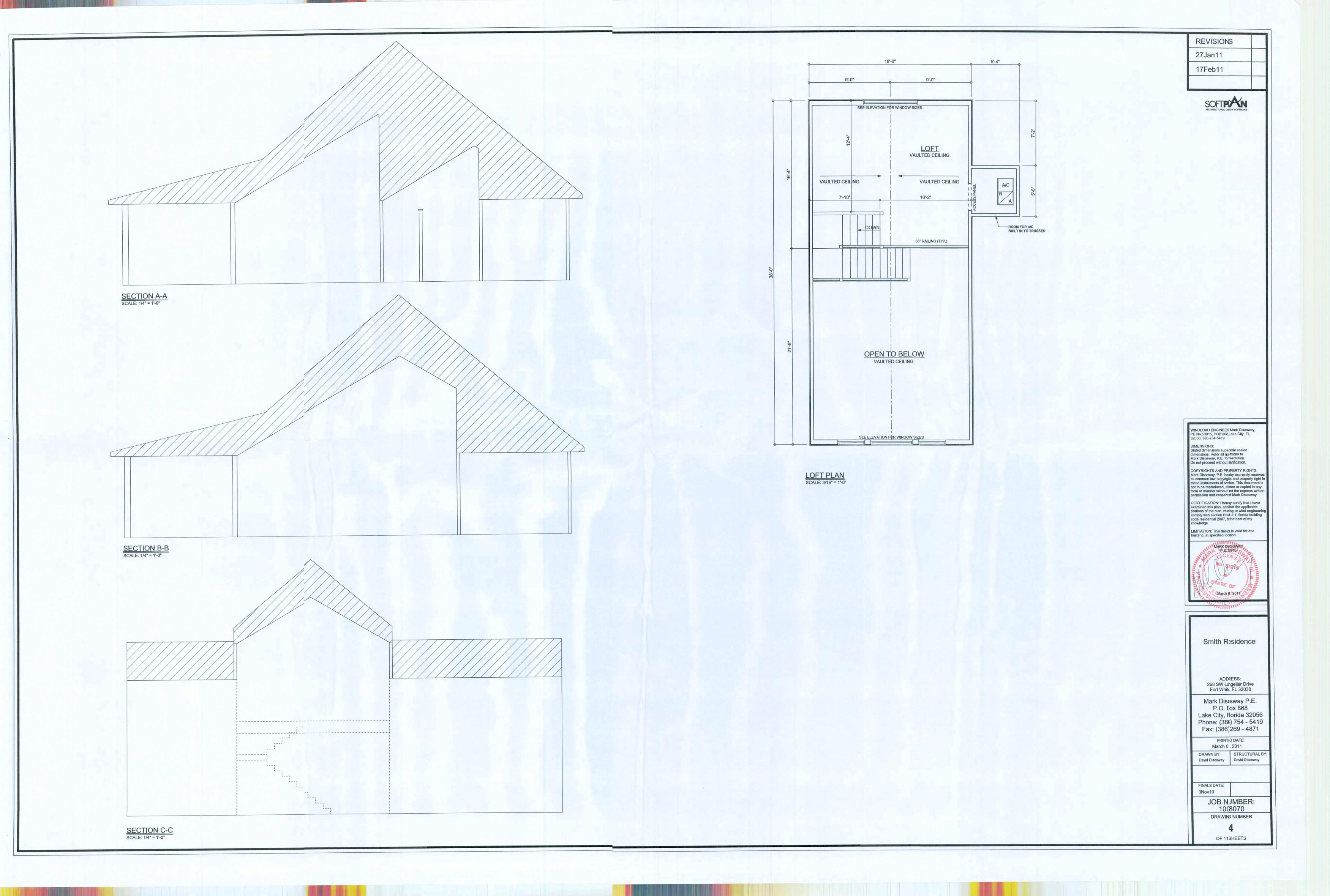
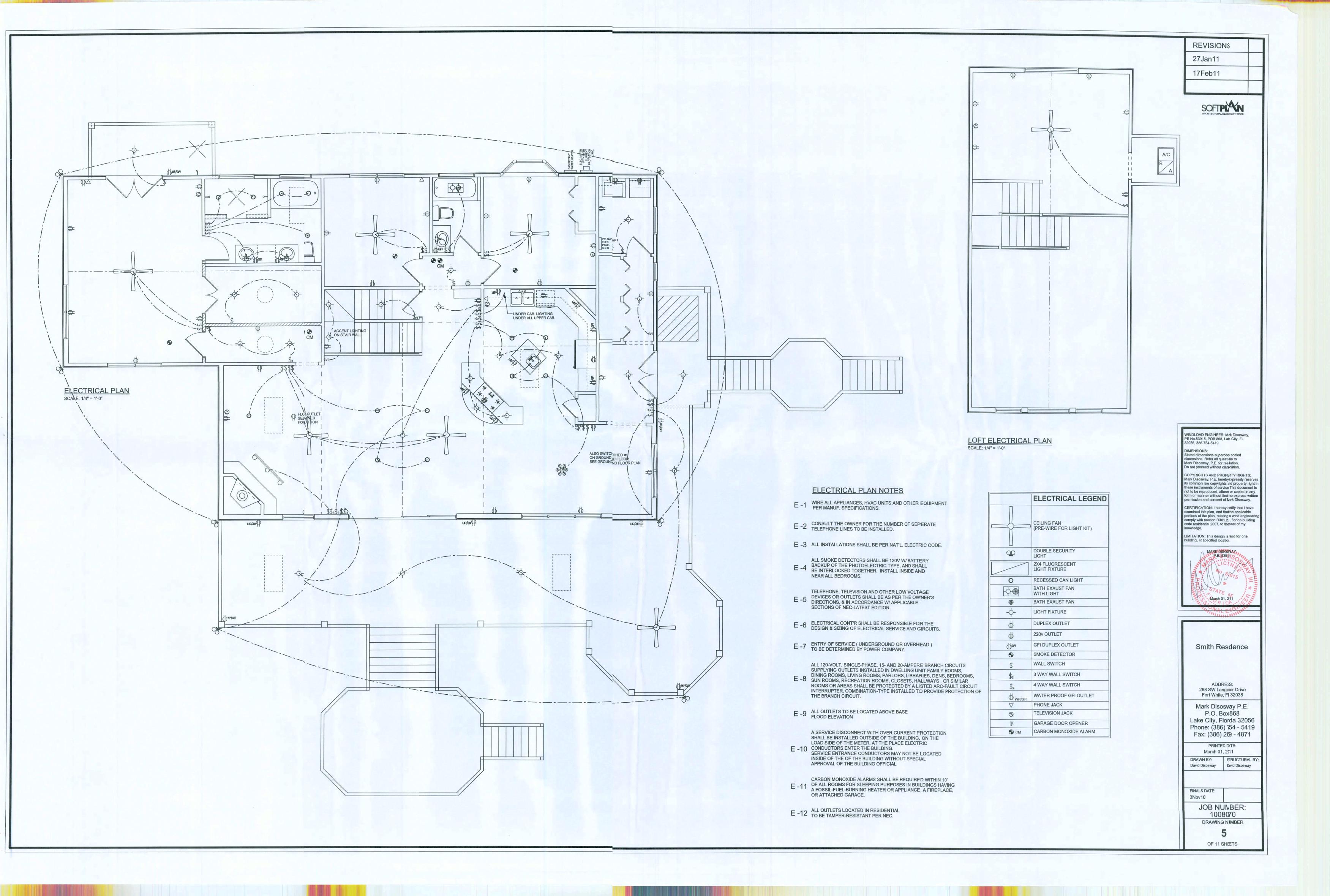


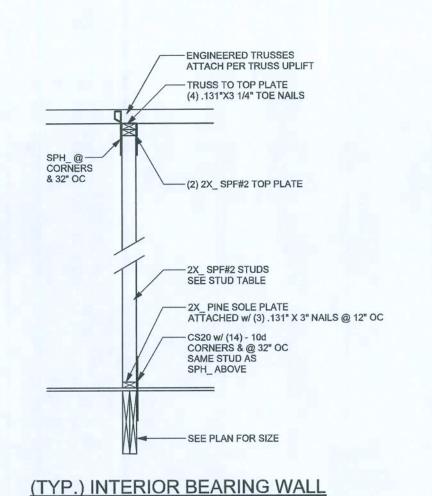
27Jan11 17Feb11 SOFTP AND ARCHITECTURAL DBIGN SOFTWAPE NOTE: ALL GROUND FLOOR CEILINGS TO BE BEAD BOARD SWITCH FOR MAIN FLOOD LIGHTS ON HOUSE WINDLOAD ENGINEER: /lark Disosway, PE No.53915, POB 868, lake City, FL 32056, 386-754-5419 DIMENSIONS:
Stated dimensions supercide scaled dimensions. Refer all questions to Mark Disosway, P.E. for risolution. Do not proceed without clirification. GROUNEFLOOR PLAN w/ ELECTRICAL

SCALE: 1/4" = 0" COPYRIGHTS AND PROPERTY RIGHTS:
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form or manner without firt the express written
permission and consent o Mark Disosway. CERTIFICATION: I hereb certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R3012.1, florida building code residential 2007, to be best of my knowledge. LIMITATION: This designs valid for one building, at specified locabn. MARK DISKSWAY P.E. 53115 No 53915 March 012011 Smith Residence ADDRISS: 268 SW Lançelier Drive Fort White, ²L 32038 Mark Disosway P.E. P.O. Box 868 Lake City, Fkrida 32056 Phone: (386)754 - 5419 Fax: (386) 269 - 4871 PRINTEDDATE: March 01, !011 DRAWN BY: STRUCTURAL BY:
David Disosway David Disosway FINALS DATE: 3Nov10 JOB NUMBER: 1008)70 DRAWING NUMBER 3.1 OF 11 SHEETS

REVISIONS







-2X4 OUTRIGGER @ 24" O.C.

BLOCKING REQUIRED BETWEEN OUT RIGGERS

-(4) .131"X3 1/4"

DIAGONAL BRACE MUST

FOR LENGTHOVER 12' IT

MAY BE "T" BRACED UP

TO 12' AND UNBRACED

-2X4X8' RAT RUN NAIL EACH

-(4) .131"X3 1/4" NAILS

-(8) .131"X3 1/4" NAILS

- 2X4 SPF#2 BLOCKING

- H3 INSTALLED HORIZONTALLY

CONNECTION w/ (4) .131"X3 1/4" NAILS

BE NAILEDTO TRUSS WEBS

/-(4) .131"X3 1/4" -

- 7/16" OSB UNBLOCKED

(4) .131"X3 1/4"—

INSTALL 2X4 SPF#2 DIAGONAL BRACE — AND NAIL TO BLOCKING AT TOP CHORD & BOTTOM CHORD AND RAT RUN @ 6' O.C.

SPACE RAT RUN & DIAGONAL BRACE 6'-0" O.C.

