACTOR OF THE PARTY OF THE		ch end	
Strap ridge beam at each en	d with min uplift re	sistance of 1800 l	bs		
Note: Three (3) 12d comm toenails				after per bearing	
point into plate to resist both later	al loads (wall to truss	and transverse loa	ds		
Horizontal Resistance (from truss		levices are in addition			
up to 110 lbs - use H2.5A		Note: hardware to be used must satisfy both			
up to 525 lbs use H10		uplift and horizontal resistance, combination			
up to 1090 lbs use H10 plus A23		of devices is acceptable			
*	33				
		top	like	bottom	
BEAM SEATS		LSTA18*	1235	LTTP2*	1680
POSTS		2-LSTA18	2400	ABU44 or ABU66	2200
	* or per trus	s engineering		Must Use proper bolt a	nchors
		2000		12 - 200	
STUDS		A		200 - 400	
Wall Sheathing Nailing Adequat				<u> </u>	
Wall Sheathing Nailing Adequat	e Exterior Walls Top	(8d nails at 3TO.C	.), as long	as sheathing covers top	
plate, otherwise use SP2 @	32" O.C. in addition	n to sheathing na	iling,		
Use SP2 top and SP1 bottom ea	ch stud an anchor b	olts @ 32" O.C. for	all interior	load bearing walls that	
have uplift. Interior anchor bo					
Please Note:All Beams must					

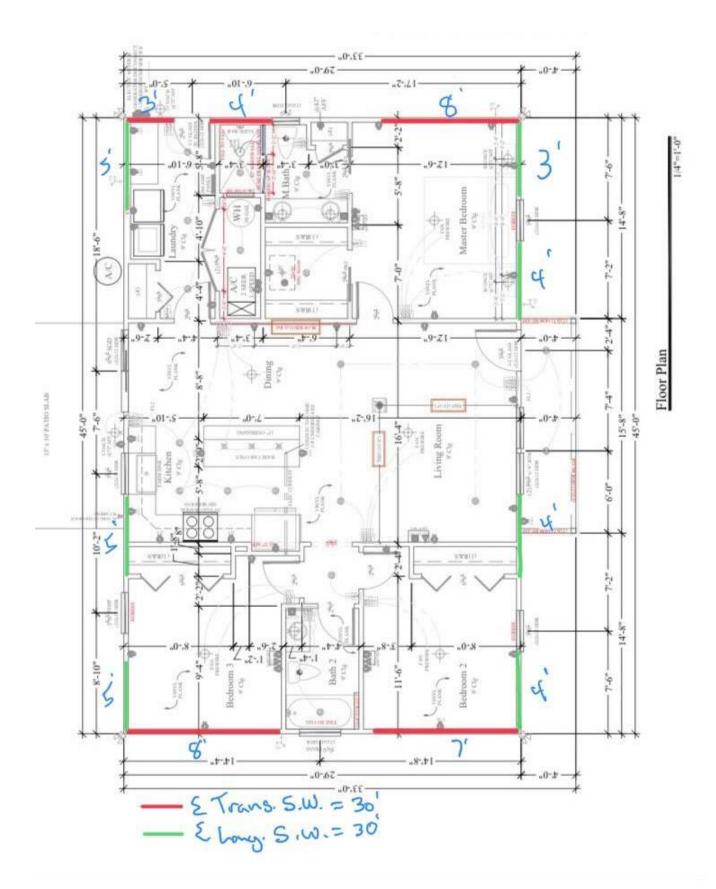
Please Note:All Beams must be sheathed or strapped to double top plate (if applicable)
An equivalent device of same or other manufactures can be substituted for any of the
devices specified on this page as long as it meets the required load capacities

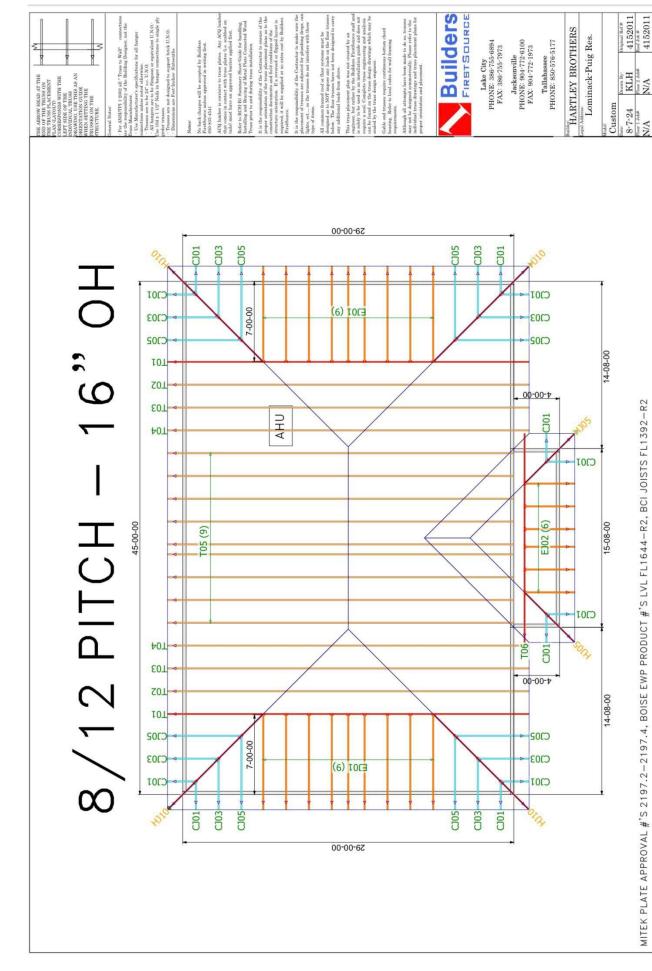
Note: For nailing into SPF members, multiply table values by .86

Number of Jack and Stud Requirements per Opening Width 2x4 or 2x6 SPF #1&2 Construction — max Wall Height=12' (based on 16" O.C. Stud Spacing)



Note — Based on uniform loads. Heavy concentrated loads require engineering review





MITEK PLATE APPROVAL #'S 2197.2-2197.4, BOISE EWP PRODUCT #'S LVL FL1644-R2, BCI JOISTS FL1392-R2