

Ridgway Roof Truss Company

(Trusses and Prefabricated Building Components)

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WARNING

**THESE TRUSSES MUST BE
HANDLED ACCORDING TO
BCSI-B1 SUMMARY SHEETS**

SEE TABLE OF CONTENTS

SEALED

DESIGN MANUAL

PROJECT NAME: Peden Residence

JOB NUMBER: 09-0117

CONTRACTOR: DWC Contracting

DATE: 04/14/2009 jrt

REVISIONS: _____

COMMENTS: _____

SEALED

Permit Number: _____ Lot Number: _____
Miscellaneous: _____ Address: _____ Alachua County _____

The information in this box is for administrative purposes only and is not part of this engineering review.

Truss Fabricator: RIDGWAY ROOF TRUSS

Job Reference: Peden Residence

Standard Loading:

T.C. Live 20 psf
T.C. Dead 7 psf
B.C. Live 0 psf
B.C. Dead 10 psf
Total 37 psf

Gary Dounson-
(PE35054) (truss
designer)

GARY DOUNSON AND ASSOC.,
INC., 2830 NW 41st STREET SUITE D
GAINESVILLE, FL 32606
(352) 375-8593, CA 5201

Engineering Index Sheet

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Job Number 090117 Date 04/14/2009 FBC - 2007 Chapter 16 and 23 Specification Quantity 42

A Professional Engineer's Seal affixed to this Index Sheet indicates the acceptance of professional Engineering responsibilities for individual truss components fabricated in accordance with the listed and attached Truss Specification Sheets. Determination as to the suitability of these individual truss components for any structure is the responsibility of the Building Designer, as defined in ANSI/TPI 1-95, Section 2.2. Sealed truss engineering only valid w/ trusses built by Ridgway Roof Truss Co. Gainesville, FL. Questions regarding this Index Sheet and/or the attached Specification Sheets may be directed to the truss fabricator listed above or Gary Dounson- (PE35054) (truss designer), (Software - Online Plus)

ANSI / ASCE 7-05
Components and Claddings*

Exterior
Wind Speed: 110
Mean Roof Ht.: 16
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load : 7.0
BC Dead Load : 10.0

Notes: Refer to individual truss design drawings for special loading conditions.

Date	Mark
1 04/06/09	R1
5 04/13/09	R5
9 04/13/09	R9
13 04/06/09	R13
17 04/13/09	R18
21 04/13/09	R22
25 04/06/09	R25
29 04/13/09	R29
33 04/06/09	T5
37 04/13/09	T9
41 04/06/09	V4

Date	Mark
2 04/06/09	R2
6 04/13/09	R6
10 04/13/09	R10
14 04/06/09	R14
18 04/13/09	R19
22 04/13/09	R23
26 04/06/09	R26
30 04/06/09	T1
34 04/06/09	T6
38 04/06/09	V1
42 04/06/09	V5

Date	Mark
3 04/13/09	R3
7 04/13/09	R7
11 04/06/09	R11
15 04/06/09	R16
19 04/06/09	R20
23 04/13/09	R24
27 04/13/09	R27
31 04/06/09	T2
35 04/06/09	T7
39 04/06/09	V2

Date	Mark
4 04/13/09	R4
8 04/13/09	R8
12 04/06/09	R12
16 04/06/09	R16A
20 04/13/09	R21
24 04/13/09	R24A
28 04/13/09	R28
32 04/06/09	T4
36 04/13/09	T8
40 04/06/09	V3

Handwritten signature and date:
4-16-09

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2. WTCA 1-1995 (STANDARD RESPONSIBILITIES IN THE DESIGN PROCESS INVOLVING METAL PLATE CONNECTED WOOD TRUSSES)

ENGINEERING (INSERT)

BLUEPRINT OF TRUSS PLACEMENT PLAN (INSERT)

6. BCSI-B1 SUMMARY SHEET

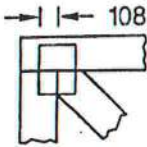
HANDLING, INSTALLING & BRACING INFORMATION

8. STANDARD CHORD & WEB REPAIRS

11. EXAMPLES OF PERMANENT WEB BRACING

ROBBINS ENG. GENERAL NOTES & SYMBOLS

PLATE LOCATION



Center plates on joints unless otherwise noted in plate list or on drawing. Dimensions are given in inches (i.e. 1 1/2" or 1.5") or IN-16ths (i.e. 108).

PLATE SIZE

6.3 x 8.8

The first dimension is the width measured perpendicular to slots. The second dimension is the length measured parallel to slots.



LATERAL BRACING

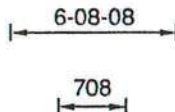
1x4 continuous lateral bracing attached with (2) 8d nails each member where indicated or 2x4 "T" or "L" brace stiffener if applicable nailed flat to edge of web with 12d nails spaced 8" o.c. "T" or "L" brace must be extended at least 90% of web length.

PLATE ORIENTATION

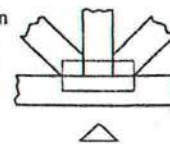


When shown, indicates direction of slots in connector plate.

DIMENSIONS



All dimensions are shown in FT-IN-SX (i.e. 6' 8 1/2" or 6-08-08). Dimensions less than one foot are shown in IN-SX only (i.e. 708).



BEARING

When truss is designed to bear on multiple supports, interior bearing locations should be marked on the truss. Interior support or temporary shoring must be in place before erecting this truss. If necessary, shim bearings to assure solid contact with truss.

ROBBINS LOCK connector plates (20 ga. galv. steel ASTM A653 SS Grade 40) shall be applied on both faces of truss at each joint. Center the plates, unless shown otherwise by circles (o) or dimensions. No loose knots or wanes in plate contact area. Splice only where shown. Overall spans assume 4" bearings at each end, unless indicated otherwise. Cutting and fabrication shall be performed on equipment which produces snug-fitting joints and plates. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication and these designs are not applicable for use with fire retardant lumber. This design was prepared in accordance with "National Design Specifications for Stress - Grade Lumber and Its Fastenings" (AFPA), "Design Specifications for Light Metal Plate Connected Wood Trusses" (TPI), and HUD Design Criteria for

Trussed Rafters. Robbins Eng. Co. bears no responsibility for the erection of trusses, field bracing or permanent truss bracing. Refer to HIB-91 as published by the Truss Plate Institute, 583 D'Onofrio Drive, Suite 200, Madison, Wisconsin 53719. Persons erecting trusses are cautioned to seek professional advice concerning proper erection bracing to prevent toppling and "dominoing". Care should be taken to prevent damage during fabrication, storage, shipping and erection. Top and bottom chords shall be adequately braced in the absence of sheathing or rigid ceiling, respectively. It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead loads imposed by the structure and the live loads imposed by the local building code or historical climatic records.

FURNISH A COPY OF THIS DESIGN TO ERECTION CONTRACTOR. IT IS THE RESPONSIBILITY OF BUILDING DESIGNER TO REVIEW THIS DRWG. & VERIFY THAT DATA INCLUDING DIM. & LOADS CONFORM TO ARCH. PLAN/SPECS & FAB. TRUSS LAYOUTS.



CORPORATE HEADQUARTERS

P.O. Box 280055
Tampa, FL 33682-0055
800-282-1299 • Fax: 813-971-6117



Wood Truss Council of America

5937 Meadowood Dr., Ste. 14 • Madison, WI 53711-4125 • 608/274-3329 (fax)

**Standard Responsibilities
in the Design Process
Involving Metal Plate
Connected Wood Trusses**

WTCA 1-1995

Developed by the WTCA Engineering Review Committee
in cooperation with the Truss Plate Institute

The Wood Truss Council of America publishes standard practice materials prepared and edited by knowledgeable authors from the construction industry to give as much assistance as possible to owners, architects, engineers, contractors, building officials, and others involved in the metal plate connected wood truss industry. The competence of the authors ensures accurate and authoritative information in regard to the subject matter covered, but, of course, neither the Wood Truss Council of America, nor the authors make either express or implied warranties in regard to the use of the materials.

1.0 SCOPE AND DEFINITIONS

- 1.1 This standard defines the design responsibilities of the individuals and organizations involved in the preparation, submittal, review and approval of each Truss Design Drawing and Truss Placement Plan associated with the use of metal plate connected wood trusses. These guidelines are presented as industry standard practice. The guidelines are not intended to preclude alternate provisions as agreed upon by the parties involved.
- 1.2 The following definitions shall apply:
 - 1.2.1 "Architect" shall mean the individual registered architect responsible for the architectural design of the structure and who produces the architectural drawings included in the Construction Design Documents.
 - 1.2.2 "Building Designer" is the individual or organization having responsibility for the overall building or structure design in accordance with the state's statutes and regulations governing the professional registration and certification of architects or engineers. This responsibility includes but is not limited to foundation design, structural member sizing, load transfer, bearing conditions, and the structure's compliance with the applicable building codes. Also referred to as registered architect or engineer, building designer, and registered building designer, but hereinafter will be referred to as Building Designer.
 - 1.2.3 "Construction Design Documents" are the architectural drawings, structural drawings, mechanical drawings, electrical drawings, and any other drawings, specifications, and addenda which set forth the overall design of the structure and issued by the Building Designer.
 - 1.2.4 "Contractor" shall mean the individual or organization responsible for the field storage, handling, and installation of trusses including, but not limited to, temporary bracing, permanent bracing, anchorage, connections and field assembly. The term "Contractor" shall include those subcontractors who have a direct contract with the Contractor to perform all or a portion of the storage, handling, and installation of the trusses.
 - 1.2.5 "Engineer-of-Record" shall mean the registered professional engineer responsible for the structural design of the structure and who produces the structural drawings included in the Construction Design Documents.
 - 1.2.6 "Owner" shall mean the individual or organization for whom the structure is designed.
 - 1.2.7 "Truss" is an individual metal plate connected wood structural component manufactured by the Truss Manufacturer.
 - 1.2.8 "Truss Designer" is the design professional, individual or organization, having responsibility for the design of metal plate connected wood trusses. This responsibility shall be in accordance with the state's statutes and regulations governing the professional registration and certification of architects or engineers. Also referred to as truss engineer, design engineer, registered engineer, and engineer, but hereinafter will be referred to as Truss Designer.
 - 1.2.9 "Truss Design Drawing" shall mean the graphic depiction of an individual Truss prepared by the Truss Designer.
 - 1.2.10 "Truss Manufacturer" shall mean an individual or organization regularly engaged in the manufacturing of Trusses.
 - 1.2.11 "Truss Placement Plan" is the drawing identifying the location assumed for each Truss based on the Truss Manufacturer's interpretation of the Construction Design Documents.

2.0 OWNER RESPONSIBILITIES

- 2.1 Directly or through its representatives, which may include the Contractor and/or Building Designer; (a) review and approve each Truss Design Drawing; (b) review and approve the Truss Placement Plan; (c) resolve and approve all design issues arising out of the preparation of each Truss Design Drawing and Truss Placement Plan; and (d) coordinate the return of each approved Truss Design Drawing and Truss Placement Plan to the Truss Manufacturer prior to truss manufacturing.

3.0 BUILDING DESIGNER RESPONSIBILITIES

- 3.1 Design a structure suitable to ensure that the intended function of each Truss is not affected by adverse influences including, but not limited to, moisture, temperature, corrosive chemicals and gases;
- 3.2 Prepare the Construction Design Documents, showing all trussed areas, which must provide as a minimum the following:
 - 3.2.1 All truss orientations and locations;
 - 3.2.2 Information to fully determine all truss profiles;
 - 3.2.3 Adequate support of the Truss and all truss bearing conditions;
 - 3.2.4 Permanent bracing design for the structure including the Trusses, except as provided in 3.4 and 6.2.12.
 - 3.2.5 The location, direction and magnitude of all dead and live loads applicable to each Truss including, but not limited to, loads attributable to: roof, floor, partition, mechanical, fire sprinkler, attic, storage, wind, snow drift and seismic;
 - 3.2.6 All Truss anchorage designs required to resist uplift, gravity, and lateral loads;
 - 3.2.7 Allowable vertical and horizontal deflection criteria;
 - 3.2.8 Proper transfer of design loads affecting the Truss; and
 - 3.2.9 Adequate connections between Truss and non-Truss components, except as noted in Section 6.2.9.
- 3.3 Review and approve the Truss Placement Plan and each Truss Design Drawing for conformance with the requirements and intent of the Construction Design Documents, the effect of each Truss Design Drawing and Truss Placement Plan on other parts of the structure, and the effect of the structure on each Truss.
- 3.4 Specify permanent lateral bracing where indicated by the Truss Designer on the Truss Design Drawings, to prevent buckling of the individual truss members due to design loads. The Building Designer shall specify how the permanent lateral bracing is to be anchored or restrained to prevent lateral movement if all truss members, so braced, buckle together. This shall be accomplished by: (a) anchorage to solid end walls; (b) permanent diagonal bracing in the plane of the web members; or (c) other means when demonstrated by the Building Designer to provide equivalent bracing.

4.0 CONTRACTOR RESPONSIBILITIES

- 4.1 Provide to the Truss Manufacturer the Construction Design Documents and all revisions and supplements thereto.
- 4.2 Review and approve the Truss Placement Plan and each Truss Design Drawing for conformance with the requirements and intent of the Construction Design Documents, and the effect of the Truss Placement Plan and each Truss Design Drawing on other trades involved in the construction of the structure and the effect of the other trades on the Trusses.
- 4.3 Coordinate the review, approval and return of each Truss Design Drawing and the Truss Placement Plan by the Owner and Building Designer.
- 4.4 Provide the approved Truss Design Drawings, approved Truss Placement Plans, and any supplemental information provided by the Truss Manufacturer to the individual or organization responsible for the installation of the Trusses.
- 4.5 Comply with the field storage, handling, installation, permanent bracing, anchorage, connections and field assembly requirements of the Construction Design Documents.
- 4.6 Determine and install the temporary bracing for the structure, including the Trusses.

5.0 TRUSS MANUFACTURER RESPONSIBILITIES

- 5.1 Communicate the design criteria from the Construction Design Documents to the Truss Designer.
- 5.2 Where required by the Construction Design Documents, prepare the Truss Placement Plan, providing as a minimum the location assumed for each Truss based on the Truss Manufacturer's interpretation of the Construction Design Documents.
- 5.3 Submit to the Contractor the Truss Placement Plan, as may be required, and each Truss Design Drawing for review and approval.
- 5.4 Manufacture the Trusses in accordance with the final approved Truss Design Drawings using the quality criteria for Metal Plate Connected Wood Trusses established by the ANSI/TPI 1-1995 "National Design Standard for Metal Plate Connected Wood Truss Construction."

6.0 TRUSS DESIGNER RESPONSIBILITIES

- 6.1 Prepare the Truss Design Drawings in conformance with the requirements set forth in the latest approved edition of ANSI/TPI 1-1995 "National Design Standard for Metal Plate Connected Wood Truss Construction."
- 6.2 For each Truss Design Drawing, set forth as a minimum the following:
 - 6.2.1 Slope or depth, span and spacing;
 - 6.2.2 Location of all joints;
 - 6.2.3 Required bearing widths;
 - 6.2.4 Design loads as applicable:
 - 6.2.4.1 Top chord live load (including snow loads);
 - 6.2.4.2 Top chord dead load;
 - 6.2.4.3 Bottom chord live load;
 - 6.2.4.4 Bottom chord dead load;
 - 6.2.4.5 Concentrated loads and their points of application; and
 - 6.2.4.6 Controlling wind and earthquake loads;
 - 6.2.5 Adjustments to lumber and metal connector plate design values for conditions of use;
 - 6.2.6 Each reaction force and direction;
 - 6.2.7 Metal connector plate type, size, thickness or gage, and the dimensioned location of each metal connector plate except where symmetrically located relative to the joint interface;
 - 6.2.8 Lumber size, species, and grade for each member;
 - 6.2.9 Connection Requirements for:
 - (a) Truss to Truss girder;
 - (b) Truss ply to ply; and
 - (c) Field splices;
 - 6.2.10 Calculated deflection ratio and/or maximum deflection for live and total load;
 - 6.2.11 Maximum axial compression forces in the Truss members to enable the Building Designer to design the size, connections and anchorage of the permanent continuous lateral bracing. Forces may be shown on the Truss Design Drawing or on supplemental documents; and
 - 6.2.12 Required permanent Truss member bracing location.

7.0 OTHER RESPONSIBILITIES

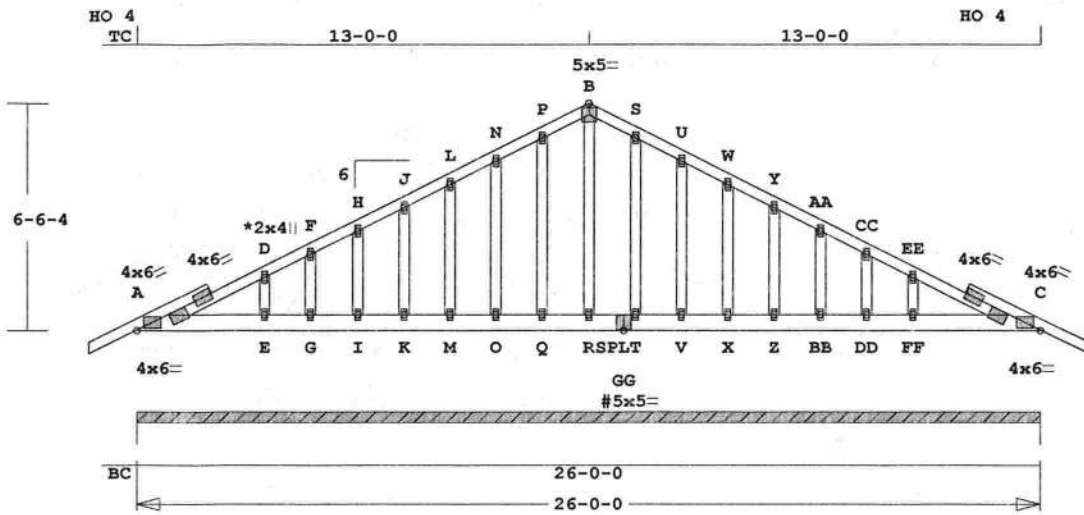
- 7.1 Any party who cuts or damages a truss shall be responsible for securing the engineering required for the repair and for subsequent costs.

Wood Truss Council of America's Objective

WTCA is committed to promoting the common interests of all engaged in the manufacture of wood trusses and related components to ensure growth, continuity, and increased professionalism in our industry. Fundamental to this is promoting the safe, economic, and structurally sound use of trusses in all applications.

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
090117	R1	1	TR	260000	6	0	0	

Peden Residence



ALL PLATES ARE MT2020, # = PLATE SELECTED IN PLATE MONITOR
See Joint D For Typical Gable Plate Size and Placement

Scale: 0.188" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 233.6 LBS

Online Plus -- Version 23.0.052
RUN DATE: 06-APR-09

CSI -Size- ----Lumber-----
TC 0.04 2x 4 SP-#2
BC 0.04 2x 6 SP-#2
GW 0.03 2x 4 SP-#3

Brace truss as follows:
O.C. From To
TC Cont. 0- 0- 0 26- 0- 0
BC Cont. 0- 0- 0 26- 0- 0

psf-Ld Dead Live
TC 7.0 20.0
BC 10.0 0.0
TC+BC 17.0 20.0
Total 37.0 Spacing 24.0"
Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)
Jt Down Uplift Horiz-
A 2009 238 U 243 R

Jt Brg Size Required
A 312.0" 0"-to- 312"

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Ax1-CSI-Bnd
-----Top Chords-----			
A -D	0.04	165 C	0.00 0.04
D -F	0.04	105 C	0.00 0.04
F -H	0.01	73 C	0.00 0.01
H -J	0.01	35 C	0.00 0.01
J -L	0.01	49 T	0.00 0.01
L -N	0.02	87 T	0.01 0.01
N -P	0.02	127 T	0.01 0.01
P -B	0.03	159 T	0.02 0.01
B -S	0.03	159 T	0.02 0.01
S -U	0.02	127 T	0.01 0.01
U -W	0.02	87 T	0.01 0.01
W -Y	0.01	49 T	0.00 0.01
Y -AA	0.01	35 C	0.00 0.01
AA -CC	0.01	73 C	0.00 0.01
CC -EE	0.04	105 C	0.00 0.04
EE -C	0.04	165 C	0.00 0.04
-----Bottom Chords-----			
A -E	0.04	2 T	0.00 0.04
E -G	0.02	0 T	0.00 0.02
G -I	0.00	0 T	

I -K	0.00	0 T
K -M	0.00	0 T
M -O	0.00	0 T
O -Q	0.00	0 T
Q -R	0.00	0 T
R -GG	0.00	0 T
GG -T	0.00	0 T
T -V	0.00	0 T
V -X	0.00	0 T
X -Z	0.00	0 T
Z -BB	0.00	0 T
BB -DD	0.00	0 T
DD -FF	0.02	0 T 0.00 0.02
FF -C	0.04	2 T 0.00 0.04
-----Gable Webs-----		
E -D	0.03	151 T
G -F	0.01	61 T
I -H	0.01	75 T
K -J	0.01	74 T
M -L	0.02	74 T
O -N	0.03	114 T
Q -P	0.03	115 T
R -B	0.03	57 C
T -S	0.03	115 T
V -U	0.03	114 T
X -W	0.02	74 T
Z -Y	0.01	74 T
BB -AA	0.01	75 T
DD -CC	0.01	61 T
FF -EE	0.03	151 T

TL Defl 0.00" in FF-C L/999
LL Defl 0.00" in FF-C L/999
Shear // Grain in A -D 0.08

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A MT20 4.0x 6.0 Ctr-0.3 0.53
D MT20 2.0x 4.0 Ctr Ctr 0.00
F MT20 2.0x 4.0 Ctr Ctr 0.00
H MT20 2.0x 4.0 Ctr Ctr 0.00
J MT20 2.0x 4.0 Ctr Ctr 0.00
L MT20 2.0x 4.0 Ctr Ctr 0.00
N MT20 2.0x 4.0 Ctr Ctr 0.00
P MT20 2.0x 4.0 Ctr Ctr 0.00
B MT20 5.0x 5.0 Ctr Ctr 0.34
S MT20 2.0x 4.0 Ctr Ctr 0.00
U MT20 2.0x 4.0 Ctr Ctr 0.00
W MT20 2.0x 4.0 Ctr Ctr 0.00
Y MT20 2.0x 4.0 Ctr Ctr 0.00
AA MT20 2.0x 4.0 Ctr Ctr 0.00
CC MT20 2.0x 4.0 Ctr Ctr 0.00
EE MT20 2.0x 4.0 Ctr Ctr 0.00
C MT20 4.0x 6.0 Ctr-0.3 0.53

E	MT20	2.0x 4.0	Ctr Ctr	0.00
G	MT20	2.0x 4.0	Ctr Ctr	0.00
I	MT20	2.0x 4.0	Ctr Ctr	0.00
K	MT20	2.0x 4.0	Ctr Ctr	0.00
M	MT20	2.0x 4.0	Ctr Ctr	0.00
O	MT20	2.0x 4.0	Ctr Ctr	0.00
Q	MT20	2.0x 4.0	Ctr Ctr	0.00
R	MT20	2.0x 4.0	Ctr Ctr	0.00
GG	MT20	5.0x 5.0	Ctr Ctr	0.36
T	MT20	2.0x 4.0	Ctr Ctr	0.00
V	MT20	2.0x 4.0	Ctr Ctr	0.00
X	MT20	2.0x 4.0	Ctr Ctr	0.00
Z	MT20	2.0x 4.0	Ctr Ctr	0.00
BB	MT20	2.0x 4.0	Ctr Ctr	0.00
DD	MT20	2.0x 4.0	Ctr Ctr	0.00
FF	MT20	2.0x 4.0	Ctr Ctr	0.00

= Plate Monitor used

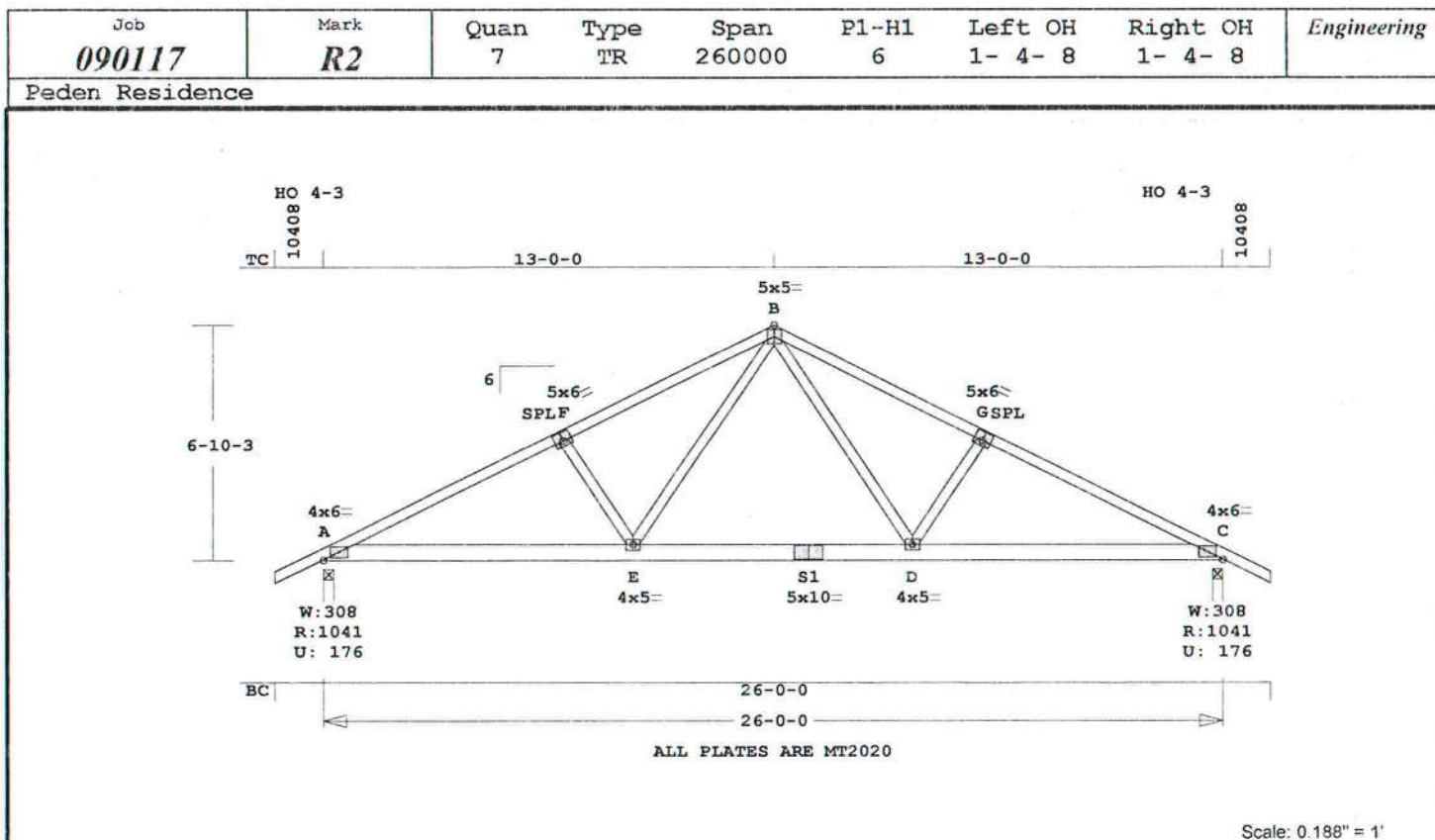
NOTES:

Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2007
Design checked for 10 psf non-concurrent LL on BC.
Refer to Gen Det 3 series for web bracing and plating.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.

Wind Loads - ANSI / ASCE 7-05
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 16'-0"
Exposure Category: B
Occupancy Factor: 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 165 lbs
Max tens. force 159 lbs
Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201



Robbins Engineering, Inc./Online Plus™
 Online Plus -- Version 23.0.052
 RUN DATE: 06-APR-09

CSI -Size- ----Lumber----

TC	0.42	2x 4	SP-#2
BC	0.35	2x 6	SP-#2
WB	0.25	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	26- 0- 0
BC Cont.	0- 0- 0	26- 0- 0

psf-Ld	Dead	Live
TC	7.0	20.0
BC	10.0	0.0
TC+BC	17.0	20.0
Total	37.0	Spacing 24.0"
Lumber Duration Factor	1.25	
Plate Duration Factor	1.25	
TC Fb=1.15	Fc=1.10	Ft=1.10
BC Fb=1.10	Fc=1.10	Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	1042	177 U	128 R
C	1042	177 U	128 R

Jt	Brg Size	Required
A	3.5"	1.5"
C	3.5"	1.5"

Plus 9 Wind Load Case(s)
 Plus 1 UBC LL Load Case(s)
 Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Ax1	CSI-Bnd
-----Top Chords-----				
A -F	0.42	1587 C	0.10	0.32
F -B	0.42	1411 C	0.10	0.32
B -G	0.42	1411 C	0.10	0.32
G -C	0.42	1587 C	0.10	0.32
-----Bottom Chords-----				

APPROX. TRUSS WEIGHT: 176.3 LBS

A -E	0.35	1428 T	0.17	0.18
E -S1	0.25	946 T	0.06	0.19
S1-D	0.25	946 T	0.06	0.19
D -C	0.35	1428 T	0.17	0.18
-----Webs-----				
F -E	0.09	358 T		
E -B	0.25	566 T		
B -D	0.25	566 T		
D -G	0.09	358 T		

TL Defl	-0.13"	in A -E	L/999
LL Defl	-0.06"	in A -E	L/999
Shear //	Grain	in A -F	0.21

Plates for each ply each face.
 Plate - MT20 20 Ga, Gross Area
 Plate - MT2H 20 Ga, Gross Area
 Jt Type Plt Size X Y JSI
 A MT20 4.0x 6.0 Ctr Ctr 0.52
 F MT20 5.0x 6.0-0.2 0.5 0.38
 B MT20 5.0x 5.0 Ctr Ctr 0.42
 G MT20 5.0x 6.0 0.2 0.5 0.38
 C MT20 4.0x 6.0 Ctr Ctr 0.52
 E MT20 4.0x 5.0 Ctr Ctr 0.22
 S1 MT20 5.0x10.0 Ctr Ctr 0.36
 D MT20 4.0x 5.0 Ctr Ctr 0.22

NOTES:

Trusses Manufactured by:
 RIDGWAY ROOF TRUSS

Analysis Conforms To:
 FBC2007

OH Loading

Soffit psf 2.0

Design checked for 10 psf non-concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-05

Truss is designed as

Components and Claddings*
 for Exterior zone location.

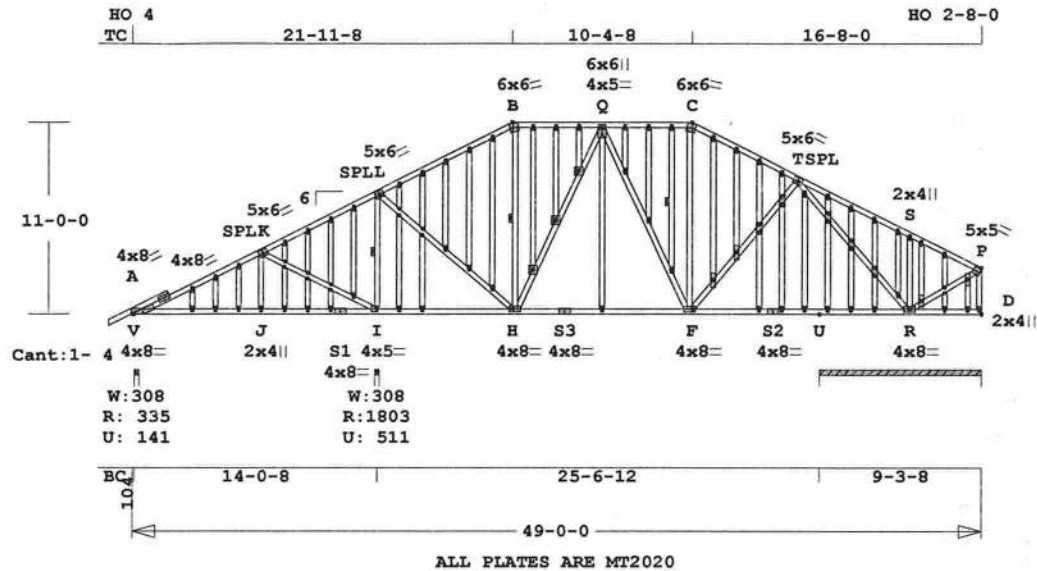
Wind Speed: 110 mph
 Mean Roof Height: 16-0

Exposure Category: B
 Occupancy Factor : 1.00
 Building Type: Enclosed
 TC Dead Load: 4.0 psf
 BC Dead Load: 6.0 psf
 Max comp. force 1587 Lbs
 Max tens. force 1428 Lbs
 Quality Control Factor 1.25
 FABRICATOR NOTES:

1. Delegated Engineer (Truss Designer)
 Gary Dounson, PE 35054
 Gary Dounson & Associates, Inc.
 2830 NW 41st Street Suite D
 Gainesville, FL 32606
 (352) 375-8593
 CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
090117	R3	1	SP	490000	6	0	0	

Peden Residence



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 712.2 LBS

Online Plus -- Version 23.0.052
RUN DATE: 13-APR-09

CSI -Size- ---Lumber---
TC 0.35 2x 4 SP-#2
BC 0.37 2x 4 SP-#2
WB 0.53 2x 4 SP-#3
GW 0.22 2x 4 SP-#3

Brace truss as follows:
O.C. From To
TC Cont. 0- 0- 0 49- 0- 0
BC Cont. 0- 0- 0 49- 0- 0
One Continuous Lateral Brace
I -L H -B D0-C9
Attach CLB with (2)-10d nails
at each web.

psf-Ld Dead Live
TC 7.0 20.0
BC 10.0 0.0
TC+BC 17.0 20.0
Total 37.0 Spacing 24.0"
Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)
Jt Down Uplift Horiz
V 336 142 U 225 R
I 1804 512 U
F2 1454 220 U 274 R

Jt Brg Size Required
V 3.5" 1.5"
I 3.5" 1.9"
F2 111.5" 477"-to- 588"

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Ax1-CSI-Bnd
-----Top Chords-----			
A -W	0.29	311 C	0.03 0.26
W -Y	0.26	289 T	0.03 0.23
Y -AA	0.19	301 T	0.03 0.16
AA-K	0.34	308 T	0.04 0.30
K -CC	0.35	289 T	0.04 0.31
CC-GG	0.10	312 T	0.04 0.06
GG-KK	0.09	338 T	0.05 0.04
KK-QO	0.11	348 T	0.06 0.05
QO-L	0.21	365 T	0.06 0.21
L -QQ	0.21	515 C	0.00 0.21
QQ-UU	0.12	525 C	0.00 0.12
UU-YY	0.15	503 C	0.04 0.11
YY-AA	0.17	466 C	0.00 0.17
AA-A6	0.20	446 C	0.04 0.16
A6-B	0.18	406 C	0.04 0.14
B -AB	0.15	417 C	0.05 0.10
AB-B2	0.10	420 C	0.00 0.10
B2-B6	0.08	423 C	0.05 0.03
B6-Q	0.18	426 C	0.00 0.18
Q -C1	0.18	572 C	0.00 0.18
C1-C5	0.10	572 C	0.06 0.04
C5-C9	0.11	569 C	0.06 0.05

C9-C	0.12	566 C	0.06 0.06
C -D1	0.17	588 C	0.06 0.11
D1-D5	0.16	622 C	0.06 0.10
D5-D9	0.11	637 C	0.05 0.06
D9-E3	0.07	649 C	0.00 0.07
E3-T	0.13	682 C	0.05 0.08
T -E9	0.11	142 T	0.01 0.10
E9-F3	0.15	142 T	0.02 0.13
F3-F7	0.06	112 T	0.02 0.04
F7-G1	0.04	108 T	0.02 0.02
G1-S	0.03	89 T	0.01 0.02
S -G3	0.04	111 T	0.02 0.02
G3-G5	0.03	96 T	0.01 0.02
G5-P	0.05	116 C	0.02 0.03
-----Bottom Chords-----			
A -V	0.00	0 T	
A -X	0.36	265 T	0.04 0.32
X -Z	0.36	265 T	0.04 0.32
Z -BB	0.26	265 T	0.04 0.22
BB-J	0.35	265 T	0.04 0.31
J -FF	0.35	265 T	0.04 0.31
FF-JJ	0.11	268 T	0.04 0.07
JJ-NN	0.12	269 T	0.04 0.08
NN-S1	0.12	267 T	0.04 0.08
S1-I	0.37	267 T	0.04 0.33
I -TT	0.33	504 T	0.00 0.33
TT-XX	0.20	501 T	0.05 0.15
XX-H	0.21	497 T	0.00 0.21
H -S3	0.28	577 T	0.09 0.19
S3-C0	0.15	577 T	0.05 0.10
C0-F	0.23	575 T	0.09 0.14
F -E2	0.21	436 T	0.07 0.14
E2-S2	0.18	427 T	0.07 0.11
S2-E6	0.12	427 T	0.07 0.05
E6-E8	0.09	420 T	0.07 0.02
E8-F2	0.25	417 T	0.07 0.18
F2-F6	0.24	412 T	0.06 0.18
F6-G0	0.07	412 T	0.06 0.01
G0-R	0.07	411 T	0.06 0.01
R -G8	0.02	45 T	0.00 0.02
G8-H0	0.01	43 T	0.00 0.01
H0-D	0.00	43 T	
-----Webs-----			
J -K	0.07	305 C	
K -PP	0.24	759 T	0.19 0.05
PP-I	0.24	788 T	0.19 0.05
I -L	0.31	1215 C	1 Br
L -ZZ	0.48	960 T	0.30 0.18
ZZ-A3	0.53	954 T	0.30 0.23
A3-A7	0.52	920 T	0.29 0.23
A7-H	0.39	922 T	0.29 0.10
H -B	0.06	125 T	1 Br
B -A9	0.06	362 C	0.05 0.01
A9-B3	0.12	376 C	0.03 0.09
B3-B7	0.12	348 C	0.02 0.09
B7-Q	0.06	355 C	0.05 0.01
Q -C2	0.02	103 C	0.00 0.02
C2-C6	0.08	105 C	0.00 0.08
C6-D0	0.08	85 C	0.00 0.08
D0-F	0.03	85 C	0.00 0.03
F -C	0.03	103 T	
F -D2	0.11	194 T	0.02 0.09
D2-D6	0.20	194 T	0.06 0.14
D6-E0	0.21	235 T	0.07 0.14
E0-E5	0.27	275 T	0.08 0.19
E5-T	0.23	323 T	0.10 0.13
T -E7	0.24	919 C	0.10 0.14