WOOD FRAME

FOR GABLE HEIGHT UP TO 25'-0" 110 MPH, EXP. C, ENCLOSED

(TYP.) GABLE BRACING DETAIL

EDGE & 12" O.C. FIELD

ATTACH RAT RUN TO-

(4) .131"X3 1/4" NAILS

TOE NAIL TRUSS ----

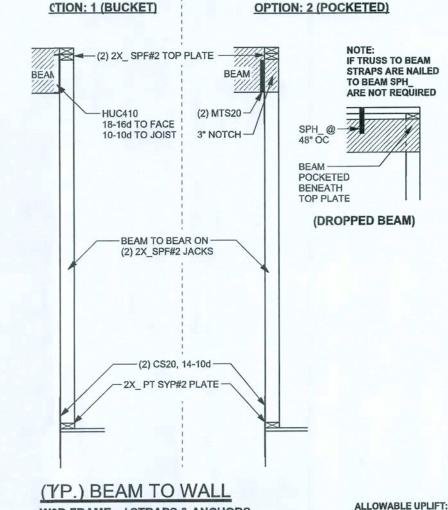
w/ (8) -16d TO TRUSS

& (8) -16d TO WALL

@ 48" O.C. U.N.O.

BLOCKING w/

12d @ 6" O.C



2X4 OUTLOOKERS @ 24" OC ---

H3 EACH OUTLOOKER

-ROOF SHEATHING

PLATE NAILED TO TRUSS

BOTTOM CHORD

w/ .131X3 1/4" @ 6" OC

EXTERIOR SHEATHING

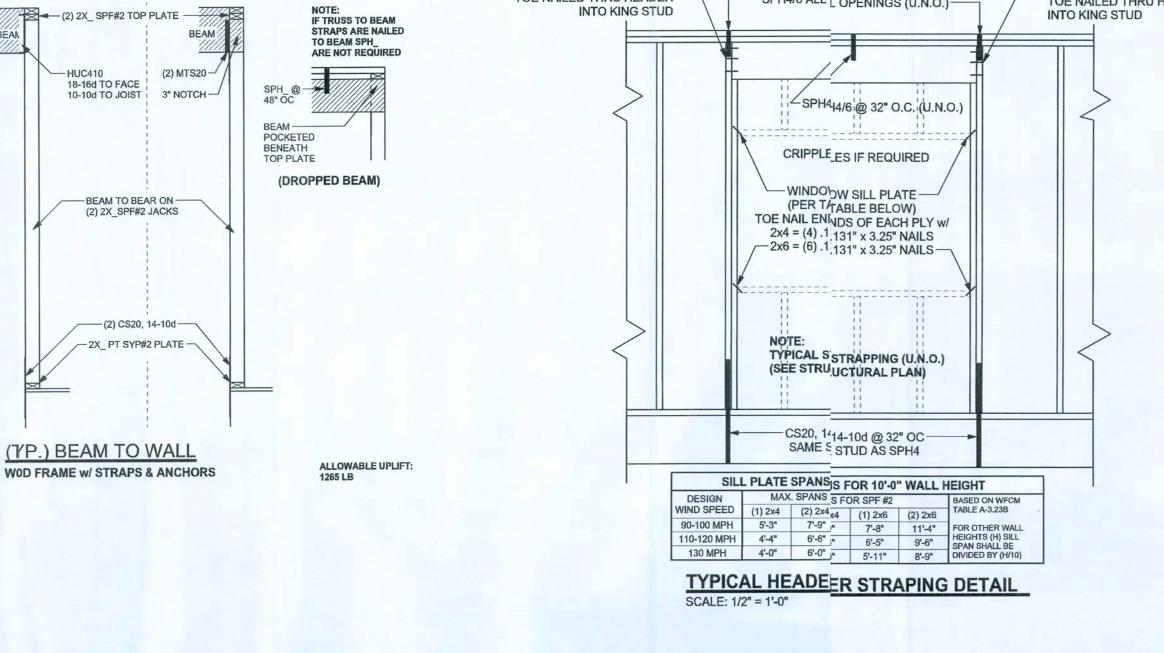
BETWEEN POINTS OF

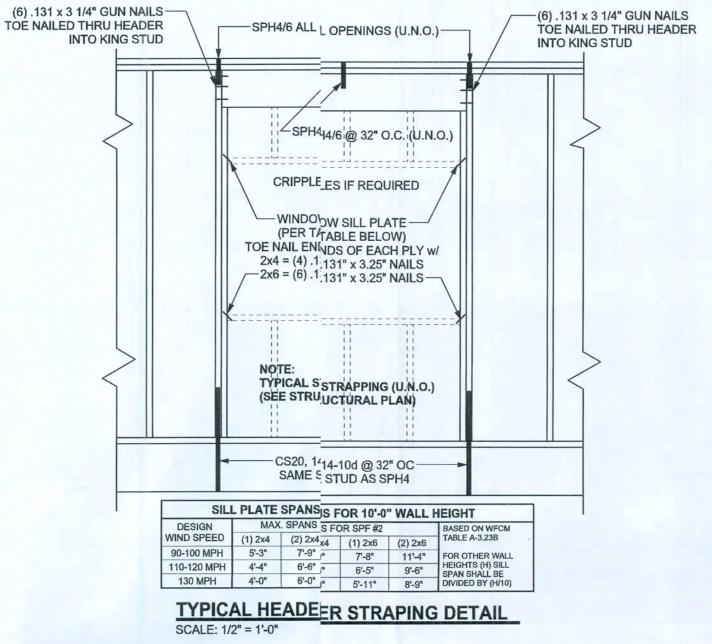
SEE STUD TABLE

(T'P.) GABLE WALL w/ VAULTED CEILING

STUDS MUST BE CONTINUOUS

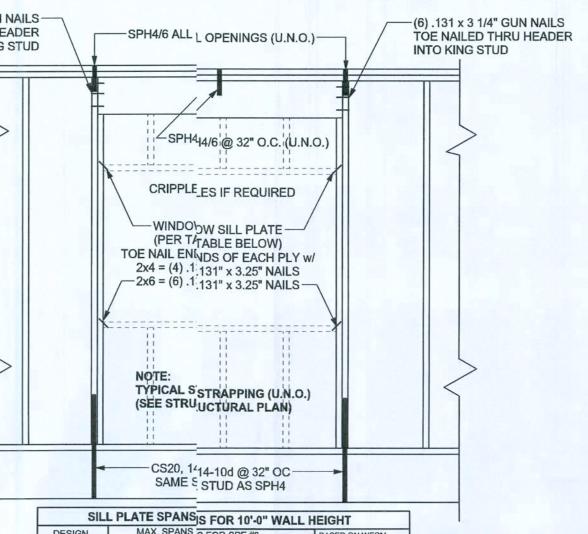
ATTACH TO TRUSS w/ (4) .131"X3 1/4" TOE NAILS





IF TRUSS TO WALL STRAPS ARE NAAILED TO THE HEADER THE SPH4/6 @ 48" (O.C.

ARE NOT REQUIRED

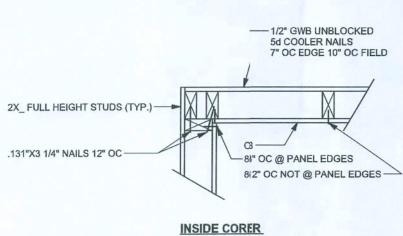


		Fb (psi)	E (10 ⁶ psi)
2x8	SYP#2	1200	1.6
2x10	SYP#2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	1600	1.9
PSL	PARALAM	2900	2.0

GRADE & SPECIES TABLE

1/2" GWB UNBLOCKED .131"X3 1/4" NAILS 12" OC -7" OC EDGE 10" OC FIELD -- 8d 3" OC @ PANEL EDGES 8d 12" OC NOT @ PANEL EDGES 2X_FULL HEIGHT STUDS (TYP.) -- EXTERIOR WALL 8d 3" OC @ PANEL EDGES -8d 12" OC NOT @ PANEL EDGES -8d 3" OC @ PANEL EDGES --8d 12" OC NOT @ PANEL EDGES .131"X3 1/4" NAILS 6" OC -**OUTSIDE CCNER** FOR SHEAR TRANSFER - 8d 3" OC @ PANEL EDGES 8d 12" OC NOT @ PANEL EDGES ---- 1/2" GWB UNBLOCKED 5d COOLER NAILS 7" OC EDGE 10" OC FIELD 2X_FULL HEIGHT STUDS (TYP.) INTERIOR SHEARWALL .131"X3 1/4" NAILS 12" OC ---1/2" GWB UNBLOCKED -2X_FULL HEIGHT STUDS (TYP.) -5d COOLER NAILS 7" OC EDGE 10" OC FIELD .131"X3 1/4" NAILS 12" OC --83" OC @ PANEL EDGES

(TYP.) INTERSECTING WALL FRAMING



(TYP.) CORNER FLAMING WOOD FRAME

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 11'-9" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 18'-10' STUD HEIGHT
(1) 2x6 @ 12" OC	TO 20'-0" STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16* O.C. x 0.85 = 13.6* O.C.

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS

PLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS	
< 420	< 245	H5A	3-8d	3-8d		
< 455	< 265	H5	4-8d	4-8d	The state of the s	
< 360	< 235	H4	4-8d	4-8d		
< 455	< 320	H3	4-8d	4-8d		
< 415	< 365	H2.5	5-8d	5-8d		
< 600	< 535	H2.5A	5-8d	5-8d		
< 950	< 820	H6	8-8d	8-8d		
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"		
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"		
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"		
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"		
< 760	< 655	H10-2	6-10d	6-10d		
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"		
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"		
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"		
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"		
< 2900	< 2490	2 - HTS24	1784	700 1 112		
< 2050	< 1785	LGT2	14 -16d	14 -16d		
		HEAVY GIRDER TIEDOWNS*		14 100	TO FOUNDATION	
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED RO 12" EMBEDMENT	
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT	
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT	
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT	
		STUD STRAP CONNECTOR*			TO STUDS	
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d	
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d	100	4 -10d	
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d	
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d	M2 = 3	8 -10d	
< 885	< 760	SP4		Supplied to	6-10d, 1 1/2"	
< 1240	< 1065	SPH4			10-10d, 1 1/2"	
< 885	< 760	SP6			6-10d, 1 1/2"	
< 1240	< 1065	SPH6		Mape Jensen	10-10d, 1 1/2"	
< 1235	< 1165	LSTA18	14-10d	No. 17 com		
< 1235	< 1235	LSTA21	16-10d			
< 1030	< 1030	CS20	18-8d			
< 1705	< 1705	CS16	28-8d			
		STUD ANCHORS*	TO STUDS		TO FOUNDATION	
< 1350	< 1305	LTT19	8-16d	TAIN TOUR	1/2" AB	
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB	
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8"AB	
< 4175	< 3695	HTT16	18 - 16d		5/8" AB	
< 1400	< 1400	PAHD42	16-16d		3/0 AB	
< 3335	< 3335	HPAHD22	000000000			
			16-16d		4/01/45	
< 2200	< 2200	ABU44	12-16d		1/2" AB	
< 2300	< 2300	ABU66	12-16d		1/2" AB	
< 2320	< 2320	ABU88	18 - 16d		2-5/8" AB	

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2007. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR
ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE
TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI. WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL.
(RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNG

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK. CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE. PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2007 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES. PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU

BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY. VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL

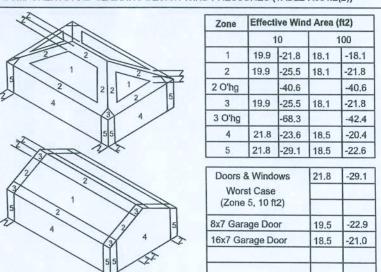
ROOF SYSTEM DESIGN

BEARING LOCATIONS.

