DATE 01/28/2013

Columbia County Building Permit

This Permit Must Be Prominently Posted on Premises During Construction

PERMIT
000030745

APPLICANT JOHN D. HARRINGTON PHONE 386-462-5323

ADDRESS 24015 NW OLD BELLAMY RD HIGH SPRINGS FL 32643

OWNER ROBERT & KAREN WHEARY PHONE

ADDRESS 1701 SW NEWARK DRIVE FORT WHITE FL 32038

CONTRACTOR JOHN D. HARRINGTON JR PHONE 352-538-5963

LOCATION OF PROPERTY 47 S, R WILSON SPRINGS RD, R NEWARK DR, 1 MILE ON RIGHT

TYPE DEVELOPMENT SFD, UTILITY ESTIMATED COST OF CONSTRUCTION 52000.00

HEATED FLOOR AREA 1024.00 TOTAL AREA 1040.00 HEIGHT 16.00 STORIES 1

FOUNDATION CONCRETE WALLS FRAMED ROOF PITCH 6/12 FLOOR SLAB

LAND USE & ZONING A-3 MAX. HEIGHT 35

Minimum Set Back Requirments: STREET-FRONT 30.00 REAR 25.00 SIDE 25.00

NO. EX.D.U. FLOOD ZONE FL X DEVELOPMENT PERMIT NO.

PARCEL ID 25-6S-15-01307-001 SUBDIVISION THREE RIVERS ESTATES

LOT 14.15 BLOCK PHASE UNIT 21 TOTAL ACRES 1.84

000001987		CGC1516998	X	
Culvert Permit No.	Culvert Waiver	Contractor's License Number		Applicant/Owner/Contractor
WAIVER	13-0018	BK	TC	N
Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance	New Resident

COMMENTS: MINIMUM FLOOR ELEVATION IS 33.4', NEED ELEVATION CONFIRMATION LETTER AT SLAB, NOC ON FILE

Check # or Cash 8393 ✓

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power _____ date/app. by | Foundation _____ date/app. by | Monolithic _____ date/app. by

Under slab rough-in plumbing _____ date/app. by | Slab _____ date/app. by | Sheathing/Nailing _____ date/app. by

Framing _____ date/app. by | Insulation _____ date/app. by

Rough-in plumbing above slab and below wood floor _____ date/app. by | Electrical rough-in _____ date/app. by

Heat & Air Duct _____ date/app. by | Peri. beam (Lintel) _____ date/app. by | Pool _____ date/app. by

Permanent power _____ date/app. by | C.O. Final _____ date/app. by | Culvert _____ date/app. by

Pump pole _____ date/app. by | Utility Pole _____ date/app. by | M/H tie downs, blocking, electricity and plumbing _____ date/app. by

Reconnection _____ date/app. by | RV _____ date/app. by | Re-roof _____ date/app. by

BUILDING PERMIT FEE $ 260.00 CERTIFICATION FEE $ 5.20 SURCHARGE FEE $ 5.20

MISC. FEES $ 0.00 ZONING CERT. FEE $ 50.00 FIRE FEE $ 0.00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ **TOTAL FEE** 345.40

INSPECTORS OFFICE _____ CLERKS OFFICE CH

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY.

NOTICE: ALL OTHER APPLICABLE STATE OR FEDERAL PERMITS SHALL BE OBTAINED BEFORE COMMENCEMENT OF THIS PERMITTED DEVELOPMENT.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED NOT SUSPENDED, ABANDONED OR INVALID WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS OT THE PREVIOUS INSPECTION.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Permit # 30745

Notice of Treatment 43000

Applicator: Florida Pest Control · (www.flapest .com)
Address: 46 NW 16 Ave
City Gnv Fl Phone 376-2661

Site Location: Subdivision
Lot # Block# Permit #
Address 1701 Newnan Dr

Product used	Active Ingredient	% Concentration
☐ Premise	Imidacloprid	0.1%
☐ Termidor	Fipronil	0.12%
☐		

Type treatment: ☑ Soil

Area Treated	Square feet	Linear feet	Gallons Applied
MB Footing	1040	142.8	120

As per Florida Building Code 104.2.6 – If soil chemical barrier method for termite prevention is used, final exterior treatment shall be completed prior to final building approval.

If this notice is for the final exterior treatment, initial this line _____

2-8-13	810	Gary
Date	Time	Print Technician's Name

Remarks:

Only 1 set of Energy Code &

Columbia County Building Permit Application

For Office Use Only Application # 1301-26 Date Received 1-15-13 By LH Permit # 30745 1987

Zoning Official RLK Date 23 Jan. 2013 Flood Zone FLOODABLE X Land Use A-3 Zoning A-3

FEMA Map # 0467C Elevation 33.4' MFE 33.4' River Santa Fe Plans Examiner T.C. Date 1-22-13

Comments Elevation Confirmation Letter Required at Slab

☑ NOC ☑ EH ☑ Deed or PA ☑ Site Plan ☑ State Road Info ☑ Well letter ☑ 911 Sheet ☐ Parent Parcel # ~~D732-A11~~

☐ Dev Permit # ☐ In Floodway ☑ Letter of Auth. from Contractor ☑ F W Comp. letter ~~D1163-A11~~

IMPACT FEES: EMS Fire Corr ☑ Sub VF Form ~~D728-w/c~~ D379 - Lic & Lic.

Road/Code School = TOTAL (Suspended) ☐ Ellisville Water ☑ App Fee Paid ~~D1153-w/c~~

Septic Permit No. 13-0018 Fax 888-769-0105

Name Authorized Person Signing Permit John D Harrington Phone 386-462-5323

Address 24015 NW 01D Bellamy RD High Springs FL 32643

Owners Name Robert & Karen Wheary Phone

911 Address 1701 Newark Drive Ft White 32038

Contractors Name John D Harrington Phone 352-538-5963

Address 12501 US Hwy 441 Alachua FL 32615

Fee Simple Owner Name & Address N/A

Bonding Co. Name & Address

Architect/Engineer Name & Address Driscoll Engineering, Inc. PO Box 357577, Gainesville, FL 32606 352-331-1513

Mortgage Lenders Name & Address First Federal Bank of Florida 4705 West US Hwy 90, Lake City, FL 32055

Circle the correct power company – FL Power & Light – (Clay Elec.) – Suwannee Valley Elec. – Progress Energy

Property ID Number 00-00-00-01307-001 Estimated Cost of Construction 65,000

Subdivision Name 3 Rivers Estates Lot 14 & 15 Block Unit 21 Phase

Driving Directions SR 47 to Ft go through Stop light TR on Wilson Springs RD - follow until Newark drive TR 1 mile down on Right side Number of Existing Dwellings on Property 0

Construction of SFD Total Acreage 1.836 Lot Size 1.836

Do you need a - Culvert Permit or (Culvert Waiver) or Have an Existing Drive Total Building Height 16'

Actual Distance of Structure from Property Lines - Front ~~30~~ Side ~~78~~ Side ~~79~~ Rear ~~70~~

Number of Stories 1 Heated Floor Area 1024 Total Floor Area 1040 Roof Pitch 6:12

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction. **CODE:** Florida Building Code 2010 and the 2008 National Electrical Code.

Left message 1-24-13
Spoke to Harrington 1-24-13

ck 8393
$395.00 w/ waiver

TIME LIMITATIONS OF APPLICATION : An application for a permit for any proposed work shall be deemed to have been abandoned 180 days after the date of filing, unless such application has been pursued in good faith or a permit has been issued; except that the building official is authorized to grant one or more extensions of time for additional periods not exceeding 90 days each. The extension shall be requested in writing and justifiable cause demonstrated.

TIME LIMITATIONS OF PERMITS: Every permit issued shall become invalid unless the work authorized by such permit is commenced within 180 days after its issuance, or if the work authorized by such permit is suspended or abandoned for a period of 180 days after the time work is commenced. A valid permit receives an approved inspection every 180 days. Work shall be considered not suspended, abandoned or invalid when the permit has received an approved inspection within 180 days of the previous approved inspection.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment: According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full.
This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE: ***YOU ARE HEREBY NOTIFIED*** as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

OWNERS CERTIFICATION: I CERTIFY THAT ALL THE FOREGOING INFORMATION IS ACCURATE AND THAT ALL WORK WILL BE DONE IN COMPLIANCE WITH ALL APPLICABLE LAWS REGULATING CONSTRUCTION AND ZONING.

NOTICE TO OWNER: There are some properties that may have deed restrictions recorded upon them. These restrictions may limit or prohibit the work applied for in your building permit. You must verify if your property is encumbered by any restrictions or face possible litigation and or fines.

(Owners Must Sign All Applications Before Permit Issuance.)

Owners Signature ***OWNER BUILDERS MUST PERSONALLY APPEAR AND SIGN THE BUILDING PERMIT.**

CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit including all application and permit time limitations.

Contractor's Signature (Permitee)

Contractor's License Number CGC1516998
Columbia County
Competency Card Number 1163

Affirmed under penalty of perjury to by the Contractor and subscribed before me this 11 **day of** January **20** 13.

Personally known ✓ **or Produced Identification** _____

State of Florida Notary Signature (For the Contractor)

SEAL:

Notary Public State of Florida
Jessica R Cason
My Commis ion EE046970
Expires 12/06/2014

1-28-13 Recd.

SUBCONTRACTOR VERIFICATION FORM

APPLICATION NUMBER 30745 CONTRACTOR Harrington PHONE _____

THIS FORM MUST BE SUBMITTED PRIOR TO THE ISSUANCE OF A PERMIT

In Columbia County one permit will cover all trades doing work at the permitted site. It is REQUIRED that we have records of the subcontractors who actually did the trade specific work under the permit. Per Florida Statute 440 and Ordinance 89-6, a contractor shall require all subcontractors to provide evidence of workers' compensation or exemption, general liability insurance and a valid Certificate of Competency license in Columbia County.

Any changes, the permitted contractor is responsible for the corrected form being submitted to this office prior to the start of that subcontractor beginning any work. Violations will result in stop work orders and/or fines.

ELECTRICAL	Print Name _____ Signature _____ License #: Phone #:
MECHANICAL/ A/C _____	Print Name _____ Signature _____ License #: Phone #:
PLUMBING/ GAS	Print Name _____ Signature _____ License #: Phone #:
ROOFING	Print Name John D Harrington Signature _____ License #: CGC 1516998 Phone #: 352-538-5963
SHEET METAL	Print Name _____ Signature _____ License #: Phone #:
FIRE SYSTEM/ SPRINKLER	Print Name _____ Signature _____ License#: Phone #:
SOLAR	Print Name _____ Signature _____ License #: Phone #:

Specialty License	License Number	Sub-Contractors Printed Name	Sub-Contractors Signature
MASON			
CONCRETE FINISHER			
FRAMING			
INSULATION			
STUCCO			
DRYWALL			
PLASTER			
CABINET INSTALLER			
PAINTING			
ACOUSTICAL CEILING			
GLASS			
CERAMIC TILE			
FLOOR COVERING			
ALUM/VINYL SIDING			
GARAGE DOOR			
METAL BLDG ERECTOR			

F. S. 440.103 Building permits; identification of minimum premium policy.--Every employer shall, as a condition to applying for and receiving a building permit, show proof and certify to the permit issuer that it has secured compensation for its employees under this chapter as provided in ss. 440.10 and 440.38, and shall be presented each time the employer applies for a building permit.

SUBCONTRACTOR VERIFICATION FORM

APPLICATION NUMBER 1301-26 CONTRACTOR Harrington PHONE _____

THIS FORM MUST BE SUBMITTED PRIOR TO THE ISSUANCE OF A PERMIT

In Columbia County one permit will cover all trades doing work at the permitted site. It is REQUIRED that we have records of the subcontractors who actually did the trade specific work under the permit. Per Florida Statute 440 and Ordinance 89-6, a contractor shall require all subcontractors to provide evidence of workers' compensation or exemption, general liability insurance and a valid Certificate of Competency license in Columbia County.

Any changes, the permitted contractor is responsible for the corrected form being submitted to this office prior to the start of that subcontractor beginning any work. Violations will result in stop work orders and/or fines.

ELECTRICAL 379	Print Name Denis Cason License #: EC13001281	Signature Denis Cason Phone #: 352-339-1980
MECHANICAL/ A/C B 1102	Print Name Greg Rhodes License #: CAC036941	Signature Greg Rhodes Phone #: 352-258-3553
PLUMBING/ GAS 728	Print Name Martin Van Musbergen License #: CFC1427326	Signature Martin Van Musbergen Phone #: 386-288-5111
ROOFING 1153	Print Name Bobby Campbell License #: CCC1326752	Signature Bobby Campbell Phone #: 904-449-2802
SHEET METAL	Print Name _____ License #: N/A	Signature _____ Phone #: N/A
FIRE SYSTEM/ SPRINKLER	Print Name _____ License#: N/A	Signature _____ Phone #: N/A
SOLAR	Print Name _____ License #: N/A	Signature _____ Phone #: N/A

Specialty License	License Number	Sub-Contractors Printed Name	Sub-Contractors Signature
MASON	↑	↑	
CONCRETE FINISHER	000310	LARRY PARRISH	
FRAMING 1163	CGC1516998	John Harrington	
INSULATION 732	CRC1327722	Gerald Kelsoe	
STUCCO 732	CRC1327722	Gerald Kelsoe	
DRYWALL 732	CRC1327722	Gerald Kelsoe	
PLASTER 732	CRC1327722	Gerald Kelsoe	
CABINET INSTALLER 1163	CGC1516998	John Harrington	
PAINTING 1163	CGC1516998	John Harrington	
ACOUSTICAL CEILING	N/A		
GLASS	N/A		
CERAMIC TILE 1163	CGC1516998	John Harrington	
FLOOR COVERING	N/A		
ALUM/VINYL SIDING	N/A		
GARAGE DOOR	N/A		
METAL BLDG ERECTOR	N/A		

F. S. 440.103 Building permits; identification of minimum premium policy.--Every employer shall, as a condition to applying for and receiving a building permit, show proof and certify to the permit issuer that it has secured compensation for its employees under this chapter as provided in ss. 440.10 and 440.38, and shall be presented each time the employer applies for a building permit.

Gave this info to John 1-15-13

Contractor Forms: Subcontractor form: 6/09

COLUMBIA COUNTY 9-1-1 ADDRESSING

P. O. Box 1787, Lake City, FL 32056-1787
PHONE: (386) 758-1125 * FAX: (386) 758-1365 * Email: ron_croft@columbiacountyfla.com

Addressing Maintenance

To maintain the Countywide Addressing Policy you must make application for a 9-1-1 Address at the time you apply for a building permit. The established standards for assigning and posting numbers to all principal buildings, dwellings, businesses and industries are contained in Columbia County Ordinance 2001-9. The addressing system is to enable Emergency Service Agencies to locate you in an emergency, and to assist the United States Postal Service and the public in the timely and efficient provision of services to residents and businesses of Columbia County.

DATE REQUESTED: 12/20/2012 **DATE ISSUED:** 12/27/2012

ENHANCED 9-1-1 ADDRESS:

1701 SW NEWARK DR

FORT WHITE FL 32038

PROPERTY APPRAISER PARCEL NUMBER:

00-00-00-01307-001

Remarks:

ADDRESS FOR PROPOSED STRUCTURE ON PARCEL.

Address Issued By: SIGNED:/ RONAL N. CROFT

Columbia County 9-1-1 Addressing / GIS Department

NOTICE: THIS ADDRESS WAS ISSUED BASED ON LOCATION INFORMATION RECEIVED FROM THE REQUESTER. SHOULD, AT A LATER DATE, THE LOCATION INFORMATION BE FOUND TO BE IN ERROR, THIS ADDRESS IS SUBJECT TO CHANGE.

2432

Columbia County Property Appraiser

CAMA updated: 12/19/2012

2012 Tax Year

Tax Collector | Tax Estimator | Property Card | Parcel List Generator | Interactive GIS Map | Print

Parcel: 00-00-00-01307-001

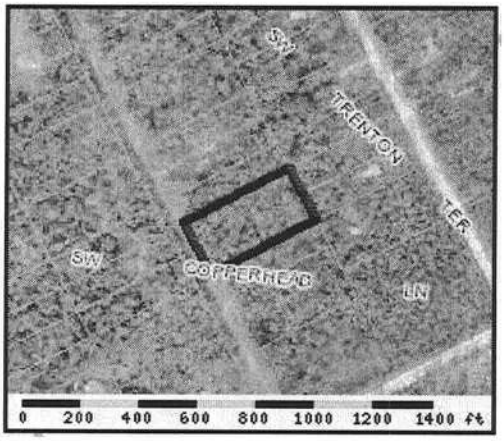
<< Next Lower Parcel | Next Higher Parcel >>

Search Result: 1 of 1

Owner & Property Info

Owner's Name	WHEARY KAREN & ROBERT		
Mailing Address	1702 SW NEWARK DRIVE FORT WHITE, FL 32038		
Site Address	NEWARK DRIVE		
Use Desc. (code)	VACANT (000000)		
Tax District	3 (County)	**Neighborhood**	100000
Land Area	1.836 ACRES	**Market Area**	02
Description	NOTE: This description is not to be used as the Legal Description for this parcel in any legal transaction.		

LOTS 14 & 15 UNIT 21 THREE RIVERS ESTATES. ORB 612-019, PROB#03-328CP ORB 1001-1245 THRU 1250, WD 1002-81, WD 1005-1506, WD 1040-1194, 1045-2493, WD 1048-1092, 1099-1227 WD 1130-1631 & WD 1241-1290



Property & Assessment Values

2012 Certified Values		
Mkt Land Value	cnt: (0)	$10,000.00
Ag Land Value	cnt: (2)	$0.00
Building Value	cnt: (0)	$0.00
XFOB Value	cnt: (0)	$0.00
Total Appraised Value		$10,000.00
Just Value		$10,000.00
Class Value		$0.00
Assessed Value		$10,000.00
Exempt Value		$0.00
Total Taxable Value		Cnty: $10,000 Other: $10,000 | Schl: $10,000

2013 Working Values

NOTE:
2013 Working Values are NOT certified values and therefore are subject to change before being finalized for ad valorem assessment purposes.

Show Working Values

Sales History

Show Similar Sales within 1/2 mile

Sale Date	OR Book/Page	OR Code	Vacant / Improved	Qualified Sale	Sale RCode	Sale Price
9/7/2012	1241/1290	WD	V	U	11	$7,500.00
9/11/2007	1130/1631	WD	V	U	01	$0.00

Building Characteristics

Bldg Item	Bldg Desc	Year Blt	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value
NONE						

Extra Features & Out Buildings

Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
NONE						

Land Breakdown

Lnd Code	Desc	Units	Adjustments	Eff Rate	Lnd Value
000000	VAC RES (MKT)	1 LT - (0000001.836AC)	1.00/1.00/1.00/1.00	$8,000.00	$8,000.00

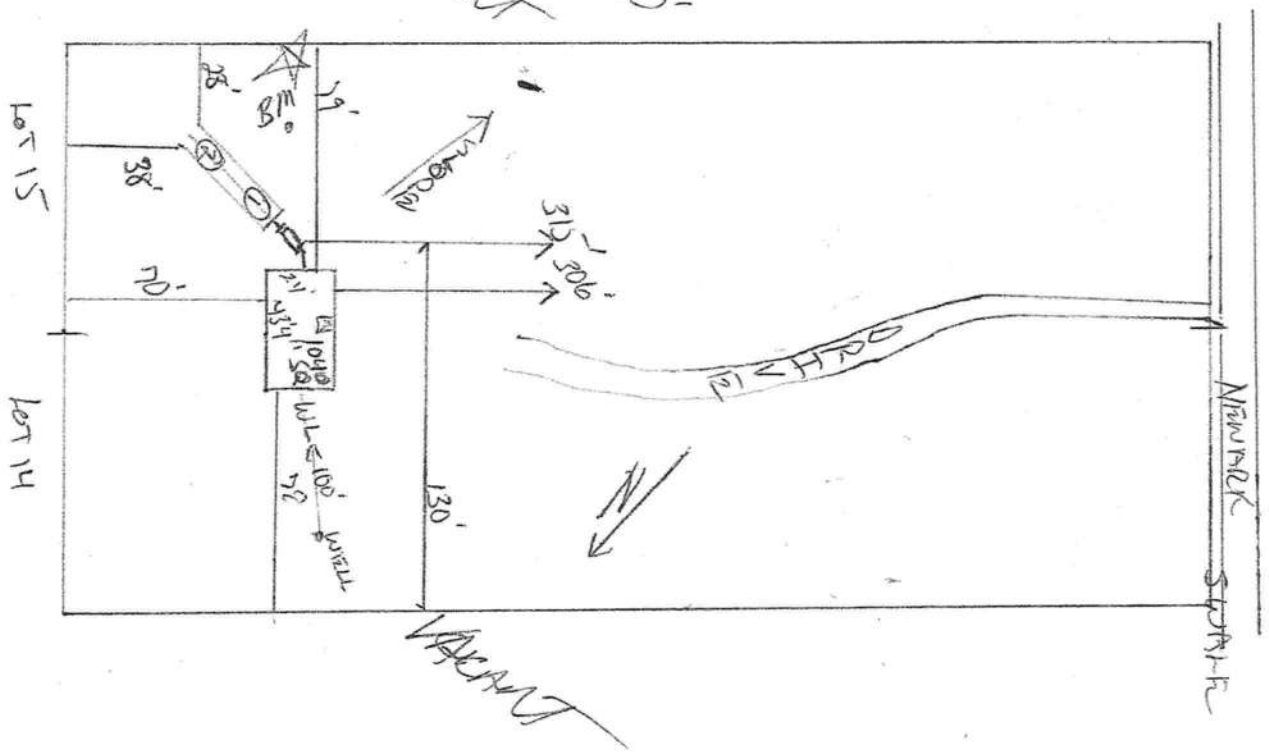
STATE OF FLORIDA
DEPARTMENT OF HEALTH
APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number_____

W HEARY

PART II - SITEPLAN

Scale: 1 inch = ~~40~~ feet.



Notes: _____

Site Plan submitted by: _____ MASTER CONTRACTOR

Plan Approved_____ Not Approved_____ Date_____

By_____ County Health Department

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

THIS INSTRUMENT PREPARED BY
AND RETURN TO:
NORTH CENTRAL FLORIDA TITLE, LLC
343 NW COLE TERRACE
SUITE 101
LAKE CITY, FLORIDA 32055

Parcel I.D. #: ***01307-001***
Permit No.

SPACE ABOVE THIS LINE FOR PROCESSING DATA — *SPACE ABOVE THIS LINE FOR RECORDING DATA*

NOTICE OF COMMENCEMENT

STATE OF FLORIDA
COUNTY OF COLUMBIA

THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement. This Notice shall be void and of no force and effect if construction is not commenced within ninety (90) days after recordation.

1. Description of property: (Legal description of property, and street address if available)

   TBD SW NEWARK DRIVE, FORT WHITE, FLORIDA 32038
   Lots 14 and 15, Three Rivers Estates, Unit 21, according to the plat thereof, recorded in Plat Book 6, Page 15, of the Public Records of Columbia County, Florida.

2. General description of improvement: **CONSTRUCTION OF A SINGLE FAMILY DWELLING**

3. Owner information:
   a. Name and address:
      ROBERT E. WHEARY and KAREN LEE WHEARY
      1702 SW NEWARK DRIVE, FORT WHITE, FLORIDA 32038
   b. Interest in property: **Fee Simple**
   c. Name and Address of Fee Simple Titleholder (if other than owner):

4. Contractor: (Name and Address)
   HOUSE CRAFT HOMES, LLC
   12501 NW US HWY 441, ALACHUA, FLORIDA 32615
   Telephone Number: **386-462-5323**

5. Surety (if any):
   a. Name and Address:
      Telephone Number: _____
   b. Amount of Bond $_____

6. Lender: (Name and Address)
   FIRST FEDERAL BANK OF FLORIDA
   4705 WEST U.S. HWY 90, LAKE CITY, FL 32055
   Telephone Number: **755-0600**

7. Persons within the State of Florida designated by Owner upon whom notice or other documents may be served as provided by Section 713.13(1)(a)(7), Florida Statutes: (Name and Address)
   N/A

8. In addition to himself, Owner designates the following person(s) to receive a copy of the Lienor's Notice as provided in Section 713.13(1)(b), Florida Statutes: (Name and Address)
   FIRST FEDERAL BANK OF FLORIDA
   4705 WEST U.S. HWY 90, LAKE CITY, FL 32055
   Telephone Number: **755-0600**

9. Expiration date of Notice of Commencement (the expiration date is 1 year from the date of recording unless a different date is specified) _____.

WARNING TO OWNER: ANY PAYMENTS MADE BY THE OWNER AFTER THE EXPIRATION OF THE NOTICE OF COMMENCEMENT ARE CONSIDERED IMPROPER PAYMENTS UNDER CHAPTER 713, PART I, SECTION 713.13, FLORIDA STATUTES, AND CAN RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE COMMENCING WORK OR RECORDING YOUR NOTICE OF COMMENCEMENT.

Signature of Owner(s) or Owner's Authorized Officer/Director/Partner/Manager:

{SEAL}
ROBERT E. WHEARY

{SEAL}
KAREN LEE WHEARY

The foregoing instrument was acknowledged before me this **20th** day of **December, 2012**, by **ROBERT E. WHEARY and KAREN LEE WHEARY**, who are personally known to me or who have produced *Driver's License* as identification.

Notary Public
My Commission Expires: 12-14-14

PATRICIA H. LANG
Commission # EE 046083
Expires December 14, 2014
Bonded Thru Troy Fain Insurance 800-385-7019

Under penalties of perjury, I declare that I have read the foregoing and that the facts stated in it are true to the best of my knowledge and belief.

Signature of Natural Person Signing Above

This Instrument Prepared by & return to:
Name: TRISH LANG, an employee of NORTH CENTRAL FLORIDA TITLE, LLC
Address: 343 NW COLE TERRACE, SUITE 101 LAKE CITY, FLORIDA 32055
File No. 12Y-09003TL

Parcel I.D. #: 01307-001

SPACE ABOVE THIS LINE FOR PROCESSING DATA

Inst:201212013664 Date:9/12/2012 Time:3:43 PM
Doc Stamp-Deed:52.50
DC,P.DeWitt Cason,Columbia County Page 1 of 2 B:1241 P:1290

SPACE ABOVE THIS LINE FOR RECORDING DATA

THIS WARRANTY DEED Made the 7th day of September, A.D. 2012, by **JOYE LYNN BALLARD** and **JOSEPH BRUCE DICKS, CONVEYING NON-HOMESTEAD PROPERTY,** hereinafter called the grantors, to **KAREN WHEARY** and **ROBERT WHEARY, HER HUSBAND,** whose post office address is , hereinafter called the grantees:

(Wherever used herein the terms "grantors" and "grantees" include all the parties to this instrument, singular and plural, the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporations, wherever the context so admits or requires.)

Witnesseth: That the grantors, for and in consideration of the sum of $10.00 and other valuable consideration, receipt whereof is hereby acknowledged, do hereby grant, bargain, sell, alien, remise, release, convey and confirm unto the grantees all that certain land situate in **Columbia County, State of Florida,** viz:

LOTS 14 AND 15, THREE RIVERS ESTATES, UNIT 21, ACCORDING TO THE MAP OR PLAT THEREOF AS RECORDED IN PLAT BOOK 6, PAGE 15, PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.

Together with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

To Have and to Hold the same in fee simple forever.

And the grantors hereby covenant with said grantees that they are lawfully seized of said land in fee simple; that they have good right and lawful authority to sell and convey said land, and hereby fully warrant the title to said land and will defend the same against the lawful claims of all persons whomsoever, and that said land is free of all encumbrances, except taxes accruing subsequent to December 31, 2012.

In Witness Whereof, the said grantors have signed and sealed these presents, the day and year first above written.

Signed, sealed and delivered in the presence of:

Terri Bell
Witness Signature
Terri Bell
Printed Name

Christol L. Gillespie
Witness Signature
Christol L. Gillespie
Printed Name

Joye Lynn Ballard L.S.
JOYE LYNN BALLARD
Address: 17 PARK CIRCLE, BLUFFTON, SOUTH CAROLINA 29910

STATE OF SOUTH CAROLINA
COUNTY OF BEAUFORT

The foregoing instrument was acknowledged before me this 7th day of September, 2012, by JOYE LYNN BALLARD, whose is known to me or who has produced SCDL as identification.

TERRI BELL NOTARY PUBLIC SOUTH CAROLINA

Terri Bell
Notary Public
My commission expires 9/24/20

Signed, sealed and delivered in the presence of:

Witness Signature

PATRICIA LANG

Printed Name

Witness Signature

BONITA HADWIN

Printed Name

L.S.

JOSEPH BRUCE DICKS

Address: 149 SW LUCILLE COURT, LAKE CITY, FLORIDA 32024

STATE OF FLORIDA
COUNTY OF COLUMBIA

The foregoing instrument was acknowledged before me this 7th day of September, 2012, by **JOSEPH BRUCE DICKS,** *who is known to me or who has produced* Personally Known *as identification.*

Notary Public
My commission expires 12-14-14

PATRICIA H. LANG
Commission # EE 046083
Expires December 14, 2014
Bonded Thru Troy Fain Insurance 800-385-7019

A&B Well Drilling, Inc.

5673 NW Lake Jeffery Road
Lake City, FL 32055
Telephone: (386) 758-3409
Cell: (386) 623-3151
Fax: (386) 758-3410
Owner: Bruce Park

January 11, 2013

To: Columbia County Building Department

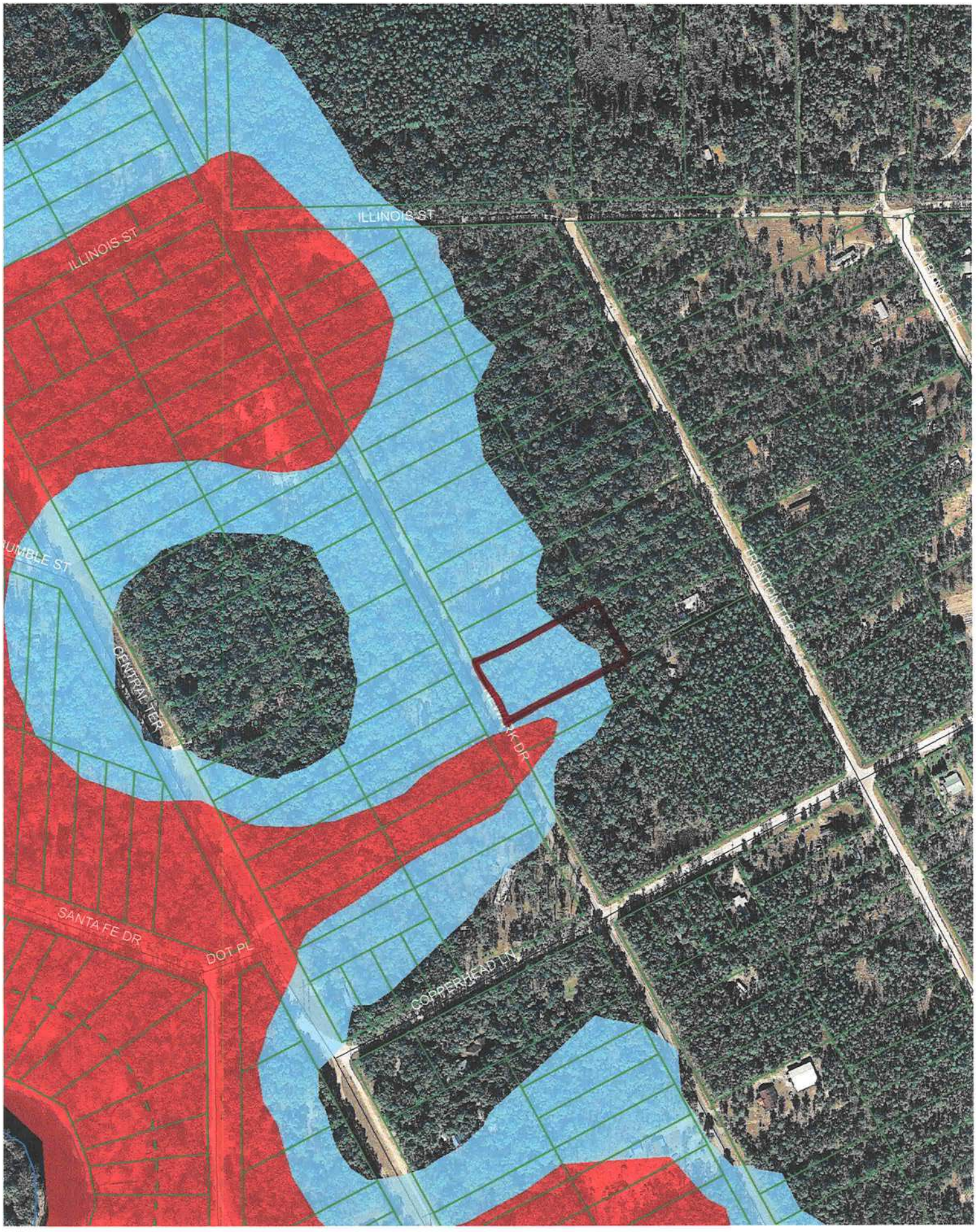
Description of Well to be installed for Customer ROBERT WHEALY

Located @ Address: LOTS 14+15 3 Rivers Estates NELMAR Dr.