E7-F0	0.45	887 C	0.10 0.35
F0-F4	0.28	771 C	0.07 0.21
F4-F9	0.09	785 C	0.07 0.02
F9-G2	0.12	763 C	0.07 0.05
G2-R	0.12	772 C	0.07 0.05
R -S	0.03	86 C	
R -G4	0.06	225 T	0.05 0.01
G4-G9	0.06	208 T	0.05 0.01
G9-P	0.05	210 T	0.05 0.00
D -P	0.04	110 C	WindLd
-----Gable Webs-----			
X -W	0.01	55 T	
Z -Y	0.00	27 C	
BB-AA	0.00	29 C	
FF-CC	0.05	30 C	0.00 0.05
JJ-GG	0.03	22 C	0.00 0.03
NN-KK	0.04	69 C	0.00 0.04
PP-OO	0.08	54 T	0.01 0.07
TT-QQ	0.19	77 C	0.00 0.19
XX-UU	0.14	143 T	0.03 0.11
ZZ-YY	0.12	34 C	0.00 0.12
A3-A2	0.21	68 T	0.00 0.21
A7-A6	0.22	16 T	0.00 0.22
A9-A8	0.08	18 T	0.00 0.08
B3-B2	0.08	28 C	0.00 0.08
B7-B6	0.03	8 T	0.00 0.03
C0-Q	0.07	179 T	0.02 0.05
C2-C1	0.00	7 T	
C6-C5	0.06	29 C	0.00 0.06
D0-C9	0.08	7 T	0.00 0.08
D2-D1	0.13	18 T	0.00 0.13
D6-D5	0.12	64 T	0.00 0.12
E0-D9	0.09	108 T	0.00 0.09
E2-E0	0.16	110 T	0.00 0.16
E5-E3	0.02	24 T	0.00 0.02
E6-E5	0.16	60 T	0.01 0.15
E8-E7	0.08	35 C	0.00 0.08
F0-E9	0.05	148 C	0.00 0.05
F2-F0	0.12	288 C	0.02 0.10
F4-F3	0.08	58 T	0.00 0.08
F6-F4	0.01	49 T	0.00 0.01
F9-F7	0.04	79 T	0.00 0.04
G0-F9	0.02	95 T	0.02 0.00
G2-G1	0.00	20 T	
G4-G3	0.01	28 T	0.00 0.01
G8-G5	0.05	181 T	0.04 0.01
H0-G9	0.00	4 C	

TL Defl -0.16" in X -Z L/999
LL Defl -0.09" in X -Z L/999
Shear // Grain in I -TT 0.21

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Jt Type Plt Size X Y JST
A MT20 4.0x 8.0 0.7 0.1 33
W MT20 2.0x 4.0 Ctr Ctr 0.23
Y MT20 2.0x 4.0 Ctr Ctr 0.23
AA MT20 2.0x 4.0 Ctr Ctr 0.23
K MT20 5.0x 6.0-0.2 0.5 43
CC MT20 2.0x 4.0 Ctr Ctr 0.23
GG MT20 2.0x 4.0 Ctr Ctr 0.23
KK MT20 2.0x 4.0 Ctr Ctr 0.23
OO MT20 2.0x 4.0 Ctr Ctr 0.23
L MT20 5.0x 6.0-0.2 0.5 43
QQ MT20 2.0x 4.0 Ctr Ctr 0.23

UU MT20	2.0x 4.0	Ctr Ctr	0.23
YY MT20	2.0x 4.0	Ctr Ctr	0.23
A2 MT20	2.0x 4.0	Ctr Ctr	0.23
A6 MT20	2.0x 4.0	Ctr Ctr	0.23
B MT20	6.0x 6.0	0.9-3.6	0.33
A8 MT20	2.0x 4.0	Ctr Ctr	0.29
B2 MT20	2.0x 4.0	Ctr Ctr	0.29
B6 MT20	2.0x 4.0	Ctr Ctr	0.29
Q MT20	4.0x 5.0	Ctr Ctr	0.20
C1 MT20	2.0x 4.0	Ctr Ctr	0.29
C5 MT20	2.0x 4.0	Ctr Ctr	0.29
C9 MT20	2.0x 4.0	Ctr Ctr	0.29
C MT20	6.0x 6.0	0.9-3.6	0.33
D1 MT20	2.0x 4.0	Ctr Ctr	0.23
D5 MT20	2.0x 4.0	Ctr Ctr	0.23
D9 MT20	2.0x 4.0	Ctr Ctr	0.23
E3 MT20	2.0x 4.0	Ctr Ctr	0.23
T MT20	5.0x 6.0	0.2 0.5	0.45
E9 MT20	2.0x 4.0	Ctr Ctr	0.23
F3 MT20	2.0x 4.0	Ctr Ctr	0.23
F7 MT20	2.0x 4.0	Ctr Ctr	0.23
G1 MT20	2.0x 4.0	Ctr Ctr	0.23
S MT20	2.0x 4.0	Ctr Ctr	0.23
G3 MT20	2.0x 4.0	Ctr Ctr	0.23
G5 MT20	2.0x 4.0	Ctr Ctr	0.23
P MT20	5.0x 5.0	Ctr Ctr	0.16
X MT20	2.0x 4.0	Ctr Ctr	0.29
Z MT20	2.0x 4.0	Ctr Ctr	0.29
BB MT20	2.0x 4.0	Ctr Ctr	0.29
J MT20	2.0x 4.0	Ctr Ctr	0.29
FF MT20	2.0x 4.0	Ctr Ctr	0.29
JJ MT20	2.0x 4.0	Ctr Ctr	0.29
NN MT20	2.0x 4.0	Ctr Ctr	0.29
S1 MT20	4.0x 8.0	Ctr 0.2	0.33
I MT20	4.0x 5.0	Ctr Ctr	0.26
TT MT20	2.0x 4.0	Ctr Ctr	0.29
XX MT20	2.0x 4.0	Ctr Ctr	0.29
H MT20	4.0x 8.0	Ctr Ctr	0.37
S3 MT20	4.0x 8.0	Ctr 0.2	0.33
C0 MT20	2.0x 4.0	Ctr Ctr	0.29
F MT20	4.0x 8.0	Ctr Ctr	0.21
E2 MT20	2.0x 4.0	Ctr Ctr	0.29
S2 MT20	4.0x 8.0	Ctr 0.2	0.33
E6 MT20	2.0x 4.0	Ctr Ctr	0.29
H8 MT20	2.0x 4.0	Ctr Ctr	0.29
F2 MT20	2.0x 4.0	Ctr Ctr	0.29
G0 MT20	2.0x 4.0	Ctr Ctr	0.29
R MT20	4.0x 8.0	Ctr Ctr	0.24
G8 MT20	2.0x 4.0	Ctr Ctr	0.29
H0 MT20	2.0x 4.0	Ctr Ctr	0.29
D MT20	2.0x 4.0	Ctr Ctr	0.29
PP MT20	2.0x 4.0	Ctr Ctr	0.35
ZZ MT20	2.0x 4.0	Ctr Ctr	0.45
A3 MT20	2.0x 4.0	Ctr Ctr	0.45
A7 MT20	2.0x 4.0	Ctr Ctr	0.43
A9 MT20	6.0x 6.0	Ctr Ctr	0.41
B3 MT20	6.0x 6.0	Ctr Ctr	0.41
B7 MT20	6.0x 6.0	Ctr Ctr	0.41
Q MT20	6.0x 6.0	Ctr Ctr	0.41
C2 MT20	2.0x 4.0	Ctr Ctr	0.19
C6 MT20	2.0x 4.0	Ctr Ctr	0.19
D0 MT20	2.0x 4.0	Ctr Ctr	0.19
D2 MT20	3.0x10.0	Ctr Ctr	0.27
D6 MT20	3.0x10.0	Ctr Ctr	0.27
E0 MT20	4.0x 8.0	Ctr Ctr	0.16
E5 MT20	4.0x 8.0	Ctr Ctr	0.13

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
090117	R3	1	SP	490000	6	0	0	<i>Cont.</i>
Peden Residence								

E7 MT20 2.0x 4.0 Ctr Ctr 0.19
 F0 MT20 4.0x 8.0 Ctr Ctr 0.13
 F4 MT20 4.0x 8.0 Ctr Ctr 0.16
 F9 MT20 4.0x 8.0 Ctr Ctr 0.13
 G2 MT20 2.0x 4.0 Ctr Ctr 0.19
 G4 MT20 3.0x 8.0 Ctr Ctr 0.26
 G9 MT20 3.0x 8.0 Ctr Ctr 0.26

NOTES:

Trusses Manufactured by:

RIDGWAY ROOF TRUSS

Analysis Conforms To:

FBC2007

Design checked for 10 psf non-concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-05

Truss is designed as

Components and Claddings*

for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 16'-0"

Exposure Category: B

Occupancy Factor : 1.00

Building Type: Enclosed

TC Dead Load: 4.0 psf

BC Dead Load: 6.0 psf

User-defined wind-exposed BC

regions --From-- ---To---

0- 1- 4 14- 1-12

Max comp. force 1215 Lbs

Max tens. force 960 Lbs

Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss

Designer)

Gary Dounson, PE 35054

Gary Dounson & Associates,

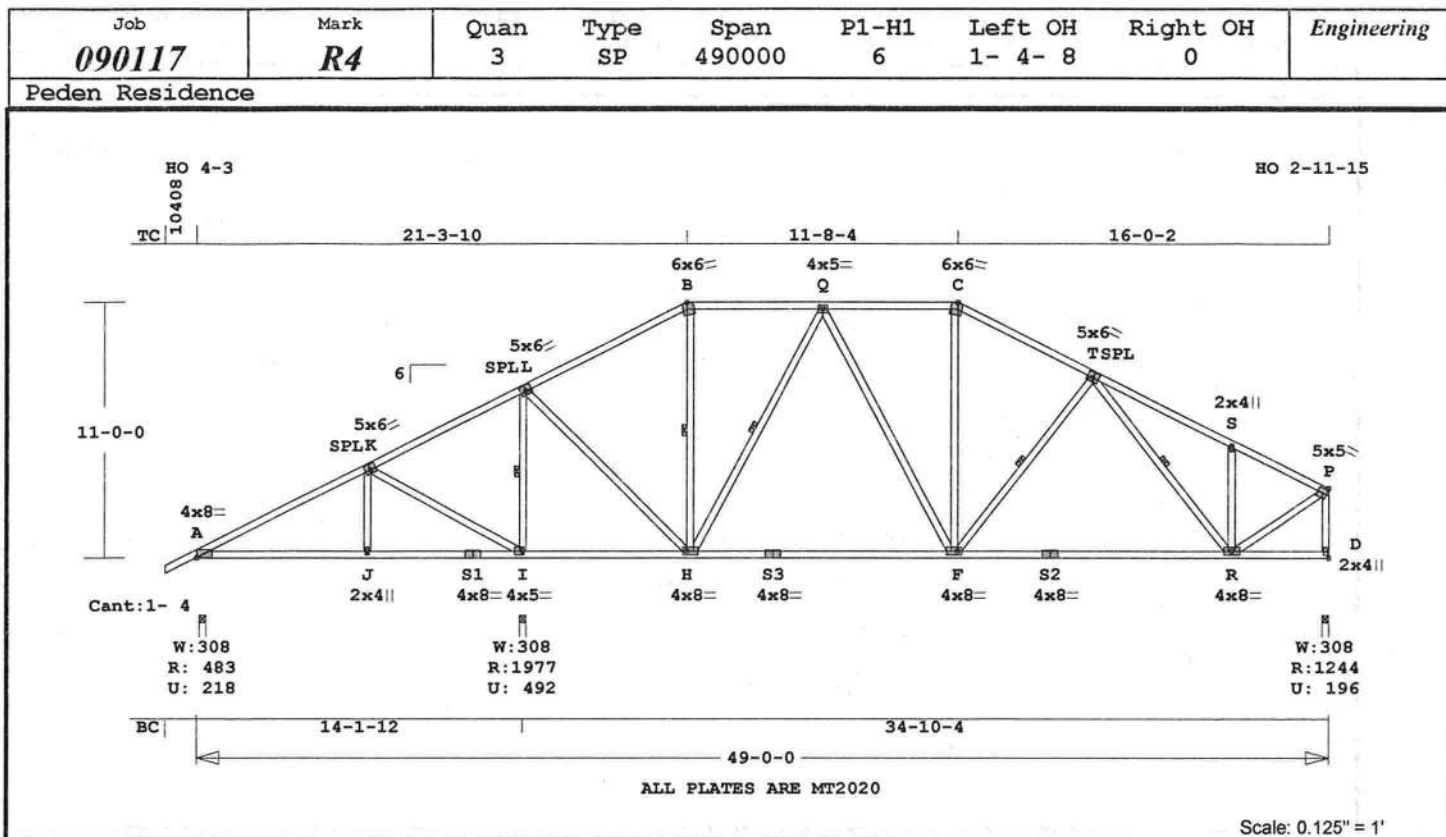
Inc.

2830 NW 41st Street Suite D

Gainesville, FL 32606

(352) 375-8593

CA 5201



Robbins Engineering, Inc./Online Plus™
 Online Plus -- Version 23.0.052
 RUN DATE: 13-APR-09

CSI -Size- ----Lumber-----
 TC 0.48 2x 4 SP-#2
 BC 0.91 2x 4 SP-#2
 WB 0.64 2x 4 SP-#3

Brace truss as follows:
 O.C. From To
 TC Cont. 0- 0- 0 49- 0- 0
 BC Cont. 0- 0- 0 49- 0- 0

One Continuous Lateral Brace
 I -L H -B H -Q F -T
 T -R
 Attach CLB with (2)-10d nails
 at each web.

psf-Ld Dead Live
 TC 7.0 20.0
 BC 10.0 0.0
 TC+BC 17.0 20.0
 Total 37.0 Spacing 24.0"
 Lumber Duration Factor 1.25
 Plate Duration Factor 1.25
 TC Fb=1.15 Fc=1.10 Ft=1.10
 BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)
 Jt Down Uplift Horiz-
 A 484 218 U 220 R
 I 1977 492 U
 D 1244 197 U 278 R

Jt Brg Size Required
 A 3.5" 1.5"
 I 3.5" 2.1"
 D 3.5" 1.5"

Plus 9 Wind Load Case(s)
 Plus 1 UBC LL Load Case(s)
 Plus 1 DL Load Case(s)

Membr CSI P Lbs Axl-Csi-Bnd
 -----Top Chords-----
 A -K 0.44 498 T 0.06 0.38
 K -L 0.48 227 T 0.05 0.43
 L -B 0.45 665 C 0.05 0.40
 B -Q 0.30 587 C 0.00 0.30
 Q -C 0.30 989 C 0.00 0.30

APPROX. TRUSS WEIGHT: 390.4 LBS
 C -T 0.39 1112 C 0.08 0.31
 T -S 0.39 1088 C 0.08 0.31
 S -P 0.28 1077 C 0.06 0.22

-----Bottom Chords-----
 A -J 0.32 359 C 0.02 0.30
 J -S1 0.32 359 C 0.02 0.30
 S1 -I 0.22 359 C 0.02 0.20
 I -H 0.49 293 T 0.00 0.49
 H -S3 0.57 876 T 0.08 0.49
 S3 -F 0.89 876 T 0.08 0.81
 F -S2 0.91 1116 T 0.10 0.81
 S2 -R 0.67 1116 T 0.10 0.57
 R -D 0.57 241 T 0.00 0.57

-----Webs-----
 J -K 0.08 306 T
 K -I 0.64 745 T
 I -L 0.43 1558 C 1 Br
 L -H 0.63 1104 T
 H -B 0.03 127 T 1 Br
 H -Q 0.42 616 C 1 Br
 Q -F 0.27 284 T
 F -C 0.15 290 T
 F -T 0.09 318 T 1 Br
 T -R 0.10 228 C 1 Br
 R -S 0.12 351 T
 R -P 0.38 1186 T
 D -P 0.25 1245 C WindLd

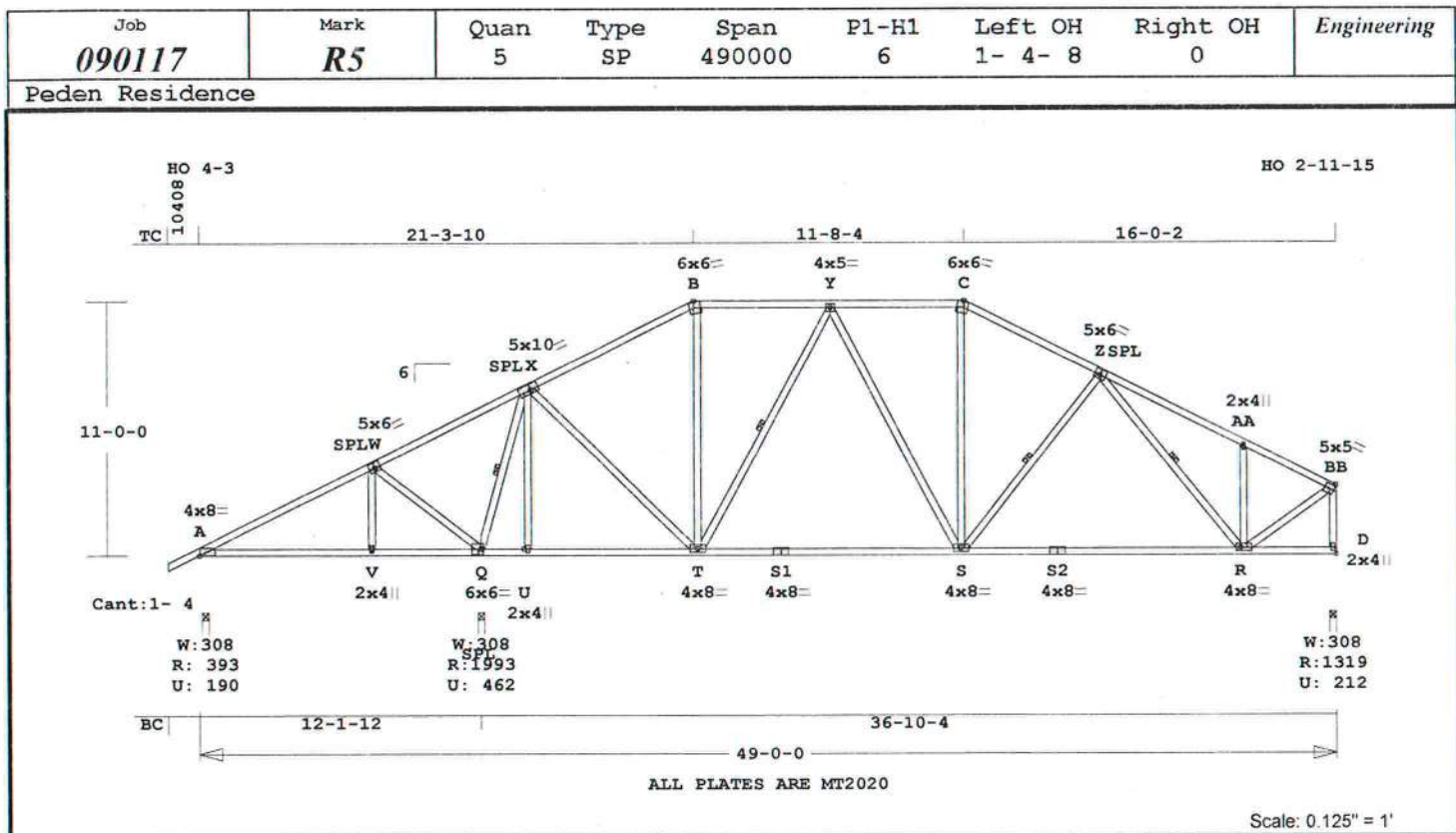
TL Defl -0.65" in F -R L/637
 LL Defl -0.32" in F -R L/999
 Shear // Grain in F -S2 0.27

Plates for each ply each face.
 Plate - MT20 20 Ga, Gross Area
 Plate - MT2H 20 Ga, Gross Area
 Jt Type Plt Size X Y JSI
 A MT20 4.0x 8.0 0.8 0.3 0.35
 K MT20 5.0x 6.0-0.2 0.5 0.43
 L MT20 5.0x 6.0-0.2 0.5 0.43
 B MT20 6.0x 6.0 0.9-3.6 0.33
 Q MT20 4.0x 5.0 Ctr Ctr 0.19
 C MT20 6.0x 6.0-0.9-3.6 0.33
 T MT20 5.0x 6.0 0.2 0.5 0.44
 S MT20 2.0x 4.0 Ctr Ctr 0.23
 P MT20 5.0x 5.0 Ctr Ctr 0.39
 J MT20 2.0x 4.0 Ctr Ctr 0.29
 S1 MT20 4.0x 8.0 Ctr 0.2 0.33
 I MT20 4.0x 5.0 Ctr Ctr 0.33
 H MT20 4.0x 8.0 Ctr Ctr 0.45
 S3 MT20 4.0x 8.0 Ctr 0.2 0.66

F MT20 4.0x 8.0 Ctr Ctr 0.20
 S2 MT20 4.0x 8.0 Ctr 0.2 0.76
 R MT20 4.0x 8.0 Ctr Ctr 0.47
 D MT20 2.0x 4.0 Ctr Ctr 0.29

NOTES:
 Trusses Manufactured by:
 RIDGWAY ROOF TRUSS
 Analysis Conforms To:
 FBC2007
 OH Loading
 Soffit psf 2.0
 Design checked for 10 psf non-
 concurrent LL on BC.
 Wind Loads - ANSI / ASCE 7-05
 Truss is designed as
 Components and Claddings*
 for Exterior zone location.
 Wind Speed: 110 mph
 Mean Roof Height: 16-0
 Exposure Category: B
 Occupancy Factor : 1.00
 Building Type: Enclosed
 TC Dead Load: 4.0 psf
 BC Dead Load: 6.0 psf
 User-defined wind-exposed BC
 regions --From-- --To--
 0- 1- 4 14- 1-12

Max comp. force 1558 Lbs
 Max tens. force 1186 Lbs
 Quality Control Factor 1.25
 FABRICATOR NOTES:
 1. Delegated Engineer (Truss
 Designer)
 Gary Dounson, PE 35054
 Gary Dounson & Associates,
 Inc.
 2830 NW 41st Street Suite D
 Gainesville, FL 32606
 (352)375-8593
 CA 5201



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 400.5 LBS
 Online Plus -- Version 23.0.052
 RUN DATE: 13-APR-09

CSI -Size- ---Lumber---
 TC 0.48 2x 4 SP-#2
 BC 0.92 2x 4 SP-#2
 WB 0.46 2x 4 SP-#3

Brace truss as follows:
 O.C. From To
 TC Cont. 0- 0- 0 49- 0- 0
 BC Cont. 0- 0- 0 49- 0- 0
 One Continuous Lateral Brace
 Q -X T -Y S -Z Z -R
 Attach CLB with (2)-10d nails
 at each web.

psf-Ld Dead Live
 TC 7.0 20.0
 BC 10.0 0.0
 TC+BC 17.0 20.0
 Total 37.0 Spacing 24.0"
 Lumber Duration Factor 1.25
 Plate Duration Factor 1.25
 TC Fb=1.15 Fc=1.10 Ft=1.10
 BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)
 Jt Down Uplift Horiz-
 A 393 190 U 220 R
 Q 1993 463 U
 D 1319 213 U 278 R

Jt Brg Size Required
 A 3.5" 1.5"
 Q 3.5" 2.1"
 D 3.5" 1.6"

Plus 9 Wind Load Case(s)
 Plus 1 UBC LL Load Case(s)
 Plus 1 DL Load Case(s)

Membr CSI P Lbs Axl-CSt-Bnd
 ---Top Chords---
 A -W 0.45 358 T 0.00 0.45
 W -X 0.48 312 T 0.03 0.45
 X -B 0.46 879 C 0.07 0.39
 B -Y 0.30 779 C 0.00 0.30
 Y -C 0.30 1101 C 0.00 0.30
 C -Z 0.40 1238 C 0.09 0.31

Z -AA 0.39 1144 C 0.08 0.31
 AA-BB 0.29 1132 C 0.07 0.22
 ---Bottom Chords---
 A -V 0.27 235 C 0.01 0.26
 V -Q 0.27 235 C 0.01 0.26
 Q -U 0.12 167 T 0.01 0.11
 U -T 0.52 167 T 0.01 0.51
 T -S1 0.61 1029 T 0.10 0.51
 S1-S 0.91 1029 T 0.10 0.81
 S -S2 0.92 1213 T 0.11 0.81
 S2-R 0.70 1213 T 0.11 0.59
 R -D 0.59 241 T 0.00 0.59
 ---Webs---
 V -W 0.07 280 T
 W -Q 0.37 661 T
 Q -X 0.46 1654 C 1 Br
 U -X 0.04 163 T
 X -T 0.42 871 T
 T -B 0.05 198 T
 T -Y 0.36 531 C 1 Br
 Y -S 0.14 217 T
 S -C 0.21 333 T
 S -Z 0.08 307 T 1 Br
 Z -R 0.14 300 C 1 Br
 R -AA 0.12 354 T
 R -BB 0.40 1262 T
 D -BB 0.26 1325 C WindLd

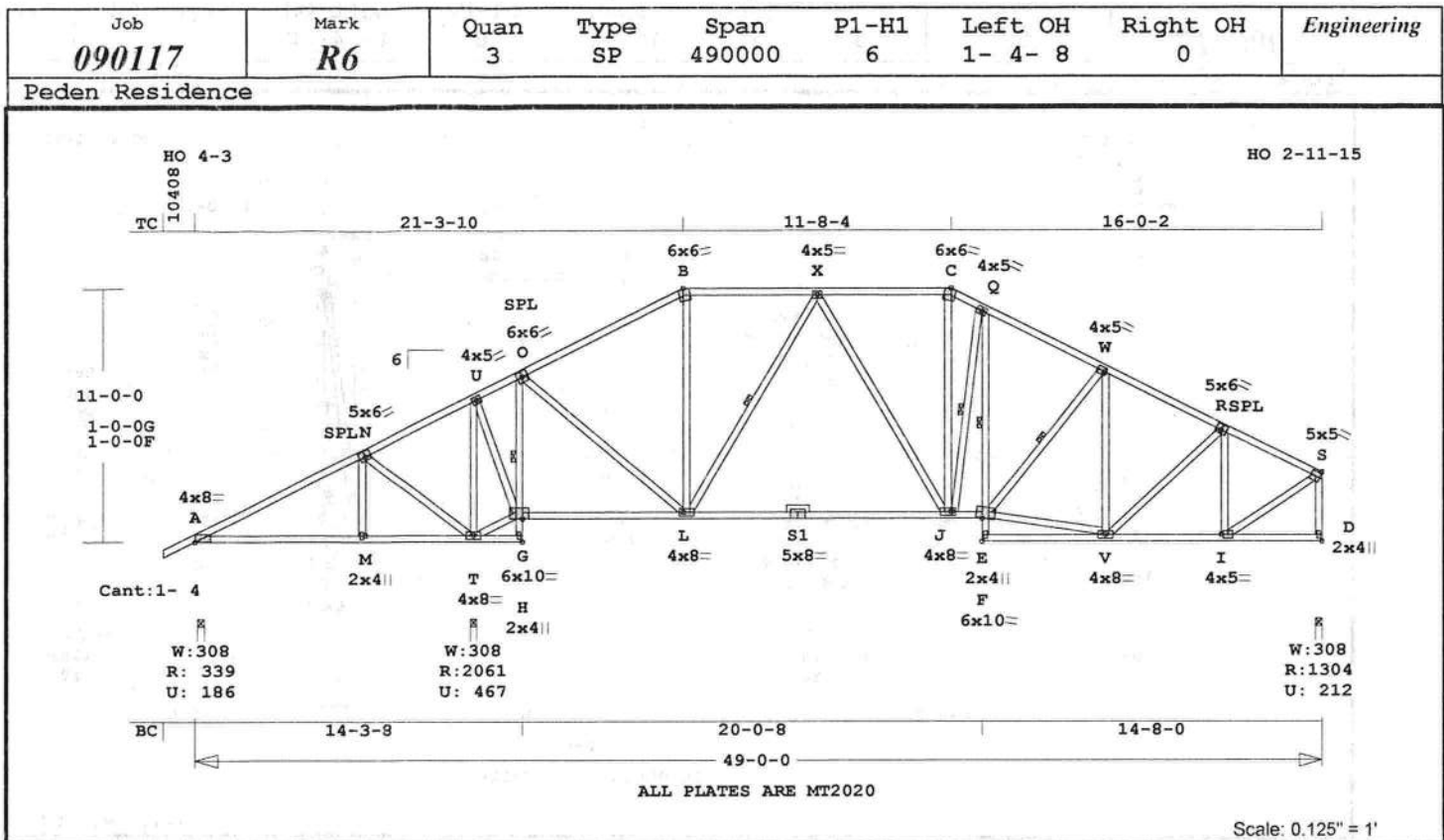
TL Defl -0.69" in S -R L/633
 LL Defl -0.34" in S -R L/999
 Shear // Grain in S -S2 0.27

Plates for each ply each face.
 Plate - MT20 20 Ga, Gross Area
 Plate - MT2H 20 Ga, Gross Area
 Jt Type Plt Size X Y JSI
 A MT20 4.0x 8.0 0.8 0.3 0.35
 W MT20 5.0x 6.0-0.2 0.5 0.43
 X MT20 5.0x10.0-1.1 Ctr 0.75
 B MT20 6.0x 6.0 0.9-3.6 0.33
 Y MT20 4.0x 5.0 Ctr Ctr 0.19
 C MT20 6.0x 6.0-0.9-3.6 0.33
 Z MT20 5.0x 6.0 0.2 0.5 0.44
 AA MT20 2.0x 4.0 Ctr Ctr 0.23
 BB MT20 5.0x 5.0 Ctr Ctr 0.41
 V MT20 2.0x 4.0 Ctr Ctr 0.29
 Q MT20 6.0x 6.0 Ctr Ctr 0.58
 U MT20 2.0x 4.0 Ctr Ctr 0.29
 T MT20 4.0x 8.0 Ctr Ctr 0.36
 S1 MT20 4.0x 8.0 Ctr 0.2 0.66

S MT20 4.0x 8.0 Ctr Ctr 0.20
 S2 MT20 4.0x 8.0 Ctr 0.2 0.80
 R MT20 4.0x 8.0 Ctr Ctr 0.50
 D MT20 2.0x 4.0 Ctr Ctr 0.31

NOTES:
 Trusses Manufactured by:
 RIDGWAY ROOF TRUSS
 Analysis Conforms To:
 FBC2007
 OH Loading
 Soffit psf 2.0
 Design checked for 10 psf non-
 concurrent LL on BC.
 Wind Loads - ANSI / ASCE 7-05
 Truss is designed as
 Components and Claddings*
 for Exterior zone location.
 Wind Speed: 110 mph
 Mean Roof Height: 16-0
 Exposure Category: B
 Occupancy Factor : 1.00
 Building Type: Enclosed
 TC Dead Load: 4.0 psf
 BC Dead Load: 6.0 psf
 User-defined wind-exposed BC
 regions --From-- ---To---
 0- 1- 4 12- 1-12
 Max comp. force 1654 Lbs
 Max tens. force 1262 Lbs
 Quality Control Factor 1.25

FABRICATOR NOTES:
 1. Delegated Engineer (Truss
 Designer)
 Gary Dounson, PE 35054
 Gary Dounson & Associates,
 Inc.
 2830 NW 41st Street Suite D
 Gainesville, FL 32606
 (352) 375-8593
 CA 5201



Online Plus -- Version 23.0.052
 RUN DATE: 13-APR-09

CSI	Size	Lumber
TC	0.47 2x 4	SP-#2
BC	0.76 2x 4	SP-#2
CW	0.35 2x 4	SP-#3
WB	1.00 2x 4	SP-#3
PB	---	2x 4 SP-#3

Brace truss as follows:

O.C.	From	To
TC	Cont.	0- 0- 0 49- 0- 0
BC	Cont.	0- 0- 0 49- 0- 0

One Continuous Lateral Brace

L	X	J	Q	F	W	H	O
E	-	Q					

Attach CLB with (2)-10d nails at each web.

psf-Ld	Dead	Live
TC	7.0	20.0
BC	10.0	0.0
TC+BC	17.0	20.0
Total	37.0	Spacing 24.0"

Lumber Duration Factor 1.25
 Plate Duration Factor 1.25
 TC Fb=1.15 Fc=1.10 Ft=1.10
 BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz
A	339	186	220 R
T	2061	468	U
D	1305	212	U 278 R

Jt	Brg Size	Required
A	3.5"	1.5"
T	3.5"	2.2"
D	3.5"	1.5"

Plus 9 Wind Load Case(s)
 Plus 1 UBC LL Load Case(s)
 Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl	CSI-Bnd
-----Top Chords-----				
A	-N	0.42	323 T	0.00 0.42
N	-U	0.47	399 T	0.05 0.42
U	-O	0.40	159 T	0.01 0.39
O	-B	0.46	923 C	0.07 0.39
B	-X	0.30	820 C	0.00 0.30
X	-C	0.30	1183 C	0.00 0.30
C	-Q	0.23	1310 C	0.11 0.12

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 455.1 LBS

Q	-W	0.30	1371 C	0.10	0.20
W	-R	0.29	1358 C	0.09	0.20
R	-S	0.26	1097 C	0.07	0.19

-----Bottom Chords-----

A	-M	0.26	204 C	0.00	0.26
M <th>-T</th> <th>0.26</th> <th>204 C</th> <th>0.00</th> <th>0.26</th>	-T	0.26	204 C	0.00	0.26
T <th>-H</th> <th>0.08</th> <th>46 C</th> <th>0.00</th> <th>0.08</th>	-H	0.08	46 C	0.00	0.08
G <th>-L</th> <th>0.56</th> <th>161 T</th> <th>0.00</th> <th>0.56</th>	-L	0.56	161 T	0.00	0.56
L <th>-S1</th> <th>0.66</th> <th>1101 T</th> <th>0.10</th> <th>0.56</th>	-S1	0.66	1101 T	0.10	0.56
S1 <th>-J</th> <th>0.74</th> <th>1101 T</th> <th>0.10</th> <th>0.64</th>	-J	0.74	1101 T	0.10	0.64
J <th>-F</th> <th>0.76</th> <th>1220 T</th> <th>0.12</th> <th>0.64</th>	-F	0.76	1220 T	0.12	0.64
E <th>-V</th> <th>0.18</th> <th>93 T</th> <th>0.01</th> <th>0.17</th>	-V	0.18	93 T	0.01	0.17
V <th>-I</th> <th>0.27</th> <th>991 T</th> <th>0.10</th> <th>0.17</th>	-I	0.27	991 T	0.10	0.17
I <th>-D</th> <th>0.11</th> <th>241 T</th> <th>0.00</th> <th>0.11</th>	-D	0.11	241 T	0.00	0.11

-----Chord-Webs-----

H	-G	0.14	22 T	0.00	0.14
G <th>-O</th> <th>0.20</th> <th>1313 C</th> <th>0.09</th> <th>0.11</th>	-O	0.20	1313 C	0.09	0.11
E <th>-F</th> <th>0.35</th> <th>82 T</th> <th>0.01</th> <th>0.34</th>	-F	0.35	82 T	0.01	0.34
F <th>-Q</th> <th>0.02</th> <th>202 C</th> <th>0.02</th> <th>0.00</th>	-Q	0.02	202 C	0.02	0.00

-----Webs-----

M	-N	0.07	277 T
N <th>-T</th> <th>0.35</th> <th>653 T</th>	-T	0.35	653 T
T <th>-U</th> <th>1.00</th> <th>1480 C</th>	-U	1.00	1480 C
U <th>-G</th> <th>0.09</th> <th>405 C</th>	-G	0.09	405 C
U <th>-G</th> <th>0.40</th> <th>1275 T</th>	-G	0.40	1275 T
O <th>-L</th> <th>0.40</th> <th>946 T</th>	-L	0.40	946 T
L <th>-B</th> <th>0.05</th> <th>212 T</th>	-B	0.05	212 T
L <th>-X</th> <th>0.32</th> <th>554 C</th>	-X	0.32	554 C
X <th>-J</th> <th>0.12</th> <th>221 T</th>	-J	0.12	221 T
J <th>-C</th> <th>0.38</th> <th>379 T</th>	-C	0.38	379 T
J <th>-Q</th> <th>0.10</th> <th>418 T</th>	-Q	0.10	418 T
F <th>-W</th> <th>0.03</th> <th>147 T</th>	-W	0.03	147 T
F <th>-V</th> <th>0.36</th> <th>1144 T</th>	-V	0.36	1144 T
V <th>-W</th> <th>0.29</th> <th>305 C</th>	-W	0.29	305 C
V <th>-R</th> <th>0.09</th> <th>306 T</th>	-R	0.09	306 T
I <th>-R</th> <th>0.24</th> <th>588 C</th>	-R	0.24	588 C
I <th>-S</th> <th>0.38</th> <th>1204 T</th>	-S	0.38	1204 T
D <th>-S</th> <th>0.25</th> <th>1270 C</th>	-S	0.25	1270 C

TL Defl -0.59" in L -J L/735
 LL Defl -0.29" in L -J L/999
 Shear // Grain in J -F 0.36

Plates for each ply each face.
 Plate - MT20 20 Ga, Gross Area
 Plate - MT2H 20 Ga, Gross Area

Jt Type	Plt Size	X	Y	JSI
A	MT20	4.0x 8.0	0.8 0.3	0.35
N	MT20	5.0x 6.0	0.2 0.5	0.43
U	MT20	4.0x 5.0	Ctr Ctr	0.61
O	MT20	6.0x 6.0	Ctr Ctr	0.43
B	MT20	6.0x 6.0	0.9-3.6	0.33
X	MT20	4.0x 5.0	Ctr Ctr	0.19
C	MT20	6.0x 6.0	0.9-3.6	0.33

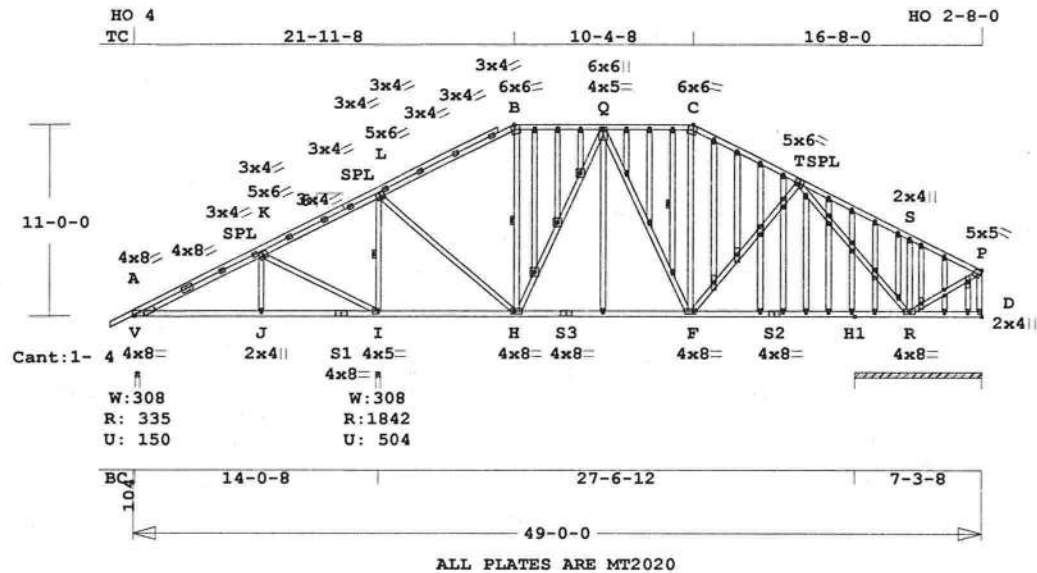
Q	MT20	4.0x 5.0	Ctr Ctr	0.23
W	MT20	4.0x 5.0 <td>Ctr Ctr <td>0.23</td> </td>	Ctr Ctr <td>0.23</td>	0.23
R	MT20	5.0x 6.0	0.2 0.5	0.43
S	MT20	5.0x 5.0	Ctr Ctr	0.40
M	MT20	2.0x 4.0	Ctr Ctr	0.29
T	MT20	4.0x 8.0	Ctr Ctr	0.25
H	MT20	2.0x 4.0	Ctr Ctr	0.58
G	MT20	6.0x10.0	Ctr	1.2 0.57
L	MT20	4.0x 8.0	Ctr Ctr	0.38
S1	MT20	5.0x 8.0	Ctr	0.8 0.88
J	MT20	4.0x 8.0	Ctr Ctr	0.23
F	MT20	6.0x10.0	0.1 1.0	0.38
E	MT20	2.0x 4.0	Ctr Ctr	0.58
V	MT20	4.0x 8.0	Ctr Ctr	0.65
I	MT20	4.0x 5.0	Ctr Ctr	0.44
D	MT20	2.0x 4.0	Ctr Ctr	0.32