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2007, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2007 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING, THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED

DESIGN DATA

WIND LOADS PER FLORIDA BUILDING CODE 2007 RESIDENTIAL, SECTION R301.2.1 (ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT: NOT ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS. BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION 1.) BASIC WIND SPEED = 110 MPH WIND EXPOSURE = B .) WIND IMPORTANCE FACTOR = 1.0 4.) BUILDING CATEGORY = II 5.) ROOF ANGLE = 10-45 DEGREES .) MEAN ROOF HEIGHT = <30 FT .) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING) 8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))



	4 1	16x7 Garage Door	18.5	-21.0
	555 27			
DESIGN	LOADS			
FLOOR	40 PSF (ALL OTHER DWELLING ROOM	S)		
	30 PSF (SLEEPING ROOMS)			
	30 PSF (ATTICS WITH STORAGE)			A.
	10 PSF (ATTICS WITHOUT STORAGE,	<3:12)		
ROOF	20 PSF (FLAT OR <4:12)			789
	16 PSF (4:12 TO <12:12)			
40.7	12 PSF (12:12 AND GREATER)			
STAIRS	40 PSF (ONE & TWO FAMILY DWELLING	SS)		
SOIL BE	ARING CAPACITY 1000PSF			
NOT IN	FLOOD ZONE (BUILDER TO VERIFY)			

27Jan11 17Feb11

SOFTPLAN

REVISIONS

VINDLOAD ENGINEER: Marl Disosway, PE No.53915, POB 868, LakeCity, FL 32056, 386-754-5419 DIMENSIONS:

Stated dimensions supercede caled

Mark Disosway, P.E. for resolution. Do not proceed without clarification.

nsions. Refer all questions to

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xamined this plan, and that the applicable portions of the plan, relating towind engineeri comply with section R301.2.1,florida building code residential 2007, to the bist of my

LIMITATION: This design is vaid for one building, at specified location.



Smith Residence

ADDRESS: 268 SW Langelier Drive Fort White, FL 32038

Mark Disosway P.E. P.O. Box \$68 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

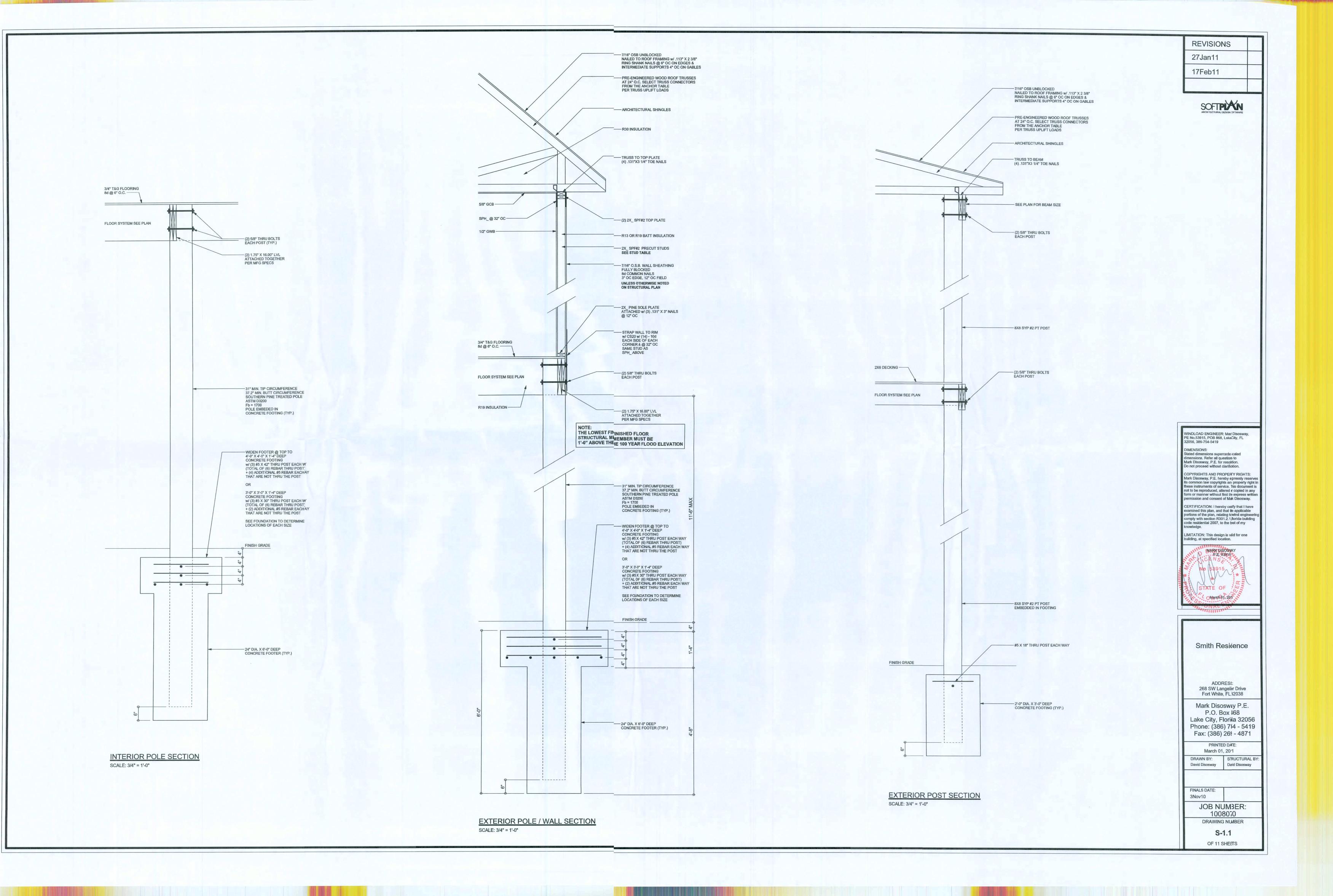
PRINTED DATE:

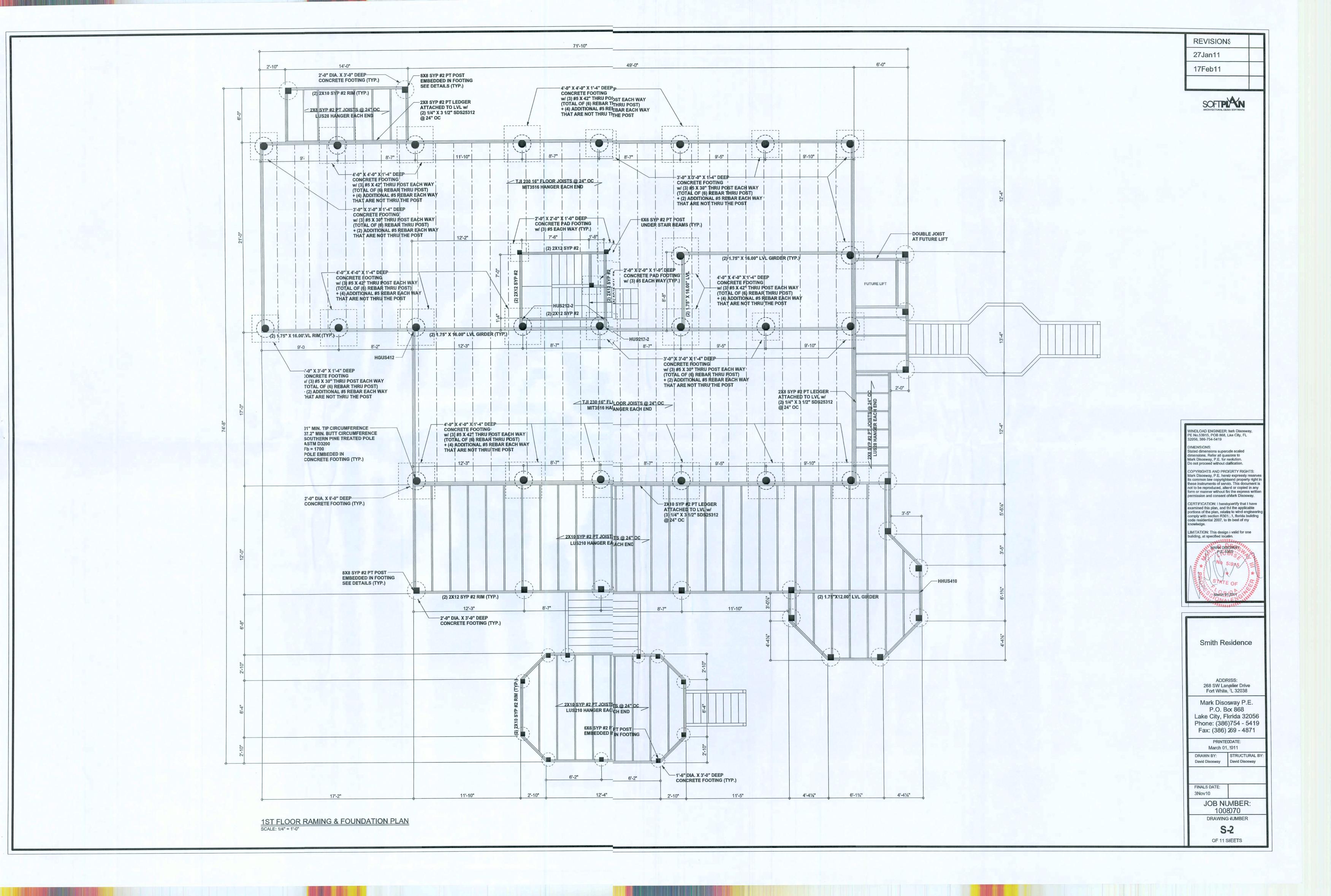
March 01, 2011 DRAWN BY: STRUCTURAL BY: David Disosway Daid Disosway

FINALS DATE: 3Nov10

> JOB NUMBER: 1008070 DRAWING NUMBER

5-1 OF 11 SHEETS





REVISIONS
27Jan11
17Feb11

SOFTPLAN

STRUCTURAL PLAN NOTES

SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP #2 (U.N.O.)

SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)

SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

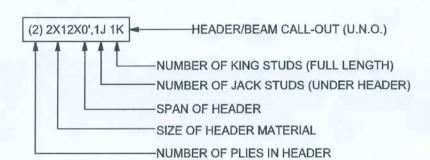
PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS.

LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

WALL LEGEND

	EXTERIOR WALL
	INTERIOR NON-LOAD BEARING WALL
	INTERIOR LOAD BEARING WALL w/ NO UPLIF
	INTERIOR LOAD BEARING WALL w/ UPLIFT

HEADER LEGEND

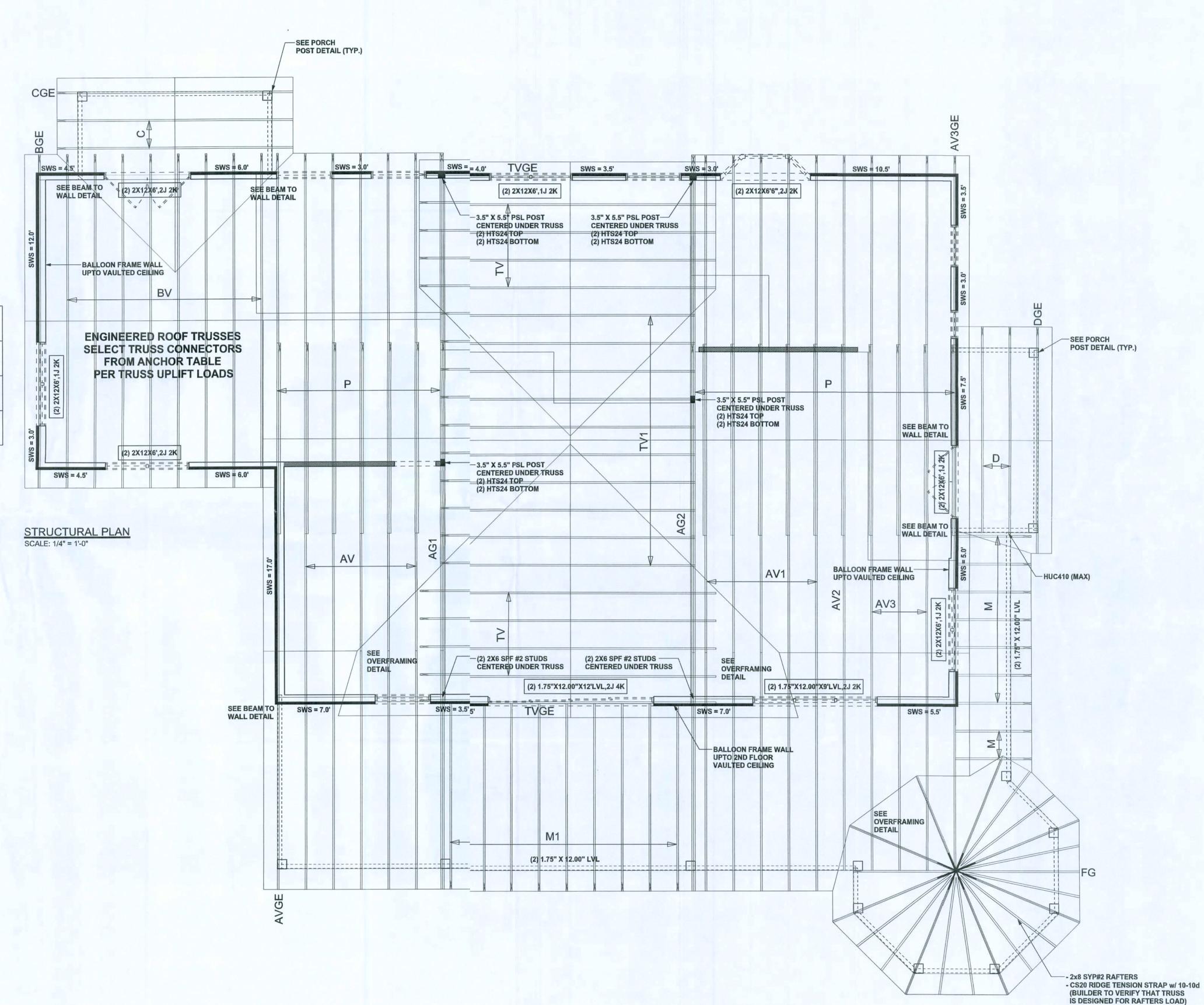


TOTAL SHEAR WALL SEGMENTS

REQUIRED ACTUAL
TRANSVERSE 45.0' 45.0'

68.0

LONGITUDINAL 38.2'



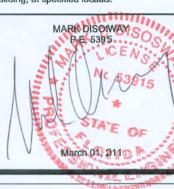
WINDLOAD ENGINEER: Mrk Disosway, PE No.53915, POB 868, Lae City, FL 32056, 386-754-5419

Stated dimensions supercee scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarication.

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permission and consent of flark Disosway.

CERTIFICATION: I hereby ertify that I have examined this plan, and that he applicable portions of the plan, relatingto wind engineerin comply with section R301.21, florida building code residential 2007, to the best of my knowledge.

LIMITATION: This design isvalid for one building, at specified locatio.



Smith Residence

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Mark Disosvay P.E. P.O. Box 868 Lake City, Floida 32056 Phone: (386) '54 - 5419 Fax: (386) 269 - 4871

PRINTED IATE: March 01, 2)11

DRAWN BY: STRUCTURAL BY:
David Disosway lavid Disosway

FINALS DATE: 3Nov10

CONNECTIONS, WALL, & HEADER DESIGN IS BASED

JOB #10-237

ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. ANDERSON TRUSS CO.

JOB NUMBER: 1008(70 DRAWING NUMBER

> S-3 OF 11 SHIETS

REVISIONS 27Jan11 17Feb11 SOFTPIAN 1 1/4" X 16" LSL RIM — (4) KING STUDS CONTINUOUS — TO VAULTED CEILING BALLOON FRAME WALL UPTO VAULTED CEILING TJI 210 16" FLOOR JOISTS @ 24" OC IUS2.06/16 HANGER EACH END 1 1/4" X 16" LSL RIM LOFT FLOOR FRAMING PLAN SCALE: 1/4" = 1'-0" WINDLOAD ENGINEER: Mar. Disosway, PE No.53915, POB 868, LakeCity, FL 32056, 386-754-5419 DIMENSIONS: Stated dimensions supercedescaled dimensions. Refer all questiors to Mark Disosway, P.E. for resoltion. Do not proceed without clarifiation. COPYRIGHTS AND PROPERTY RIGHTS:
Mark Disosway, P.E. hereby expressly reserves its common law copyrights and property right in these instruments of service. This document is not to be reproduced, altered it copied in any form or manner without first the express written permission and consent of Mak Disosway. CERTIFICATION: I hereby ceilfy that I have examined this plan, and that the applicable portions of the plan, relating trwind engineering comply with section R301.2.1 florida building code residential 2007, to the last of my knowledge. LIMITATION: This design is vlid for one building, at specified location. MARK DISOSVAY — (4) KING STUDS CONTINUOUS — FROM 1ST FLOOR TO VAULTED CEILING BALLOON FRAME WALL **UPTO VAULTED CEILING** March 01, 201 — (4) KING STUDS CONTINUOUS — TO VAULTED CEILING Smith Residence LOFT STRUCTURAL PLAN
SCALE: 1/4" = 1'-0" ADDRES: 268 SW Langelèr Drive Fort White, FL32038 Mark Disosway P.E. P.O. Box 368 Lake City, Florila 32056 Phone: (386) 7¼ - 5419 Fax: (386) 26! - 4871 PRINTED DATE: March 01, 201 DRAWN BY: STRUCTURAL BY: David Disosway Daid Disosway FINALS DATE: 3Nov10 JOB NUMBER: 1008070 DRAWING NUMBER OF 11 SHEETS



BUILDER MUST VERIFY THE FOLLOWING MINIMUM NET FREE VENT AREAS:

REVISIONS

SOFT**PAN**

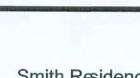
WINDLOAD ENGINEER Mark Disosway, PE No.53915, POB 868, ake City, FL 32056, 386-754-5419

DIMENSIONS: Stated dimensions dimensions. Refer all questions to Mark Disosway, P.E. for esolution. Do not proceed without carification.

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permission and consent f Mark Disosway.

CERTIFICATION: I herely certify that I have examined this plan, and 1at the applicable portions of the plan, relang to wind engineering comply with section R30.2.1, florida building code residential 2007, tohe best of my knowledge.

LIMITATION: This design is valid for one building, at specified location. MARK DISSWAY



Smith Residence

ADDRESS: 268 SW Langelier Drive Fort White,FL 32038

Mark Disosway P.E. P.O. Box 868 Lake City, Fbrida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