1 HP 15 GPM submersible pump, 1 ¼" drop pipe, 86 gallon captive tank, and backflow prevention. With SRWMD permit.

Bruce N Park

Sincerely,
Bruce N. Park
President


ILLINOIS ST
ILLINOIS ST
UMBLE ST
CENTRAL TER
SANTA FE DR
DOT PL
COPPERHEAD LN
PRENTON TER

1301-26

STATE OF FLORIDA
DEPARTMENT OF HEALTH
ONSITE SEWAGE TREATMENT AND DISPOSAL SYSTEM
APPLICATION FOR CONSTRUCTION PERMIT

PERMIT NO. 13-0018
DATE PAID: 1/14/13
FEE PAID: 310.00
RECEIPT #: 1894005

APPLICATION FOR:
[X] New System [ ] Existing System [ ] Holding Tank [ ] Innovative
[ ] Repair [ ] Abandonment [ ] Temporary [ ] _____

APPLICANT: Robert Wheary

AGENT: ROCKY FORD, A & B CONSTRUCTION TELEPHONE: 386-497-2311

MAILING ADDRESS: P.O. BOX 39 FT. WHITE, FL, 32038

TO BE COMPLETED BY APPLICANT OR APPLICANT'S AUTHORIZED AGENT. SYSTEMS MUST BE CONSTRUCTED BY A PERSON LICENSED PURSUANT TO 489.105(3)(m) OR 489.552, FLORIDA STATUTES. IT IS THE APPLICANT'S RESPONSIBILITY TO PROVIDE DOCUMENTATION OF THE DATE THE LOT WAS CREATED OR PLATTED (MM/DD/YY) IF REQUESTING CONSIDERATION OF STATUTORY GRANDFATHER PROVISIONS.

PROPERTY INFORMATION

LOT: 14/15 BLOCK: na SUB: Three Rivers Estates unit 21 PLATTED: 1964

PROPERTY ID #: 00-00-00-013307-001 ZONING: RES. I/M OR EQUIVALENT: [Y / (N)]

PROPERTY SIZE: 1.836 ACRES WATER SUPPLY: [X] PRIVATE PUBLIC [ ]<=2000GPD [ ]>2000GPD

IS SEWER AVAILABLE AS PER 381.0065, FS? [Y / (N)] DISTANCE TO SEWER: ____ FT

PROPERTY ADDRESS: SW Newark Drive, Fort White, FL, 32038

DIRECTIONS TO PROPERTY: 47 South, TR on Wilson Springs Road, TR on Newark Drive

1.3 miles on right, 700 feet past Copperhead

BUILDING INFORMATION [X] RESIDENTIAL [ ] COMMERCIAL

Unit No	Type of Establishment	No. of Bedrooms	Building Area Sqft	Commercial/Institutional System Design Table 1, Chapter 64E-6, FAC
1	SF Residential	3	1040	
2				out of Flood Zone
3				

[N] Floor/Equipment Drains [N] Other (Specify) _____

SIGNATURE: Rocky D Ford DATE: 1/10/2013

DH 4015, 08/09 (Obsoletes previous editions which may not be used)
Incorporated 64E-6.001, FAC

STATE OF FLORIDA
DEPARTMENT OF HEALTH
APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

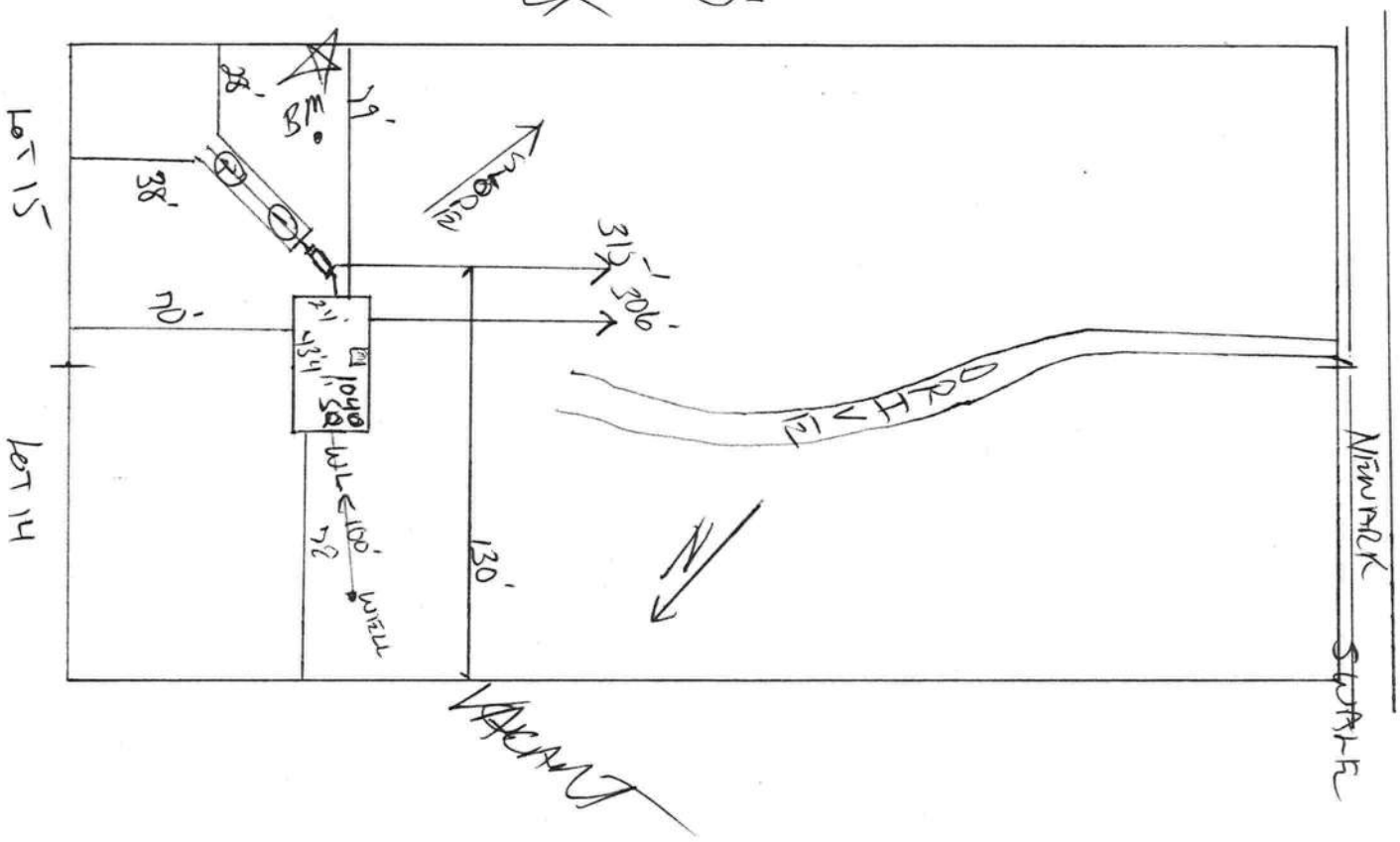
Permit Application Number 13-0018

W HEARY

60'

PART II - SITEPLAN

Scale: 1 inch = ~~40~~ feet.



Notes: _____

Site Plan submitted by: Rocky D 7-0

Plan Approved ✓ Not Approved ____

By Sallie Ford Env Health Director - Columbia

MASTER CONTRACTOR

Date 1-17-13

County Health Department

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

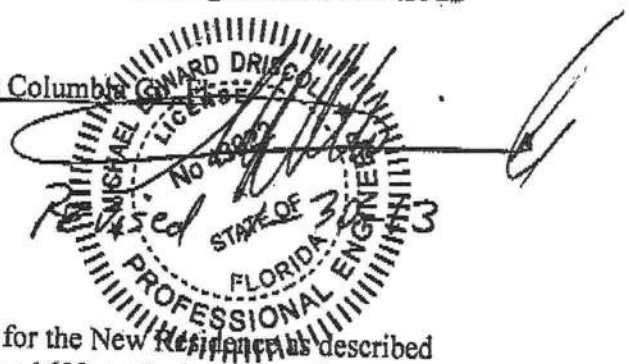
WIND ANALYSIS SUMMARY

DRISCOLL ENGINEERING, Inc.
P.O. BOX 357577.
Gainesville, FL 32606
352-331-1513
C.A. 8690

Project No. **DW12-20**

Michael E. Driscoll, P.E.
FL Registration No. 43922

1. Name: Wheary Residence
2. Address Lot 14 Three Rivers Estates Columbia County, FL
3. Description: New Residence



Certification

I hereby certify that the accompanying wind load analysis for the New Residence as described above demonstrates compliance with the FBC 2010 Section 1609, to the best of my knowledge.

Project Wind load Information

1. Ultimate wind speed = 130 MPH
2. Nominal wind speed = 101 MPH
3. Risk Category = II
4. Wind exposure for this design is Exposure B
5. Interior Pressure Coefficient or Gcpi = +/- 0.18
6. For design of MWFRS: see attached MECAWind Version 2.1.0.6 per ASCE 7-10
7. Roof Design live load 20 psf.
8. Floor Design load 40 psf.

Drawings

See drawings for additional details. In case of conflict, the more restrictive requirements of the drawings or these calculations govern.

Roof Structure

1. Trusses: Pre-engineered wood trusses at 24" o.c. The Truss engineering for this project was not available prior to the preparation of these wind-load calculations. A Typical Connector Schedule is provided for the convenience of the owner/builder as a selection guide only. If the truss uplift from the truss engineering exceeds the capacity of the specified connector, contact the Engineer immediately. Signed & Sealed Truss engineering shall be provided to Driscoll Engineering for review and confirmation of connector selection prior to beginning construction.

2. Roof Sheathing: Sheathing to be or 7/16" OSB min. to adequately resist exterior shear and uplift forces due to nailing. Panels to be facenailed w/ #8 ring shank @ 4" oc along edges and @ 8" oc along interior supports. Galv. metal edging to be nailed @ 4" oc.

3. Roofing : Asphalt Shingles shall be installed per mfg. specifications to meet 130 m.p.h. wind loading & in accord with the Florida Building Code 2010

Exterior Walls

1. Exterior Wall: 8" Concrete Masonry Units (ASTM C90 or C145, 1500 psi min) will adequately resist exterior shear forces. Mortar type M.
   a) Shearwalls: Transverse = 48 LF
      Longitudinal = 62 LF

2. Bond Beam to be (1) 8" min. Masonry with (1) #5 reinforcement with grout continuous. Note bond beam to remain continuous without breaks or interruptions to maintain shear transfer capacity. Minimum splice lap of #5 rebar is 25" at all locations. Install plated steel bearing plate at truss/masonry bearing points.

   Vertical spacing of grouted reinforced cells w/ (1) #5 rebar is to be 7'-0" o.c. typical. Install a minimum of 1 each vertical #5 bar in each cell on either side of each corner and on each side of any openings. Minimum splice lap of #5 rebar is 25".

Headers

1. Provide headers in accordance with Section 2308 of the *Florida Building Code, 2010.u.n.o.*
2. All wood header & beam connections to trusses shall be designed & engineered by the roof truss mfg.

Foundations (sizes based on wind load requirements only :

Footing: Stem wall footing: 20" wide x 10" deep w/ 2 #5 bars cont. 25"min bar lap.

CERTIFICATE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 25-6S-15-01307-001 **Building permit No.** 000030745

Use Classification SFD, UTILITY **Fire:** 32.10

Permit Holder JOHN D. HARRINGTON JR **Waste:** 83.75

Owner of Building ROBERT & KAREN WHEARY **Total:** 115.85

Location: 1701 SW NEWARK DRIE, FT. WHITE,FL 32038

Date: 05/16/2013

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)

FLORIDA PEST CONTROL & CHEMICAL CO.
Since 1949

FOUNDED 1949

CORPORATE HEADQUARTERS:
P.O. BOX 5369
116 N.W. 16TH AVENUE
GAINESVILLE, FL 32602-5369
(352) 376-2661
FAX (352) 376-2791

SCIENTIFIC PEST CONTROL DIRECTED BY GRADUATE ENTOMOLOGISTS

Complete Pest Control Service
Member Florida & National Pest Control Associations

HOUSE CRAFT HOMES
24113 OLD BELLAMY RD
HIGH SPRINGS, FL 32643-8931

Reply to: 116 NW 16th Ave
Gainesville, FL 32601
Phone (352) 376-2661 Fax (352) 376-2791

30745

TERMITE TREATMENT CERTIFICATION

Owner: ROBERT WHERERY	Permit Number:
Lot:	Block:
Subdivision:	Street Address: 1701 SW NEWARK DR
City: FT WHITE	County: Alachua
General Contractor: HOUSE CRAFT HOMES	Area Treated: EXTERIOR PERIMETER
Date: 05/16/13	Time: PM
Name of applicator GAREY HARRELL	Applicator ID Number: JE 185875
Product Used: Active Ingredient: % Concentration Premise: Imidacloprid: 0.10%	Number of gallons used: 33
Method of termite prevention treatment: **Soil treatment**	

The building has received a complete treatment for the prevention of subterranean termites. Treatment is in accordance with rules and laws established by the Florida Department of Agriculture and Consumer Services.

This form is proof of complete treatment for Certificate of Occupancy or Closing.

THIS IS PROOF OF WARRANTY

Warranty and Treatment Certifications Have Been Issued.

Authorized Signature:	Date:
Dempsey R Sapp Jr	5-16-13

OK
JC
5-17-13

Attention: Connie

Columbia County Building Department
Culvert Waiver

Culvert Waiver No. 000001987

DATE: 01/28/2013 BUILDING PERMIT NO. 30745

APPLICANT JOHN D. HARRINGTON PHONE 386-462-5323

ADDRESS 24015 NW OLD BELLAMY RD HIGH SPRINGS FL 32643

OWNER ROBERT & KAREN WHEARY PHONE

ADDRESS 1701 SW NEWARK DRIVE FORT WHITE FL 32038

CONTRACTOR JOHN D. HARRINGTON JR PHONE 352-538-5963

LOCATION OF PROPERTY 47 S, R WILSON SPRINGS RD, R NEWARK, 1 MILE ON RIGHT

SUBDIVISION/LOT/BLOCK/PHASE/UNIT THREE RIVERS ESTATES 14.15 21

PARCEL ID # 25-6S-15-01307-001

I HEREBY CERTIFY THAT I UNDERSTAND AND WILL FULLY COMPLY WITH THE DECISION OF THE COLUMBIA COUNTY PUBLIC WORKS DEPARTMENT IN CONNECTION WITH THE HEREIN PROPOSED APPLICATION.

SIGNATURE: John Harrington

A SEPARATE CHECK IS REQUIRED
MAKE CHECKS PAYABLE TO BCC

Amount Paid 50.00

PUBLIC WORKS DEPARTMENT USE ONLY

I HEREBY CERTIFY THAT I HAVE EXAMINED THIS APPLICATION AND DETERMINED THAT THE CULVERT WAIVER IS:

✓ APPROVED _____ NOT APPROVED - NEEDS A CULVERT PERMIT

COMMENTS:

SIGNED: James Dunaven DATE: 2-1-13

ANY QUESTIONS PLEASE CONTACT THE PUBLIC WORKS DEPARTMENT AT 386-752-5955

COLUMBIA COUNTY FLORIDA

Mark D. Duren and Associates, Inc.
Professional Surveyor and Mapper

1604 SW Sisters Welcome Road
Lake City, FL 32025
386-758-9831 Phone
386-758-8010 Fax

Permit # 30745

OK
02/18/13
BLK

February 15, 2013

To whom it may concern,

The elevation of the floor of the dwelling under construction on Lots 14 and 15, "Three Rivers Estates, Unit 21", Columbia County Tax Parcel No. 00-00-00-01307-001 is 39.01 feet, NAVD 1988 datum. The ground elevation around the dwelling is 37 feet to 38 feet, NAVD 1988 Datum.

Mark D. Duren, LS 4708

Mark D. Duren And Associates, Inc.

352.538.5963

Brian Kepner

From: Jean Duren [jldmddpsm@gmail.com]
Sent: Friday, February 15, 2013 9:21 AM
To: Brian Kepner
Subject: Wheary Elevation letter on Slab
Attachments: 13-043 elev letter.pdf

Brian,
Robert & Karen Wheary on Lots 14 &15 Three Rivers Est. Un. 21 needed a letter on the elevation of their slab; It is attached.

I was asked to send it to you.
Thanks,
Jean Lea
@ Mark D. Duren and Associates, Inc.
386-758-9831

Permit # _____
User ID _____

PRODUCT APPROVAL SPECIFICATION SHEET

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and approval numbers on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. Statewide approved products are listed online @ www.floridabuilding.org

Category/Subcategory	Manufacturer	Product Description	Approval Number(s)	X
1. EXTERIOR DOORS				
A. SWINGING	Masonite	Entry Door	FL8228.1	
B. SLIDING	HR Danvid	Glass Door	FL5600.1	
C. SECTIONAL/ROLL UP	Overhead Door	Garage door	FL 674	
D. OTHER				
2. WINDOWS				
A. SINGLE/DOUBLE HUNG	SILVERLINE	SINGLE	FL12068.4	
B. HORIZONTAL SLIDER				
C. CASEMENT				
D. FIXED	SLIVERLINE	FIXED	FL8063.1	
E. MULLION			FL8478.1	
F. SKYLIGHTS				
G. OTHER / GLASS BLOCK	Hy-Lite	Glass Block window	FL 1956.3	
3. PANEL WALL				
A. SIDING				
B. SOFFITS	Kaycan	Aluminum soffits	FL 1146.5	
C. STOREFRONTS				
D. GLASS BLOCK				
F. OTHER				
4. ROOFING PRODUCTS				
A. ASPHALT SHINGLES	Tamko	Heritage 38-R	FL 1956.3	
B. NON-STRUCT METAL				
C. ROOFING TILES				
D. SINGLE PLY ROOF				
E. OTHER				
5. STRUCT COMPONENTS				
A. WOOD CONNECTORS				
B. WOOD ANCHORS	Simpson	Truss anchors	1901.17 1901.45	
C. TRUSS PLATES			1901.25 1901.21	
D. INSULATION FORMS				
E. LINTELS	Cenemt Precast	Concrete lintels	FI 3048	
F. TRUSSES	Julius Lee	engineer	PE 34869	
6. NEW EXTERIOR				
ENVELOPE PRODUCTS				
A.				

The products listed below did not demonstrate product approval at plan review. I understand that at the time of inspection of these products, the following information must be available to the inspector on the jobsite: 1) copy of the product approval, 2) performance characteristics which the product was tested and certified to comply with, 3) copy of the applicable manufacturers installation requirements. Further, I understand these products may have to be removed if approval cannot be demonstrated during inspection.

COLUMBIA COUNTY BUILDING DEPARTMENT
Received for FILE COPY
Code Compliance
PLANS EXAMINER

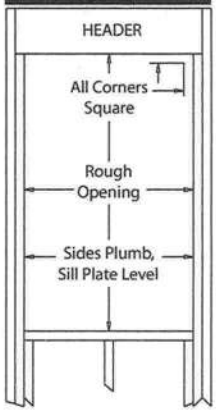
_____ _____
APPLICANT SIGNATURE DATE

Installation Instructions For New Construction
Single-Hung, Double-Hung, Sliding, Casement, Awning And Picture Windows

Window Opening

The rough opening must be plumb, level, square and 1/2" larger than the window size in width and height, not including the nailing fins (See fig.1). Close and lock the sash to aid in keeping the window square during installation. **Note:** Install sill flashing before the window is installed (refer to "Flashing" below). Apply a 3/8" continuous bead of silicone caulking to the interior surface of the nailing fin, covering the holes in the fin, to seal the window's fin to the sheathing or house wrap. If the rough opening is larger than the window unit by more than 1/2", also apply the caulk to the sheathing or house wrap making sure the bead is no more than 1/4" from the edge of the rough opening, so that it is covered by the nailing fin when the window is installed.

FIGURE 1



Setting Shims

The sill of the window must be supported in a straight and level position, with shims at all locations where the jamb, intermediate jamb, or the stiles of a slider meet the sill (See fig. 2A & 2B). Place 1/4" shims on the sill plate of the window opening spaced as described above. Multiple, twin or triple windows should have a shim under each mullion, intermediate jamb or the center stiles of sliders (See fig. 2A & 2B).

Setting the Window

Set window on the shims and adjust side clearance to be equal on both sides. Fasten one upper corner of the nailing fin to keep the window in place. Check the sill with a level and adjust the shims as required to level sill. Do not force shims into place, possibly bowing the window frame. Shim both sides of window as needed to assure window is plumb and margins are equal (see fig. 2A & 2B). Measure window diagonally from bottom, left corner to top, right corner and from bottom, right corner to top, left corner to ensure it is square. If the above has been done correctly the width across the top, middle and bottom of the window will measure the same. The weatherstripping clearance between the sash and frame should be equal. The meeting rail and lock rail should align evenly, with parallel sight lines. The window locks should engage smoothly.

FIGURE 2B
SLIDING INTERIOR

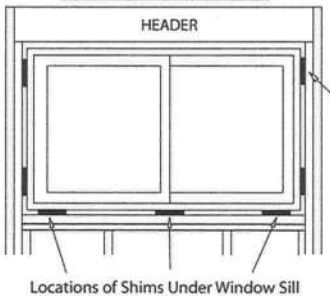
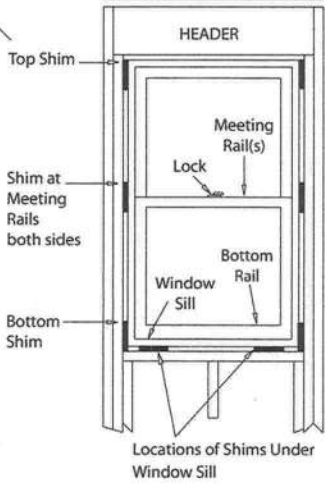


FIGURE 2A
SINGLE- / DOUBLE-HUNG
INTERIOR



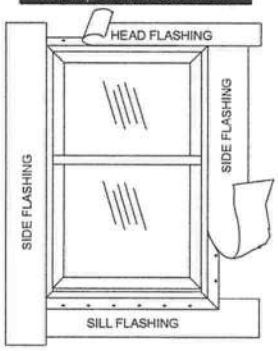
Fastening the Window

Use stainless or galvanized steel fasteners, long enough to penetrate studs a minimum of 1".

Fasten the entire perimeter of the nailing fin to the sheathing using every other slotted hole at minimum on single windows. Multiple windows, twins, & triples should be nailed in every slotted hole.

Fasten the fin snug but do not "sink" the nails. Fasteners should be just tight enough to hold the window but not stop the movement of the framing underneath during expansion and contraction. Make sure the head and sill are NOT crowned up or down, or the jambs bowed in or out.

FIG.3, EXTERIOR VIEW

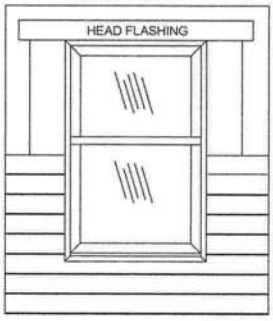


Flashing

Use self-adhesive flexible flashing, a minimum of 4" wide (6" for windows rated above DP/PG 50), approved for use on vinyl, wood and other substances, such as house wrap. Must meet or exceed the Minimum Requirements for Flashing Material in ASTM E 2112-07 Standard Practice for Installation of Exterior Windows, Doors and Skylights.

Sill flashing should already be applied prior to window installation and extend beyond the sides of the window nailing fin at least 2" (See fig. 3). Now apply jamb flashing over the jamb nailing fin, continuing over and beyond the sill flashing, at least 2". Apply head flashing similarly extending 2" past either side of the jamb flashing, to complete the window flashing detail.

FIG.4, EXTERIOR VIEW



Install batt insulation between the window and rough opening. It is very important that these openings are not "over stuffed" and warp the frame. Only use low expansion, low pressure foam products, made specifically for use with windows and doors, that comply with AAMA 812. Do not overfill to avoid bowing frame.

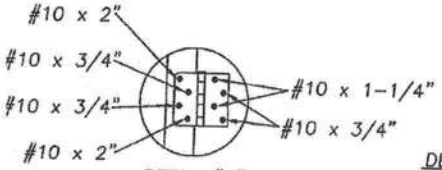
If the exterior finish is brick, stone or stucco, make sure to leave a 1/4" gap around the entire window to allow for the expansion of materials.

Cautions

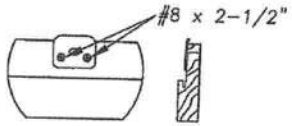
- Remove or cut ventilation holes in plastic shipping wrap if windows are not installed immediately.
- Do not lay windows flat or store in the sun. The heat will shrink the plastic wrapping and warp the frame.
- Do not caulk or plug weep holes.
- Do not drill into or through the sill of the window.
- Protect vinyl sill from traffic and damage.
- Do not lift window by top of frame, only by jambs.
- Protect the window during construction and plastering.

Please call our hot line 1(800)-234-4228 for any installation help that you might need.

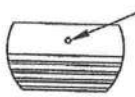
Note: The manufacturer's warranty can be voided if these instructions are not followed. If special applications are needed during the installation you must contact the manufacturer for approval.



DETAIL "C"

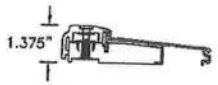


DETAIL "E" ASTRAGAL
ATTACH ASTRAGAL RETAINER BOLT STRIKE PLATE TO FRAME AS SHOWN.



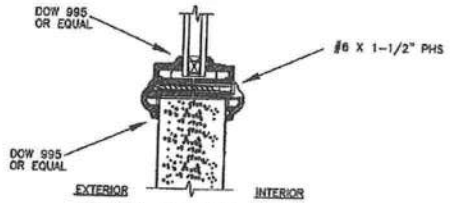
ASTRAGAL RETAINER BOLT HOLE MUST BE DRILLED THROUGH THE THRESHOLD & INTO THE STRUCTURE DEEP ENOUGH FOR A 1.375" THROW

DETAIL "F" ASTRAGAL



INSWING THRESHOLD

OUTSWING THRESHOLD



TYPICAL GLAZING DETAIL

Addendum to NAMI
Certification No.: NT 00L215
Reviewed By:
Date Reviewed: 9/23/05

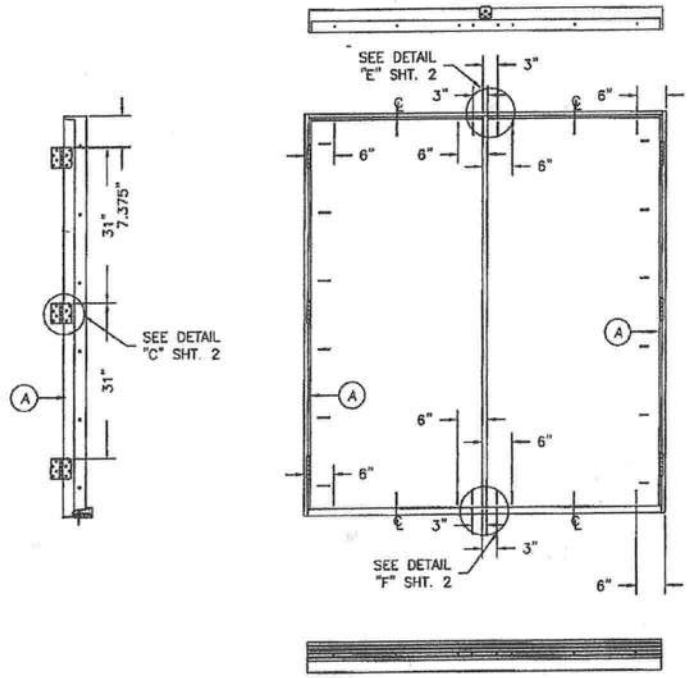
MASONITE INTERNATIONAL CORP.
7300 REAMES RD.
CHARLOTTE, NC 28216

PRODUCT: "EXTERIOR DOOR PRODUCT" 6'-8" GLAZED DOUBLE DOOR UNIT

PART OR ASSEMBLY: ANCHORING LOCATIONS & DETAILS

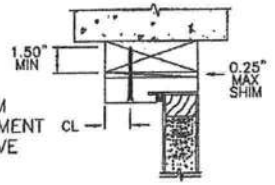
NO.	DATE	REVISIONS	BY

DATE: 9/20/05
SCALE: N.T.S.
DWG. BY: SWS
CHK. BY:
DRAWING NO.: DWG-MA-FL0142-05
SHEET 2 OF 3



ATTACHMENT DETAIL

1. ANCHOR ANALYSIS FOR LOADING CONDITIONS PREPARED, SIGNED AND SEALED BY HAROLD E. RUPP, PE (FLORIDA #15935) WITH THE LOWEST (LEAST) FASTENER RATING FROM THE DIFFERENT FASTENERS BEING CONSIDERED FOR USE. JAMB, HEAD, AND THRESHOLD FASTENERS ANALYZED FOR THIS UNIT INCLUDE #10 WOOD SCREWS OR 3/16" TAPCONS. A PHYSICAL SHIM MUST BE PLACED IN SHIM SPACE AT EACH ANCHOR LOCATION.
2. THE WOOD SCREW SINGLE SHEAR DESIGN VALUES COME FROM ANSI/AF&PA NDA FOR SOUTHERN PINE LUMBER AND ACHEIVEMENT OF 1-1/2" MINIMUM EMBEDMENT. THE TAPCON MUST ACHIEVE MINIMUM EMBEDMENT OF 1-1/4".
3. WOOD BUCKS BY OTHERS MUST BE ANCHORED PROPERLY TO TRANSFER LOADS TO STRUCTURE.
4. MINIMUM DESIGN VALUE STRENGTH OF ANCHORS 171 LBS.



TYPICAL ANCHOR INSTALLATION

HARDWARE SCHEDULE

1.	KWIKSET OR SCHLEGE ANSI/BHMA GRADE 3 OR BETTER CYLINDRICAL AND DEADLOCK HARDWARE TO BE INSTALLEI AT 5-1/2" CENTERLINE WITH 8" GRADE 1 (ANSI/BHMA SURFACE BOLTS INSTALLED ON LATCH SIDE OF ACTIVE DOOR PANEL - (1) AT TOP AND (1) AT BOTTOM.
2.	4" X 4" FULL MORTISE BUTT HINGES

Addendum to NAMI
Certification No: NJ-0016215
Reviewed By:
Date Reviewed: 9/23/05

Masonite®

HINGED WOOD-EDGE STEEL DOOR UNIT
6'-8" GLAZED DOUBLE DOOR

ENERAL NOTES

EVALUATED FOR USE IN LOCATIONS ADHERING TO THE FLORIDA BUILDING CODE AND WHERE PRESSURE REQUIREMENTS AS DETERMINED BY ASCE 7, MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES, DOES NOT EXCEED THE DESIGN PRESSURES LISTED.

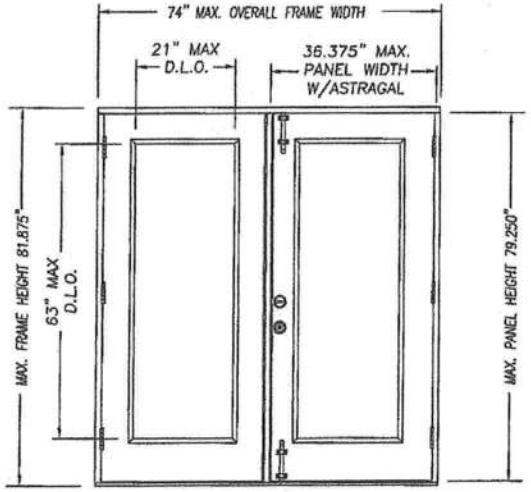
HURRICANE PROTECTIVE SYSTEM (SHUTTERS) IS REQUIRED.

POLYURETHANE CORE FLAME SPREAD INDEX OF 50 AND SMOKE DEVELOPED INDEX OF 60 PER ASTM E84

PLASTICS TESTING OF LITE FRAME MATERIAL:

TEST DESCRIPTION	DESIGNATION	RESULT
SELF IGNITION TEMP	ASTM D1929	680 °F > 650 °F
RATE OF BURNING	ASTM D635	1.10 IN/MIN
SMOKE DENSITY	ASTM D2843	69.6%
TENSILE STRENGTH*	ASTM D638	-7.48% DIFF

* COMPARATIVE TENSILE STRENGTH AFTER WEATHERING 4500 HOURS XENON ARC METHOD 1



DOUBLE DOOR UNIT

Addendum to NAMI
Certification No.: NJ 006215
Reviewed By:
Date Reviewed: 9/23/05

CONFIG	MAX WIDTH	DESIGN PRESSURE RATING INSWING	DESIGN PRESSURE RATING OUTSWING	WHERE WATER INFILTRATION PERFORMANCE IS REQUIRED TO BE 15% OF DESIGN PRESSURE INSWING	WHERE WATER INFILTRATION PERFORMANCE IS REQUIRED TO BE 15% OF DESIGN PRESSURE OUTSWING
XX	74"	+50.5 / -50.5	+50.5 / -50.5	+19.0 / -19.0	+50.5 / -50.5

TABLE OF CONTENTS

MASONITE INTERNATIONAL CORP.
7300 REAMES RD.
CHARLOTTE, NC 28216

PRODUCT: "EXTERIOR DOOR PRODUCT" 6'8" GLAZED DOUBLE DOOR UNIT

PART OR ASSEMBLY: TYPICAL ELEVATIONS & GENERAL NOTES

NO.	DATE	REVISIONS	BY

DATE: 9/20/05
SCALE: N.T.S.
DWG. BY: SWS
CHK. BY:
DRAWING NO.: DWG-MA-FL0142-05
SHEET 1 OF 3

WINDLOAD ANALYSIS

PREPARED BY

DRISCOLL ENGINEERING,INC.
CONSULTING ENGINEERS
PO BOX 357577 CA 8690
GAINESVILLE, FL. 32606 PH (352) 331-1513

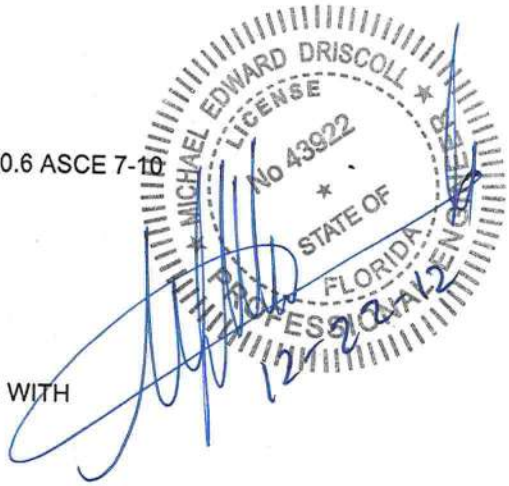
12-22-12

PREPARED FOR

CLIENT:

WHEARY RESIDENCE
LOT 14 THREE RIVERS ESTATES
COLUMBIA CO , FL
DW12-20

INDEX



WINDLOAD ANALSIS HAVE BEEN PREPARED IN COMPLIANCE WITH THE 2010 FLORIDA BUILDING CODE

FOR WINDLOAD ANALYSIS ONLY

MICHAEL E. DRISCOLL P.E.
FL REG. #43922

WIND ANALYSIS SUMMARY

DRISCOLL ENGINEERING, Inc.
P.O. BOX 357577.
Gainesville, FL 32606
352-331-1513
C.A. 8690

Project No. **DW12-20**

Michael E. Driscoll, P.E.
FL Registration No. 43922

1. Name: Wheary Residence
2. Address Lot 14 Three Rivers Estates Columbia Co, Fl
3. Description: New Residence

Certification

I hereby certify that the accompanying wind load analysis for the New Residence as described above demonstrates compliance with the FBC 2010 Section 1609, to the best of my knowledge.

