JULIUS LEE PE.

RE: 687630 - AARON SIMQUE - LOT 100 PRESERVE

1109 COASTAL BAY BLVD,
BOYNTON BEACH, FL 33435

Site Information:
Project Customer Aaron Simque Cosnt Project Name. 687630 Model: Bristol Modified
Lot/Block: 100 Subdivision: The Preserve
Address:
City: Columbia Cty State FL

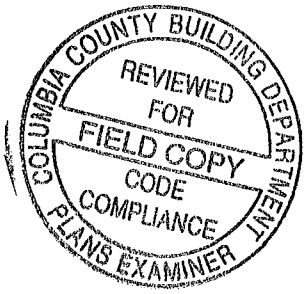
Name Address and License # of Structural Engineer of Record, If there is one, for the building.
Name: Unknown at time of Seal License #: Unknown at time of Seal
Address. Unknown at time of Seal
City: Unknown at time of Seal State Unknown at time of Seal

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):
Design Code: FBC2014/TPI2007 Design Program: MiTek 20/20 7.6
Wind Code: ASCE 7-10 Wind Speed: 130 mph Floor Load: N/A psf
Roof Load: 32.0 psf

This package includes 59 individual, dated Truss Design Drawings and 0 Additional Drawings
With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31 003, section 5 of the Florida Board of Professional Engineers Rules
This document processed per section 16G15-23 003 of the Florida Board of Professionals Rules

In the event of changes from Builder or E.O.R. additional coversheets and drawings may accompany this coversheet. The latest approval dates supersede and replace the previous drawings.

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	I10030429	CJ01	6/24/015	18	I10030446	T03G	6/24/015
2	I10030430	CJ02	6/24/015	19	I10030447	T04 644	6/24/015
3	I10030431	CJ02T	6/24/015	20	I10030448	T04D 5295	6/24/015
4	I10030432	CJ03	6/24/015	21	I10030449	T04G 2238	6/24/015
5	I10030433	CJ03T	6/24/015	22	I10030450	T05	6/24/015
6	I10030434	EJ01	6/24/015	23	I10030451	T05D 2502	6/24/015
7	I10030435	EJ02	6/24/015	24	I10030452	T06 1397	6/24/015
8	I10030436	HJ01	6/24/015	25	I10030453	T07	6/24/015
9	I10030437	HJ01T	6/24/015	26	I10030454	T07G 914	6/24/015
10	I10030438	PB01	6/24/015	27	I10030455	T08	6/24/015
11	I10030439	PB01G	6/24/015	28	I10030456	T09 5299	6/24/015
12	I10030440	PB04	6/24/015	29	I10030457	T10	6/24/015
13	I10030441	PB04G	6/24/015	30	I10030458	T11	6/24/015
14	I10030442	T01	6/24/015	31	I10030459	T12	6/24/015
15	I10030443	T01G	6/24/015	32	I10030460	T12D	6/24/015
16	I10030444	T02	6/24/015	33	I10030461	T13 1756	6/24/015
17	I10030445	T03	6/24/015	34	I10030462	T14	6/24/015

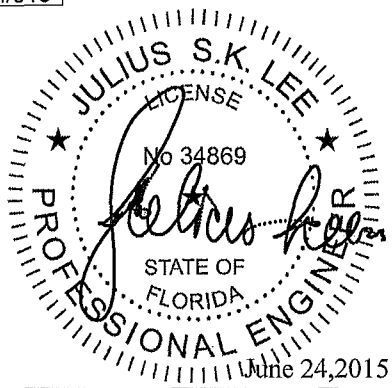


The truss drawing(s) referenced above have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by Builders FirstSource (Jax).

Truss Design Engineer's Name: Julius Lee

My license renewal date for the state of Florida is February 28, 2017.

NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Chapter 2.



June 24,2015
Julius Lee

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE - LOT 100 PRESERVE
687630	CJ01	Jack-Open Truss	6	1	I10030429
Builders FirstSource, Lake City, FL 32055, Kim Holloway					Job Reference (optional)

7.610 s Jan 29 2015 MiTek Industries, Inc. Tue Jun 23 15:47:56 2015 Page 1
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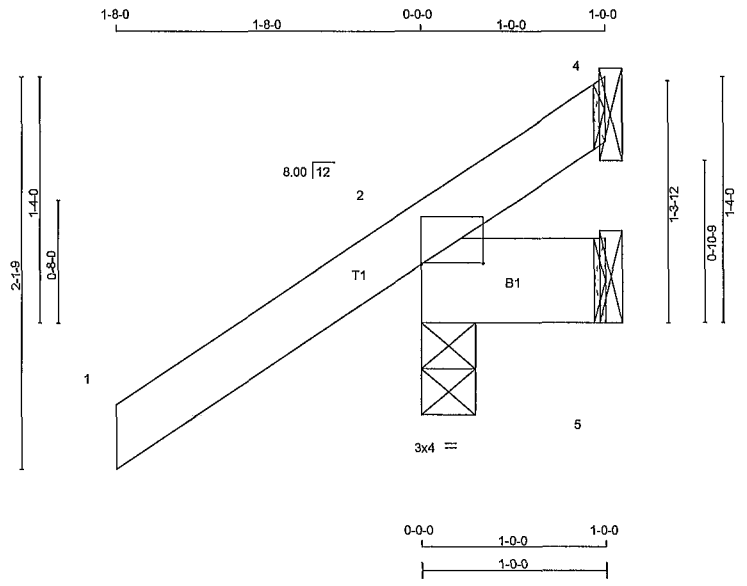

Plate Offsets (X,Y)-- [2:0-4-0,0-0-2]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20 0	Plate Grip DOL	1.25	TC	0 24	Vert(LL)	0.00	9	>999	240	MT20	244/190
TCDL	7 0	Lumber DOL	1.25	BC	0 04	Vert(TL)	0.00	9	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0 00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2014/TPI2007		(Matrix-M)							Weight: 7 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x6 SP No.2

BRACING-
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=201/0-3-8 (min. 0-1-8) 3=-3/Mechanical 6=-47/Mechanical
Max Horz 2=81(LC 12)
Max Uplift 2=-118(LC 12), 3=-12(LC 12), 6=-52(LC 21)
Max Grav 2=201(LC 1) 3=11(LC 8) 6=39(LC 16)

FORCES. (lb) - Max. Comp./Max. Ten - All forces 250 (lb) or less except when shown

NOTES- (8-10)
1) Wind ASCE 7-10 Vult=130mph (3-second gust) Vasd=101mph TCDL=4 2psf BCDL=3 0psf h=20ft; Cat. II Exp C, Encl GCpi=0.18 MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1 60 plate grip DOL=1 60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
3) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 118 lb uplift at joint 2 12 lb uplift at joint 3 and 52 lb uplift at joint 6
7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss
8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
9) Note Visually graded lumber designation SPp represents new lumber design values as per SPIB
10) Truss Design Engineer Julius Lee PE. Florida P E. License No 34869 Address. 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

June 24,2015

Julius Lee PE
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE - LOT 100 PRESERVE	I10030431
687630	CJ02T	Jack-Open Truss	4	1		
Builders FirstSource, Lake City, FL, 32055, Kim Holloway					Job Reference (optional)	

7.610 s Jan 29 2015 MiTek Industries, Inc. Tue Jun 23 15:47:58 2015 Page 1
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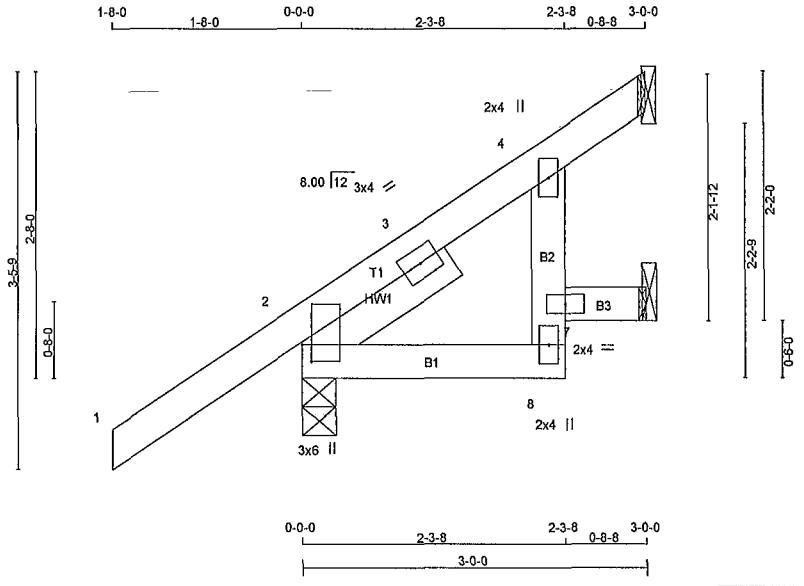


Plate Offsets (X,Y)- [2,0-1-12,0-0-15]

LOADING (psf)		SPACING-	2-0-0	CSI		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.24	Vert(LL)	-0.00	11	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.13	Vert(TL)	-0.00	11	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	2	n/a	n/a		
BCDL	5.0	Code FBC2014/TPI2007		(Matrix-M)							Weight: 18 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2 *Except*
B2 2x4 SP No 3
SLIDER Left 2x4 SP No 3 1-6-0

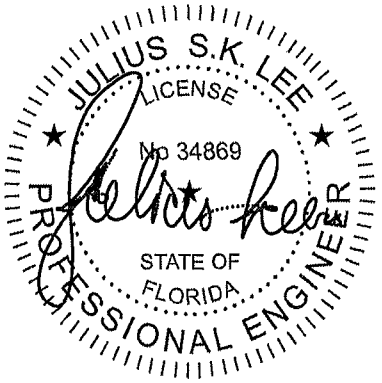
BRACING-
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 5=37/Mechanical, 2=210/0-3-8 (min. 0-1-8), 6=31/Mechanical
Max Horz 2=145(LC 12)
Max Uplift 5=-40(LC 12), 2=-85(LC 12), 6=-36(LC 12)
Max Grav 5=44(LC 21), 2=210(LC 1), 6=41(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES- (8-10)
1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 40 lb uplift at joint 5, 85 lb uplift at joint 2 and 36 lb uplift at joint 6.
7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
9) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
10) Truss Design Engineer: Julius Lee PE, Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd Boynton Beach FL 33435

LOAD CASE(S) Standard



June 24,2015

Julius Lee PE
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE - LOT 100 PRESERVE	I10030433
687630	CJ03T	Jack-Open Truss	4	1		
Builders FirstSource, Lake City, FL 32055, Kim Holloway					Job Reference (optional)	

7.610 s Jan 29 2015 MiTek Industries, Inc. Tue Jun 23 15:47:59 2015 Page 1
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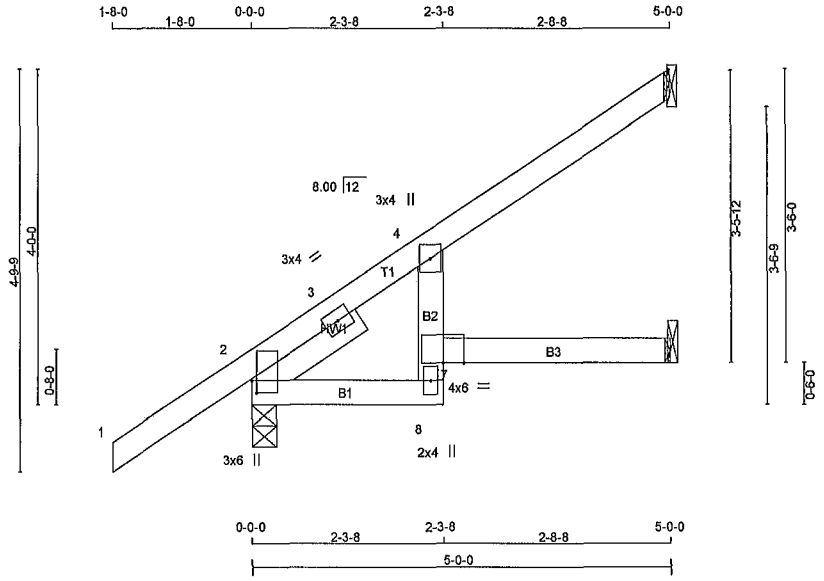


Plate Offsets (X,Y)-- [2:0-1-12,0-0-11]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.26	Vert(LL)	0.05	6-7	>999	240	MT20	244/190
TCDL	7 0	Lumber DOL	1.25	BC	0 37	Vert(TL)	-0.06	6-7	>982	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0 00	Horz(TL)	0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2014/TPI2007		(Matrix-M)							Weight: 24 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2 *Except*
B2 2x4 SP No.3
SLIDER Left 2x4 SP No.3 1-6-0

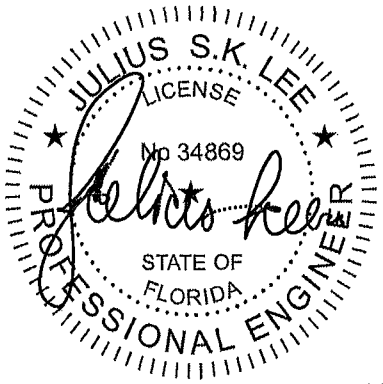
BRACING-
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing

REACTIONS. (lb/size) 5=92/Mechanical 2=263/0-3-8 (min. 0-1-8) 6=50/Mechanical
Max Horz 2=211(LC 12)
Max Uplift 5=-105(LC 12) 2=-90(LC 12) 6=-37(LC 12)
Max Grav 5=110(LC 21) 2=263(LC 1), 6=65(LC 3)

FORCES. (lb) Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown

NOTES- (8-10)
1) Wind ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II Exp C Encl GCpi=0.18 MWFRS (envelope) gable end zone and C-C Exterior(2) zone C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1 60
2) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 105 lb uplift at joint 5, 90 lb uplift at joint 2 and 37 lb uplift at joint 6
7) 'Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
9) Note Visually graded lumber designation SPp represents new lumber design values as per SPIB
10) Truss Design Engineer: Julius Lee, PE Florida P E. License No. 34869: Address 1109 Coastal Bay Blvd. Boynton Beach FL 33435

LOAD CASE(S) Standard



June 24,2015

Julius Lee PE
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE - LOT 100 PRESERVE
687630	EJ02	Jack-Partial Truss	2	1	I10030435
Builders FirstSource, Lake City, FL 32055, Kim Holloway					Job Reference (optional)

7.610 s Jan 29 2015 MiTek Industries, Inc. Tue Jun 23 15:48:00 2015 Page 1
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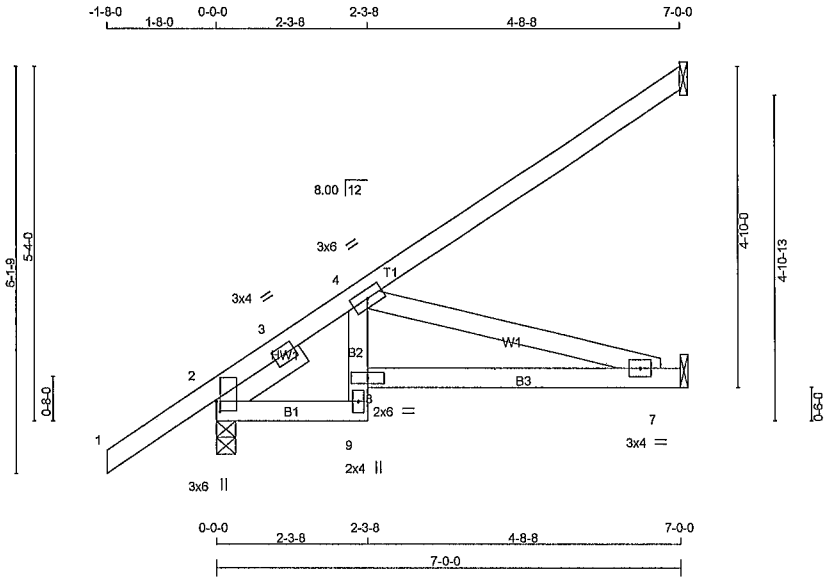


Plate Offsets (X,Y)-- [2:0-1-12,0-0-11]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.36	Vert(LL)	-0.04	7-8	>999	240	MT20	244/190
TCDL	7 0	Lumber DOL	1.25	BC	0.57	Vert(TL)	-0.07	7-8	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.26	Horz(TL)	0 02	6	n/a	n/a		
BCDL	5.0	Code FBC2014/TPI2007		(Matrix-M)							Weight: 37 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2 *Except*
B2 2x4 SP No.3
WEBS 2x4 SP No.3
SLIDER Left 2x4 SP No 3 1-6-0

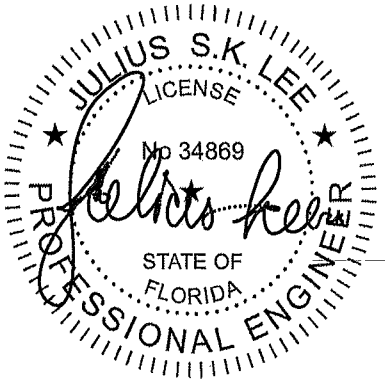
BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing

REACTIONS. (lb/size) 5=110/Mechanical, 2=323/0-3-8 (min 0-1-8) 6=101/Mechanical
Max Horz 2=191(LC 12)
Max Uplift 5=-83(LC 12), 2=-45(LC 12), 6=-44(LC 12)
Max Grav 5=123(LC 21) 2=323(LC 1), 6=120(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten - All forces 250 (lb) or less except when shown
TOP CHORD 3-4=-265/39
BOT CHORD 2-9=-262/393 7-8=-365/547
WEBS 4-7=-567/379

NOTES- (8-10)
1) Wind ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph TCDL=4.2psf BCDL=3.0psf h=20ft; Cat. II Exp C Encl GCpi=0.18, MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1 60
2) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 83 lb uplift at joint 5 45 lb uplift at joint 2 and 44 lb uplift at joint 6
7) 'Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss
8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
9) Note Visually graded lumber designation SPp, represents new lumber design values as per SPIB
10) Truss Design Engineer Julius Lee PE Florida P E License No. 34869 Address. 1109 Coastal Bay Blvd Boynton Beach FL 33435

LOAD CASE(S) Standard



June 24,2015

Julius Lee PE
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE - LOT 100 PRESERVE	I10030437
687630	HJ01T	Diagonal Hip Girder	2	1		
Builders FirstSource, Lake City, FL 32055, Kim Holloway					Job Reference (optional)	

7.610 s Jan 29 2015 MiTek Industries, Inc. Tue Jun 23 15:48:02 2015 Page 1
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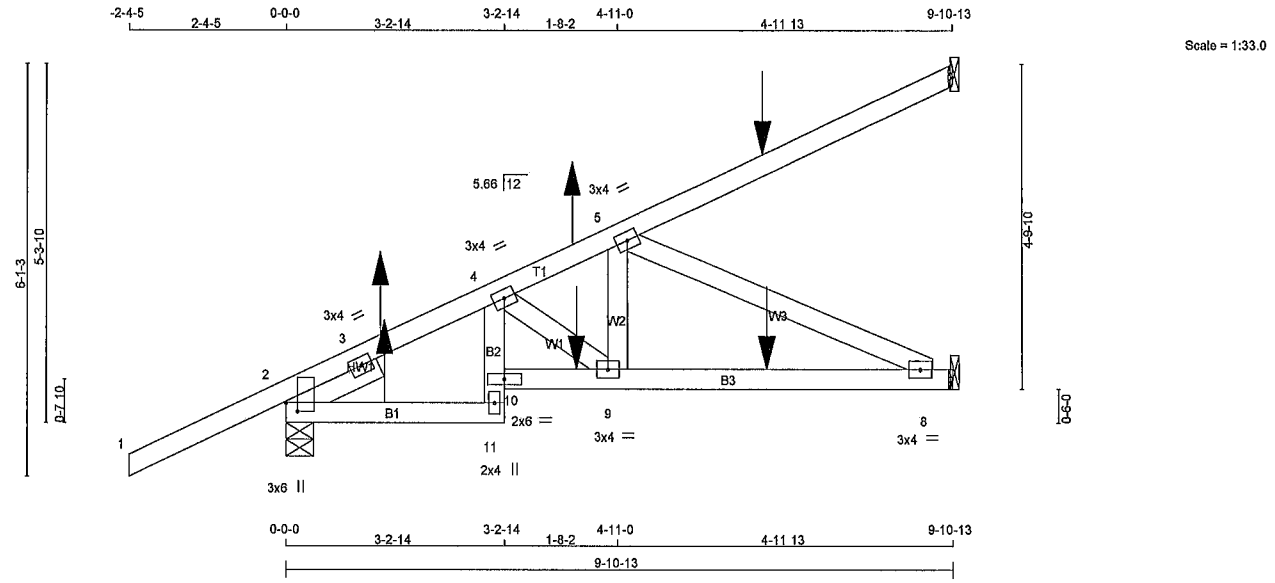

Plate Offsets (X,Y)-- [2:0-1-8,0-2-1]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.40	Vert(LL)	0.07	8-9	>999	240	MT20	244/190
TCDL	7 0	Lumber DOL	1.25	BC	0.43	Vert(TL)	-0.08	8-9	>999	180		
BCLL	0 0 *	Rep Stress Incr	NO	WB	0.28	Horz(TL)	-0.02	7	n/a	n/a		
BCDL	5.0	Code FBC2014/TPI2007		(Matrix-M)							Weight: 53 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No 2
BOT CHORD 2x4 SP No 2 *Except*
B2. 2x4 SP No 3
WEBS 2x4 SP No.3
SLIDER Left 2x4 SP No 3 1-6-0

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing

REACTIONS (lb/size) 6=119/Mechanical, 2=392/0-4-15 (min 0-1-8) 7=225/Mechanical
Max Horz 2=278(LC 8)
Max Uplift 6=-134(LC 8), 2=-285(LC 4), 7=-244(LC 8)
Max Grav 6=119(LC 1) 2=392(LC 1), 7=259(LC 32)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown
TOP CHORD 2-3=-417/288, 3-16=-586/360 4-16=-520/353 4-17=-578/468, 5-17=-529/430
BOT CHORD 2-19=-366/345 11-19=-366/345, 10-20=-448/413, 9-20=-448/413 9-21=-532/492 8-21=-532/492
WEBS 5-8=-541/585

NOTES- (10-12)
1) Wind ASCE 7 10; Vult=130mph (3-second gust) Vasd=101mph TCDL=4.2psf BCDL=3.0psf h=20ft; Cat. II Exp C Encl , GCpi=0 18; MWFRS (envelope) gable end zone Lumber DOL=1 60 plate grip DOL=1 60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 134 lb uplift at joint 6 285 lb uplift at joint 2 and 244 lb uplift at joint 7
7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 77 lb down and 79 lb up at 1-5-12 77 lb down and 79 lb up at 1-5-12, 92 lb down and 49 lb up at 4-3-11 92 lb down and 49 lb up at 4-3-11 and 134 lb down and 114 lb up at 7-1-10 and 134 lb down and 114 lb up at 7-1-10 on top chord and 34 lb down and 61 lb up at 1-5-12 34 lb down and 61 lb up at 1-5-12 36 lb down and 50 lb up at 4-3-11 36 lb down and 50 lb up at 4-3-11 and 45 lb down and 51 lb up at 7-1-10 and 45 lb down and 51 lb up at 7-1-10 on bottom chord The design/selection of such connection device(s) is the responsibility of others.
9) In the LOAD CASE(S) section loads applied to the face of the truss are noted as front (F) or back (B).
10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
11) Note Visually graded lumber designation SPp represents new lumber design values as per SPIB.
12) Truss Design Engineer Julius Lee PE Florida P E License No 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach FL 33435

LOAD CASE(S) Standard
1) Dead + Roof Live (balanced): Lumber Increase=1 25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-6=-54, 11-12=-10 7-10=-10
Concentrated Loads (lb)
Vert: 16=37(F=19 B=19) 17=79(F=39 B=39) 18=-32(F=-16 B=-16) 19=44(F=22, B=22) 20=-35(F=-17, B=-17) 21=-73(F=-36, B=-36)

JULIUS S.K. LEE LICENSE No 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

June 24,2015

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer Bracing shown is for lateral support of individual web members only Additional temporary bracing to insure stability during construction is the responsibility of the erector Additional permanent bracing of the overall structure is the responsibility of the building designer For general guidance regarding fabrication quality control storage, delivery erection and bracing consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive Madison, WI 53719.