NOTES:
 Trusses Manufactured by:
 RIDGWAY ROOF TRUSS
 Analysis Conforms To:
 FBC2007
 OH Loading
 Soffit psf 2.0
 Design checked for 10 psf non-concurrent LL on BC.
 Wind Loads - ANSI / ASCE 7-05
 Truss is designed as
 Components and Claddings*
 for Exterior zone location.
 Wind Speed: 110 mph
 Mean Roof Height: 16-0
 Exposure Category: B
 Occupancy Factor : 1.00
 Building Type: Enclosed
 TC Dead Load: 4.0 psf
 BC Dead Load: 6.0 psf
 User-defined wind-exposed BC regions --From-- --To--
 0- 1- 4 12- 1-12
 Max comp. force 1480 Lbs
 Max tens. force 1275 Lbs
 Quality Control Factor 1.25
 FABRICATOR NOTES:

1. Delegated Engineer (Truss Designer)
 Gary Dounson, PE 35054
 Gary Dounson & Associates, Inc.
 2830 NW 41st Street Suite D
 Gainesville, FL 32606
 (352)375-8593
 CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
090117	R8	1	SP	490000	6	0	0	

Peden Residence



Scale: 0.094" = 1'

Online Plus -- Version 23.0.052
RUN DATE: 13-APR-09

CSI -Size- ---Lumber---
TC 0.56 2x 4 SP-#2
BC 0.33 2x 4 SP-#2
WB 0.73 2x 4 SP-#3
GW 0.16 2x 4 SP-#3

Brace truss as follows:
O.C. From To
TC Cont. 0-0-0 49-0-0
BC Cont. 0-0-0 49-0-0
One Continuous Lateral Brace
I-L H-B D0-C9
Attach CLB with (2)-10d nails
at each web.

psf-Ld Dead Live
TC 7.0 20.0
BC 10.0 0.0
TC+BC 17.0 20.0
Total 37.0 Spacing 24.0"
Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)
Jt Down Uplift Horiz
V 336 150 U 225 R
I 1842 505 U
H1 1415 218 U 274 R

Jt Brg Size Required
V 3.5" 1.5"
I 3.5" 2.0"
H1 87.5" 501"-to- 588"

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)
Plus 1 DL Load Case(s)

Membr CSI P Lbs Axl-CSI-Bnd
-----Top Chords-----
A-K 0.40 315 T 0.00 0.40
K-L 0.56 325 T 0.06 0.50
L-B 0.50 512 C 0.04 0.46
B-A 0.27 449 C 0.05 0.22
A8-B2 0.10 451 C 0.05 0.05
B2-B6 0.07 454 C 0.05 0.02
B6-Q 0.16 457 C 0.00 0.16
Q-C1 0.16 637 C 0.00 0.16
C1-C5 0.10 636 C 0.06 0.04
C5-C9 0.12 633 C 0.06 0.06
C9-C 0.12 631 C 0.06 0.06
C-D1 0.17 660 C 0.06 0.11
D1-D5 0.16 695 C 0.06 0.10
D5-D9 0.12 710 C 0.06 0.06
D9-E3 0.07 720 C 0.05 0.02
E3-T 0.14 769 C 0.05 0.09
T-E9 0.12 177 T 0.01 0.11
E9-F3 0.10 149 T 0.00 0.10
F3-F7 0.09 96 T 0.01 0.08
F7-G1 0.05 93 T 0.01 0.04
G1-S 0.03 72 T 0.01 0.02
S-G3 0.03 93 T 0.01 0.02

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 639.9 LBS

G3-G5 0.04 77 T 0.01 0.03
G5-P 0.04 103 C 0.01 0.03

-----Bottom Chords-----

A-V 0.00 0 T
A-J 0.31 264 T 0.04 0.27
J-S1 0.28 264 T 0.02 0.26
S1-I 0.33 264 T 0.02 0.31
I-H 0.31 470 T 0.00 0.31
H-S3 0.32 623 T 0.06 0.26
S3-C0 0.14 623 T 0.06 0.08
C0-F 0.23 622 T 0.10 0.13
F-E2 0.22 563 T 0.09 0.13
E2-S2 0.19 554 T 0.09 0.10
S2-E6 0.11 554 T 0.09 0.02
E6-E8 0.10 548 T 0.09 0.01
E8-F2 0.09 548 T 0.09 0.00
F2-F6 0.15 543 T 0.09 0.06
H1-G0 0.24 537 T 0.09 0.15
G0-R 0.09 534 T 0.08 0.01
R-G8 0.02 45 T 0.00 0.02
G8-H0 0.01 43 T 0.00 0.01
H0-D 0.00 43 T

-----Webs-----

J-K 0.07 286 T
K-I 0.61 754 T
I-L 0.36 1417 C 1 Br
L-H 0.73 968 T
H-B 0.07 233 T
H-A9 0.08 384 C 0.03 0.05
A9-B3 0.12 417 C 0.04 0.08
B3-B7 0.11 387 C 0.03 0.08
B7-Q 0.07 393 C 0.03 0.04
Q-C2 0.03 116 C 0.01 0.02
C2-C6 0.09 114 C 0.01 0.08
C6-D0 0.08 93 C 0.00 0.08
D0-F 0.03 93 C 0.01 0.02
F-C 0.05 126 T
F-D2 0.11 119 T 0.02 0.09
D2-D6 0.17 103 T 0.03 0.14
D6-E0 0.18 138 T 0.04 0.14
E0-E5 0.22 173 T 0.05 0.17
E5-T 0.19 211 T 0.06 0.13
T-E7 0.47 955 C 0.11 0.36
E7-F0 0.47 955 C 0.11 0.36
F0-F4 0.32 969 C 0.12 0.20
F4-F9 0.19 944 C 0.11 0.08
F9-G2 0.15 926 C 0.10 0.05
G2-R 0.16 932 C 0.11 0.05
R-S 0.03 82 C
R-G4 0.06 205 T 0.05 0.01
G4-G9 0.05 189 T 0.04 0.01
G9-P 0.04 191 T 0.04 0.00
D-P 0.04 98 C WindLd

-----Gable Webs-----

A9-A8 0.07 48 C 0.01 0.06
B3-B2 0.07 31 C 0.00 0.07
B7-B6 0.03 8 T 0.00 0.03
C0-Q 0.06 169 T 0.02 0.04
C2-C1 0.01 4 C 0.00 0.01
C6-C5 0.06 30 C 0.00 0.06
D0-C9 0.08 6 T 0.00 0.08
D2-D1 0.12 16 T 0.00 0.12
D6-D5 0.12 64 T 0.00 0.12
E0-D9 0.08 108 T 0.00 0.08
E2-E0 0.15 112 T 0.00 0.15
E5-E3 0.04 51 T 0.01 0.03
E6-E5 0.16 78 T 0.02 0.14

E8-E7 0.01 46 T 0.01 0.00
F0-E9 0.15 46 T 0.00 0.15
F2-F0 0.10 40 T 0.00 0.10
F4-F3 0.05 168 C 0.00 0.05
F6-F4 0.09 202 C 0.00 0.09
F9-F7 0.03 81 T 0.02 0.01
G0-F9 0.02 96 T 0.00 0.02
G2-G1 0.00 19 T
G4-G3 0.01 27 T 0.00 0.01
G8-G5 0.05 181 T 0.04 0.01
H0-G9 0.00 5 C

TL Defl -0.14" in V -J L/999
LL Defl -0.07" in V -J L/999
Shear // Grain in L-B 0.26

Plates for each ply each face.

Plate - MT20 20 Ga, Gross Area

Plate - MT2H 20 Ga, Gross Area

Jt Type Plt Size X Y JSI

A MT20 4.0x 8.0 0.7 0.1 0.33

K MT20 5.0x 6.0-0.2 0.5 0.43

L MT20 5.0x 6.0-0.2 0.5 0.43

B MT20 6.0x 6.0-0.9-3.6 0.33

A8 MT20 2.0x 4.0 Ctr Ctr 0.29

B2 MT20 2.0x 4.0 Ctr Ctr 0.29

B6 MT20 2.0x 4.0 Ctr Ctr 0.29

Q MT20 4.0x 5.0 Ctr Ctr 0.20

C1 MT20 2.0x 4.0 Ctr Ctr 0.29

C5 MT20 2.0x 4.0 Ctr Ctr 0.29

C9 MT20 2.0x 4.0 Ctr Ctr 0.29

C MT20 6.0x 6.0-0.9-3.6 0.33

D1 MT20 2.0x 4.0 Ctr Ctr 0.23

D5 MT20 2.0x 4.0 Ctr Ctr 0.23

D9 MT20 2.0x 4.0 Ctr Ctr 0.23

E3 MT20 2.0x 4.0 Ctr Ctr 0.23

T MT20 5.0x 6.0 0.2 0.5 0.45

E9 MT20 2.0x 4.0 Ctr Ctr 0.23

F3 MT20 2.0x 4.0 Ctr Ctr 0.23

F7 MT20 2.0x 4.0 Ctr Ctr 0.23

G1 MT20 2.0x 4.0 Ctr Ctr 0.23

S MT20 2.0x 4.0 Ctr Ctr 0.23

G3 MT20 2.0x 4.0 Ctr Ctr 0.23

G5 MT20 2.0x 4.0 Ctr Ctr 0.23

P MT20 5.0x 5.0 Ctr Ctr 0.16

J MT20 2.0x 4.0 Ctr Ctr 0.29

S1 MT20 4.0x 6.0 Ctr Ctr 0.23

I MT20 4.0x 5.0 Ctr Ctr 0.33

H MT20 4.0x 8.0 Ctr Ctr 0.39

S3 MT20 4.0x 8.0 Ctr Ctr 0.23

C0 MT20 2.0x 4.0 Ctr Ctr 0.29

F MT20 4.0x 8.0 Ctr Ctr 0.21

E2 MT20 2.0x 4.0 Ctr Ctr 0.29

S2 MT20 4.0x 8.0 Ctr Ctr 0.23

E6 MT20 2.0x 4.0 Ctr Ctr 0.29

E8 MT20 2.0x 4.0 Ctr Ctr 0.29

F2 MT20 2.0x 4.0 Ctr Ctr 0.29

F6 MT20 2.0x 4.0 Ctr Ctr 0.29

G0 MT20 2.0x 4.0 Ctr Ctr 0.29

R MT20 4.0x 8.0 Ctr Ctr 0.29

G8 MT20 2.0x 4.0 Ctr Ctr 0.29

H0 MT20 2.0x 4.0 Ctr Ctr 0.29

D MT20 2.0x 4.0 Ctr Ctr 0.29

A9 MT20 6.0x 6.0 Ctr Ctr 0.41

B3 MT20 6.0x 6.0 Ctr Ctr 0.41

B7 MT20 6.0x 6.0 Ctr Ctr 0.41

Q MT20 6.0x 6.0 Ctr Ctr 0.41

NOTES:

Trusses Manufactured by:

RIDGWAY ROOF TRUSS

Analysis Conforms To:

FBC2007

Design checked for 10 psf non-

concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-05

Truss is designed as

Components and Claddings*

for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 16-0

Exposure Category: B

Occupancy Factor: 1.00

Building Type: Enclosed

TC Dead Load: 4.0 psf

BC Dead Load: 6.0 psf

User-defined wind-exposed BC

regions --From-- --To--

0- 1- 4 14- 1-12

Max comp. force 1417 Lbs

Max tens. force 968 Lbs

Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss

Designer)

Gary Dounson, PE 35054

Gary Dounson & Associates,

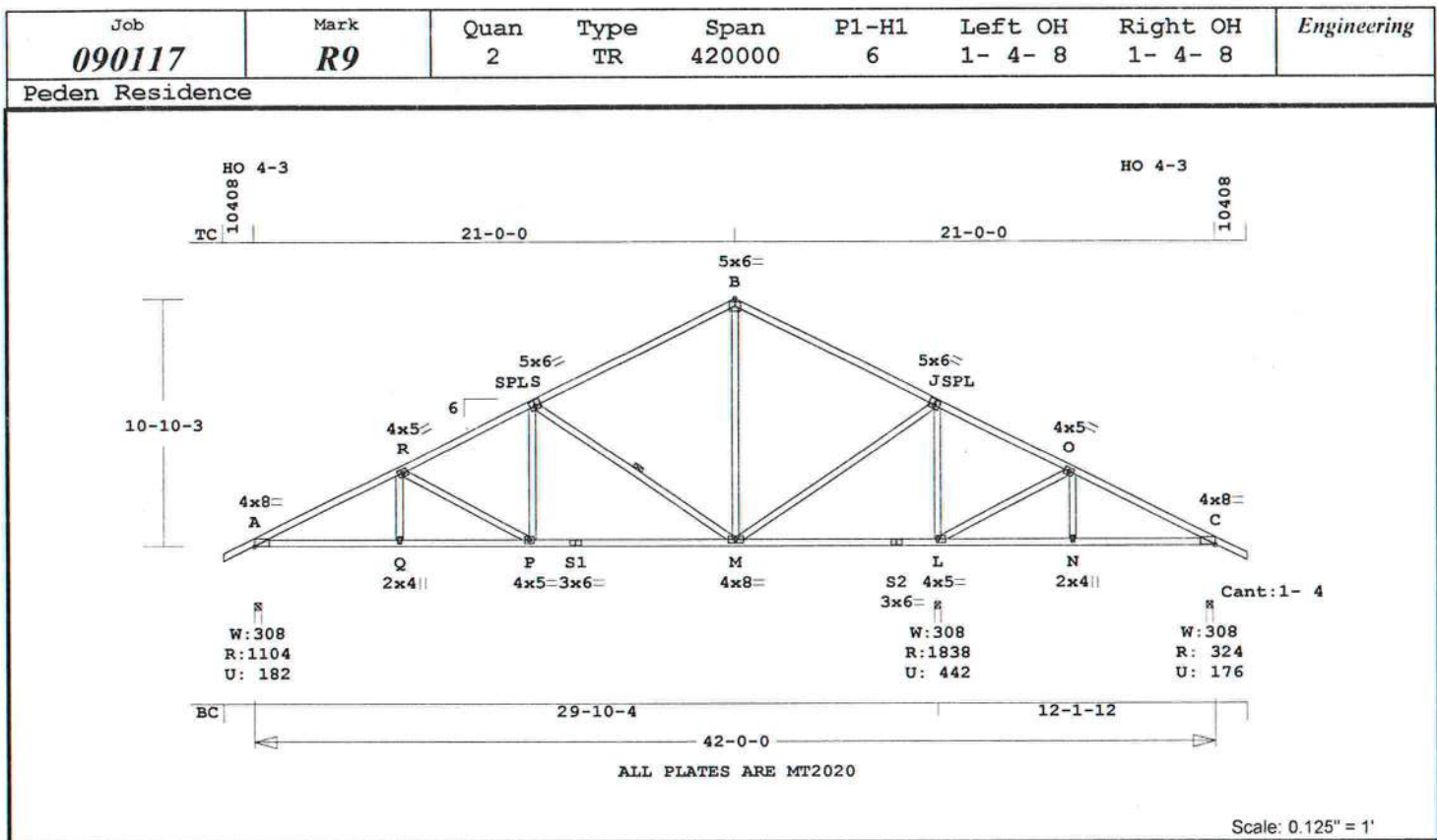
Inc.

2830 NW 41st Street Suite D

Gainesville, FL 32606

(352)375-8593

CA 5201



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 290.1 LBS
 Online Plus -- Version 23.0.052
 RUN DATE: 13-APR-09

CSI -Size- ----Lumber----
 TC 0.60 2x 4 SP-#2
 BC 0.57 2x 4 SP-#2
 WB 0.99 2x 4 SP-#3

Brace truss as follows:

O.C. From To
 TC Cont. 0- 0- 0 42- 0- 0
 BC Cont. 0- 0- 0 42- 0- 0
 One Continuous Lateral Brace
 S -M
 Attach CLB with (2)-10d nails
 at each web.

psf-Ld Dead Live
 TC 7.0 20.0
 BC 10.0 0.0
 TC+BC 17.0 20.0
 Total 37.0 Spacing 24.0"
 Lumber Duration Factor 1.25
 Plate Duration Factor 1.25
 TC Fb=1.15 Fc=1.10 Ft=1.10
 BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)

Jt Down Uplift Horiz-
 A 1105 182 U 231 R
 L 1838 442 U
 C 324 177 U 230 R

Jt Brg Size Required
 A 3.5" 1.5"
 L 3.5" 2.0"
 C 3.5" 1.5"

Plus 9 Wind Load Case(s)
 Plus 1 UBC LL Load Case(s)
 Plus 1 DL Load Case(s)

Membr CSI P Lbs Axl-CSI-Bnd

-----Top Chords-----
 A -R 0.32 1748 C 0.11 0.21
 R -S 0.56 1306 C 0.08 0.48
 S -B 0.53 583 C 0.05 0.48
 B -J 0.53 583 C 0.05 0.48
 J -O 0.60 401 T 0.07 0.53

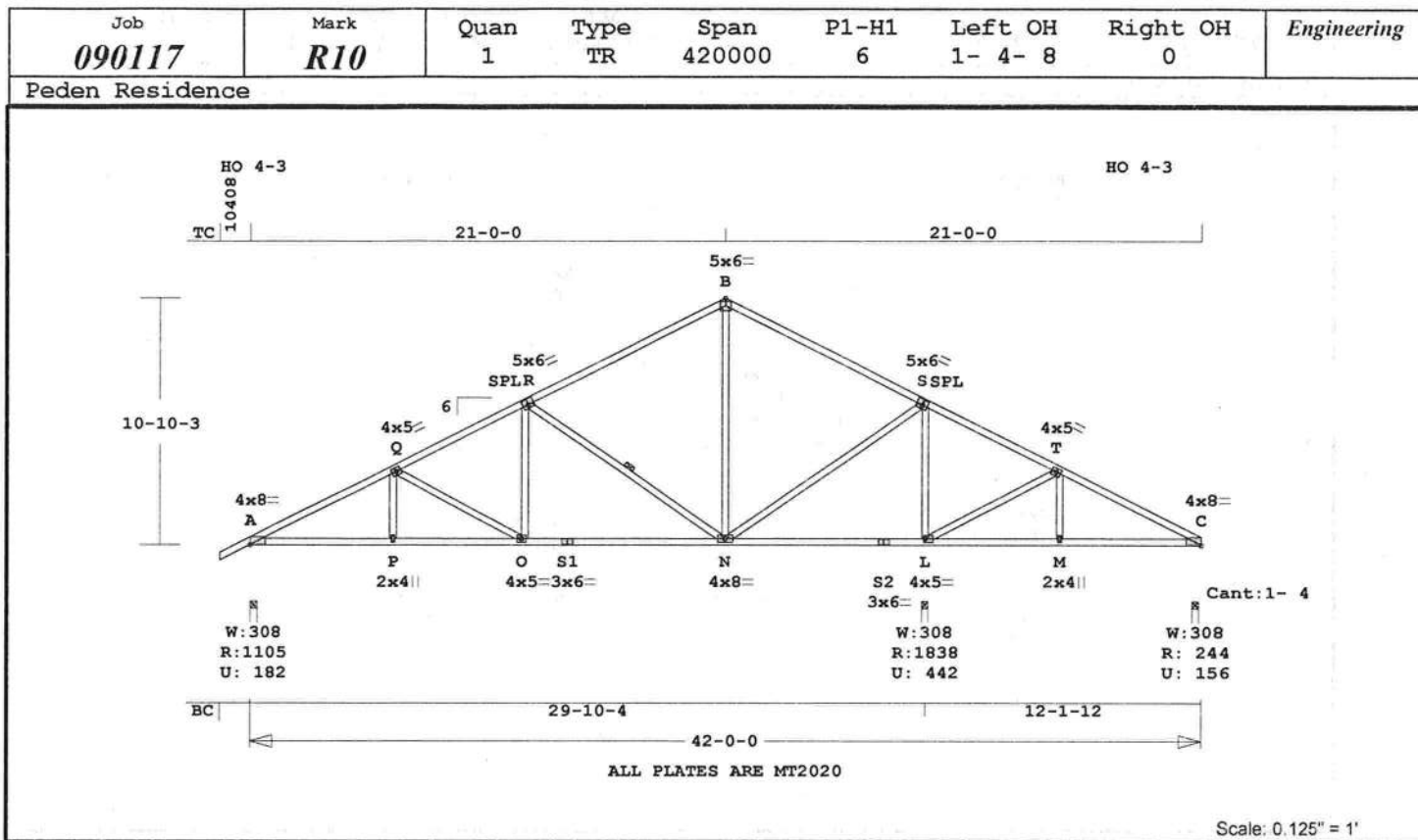
O -C 0.29 348 T 0.04 0.25
 -----Bottom Chords-----
 A -Q 0.37 1564 T 0.26 0.11
 Q -P 0.45 1564 T 0.15 0.30
 P -S1 0.41 1180 T 0.11 0.30
 S1-M 0.57 1180 T 0.11 0.46
 M -S2 0.46 354 T 0.00 0.46
 S2-L 0.31 354 T 0.00 0.31
 L -N 0.31 198 C 0.00 0.31
 N -C 0.21 198 C 0.00 0.21
 -----Webs-----
 Q -R 0.05 218 T
 R -P 0.33 435 C
 P -S 0.12 455 T
 S -M 0.43 803 C 1 Br
 M -B 0.06 243 T
 M -J 0.78 1056 T
 L -J 0.99 1458 C
 L -O 0.38 629 T
 N -O 0.06 237 T

TL Defl -0.24" in P -M L/999
 LL Defl -0.11" in P -M L/999
 Shear // Grain in S -B 0.29

Plates for each ply each face.
 Plate - MT20 20 Ga, Gross Area
 Plate - MT2H 20 Ga, Gross Area
 Jt Type Plt Size X Y JSI
 A MT20 4.0x 8.0 0.8 0.3 0.35
 R MT20 4.0x 5.0 Ctr Ctr 0.23
 S MT20 5.0x 6.0-0.2 0.5 0.43
 B MT20 5.0x 6.0 Ctr-0.1 0.33
 J MT20 5.0x 6.0 0.2 0.5 0.43
 O MT20 4.0x 5.0 Ctr Ctr 0.23
 C MT20 4.0x 8.0-0.8 0.3 0.35
 Q MT20 2.0x 4.0 Ctr Ctr 0.29
 P MT20 4.0x 5.0 Ctr Ctr 0.32
 S1 MT20 3.0x 6.0 Ctr Ctr 0.39
 M MT20 4.0x 8.0 Ctr Ctr 0.42
 S2 MT20 3.0x 6.0 Ctr Ctr 0.39
 L MT20 4.0x 5.0 Ctr Ctr 0.32
 N MT20 2.0x 4.0 Ctr Ctr 0.29

NOTES:
 Trusses Manufactured by:
 RIDGWAY ROOF TRUSS
 Analysis Conforms To:

FBC2007
 OH Loading
 Soffit psf 2.0
 Design checked for 10 psf non-
 concurrent LL on BC.
 Wind Loads - ANSI / ASCE 7-05
 Truss is designed as
 Components and Claddings*
 for Exterior zone location.
 Wind Speed: 110 mph
 Mean Roof Height: 16-0
 Exposure Category: B
 Occupancy Factor: 1.00
 Building Type: Enclosed
 TC Dead Load: 4.0 psf
 BC Dead Load: 6.0 psf
 User-defined wind-exposed BC
 regions --From-- --To--
 29-10- 4 41-10-12
 Max comp. force 1748 Lbs
 Max tens. force 1564 Lbs
 Quality Control Factor 1.25
 FABRICATOR NOTES:
 1. Delegated Engineer (Truss
 Designer)
 Gary Dounson, PE 35054
 Gary Dounson & Associates,
 Inc.
 2830 NW 41st Street Suite D
 Gainesville, FL 32606
 (352)375-8593
 CA 5201



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 287.2 LBS

Online Plus -- Version 23.0.052
RUN DATE: 13-APR-09

CSI -Size- ----Lumber-----
TC 0.60 2x 4 SP-#2
BC 0.57 2x 4 SP-#2
WB 0.99 2x 4 SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	42- 0- 0
BC Cont.	0- 0- 0	42- 0- 0

One Continuous Lateral Brace

R -N

Attach CLB with (2)-10d nails
at each web.

psf-Ld	Dead	Live
TC	7.0	20.0
BC	10.0	0.0
TC+BC	17.0	20.0
Total	37.0	Spacing 24.0"
Lumber Duration Factor	1.25	
Plate Duration Factor	1.25	
TC Fb=1.15	Fc=1.10	Ft=1.10
BC Fb=1.10	Fc=1.10	Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	1105	182 U	231 R
L	1838	442 U	
C	244	157 U	230 R

Jt	Brg Size	Required
A	3.5"	1.5"
L	3.5"	2.0"
C	3.5"	1.5"

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)
Plus 1 DL Load Case(s)

Membr CSI P Lbs Ax1-CSI-Bnd

-----Top Chords-----
A -Q 0.32 1749 C 0.11 0.21
Q -R 0.56 1307 C 0.08 0.48
R -B 0.53 583 C 0.05 0.48
B -S 0.53 583 C 0.05 0.48
S -T 0.60 400 T 0.07 0.53

T -C	0.29	354	T	0.04	0.25
-----Bottom Chords-----					
A -P	0.37	1565	T	0.26	0.11
P -O	0.45	1565	T	0.15	0.30
O -S1	0.41	1181	T	0.11	0.30
S1 -N	0.57	1181	T	0.11	0.46
N -S2	0.46	352	T	0.00	0.46
S2 -L	0.31	352	T	0.00	0.31
L -M	0.31	204	C	0.00	0.31
M -C	0.21	204	C	0.00	0.21
-----Webs-----					
P -Q	0.05	218	T		
Q -O	0.33	435	C		
O -R	0.12	455	T		
R -N	0.43	803	C		1 Br
N -B	0.06	243	T		
N -S	0.77	1055	T		
L -S	0.99	1457	C		
L -T	0.38	633	T		
M -T	0.06	237	T		

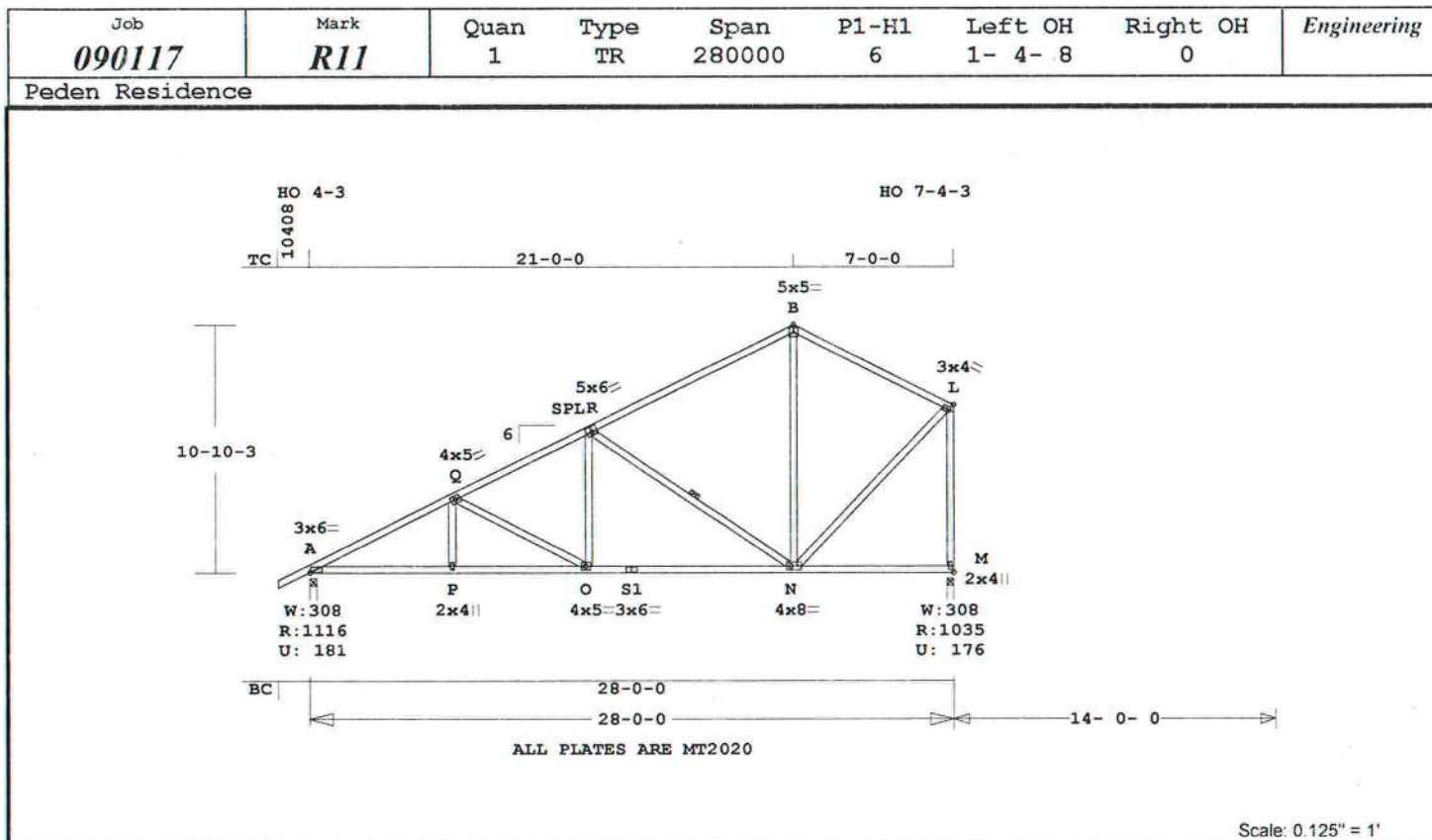
TL Defl -0.24" in O -N L/999
LL Defl -0.11" in O -N L/999
Shear // Grain in R -B 0.29

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A MT20 4.0x 8.0 0.8 0.3 0.35
Q MT20 4.0x 5.0 Ctr Ctr 0.23
R MT20 5.0x 6.0-0.2 0.5 0.43
B MT20 5.0x 6.0 Ctr-0.1 0.33
S MT20 5.0x 6.0 0.2 0.5 0.42
T MT20 4.0x 5.0 Ctr Ctr 0.23
C MT20 4.0x 8.0-0.8 0.3 0.35
P MT20 2.0x 4.0 Ctr Ctr 0.29
O MT20 4.0x 5.0 Ctr Ctr 0.20
S1 MT20 3.0x 6.0 Ctr Ctr 0.39
N MT20 4.0x 8.0 Ctr Ctr 0.42
S2 MT20 3.0x 6.0 Ctr Ctr 0.39
L MT20 4.0x 5.0 Ctr Ctr 0.32
M MT20 2.0x 4.0 Ctr Ctr 0.29

NOTES:

Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:

FBC2007
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-
concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-05
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 16-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
User-defined wind-exposed BC
regions --From-- ---To---
29-10- 4 41-10-12
Max comp. force 1749 Lbs
Max tens. force 1565 Lbs
Quality Control Factor 1.25
FABRICATOR NOTES:
1. Delegated Engineer (Truss
Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates,
Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 213.9 LBS
 Online Plus -- Version 23.0.052
 RUN DATE: 06-APR-09

CSI -Size- ---Lumber---
 TC 0.57 2x 4 SP-#2
 BC 0.53 2x 4 SP-#2
 WB 0.89 2x 4 SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	28- 0- 0
BC	Cont.	0- 0- 0	28- 0- 0

One Continuous Lateral Brace
 R -N
 Attach CLB with (2)-10d nails
 at each web.

psf-Ld Dead Live
 TC 7.0 20.0
 BC 10.0 0.0
 TC+BC 17.0 20.0
 Total 37.0 Spacing 24.0"
 Lumber Duration Factor 1.25
 Plate Duration Factor 1.25
 TC Fb=1.15 Fc=1.10 Ft=1.10
 BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)
 Jt Down Uplift Horiz-
 A 1116 181 U 190 R
 M 1036 176 U 326 R

Jt	Brg Size	Required
A	3.5"	1.5"
M	3.5"	1.5"

Plus 9 Wind Load Case(s)
 Plus 1 UBC LL Load Case(s)
 Plus 1 DL Load Case(s)

Membr CSI P Lbs Ax1-CSI-Bnd
 -----Top Chords-----
 A -Q 0.32 1772 C 0.11 0.21

Q -R	0.57	1332 C	0.09	0.48
R -B	0.55	605 C	0.00	0.55
B -L	0.48	586 C	0.06	0.42
-----Bottom Chords-----				
A -P	0.37	1585 T	0.26	0.11
P -O	0.47	1585 T	0.16	0.31
O -S1	0.43	1205 T	0.12	0.31
S1 -N	0.53	1205 T	0.12	0.41
N -M	0.41	232 T	0.00	0.41
-----Webs-----				
P -Q	0.05	215 T		
Q -O	0.33	431 C		
O -R	0.12	459 T		
R -N	0.43	810 C		1 Br
N -B	0.07	278 T		
N -L	0.50	775 T		
M -L	0.89	983 C	WindLd	

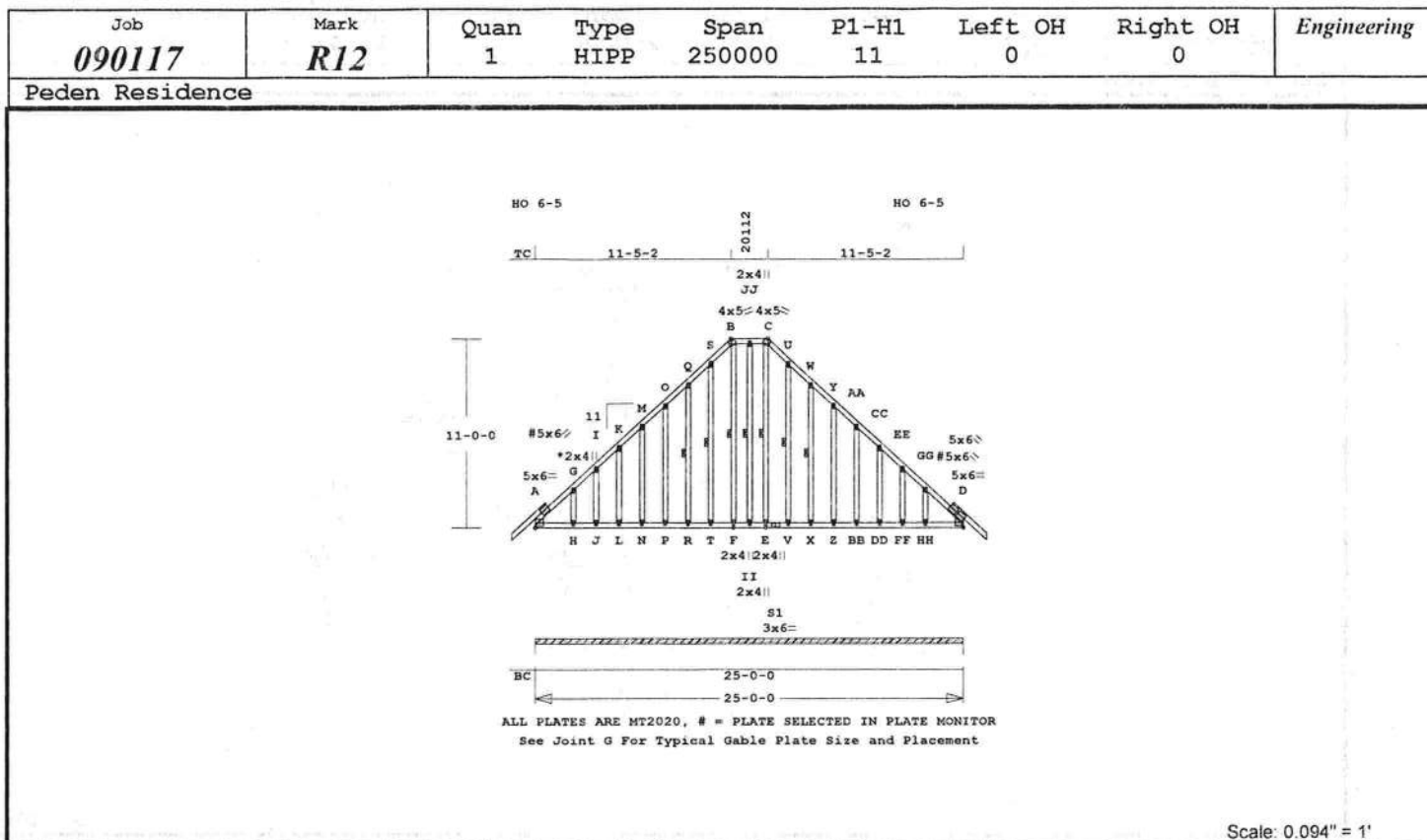
TL Defl -0.25" in O -N L/999
 LL Defl -0.12" in O -N L/999
 Shear // Grain in R -B 0.29

Plates for each ply each face.
 Plate - MT20 20 Ga, Gross Area
 Plate - MT2H 20 Ga, Gross Area
 Jt Type Plt Size X Y JSI
 A MT20 3.0x 6.0 Ctr Ctr 0.53
 Q MT20 4.0x 5.0 Ctr Ctr 0.23
 R MT20 5.0x 6.0-0.2 0.5 0.43
 B MT20 5.0x 5.0 Ctr Ctr 0.34
 L MT20 3.0x 4.0 Ctr Ctr 0.50
 P MT20 2.0x 4.0 Ctr Ctr 0.29
 O MT20 4.0x 5.0 Ctr Ctr 0.20
 S1 MT20 3.0x 6.0 Ctr Ctr 0.39
 N MT20 4.0x 8.0 Ctr Ctr 0.32
 M MT20 2.0x 4.0 Ctr Ctr 0.31