Project Wind load Information

1. Ultimate wind speed = 130 MPH
2. Nominal wind speed = 101 MPH
3. Risk Category = II
4. Wind exposure for this design is Exposure B
5. Interior Pressure Coefficient or Gcpi = +/- 0.18
6. For design of MWFRS: see attached MECAWind Version 2.1.0.6 per ASCE 7-10
7. Roof Design live load 20 psf.
8. Floor Design load 40 psf.

Drawings

See drawings for additional details. In case of conflict, the more restrictive requirements of the drawings or these calculations govern.

Roof Structure

1. Trusses: Pre-engineered wood trusses at 24" o.c. The Truss engineering for this project was not available prior to the preparation of these wind-load calculations. A Typical Connector Schedule is provided for the convenience of the owner/builder as a selection guide only. If the truss uplift from the truss engineering exceeds the capacity of the specified connector, contact the Engineer immediately. Signed & Sealed Truss engineering shall be provided to Driscoll Engineering for review and confirmation of connector selection prior to beginning construction.

2. Roof Sheathing: Sheathing to be or 7/16" OSB min. to adequately resist exterior shear and uplift forces due to nailing. Panels to be facenailed w/ #8 ring shank @ 4" oc along edges and @ 8" oc along interior supports. Galv. metal edging to be nailed @ 4" oc.

3. Roofing : Asphalt Shingles shall be installed per mfg. specifications to meet 130 m.p.h. wind loading & in accord with the Florida Building Code 2010

Exterior Walls

1. Exterior Wall: 8" Concrete Masonry Units (ASTM C90 or C145, 1500 psi min) will adequately resist exterior shear forces. Mortar type M.
   a) Shearwalls: Transverse = 48 LF
      Longitudinal = 62 LF

2. Bond Beam to be (1) 8" min. Masonry with (1) #5 reinforcement with grout continuous. Note bond beam to remain continuous without breaks or interruptions to maintain shear transfer capacity. Minimum splice lap of #5 rebar is 25" at all locations. Install plated steel bearing plate at truss/masonry bearing points.

   Vertical spacing of grouted reinforced cells w/ (1) #5 rebar is to be 7'-0" o.c. typical. Install a minimum of 1 each vertical #5 bar in each cell on either side of each corner and on each side of any openings. Minimum splice lap of #5 rebar is 25".

Headers

1. Provide headers in accordance with Section 2308 of the *Florida Building Code, 2010.u.n.o.*
2. All wood header & beam connections to trusses shall be designed & engineered by the roof truss mfg.

Foundations (sizes based on wind load requirements only :

Footing: Stem wall footing: 20" wide x 12" deep w/ 2 #5 bars cont. 25"min bar lap.

M.E. Driscoll, P.E.
Effective 01-01-12

+GENERAL NOTES

Design Criteria

1. Structure to meet wind load requirements of FBC 2010 amendments to SEC. 1609 for a design wind speed of 140 mph.
2. Wood framing and fasteners to meet NDS-2005 requirements.
3. Fastener requirements: (1) All nails are Common galvanized; (UNO) (2) all bolts are to be galvanized steel and include nuts and washers; and (3) all other hardware (Simpson, etc.) is to be installed according to manufacturer's specifications and recommendations. Nailing (size and number) shall satisfy Tables 2306.3.1, 2306.3.2 and 2306.4.1 FBC unless otherwise indicated. Note: fasteners exposed to the weather are to be treated for weather resistance and compatible with the type of pressure treated wood used (connectors, nails, bolts, nuts and washers).
4. Fasteners shall be driven flush with surface of sheathing.

Concrete Construction Notes

1. Concrete work shall conform to "Building Code Requirements for Reinforced Concrete" (ACI-318) and "Specifications for Structural Concrete" (ACI-301), Latest Edition.
2. Concrete Mix "A" shall be used for foundation walls, footings and interior slabs on grade. Concrete mix "B" shall be used for exterior slabs, curbs and all other exterior concrete. All concrete mixes shall contain a water-reducing admixture conforming to ASTM C-494. Air-entraining admixture shall conform to ASTM C-260.

	Mix A	Mix B
Ultimate Compressive Strength @ 28 days	3000 psi	3000 psi
Slump Range	4"+/-1"	3"+/-1"
Maximum Aggregate Size	1"	1"
Entrained Air	None	5-7%
Dry Weight per Cubic Foot	150#	150#

3. All concrete shall be cured for a minimum of 28 days. If forms for vertical surfaces are removed prior to the end of the curing period, spray surfaces with liquid membrane curing compound.
4. Reinforcing steel shall conform to ASTM A615, Grade 40 (Fy=40 ksi). Lap continuous bars for tension lap splice per ACI-318, unless otherwise noted. Provide corner bars of same size and spacing as horizontal wall reinforcement. Cover for concrete reinforcing steel shall be in accordance with ACI-318, Paragraph 7.7.
5. Welded wire fabric (WWF) shall conform to ASTM A185. Lap sheets on mesh space and wire tie adjacent sheets together securely. Cut alternate reinforcement at control joints.
6. All slabs on grade shall have construction or control joints not to exceed 15' – 0" spacing, unless otherwise noted.
7. Electrical conduit and other pipes to be embedded in structural concrete floor slabs or walls shall be placed in accordance with the requirements of ACI-318, Paragraph 6.3.

Masonry Construction Notes

1. Concrete masonry work shall conform to "Building Code Requirements for Masonry Structures" (ACI 530-02/ASCE5-05) and "Specifications for Masonry Structures: (ACI 530.1-05/ASCE6-05).
2. Concrete masonry units shall be Type 1 and comply with "Standard Specifications for Hollow Load-Bearing Concrete Masonry Units" (ASTM C90-90).
3. The minimum net area compressive strength of masonry (f'm), as determined by the unit strength method, shall be 1500 psi.
4. Mortar shall conform to ASTM C270. Type M Mortar shall be used unless otherwise noted. Type S Mortar shall be used with masonry in contact with earth.
5. Masonry column reinforcement shall have #2 ties in the bed joints at 8" oc, unless otherwise noted.
6. Grout for filling block cores and bond beams shall have a minimum compressive strength (f'c) of 3000 psi at the age of 28 days.

PROFESSIONAL SERVICES BY
DRISCOLL ENGINEERING,INC.
PO BOX 357577.
GAINESVILLE, FL 32609
PH (352)-331-1513
CA 8690

PLANS AND SPECIFICATIONS

The plans and specifications presented herein are applicable only for the anticipated construction at the locations shown. If construction plans change, the Design Professional should be notified so the plans and specifications can be re-evaluated. The Design Professional should be given the opportunity to review final plans and specifications to see if the intent of the plans and specifications has been followed and/or if supplemental details and recommendations are needed. The Design Professional warrants that the plans and specifications contained herein, have been prepared in accordance with generally accepted professional engineering practice. No other warranties are implied or expressed.

CORPORATE PROTECTION

It is understood and agreed that the Design Professional's Basic Services under this Agreement do not include project observation or review of the Contractor's performance or any other construction phase services, and that such services will be provided by the Client. The Client assumes all responsibility for interpretation of the contractor Documents and for construction observation and supervision and waives any claims against the Design Professional that may be in any way connected thereto.

In addition, the Client agrees, to the fullest extent permitted by law, to indemnify and hold the Design Professional harmless from any loss, claim or cost, including reasonable attorney's fees and costs of defense, arising or resulting from the performance of such services by other person or entities and from any and all claims arising from modifications, clarifications, interpretations, adjustments or changes made to Contract Documents to reflect changed field or other conditions, except for claims arising from the sole negligence or willful misconduct to the Design Professional.

OWNERSHIP OF INSTRUMENTS OF SERVICE

All reports, plans, specifications, computer files, field data, notes and other documents and instruments prepared by the Design Professional as instruments of service shall remain the property of the Design Professional. The Design Professional shall retain all common law, statutory and other reserved rights, including the copyright thereto.

DEFECTS IN SERVICE

The Client shall promptly report to the Design Professional any defects or suspected defects in the Design Professional's work or services of which the Client becomes aware, so that the Design Professional may take measures to minimize the consequences of such a defect. The Client warrants that he or she will impose a similar notification requirement on all contractors in his or her Client/Contractor contract and shall require all subcontractors at any level to contain a like requirement. Failure by the Client, and the Client's contractors or subcontractors to notify the Design Professional, shall relieve the Design Professional of the costs of remedying the defects above the sum such remedy would have cost had prompt notification been given.

VERIFICATION OF EXISTING CONDITIONS

Inasmuch as the remodeling and/or rehabilitation of an existing building requires that certain assumptions be made regarding existing conditions, and because some of these assumptions may not be verifiable without expending additional sums of money or destroying otherwise adequate or serviceable portions of the building, the Client agrees, to the fullest extent permitted by law, to indemnify and hold the Design Professional harmless from any claim, liability or cost (including reasonable attorney's fees and costs of defense) for injury or economic loss arising or allegedly arising out of the professional services provided under this Agreement, excepting only those damages, liabilities, or costs attributable to the sole negligence or willful misconduct of the Design Professional.

MECAWind Version 2.1.0.6 per ASCE 7-10

Developed by MECA Enterprises, Inc. Copyright 2012 www.mecaenterprises.com

Date	: 12/22/2012	Project No.	: DW12-20
Company Name	: Driscoll Engineering, Inc	Designed By	: MED
Address	: PO Box 357577	Description	: New Residence
City	: Gainesville	Customer Name	: Wheary
State	: Fl	Proj Location	: Lot 14 Three Rivers Estates Co

File Location: C:\Program Files\MECAWind\Default.wnd

Directional Procedure Simplified Diaphragrm Building (Ch 27 Part 2)

All pressures shown are based upon STRENGTH Design, with a Load Factor of 1

Basic Wind Speed(V)	= 130.00 mph		
Structural Category	= II	Exposure Category	= B
Natural Frequency	= N/A	Flexible Structure	= No
Importance Factor	= 1.00	Kd Directional Factor	= 0.85
Alpha	= 7.00	Zg	= 1200.00 ft
At	= 0.14	Bt	= 0.84
Am	= 0.25	Bm	= 0.45
Cc	= 0.30	l	= 320.00 ft
Epsilon	= 0.33	Zmin	= 30.00 ft
Slope of Roof	= 1.67 : 12	Slope of Roof(Theta)	= 7.91 Deg
Ht: Mean Roof Ht	= 10.50 ft	Type of Roof	= Hipped
RHt: Ridge Ht	= 13.00 ft	Eht: Eave Height	= 8.00 ft
OH: Roof Overhang at Eave= 2	24.00 ft	Roof Area	= 3893.00 ft^
Bldg Length Along Ridge	= 44.00 ft	Bldg Width Across Ridge=	24.00 ft

Gust Factor Category I Rigid Structures - Simplified Method

Gust1: For Rigid Structures (Nat. Freq.>1 Hz) use 0.85 = 0.85

Gust Factor Category II Rigid Structures - Complete Analysis

Zm: 0.6*Ht	=	30.00 ft
lzm: Cc*(33/Zm)^0.167	=	0.30
Lzm: l*(Zm/33)^Epsilon	=	309.99 ft
Q: (1/(1+0.63*((B+Ht)/Lzm)^0.63))^0.5	=	0.93
Gust2: 0.925*((1+1.7*lzm*3.4*Q)/(1+1.7*3.4*lzm))	=	0.88

Gust Factor Summary

Not a Flexible Structure use the Lessor of Gust1 or Gust2 = 0.85

Table 26.11-1 Internal Pressure Coefficients for Buildings, GCpi

GCPi : Internal Pressure Coefficient = +/-0.18

Topographic Adjustment

0.33*z	=	3.47
Kzt (0.33*z): Topographic factor at elevation 0.33*z	=	1.00
Vtopo: Adjust V per Para 27.5.2: V * [Kzt(0.33*z)]^0.5	=	130.00 mph

Net Wind Pressures on Walls (Table 27.6-1)

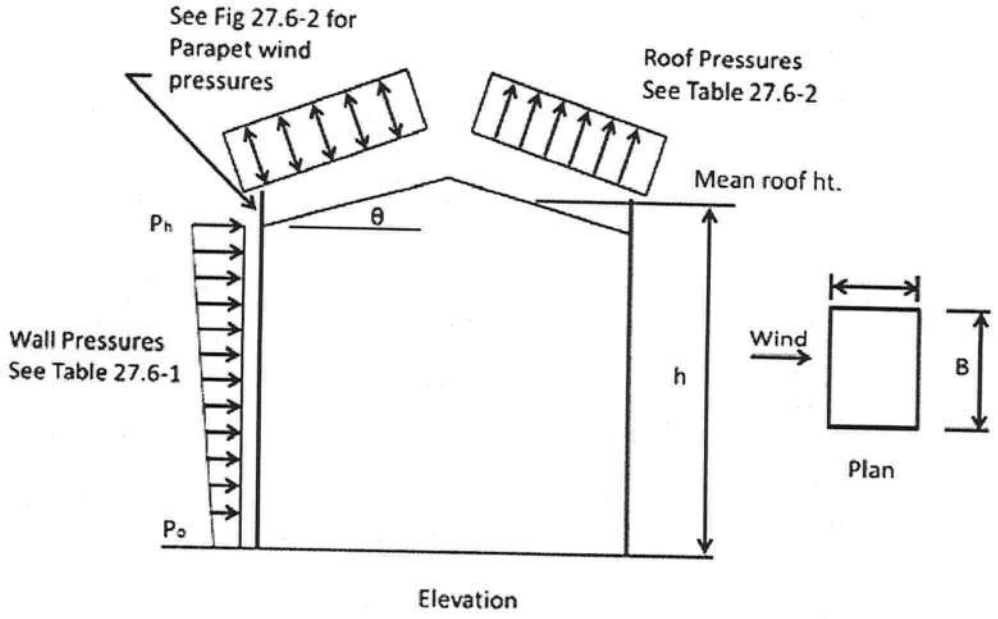
Wall Pressures do not include effect of internal pressure

MWFRS-Wall Pressures for Wind Normal to 24 ft wall

L/B	=	1.83
ph: Net Pressure at top of wall (windward + leeward)	=	20.80 psf
p0: Net Pressure at bottom of wall (windward + leeward)	=	20.80 psf
ps: Side wall pressure acting uniformly outward = .62 * ph	=	12.97 psf
pl: Leeward wall pressure acting uniformly outward = .29 * ph	=	6.00 psf
pwh: Windward wall pressure acting uniformly outward = ph-pl	=	14.80 psf
pw0: Windward wall pressure acting uniformly outward = p0-pl	=	14.80 psf

MWFRS-Wall Pressures for Wind Normal to 44 ft wall
L/B

ph: Net Pressure at top of wall (windward + leeward)	=	0.55
p0: Net Pressure at bottom of wall (windward + leeward)	=	23.30 psf
		23.30 psf
ps: Side wall pressure acting uniformly outward = .54 * ph	=	12.58 psf
pl: Leeward wall pressure acting uniformly outward = .38 * ph	=	8.85 psf
pwh: Windward wall pressure acting uniformly outward = ph-pl	=	14.45 psf
pw0: Windward wall pressure acting uniformly outward = p0-pl	=	14.45 psf

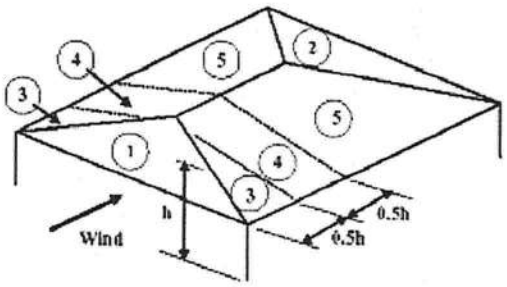


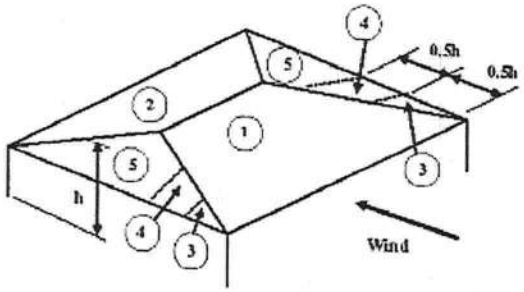
Net Wind Pressures on Roof (Table 27.6-2):

Exposure Adjustment Factor = 0.677

Zone	Load Case1 psf	Load Case2 psf
1	.00	.00
2	.00	.00
3	-22.41	.00
4	-19.97	.00
5	-16.38	.00

Note: A value of '0' indicates that the zone/load case is not applicable.





Hipped Roof

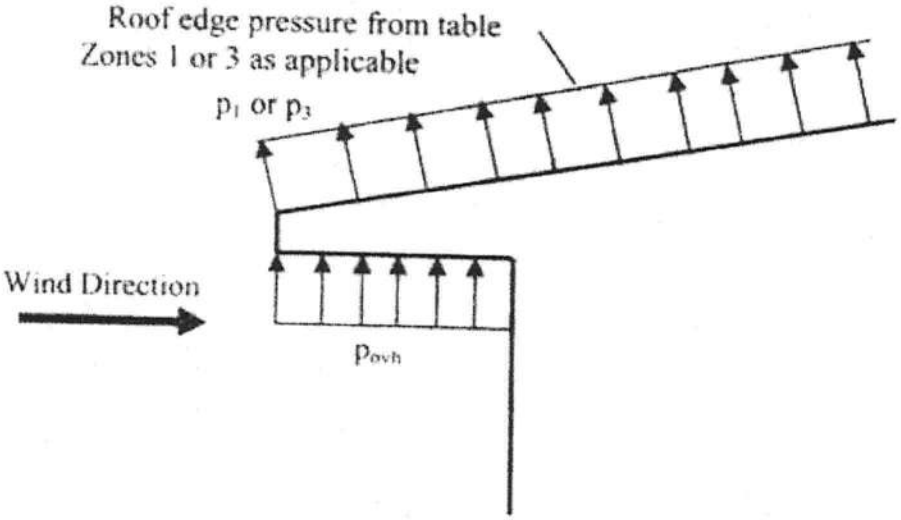
Roof Overhang Loads (Figure 27.6-3):

Load Case 1:

Povh1: Overhang pressure for zone 1	=	.00 psf
Povh3: Overhang pressure for zone 3	=	-16.81 psf

Load Case 2:

Povh1: Overhang pressure for zone 1	=	.00 psf
Povh3: Overhang pressure for zone 3	=	.00 psf



MECAWind Version 2.1.0.6 ASCE 7-10

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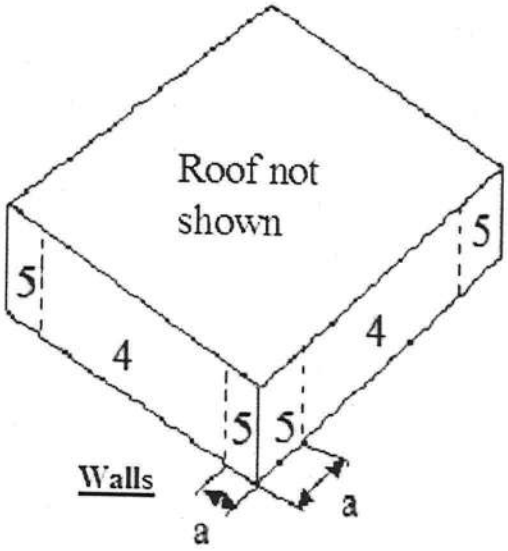
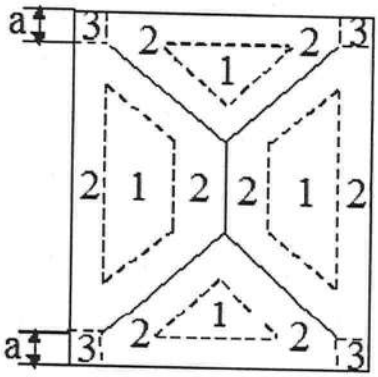
Date	: 12/22/2012	Project No.	: DW12-20
Company Name	: Driscoll Engineering, Inc	Designed By	: MED
Address	: PO Box 357577	Description	: New Residence
City	: Gainesville	Customer Name	: Wheary
State	: Fl	Proj Location	: Lot 14 Three Rivers Estates Co
File Location:	C:\Program Files\MECAWind\Default.wnd		





Hip Roof $7 < \theta <= 27$

Wind Pressure on Components and Cladding (Ch 30 Part 1)

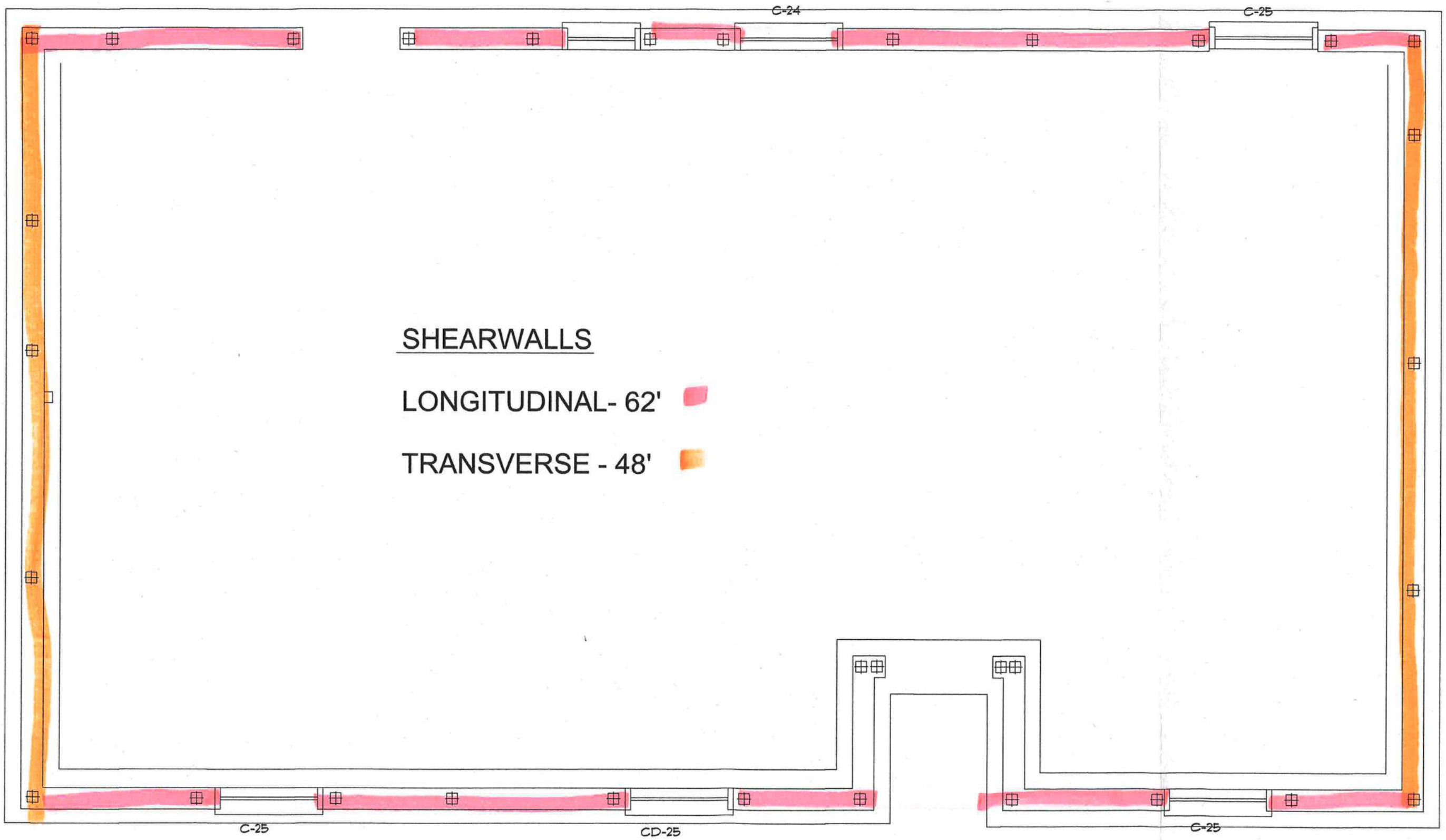
All pressures shown are based upon STRENGTH Design, with a Load Factor of 1

Width of Pressure Coefficient Zone "a" = 3 ft

Description	Width ft	Span ft	Area ft^2	Zone	Max GCp	Min GCp	Max P psf	Min P psf
Zone 1	1.00	1.00	1.0	1	0.50	-0.90	17.52	-27.82
Zone 2	1.00	1.00	1.0	2	0.50	-1.70	17.52	-48.44
Zone 3	1.00	1.00	1.0	3	0.50	-1.70	17.52	-48.44
Zone 4	1.00	1.00	1.0	4	0.90	-1.10	27.82	-32.98
Zone 5	1.00	1.00	1.0	5	0.90	-1.40	27.82	-40.71
Zone 2H	1.00	1.00	1.0	2H	0.50	-2.20	16.00	-56.68
Zone 3H	1.00	1.00	1.0	3H	0.50	-3.70	16.00	-95.33

Khcc:Comp. & Clad. Table 6-3 Case 1 = 0.70

Qhcc:.00256*V^2*Khcc*Kht*Kd = 25.76 psf


C-24
C-25
SHEARWALLS
LONGITUDINAL- 62'
TRANSVERSE - 48'
C-25
CD-25
C-25

CONNECTOR SCHEDULE FOR LOAD BEARING & SHEAR WALLS

TO CONNECT	TO	NO.	PRODUCT CODE	FASTENER	UPLIFT CAPACITY LBS
TRUSS	LINTEL	1	HETA 20	9- 10dX 1-1/2"	1810
GIRDER TRUSS	LINTEL	1	HETA 20	9- 10dX 1-1/2"	1810

DRISCOLL ENGINEERING, INC.
CONSULTING ENGINEER
PO BOX 357577. CA 8690
GAINESVILLE, FL. 32606 PH (352) 332-1513

NAME:

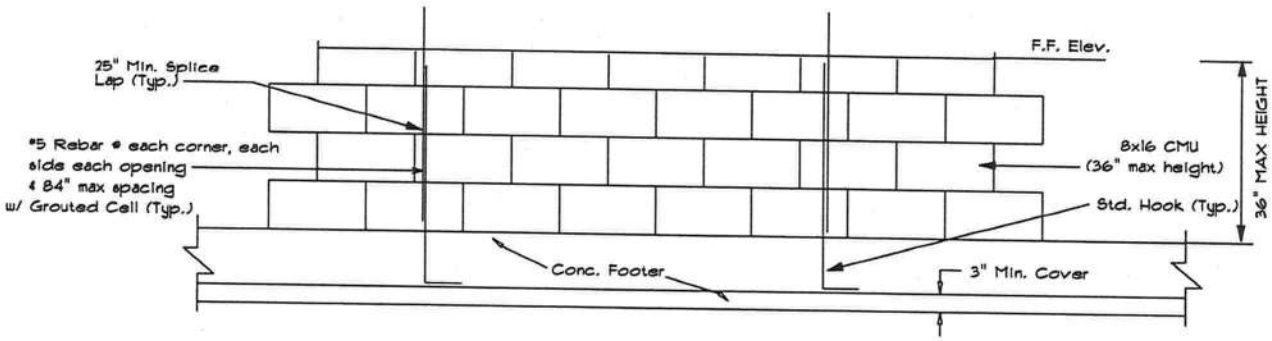
WHEARY
LOT 14 THREE RIVERS ESTATES
COLUMBIA CO, FL DW12-20

DATE:
DESIGNER:
CHECKED:
DRAWN:
REVISED:

JOB NO.
SHEET
OF

Connector Schedule Notes:

1. Product codes refer to connector hardware as manufactured by Simpson Strong-Tie Company, Inc., Pleasanton, CA. Other manufacturers' products of equal or higher capacity may be substituted.
2. Use one connector on every other stud -32" OC max.
3. Use one connector each jack stud, each side of header.
4. Connector spacing: within 6" of each end of each plate, within 6" of corners, and at 32" o.c. maximum.
5. All metal hardware and fasteners in contact with pressure-treated wood shall be corrosion-resistant compatible with the chemical application of the treated wood. Type 304 or Type 316 stainless steel may also be used. Where allowed, provide moisture barrier between untreated wood and concrete in lieu of using pressure-treated wood.
6. Unless noted otherwise, all nails to be common wire nails with the following diameters:
    a. 8d: 0.131 in.
    b. 10d: 0.148 in.
    c. 16d: 0.162 in.
    d. #8 Ringshank
7. See truss calculations for truss-to-truss connectors.
8. Connections not otherwise specified herein or shown on the drawings shall be in accordance with Section 2306 of the 2010 Florida Building Code.



Masonry Stemwall Detail

DRISCOLL ENGINEERING, INC.
CONSULTING ENGINEERS
PO BOX 357577
GAINESVILLE, FL. 32606
EB 8690
PH (352) 332-1513

PROJECT:

WHEARY
LOT 14 THREE RIVERS ESTATES
COLUMBIA CO, FL DW12-20

DATE:
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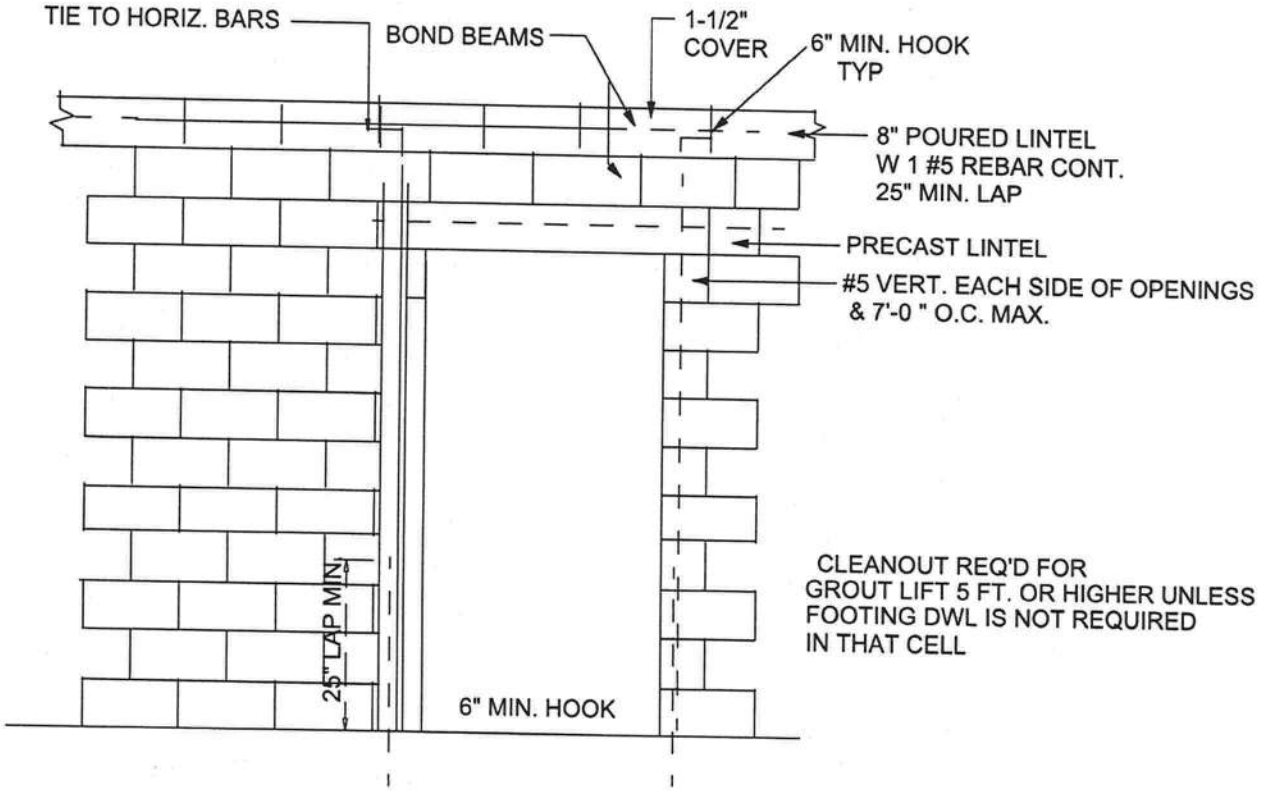

CMU WALL OPENING REINFORCING

NTS

DRISCOLL ENGINEERING,INC.
CONSULTING ENGINEERS
PO BOX 357577
GAINESVILLE, FL. 32606
EB 8690
PH (352) 332-1513

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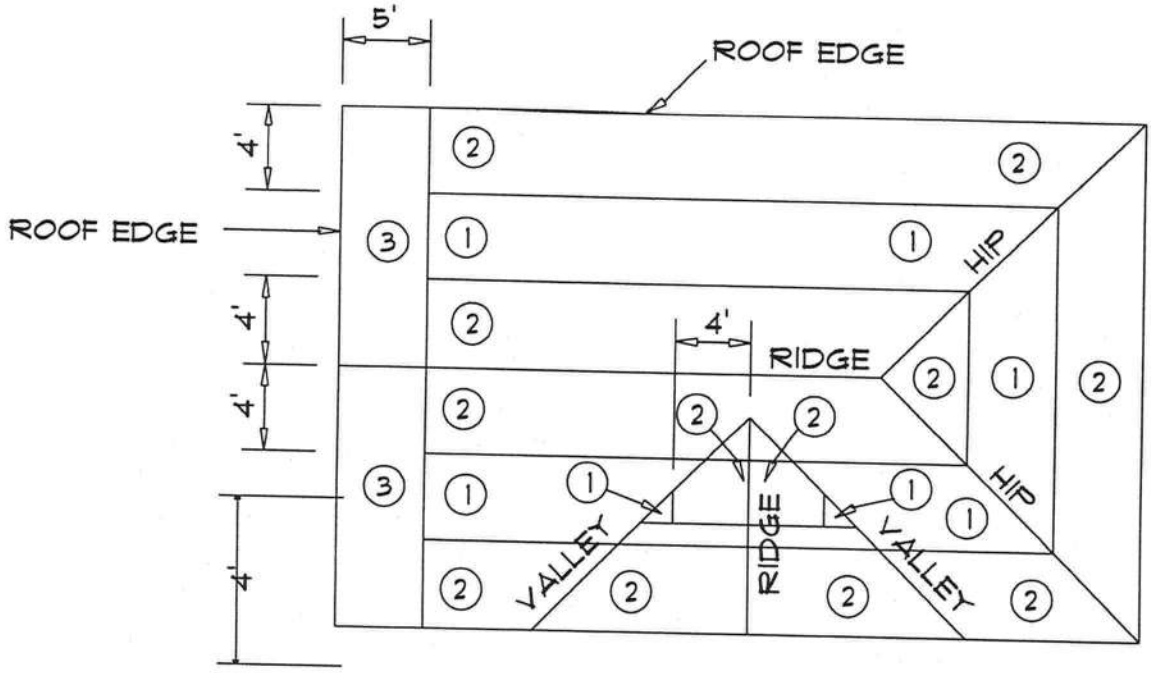

ROOF ATTACHMENT PLAN

ZONE 1 EDGES AT TRUSSES 6" O.C. AND INTERMEDIATE TRUSSES 10" O.C.
ZONE 2 EDGES AT TRUSSES 6" O.C. AND INTERMEDIATE TRUSSES 6" O.C.
ZONE 3 EDGES AT TRUSSES 4" O.C. AND INTERMEDIATE TRUSSES 6" O.C.