Julius Lee PE
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE - LOT 100 PRESERVE
687630	PB01G	GABLE	2	1	I10030439
Builders FirstSource, Lake City, FL 32055, Kim Holloway					Job Reference (optional)

7.610 s Jan 29 2015 MiTek Industries, Inc. Tue Jun 23 15:48:04 2015 Page 1
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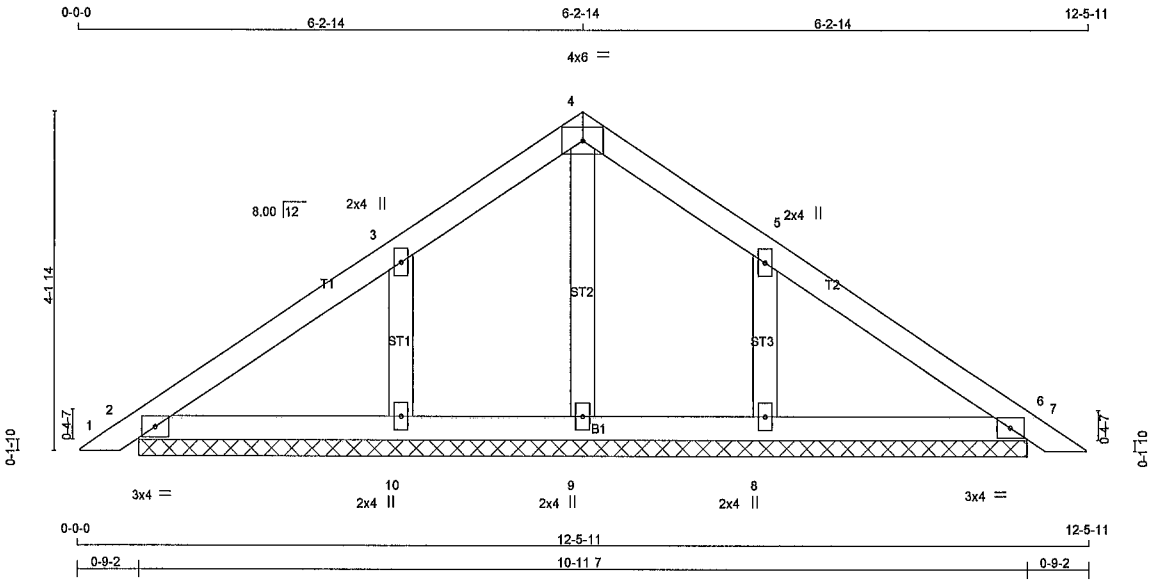

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.12	Vert(LL)	0.00	7	n/r	120	MT20	244/190
TCDL	7 0	Lumber DOL	1.25	BC	0.06	Vert(TL)	0.00	7	n/r	120		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.06	Horz(TL)	0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2014/TPI2007		(Matrix)							Weight: 49 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

REACTIONS. All bearings 10-11-7
(lb) Max Horz 2=101(LC 11)
Max Uplift All uplift 100 lb or less at joint(s) 2 6 except 8=-123(LC 13) 10=-123(LC 12)
Max Grav All reactions 250 lb or less at joint(s) 2, 6 9 8, 10

FORCES (lb) Max. Comp./Max. Ten - All forces 250 (lb) or less except when shown
WEBS 5-8=-286/204, 3-10=-286/204

NOTES- (12-14)
1) Unbalanced roof live loads have been considered for this design
2) Wind ASCE 7 10; Vult=130mph (3-second gust) Vasd=101mph TCDL=4.2psf· BCDL=3.0psf· h=20ft; Cat. II Exp C Encl GCpi=0 18 MWFRS (envelope) and C-C Exterior(2) zone·C-C for members and forces & MWFRS for reactions shown Lumber DOL=1 60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable or consult qualified building designer as per ANSI/TPI 1
4) Gable requires continuous bottom chord bearing
5) Gable studs spaced at 4-0-0 oc.
6) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads.
7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
8) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 6 except (jt=lb) 8=123 10=123.
10) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
11) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer
12) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
13) Note Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
14) Truss Design Engineer· Julius Lee, PE Florida P E. License No. 34869· Address 1109 Coastal Bay Blvd. Boynton Beach FL 33435

LOAD CASE(S) Standard

June 24,2015

Julius Lee PE
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE - LOT 100 PRESERVE	I10030441
687630	PB04G	PIGGYBACK TRUSS	1	1		
Builders FirstSource, Lake City, FL 32055, Kim Holloway					Job Reference (optional)	

7.610 s Jan 29 2015 MiTek Industries, Inc. Tue Jun 23 15:48:05 2015 Page 1
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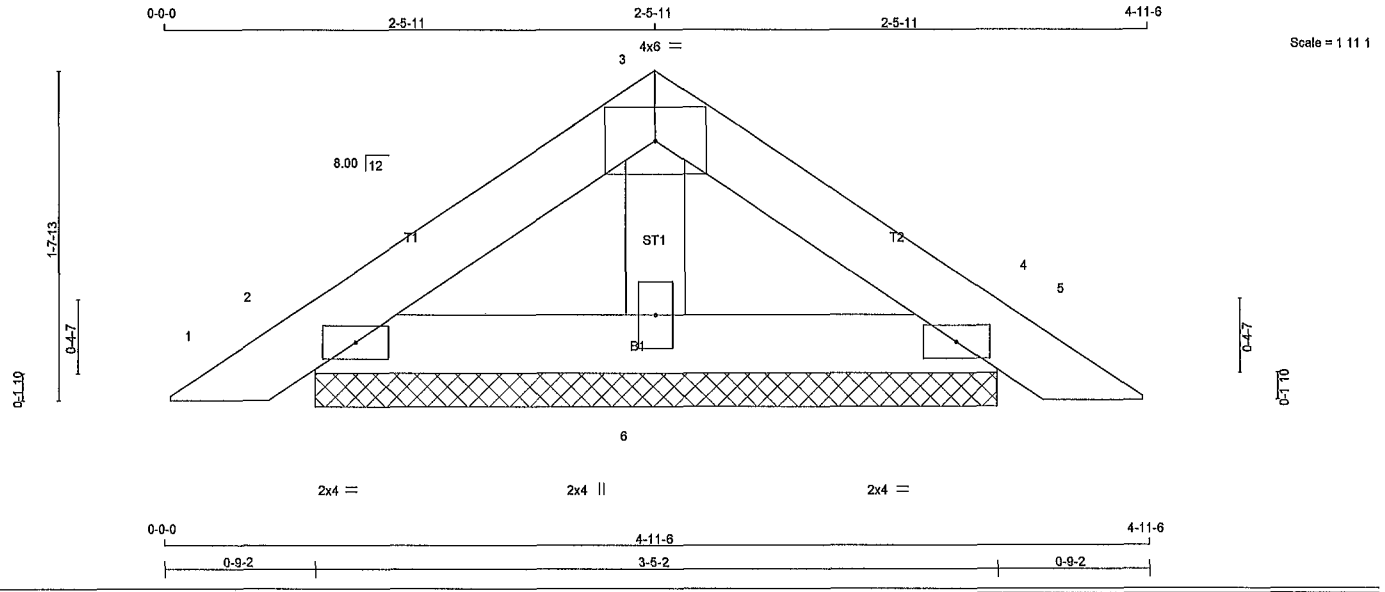


LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) l/defl L/d	PLATES GRIP
TCLL 20 0	Plate Grip DOL 1 25	TC 0 05	Vert(LL) 0.00 4 n/r 120	MT20 244/190
TCDL 7 0	Lumber DOL 1 25	BC 0 02	Vert(TL) 0.00 4 n/r 120	
BCLL 0 0 *	Rep Stress Incr YES	WB 0.01	Horz(TL) 0.00 4 n/a n/a	
BCDL 5.0	Code FBC2014/TPI2007	(Matrix)		Weight: 15 lb FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

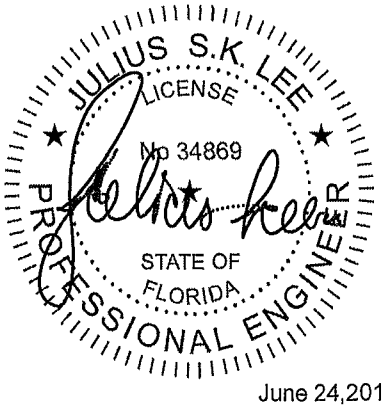
BRACING-
TOP CHORD Structural wood sheathing directly applied or 4-11-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 2=91/3-5-2 (min 0-1-8), 4=91/3-5-2 (min 0-1-8) 6=90/3-5-2 (min 0-1-8)
Max Horz 2=-37(LC 10)
Max Uplift 2=-33(LC 12) 4=-36(LC 13), 6=-7(LC 12)

FORCES. (lb) Max. Comp./Max. Ten - All forces 250 (lb) or less except when shown

NOTES- (10-12)
1) Unbalanced roof live loads have been considered for this design
2) Wind ASCE 7 10 Vult=130mph (3-second gust) Vasd=101mph TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II Exp C Encl GCpi=0.18, MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1 60 plate grip DOL=1 60
3) Gable requires continuous bottom chord bearing.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 4, 6.
8) 'Semi-rigid pitchbreaks including heels' Member end fixity model was used in the analysis and design of this truss.
9) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer
10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
11) Note: Visually graded lumber designation SPp represents new lumber design values as per SPIB.
12) Truss Design Engineer: Julius Lee, PE Florida P E License No. 34869 Address. 1109 Coastal Bay Blvd. Boynton Beach FL 33435

LOAD CASE(S) Standard



June 24,2015

Julius Lee PE
1109 Coastal Bay
Boynton Beach FL 33435

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE - LOT 100 PRESERVE
687630	T01G	GABLE	1	1	I10030443
Builders FirstSource, Lake City, FL 32055, Kim Holloway					Job Reference (optional)

7.610 s Jan 29 2015 MiTek Industries, Inc. Tue Jun 23 15:48:07 2015 Page 1
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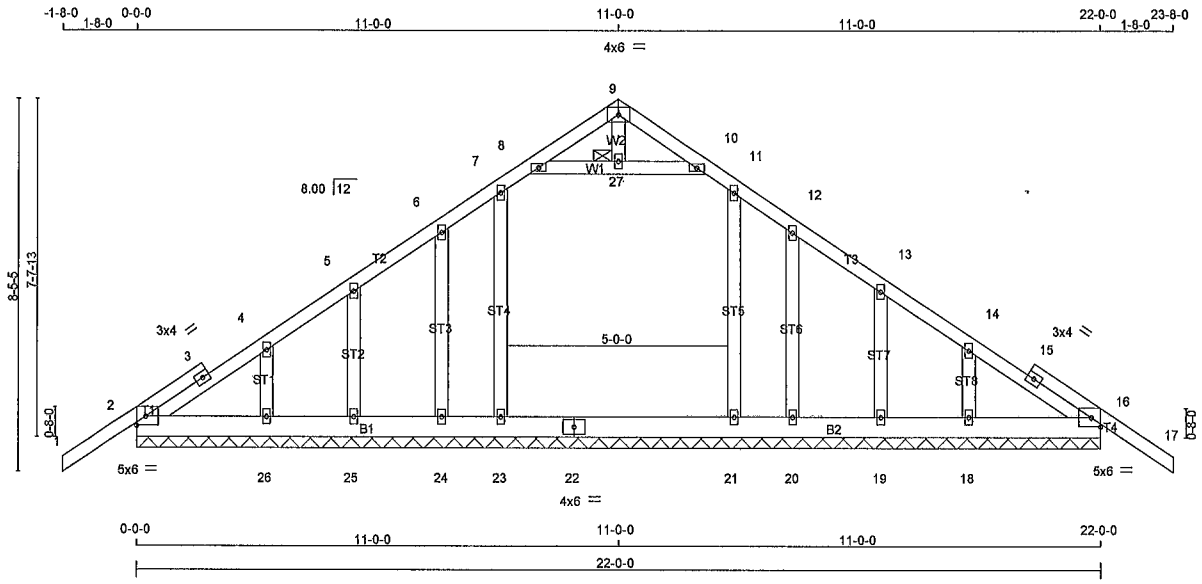

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.18	Vert(LL)	-0.01	17	n/r	120	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.09	Vert(TL)	-0.01	17	n/r	120		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.10	Horz(TL)	0.00	16	n/a	n/a		
BCDL	5.0	Code FBC2014/TPI2007		(Matrix)							Weight: 151 lb	FT = 20%

LUMBER-

TOP CHORD	2x4 SP No.2
BOT CHORD	2x6 SP No.2
WEBS	2x4 SP No.3
OTHERS	2x4 SP No.3

BRACING-

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
JOINTS	1 Brace at Jt(s): 27

REACTIONS. All bearings 22-0-0
(lb) - Max Horz 2=-258(LC 10)
Max Uplift All uplift 100 lb or less at joint(s) 2 16, 23, 21 except 24=-103(LC 12), 25=-115(LC 12), 26=-127(LC 12) 20=-109(LC 13) 19=-114(LC 13), 18=-129(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 24 25, 26, 20, 19 18 except 2=256(LC 1), 16=256(LC 1), 23=357(LC 21), 21=326(LC 22)

FORCES. (lb) - Max. Comp./Max. Ten - All forces 250 (lb) or less except when shown
TOP CHORD 7-8=-254/196, 10-11=-254/196

NOTES- (12-14)
1) Unbalanced roof live loads have been considered for this design.
2) Wind ASCE 7 10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II, Exp C, Encl GCpi=0.18, MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1
4) All plates are 2x4 MT20 unless otherwise indicated
5) Gable requires continuous bottom chord bearing
6) Gable studs spaced at 2-0-0 oc.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf
9) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 16, 23, 21 except (jt=lb) 24=103 25=115 26=127 20=109, 19=114 18=129
11) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
12) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
13) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
14) Truss Design Engineer: Julius Lee PE. Florida P E. License No. 34869: Address 1109 Coastal Bay Blvd. Boynton Beach FL 33435

LOAD CASE(S) Standard

June 24,2015

Julius Lee PE
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE - LOT 100 PRESERVE
687630	T03	Piggyback Base Truss	6	1	I10030445
Builders FirstSource, Lake City, FL 32055, Kim Holloway					Job Reference (optional)

7.610 s Jan 29 2015 MiTek Industries, Inc. Tue Jun 23 15:48:09 2015 Page 1
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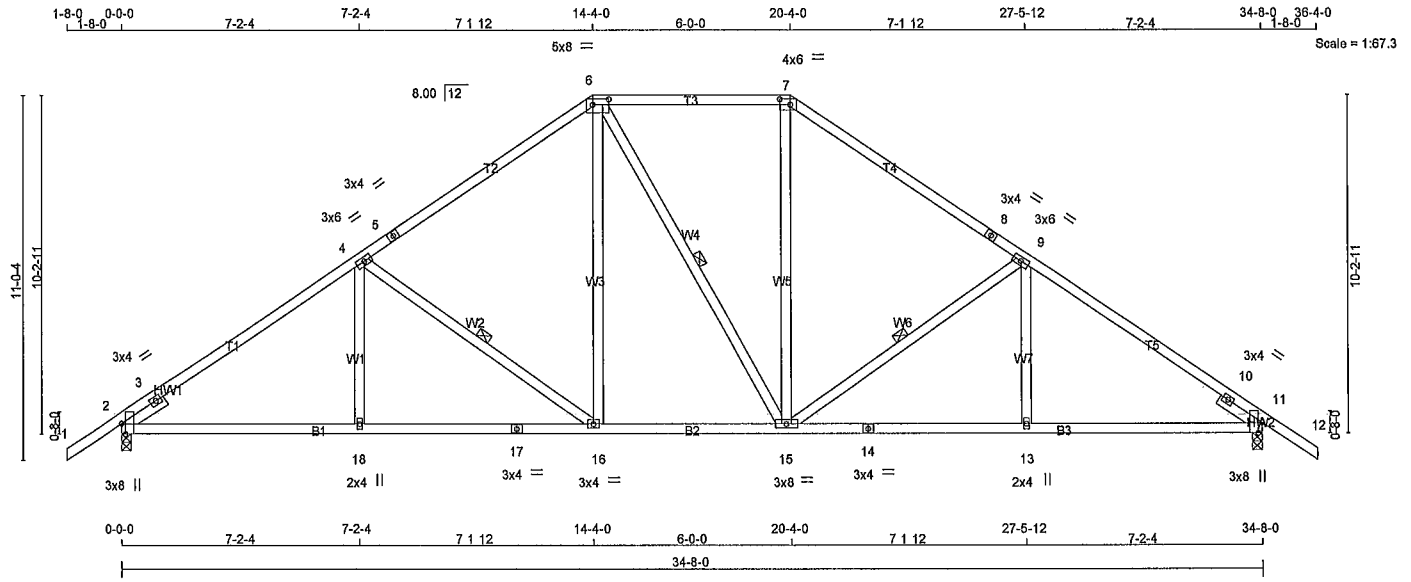

Plate Offsets (X,Y)- [2:0-3-13,Edge], [6:0-5-12,0-2-0], [7:0-3-12,0-2-0], [11:0-3-13,Edge]

LOADING (psf)		SPACING-	2-0-0	CSI		DEFL.	In	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.87	Vert(LL)	-0.10	15-16	>999	240	MT20	244/190
TCDL	7 0	Lumber DOL	1.25	BC	0.48	Vert(TL)	-0.22	16-18	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0 37	Horz(TL)	0.09	11	n/a	n/a		
BCDL	5.0	Code FBC2014/TPI2007		(Matrix-M)							Weight: 208 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3
SLIDER Left 2x4 SP No.3 1-6-0 Right 2x4 SP No 3 1-6-0

BRACING-
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied or 7-7-12 oc bracing
WEBS 1 Row at midpt 4-16 6-15, 9-15

REACTIONS (lb/size) 2=1199/0-3-8 (min 0-1 13), 11=1199/0-3-8 (min 0-1-13)
Max Horz 2=-341(LC 10)
Max Uplift 2=-512(LC 12) 11=-512(LC 13)

FORCES. (lb) Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown
TOP CHORD 2-3=-812/104 3-4=-2085/843, 4-5=-1682/739 5-6=-1586/769, 6-7=-1377/729 7-8=-1574/769 8-9=-1669/739 9-10=-2074/843, 10-11=-805/104
BOT CHORD 2-18=-577/1620 17-18=-577/1620, 16-17=-577/1620 16-27=-259/1133 15-27=-259/1133, 14-15=-530/1623, 13-14=-530/1623 11-13=-530/1623
WEBS 4-16=-620/410 6-16=-194/502 7 15=-170/476, 9-15=-621/410

NOTES- (9-11)
1) Unbalanced roof live loads have been considered for this design.
2) Wind ASCE 7-10 Vult=130mph (3-second gust) Vasd=101mph TCDL=4.2psf BCDL=3 0psf h=20ft; Cat. II Exp C, Encl GCpi=0 18, MWFRS (envelope) gable end zone and C-C Exterior(2) zone C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=512 11=512.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
10) Note Visually graded lumber designation SPp represents new lumber design values as per SPIB
11) Truss Design Engineer Julius Lee, PE Florida P E. License No. 34869 Address. 1109 Coastal Bay Blvd. Boynton Beach FL 33435

LOAD CASE(S) Standard

June 24,2015

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE - LOT 100 PRESERVE	I10030447
687630	T04	ATTIC TRUSS	3	1		
Builders FirstSource, Lake City, FL 32055, Kim Holloway					Job Reference (optional)	

7.610 s Jan 29 2015 MiTek Industries, Inc. Tue Jun 23 15:48:15 2015 Page 1
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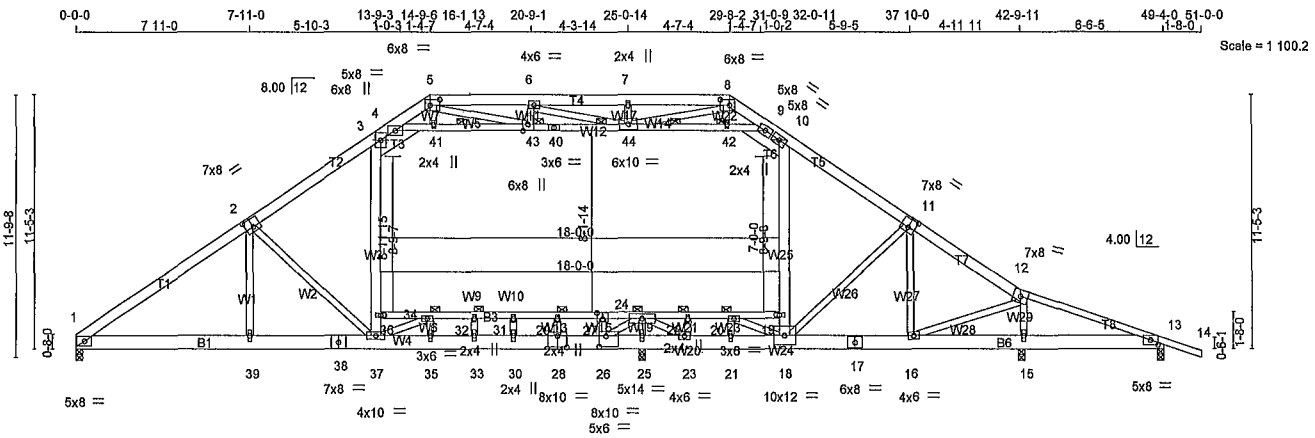

