

DATE 11/20/2006

Columbia County Building Permit

PERMIT

This Permit Expires One Year From the Date of Issue

000025241

APPLICANT MIKE TODD PHONE 755-4387  
ADDRESS 129 NE COLBURN AVE LAKE CITY FL 32055  
OWNER JIMMY & SUSAN SPARKS PHONE  
ADDRESS 146 SW WISE DRIVE LAKE CITY FL 32024  
CONTRACTOR MIKE TODD PHONE 755-4387  
LOCATION OF PROPERTY 47S, TR ON 242, TR ON WISE DR, 2ND LOT ON LEFT

TYPE DEVELOPMENT SFD,UTILITY ESTIMATED COST OF CONSTRUCTION 84700.00  
HEATED FLOOR AREA 1694.00 TOTAL AREA 2426.00 HEIGHT 1 STORIES 1  
FOUNDATION CONC WALLS FRAMED ROOF PITCH 6/12 FLOOR SLAB  
LAND USE & ZONING RSF-2 MAX. HEIGHT 16  
Minimum Set Back Requirments: STREET-FRONT 25.00 REAR 15.00 SIDE 10.00  
NO. EX.D.U. 0 FLOOD ZONE X PP DEVELOPMENT PERMIT NO.

PARCEL ID 23-4S-16-03113-102 SUBDIVISION WISE ESTATES  
LOT 2 BLOCK A PHASE UNIT TOTAL ACRES

000001260 CGC006209  
Culvert Permit No. Culvert Waiver Contractor's License Number Applicant/Owner/Contractor  
CULVERT 06-0972-N BK JH Y  
Driveway Connection Septic Tank Number LU & Zoning checked by Approved for Issuance New Resident

COMMENTS: PLAT REQUIRES MFE TO BE AT 99.0', ELEVATION LETTER REQUIRED

Check # or Cash 12066

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power date/app. by Foundation date/app. by Monolithic date/app. by  
Under slab rough-in plumbing date/app. by Slab date/app. by Sheathing/Nailing date/app. by  
Framing date/app. by Rough-in plumbing above slab and below wood floor date/app. by  
Electrical rough-in date/app. by Heat & Air Duct date/app. by Peri. beam (Lintel) date/app. by  
Permanent power date/app. by C.O. Final date/app. by Culvert date/app. by  
M/H tie downs, blocking, electricity and plumbing date/app. by Pool date/app. by  
Reconnection date/app. by Pump pole date/app. by Utility Pole date/app. by  
M/H Pole date/app. by Travel Trailer date/app. by Re-roof date/app. by

BUILDING PERMIT FEE \$ 425.00 CERTIFICATION FEE \$ 12.13 SURCHARGE FEE \$ 12.13  
MISC. FEES \$ 0.00 ZONING CERT. FEE \$ 50.00 FIRE FEE \$ 0.00 WASTE FEE \$  
FLOOD DEVELOPMENT FEE \$ FLOOD ZONE FEE \$ 25.00 CULVERT FEE \$ 25.00 TOTAL FEE 549.26  
INSPECTORS OFFICE CLERKS OFFICE

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

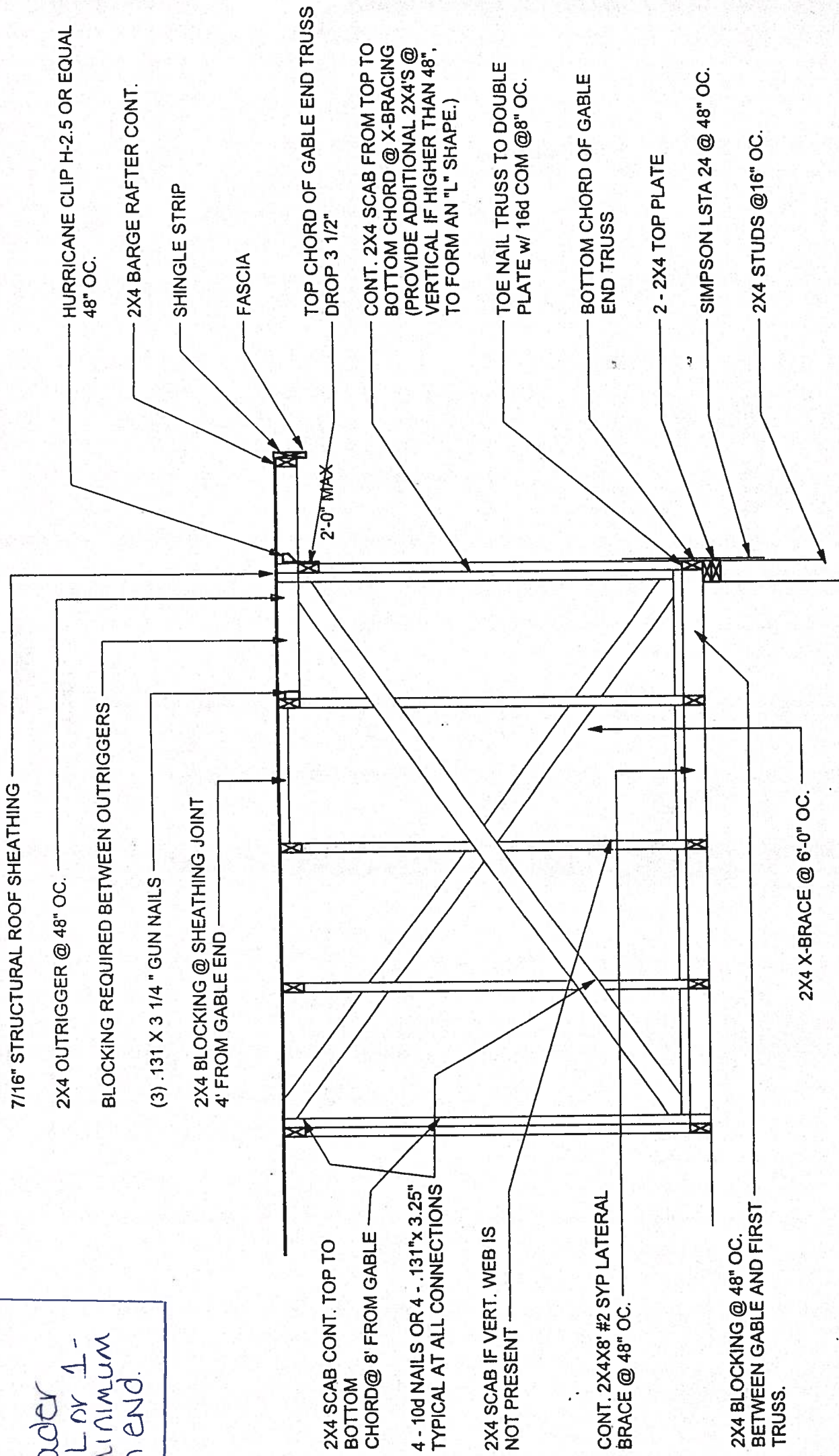
"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Note:  
 Garage Door Header  
 2-1 3/4" x 12" 2.0E LVL or 1-  
 3 1/2" x 12" GLU-LAM, MINIMUM  
 BEARING 4 1/2" each end.



## TYPICAL GABLE END ( X-BRACING )

ALL MEMBERS SHALL BE SYP

**Columbia County Building Permit Application**

CK# 12066  
Called Left  
Message LH  
11-20-06

**For Office Use Only** Application # 06011-24 Date Received 11/9 By JIN Permit # 1260/25241  
 Application Approved by - Zoning Official BLK Date 11.11.06 Plans Examiner OK 5/11 Date 11-25-06  
 Flood Zone aprilat Development Permit N/A Zoning RSF-2 Land Use Plan Map Category Res. La Dev.  
 Comments signed the plan, NOC Plat Require MFE to be at 99.0' Elevation Letter Required

Applicants Name Mike Todd Construction Inc Phone 386 755 4387  
 Address 129 NE Colburn Avenue Lake City FL 32055  
 Owners Name Jimmy & Susan Sparks Phone \_\_\_\_\_  
 911 Address 14th SW wise Drive Lake City FL 32024  
 Contractors Name Mike Todd Construction Inc Phone 386 755 4387  
 Address 129 NE Colburn Avenue Lake City FL 32055  
 Fee Simple Owner Name & Address N/A  
 Bonding Co. Name & Address N/A  
 Architect/Engineer Name & Address \_\_\_\_\_  
 Mortgage Lenders Name & Address NONE  
 Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progressive Energy  
 Property ID Number 23-4S-14-03113-102 Estimated Cost of Construction \$149,000  
 Subdivision Name Wise Estates Lot 2 Block A Unit \_\_\_\_\_ Phase \_\_\_\_\_  
 Driving Directions Hwy 47 South to CR 242 west - TO Wise Estates - Lot 2 on Right

Type of Construction Single Family New Number of Existing Dwellings on Property 0  
 Total Acreage .52 Lot Size .52 Do you need a Culvert Permit or Culvert Waiver or Have an Existing Drive  
 Actual Distance of Structure from Property Lines - Front 30' Side 55' Side 26' Rear 70'  
 Total Building Height 110' Number of Stories 1.28 Heated Floor Area 1694 Roof Pitch 6/12  
TOTAL 2426

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

**OWNERS AFFIDAVIT:** I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

**WARNING TO OWNER:** YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

[Signature]  
 Owner Builder or Agent (Including Contractor)

[Signature]  
 Contractor Signature  
 Contractors License Number CGC006209

STATE OF FLORIDA  
 COUNTY OF COLUMBIA



TERESA N. PIERCE  
 MY COMMISSION #007398-0153  
 EXPIRES: Sept. 21, 2009  
 Florida Notary Services, Inc.

Sworn to (or affirmed) and subscribed before me  
 this 8th day of November 2006

[Signature]  
 Notary Signature

Personally known ☒ or Produced Identification



LOT 2 BLOCK A  
WISE ESTATES  
COLUMBIA COUNTY

SEPTIC

WELL



192.24'

56'

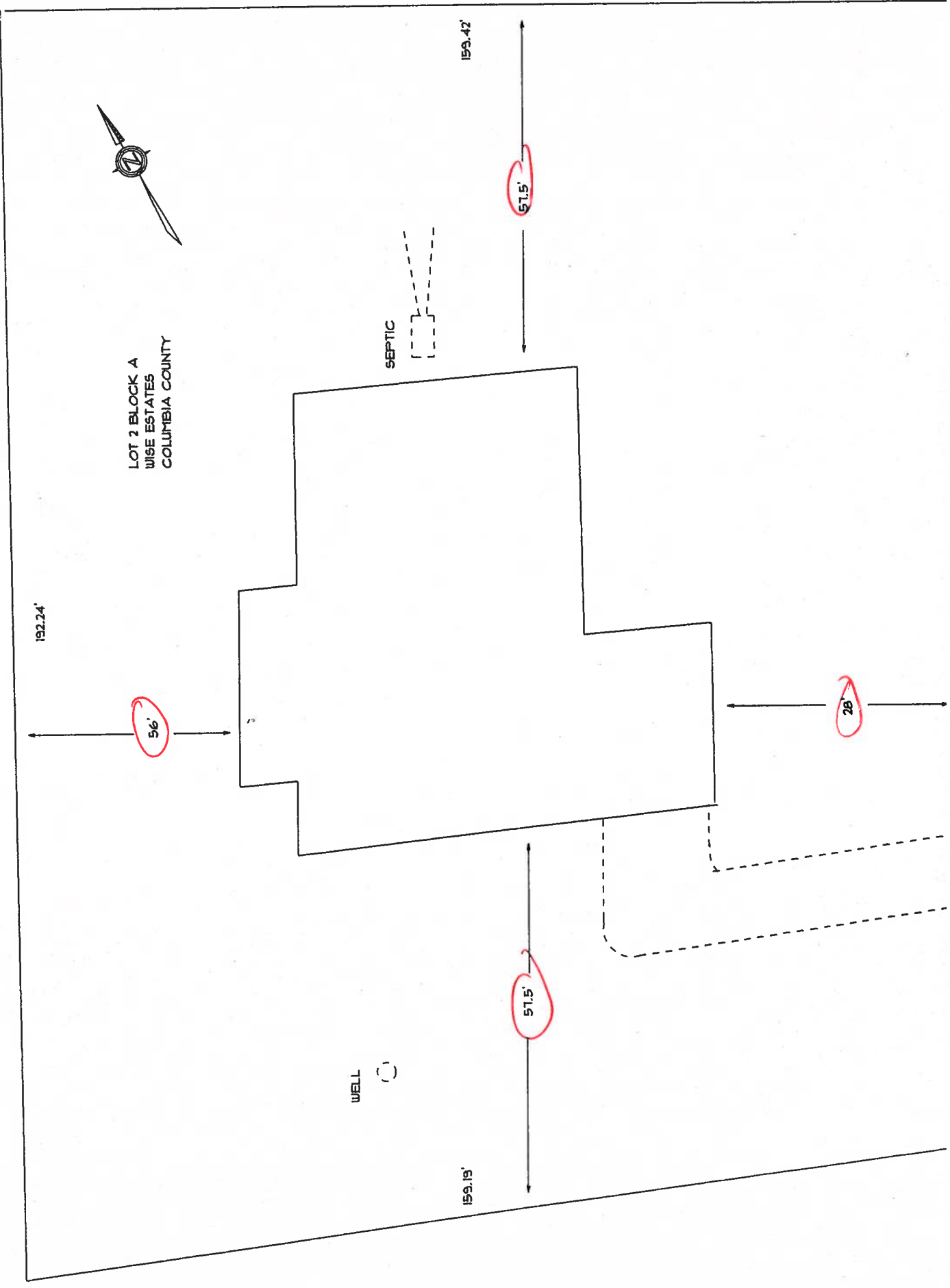
159.42'

51.5'

159.19'

51.5'

28'

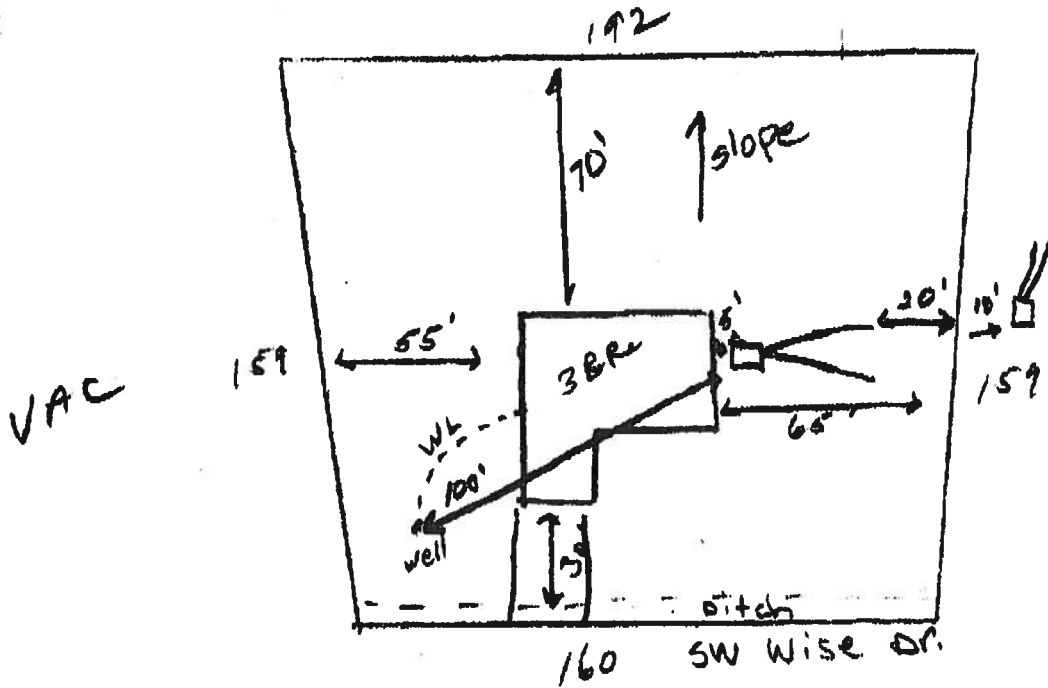


**STATE OF FLORIDA  
DEPARTMENT OF HEALTH  
APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT**

Permit Application Number \_\_\_\_\_

-----PART II - SITEPLAN-----

1 inch = 50 feet.