NOTES:

1. ALL NAILS TO BE #8 RING SHANK NAILS MIN.
2. IF BUILDING WIDTH EXCEEDS 40 FT OR HEIGHT IS MORE THAN 2 STORIES USE #10 RINGSHANK INSTEAD OF #8 RINGSHANK FOR ATTACHMENT OF ROOF SHEATHING
3. ALL STRUCTURAL SHEATHING PANELS TO BE 7/16" OSB MIN.

DRISCOLL ENGINEERING, INC.
CONSULTING ENGINEERS
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GAINESVILLE, FL. 32606
CA 8690
PH (352) 332-1513

PROJECT:
WHEARY
LOT 14 THREE RIVERS ESTATES
COLUMBIA CO, FL DW12-20

DATE:
DESIGNER:
CHECKED:
DRAWN:
REVISED:
JOB NO.
SHEET
OF

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE INDEX* = 78

The lower the EnergyPerformance Index, the more efficient the home.

, Gainesville, FL, 32608-

1. New construction or existing		New (From Plans)
2. Single family or multiple family		Single-family
3. Number of units, if multiple family		1
4. Number of Bedrooms		3
5. Is this a worst case?		No
6. Conditioned floor area (ft²)		1025
7. Windows**	Description	Area
a. U-Factor: SHGC:	Dbl, U=0.34 SHGC=0.29	84.00 ft²
b. U-Factor: SHGC:	N/A	ft²
c. U-Factor: SHGC:	N/A	ft²
d. U-Factor: SHGC:	N/A	ft²
Area Weighted Average Overhang Depth:		1.500 ft.
Area Weighted Average SHGC:		0.290

8. Floor Types	Insulation	Area
a. Slab-On-Grade Edge Insulation	R=0.0	1025.00 ft²
b. N/A	R=	ft²
c. N/A	R=	ft²

9. Wall Types	Insulation	Area
a. Concrete Block - Int Insul, Exterior	R=5.0	1056.00 ft²
b. N/A	R=	ft²
c. N/A	R=	ft²
d. N/A	R=	ft²

10. Ceiling Types	Insulation	Area
a. Under Attic (Vented)	R=30.0	1025.00 ft²
b. N/A	R=	ft²
c. N/A	R=	ft²

11. Ducts	R	ft²
a. Sup: Attic, Ret: RoomsInBlock1, AH: RoomsInBlo	8	205

12. Cooling systems	kBtu/hr	Efficiency
a. Central Unit	24.0	SEER:15.00

13. Heating systems	kBtu/hr	Efficiency
a. Electric Heat Pump	24.0	HSPF:8.60

14. Hot water systems
a. Electric — Cap: 40 gallons EF: 0.92

b. Conservation features
None

15. Credits — Pstat

COLUMBIA COUNTY BUILDING DEPARTMENT Received for FILE COPY Code Compliance PLANS EXAMINER

I certify that this home has complied with the Florida Energy Efficiency Code for Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

GREAT SEAL OF THE STATE OF FLORIDA * IN GOD WE TRUST *

Builder Signature: _____ Date: _____

Address of New Home: _____ City/FL Zip: _____

*Note: This is not a Building Energy Rating. If your Index is below 70, your home may qualify for energy efficient mortgage (EEM) incentives if you obtain a Florida EnergyGauge Rating. Contact the EnergyGauge Hotline at (321) 638-1492 or see the EnergyGauge web site at energygauge.com for information and a list of certified Raters. For information about the Florida Building Code, Energy Conservation, contact the Florida Building Commission's support staff.

**Label required by Section 303.1.3 of the Florida Building Code, Energy Conservation, if not DEFAULT.

FORM 405-10

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Business and Professional Regulation - Residential Performance Method

Project Name:	Housecraft Wheary	Builder Name:	Housecraft
Street:		Permit Office:	
City, State, Zip:	Gainesville , FL , 32608-	Permit Number:	
Owner:	Wheary	Jurisdiction:	
Design Location:	FL, Gainesville		

1. New construction or existing	New (From Plans)
2. Single family or multiple family	Single-family
3. Number of units, if multiple family	1
4. Number of Bedrooms	3
5. Is this a worst case?	No
6. Conditioned floor area above grade (ft²)	1025
Conditioned floor area below grade (ft²)	0

7. Windows(84.0 sqft.)

	Description	Area
a. U-Factor: SHGC:	Dbl, U=0.34 SHGC=0.29	84.00 ft²
b. U-Factor: SHGC:	N/A	ft²
c. U-Factor: SHGC:	N/A	ft²
d. U-Factor: SHGC:	N/A	ft²

Area Weighted Average Overhang Depth: 1.500 ft.
Area Weighted Average SHGC: 0.290

8. Floor Types (1025.0 sqft.)

	Insulation	Area
a. Slab-On-Grade Edge Insulation	R=0.0	1025.00 ft²
b. N/A	R=	ft²
c. N/A	R=	ft²

9. Wall Types (1056.0 sqft.)

	Insulation	Area
a. Concrete Block - Int Insul, Exterior	R=5.0	1056.00 ft²
b. N/A	R=	ft²
c. N/A	R=	ft²
d. N/A	R=	ft²

10. Ceiling Types (1025.0 sqft.)

	Insulation	Area
a. Under Attic (Vented)	R=30.0	1025.00 ft²
b. N/A	R=	ft²
c. N/A	R=	ft²

11. Ducts

	R	ft²
a. Sup: Attic, Ret: RoomsInBlock1, AH: RoomsInBlo	8	205

12. Cooling systems

	kBtu/hr	Efficiency
a. Central Unit	24.0	SEER:15.00

13. Heating systems

	kBtu/hr	Efficiency
a. Electric Heat Pump	24.0	HSPF:8.60

14. Hot water systems
a. Electric — Cap: 40 gallons, EF: 0.920
b. Conservation features
None

15. Credits — Pstat

Glass/Floor Area: 0.082

Total Proposed Modified Loads: 23.57
Total Standard Reference Loads: 30.04

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: [signature]
DATE: 1-10-13

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____
DATE: _____

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL: _____
DATE: _____

- Compliance requires completion of a Florida Air Barrier and Insulation Inspection Checklist

PROJECT

Title:	Housecraft Wheary	Bedrooms:	3	Address Type:	Street Address
Building Type:	FLProp2010	Conditioned Area:	1025	Lot #	
Owner:	Wheary	Total Stories:	1	Block/SubDivision:	
# of Units:	1	Worst Case:	No	PlatBook:	
Builder Name:	Housecraft	Rotate Angle:	0	Street:	
Permit Office:		Cross Ventilation:		County:	Alachua
Jurisdiction:		Whole House Fan:		City, State, Zip:	Gainesville , FL , 32608-
Family Type:	Single-family				
New/Existing:	New (From Plans)				
Comment:					

CLIMATE

√	Design Location	TMY Site	IECC Zone	Design Temp 97.5 %	Design Temp 2.5 %	Int Design Temp Winter	Int Design Temp Summer	Heating Degree Days	Design Moisture	Daily Temp Range
____	FL, Gainesville	FL_GAINESVILLE_REGI	2	32	92	70	75	1305.5	51	Medium

BLOCKS

Number	Name	Area	Volume
1	Block1	1025	8200

SPACES

Number	Name	Area	Volume	Kitchen	Occupants	Bedrooms	Infil ID	Finished	Cooled	Heated
1	RoomsInBlock1	1025	8200	Yes	3	3	1	Yes	Yes	Yes

FLOORS

√	#	Floor Type	Space	Perimeter	R-Value	Area		Tile	Wood	Carpet
____	1	Slab-On-Grade Edge Insulatio	RoomsInBlock1	132 ft	0	1025 ft²	----	0.2	0	0.8

ROOF

√	#	Type	Materials	Roof Area	Gable Area	Roof Color	Solar Absor.	SA Tested	Emitt	Emitt Tested	Deck Insul.	Pitch (deg)
____	1	Hip	Composition shingles	1110 ft²	0 ft²	Medium	0.96	No	0.9	No	0	22.6

ATTIC

√	#	Type	Ventilation	Vent Ratio (1 in)	Area	RBS	IRCC
____	1	Full attic	Vented	300	1025 ft²	N	N

CEILING

√	#	Ceiling Type	Space	R-Value	Area	Framing Frac	Truss Type
____	1	Under Attic (Vented)	RoomsInBlock1	30	1025 ft²	0.11	Wood

WALLS

√	#	Ornt	Adjacent To	Wall Type	Space	Cavity R-Value	Width Ft	Width In	Height Ft	Height In	Area	Sheathing R-Value	Framing Fraction	Solar Absor.	Below Grade%
_____	1	N	Exterior	Concrete Block - Int Ins	RoomsInBloc	5	24		8		192 ft²		0	0.75	0
_____	2	E	Exterior	Concrete Block - Int Ins	RoomsInBloc	5	42		8		336 ft²		0	0.75	0
_____	3	S	Exterior	Concrete Block - Int Ins	RoomsInBloc	5	24		8		192 ft²	0	0	0.75	0
_____	4	W	Exterior	Concrete Block - Int Ins	RoomsInBloc	5	42		8		336 ft²	0	0	0.75	0

DOORS

√	#	Ornt	Door Type	Space	Storms	U-Value	Width Ft	Width In	Height Ft	Height In	Area
_____	1	E	Insulated	RoomsInBloc	None	0.4	3		6	6	19.5 ft²
_____	2	W	Insulated	RoomsInBloc	None	0.4	3		6	6	19.5 ft²

WINDOWS

Orientation shown is the entered, Proposed orientation.

√	#	Ornt	Wall ID	Frame	Panes	NFRC	U-Factor	SHGC	Storms	Area	Overhang Depth	Overhang Separation	Int Shade	Screening
_____	1	E	2	Vinyl	Double (Tinted)	Yes	0.34	0.29	N	9 ft²	1 ft 6 in	1 ft 0 in	HERS 2006	None
_____	2	E	2	Vinyl	Double (Tinted)	Yes	0.34	0.29	N	30 ft²	1 ft 6 in	1 ft 0 in	HERS 2006	None
_____	3	W	4	Vinyl	Double (Tinted)	Yes	0.34	0.29	N	45 ft²	1 ft 6 in	1 ft 0 in	HERS 2006	None

INFILTRATION

#	Scope	Method	SLA	CFM 50	ELA	EqLA	ACH	ACH 50
1	BySpaces	Proposed SLA	0.000360	967.89	53.136	99.930	0.2771	7.0821

HEATING SYSTEM

√	#	System Type	Subtype	Efficiency	Capacity	Block	Ducts
_____	1	Electric Heat Pump	None	HSPF: 8.6	24 kBtu/hr	1	sys#1

COOLING SYSTEM

√	#	System Type	Subtype	Efficiency	Capacity	Air Flow	SHR	Block	Ducts
_____	1	Central Unit	Split	SEER: 15	24 kBtu/hr	720 cfm	0.75	1	sys#1

HOT WATER SYSTEM

√	#	System Type	SubType	Location	EF	Cap	Use	SetPnt	Conservation
_____	1	Electric	None	RoomsInBlock	0.92	40 gal	60 gal	120 deg	None

SOLAR HOT WATER SYSTEM

√	FSEC Cert #	Company Name	System Model #	Collector Model #	Collector Area	Storage Volume	FEF
_____	None	None			ft²		

DUCTS

√	#	---- Supply ---- Location	R-Value	Area	---- Return ---- Location	Area	Leakage Type	Air Handler	CFM 25	Percent Leakage	QN	RLF	HVAC # Heat	Cool
_____	1	Attic	8	205 ft²	RoomsInBloc	20 ft²	DSE=0.88	RoomsInBl	0.0 cfm	0.00 %	0.00	0.60	1	1

TEMPERATURES

Programable Thermostat: Y

Ceiling Fans:

	Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	Oct	Nov	Dec
Cooling	[X]	[X]	[X]	[X]	[X]	[X]	[X]	[X]	[X]	[X]	[X]	[X]
Heating	[X]	[X]	[X]	[X]	[X]	[X]	[X]	[X]	[X]	[X]	[X]	[X]
Venting	[X]	[X]	[X]	[X]	[X]	[X]	[X]	[X]	[X]	[X]	[X]	[X]

Thermostat Schedule: HERS 2006 Reference

Schedule Type		Hours 1	2	3	4	5	6	7	8	9	10	11	12
Cooling (WD)	AM	78	78	78	78	78	78	78	78	80	80	80	80
	PM	80	80	78	78	78	78	78	78	78	78	78	78
Cooling (WEH)	AM	78	78	78	78	78	78	78	78	78	78	78	78
	PM	78	78	78	78	78	78	78	78	78	78	78	78
Heating (WD)	AM	66	66	66	66	66	68	68	68	68	68	68	68
	PM	68	68	68	68	68	68	68	68	68	68	66	66
Heating (WEH)	AM	66	66	66	66	66	68	68	68	68	68	68	68
	PM	68	68	68	68	68	68	68	68	68	68	66	66

MECHANICAL VENTILATION

Type	Supply CFM	Exhaust CFM	Fan Watts	HRV	Heating System	Run Time	Cooling System
None	0	0	0	0	1 - Electric Heat Pump	0%	1 - Central Unit

FORM 405-10

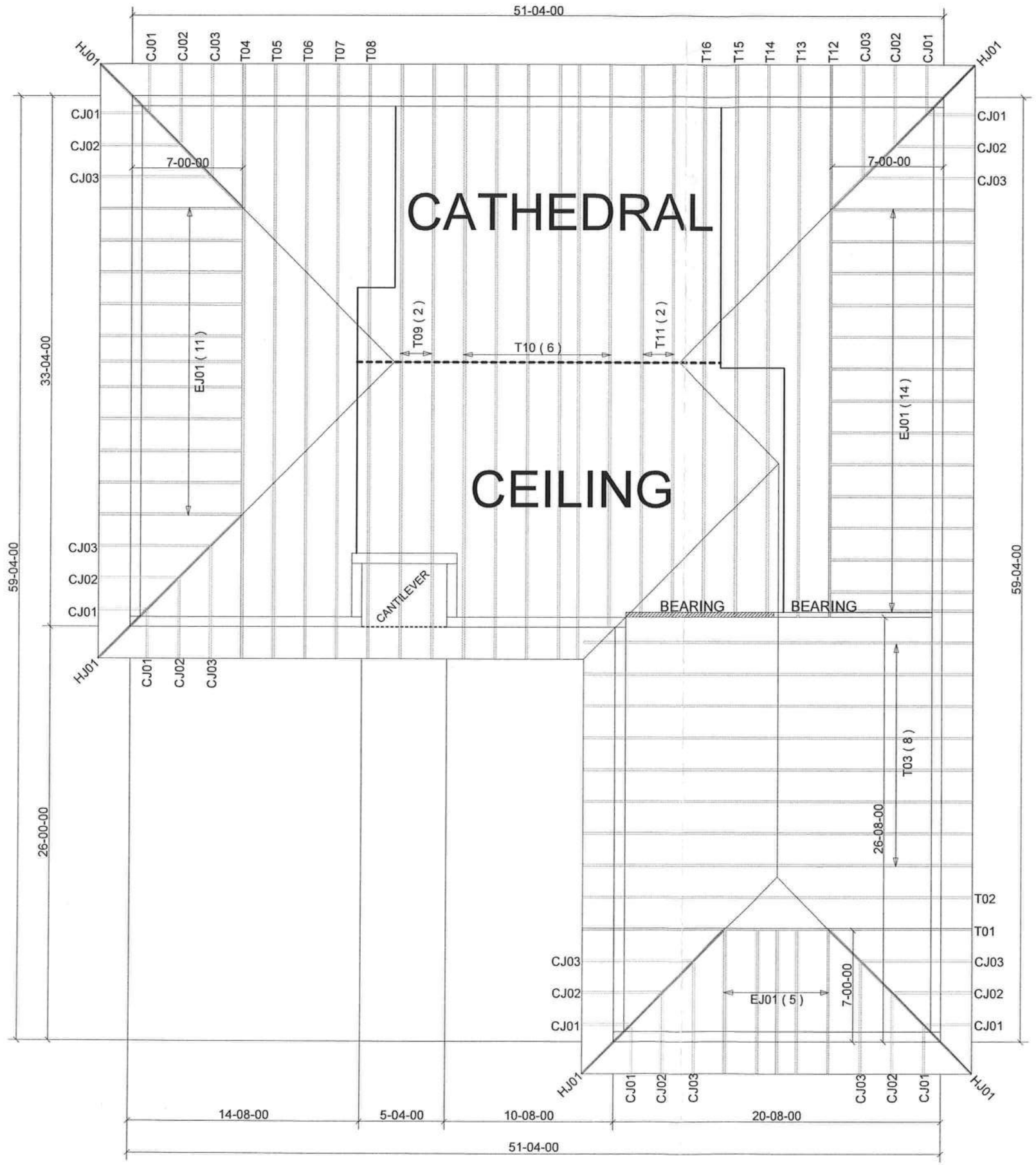
Florida Code Compliance Checklist

Florida Department of Business and Professional Regulations
Residential Whole Building Performance Method

ADDRESS:	PERMIT #:
Gainesville, FL, 32608-	

MANDATORY REQUIREMENTS SUMMARY - See individual code sections for full details.

COMPONENT	SECTION	SUMMARY OF REQUIREMENT(S)	CHECK
Air leakage	402.4	To be caulked, gasketed, weatherstripped or otherwise sealed. Recessed lighting IC-rated as meeting ASTM E 283. Windows and doors = 0.30 cfm/sq.ft. Testing or visual inspection required. Fireplaces: gasketed doors & outdoor combustion air. Must complete envelope leakage report or visually verify Table 402.4.2.	
Thermostat & controls	403.1	At least one thermostat shall be provided for each separate heating and cooling system. Where forced-air furnace is primary system, programmable thermostat is required. Heat pumps with supplemental electric heat must prevent supplemental heat when compressor can meet the load.	
Ducts	403.2.2 403.3.3	All ducts, air handlers, filter boxes and building cavities which form the primary air containment passageways for air distribution systems shall be considered ducts or plenum chambers, shall be constructed and sealed in accordance with Section 503.2.7.2 of this code. Building framing cavities shall not be used as supply ducts.	
Water heaters	403.4	Heat trap required for vertical pipe risers. Comply with efficiencies in Table 403.4.3.2. Provide switch or clearly marked circuit breaker (electric) or shutoff (gas). Circulating system pipes insulated to = R-2 + accessible manual OFF switch.	
Mechanical ventilation	403.5	Homes designed to operate at positive pressure or with mechanical ventilation systems shall not exceed the minimum ASHRAE 62 level. No make-up air from attics, crawlspaces, garages or outdoors adjacent to pools or spas.	
Swimming Pools & Spas	403.9	Pool pumps and pool pump motors with a total horsepower (HP) of = 1 HP shall have the capability of operating at two or more speeds. Spas and heated pools must have vapor-retardant covers or a liquid cover or other means proven to reduce heat loss except if 70% of heat from site-recovered energy. Off/timer switch required. Gas heaters minimum thermal efficiency=78% (82% after 4/16/13). Heat pump pool heaters minimum COP= 4.0.	
Cooling/heating equipment	403.6	Sizing calculation performed & attached. Minimum efficiencies per Tables 503.2.3. Equipment efficiency verification required. Special occasion cooling or heating capacity requires separate system or variable capacity system. Electric heat >10kW must be divided into two or more stages.	
Ceilings/knee walls	405.2.1	R-19 space permitting.	



6/12 PITCH
24" O/H

BEARING HEIGHT SCHEDULE

	8' - 0"
	8' 1-1/2"

NOTES:

1) REFER TO HIB 91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BRACING.) REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED.

2.) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL V10S FOR ALTERNATE BRACING REQUIREMENTS.

3.) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.

4.) ALL TRUSSES ARE DESIGNED FOR 2' o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.

5.) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.

6.) SY42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.

7.) BEAM/HEADER/LINTEL (HDR) TO BE FURNISHED BY BUILDER.

Builders FirstSource

Jacksonville
PHONE: 904-772-6100 FAX: 904-772-1973

Tampa
PHONE: 813-621-9831 FAX: 813-628-8956

Freeport
PHONE: 850-835-4541 FAX: 850-835-6835

BUILDER: HOUSECRAFT		
LEGAL ADDRESS: STEPHENS RES.		
MODEL: CUSTOM		Revision: Rev. By:
DATE: 12-23-12	DRAWN BY: KLH	Original Reference #: 458217
1st Level Job #:	2nd Level Job #:	Roof Job #: 458217

MITEK PLATE APPROVAL #'s 2197.2 - 2197.4, WEYERHAUSER PRODUCT #'s 1630.2 - 1630.10

JULIUS LEE PE.

RE: 458217 - HOUSECRAFT - STEPHENS RES.

1109 COASTAL BAY BLVD,
BOYNTON BEACH, FL 33435

Site Information:
Project Customer: HOUSECRAFT HOMES Project Name: 458217 Model: STEPHENS RES.
Lot/Block: Subdivision:
Address: TBD
City: GILCHRIST CTY State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.
Name: JOHN D. HARRINGTON License #: CGC038861
Address: 24113 NW OLD BELLAMY RD
City: HIGH SPRINGS, State: FL

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):
FBC 2010/TPI 2007
ASCE 7-10 Wind Speed: 130 mph Design Program: MiTek 20/20 7.3
Roof Load: 32.0 psf Floor Load: N/A psf

This package includes 21 individual, dated Truss Design Drawings and 0 Additional Drawings.
With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.
This document processed per section 16G15-23.003 of the Florida Board of Professionals Rules

In the event of changes from Builder or E.O.R. additional coversheets and drawings may accompany this coversheet. The latest approval dates supersede and replace the previous drawings.

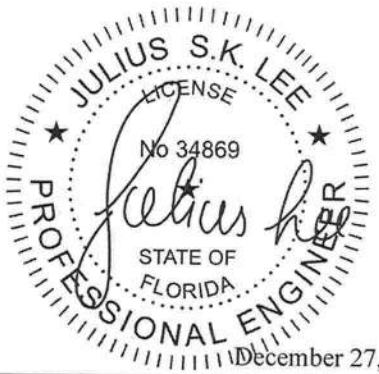
No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	I6243880	CJ01	12/27/012	18	I6243897	T13	12/27/012
2	I6243881	CJ02	12/27/012	19	I6243898	T14	12/27/012
3	I6243882	CJ03	12/27/012	20	I6243899	T15	12/27/012
4	I6243883	EJ01	12/27/012	21	I6243900	T16	12/27/012
5	I6243884	HJ01	12/27/012				
6	I6243885	T01	12/27/012				
7	I6243886	T02	12/27/012				
8	I6243887	T03	12/27/012				
9	I6243888	T04	12/27/012				
10	I6243889	T05	12/27/012				
11	I6243890	T06	12/27/012				
12	I6243891	T07	12/27/012				
13	I6243892	T08	12/27/012				
14	I6243893	T09	12/27/012				
15	I6243894	T10	12/27/012				
16	I6243895	T11	12/27/012				
17	I6243896	T12	12/27/012				

The truss drawing(s) referenced above have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by Builders FirstSource (Jax).

Truss Design Engineer's Name: Julius Lee

My license renewal date for the state of Florida is February 28, 2013.

NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Chapter 2.



December 27,2012

Julius Lee

Job	Truss	Truss Type	Qty	Ply	HOUSECRAFT - STEPHENS RES.
458217	CJ01	Jack-Open Truss	10	1	I6243880
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

7.350 s Jul 31 2012 MiTek Industries, Inc. Thu Dec 27 09:06:06 2012 Page 1

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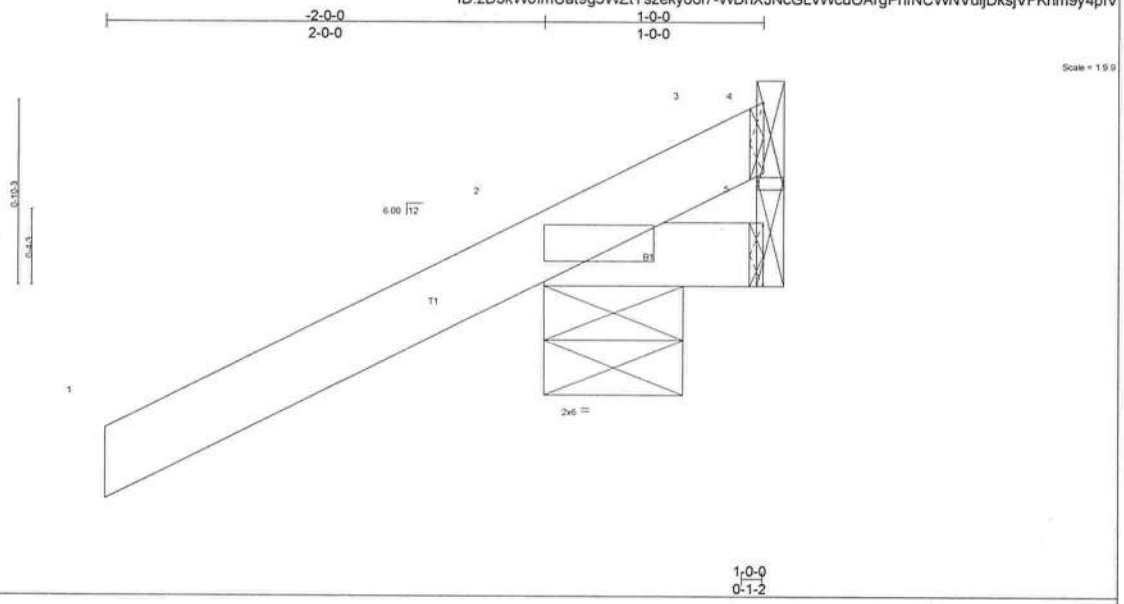

Plate Offsets (X,Y): [2:0-6-0,0-1-2]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	-0.00	8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.07	Vert(TL)	-0.00	8	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	2	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 7 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=184/0-7-10 (min. 0-1-8), 5=-24/Mechanical, 3=-21/Mechanical
Max Horz 2=67(LC 12)
Max Uplift2=-138(LC 12), 5=-30(LC 2), 3=-27(LC 2)
Max Grav 2=225(LC 2), 5=24(LC 16), 3=26(LC 10)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
BOT CHORD 2-5=-158/255

NOTES (7-9)
1) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 138 lb uplift at joint 2, 30 lb uplift at joint 5 and 27 lb uplift at joint 3.
6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
8) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

December 27,201

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	HOUSECRAFT - STEPHENS RES.	
458217	CJ03	Jack-Open Truss	10	1	Job Reference (optional)	I6243882

Builders FirstSource, Lake City, FL 32055

7.350 s Jul 31 2012 MiTek Industries, Inc. Thu Dec 27 09:06:09 2012 Page 1
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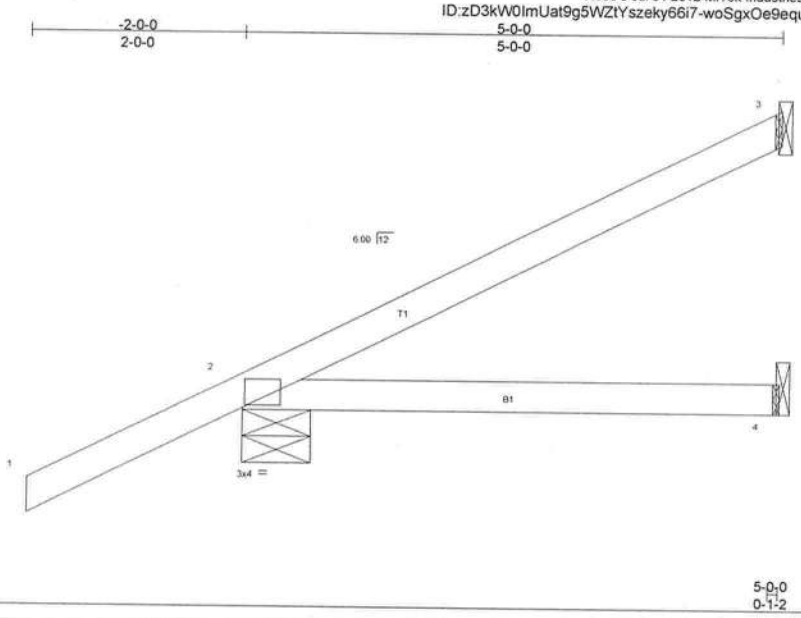


Plate Offsets (X,Y): [2:0-4-4,0-0-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	-0.02	4-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.19	Vert(TL)	-0.03	4-7	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	2	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 19 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

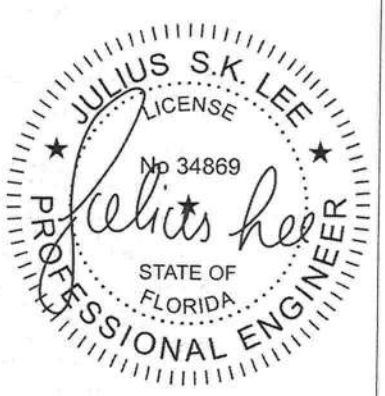
BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 3=79/Mechanical, 2=253/0-7-10 (min. 0-1-8), 4=23/Mechanical
Max Horz 2=162(LC 12)
Max Uplift 3=-93(LC 12), 2=-148(LC 12)
Max Grav 3=97(LC 2), 2=304(LC 2), 4=56(LC 3)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-824/105
BOT CHORD 2-4=-352/1106

NOTES (7-9)
1) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 93 lb uplift at joint 3 and 148 lb uplift at joint 2.
6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
8) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



December 27,2

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	HOUSECRAFT - STEPHENS RES.
458217	HJ01	Diagonal Hip Girder	5	1	I6243884
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

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ID:zD3kW0ImUat9g5WZtYszeky66i7-lyqxCSjvDgeLTnMaioSmE6OsGWhWqkS2a80la7y4plM

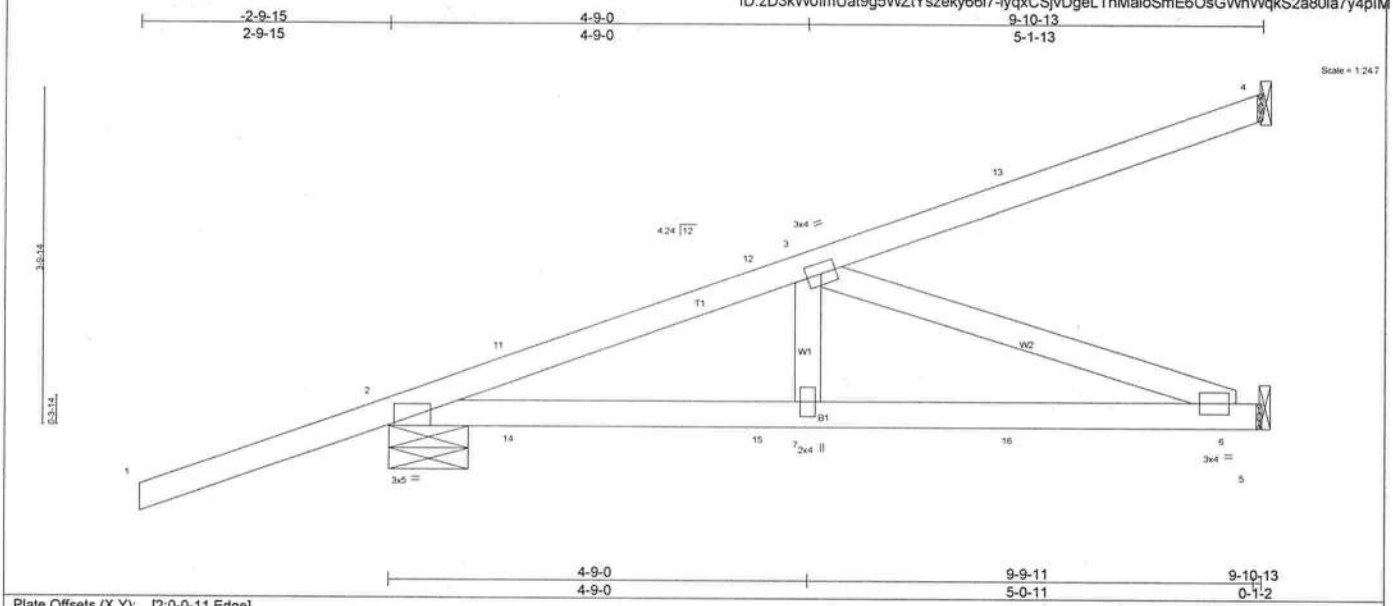

Plate Offsets (X,Y): [2:0-0-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.59	Vert(LL)	-0.05	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.47	Vert(TL)	-0.10	6-7	>999	180		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.36	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 44 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-2-5 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 4=120/Mechanical, 2=369/0-10-13 (min. 0-1-8), 5=194/Mechanical
Max Horz 2=234(LC 4)
Max Uplift 4=-137(LC 4), 2=-266(LC 4), 5=-120(LC 8)
Max Grav 4=147(LC 2), 2=454(LC 2), 5=225(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-11=-621/277, 11-12=-570/290, 3-12=-570/285
BOT CHORD 2-14=-397/570, 14-15=-397/570, 7-15=-397/570, 7-16=-397/570, 6-16=-397/570
WEBS 3-6=-604/421