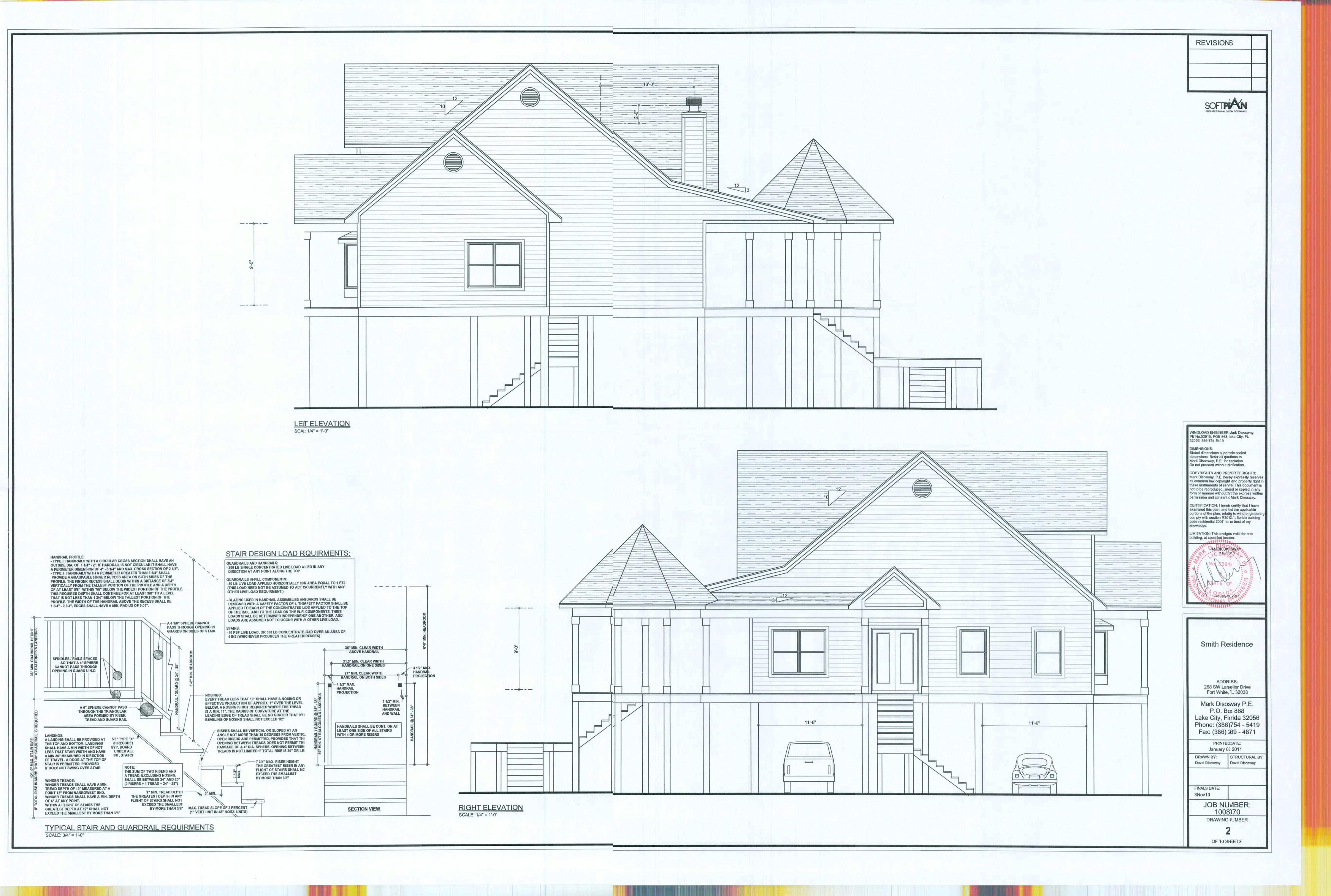
PRINTELDATE: January 0i, 2011

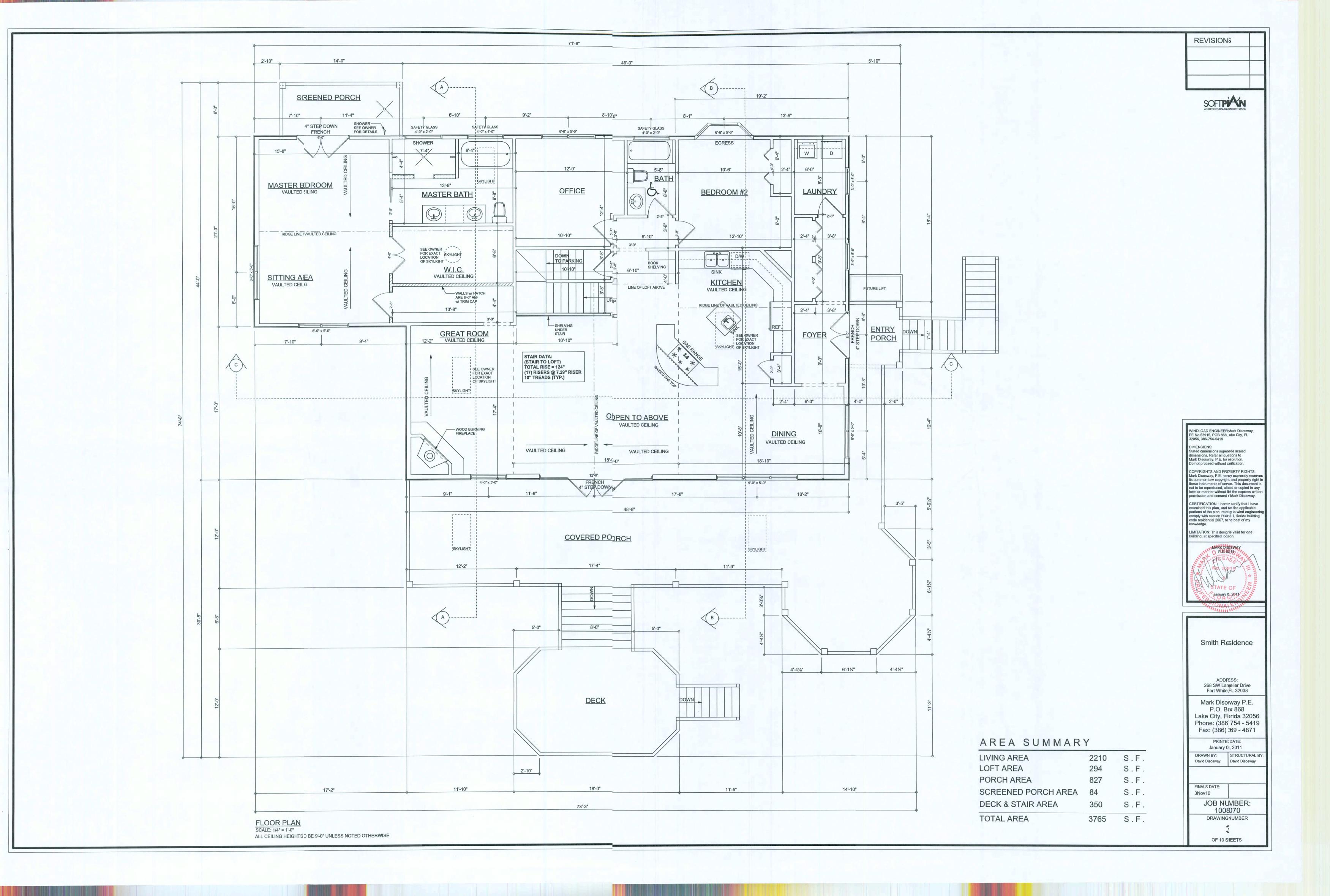
DRAWN BY: STRUCTURAL BY:
David Disosway David Disosway