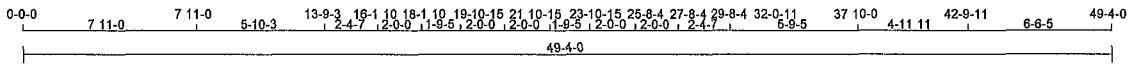

Plate Offsets (X,Y)- [2:0-4-0,0-4-8], [5:0-5-4,0-3-0], [8:0-5-4,0-3-0], [11:0-4-0,0-4-8], [13:0-4-0,0-2-10], [26:0-3-8,0-5-12], [27:0-3-0,0-3-4], [28:0-5-0,0-6-0], [43:0-3-8,0-3-0]

LOADING (psf)		SPACING- 2-0-0		CSI		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.95	Vert(LL)	-0.44	34-36	>703	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.80	Vert(TL)	-0.74	34-36	>417	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.93	Horz(TL)	0.07	13	n/a	n/a		
BCDL	5.0	Code FBC2014/TPI2007		(Matrix-M)		Attic	-0.31	19-36	715	360	Weight: 477 lb	FT = 20%

LUMBER-

TOP CHORD 2x6 SP No.2 *Except*
T8: 2x4 SP No.2 T3,T6: 2x8 SP 2400F 2.0E
BOT CHORD 2x8 SP 2400F 2.0E *Except*
B3 B5: 2x4 SP No.2
WEBS 2x4 SP No.3 *Except*
W3 W25: 2x6 SP No.2, W16: 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 1-11-14 oc purlins
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 4-43
2 Rows at 1/3 pts 9-44
JOINTS 1 Brace at Jt(s): 24, 22, 20, 34, 32, 29, 43, 44

REACTIONS. All bearings 0-3-8.
(lb) - Max Horz 1=-373(LC 8)
Max Uplift All uplift 100 lb or less at joint(s) except 1=-398(LC 12), 15=-644(LC 13), 13=-539(LC 9)
Max Grav All reactions 250 lb or less at joint(s) except 1=1952(LC 22), 15=1427(LC 27), 25=2084(LC 18), 13=983(LC 22)

FORCES (lb) Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown

TOP CHORD 1-2=-3873/762, 2-3=-3393/700, 3-4=-2568/726, 4-5=-918/480, 5-6=-1590/924, 6-7=-1536/999, 7-8=-1536/999, 8-9=-782/541, 9-10=-2624/727, 10-11=-3173/694, 11-12=-3116/550, 12-13=-2599/769

BOT CHORD 1-39=-844/3278, 38-39=-525/3113, 37-38=-525/3113, 35-37=-249/3426, 33-35=-249/3426, 30-33=-249/3426, 28-30=-249/3426, 26-28=-249/3425, 25-26=-411/924, 23-25=-411/924, 21-23=-166/1282, 18-21=-166/1282, 17-18=-264/2508, 16-17=-264/2508, 15-16=-607/2494, 13-15=-618/2457, 34-36=-288/90, 32-34=-1392/69, 31-32=-1392/69, 29-31=-1392/69, 27-29=-1391/69, 24-27=-1391/69, 22-24=-172/1177, 20-22=-172/1177, 19-20=-102/382

WEBS 2-39=-76/261, 2-37=-849/473, 36-37=-73/925, 3-36=-63/1174, 18-19=-148/681, 10-19=-126/892, 12-16=-579/833, 12-15=-1313/687, 4-41=-2036/388, 41-43=-2019/387, 40-43=-1492/333, 40-44=-1492/333, 42-44=-2600/455, 9-42=-2622/455, 24-25=-2427/83, 22-23=-336/62, 20-21=-764/74, 18-20=-111/941, 34-37=-1335/105, 32-33=-282/0, 30-31=-317/0, 28-29=-335/0, 26-27=-465/0, 11-16=-379/256, 5-43=-647/978, 8-44=-620/1159, 6-44=-269/60, 24-26=0/3277, 23-24=-285/779

NOTES- (13-15)

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) All plates are 2x6 MT20 unless otherwise indicated.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
7) Ceiling dead load (5.0 psf) on member(s). 3-4, 9-10, 4-41, 41-43, 43-44, 42-44, 9-42; Wall dead load (5.0psf) on member(s) 3-36, 10-19
8) Bottom chord live load (75.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room: 34-36, 32-34, 31-32, 29-31, 27-29, 24-27, 22-24, 20-22, 19-20
9) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 398 lb uplift at joint 1, 644 lb uplift at joint 15 and 539 lb uplift at joint 13.
11) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
12) Attic room checked for L/360 deflection.
13) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
14) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
15) Truss Design Engineer: Julius Lee, PE, Florida P.E. License No. 34869; Address. 1109 Coastal Bay Blvd, Boynton Beach, FL 33435

LOAD CASE(S) Standard

June 24,2015

Julius Lee PE
1109 Coastal Bay
Boynton Beach,FL 33435

NOTES- (17-19)

10) Bottom chord live load (75.0 psf) and additional bottom chord dead load (10 0 psf) applied only to room 35-37 33-35, 32-33 30-32, 28-30, 25-28, 23-25, 21-23 20-21
11) WARNING. Required bearing size at joint(s) 16, 26 greater than input bearing size
12) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1836 lb uplift at joint 1 4049 lb uplift at joint 16, 5295 lb uplift at joint 26 and 219 lb uplift at joint 14
14) 'Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
15) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 3129 lb down and 771 lb up at 32-11-4 on top chord and 4737 lb down and 1168 lb up at 13-8-3 on bottom chord The design/selection of such connection device(s) is the responsibility of others.
16) Attic room checked for L/360 deflection.
17) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
18) Note: Visually graded lumber designation SPp represents new lumber design values as per SPIB
19) Truss Design Engineer: Julius Lee, PE Florida P E, License No. 34869 Address 1109 Coastal Bay Blvd Boynton Beach, FL 33435

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1 25 Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-4=-54 4-5=-64, 5-6=-54, 6-9=-54, 9-10=-54, 10-11=-64, 13-15=-54, 1-48=-10, 16-48=-255(F=-245) 14-16=-10, 37-49=-30 35-49=-275(F=-245) 21-35=-275(F=-245) 20-21=-275(F=-245), 5-10=-10
   Drag 4-37=-10, 11-20=-10
   Concentrated Loads (lb)
   Vert: 38=-1702(F) 47=-1124
   Trapezoidal Loads (plf)
   Vert. 11=-201(F=-147)-to-13=-319(F=-265)

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Boynton Beach,FL 33435

15) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
16) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
17) Truss Design Engineer: Julius Lee PE Florida P E. License No. 34869. Address 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Julius Lee PE
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE - LOT 100 PRESERVE	I10030451
687630	T05D	ATTIC TRUSS	1	3	Job Reference (optional)	
Builders FirstSource, Lake City, FL, 32055, Kim Holloway						

7.610 s Jan 29 2015 MiTek Industries, Inc. Tue Jun 23 15:48:31 2015 Page 1
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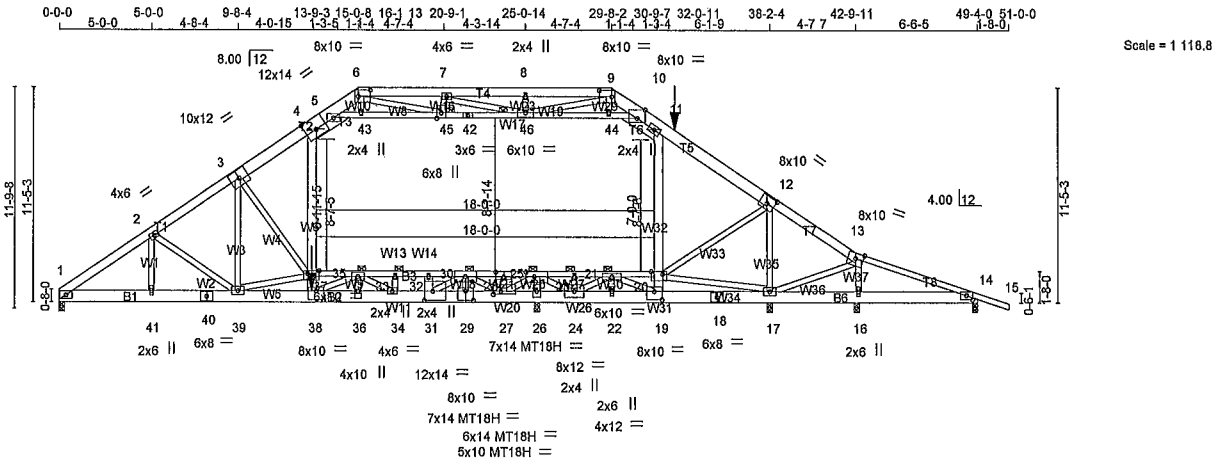


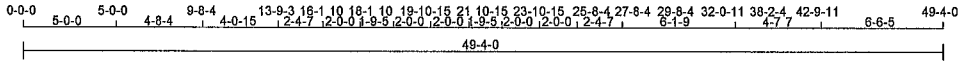


Plate Offsets (X,Y)- [4 0-7-0 Edge] [6 0-7-12 0-4-0] [9 0-7-12 0-4-0] [10:Edge 0-5-6] [12 0-5-0,0-4-8] [14 0-4-0,0-2-10], [19 0-5-0 0-5-4] [20:0-7-8,Edge], [25 0-5-5 0-3-1] [27 0-4-12 0-2-0] [28.0-5-0 0-3-0], [29:0-5-0,0-6-0], [31.0-5-8,0-6-0], [37:0-7-8,Edge], [38:0-5-0,0-5-12], [45:0-3-8,0-3-0]

LOADING (psf)		SPACING- 2-0-0		CSI		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1 25	TC	0 88	Vert(LL)	-0.48	35-37	>641	240	MT20	244/190
TCDL	7 0	Lumber DOL	1 25	BC	0.95	Vert(TL)	-0.63	35-37	>490	180	MT18H	244/190
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.95	Horz(TL)	0.07	14	n/a	n/a		
BCDL	5.0	Code FBC2014/TPI2007		(Matrix)		Attic	-0.25	20-37	868	360	Weight: 1570 lb	FT = 20%

LUMBER-
TOP CHORD 2x6 SP No.2 *Except*
T2 T5 2x8 SP 2400F 2 0E T8: 2x4 SP No 2
BOT CHORD 2x8 SP 2400F 2.0E *Except*
B3,B5: 2x4 SP M 31
WEBS 2x4 SP No 3 *Except*
W6,W32 W14. 2x6 SP No 2 W25,W22 2x4 SP M 31 W19 2x4 SP No.2
W26,W20 W11 W15 W28: 2x4 SP No.1

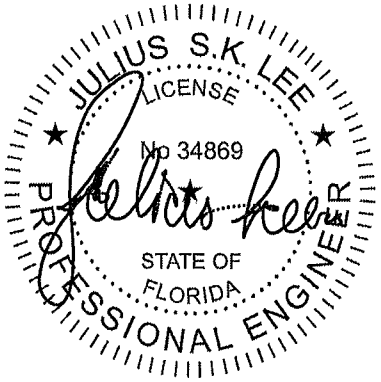
BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
6-0-0 oc bracing 26-27,24-26,22-24 19-22
JOINTS 1 Brace at Jt(s) 25, 23, 21 35, 33, 30 45, 46

REACTIONS. All bearings 0-3-8 except (jt=length) 26=0-9-4 (input: 0-3-8)
(lb) Max Horz 1=-290(LC 6)
Max Uplift All uplift 100 lb or less at joint(s) except 1=-1816(LC 5), 17=-2502(LC 4), 16=-2147(LC 5) 26=-4878(LC 5), 14=-460(LC 5)
Max Grav All reactions 250 lb or less at joint(s) except 1=8731(LC 16) 17=8533(LC 17) 16=8925(LC 16), 26=23519(LC 16) 14=1278(LC 2)

FORCES. (lb) Max. Comp./Max. Ten - All forces 250 (lb) or less except when shown
TOP CHORD 1-2=-14586/3048, 2-3=-14958/3190, 3-4=-13246/2862, 4-5=-8210/1847 5-6=-2006/528 6-7=-1553/460, 7-8=-791/1408 8-9=-791/1408, 9-10=-206/1404, 10-11=-8747/1972 11-47=-9873/2173 12-47=-13156/2928, 12-13=-10268/2358, 13-14=-3773/851
BOT CHORD 1-41=-2610/12035, 40-41=-2610/12035 39-40=-2610/12035, 38-39=-4415/20405 38-48=-5387/25390, 36-48=-5387/25390, 34-36=-5387/25390 31-34=-4567/21743, 29-31=-1915/9231 27-29=-1915/9227 26-27=-18022/3777 24-26=-18022/3777 22-24=-752/211 19-22=-752/211 18-19=-1051/4681 17-18=-1051/4681 16-17=-848/4053, 14-16=-732/3549 37-49=-10912/2399 35-49=-10898/2397 33-35=-11756/2437 32-33=-11756/2437 30-32=-11756/2437 28-30=-3000/14160, 25-28=-3000/14160 23-25=-3889/18319, 21-23=-3889/18319 20-21=-1239/5704
WEBS 3-39=-519/2417 3-37=-4489/1001 37-38=-1620/6574 4-37=-1900/8790, 19-20=-381/1462 11-20=-384/1691 12-20=-563/2042, 13-17=-988/4600 13-16=-7499/1722 5-43=-7821/1668, 43-45=-7729/1652, 42-45=-8994/1760 42-46=-8994/1760 44-46=-12755/2789 10-44=-12924/2823 25-26=-17306/3568, 23-24=-1514/270 19-21=-1244/5721 35-36=-699/2999 35-38=-5322/1138, 33-34=-1049/187 31-32=-2043/383 29-30=-869/162 27-28=-1583/286, 12-17=-6977/1847 6-43=-103/606, 9-44=-218/1116 7-45=-40/442 6-45=-1451/324, 9-46=-590/1927 7-46=-2129/478, 25-27=-3526/16835 24-25=-2380/11396, 27-30=-15654/3261 34-35=-4271/960, 30-31=-3105/14653, 37-39=-7902/1730, 17-20=-841/3851 2-41=-318/109, 2-39=-299/1070, 21-24=-8941/1935

NOTES- (18-20)
1) 3-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2x6 - 2 rows staggered at 0-6-0 oc, 2x8 - 2 rows staggered at 0-8-0 oc, 2x4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows. 2x8 - 2 rows staggered at 0-5-0 oc, 2x4 2 rows staggered at 0-5-0 oc.
Webs connected as follows: 2x4 - 1 row at 0-9-0 oc, Except member 42-10 2x4 - 1 row at 0-7-0 oc, member 26-25 2x4 1 row at 0-4-0 oc, 2x6 - 2 rows staggered at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated
3) Unbalanced roof live loads have been considered for this design.
4) Wind ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph, TCDL=4 2psf; BCDL=3 0psf; h=20ft; Cat. II, Exp C, Encl GCpi=0 18; MWFRS (envelope) porch right exposed, Lumber DOL=1 60 plate grip DOL=1 60
5) Provide adequate drainage to prevent water ponding
6) All plates are MT20 plates unless otherwise indicated
7) All plates are 5x8 MT20 unless otherwise indicated.
8) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads
9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

Continued on page 2



June 24,2015

Julius Lee PE
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE - LOT 100 PRESERVE
687630	T06	Attic Truss	1	3	I10030452
Builders FirstSource, Lake City, FL 32055, Kim Holloway					Job Reference (optional)

7.610 s Jan 29 2015 MiTek Industries, Inc. Tue Jun 23 15:48:36 2015 Page 1
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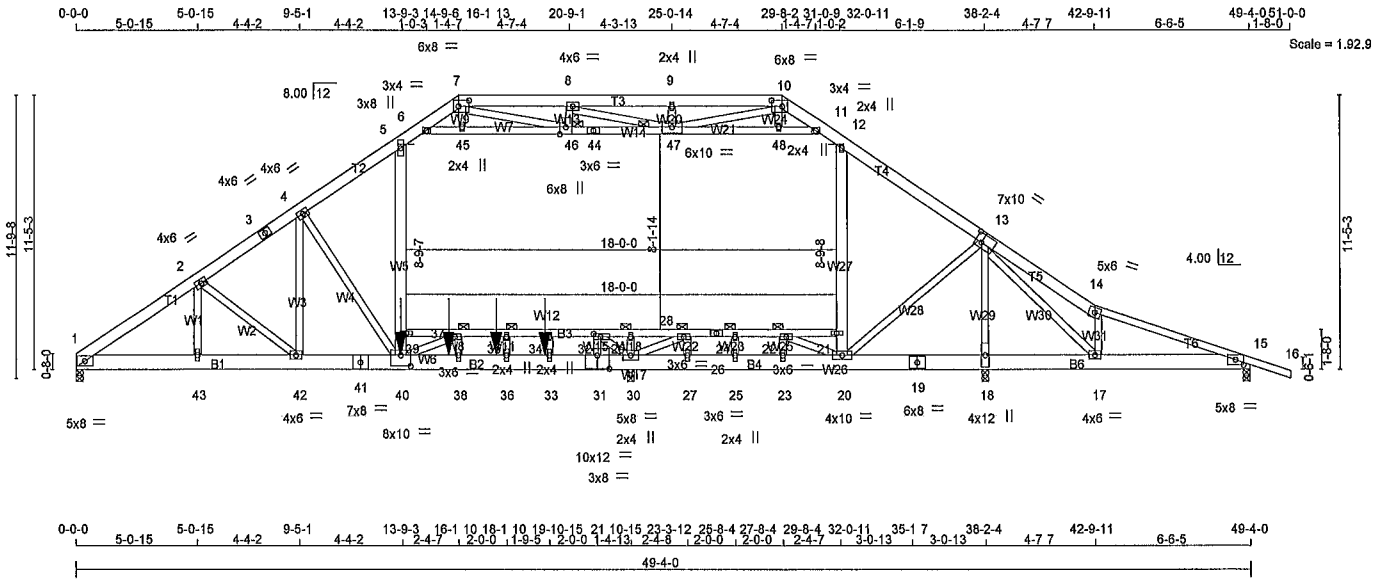

Plate Offsets (X,Y)-- [7,0-5-4,0-3-0], [10:0-5-4,0-3-0], [13,0-3-4,0-4-8], [15:0-4-0,0-2-10], [31,0-6-0,0-6-12], [32,0-3-8,0-1-8], [40:0-5-0,0-5-8], [46,0-3-8,0-3-0]

LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) l/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.25	TC 0.90	Vert(LL) -0.39 40 >718 240	MT20 244/190
TCDL 7.0	Lumber DOL 1.25	BC 0.68	Vert(TL) -0.74 40 >377 180	
BCLL 0.0 *	Rep Stress Incr NO	WB 0.68	Horz(TL) 0.06 15 n/a n/a	
BCDL 5.0	Code FBC2014/TPI2007	(Matrix-M)	Attic -0.25 21-39 881 360	Weight: 1437 lb FT = 20%

LUMBER-

TOP CHORD 2x6 SP No.2 *Except*
T6: 2x4 SP No.2
BOT CHORD 2x8 SP 2400F 2 0E *Except*
B3 B5: 2x4 SP No.2
WEBS 2x4 SP No 3 *Except*
W5,W27 2x6 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
JOINTS 1 Brace at Jt(s) 46 47 29 37 35, 22 24 28

REACTIONS All bearings 0-3-8.
(lb) Max Horz 1=-290(LC 24)
Max Uplift All uplift 100 lb or less at joint(s) except 1=-1118(LC 8) 18=-2712(LC 20), 30=-371(LC 8) 15=-1347(LC 5)
Max Grav All reactions 250 lb or less at joint(s) except 1=4438(LC 1) 18=1588(LC 17) 30=3084(LC 2), 15=3855(LC 1)

FORCES. (lb) Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown

TOP CHORD 1-2=-7211/1806 2-3=-7516/1935 3-4=-7399/1941 4-5=-6672/1723, 5-6=-4436/1143 6-7=-1109/336
7-8=-1433/649 8-9=-1279/911 9-10=-1279/911 10-11=-695/619 11-12=-4586/1193 12-13=-5985/1543,
13-14=-11263/3665, 14-15=-9816/3151

BOT CHORD 1-43=-1548/5900 42-43=-1548/5900, 41-42=-1615/6289 40-41=-1615/6289 40-55=-1239/5613,
38-55=-1239/5613, 38-56=-1239/5613 36-56=-1239/5613, 33-36=-1239/5613 31-33=-1239/5613,
30-31=-1243/5631 27-30=0/1038, 25-27=0/1038, 23-25=0/1038, 20-23=0/1038 19-20=-2102/7289
18-19=-2102/7289, 17-18=-2106/7298, 15-17=-2905/9260, 39-57=-1150/366, 37-57=-1150/366,
37-58=-691/80 35-58=-691/80 34-35=-691/80, 32-34=-691/80 29-32=-1256/4807 28-29=-1256/4807
26-28=-1372/4353, 24-26=-1372/4353, 22-24=-1372/4353 21-22=-273/883

WEBS 2-43=-513/182 2-42=-314/614 4-42=-428/1292 4-40=-2006/727 39-40=-1035/3302 5-39=-1063/3566,
20-21=-815/2455, 12-21=-794/2556, 13-20=-3155/1257, 13-17=-1278/3443, 14-17=-3153/1015,
6-45=-4058/1208, 45-46=-4034/1205, 44-46=-3596/1324, 44-47=-3596/1324 47-48=-5572/1687
11-48=-5610/1693, 13-18=-1479/2427 10-48=-49/288, 7-46=-562/767 8-47=-666/204, 10-47=-458/1392
31-32=-1016/4204, 22-23=-1808/504, 30-32=-7164/1671 28-30=-957/0, 20-22=-1227/3875, 37-40=-403/655

NOTES- (17-19)