NOTES (9-11)
1) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 137 lb uplift at joint 4, 266 lb uplift at joint 2 and 120 lb uplift at joint 5.
6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 45 lb down and 44 lb up at 1-5-12, 45 lb down and 44 lb up at 1-5-12, 3 lb down and 50 lb up at 4-3-11, 3 lb down and 50 lb up at 4-3-11, and 43 lb down and 93 lb up at 7-1-10, and 43 lb down and 93 lb up at 7-1-10 on top chord, and 29 lb up at 1-5-12, 29 lb up at 1-5-12, 3 lb down and 3 lb up at 4-3-11, 3 lb down and 3 lb up at 4-3-11, and 26 lb down at 7-1-10, and 26 lb down at 7-1-10 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
10) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-44, 5-8=-10
Concentrated Loads (lb)
Vert: 11=36(F=18, B=18) 12=4(F=2, B=2) 13=-70(F=-35, B=-35) 14=31(F=15, B=15) 15=-2(F=-1, B=-1) 16=-26(F=-13, B=-13)

December 27,2(

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	HOUSECRAFT - STEPHENS RES.
458217	T01	Hip Truss	1	1	I6243885
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

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ID:zD3kW0ImUat9g5WZtYszeky66i7-dj3S2pmQHv8mxOgLxeWiPyYXX70cmbJeUm_zjuy4pl

LOAD CASE(S) Standard

Uniform Loads (plf)

Vert: 1-3=-44, 3-5=-44, 5-7=-44, 12-15=-10

Concentrated Loads (lb)

Vert: 3=-69(B) 5=-146(B) 11=-205(B) 9=-22(B) 4=-69(B) 8=-205(B) 18=-69(B) 19=-69(B) 20=-22(B) 21=-22(B)

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	HOUSECRAFT - STEPHENS RES.
458217	T03	Common Truss	8	1	I6243887
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

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ID:zD3kW0ImUat9g5WZtYszeky66i7-tS6rxut39gHVXns4y1BpGrQ7YI1ANdUzZgfxYty4pI9

-2-0-0 2-0-0 | 5-5-4 5-5-4 | 10-4-0 4-10-12 | 15-2-12 4-10-12 | 20-8-0 5-5-4 | 22-8-0 2-0-0

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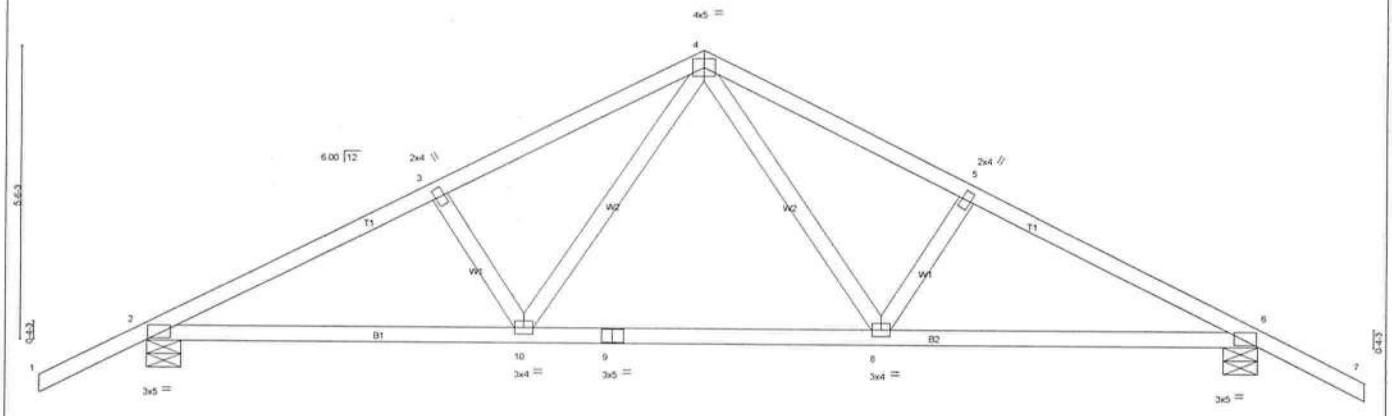

7-0-2 7-0-2 | 13-7-14 6-7-12 | 20-8-0 7-0-2

Plate Offsets (X,Y): [2:0-5-4,0-0-4], [6:0-5-4,0-0-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.43	Vert(LL)	0.24	8-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.88	Vert(TL)	-0.36	8-10	>689	180		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.31	Horz(TL)	0.05	6	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 98 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-10 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-8-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=814/0-7-10 (min. 0-1-8), 6=814/0-7-10 (min. 0-1-8)
Max Horz 2=77(LC 16)
Max Uplift 2=-260(LC 12), 6=-260(LC 13)
Max Grav 2=969(LC 2), 6=969(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1689/971, 3-4=-1572/968, 4-5=-1571/968, 5-6=-1689/971
BOT CHORD 2-10=-727/1499, 9-10=-390/1020, 8-9=-390/1020, 6-8=-733/1510
WEBS 4-8=-385/647, 5-8=-273/262, 4-10=-385/647, 3-10=-274/263

NOTES (9-11)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 260 lb uplift at joint 2 and 260 lb uplift at joint 6.
7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
10) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-44, 4-7=-44, 10-11=-10, 8-10=-61(F=-51), 8-14=-10

JULIUS S.K. LEE LICENSE No 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

December 27,20

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	HOUSECRAFT - STEPHENS RES.
458217	T04	Hip Truss	1	1	I6243888
					Job Reference (optional)

11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
12) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
13) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-44, 3-7=-44, 7-9=-44, 2-8=-10
Concentrated Loads (lb)
Vert: 3=-69(F) 5=-69(F) 7=-146(F) 15=-205(F) 10=-205(F) 20=-69(F) 21=-69(F) 22=-69(F) 23=-69(F) 24=-69(F) 25=-69(F) 26=-69(F) 27=-69(F) 28=-22(F) 29=-22(F) 30=-22(F) 31=-22(F) 32=-22(F) 33=-22(F) 34=-22(F) 35=-22(F) 36=-22(F)

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	HOUSECRAFT - STEPHENS RES.	I6243890
458217	T06	Hip Truss	1	1	Job Reference (optional)	

Builders FirstSource, Lake City, FL 32055

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-2-0-0	5-7-11	11-0-0	16-8-0	22-4-0	27-8-5	33-4-0	35-4-0
2-0-0	5-7-11	5-4-5	5-8-0	5-8-0	5-4-5	5-7-11	2-0-0

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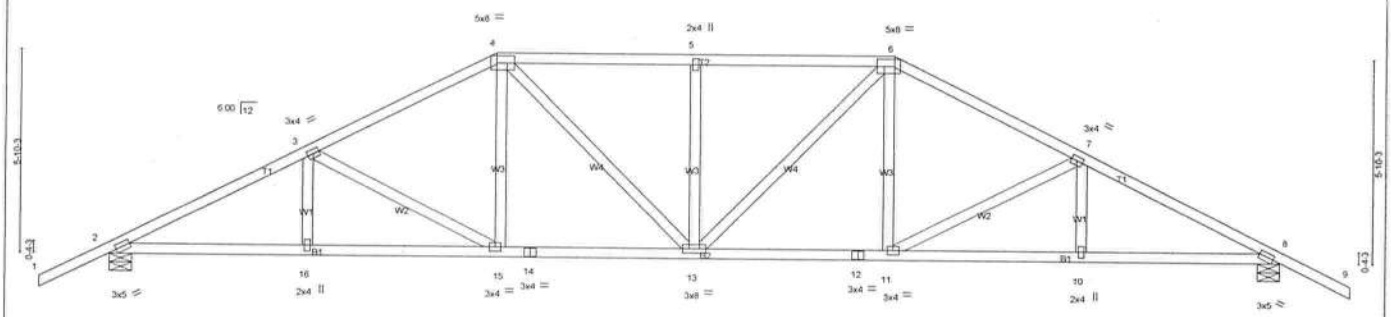

5-7-11	11-0-0	16-8-0	22-4-0	27-8-5	33-4-0
5-7-11	5-4-5	5-8-0	5-8-0	5-4-5	5-7-11

Plate Offsets (X,Y): [2:0-2-10,0-1-8], [4:0-6-0,0-2-8], [6:0-6-0,0-2-8], [8:0-2-10,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.41	Vert(LL)	0.17	13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.51	Vert(TL)	-0.25	13-15	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.32	Horz(TL)	0.10	8	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 181 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-11-10 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-3-6 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=988/0-7-10 (min. 0-1-8), 8=988/0-7-10 (min. 0-1-8)
Max Horz 2=81(LC 12)
Max Uplift 2=-264(LC 12), 8=-264(LC 13)
Max Grav 2=1175(LC 2), 8=1175(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-2049/1135, 3-4=-1693/994, 4-5=-1685/1059, 5-6=-1685/1059, 6-7=-1693/994, 7-8=-2048/1135
BOT CHORD 2-16=-868/1813, 15-16=-868/1813, 14-15=-605/1471, 13-14=-605/1471, 12-13=-606/1474, 11-12=-606/1474, 10-11=-877/1829, 8-10=-877/1829
WEBS 3-15=-409/309, 4-15=-116/282, 4-13=-155/384, 5-13=-346/254, 6-13=-155/384, 6-11=-116/282, 7-11=-408/309

NOTES (9-11)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 264 lb uplift at joint 2 and 264 lb uplift at joint 8.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
10) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

December 27,2

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	HOUSECRAFT - STEPHENS RES.
458217	T08	Hip Truss	1	1	I6243892

Builders FirstSource, Lake City, FL 32055

Job Reference (optional)

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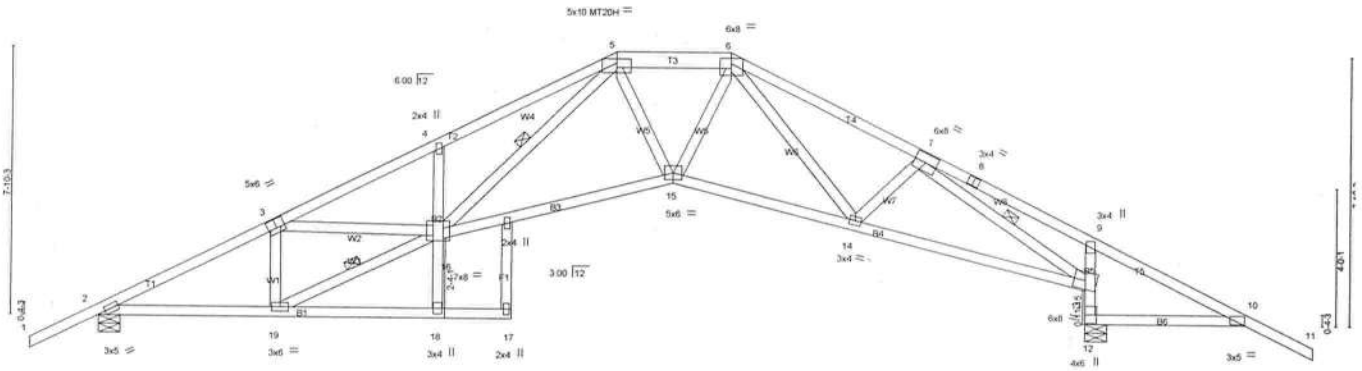
-2-0-0	5-1-9	10-0-0	15-0-0	18-4-0	23-10-9	28-8-6	33-4-0	35-4-0
2-0-0	5-1-9	4-10-7	5-0-0	3-4-0	5-6-9	4-9-13	4-7-10	2-0-0

Scale: 3/16"=1



5-1-9	10-0-0	12-0-0	16-8-0	21-11-14	28-8-6	29-0-3	33-4-0
5-1-9	4-10-7	2-0-0	4-8-0	5-3-14	6-8-8	0-3-13	4-3-13

Plate Offsets (X,Y): [2:0-2-10,0-1-8], [3:0-3-0,0-3-4], [5:0-5-0,0-1-7], [6:0-4-0,0-1-15], [10:0-5-0,0-0-4], [16:0-2-4,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.91	Vert(LL)	0.81	15-16	>429	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.85	Vert(TL)	0.68	15-16	>510	180	MT20H	187/143
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.66	Horz(TL)	-0.27	12	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 192 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2 *Except*
T3: 2x6 SYP No.2
BOT CHORD 2x4 SP No.2 *Except*
B2: 2x4 SP No.3, B3,B5: 2x4 SYP No.1
WEBS 2x4 SP No.3 *Except*
W8: 2x4 SP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 3-1-4 oc bracing. Except:
10-0-0 oc bracing: 16-18
WEBS 1 Row at midpt 16-19, 5-16, 7-13

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=856/0-7-10 (min. 0-1-8), 12=1142/0-7-10 (min. 0-1-10)
Max Horz 2=168(LC 9)
Max Uplift2=-452(LC 9), 12=-525(LC 9)
Max Grav 2=1015(LC 2), 12=1356(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1588/2544, 3-4=-2441/3976, 4-5=-2473/4195, 5-6=-1651/2916, 6-7=-1486/2887, 7-8=-932/927, 8-9=-948/840, 9-10=-717/595
BOT CHORD 2-19=-2179/1364, 4-16=-314/327, 15-16=-2090/1593, 14-15=-1900/1438, 13-14=-1890/1164, 12-13=-1345/2248, 10-12=-740/878
WEBS 3-19=-650/881, 16-19=-2290/1471, 3-16=-1186/783, 5-16=-1638/880, 5-15=-589/297, 6-15=-981/618, 6-14=-467/39, 7-14=-456/242, 7-13=-2299/3596

NOTES (10-12)

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; cantilever right exposed ; porch right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
7) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 452 lb uplift at joint 2 and 525 lb uplift at joint 12.
9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
11) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
12) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

JULIUS S.K. LEE LICENSE No 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

December 27

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	HOUSECRAFT - STEPHENS RES.
458217	T10	SPECIAL TRUSS	6	1	I6243894
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055 — 7.350 s Jul 31 2012 MiTek Industries, Inc. Thu Dec 27 09:06:57 2012 Page 1

ID:zD3kW0ImUat9g5WZtYszeky66i7-W1Mp0pDGKaxNVEZf9dCdxz7ybRyfgd2EEhBz2hy4pH

-2-0-0	3-10-7	8-3-2	12-5-13	16-8-0	20-10-3	25-0-14	29-5-9	33-4-0	35-4-0
2-0-0	3-10-7	4-4-11	4-2-11	4-2-3	4-2-3	4-2-11	4-4-11	3-10-7	2-0-0

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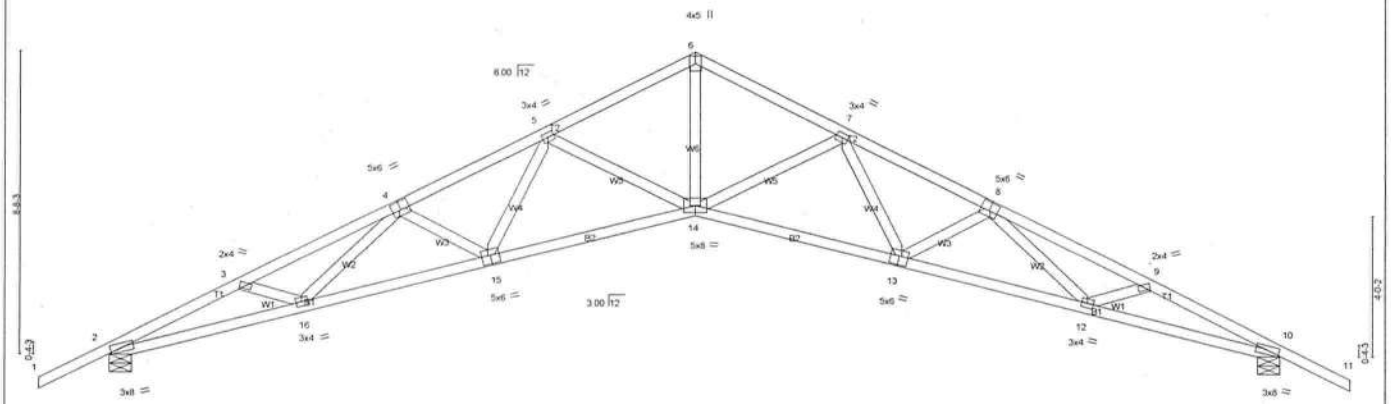

5-5-12	10-9-4	16-8-0	22-6-12	27-10-4	33-4-0
5-5-12	5-3-8	5-10-12	5-10-12	5-3-8	5-5-12

Plate Offsets (X,Y): [2:0-0-10,Edge], [4:0-3-0,0-3-0], [8:0-3-0,0-3-0], [10:0-0-10,Edge], [13:0-3-0,0-3-0], [15:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.92	Vert(LL)	0.52	14	>773	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.71	Vert(TL)	-0.79	14-15	>504	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.73	Horz(TL)	0.55	10	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 170 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2 *Except*
B1: 2x4 SYP No.1
WEBS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-10-10 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=988/0-7-10 (min. 0-1-8), 10=988/0-7-10 (min. 0-1-8)
Max Horz 2=-116(LC 13)
Max Uplift2=-297(LC 12), 10=-297(LC 13)
Max Grav 2=1175(LC 2), 10=1175(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-3727/1964, 3-4=-3574/1871, 4-5=-3283/1737, 5-6=-2532/1295, 6-7=-2533/1295, 7-8=-3303/1748, 8-9=-3641/1907, 9-10=-3791/1998
BOT CHORD 2-16=-1625/3334, 15-16=-1532/3295, 14-15=-1157/2806, 13-14=-1164/2819, 12-13=-1552/3333, 10-12=-1696/3467
WEBS 6-14=-971/1988, 7-14=-615/471, 7-13=-259/437, 8-13=-371/316, 5-14=-616/472, 5-15=-260/438, 4-15=-373/317

NOTES (9-11)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
6) Bearing at joint(s) 2, 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 297 lb uplift at joint 2 and 297 lb uplift at joint 10.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
10) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

December 27,2(

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	HOUSECRAFT - STEPHENS RES.
458217	T12	Half Hip Truss	1	1	I6243896

Builders FirstSource, Lake City, FL 32055

Job Reference (optional)

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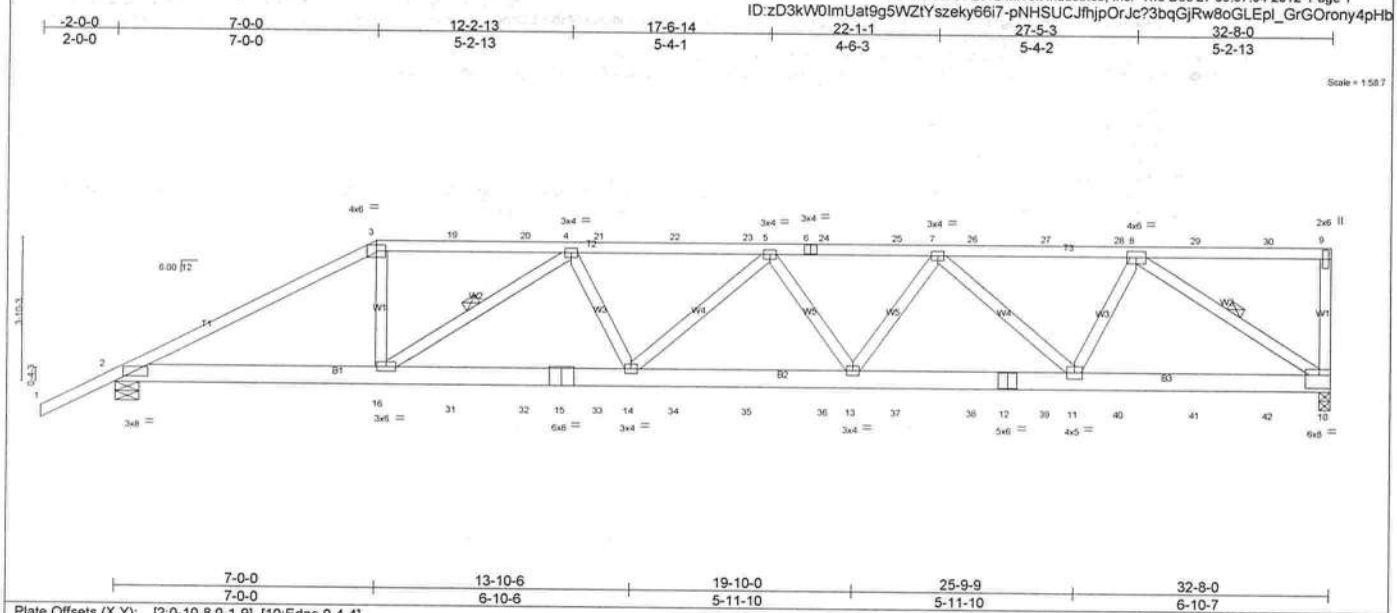

Plate Offsets (X,Y): [2:0-10-8,0-1-9], [10:Edge,0-4-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.98	Vert(LL)	-0.29	13-14	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.71	Vert(TL)	-0.57	13-14	>687	180		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.91	Horz(TL)	0.13	10	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 192 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2 *Except*
T1: 2x4 SYP No.1
BOT CHORD 2x6 SYP No.2
WEBS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-11-12 oc bracing.
WEBS 1 Row at midpt 4-16, 8-10

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 10=1698/0-3-8 (min. 0-2-6), 2=1601/0-7-10 (min. 0-2-4)
Max Horz 2=146(LC 8)
Max Uplift 10=-770(LC 5), 2=-657(LC 8)
Max Grav 10=2011(LC 2), 2=1898(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-3627/1334, 3-19=-3223/1236, 19-20=-3223/1236, 4-20=-3223/1236, 4-21=-4434/1674, 21-22=-4434/1674, 22-23=-4434/1674, 5-23=-4434/1674, 5-6=-4296/1619, 6-24=-4296/1619, 24-25=-4296/1619, 7-25=-4296/1619, 7-26=-2953/1095, 26-27=-2953/1095, 27-28=-2953/1095, 8-28=-2953/1095, 9-10=-276/166

BOT CHORD 2-16=-1220/3174, 16-31=-1680/4333, 31-32=-1680/4333, 15-32=-1680/4333, 15-33=-1680/4333, 14-33=-1680/4333, 14-34=-1748/4535, 34-35=-1748/4535, 35-36=-1748/4535, 13-36=-1748/4535, 13-37=-1528/3974, 37-38=-1528/3974, 12-38=-1528/3974, 12-39=-1528/3974, 11-39=-1528/3974, 11-40=-946/2440, 40-41=-946/2440, 41-42=-946/2440, 10-42=-946/2440

WEBS 3-16=-386/1168, 4-16=-1424/580, 4-14=-4/356, 5-13=-439/236, 7-13=-167/591, 7-11=-1398/594, 8-11=-349/1204, 8-10=-2911/1129

NOTES (11-13)

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 770 lb uplift at joint 10 and 657 lb uplift at joint 2.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 85 lb down and 92 lb up at 7-0-0, 85 lb down and 92 lb up at 9-0-12, 85 lb down and 92 lb up at 11-0-12, 85 lb down and 92 lb up at 13-0-12, 85 lb down and 92 lb up at 15-0-12, 85 lb down and 92 lb up at 17-0-12, 85 lb down and 92 lb up at 19-0-12, 85 lb down and 92 lb up at 21-0-12, 85 lb down and 92 lb up at 23-0-12, 85 lb down and 92 lb up at 25-0-12, 85 lb down and 92 lb up at 27-0-12, 85 lb down and 92 lb up at 29-0-12, and 85 lb down and 92 lb up at 31-0-12, and 85 lb down and 92 lb up at 32-6-4 on top chord, and 240 lb down and 123 lb up at 7-0-0, 49 lb down at 9-0-12, 49 lb down at 11-0-12, 49 lb down at 13-0-12, 49 lb down at 15-0-12, 49 lb down at 17-0-12, 49 lb down at 19-0-12, 49 lb down at 21-0-12, 49 lb down at 23-0-12, 49 lb down at 25-0-12, 49 lb down at 27-0-12, 49 lb down at 29-0-12, and 49 lb down at 31-0-12, and 49 lb down at 32-6-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

Continued on page 2

JULIUS S.K. LEE LICENSE No 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

December 27,20

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	HOUSECRAFT - STEPHENS RES.
458217	T13	Half Hip Truss	1	1	I6243897
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

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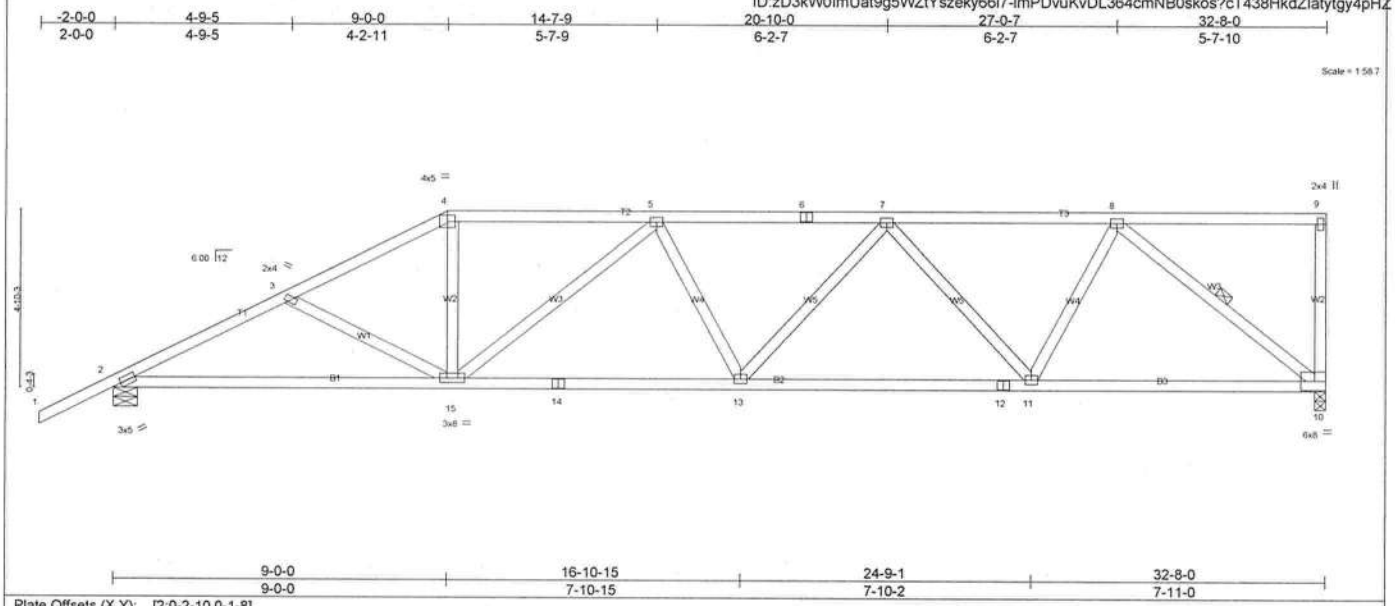


9-0-0	16-10-15	24-9-1	32-8-0
9-0-0	7-10-15	7-10-2	7-11-0

Plate Offsets (X,Y): [2:0-2-10,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	0.18	13-15	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.62	Vert(TL)	-0.29	13-15	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.58	Horz(TL)	0.10	10	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 174 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-11-15 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-5-12 oc bracing.
WEBS 1 Row at midpt 8-10

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 10=871/0-3-8 (min. 0-1-8), 2=973/0-7-10 (min. 0-1-8)
Max Horz 2=180(LC 12)
Max Uplift 10=-312(LC 9), 2=-254(LC 9)
Max Grav 10=1033(LC 2), 2=1157(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1995/1107, 3-4=-1749/967, 4-5=-1556/922, 5-6=-1883/1069, 6-7=-1883/1069, 7-8=-1371/757
BOT CHORD 2-15=-1152/1789, 14-15=-1117/1908, 13-14=-1117/1908, 12-13=-1023/1774, 11-12=-1023/1774, 10-11=-629/1096
WEBS 3-15=-281/266, 4-15=-230/502, 5-15=-546/254, 7-11=-617/408, 8-11=-286/615, 8-10=-1405/811

NOTES (10-12)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) All plates are 3x4 MT20 unless otherwise indicated.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
7) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 312 lb uplift at joint 10 and 254 lb uplift at joint 2.
9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
11) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
12) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

December 27,20

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	HOUSECRAFT - STEPHENS RES.
458217	T15	Hip Truss	1	1	I6243899
					Job Reference (optional)

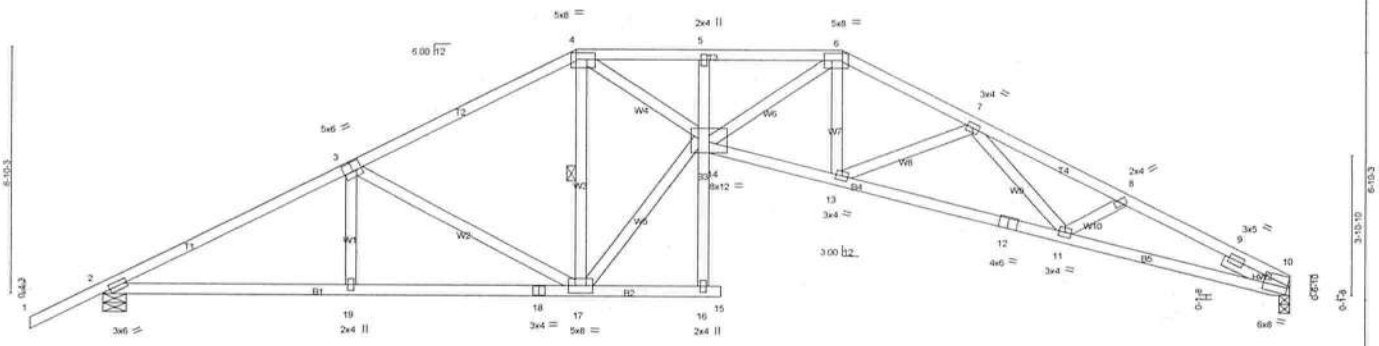
Builders FirstSource, Lake City, FL 32055

7.350 s Jul 31 2012 MiTek Industries, Inc. Thu Dec 27 09:07:13 2012 Page 1

ID:zD3kW0ImUat9g5WZtYszeky66i7-26KsNHQIZUy6Qhoj5_UNbLohduMvQtnbvA3pcmy4pHS

-2-0-0	6-9-14	13-0-0	16-8-0	20-4-0	23-11-3	28-0-0	32-8-0
2-0-0	6-9-14	6-2-2	3-8-0	3-8-0	3-7-3	4-0-13	4-8-1

Scale = 1:60.1



6-9-14	13-0-0	16-8-0	17-0-0	20-4-0	26-5-12	32-8-0
6-9-14	6-2-2	3-8-0	0-4-0	3-4-0	6-1-12	6-2-4

Plate Offsets (X,Y): [2:0-2-9,0-1-8], [3:0-3-0,0-3-0], [4:0-6-4,0-2-4], [6:0-6-0,0-2-8], [10:0-0-13,0-2-12], [14:0-6-0,0-3-10]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.91	Vert(LL)	0.44	15	>888	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.99	Vert(TL)	-0.69	15	>572	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.75	Horz(TL)	0.43	10	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 181 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2 *Except* B3: 2x4 SP No.3
WEBS	2x4 SP No.3 *Except* W4: 2x4 SP No.2
SLIDER	Right 2x4 SP No.3 2-0-0

BRACING

TOP CHORD	Structural wood sheathing directly applied or 1-9-15 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 2-2-0 oc bracing. Except: 10-0-0 oc bracing: 14-16
WEBS	1 Row at midpt 4-17

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 10=875/0-3-8 (min. 0-1-8), 2=982/0-7-10 (min. 0-1-8)
Max Horz 2=115(LC 12)
Max Uplift 10=-228(LC 13), 2=-275(LC 12)
Max Grav 10=1036(LC 2), 2=1167(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD	2-3=-1968/1096, 3-4=-1541/928, 4-5=-3408/1839, 5-6=-3429/1851, 6-7=-2742/1500, 7-8=-3092/1745, 8-9=-3176/1793, 9-10=-1535/833
BOT CHORD	2-19=-890/2059, 18-19=-890/1684, 17-18=-890/1684, 13-14=-1083/2414, 12-13=-1412/2791, 11-12=-1414/2778, 10-11=-1537/2818
WEBS	3-17=-495/373, 4-17=-1144/494, 14-17=-838/1864, 4-14=-1179/2587, 6-14=-561/1193, 6-13=-214/362, 7-13=-393/351

NOTES (10-12)

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
7) Bearing at joint(s) 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 228 lb uplift at joint 10 and 275 lb uplift at joint 2.
9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
11) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
12) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

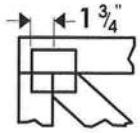
December 27,201

⚠ ***WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.***
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

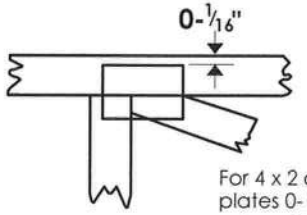
Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

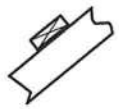
*** Plate location details available in MiTek 20/20 software or upon request.**

PLATE SIZE

4 x 4

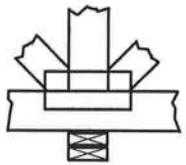
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

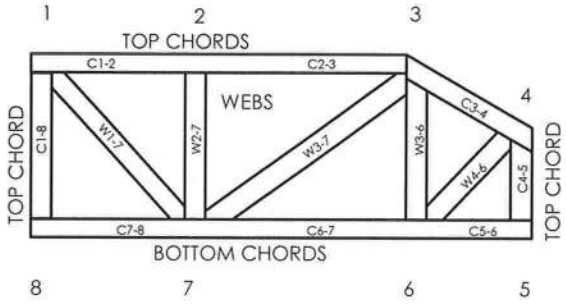


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI1: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ER-5243, 9604B,
9730, 95-43, 96-31, 9667A
NER-487, NER-561
95110, 84-32, 96-67, ER-3907, 9432A

Julius Lee PE
1109 Coastal Bay ,
Boynton Beach ,FL 33435

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI1.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.