Notes: \_\_\_\_\_

its Plan submitted by: \_\_\_\_\_

**MASTER CONTRACTOR**

I am Approved \_\_\_\_\_ Not Approved \_\_\_\_\_

Date \_\_\_\_\_

y \_\_\_\_\_ County Health Department

**ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT**



# A & B CONSTRUCTION

## Site Evaluation Checklist

Customer Name: Mike Todd Wise Est

Directions:

417 South TR on 242  
TR in to Wise Est 2nd lot on Left

NEW: X REPAIR:        MOD:        EXIST:       

Soil Evaluation 1: 4/2 0-8 Soil Evaluation 2:        WSWT: 48 MOTTELS: NA

5/2 8-16       

Apparent WSWT: >72

2/6 16-48       

BM: Nail in ROAD  
3" Below

7/3 48-59       

8/2 59-72       

MEASUREMENTS FROM AT LEAST TWO PROPERTY LINES         
SLOPE        WELLS        SEPTICS        DRIVE        BM         
SITE HOLES        WATER LINE        STANDING WATER         
FLAGS        NORTH        DRAINAGE FEATURES

A & B CONSTRUCTION SITE PLAN ATTACHMENT

- |  |   |
|--|---|
| 1. Is there any slope to the property?   | <input checked="" type="radio"/> Yes <input type="radio"/> No |
| 2. Are there any public wells within 200' of the property lines?   | Yes <input checked="" type="radio"/> No                       |
| 3. Are there any private wells within 75' of the property lines?   | Yes <input checked="" type="radio"/> No                       |
| 4. Are there any lakes, streams, canals or standing bodies of water on or within 75' of your property lines?             | Yes <input checked="" type="radio"/> No                       |
| 5. Are there any drainage features (i.e. ditches, swales, retention areas, etc.) on or within 75' of the property lines? | Yes <input checked="" type="radio"/> No                       |
| 6. Are there any septic systems on adjacent properties within 75' of the property lines?                                 | Yes <input checked="" type="radio"/> No                       |
| 7. Are there any recorded easements on the property?   | Yes <input checked="" type="radio"/> No                       |
| 8. Is there a swimming pool on the property?   | Yes <input checked="" type="radio"/> No                       |
| 9. Are there any non-potable water wells on or within 50' of the property lines?   | Yes <input checked="" type="radio"/> No                       |
| 10. Are there any other structures on the property?  | Yes <input checked="" type="radio"/> No                       |
| 11. Are there any paved or obstructed areas on the property?   | Yes <input checked="" type="radio"/> No                       |
| 12. Is the distance from the well and the building foundation equal to or greater than 25 feet?                          | <input checked="" type="radio"/> Yes <input type="radio"/> No |

IF YOU ANSWERED YES TO ANY OF THESE QUESTIONS, PLEASE SHOW LOCATION ON THE SITE PLAN

PLEASE USE THIS CHECKLIST WHILE COMPLETING SITE PLAN TO BE SURE THAT ALL REQUIREMENTS AND DISTANCES ARE SHOWN

- |     |                                     |   |
|-----|-------------------------------------|---|
| 1.  | <input checked="" type="checkbox"/> | Property dimensions.  |
| 2.  | <input checked="" type="checkbox"/> | Distance from front, back and side property lines to the residence. |
| 3.  | <input checked="" type="checkbox"/> | Building dimensions.  |
| 4.  | <input checked="" type="checkbox"/> | Location of proposed septic system and drain field.                 |
| 5.  | <input checked="" type="checkbox"/> | Distance from well to septic.                                       |
| 6.  | <input checked="" type="checkbox"/> | Distance from septic to nearest property line.                      |
| 7.  | <input checked="" type="checkbox"/> | Water lines must be shown.  |
| 8.  | <input checked="" type="checkbox"/> | Distance from residence to septic.                                  |
| 9.  | <input checked="" type="checkbox"/> | Show driveway.  |
| 10. | <input checked="" type="checkbox"/> | Septic tank and drain field location staked and marked.             |

  
Owner / Agent Signature / Date

\_\_\_\_\_  
Clerical Signature / Date

NOV 01 2008



PAYMENT FORM: Check 13859

PAYMENT DATE: November 2, 2006

RECEIVED FROM: SPARKS, JAMES C./L-2/B-A/WISE AMOUNT PAID: \$ 215.00

PAYING ON: 12-SC-08733 06-0972-N

PROPERTY LOCATION:

Lot: 2 Block: A Wise Estates  
Property ID 23-4S-16-03113-102

<u>EXPLANATION or DESCRIPTION:</u>	<u>FEE</u>
Application for permitting of an onsite sewage treatment and disposal system, which includes application and plan review	\$ 50.00
Site evaluation for a new system which includes an evaluation of criteria specified in rule 64E-6.004(3)	\$ 0.00
Site evaluation for a system repair which includes an evaluation of criteria specified in rule 64E-6.015(1)	\$ 0.00
Site re-evaluation, new or repair	\$ 0.00
Permit for new system, including standard subsurface, filled or mounded system	\$ 55.00
New system installation inspection	\$ 80.00
Research fee to be collected in addition to and concurrent with the permit for a new system installation fee	\$ 5.00
Repair permit issuance, which includes inspection	\$ 0.00
Inspection of a system previously in use	\$ 0.00
Reinspection fee per visit for site inspections after system construction approval or installation reinspection for non-compliant system per each visit	\$ 0.00
System abandonment permit, includes permit issuance and inspection	\$ 0.00
Variance application for a single-family residence per each lot or building site	\$ 0.00
Variance application for a multi-family residence or commercial building per each building site	\$ 0.00

RECEIVED BY: SJM

AUDIT CONTROL NO. 5061102021





STATE OF FLORIDA  
DEPARTMENT OF HEALTH  
ON-SITE SEWAGE TREATMENT AND DISPOSAL SYSTEM  
CONSTRUCTION PERMIT

PERMIT NO. \_\_\_\_\_  
DATE PAID: \_\_\_\_\_  
FEE PAID: \_\_\_\_\_  
RECEIPT #: \_\_\_\_\_

CONSTRUCTION PERMIT FOR:

☒ New System ☐ Existing System ☐ Holding Tank ☐ Innovative  
☐ Repair ☐ Abandonment ☐ Temporary ☐

APPLICANT: SPARKS, JAMES CLAYTON

PROPERTY ADDRESS: \_\_\_\_\_

LOT: 2 BLOCK: A SUBDIVISION: WISE ESTATES

PROPERTY ID #: 23-45-16-03113-102 [SECTION, TOWNSHIP, RANGE, PARCEL NUMBER]  
[OR TAX ID NUMBER]

SYSTEM MUST BE CONSTRUCTED IN ACCORDANCE WITH SPECIFICATIONS AND STANDARDS OF SECTION 381.0065, F.S., AND CHAPTER 64E-6, F.A.C. DEPARTMENT APPROVAL OF SYSTEM DOES NOT GUARANTEE SATISFACTORY PERFORMANCE FOR ANY SPECIFIC PERIOD OF TIME. ANY CHANGE IN MATERIAL FACTS, WHICH SERVED AS A BASIS FOR ISSUANCE OF THIS PERMIT, REQUIRE THE APPLICANT TO MODIFY THE PERMIT APPLICATION. SUCH MODIFICATIONS MAY RESULT IN THIS PERMIT BEING MADE NULL AND VOID. ISSUANCE OF THIS PERMIT DOES NOT EXEMPT THE APPLICANT FROM COMPLIANCE WITH OTHER FEDERAL, STATE, OR LOCAL PERMITTING REQUIRED FOR DEVELOPMENT OF THIS PROPERTY.

SYSTEM DESIGN AND SPECIFICATIONS

T 900 GALLONS / GPD SEPTIC TANK/AEROBIC UNIT CAPACITY MULTI-CHAMBERED/IN-SERIES ☐  
A ☐ GALLONS / GPD CAPACITY MULTI-CHAMBERED/IN-SERIES ☐  
N ☐ GALLONS GREASE INTERCEPTOR CAPACITY [MAXIMUM CAPACITY SINGLE TANK: 1250 GALLONS]  
K ☐ GALLONS DOSING TANK CAPACITY ☐ GALLONS ☐ DOSES PER 24 HRS # PUMPS ☐  
D 334 SQUARE FEET PRIMARY DRAINFIELD SYSTEM  
R ☐ SQUARE FEET SYSTEM  
A TYPE SYSTEM: ☒ STANDARD ☐ FILLED ☐ MOUND ☐  
I CONFIGURATION: ☒ TRENCH ☐ BED ☐  
N  
F LOCATION OF BENCHMARK: nail in road  
I ELEVATION OF PROPOSED SYSTEM SITE 3 [INCHES/FT] [ABOVE/BELOW] BENCHMARK/REFERENCE POINT  
E BOTTOM OF DRAINFIELD TO BE 3 [INCHES/FT] [ABOVE/BELOW] BENCHMARK/REFERENCE POINT  
L  
D FILL REQUIRED: 4 INCHES EXCAVATION REQUIRED: NA INCHES

O  
T  
R  
E  
R

SPECIFICATIONS BY: Rock 7 TITLE: MASTER CONTRACTOR

APPROVED BY: \_\_\_\_\_ TITLE: \_\_\_\_\_  
DATE ISSUED: \_\_\_\_\_

ON 4016, 10/97 (Previous Editions May Be Used)

EXPIRATION DATE: \_\_\_\_\_  
Page 3



STATE OF FLORIDA  
DEPARTMENT OF HEALTH  
ON-SITE SEWAGE TREATMENT AND DISPOSAL SYSTEM  
SITE EVALUATION AND SYSTEM SPECIFICATIONS

PERMIT #. \_\_\_\_\_

APPLICANT: SPARKS, JAMES CLAYTON

AGENT: Rocky Ford, A & B Const

LOT: 2 BLOCK: A SUBDIVISION: WISE ESTATES

PROPERTY ID #: 23-45-16-03113-102 [Section/Township/Parcel No. or Tax ID Number]

TO BE COMPLETED BY ENGINEER, HEALTH DEPARTMENT EMPLOYEE, OR OTHER QUALIFIED PERSON. ENGINEERS MUST PROVIDE REGISTRATION NUMBER AND SIGN AND SEAL EACH PAGE OF SUBMITTAL. COMPLETE ALL ITEMS.

PROPERTY SIZE CONFORMS TO SITE PLAN: ☒ YES ☐ NO NET USABLE AREA AVAILABLE: .5 ACRES  
TOTAL ESTIMATED SEWAGE FLOW: 300 GALLONS PER DAY [RESIDENCES-TABLE 1/OTHER-TABLE 2]  
AUTHORIZED SEWAGE FLOW: 7500 GALLONS PER DAY [1500 GPD/ACRE OR 2500 GPD/ACRE]  
UNOBSTRUCTED AREA AVAILABLE: > 2000 SQFT UNOBSTRUCTED AREA REQUIRED: 668 SQFT

BENCHMARK/REFERENCE POINT LOCATION: Nail in Road  
ELEVATION OF PROPOSED SYSTEM SITE IS 3 [INCHES/FT] [ABOVE/BELOW] BENCHMARK/REFERENCE POINT

THE MINIMUM SETBACK WHICH CAN BE MAINTAINED FROM THE PROPOSED SYSTEM TO THE FOLLOWING FEATURES  
SURFACE WATER: NA FT DITCHES/SWALES: NA FT NORMALLY WET? ☐ YES ☒ NO  
WELLS: PUBLIC: NA FT LIMITED USE: NA FT PRIVATE: 100 FT NON-POTABLE: NA FT  
BUILDING FOUNDATIONS: 10 FT PROPERTY LINES: 50 FT POTABLE WATER LINES: 30 FT

SITE SUBJECT TO FREQUENT FLOODING: ☐ YES ☒ NO 10 YEAR FLOODING? ☐ YES ☒ NO  
10 YEAR FLOOD ELEVATION FOR SITE: NA FT MSL/NGVD SITE ELEVATION: NA FT MSL/NGVD

SOIL PROFILE INFORMATION SITE 1

MUNSELL #/COLOR	TEXTURE	DEPTH
10 YR <u>4/2</u>	<u>FS</u>	<u>0 TO 8</u>
<u>5/2</u>	<u>FS</u>	<u>8 TO 16</u>
<u>7/6</u>	<u>FS</u>	<u>16 TO 48</u>
<u>7/3</u>	<u>FS</u>	<u>48 TO 54</u>
<u>8/2</u>	<u>FS</u>	<u>54 TO 72</u>
		<u>TO</u>
		<u>TO</u>
		<u>TO</u>
10 YR _____		<u>TO</u>
USDA SOIL SERIES: <u>Alpa Like</u>		

SOIL PROFILE INFORMATION SITE 2

MUNSELL #/COLOR	TEXTURE	DEPTH
10 YR <u>4/2</u>	<u>FS</u>	<u>0 TO 6</u>
<u>5/2</u>	<u>FS</u>	<u>6 TO 19</u>
<u>7/6</u>	<u>FS</u>	<u>19 TO 47</u>
<u>7/3</u>	<u>FS</u>	<u>47 TO 54</u>
<u>8/2</u>	<u>FS</u>	<u>54 TO 72</u>
		<u>TO</u>
		<u>TO</u>
		<u>TO</u>
10 YR _____		<u>TO</u>
USDA SOIL SERIES: <u>Alpa Like</u>		

OBSERVED WATER TABLE: 772 INCHES [ABOVE / BELOW] EXISTING GRADE. TYPE: [PERCHED / APPARENT]  
ESTIMATED WET SEASON WATER TABLE ELEVATION: 48 INCHES [ABOVE / BELOW] EXISTING GRADE  
HIGH WATER TABLE VEGETATION: ☐ YES ☒ NO MOTTLING: ☒ YES ☐ NO DEPTH: NA INCHES

SOIL TEXTURE/LOADING RATE FOR SYSTEM SIZING: FS .90 DEPTH OF EXCAVATION: NA INCHES  
DRAINFIELD CONFIGURATION: ☒ TRENCH ☐ BED ☐ OTHER (SPECIFY) \_\_\_\_\_  
REMARKS/ADDITIONAL CRITERIA: \_\_\_\_\_

SITE EVALUATED BY Rocky Ford

Master Contractor DATE: 11/1/2006

DH 4015, 10/96 (Replaces HRS-H Form 4015 [page 3] which may be used)



STATE OF FLORIDA  
DEPARTMENT OF HEALTH  
ONSITE SEWAGE TREATMENT AND DISPOSAL  
SYSTEM  
APPLICATION FOR CONSTRUCTION PERMIT

PERMIT NO. \_\_\_\_\_  
DATE PAID: \_\_\_\_\_  
FEE PAID: \_\_\_\_\_  
RECEIPT #: \_\_\_\_\_

APPLICATION FOR:

☒ New System    ☐ Existing System    ☐ Holding Tank    ☐ Innovative  
☐ Repair    ☐ Abandonment    ☐ Temporary    ☐ \_\_\_\_\_

APPLICANT: SPARKS, JAMES CLAYTON

AGENT: ROCKY FORD, A & B CONSTRUCTION

TELEPHONE: 386-497-2311

MAILING ADDRESS: P.O. BOX 39 FT. WHITE, FL, 32038

TO BE COMPLETED BY APPLICANT OR APPLICANT'S AUTHORIZED AGENT. SYSTEMS MUST BE CONSTRUCTED BY A PERSON LICENSED PURSUANT TO 489.105(3)(m) OR 489.552, FLORIDA STATUTES. IT IS THE APPLICANT'S RESPONSIBILITY TO PROVIDE DOCUMENTATION OF THE DATE THE LOT WAS CREATED OR PLATTED (MM/DD/YY) IF REQUESTING CONSIDERATION OF STATUTORY GRANDFATHER PROVISIONS.

PROPERTY INFORMATION

LOT: 2 BLOCK: A SUB: WISE ESTATES PLATTED: \_\_\_\_\_

PROPERTY ID #: 23-48-16-03113-102 ZONING: \_\_\_\_\_ I/M OR EQUIVALENT: ☐ Y ☒ N

PROPERTY SIZE: .5 ACRES WATER SUPPLY: ☒ PRIVATE PUBLIC ☐ <=2000GPD ☐ >2000GPD

IS SEWER AVAILABLE AS PER 381.0065, F8? ☐ Y ☒ N DISTANCE TO SEWER: NA FT

PROPERTY ADDRESS: \_\_\_\_\_

DIRECTIONS TO PROPERTY: 47 SOUTH TURN RIGHT ON 242 TURN RIGHT IN TO WISE  
ESTATES 2<sup>ND</sup> LOT ON LEFT.

BUILDING INFORMATION

☒ RESIDENTIAL ☐ COMMERCIAL

Unit No.	Type of Establishment	No. of Bedrooms	Building Area Sqft	Commercial/Institutional System Design Table 1, Chapter 64E-6, FAC
1	SF Residential	3	1698	
2				
3				

☐ Floor/Equipment Drains ☒ Other (Specify) \_\_\_\_\_

SIGNATURE: Rocky Ford DATE: 11/1/2006

DH 4015, 10/97 (Previous Editions May Be Used)

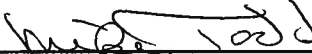
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# Columbia County Building Department Culvert Permit

Culvert Permit No.  
**000001260**

DATE 11/20/2006 PARCEL ID # 23-4S-16-03113-102  
APPLICANT MIKE TODD PHONE 755-4387  
ADDRESS 129 NE COLBURN AVE LAKE CITY FL 32055  
OWNER JIMMY & SUSAN SPARKS PHONE \_\_\_\_\_  
ADDRESS 146 SW WISE DRIVE LAKE CITY FL 32024  
CONTRACTOR MIKE TODD PHONE 755-4387  
LOCATION OF PROPERTY 47S, TR ON 242, TR ON WISE DR, 2ND LOT ON LEFT

SUBDIVISION/LOT/BLOCK/PHASE/UNIT WISE ESTATES 2 A

SIGNATURE 

## INSTALLATION REQUIREMENTS



Culvert size will be 18 inches in diameter with a total length of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4 : 1 slope and poured with a 4 inch thick reinforced concrete slab.

INSTALLATION NOTE: Turnouts will be required as follows:

- a) a majority of the current and existing driveway turnouts are paved, or;
- b) the driveway to be served will be paved or formed with concrete.

Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concreted turnouts.



Culvert installation shall conform to the approved site plan standards.



Department of Transportation Permit installation approved standards.