1) 3-ply truss to be connected together with 10d (0.131"x3") nails as follows.
Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc, 2x4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows. 2x8 - 2 rows staggered at 0-9-0 oc, 2x4 - 1 row at 0-9-0 oc.
Webs connected as follows. 2x4 - 1 row at 0-9-0 oc, 2x6 - 2 rows staggered at 0-9-0 oc.
2) All loads are considered equally applied to all plies except if noted as front (F) or back (B) face in the LOAD CASE(S) section Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated
3) Unbalanced roof live loads have been considered for this design.
4) Wind ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft, Cat. II Exp C Encl GCpi=0.18; MWFRS (envelope); porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60
5) Provide adequate drainage to prevent water ponding.
6) All plates are 2x6 MT20 unless otherwise indicated.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
9) Ceiling dead load (5.0 psf) on member(s). 5-6, 11-12 6-45, 45-46, 46-47 47-48 11-48, Wall dead load (5.0psf) on member(s).5-39 12-21
10) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room 37-39 35-37, 34-35, 32-34 29-32 28-29, 24-28, 22-24, 21-22
11) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1118 lb uplift at joint 1, 2712 lb uplift at joint 18, 371 lb uplift at joint 30 and 1347 lb uplift at joint 15
13) Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

Continued on page 2

JULIUS S.K. LEE LICENSE No 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

June 24,2015

Julius Lee PE
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE - LOT 100 PRESERVE	I10030453
687630	T07	Attic Truss	5	1		
Builders FirstSource, Lake City, FL 32055, Kim Holloway					Job Reference (optional)	

7.610 s Jan 29 2015 MiTek Industries, Inc. Tue Jun 23 15:48:38 2015 Page 1
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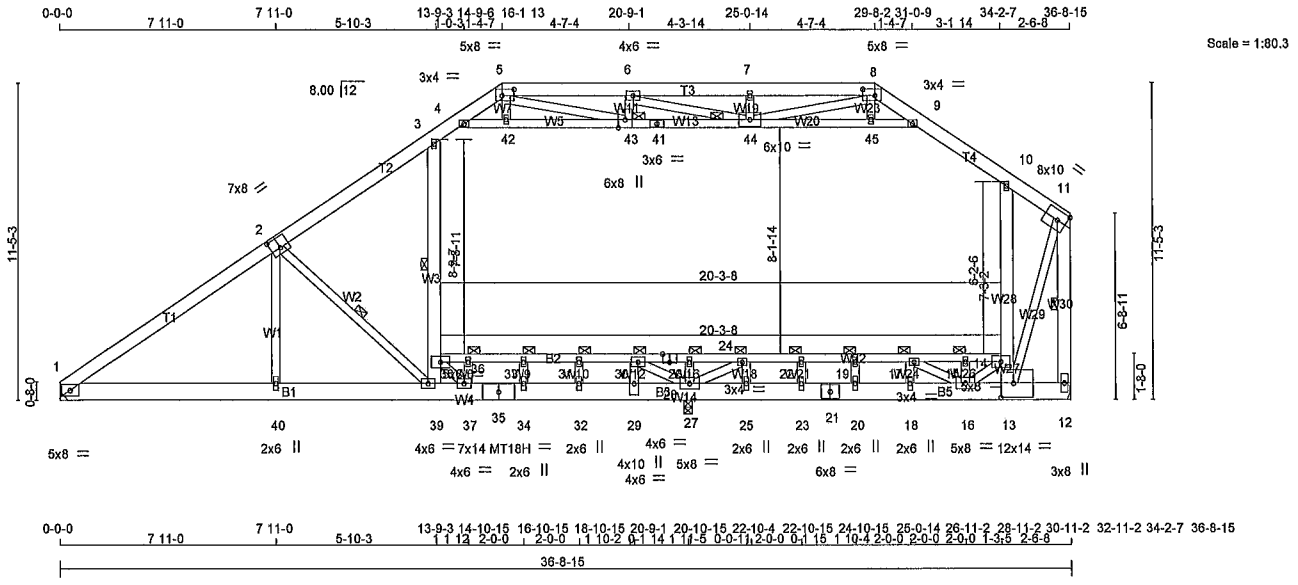


Plate Offsets (X,Y)-- [2,0-4-0,0-4-8], [5,0-5-4,0-2-12], [8,0-5-4,0-2-12], [13,0-5-8,0-6-4], [28,0-3-0,Edge], [43,0-3-8,0-3-0]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20 0	Plate Grip DOL	1.25	TC	0 64	Vert(LL)	0 47	39-40	>591	240	MT20	244/190
TCDL	7 0	Lumber DOL	1.25	BC	0.85	Vert(TL)	-0 92	39-40	>299	180	MT18H	244/190
BCLL	0.0 *	Rep Stress Incr	YES	WB	0 80	Horz(TL)	0 02	27	n/a	n/a		
BCDL	5.0	Code FBC2014/TPI2007		(Matrix-M)		Attic	-0.23	14-38	1087	360	Weight: 392 lb	FT = 20%

LUMBER-
TOP CHORD 2x6 SP No.2
BOT CHORD 2x8 SP 2400F 2.0E *Except*
B2,B4. 2x4 SP No.2
WEBS 2x4 SP No 3 *Except*
W3,W28 W30: 2x6 SP No 2

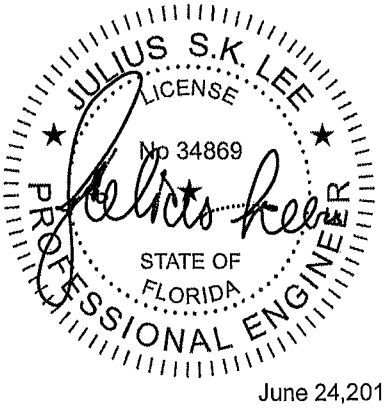
BRACING-
TOP CHORD Structural wood sheathing directly applied or 4-9-5 oc purlins except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing
WEBS 1 Row at midpt 2-39 3-38, 11-12
JOINTS 1 Brace at Jt(s): 43, 44 26, 30, 31 33 24 22 19 36, 17 15

REACTIONS. (lb/size) 1=1116/Mechanical 12=1136/Mechanical 27=1060/0-3-8 (min. 0-2-5)
Max Horz 1=408(LC 12)
Max Uplift 1=-269(LC 12) 12=-140(LC 8)
Max Grav 1=1116(LC 1) 12=1329(LC 29), 27=1839(LC 22)

FORCES (lb) - Max. Comp./Max. Ten - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-2208/557 2-3=-1391/375, 3-4=-1171/477 4-5=-1179/564 5-6=-1912/1067 6-7=-1838/1069
7-8=-1838/1069 8-9=-1019/631 9-10=-1284/478 10-11=-862/164 11-12=-2779/564
BOT CHORD 1-40=-1103/3081 39-40=-607/1863 37-39=-387/1559 35-37=-130/1208 34-35=-130/1208,
32-34=-130/1208, 29-32=-130/1208 27-29=-130/1208, 25-27=-1167/736, 23-25=-1167/736,
21-23=-1167/736, 20-21=-1167/736 18-20=-1167/736, 16-18=-1167/736, 13-16=-75/278, 36-38=-373/0
33-36=-373/0, 31-33=-373/0 30-31=-373/0, 28-30=-631/2962, 26-28=-631/2962 24-26=-631/2962,
22-24=-890/2022, 19-22=-890/2022 17 19=-890/2022 15-17=-728/1965, 14-15=-728/1965
WEBS 2-40=-173/668 2-39=-1183/556 3-38=-238/276, 13-14=-298/512, 10-14=-805/478, 4-42=-392/377
42-43=-389/382 41-43=-892/1238, 41-44=-892/1238, 44-45=-535/267 9-45=-545/261 11-13=-596/2569
5-43=-668/979, 8-44=-607/1163, 29-30=-339/1640 24-25=0/435, 37-38=-540/362, 14-16=-1707/732
27-30=-3809/643, 24-27=-1356/0, 15-16=-261/0

NOTES- (15-18)
1) Unbalanced roof live loads have been considered for this design
2) Wind ASCE 7-10 Vult=130mph (3-second gust) Vasd=101mph TCDL=4.2psf BCDL=3 0psf h=20ft; Cat. II Exp C, Encl GCpi=0 18 MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1.60 plate grip DOL=1 60
3) Provide adequate drainage to prevent water ponding
4) All plates are MT20 plates unless otherwise indicated
5) All plates are 2x4 MT20 unless otherwise indicated.
6) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads.
7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
8) Ceiling dead load (5.0 psf) on member(s). 3-4, 9-10 4-42 42-43, 43-44, 44-45 9-45 Wall dead load (5.0psf) on member(s).3-38 10-14
9) Bottom chord live load (40 0 psf) and additional bottom chord dead load (10.0 psf) applied only to room 36-38, 33-36, 31-33, 30-31 26-30 24-26, 22-24, 19-22 17-19 15-17 14-15
10) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
11) Refer to girder(s) for truss to truss connections.
12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 269 lb uplift at joint 1 and 140 lb uplift at joint 12
13) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
14) Attic room checked for L/360 deflection
15) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
16) Note Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
17) Truss Design Engineer Julius Lee PE. Florida P E License No. 34869 Address 1109 Coastal Bay Blvd Boynton Beach, FL 33435
18) Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard



June 24,2015

⚠ *WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.*
Design valid for use only with MiTek connectors. This design is based only upon parameters shown and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer Bracing shown is for lateral support of individual web members only Additional temporary bracing to insure stability during construction is the responsibility of the erector Additional permanent bracing of the overall structure is the responsibility of the building designer For general guidance regarding fabrication, quality control storage, delivery erection and bracing consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information** available from Truss Plate Institute 583 D'Onofrio Drive Madison, WI 53719

Julius Lee PE
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE - LOT 100 PRESERVE	I10030455
687630	T08	Attic Truss	1	1		
Builders FirstSource,	Lake City, FL 32055, Kim Holloway				Job Reference (optional)	

7.610 s Jan 29 2015 MiTek Industries, Inc. Tue Jun 23 15:48:44 2015 Page 1
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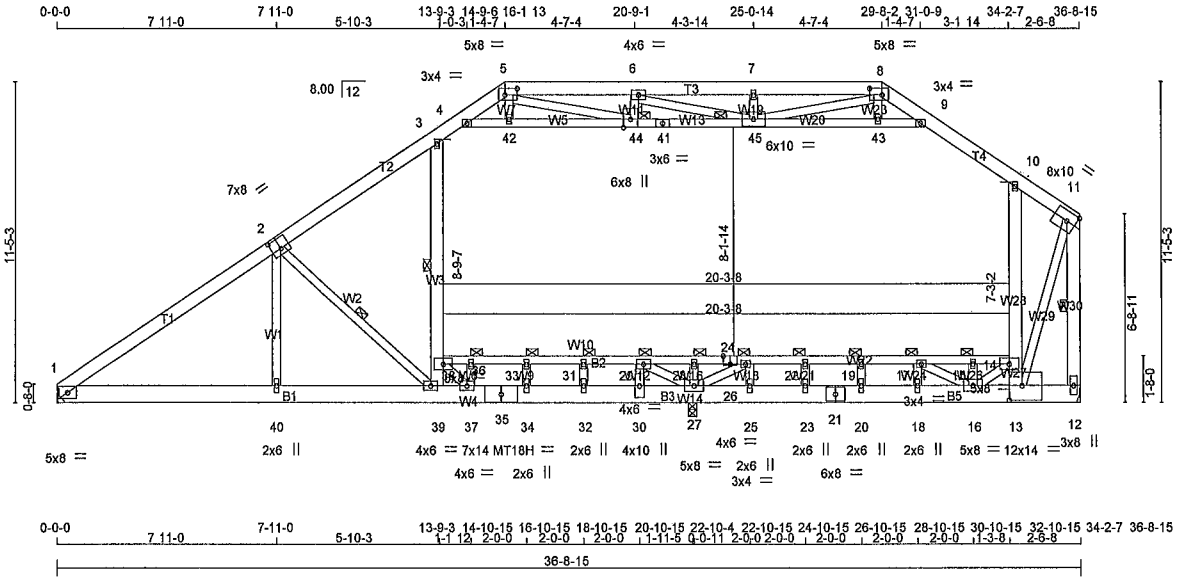

Plate Offsets (X,Y)-- [2,0-4-0,0-4-8], [5,0-5-4,0-2-12], [8,0-5-4,0-2-12], [13,0-5-8,0-6-4], [26:0-3-0,Edge], [44:0-3-8,0-3-0]

LOADING (psf)		SPACING-	2-0-0	CSI		DEFL.	In	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.64	Vert(LL)	-0.41	39-40	>674	240	MT20	244/190
TCDL	7 0	Lumber DOL	1.25	BC	0.85	Vert(TL)	-0.92	39-40	>299	180	MT18H	244/190
BCLL	0.0 *	Rep Stress Incr	YES	WB	0 80	Horz(TL)	0.02	27	n/a	n/a		
BCDL	5.0	Code FBC2014/TPI2007		(Matrix-M)		Attic	0.23	14-38	1086	360	Weight: 392 lb	FT = 20%

LUMBER-
TOP CHORD 2x6 SP No.2
BOT CHORD 2x8 SP 2400F 2 0E *Except*
B2 B4. 2x4 SP No.2
WEBS 2x4 SP No 3 *Except*
W3,W28,W30· 2x6 SP No.2

BRACING-
TOP CHORD Structural wood sheathing directly applied or 4-9-5 oc purlins except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 2-39 3-38 11-12
JOINTS 1 Brace at Jt(s): 44, 45 28 24, 22, 19 17 29, 31 33, 36, 15

REACTIONS (lb/size) 1=1116/Mechanical, 12=1136/Mechanical 27=1060/0-3-8 (min. 0-2-5)
Max Horz 1=274(LC 12)
Max Uplift 1=-107(LC 12), 12=-10(LC 8)
Max Grav 1=1116(LC 1) 12=1329(LC 29), 27=1847(LC 22)

FORCES. (lb) - Max. Comp./Max. Ten - All forces 250 (lb) or less except when shown
TOP CHORD 1-2=-2208/557 2-3=-1390/374, 3-4=-1171/477 4-5=-1180/564 5-6=-1912/1067 6-7=-1838/1062
7-8=-1838/1062 8-9=-1019/555, 9-10=-1283/478, 10-11=-861/164, 11-12=-2776/563
BOT CHORD 1-40=-1086/3082 39-40=-607/1863, 37-39=-387/1559, 35-37=-130/1208 34-35=-130/1208,
32-34=-130/1208, 30-32=-130/1208, 27-30=-130/1208, 25-27=-1166/678 23-25=-1166/678,
21-23=-1166/678, 20-21=-1166/678, 18-20=-1166/678 16-18=-1166/678, 13-16=-51/277 36-38=-373/0
33-36=-373/0 31-33=-373/0, 29-31=-373/0 28-29=-631/2962 26-28=-631/2962, 24-26=-631/2962,
22-24=-889/2021 19-22=-889/2021 17 19=-889/2021 15-17=-730/1963, 14-15=-730/1963
WEBS 2-40=-163/668, 2-39=-1183/511 13-14=-229/435 10-14=-805/479 4-42=-259/378, 42-44=-256/383
41-44=-640/1238, 41-45=-640/1238 43-45=-445/268, 9-43=-454/262, 11-13=-595/2567 5-44=-568/979
8-45=-577/1163, 24-25=0/437 14-16=-1695/730, 29-30=-339/1641 37-38=-540/362 27-29=-3809/644
24-27=-1357/0, 15-16=-262/0

NOTES- (15-18)
1) Unbalanced roof live loads have been considered for this design
2) Wind ASCE 7-10· Vult=130mph (3-second gust) Vasd=101mph, TCDL=4 2psf· BCDL=3.0psf· h=20ft; Cat. II Exp C Encl GCpi=0.18 MWFRS (envelope) and C-C Exterior(2) zone·C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1 60
3) Provide adequate drainage to prevent water ponding
4) All plates are MT20 plates unless otherwise indicated
5) All plates are 2x4 MT20 unless otherwise indicated.
6) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads.
7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
8) Ceiling dead load (5 0 psf) on member(s). 3-4 9-10, 4-42 42-44, 44-45, 43-45, 9-43, Wall dead load (5.0psf) on member(s).3-38, 10-14
9) Bottom chord live load (40 0 psf) and additional bottom chord dead load (10 0 psf) applied only to room 36-38, 33-36, 31-33, 29-31 28-29 24-26, 22-24, 19-22, 17-19 15-17 14-15
10) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
11) Refer to girder(s) for truss to truss connections.
12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 12 except (jt=lb) 1=107
13) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
14) Attic room checked for L/360 deflection
15) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
16) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
17) Truss Design Engineer· Julius Lee, PE. Florida P E License No. 34869: Address. 1109 Coastal Bay Blvd Boynton Beach, FL 33435
18) Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard

June 24,2015

Julius Lee PE
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE - LOT 100 PRESERVE
687630	T09	Attic Truss	1	2	I10030456
Builders FirstSource, Lake City, FL 32055, Kim Holloway					Job Reference (optional)

NOTES- (19-22)

15) 'Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
16) This truss has large uplift reaction(s) from gravity load case(s). Proper connection is required to secure truss against upward movement at the bearings. Building designer must provide for uplift reactions indicated
17) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 3376 lb down and 1210 lb up at 13-8-3, 499 lb down and 104 lb up at 15-8-10, 499 lb down and 104 lb up at 17-8-10, and 499 lb down and 104 lb up at 19-8-10, and 3095 lb down and 1159 lb up at 32-1-11 on bottom chord The design/selection of such connection device(s) is the responsibility of others.
18) Attic room checked for L/360 deflection.
19) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
20) Note Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
21) Truss Design Engineer· Julius Lee PE. Florida P E License No. 34869· Address. 1109 Coastal Bay Blvd Boynton Beach FL 33435
22) Use Simpson HHUS26-2 to attach Truss to Carrying member

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced)· Lumber Increase=1 25, Plate Increase=1 25
   Uniform Loads (plf)
   Vert: 1-5=-54, 5-6=-64, 6-7=-54, 7-12=-54, 12-13=-54 13-14=-64 14-15=-54 16-48=-10, 18-39=-30 6-13=-10
   Drag 5-39=-10, 14-18=-10
   Concentrated Loads (lb)
   Vert. 40=-3376(F) 17=-3095(F) 51=-499(F) 52=-499(F) 53=-499(F)

Julius Lee PE
1109 Coastal Bay
Boynton Beach FL 33435

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE - LOT 100 PRESERVE
687630	T11	MONOPITCH TRUSS	4	1	I10Q30458
Builders FirstSource, Lake City, FL 32055, Kim Holloway					Job Reference (optional)

7.610 s Jan 29 2015 MiTek Industries, Inc. Tue Jun 23 15:48:49 2015 Page 1
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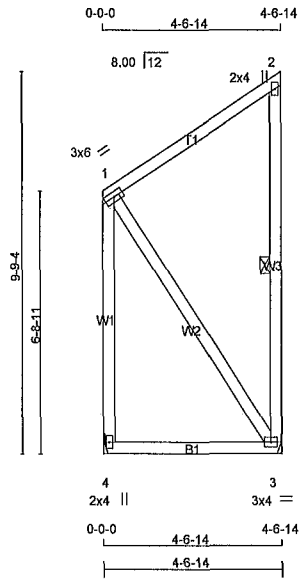

LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) l/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1 25	TC 0.40	Vert(LL) -0.02 3-4 >999 240	MT20 244/190
TCDL 7 0	Lumber DOL 1 25	BC 0 17	Vert(TL) -0.04 3-4 >999 180	
BCLL 0 0 *	Rep Stress Incr YES	WB 0.26	Horz(TL) -0.00 3 n/a n/a	
BCDL 5.0	Code FBC2014/TPI2007	(Matrix-M)		Weight: 49 lb FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 4-6-14 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 2-3

REACTIONS. (lb/size) 4=137/Mechanical 3=137/Mechanical
Max Horz 4=99(LC 12)
Max Uplift 3=-226(LC 12)
Max Grav 4=163(LC 23) 3=202(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown
WEBS 1-3=-223/285

NOTES- (8-11)
1) Wind ASCE 7 10· Vult=130mph (3-second gust) Vasd=101mph TCDL=4.2psf· BCDL=3.0psf· h=20ft; Cat. II Exp C Encl GCpi=0.18 MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown Lumber DOL=1 60 plate grip DOL=1 60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
3) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 226 lb uplift at joint 3
7) 'Semi-rigid pitchbreaks including heels' Member end fixity model was used in the analysis and design of this truss.
8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
9) Note Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
10) Truss Design Engineer· Julius Lee PE Florida P E License No. 34869 Address. 1109 Coastal Bay Blvd Boynton Beach, FL 33435
11) Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard

June 24,2015

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE - LOT 100 PRESERVE
687630	T12D	Roof Special	3	1	I10030460
Builders FirstSource, Lake City, FL 32055, Kim Holloway					Job Reference (optional)

7.610 s Jan 29 2015 MiTek Industries, Inc. Tue Jun 23 15:48:51 2015 Page 1
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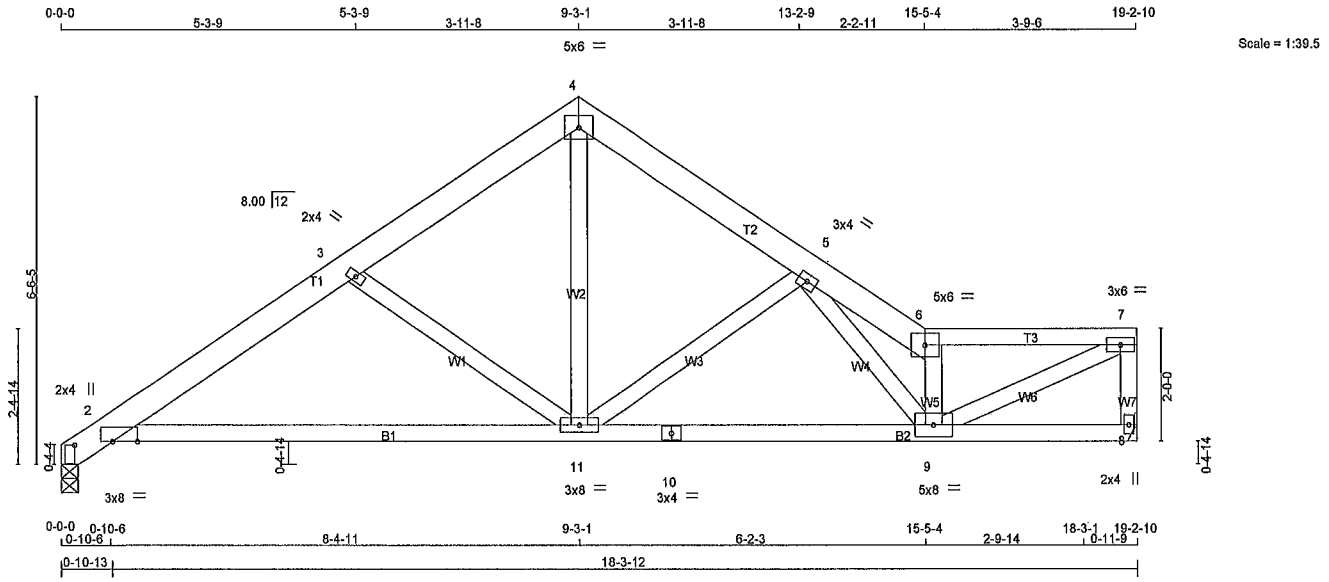