August 10, 2010	T-BRACE / I-BRACE DETAIL WITH 2X BRACE ONLY	ST - T-BRACE 2

MiTek Industries, Inc.

Note: T-Bracing / I-Bracing to be used when continuous lateral bracing is impractical. T-Brace / I-Brace must cover 90% of web length.

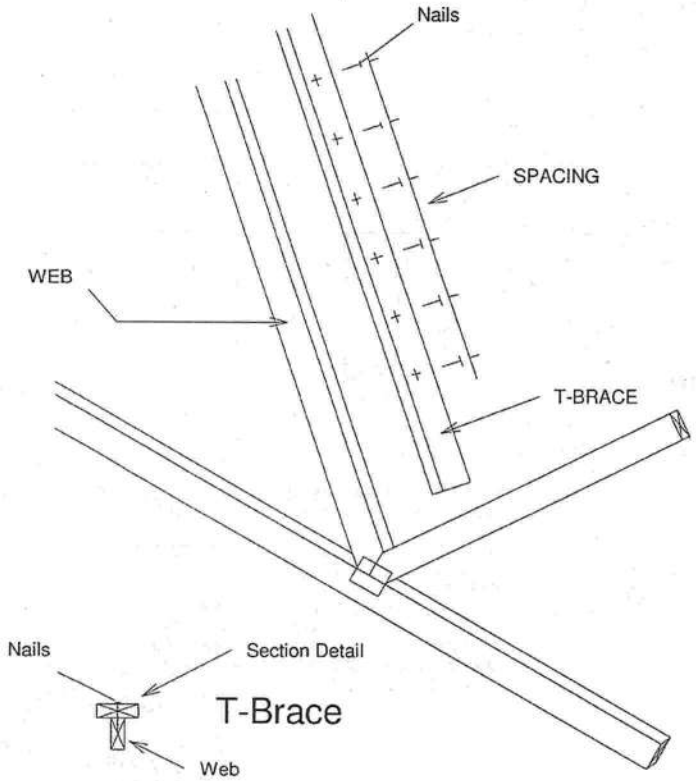
Note: This detail NOT to be used to convert T-Brace / I-Brace webs to continuous lateral braced webs.

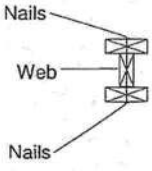
Nailing Pattern		
T-Brace size	Nail Size	Nail Spacing
2x4 or 2x6 or 2x8	10d	6" o.c.
Note: Nail along entire length of T-Brace / I-Brace (On Two-Ply's Nail to Both Plies)		

	Brace Size for One-Ply Truss	
	Specified Continuous Rows of Lateral Bracing	
Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

	Brace Size for Two-Ply Truss	
	Specified Continuous Rows of Lateral Bracing	
Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

T-Brace / I-Brace must be same species and grade (or better) as web member.





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JANUARY 1, 2009	LATERAL TOE-NAIL DETAIL	ST-TOENAIL_SP

MiTek Industries, Inc.

NOTES:
1. TOE-NAILS SHALL BE DRIVEN AT AN ANGLE OF 45 DEGREES WITH THE MEMBER AND MUST HAVE FULL WOOD SUPPORT. (NAIL MUST BE DRIVEN THROUGH AND EXIT AT THE BACK CORNER OF THE MEMBER END AS SHOWN.
2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
3. ALLOWABLE VALUE SHALL BE THE LESSER VALUE OF THE TWO SPECIES FOR MEMBERS OF DIFFERENT SPECIES.

	TOE-NAIL SINGLE SHEAR VALUES PER NDS 2001 (lb/nail)					
	DIAM.	SYP	DF	HF	SPF	SPF-S
3.5" LONG	.131	88.0	80.6	69.9	68.4	59.7
	.135	93.5	85.6	74.2	72.6	63.4
	.162	108.8	99.6	86.4	84.5	73.8
3.25" LONG	.128	74.2	67.9	58.9	57.6	50.3
	.131	75.9	69.5	60.3	59.0	51.1
	.148	81.4	74.5	64.6	63.2	52.5

VALUES SHOWN ARE CAPACITY PER TOE-NAIL.
APPLICABLE DURATION OF LOAD INCREASES MAY BE APPLIED.

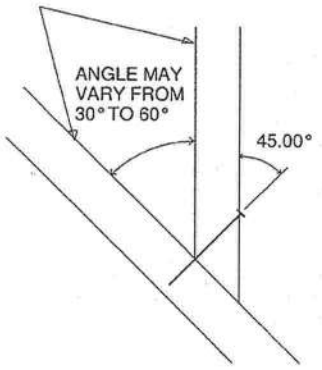
THIS DETAIL APPLICABLE TO THE THREE END DETAILS SHOWN BELOW

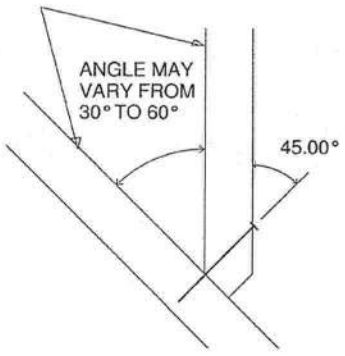
VIEWS SHOWN ARE FOR ILLUSTRATION PURPOSES ONLY

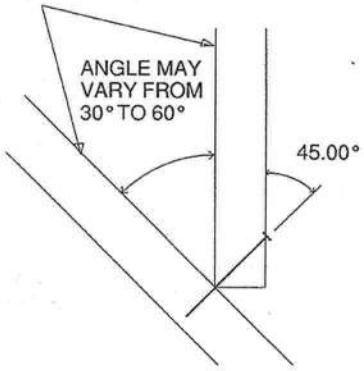
SIDE VIEW

EXAMPLE:
(3) - 16d NAILS (.162" diam. x 3.5") WITH SPF SPECIES BOTTOM CHORD

For load duration increase of 1.15:
3 (nails) X 84.5 (lb/nail) X 1.15 (DOL) = 291.5 lb Maximum Capacity







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FEBRUARY 14, 2012	STANDARD PIGGYBACK TRUSS CONNECTION DETAIL	ST-PIGGY-7-10

MiTek Industries, Chesterfield, MO

MiTek Industries, Inc.

MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E MAX MEAN ROOF HEIGHT = 30 FEET MAX TRUSS SPACING = 24 " O.C. CATEGORY II BUILDING EXPOSURE B or C ASCE 7-10 DURATION OF LOAD INCREASE : 1.60
DETAIL IS NOT APPLICABLE FOR TRUSSES TRANSFERING DRAG LOADS (SHEAR TRUSSES). ADDITIONAL CONSIDERATIONS BY BUILDING ENGINEER/DESIGNER ARE REQUIRED.

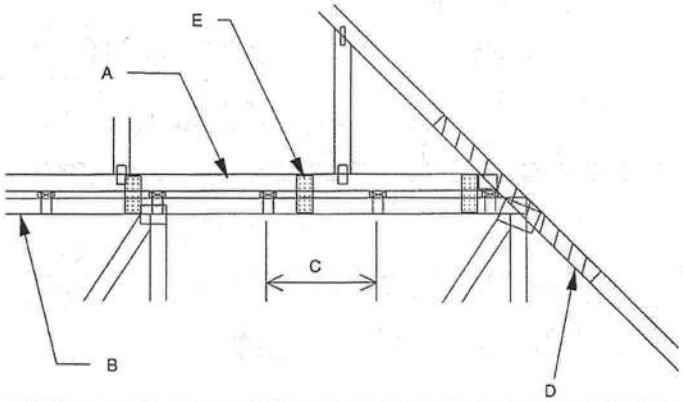
A - PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING. SHALL BE CONNECTED TO EACH PURLIN WITH (2) 0.131" X 3.5" TOE NAILED.

B - BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.

C - PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24" O.C. UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING. CONNECT TO BASE TRUSS WITH (2) 0.131" X 3.5" NAILS EACH.

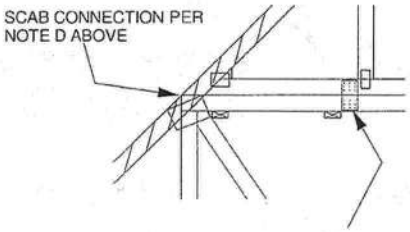
D - 2 X __ X 4'-0" SCAB, SIZE AND GRADE TO MATCH TOP CHORD OF PIGGYBACK TRUSS, ATTACHED TO ONE FACE, CENTERED ON INTERSECTION, WITH (2) ROWS OF 0.131" X 3" NAILS @ 4" O.C. SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH DIRECTIONS AND:
1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR
2. WIND SPEED OF 116 MPH TO 160 MPH WITH A MAXIMUM PIGGYBACK SPAN OF 12 ft.

E - FOR WIND SPEEDS BETWEEN 126 AND 160 MPH, ATTACH MITEK 3X8 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 72" O.C. W/ (4) 0.131" X 1.5" PER MEMBER. STAGGER NAILS FROM OPPOSING FACES. ENSURE 0.5" EDGE DISTANCE. (MIN. 2 PAIRS OF PLATES REQ. REGARDLESS OF SPAN)

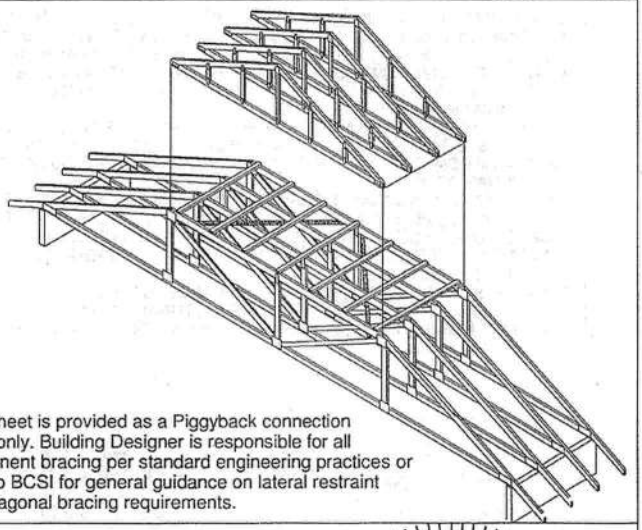


WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH Nail-On PLATES AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.

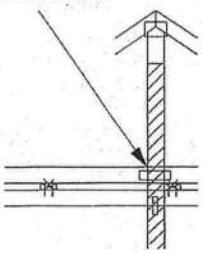


FOR ALL WIND SPEEDS, ATTACH MITEK 3X6 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 48" O.C. W/ (4) 0.131" X 1.5" PER MEMBER. STAGGER NAILS FROM OPPOSING FACES ENSURE 0.5" EDGE DISTANCE.



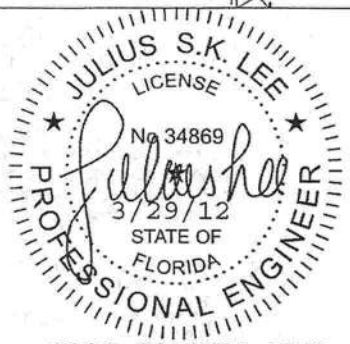
This sheet is provided as a Piggyback connection detail only. Building Designer is responsible for all permanent bracing per standard engineering practices or refer to BCSI for general guidance on lateral restraint and diagonal bracing requirements.

VERTICAL WEB TO EXTEND THROUGH BOTTOM CHORD OF PIGGYBACK



FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

1) VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL.
2) ATTACH 2 x ___ x 4'-0" SCAB TO EACH FACE OF TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 4" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.) (MINIMUM 2X4)
3) THIS CONNECTION IS ONLY VALID FOR A MAXIMUM CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS GREATER THAN 4000 LBS.
4) FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS, NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS.
5) CONCENTRATED LOAD MUST BE APPLIED TO BOTH THE PIGGYBACK AND THE BASE TRUSS DESIGN.



1109 COASTAL BAY
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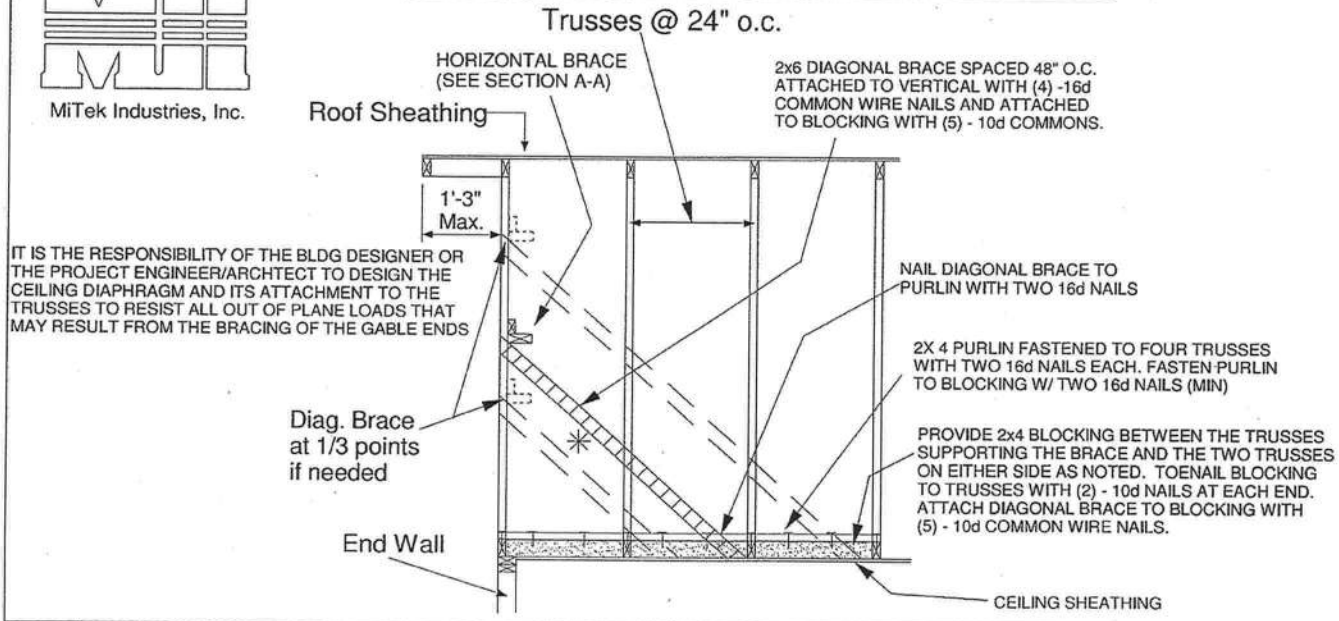
FEBRUARY 14, 2012	Standard Gable End Detail	SHEET 2

MiTek Industries, Chesterfield, MO

MiTek Industries, Inc.

ALTERNATE DIAGONAL BRACING TO THE BOTTOM CHORD



BRACING REQUIREMENTS FOR STRUCTURAL GABLE TRUSSES

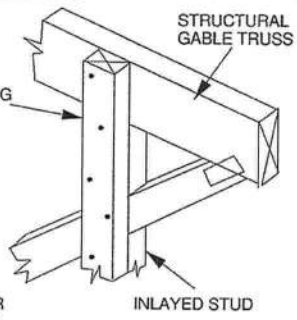
STRUCTURAL GABLE TRUSSES MAY BE BRACED AS NOTED:

METHOD 1 : ATTACH A MATCHING GABLE TRUSS TO THE INSIDE FACE OF THE STRUCTURAL GABLE AND FASTEN PER THE FOLLOWING NAILING SCHEDULE.

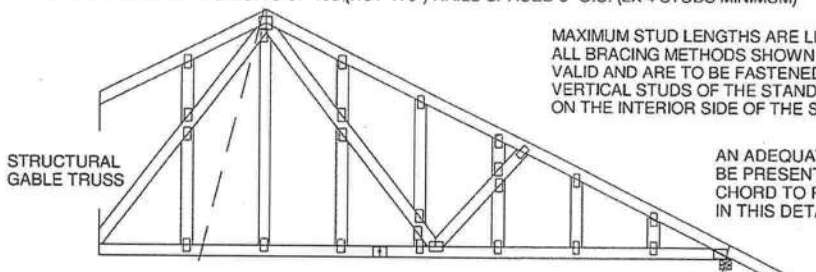
METHOD 2 : ATTACH 2X__ SCABS TO THE FACE OF EACH VERTICAL MEMBER ON THE STRUCTURAL GABLE PER THE FOLLOWING NAILING SCHEDULE. SCABS ARE TO BE OF THE SAME SIZE, GRADE AND SPECIES AS THE TRUSS VERTICALS

NAILING SCHEDULE:

- FOR WIND SPEEDS 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7-10) OR LESS, NAIL ALL MEMBERS WITH ONE ROW OF 10d (.131" X 3") NAILS SPACED 6" O.C.
- FOR WIND SPEEDS GREATER 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7-10) NAIL ALL MEMBERS WITH TWO ROWS OF 10d (.131" X 3") NAILS SPACED 6" O.C. (2X 4 STUDS MINIMUM)

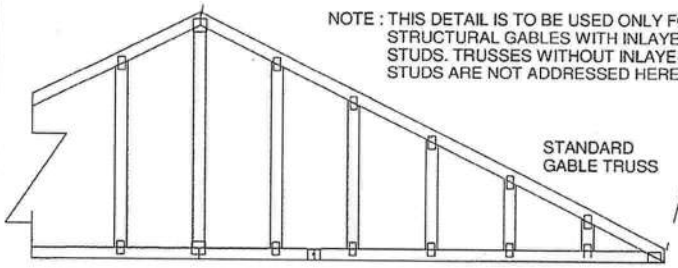


MAXIMUM STUD LENGTHS ARE LISTED ON PAGE 1. ALL BRACING METHODS SHOWN ON PAGE 1 ARE VALID AND ARE TO BE FASTENED TO THE SCABS OR VERTICAL STUDS OF THE STANDARD GABLE TRUSS ON THE INTERIOR SIDE OF THE STRUCTURE.



AN ADEQUATE DIAPHRAGM OR OTHER METHOD OF BRACING MUST BE PRESENT TO PROVIDE FULL LATERAL SUPPORT OF THE BOTTOM CHORD TO RESIST ALL OUT OF PLANE LOADS. THE BRACING SHOWN IN THIS DETAIL IS FOR THE VERTICAL STUDS ONLY.

NOTE : THIS DETAIL IS TO BE USED ONLY FOR STRUCTURAL GABLES WITH INLAYED STUDS. TRUSSES WITHOUT INLAYED STUDS ARE NOT ADDRESSED HERE.



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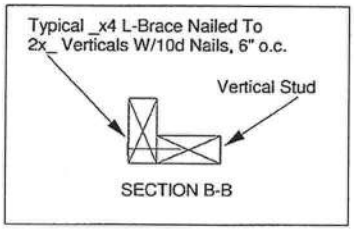
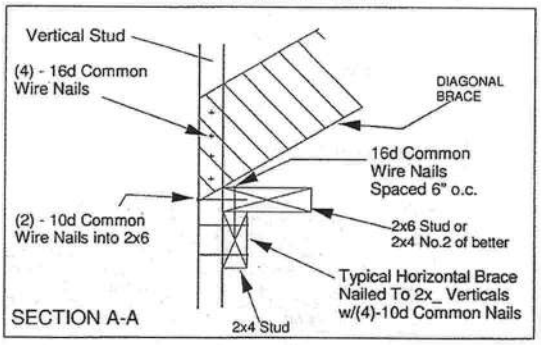
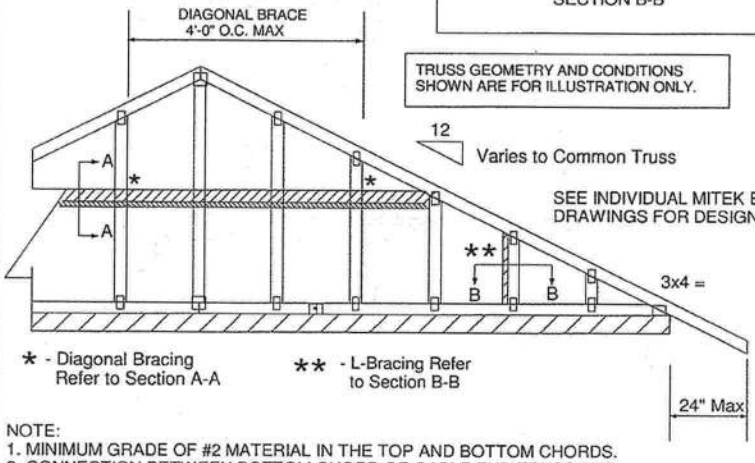
FEBRUARY 14, 2012	Standard Gable End Detail	ST-GE130-001

MiTek Industries, Inc.







TRUSS GEOMETRY AND CONDITIONS SHOWN ARE FOR ILLUSTRATION ONLY.

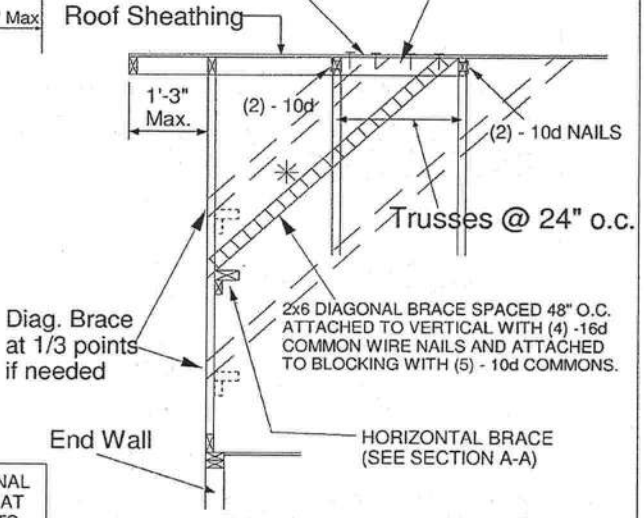
SEE INDIVIDUAL MITEK ENGINEERING DRAWINGS FOR DESIGN CRITERIA

* - Diagonal Bracing Refer to Section A-A

** - L-Bracing Refer to Section B-B

PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d COMMON WIRE NAILS.

(4) - 8d NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SPF BLOCK



NOTE:
1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 1x4 SRB OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.
6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.

Minimum Stud Size Species and Grade	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
		Maximum Stud Length				
2x4 SPF Std/Stud	12" O.C.	4-0-7	4-3-2	6-0-4	8-0-15	12-1-6
2x4 SPF Std/Stud	16" O.C.	3-7-0	3-8-4	5-2-10	7-1-15	10-8-15
2x4 SPF Std/Stud	24" O.C.	2-11-1	3-0-2	4-3-2	5-10-3	8-9-4

✳ Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of web with 10d common wire nails 8in o.c., with 3in minimum end distance. Brace must cover 90% of diagonal length.

MAX MEAN ROOF HEIGHT = 30 FEET
CATEGORY II BUILDING
EXPOSURE B or C
ASCE 7-98, ASCE 7-02, ASCE 7-05 130 MPH
ASCE 7-10 160 MPH
DURATION OF LOAD INCREASE : 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.

1109 COASTAL BAY
BOYNTON BC, FL 33435

FEBRUARY 8, 2008	LATERAL BRACING RECOMMENDATIONS	ST-STRGBCK

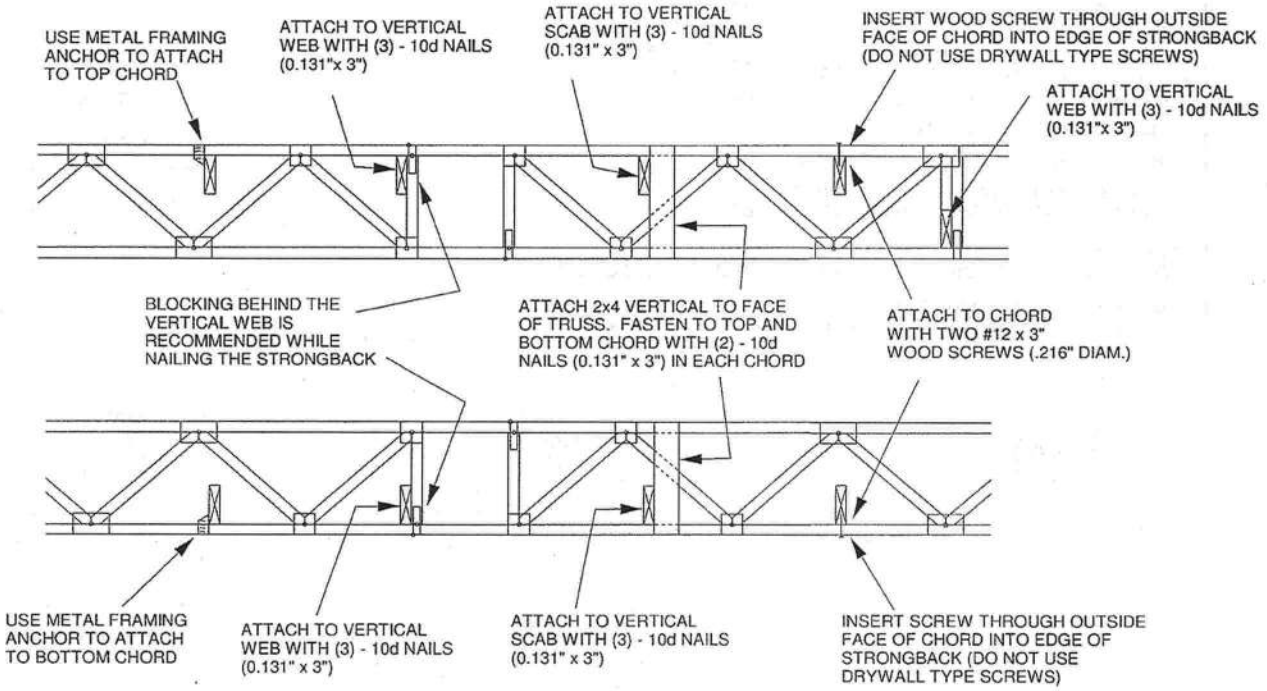
MiTek Industries, Chesterfield, MO

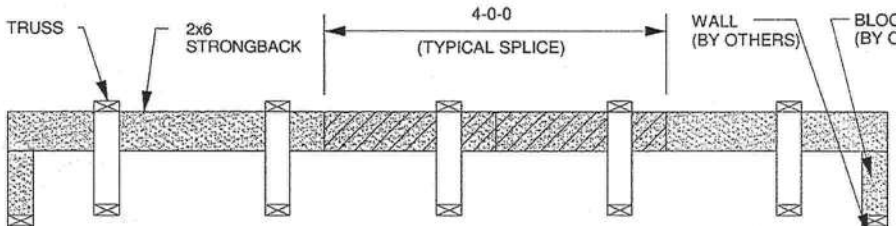
MiTek Industries, Inc.

TO MINIMIZE VIBRATION COMMON TO ALL SHALLOW FRAMING SYSTEMS, 2x6 "STRONGBACK" IS RECOMMENDED, LOCATED EVERY 8 TO 10 FEET ALONG A FLOOR TRUSS.

NOTE 1: 2X6 STRONGBACK ORIENTED VERTICALLY MAY BE POSITIONED DIRECTLY UNDER THE TOP CHORD OR DIRECTLY ABOVE THE BOTTOM CHORD. SECURELY FASTENED TO THE TRUSS USING ANY OF THE METHODS ILLUSTRATED BELOW.

NOTE 2: STRONGBACK BRACING ALSO SATISFIES THE LATERAL BRACING REQUIREMENTS FOR THE BOTTOM CHORD OF THE TRUSS WHEN IT IS PLACED ON TOP OF THE BOTTOM CHORD, IS CONTINUOUS FROM END TO END, CONNECTED WITH A METHOD OTHER THAN METAL FRAMING ANCHOR, AND PROPERLY CONNECTED, BY OTHERS, AT THE ENDS.





THE STRONGBACKS SHALL BE SECURED AT THEIR ENDS TO ADEQUATE SUPPORT, DESIGNED BY OTHERS. IF SPLICING IS NECESSARY, USE A 4'-0" LONG SCAB CENTERED ON THE SPLICE AND JOINED WITH (12) - 10d NAILS (0.131" x 3") EQUALLY SPACED.

ALTERNATE METHOD OF SPLICING:
OVERLAP STRONGBACK MEMBERS A MINIMUM OF 4'-0" AND FASTEN WITH (12) - 10d NAILS (0.131" x 3") STAGGERED AND EQUALLY SPACED.
(TO BE USED ONLY WHEN STRONGBACK IS NOT ALIGNED WITH A VERTICAL)

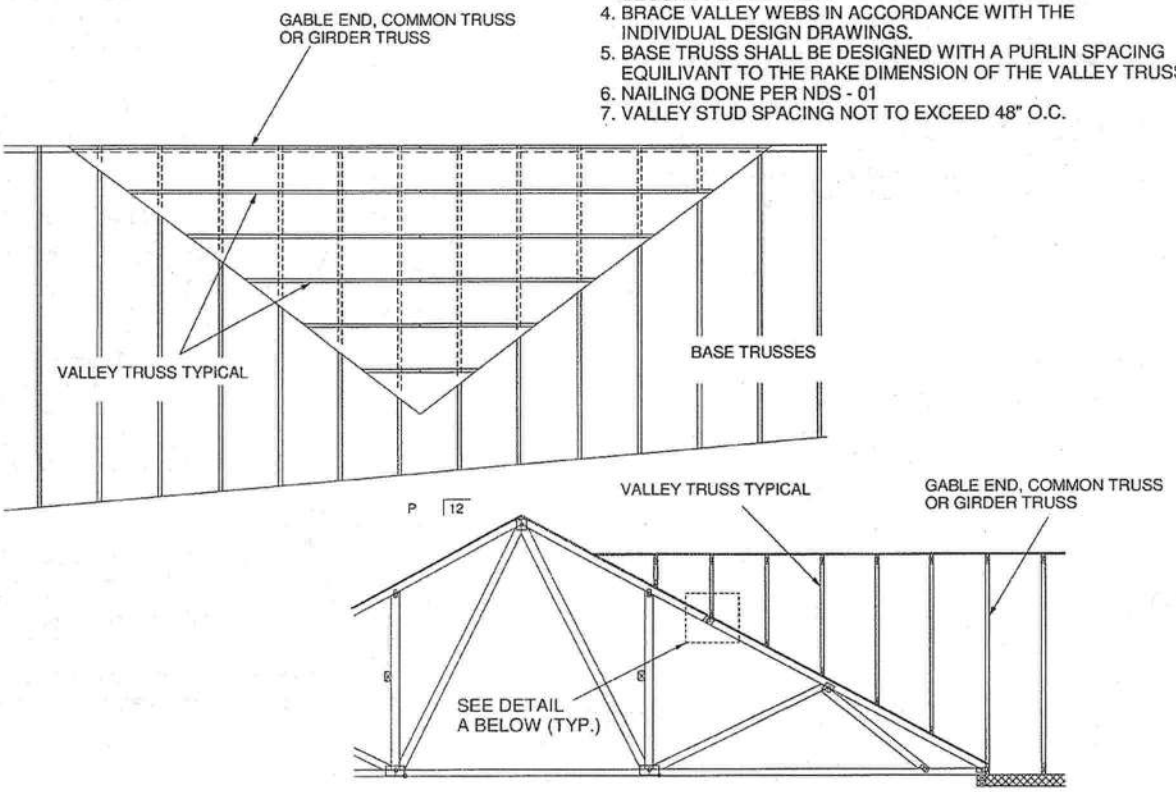
1109 COASTAL BAY
BOYNTON BC, FL 33435

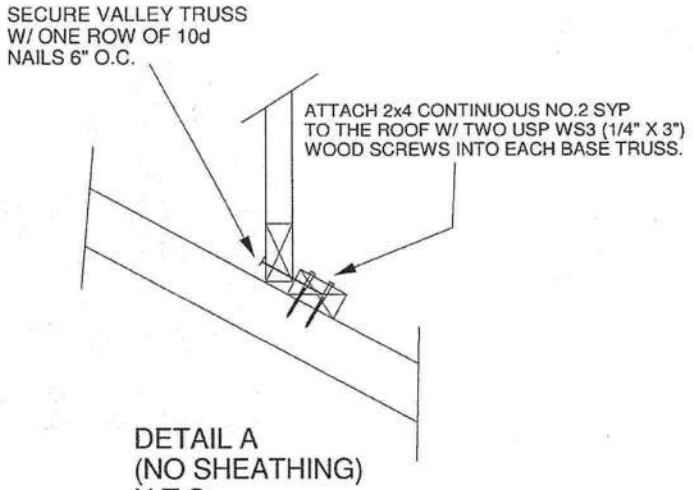
FEBRUARY 14, 2012	TRUSSED VALLEY SET DETAIL	ST-VALLEY HIGH WIND1

MiTek Industries, Inc.