Other \_\_\_\_\_

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED  
DURING THE INSTALATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21  
Lake City, FL 32055  
Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00



# Columbia County Property Appraiser

DB Last Updated: 10/4/2006

Parcel: 23-4S-16-03113-102

Tax Record

Property Card

Interactive GIS Map

Print

## 2006 Proposed Values

### Owner & Property Info

<b>Owner's Name</b>	SPARKS JAMES CLAYTON JR &		
<b>Site Address</b>	WISE		
<b>Mailing Address</b>	SUSAN DIANNE SPARKS 394 SW ROSE CREEK DR LAKE CITY, FL 32024		
<b>Use Desc. (code)</b>	VACANT (000000)		
<b>Neighborhood</b>	24416.00	<b>Tax District</b>	2
<b>UD Codes</b>	MKTA06	<b>Market Area</b>	06
<b>Total Land Area</b>	0.520 ACRES		
<b>Description</b>	LOT 2 BLOCK A WISE ESTATE S/D WD 1022-1742. WD 1077-1342.		

&lt;&lt; Prev Search Result: 22 of 38 Next &gt;&gt;

### GIS Aerial



### Property & Assessment Values

<b>Mkt Land Value</b>	cnt: (1)	\$25,500.00
<b>Ag Land Value</b>	cnt: (0)	\$0.00
<b>Building Value</b>	cnt: (0)	\$0.00
<b>XFOB Value</b>	cnt: (0)	\$0.00
<b>Total Appraised Value</b>		\$25,500.00

<b>Just Value</b>	\$25,500.00
<b>Class Value</b>	\$0.00
<b>Assessed Value</b>	\$25,500.00
<b>Exempt Value</b>	\$0.00
<b>Total Taxable Value</b>	\$25,500.00

### Sales History

Sale Date	Book/Page	Inst. Type	Sale VImp	Sale Qual	Sale RCode	Sale Price
3/14/2006	1077/1342	WD	V	Q		\$53,100.00
7/23/2004	1022/1742	WD	V	Q		\$22,900.00

### Building Characteristics

Bldg Item	Bldg Desc	Year Blt	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value
NONE						

### Extra Features & Out Buildings

Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
NONE						

### Land Breakdown

Lnd Code	Desc	Units	Adjustments	Eff Rate	Lnd Value
000000	VAC RES (MKT)	1.000 LT - (.520AC)	1.00/1.00/1.00/1.00	\$25,500.00	\$25,500.00

Columbia County Property Appraiser

DB Last Updated: 10/4/2006

&lt;&lt; Prev

22 of 38

Next &gt;&gt;

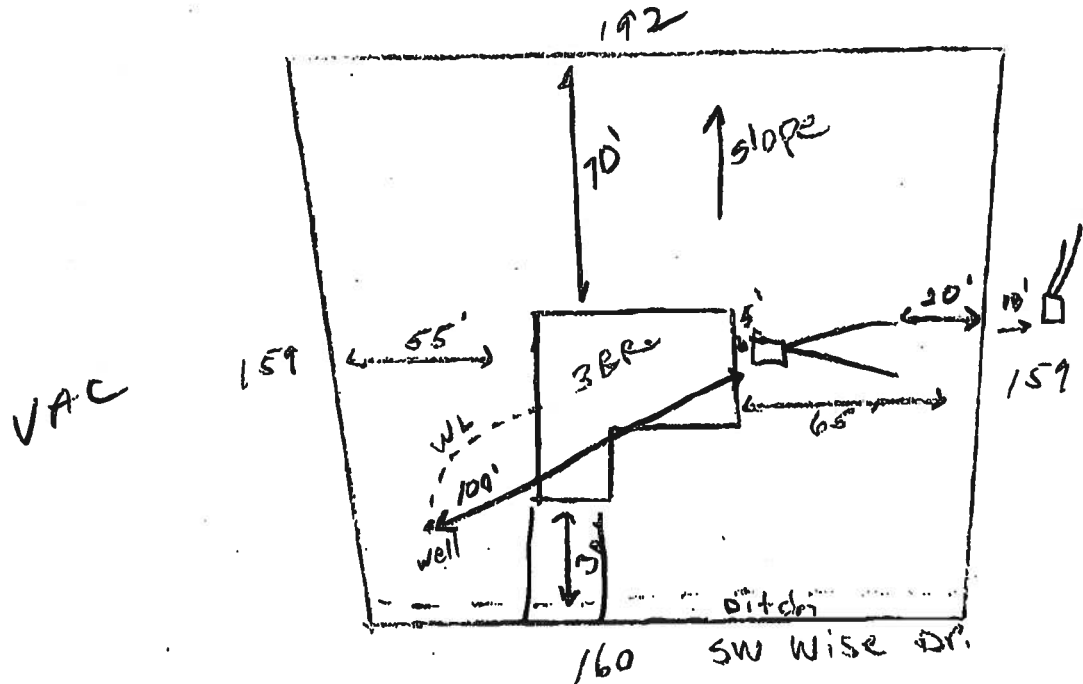
**STATE OF FLORIDA  
DEPARTMENT OF HEALTH  
APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT**

Permit Application Number

06-0972N

----- **PART II - SITEPLAN** -----

1 inch = 50 feet.



Notes:

Its Plan submitted by: Rock 37-1

Plan Approved X

Not Approved \_\_\_\_\_

by Sally Graddy ESII

**MASTER CONTRACTOR**

Date 11-7-06

County Health Department

**Columbia CHD**

**ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT**

H 4016, 10/96 (Replaces HRS-H Form 4016 which may be used)  
Book Number: 6744-002-4016-8)

Page 2 of 4

**RECEIVED**  
11-9-06  
SM



**NOTICE OF COMMENCEMENT**

25241

**To Whom It May Concern:**

The undersigned hereby informs you that improvements will be made to certain real property, and in accordance with Section 713.13, Florida Statutes, the following information is stated in this Notice of Commencement.

**DESCRIPTION OF REAL PROPERTY TO BE IMPROVED:** 23-4S-16-03113-102, Lot 2, Block A, Wise Estates

**GENERAL DESCRIPTION OF IMPROVEMENTS:** New Single Family Construction

**OWNER:** Jimmy & Susan Sparks

**ADDRESS:** 394 SW Rose Creek Drive, Lake City, Florida 32024

**OWNER'S INTEREST IN THE SITE OF THE IMPROVEMENTS (IF OTHER THAN FEE SIMPLE TITLE HOLDER):**

**ADDRESS:** N/A

**CONTRACTOR:** Mike Todd Construction, Inc

**ADDRESS:** 129 NE Colburn Avenue, Lake City, Florida 32055

**SURETY ON ANY PAYMENT BOND:** N/A

Any person within the State of Florida designated by owner upon whom notices or other documents may be served under Part 1 of Chapter 713, Florida Statutes, which service shall constitute service upon owner:

**NAME:**

**ADDRESS:**

In addition to himself/herself, owner designates the following person to receive a copy of the Lienor's notice as provided in Section 713.06(2)(b), Florida Statutes:

**NAME:** Mike Todd Construction, Inc

**ADDRESS:** 129 NE Colburn Avenue, Lake City, Florida 32055

Inst:2006027505 Date:11/21/2006 Time:10:46

J. P. DC, P. Dewitt Cason, Columbia County B:1102 P:1523

Jimmy A. Parker 11.20.06  
OWNER/DATE

Susan D. Sparks 11.20.06  
OWNER/DATE

**This Notice of Commencement shall expire upon completion of contract.**

Sworn to and subscribed before me this 20<sup>th</sup> day of NOV, 2006.

Teresa N. Pierce  
Notary Public

My Commission Expires: Sept 21, 2009



011205  
**COLUMBIA COUNTY**  
**DEPT**  
**OF**  
**ENVIRONMENTAL**  
**PROTECTION**

# OCCUPANCY

**COLUMBIA COUNTY, FLORIDA**

## Department of Building and Zoning Inspection

*This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.*

Parcel Number 23-4S-16-03113-102

Building permit No. 000025241

Use Classification SFD, UTILITY

Fire: 33.48

Permit Holder MIKE TODD

Waste: 100.50

Owner of Building JIMMY & SUSAN SPARKS

Total: 133.98

Location: 146 SW WISE DRIVE

Date: 04/13/2007



*[Signature]*  
Building Inspector

**POST IN A CONSPICUOUS PLACE**  
*(Business Places Only)*

25241



## BRITT SURVEYING

830 West Duval Street • Lake City, FL 32055  
Phone (386) 752-7163 • Fax (386) 752-5573

*Land Surveyors  
and Mappers*

12/07/06

L-17995

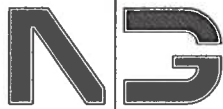
To Whom It May Concern:

C/o: Mike Todd

Re: Lot 2 in Block "A" Wise Estates

The elevation of the foundation wall is found to be 99.20 feet. The minimum finished floor elevation is 99.00 feet according to the plat of record. The centerline of the road is 98.09 feet. The highest adjacent grade is 97.4 feet and the lowest adjacent grade is 97.3 feet. The elevations shown hereon are based on NGVD 29 datum.

L. Scott Britt  
PLS #5757



**NICHOLAS  
PAUL  
GEISLER  
ARCHITECT**  
N.C.A.R.B. Certified

1758 NW Brown Road  
Lake City, FL 32055  
386/755-9021

---

## FLORIDA BUILDING CODE SECTION 1609

---

### COMPLIANCE SUMMARY

**PROJECT:** SPARKS RESIDENCE, COLUMBIA COUNTY, FL (110 WIND ZONE)

#### TYPE OF CONSTRUCTION

ROOF: Gable Construction, Wood Trusses @ 24" O.C., SYP  
WALLS: 2x4 Wood Studs @ 16" O.C.  
FLOOR: 4" Thk. Conc. Slab, w/ 6x6 10/10 W.W.M., dbl. 3' from edge  
FOUNDATION: Continuous Footer/Stemwall  
EDGE STRIP: 3.6 ft.      END ZONE: 7.2 ft.

#### ROOF DECKING

MATERIAL: 1/2" CDX Plywood or 7/16" O.S.B.  
SHEET SIZE: 48"x96" Sheets Placed Perpendicular to Roof Framing  
FASTENERS: 8d Common Nails @ 6" O.C. Ends, 12" O.C. Interior

#### SHEAR WALLS

MATERIAL: 7/16" OSB  
SHEET SIZE: 48"x96" Sheets Placed Vertical  
FASTENERS: 1 1/2" Roofing Nails @ 6" O.C. Edges, 12" O.C. Interior  
DRAGSTRUT: Dbl. Top Plate Nailed w/ 12d Nails @ 16" O.C.  
WALL STUDS: S-P-F Nr. 2 and better, 2x4 Studs @ 16" O.C.

#### HURRICANE UPLIFT CONNECTORS

TRUSS CLIPS: "SIMPSON" H16S  
WALL TENSION: 7/16" OSB w/ 8d Common Nails @ 4" O.C. Edges,  
8" O.C. Interior for all exterior non-shear walls  
HOLD-DOWN CONNECTORS: A307 Bolts, within 6" of corners  
WALL SILL: 1/2" x 10" A.B., w/ 2" washers @ 48" o.c., 6" embedment  
CORNER HOLD-DOWN DEVICE: "SIMPSON" SPH4, Ea. Corner

#### FOOTINGS AND FOUNDATIONS

HOUSE FOOTINGS: 20"x10" Continuous w/ 2 - #5 Rebars  
HOUSE STEMWALL: 8" CMU w/ #5 Rebar Dowels Gd. 40, @ 72" O.C.  
CONCRETE: Fb = 2500 p.s.i. or greater

#### PREPARER'S CERTIFICATION

I hereby certify that the attached Wind Load Design and Analysis calculations are in compliance with the 2004 Florida Building Code, Section 1609, to the best of my knowledge and belief.

  
Nicholas Paul Geisler, Architect    AR0007005

Date: 07 Nov 2006



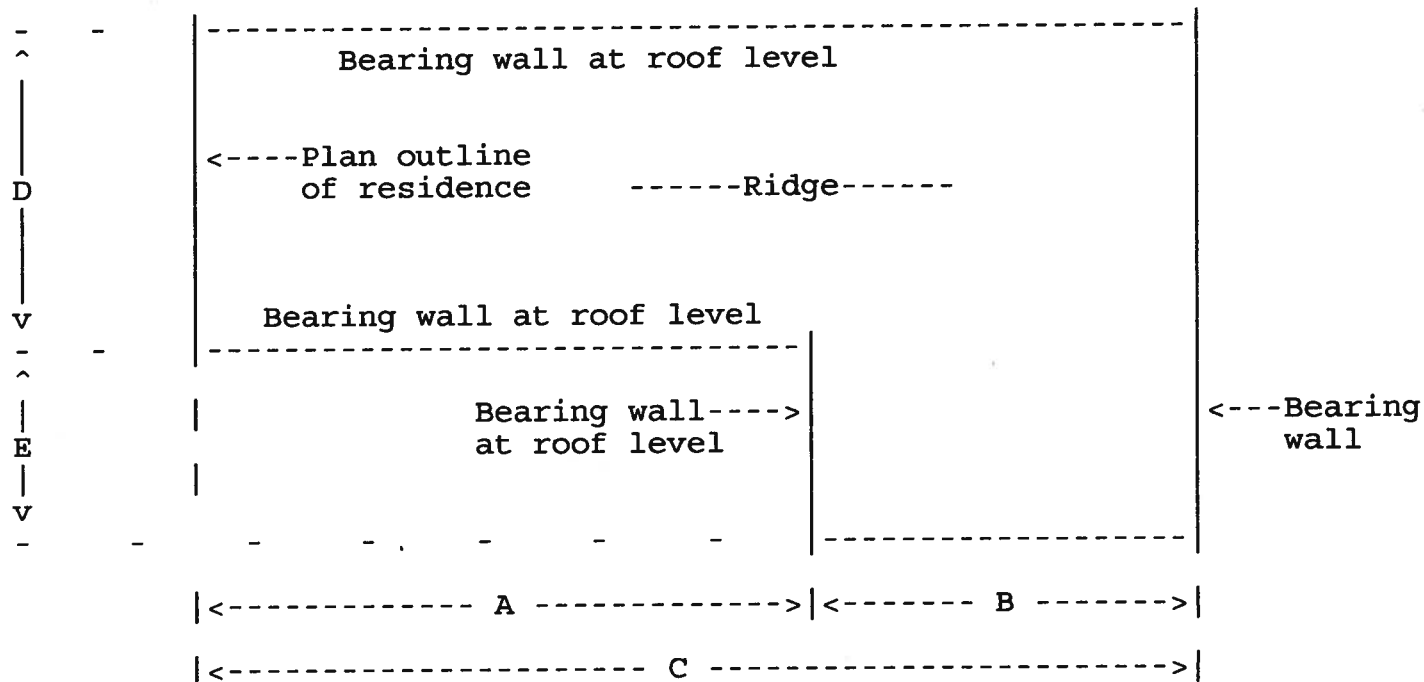
Data entry by: MT      Date: 10 26 06

Project name: SPARKS  
Location : COLUMBIA COUNTY

-----  
R E S I D E N T I A L   W I N D   D E S I G N   A N D   A N A L Y S I S  
A product of EDA Software, Inc.  
Based on the Standard Building Code, 1994 edition  
-----

\*\*\*\* GENERAL INPUT DATA \*\*\*\*

Permanent construction  
L-shaped building



All dimensions are out to out of studs.