Plate Offsets (X,Y)-- [2:0-5-6,Edge], [2:0-0-12,0-8-1]

LOADING (psf)		SPACING-	2-0-0	CSI		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1 25	TC	0.40	Vert(LL)	-0.09	11-14	>999	240	MT20	244/190
TCDL	7 0	Lumber DOL	1 25	BC	0.73	Vert(TL)	-0 19	11-14	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.42	Horz(TL)	0 04	8	n/a	n/a		
BCDL	5.0	Code FBC2014/TPI2007		(Matrix-M)							Weight: 114 lb	FT = 20%

LUMBER-
TOP CHORD 2x6 SP No.2 *Except*
T3. 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No 3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 5-1-0 oc purlins
BOT CHORD Rigid ceiling directly applied or 8-10-10 oc bracing

REACTIONS (lb/size) 1=615/0-3-8 (min. 0-1-8), 8=603/Mechanical
Max Horz 1=152(LC 9)
Max Uplift 1=-141(LC 12), 8=-154(LC 13)

FORCES. (lb) Max. Comp./Max. Ten - All forces 250 (lb) or less except when shown
TOP CHORD 1-2=-385/93, 2-3=-1859/718, 3-4=-915/411 4-5=-898/404 5-6=-1568/678, 6-7=-1244/520
BOT CHORD 2-11=-720/1665 10-11=-442/1053, 9-10=-442/1053
WEBS 3-11=-480/281, 4-11=-294/673, 5-11=-521/291 7-8=-707/327 6-9=-947/453, 7-9=-579/1385, 5-9=-196/456

NOTES- (11 14)
1) Unbalanced roof live loads have been considered for this design
2) Wind ASCE 7 10· Vult=130mph (3-second gust) Vasd=101mph, TCDL=4 2psf· BCDL=3 0psf· h=20ft; Cat. II Exp C Encl GCpi=0.18 MWFRS (envelope) and C-C Exterior(2) zone·C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
5) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
7) Refer to girder(s) for truss to truss connections.
8) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 141 lb uplift at joint 1 and 154 lb uplift at joint 8.
10) 'Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
12) Note Visually graded lumber designation SPp, represents new lumber design values as per SPIB
13) Truss Design Engineer· Julius Lee, PE. Florida P E License No 34869· Address. 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
14) Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard

June 24,2015

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE - LOT 100 PRESERVE
687630	T13	Half Hip Truss	1	1	I10030461
Builders FirstSource, Lake City, FL 32055, Kim Holloway					Job Reference (optional)

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced) Lumber Increase=1 25, Plate Increase=1 25

Uniform Loads (plf)

Vert. 1-3=-54, 3-8=-54, 9-14=-10

Concentrated Loads (lb)

Vert. 3=-24(B) 6=-24(B) 13=-312(B) 17=-24(B) 18=-24(B) 19=-24(B) 20=-24(B) 21=-24(B) 22=-24(B) 23=-24(B) 24=-24(B) 25=-24(B) 26=-24(B) 27=-123(B) 28=-123(B) 29=-123(B) 30=-123(B) 31=-123(B) 32=-123(B) 33=-123(B) 34=-123(B) 35=-123(B) 36=-123(B) 37=-123(B)

Julius Lee PE
1109 Coastal Bay
Boynton Beach FL 33435

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE - LOT 100 PRESERVE	I10030463
687630	T15	Special Truss	3	1		
Builders FirstSource, Lake City, FL 32055, Kim Holloway					Job Reference (optional)	

7.610 s Jan 29 2015 MiTek Industries, Inc. Tue Jun 23 15:48:55 2015 Page 1
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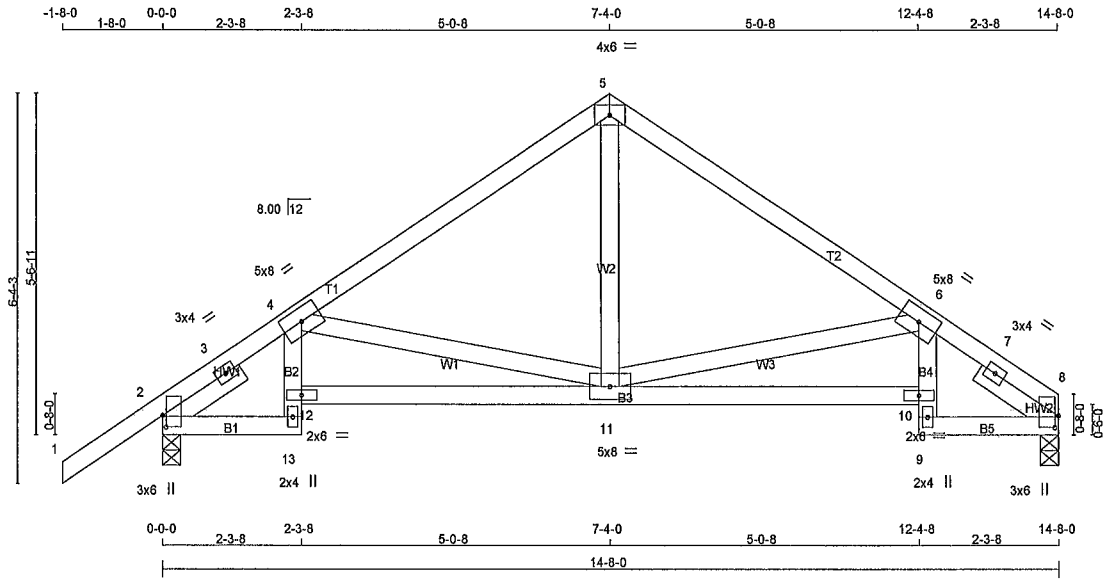

Plate Offsets (X,Y)- [2,0-2-5,0-0-11], [8,0-2-5,0-0-11]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0 29	Vert(LL)	-0 03	10-11	>999	240	MT20	244/190
TCDL	7 0	Lumber DOL	1.25	BC	0 71	Vert(TL)	-0 07	10-11	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0 29	Horz(TL)	0.05	8	n/a	n/a		
BCDL	5.0	Code FBC2014/TPI2007		(Matrix-M)							Weight: 82 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2 *Except*
B2,B4. 2x4 SP No.3
WEBS 2x4 SP No.3
SLIDER Left 2x4 SP No 3 1-6-0, Right 2x4 SP No.3 1-6-0

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing

REACTIONS. (lb/size) 8=464/0-3-8 (min 0-1-8) 2=564/0-3-8 (min 0-1-8)
Max Horz 2=144(LC 9)
Max Uplift 8=-112(LC 13), 2=-147(LC 12)

FORCES (lb) Max. Comp./Max. Ten - All forces 250 (lb) or less except when shown
TOP CHORD 2-3=-367/108 3-4=-848/336, 4-5=-702/295, 5-6=-702/295, 6-7=-859/363, 7-8=-392/171
BOT CHORD 2-13=-228/642, 11-12=-364/952 10-11=-394/964 8-9=-257/653
WEBS 5-11=-121/341, 6-11=-547/324, 4-11=-534/293

NOTES- (8-10)
1) Unbalanced roof live loads have been considered for this design
2) Wind ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph TCDL=4 2psf BCDL=3.0psf h=20ft; Cat. II Exp C Encl GCpi=0 18, MWFRS (envelope) and C-C Exterior(2) zone C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1 60
3) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
5) All bearings are assumed to be SP No 2 crushing capacity of 565 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 112 lb uplift at joint 8 and 147 lb uplift at joint 2
7) 'Semi-rigid pitchbreaks including heels' Member end fixity model was used in the analysis and design of this truss.
8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
9) Note Visually graded lumber designation SPp represents new lumber design values as per SPIB.
10) Truss Design Engineer Julius Lee PE Florida P E. License No. 34869 Address 1109 Coastal Bay Blvd Boynton Beach FL 33435

LOAD CASE(S) Standard

June 24,2015

Julius Lee PE.
1109 Coastal Bay
Boynton Beach FL 33435

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE - LOT 100 PRESERVE	I10030465
687630	T17	Half Hip Truss	1	2	Job Reference (optional)	
Builders FirstSource, Lake City, FL 32055, Kim Holloway						

7.610 s Jan 29 2015 MiTek Industries, Inc. Tue Jun 23 15:48 57 2015 Page 1
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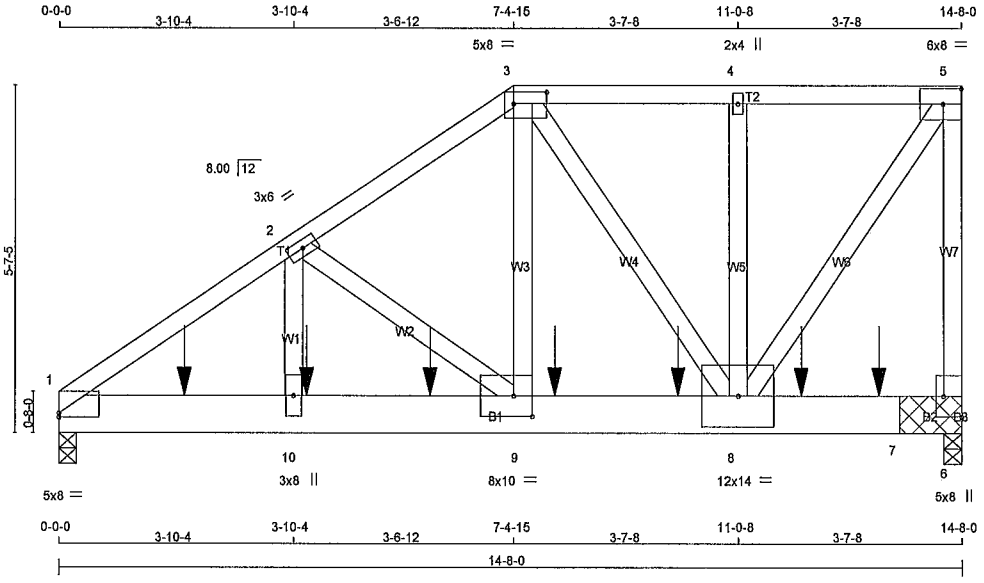


Plate Offsets (X,Y)- [1,0-0-0,0-0-13], [3,0-6-4,0-2-4], [6,Edge,0-3-8], [9,0-3-8,0-4-0]

LOADING (psf)		SPACING-	2-0-0	CSI		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1 25	TC	0.80	Vert(LL)	-0 05	9-10	>999	240	MT20	244/190
TCDL	7 0	Lumber DOL	1 25	BC	0.38	Vert(TL)	-0 12	9-10	>999	180		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.62	Horz(TL)	0 02	6	n/a	n/a		
BCDL	5 0	Code FBC2014/TPI2007		(Matrix-M)							Weight: 247 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x8 SP 2400F 2.0E
WEBS 2x4 SP No.3 *Except*
W4,W6. 2x4 SP No.2

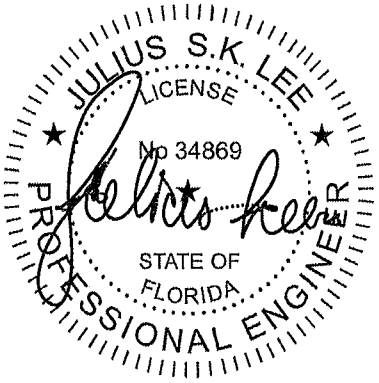
BRACING-
TOP CHORD Structural wood sheathing directly applied or 5-1-6 oc purlins except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 1=4228/0-3-8 (min 0-2-8), 6=7367/(0-3-8 + bearing block) (req 0-4-6)
Max Horz 1=172(LC 8)
Max Uplift 1=-1021(LC 8) 6=-1786(LC 5)

FORCES (lb) - Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown
TOP CHORD 1-2=-5951/1429 2-3=-4308/1041 3-4=-3114/747 4-5=-3114/747 5-6=-4669/1138
BOT CHORD 1-14=-1305/4901 10-14=-1305/4901 10-15=-1305/4901 9-15=-1305/4901 9-16=-913/3624 16-17=-913/3624, 8-17=-913/3624
WEBS 2-10=-426/1738 2-9=-1693/515 3-9=-757/2899 3-8=-896/312 5-8=-1320/5506

NOTES- (13-15)
1) 2-ply truss to be connected together with 10d (0 131"x3") nails as follows.
Top chords connected as follows. 2x4 1 row at 0-9-0 oc.
Bottom chords connected as follows 2x8 - 2 rows staggered at 0-3-0 oc.
Webs connected as follows. 2x4 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section Ply to ply connections have been provided to distribute only loads noted as (F) or (B) unless otherwise indicated.
3) 2x8 SP 2400F 2.0E bearing block 12" long at jt. 6 attached to each face with 4 rows of 10d (0 131 'x3") nails spaced 3" o.c. 16 Total fasteners per block. Bearing is assumed to be SP No.2
4) Unbalanced roof live loads have been considered for this design
5) Wind ASCE 7-10· Vult=130mph (3-second gust) Vasd=101mph TCDL=4.2psf· BCDL=3 0psf· h=20ft; Cat. II Exp C, Encl GCpi=0.18· MWFRS (envelope)· Lumber DOL=1 60 plate grip DOL=1 60
6) Provide adequate drainage to prevent water ponding
7) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
9) All bearings are assumed to be SP No 2 crushing capacity of 565 psi
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1021 lb uplift at joint 1 and 1786 lb uplift at joint 6.
11) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
12) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1109 lb down and 285 lb up at 2-0-12 1109 lb down and 285 lb up at 4-0-12 1109 lb down and 285 lb up at 6-0-12 1109 lb down and 285 lb up at 8-0-12, 1109 lb down and 285 lb up at 10-0-12, and 1109 lb down and 121 lb up at 12-0-12, and 4010 lb down and 1046 lb up at 13-4-0 on bottom chord The design/selection of such connection device(s) is the responsibility of others.
13) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
14) Note: Visually graded lumber designation SPp represents new lumber design values as per SPIB
15) Truss Design Engineer· Julius Lee PE Florida P E License No. 34869 Address. 1109 Coastal Bay Blvd Boynton Beach FL 33435

LOAD CASE(S) Standard
1) Dead + Roof Live (balanced): Lumber Increase=1.25 Plate Increase=1 25
Uniform Loads (plf)
Vert: 1-3=-54, 3-5=-54, 6-11=-10
Concentrated Loads (lb)
Vert: 10=-1109(B) 14=-1109(B) 15=-1109(B) 16=-1109(B) 17=-1109(B) 18=-1109(B) 19=-4010(B)



June 24,2015

⚠ *WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MiTEK REFERENCE PAGE MII-7473 BEFORE USE.*
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer Bracing shown is for lateral support of individual web members only Additional temporary bracing to insure stability during construction is the responsibility of the erector Additional permanent bracing of the overall structure is the responsibility of the building designer For general guidance regarding fabrication, quality control, storage, delivery erection and bracing consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive Madison, WI 53719.

Julius Lee PE
1109 Coastal Bay
Boynton Beach FL 33435

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE - LOT 100 PRESERVE
687630	T18G	GABLE	1	1	I10030467
Builders FirstSource, Lake City, FL 32055, Kim Holloway					Job Reference (optional)

7.610 s Jan 29 2015 MiTek Industries, Inc. Tue Jun 23 15:48:59 2015 Page 1
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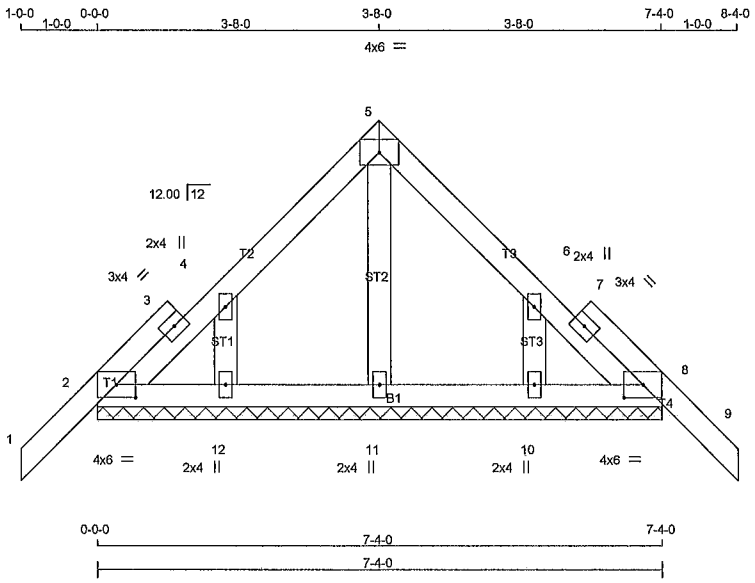

Plate Offsets (X,Y)-- [2.0-3-1,0-2-0], [8.0-3-1,0-2-0]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20 0	Plate Grip DOL	1 25	TC	0 10	Vert(LL)	-0 00	9	n/r	120	MT20	244/190
TCDL	7 0	Lumber DOL	1.25	BC	0 03	Vert(TL)	-0 00	9	n/r	120		
BCLL	0 0 *	Rep Stress Incr	YES	WB	0.05	Horz(TL)	0 00	8	n/a	n/a		
BCDL	5.0	Code FBC2014/TPI2007		(Matrix)							Weight: 42 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 7-4-0.
(lb) Max Horz 2=-135(LC 10)
Max Uplift All uplift 100 lb or less at joint(s) 2 8 except 12=-134(LC 12) 10=-137(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 2 8, 11 12 10

FORCES. (lb) Max. Comp./Max. Ten All forces 250 (lb) or less except when shown

NOTES- (11-13)
1) Unbalanced roof live loads have been considered for this design
2) Wind ASCE 7 10· Vult=130mph (3-second gust) Vasd=101mph TCDL=4.2psf· BCDL=3.0psf· h=20ft; Cat. II Exp C, Encl GCpi=0.18, MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only For studs exposed to wind (normal to the face) see Standard Industry Gable End Details as applicable or consult qualified building designer as per ANSI/TPI 1
4) Gable requires continuous bottom chord bearing
5) Gable studs spaced at 2-0-0 oc.
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
8) All bearings are assumed to be SP No 2 crushing capacity of 565 psi
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 8 except (jt=lb) 12=134 10=137
10) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss
11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
12) Note Visually graded lumber designation SPp represents new lumber design values as per SPIB.
13) Truss Design Engineer· Julius Lee PE Florida P E License No 34869· Address 1109 Coastal Bay Blvd Boynton Beach, FL 33435

LOAD CASE(S) Standard

June 24,2015

Julius Lee PE
1109 Coastal Bay
Boynton Beach FL 33435

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE - LOT 100 PRESERVE
687630	T20	Monopitch	3	1	I10030469
Builders FirstSource, Lake City, FL 32055, Kim Holloway					Job Reference (optional)

7.610 s Jan 29 2015 MiTek Industries, Inc. Tue Jun 23 15:49:00 2015 Page 1
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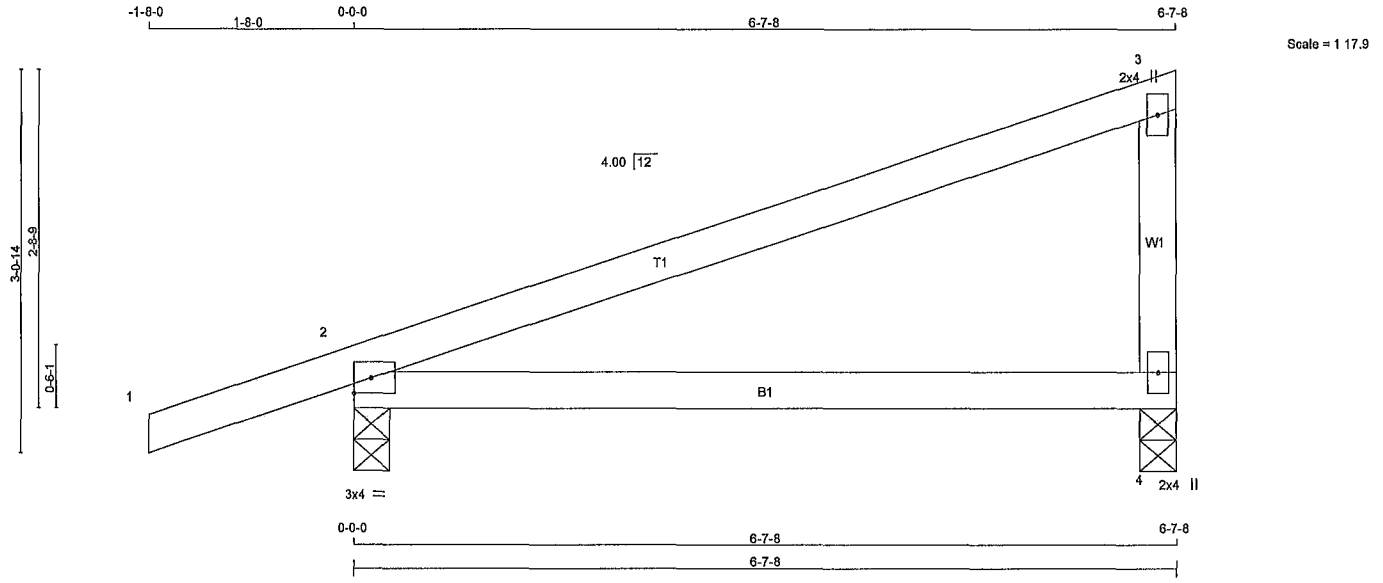

LOADING (psf)		SPACING-	2-0-0	CSI		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1 25	TC	0 67	Vert(LL)	0.25	4-7	>305	240	MT20	244/190
TCDL	7 0	Lumber DOL	1 25	BC	0.66	Vert(TL)	0.21	4-7	>378	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0 00	Horz(TL)	-0 03	2	n/a	n/a		
BCDL	5 0	Code FBC2014/TPI2007		(Matrix-M)							Weight: 26 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No 2
WEBS 2x4 SP No 3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 4=196/0-3-8 (min. 0-1-8), 2=309/0-3-8 (min 0-1-8)
Max Horz 2=144(LC 8)
Max Uplift 4=-200(LC 8), 2=-279(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-220/276