GENERAL SPECIFICATIONS

1. NAIL SIZE = 3" X 0.131" = 10d
2. WOOD SCREW = 3" WS3 USP OR EQUIVALENT
   DO NOT USE DRYWALL OR DECKING TYPE SCREW
3. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND
   SECURE PER DETAIL A
4. BRACE VALLEY WEBS IN ACCORDANCE WITH THE
   INDIVIDUAL DESIGN DRAWINGS.
5. BASE TRUSS SHALL BE DESIGNED WITH A PURLIN SPACING
   EQUILIVANT TO THE RAKE DIMENSION OF THE VALLEY TRUSS SPACING.
6. NAILING DONE PER NDS - 01
7. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.





DETAIL A
(NO SHEATHING)
N.T.S.

WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH
WIND DESIGN PER ASCE 7-10 160 MPH
MAX MEAN ROOF HEIGHT = 30 FEET
ROOF PITCH = MINIMUM 3/12 MAXIMUM 6/12
CATEGORY II BUILDING
EXPOSURE C
WIND DURATION OF LOAD INCREASE : 1.60
MAX TOP CHORD TOTAL LOAD = 50 PSF
MAX SPACING = 24" O.C. (BASE AND VALLEY)
MINIMUM REDUCED DEAD LOAD OF 6 PSF
ON THE TRUSSES

1109 COASTAL BAY
BOYNTON BC, FL 33435

Job	Truss	Truss Type	Qty	Ply	HOUSECRAFT - STEPHENS RES.	
458217	T16	Hip Truss	1	1	Job Reference (optional)	I6243900

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ID:zD3kW0ImUat9g5WZtYszeky66i7-xtZNDeTodjSYvJ5VKqZJlByNrVnCMoXAqo11IXy4pHO

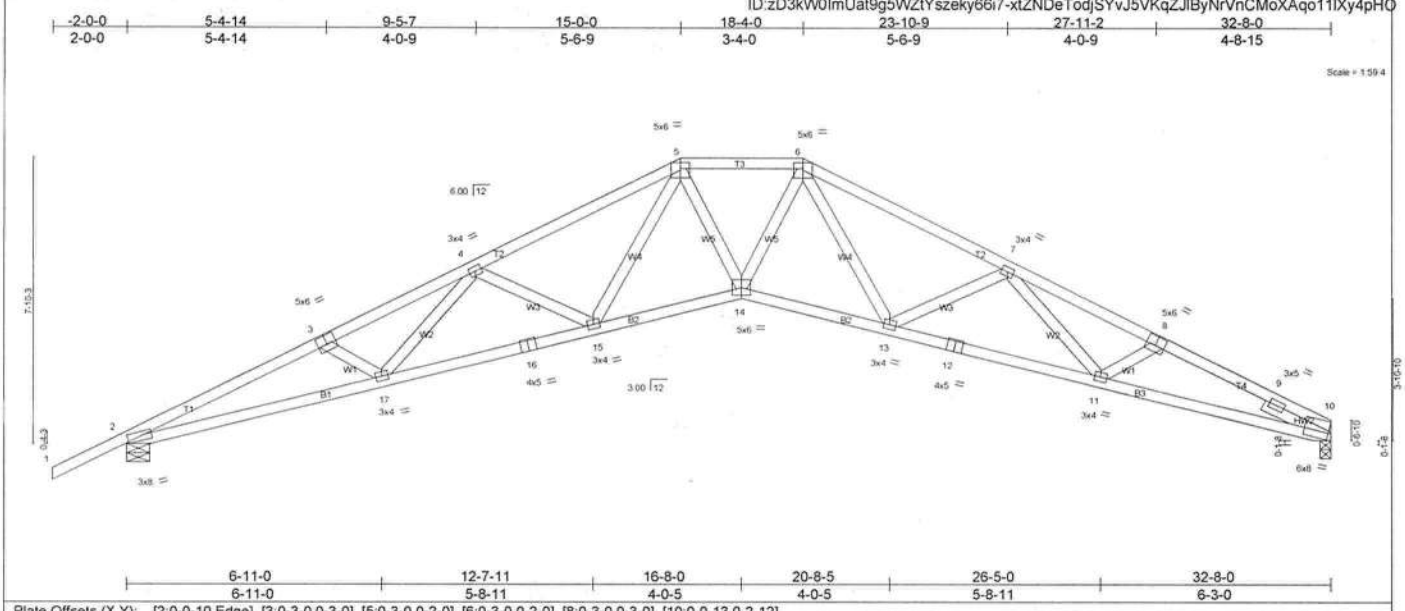


Plate Offsets (X,Y): [2:0-0-10,Edge], [3:0-3-0,0-3-0], [5:0-3-0,0-2-0], [6:0-3-0,0-2-0], [8:0-3-0,0-3-0], [10:0-0-13,0-2-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.90	Vert(LL)	0.48	15-17	>821	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.72	Vert(TL)	-0.71	15-17	>549	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.25	Horz(TL)	0.49	10	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 163 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2 *Except* B1,B3: 2x4 SYP No.1
WEBS 2x4 SP No.3
SLIDER Right 2x4 SP No.3 2-0-0

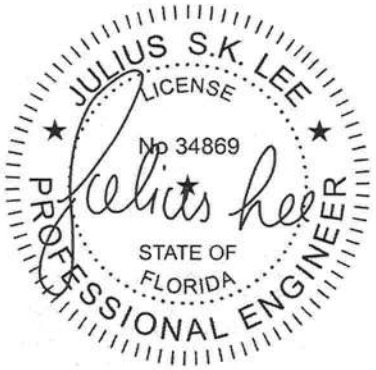
BRACING
TOP CHORD Structural wood sheathing directly applied or 1-11-5 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-9-7 oc bracing.
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 10=879/0-3-8 (min. 0-1-8), 2=973/0-7-10 (min. 0-1-8)
Max Horz 2=128(LC 12)
Max Uplift 10=-242(LC 13), 2=-285(LC 12)
Max Grav 10=1042(LC 2), 2=1157(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-3650/1994, 3-4=-3469/1930, 4-5=-2894/1614, 5-6=-2539/1432, 6-7=-2835/1587, 7-8=-3143/1802, 8-9=-3224/1832, 9-10=-1599/862
BOT CHORD 2-17=-1748/3279, 16-17=-1539/2989, 15-16=-1534/3001, 14-15=-978/2257, 13-14=-968/2235, 12-13=-1479/2864, 11-12=-1484/2852, 10-11=-1570/2861
WEBS 4-15=-515/423, 5-15=-367/566, 5-14=-235/588, 6-14=-257/638, 6-13=-334/493, 7-13=-426/392

NOTES (10-12)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
7) Bearing at joint(s) 10, 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 242 lb uplift at joint 10 and 285 lb uplift at joint 2.
9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
11) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
12) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



December 27,2012

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	HOUSECRAFT - STEPHENS RES.
458217	T14	Hip Truss	1	1	I624389
					Job Reference (optional)

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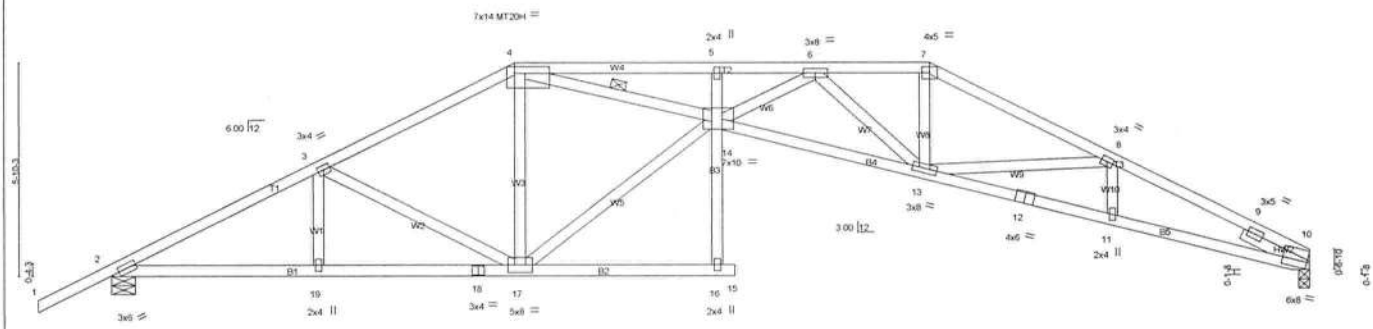
ID:zD3kW0ImUat9g5WZtYszeky66i7-eXekIFNPGZaXZE39QsxgziABZhLqDSW8DCr90Ry4pI

-2-0-0	5-7-11	11-0-0	16-8-0	19-2-9	22-4-0	27-3-11	32-8-0
2-0-0	5-7-11	5-4-5	5-8-0	2-6-9	3-1-7	4-11-11	5-4-5

Scale = 1:5



5-7-11	11-0-0	16-8-0	17-0-0	22-4-0	27-3-11	32-8-0
5-7-11	5-4-5	5-8-0	0-4-0	5-4-0	4-11-11	5-4-5

Plate Offsets (X,Y): [2:0-2-9,0-1-8], [4:0-11-8,0-2-4], [10:0-0-13,0-2-12], [14:0-3-12,0-3-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.84	Vert(LL)	0.74	15	>530	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.95	Vert(TL)	-1.17	15	>336	180	MT20H	187/143
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.97	Horz(TL)	0.66	10	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 178 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2 *Except* T2: 2x4 SYP No.1
BOT CHORD	2x4 SP No.2 *Except* B3: 2x4 SP No.3, B5: 2x4 SYP No.1
WEBS	2x4 SP No.3 *Except* W4: 2x4 SP No.2
SLIDER	Right 2x4 SP No.3 2-0-0

BRACING

TOP CHORD	Structural wood sheathing directly applied or 2-2-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 2-2-0 oc bracing. Except: 10-0-0 oc bracing: 14-16
WEBS	1 Row at midpt 4-14

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=984/0-7-10 (min. 0-1-8), 10=873/0-3-8 (min. 0-1-8)
Max Horz 2=103(LC 12)
Max Uplift 2=-261(LC 12), 10=-214(LC 13)
Max Grav 2=1169(LC 2), 10=1034(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD	2-3=-1955/1083, 3-4=-1647/966, 4-5=-5479/2882, 5-6=-5537/2894, 6-7=-2663/1498, 7-8=-2936/1582, 8-9=-3148/1754, 9-10=-1715/908
BOT CHORD	2-19=-904/2250, 18-19=-892/1679, 17-18=-892/1679, 13-14=-1848/3843, 12-13=-1511/2832, 11-12=-1515/2821, 10-11=-1500/2799
WEBS	3-17=-349/279, 4-17=-756/361, 14-17=-767/1626, 4-14=-2044/4211, 6-14=-947/1995, 6-13=-1602/805, 7-13=-507/1026, 8-13=-197/269

NOTES (11-13)

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
7) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
8) Bearing at joint(s) 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 261 lb uplift at joint 2 and 214 lb uplift at joint 10.
10) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
12) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
13) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

JULIUS S.K. LEE LICENSE No 34869 STATE OF FLORIDA PROFESSIONAL ENG

December

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 3343

Job	Truss	Truss Type	Qty	Ply	HOUSECRAFT - STEPHENS RES.
458217	T12	Half Hip Truss	1	1	I6243896 Job Reference (optional)

11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
12) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
13) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-44, 3-9=-44, 2-10=-10
Concentrated Loads (lb)
Vert: 3=-69(B) 9=-69(B) 10=-22(B) 16=-205(B) 19=-69(B) 20=-69(B) 21=-69(B) 22=-69(B) 23=-69(B) 24=-69(B) 25=-69(B) 26=-69(B) 27=-69(B) 28=-69(B) 29=-69(B) 30=-69(B) 31=-22(B) 32=-22(B) 33=-22(B) 34=-22(B) 35=-22(B) 36=-22(B) 37=-22(B) 38=-22(B) 39=-22(B) 40=-22(B) 41=-22(B) 42=-22(B)

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	HOUSECRAFT - STEPHENS RES.
458217	T11	Special Truss	2	1	I6243895
					Job Reference (optional)

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ID:zD3kW0ImUat9g5WZtYszeky66i7-xc2yeqG8dVJyMhlEqlIKZbIT8fzNt_CgwfQdf0y4pH?

-2-0-0	3-10-7	8-3-2	12-5-13	16-8-0	20-10-3	25-0-14	29-5-9	32-8-0
2-0-0	3-10-7	4-4-11	4-2-11	4-2-3	4-2-3	4-2-11	4-4-11	3-2-8

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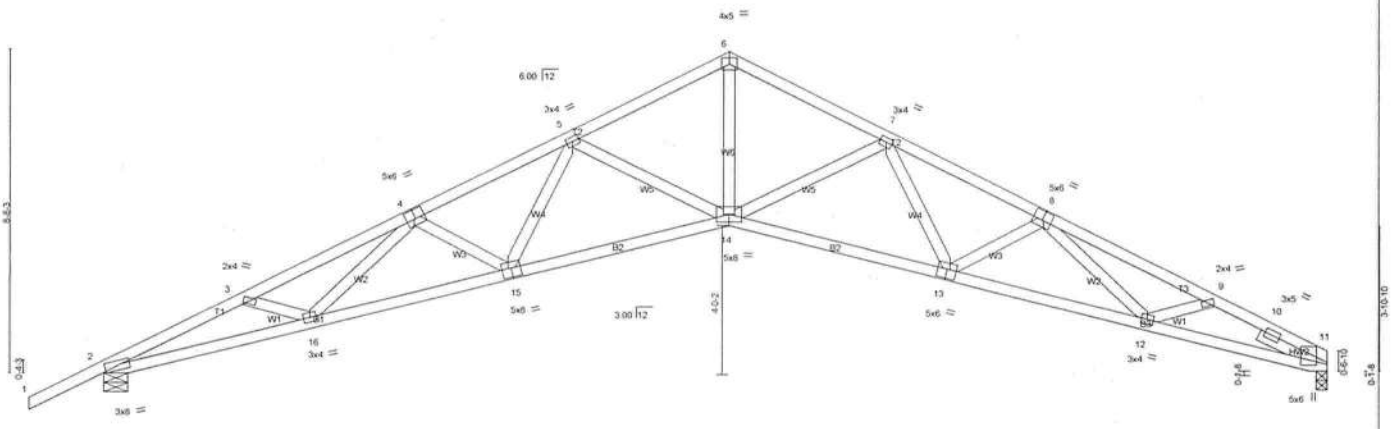

5-5-12	10-9-4	16-8-0	22-6-12	27-10-4	32-6-4	32-8-0
5-5-12	5-3-8	5-10-12	5-10-12	5-3-8	4-8-0	0-1-12

Plate Offsets (X,Y): [2:0-0-10,Edge], [4:0-3-0,0-3-0], [8:0-3-0,0-3-0], [11:0-0-13,0-3-8], [13:0-3-0,0-3-0], [15:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.90	Vert(LL)	0.48	14	>811	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.77	Vert(TL)	-0.72	14-15	>546	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.71	Horz(TL)	0.49	11	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 167 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2 *Except*
B1: 2x4 SYP No.1
WEBS 2x4 SP No.3
SLIDER Right 2x4 SP No.3 2-0-0

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-7-8 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=973/0-7-10 (min. 0-1-8), 11=879/0-3-8 (min. 0-1-8)
Max Horz 2=138(LC 12)
Max Uplift 2=-294(LC 12), 11=-251(LC 13)
Max Grav 2=1157(LC 2), 11=1042(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-3669/2027, 3-4=-3514/1937, 4-5=-3159/1783, 5-6=-2386/1331, 6-7=-2386/1331, 7-8=-3049/1733, 8-9=-3143/1798, 9-10=-3097/1808, 10-11=-1486/844
BOT CHORD 2-16=-1789/3296, 15-16=-1650/3143, 14-15=-1263/2625, 13-14=-1235/2560, 12-13=-1573/2960, 11-12=-1548/2722
WEBS 4-15=-374/315, 5-15=-258/439, 5-14=-617/470, 6-14=-1003/1859, 7-14=-546/439, 7-13=-218/349, 8-13=-287/282

NOTES (9-11)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
6) Bearing at joint(s) 2, 11 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 294 lb uplift at joint 2 and 251 lb uplift at joint 11.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
10) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

December 27,2

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	HOUSECRAFT - STEPHENS RES.
458217	T09	Special Truss	2	1	I6243893
					Job Reference (optional)

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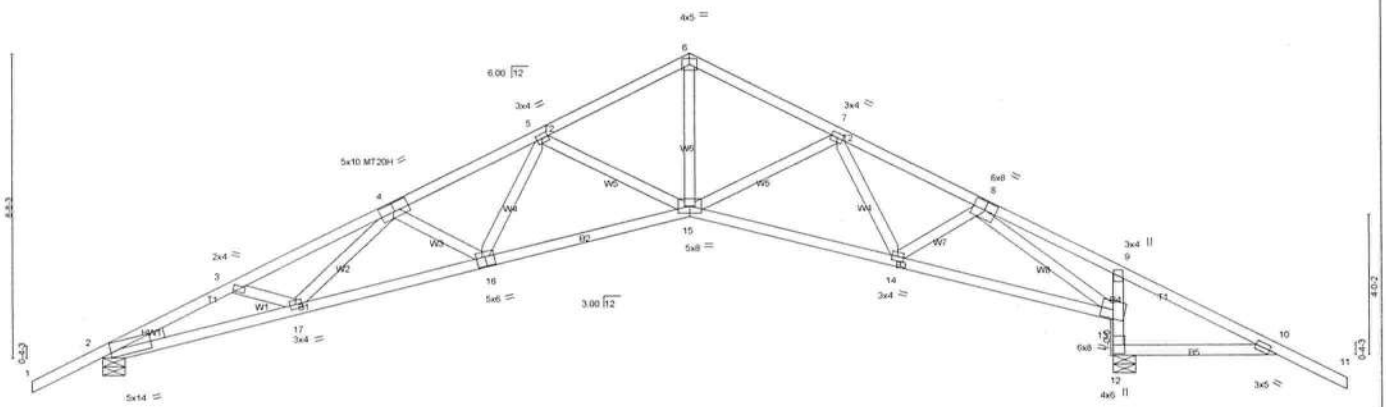
ID:zD3kW0ImUat9g5WZtYszeky66i7-eF6IARAIGLRy0dGtwn7hn7zFTqZvkm?eJ3Dmvwy4pHm

-2-0-0	3-10-7	8-3-2	12-5-13	16-8-0	20-10-3	24-11-0	28-8-6	33-4-0	35-4-0
2-0-0	3-10-7	4-4-11	4-2-11	4-2-3	4-2-2	4-0-13	3-9-6	4-7-10	2-0-0

Scale = 1:62.0



5-5-12	10-9-4	16-8-0	22-6-11	28-8-6	33-4-0
5-5-12	5-3-8	5-10-12	5-10-11	6-1-11	4-7-10

Plate Offsets (X,Y): [2:0-3-2,0-0-15], [4:0-5-0,0-3-0], [10:0-2-10,0-1-8], [16:0-3-0,0-3-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.99	Vert(LL)	0.81	15-16	>427	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.89	Vert(TL)	0.69	15-16	>504	180	MT20H	187/143
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.99	Horz(TL)	-0.47	12	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 173 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2 *Except*
T1: 2x4 SYP No.1
BOT CHORD 2x4 SP No.2 *Except*
B4: 2x4 SYP No.1
WEBS 2x4 SP No.3 *Except*
W8: 2x4 SP No.2
WEDGE
Left: 2x4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 12=1141/0-7-10 (min. 0-1-10), 2=835/0-7-10 (min. 0-1-8)
Max Horz 2=129(LC 9)
Max Uplift 12=-504(LC 9), 2=-431(LC 9)
Max Grav 12=1356(LC 2), 2=993(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-2723/5260, 3-4=-2587/5297, 4-5=-2255/4466, 5-6=-1543/2891, 6-7=-1546/2895, 7-8=-1413/2998, 8-9=-1081/905, 9-10=-766/664
BOT CHORD 2-17=-4675/2424, 16-17=-4224/2329, 15-16=-3078/1877, 14-15=-2242/1406, 13-14=-1681/906, 12-13=-1355/2363, 10-12=-759/797
WEBS 3-17=-312/110, 4-17=-611/176, 4-16=-334/605, 5-16=-1197/391, 5-15=-559/1198, 6-15=-2398/1122, 7-15=-122/266, 7-14=-464/86, 8-14=-830/414, 8-13=-2138/3598

NOTES (10-12)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; cantilever right exposed ; porch right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) All plates are MT20 plates unless otherwise indicated.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
7) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 504 lb uplift at joint 12 and 431 lb uplift at joint 2.
9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
11) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
12) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

JULIUS S.K. LEE LICENSE No 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

December 27,2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	HOUSECRAFT - STEPHENS RES.
458217	T07	Hip Truss	1	1	I6243891
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

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ID:zD3kW0ImUat9g5WZtYszeky66i7-tjeHVi4k9uQ43OfLS6_pSRdwicwJsFWTTpHLdOy4pHu

-2-0-0	6-9-14	13-0-0	20-4-0	26-6-2	33-4-0	35-4-0
2-0-0	6-9-14	6-2-2	7-4-0	6-2-2	6-9-14	2-0-0

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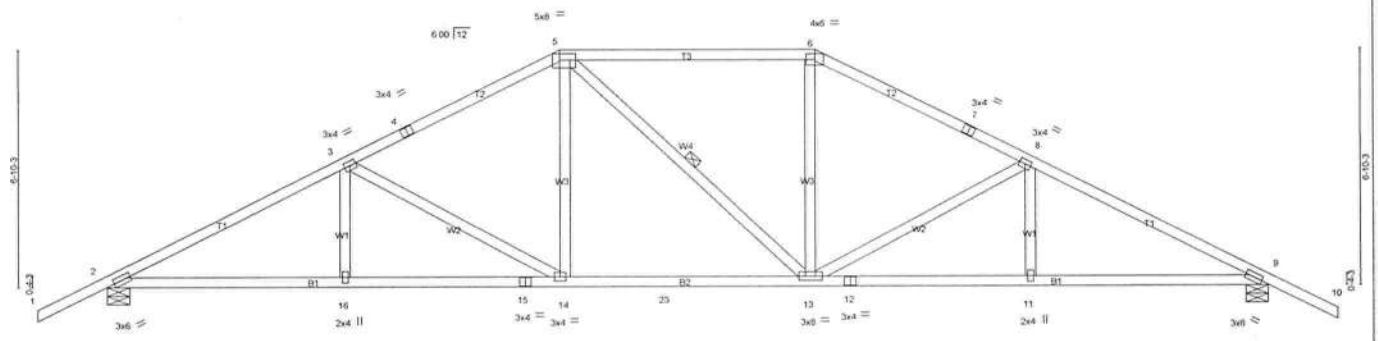

6-9-14	13-0-0	20-4-0	26-6-2	33-4-0
6-9-14	6-2-2	7-4-0	6-2-2	6-9-14

Plate Offsets (X,Y): [2:0-2-9,0-1-8], [5:0-5-8,0-2-4], [9:0-2-9,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.85	Vert(LL)	0.15	14-16	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.49	Vert(TL)	-0.27	13-14	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.51	Horz(TL)	0.10	9	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 173 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-4-14 oc bracing.
WEBS 1 Row at midpt 5-13

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=1037/0-7-10 (min. 0-1-8), 9=1026/0-7-10 (min. 0-1-8)
Max Horz 2=-94(LC 13)
Max Uplift 2=-278(LC 12), 9=-278(LC 13)
Max Grav 2=1175(LC 2), 9=1175(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-2109/1120, 3-4=-1690/927, 4-5=-1626/948, 5-6=-1457/918, 6-7=-1600/948, 7-8=-1663/927, 8-9=-2086/1120
BOT CHORD 2-16=-841/1869, 15-16=-841/1858, 14-15=-841/1858, 14-23=-520/1453, 13-23=-520/1453, 12-13=-847/1850, 11-12=-847/1850, 9-11=-847/1909
WEBS 3-14=-483/375, 5-14=-141/382, 6-13=-140/346, 8-13=-487/375

NOTES (9-11)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 278 lb uplift at joint 2 and 278 lb uplift at joint 9.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
10) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

December 27,2

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	HOUSECRAFT - STEPHENS RES.
458217	T05	Hip Truss	1	1	I6243889
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

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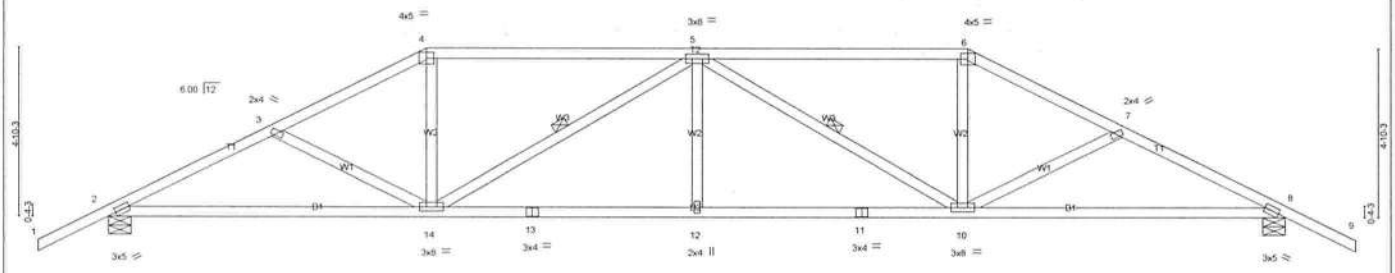
ID:zD3kW0ImUat9g5WZtYszeky66i7-e_btcdz4H7IMU?TcQjKibXIP9_q?FFn8PwcMqPy4pl1

-2-0-0	4-9-5	9-0-0	16-8-0	24-4-0	28-6-11	33-4-0	35-4-0
2-0-0	4-9-5	4-2-11	7-8-0	7-8-0	4-2-11	4-9-5	2-0-0

Scale = 1:62.0



9-0-0	16-8-0	24-4-0	33-4-0
9-0-0	7-8-0	7-8-0	9-0-0

Plate Offsets (X,Y): [2:0-2-10,0-1-8], [8:0-2-10,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.71	Vert(LL)	0.20	12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.63	Vert(TL)	-0.32	10-12	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.29	Horz(TL)	0.11	8	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 168 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-6-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-11-11 oc bracing.
WEBS 1 Row at midpt 5-14, 5-10

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=988/0-7-10 (min. 0-1-8), 8=988/0-7-10 (min. 0-1-8)
Max Horz 2=69(LC 12)
Max Uplift2=-248(LC 12), 8=-248(LC 13)
Max Grav 2=1175(LC 2), 8=1175(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-2043/1173, 3-4=-1809/1040, 4-5=-1612/990, 5-6=-1612/990, 6-7=-1808/1039, 7-8=-2042/1173
BOT CHORD 2-14=-911/1814, 13-14=-964/2069, 12-13=-964/2069, 11-12=-964/2069, 10-11=-964/2069, 8-10=-920/1830
WEBS 3-14=-263/256, 4-14=-224/476, 5-14=-621/312, 5-10=-621/313, 6-10=-224/476, 7-10=-262/255

NOTES (9-11)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 248 lb uplift at joint 2 and 248 lb uplift at joint 8.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
10) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

December 27,2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

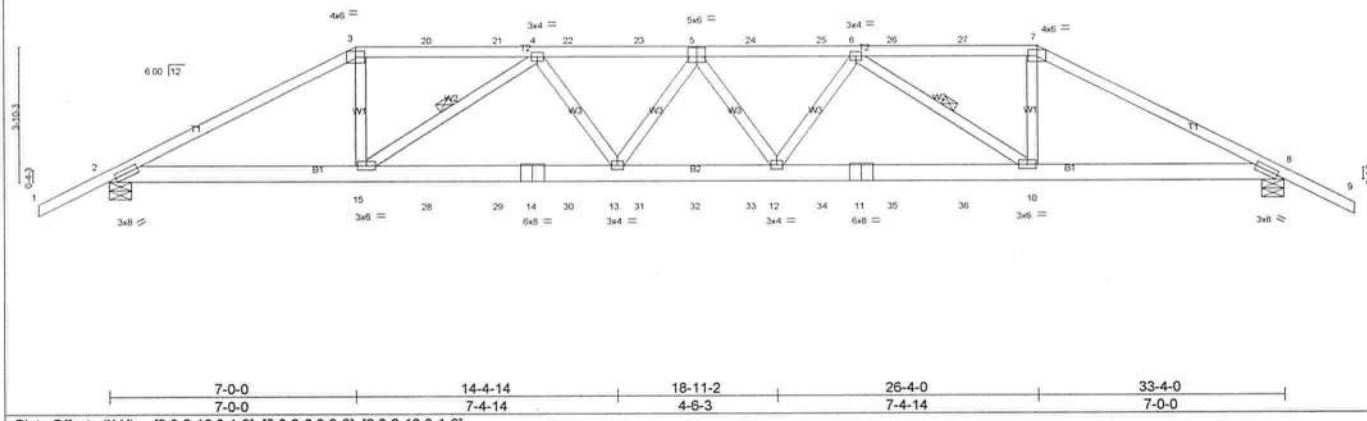
Job	Truss	Truss Type	Qty	Ply	HOUSECRAFT - STEPHENS RES.
458217	T04	Hip Truss	1	1	I6243888
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055 — 7.350 s Jul 31 2012 MiTek Industries, Inc. Thu Dec 27 09:06:33 2012 Page 1

ID:zD3kW0ImUat9g5WZtYszeky66i7-DPvk_bxC_CvndYk1lam?zv7tbmmK2qTiiyNiD4y4pI4

-2-0-0	7-0-0	12-1-12	16-8-0	21-2-4	26-4-0	33-4-0	35-4-0
2-0-0	7-0-0	5-1-12	4-6-4	4-6-4	5-1-12	7-0-0	2-0-0

Scale = 1:62.0



7-0-0	14-4-14	18-11-2	26-4-0	33-4-0
7-0-0	7-4-14	4-6-3	7-4-14	7-0-0

Plate Offsets (X,Y): [2:0-2-10,0-1-8], [5:0-3-0,0-3-0], [8:0-2-10,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.79	Vert(LL)	-0.32	12-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.75	Vert(TL)	-0.61	12-13	>653	180		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.52	Horz(TL)	0.16	8	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 186 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SYP No.1
BOT CHORD 2x6 SYP No.2
WEBS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-7-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-8-4 oc bracing.
WEBS 1 Row at midpt 4-15, 6-10

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=1687/0-7-10 (min. 0-2-6), 8=1731/0-7-10 (min. 0-2-7)
Max Horz 2=-56(LC 9)
Max Uplift 2=-724(LC 8), 8=-786(LC 9)
Max Grav 2=2001(LC 2), 8=2055(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-3862/1451, 3-20=-3437/1342, 20-21=-3437/1342, 4-21=-3437/1342, 4-22=-4890/1904, 22-23=-4890/1904, 5-23=-4890/1904, 5-24=-4916/1928, 24-25=-4916/1928, 6-25=-4916/1928, 6-26=-3543/1474, 26-27=-3543/1474, 7-27=-3543/1474, 7-8=-3982/1600
BOT CHORD 2-15=-1259/3384, 15-28=-1794/4660, 28-29=-1794/4660, 14-29=-1794/4660, 14-30=-1794/4660, 13-30=-1794/4660, 13-31=-1933/5045, 31-32=-1933/5045, 32-33=-1933/5045, 12-33=-1933/5045, 12-34=-1834/4710, 11-34=-1834/4710, 11-35=-1834/4710, 35-36=-1834/4710, 10-36=-1834/4710, 8-10=-1354/3492
WEBS 3-15=-444/1270, 4-15=-1567/690, 4-13=-115/464, 5-13=-308/193, 5-12=-251/128, 6-12=-52/449, 6-10=-1486/597, 7-10=-390/1224

NOTES (11-13)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 724 lb uplift at joint 2 and 786 lb uplift at joint 8.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 85 lb down and 92 lb up at 7-0-0, 85 lb down and 92 lb up at 9-0-12, 85 lb down and 92 lb up at 11-0-12, 85 lb down and 92 lb up at 13-0-12, 85 lb down and 92 lb up at 15-0-12, 85 lb down and 92 lb up at 16-8-0, 85 lb down and 92 lb up at 18-3-4, 85 lb down and 92 lb up at 20-3-4, 85 lb down and 92 lb up at 22-3-4, and 85 lb down and 92 lb up at 24-3-4, and 218 lb down and 236 lb up at 26-4-0 on top chord, and 240 lb down and 123 lb up at 7-0-0, 49 lb down at 9-0-12, 49 lb down at 11-0-12, 49 lb down at 13-0-12, 49 lb down at 15-0-12, 49 lb down at 16-8-0, 49 lb down at 18-3-4, 49 lb down at 20-3-4, 49 lb down at 22-3-4, and 49 lb down at 24-3-4, and 240 lb down and 123 lb up at 26-3-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

Continued on page 2

JULIUS S.K. LEE LICENSE No 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

December 27

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	HOUSECRAFT - STEPHENS RES.
458217	T02	Hip Truss	1	1	I6243886
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