Dimension A = 28 feet

Dimension B = 22 feet

Dimension C = 50 feet

Dimension D = 32 feet

Dimension E = 22 feet

Roof overhang in long direction from outer face of stud = 2 feet generally

Roof overhang at short end wall from outer face of stud = 2 feet generally

Height of exterior wall to top of plate on long side = 8 feet constant

Roof cross slope = 6 /12

Wind velocity = 110 mph

\*\*\*\* DEGREE OF ENCLOSURE \*\*\*\*

-----  
Assume that this building is an 'Enclosed building' per Code 1606.2.3.  
-----

\*\*\*\* STRUCTURAL FRAMING INPUT DATA \*\*\*\*

\*\*\* Roof Structural Data \*\*\*

Member number 1

Normal gable type house truss--supported by exterior walls only

Span length out to out of supports = 22 feet

Roof cross slope = 6 /12

Truss spacing = 24 inches

Overhang = 2 feet

Member number 2

Jack truss--hip-ended roof

Span length out to out of supports = 32 feet

Roof cross slope = 6 /12

Truss spacing = 24 inches

Overhang = 2 feet

\*\*\* Wall Structural Data \*\*\*

Spacing of wall studs = 16 inches

Total number of plates = 3

Wall stud number 1 is 8 feet high out to out of plates

COEFFICIENTS AND PRESSURES  
Main Wind Force Resisting Systems

Actual pressure = Velocity pressure x Use factor x Coefficient

Wind velocity is 110 mph

Mean roof height is 12.37268 feet

Velocity pressure is 24.7 psf

Use factor is 1.0

Roof cross slope is 6 on 12, which equals 26.56505 degrees to horizontal

End zone width is 6.4 feet

	Coefficient	Design Pressure (psf)
-----		
End zone		
Windward wall (1E)	.7	17.29
Windward roof (2E)	-1	-24.7
Leeward roof (3E)	-1	-24.7
Leeward wall (4E)	-.95	-23.47
Overhang	-1.5	-37.06
Interior zone		
Windward wall (1)	.4	9.88
Windward roof (2)	-.75	-18.53
Leeward roof (3)	-.75	-18.53
Leeward wall (4)	-.7	-17.3
Overhang	-1.5	-37.06



ROOF LOADING--Roof Number 1 (pounds per square foot)

Roof cross slope = 6 inches per foot

Fiberglass shingles 240 # per square and 1 layer of 15 # felt	= 2.55
No insulation	
7/16 in. roof sheathing	= 1.31
2 in. x 4 in. wood trusses at 24 in. spacing	= 2.215147
-----	
Total roof unit weight on slope	= 6.075148
Cosine of roof cross slope	= .8944272
-----	
Roof unit weight on horizontal	= 6.792222
1 layer of 1/2 in. gypsum board ceiling--plain	= 2
Ceiling insulation R-30	= .5
Air-conditioning ductwork	= 1
Full lighting	= .3
Miscellaneous	= 0
=====	
Total	= 10.59222

Roof Unit Dead Load = 11 psf

Roof dead load supported generally by wall = 180.9755 plf

ROOF LOADING--Roof Number 2 (pounds per square foot)

Roof cross slope = 6 inches per foot

Fiberglass shingles 240 # per square and 1 layer of 15 # felt	= 2.55
No insulation	
7/16 in. roof sheathing	= 1.31
2 in. x 4 in. wood trusses at 24 in. spacing	= 2.215147
-----	
Total roof unit weight on slope	= 6.075148
Cosine of roof cross slope	= .8944272
-----	
Roof unit weight on horizontal	= 6.792222
1 layer of 1/2 in. gypsum board ceiling--plain	= 2
Ceiling insulation R-30	= .5
Air-conditioning ductwork	= 1
Full lighting	= .3
Miscellaneous	= 0
=====	
Total	= 10.59222

Roof Unit Dead Load = 11 psf

Roof dead load supported generally by wall = 180.9755 plf

ROOF LOADING--Roof Number 3 (pounds per square foot)

Roof cross slope = 6 inches per foot

```
-----
Fiberglass shingles 240 # per square and 1 layer of 15 # felt = 2.55
No insulation
7/16 in. roof sheathing = 1.31
2 in. x 4 in. wood trusses at 24 in. spacing = 2.215147
-----
Total roof unit weight on slope = 6.075148
Cosine of roof cross slope = .8944272
-----
Roof unit weight on horizontal = 6.792222
1 layer of 1/2 in. gypsum board ceiling--plain = 2
Ceiling insulation R-30 = .5
Air-conditioning ductwork = 1
Full lighting = .3
Miscellaneous = 0
=====
Total = 10.59222
```

Roof Unit Dead Load = 11 psf

Roof dead load supported generally by wall = 180.9755 plf

ROOF LOADING--Roof Number 4 (pounds per square foot)

Roof cross slope = 6 inches per foot

```
-----
Fiberglass shingles 240 # per square and 1 layer of 15 # felt = 2.55
No insulation
7/16 in. roof sheathing = 1.31
2 in. x 4 in. wood trusses at 24 in. spacing = 2.215147
-----
Total roof unit weight on slope = 6.075148
Cosine of roof cross slope = .8944272
-----
Roof unit weight on horizontal = 6.792222
1 layer of 1/2 in. gypsum board ceiling--plain = 2
Ceiling insulation R-30 = .5
Air-conditioning ductwork = 1
Full lighting = .3
Miscellaneous = 0
=====
Total = 10.59222
```

Roof Unit Dead Load = 11 psf

Roof dead load supported generally by wall = 180.9755 plf

# ROOF MEMBER DEAD LOAD REACTIONS AT BEARINGS

All values are in pounds

Roof member number 1	--Span 22 feet, Slope 6 /12, interior zone----	256
Roof member number 2	--Span 22 feet, Slope 6 /12, end zone-----	256
Roof member number 3	--Span 32 feet, Slope 6 /12, interior zone----	361
Roof member number 4	--Span 32 feet, Slope 6 /12, end zone-----	361

## EXTERIOR WALL LOADING (pounds per linear foot)

Wood frame wall-- 8 ft. out to out plates

3--2 in. x 4 in. plates	= 2.865625
2 in. x 4 in. studs at 16 in. spacing	= 5.462598
R-13 Insulation	= 1.90625
Brick veneer siding	= 373.3333
1/2 in. Gypsum board--Total 1 layer---	= 16
=====	=====
Total	= 399.5678

Exterior Wall Unit Dead Load = 400 plf

# S U M M A R Y   O F   H U R R I C A N E   A N C H O R   A N A L Y S I S

All values of forces are in pounds. Resistances have been increased for wind.  
End zone width = 6.4 feet

Code: C = Compliance

N = Non-compliance

## Simpson hurricane anchors

Member 1 --Gable roof--Span 22 feet, at 24 inches oc--in interior zone:  
Uplift = 789 Dead = 256 Net = 533 Model Special, Resistance = 1205 C  
Model H16S--all nails installed per manufacturers catalog  
Data supplied by operator--not from EDA database

Member 2 --Gable roof--Span 22 feet, at 24 inches oc--in end zone:  
Uplift = 1143 Dead = 256 Net = 887 Model Special, Resistance = 1205 C  
Model H16S--all nails installed per manufacturers catalog  
Data supplied by operator--not from EDA database

Member 3 --Hip roof--Span 32 feet, at 24 inches oc--in interior zone:  
Uplift = 1043 Dead = 361 Net = 682 Model Special, Resistance = 1205 C  
Model H16S--all nails installed per manufacturers catalog  
Data supplied by operator--not from EDA database

Member 4 --Hip roof--Span 32 feet, at 24 inches oc--in end zone:  
Uplift = 1043 Dead = 361 Net = 682 Model Special, Resistance = 1205 C  
Model H16S--all nails installed per manufacturers catalog  
Data supplied by operator--not from EDA database

\*\*\*\* ANALYSIS OF ROOF SHEATHING AS SHEAR DIAPHRAGM TRANSVERSE \*\*\*\*  
Shear analysis applies along supporting shearwalls.

Roof trusses are Southern Pine lumber, spaced at 24 inches  
Sheathing is Oriented Strand Board, 7/16 inch thick  
Sheathing has no intermediate blocking  
Fasteners on panel ends are 8d nails spaced at 4 inches  
Fasteners in panel interior are 8d nails spaced at 8 inches

Total lateral wind force on building = 11300 pounds  
Total force transferred through diaphragm to shearwalls = 5650 pounds  
Total length of shearwalls = 64 feet  
MINIMUM REQUIRED TOTAL SHEARWALL LENGTH = 18 FT.--LOCATE EVENLY THROUGHOUT

Actual diaphragm force per unit length of shearwall = 88 plf  
Allowable diaphragm force per unit length of shearwall = 314 plf

---

\*\*\* Summary of Analysis \*\*\*  
Roof sheathing diaphragm satisfies Code requirements.

\*\*\*\* ANALYSIS OF ROOF SHEATHING AS SHEAR DIAPHRAGM LONGITUDINAL \*\*\*\*  
Shear analysis applies along supporting shearwalls.

Roof trusses are Southern Pine lumber, spaced at 24 inches  
Sheathing is Oriented Strand Board, 7/16 inch thick  
Sheathing has no intermediate blocking  
Fasteners on panel ends are 8d nails spaced at 4 inches  
Fasteners in panel interior are 8d nails spaced at 8 inches

Total lateral wind force on building = 8798 pounds  
Total force transferred through diaphragm to shearwalls = 4399 pounds  
Total length of shearwalls = 100 feet  
MINIMUM REQUIRED TOTAL SHEARWALL LENGTH = 13.7 FT.--LOCATE EVENLY THROUGHOUT

Actual diaphragm force per unit length of shearwall = 43 plf  
Allowable diaphragm force per unit length of shearwall = 314 plf

---

\*\*\* Summary of Analysis \*\*\*  
Roof sheathing diaphragm satisfies Code requirements.

\*\*\*\* ANALYSIS OF ROOF SHEATHING FOR FASTENER WITHDRAWAL \*\*\*\*

Interior zone (area Ri)

Roof trusses are Southern Pine lumber, spaced at 24 inches

Sheathing is 7/16 inch with no intermediate blocking

Size of sheathing is 48 inches by 96 inches

Fasteners along end trusses are 8d nails spaced at 4 inches

Fasteners along int. trusses are 8d nails spaced at 8 inches

Total outward wind force on sheathing = 818 pounds

Total withdrawal resistance of 47 nails = 3569 pounds (increased for wind)

Fastening of roof sheathing satisfies Code requirements.

Edge strip (area Si) width = 3.2 feet

Roof trusses are Southern Pine lumber, spaced at 24 inches

Sheathing is 7/16 inch with no intermediate blocking

Size of sheathing is 48 inches by 96 inches

Fasteners along end trusses are 8d nails spaced at 4 inches

Fasteners along int. trusses are 8d nails spaced at 8 inches

Total outward wind force on sheathing = 1263 pounds

Total withdrawal resistance of 47 nails = 3569 pounds (increased for wind)

Fastening of roof sheathing satisfies Code requirements.

End zone (areas Se and C) width = 6.4 feet

Roof trusses are Southern Pine lumber, spaced at 24 inches

Sheathing is 7/16 inch with no intermediate blocking

Size of sheathing is 48 inches by 96 inches

Fasteners along end truss are 8d nails spaced at 4 inches

Fasteners along end wall are 8d nails spaced at 4 inches

Fasteners along int. trusses are 8d nails spaced at 8 inches

Total outward wind force on sheathing = 1738 pounds

Total withdrawal resistance of 47 nails = 3569 pounds (increased for wind)

Fastening of roof sheathing satisfies Code requirements.



\*\*\*\* ANALYSIS OF WALL STUDS \*\*\*\*

\*\*\* Analysis of Wall Stud Number 1 \*\*\*

2 in. x 4 in. single studs at 16 in. spacing  
Stud height is 7.625 feet--located in interior zone  
Top of studs is laterally supported by ceiling diaphragm or other method  
Spruce--Pine--Fir lumber----Number 1--Number 2 grade  
Sheathing is inch rated OSB, span rating 24/16

Cross-sectional area = 5.25 sq.in.  
Moment of inertia = 5.359375 in.^4  
Section Modulus = 3.0625 in.^3  
Elastic modulus of wood stud = 1400000 in.^2

Total outward force on stud = 325 pounds  
Stud moment = 309 ft-lb.

Stresses:

Stud bending vert : Actual = 1213 psi Allowable = 2415 psi (adjusted)  
Stud shear : Actual = 42 psi Allowable = 112 psi (adjusted)  
Stud tensile : Actual = 55 psi Allowable = 1020 psi (adjusted)  
Interaction bending and tension actual/allowable stress ratio total = .556199  
Sheathing bending hor: Actual = 178 psi Allowable = 222 psi(adjusted)

Deflections:

Stud : Actual = .27 in. Allowable = .5083 in.

-----  
\*\*\* Summary of Analysis \*\*\*

Wall structure satisfies all Code requirements.

\*\*\*\* ANALYSIS OF WALL STUDS \*\*\*\*

\*\*\* Analysis of Wall Stud Number 2 \*\*\*

2 in. x 4 in. single studs at 16 in. spacing  
Stud height is 7.625 feet--located in end zone  
Top of studs is laterally supported by ceiling diaphragm or other method  
Spruce--Pine--Fir lumber----Number 1--Number 2 grade  
Sheathing is inch rated OSB, span rating 24/16

Cross-sectional area = 5.25 sq.in.  
Moment of inertia = 5.359375 in.^4  
Section Modulus = 3.0625 in.^3  
Elastic modulus of wood stud = 1400000 in.^2

Total outward force on stud = 374 pounds  
Stud moment = 356 ft-lb.

Stresses:

Stud bending vert : Actual = 1396 psi Allowable = 2415 psi (adjusted)  
Stud shear : Actual = 49 psi Allowable = 112 psi (adjusted)  
Stud tensile : Actual = 55 psi Allowable = 1020 psi (adjusted)  
Interaction bending and tension actual/allowable stress ratio total = .6319754  
Sheathing bending hor: Actual = 205 psi Allowable = 222 psi (adjusted)

Deflections:

Stud : Actual = .3107 in. Allowable = .5083 in.

-----  
\*\*\* Summary of Analysis \*\*\*

Wall structure satisfies all Code requirements.

\*\*\*\* ALLOWABLE STRESS PROPERTIES \*\*\*\*

Base stresses (psi):

Wood:

Bending = 875  
Tension = 425  
Shear = 70  
Elastic modulus = 1400000

Adjustment factors for wood:

Duration (Du) = 1.6  
Wet service (Wt) = 1  
Temperature (Tm) = 1  
Stability (St) = 1  
Size (Sz) = 1.5  
Volume (Vm) = 1  
Flat use (Fu) = 1  
Repetitive (Rp) = 1.15  
Curvature (Cu) = 1  
Form (Fm) = 1  
Shear stress (Sh) = 1

Allowable stresses (psi):

Wood:

Bending = 2415 (Base x Du x Wt x Tm x St x Sz x Vm x Fu x Rp x Cu x Fm)  
Tension = 1020 (Base x Du x Wt x Tm x Sz)  
Shear = 112 (Base x Du x Wt x Tm x Sh)  
Elastic modulus = 2240000 (Base x Wt x Tm)

Sheathing:

Bending = 222 (Base x 1.33)  
Elastic modulus = 61904.76 (Base)

## TRANSVERSE DRAGSTRUT NAIL ANALYSIS

Wall framing is 2 in. x 4 in. studs  
Wall stud framing lumber is Spruce--Pine--Fir  
Fasteners are 16d common nails  
Approximate nail spacing = 16 inches

Total lateral force on building = 11300 pounds  
Force applied at top of walls = 5650 pounds  
Total dragstrut length = 64 feet  
Shear per unit dragstrut length = 88 pounds per linear foot

Actual shear on each nail = 117 pounds  
Allowable shear on each nail = 192 pounds

Dragstrut nailing satisfies Code requirements.

-----

## LONGITUDINAL DRAGSTRUT NAIL ANALYSIS

Wall framing is 2 in. x 4 in. studs  
Wall stud framing lumber is Spruce--Pine--Fir  
Fasteners are 16d common nails  
Approximate nail spacing = 16 inches

Total lateral force on building = 8798 pounds  
Force applied at top of walls = 4399 pounds  
Total dragstrut length = 100 feet  
Shear per unit dragstrut length = 43 pounds per linear foot

Actual shear on each nail = 57 pounds  
Allowable shear on each nail = 192 pounds

Dragstrut nailing satisfies Code requirements.

\*\*\*\* T R A N S V E R S E    S H E A R W A L L    A N A L Y S I S \*\*\*\*

Wall framing is 2 in. x 4 in. studs at 16 inch spacing  
Wall stud framing lumber is Spruce--Pine--Fir  
Wall shear siding is Oriented Strand Board -- 7/16 inch thick  
Wall sheathing has all edges nailed  
Fasteners: 8d common nails spaced along edges at 4 inch centers  
Fasteners: 8d common nails spaced in interior at 8 inch centers

Total lateral force on building        = 11300 pounds  
Force applied at top of walls           = 5650 pounds  
Accumulated total shearwall length = 64 feet

Actual unit shear on shearwalls        = 88 pounds per linear foot  
Allowable unit shear on shearwalls = 322 pounds per linear foot

Shearwall satisfies Code requirements.

-----

\*\*\*\* L O N G I T U D I N A L    S H E A R W A L L    A N A L Y S I S \*\*\*\*

Wall framing is 2 in. x 4 in. studs at 16 inch spacing  
Wall stud framing lumber is Spruce--Pine--Fir  
Wall shear siding is Oriented Strand Board -- 7/16 inch thick  
Wall sheathing has all edges nailed  
Fasteners: 8d common nails spaced along edges at 4 inch centers  
Fasteners: 8d common nails spaced in interior at 8 inch centers

Total lateral force on building        = 8798 pounds  
Force applied at top of walls           = 4399 pounds  
Accumulated total shearwall length = 100 feet

Actual unit shear on shearwalls        = 43 pounds per linear foot  
Allowable unit shear on shearwalls = 322 pounds per linear foot

Shearwall satisfies Code requirements.

-----

\*\*\* ANALYSIS OF OUTWARD FORCES ON WALL SHEATHING \*\*\*

Wall number 1 : Total outward wind force on sheathing = 975 pounds  
: Total withdrawal resistance of 92 nails = 5133 pounds

Wall number 2 : Total outward wind force on sheathing = 1122 pounds  
: Total withdrawal resistance of 92 nails = 5133 pounds

\*\*\*\* ANALYSIS OF SHEATHING FASTENERS \*\*\*\*

Wall framing is Spruce--Pine--Fir lumber  
Sheathing is 7/16 inch Oriented Strand Board  
Sheathing extends from bottom of bottom plate to top of top plate  
Fasteners are 8d common nails at 4 inch spacing

Total uniform wind uplift in first story at top of wall level = 398 plf  
Uniform dead loads per linear foot:

Roof = 180.9755 plf  
-----

Total = 180.9755 plf

Total uniform dead load in first story at top of wall level = 180 plf

Net wind uplift in first story at top of wall level = 218 plf

Total uplift force on each nail = 72 pounds

Allowable shear on each nail = 97 pounds (increased for wind)

Sheathing to plate fastening satisfies all Code requirements.