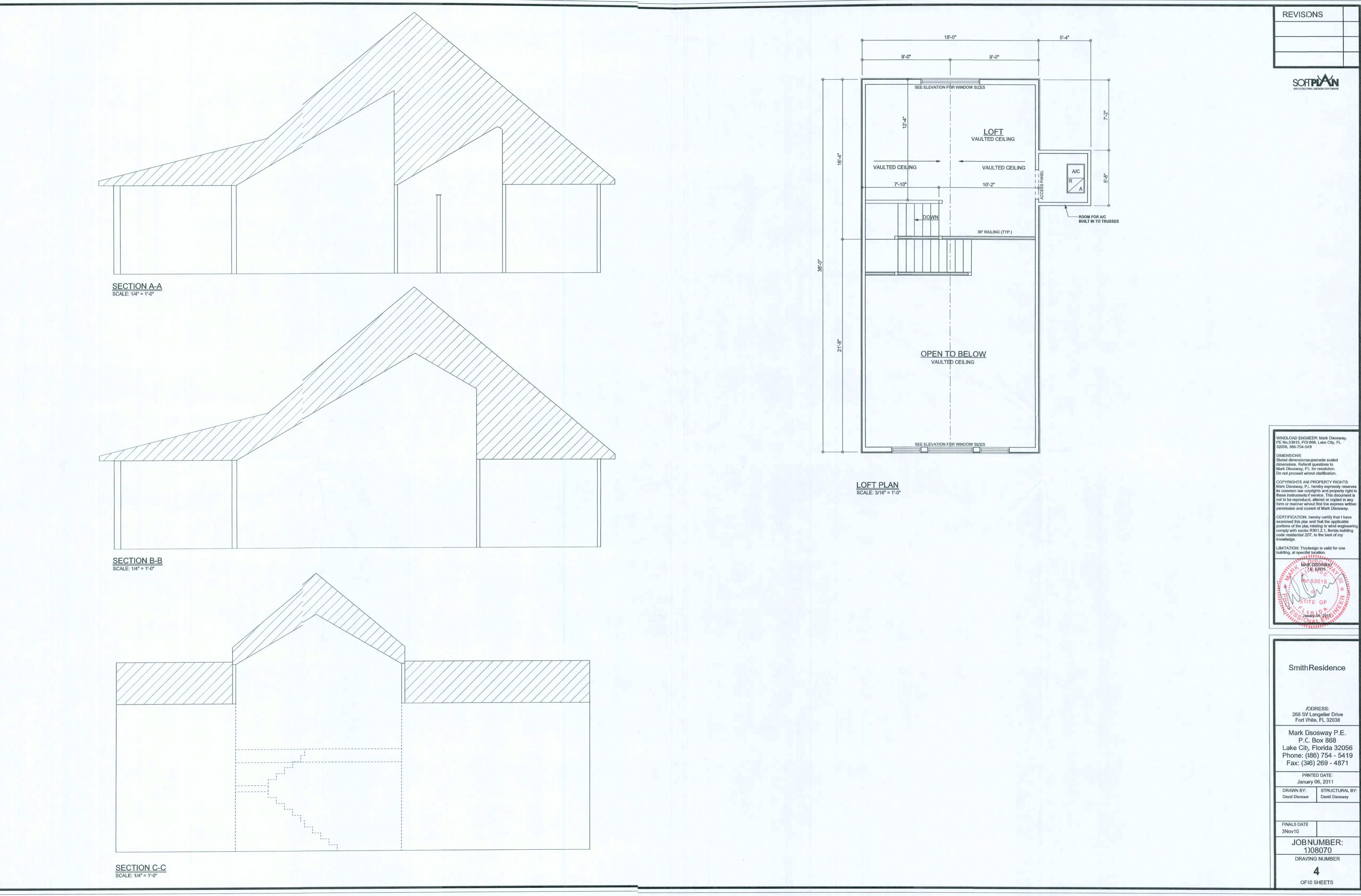
FINALS DATE: 3Nov10

> JOB NUMBER: 1008070 **DRAWING NUMBER**

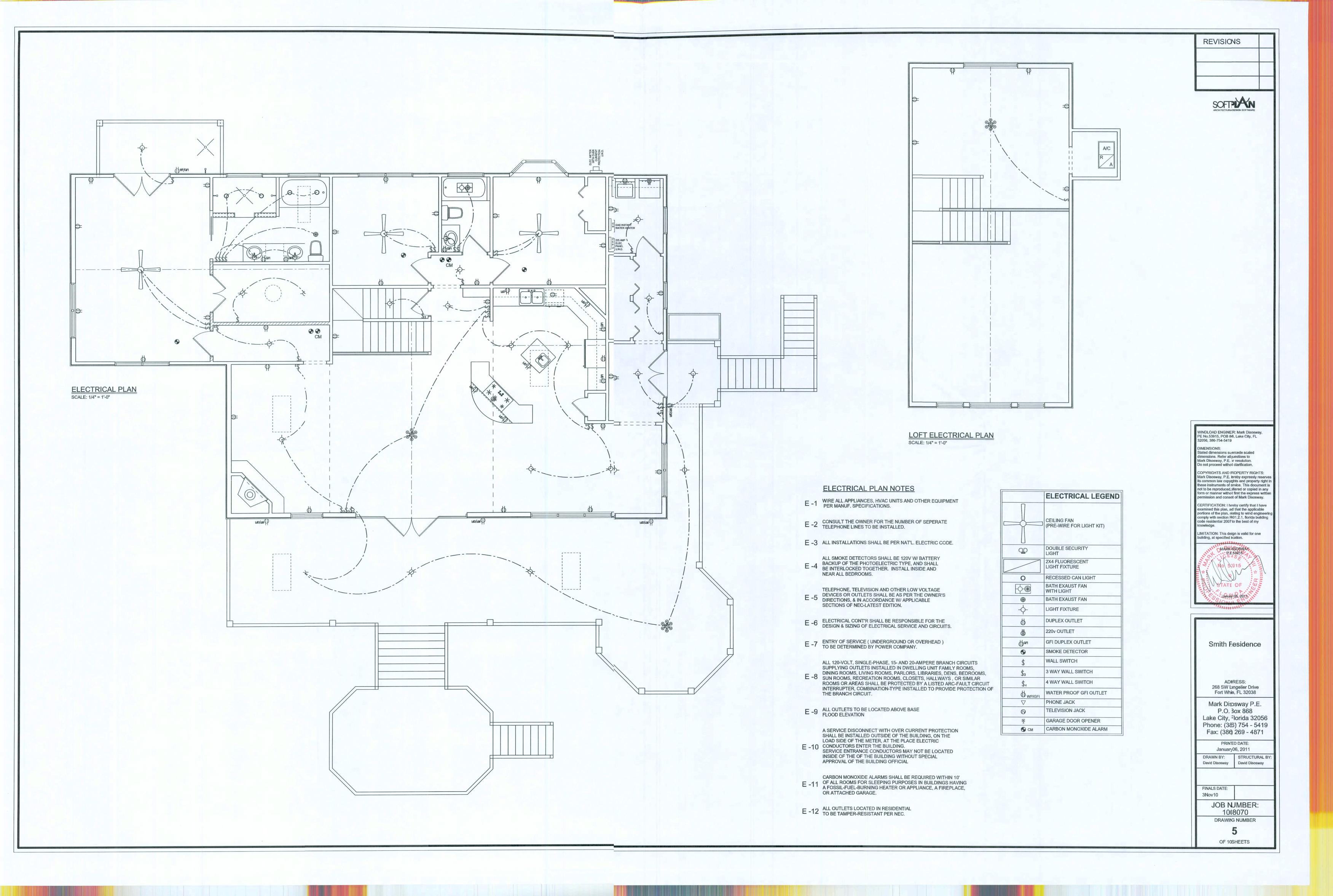
OF 10 SHEETS

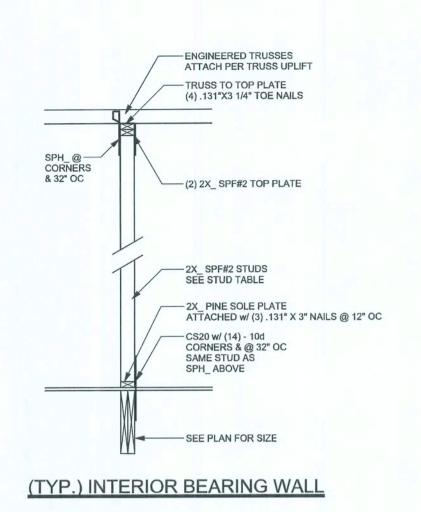






Mark Eisosway P.E.





-2X4 OUTRIGGER @ 24" O.C.

- BLOCKING REQUIRED BETWEEN OUT RIGGERS

-(4) .131"X3 1/4" -

DIAGONAL BRACE MUST BE NAILEDTO TRUSS WEBS FOR LENGTHOVER 12' IT

MAY BE "T" BRACED UF

2X4X8' RAT RUN NAIL EACH

-(4) .131"X3 1/4" NAILS

-(8) .131"X3 1/4" NAILS

- 2X4 SPF#2 BLOCKING

- H3 INSTALLED HORIZONTALLY

CONNECTION w/ (4) .131"X3 1/4" NAILS

TO 12' AND UNBRACED

┌─(4) .131"X3 1/4" ¬

(4) .131"X3 1/4"

-7/16" OSB UNBLOCKED

(4) .131"X3 1/4"-

INSTALL 2X4 SPF#2 DIAGONAL BRACE AND NAIL TO BLOCKING AT TOP CHORD &

BOTTOM CHORD AND RAT RUN @ 6' O.C

SPACE RAT RUN & DIAGONAL BRACE 6'-0" O.C.

FOR GABLE HEIGHT UP TO 25'-0" 110 MPH, EXP. C, ENCLOSED

(TYP.) GABLE BRACING DETAIL

NAILS

ATTACH RAT RUN TO -

(4) .131"X3 1/4" NAILS

TOE NAIL TRUSS ---

BLOCKING w/

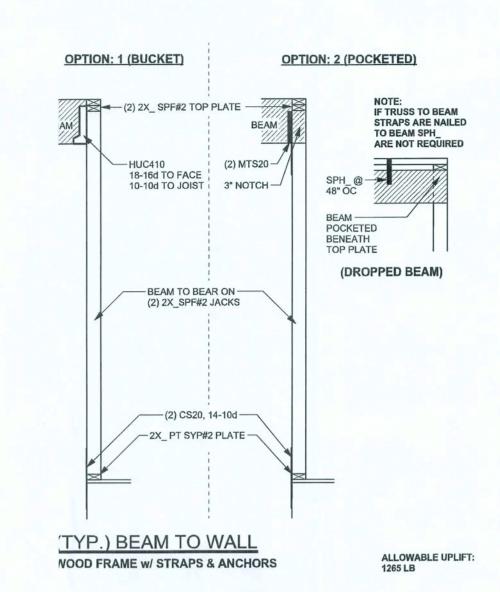
12d @ 6" O.C.

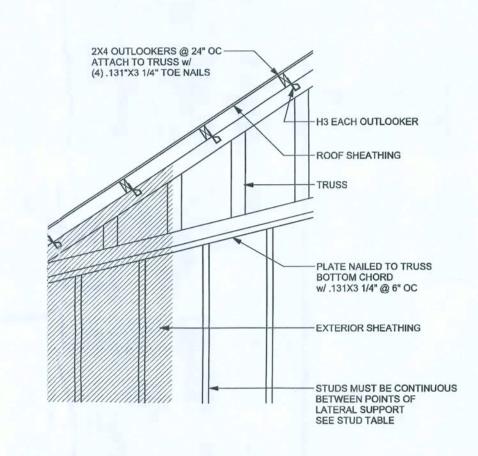
SIMPSON LSTA21

w/ (8) -16d TO TRUSS

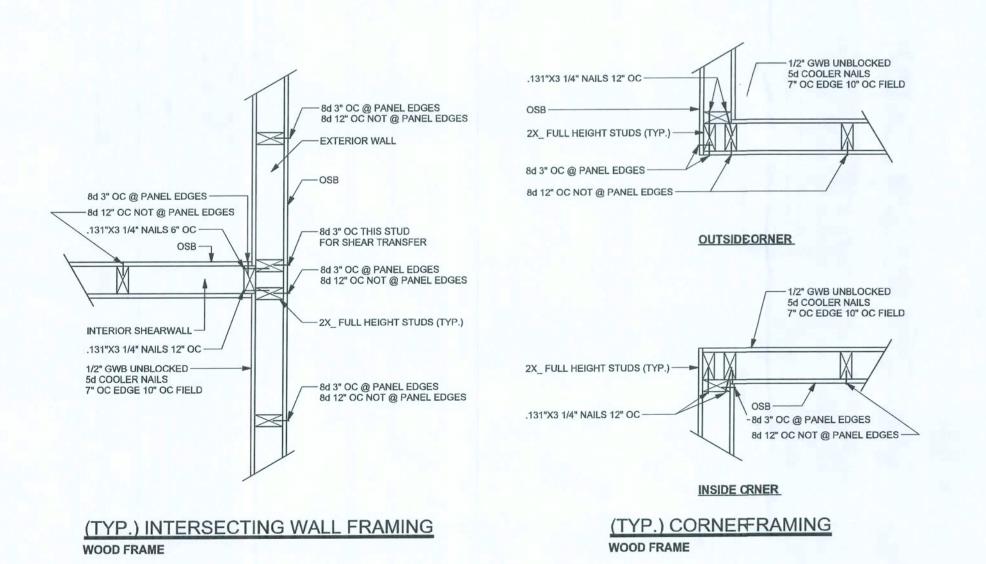
& (8) -16d TO WALL

@ 48" O.C. U.N.O.

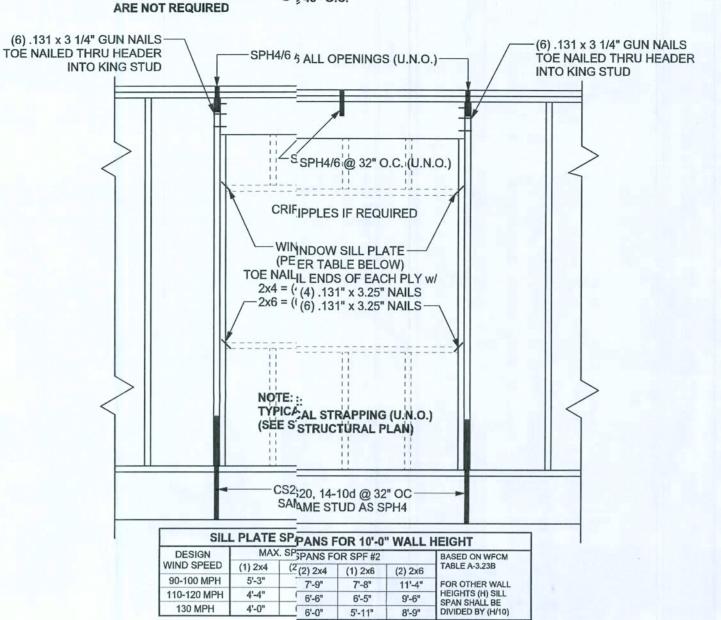




TYP.) GABLE WALL w/ VAULTED CEILING



IF TRUSS TO WALL STRAPS ARE NAILED TO THE HEADER THE SPH4/6 @ 9 48" O.C.