NOTES:
 Trusses Manufactured by:
 RIDGWAY ROOF TRUSS
 Analysis Conforms To:
 FBC2007

OH Loading
 Soffit psf 2.0
 Design checked for 10 psf non-
 concurrent LL on BC.
 Wind Loads - ANSI / ASCE 7-05
 Truss is designed as
 Components and Claddings*
 for Exterior zone location.
 Wind Speed: 110 mph
 Mean Roof Height: 16-0
 Exposure Category: B
 Occupancy Factor : 1.00
 Building Type: Enclosed
 TC Dead Load: 4.0 psf
 BC Dead Load: 6.0 psf
 Max comp. force 1772 Lbs
 Max tens. force 1585 Lbs
 Quality Control Factor 1.25
 FABRICATOR NOTES:

1. Delegated Engineer (Truss
 Designer)
 Gary Dounson, PE 35054
 Gary Dounson & Associates,
 Inc.
 2830 NW 41st Street Suite D
 Gainesville, FL 32606
 (352) 375-8593
 CA 5201



Online Plus -- Version 23.0.052
RUN DATE: 06-APR-09

CSI -Size- ---Lumber---
TC 0.06 2x 4 SP-#2
BC 0.07 2x 4 SP-#2
WB 0.04 2x 4 SP-#3
GW 0.09 2x 4 SP-#3
PB --- 2x 4 SP-#3

Brace truss as follows:
O.C. From To
TC Cont. 0- 0- 0 25- 0- 0
BC Cont. 0- 0- 0 25- 0- 0
One Continuous Lateral Brace
F -B II-JJ E -C R -Q
T -S V -U X -W
Attach CLB with (2)-10d nails
at each web.

psf-Ld Dead Live
TC 7.0 20.0
BC 10.0 0.0
TC+BC 17.0 20.0
Total 37.0 Spacing 24.0"
Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)
Jt Down Uplift Horiz
A 1850 302 U 258 R
Jt Brg Size Required
A 300.0" 0"-to- 300"

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl	CSI-Bnd
Top Chords				
A -G	0.06	267 C	0.02	0.04
G -I	0.05	152 C	0.01	0.04
I -K	0.01	113 C	0.00	0.01
K -M	0.01	96 C	0.00	0.01
M -O	0.01	79 C	0.00	0.01
O -Q	0.02	126 T	0.01	0.01
Q -S	0.03	204 T	0.02	0.01
S -B	0.04	253 T	0.03	0.01
B -JJ	0.04	222 T	0.02	0.02
JJ -C	0.04	222 T	0.02	0.02
C -U	0.04	253 T	0.03	0.01
U -W	0.03	204 T	0.02	0.01
W -Y	0.02	126 T	0.01	0.01
Y -AA	0.01	79 C	0.00	0.01
AA -CC	0.01	96 C	0.00	0.01
CC -EE	0.01	113 C	0.00	0.01
EE -GG	0.05	152 C	0.01	0.04
GG -D	0.06	267 C	0.02	0.04
Bottom Chords				

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 331.4 LBS

A -H	0.07	1 T	0.00	0.07
H -J	0.02	0 T	0.00	0.02
J -L	0.00	0 T		
L -N	0.00	0 T		
N -P	0.00	0 T		
P -R	0.00	0 T		
R -T	0.00	0 T		
T -F	0.00	0 T		
F -II	0.00	0 T		
II -E	0.00	0 T		
E -S1	0.00	0 T		
S1 -V	0.00	0 T		
V -X	0.00	0 T		
X -Z	0.00	0 T		
Z -BB	0.00	0 T		
BB -DD	0.00	0 T		
DD -FF	0.00	0 T		
FF -HH	0.02	0 T	0.00	0.02
HH -D	0.07	1 T	0.00	0.07

Webs				
F -B	0.04	87 C	1 Br	
II -JJ	0.02	42 C	1 Br	
E -C	0.04	87 C	1 Br	

Gable Webs				
H -G	0.03	160 C		
J -I	0.02	82 C		
L -K	0.04	100 C		
N -M	0.06	99 C		
P -O	0.09	99 C		
R -Q	0.03	107 C	1 Br	
T -S	0.03	71 C	1 Br	
V -U	0.03	71 C	1 Br	
X -W	0.03	107 C	1 Br	
Z -Y	0.09	99 C		
BB -AA	0.06	99 C		
DD -CC	0.04	100 C		
FF -EE	0.02	82 C		
HH -GG	0.03	160 C		

TL Defl 0.00" in HH-D L/999
LL Defl 0.00" in HH-D L/999
Shear // Grain in A -G 0.07

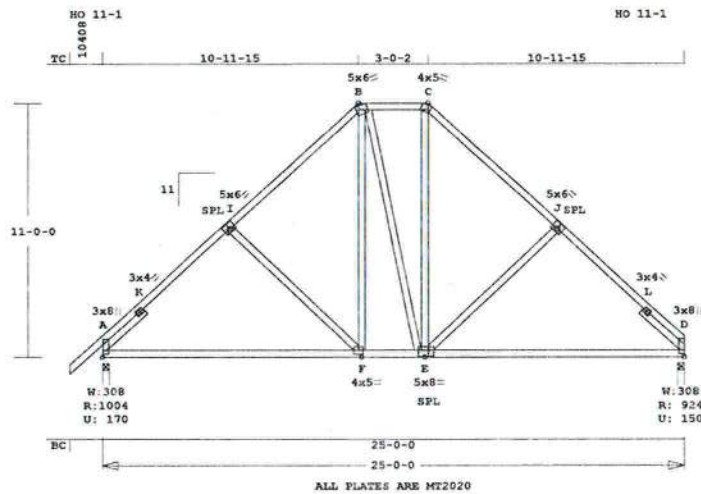
Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A MT20 5.0x 6.0 1.9 1.1 0.49
G MT20 2.0x 4.0 Ctr Ctr 0.00
I MT20 2.0x 4.0 Ctr Ctr 0.00
K MT20 2.0x 4.0 Ctr Ctr 0.00
M MT20 2.0x 4.0 Ctr Ctr 0.00
O MT20 2.0x 4.0 Ctr Ctr 0.00
Q MT20 2.0x 4.0 Ctr Ctr 0.00
S MT20 2.0x 4.0 Ctr Ctr 0.00
B MT20 4.0x 5.0 1.1-2.8 0.33
JJ MT20 2.0x 4.0 Ctr Ctr 0.29
C MT20 4.0x 5.0-1.1-2.8 0.33
U MT20 2.0x 4.0 Ctr Ctr 0.00
W MT20 2.0x 4.0 Ctr Ctr 0.00
Y MT20 2.0x 4.0 Ctr Ctr 0.00
AA MT20 2.0x 4.0 Ctr Ctr 0.00

CC MT20	2.0x 4.0	Ctr	Ctr	0.00
EE MT20	2.0x 4.0	Ctr	Ctr	0.00
GG MT20	2.0x 4.0	Ctr	Ctr	0.00
D MT20	5.0x 6.0-1.9	1.1	0.49	
H MT20	2.0x 4.0	Ctr	Ctr	0.00
J MT20	2.0x 4.0	Ctr	Ctr	0.00
L MT20	2.0x 4.0	Ctr	Ctr	0.00
N MT20	2.0x 4.0	Ctr	Ctr	0.00
P MT20	2.0x 4.0	Ctr	Ctr	0.00
R MT20	2.0x 4.0	Ctr	Ctr	0.00
T MT20	2.0x 4.0	Ctr	Ctr	0.00
F MT20	2.0x 4.0	Ctr	Ctr	0.29
II MT20	2.0x 4.0	Ctr	Ctr	0.29
E MT20	2.0x 4.0	Ctr	Ctr	0.29
S1 MT20	3.0x 6.0	Ctr	Ctr	0.39
V MT20	2.0x 4.0	Ctr	Ctr	0.00
X MT20	2.0x 4.0	Ctr	Ctr	0.00
Z MT20	2.0x 4.0	Ctr	Ctr	0.00
BB MT20	2.0x 4.0	Ctr	Ctr	0.00
DD MT20	2.0x 4.0	Ctr	Ctr	0.00
FF MT20	2.0x 4.0	Ctr	Ctr	0.00
HH MT20	2.0x 4.0	Ctr	Ctr	0.00

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2007
Design checked for 10 psf non-
concurrent LL on BC.
Refer to Gen Det 3 series for
web bracing and plating.
NOTE: USER MODIFIED PLATES
This design may have plates
selected through a plate
monitor.
Wind Loads - ANSI / ASCE 7-05
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 16-0
Exposure Category: B
Occupancy Factor: 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 267 Lbs
Max tens. force 253 Lbs
Quality Control Factor 1.25
FABRICATOR NOTES:
1. Delegated Engineer (Truss
Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates,
Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352) 375-8593
CA 5201

Job 090117	Mark R13	Quan 2	Type HIPP	Span 250000	Pl-H1 11	Left OH 1- 4- 8	Right OH 0	Engineering
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Peden Residence



Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 211.6 LBS

Online Plus -- Version 23.0.052
RUN DATE: 06-APR-09

CSI	Size	Lumber
TC	0.31	2x 4 SP-#2
BC	0.67	2x 4 SP-#2
WB	0.35	2x 4 SP-#3
SL	0.12	2x 4 SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	25- 0- 0
BC Cont.	0- 0- 0	25- 0- 0

psf-Ld	Dead	Live
TC	7.0	20.0
BC	10.0	0.0
TC+BC	17.0	20.0
Total	37.0	Spacing 24.0"
Lumber Duration Factor	1.25	
Plate Duration Factor	1.25	
TC Fb=1.15	Fc=1.10	Ft=1.10
BC Fb=1.10	Fc=1.10	Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz
A	1005	171 U	248 R
D	925	151 U	248 R

Jt	Brg Size	Required
A	3.5"	1.5"
D	3.5"	1.5"

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Ax1-CSI-Bnd
-----Top Chords-----			
A -K	0.14	857 C	0.00 0.14
K -I	0.31	932 C	0.05 0.26
I -B	0.31	748 C	0.05 0.26
B -C	0.14	572 C	0.05 0.09

C -J	0.31	749 C	0.05 0.26
J -L	0.31	933 C	0.05 0.26
L -D	0.14	855 C	0.00 0.14
-----Bottom Chords-----			
A -F	0.67	699 T	0.06 0.61
F -E	0.54	543 T	0.05 0.49
E -D	0.67	700 T	0.06 0.61
-----Webs-----			
I -F	0.35	321 C	
F -B	0.25	321 T	
B -E	0.29	148 T	
E -C	0.24	313 T	
E -J	0.35	323 C	
-----Sliders-----			
A -K	0.12	497 T	
L -D	0.12	491 T	

TL Defl -0.50" in A -F L/587
LL Defl -0.25" in A -F L/999
Shear // Grain in A -F 0.24

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A MT20 3.0x 8.0 1.5 0.7 0.61
K MT20 3.0x 4.0 Ctr Ctr 0.29
I MT20 5.0x 6.0-0.4 0.5 0.37
B MT20 5.0x 6.0 1.9-3.4 0.64
C MT20 4.0x 5.0-1.1-2.8 0.33
J MT20 5.0x 6.0 0.4 0.5 0.37
L MT20 3.0x 4.0 Ctr Ctr 0.29
D MT20 3.0x 8.0-1.5 0.7 0.61
F MT20 4.0x 5.0 Ctr Ctr 0.20
E MT20 5.0x 8.0-1.0-0.5 0.75

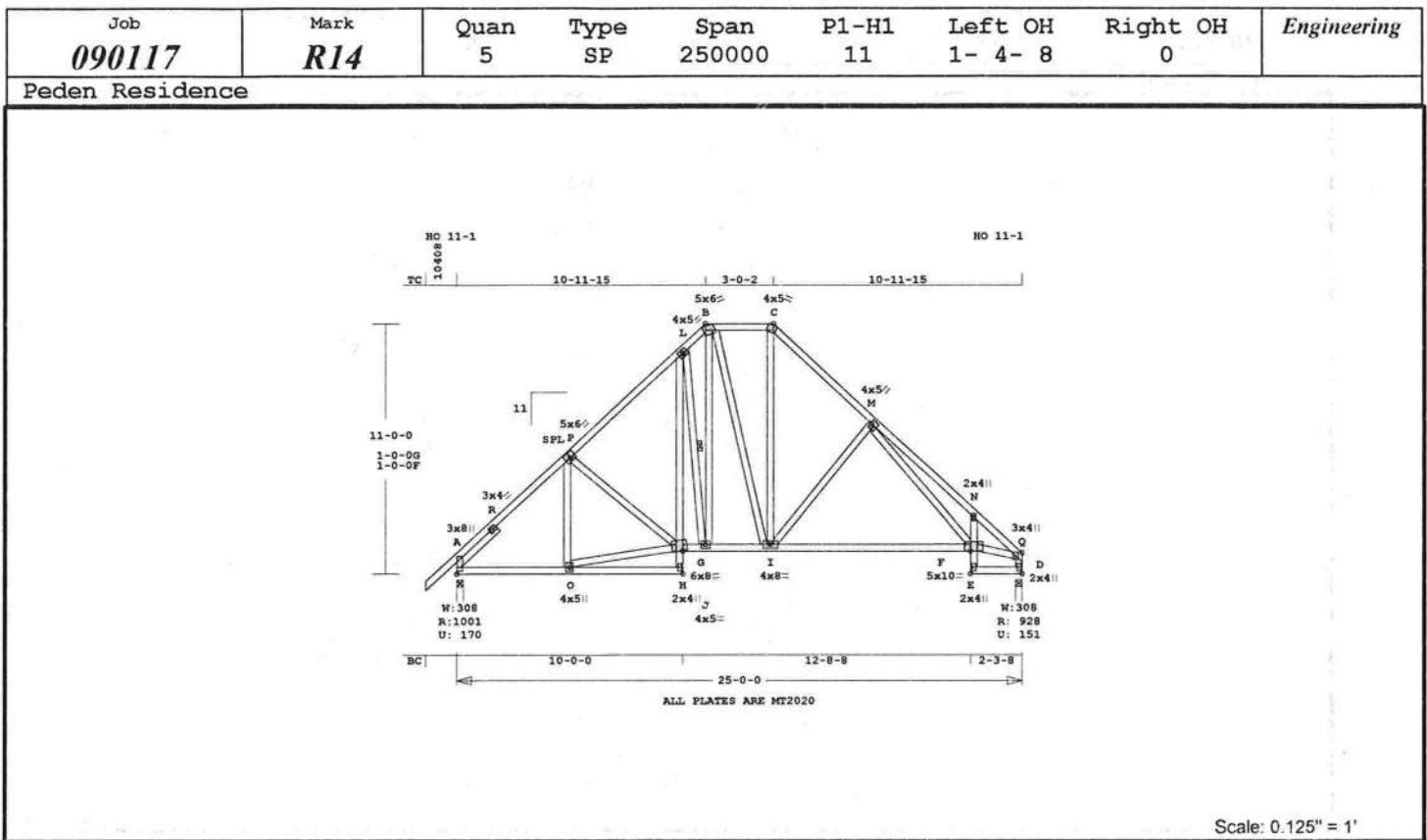
NOTES:

Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2007
OH Loading

Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-05
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 16-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 933 Lbs
Max tens. force 700 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 270.7 LBS
 Online Plus -- Version 23.0.052
 RUN DATE: 06-APR-09

CSI -Size- ----Lumber-----
 TC 0.28 2x 4 SP-#2
 BC 0.44 2x 4 SP-#2
 CW 0.73 2x 4 SP-#3
 WB 0.53 2x 4 SP-#3
 SL 0.14 2x 4 SP-#3

Brace truss as follows:
 O.C. From To
 TC Cont. 0- 0- 0 25- 0- 0
 BC Cont. 0- 0- 0 25- 0- 0
 One Continuous Lateral Brace
 L -J
 Attach CLB with (2)-10d nails
 at each web.

psf-Ld Dead Live
 TC 7.0 20.0
 BC 10.0 0.0
 TC+BC 17.0 20.0
 Total 37.0 Spacing 24.0"
 Lumber Duration Factor 1.25
 Plate Duration Factor 1.25
 TC Fb=1.15 Fc=1.10 Ft=1.10
 BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)
 Jt Down Uplift Horiz-
 A 1002 170 U 253 R
 D 928 151 U 263 R

Jt Brg Size Required
 A 3.5" 1.5"
 D 3.5" 1.5"

Plus 9 Wind Load Case(s)
 Plus 1 UBC LL Load Case(s)
 Plus 1 DL Load Case(s)

Membr CSI P Lbs Ax1-CSI-Bnd
 -----Top Chords-----
 A -R 0.19 341 C 0.00 0.19
 R -P 0.26 987 C 0.04 0.22
 P -L 0.27 856 C 0.05 0.22
 L -B 0.18 782 C 0.06 0.12
 B -C 0.09 594 C 0.05 0.04

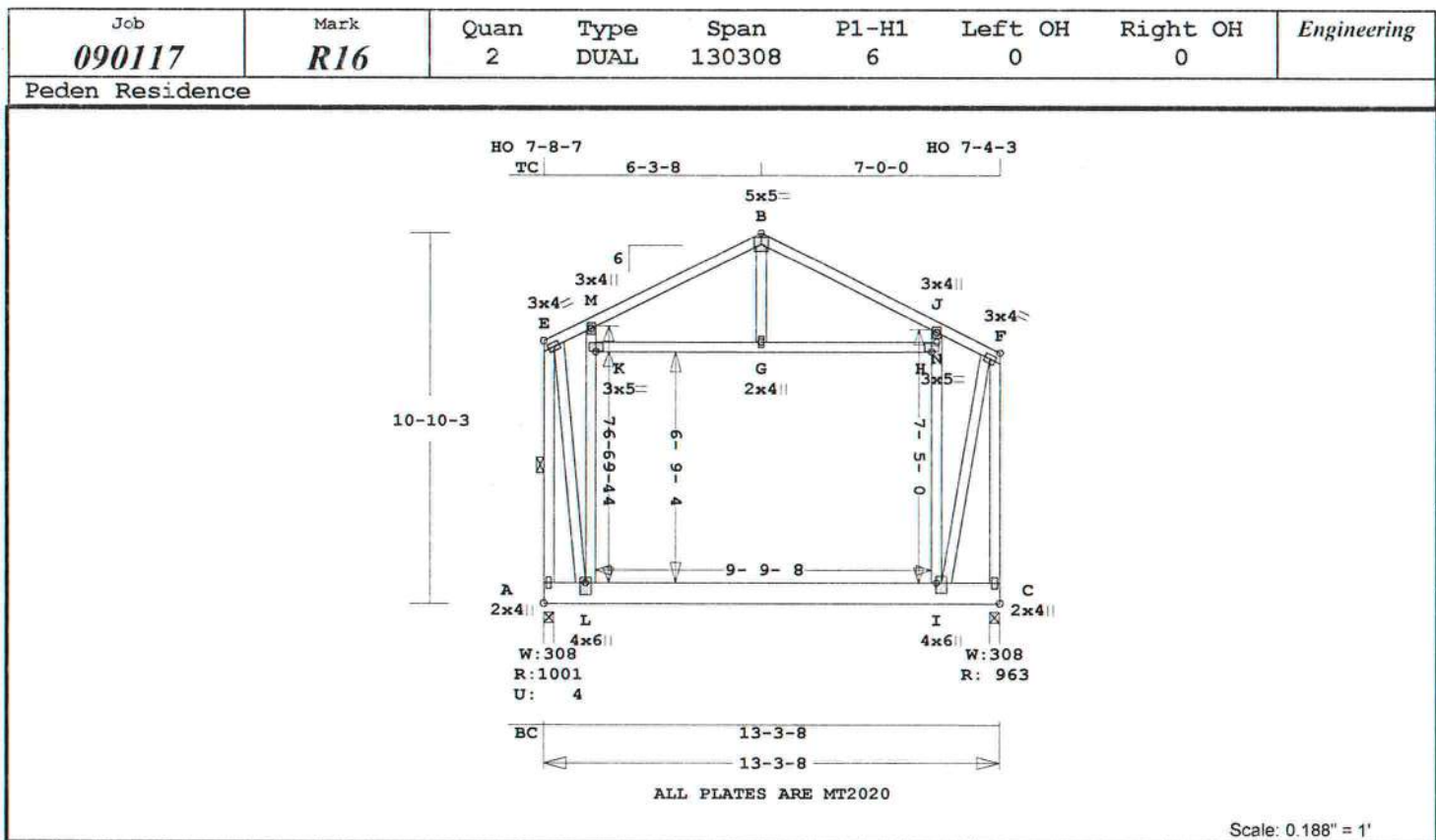
C -M 0.25 815 C 0.05 0.20	G -MT20 6.0x 8.0 Ctr 1.0 0.35
M -N 0.28 1559 C 0.08 0.20	J -MT20 4.0x 5.0 Ctr Ctr 0.20
N -Q 0.15 1490 C 0.06 0.09	I -MT20 4.0x 8.0 Ctr Ctr 0.23
-----Bottom Chords-----	
A -O 0.37 720 T 0.12 0.25	F -MT20 5.0x10.0 Ctr 2.5 0.42
O -H 0.14 58 T 0.00 0.14	E -MT20 2.0x 4.0 Ctr Ctr 0.58
G -J 0.21 632 T 0.10 0.11	D -MT20 2.0x 4.0 Ctr Ctr 0.29
J -I 0.42 577 T 0.05 0.37	
I -F 0.44 777 T 0.07 0.37	
E -D 0.03 254 T 0.00 0.03	
-----Chord-Webs-----	
H -G 0.22 85 T 0.01 0.21	
G -L 0.07 252 T 0.07 0.00	
E -F 0.73 39 T 0.00 0.73	
F -N 0.73 265 C 0.00 0.73	
-----Webs-----	
O -P 0.02 96 T	
O -G 0.21 674 T	
P -G 0.17 245 C	
L -J 0.15 402 C	1 Br
J -B 0.53 431 T	
B -I 0.15 166 T	
I -C 0.24 351 T	
I -M 0.32 363 C	
M -F 0.19 613 T	
F -Q 0.35 1114 T	
D -Q 0.14 905 C WindLd	
-----Sliders-----	
A -R 0.14 748 C	

TL Defl -0.28" in I -F L/999
 LL Defl -0.13" in I -F L/999
 Shear // Grain in E -F 0.25

Plates for each ply each face.
 Plate - MT20 20 Ga, Gross Area
 Plate - MT2H 20 Ga, Gross Area
 Jt Type Plt Size X Y JSI
 A MT20 3.0x 8.0 1.5 0.7 0.61
 R MT20 3.0x 4.0 Ctr Ctr 0.32
 P MT20 5.0x 6.0-0.4 0.5 0.52
 L MT20 4.0x 5.0 Ctr Ctr 0.27
 B MT20 5.0x 6.0 1.9-3.4 0.66
 C MT20 4.0x 5.0-1.1-2.8 0.33
 M MT20 4.0x 5.0 1.0-1.2 0.28
 N MT20 2.0x 4.0 Ctr Ctr 0.22
 Q MT20 3.0x 4.0 Ctr Ctr 0.65
 O MT20 4.0x 5.0 1.2 0.1 0.37
 H MT20 2.0x 4.0 Ctr Ctr 0.58

NOTES:
 Trusses Manufactured by:
 RIDGWAY ROOF TRUSS
 Analysis Conforms To:
 FBC2007
 OH Loading
 Soffit psf 2.0
 Design checked for 10 psf non-
 concurrent LL on BC.
 Wind Loads - ANSI / ASCE 7-05
 Truss is designed as
 Components and Claddings*
 for Exterior zone location.
 Wind Speed: 110 mph
 Mean Roof Height: 16-0
 Exposure Category: B
 Occupancy Factor: 1.00
 Building Type: Enclosed
 TC Dead Load: 4.0 psf
 BC Dead Load: 6.0 psf
 Max comp. force 1559 Lbs
 Max tens. force 1114 Lbs
 Quality Control Factor 1.25
 FABRICATOR NOTES:

1. Delegated Engineer (Truss Designer)
 Gary Dounson, PE 35054
 Gary Dounson & Associates, Inc.
 2830 NW 41st Street Suite D
 Gainesville, FL 32606
 (352)375-8593
 CA 5201



Online Plus -- Version 23.0.052
 RUN DATE: 06-APR-09

CSI -Size- ----Lumber----

TC	0.27	2x 4	SP-#2
BC	0.27	2x 8	SP-2400f-2.0E
WB	0.99	2x 4	SP-#3
ACT	0.10	2x 4	SP-#2
AWT	0.02	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	13- 3- 8
BC Cont.	0- 0- 0	13- 3- 8

One Continuous Lateral Brace
 A -E
 Attach CLB with (2)-10d nails
 at each web.

psf-Ld	Dead	Live
TC	7.0	20.0
BC	10.0	0.0
TC+BC	17.0	20.0
Total	37.0	Spacing 24.0"

Lumber Duration Factor 1.25
 Plate Duration Factor 1.25
 TC Fb=1.15 Fc=1.10 Ft=1.10
 BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	1002	4 U	57 R
C	964		56 R

Jt	Brg Size	Required
A	3.5"	1.5"
C	3.5"	1.5"

LC# 1 Attic Loading

Dur Fctrs	Lbr	1.25	Plt	1.25
plf - Dead	Live*	From	To	
TC V	14	40	0.0'	13.3'
BC V	20	0	0.0'	13.3'
BC V	10	80	1.5'	11.3'
MA V	10	0	1.4'	11.4'
MA V	10	0	0.3'	8.1'
MA V	10	0	0.3'	7.9'

Plus 9 Wind Load Case(s)

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 192.6 LBS

Plus 2 Unbalanced Load Cases
 Plus 1 UBC LL Load Case(s)
 Plus 1 DL Load Case(s)
 Plus 1 Attic 2 Load Case(s)

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Top Chords-----					
E -M	0.24	264	C	0.00	0.24
M -B	0.27	358	C	0.03	0.24
B -J	0.26	354	C	0.03	0.23
J -F	0.23	274	C	0.00	0.23

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Bottom Chords-----					
A -L	0.25	57	T	0.00	0.25
L -I	0.27	252	T	0.00	0.27
I -C	0.16	56	T	0.00	0.16

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Webs-----					
A -E	0.45	1635	C		1 Br
E -L	0.50	1583	T		
K -M	0.19	418	T	0.10	0.09
L -K	0.31	403	T	0.10	0.21
N -J	0.17	382	T	0.09	0.08
I -N	0.24	365	T	0.07	0.17
I -F	0.34	1079	T		
C -F	0.99	1186	C		

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Attic Chords (Top)-----					
K -G	0.10	74	C	0.00	0.10
G -N	0.10	74	C	0.00	0.10
-----Attic Webs (Top)-----					
G -B	0.02	70	T		

TL Defl	-0.18"	in L	-I	L/831
LL Defl	-0.14" <th>in L</th> <th>-I</th> <th>L/999</th>	in L	-I	L/999
Shear //	Grain <th>in A</th> <th>-L</th> <th>0.45</th>	in A	-L	0.45

Plates for each ply each face.

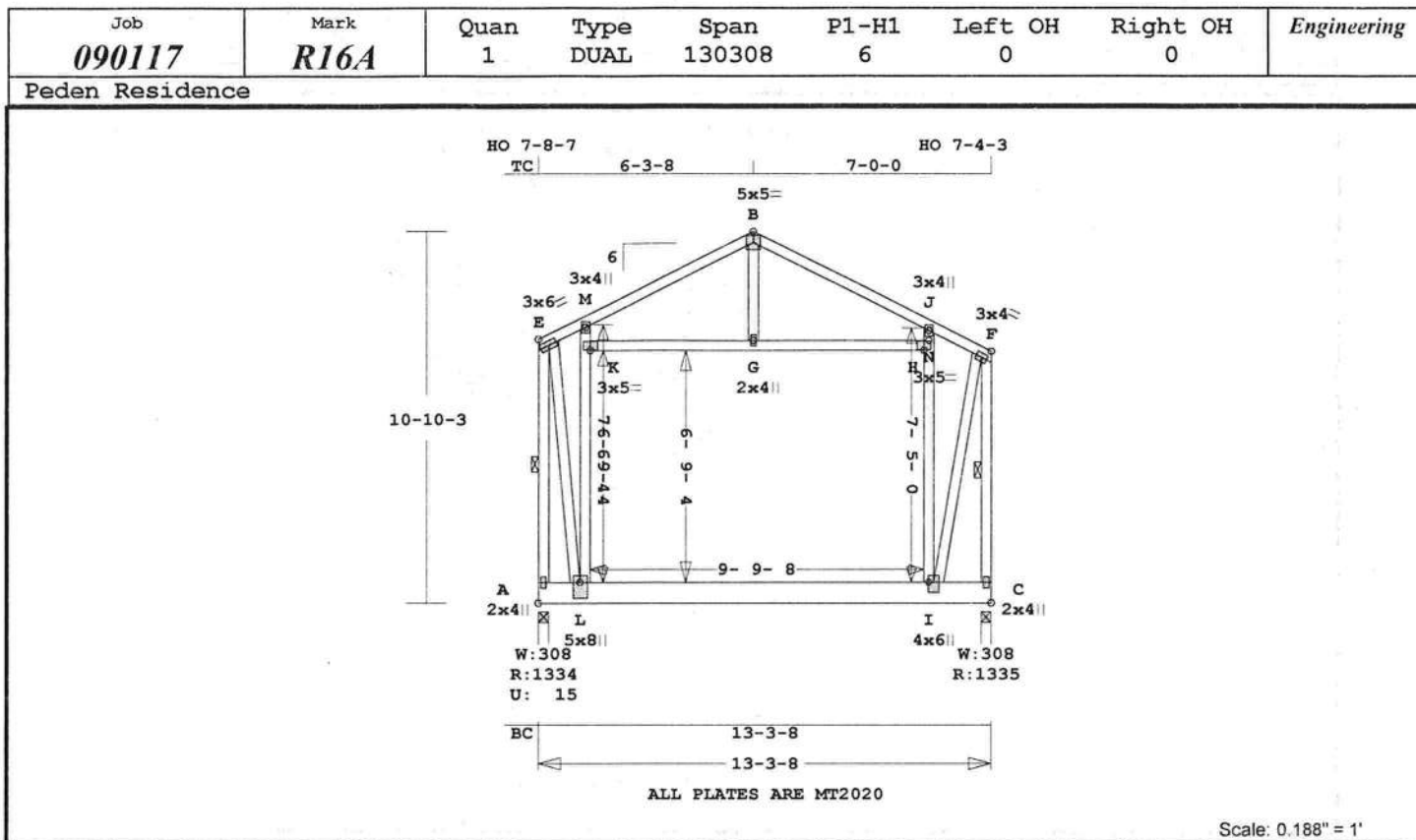
Plate	MT20	20 Ga,	Gross Area
Plate	MT2H	20 Ga, <td>Gross Area</td>	Gross Area

Jt	Type	Plt	Size	X	Y	JSI
E	MT20	3.0x	4.0	Ctr	Ctr	0.91
M	MT20	3.0x	4.0	Ctr	Ctr	0.12
B	MT20	5.0x	5.0	Ctr	Ctr	0.34
J	MT20	3.0x	4.0	Ctr	Ctr	0.11
F	MT20	3.0x	4.0	Ctr	Ctr	0.65
A	MT20	2.0x	4.0	Ctr	Ctr	0.34
L	MT20	4.0x	6.0	Ctr	-1.2	0.89
I	MT20	4.0x	6.0	Ctr	-0.6	0.61
C	MT20	2.0x	4.0	Ctr	Ctr	0.24
K	MT20	3.0x	5.0	Ctr	Ctr	0.46
G	MT20	2.0x	4.0	Ctr	Ctr	0.12

NOTES:

Trusses Manufactured by:
 RIDGWAY ROOF TRUSS
 Analysis Conforms To:
 FBC2007
 Design checked for 10 psf non-
 concurrent LL on BC.
 No wind exposure on left end.
 No wind exposure on right end.
 Wind Loads - ANSI / ASCE 7-05
 Truss is designed as
 Components and Claddings*
 for Exterior zone location.
 Wind Speed: 110 mph
 Mean Roof Height: 16-0
 Exposure Category: B
 Occupancy Factor : 1.00
 Building Type: Enclosed
 TC Dead Load: 4.0 psf
 BC Dead Load: 6.0 psf
 Unbalanced Loads Checked
 Load Factors = 1.00 and 0.00
 Max comp. force 1635 Lbs
 Max tens. force 1583 Lbs
 Quality Control Factor 1.25
 FABRICATOR NOTES:

1. Delegated Engineer (Truss
 Designer)
 Gary Dounson, PE 35054
 Gary Dounson & Associates,
 Inc.
 2830 NW 41st Street Suite D
 Gainesville, FL 32606
 (352)375-8593
 CA 5201



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 182.6 LBS

Online Plus -- Version 23.0.052
RUN DATE: 06-APR-09

CSI -Size- ----Lumber-----
TC 0.42 2x 4 SP-#2
BC 0.49 2x 8 SP-2400f-2.0E
WB 0.68 2x 4 SP-#3
ACT 0.15 2x 4 SP-#2
AWT 0.03 2x 4 SP-#3

Brace truss as follows:

O.C. From To
TC 48.0" 0- 0- 0 13- 3- 8
BC 120.0" 0- 0- 0 13- 3- 8

One Continuous Lateral Brace
A -E C -F
Attach CLB with (2)-10d nails
at each web.

psf-Ld Dead Live
TC 7.0 20.0
BC 10.0 0.0
TC+BC 17.0 20.0
Total 37.0 Spacing 31.8"
Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Total Load Reactions (Lbs)

Jt Down Uplift Horiz-
A 1334 16 U 77 R
C 1335 74 R

Jt Brg Size Required
A 3.5" 1.5"
C 3.5" 1.5"

LC# 1 Attic Loading

Dur Fctrs - Lbr 1.25 Plt 1.25
plf - Dead Live* From To
TC V 19 53 0.0' 13.3'
BC V 26 0 0.0' 13.3'
TC V 4 10 9.6' 13.3'
BC V 5 0 9.6' 13.3'
BC V 13 106 1.5' 11.3'
MA V 13 0 1.4' 11.4'
MA V 13 0 0.3' 8.1'
MA V 13 0 0.3' 7.9'

Plus 9 Wind Load Case(s)
Plus 2 Unbalanced Load Cases
Plus 1 UBC LL Load Case(s)
Plus 1 DL Load Case(s)
Plus 1 Attic 2 Load Case(s)

Membr CSI P Lbs Ax1-CSI-Bnd

-----Top Chords-----
E -M 0.38 352 C 0.00 0.38
M -B 0.42 477 C 0.04 0.38
B -J 0.40 477 C 0.01 0.39
J -F 0.35 366 C 0.00 0.35

-----Bottom Chords-----
A -L 0.49 76 T 0.00 0.49
L -I 0.41 340 T 0.01 0.40
I -C 0.32 73 T 0.00 0.32

-----Webs-----
A -E 0.61 2197 C 1 Br
E -L 0.68 2129 T
K -M 0.23 554 T 0.13 0.10
L -K 0.51 535 T 0.18 0.33
N -J 0.21 547 T 0.13 0.08
I -N 0.43 525 T 0.16 0.27
I -F 0.46 1451 T
C -F 0.41 1604 C 1 Br

-----Attic Chords (Top)-----
K -G 0.15 99 C 0.00 0.15
G -N 0.15 99 C 0.00 0.15
-----Attic Webs (Top)-----
G -B 0.03 93 T

TL Defl -0.25" in L -I L/621
LL Defl -0.18" in L -I L/825
Shear // Grain in A -L 0.60

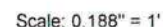
Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
E MT20 3.0x 6.0 Ctr Ctr 0.85
M MT20 3.0x 4.0 Ctr Ctr 0.16
B MT20 5.0x 5.0 Ctr Ctr 0.34
J MT20 3.0x 4.0 Ctr Ctr 0.16
F MT20 3.0x 4.0 Ctr Ctr 0.87
A MT20 2.0x 4.0 Ctr Ctr 0.45
L MT20 5.0x 8.0 Ctr-1.5 0.96
I MT20 4.0x 6.0 Ctr-0.6 0.83
C MT20 2.0x 4.0 Ctr Ctr 0.33

K MT20 3.0x 5.0 Ctr Ctr 0.46
G MT20 2.0x 4.0 Ctr Ctr 0.12
N MT20 3.0x 5.0 Ctr Ctr 0.46

NOTES:

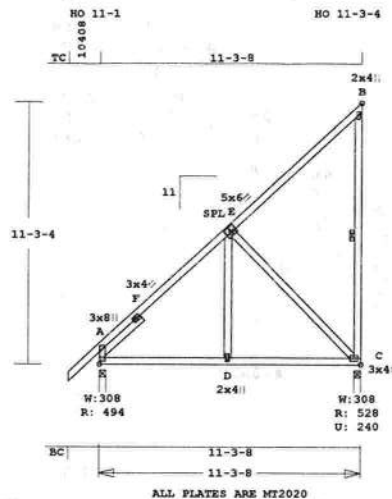
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2007
Design checked for 10 psf non-
concurrent LL on BC.
No wind exposure on left end.
No wind exposure on right end.
Wind Loads - ANSI / ASCE 7-05
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 16-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Unbalanced Loads Checked
Load Factors = 1.00 and 0.00
Max comp. force 2197 Lbs
Max tens. force 2129 Lbs
Quality Control Factor 1.25
FABRICATOR NOTES:

1. Delegated Engineer (Truss
Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates,
Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352) 375-8593
CA 5201



Job 090117	Mark R19	Quan 4	Type MONO	Span 110308	P1-H1 11	Left OH 1- 4- 8	Right OH 0	Engineering
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Peden Residence



Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 103.6 LBS

Online Plus -- Version 23.0.052
RUN DATE: 13-APR-09

CSI -Size- ----Lumber----

TC	0.41	2x 4	SP-#2
BC	0.24	2x 4	SP-#2
WB	0.51	2x 4	SP-#3
SL	0.05	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	11- 3- 8
BC Cont.	0- 0- 0	11- 3- 8

One Continuous Lateral Brace
C -B
Attach CLB with (2)-10d nails
at each web.

psf-Ld	Dead	Live
TC	7.0	20.0
BC	10.0	0.0
TC+BC	17.0	20.0
Total	37.0	Spacing 24.0"
Lumber Duration Factor	1.25	
Plate Duration Factor	1.25	
TC Fb=1.15	Fc=1.10	Ft=1.10
BC Fb=1.10	Fc=1.10	Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	495		479 R
C	529	241 U	220 R

Jt	Brg Size	Required
A	3.5"	1.5"
C	3.5"	1.5"

Plus 8 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)
Plus 1 DL Load Case(s)

Membr CSI P Lbs Ax1-CSI-Bnd
-----Top Chords-----

A -F	0.17	177 C	0.00	0.17
F -E	0.41	300 C	0.00	0.41
E -B	0.41	136 C	0.00	0.41

-----Bottom Chords-----

A -D	0.24	328 T	0.00	0.24
D -C	0.22	328 T	0.02	0.20

-----Webs-----

D -E	0.06	254 T	
E -C	0.51	462 C	
C -B	0.09	179 C	1 Br

-----Sliders-----

A -F	0.05	266 C
------	------	-------

TL Defl -0.06" in D -C L/999
LL Defl -0.03" in D -C L/999
Shear // Grain in E -B 0.20

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A MT20 3.0x 8.0 1.5 0.7 0.61
F MT20 3.0x 4.0 Ctr Ctr 0.11
E MT20 5.0x 6.0-0.4 0.5 0.52
B MT20 2.0x 4.0 Ctr Ctr 0.13
D MT20 2.0x 4.0 Ctr Ctr 0.17
C MT20 3.0x 4.0 Ctr Ctr 0.21

NOTES:

Trusses Manufactured by:

RIDGWAY ROOF TRUSS

Analysis Conforms To:

FBC2007

OH Loading

Soffit psf 2.0

Design checked for 10 psf non-concurrent LL on BC.