\*\*\*\* ANALYSIS OF SHEATHING FASTENERS \*\*\*\*

Wall framing is Spruce--Pine--Fir lumber  
Sheathing is 7/16 inch Oriented Strand Board  
Sheathing extends from bottom of bottom plate to top of top plate  
Fasteners are 8d common nails at 4 inch spacing

Total uniform wind uplift in first story at floor level = 398 plf

Uniform dead loads per linear foot:

Roof = 180.9755 plf

Wall = 399.5678 plf  
-----

Total = 580.5433 plf

Total uniform dead load in first story at floor level = 580 plf

Net wind uplift in first story at floor level = -182 plf

Total uplift force on each nail = -61 pounds

Allowable shear on each nail = 97 pounds (increased for wind)

Sheathing to plate fastening satisfies all Code requirements.

\*\*\*\* ANALYSIS OF FOUNDATION ANCHORAGE \*\*\*\*

Anchor bolts are 1/2 inch A307, with 2 inch round washer at 72 inch centers.

Total uniform wind uplift on foundation = 398 pounds per linear foot

Uniform dead loads in pounds per linear foot:

Roof = 180.9755 plf

Wall = 399.5678 plf

-----

Total = 580.5433 plf

Total uniform dead load times 2/3 = 387 pounds per linear foot

Net uplift force on foundation = 11 pounds per linear foot

Total uplift force on each anchor bolt = 66 pounds

Safe tension value of each anchor bolt = 1634 pounds (increased by 1/3)

Bolt safe tension value is governed by washer failure

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\*\*\* Summary of Analysis \*\*\*

Foundation anchorage satisfies all Code requirements.

\*\*\*\* ANALYSIS OF CORNER HOLD-DOWN REQUIREMENTS \*\*\*\*

Hold-down is one typical anchor bolt with washer, each wall

Normal anchor bolt spacing = 72 inches

Distance from corner to hold-down device = 6 inches

Distance from corner to first interior anchor bolt = 6 inches

Net uplift force on foundation = 11 pounds per linear foot

Tributary distance to corner device = .5 feet

Net uplift on corner hold-down device = 5 pounds

Uplift tension due to shearwall action in a transverse shearwall segment:

Distance from corner to hold-down device = 6 inches

Distance from corner to first interior anchor bolt = 6 inches

Total shear from shearwall segment = 264 pounds

Height of wall = 8 feet

Uniform dead load times 2/3 = 266 pounds per linear foot

Shearwall moment at bottom of wall = 2118 foot-pounds

Additional tension at corner device = 348 pounds

Total uplift tension on corner hold-down devices = 353 pounds

Allowable tension on corner hold-down devices = 3268 pounds

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\*\*\* Summary of Analysis \*\*\*

Corner hold-down device COMPLIES with Code requirements.



\*\*\*\* ANALYSIS OF FOUNDATION \*\*\*\*

Stemwall is 8 inch concrete masonry, filled with grout, 16 inches high  
Footing is 20 inches wide by 10 inches deep  
Earth cover over top of footing is 4 inches

Total uniform wind uplift on foundation = 398 pounds per linear foot  
Uniform dead loads in pounds per linear foot:

Roof = 180.9755 plf  
Wall = 399.5678 plf  
-----

Total = 580.5433 plf

Total uniform dead load times 2/3 = 387 pounds per linear foot  
Net uplift force at top of foundation = 11 pounds per linear foot  
Weight of stemwall footing earth x 2/3 = 261 pounds per linear foot  
Net uplift at bottom of footing = 0 pounds per linear foot  
-----

\*\*\* Summary of Analysis \*\*\*

Foundation is stable.

\*\*\*\* ANALYSIS OF REINFORCING STEEL \*\*\*\*

Grade 40 reinforcing steel, Number 5 vert. bars at 72 inch centers

Total uniform wind uplift on foundation = 398 pounds per linear feet  
Uniform dead loads in pounds per linear foot:

Roof = 180.9755 plf  
Wall = 399.5678 plf  
-----

Total = 580.5433 plf

Total uniform dead load times 2/3 = 387 pounds per linear foot  
Net uplift force on foundation = 11 pounds per linear foot  
Weight of concrete block stemwall x 2/3 = 81 pounds per linear foot  
Net uplift at top of footing = 0 pounds per linear foot

Total uplift force on each re-bar = 0 pounds  
Safe tension value of each re-bar = 8181 pounds (increased by 1/3)  
-----

\*\*\* Summary of Analysis \*\*\*

Reinforcing steel satisfies all Code requirements.

\*\*\*\* SUMMARY OF REINFORCING DATA \*\*\*\*

Foundation wall data:

Wall is composed of 8 inch concrete masonry, fully grouted.

Wall reinforcing is Grade 40 steel, Number 5 at 72 inch centers

Minimum required lap splice for Number 5 bar is 25 inches.

Minimum required clearance for Number 5 bar is 1.5 inches.

Wall reinf. in footing has a std. A.C.I. hook, 6 inches below top of footing.

Footing data:

Footing is continuous, 20 inches wide by 10 inches deep.

Footing concrete is 2500 psi

Footing reinforcing is Grade 40 steel, 2--#(    ) longitudinal.

Minimum required splice length = 25 inches

Reinforcing steel shall have cover as follows:    ,

Top-----6 inches

Sides-----3 inches

Bottom----3 inches

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# RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2004 and FLORIDA RESIDENTIAL CODE 2004 WITH AMENDMENTS ONE (1) AND TWO (2) FAMILY DWELLINGS

ALL REQUIREMENTS ARE SUBJECT TO CHANGE  
EFFECTIVE OCTOBER 1, 2005

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 16 OF THE FLORIDA BUILDING CODE 2004 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1609 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ----- 100 MPH
2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE -----110 MPH
3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

## GENERAL REQUIREMENTS: Two (2) complete sets of plans containing the following:

Applicant	Plans Examiner	
<input checked="" type="checkbox"/>	<input type="checkbox"/>	All drawings must be clear, concise and drawn to scale ("Optional " details that are not used shall be marked void or crossed off). Square footage of different areas shall be shown on plans.
<input checked="" type="checkbox"/>	<input type="checkbox"/>	Designers name and signature on document (FBC 106.1). If licensed architect or engineer, official seal shall be affixed.
<input checked="" type="checkbox"/>	<input type="checkbox"/>	<u>Site Plan including:</u> <ol style="list-style-type: none"> <li>a) Dimensions of lot</li> <li>b) Dimensions of building set backs</li> <li>c) Location of all other buildings on lot, well and septic tank if applicable, and all utility easements.</li> <li>d) Provide a full legal description of property.</li> </ol>
<input checked="" type="checkbox"/>	<input type="checkbox"/>	<u>Wind-load Engineering Summary, calculations and any details required</u> Plans or specifications must state compliance with FBC Section 1609. The following information must be shown as per section 1603.1.4 FBC <ol style="list-style-type: none"> <li>a. Basic wind speed (3-second gust), miles per hour (km/hr).</li> <li>b. Wind importance factor, <math>I_w</math>, and building classification from Table 1604.5 or Table 6-1, ASCE 7 and building classification in Table 1-1, ASCE 7.</li> <li>c. Wind exposure, if more than one wind exposure is utilized, the wind exposure and applicable wind direction shall be indicated.</li> <li>d. The applicable enclosure classifications and, if designed with ASCE 7, internal pressure coefficient.</li> <li>e. Components and Cladding. The design wind pressures in terms of psf (<math>kN/m^2</math>) to be used for the design of exterior component and cladding materials not specifically designed by the registered design professional.</li> </ol>
<input checked="" type="checkbox"/>	<input type="checkbox"/>	<u>Elevations including:</u> <ol style="list-style-type: none"> <li>a) All sides</li> <li>b) Roof pitch</li> <li>c) Overhang dimensions and detail with attic ventilation</li> </ol>
<input checked="" type="checkbox"/>	<input type="checkbox"/>	
<input checked="" type="checkbox"/>	<input type="checkbox"/>	
<input checked="" type="checkbox"/>	<input type="checkbox"/>	

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- ☒ ☐
- ☒ ☐
- ☒ ☐

- d) Location, size and height above roof of chimneys.
- e) Location and size of skylights
- f) Building height
- e) Number of stories

**Floor Plan including:**

- ☒ ☐
- ☒ ☐
- ☒ ☐

- a) Rooms labeled and dimensioned.
- b) Shear walls identified.
- c) Show product approval specification as required by Fla. Statute 553.842 and Fla. Administrative Code 9B-72 (**see attach forms**).
- d) Show safety glazing of glass, where required by code.
- e) Identify egress windows in bedrooms, and size.
- f) Fireplace (gas vented), (gas non-vented) or wood burning with hearth, (**Please circle applicable type**).
- g) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails.
- h) Must show and identify accessibility requirements (accessible bathroom)

**Foundation Plan including:**

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- ☒ ☐
- ☒ ☐
- ☒ ☐

- a) Location of all load-bearing wall with required footings indicated as standard or monolithic and dimensions and reinforcing.
- b) All posts and/or column footing including size and reinforcing
- c) Any special support required by soil analysis such as piling
- d) Location of any vertical steel.

**Roof System:**

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- a) Truss package including:
  1. Truss layout and truss details signed and sealed by Fl. Pro. Eng.
  2. Roof assembly (FBC 106.1.1.2 )Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
- b) Conventional Framing Layout including:
  1. Rafter size, species and spacing
  2. Attachment to wall and uplift
  3. Ridge beam sized and valley framing and support details
  4. Roof assembly (FBC 106.1.1.2)Roofing systems, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

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**Wall Sections including:**

- ☐ ☐

**a) Masonry wall**

1. All materials making up wall
2. Block size and mortar type with size and spacing of reinforcement
3. Lintel, tie-beam sizes and reinforcement
4. Gable ends with rake beams showing reinforcement or gable truss and wall bracing details
5. All required connectors with uplift rating and required number and size of fasteners for continuous tie from roof to foundation
6. Roof assembly shown here or on roof system detail (FBC 106.1.1.2) Roofing system, materials, manufacturer, fastening requirements and product evaluation with resistance rating)
7. Fire resistant construction (if required)
8. Fireproofing requirements
9. Shoe type of termite treatment (termicide or alternative method)
10. Slab on grade
  - a. Vapor retarder (6mil. Polyethylene with joints lapped 6 inches and sealed)
  - b. Must show control joints, synthetic fiber reinforcement or Welded fire fabric reinforcement and supports
11. Indicate where pressure treated wood will be placed
12. Provide insulation R value for the following:
  - a. Attic space
  - b. Exterior wall cavity

c. Crawl space (if applicable)

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b) Wood frame wall

1. All materials making up wall
2. Size and species of studs
3. Sheathing size, type and nailing schedule
4. Headers sized
5. Gable end showing balloon framing detail or gable truss and wall hinge bracing detail
6. All required fasteners for continuous tie from roof to foundation (truss anchors, straps, anchor bolts and washers)
7. Roof assembly shown here or on roof system detail (FBC 106.1.1.2) Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
8. Fire resistant construction (if applicable)
9. Fireproofing requirements
10. Show type of termite treatment (termiteicide or alternative method)
11. Slab on grade
  - a. Vapor retarder (6Mil. Polyethylene with joints lapped 6 inches and sealed
  - b. Must show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and supports
12. Indicate where pressure treated wood will be placed
13. Provide insulation R value for the following:
  - a. Attic space
  - b. Exterior wall cavity
  - c. Crawl space (if applicable)

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c) Metal frame wall and roof (designed, signed and sealed by Florida Prof. Engineer or Architect)

**Floor Framing System:**

- a) Floor truss package including layout and details, signed and sealed by Florida Registered Professional Engineer
- b) Floor joist size and spacing
- c) Girder size and spacing
- d) Attachment of joist to girder
- e) Wind load requirements where applicable

**Plumbing Fixture layout**

**Electrical layout including:**

- a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified
- b) Ceiling fans
- c) Smoke detectors
- d) Service panel and sub-panel size and location(s)
- e) Meter location with type of service entrance (overhead or underground)
- f) Appliances and HVAC equipment
- g) Arc Fault Circuits (AFCI) in bedrooms
- h) Exhaust fans in bathroom

**HVAC information**

- a) Energy Calculations (dimensions shall match plans)
- b) Manual J sizing equipment or equivalent computation
- c) Gas System Type (LP or Natural) Location and BTU demand of equipment

**Disclosure Statement for Owner Builders**

**\*\*\*Notice Of Commencement Required Before Any Inspections Will Be Done**

**Private Potable Water**

- a) Size of pump motor
- b) Size of pressure tank
- c) Cycle stop valve if used

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LAMAR BOOZER  
900 EAST PUTNAM STREET  
LAKE CIT, FLORIDA 32055

PROJECT: SPARKS/ WISE EST  
CLIENT: MIKE TODD  
DATE: OCTOBER 30, 2006

RESIDENTIAL/ LIGHT COMMERCIAL HVAC LOADS

DESIGNER: LAMAR BOOZER

CLIENT INFORMATION:

NAME: MIKE TODD  
ADDRESS: 129 N.E. COLBURN AVENUE  
LAKE CITY, FL 32055

TOTAL BUILDING LOADS:

BLDG LOAD DESCRIPTION	AREA QUAN	SEN. LOSS	LAT. GAIN	+	SEN. GAIN	=	TOTAL GAIN
3-C WINDOW DBL PANE CLR GLS METL FR	148	4,828	0		9,694		9,694
9-I FRENCH DOOR DBL CLR GLS METL FR	80	2,715	0		5,792		5,792
12-D WALL R-11 = 1/4" ASPHLT BRD (R-1.3)	1,277	4,597	0		2,513		2,513
11-C DOOR METAL POLYSTRENE CORE	57	1,206	0		659		659
16-G CEILING R- 30 INSULATION	1,716	2,548	0		2,548		2,548
22-A SLAB ON GRADE NO EDGE INSUL	208	7,581	0		0		0
SUBTOTALS FOR STRUCTURE:	3,486	23,475	0		21,206		21,206
PEOPLE	20	0	0		6,000		6,000
APPLIANCES	0	0	1,800		1,500		1,500
DUCTWORK	0	1,174	0		2,871		2,871
INFILTRATION W.CFM: 0.0 S.CFM: 0.0	0	0	0		0		0
VENTILATION W.CFM: 0.00 S.CFM: 0.00	0	0	0		0		0
SENSIBLE GAIN TOTALS					31,577		
TEMP. SWING MULTIPLIER				X 1.00			
BUILDING LOAD TOTALS		24,649	1,800		31,577		33,377

SUPPLY CFM AT 20 DEG DT:	1435	CFM PER SUARE FOOT:	0.836
SQUARE FT. OF ROOM AREA:	1694	SQUARE FOOT PER TON:	616.925

TOTAL HEATING REQUIRED WITH OUTSIDE AIR: 24.649 MBH  
TOTAL COOLING REQUIRED WITH OUTSIDE AIR: 2.781 TONS

CALCULATIONS ARE BASED ON 7<sup>TH</sup> EDITION OF ACCA MANUAL J.  
ALL COMPUTED RESULTS ARE ESTIMATES AS BUILDING USE AND WEATHER MAY VARY.  
BE SURE TO SELECT A UNIT THAT MEETS BOTH SENSIBLE AND LATENT LOADS.

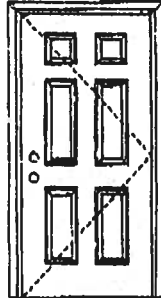
X

Opaque Inswing Unit

COP-WI-JH4101-02

## WOOD-EDGE STEEL DOORS

### APPROVED ARRANGEMENT:



**Note:**  
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".



Test Data Review Certificate #3026447A and COP/First Report Validation Matrix #3026447A-001 provides additional information - available from the ITG/WH website ([www.itg-wh.com](http://www.itg-wh.com)), the Masonite website ([www.masonite.com](http://www.masonite.com)) or the Masonite technical center.

**Single Door**  
Maximum unit size = 3'0" x 6'8"

**Design Pressure**  
+66.0/-66.0

Limited water unless special threshold design is used.

**Large Missile Impact Resistance**

**Hurricane protective system (shutters) is NOT REQUIRED.**

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

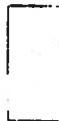
### MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed - see MAD-WL-MA0001-02.

### MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed - see MID-WL-MA0001-02.

### APPROVED DOOR STYLES:



Flush



Arch Top 8-panel



3-panel



6-panel



New England 4-panel



Eyebrow 4-panel



8-panel



9-panel



16-panel



5-panel



5-panel with scroll



Eyebrow 5-panel



Eyebrow 8-panel with scroll

**Johnson**  
**EntrySystems**

June 17, 2002  
Our continuing program of product improvement includes specifications, design and product development to change without notice.



**Masonite**  
Masonite International Corporation



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Opaque Inswing Unit

COP-WL-JH4101-02

## WOOD-EDGE STEEL DOORS

### CERTIFIED TEST REPORTS:

NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested In Accordance with Miami-Dade BCCO PA201, PA202 and PA203.

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core.

Frama constructed of wood with an extruded aluminum threshold.

### PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH  
MIAMI-DADE BCCO  
PA201, PA202 & PA203

COMPANY NAME  
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

*Kurt L. Bathaz*

State of Florida, Professional Engineer  
Kurt Bathazor, P.E. - License Number 56533

Memoack Hargay



Test Data Review Certificate #3028447A  
and COI/Field Report Validation Matrix  
#3028447A-COI provides additional  
information - available from the ITSAWH  
website (www.itswah.com), the  
Masonite website (www.masonite.com)  
or the Memoack technical center.

**Johnson**  
EntrySystems

June 17, 2002

Our engineering division of product performance makes specifications, design and product  
drawings subject to change without notice.