NOTES- (7-9)
1) Wind ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph, TCDL=4.2psf BCDL=3.0psf h=20ft; Cat. II Exp C, Encl GCpi=0 18, MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1 60 plate grip DOL=1 60
2) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads
3) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 4=200, 2=279
6) 'Semi-rigid pitchbreaks including heels' Member end fixity model was used in the analysis and design of this truss.
7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
8) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
9) Truss Design Engineer: Julius Lee, PE Florida P E. License No. 34869 Address. 1109 Coastal Bay Blvd. Boynton Beach FL 33435

LOAD CASE(S) Standard

June 24,2015

Julius Lee PE
1109 Coastal Bay
Boynton Beach FL 33435

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1 25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-9=-54, 10-20=-20

Concentrated Loads (lb)

Vert: 14=-7435(F) 12=-117(F) 11=-117(F) 21=-1122(F) 22=-1122(F) 23=-1122(F) 24=-1122(F) 25=-1122(F) 26=-1122(F) 27=-117(F) 28=-117(F)

Julius Lee PE
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE - LOT 100 PRESERVE
687630	TG03	FLAT TRUSS	1	2	I10030472
Builders FirstSource, Lake City, FL 32055, Kim Holloway					Job Reference (optional)

7.610 s Jan 29 2015 MiTek Industries, Inc. Tue Jun 23 15:49 04 2015 Page 1
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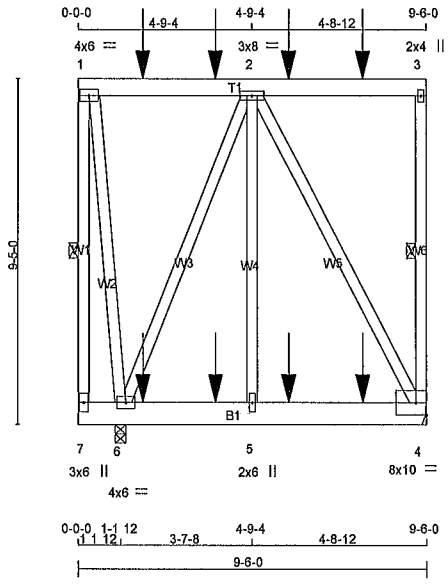


Scale = 1:60 7

LOADING (psf)	SPACING- 2-0-0	CSI	DEFL. in (loc) l/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1 00	TC 0.89	Vert(LL) -0.02 4-5 >999 240	MT20 244/190
TCDL 7 0	Lumber DOL 1 00	BC 0.14	Vert(TL) -0.04 4-5 >999 180	
BCLL 0.0 *	Rep Stress Incr NO	WB 0.85	Horz(TL) 0.00 4 n/a n/a	
BCDL 10.0	Code FBC2014/TPI2007	(Matrix-M)		Weight: 257 lb FT = 20%

LUMBER-
TOP CHORD 2x6 SP No 2
BOT CHORD 2x8 SP 2400F 2 0E
WEBS 2x4 SP No 3

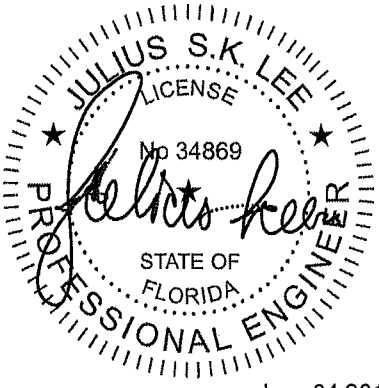
BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 1-7 3-4

REACTIONS. (lb/size) 6=3391/0-3-8 (min 0-2-0), 4=3105/Mechanical
Max Uplift 6=-1310(LC 4) 4=-1149(LC 4)

FORCES. (lb) - Max. Comp./Max. Ten - All forces 250 (lb) or less except when shown
TOP CHORD 1-7=-526/235, 3-4=-858/240
BOT CHORD 6-12=-315/837 12-13=-315/837 5-13=-315/837 5-14=-315/837 14-15=-315/837 15-16=-315/837 4-16=-315/837
WEBS 1-6=-341/83, 2-6=-2311/833, 2-5=-524/712 2-4=-1801/678

NOTES- (13-16)
1) 2-ply truss to be connected together with 10d (0 131"x3 ') nails as follows.
Top chords connected as follows. 2x4 - 1 row at 0-9-0 oc, 2x6 - 2 rows staggered at 0-9-0 oc.
Bottom chords connected as follows: 2x8 - 2 rows staggered at 0-9-0 oc.
Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies except if noted as front (F) or back (B) face in the LOAD CASE(S) section Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Wind ASCE 7-10· Vult=130mph (3-second gust) Vasd=101mph TCDL=4.2psf· BCDL=3 0psf· h=20ft; Cat. II Exp C Encl GCpi=0 18, MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60
4) Provide adequate drainage to prevent water ponding
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10 0psf
7) All bearings are assumed to be SP No 2 crushing capacity of 565 psi
8) Refer to girder(s) for truss to truss connections.
9) Provide metal plate or equivalent at bearing(s) 4 to support reaction shown
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 6=1310 4=1149
11) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
12) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 532 lb down and 136 lb up at 1-9-3, 532 lb down and 136 lb up at 3-9-3, and 532 lb down and 136 lb up at 5-9-3 and 532 lb down and 136 lb up at 7-9-3 on top chord and 166 lb down and 246 lb up at 1-9-3, 166 lb down and 246 lb up at 3-9-3, and 166 lb down and 246 lb up at 5-9-3 and 166 lb down and 246 lb up at 7-9-3 on bottom chord. The design/selection of such connection device(s) is the responsibility of others
13) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
14) Note Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
15) Truss Design Engineer· Julius Lee PE Florida P E License No. 34869· Address. 1109 Coastal Bay Blvd Boynton Beach FL 33435
16) Use Simpson HHUS26-2 to attach Truss to Carrying member

LOAD CASE(S) Standard
1) Dead + Roof Live (balanced): Lumber Increase=1 00, Plate Increase=1 00
Uniform Loads (plf)
Vert: 1-3=-304(F=-250), 5-7=-20 4-5=-220(B=-200)
Concentrated Loads (lb)
Vert: 8=-532 9=-532 10=-532 11=-532 12=-117(F) 13=-117(F) 14=-117(F) 16=-117(F)



June 24,2015

Julius Lee PE
1109 Coastal Bay
Boynton Beach FL 33435

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE LOT 100 PRESERVE	I10030474
687630	TG05	ROOF TRUSS	3	1		
Builders FirstSource, Lake City, FL 32055, Kim Holloway					Job Reference (optional)	

7.610 s Jan 29 2015 MiTek Industries, Inc. Tue Jun 23 15:49:06 2015 Page 1
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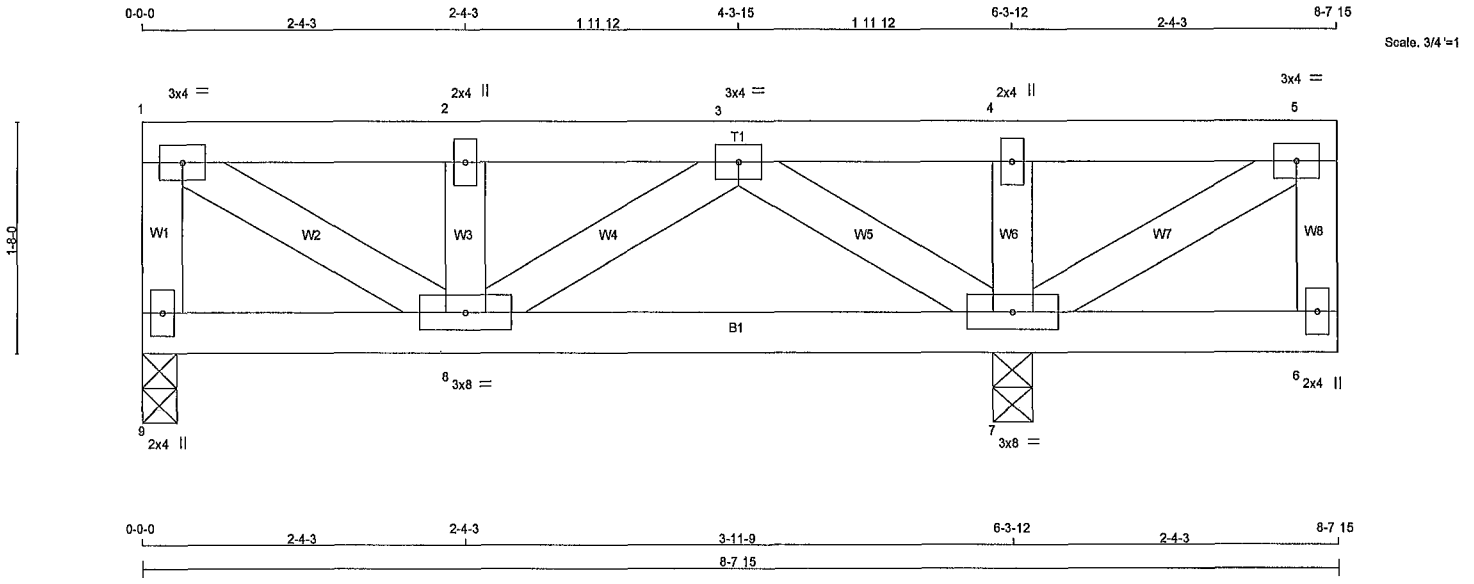

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	40.0	Plate Grip DOL	1 00	TC	0.17	Vert(LL)	-0.01	7-8	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1 00	BC	0.10	Vert(TL)	-0 01	7-8	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.16	Horz(TL)	0 00	7	n/a	n/a		
BCDL	5.0	Code FBC2014/TPI2007		(Matrix-M)							Weight: 45 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

REACTIONS. (lb/size) 9=296/0-3-0 (min. 0-1-8) 7=625/0-3-8 (min. 0-1-8)
Max Uplift 9=-54(LC 8) 7=-115(LC 8)

FORCES (lb) - Max. Comp./Max. Ten - All forces 250 (lb) or less except when shown
TOP CHORD 1-9=-280/142, 1-2=-302/141 2-3=-302/141
WEBS 1-8=-155/333, 3-7=-423/221

NOTES- (8-10)
1) Wind ASCE 7 10, Vult=130mph (3-second gust) Vasd=101mph TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II Exp C, Encl GCpi=0 18, MWFRS (envelope) and C-C Exterior(2) zone C-C for members and forces & MWFRS for reactions shown Lumber DOL=1 60 plate grip DOL=1 60
2) Provide adequate drainage to prevent water ponding
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
4) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 9 except (jt=lb) 7=115.
7) 'Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
9) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
10) Truss Design Engineer: Julius Lee PE Florida P E License No 34869: Address 1109 Coastal Bay Blvd. Boynton Beach FL 33435

LOAD CASE(S) Standard

June 24,2015

Julius Lee PE.
1109 Coastal Bay
Boynton Beach FL 33435

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE - LOT 100 PRESERVE	I10030476
687630	TG07	Flat	2	1		
Builders FirstSource, Lake City, FL 32055, Kim Holloway					Job Reference (optional)	

7.610 s Jan 29 2015 MiTek Industries, Inc. Tue Jun 23 15:49:07 2015 Page 1
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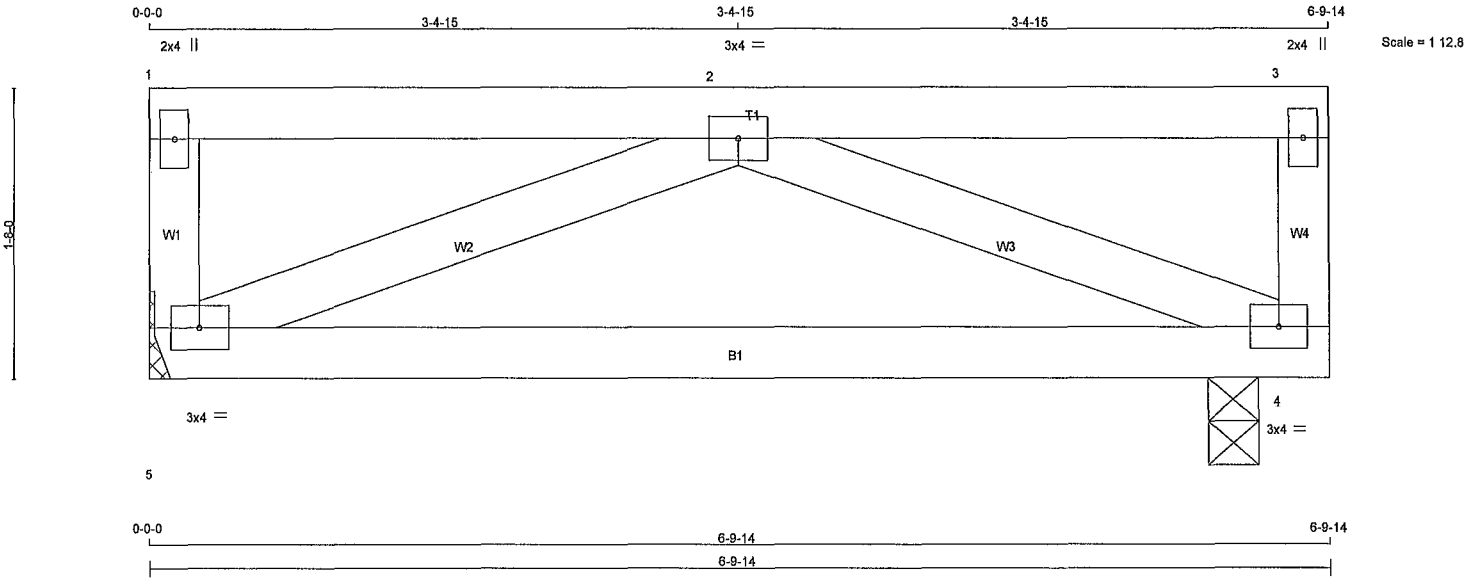


LOADING (psf)	SPACING- 2-0-0	CSI	DEFL. in (loc) l/defl L/d	PLATES GRIP
TCLL 40 0	Plate Grip DOL 1 00	TC 0.28	Vert(LL) -0 11 4-5 >718 240	MT20 244/190
TCDL 10 0	Lumber DOL 1 00	BC 0.44	Vert(TL) -0.19 4-5 >410 180	
BCLL 0.0 *	Rep Stress Incr YES	WB 0 16	Horz(TL) 0 01 4 n/a n/a	
BCDL 5.0	Code FBC2014/TPI2007	(Matrix-M)		Weight: 33 lb FT = 20%

LUMBER-
TOP CHORD 2x4 SP No 2
BOT CHORD 2x4 SP No 2
WEBS 2x4 SP No 3

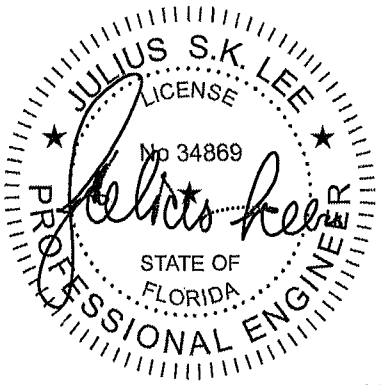
BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-9-14 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 5=359/Mechanical 4=359/0-3-8 (min. 0-1-8)
Max Uplift 5=-66(LC 8) 4=-66(LC 8)

FORCES. (lb) Max. Comp./Max. Ten - All forces 250 (lb) or less except when shown.
BOT CHORD 4-5=-307/472
WEBS 2-5=-513/334 2-4=-513/334

NOTES- (9-12)
1) Wind ASCE 7 10· Vult=130mph (3-second gust) Vasd=101mph· TCDL=4.2psf· BCDL=3.0psf· h=20ft, Cat. II Exp C, Encl GCpi=0 18 MWFRS (envelope) and C-C Exterior(2) zone·C-C for members and forces & MWFRS for reactions shown Lumber DOL=1 60 plate grip DOL=1 60
2) Provide adequate drainage to prevent water ponding.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
4) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
5) All bearings are assumed to be SP No 2 crushing capacity of 565 psi
6) Refer to girder(s) for truss to truss connections.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 4
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
10) Note Visually graded lumber designation SPp represents new lumber design values as per SPIB.
11) Truss Design Engineer· Julius Lee PE Florida P E. License No. 34869 Address. 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
12) Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard



June 24,2015

Julius Lee PE
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE - LOT 100 PRESERVE	I10030478
687630	V01	Valley Truss	1	1		
Builders FirstSource, Lake City, FL 32055, Kim Holloway					Job Reference (optional)	

7.610 s Jan 29 2015 MiTek Industries, Inc. Tue Jun 23 15:49:09 2015 Page 1
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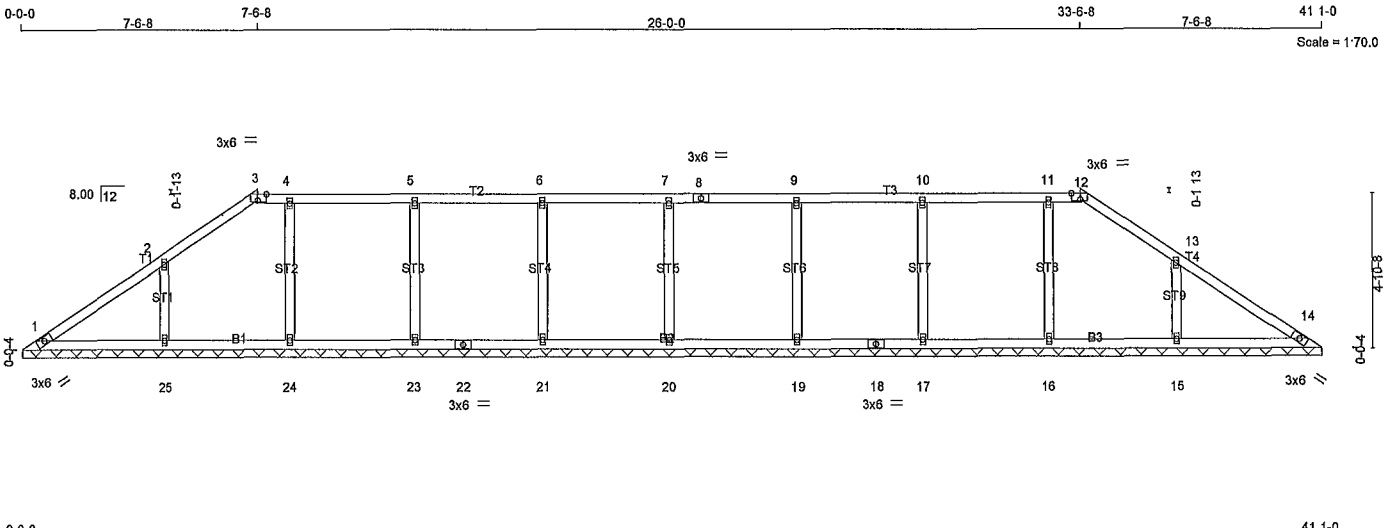


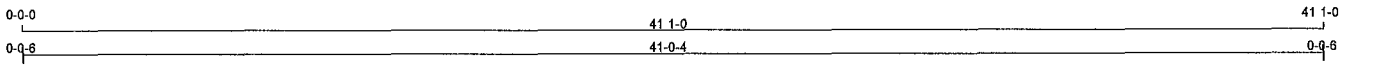


Plate Offsets (X,Y)- [3,0-3-5,Edge], [12,0-3-5,Edge]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1 25	TC	0 21	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	7 0	Lumber DOL	1 25	BC	0.14	Vert(TL)	n/a		n/a	999		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.10	Horz(TL)	0 01	14	n/a	n/a		
BCDL	5.0	Code FBC2014/TPI2007		(Matrix)							Weight: 177 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No 2
BOT CHORD 2x4 SP No 2
OTHERS 2x4 SP No 3

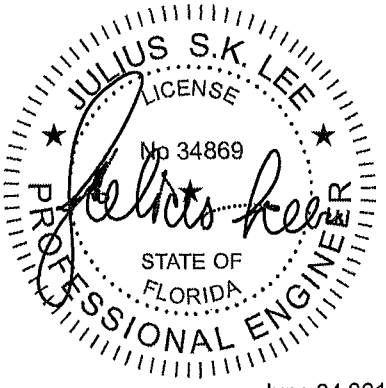
BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 41-0-4.
(lb) Max Horz 1=118(LC 9)
Max Uplift All uplift 100 lb or less at joint(s) 1 20, 21 23 24, 19 17 16 except 25=-155(LC 12), 15=-153(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 1 14 except 20=337(LC 27) 21=335(LC 27) 23=345(LC 28), 24=320(LC 27), 25=336(LC 21) 19=335(LC 28) 17=345(LC 27) 16=320(LC 28), 15=333(LC 22)

FORCES. (lb) - Max. Comp./Max. Ten - All forces 250 (lb) or less except when shown
WEBS 2-25=-366/243 13-15=-366/243

NOTES- (11-13)
1) Unbalanced roof live loads have been considered for this design
2) Wind ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph TCDL=4.2psf; BCDL=3 0psf; h=20ft; Cat. II Exp C, Encl , GCpi=0 18, MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1 60
3) Provide adequate drainage to prevent water ponding.
4) All plates are 2x4 MT20 unless otherwise indicated
5) Gable requires continuous bottom chord bearing
6) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads.
7) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members with BCDL = 5.0psf
8) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 20 21 23 24, 19 17 16 except (jt=lb) 25=155 15=153.
10) 'Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
12) Note Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
13) Truss Design Engineer: Julius Lee, PE Florida P E. License No 34869 Address 1109 Coastal Bay Blvd. Boynton Beach FL 33435

LOAD CASE(S) Standard



June 24,2015

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE - LOT 100 PRESERVE
667630	V03	Valley Truss	1	1	I10030480
Builders FirstSource, Lake City, FL, 32055, Kim Holloway					Job Reference (optional)