No wind exposure on right end.

Wind Loads - ANSI / ASCE 7-05

Truss is designed as

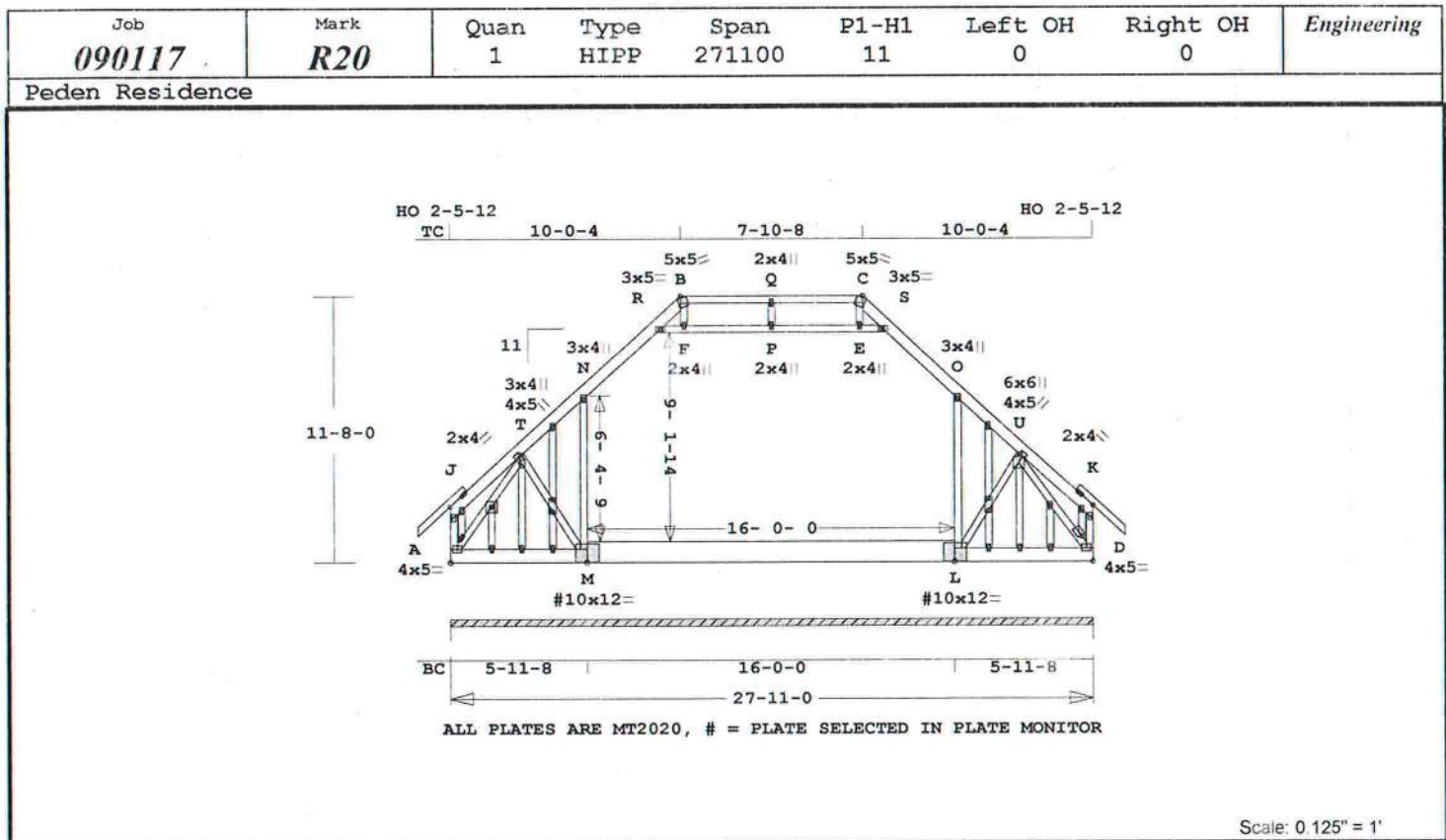
Components and Claddings*

for Exterior zone location.

Wind Speed: 110 mph
Mean Roof Height: 16-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 462 Lbs
Max tens. force 362 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201



Online Plus -- Version 23.0.052
RUN DATE: 06-APR-09

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 381.2 LBS

CSI -Size- ---Lumber---
TC 0.16 2x 4 SP-#2
-- 0.04 2x 6 SP-2400f-2.0E
V -B C -LL
BC 0.15 2x 8 SP-2400f-2.0E
-- 0.15 2x12 SP-#2
M -L
WB 0.40 2x 4 SP-#3
GW 0.13 2x 4 SP-#3
ACT 0.30 2x 4 SP-#2
AWT 0.04 2x 4 SP-#3

Brace truss as follows:
O.C. From To
TC Cont. 0- 0- 0 27-11- 0
BC Cont. 0- 0- 0 27-11- 0

psf-Ld Dead Live
TC 7.0 20.0
BC 10.0 0.0
TC+BC 17.0 20.0
Total 37.0 Spacing 24.0"
Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)
Jt Down Uplift Horiz
A 2066 337 U 287 R

Jt Brg Size Required
A 335.0" 0"-to- 335"

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl	CSI-Bnd
-----Top Chords-----				
J -V	0.00	2 T		
V -V	0.01	73 T	0.00	0.01
V -T	0.02	121 T	0.00	0.02
T -AA	0.02	431 C	0.00	0.02
AA-N	0.04	450 C	0.00	0.04
N -R	0.04	496 C	0.00	0.04
R -B	0.02	548 C	0.00	0.02
B -Q	0.16	438 C	0.00	0.16
Q -C	0.16	438 C	0.00	0.16
C -S	0.02	548 C	0.00	0.02
S -O	0.04	496 C	0.00	0.04
O -EE	0.04	441 C	0.00	0.04
EE-U	0.01	457 C	0.00	0.01
U -LL	0.01	131 T	0.00	0.01
LL-LL	0.00	79 T		
LL-K	0.00	2 T		
-----Bottom Chords-----				
A -Y	0.00	0 T		
Y -Z	0.00	0 T		
Z -DD	0.02	0 T	0.00	0.02
DD-M	0.15	0 T	0.00	0.15

M	-L	0.15	0 T	0.00	0.15
L	-HH	0.15	0 T	0.00	0.15
HH	-II	0.02	0 T	0.00	0.02
II	-KK	0.00	0 T		
KK	-D	0.00	0 T		

A	-V	0.03	75 T	WindLd
A	-W	0.06	434 C	0.05 0.00
W	-X	0.03	271 C	0.01 0.02
X	-T	0.26	363 C	0.00 0.26
T	-T	0.40	647 C	0.01 0.39
T	-CC	0.13	351 T	0.11 0.02
CC	-M	0.11	329 T	0.10 0.01
M	-N	0.17	200 C	
L	-O	0.18	209 C	

L	-GG	0.05	158 T	0.05 0.00
GG	-U	0.11	172 T	0.05 0.06
U	-U	0.11	153 C	0.01 0.10
U	-JJ	0.07	499 C	
JJ	-MM	0.04	419 C	0.02 0.02
MM	-D	0.07	507 C	0.07 0.00
D	-LL	0.02	83 T	WindLd

W	-V	0.03	190 C
Y	-X	0.05	109 C
Z	-T	0.13	343 C
CC	-AA	0.02	98 C
DD	-CC	0.01	77 C
GG	-EE	0.02	69 C
HH	-GG	0.01	78 C
II	-U	0.03	106 C
KK	-JJ	0.01	94 C
MM	-LL	0.10	94 C

R	-F	0.22	137 C	0.01 0.21
F	-P	0.30	137 C	0.01 0.29
P	-E	0.30	137 C	0.01 0.29
E	-S	0.22	137 C	0.01 0.21

F	-B	0.04	155 T
P	-Q	0.02	142 C
E	-C	0.04	155 T

TL Defl -0.09" in M -L L/999

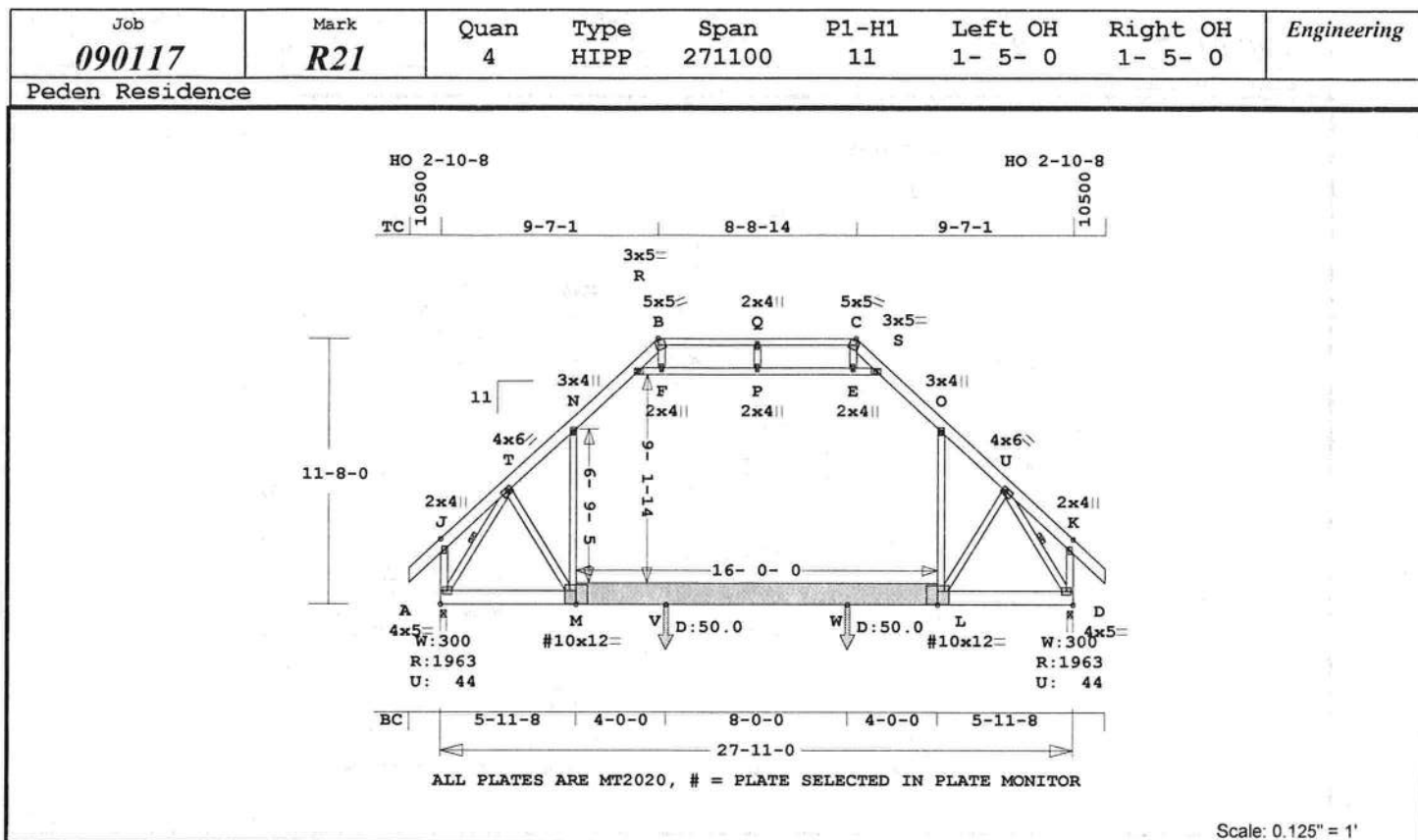
LL Defl -0.04" in M -L L/999

Shear // Grain in DD-M 0.22

Plates for each ply each face.
Plate - MT20 20 Ga. Gross Area
Plate - MT2H 20 Ga. Gross Area
Jt Type Plt Size X Y JSI
V MT20 2.0x 4.0 Ctr Ctr 0.22
V MT20 2.0x 4.0 Ctr Ctr 0.22
T MT20 4.0x 5.0-0.9-1.2 0.35
AA MT20 2.0x 4.0 Ctr Ctr 0.22
N MT20 3.0x 4.0 Ctr 0.4 0.16
R MT20 3.0x 5.0 Ctr Ctr 0.11
B MT20 5.0x 5.0 1.3-3.2 0.38
Q MT20 2.0x 4.0 Ctr Ctr 0.29
C MT20 5.0x 5.0-1.3-3.2 0.38
S MT20 3.0x 5.0 Ctr Ctr 0.11
O MT20 3.0x 4.0 Ctr 0.4 0.16
EE MT20 2.0x 4.0 Ctr Ctr 0.22
U MT20 4.0x 5.0 0.9-1.2 0.26

LL	MT20	2.0x 4.0	Ctr	Ctr	0.22
LL	MT20	2.0x 4.0	Ctr	Ctr	0.22
A	MT20	4.0x 5.0	Ctr	Ctr	0.18
Y	MT20	2.0x 4.0	Ctr	Ctr	0.29
Z	MT20	2.0x 4.0	Ctr	Ctr	0.29
DD	MT20	2.0x 4.0	Ctr	Ctr	0.29
M	MT20	10.0x12.0	Ctr	1.6	0.37
L	MT20	10.0x12.0	Ctr	1.6	0.37
HH	MT20	2.0x 4.0	Ctr	Ctr	0.29
II	MT20	2.0x 4.0	Ctr	Ctr	0.29
KK	MT20	2.0x 4.0	Ctr	Ctr	0.29
D	MT20	4.0x 5.0	Ctr	Ctr	0.20
F	MT20	2.0x 4.0	Ctr	Ctr	0.29
P	MT20	2.0x 4.0	Ctr	Ctr	0.29
E	MT20	2.0x 4.0	Ctr	Ctr	0.29
W	MT20	2.0x 4.0	Ctr	Ctr	0.16
X	MT20	6.0x 6.0	Ctr	Ctr	0.33
T	MT20	3.0x 4.0-0.1	Ctr	Ctr	0.23
CC	MT20	4.0x 8.0	Ctr	Ctr	0.20
GG	MT20	4.0x 8.0	Ctr	Ctr	0.20
U	MT20	6.0x 6.0	Ctr	Ctr	0.35
JJ	MT20	2.0x 4.0	Ctr	Ctr	0.19
MM	MT20	4.0x 5.0	Ctr	Ctr	0.09

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2007
Design checked for 10 psf non-concurrent LL on BC.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-05
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 16'-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 647 Lbs
Max tens. force 367 Lbs
Quality Control Factor 1.25
FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201



Online Plus -- Version 23.0.052
 RUN DATE: 13-APR-09

CSI	Size	Lumber
TC	0.64 2x 6	SP-2400f-2.0E
--	0.30 2x 4	SP-#2
B-C	0.98 2x 8	SP-#2
--	0.64 2x12	SP-#2
M-L	0.46 2x 4	SP-#3
WB	0.64 2x 4	SP-#2
ACT	0.12 2x 4	SP-#3
AWT	0.12 2x 4	SP-#3
SCAB (1)	2x12	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	27-11- 0
BC Cont.	0- 0- 0	27-11- 0

One Continuous Lateral Brace

A -T U -D

Attach CLB with (2)-10d nails at each web.

psf-Ld	Dead	Live
TC	7.0	20.0
BC	10.0	0.0
TC+BC	17.0	20.0
Total	37.0	24.0

Lumber Duration Factor 1.25

Plate Duration Factor 1.25

TC Fb=1.00 Fc=1.00 Ft=1.00

BC Fb=1.00 Fc=1.00 Ft=1.00

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz
A	1963	45 U	290 R
D	1963	45 U	290 R

Jt	Brg Size	Required
A	3.0"	2.3"
D	3.0"	2.3"

LC# 1 Attic Loading

plf - Dead	Live*	From	To
TC V	14	40	0.0' 27.9'
BC V	20	0	0.0' 27.9'
TC V	10	0	6.0' 8.6'
TC V	10	0	19.4' 22.0'
BC V	10	80	6.0' 22.0'
MA V	10	0	8.7' 19.2'
MA V	10	0	0.4' 7.6'
MA V	10	0	0.4' 7.6'
BC V	50	0	10.0' CL-LB
BC V	50	0	18.0' CL-LB

Plus 9 Wind Load Case(s)

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 428.9 LBS

Plus 2 Unbalanced Load Cases

Plus 1 UBC LL Load Case(s)

Plus 1 DL Load Case(s)

Plus 1 Attic 2 Load Case(s)

Membr	CSI	P Lbs	Axl	CSI-Bnd
J -T	0.06	119 T	0.00	0.06
T -N	0.64	1975 C	0.00	0.64
N -R	0.64	1222 C	0.00	0.64
R -B	0.53	303 C	0.01	0.52
B -Q	0.30	564 T	0.01	0.29
Q -C	0.30	564 T	0.01	0.29
C -S	0.53	303 C	0.01	0.52
S -O	0.64	1222 C	0.00	0.64
O -U	0.64	1975 C	0.00	0.64
U -K	0.06	119 T	0.00	0.06

Bottom Chords

A	M	L	D
0.98	1125 T	0.11	0.87
0.64	1258 T	0.10	0.54
0.98	1125 T	0.11	0.87

Webbs

A	J	T	M	N	O	L	U	D
0.08	201 T	WindLd						
0.46	2275 C							
0.10	313 T							
0.41	1258 T							
0.41	1258 T							
0.10	313 T							
0.46	2275 C							
0.05	201 T	WindLd						

Attic Chords (Top)

R	F	P	E
0.64	1663 C	0.01	0.63
0.64	1663 C	0.01	0.63
0.64	1663 C	0.01	0.63
0.64	1663 C	0.01	0.63

Attic Webs (Top)

F	B	P	E
0.12	380 T		
0.03	190 C		
0.12	380 T		

TL Defl -0.44" in M -L L/755

LL Defl -0.32" in M -L L/999

Shear // Grain in N -R 0.51

Plates for each ply each face.

Plate - MT20 20 Ga, Gross Area

Plate - MT2H 20 Ga, Gross Area

Jt Type Plt Size X Y JSI

J MT20 2.0x 4.0 Ctr Ctr 0.22

T MT20 4.0x 6.0-0.6-0.6 0.65

N MT20 3.0x 4.0 Ctr Ctr 0.48

R MT20 3.0x 5.0 Ctr Ctr 0.37

B MT20 5.0x 5.0 1.3-3.2 0.38

Q MT20 2.0x 4.0 Ctr Ctr 0.29

C MT20 5.0x 5.0-1.3-3.2 0.38

S MT20 3.0x 5.0 Ctr Ctr 0.37

O MT20 3.0x 4.0 Ctr Ctr 0.48

U MT20 4.0x 6.0 0.6-0.6 0.65

K	MT20	2.0x 4.0	Ctr	Ctr	0.22
A	MT20	4.0x 5.0	Ctr	Ctr	0.61
M	MT20	10.0x12.0	Ctr	Ctr	0.41
L	MT20	10.0x12.0	Ctr	Ctr	0.41
D	MT20	4.0x 5.0	Ctr	Ctr	0.61
F	MT20	2.0x 4.0	Ctr	Ctr	0.29
P	MT20	2.0x 4.0	Ctr	Ctr	0.29
E	MT20	2.0x 4.0	Ctr	Ctr	0.29

NOTES:

Trusses Manufactured by:

RIDGWAY ROOF TRUSS

Analysis Conforms To:

FBC2007

Fasten each scab (shaded) with 2 rows of 10d nails at 6 in o.c. each row, staggered along entire length.

OH Loading

Soffit psf 2.0

Design checked for 10 psf non-concurrent LL on BC.

NOTE: USER MODIFIED PLATES

This design may have plates selected through a plate monitor.

Wind Loads - ANSI / ASCE 7-05

Truss is designed as

Components and Claddings* for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 16-0

Exposure Category: B

Occupancy Factor: 1.00

Building Type: Enclosed

TC Dead Load: 4.0 psf

BC Dead Load: 6.0 psf

Unbalanced Loads Checked

Load Factors = 1.00 and 0.00

Max comp. force 2275 Lbs

Max tens. force 1258 Lbs

Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss Designer)

Gary Dounson, PE 35054

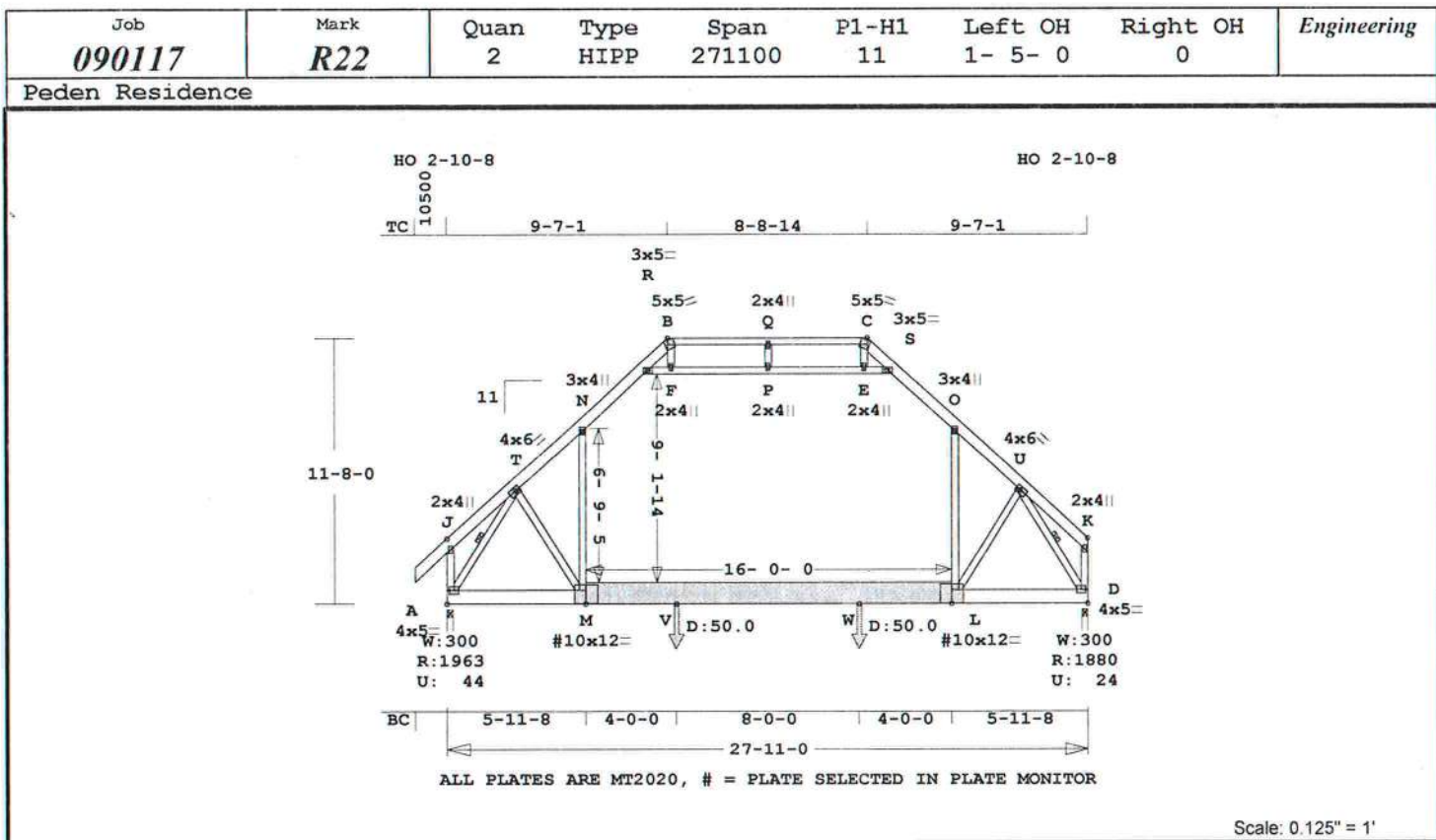
Gary Dounson & Associates, Inc.

2830 NW 41st Street Suite D

Gainesville, FL 32606

(352) 375-8593

CA 5201



Online Plus -- Version 23.0.052
 RUN DATE: 13-APR-09

CSI	-Size-	---Lumber---
TC	0.64	2x 6 SP-2400f-2.0E
--	0.30	2x 4 SP-#2
B -C		
BC	0.98	2x 8 SP-#2
--	0.64	2x12 SP-#2
M -L		
WB	0.46	2x 4 SP-#3
ACT	0.64	2x 4 SP-#2
AWT	0.12	2x 4 SP-#3
SCAB (1)	2x12	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	27-11- 0
BC Cont.	0- 0- 0	27-11- 0

One Continuous Lateral Brace
 A -T U -D
 Attach CLB with (2)-10d nails
 at each web.

psf-Ld	Dead	Live
TC	7.0	20.0
BC	10.0	0.0
TC+BC	17.0	20.0
Total	37.0	Spacing 24.0"
Lumber Duration Factor	1.25	
Plate Duration Factor	1.25	
TC Fb=1.00	Fc=1.00	Ft=1.00
BC Fb=1.00	Fc=1.00	Ft=1.00

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	1964	45 U	290 R
D	1881	24 U	290 R

Jt	Brq	Size	Required
A	3.0"	2.3"	
D	3.0"	2.2"	

LC# 1 Attic Loading

Dur	Fctrs	- Lbr	1.25	Plt	1.25
plf	- Dead	Live*	From	To	
TC V	14	40	0.0'	27.9'	
BC V	20	0	0.0'	27.9'	
TC V	10	0	6.0'	8.6'	
TC V	10	0	19.4'	22.0'	
BC V	10	80	6.0'	22.0'	
MA V	10	0	8.7'	19.2'	
MA V	10	0	0.4'	7.6'	
MA V	10	0	0.4'	7.6'	
BC V	50	0	10.0'	CL-LB	
BC V	50	0	18.0'	CL-LB	

Plus 9 Wind Load Case(s)

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 423.5 LBS

Plus 2 Unbalanced Load Cases
 Plus 1 UBC LL Load Case(s)
 Plus 1 DL Load Case(s)
 Plus 1 Attic 2 Load Case(s)

Membr	CSI	P Lbs	Ax1-CSI-Bnd
-----Top Chords-----			
J -T	0.06	119 T	0.00 0.06
T -N	0.64	1976 C	0.00 0.64
N -R	0.64	1223 C	0.00 0.64
R -B	0.53	303 C	0.01 0.52
B -Q	0.30	564 T	0.01 0.29
Q -C	0.30	564 T	0.01 0.29
C -S	0.53	302 C	0.01 0.52
S -O	0.64	1223 C	0.00 0.64
O -U	0.64	1976 C	0.00 0.64
U -K	0.06	116 T	0.00 0.06

Membr	CSI	P Lbs	Ax1-CSI-Bnd
-----Bottom Chords-----			
A -M	0.98	1125 T	0.11 0.87
M -L	0.64	1258 T	0.10 0.54
L -D	0.98	1127 T	0.11 0.87

Membr	CSI	P Lbs	Ax1-CSI-Bnd
-----Webs-----			
A -J	0.08	201 T	WindLd
A -T	0.46	2276 C	1 Br
T -M	0.10	314 T	
M -N	0.41	1258 T	
L -O	0.41	1259 T	
L -U	0.10	312 T	
U -D	0.46	2279 C	1 Br
D -K	0.03	120 T	WindLd

Membr	CSI	P Lbs	Ax1-CSI-Bnd
-----Attic Chords (Top)-----			
R -F	0.64	1663 C	0.01 0.63
F -P	0.64	1663 C	0.01 0.63
P -E	0.64	1663 C	0.01 0.63
E -S	0.64	1663 C	0.01 0.63

Membr	CSI	P Lbs	Ax1-CSI-Bnd
-----Attic Webs (Top)-----			
F -B	0.12	381 T	
P -Q	0.03	190 C	
E -C	0.12	380 T	

TL Defl	-0.44"	in M	-L	L/755
LL Defl	-0.32"	in M	-L	L/999
Shear //	Grain	in N	-R	0.51

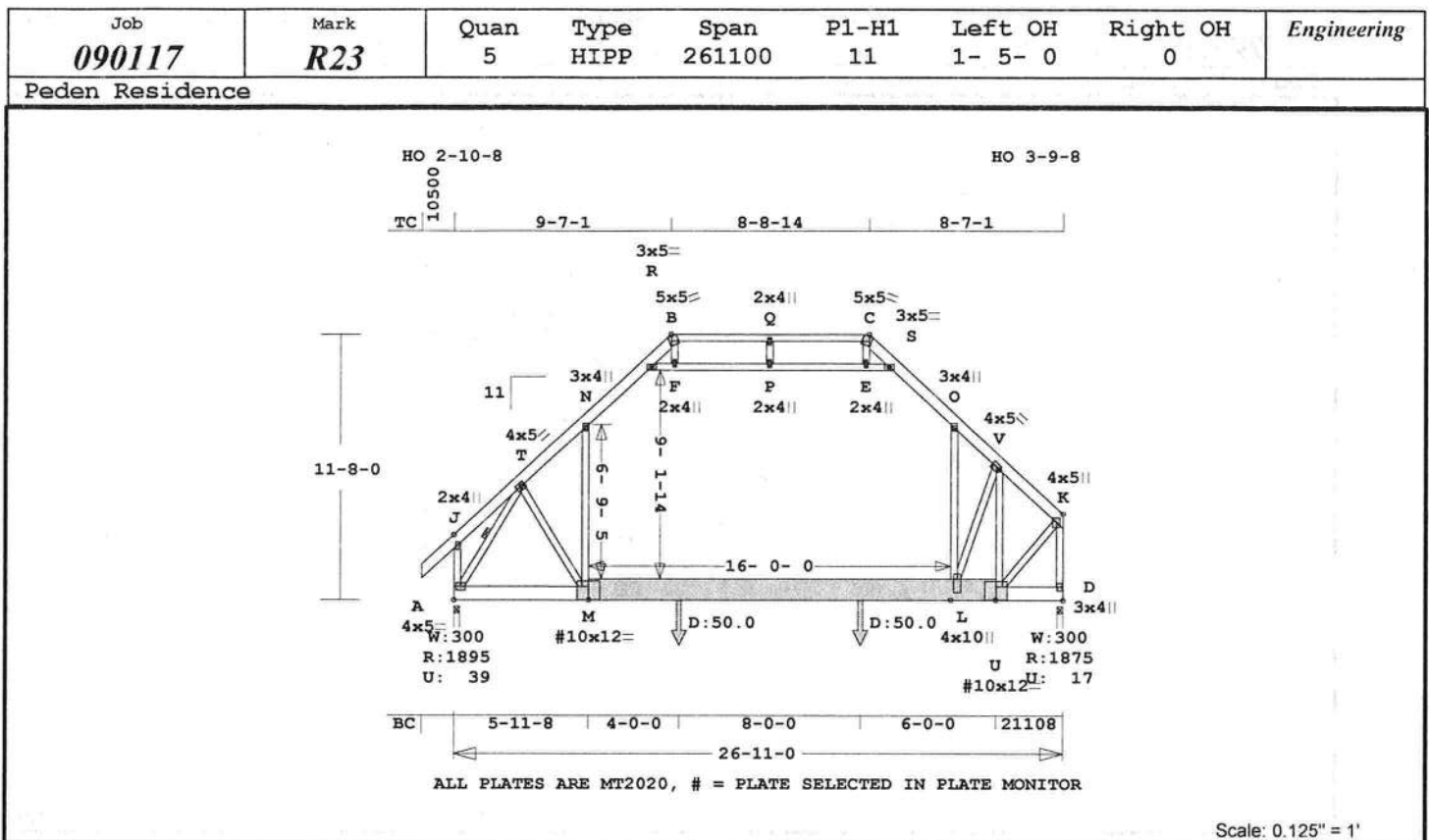
Plates for each ply each face.

Plate	- MT20	20 Ga,	Gross Area
Plate	- MT2H	20 Ga, <td>Gross Area</td>	Gross Area
Jt Type	Plt Size	X	Y JSI
J	MT20	2.0x 4.0	Ctr Ctr 0.22
T	MT20	4.0x 6.0	-0.6-0.6 0.65
N	MT20	3.0x 4.0	Ctr 0.4 0.48
R	MT20	3.0x 5.0	Ctr Ctr 0.37
B	MT20	5.0x 5.0	1.3-3.2 0.38
Q	MT20	2.0x 4.0	Ctr Ctr 0.29
C	MT20	5.0x 5.0	-1.3-3.2 0.38
S	MT20	3.0x 5.0	Ctr Ctr 0.37
O	MT20	3.0x 4.0	Ctr 0.4 0.48
U	MT20	4.0x 6.0	0.6-0.6 0.65

K	MT20	2.0x 4.0	Ctr Ctr	0.22
A	MT20	4.0x 5.0	Ctr Ctr <td>0.61</td>	0.61
M	MT20	10.0x12.0	Ctr 1.6	0.41
L	MT20	10.0x12.0	Ctr 1.5	0.41
D	MT20	4.0x 5.0	Ctr Ctr <td>0.61</td>	0.61
F	MT20	2.0x 4.0	Ctr Ctr <td>0.29</td>	0.29
P	MT20	2.0x 4.0	Ctr Ctr <td>0.29</td>	0.29
E	MT20	2.0x 4.0	Ctr Ctr <td>0.29</td>	0.29

NOTES:

Trusses Manufactured by:
 RIDGWAY ROOF TRUSS
 Analysis Conforms To:
 FBC2007
 Fasten each scab (shaded) with
 2 rows of 10d nails at 6 in
 o.c. each row, staggered
 along entire length.
 OH Loading
 Soffit psf 2.0
 Design checked for 10 psf non-
 concurrent LL on BC.
 NOTE: USER MODIFIED PLATES
 This design may have plates
 selected through a plate
 monitor.
 Wind Loads - ANSI / ASCE 7-05
 Truss is designed as
 Components and Claddings*
 for Exterior zone location.
 Wind Speed: 110 mph
 Mean Roof Height: 16-0
 Exposure Category: B
 Occupancy Factor : 1.00
 Building Type: Enclosed
 TC Dead Load: 4.0 psf
 BC Dead Load: 6.0 psf
 Unbalanced Loads Checked
 Load Factors = 1.00 and 0.00
 Max comp. force 2279 Lbs
 Max tens. force 1259 Lbs
 Quality Control Factor 1.25
 FABRICATOR NOTES:
 1. Delegated Engineer (Truss
 Designer)
 Gary Dounson, PE 35054
 Gary Dounson & Associates,
 Inc.
 2830 NW 41st Street Suite D
 Gainesville, FL 32606
 (352) 375-8593
 CA 5201



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CSI	Size	Lumber
TC	0.65	2x 6 SP-2400f-2.0E
--	0.29	2x 4 SP-#2
BC	0.87	2x 8 SP-#2
--	0.56	2x12 SP-#2
M-U		
WB	0.98	2x 4 SP-#3
ACT	0.62	2x 4 SP-#2
AWT	0.11	2x 4 SP-#3
SCAB (1)	2x12	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	26-11- 0
BC Cont.	0- 0- 0	26-11- 0

One Continuous Lateral Brace
 A - T
 Attach CLB with (2)-10d nails
 at each web.

psf-Ld	Dead	Live
TC	7.0	20.0
BC	10.0	0.0
TC+BC	17.0	20.0
Total	37.0	24.0"
Lumber Duration Factor	1.25	
Plate Duration Factor	1.25	
TC Fb=1.00	Fc=1.00	Ft=1.00
BC Fb=1.00	Fc=1.00	Ft=1.00

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz
A	1895	39 U	265 R
D	1876	18 U	221 R

Jt	Brg Size	Required
A	3.0"	2.2"
D	3.0"	2.2"

LC# 1 Attic Loading

Dur Fctrs	Lbr 1.25	Plt 1.25	
plf - Dead	Live*	From To	
TC V	14	40	0.0' 26.9'
BC V	20	0	0.0' 26.9'
TC V	10	0	6.0' 8.6'
TC V	10	0	19.4' 22.0'
BC V	10	80	6.0' 22.0'
MA V	10	0	8.7' 19.2'
MA V	10	0	0.4' 7.6'
MA V	10	0	0.5' 7.6'
BC V	50	0	10.0' CL-LB
BC V	50	0	18.0' CL-LB

Plus 9 Wind Load Case(s)

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 439.6 LBS

Plus 2 Unbalanced Load Cases
 Plus 1 UBC LL Load Case(s)
 Plus 1 DL Load Case(s)
 Plus 1 Attic 2 Load Case(s)

Membr	CSI	P Lbs	Ax1	CSI-Bnd
Top Chords				
J - T	0.06	119 T	0.00	0.06
T - N	0.60	1869 C	0.00	0.60
N - R	0.60	1177 C	0.00	0.60
R - B	0.50	354 C	0.00	0.50
B - Q	0.28	504 T	0.00	0.28
Q - C	0.29	504 T	0.00	0.29
C - S	0.47	322 C	0.00	0.47
S - O	0.65	1174 C	0.00	0.65
O - V	0.65	1969 C	0.00	0.65
V - K	0.03	1211 C	0.00	0.03
Bottom Chords				
A - M	0.87	1067 T	0.11	0.76
M - L	0.56	1194 T	0.09	0.47
L - U	0.43	939 T	0.07	0.36
U - D	0.26	220 T	0.00	0.26

Web	CSI	P Lbs	Ax1	CSI-Bnd
A - J	0.08	201 T	WindLd	
A - T	0.43	2159 C		1 Br
T - M	0.09	294 T		
M - N	0.38	1181 T		
L - O	0.45	1386 T		
L - V	0.30	952 T		
U - V	0.98	1758 C		
U - K	0.45	1422 T		
D - K	0.43	1980 C		
Attic Chords (Top)				
R - F	0.62	1556 C	0.01	0.61
F - P	0.62	1556 C	0.01	0.61
P - E	0.58	1556 C	0.01	0.57
E - S	0.58	1556 C	0.01	0.57
Attic Webs (Top)				
F - B	0.11	370 T		
P - Q	0.03	185 C		
E - C	0.11	353 T		

TL Defl -0.37" in M - L L/855
 LL Defl -0.28" in M - L L/999
 Shear // Grain in S - O 0.49

Plates for each ply each face.