**PREMDOR**  
Premium Quality Doors



Exclusively from  
**Masonite**  
Masonite International Corporation



# ELK



**PRESTIQUE®  
HIGH DEFINITION®**



**RAISED PROFILE™**

## Prestique Plus *High Definition* and Prestique Gallery Collection™

Product size ..... 13⅝" x 39⅝"  
Exposure ..... 5⅞"  
Pieces/Bundle ..... 16  
Bundles/Square ..... 4/98.6 sq.ft.  
Squares/Pallet ..... 11

50-year limited warranty period:  
non-prorated coverage for  
shingles and application labor for  
the initial 5 years, plus an option  
for transferability\*; prorated  
coverage for application labor and  
shingles for balance of limited  
warranty period; 5-year limited  
wind warranty\*.

## Raised Profile

Product size ..... 13⅝" x 39⅝"  
Exposure ..... 5⅞"  
Pieces/Bundle ..... 22  
Bundles/Square ..... 3/100 sq.ft.  
Squares/Pallet ..... 19

30-year limited warranty period:  
non-prorated coverage for  
shingles and application labor for  
the initial 5 years, plus an option  
for transferability\*; prorated  
coverage for application labor and  
shingles for balance of limited  
warranty period; 5-year limited  
wind warranty\*.

## Prestique I *High Definition*

Product size ..... 13⅝" x 39⅝"  
Exposure ..... 5⅞"  
Pieces/Bundle ..... 16  
Bundles/Square ..... 4/98.6 sq.ft.  
Squares/Pallet ..... 14

40-year limited warranty period:  
non-prorated coverage for  
shingles and application labor for  
the initial 5 years, plus an option  
for transferability\*; prorated  
coverage for application labor and  
shingles for balance of limited  
warranty period; 5-year limited  
wind warranty\*.

## HIP AND RIDGE SHINGLES

### Seal-A-Ridge® w/FLX™

Size: 12" x 12"  
Exposure: 6⅞"  
Pieces/Bundle: 45  
Coverage: 4 Bundles = 100 linear feet

## Prestique *High Definition*

Product size ..... 13⅝" x 39⅝"  
Exposure ..... 5⅞"  
Pieces/Bundle ..... 22  
Bundles/Square ..... 3/100 sq.ft.  
Squares/Pallet ..... 16

30-year limited warranty period:  
non-prorated coverage for  
shingles and application labor for  
the initial 5 years, plus an option  
for transferability\*; prorated  
coverage for application labor and  
shingles for balance of limited  
warranty period; 5-year limited  
wind warranty\*.

## Elk Starter Strip

52 Bundles/Pallet  
18 Pallets/Truck  
936 Bundles/Truck  
19 Pieces/Bundle  
1 Bundle = 120.33 linear feet

Available Colors: Antique Slate, Weatheredwood, Shekewood, Sablewood, Hickory, Barkwood\*\*, Forest Green, Wedgewood\*\*, Birchwood\*\*, Sandalwood, Gallery Collection: Balsam Forest™, Weathered Sage™, Sienna Sunset™.

All Prestique, Raised Profile and Seal-A-Ridge roofing products contain Elk WindGuard® sealant. WindGuard activates with the sun's heat, bonding shingles into a wind and weather resistant cover that resists blow-offs and leaks.

Check for availability with built-in StainGuard® treatment to inhibit the discoloration of roofing granules caused by the growth of certain types of algae. Not available in Sablewood.

All Prestique and Raised Profile shingles meet UL® Wind Resistant (UL 997) and Class "A" Fire Ratings (UL 790); and ASTM Specifications D 3018, Type-I; D 3161, Type-I; E 108 and the requirements of ASTM D 3482.

All Prestique and Raised Profile shingles meet the latest Metro Data building code requirements.

\*See actual limited warranty for conditions and limitations.

\*\*Check for product availability.

## SPECIFICATIONS

**SCOPE:** Work includes furnishing all labor, materials and equipment necessary to complete installation of (name) shingles specified herein. Color shall be (name of color). Hip and ridge type to be Elk Seal-A-Ridge with formula FLX.

All exposed metal surfaces (flashing, vents, etc.) to be painted with matching Elk roof accessory paint.

**PREPARATION OF ROOF DECK:** Roof deck to be dry, well-

**MATERIALS:** Underlayment for standard roof slopes, 4" per foot (101.6/304.8mm) or greater: apply non-perforated No. 15 or 30 asphalt-saturated felt underlayment. For low slopes (4" per foot (101.6/304.8mm) to a minimum of 2" per foot (50.8/304.8mm)), use two plies of underlayment overlapped a minimum of 19". Fasteners shall be of sufficient length and holding power for securing material as required by the application instructions printed on shingle wrapper.

warranties are contingent upon the correct installation as shown on the instructions. These instructions are the minimum required to meet Elk application requirements. In some areas, building codes may require additional application techniques or methods beyond our instructions. In these cases, the local code must be followed. Under no circumstances will Elk accept application requirements less than those contained in its application instructions.

(9.525mm) thick conforming to the specifications of the American Plywood Association; 7/16" (11.074mm) oriented strandboard; or chipboard. Most fire retardant plywood decks are NOT approved substrates for Elk shingles. Consult Elk Field Service for application specifications over other decks and other slopes.

(same) with stainguard treatment, as manufactured by the Elk Tuscaloosa plant. Hip and ridge type to be Seal-A-Ridge with formula FLX with StainGuard treatment.

or e-mail [specinfo@elkcorp.com](mailto:specinfo@elkcorp.com).

Complete application instructions are published by Elk and printed on the back of every shingle bundle. All

**SOUTHEAST &  
ATLANTIC OFFICE:**  
800.945.5551

**CORPORATE HEADQUARTERS:**  
800.354.7732

**PLANT LOCATION:**  
800.945.5545

**ELK** 

[www.elkcorp.com](http://www.elkcorp.com)

SSOOT 01/02



Architectural Testing

AAMA/NWDA 101/I.S.2-97  
TEST REPORT SUMMARY

Rendered to:

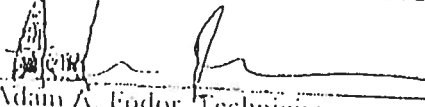
MI HOME PRODUCTS, INC.

SERIES/MODEL: 450  
TYPE: Aluminum Single Hung Window  
RATING: H-C30 54 x 90; H-C45 52 x 72\*

Title of Test	Results	
	Test Specimen #1	Test Specimen #2
Overall Design Pressure	30 psf	47 psf
Operating Force	20 lb max.	N/A
Air Infiltration	0.27 cfm/ft <sup>2</sup>	N/A
Water Resistance	5.25 psf	6.0 psf
Structural Test Pressure	±45.0 psf	±70.5 psf
Deglazing	Passed	N/A
Forced Entry Resistance	Grade 10	N/A

Reference should be made to Report No. 01-37589.01 for complete test specimen description and data.

For ARCHITECTURAL TESTING, INC.

  
Adam A. Fodor, Technician

AAF:jp

130 Derry Court  
York, PA 17402-9405  
phone: 717.764.7700  
fax: 717.764.4129  
www.testintl.com



Architectural Testing

AAMA/NWDA 101/1.S.2-97 TEST REPORT

Rendered to:

MI HOME PRODUCTS, INCORPORATED  
650 West Market Street  
Gratz, Pennsylvania 17030-0370

Report No: 01-37589.01  
Test Date: 06/29/00  
Report Date: 09/11/00  
Expiration Date: 06/29/04

**Project Summary:** Architectural Testing, Inc. (ATI) was contracted to witness tests on a Series/Model 450, aluminum single hung window at the MI Home Products in-plant test facility in Elizabethville, Pennsylvania. The samples tested successfully met the performance requirements for the following ratings: Test Specimen #1 H-C30 54 x 90; Test Specimen #2 H-C40 52 x 72\*. Test specimen descriptions and results are reported herein.

**General Note:** An asterisk (\*) next to the performance grade indicates that the size tested for optional performance was smaller than the minimum test size for the product type and class.

**Test Specification:** The test specimen was evaluated in accordance with AAMA/NWDA 101/1.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors*.

**Test Specimen Description**

Series/Model: 450

Type: Aluminum Single Hung Window

Test Specimen #1 H-C30 54 x 90

Overall Size: 4' 6-1/2" wide by 7' 6-1/2" high

Sash Size: 4' 4" wide by 3' 9-3/4" high

Fixed Daylight Opening Size: 4' 1-1/2" wide by 3' 6-1/2" high

Screen Size: 4' 2-1/4" wide by 3' 8-1/2" high

130 Derry Court  
York, PA 17402-9405  
phone: 717.764.7700  
fax: 717.764.4129  
www.testati.com



Test Specimen Description: (Continued)

Test Specimen #2: 11-C40 52 x 72\*

Overall Size: 4' 4-1/4" wide by 6' 0" high

Sash Size: 4' 2" wide by 3' 0-1/2" high

Fixed Daylight Opening Size: 3' 11-1/2" wide by 2' 9-1/2" high

Screen Size: 4' 0" wide by 2' 11" high

*The following descriptions apply to all specimens.*

Finish: All aluminum was painted.

Glazing Details: The lites utilized 5/8" thick sealed insulating glass units fabricated from two sheets of 3/32" thick clear annealed glass and an Intercept™ spacer system. The sash was channel glazed with a flexible gasket. The fixed lite was interior glazed onto single-sided adhesive foam tape and secured with extruded PVC glazing beads.

Weatherstripping:

<u>Description</u>	<u>Quantity</u>	<u>Location</u>
0.210" high by 0.270" backed polypile with center fin	Row	Fixed meeting rail
0.250" high by 0.187" backed polypile with center fin	2 Rows	Stiles
0.300" diameter by 0.187" backed foam-filled vinyl bulb gasket	Row	Bottom rail
0.400" high by 1/2" square polypile dust plug	4	One on each sash corner

Frame Construction: The main frame was constructed of thermally-broken extruded aluminum members with coped, butted and sealed corners. The fixed meeting rail was constructed of an extruded aluminum member with coped, butted and sealed ends fastened with two screws each.



Test Specimen Description: (Continued)

**Sash Construction:** The sash members were constructed of thermally-broken extruded aluminum members with coped, butted and sealed corners fastened with one screw each.

**Screen Construction:** The screen was constructed of rolled aluminum members with plastic keyed corners. The fiberglass mesh was secured with a flexible spline.

**Hardware:**

<u>Description</u>	<u>Quantity</u>	<u>Location</u>
Plastic snap latch	1	Midspace of bottom rail
Block and tackle balance system	2	One per jamb
Plastic tilt latch	2	One on each end of sash meeting rail
Metal pivot bar	2	One on each end of bottom rail

**Drainage:** Sloped sill

**Reinforcement:** No reinforcement was utilized.

**Installation:** The test unit was installed into the nominal 2" x 8" Spruce-Pine-Fir #2 wood test buck utilizing the integral nailing fin secured with 1" long galvanized roofing nails, 6" from each corner and every 18" on center. The nailing fin was also bedded in polyurethane. The exterior perimeter was blindstopped with wood members and secured with #8 x 3" screws every 24" on center.





Test Results:

The results are tabulated as follows:

<u>Paragraph</u>	<u>Title of Test - Test Method</u>	<u>Results</u>	<u>Allowed</u>
<u>Test Specimen #1: H-C30 54 x 90</u>			
2.2.1.6.1	Operating Force	20 lbs	45 lbs max.
	Air Infiltration per ASTM E 283 (See Note #1) @ 1.57 psf (25 mph)	0.27 cfm/ft <sup>2</sup>	0.3 cfm/ft <sup>2</sup> max.
<i>Note #1: The tested specimen meets (or exceeds) the performance levels specified in AAMA/NWFA 101/1.S. 2-97 for air infiltration.</i>			
	Water Resistance per ASTM E 547 (with and without screen) WTP = 4.5 psf	No leakage	No leakage
2.1.4.2	Uniform Load Structural per ASTM E 330 (Measurements reported were taken on the fixed meeting rail) (a) 45.0 psf (exterior) (a) 45.0 psf (interior)	0.03" 0.04"	0.22" max. 0.22" max.
2.2.1.6.2	Deglazing Test per ASTM E 987 In operating direction at 70 lbs		
	Meeting rail	0.06"/12%	0.50"/100%
	Bottom rail	0.06"/12%	0.50"/100%
	In remaining direction at 50 lbs		
	Left stile	0.06"/12%	0.50"/100%
	Right stile	0.06"/12%	0.50"/100%
	Forced Entry Resistance per ASTM F 588-97		
	Type: A		
	Grade: 10		
	Lock Manipulation Test	No entry	No entry
	Test A1 through A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry



## Test Results:

<u>Paragraph</u>	<u>Title of Test - Test Method</u>	<u>Results</u>	<u>Allowed</u>
<u>Test Specimen #1: (Continued)</u>			
<u>Optional Performance</u>			
4.3	Water Resistance per ASTM E 547 (with and without screen) WTP - 5.25 psf	No leakage	No leakage
<u>Test Specimen #2: H-C40 52 X 72*</u>			
<u>Optional Performance</u>			
4.3	Water Resistance per ASTM E 547 and 331 (with and without screen) WTP - 6.0 psf	No leakage	No leakage
4.4.2	Uniform Load Structural per ASTM E 330 (Measurements reported were taken on the fixed meeting rail) (Loads held for 33 seconds) (a) 47.0 psf (exterior) (a) 47.0 psf (interior)	0.04" 0.03"	N/A N/A
	(Loads held for 10 seconds) (a) 70.5 psf (exterior) (a) 70.5 psf (interior)	0.07" 0.04"	0.21" max. 0.21" max.

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product which may only be granted by the certification program administrator.

For ARCHITECTURAL TESTING, INC:

Adam A. Fodor  
Technician

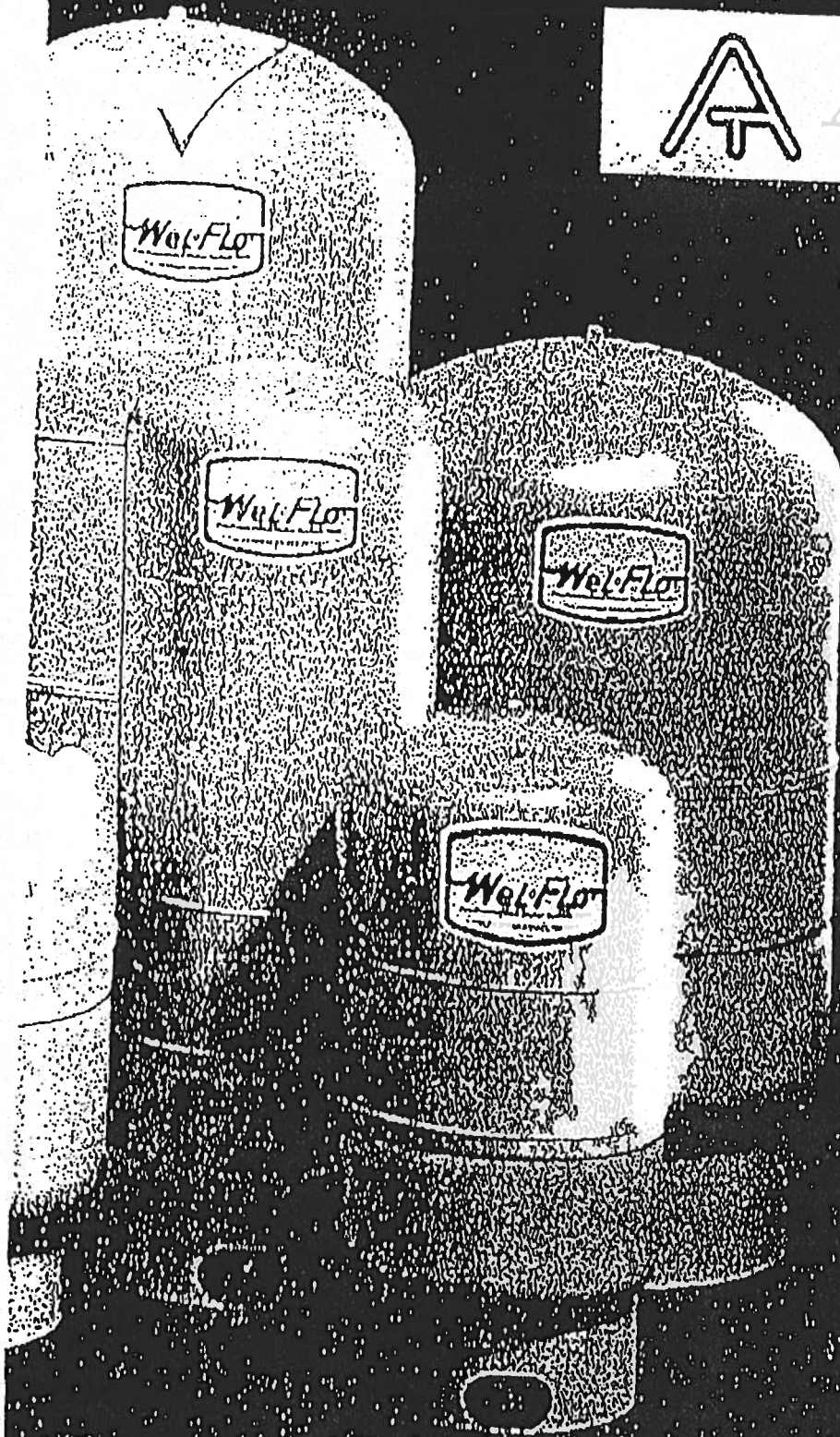
Bruce W. Crook  
Director - Product/Physical Testing



AMTROL INC.