7.610 s Jan 29 2015 MiTek Industries, Inc. Tue Jun 23 15:49:12 2015 Page 1
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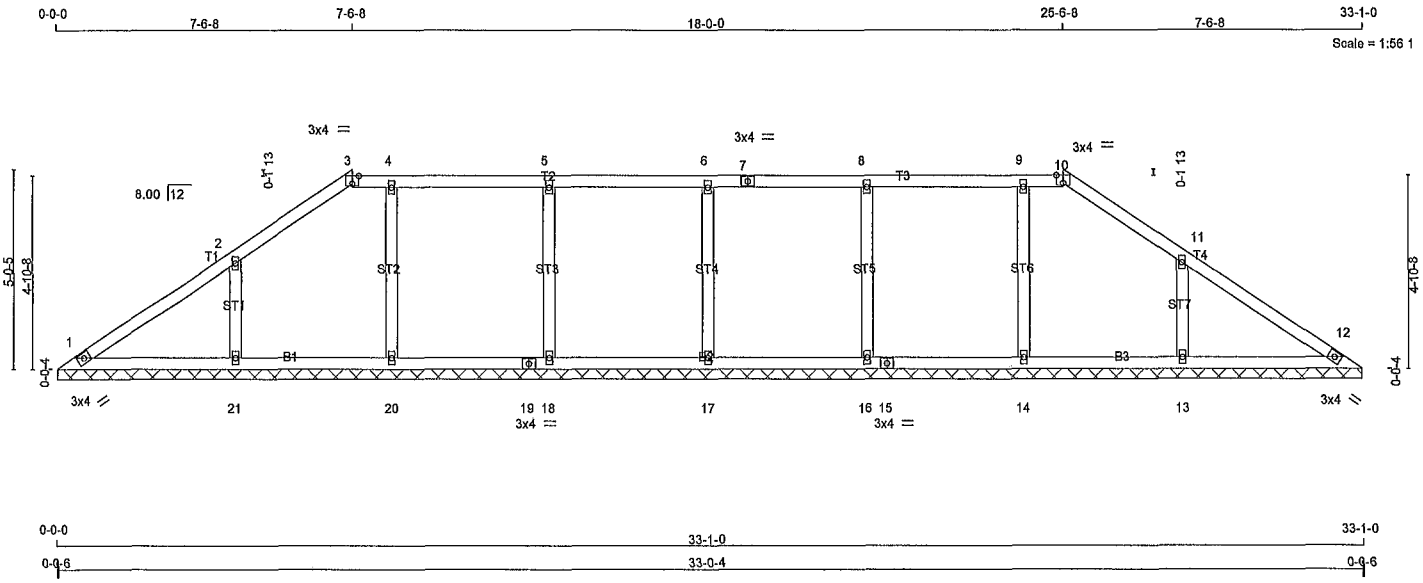


Plate Offsets (X,Y)- [3,0-2-0,Edge], [10,0-2-0,Edge]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.21	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	7 0	Lumber DOL	1.25	BC	0.14	Vert(TL)	n/a	-	n/a	999		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0 10	Horz(TL)	0 00	12	n/a	n/a		
BCDL	5.0	Code FBC2014/TPI2007		(Matrix)							Weight: 140 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

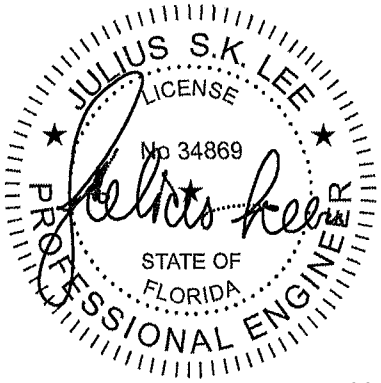
BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 33-0-4.
(lb) Max Horz 1=-118(LC 8)
Max Uplift All uplift 100 lb or less at joint(s) 1 17 18 20 16, 14 except 21=-155(LC 12) 13=-153(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 1 12 except 17=333(LC 2), 18=346(LC 28) 20=319(LC 27), 21=336(LC 21) 16=346(LC 27), 14=319(LC 28) 13=333(LC 22)

FORCES (lb) Max. Comp./Max. Ten - All forces 250 (lb) or less except when shown
WEBS 2-21=-365/243 11-13=-365/243

NOTES- (11 13)
1) Unbalanced roof live loads have been considered for this design.
2) Wind ASCE 7 10 Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II Exp C Encl GCpi=0 18, MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1 60
3) Provide adequate drainage to prevent water ponding
4) All plates are 2x4 MT20 unless otherwise indicated
5) Gable requires continuous bottom chord bearing
6) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads.
7) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5 0psf
8) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 17 18, 20, 16, 14 except (jt=lb) 21=155, 13=153.
10) Semi-rigid pitchbreaks including heels' Member end fixity model was used in the analysis and design of this truss.
11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
12) Note Visually graded lumber designation SPp, represents new lumber design values as per SPIB
13) Truss Design Engineer: Julius Lee PE Florida P E License No. 34869 Address 1109 Coastal Bay Blvd. Boynton Beach FL 33435

LOAD CASE(S) Standard



June 24,2015

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE LOT 100 PRESERVE
687630	V05	Valley Truss	1	1	I10030482
Builders FirstSource, Lake City, FL 32055, Kim Holloway					Job Reference (optional)

7.610 s Jan 29 2015 MiTek Industries, Inc. Tue Jun 23 15:49:14 2015 Page 1
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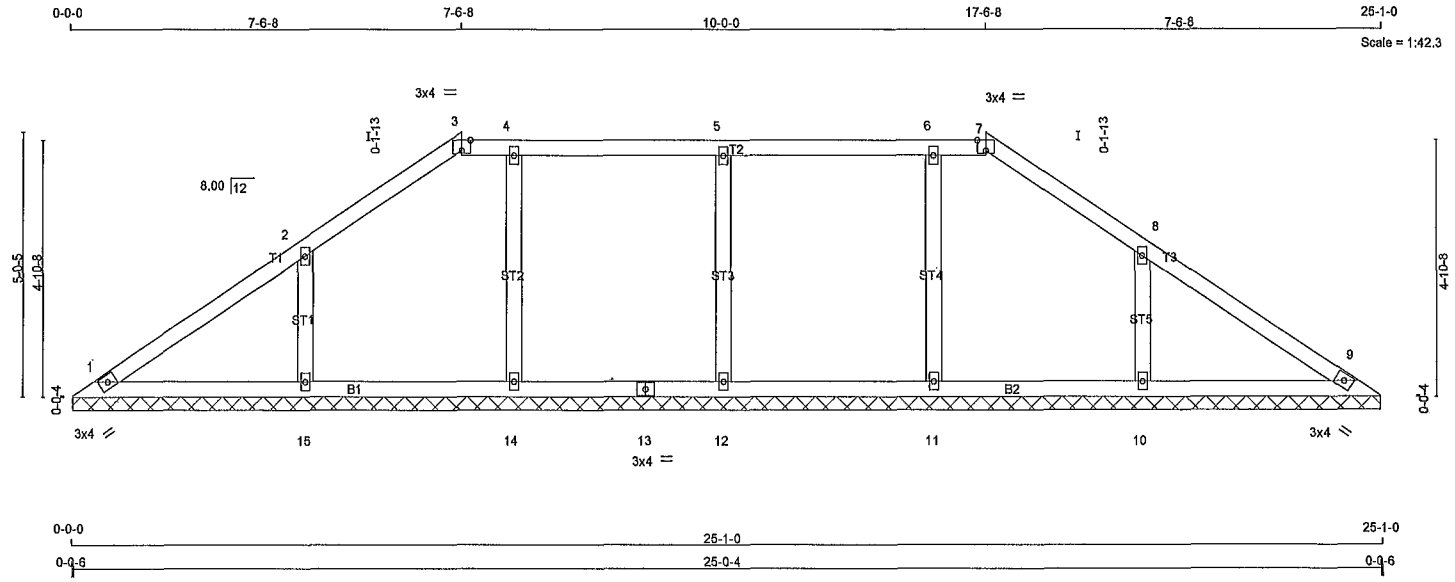

Plate Offsets (X,Y)- [3:0-2-0,Edge], [7:0-2-0,Edge]

LOADING (psf)		SPACING-	2-0-0	CSI		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20 0	Plate Grip DOL	1.25	TC	0.20	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	7 0	Lumber DOL	1 25	BC	0.14	Vert(TL)	n/a	-	n/a	999		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.10	Horz(TL)	0 00	9	n/a	n/a		
BCDL	5.0	Code FBC2014/TPI2007		(Matrix)							Weight: 104 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 25-0-4.
(lb) - Max Horz 1=-118(LC 8)
Max Uplift All uplift 100 lb or less at joint(s) 1 12 14, 11 except 15=-155(LC 12), 10=-153(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 1 9 except 12=349(LC 27) 14=315(LC 27) 15=335(LC 21), 11=315(LC 28) 10=333(LC 22)

FORCES. (lb) Max. Comp./Max. Ten - All forces 250 (lb) or less except when shown
WEBS 2-15=-365/244, 8-10=-365/244

NOTES- (11-13)
1) Unbalanced roof live loads have been considered for this design.
2) Wind ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph TCDL=4.2psf; BCDL=3.0psf; h=20ft, Cat. II Exp C, Encl GCpi=0 18 MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1 60
3) Provide adequate drainage to prevent water ponding
4) All plates are 2x4 MT20 unless otherwise indicated
5) Gable requires continuous bottom chord bearing
6) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads
7) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf
8) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 12 14, 11 except (jt=lb) 15=155, 10=153.
10) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
12) Note: Visually graded lumber designation SPp represents new lumber design values as per SPIB
13) Truss Design Engineer: Julius Lee, PE. Florida P E. License No 34869; Address. 1109 Coastal Bay Blvd Boynton Beach FL 33435

LOAD CASE(S) Standard

June 24,2015

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE - LOT 100 PRESERVE
687630	V07	Valley Truss	1	1	I10030484
Builders FirstSource, Lake City, FL 32055, Kim Holloway					Job Reference (optional)

7.610 s Jan 29 2015 MiTek Industries Inc. Tue Jun 23 15:49:16 2015 Page 1
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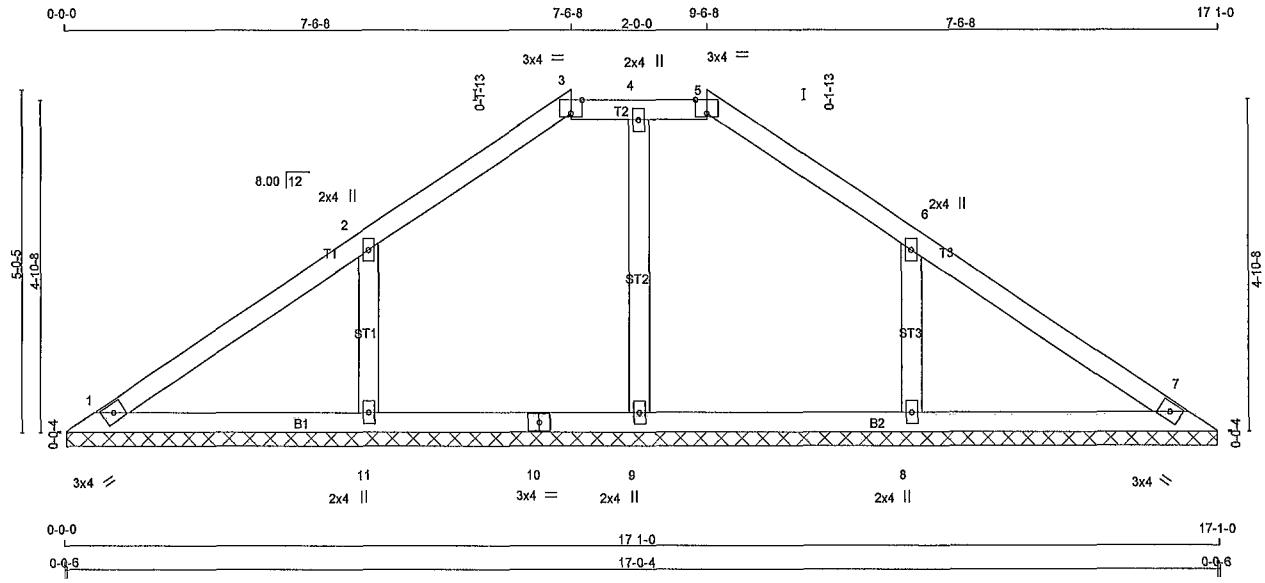


Plate Offsets (X,Y)-- [3:0-2-0,Edge], [5:0-2-0,Edge]

LOADING (psf)		SPACING-	2-0-0	CSI		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0 20	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	7 0	Lumber DOL	1.25	BC	0 12	Vert(TL)	n/a	-	n/a	999		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0 07	Horz(TL)	0 00	7	n/a	n/a		
BCDL	5.0	Code FBC2014/TPI2007		(Matrix)							Weight: 67 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

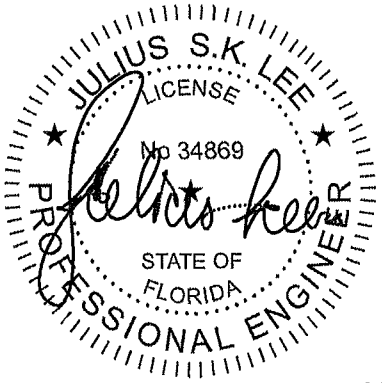
BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

REACTIONS. All bearings 17-0-4.
(lb) - Max Horz 1=118(LC 9)
Max Uplift All uplift 100 lb or less at joint(s) 1 7 9 except 11=-157(LC 12) 8=-156(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 1 7 except 9=258(LC 24), 11=338(LC 21) 8=336(LC 22)

FORCES. (lb) - Max. Comp./Max. Ten - All forces 250 (lb) or less except when shown.
WEBS 2-11=-365/246, 6-8=-365/246

NOTES- (10-12)
1) Unbalanced roof live loads have been considered for this design.
2) Wind ASCE 7-10 Vult=130mph (3-second gust) Vasd=101mph TCDL=4.2psf BCDL=3.0psf h=20ft; Cat. II Exp C Encl GCpi=0 18 MWFRS (envelope) and C-C Exterior(2) zone C-C for members and forces & MWFRS for reactions shown Lumber DOL=1 60 plate grip DOL=1 60
3) Provide adequate drainage to prevent water ponding
4) Gable requires continuous bottom chord bearing
5) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads
6) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members with BCDL = 5.0psf
7) All bearings are assumed to be SP No 2 crushing capacity of 565 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 7 9 except (jt=lb) 11=157 8=156.
9) 'Semi-rigid pitchbreaks including heels' Member end fixity model was used in the analysis and design of this truss.
10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
11) Note Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
12) Truss Design Engineer Julius Lee, PE. Florida P E License No. 34869 Address 1109 Coastal Bay Blvd. Boynton Beach FL 33435

LOAD CASE(S) Standard



June 24,2015

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE LOT 100 PRESERVE
687630	V09	Valley Truss	1	1	I10030486
Builders FirstSource, Lake City, FL 32055, Kim Holloway					Job Reference (optional)

7.610 s Jan 29 2015 MiTek Industries, Inc. Tue Jun 23 15:49:18 2015 Page 1
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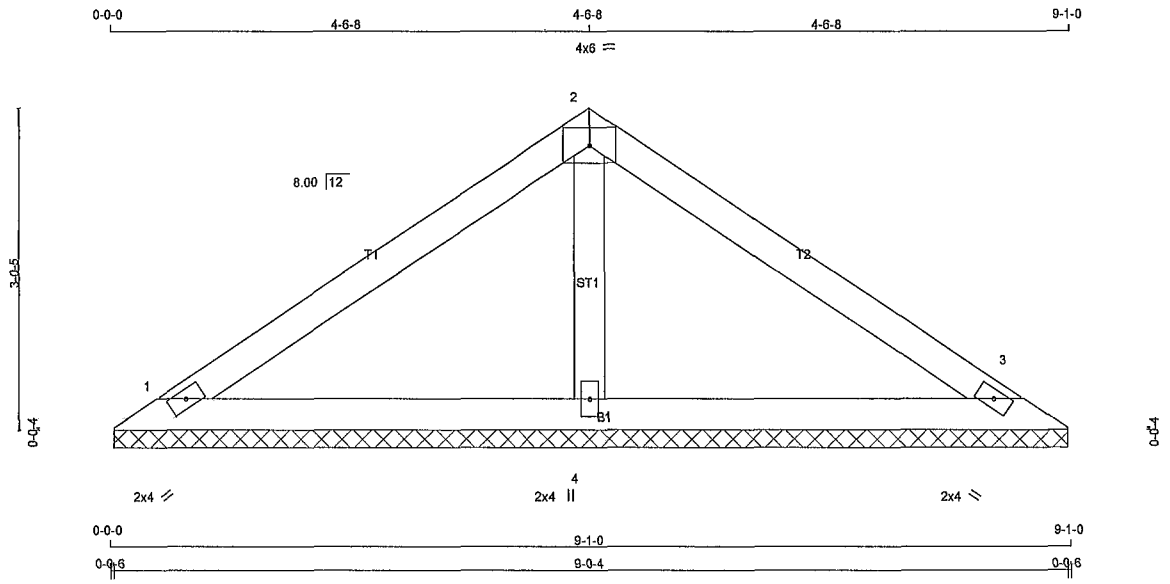


LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) l/defl L/d	PLATES GRIP
TCLL 20 0	Plate Grip DOL 1.25	TC 0.22	Vert(LL) n/a - n/a 999	MT20 244/190
TCDL 7 0	Lumber DOL 1.25	BC 0.12	Vert(TL) n/a - n/a 999	
BCLL 0 0 *	Rep Stress Incr YES	WB 0 05	Horz(TL) 0 00 3 n/a n/a	
BCDL 5.0	Code FBC2014/TPI2007	(Matrix)		Weight: 32 lb FT = 20%

LUMBER-
TOP CHORD 2x4 SP No 2
BOT CHORD 2x4 SP No 2
OTHERS 2x4 SP No.3

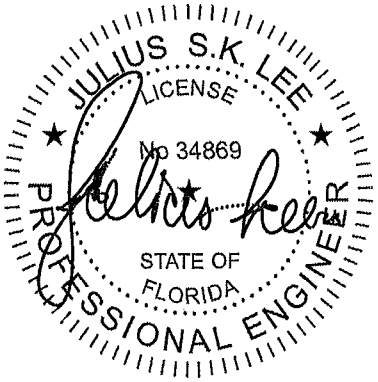
BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 1=131/9-0-4 (min 0-1-8), 3=131/9-0-4 (min. 0-1-8) 4=257/9-0-4 (min. 0-1-8)
Max Horz 1=-69(LC 8)
Max Uplift 1=-41(LC 12), 3=-48(LC 13) 4=-45(LC 12)

FORCES. (lb) Max. Comp./Max. Ten - All forces 250 (lb) or less except when shown

NOTES- (9-11)
1) Unbalanced roof live loads have been considered for this design
2) Wind ASCE 7 10· Vult=130mph (3-second gust) Vasd=101mph TCDL=4.2psf· BCDL=3.0psf· h=20ft; Cat. II Exp C Encl GCpi=0.18, MWFRS (envelope) and C-C Exterior(2) zone·C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1 60 plate grip DOL=1 60
3) Gable requires continuous bottom chord bearing
4) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 3, 4.
8) 'Semi-rigid pitchbreaks including heels' Member end fixity model was used in the analysis and design of this truss.
9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
10) Note Visually graded lumber designation SPp, represents new lumber design values as per SPIB
11) Truss Design Engineer· Julius Lee, PE Florida P E. License No. 34869 Address· 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

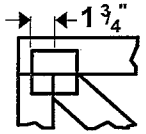


June 24,2015

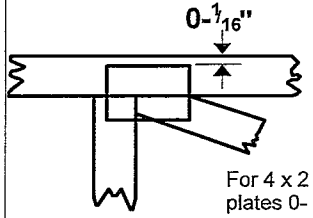
Julius Lee PE
1109 Coastal Bay
Boynton Beach,FL 33435

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- 1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*** Plate location details available in MiTek 20/20 software or upon request.**

PLATE SIZE

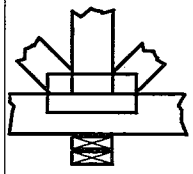
4 x 4

The first dimension is the plate width measured perpendicular to slots Second dimension is the length parallel to slots

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

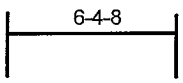


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur

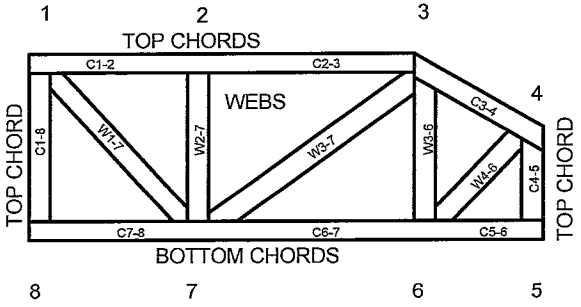
Industry Standards:

ANSI/TPI1 National Design Specification for Metal Plate Connected Wood Truss Construction
DSB-89 Design Standard for Bracing
BCSI1 Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports

ESR-1311, ESR-1352, ER-5243, 9604B,
9730, 95-43, 96-31, 9667A
NER-487, NER-561
95110, 84-32, 96-67, ER-3907, 9432A

Julius Lee PE
1109 Coastal Bay ,
Boynton Beach ,FL 33435

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1 Additional stability bracing for truss system, e.g diagonal or X-bracing, is always required See BCSI1

2. Truss bracing must be designed by an engineer For wide truss spacing individual lateral braces themselves may require bracing or alternative T I, or Eliminator bracing should be considered

3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.

4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.

5. Cut members to bear tightly against each other

6. Place plates on each face of truss at each joint and embed fully Knots and wane at joint locations are regulated by ANSI/TPI 1

7 Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1

8 Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication

9 Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber

10. Camber is a non-structural consideration and is the responsibility of truss fabricator General practice is to camber for dead load deflection

11 Plate type, size, orientation and location dimensions indicated are minimum plating requirements.

12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.

13. Top chords must be sheathed or purlins provided at spacing indicated on design.

14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed unless otherwise noted.

15. Connections not shown are the responsibility of others.

16. Do not cut or alter truss member or plate without prior approval of an engineer

17 Install and load vertically unless indicated otherwise.

18 Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.

19 Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.

20 Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.

August 10, 2010	T-BRACE / I-BRACE DETAIL WITH 2X BRACE ONLY	ST - T-BRACE 2

MiTek Industries, Inc.

Note: T-Bracing / I-Bracing to be used when continuous lateral bracing is impractical. T-Brace / I-Brace must cover 90% of web length.

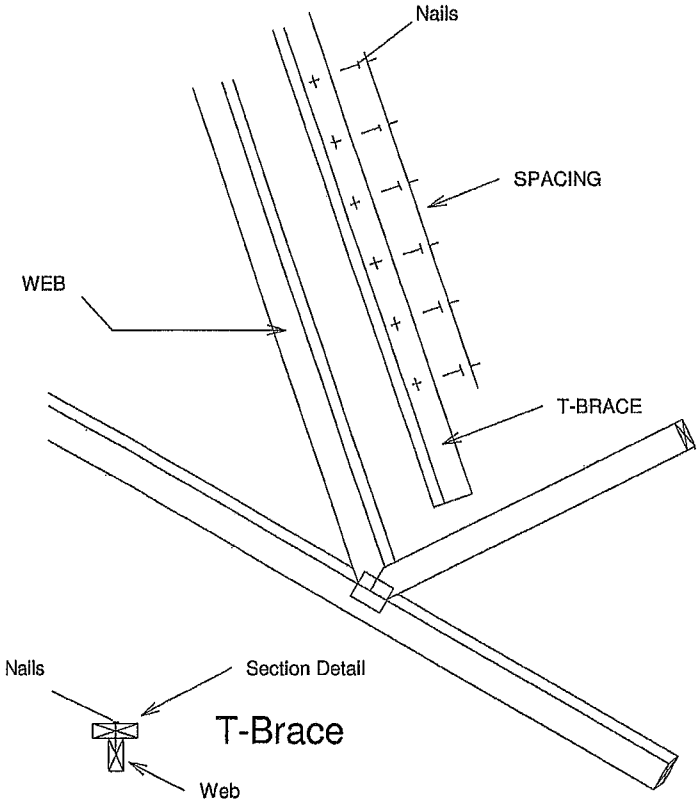
Note: This detail NOT to be used to convert T-Brace / I-Brace webs to continuous lateral braced webs.