Plate	MT20	20 Ga	Gross Area
Plate - MT2H	20 Ga <td></td> <td></td>		
Jt Type	Plt Size	X	Y JSI
J	MT20	2.0x 4.0	Ctr Ctr 0.22
T	MT20	4.0x 5.0	-0.2-0.2 0.94
N	MT20	3.0x 4.0	Ctr 0.4 0.45
R	MT20	3.0x 5.0	Ctr Ctr 0.35
B	MT20	5.0x 5.0	1.3-3.2 0.38
Q	MT20	2.0x 4.0	Ctr Ctr 0.29
C	MT20	5.0x 5.0	-1.3-3.2 0.38
S	MT20	3.0x 5.0	Ctr Ctr 0.35

	MT20	3.0x 4.0	Ctr	0.4	0.53
V	MT20	4.0x 5.0	Ctr Ctr	0.80	
K	MT20	4.0x 5.0	Ctr Ctr	0.79	
A	MT20	4.0x 5.0	Ctr Ctr	0.58	
M	MT20	10.0x12.0	Ctr	1.6	0.39
L	MT20	4.0x10.0	Ctr	-2.1	0.80
U	MT20	10.0x12.0	0.1-2.1	0.46	
D	MT20	3.0x 4.0	Ctr Ctr	0.27	
F	MT20	2.0x 4.0	Ctr Ctr	0.29	
P	MT20	2.0x 4.0	Ctr Ctr	0.29	
E	MT20	2.0x 4.0	Ctr Ctr	0.29	

NOTES:

Trusses Manufactured by:
 RIDGWAY ROOF TRUSS

Analysis Conforms To:
 FBC2007

Fasten each scab (shaded) with
 2 rows of 10d nails at 6 in
 o.c. each row, staggered
 along entire length.

OH Loading
 Soffit psf 2.0
 Design checked for 10 psf non-
 concurrent LL on BC.

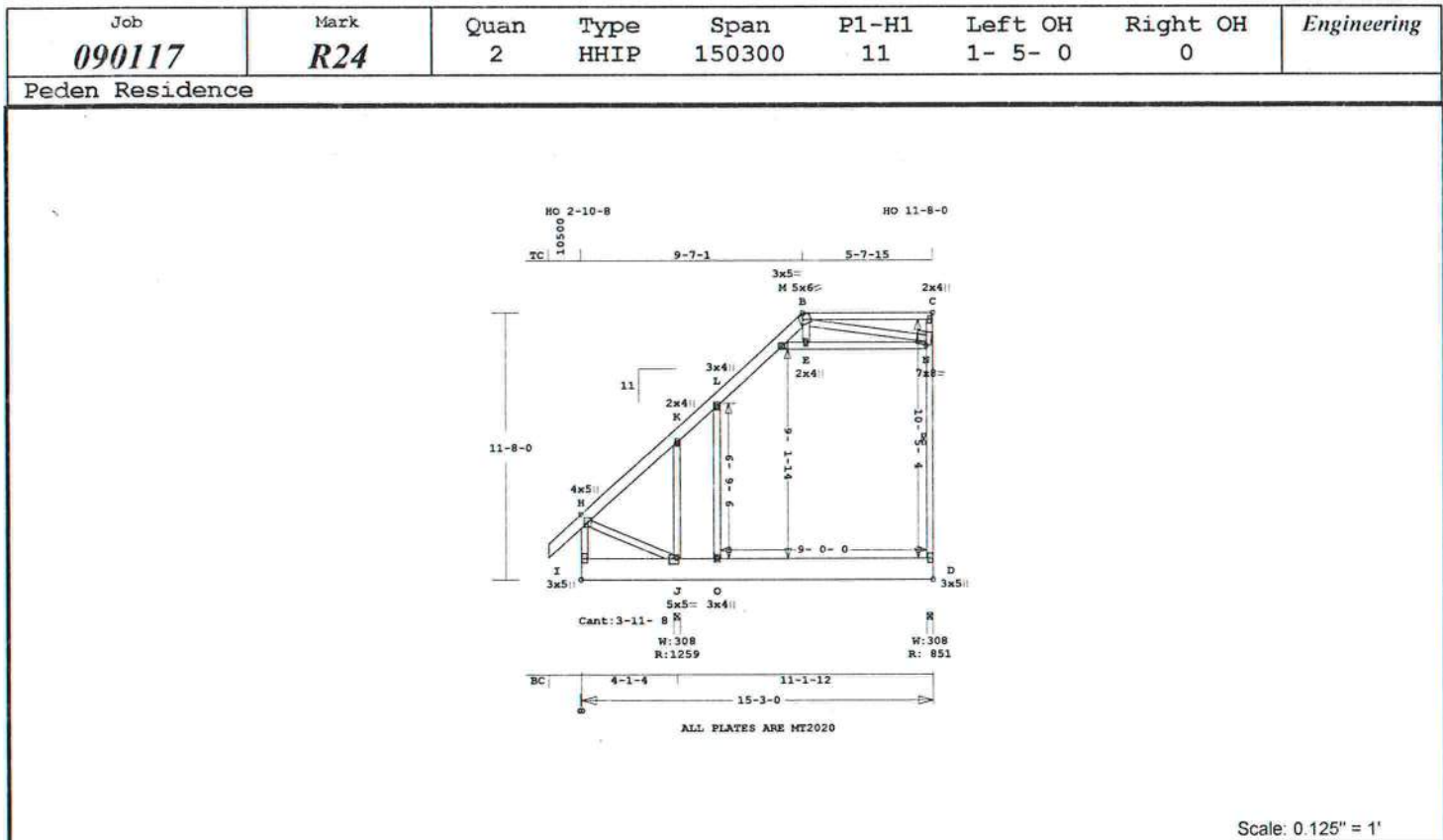
No wind exposure on right end.
 NOTE: USER MODIFIED PLATES
 This design may have plates
 selected through a plate
 monitor.

Wind Loads - ANSI / ASCE 7-05
 Truss is designed as
 Components and Claddings*
 for Exterior zone location.
 Wind Speed: 110 mph
 Mean Roof Height: 16-0
 Exposure Category: B
 Occupancy Factor : 1.00
 Building Type: Enclosed
 TC Dead Load: 4.0 psf
 BC Dead Load: 6.0 psf

Unbalanced Loads Checked
 Load Factors = 1.00 and 0.00
 Max comp. force 2159 Lbs
 Max tens. force 1422 Lbs
 Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss
 Designer)
 Gary Dounson, PE 35054
 Gary Dounson & Associates,
 Inc.
 2830 NW 41st Street Suite D
 Gainesville, FL 32606
 (352)375-8593
 CA 5201



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 217.2 LBS
 Online Plus -- Version 23.0.052
 RUN DATE: 13-APR-09

	CSI	-Size-	-----Lumber-----
TC	0.30	2x 4	SP-#2
--	0.21	2x 6	SP-2400f-2.0E
H -B			
BC	0.50	2x12	SP-#2
WB	0.38	2x 4	SP-#3
ACT	0.17	2x 4	SP-#2
AWT	0.38	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	15- 3- 0
BC	Cont.	0- 0- 0	15- 3- 0

One Continuous Lateral Brace
 D -C
 Attach CLB with (2)-10d nails
 at each web.

psf-Ld	Dead	Live
TC	7.0	20.0
BC	10.0	0.0
TC+BC	17.0	20.0
Total	37.0	Spacing 24.0"
Lumber Duration Factor 1.25		
Plate Duration Factor 1.25		
TC Fb=1.15	Fc=1.10	Ft=1.10
BC Fb=1.10	Fc=1.10	Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz
J	1260		394 R
D	851		223 R

Jt	Brg Size	Required
J	3.5"	1.5"
D	3.5"	1.5"

LC# 1 Attic Loading
 Dur Fctrs - Lbr 1.25 Plt 1.25
 plf - Dead Live* From To
 TC V 14 40 0.0' 15.2'
 BC V 20 0 0.0' 15.2'
 TC V 10 0 6.0' 8.6'
 BC V 10 80 6.0' 15.0'
 MA V 10 0 8.7' 15.1'
 MA V 10 0 0.5' 7.6'

MA V 10 0 0.0' 0.0'
 Plus 9 Wind Load Case(s)
 Plus 2 Unbalanced Load Cases
 Plus 1 UBC LL Load Case(s)
 Plus 1 DL Load Case(s)
 Plus 1 Attic 2 Load Case(s)

Membr	CSI	P Lbs	Axl	CSI-Bnd
-----Top Chords-----				
H -K	0.09	320 T	0.00	0.09
K -L	0.09	227 T	0.00	0.09
L -M	0.21	150 C	0.00	0.21
M -B	0.21	524 C	0.00	0.21
B -C	0.30	67 T	0.00	0.30

-----Bottom Chords-----				
I -J	0.26	51 T	0.00	0.26
J -O	0.38	221 T	0.00	0.38
O -D	0.50	221 T	0.00	0.50

-----Webs-----				
I -H	0.06	273 C	WindLd	
H -J	0.13	397 C		
J -K	0.14	270 C		
O -L	0.38	439 C		
N -C	0.20	142 C	0.00	0.20
D -N	0.24	298 C	0.01	0.23

-----Attic Chords (Top)-----				
M -E	0.11	647 T	0.10	0.01
E -N	0.17	648 T	0.10	0.07

-----Attic Webs (Top)-----				
E -B	0.01	73 T		
B -N	0.38	729 C		

TL Defl -0.12" in O -D L/999
 LL Defl -0.09" in O -D L/999
 LL Cant 0.09" in I -J L/500
 Shear // Grain in J -O 0.30

Plates for each ply each face.
 Plate - MT20 20 Ga, Gross Area
 Plate - MT2H 20 Ga, Gross Area
 Jt Type Plt Size X Y JSI
 H MT20 4.0x 5.0 Ctr Ctr 0.16
 K MT20 2.0x 4.0 Ctr Ctr 0.13
 L MT20 3.0x 4.0 Ctr 0.4 0.11
 M MT20 3.0x 5.0 Ctr Ctr 0.14
 B MT20 5.0x 6.0 1.3-3.4 0.66
 C MT20 2.0x 4.0 Ctr Ctr 0.12
 I MT20 3.0x 5.0 Ctr Ctr 0.07

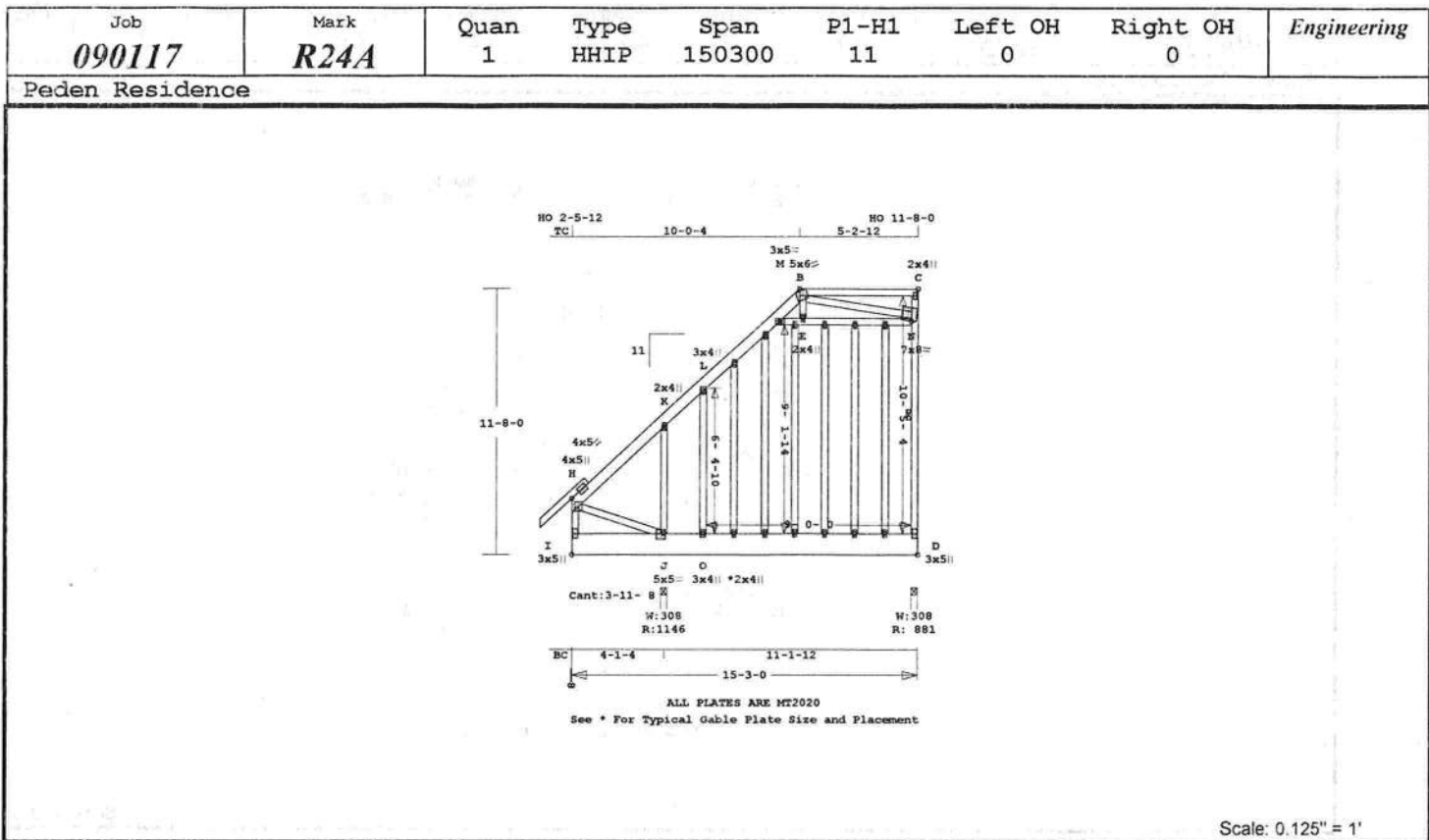
J	MT20	5.0x 5.0	Ctr-0.5	0.12	
O	MT20	3.0x 4.0	Ctr Ctr	0.08	
D	MT20	3.0x 5.0	Ctr Ctr	0.11	
E	MT20	2.0x 4.0	Ctr Ctr	0.18	
N	MT20	7.0x 8.0	2.7 0.4	0.76	

NOTES:

Trusses Manufactured by:
 RIDGWAY ROOF TRUSS
 Analysis Conforms To:
 FBC2007
 OH Loading
 Soffit psf 2.0
 Design checked for 10 psf non-
 concurrent LL on BC.
 No wind exposure on right end.
 Wind Loads - ANSI / ASCE 7-05
 Truss is designed as
 Components and Claddings*
 for Exterior zone location.
 Wind Speed: 110 mph
 Mean Roof Height: 16-0
 Exposure Category: B
 Occupancy Factor: 1.00
 Building Type: Enclosed
 TC Dead Load: 4.0 psf
 BC Dead Load: 6.0 psf
 Unbalanced Loads Checked
 Load Factors = 1.00 and 0.00
 Max comp. force 729 Lbs
 Max tens. force 648 Lbs
 Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss
 Designer)
 Gary Dounson, PE 35054
 Gary Dounson & Associates,
 Inc.
 2830 NW 41st Street Suite D
 Gainesville, FL 32606
 (352)375-8593
 CA 5201



Online Plus -- Version 23.0.052
 RUN DATE: 13-APR-09

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 314.2 LBS

CSI -Size- ----Lumber-----
 TC 0.25 2x 4 SP-#2
 -- 0.23 2x 6 SP-2400f-2.OE
 H -B
 BC 0.52 2x12 SP-#2
 WB 0.37 2x 4 SP-#3
 ACT 0.16 2x 4 SP-#2
 AWT 0.33 2x 4 SP-#3

Brace truss as follows:

O.C. From To
 TC Cont. 0- 0- 0 15- 3- 0
 BC Cont. 0- 0- 0 15- 3- 0
 One Continuous Lateral Brace
 D -C
 Attach CLB with (2)-10d nails
 at each web.

psf-Ld Dead Live
 TC 7.0 20.0
 BC 10.0 0.0
 TC+BC 17.0 20.0
 Total 37.0 Spacing 24.0"
 Lumber Duration Factor 1.25
 Plate Duration Factor 1.25
 TC Fb=1.15 Fc=1.10 Ft=1.10
 BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)

Jt Down Uplift Horiz-
 J 1147 394 R
 D 882 224 R

Jt Brg Size Required
 J 3.5" 1.5"
 D 3.5" 1.5"

LC# 1 Attic Loading
 Dur Fctrs - Lbr 1.25 Plt 1.25
 plf - Dead Live* From To
 TC V 14 40 0.0' 15.2'
 BC V 20 0 0.0' 15.2'
 TC V 10 0 6.0' 9.0'
 BC V 10 80 6.0' 15.0'
 MA V 10 0 9.1' 15.1'
 MA V 10 0 0.5' 7.2'

MA V 10 0 0.0' 0.0'
 Plus 9 Wind Load Case(s)
 Plus 2 Unbalanced Load Cases
 Plus 1 UBC LL Load Case(s)
 Plus 1 DL Load Case(s)
 Plus 1 Attic 2 Load Case(s)

Membr CSI P Lbs Axl-CSI-Bnd

-----Top Chords-----
 H -K 0.07 337 T 0.00 0.07
 K -L 0.07 264 T 0.00 0.07
 L -M 0.23 142 C 0.00 0.23
 M -B 0.23 499 C 0.00 0.23
 B -C 0.25 80 T 0.00 0.25

-----Bottom Chords-----
 I -J 0.30 42 T 0.00 0.30
 J -O 0.43 221 T 0.00 0.43
 O -D 0.52 221 T 0.00 0.52

-----Webs-----
 I -H 0.07 289 T WindLd
 H -J 0.12 387 C
 J -K 0.11 242 T
 O -L 0.37 473 C
 N -C 0.24 132 C 0.00 0.24
 D -N 0.28 299 C 0.01 0.27

-----Attic Chords (Top)-----
 M -E 0.11 639 T 0.10 0.01
 E -N 0.16 641 T 0.10 0.06

-----Attic Webs (Top)-----
 E -B 0.01 66 T
 B -N 0.33 739 C

TL Defl -0.13" in O -D L/999
 LL Defl -0.09" in O -D L/999
 LL Cant 0.09" in I -J L/503
 Shear // Grain in J -O 0.30

Plates for each ply each face.
 Plate - MT20 20 Ga, Gross Area
 Plate - MT2H 20 Ga, Gross Area
 Jt Type Plt Size X Y JSI
 H MT20 4.0x 5.0 Ctr Ctr 0.16
 K MT20 2.0x 4.0 Ctr Ctr 0.13
 L MT20 3.0x 4.0 Ctr 0.4 0.11
 M MT20 3.0x 5.0 Ctr Ctr 0.14
 B MT20 5.0x 6.0 1.3-3.4 0.67
 C MT20 2.0x 4.0 Ctr Ctr 0.12
 I MT20 3.0x 5.0 Ctr Ctr 0.09

J MT20 5.0x 5.0 Ctr-0.5 0.13
 O MT20 3.0x 4.0 Ctr Ctr 0.08
 D MT20 3.0x 5.0 Ctr Ctr 0.11
 E MT20 2.0x 4.0 Ctr Ctr 0.18
 N MT20 7.0x 8.0-2.7 0.4 0.77

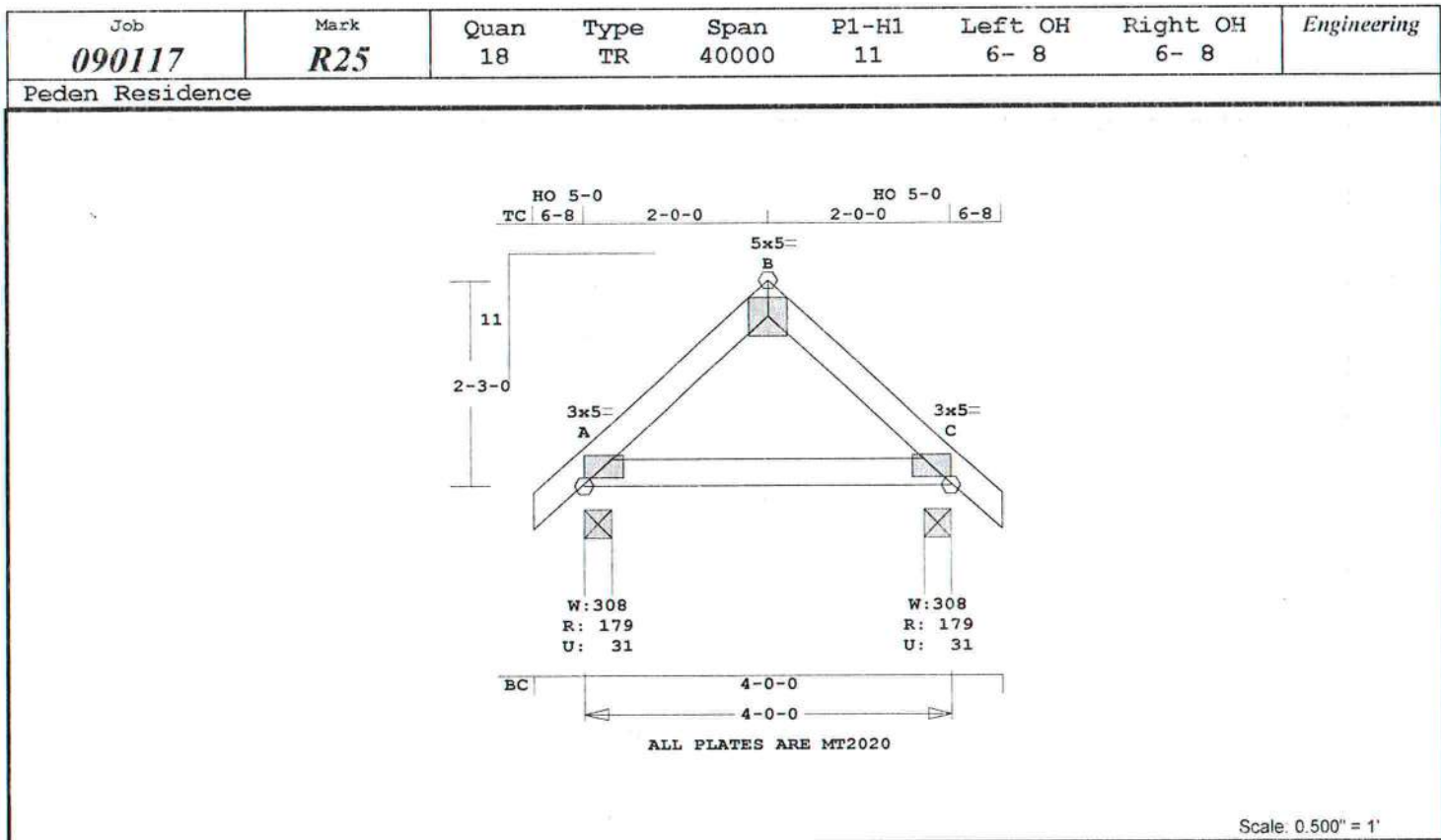
6 Gable studs to be attached
 with 2.0x4.0 plates each end.

NOTES:

Trusses Manufactured by:
 RIDGWAY ROOF TRUSS
 Analysis Conforms To:
 FBC2007
 Design checked for 10 psf non-
 concurrent LL on BC.
 No wind exposure on right end.
 Refer to Gen Det 3 series for
 web bracing and plating.
 Wind Loads - ANSI / ASCE 7-05
 Truss is designed as
 Components and Claddings*
 for Exterior zone location.
 Wind Speed: 110 mph
 Mean Roof Height: 16-0
 Exposure Category: B
 Occupancy Factor : 1.00
 Building Type: Enclosed
 TC Dead Load: 4.0 psf
 BC Dead Load: 6.0 psf
 Unbalanced Loads Checked
 Load Factors = 1.00 and 0.00
 Max comp. force 739 Lbs
 Max tens. force 641 Lbs
 Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss
 Designer)
 Gary Dounson, PE 35054
 Gary Dounson & Associates,
 Inc.
 2830 NW 41st Street Suite D
 Gainesville, FL 32606
 (352)375-8593
 CA 5201



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 21.6 LBS
 Online Plus -- Version 23.0.052
 RUN DATE: 06-APR-09

LL Defl 0.00" in A -C L/999 (352)375-8593
 Shear // Grain in A -C 0.07 CA 5201

CSI -Size- ----Lumber----
 TC 0.03 2x 4 SP-#2
 BC 0.05 2x 4 SP-#2

Brace truss as follows:

	O.C.	From	To
TC Cont.	0- 0- 0	4- 0- 0	
BC Cont.	0- 0- 0	4- 0- 0	

psf-Ld Dead Live
 TC 7.0 20.0
 BC 10.0 0.0
 TC+BC 17.0 20.0
 Total 37.0 Spacing 24.0"
 Lumber Duration Factor 1.25
 Plate Duration Factor 1.25
 TC Fb=1.15 Fc=1.10 Ft=1.10
 BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	179	32 U	39 R
C	179	32 U	39 R

Jt	Brg Size	Required
A	3.5"	1.5"
C	3.5"	1.5"

Plus 9 Wind Load Case(s)
 Plus 1 UBC LL Load Case(s)
 Plus 1 DL Load Case(s)

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Top Chords-----					
A -B	0.03	80 C	0.00	0.03	
B -C	0.03	80 C	0.00	0.03	
-----Bottom Chords-----					
A -C	0.05	61 T	0.00	0.05	

TL Defl -0.01" in A -C L/999

Plates for each ply each face.
 Plate - MT20 20 Ga, Gross Area
 Plate - MT2H 20 Ga, Gross Area
 Jt Type Plt Size X Y JSI
 A MT20 3.0x 5.0 0.7 0.7 0.47
 B MT20 5.0x 5.0 Ctr-2.4 0.33
 C MT20 3.0x 5.0-0.7 0.7 0.47

NOTES:

Trusses Manufactured by:

RIDGWAY ROOF TRUSS

Analysis Conforms To:

FBC2007

OH Loading

Soffit psf 2.0

Design checked for 10 psf non-concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-05

Truss is designed as

Components and Claddings*

for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 16-0

Exposure Category: B

Occupancy Factor : 1.00

Building Type: Enclosed

TC Dead Load: 4.0 psf

BC Dead Load: 6.0 psf

Max comp. force 80 Lbs

Max tens. force 61 Lbs

Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss

Designer)

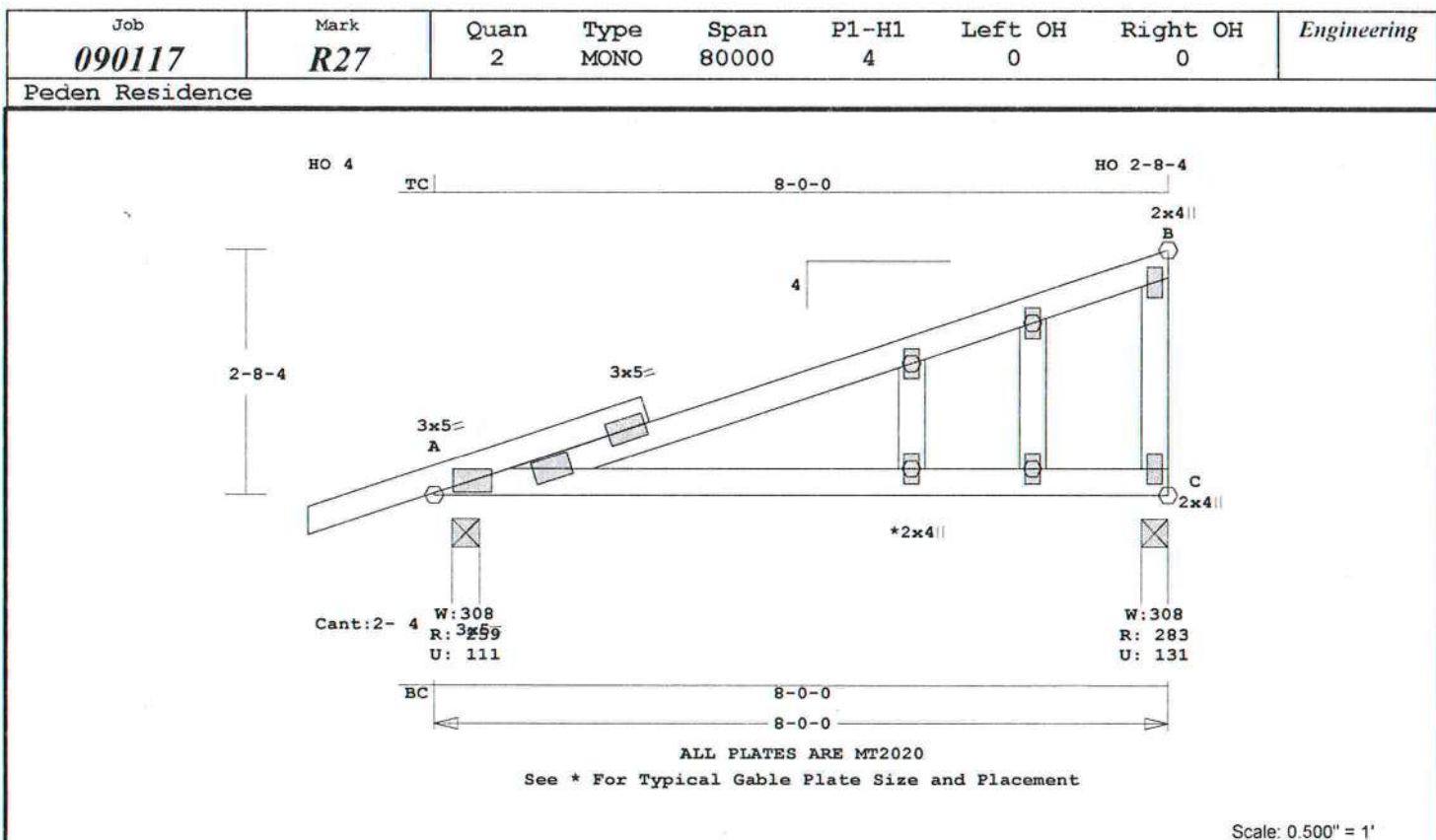
Gary Dounson, PE 35054

Gary Dounson & Associates,

Inc.

2830 NW 41st Street Suite D

Gainesville, FL 32606



Online Plus -- Version 23.0.052
 RUN DATE: 13-APR-09

CSI	Size	Lumber
TC	0.64	2x 4 SP-#2
BC	0.63	2x 4 SP-#2
WB	0.07	2x 4 SP-#3

Brace truss as follows:

	O.C.	From	To
TC Cont.	0- 0- 0	8- 0- 0	
BC Cont.	0- 0- 0	8- 0- 0	

psf-Ld	Dead	Live
TC	7.0	20.0
BC	10.0	0.0
TC+BC	17.0	20.0
Total	37.0	Spacing 24.0"
Lumber Duration Factor	1.25	
Plate Duration Factor	1.25	
TC Fb=1.15	Fc=1.10	Ft=1.10
BC Fb=1.10	Fc=1.10	Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz
A	260	111 U	69 R
C	283	131 U	84 R

Jt	Brg Size	Required
A	3.5"	1.5"
C	3.5"	1.5"

Plus 8 Wind Load Case(s)
 Plus 1 UBC LL Load Case(s)
 Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Ax1	CSI-Bnd
-----Top Chords-----				
A -B	0.64	53 C	0.00	0.64
-----Bottom Chords-----				
A -A	0.19	69 C	0.00	0.19
A -C	0.63	66 T	0.00	0.63
-----Webs-----				

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 46.1 LBS

C -B 0.07 306 T WindLd

TL Defl -0.26" in A -C L/328
 LL Defl -0.12" in A -C L/700
 Shear // Grain in A -A 0.38

Plates for each ply each face.
 Plate - MT20 20 Ga, Gross Area
 Plate - MT2H 20 Ga, Gross Area
 Jt Type Plt Size X Y JSI
 A MT20 3.0x 5.0 Ctr Ctr 0.52
 B MT20 2.0x 4.0 Ctr Ctr 0.14
 C MT20 2.0x 4.0 Ctr Ctr 0.15

2 Gable studs to be attached
 with 2.0x4.0 plates each end.

NOTES:

Trusses Manufactured by:

RIDGWAY ROOF TRUSS

Analysis Conforms To:

FBC2007

Design checked for 10 psf non-
 concurrent LL on BC.

Refer to Gen Det 3 series for
 web bracing and plating.

Wind Loads - ANSI / ASCE 7-05

Truss is designed as

Components and Claddings*

for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 16-0

Exposure Category: B

Occupancy Factor : 1.00

Building Type: Enclosed

TC Dead Load: 4.0 psf

BC Dead Load: 6.0 psf

User-defined wind-exposed BC

regions --From-- --To--

0- 2- 4 8- 0- 0

Max comp. force 185 Lbs

Max tens. force 306 Lbs

Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss
 Designer)

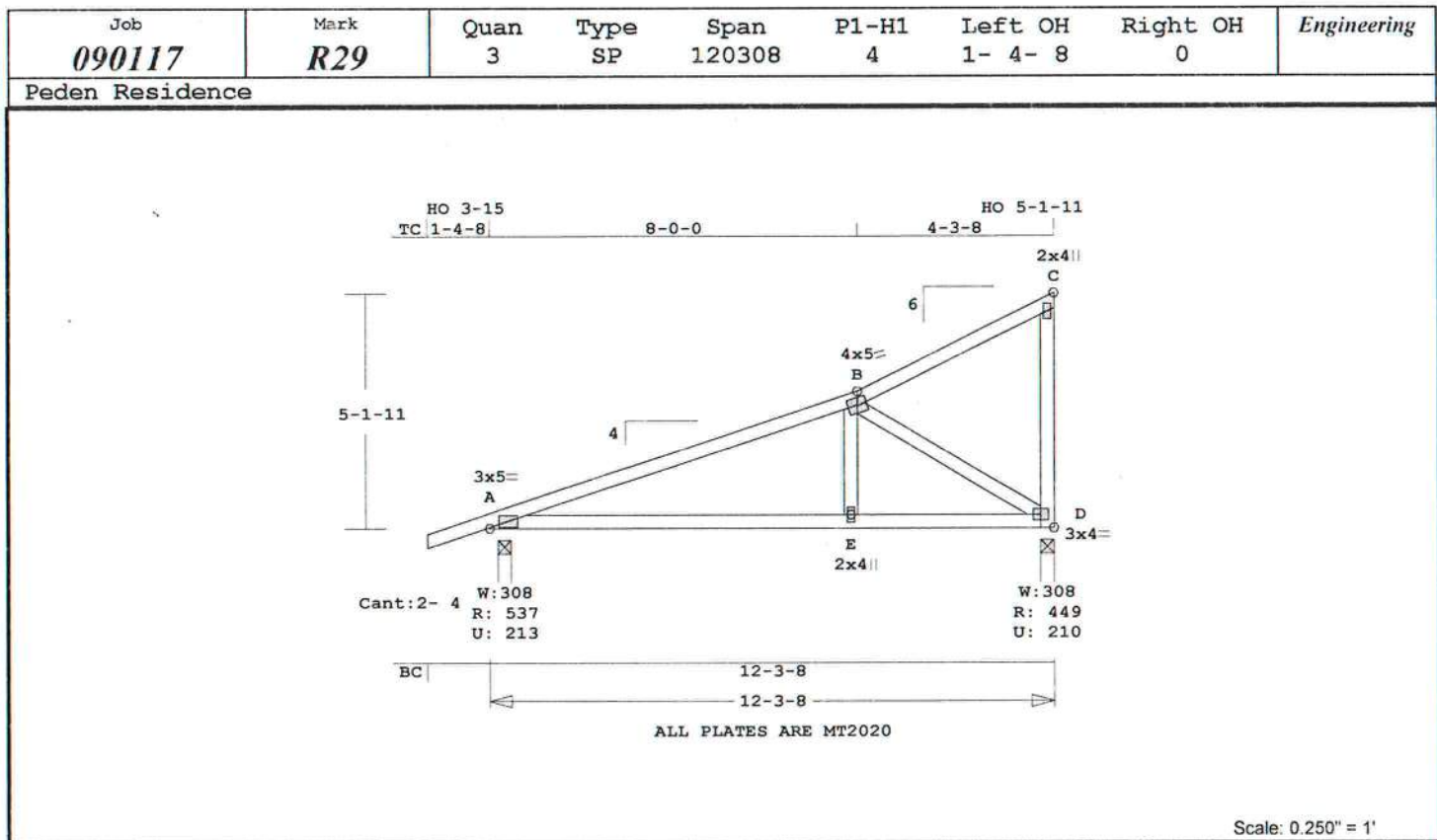
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(352) 375-8593

CA 5201



Online Plus -- Version 23.0.052
 RUN DATE: 13-APR-09

CSI -Size- ----Lumber----

TC	0.50	2x 4	SP-#2
BC	0.42	2x 4	SP-#2
WB	0.29	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	12- 3- 8
BC	Cont.	0- 0- 0	12- 3- 8

psf-Ld	Dead	Live
TC	7.0	20.0
BC	10.0	0.0
TC+BC	17.0	20.0
Total	37.0	Spacing 24.0"
Lumber Duration Factor 1.25		
Plate Duration Factor 1.25		
TC	Fb=1.15	Fc=1.10 Ft=1.10
BC	Fb=1.10	Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	537	214 U	128 R
D	449	211 U	172 R

Jt	Brg Size	Required
A	3.5"	1.5"
D	3.5"	1.5"

Plus 8 Wind Load Case(s)
 Plus 1 UBC LL Load Case(s)
 Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl	CSI-Bnd
-----Top Chords-----				
A -B	0.50	858 T	0.11	0.39
B -C	0.21	80 C	0.00	0.21
-----Bottom Chords-----				
A -E	0.42	867 C	0.01	0.41
E -D	0.38	853 C	0.00	0.38

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 73.3 LBS

-----Webs-----

E -B	0.07	385 C
B -D	0.29	1128 T
D -C	0.20	140 T WindLd

TL Defl	-0.17"	in A -E	L/832
LL Defl	-0.07"	in A -E	L/999
Shear //	Grain	in A -B	0.30

Plates for each ply each face.
 Plate - MT20 20 Ga, Gross Area
 Plate - MT2H 20 Ga, Gross Area
 Jt Type Plt Size X Y JSI

A	MT20	3.0x 5.0	Ctr Ctr	0.52
B	MT20	4.0x 5.0	Ctr Ctr	0.58
C	MT20	2.0x 4.0	Ctr Ctr	0.12
E	MT20	2.0x 4.0	Ctr Ctr	0.19
D	MT20	3.0x 4.0	Ctr Ctr	0.63

NOTES:

Trusses Manufactured by:

RIDGWAY ROOF TRUSS

Analysis Conforms To:

FBC2007

OH Loading

Soffit psf 2.0

Design checked for 10 psf non-concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-05

Truss is designed as

Components and Claddings*

for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 16-0

Exposure Category: B

Occupancy Factor : 1.00

Building Type: Enclosed

TC Dead Load: 4.0 psf

BC Dead Load: 6.0 psf

User-defined wind-exposed BC

regions --From-- ---To---

0- 2- 4 12- 3- 8

Max comp. force 867 Lbs
 Max tens. force 1128 Lbs
 Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss

Designer)

Gary Dounson, PE 35054

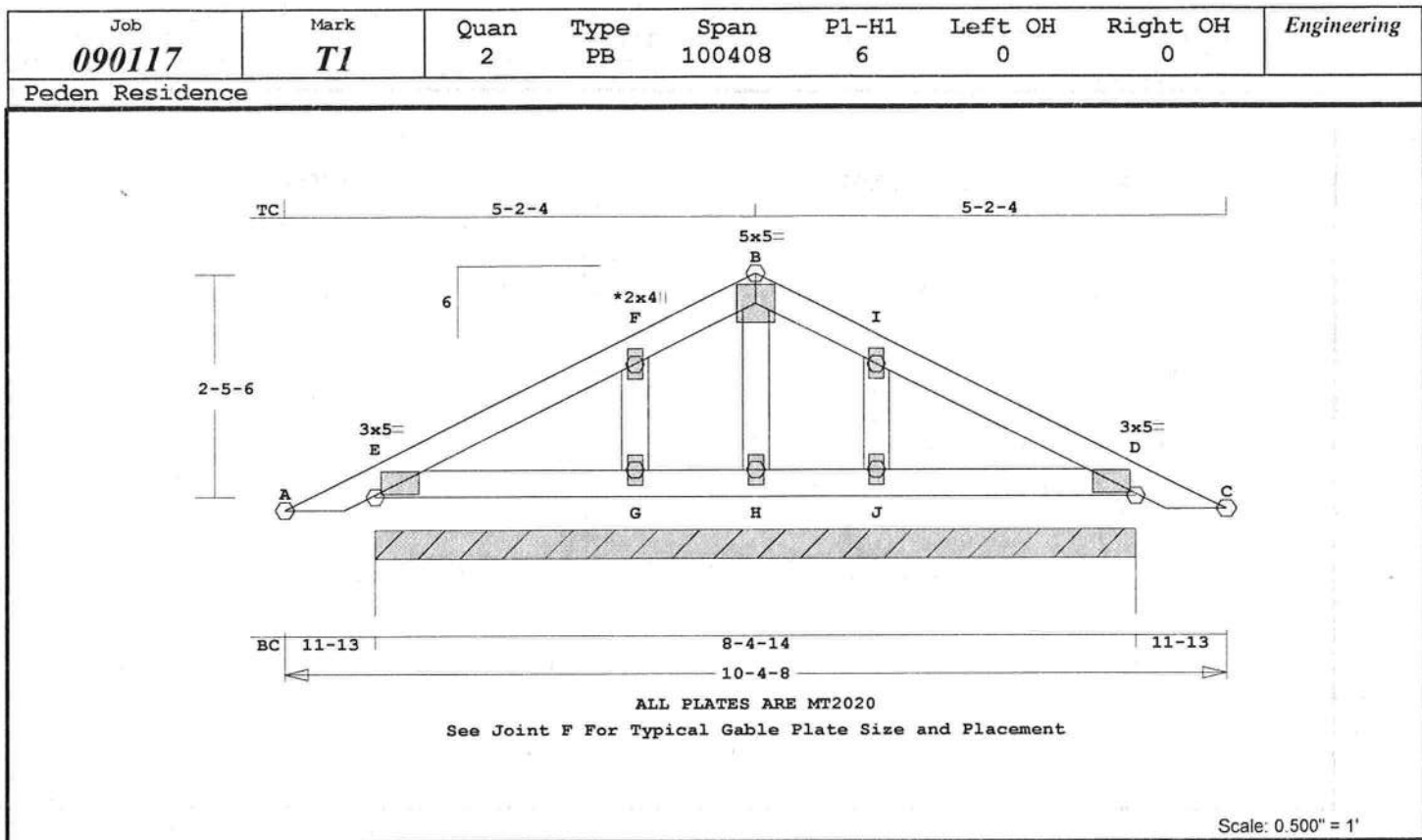
Gary Dounson & Associates, Inc.