## **WEL-FLO®** Pre-pressurized Water System Tanks

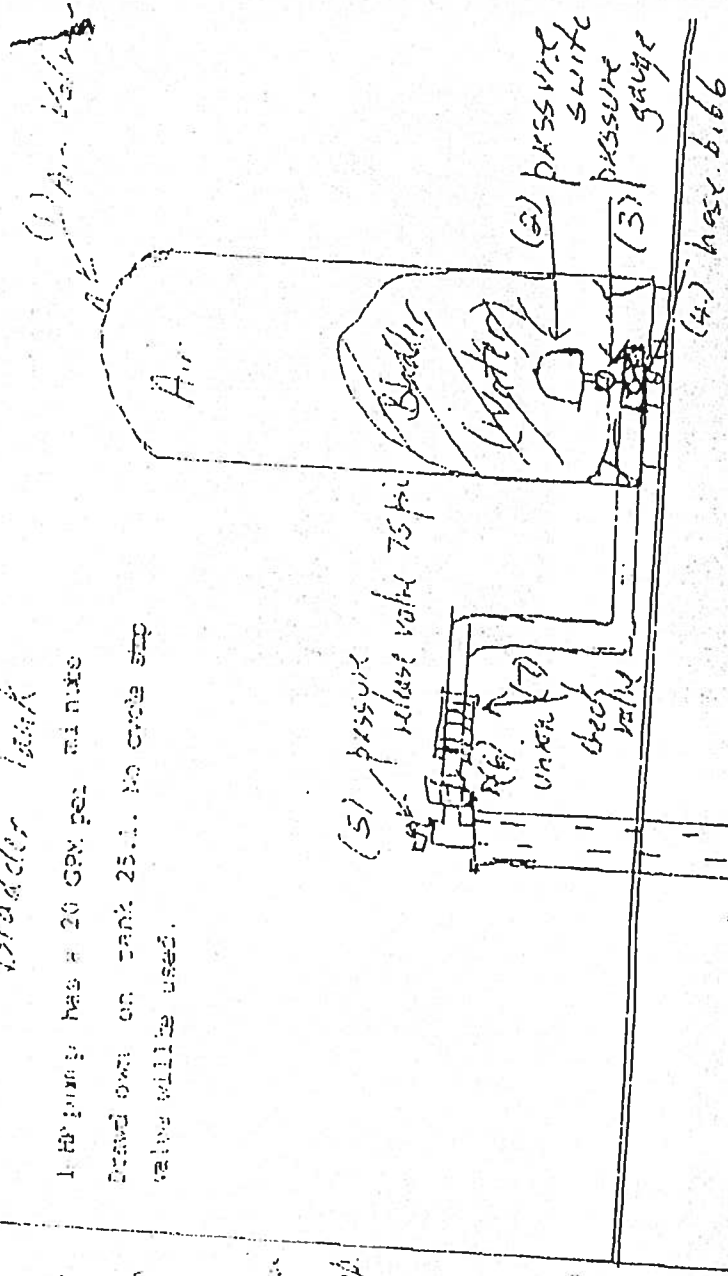
- Proven Diaphragm Design
- Tough Glass Finish
- Sizes from 14 to 119 Gallons
- Outstanding Value



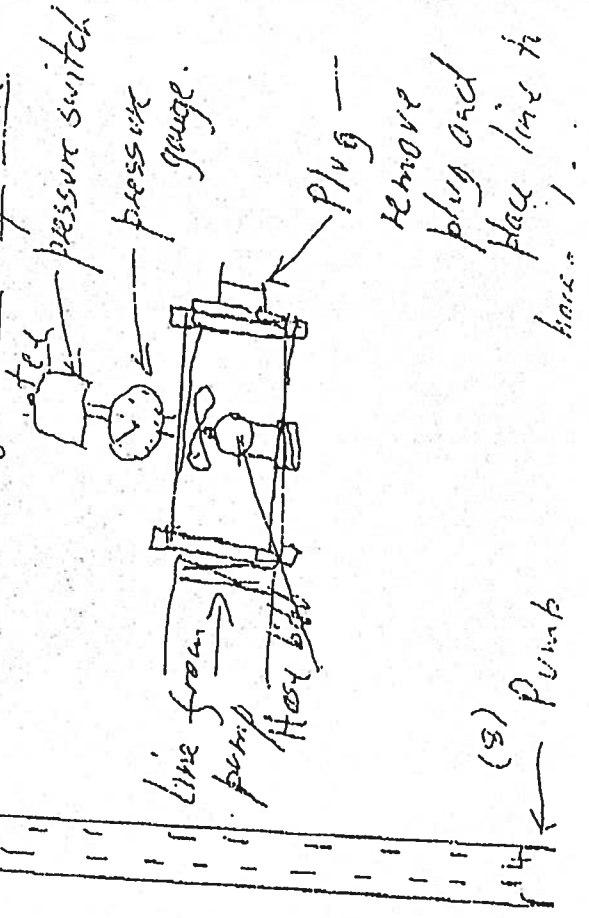
1. Air Valve. Allows air to be put into tank. Must be at or 2 psi below cut-in pressure with tank empty.
2. Pressure switch. Sets cut-in and cut-off pressure for pump.
3. Pressure gauge. Shows actual pressure in tank.
4. Hose bibb. May be used to drain tank or for watering purposes.
5. Pressure release valve. Safety device to prevent explosion of tank.
- Union. Used to separate tank from well.
- Check valve. Prevents water from running back down well.
- Pump. Pushes water up from well into tank.

## Bladder Tank

1 HP pump has a 20 GPM per minute demand on tank 25.1. No cycle stop valve will be used.



## Enlarged view of tank





# Alpine Engineered Products, Inc.

1950 Marley Drive Haines City, FL 33844  
Florida Engineering Certificate of Authorization Number: 567  
Florida Certificate of Product Approval # FL1999  
Page 1 of 1 Document ID:1T1Z487-Z0102104721

Truss Fabricator: Anderson Truss Company  
Job Identification: 6-372-MIKE TODD/KARLTON *Sparks*  
Truss Count: 34  
Model Code: Florida Building Code 2004  
Truss Criteria: ANSI/TPI-2002(STD)/FBC  
Engineering Software: Alpine Software, Version 7.24.  
Structural Engineer of Record: The identity of the structural EOR did not exist as of  
the seal date per section 61G15-31.003(5a) of the FAC  
Address:  
Minimum Design Loads: Roof - 40.0 PSF @ 1.25 Duration  
Floor - N/A  
Wind - 110 MPH ASCE 7-02 - Closed

## Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR487

Details: TCFILLER-BCFILLER-CN8RGBLK-BRCLBSUB-MAX DEAD LOAD-A11015EE-GBLLETIN-

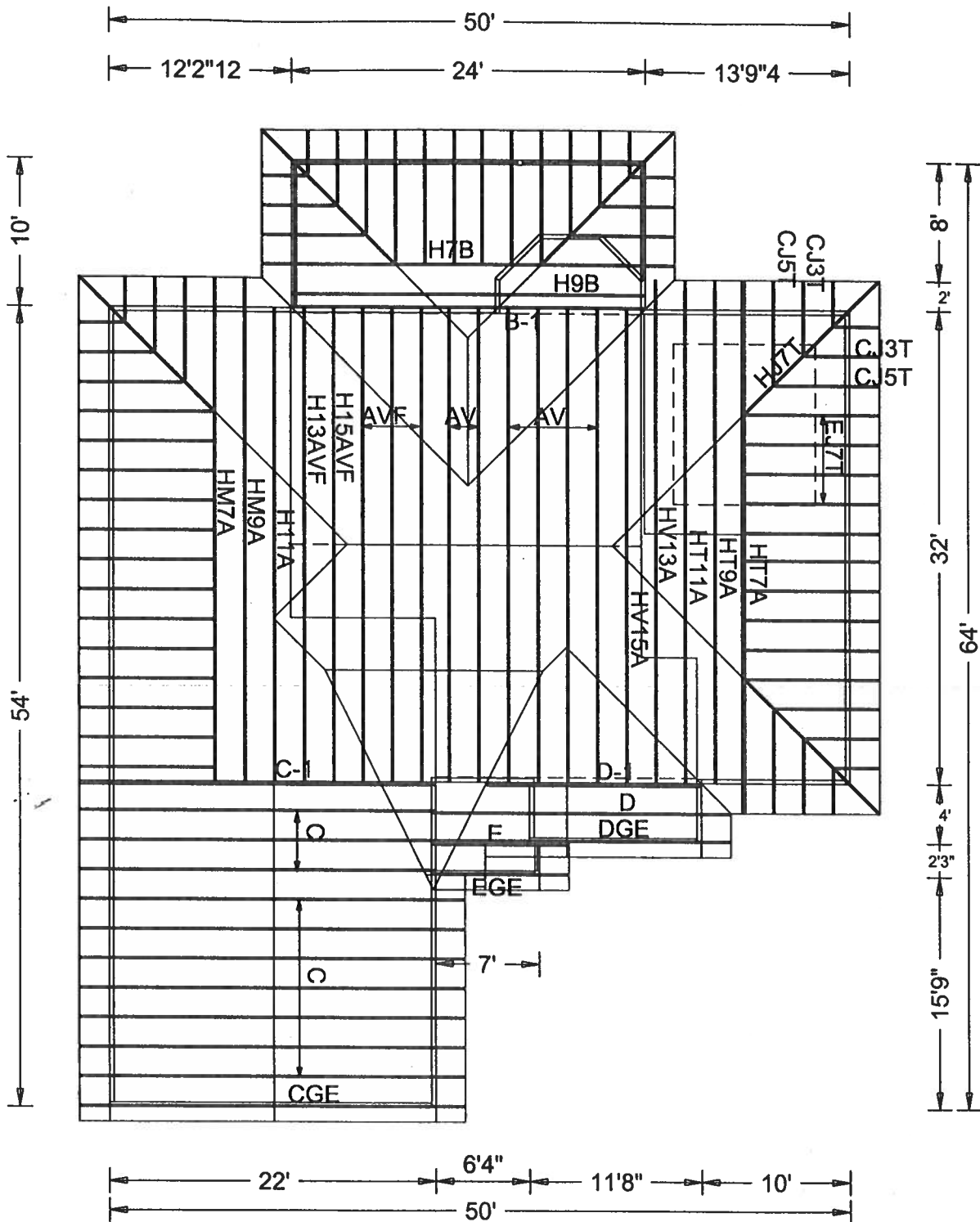
Seal Date: 11/02/2006

-Truss Design Engineer-  
Arthur R. Fisher

Florida License Number: 59687  
1950 Marley Drive  
Haines City, FL 33844

#	Ref	Description	Drawing#	Date
1	56335--	HM7A	06306009	11/02/06
2	56336--	HM9A	06306010	11/02/06
3	56337--	H11A	06306011	11/02/06
4	56338--	H13AVF	06306012	11/02/06
5	56339--	H15AVF	06306013	11/02/06
6	56340--	AVF	06306014	11/02/06
7	56341--	AV	06306015	11/02/06
8	56342--	AV	06306016	11/02/06
9	56343--	HV15A	06306017	11/02/06
10	56344--	HV13A	06306018	11/02/06
11	56345--	HT11A	06306019	11/02/06
12	56346--	HT9A	06306020	11/02/06
13	56347--	HT7A	06306021	11/02/06
14	56348--	H7B	06306022	11/02/06
15	56349--	H9B	06306023	11/02/06
16	56350--	B-1	06306024	11/02/06
17	56351--	CGE	06306025	11/02/06
18	56352--	C	06306001	11/02/06
19	56353--	C	06306002	11/02/06
20	56354--	C-1	06306026	11/02/06
21	56355--	DGE	06306027	11/02/06
22	56356--	D	06306028	11/02/06
23	56357--	D-1	06306029	11/02/06
24	56358--	E	06306003	11/02/06
25	56359--	EGE	06306030	11/02/06
26	56360--	HJ7	06306031	11/02/06
27	56361--	EJ7	06306001	11/02/06
28	56362--	CJ5	06306004	11/02/06
29	56363--	CJ3	06306005	11/02/06
30	56364--	CJ1	06306032	11/02/06
31	56365--	HJ7T	06306033	11/02/06
32	56366--	EJ7T	06306006	11/02/06
33	56367--	CJ5T	06306007	11/02/06
34	56368--	CJ3T	06306008	11/02/06





#6-372 MIKE TODD-SPARKS

Scale: 3/32" = 1'



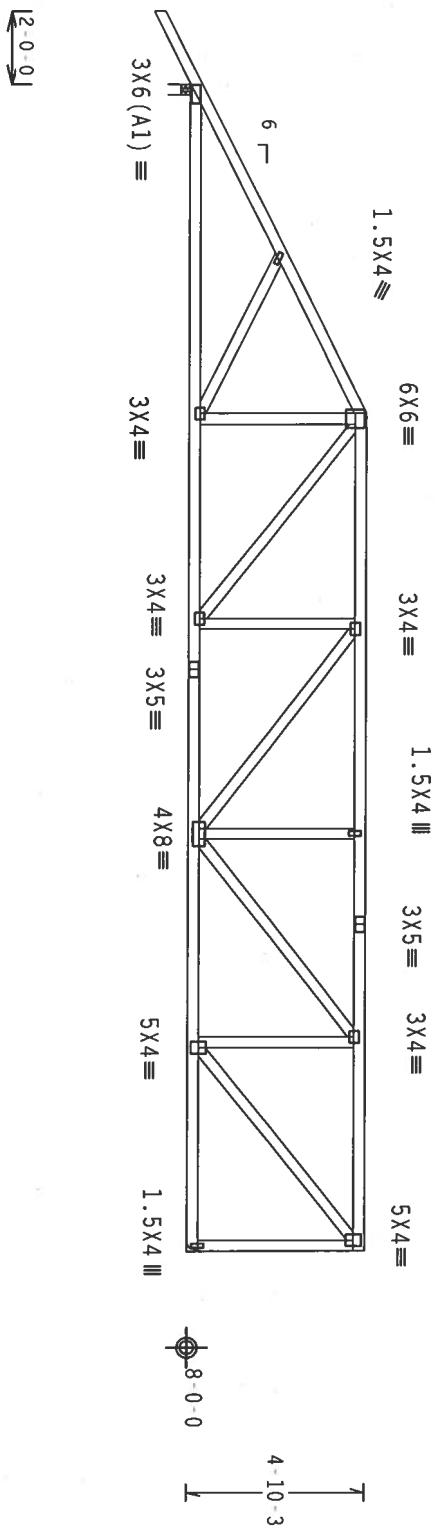
Top chord 2x4 SP #2 Dense  
Bot chord 2x4 SP #2 Dense  
Webs 2x4 SP #3

Wind reactions based on MWFRS pressures.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Right end vertical not exposed to wind pressure.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC  
Cq/RT=1.00(1.25)/10(0)

Nov 02 '06

FL/-/4/-/R/-

Scale = .1875"/ft.

\*\*WARNING\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST (BUILDING COMPONENT SAFETY TEST) INFORMATION, SUITE 312, ALEXANDRIA, VA 22314, AND WICK AND TRUSS COMPANY, 4300 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA 22314, FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

\*\*IMPORTANT\*\* FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE

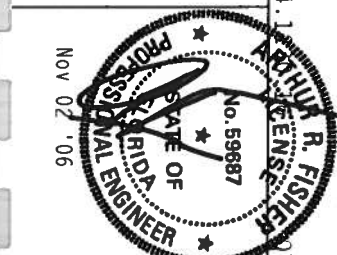
CONNECTION PLATES ARE MADE OF 20/18/16GA (U/H/SS/K) ASTM A653 GRADE 40/60 (U. K/H/SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2.

SEAL OF THE PROFESSIONAL ENGINEER RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SEAL IS THE PROPERTY OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

Alpine Engineered Products, Inc.  
1990 Marley Drive  
Haines City, FL 33844

License # 12000

Professional Engineer



TC LL	20.0 PSF	REF R487 -- 56336
TC DL	10.0 PSF	DATE 11/02/06
BC DL	10.0 PSF	DRW HCUSR487 06306010
BC LL	0.0 PSF	HC-ENG DAB/AF
TOT.LD.	40.0 PSF	SEQN- 36456
DUR.FAC.	1.25	
SPACING	24.0"	JRFF- 1T17487_201

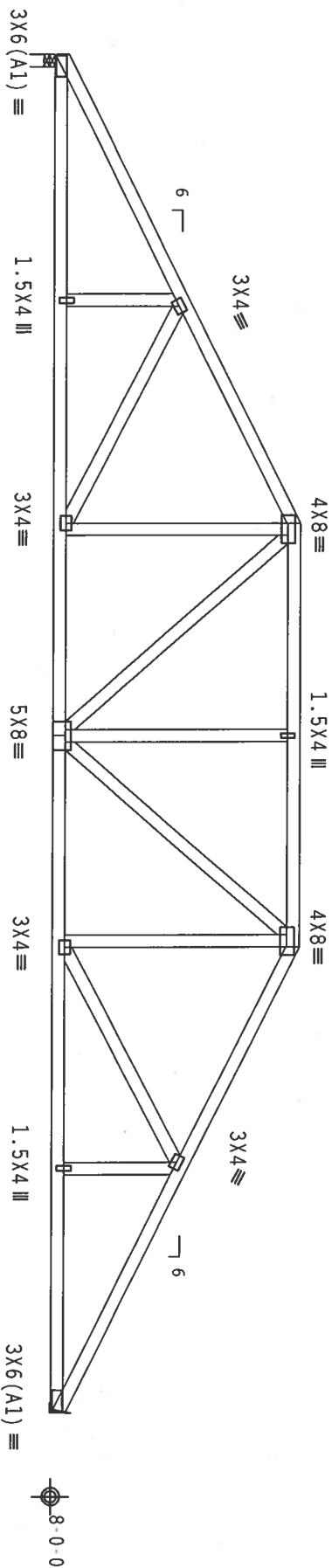


Top chord 2x4 SP #2 Dense  
Bot chord 2x4 SP #2 Dense  
Webs 2x4 SP #3

Wind reactions based on MMFRS pressures.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located  
within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind  
BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase  
factor for dead load is 1.50.