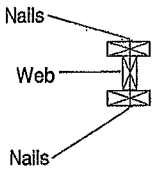
Nailing Pattern		
T-Brace size	Nail Size	Nail Spacing
2x4 or 2x6 or 2x8	10d	6" o.c.
Note: Nail along entire length of T-Brace / I-Brace (On Two-Ply's Nail to Both Plies)		

	Brace Size for One-Ply Truss	
	Specified Continuous Rows of Lateral Bracing	
Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

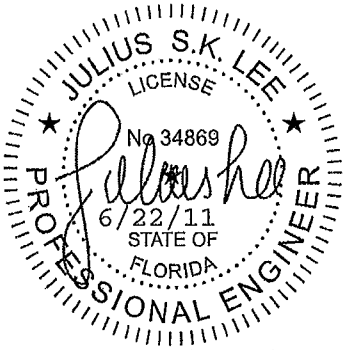
	Brace Size for Two-Ply Truss	
	Specified Continuous Rows of Lateral Bracing	
Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

T-Brace / I-Brace must be same species and grade (or better) as web member.





I-Brace



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JANUARY 1, 2009	LATERAL TOE-NAIL DETAIL	ST-TOENAIL_SP

MiTek Industries, Inc.

NOTES
1. TOE-NAILS SHALL BE DRIVEN AT AN ANGLE OF 45 DEGREES WITH THE MEMBER AND MUST HAVE FULL WOOD SUPPORT. (NAIL MUST BE DRIVEN THROUGH AND EXIT AT THE BACK CORNER OF THE MEMBER END AS SHOWN.
2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
3. ALLOWABLE VALUE SHALL BE THE LESSER VALUE OF THE TWO SPECIES FOR MEMBERS OF DIFFERENT SPECIES.

	TOE-NAIL SINGLE SHEAR VALUES PER NDS 2001 (lb/nail)					
	DIAM.	SYP	DF	HF	SPF	SPF-S
3.5" LONG	131	88.0	80.6	69 9	68.4	59.7
	135	93 5	85.6	74.2	72.6	63.4
	162	108 8	99.6	86.4	84.5	73.8
3.25" LONG	.128	74.2	67.9	58.9	57 6	50.3
	.131	75.9	69.5	60.3	59.0	51.1
	148	81 4	74.5	64.6	63.2	52.5

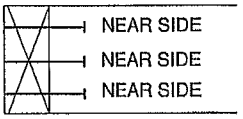
VALUES SHOWN ARE CAPACITY PER TOE-NAIL.
APPLICABLE DURATION OF LOAD INCREASES MAY BE APPLIED

THIS DETAIL APPLICABLE TO THE THREE END DETAILS SHOWN BELOW

VIEWS SHOWN ARE FOR ILLUSTRATION PURPOSES ONLY

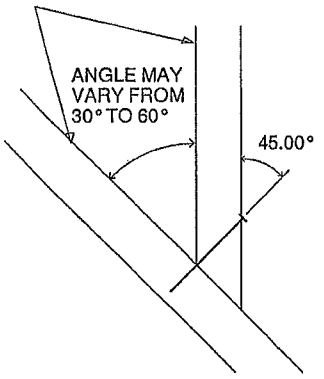
SIDE VIEW

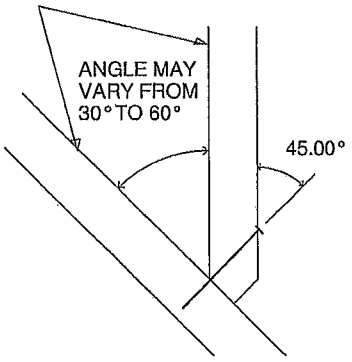
3 NAILS

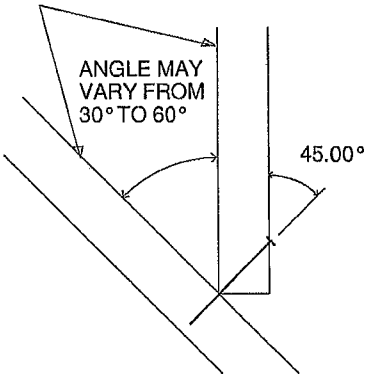


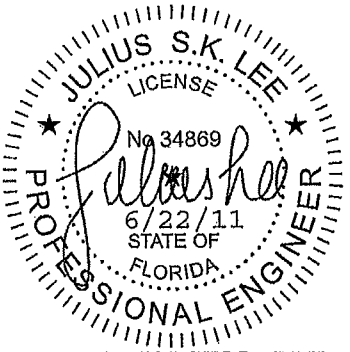
EXAMPLE
(3) - 16d NAILS (.162" diam. x 3.5") WITH SPF SPECIES BOTTOM CHORD

For load duration increase of 1.15:
3 (nails) X 84.5 (lb/nail) X 1.15 (DOL) = 291 5 lb Maximum Capacity









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FEBRUARY 14, 2012	STANDARD PIGGYBACK TRUSS CONNECTION DETAIL	ST-PIGGY-7-10

MiTek Industries, Chesterfield, MO

MiTek Industries, Inc.

MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E MAX MEAN ROOF HEIGHT = 30 FEET MAX TRUSS SPACING = 24 " O.C. CATEGORY II BUILDING EXPOSURE B or C ASCE 7-10 DURATION OF LOAD INCREASE . 1.60
DETAIL IS NOT APPLICABLE FOR TRUSSES TRANSFERING DRAG LOADS (SHEAR TRUSSES). ADDITIONAL CONSIDERATIONS BY BUILDING ENGINEER/DESIGNER ARE REQUIRED

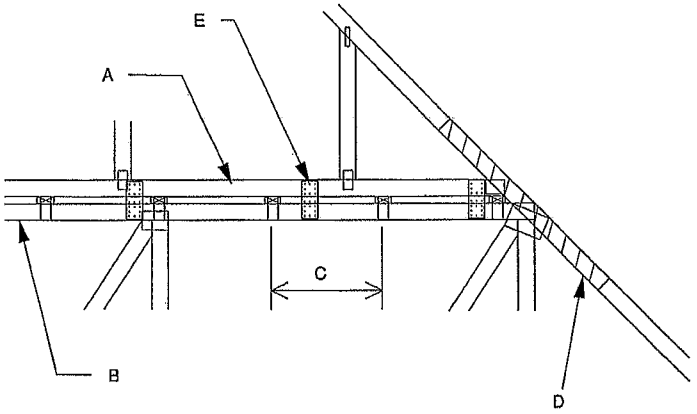
A - PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING. SHALL BE CONNECTED TO EACH PURLIN WITH (2) 0.131" X 3.5" TOE NAILED

B BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.

C - PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24" O.C. UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING. CONNECT TO BASE TRUSS WITH (2) 0.131" X 3.5" NAILS EACH.

D - 2 X __ X 4'-0" SCAB, SIZE AND GRADE TO MATCH TOP CHORD OF PIGGYBACK TRUSS, ATTACHED TO ONE FACE, CENTERED ON INTERSECTION, WITH (2) ROWS OF 0.131" X 3" NAILS @ 4 O.C. SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT IN BOTH DIRECTIONS AND·
1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR
2. WIND SPEED OF 116 MPH TO 160 MPH WITH A MAXIMUM PIGGYBACK SPAN OF 12 ft.

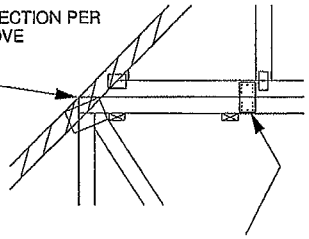
E - FOR WIND SPEEDS BETWEEN 126 AND 160 MPH, ATTACH MITEK 3X8 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 72" O.C. W/ (4) 0.131" X 1.5" PER MEMBER STAGGER NAILS FROM OPPOSING FACES ENSURE 0.5 EDGE DISTANCE. (MIN. 2 PAIRS OF PLATES REQ. REGARDLESS OF SPAN)



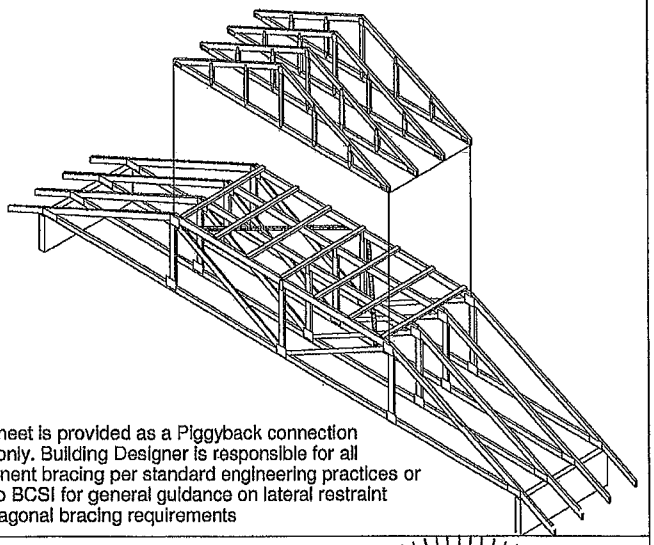
WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS·

REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH Nail-On PLATES AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING

SCAB CONNECTION PER NOTE D ABOVE

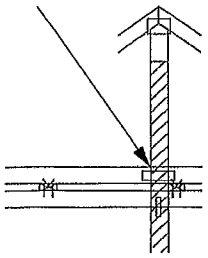


FOR ALL WIND SPEEDS, ATTACH MITEK 3X6 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 48" O C. W/ (4) 0 131" X 1 5" PER MEMBER STAGGER NAILS FROM OPPOSING FACES ENSURE 0.5 EDGE DISTANCE.



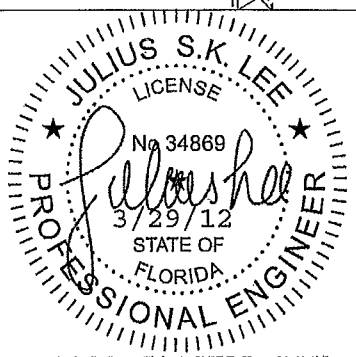
This sheet is provided as a Piggyback connection detail only. Building Designer is responsible for all permanent bracing per standard engineering practices or refer to BCSI for general guidance on lateral restraint and diagonal bracing requirements

VERTICAL WEB TO EXTEND THROUGH BOTTOM CHORD OF PIGGYBACK



FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

1) VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL.
2) ATTACH 2 x ___ x 4'-0" SCAB TO EACH FACE OF TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 4" O C FROM EACH FACE. (SIZE AND GRADE TO MATCH VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.) (MINIMUM 2X4)
3) THIS CONNECTION IS ONLY VALID FOR A MAXIMUM CONCENTRATED LOAD OF 4000 LBS (@1.15) REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS GREATER THAN 4000 LBS.
4) FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS, NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS.
5) CONCENTRATED LOAD MUST BE APPLIED TO BOTH THE PIGGYBACK AND THE BASE TRUSS DESIGN



1109 COASTAL BAY
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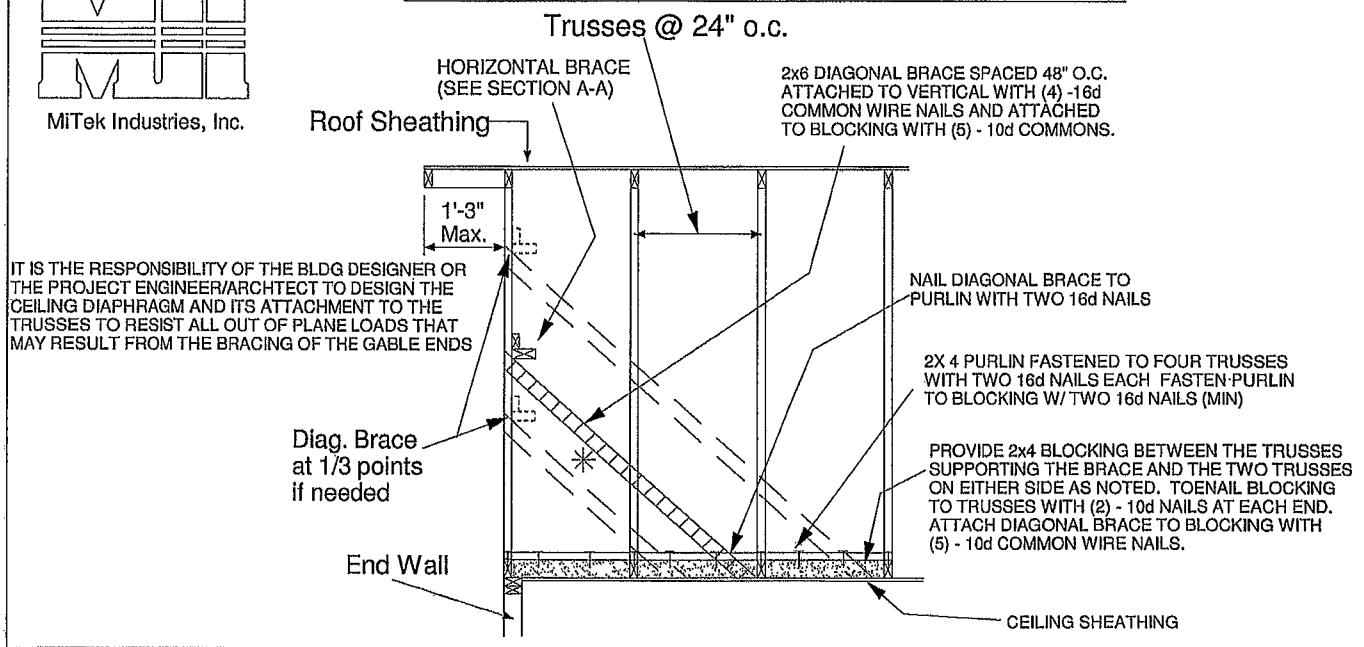
FEBRUARY 14, 2012	Standard Gable End Detail	SHEET 2

MiTek Industries, Chesterfield, MO

MiTek Industries, Inc.

ALTERNATE DIAGONAL BRACING TO THE BOTTOM CHORD



BRACING REQUIREMENTS FOR STRUCTURAL GABLE TRUSSES

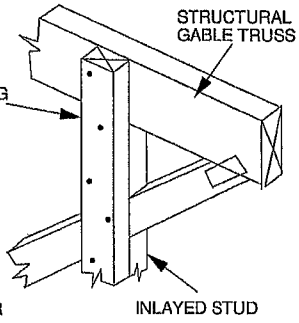
STRUCTURAL GABLE TRUSSES MAY BE BRACED AS NOTED·

METHOD 1 ATTACH A MATCHING GABLE TRUSS TO THE INSIDE FACE OF THE STRUCTURAL GABLE AND FASTEN PER THE FOLLOWING NAILING SCHEDULE.

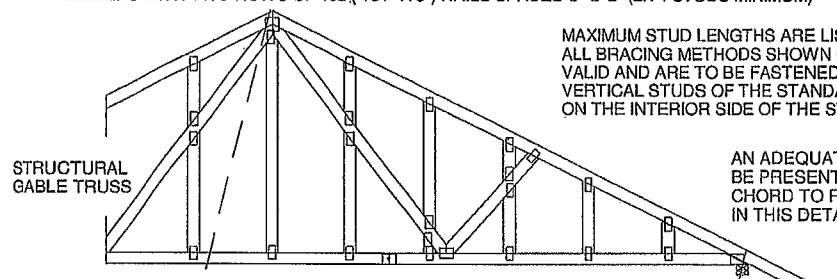
METHOD 2 ATTACH 2X_ SCABS TO THE FACE OF EACH VERTICAL MEMBER ON THE STRUCTURAL GABLE PER THE FOLLOWING NAILING SCHEDULE. SCABS ARE TO BE OF THE SAME SIZE, GRADE AND SPECIES AS THE TRUSS VERTICALS

NAILING SCHEDULE·

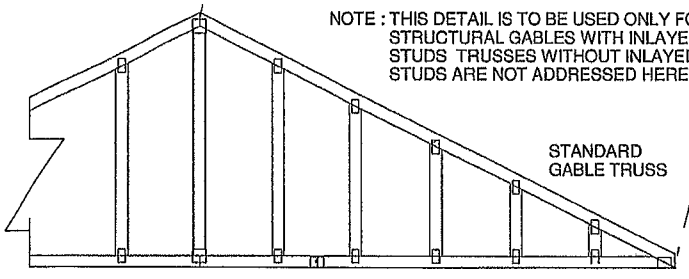
- FOR WIND SPEEDS 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7-10) OR LESS, NAIL ALL MEMBERS WITH ONE ROW OF 10d (.131" X 3") NAILS SPACED 6" O.C
- FOR WIND SPEEDS GREATER 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7-10) NAIL ALL MEMBERS WITH TWO ROWS OF 10d (131" X 3") NAILS SPACED 6" O C (2X 4 STUDS MINIMUM)

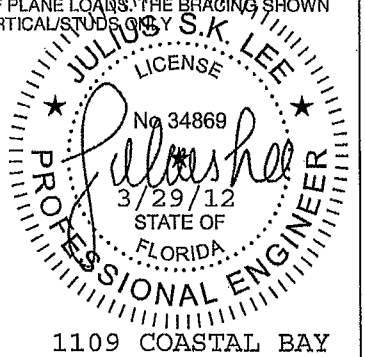


MAXIMUM STUD LENGTHS ARE LISTED ON PAGE 1 ALL BRACING METHODS SHOWN ON PAGE 1 ARE VALID AND ARE TO BE FASTENED TO THE SCABS OR VERTICAL STUDS OF THE STANDARD GABLE TRUSS ON THE INTERIOR SIDE OF THE STRUCTURE.



AN ADEQUATE DIAPHRAGM OR OTHER METHOD OF BRACING MUST BE PRESENT TO PROVIDE FULL LATERAL SUPPORT OF THE BOTTOM CHORD TO RESIST ALL OUT OF PLANE LOADS. THE BRACING SHOWN IN THIS DETAIL IS FOR THE VERTICAL STUDS ONLY





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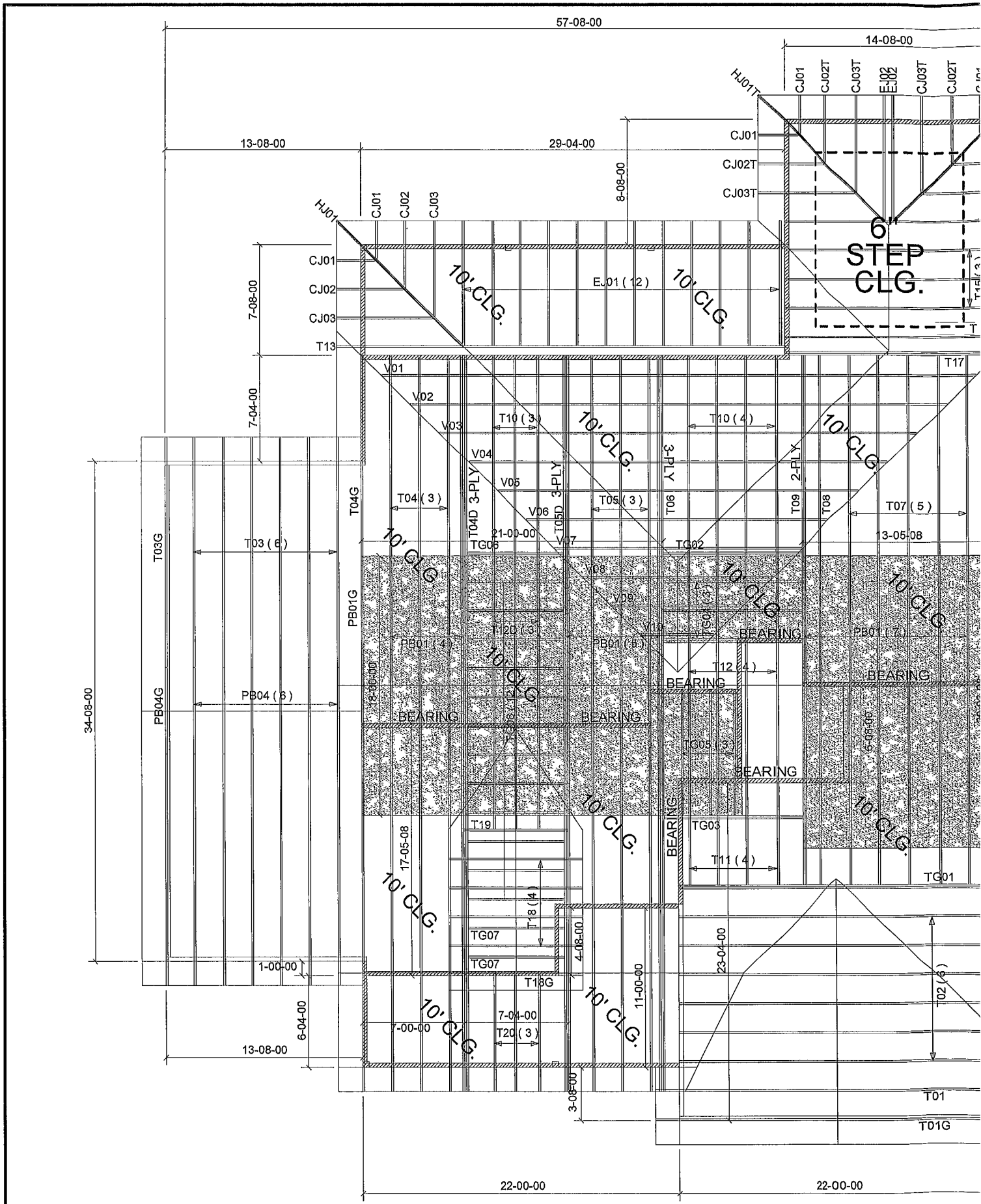

MITEK PLATE APPROVAL #'s 2197.2 - 2197.4, WEYERH