TYPICAL HEALDER STRAPING DETAIL

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

PLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS	
< 420	< 245	H5A	3-8d	3-8d		
< 455	< 265	H5	4-8d	4-8d		
< 360	< 235	H4	4-8d	4-8d		
< 455	< 320	H3	4-8d	4-8d		
< 415	< 365	H2.5	5-8d	5-8d		
< 600	< 535	H2.5A	5-8d	5-8d		
< 950	< 820	H6	8-8d	8-8d		
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"		
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"		
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"		
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	3 7 7	
< 760	< 655	H10-2	6-10d	6-10d		
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"		
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"		
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"		
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"		
< 2900	< 2490	2 - HTS24				
< 2050	< 1785	LGT2	14 -16d	14 -16d		
10 12		HEAVY GIRDER TIEDOWNS*			TO FOUNDATION	
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED RO	
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED ROI 12" EMBEDMENT	
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED RO 12" EMBEDMENT	
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED RO 12" EMBEDMENT	
		STUD STRAP CONNECTOR*			TO STUDS	
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d	
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d	
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d	
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d	
< 885	< 760	SP4			6-10d, 1 1/2"	
< 1240	< 1065	SPH4			10-10d, 1 1/2"	
< 885	< 760	SP6			6-10d, 1 1/2"	
< 1240	< 1065	SPH6			10-10d, 1 1/2"	
< 1235	< 1165	LSTA18	14-10d			
< 1235	< 1235	LSTA21	16-10d		1/20/10/10	
< 1030	< 1030	CS20	18-8d			
< 1705	< 1705	CS16	28-8d		78 1 50 1 1 1	
		STUD ANCHORS*	TO STUDS		TO FOUNDATION	
< 1350	< 1305	LTT19	8-16d		1/2" AB	
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB	
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB	
< 4175	< 3695	HTT16	18 - 16d		5/8" AB	
< 1400	< 1400	PAHD42	16-16d		WO AD	
< 3335	< 3335	HPAHD22	16-16d			
< 2200	< 2200	ABU44	12-16d		4/0" AD	
< 2300	< 2300	ABU66	12-16d		1/2" AB	
2000	2000	7,5000	12-100		1/2" AB	

GRADE & SPECIES TABLE

		Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	1600	1.9
PSL	PARALAM	2900	2.0

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 11'-9" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 18'-10' STUD HEIGHT
(1) 2x6 @ 12" OC	TO 20'-0" STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.

GENERAL NOTES:

NOT TO EXCEED 3'.

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2007. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI. WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS

FIBER CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWW OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"0C INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT.
AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK. CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE. PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2007 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY. VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2007, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2007 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED

DESIGN DATA

MEAN R ON UPP	SED SIMPLE DIAPHRAGM BUILDINGS I OOF HEIGHT NOT EXCEEDING LEAST ER HALF OF HILL OR ESCARPMENT 60 AND UNOBSTRUCTED UPWIND FOR 50	HORIZONTA OFT IN EXP. E	L DIME 3. 30FT	NSION IN EXE	OR 60	FT; N D > 10°
BUILDIN	G IS NOT IN THE HIGH VELOCITY HUR	RICANE ZON	1E			
BUILDIN	G IS NOT IN THE WIND-BORNE DEBRI	S REGION				
1.) BA	SIC WIND SPEED = 110 MPH					
2.) WII	ND EXPOSURE = B					
3.) WII	ND IMPORTANCE FACTOR = 1.0					
	LDING CATEGORY = II		_			
5.) RO	OF ANGLE = 10-45 DEGREES					
6.) ME	AN ROOF HEIGHT = <30 FT					
-10 002	ERNAL PRESSURE COEFFICIENT = N/	A (ENCLOSE	D BUIL	DING)		
	MPONENTS AND CLADDING DESIGN W				R301 2	(2))
- T. T.	*		_			11/18
		Zone	-		d Area	
		1	19.9	0	-	100
K		2	19.9	-21.8 -25.5	18.1	-18.
5	2 2 1	2 O'hg	10.0	-40.6	10.1	-40.
3	4 5 5	3	19.9	-25.5	18.1	-21.8
	1 4	3 O'hg		-68.3		-42.4
	55	4	21.8	-23.6	18.5	-20.
4	N. T.	5	21.8	-29.1	18.5	-22.
,	12/1/11/11	Doors 8	k Windo	ows	21.8	-29.
1		Wors	t Case			
5	13/ 13/4	(Zone	5, 10 ft	2)		
2	4 /3/ 5	8x7 Gara	ge Doo	or	19.5	-22.
	A 4 A	16x7 Gai	rage Do	oor	18.5	-21.0
	55 22				-	
	24				<u> </u>	1
DESIGN						
FLOOR	40 PSF (ALL OTHER DWELLING ROO	MS)				
	30 PSF (SLEEPING ROOMS)				111	
	30 PSF (ATTICS WITH STORAGE)					
	10 PSF (ATTICS WITHOUT STORAGE	, <3:12)				
ROOF	20 PSF (FLAT OR <4:12)					
	16 PSF (4:12 TO <12:12)					
	101 01 (4.12 10 -12.12)					

STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

SOIL BEARING CAPACITY 1000PSF

REVISIONS

SOFTEN

INDLOAD ENGINEER Mark Disosway, PE No.53915, POB 868 Lake City, FL 32056, 386-754-5419

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portions of the plan, relang to wind engineer comply with section R30.2.1, florida building code residential 2007, to the best of my



Smith Residence

ADDRESS: 268 SW Largelier Drive

Fort White,FL 32038 Mark Disosway P.E. P.O. Box 868 Lake City, Fbrida 32056

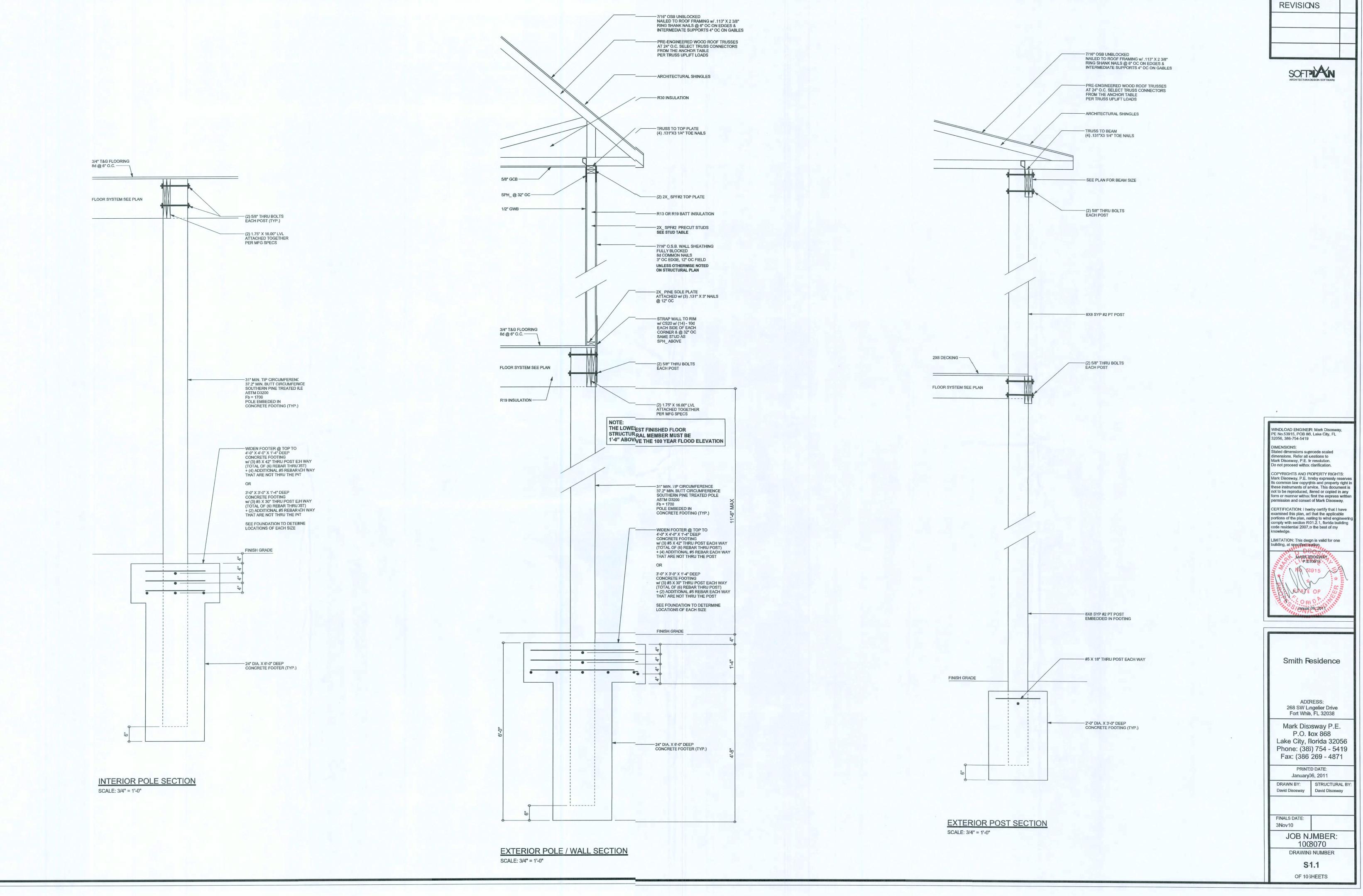
Phone: (386 754 - 5419 Fax: (386) 269 - 4871 PRINTEI DATE:

January 0i, 2011 DRAWN BY: STRUCTURAL BY: David Disosway David Disosway

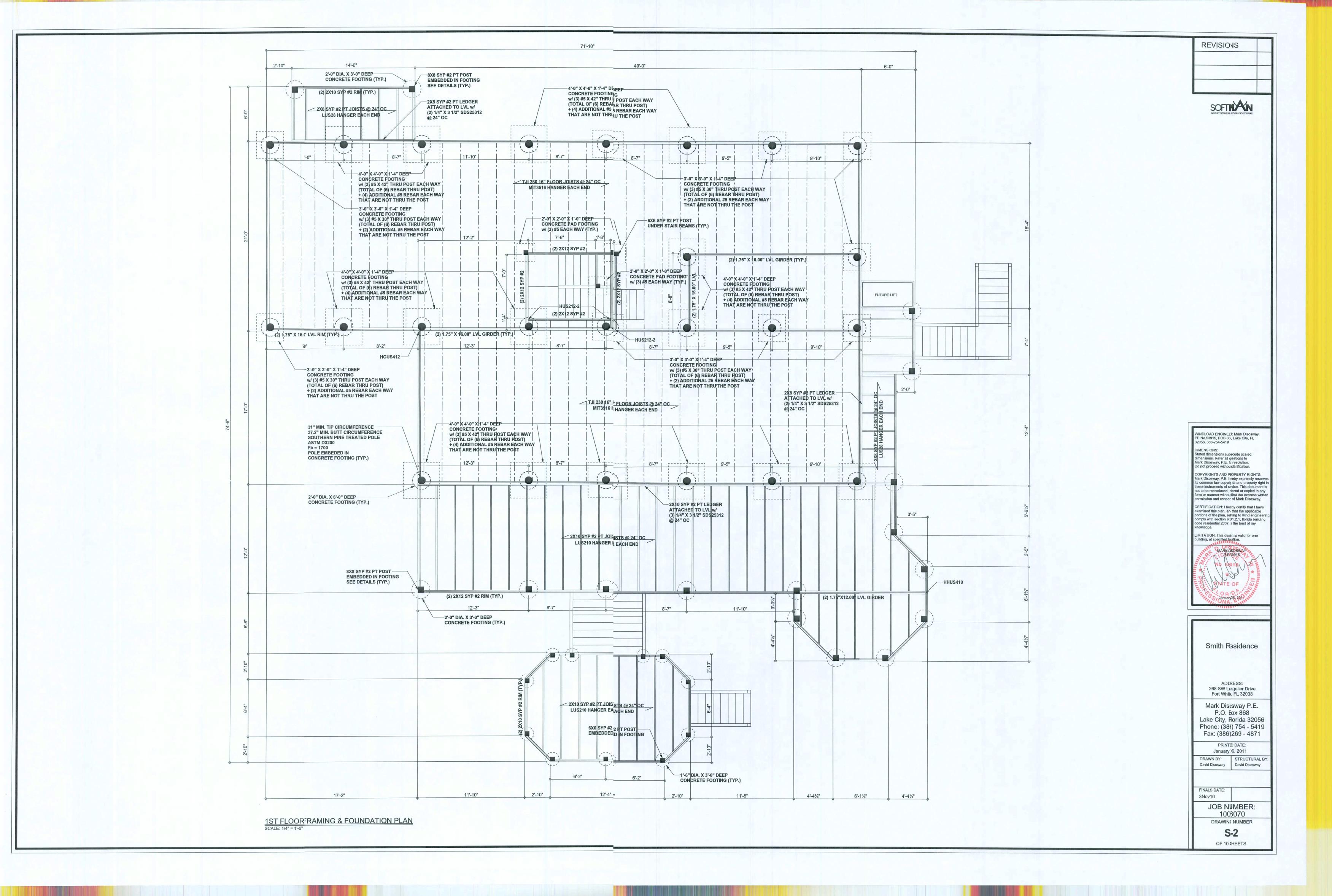
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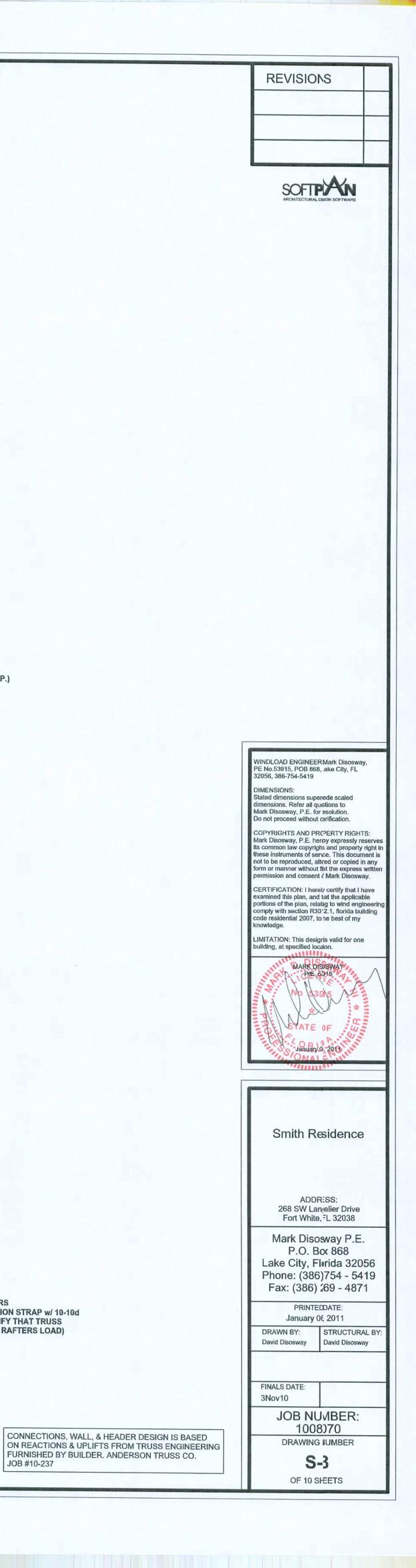
JOB NUMBER: 1008070 DRAWINGNUMBER

OF 10 SHEETS



REVISIONS





(BUILDER TO VERIFY THAT TRUSS IS DESIGNED FOR RAFTERS LOAD)

JOB #10-237

STRUCTURAL PLAN NOTES

ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP #2 (U.N.O.)

ALL LOAD BEARING FRAME WALL HEADERS SN-2 SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)

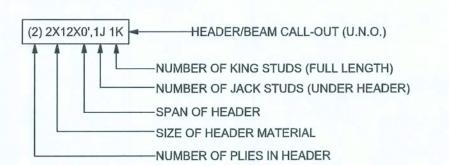
DIMENSIONS ON STRUCTURAL SHEETS SN-3 ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

WALL LEGEND

EXTERIOR WALL
INTERIOR NON-LOAD BEARING WALL
INTERIOR LOAD BEARING WALL w/ NO UPIT
INTERIOR LOAD BEARING WALL w/ UPLIFT

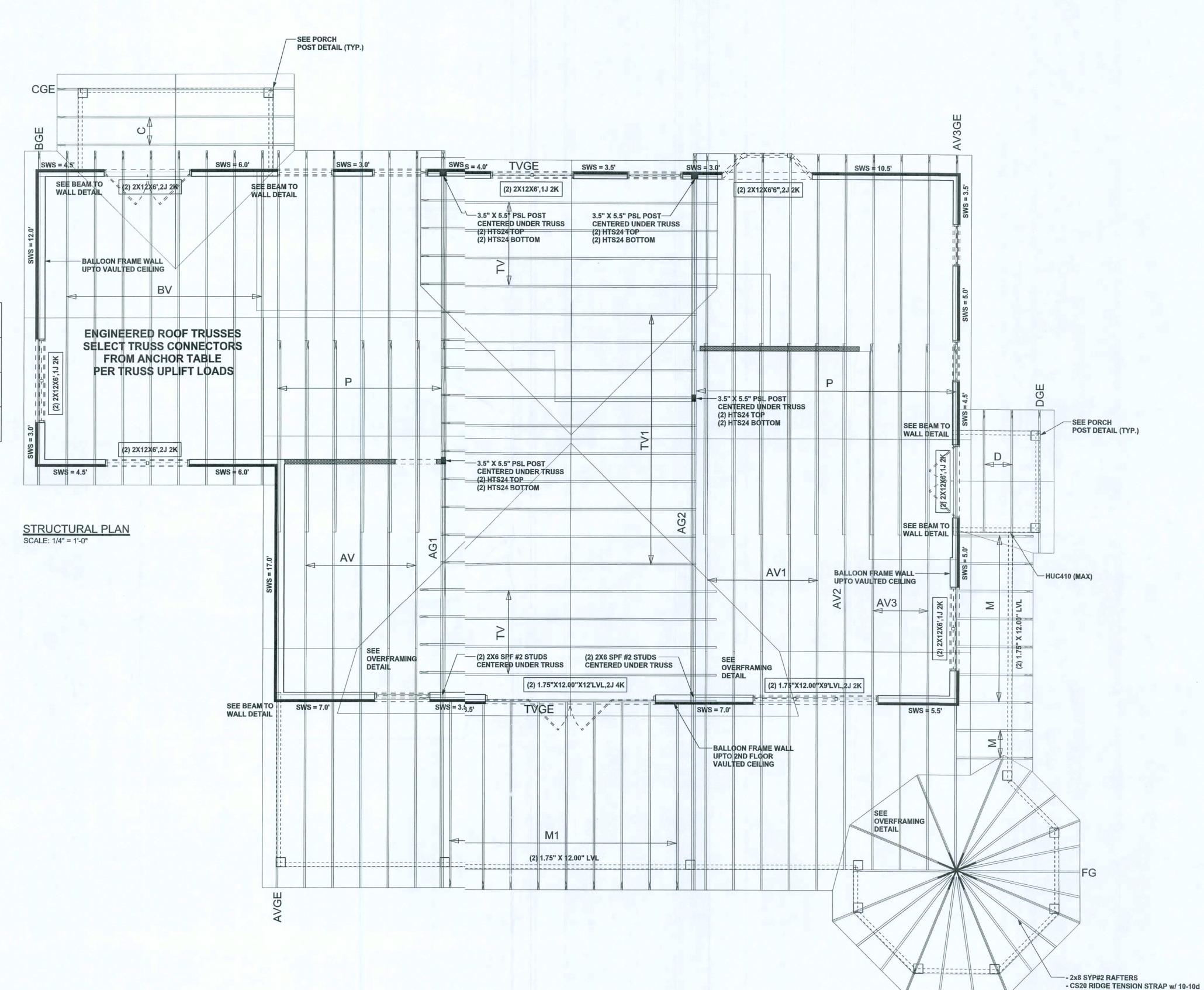
HEADER LEGEND



TOTAL SHEAR WALL SEGMENTS

INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	45.0'	45.0'
LONGITUDINAL	38.2'	68.0'



REVISION\$ 1 1/4" X 16" LSL RIM (4) KING STUDS CONTINUOUS — TO VAULTED CEILING BALLOON FRAME WALL UPTO VAULTED CEILING TJI 210 16" FLOOR JOISTS @ 24" OC IUS2.06/16 HANGER EACH END 1 1/4" X 16" LSL RIM WINDLOAD ENGINEER: Nark Disosway, PE No.53915, POB 868, Lαe City, FL 32056, 386-754-5419 LOFT FLOOR FRAMING PLAN SCALE: 1/4" = 1'-0" DIMENSIONS: Stated dimensions supercele scaled dimensions. Refer all questons to Mark Disosway, P.E. for replution. Do not proceed without claffication. COPYRIGHTS AND PROPRTY RIGHTS:
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permission and consent of flark Disosway. CERTIFICATION: I hereby ertify that I have examined this plan, and the the applicable portions of the plan, relating to wind engineering comply with section R301.£1, florida building code residential 2007, to the best of my knowledge. LIMITATION: This design isvalid for one building, at specified locatio. MARK DISO/WAY P.E. 5385 — (4) KING STUDS CONTINUOUS — FROM 1ST FLOOR TO VAULTED CEILING My de form BALLOON FRAME WALL UPTO VAULTED CEILING STATE OF January 06, 2011 (4) KING STUDS CONTINUOUS — TO VAULTED CEILING Smith Resdence LOFT STRUCTURAL PLAN
SCALE: 1/4" = 1'-0" ADDRESS: 268 SW Langdier Drive Fort White, F. 32038 Mark Disosvay P.E. P.O. Box868 Lake City, Floida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871 PRINTED DATE: January 06, 2011 DRAWN BY: STRUCTURAL BY: David Disosway Eavid Disosway FINALS DATE: JOB NUMBER: 1008070 DRAWING NIMBER S-3.1 OF 10 SHETS