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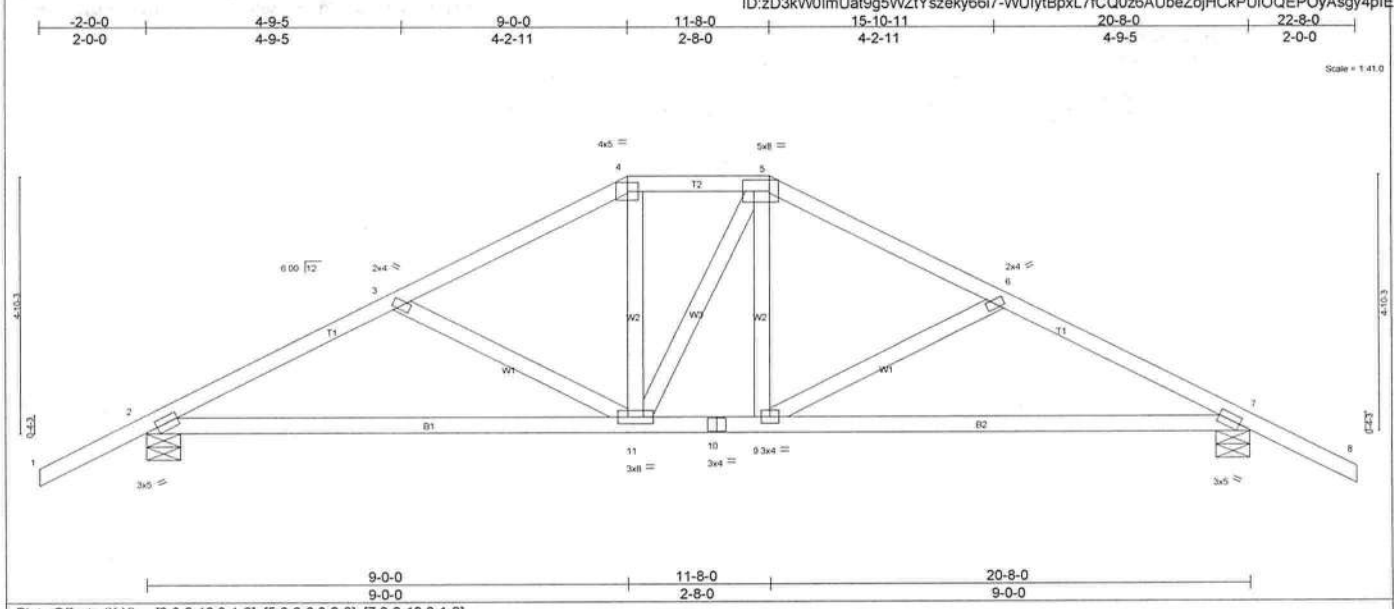

Plate Offsets (X,Y): [2:0-2-10,0-1-8], [5:0-6-0,0-2-8], [7:0-2-10,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	-0.12	9-17	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.53	Vert(TL)	-0.22	9-17	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.17	Horz(TL)	0.03	7	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 106 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-4-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-3-2 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=646/0-7-10 (min. 0-1-8), 7=646/0-7-10 (min. 0-1-8)
Max Horz 2=69(LC 12)
Max Uplift2=-192(LC 12), 7=-192(LC 13)
Max Grav 2=769(LC 2), 7=769(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1172/704, 3-4=-905/551, 4-5=-786/543, 5-6=-903/551, 6-7=-1170/704
BOT CHORD 2-11=-499/1230, 10-11=-253/786, 9-10=-253/786, 7-9=-506/1308
WEBS 3-11=-318/289, 6-9=-318/289

NOTES (9-11)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 192 lb uplift at joint 2 and 192 lb uplift at joint 7.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
10) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

JULIUS S.K. LEE LICENSE No 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

December 27,

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	HOUSECRAFT - STEPHENS RES.
458217	T01	Hip Truss	1	1	I6243885
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

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ID:zD3kW0ImUat9g5WZtYszeky66i7-dj3S2pmQHv8mxOgLxeWiPyYXX70cmbJeUm_zjuy4pl

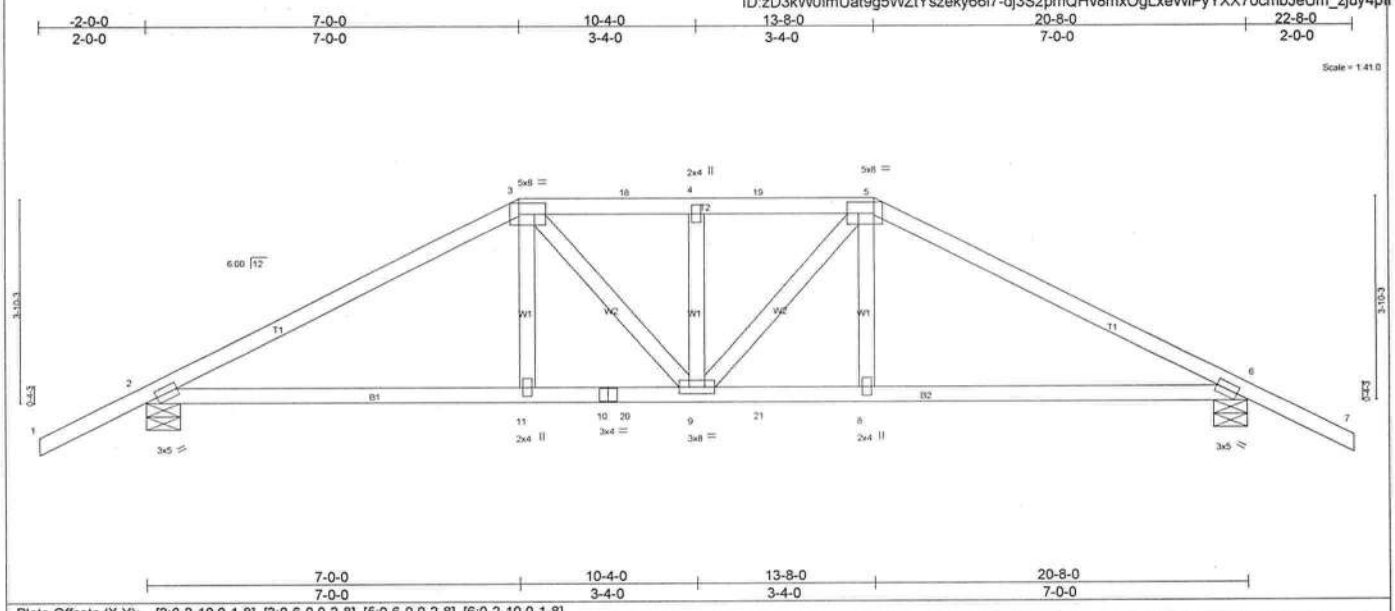


Plate Offsets (X,Y): [2:0-2-10,0-1-8], [3:0-6-0,0-2-8], [5:0-6-0,0-2-8], [6:0-2-10,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.64	Vert(LL)	0.10	9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.59	Vert(TL)	-0.18	9	>999	180		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.18	Horz(TL)	0.07	6	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 99 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-3-3 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-8-11 oc bracing.

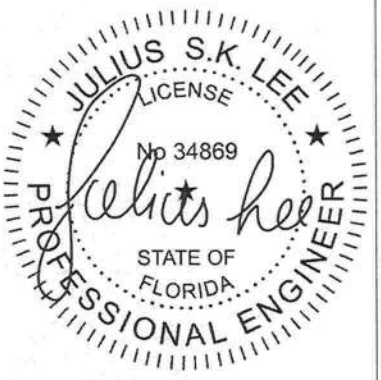
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=1082/0-7-10 (min. 0-1-8), 6=1107/0-7-10 (min. 0-1-9)
Max Horz 2=-56(LC 13)
Max Uplift2=-505(LC 8), 6=-545(LC 9)
Max Grav 2=1285(LC 2), 6=1315(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-2105/864, 3-18=-2054/919, 4-18=-2054/919, 4-19=-2054/919, 5-19=-2054/919, 5-6=-2170/952
BOT CHORD 2-11=-720/1801, 10-11=-723/1813, 10-20=-723/1813, 9-20=-723/1813, 9-21=-766/1871, 8-21=-766/1871, 6-8=-763/1859
WEBS 3-11=-76/414, 3-9=-248/436, 4-9=-394/244, 5-9=-128/351, 5-8=-75/414

NOTES (11-13)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 505 lb uplift at joint 2 and 545 lb uplift at joint 6.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 85 lb down and 92 lb up at 7-0-0, 85 lb down and 92 lb up at 9-0-12, 85 lb down and 92 lb up at 10-4-0, and 85 lb down and 92 lb up at 11-7-4, and 178 lb down and 236 lb up at 13-8-0 on top chord, and 240 lb down and 123 lb up at 7-0-0, 49 lb down at 9-0-12, 49 lb down at 10-4-0, and 49 lb down at 11-7-4, and 240 lb down and 123 lb up at 13-7-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
12) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
13) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25



Continued on page 2

December 27,2

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	HOUSECRAFT - STEPHENS RES.
458217	EJ01	Jack-Partial Truss	30	1	I6243883
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055 — 7.350 s Jul 31 2012 MiTek Industries, Inc. Thu Dec 27 09:06:12 2012 Page 1

ID:zD3kW0ImUat9g5WZtYszeky66i7-LN8oaQh1wlGmcJd?0gu3cTmMYJiFdSLctBn5zoy4pIP

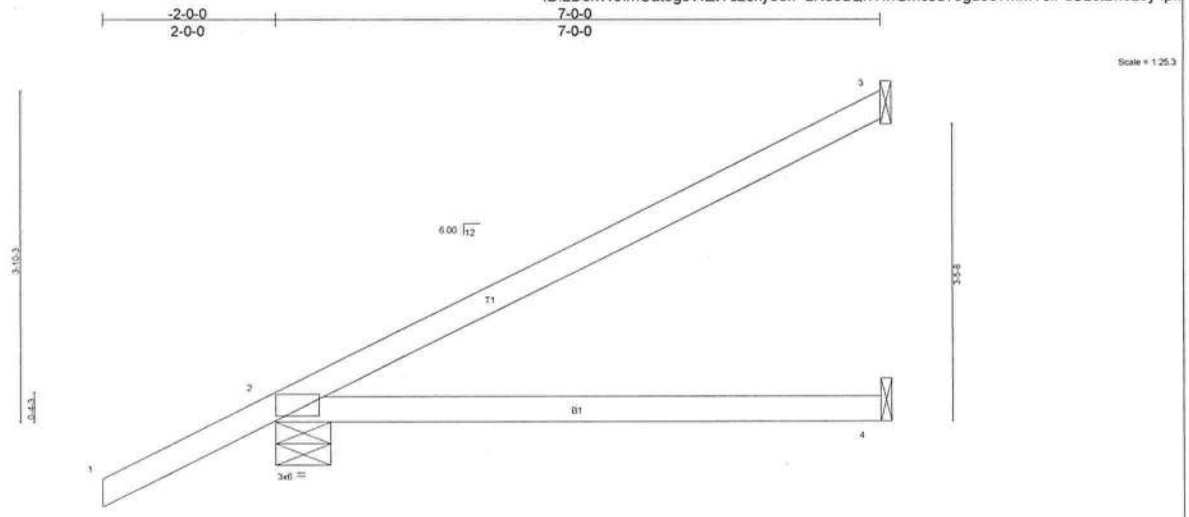


Plate Offsets (X,Y): [2:0-6-0,0-0-10]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.56	Vert(LL)	-0.06	4-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.32	Vert(TL)	-0.11	4-7	>745	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.01	2	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 26 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-1-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 3=113/Mechanical, 2=318/0-7-10 (min. 0-1-8), 4=32/Mechanical
Max Horz 2=144(LC 12)
Max Uplift3=-86(LC 12), 2=-97(LC 12)
Max Grav 3=139(LC 2), 2=380(LC 2), 4=79(LC 3)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1212/532
BOT CHORD 2-4=-1041/1713

NOTES (7-9)
1) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 86 lb uplift at joint 3 and 97 lb uplift at joint 2.
6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
8) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

December 27,2

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	HOUSECRAFT - STEPHENS RES.
458217	CJ02	Jack-Open Truss	10	1	I6243881
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

7.350 s Jul 31 2012 MiTek Industries, Inc. Thu Dec 27 09:06:08 2012 Page 1

ID:zD3kW0ImUat9g5WZtYszeky66i7-SbuHk2dWtWmK7iKEnqq7Sdbj_hNAheL0zZptq1y4pIT

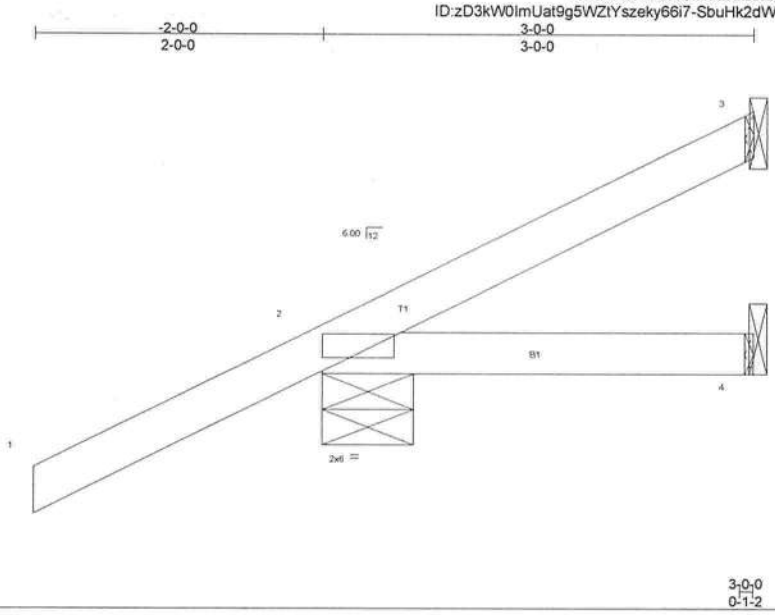


Plate Offsets (X,Y): [2:0-6-0,0-1-2]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	-0.01	4-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.13	Vert(TL)	-0.01	4-7	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	2	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 13 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 3=42/Mechanical, 2=193/0-7-10 (min. 0-1-8), 4=11/Mechanical
Max Horz 2=113(LC 12)
Max Uplift 3=-50(LC 12), 2=-121(LC 12)
Max Grav 3=52(LC 2), 2=233(LC 2), 4=33(LC 3)

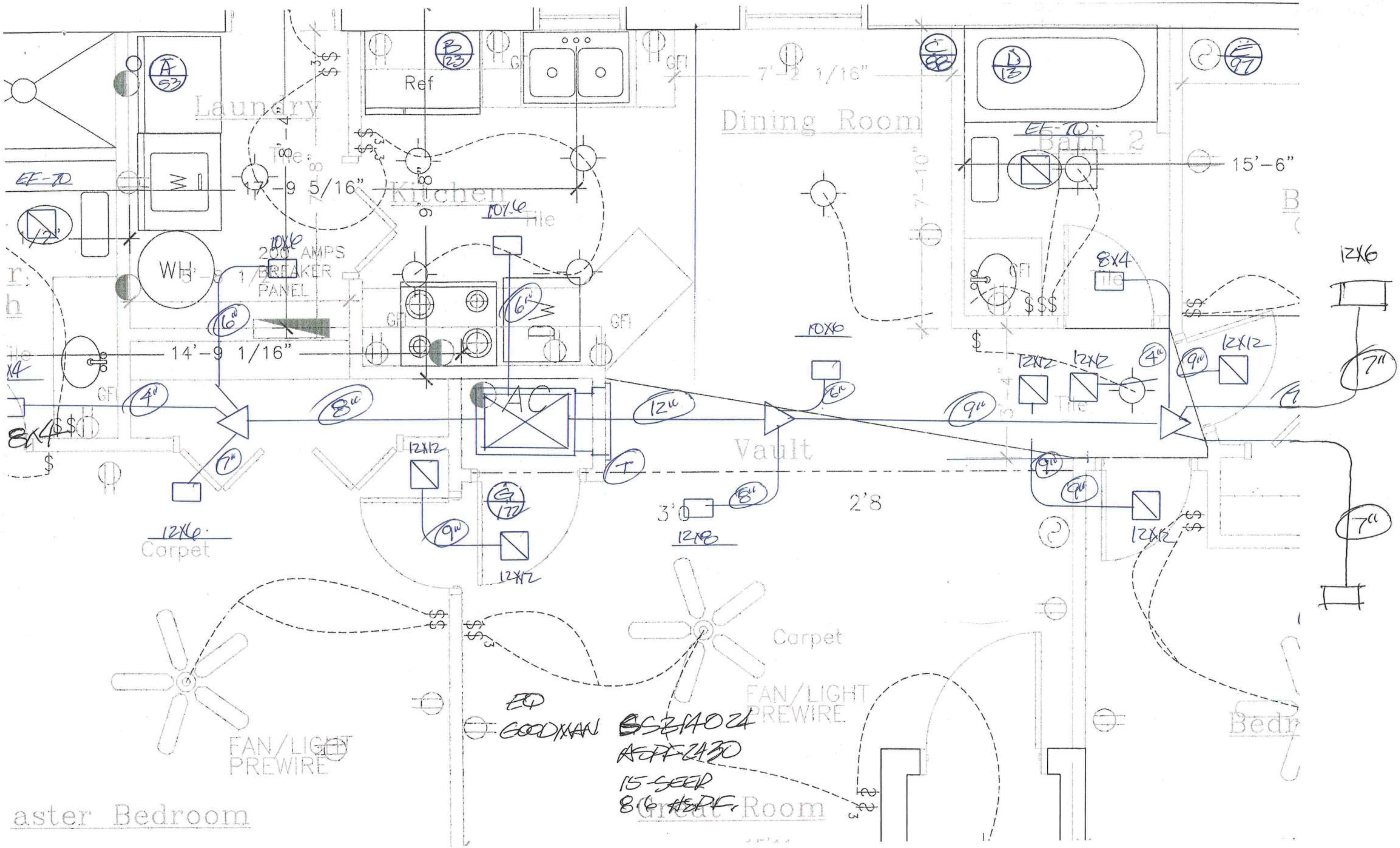
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-508/112
BOT CHORD 2-4=-106/642

NOTES (7-9)
1) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 50 lb uplift at joint 3 and 121 lb uplift at joint 2.
6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
8) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

December 27,

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435


Laundry
Ref
Dining Room
Bath 2
15'-6"
7'-2 1/16"
7'-10"
EF-70
17'-9 5/16"
Kitchen
WH
W
DW
200 AMPS
BREAKER
PANEL
14'-9 1/16"
GFI
AC
Vault
3'0
2'8
Carpet
FAN/LIGHT
PREWIRE
Master Bedroom
Great Room
Bedr
Tile
10X6
12X6
12X8
12X12
8X4
GOODMAN
15-SEER

Project: Housecraft - Wheary				Room	A			
Location: Gainesville Florida				Running ft wall	6			
Indoor Heating Db	70	Outdoor 99% db	42	Ceiling Height	8			
Indoor Cooling db	75	Outdoor 1% db	93	Gross Wall	48			
Indoor Cooling RH	55%	Grains Differance	39	Square Feet	48			
Latitude	28	Elevation	100	Cubic feet	384	0	0	0

	Type of Exposure		Construction Number	Panel Faces	HTM Htg	HTM Clg	Area	Htg	Clg	L-Clg
6A	Windows Glass Doors	a	1D-h Dbl low E	N	15.96	24		0	0	
		b	1D-h Dbl low E	E/W	15.96	73	0	0	0	
		c	1D-h Dbl low E	S	15.96	38		0	0	
		d						0	0	
		e						0	0	
6B	Skylights	a	8Ac-1 Metal singl		43.66	208		0	0	
		b	8Bc-1 Metal doubl		27.38	171		0	0	
7	Wood & Metal Doors	a	11-D Wood solid		14.43	12.09		0	0	
		b	11-J Metal fiber		22.2	18.6	21	466.2	390.6	
8	Above Grade Walls & Partitions		NET WALL				27			
		a	12C-Os R-13 frame		3.36	1.65		0	0	
		b	12E-Os r-19 frame		2.51	1.16		0	0	
		c	13A-5oc R-5 block		4.63	2.13	27	125.01	57.51	
		d						0	0	
9	Below Grade	a						0	0	
10	Ceilings		NET CEILINGS				48			
		a	16C-19 Vented attic		1.81	2.2		0	0	
		b	16C-30 Vented attic		1.19	1.44	48	57.12	69.12	
11	Floors	a	22A-ph slab no insul		1.358	0	6	8.148	0	
		b	20P-13 Garage craw		2.52	1.16		0	0	
12	Infiltration	a	5-A Semi tight A/C		26	14	21	546	294	0
		b						0	0	0
13	Internal loads	a	6A- Appliance load			1200		0	0	0
		b	Occupants	200	0	230	0	0	0	0
14	Subtotals							1202.48	811.23	0
15	Duct loads	a	7B-T Trunk branch	0	0.18	0.15		216.446	121.685	0
		b		0	0	0		0	0	0
16	Ventilation load			0	0	0		0	0	0
17	Winter Humid			0	0	0		0	0	0
18	Blower heat			0	0	0		0	0	0
19	Latent Migration			0	0	0		0	0	0
20	Total heating load		11271.57948					1418.92		
21	Total cooling sensible		13922.199						932.915	
22	Total latent load		2400							0
23	Room CFM heating							100.708		
24	Room CFM cooling								53.6073	

Builder's Air Of North Florida Inc.
5510 SW 41 Blvd. Gainesville, Florida 32608
352-373-3111, 352-373-3144
www.buildersair.com

Air Changes	619.2
Design CFM	800
Heating MTL	0.07097
Cooling MTL	0.05746

ACCA
MANUAL J
EIGHTH EDITION
VERSION TWO

B				C				D			
9				8				6			
12				12				8			
108				96				48			
81				99				66			
972	0	0	0	1188	0	0	0	528	0	0	0
Area	Htg	Clg	L-Clg	Area	Htg	Clg	L-Clg	Area	Htg	Clg	L-Clg
	0	0			0	0			0	0	
	0	0			0	0			0	0	
9	143.64	342		15	239.4	570			0	0	
	0	0			0	0			0	0	
	0	0			0	0			0	0	
	0	0			0	0			0	0	
	0	0			0	0			0	0	
	0	0			0	0			0	0	
99				81				48			
	0	0			0	0			0	0	
	0	0			0	0			0	0	
99	458.37	210.87		81	375.03	172.53		48	222.24	102.24	
	0	0			0	0			0	0	
	0	0			0	0			0	0	
81				99				66			
	0	0			0	0			0	0	
81	96.39	116.64		99	117.81	142.56		66	78.54	95.04	
9	12.222	0		8	10.864	0		6	8.148	0	
	0	0			0	0			0	0	
	0	0	0		0	0	0		0	0	0
	0	0	0		0	0	0		0	0	0
1	0	1200	0		0	0	0		0	0	0
0	0	0	0	2	0	460	400		0	0	0
	710.622	1869.51	0		743.104	1345.09	400		308.928	197.28	0
	127.912	280.427	0		133.759	201.764	0		55.607	29.592	0
	0	0	0		0	0	0		0	0	0
	0	0	0		0	0	0		0	0	0
	0	0	0		0	0	0		0	0	0
	0	0	0		0	0	0		0	0	0
	0	0	0		0	0	0		0	0	0
	838.534				876.863				364.535		
		2149.94				1546.85				226.872	
			0				400				0
	59.5149				62.2353				25.8729		
		123.54				88.8856				13.0366	

E				F				G			
20				29				21			
8				8				12			
160				232				252			
99				132				192			
792	0	0	0	1056	0	0	0	2304	0	0	0
Area	Htg	Clg	L-Clg	Area	Htg	Clg	L-Clg	Area	Htg	Clg	L-Clg
	0	0			0	0			0	0	
	0	0			0	0			0	0	
15	239.4	570		15	239.4	570		15	239.4	570	
	0	0			0	0			0	0	
	0	0			0	0			0	0	
0	0	0			0	0			0	0	
	0	0			0	0			0	0	
	0	0			0	0			0	0	
	0	0			0	0		21	466.2	390.6	
145				217				216			
	0	0			0	0			0	0	
	0	0			0	0			0	0	
145	671.35	308.85		217	1004.71	462.21		216	1000.08	460.08	
	0	0			0	0			0	0	
	0	0			0	0			0	0	
99				132				192			
	0	0			0	0			0	0	
99	117.81	142.56		132	157.08	190.08		192	228.48	276.48	
20	27.16	0		29	39.382	0		21	28.518	0	
	0	0			0	0			0	0	
	0	0	0		0	0	0		0	0	0
	0	0	0		0	0	0		0	0	0
	0	0	0		0	0	0		0	0	0
2	0	460	400	2	0	460	400	4	0	920	800
	1055.72	1481.41	400		1440.57	1682.29	400		1962.68	2617.16	800
	190.03	222.212	0		259.303	252.344	0		353.282	392.574	0
	0	0	0		0	0	0		0	0	0
	0	0	0		0	0	0		0	0	0
	0	0	0		0	0	0		0	0	0
	0	0	0		0	0	0		0	0	0
	0	0	0		0	0	0		0	0	0
	1245.75				1699.87				2315.96		
		1703.62				1934.63				3009.73	
			400				400				800
	88.417				120.649				164.375		
		97.8938				111.168				172.946	

H				I				J			
26				17							
8				8							
208				136				0			
192				66							
1536	0	0	0	528	0	0	0	0	0	0	0
Area	Htg	Clg	L-Clg	Area	Htg	Clg	L-Clg	Area	Htg	Clg	L-Clg
	0	0			0	0			0	0	
	0	0			0	0			0	0	
15	239.4	570			0	0			0	0	
	0	0			0	0			0	0	
	0	0			0	0			0	0	
	0	0			0	0			0	0	
	0	0			0	0			0	0	
	0	0			0	0			0	0	
	0	0			0	0			0	0	
193				136				0			
	0	0			0	0			0	0	
	0	0			0	0			0	0	
193	893.59	411.09		136	629.68	289.68			0	0	
	0	0			0	0			0	0	
	0	0			0	0			0	0	
192				66				0			
	0	0			0	0			0	0	
192	228.48	276.48		66	78.54	95.04			0	0	
26	35.308	0		17	23.086	0			0	0	
	0	0			0	0			0	0	
	0	0	0		0	0	0		0	0	0
	0	0	0		0	0	0		0	0	0
	0	0	0		0	0	0		0	0	0
2	0	460	400		0	0	0		0	0	0
	1396.78	1717.57	400		731.306	384.72	0		0	0	0
	251.42	257.636	0		131.635	57.708	0		0	0	0
	0	0	0		0	0	0		0	0	0
	0	0	0		0	0	0		0	0	0
	0	0	0		0	0	0		0	0	0
	0	0	0		0	0	0		0	0	0
	0	0	0		0	0	0		0	0	0
	1648.2				862.941				0		
		1975.21				442.428				0	
			400				0				0
	116.981				61.2472				0		
		113.5				25.4229				0	

Project: Housecraft - Wheary				Room		A			
Location: Gainesville Florida				Running ft wall		6			
Indoor Heating Db	70	Outdoor 99% db	42	Ceiling Height		8			
Indoor Cooling db	75	Outdoor 1% db	93	Gross Wall		48			
Indoor Cooling RH	55%	Grains Differance	39	Square Feet		48			
Latitude	28	Elevation	100	Cubic feet		384	0	0	0

	Type of Exposure		Construction Number	Panel Faces	HTM Htg	HTM Clg	Area	Htg	Clg	L-Clg
6A	Windows Glass Doors	a	1D-h Dbl low E	N	15.96	24		0	0	
		b	1D-h Dbl low E	E/W	15.96	73	0	0	0	
		c	1D-h Dbl low E	S	15.96	38		0	0	
		d						0	0	
		e						0	0	
6B	Skylights	a	8Ac-1 Metal singl		43.66	208		0	0	
		b	8Bc-1 Metal doubl		27.38	171		0	0	
7	Wood & Metal Doors	a	11-D Wood solid		14.43	12.09		0	0	
		b	11-J Metal fiber		22.2	18.6	21	466.2	390.6	
8	Above Grade Walls & Partitions		NET WALL				27			
		a	12C-Os R-13 frame		3.36	1.65		0	0	
		b	12E-Os r-19 frame		2.51	1.16		0	0	
		c	13A-5oc R-5 block		4.63	2.13	27	125.01	57.51	
		d						0	0	
9	Below Grade	a						0	0	
10	Ceilings		NET CEILINGS				48			
		a	16C-19 Vented attic		1.81	2.2		0	0	
		b	16C-30 Vented attic		1.19	1.44	48	57.12	69.12	
11	Floors	a	22A-ph slab no insul		1.358	0	6	8.148	0	
		b	20P-13 Garage craw		2.52	1.16		0	0	
12	Infiltration	a	5-A Semi tight A/C		26	14	21	546	294	0
		b						0	0	0
13	Internal loads	a	6A- Appliance load			1200		0	0	0
		b	Occupants	200	0	230	0	0	0	0
14	Subtotals							1202.48	811.23	0
15	Duct loads	a	7B-T Trunk branch	0	0.18	0.15		216.446	121.685	0
		b		0	0	0		0	0	0
16	Ventilation load			0	0	0		0	0	0
17	Winter Humid			0	0	0		0	0	0
18	Blower heat			0	0	0		0	0	0
19	Latent Migration			0	0	0		0	0	0
20	Total heating load		11271.57948					1418.92		
21	Total cooling sensible		13922.199						932.915	
22	Total latent load		2400							0
23	Room CFM heating							100.708		
24	Room CFM cooling								53.6073	

Air Changes	619.2
Design CFM	800
Heating MTL	0.07097
Cooling MTL	0.05746

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352-373-3111, 352-373-3144
www.buildersair.com

ACCA
MANUAL J
EIGHTH EDITION
VERSION TWO

B				C				D			
9				8				6			
12				12				8			
108				96				48			
81				99				66			
972	0	0	0	1188	0	0	0	528	0	0	0
Area	**Htg**	**Clg**	**L-Clg**	**Area**	**Htg**	**Clg**	**L-Clg**	**Area**	**Htg**	**Clg**	**L-Clg**
	0	0			0	0			0	0	
	0	0			0	0			0	0	
9	143.64	342		15	239.4	570			0	0	
	0	0			0	0			0	0	
	0	0			0	0			0	0	
	0	0			0	0			0	0	
	0	0			0	0			0	0	
	0	0			0	0			0	0	
	0	0			0	0			0	0	
99				81				48			
	0	0			0	0			0	0	
	0	0			0	0			0	0	
99	458.37	210.87		81	375.03	172.53		48	222.24	102.24	
	0	0			0	0			0	0	
	0	0			0	0			0	0	
81				99				66			
	0	0			0	0			0	0	
81	96.39	116.64		99	117.81	142.56		66	78.54	95.04	
9	12.222	0		8	10.864	0		6	8.148	0	
	0	0			0	0			0	0	
	0	0	0		0	0	0		0	0	0
	0	0	0		0	0	0		0	0	0
1	0	1200	0		0	0	0		0	0	0
0	0	0	0	2	0	460	400		0	0	0
	710.622	1869.51	0		743.104	1345.09	400		308.928	197.28	0
	127.912	280.427	0		133.759	201.764	0		55.607	29.592	0
	0	0	0		0	0	0		0	0	0
	0	0	0		0	0	0		0	0	0
	0	0	0		0	0	0		0	0	0
	0	0	0		0	0	0		0	0	0
	0	0	0		0	0	0		0	0	0
	838.534				876.863				364.535		
		2149.94				1546.85				226.872	
			0				400				0
	59.5149				62.2353				25.8729		
		123.54				88.8856				13.0366	

E				F				G			
20				29				21			
8				8				12			
160				232				252			
99				132				192			
792	0	0	0	1056	0	0	0	2304	0	0	0
Area	**Htg**	**Clg**	**L-Clg**	**Area**	**Htg**	**Clg**	**L-Clg**	**Area**	**Htg**	**Clg**	**L-Clg**
	0	0			0	0			0	0	
	0	0			0	0			0	0	
15	239.4	570		15	239.4	570		15	239.4	570	
	0	0			0	0			0	0	
	0	0			0	0			0	0	
0	0	0			0	0			0	0	
	0	0			0	0			0	0	
	0	0			0	0			0	0	
	0	0			0	0		21	466.2	390.6	
145				217				216			
	0	0			0	0			0	0	
	0	0			0	0			0	0	
145	671.35	308.85		217	1004.71	462.21		216	1000.08	460.08	
	0	0			0	0			0	0	
	0	0			0	0			0	0	
99				132				192			
	0	0			0	0			0	0	
99	117.81	142.56		132	157.08	190.08		192	228.48	276.48	
20	27.16	0		29	39.382	0		21	28.518	0	
	0	0			0	0			0	0	
	0	0	0		0	0	0		0	0	0
	0	0	0		0	0	0		0	0	0
	0	0	0		0	0	0		0	0	0
2	0	460	400	2	0	460	400	4	0	920	800
	1055.72	1481.41	400		1440.57	1682.29	400		1962.68	2617.16	800
	190.03	222.212	0		259.303	252.344	0		353.282	392.574	0
	0	0	0		0	0	0		0	0	0
	0	0	0		0	0	0		0	0	0
	0	0	0		0	0	0		0	0	0
	0	0	0		0	0	0		0	0	0
	0	0	0		0	0	0		0	0	0
	1245.75				1699.87				2315.96		
		1703.62				1934.63				3009.73	
			400				400				800
	88.417				120.649				164.375		
		97.8938				111.168				172.946	

H				I				J			
26				17							
8				8							
208				136				0			
192				66							
1536	0	0	0	528	0	0	0	0	0	0	0
Area	Htg	Clg	L-Clg	Area	Htg	Clg	L-Clg	Area	Htg	Clg	L-Clg
	0	0			0	0			0	0	
	0	0			0	0			0	0	
15	239.4	570			0	0			0	0	
	0	0			0	0			0	0	
	0	0			0	0			0	0	
	0	0			0	0			0	0	
	0	0			0	0			0	0	
	0	0			0	0			0	0	
	0	0			0	0			0	0	
193				136				0			
	0	0			0	0			0	0	
	0	0			0	0			0	0	
193	893.59	411.09		136	629.68	289.68			0	0	
	0	0			0	0			0	0	
	0	0			0	0			0	0	
192				66				0			
	0	0			0	0			0	0	
192	228.48	276.48		66	78.54	95.04			0	0	
26	35.308	0		17	23.086	0			0	0	
	0	0			0	0			0	0	
	0	0	0		0	0	0		0	0	0
	0	0	0		0	0	0		0	0	0
	0	0	0		0	0	0		0	0	0
2	0	460	400		0	0	0		0	0	0
	1396.78	1717.57	400		731.306	384.72	0		0	0	0
	251.42	257.636	0		131.635	57.708	0		0	0	0
	0	0	0		0	0	0		0	0	0
	0	0	0		0	0	0		0	0	0
	0	0	0		0	0	0		0	0	0
	0	0	0		0	0	0		0	0	0
	0	0	0		0	0	0		0	0	0
	1648.2				862.941				0		
		1975.21				442.428				0	
			400				0				0
	116.981				61.2472				0		
		113.5				25.4229				0	