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(352)375-8593

CA 5201



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 45.9 LBS
 Online Plus -- Version 23.0.052
 RUN DATE: 06-APR-09

	CSI	-Size-	----	Lumber-----
TC	0.07	2x 4	SP-#2	
BC	0.06	2x 4	SP-#2	
GW	0.05	2x 4	SP-#3	

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	10- 4- 8
BC	Cont.	0- 0- 0	10- 4- 8

psf-Ld	Dead	Live
TC	7.0	20.0
BC	10.0	0.0
TC+BC	17.0	20.0
Total	37.0	Spacing 24.0"
Lumber Duration Factor	1.25	
Plate Duration Factor	1.25	
TC Fb=1.15	Fc=1.10	Ft=1.10
BC Fb=1.10	Fc=1.10	Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
E	625	111 U	35 R

Jt	Brg Size	Required
E	100.9"	0"-to- 101"

Plus 9 Wind Load Case(s)
 Plus 1 UBC LL Load Case(s)
 Plus 1 DL Load Case(s)

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Top Chords-----					
E -F	0.06		58 C	0.00	0.06
F -B	0.07		139 T	0.01	0.06
B -I	0.07		139 T	0.01	0.06
I -D	0.06		58 C	0.00	0.06
-----Bottom Chords-----					
E -G	0.06		12 T	0.00	0.06
G -H	0.03		0 T	0.00	0.03

H -J	0.03	0 T	0.00	0.03
J -D	0.06	12 T	0.00	0.06
-----Gable Webs-----				
G -F	0.05	237 T		
H -B	0.00	70 C		
J -I	0.05	237 T		

TL Defl	0.00"	in E -G	L/999
LL Defl	0.00"	in E -G	L/999
Shear // Grain		in E -F	0.12

Plates for each ply each face.
 Plate - MT20 20 Ga, Gross Area
 Plate - MT2H 20 Ga, Gross Area
 Jt Type Plt Size X Y JSI
 E MT20 3.0x 5.0 Ctr Ctr 0.47
 F MT20 2.0x 4.0 Ctr Ctr 0.00
 B MT20 5.0x 5.0 Ctr Ctr 0.34
 I MT20 2.0x 4.0 Ctr Ctr 0.00
 D MT20 3.0x 5.0 Ctr Ctr 0.47
 G MT20 2.0x 4.0 Ctr Ctr 0.00
 H MT20 2.0x 4.0 Ctr Ctr 0.00
 J MT20 2.0x 4.0 Ctr Ctr 0.00

NOTES:

Trusses Manufactured by:

RIDGWAY ROOF TRUSS

Analysis Conforms To:

FBC2007

OH Loading

Soffit psf 2.0

Design checked for 10 psf non-concurrent LL on BC.

Refer to Gen Det 3 series for web bracing and plating.

Wind Loads - ANSI / ASCE 7-05

Truss is designed as

Components and Claddings*

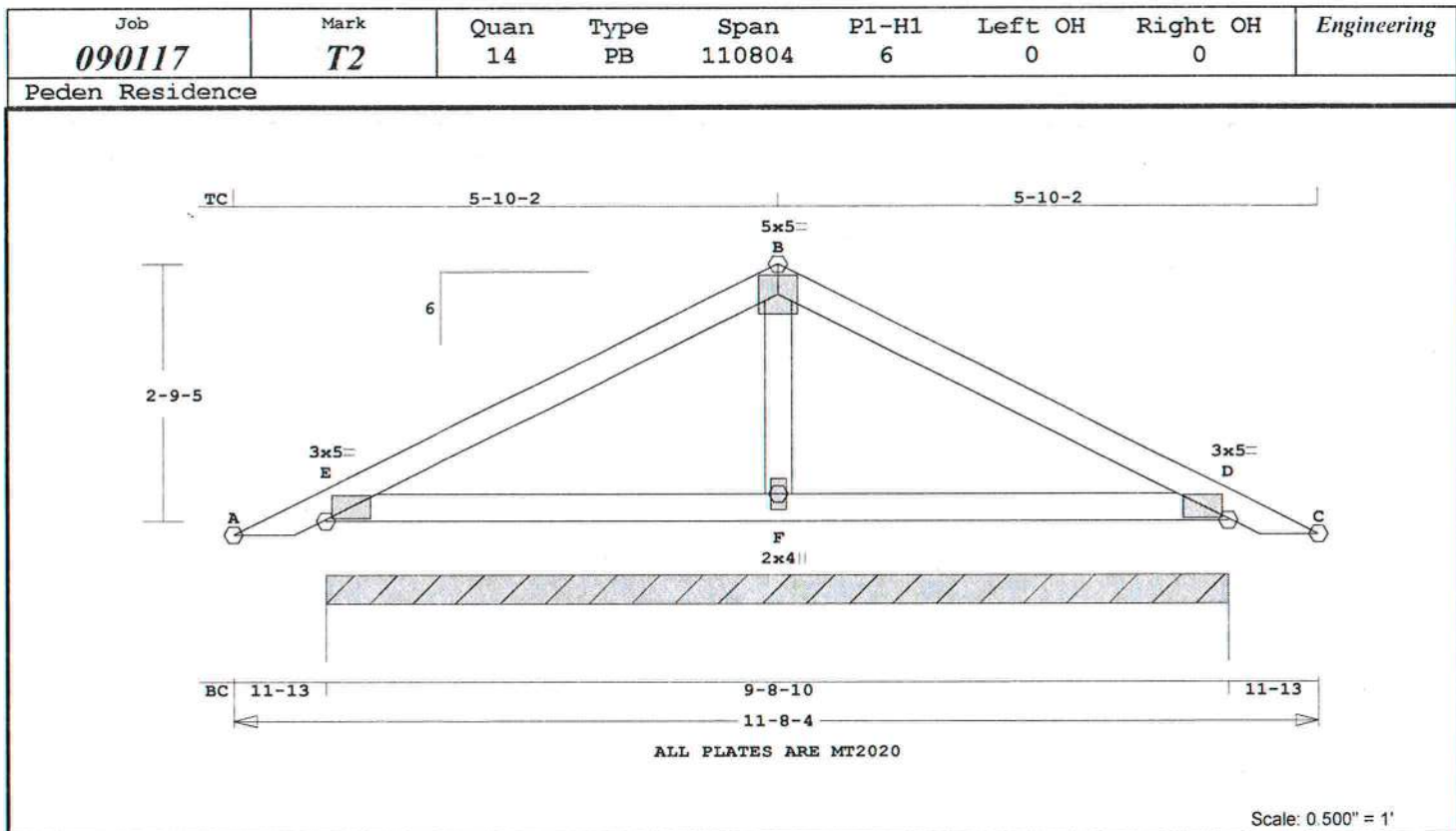
for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 16-0

Exposure Category: B

Occupancy Factor : 1.00
 Building Type: Enclosed
 TC Dead Load: 4.0 psf
 BC Dead Load: 6.0 psf
 Max comp. force 127 Lbs
 Max tens. force 237 Lbs
 Quality Control Factor 1.25
FABRICATOR NOTES:
 1. Delegated Engineer (Truss Designer)
 Gary Dounson, PE 35054
 Gary Dounson & Associates, Inc.
 2830 NW 41st Street Suite D
 Gainesville, FL 32606
 (352)375-8593
 CA 5201



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CSI -Size- ----Lumber-----
 TC 0.22 2x 4 SP-#2
 BC 0.15 2x 4 SP-#2
 WB 0.01 2x 4 SP-#3

Brace truss as follows:

	O.C.	From	To
TC Cont.	0- 0- 0	11- 8- 4	
BC Cont.	0- 0- 0	11- 8- 4	

psf-Ld Dead Live
 TC 7.0 20.0
 BC 10.0 0.0
 TC+BC 17.0 20.0
 Total 37.0 Spacing 24.0"
 Lumber Duration Factor 1.25
 Plate Duration Factor 1.25
 TC Fb=1.15 Fc=1.10 Ft=1.10
 BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
E	722	127 U	40 R

Jt	Brg Size	Required
E	116.6"	0"-to- 117"

Plus 9 Wind Load Case(s)
 Plus 1 UBC LL Load Case(s)
 Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl	CSI-Bnd
-----Top Chords-----				
E -B	0.22	333 T	0.04	0.18
B -D	0.22	333 T	0.04	0.18
-----Bottom Chords-----				
E -F	0.15	22 T	0.00	0.15
F -D	0.15	22 T	0.00	0.15
-----Webs-----				
F -B	0.01	65 T		

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 47.1 LBS

TL Defl -0.02" in F -D L/999
 LL Defl -0.01" in F -D L/999
 Shear // Grain in E -B 0.19

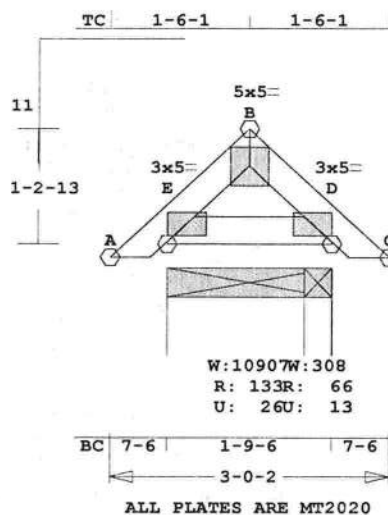
Plates for each ply each face.
 Plate - MT20 20 Ga, Gross Area
 Plate - MT2H 20 Ga, Gross Area
 Jt Type Plt Size X Y JSI
 E MT20 3.0x 5.0 Ctr Ctr 0.47
 B MT20 5.0x 5.0 Ctr Ctr 0.34
 D MT20 3.0x 5.0 Ctr Ctr 0.47
 F MT20 2.0x 4.0 Ctr Ctr 0.12

NOTES:

Trusses Manufactured by:
 RIDGWAY ROOF TRUSS
 Analysis Conforms To:
 FBC2007
 OH Loading
 Soffit psf 2.0
 Design checked for 10 psf non-
 concurrent LL on BC.
 Refer to Gen Det 3 series for
 web bracing and plating.
 Wind Loads - ANSI / ASCE 7-05
 Truss is designed as
 Components and Claddings*
 for Exterior zone location.
 Wind Speed: 110 mph
 Mean Roof Height: 16-0
 Exposure Category: B
 Occupancy Factor : 1.00
 Building Type: Enclosed
 TC Dead Load: 4.0 psf
 BC Dead Load: 6.0 psf
 Max comp. force 279 Lbs
 Max tens. force 333 Lbs
 Quality Control Factor 1.25
 FABRICATOR NOTES:
 1. Delegated Engineer (Truss
 Designer)
 Gary Dounson, PE 35054
 Gary Dounson & Associates,

Job	Mark	Quan	Type	Span	Pl-H1	Left OH	Right OH	Engineering
090117	T4	7	PB	30002	11	0	0	

Peden Residence



Scale: 0.500" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 11.1 LBS

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Shear // Grain in E -D 0.02

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(352) 375-8593
CA 5201

CSI -Size- ----Lumber-----
TC 0.00 2x 4 SP-#2
BC 0.00 2x 4 SP-#2

Brace truss as follows:

	O.C.	From	To
TC Cont.	0- 0- 0	3- 0- 2	
BC Cont.	0- 0- 0	3- 0- 2	

psf-Ld Dead Live
TC 7.0 20.0
BC 10.0 0.0
TC+BC 17.0 20.0
Total 37.0 Spacing 24.0"
Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
D	133	27 U	16 R
D	67	13 U	16 R

Jt	Brg Size	Required
D	21.4"	0"-to- 21"
D	3.5"	1.5"

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)
Plus 1 DL Load Case(s)

Membr CSI P Lbs Axl-CSI-Bnd
-----Top Chords-----
E -B 0.00 31 C
B -D 0.00 31 C
-----Bottom Chords-----
E -D 0.00 13 T

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
E MT20 3.0x 5.0 0.7 0.7 0.47
B MT20 5.0x 5.0 Ctr-2.4 0.33
D MT20 3.0x 5.0-0.7 0.7 0.47

NOTES:

Trusses Manufactured by:

RIDGWAY ROOF TRUSS

Analysis Conforms To:

FBC2007

OH Loading

Soffit psf 2.0

Design checked for 10 psf non-concurrent LL on BC.

Refer to Gen Det 3 series for web bracing and plating.

Wind Loads - ANSI / ASCE 7-05

Truss is designed as

Components and Claddings*

for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 16-0

Exposure Category: B

Occupancy Factor : 1.00

Building Type: Enclosed

TC Dead Load: 4.0 psf

BC Dead Load: 6.0 psf

Max comp. force 31 Lbs

Max tens. force 24 Lbs

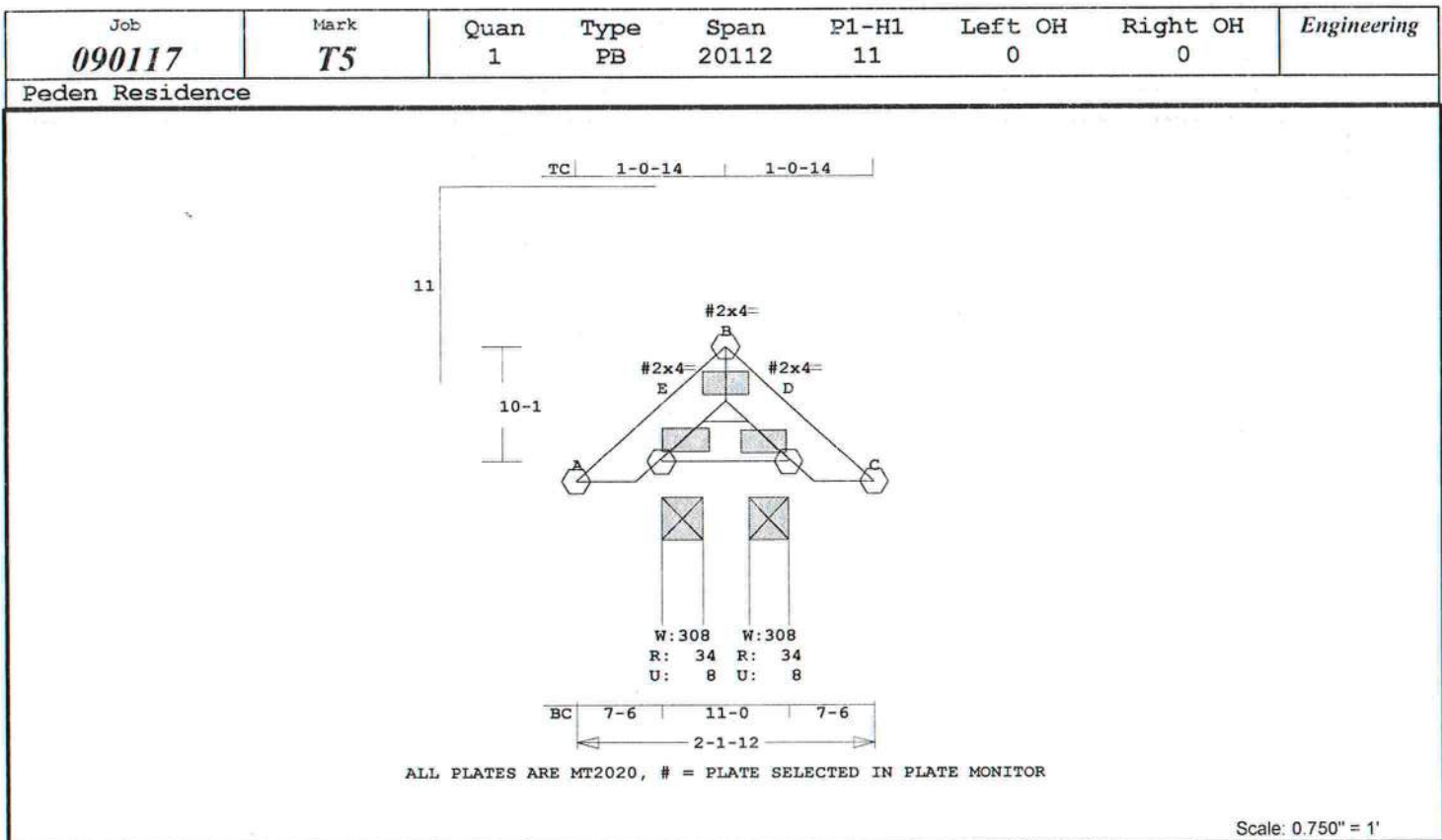
Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss Designer)

Gary Dounson, PE 35054

Gary Dounson & Associates, Inc.



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 RUN DATE: 06-APR-09

CSI -Size- ----Lumber-----
 TC 0.00 2x 4 SP-#2
 BC 0.00 2x 4 SP-#2

Brace truss as follows:

	O.C.	From	To
TC Cont.	0- 0- 0	2- 1-12	
BC Cont.	0- 0- 0	2- 1-12	

psf-Ld	Dead	Live
TC	7.0	20.0
BC	10.0	0.0
TC+BC	17.0	20.0
Total	37.0	24.0"
Lumber Duration Factor	1.25	
Plate Duration Factor	1.25	
TC Fb=1.15 Fc=1.10 Ft=1.10		
BC Fb=1.10 Fc=1.10 Ft=1.10		

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
E	35	8 U	6 R
D	35	8 U	6 R

Jt	Brg Size	Required
E	3.5"	1.5"
D	3.5"	1.5"

Plus 9 Wind Load Case(s)
 Plus 1 UBC LL Load Case(s)
 Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Ax1-CSI-Bnd
-----Top Chords-----			
-----Bottom Chords-----			
E -D	0.00	6 T	

Shear // Grain in E -D 0.00

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 7.2 LBS

Plates for each ply each face.
 Plate - MT20 20 Ga, Gross Area
 Plate - MT2H 20 Ga, Gross Area
 Jt Type Plt Size X Y JSI
 E MT20 2.0x 4.0 0.3 0.1 0.60
 B MT20 2.0x 4.0 Ctr-0.8 0.41
 D MT20 2.0x 4.0-0.4-0.1 0.68

NOTES:

Trusses Manufactured by:

RIDGWAY ROOF TRUSS

Analysis Conforms To:

FBC2007

OH Loading

Soffit psf 2.0

Design checked for 10 psf non-concurrent LL on BC.

NOTE: USER MODIFIED PLATES

This design may have plates selected through a plate monitor.

Wind Loads - ANSI / ASCE 7-05

Truss is designed as

Components and Claddings*

for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 16-0

Exposure Category: B

Occupancy Factor : 1.00

Building Type: Enclosed

TC Dead Load: 4.0 psf

BC Dead Load: 6.0 psf

Max comp. force 12 Lbs

Max tens. force 9 Lbs

Quality Control Factor 1.25

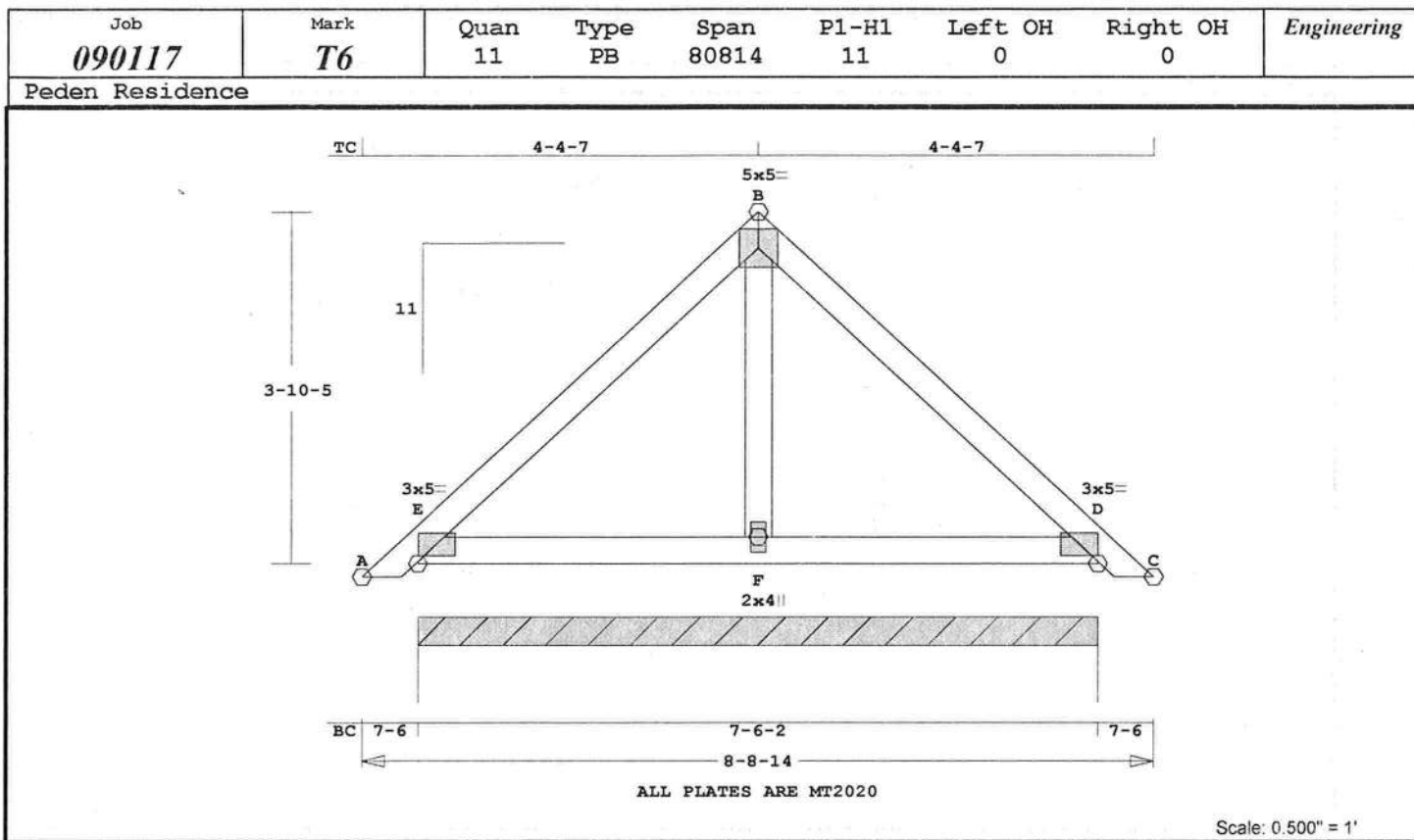
FABRICATOR NOTES:

1. Delegated Engineer (Truss Designer)

Gary Dounson, PE 35054

Gary Dounson & Associates, Inc.

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Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 42.6 LBS

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CSI -Size- ----Lumber----

TC	0.10	2x 4	SP-#2
BC	0.08	2x 4	SP-#2
WB	0.02	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	8- 8-14
BC	Cont.	0- 0- 0	8- 8-14

psf-Ld	Dead	Live
TC	7.0	20.0
BC	10.0	0.0
TC+BC	17.0	20.0
Total	37.0	Spacing 24.0"
Lumber Duration Factor	1.25	
Plate Duration Factor	1.25	
TC Fb=1.15	Fc=1.10	Ft=1.10
BC Fb=1.10	Fc=1.10	Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
E	557	96 U	76 R

Jt	Brg Size	Required
E	90.1"	0"-to- 90"

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)
Plus 1 DL Load Case(s)

Membr CSI P Lbs Ax1-CSI-Bnd

-----Top Chords-----

E -B	0.10	201 C	0.02	0.08
B -D	0.10	201 C	0.02	0.08

-----Bottom Chords-----

E -F	0.08	0 T	0.00	0.08
F -D	0.08	0 T	0.00	0.08

-----Webs-----

F -B	0.02	103 T
------	------	-------

TL Defl	0.00"	in E -F	L/999
LL Defl	0.00"	in E -F	L/999
Shear //	Grain	in E -B	0.10

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
E MT20 3.0x 5.0 0.7 0.7 0.47
B MT20 5.0x 5.0 Ctr Ctr 0.33
D MT20 3.0x 5.0-0.7 0.7 0.47
F MT20 2.0x 4.0 Ctr Ctr 0.12

NOTES:

Trusses Manufactured by:

RIDGWAY ROOF TRUSS

Analysis Conforms To:

FBC2007

OH Loading

Soffit psf 2.0

Design checked for 10 psf non-concurrent LL on BC.

Refer to Gen Det 3 series for web bracing and plating.

Wind Loads - ANSI / ASCE 7-05

Truss is designed as

Components and Claddings*
for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 16'-0"

Exposure Category: B

Occupancy Factor : 1.00

Building Type: Enclosed

TC Dead Load: 4.0 psf

BC Dead Load: 6.0 psf

Max comp. force 201 Lbs

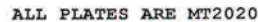
Max tens. force 170 Lbs

Quality Control Factor 1.25

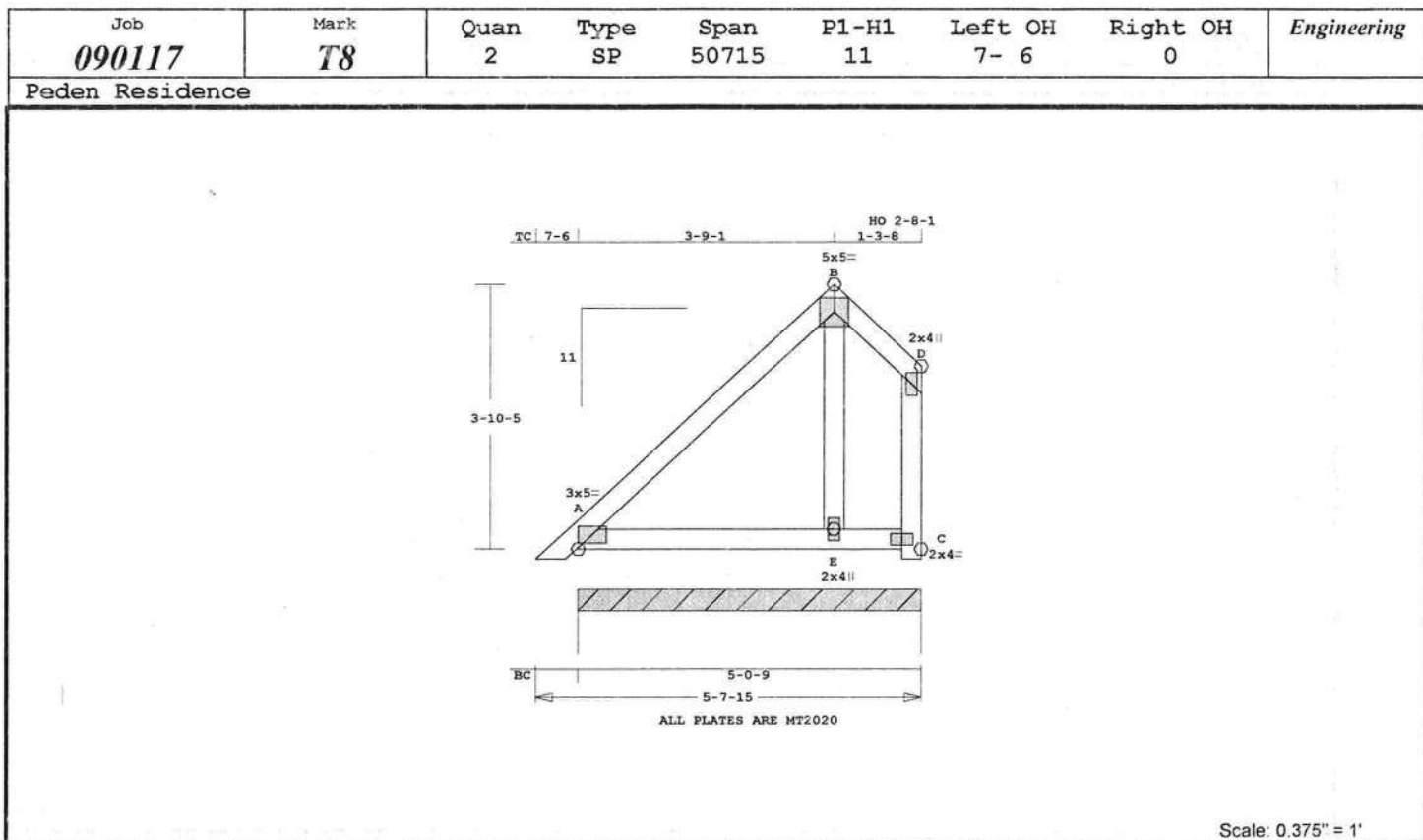
FABRICATOR NOTES:

1. Delegated Engineer (Truss Designer)

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Inc.
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CA 5201



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 RUN DATE: 13-APR-09

CSI	Size	-----Lumber-----
TC	0.10	2x 4 SP-#2
BC	0.11	2x 4 SP-#2
WB	0.08	2x 4 SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	5- 7-15
BC Cont.	0- 0- 0	5- 7-15

psf-Ld	Dead	Live
TC	7.0	20.0
BC	10.0	0.0
TC+BC	17.0	20.0
Total	37.0	Spacing 24.0"
Lumber Duration Factor	1.25	
Plate Duration Factor	1.25	
TC Fb=1.15	Fc=1.10	Ft=1.10
BC Fb=1.10	Fc=1.10	Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz
A	371	65 U	107 R

Jt	Brg Size	Required
A	60.6"	0"-to- 61"

Plus 9 Wind Load Case(s)
 Plus 1 UBC LL Load Case(s)
 Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl	CSI-Bnd
-----Top Chords-----				
A -B	0.10	68 C	0.00	0.10
B -D	0.04	117 T	0.01	0.03
-----Bottom Chords-----				
A -E	0.11	1 T	0.00	0.11
E -C	0.04	0 T	0.00	0.04
-----Webs-----				
E -B	0.07	117 C	0.00	0.07

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 35.1 LBS

C -D 0.08 104 T 0.02 0.06

TL Defl -0.01" in A -E L/999
 LL Defl 0.00" in A -E L/999
 Shear // Grain in A -B 0.11

Plates for each ply each face.
 Plate - MT20 20 Ga, Gross Area
 Plate - MT2H 20 Ga, Gross Area
 Jt Type Plt Size X Y JSI
 A MT20 3.0x 5.0 0.7 0.7 0.47
 B MT20 5.0x 5.0 Ctr-0.1 0.33
 D MT20 2.0x 4.0 Ctr Ctr 0.13
 E MT20 2.0x 4.0 Ctr Ctr 0.12
 C MT20 2.0x 4.0 Ctr Ctr 0.12

NOTES:

Trusses Manufactured by:

RIDGWAY ROOF TRUSS

Analysis Conforms To:

FBC2007

OH Loading

Soffit psf 2.0

Design checked for 10 psf non-concurrent LL on BC.

Refer to Gen Det 3 series for web bracing and plating.

Wind Loads - ANSI / ASCE 7-05

Truss is designed as

Components and Claddings*

for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 16-0

Exposure Category: B

Occupancy Factor : 1.00

Building Type: Enclosed

TC Dead Load: 4.0 psf

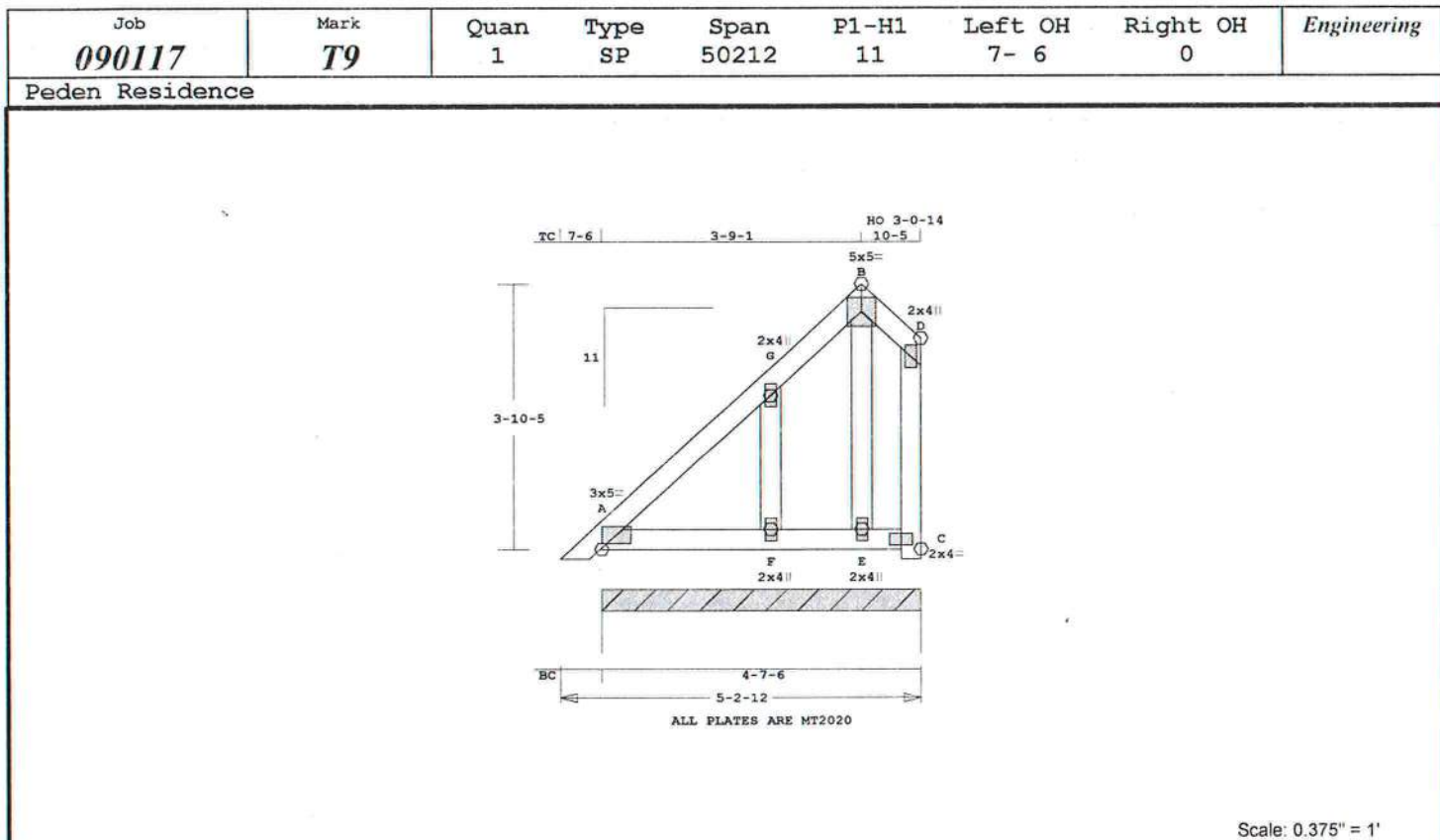
BC Dead Load: 6.0 psf

Max comp. force 117 Lbs

Max tens. force 117 Lbs

Quality Control Factor 1.25

FABRICATOR NOTES:



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 37.8 LBS
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CSI	Size	----Lumber----
TC	0.04	2x 4 SP-#2
BC	0.06	2x 4 SP-#2
WB	0.15	2x 4 SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	5- 2-12
BC Cont.	0- 0- 0	5- 2-12

psf-Ld	Dead	Live
TC	7.0	20.0
BC	10.0	0.0
TC+BC	17.0	20.0
Total	37.0	Spacing 24.0"
Lumber Duration Factor	1.25	
Plate Duration Factor	1.25	
TC Fb=1.15	Fc=1.10	Ft=1.10
BC Fb=1.10	Fc=1.10	Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz
A	418	62 U	157 R

Jt	Brg Size	Required
A	55.4"	0"-to- 55"

Plus 9 Wind Load Case(s)
 Plus 1 UBC LL Load Case(s)
 Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl	CSI-Bnd
-------	-----	-------	-----	---------

-----Top Chords-----				
A	-G	0.04	76 C	0.00 0.04
G	-B	0.03	89 T	0.00 0.03
B	-D	0.02	107 T	0.01 0.01

-----Bottom Chords-----				
A	-F	0.04	1 T	0.00 0.04
F	-E	0.02	0 T	0.00 0.02
E	-C	0.06	1 C	0.00 0.06

-----Webs-----					
F	-G	0.08	143 C	0.03	0.05
E	-B	0.01	83 C	0.00	0.01
C	-D	0.15	90 T	0.02	0.13

TL Defl	0.00"	in A -F	L/999
LL Defl	0.00"	in A -F	L/999
Shear // Grain	in E -C	0.08	

Plates for each ply each face.
 Plate - MT20 20 Ga, Gross Area
 Plate - MT2H 20 Ga, Gross Area
 Jt Type Plt Size X Y JSI

A	MT20	3.0x	5.0	0.7	0.7	0.47
G	MT20	2.0x	4.0	Ctr	Ctr	0.13
B	MT20	5.0x	5.0	Ctr	Ctr	0.33
D	MT20	2.0x	4.0	Ctr	Ctr	0.13
F	MT20	2.0x	4.0	Ctr	Ctr	0.12
E	MT20	2.0x	4.0	Ctr	Ctr	0.12
C	MT20	2.0x	4.0	Ctr	Ctr	0.12

NOTES:

Trusses Manufactured by:

RIDGWAY ROOF TRUSS

Analysis Conforms To:

FBC2007

OH Loading

Soffit psf 2.0

Design checked for 10 psf non-concurrent LL on BC.

Refer to Gen Det 3 series for web bracing and plating.

Wind Loads - ANSI / ASCE 7-05

Truss is designed as

Components and Claddings*

for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 16-0

Exposure Category: B

Occupancy Factor : 1.00

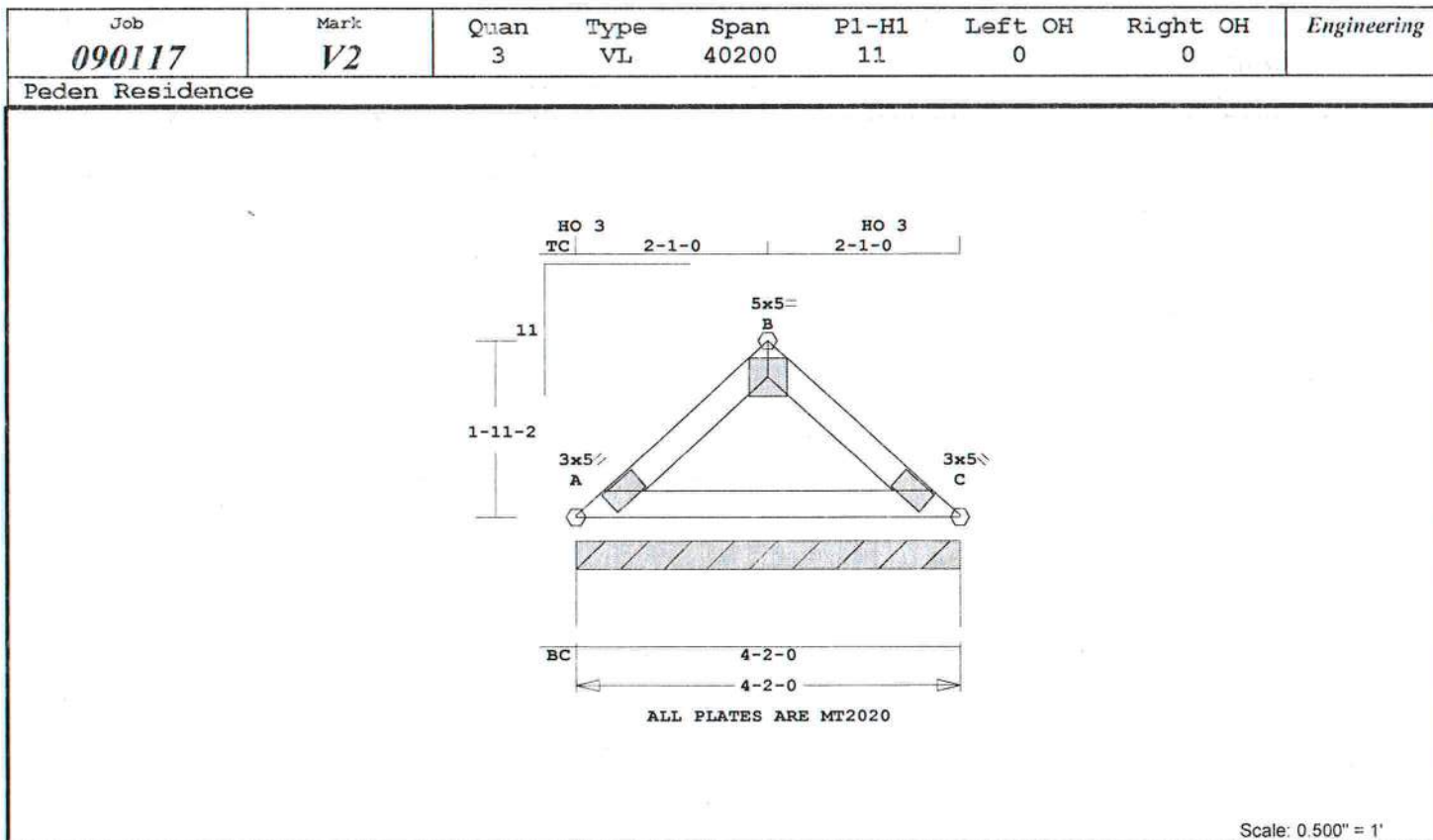
Building Type: Enclosed

TC Dead Load: 4.0 psf

Peden Residence



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Online Plus -- Version 23.0.052
 RUN DATE: 06-APR-09

CSI -Size- ----Lumber----

TC	0.03	2x 4	SP-#2
BC	0.04	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	4- 2- 0
BC	Cont.	0- 0- 0	4- 2- 0

psf-Ld	Dead	Live
TC	7.0	20.0
BC	10.0	0.0
TC+BC	17.0	20.0
Total	37.0	24.0
Spacing 24.0"		
Lumber Duration Factor 1.25		
Plate Duration Factor 1.25		
TC Fb=1.15	Fc=1.10	Ft=1.10
BC Fb=1.10	Fc=1.10	Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz
A	256	42 U	34 R

Jt	Brg Size	Required
A	50.0"	0"-to- 50"

Plus 9 Wind Load Case(s)
 Plus 1 UBC LL Load Case(s)
 Plus 1 DL Load Case(s)

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Top Chords-----					
A -B	0.03	78	C	0.00	0.03
B -C	0.03	78	C	0.00	0.03
-----Bottom Chords-----					
A -C	0.04	1	C	0.00	0.04

TL Defl	-0.01"	in A -C	L/999
LL Defl	0.00"	in A -C	L/999
Shear //	Grain	in A -C	0.06

Plates for each ply each face.
 Plate - MT20 20 Ga, Gross Area
 Plate - MT2H 20 Ga, Gross Area
 Jt Type Plt Size X Y JSI
 A MT20 3.0x 5.0 Ctr Ctr 0.41
 B MT20 5.0x 5.0 Ctr-2.4 0.33
 C MT20 3.0x 5.0 Ctr Ctr 0.41

NOTES:

Trusses Manufactured by:

RIDGWAY ROOF TRUSS

Analysis Conforms To:

FBC2007

Design checked for 10 psf non-concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-05

Truss is designed as

Components and Claddings*

for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 16'-0"

Exposure Category: B

Occupancy Factor : 1.00

Building Type: Enclosed

TC Dead Load: 4.0 psf

BC Dead Load: 6.0 psf

Max comp. force 78 Lbs

Max tens. force 54 Lbs

Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss Designer)

Gary Dounson, PE 35054

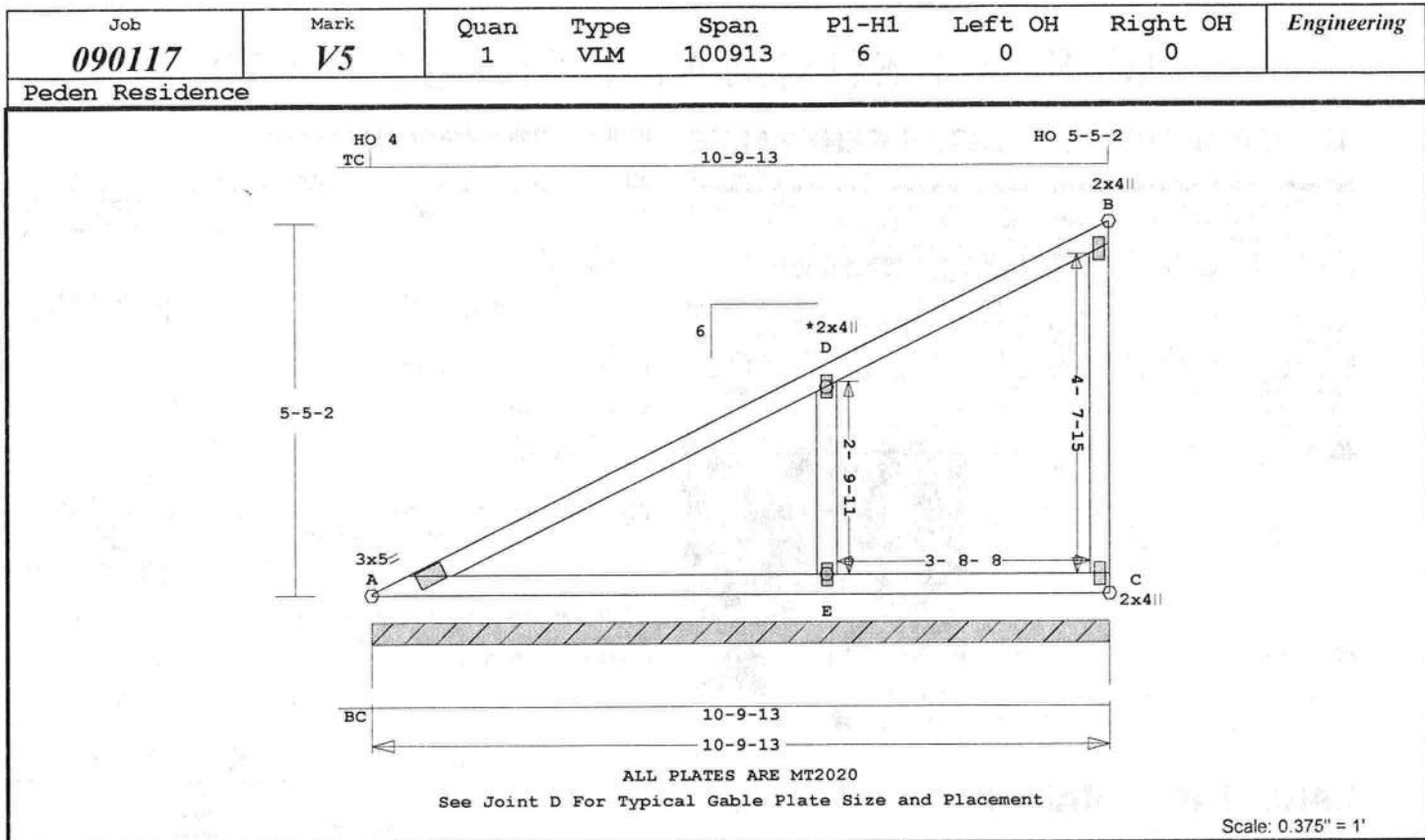
Gary Dounson & Associates, Inc.

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Gainesville, FL 32606

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CA 5201



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 56.5 LBS

Online Plus -- Version 23.0.052
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	CSI	-Size-	----	Lumber	----
TC	0.34	2x 4	SP-#2		
BC	0.22	2x 4	SP-#2		
WB	0.39	2x 4	SP-#3		
GW	0.26	2x 4	SP-#3		

Brace truss as follows:

	O.C.	From	To
TC Cont.	0- 0- 0	10- 9-13	
BC Cont.	0- 0- 0	10- 9-13	

psf-Ld	Dead	Live
TC	7.0	20.0
BC	10.0	0.0
TC+BC	17.0	20.0
Total	37.0	24.0"
Lumber Duration Factor 1.25		
Plate Duration Factor 1.25		
TC Fb=1.15	Fc=1.10	Ft=1.10
BC Fb=1.10	Fc=1.10	Ft=1.10

Total Load Reactions (Lbs)
Jt Down Uplift Horiz-
A 957 124 U 323 R

Jt	Brg Size	Required
A	129.8"	0"-to- 130"

Plus 8 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Ax1	CSI-Bnd
-----Top Chords-----				
A -D	0.34	125 C	0.00	0.34
D -B	0.22	84 C	0.00	0.22
-----Bottom Chords-----				
A -E	0.22	2 C	0.00	0.22
E -C	0.15	0 T	0.00	0.15
-----Webs-----				

C -B	0.39	154 T	0.03	0.36
-----Gable Webs-----				
E -D	0.26	372 T	0.07	0.19
TL Defl	-0.06" in A -E L/999			
LL Defl	-0.02" in A -E L/999			
Shear // Grain	in A -D 0.21			

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A MT20 3.0x 5.0 Ctr Ctr 0.47
D MT20 2.0x 4.0 Ctr Ctr 0.00
B MT20 2.0x 4.0 Ctr Ctr 0.12
E MT20 2.0x 4.0 Ctr Ctr 0.00
C MT20 2.0x 4.0 Ctr Ctr 0.12

NOTES:

Trusses Manufactured by:

RIDGWAY ROOF TRUSS

Analysis Conforms To:

FBC2007

Design checked for 10 psf non-concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-05

Truss is designed as

Components and Claddings*

for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 16-0

Exposure Category: B

Occupancy Factor : 1.00

Building Type: Enclosed

TC Dead Load: 4.0 psf

BC Dead Load: 6.0 psf

Max comp. force 298 Lbs

Max tens. force 372 Lbs

Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss Designer)

Gary Dounson, PE 35054
Gary Dounson & Associates,
Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

BCSI-B1 SUMMARY SHEET 1 of 2

GENERAL NOTES

Trusses are not marked in any way to identify the frequency or location of temporary bracing. Follow the recommendations for handling, installing and temporary bracing of trusses. Refer to BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses for more detailed information.

Truss Design Drawings may specify locations of permanent bracing on individual compression members. Refer to the BCSI-B3 Summary Sheet - Web Member Permanent Bracing/Web Reinforcement for more information. All other permanent bracing design is the responsibility of the Building Designer.

NOTAS GENERALES

Los trusses no están marcados de ningún modo que identifique la frecuencia o localización de los arriostres (bracing) temporales. Use las recomendaciones de manejo, instalación y arrioste temporal de los trusses. Vea el folleto BCSI 1-03 Guía de Buena Práctica para el Manejo, Instalación y Arrioste de los Trusses de Madera Conectados con Placas de Metal para mayor información.

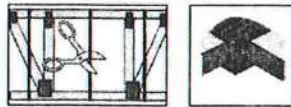
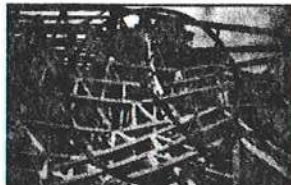
Los dibujos de diseño de los trusses pueden especificar las localizaciones de los arriostres permanentes en los miembros individuales en compresión. Vea la hoja resumen BCSI-B3 para los arriostres permanentes y refuerzos de los miembros secundarios (webs) para mayor información. El resto de arriostres permanentes son la responsabilidad del Diseñador del Edificio.

⚠ The consequences of improper handling, installing and bracing may be a collapse of the structure, or worse, serious personal injury or death.

El resultado de un manejo, instalación y arrioste inadecuados, puede ser la caída de la estructura o aun peor, muertos o heridos.

⚠ Banding and truss plates have sharp edges. Wear gloves when handling and safety glasses when cutting banding.

Empaques y placas de metal tienen bordes afilados. Use guantes y lentes protectores cuando corte los empaques.



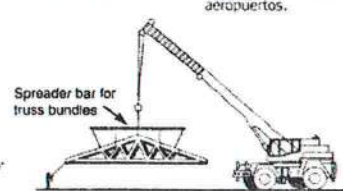
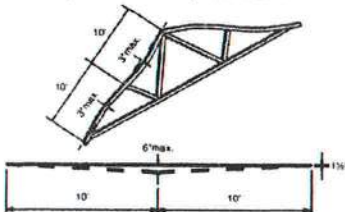
HANDLING — MANEJO

✓ Allow no more than 3" of deflection for every 10' of span.

No permita mas de 3 pulgadas de pandeo por cada 10 pies de tramo.

⚠ Use special care in windy weather or near power lines and airports.

Utilice cuidado especial en días ventosos o cerca de cables eléctricos o de aeropuertos.



✓ Check banding prior to moving bundles.

Revise los empaques antes de mover los paquetes de trusses.

⚠ Avoid lateral bending. — Evite la flexión lateral.



✓ Pick up vertical bundles at the top chord.

Levante de la cuerda superior los grupos verticales de trusses.



ONE WEEK OR LESS

MORE THAN ONE WEEK



✓ Bundles stored on the ground for one week or more should be raised by blocking at 8' to 10' on center.

Los paquetes almacenados en la tierra por una semana o más deben ser elevados con bloques a cada 8 o 10 pies.

✓ For long term storage, cover bundles to prevent moisture gain but allow for ventilation.

Para almacenamiento por mayor tiempo, cubra los paquetes para prevenir aumento de humedad pero permita ventilación.

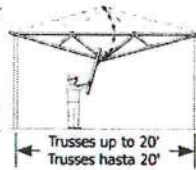


⚠ Do not store on uneven ground.

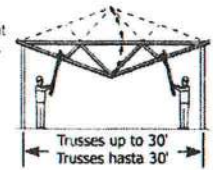
No almacene en tierra desigual.

HAND ERECTION — LEVANTAMIENTO A MANO

✓ Trusses 20' or less, support at peak.
Levante del pico los trusses de 20 pies o menos.



✓ Trusses 30' or less, support at quarter points.
Levante de los cuartos de tramo los trusses de 30 pies o menos.



HOISTING — LEVANTAMIENTO

✓ Hold each truss in position with the erection equipment until temporary bracing is installed and truss is fastened to the bearing points.

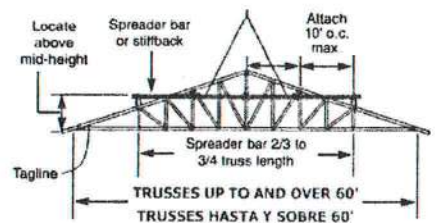
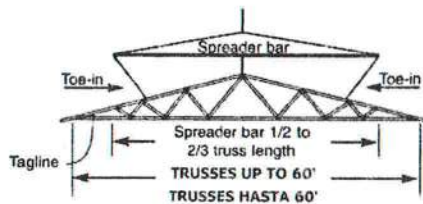
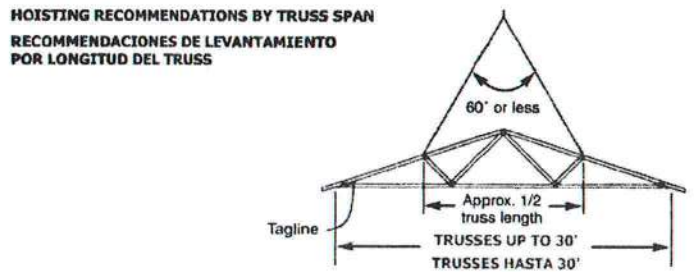
Sostenga cada truss en posición con la grúa hasta que el arrioste temporal esté instalado y el truss asegurado en los soportes.

⚠ Do not lift trusses over 30' by the peak.

No levante del pico los trusses de más de 30 pies.



HOISTING RECOMMENDATIONS BY TRUSS SPAN RECOMENDACIONES DE LEVANTAMIENTO POR LONGITUD DEL TRUSS



BRACING — ARRIOSTRE

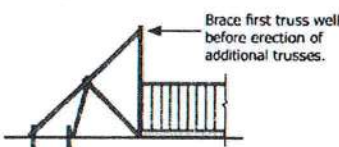
⚠ Refer to BCSI-B2 Summary Sheet - Truss Installation and Temporary Bracing for more information. Vea el resumen BCSI-B2 - Instalación de Trusses y Arrioste Temporal para mayor información.

⚠ Do not walk on unbraced trusses.

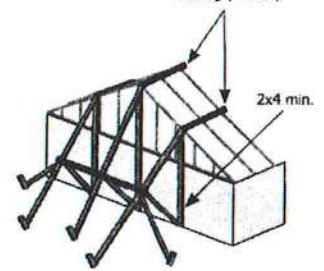
No camine en trusses sueltos.

✓ Locate ground braces for first truss directly in line with all rows of top chord temporary lateral bracing.

Coloque los arriostres de tierra para el primer truss directamente en línea con cada una de las filas de arriostres laterales temporales de la cuerda superior.



Top Chord Temporary Lateral Bracing (TCTLB)



BCSI-B1 SUMMARY SHEET 2 of 2

BRACING FOR THREE PLANES OF ROOF EL ARRIOSTRE EN TRES PLANOS DE TECHO

- ✓ This bracing method is for all trusses except 3x2 and 4x2 parallel chord trusses.
Este método de arriostre es para todo trusses de cuerdas paralelas 3x2 y 4x2

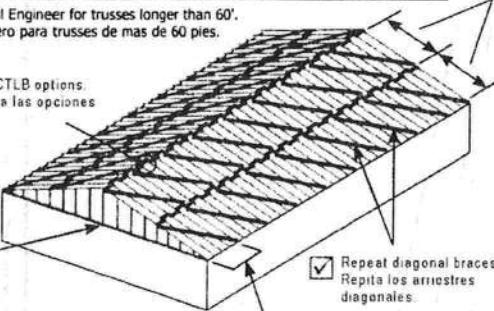
1) TOP CHORD — CUERDA SUPERIOR

Truss Span Longitud de Tramo	Top Chord Temporary Lateral Brace (TCLB) Spacing Espaciamiento del Arriostre Temporal de la Cuerda Superior
Up to 30'	10' o.c. max.
Hasta 30 pies	10 pies máximo
30' to 45'	8' o.c. max.
30 a 45 pies	8 pies máximo
45' to 60'	6' o.c. max.
45 a 60 pies	6 pies máximo
60' to 80'	4' o.c. max.
60 a 80 pies*	4 pies máximo

*Consult a Professional Engineer for trusses longer than 60'.
*Consulte a un ingeniero para trusses de mas de 60 pies.

- ✓ See BCSI-B2 for TCLB options.
Vea el BCSI-B2 para las opciones de TCLB.

- Refer to BCSI-B6 Summary Sheet: Cable End Frame Bracing.
Vea el resumen BCSI-B6: Arriostre del truss terminal de un techo a dos aguas.



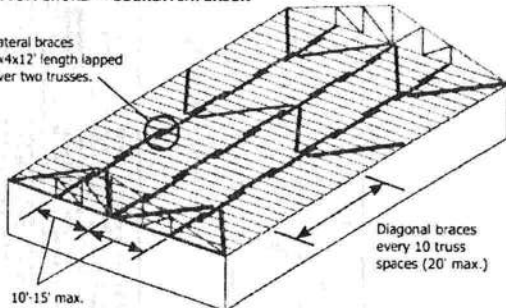
- Repeat diagonal braces.
Repita los arriostres diagonales.

- ✓ Set first five trusses with spacer pieces, then add diagonals. Repeat process on groups of four trusses until all trusses are set.

Instale los cinco primeros trusses con espaciadores. Luego los arriostres diagonales. Repita este procedimiento en grupos de cuatro trusses hasta que todos los trusses estén instalados.

2) BOTTOM CHORD — CUERDA INFERIOR

Lateral braces
2x4x12' length lapped
over two trusses.



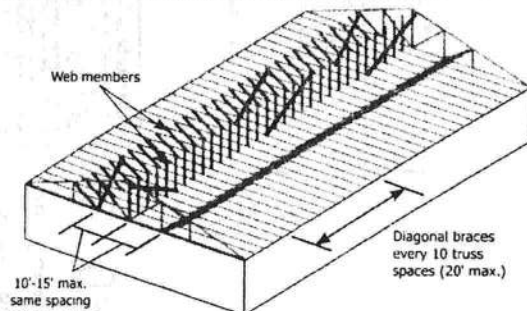
Diagonal braces
every 10 truss
spaces (20' max.)

10'-15' max.

Some chord and web members not shown for clarity.

3) WEB MEMBER PLANE — PLANO DE LOS MIEMBROS SECUNDARIOS

Web members



Diagonal braces
every 10 truss
spaces (20' max.)

10'-15' max.

same spacing
as bottom chord
lateral bracing

Some chord and web members not shown for clarity.

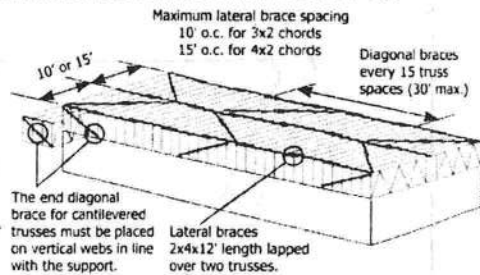
⚠ **DIAGONAL BRACING IS VERY IMPORTANT**
¡EL ARRIOSTRE DIAGONAL ES MUY IMPORTANTE! ⚠

BRACING FOR 3x2 AND 4x2 PARALLEL CHORD TRUSSES

EL ARRIOSTRE PARA TRUSSES DE CUERDAS PARALELAS 3x2 Y 4x2

- Refer to BCSI-B7 Summary Sheet: Temporary and Permanent Bracing for Parallel Chord Trusses for more information.

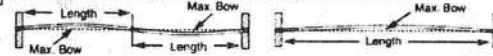
Vea el resumen BCSI-B7: Arriostre temporal y permanente de trusses de cuerdas paralelas para mayor información.



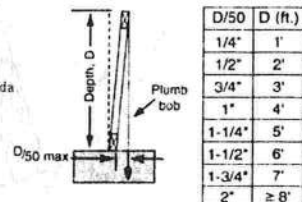
The end diagonal brace for cantilevered trusses must be placed on vertical webs in line with the support.

INSTALLING — INSTALACION

- ✓ Tolerances for Out-of-Plane. Tolerancias para Fuera-de-Plano.



- ✓ Tolerances for Out-of-Plumb. Tolerancias para Fuera-de-Plomada.



Max. Bow	Truss Length
3/4"	12.5'
7/8"	14.6'
1"	16.7'
1-1/8"	18.8'
1-1/4"	20.8'
1-3/8"	22.9'
1-1/2"	25.0'
1-3/4"	29.2'
2"	≥ 33.3'

CONSTRUCTION LOADING — CARGA DE CONSTRUCCION

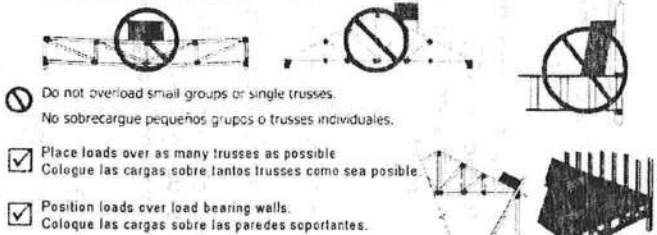
- Do not proceed with construction until all bracing is securely and properly in place.

No proceda con la construcción hasta que todos los arriostres estén colocados en forma apropiada y segura.

- Do not exceed maximum stack heights. Refer to BCSI-B4 Summary Sheet: Construction Loading for more information.

No exceda las máximas alturas recomendadas. Vea el resumen BCSI-B4 Carga de Construcción para mayor información.

Material	Height (h)
Gypsum Board	12'
Plywood or OSB	16'
Asphalt Shingles	2 bundles
Concrete Block	8'
Clay Tile	3-4 tiles high



ALTERATIONS — ALTERACIONES

- Refer to BCSI-B5 Summary Sheet: Truss Damage, Jobsite Modifications and Installation Errors. Vea el resumen BCSI-B5 Daños de trusses, Modificaciones en la Obra y Errores de Instalación.

- Do not cut, alter, or drill any structural member of a truss unless specifically permitted by the Truss Design Drawing.

No corte, altere o perforo ningún miembro estructural de los trusses, a menos que esté específicamente permitido en el dibujo del diseño del truss.

- Trusses that have been overloaded during construction or altered without the Truss Manufacturer's prior approval may render the Truss Manufacturer's limited warranty null and void.

Trusses que se han sobrecargado durante la construcción o han sido alterados sin una autorización previa del Fabricante de Trusses, pueden reducir o eliminar la garantía del Fabricante de Trusses.

NOTE: The Truss Manufacturer and Truss Designer must rely on the fact that the Contractor and crane operator (if applicable) are capable to undertake the work they have agreed to do on a particular project. The Contractor should seek any required approvals regarding construction practices from a competent party. The methods and procedures outlined are intended to ensure that the overall construction techniques employed will put floor and roof trusses into place SAFELY. These recommendations for handling, installing and bracing wood trusses are based upon the collective experience of leading technical personnel in the wood truss industry, but must, due to the nature of responsibilities involved, be presented only as a GUIDE for use by a qualified Building Designer or Erection/Installation Contractor. It is not intended that these recommendations be interpreted as superior to any design specification (provided by either an Architect, Engineer, the Building Designer, the Erection/Installation Contractor or otherwise) for handling, installing and bracing wood trusses and it does not preclude the use of other equivalent methods for bracing and providing stability for the walls and columns as may be determined by the Truss Erection/Installation Contractor. Thus, the Wood Truss Council of America and the Truss Plate Institute expressly disclaim any responsibility for damages arising from the use, application, or reliance on the recommendations and information contained herein.

WOOD TRUSS COUNCIL OF AMERICA
One WTCA Center • 6300 Enterprise Lane • Madison, WI 53719
608/274-4849 • www.woodtruss.com

TRUSS PLATE INSTITUTE
583 D'Ondroff Drive • Madison, WI 53719
608/833-5900 • www.tpin.org

General detail for repair of broken chords and damaged or missing chord splices that meet the following conditions:

- Chord size must be maximum 2x6 and southern pine species.
- Scab must be the same size and grade as broken chord.
- Truss must be single ply.
- No more than two broken or cracked chords per truss.
- Perimeter of break area must be minimum of X distance from any heel or peak, and minimum of 6" away from any interior joint locations.

C = Maximum length of damaged area not to exceed 12".

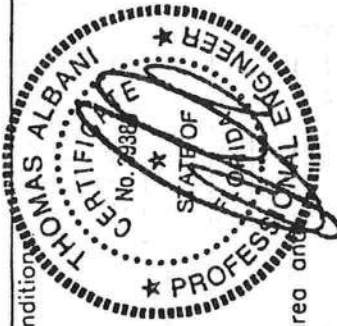
S = Overall length of scab member (shaded) must be equal or greater than $2(X)+C$.

X = Minimum length of scab member at each end of break area.

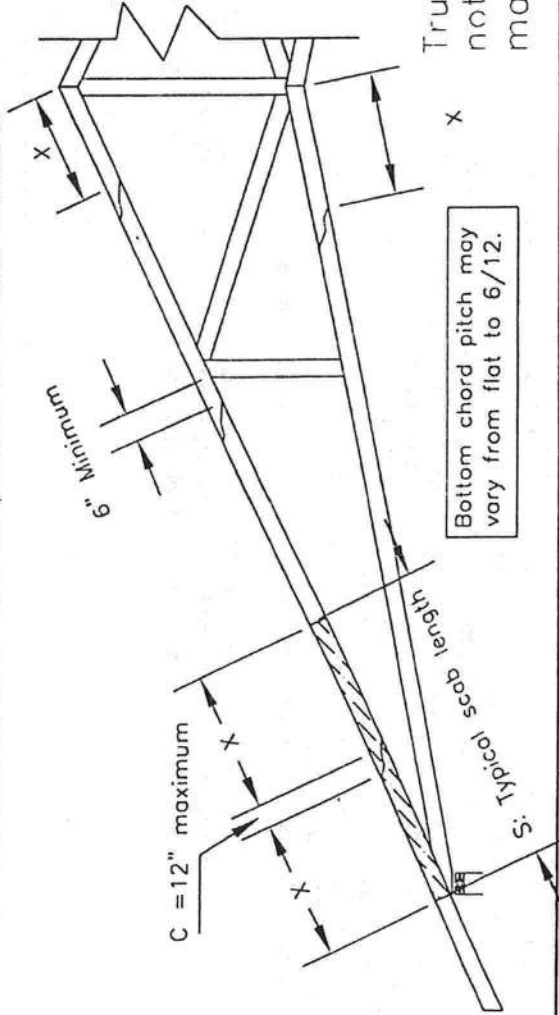
Refer to following table for minimum length of scab, and minimum number of nails at each end of break area and maximum axial force of broken member.

For all lumber, plates, web bracing, etc. refer to original drawing sealed by Robbins Engineering Inc.

- 1) Apply all nails so as to avoid damaging of lumber and loosening of plates at joints.
- 2) Use 2 rows of 10d common nails 3" on center each row and staggered into 2x4 members.
- Use 3 rows of 10d common nails 3" on center each row and staggered into 2x6 members.
- Use 6" spacing if scabs applied to each face of truss.
- Minimum end distance of 2" must be provided for all scab members.



TOTAL NUMBER OF NAILS ON EACH SIDE OF BREAK		.X (INCHES)	MAXIMUM FORCE (LBS.) AT 33% LOAD DURATION	
2X4 SYP	2X6 SYP		2X4 SYP	2X6 SYP
13	20	24"	1924	2886
17	26	30"	2501	3752
21	32	36"	3079	4618
25	38	42"	3656	5484
29	44	48"	4233	6349



Truss manufacturer must be notified if field conditions do not match this general detail.

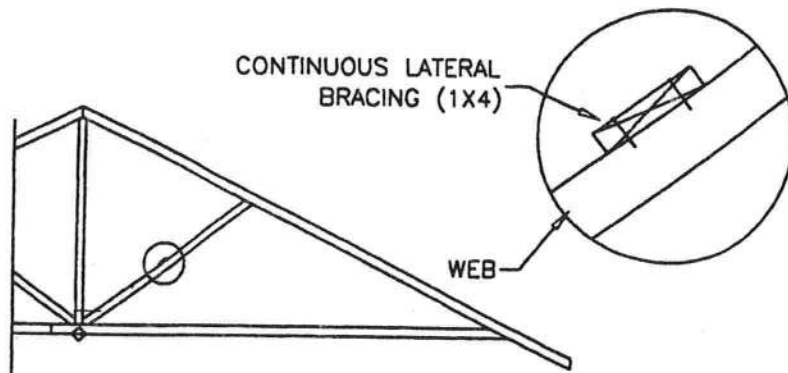
ROBBINS
Engineering Inc.
P.O. Box 280055, Tampa, FL 33682

ROBBINS LOCK connector plates (20 gp. galv. steel-ASTM A53 55 Grade 40) shall be applied on both faces of truss at each joint. Center the plates, unless shown otherwise by circles (c) or dimensions. Unless otherwise indicated by a "V", all slots in plates run parallel with the chords or horizontally at the ends and for the loose ends or ends in pole contact area. Splice ends shall be "Overlapped" and secured with 10d common nails. The indicated otherwise. Cutting and fabrications shall be performed on equipment which produces snug-fitting joints and plates. This design was prepared in accordance with National Design Specifications for Stress-Grade Lumber (NDS) and the National Design Specification for Light and Medium Duty Connected Wood Trusses (NDS-C) (199). See HUD Design Criteria for Trussed Roofs.

FURNISH A COPY OF THIS DESIGN TO ERECTION CONTRACTOR. IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER AND TRUSS FAB. TO REVIEW THIS DRAWING & VERIFY THAT DATA INCLUDING DIM. & LOADS CONFORM TO ARCH. PLAN/SPECS & FAB. TRUSS LAYOUTS.

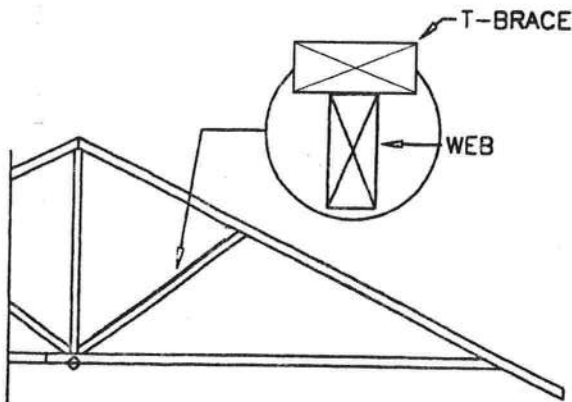
Designed By: MC
Checked By: TAA
Rev. Date: 12/01/03
Dwg. No: GD-62-33

PERMANENT WEB BRACING



CONTINUOUS LATERAL BRACING

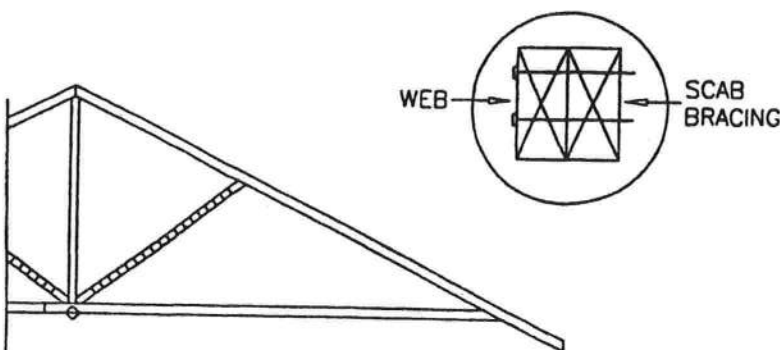
1x4 #3 HEM-FIR OR BETTER CONTINUOUS LATERAL BRACING TO BE EQUALLY SPACED. ATTACH WITH (2) 8d NAILS. BRACING MATERIAL TO BE SUPPLIED AND ATTACHED AT BOTH ENDS TO A SUITABLE SUPPORT BY ERECTION CONTRACTOR.



T-BRACE

THESE DETAILS APPLY TO 1.5" WIDE WOOD TRUSSES.

- USE A 2x4 T-BRACE IF THE TRUSS DESIGN SPECIFIES ONE LATERAL BRACE (MID POINT OF WEB).
- USE A 2x6 T-BRACE IF THE TRUSS DESIGN SPECIFIES TWO LATERAL BRACES (AT THE THIRD POINTS OF THE WEB).
- USE A CONTINUOUS PIECE FOR THE T-BRACE, OF THE SAME GRADE AS THE WEB AND COVERING AT LEAST 90% OF THE WEB LENGTH.
- CENTER THE T-BRACE ON THE WEB AND FASTEN WITH 10d COMMON NAILS SPACED 4" ON CENTER.



SCAB BRACE

SCAB BRACE SAME SIZE, GRADE, AND LENGTH AS WEB MEMBER. ATTACH WITH 10d NAILS @ 4" O.C. BRACING MATERIAL TO BE SUPPLIED BY ERECTION CONTRACTOR.