11'-0'-0  
10'-0'-0  
11'-0'-0  
32'-0'-0 Over 2 Supports  
R=1319 U=180 W=3.5"  
R=1317 U=180

PLT TYP. Wave

Design Cr1t: TPI-2002(STD)/FBC

Cq/RT=1.00(1.25)/10(0)

7.24

QTY:1 FL/-4/-/R/-

Scale = .25"/Ft.

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 (BUILDING COMPONENT SAFETY INFORMATION) PUBLISHED BY TPI TRUSS SYSTEMS, INC., 6300 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA. 22314, AND WICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY ASEP) AND TPI. ALPINE CONNECTION PLATES ARE MADE OF 20/18/16GA (W/N/SS) ASTM A653 GRADE 40/60 (W. K/H/SS) GALV. STEEL. APPLY ANY RESPECTIVE OF THIS DESIGN. UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROVISIONS FOR THE TRUSS COMPONENT. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ALPINE

Alpine Engineered Products, Inc.  
1950 Marley Drive  
Haines City, FL 33844



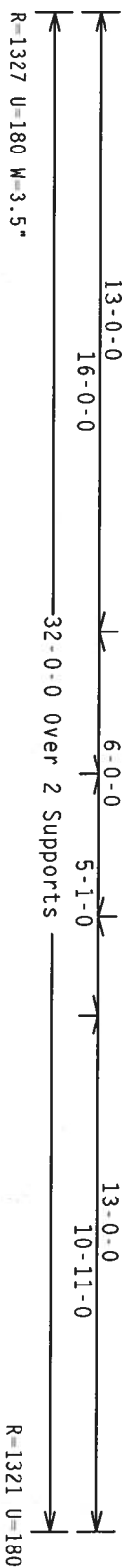
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TOT.LD.	40.0 PSF	SEQN-	36459	
DUR.FAC.	1.25			
SPACING	24.0"	QRFF	1T17487	201

МРК. ВЪЗМОЖНОСТ НА СЪВРЕМЕННИТЕ (ЛЪЗОВИ) КОМПЮТЕРИ ДА РАБОТЯТ СЪС СЪВРЕМЕННИ

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP 8, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Calculated horizontal deflection is 0.22" due to live load and 0.36" due to dead load.

due to dead load.



Scale = .25" / Ft.

BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

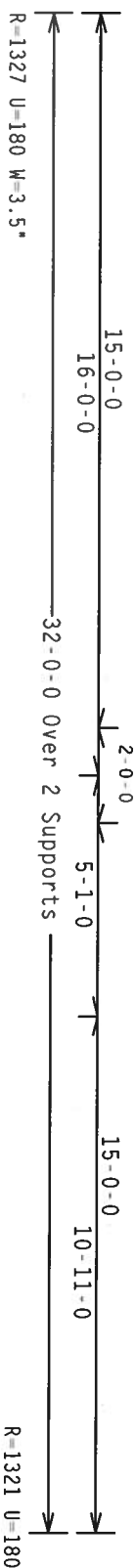
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BC DL	10.0 PSF	DRW	HCSR487 06306012
BC LL	0.0 PSF	HC-ENG	DAB/AF
TOT.LD.	40.0 PSF	SEQN -	36450
DUR.EAC.	1.25		
SPACING	24.0"	JRFF -	1T17487_201

THIS WORK PREPARED FROM COMPUTER INPUT (LVAUS & DIMENSIONS) SUBMITTED BY KRUSZ MFK.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Calculated horizontal deflection is 0.22" due to live load and 0.35" due to dead load.

Calculated horizontal deflection is 0.22" due to live load and 0.35" due to dead load.



Scale = .25" / Ft.

No. 59687

...A...  
...E...  
...N...



**UNITAL**



Nov 2 1965

90. ~~2002~~ 06

1501

1



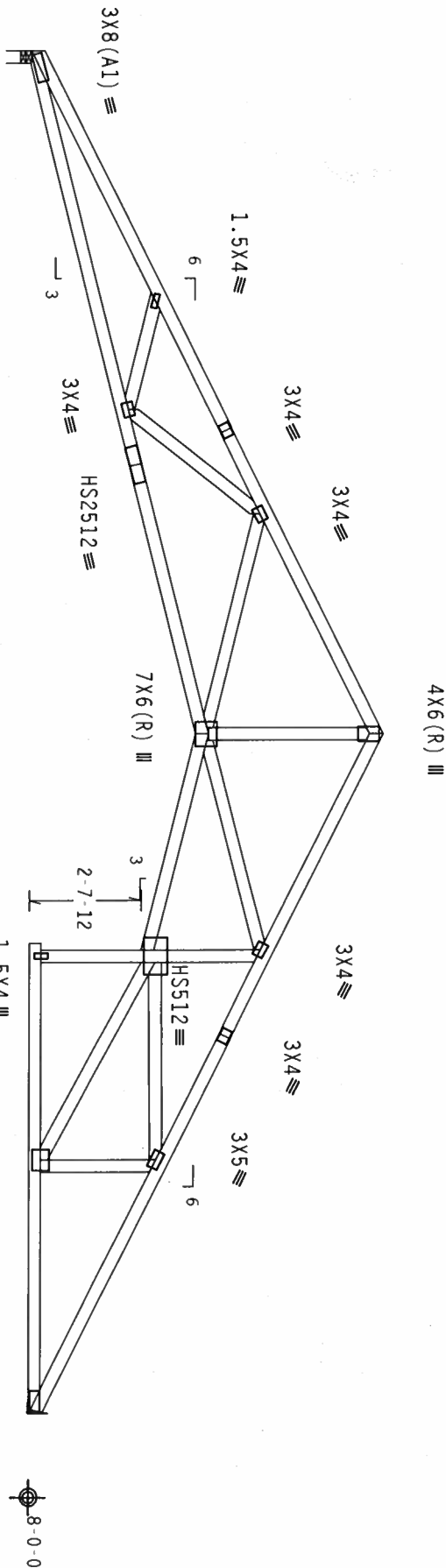
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TOT.LD.	40.0 PSF	SEON-	36476
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1TJ7487_201

Top chord 2x4 SP #2 Dense  
Bot chord 2x4 SP #2 Dense  
Webs 2x4 SP #3

Wind reactions based on MFERS pressures.  
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Calculated horizontal deflection is 0.22" due to live load and 0.35" due to dead load.



16-0-0  
16-0-0  
16-0-0  
32-0-0 Over 2 Supports  
R=1327 U=180 W=3.5"  
R=1321 U=180

PLT TYP. 20 Gauge HS, Wave

Design Crit: TPI-2002(STD)/FBC  
Cq/RT=1.00(1.25)/10(0)

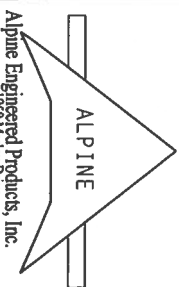
Nov 2 2006

TY: 3 FL/-/4/-/R/-

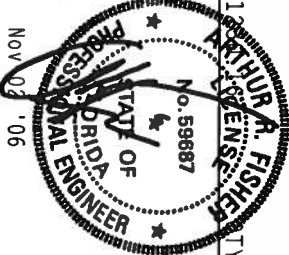
Scale = .25"/ft.

WARNING: TRUSSES REQUIRING EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. THE FOLLOWING INFORMATION, PUBLISHED BY THE TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, MOHAWK, NY 13529, FOR SAFETY PRACTICES PRIOR TO PERFORMANCE OF THE TRUSS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT: FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY ASEA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W/H/55/K) ASTM A653 GRADE 40/60 (W. K/H/55) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMER AS OF TPI-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



Alpine Engineered Products, Inc.  
1950 Marley Drive  
Haines City, FL 33844  
Certification #



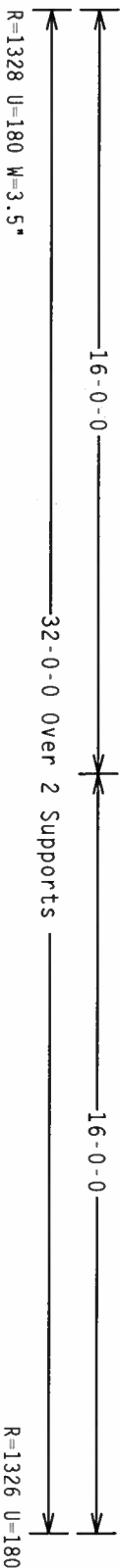
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BC DL	10.0 PSF	DRW HCUSR487 06306014
BC LL	0.0 PSF	HC-ENG DAB/AF
TOT.LD.	40.0 PSF	SEQN- 36431
DUR.FAC.	1.25	
SPACING	24.0"	JBEF- 1177487-201

THIS UMG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY IKUSS MFK.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

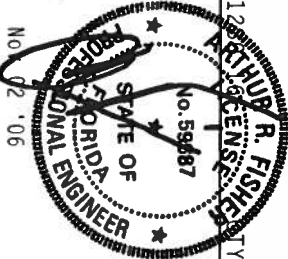
Calculated horizontal deflection is 0.26" due to live load and 0.41" due to dead load.

Calculated horizontal deflection is 0.26" due to live load and 0.41" due to dead load.



Scale = .25"/Ft.

1



TC LL	20.0 PSF	REF	R487 - 56341
TC DL	10.0 PSF	DATE	11/02/06
BC DL	10.0 PSF	DRW	HCUSR487 06306015
BC LL	0.0 PSF	HC-ENG	DAB/AF
TOT.LD.	40.0 PSF	SEQN -	36440
DUR.FAC.	1.25		
SPACING	24.0"	JREF -	1T1Z487_201

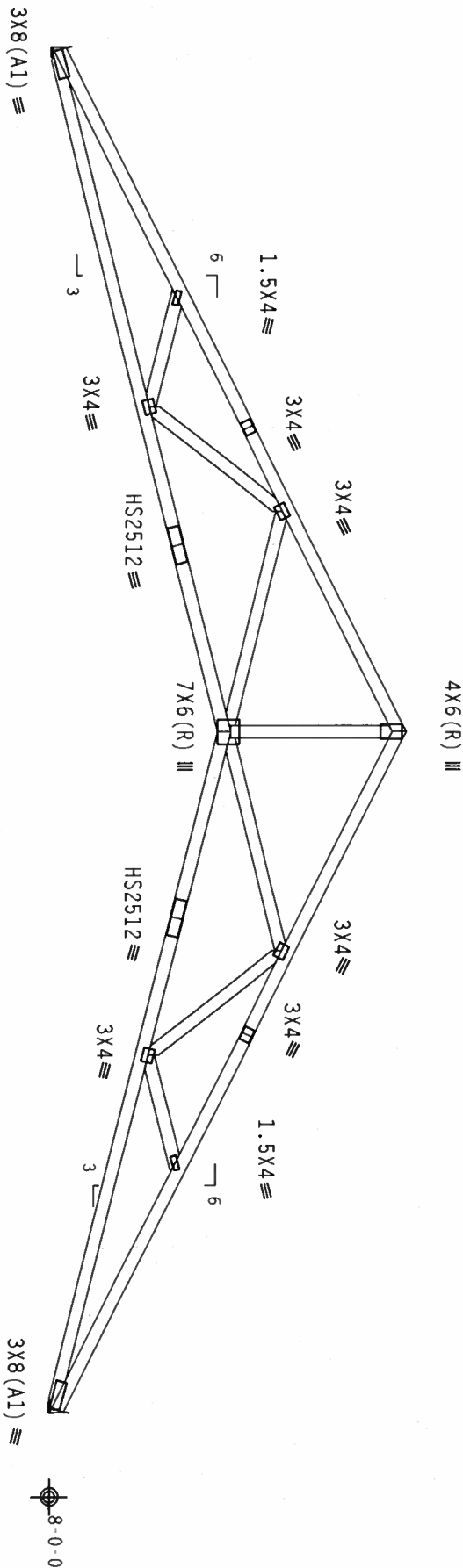
Top chord 2x4 SP #2 Dense  
Bot chord 2x4 SP #2 Dense  
Webs 2x4 SP #3

Wind reactions based on MMFRS pressures.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP. B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Calculated horizontal deflection is 0.26" due to live load and 0.41" due to dead load.



PLT TYP. 20 Gauge HS, Wave

Design Cmt: TPI-2002(STD)/FBC

Cq/RT=1.00(1.25)/10(0)

7.24.12

FL/-/4/-/-/R/-

Scale = .25"/ft.

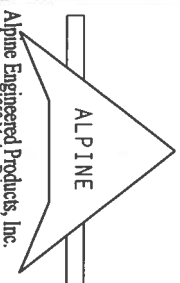
\*\*WARNING\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST PRACTICES FOR TRUSS FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. 1106 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA 22314 AND WICOMM TRUSS COMPANY, INC., 4300 ENTERPRISE LANE, WILMINGTON, DE 19806. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

\*\*IMPORTANT\*\* FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR.

ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NOS (NATIONAL DESIGN SPEC. BY AEPN) AND TPI.

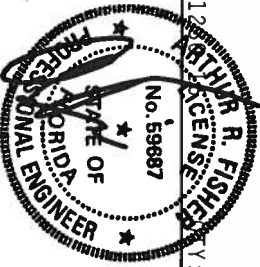
CONNECTOR PLATES ARE MADE OF 20/18/16GA (W/H/55/K) ASH 4653 GRADE 40/60 (W. K/H/55) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2.

AN INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMES AS OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES THE SUITABILITY OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/771.1 SEC. 2.



Alpine Engineered Products, Inc.  
1990 Marley Drive  
Haines City, FL 33844

License # 13344



TC LL	20.0 PSF	REF R487--	56342
TC DL	10.0 PSF	DATE	11/02/06
BC DL	10.0 PSF	DRW HCUSR487	06306016
BC LL	0.0 PSF	HC-ENG DAB/AF	
TOT.LD.	40.0 PSF	SEQN-	36479
DUR.FAC.	1.25		
SPACING	24.0"	JBFF-1177487-201	



110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Calculated horizontal deflection is 0.15" due to live load and 0.23" due to dead load.

Calculated horizontal deflection is 0.15" due to live load and 0.23" due to dead load.



R-1317 U-180

Design Crit: TPI-2002(STD)/FBC  
Cq/RT=1.00(1.25)

 $Cq/RT=1.00(1.25)/10(0)$ 

7.24.

PROPERTY: LICENSE

FL/-/4/-/-/R/-/

Scale = .1875"/Ft.

STATE OF  
N. 59687

**\*\*IMPORTANT\*\***  
FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR.

Alpine Engineered Products, Inc.

1950 Marley Drive  
Haines City, FL 33844

ization # 5

[illegible]

1

SPACING 24.0"

JREF - IT17487\_Z01



Calculated horizontal deflection is 0.19" due to live load and 0.29" due to dead load.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED brdg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Wind reactions based on MMFRS pressures.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



R=1317 U=180

Design Crit: TPI-2002(STD)/FBC

$$C_q/RT=1.00(1.25)/10(0)$$

7.24.13

FL/4/-/R/

Scale = .25" / Ft.

\*\*WARNING\*\* THESE REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING  
 REFER TO GC51 (BUILDING COMPONENT SAFETY INFORMATION) - PUBLISHED BY IP1 (TRUSS PLATE INSTITUTE), 218  
 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA 22314, AND WCA (WOOD ROSS COUNCIL OF AMERICA), 6300  
 ENTERPRISE LANE, MOHAWK, NY 13119 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS  
 OTHERWISE INDICATED THE CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE  
 PROPERLY ATTACHED RIGID CEILING.

\*\*\* IMPORTANT \*\*\*  
FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF MSDS (NATIONAL DESIGN SPEC BY AREA) AND TO CROSS IN COMPLIANCE WITH 1P1: OR FABRICATING, HANDLING, SHIPPING, (INSTALLING & BRACING

CONNECTOR PLATES ARE MADE OF 20/18/16GA (M H/SS/K) ASTM A653 GRADE 40/60 (M K/M SS) GALV STEEL APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC, B1 AFAPA) AND IPI. ALLPINE


PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-7

ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TP11-2002 SEC.3. A SEAL ON THIS

DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT

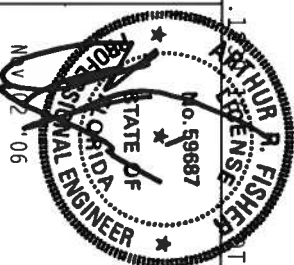
**DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE**

**BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.**



1950 Mulvey Drive  
Haines City, FL 33844

Trufide Station # \_\_\_\_\_



TC LL	20.0 PSF	REF	R487 - 56345
TC DL	10.0 PSF	DATE	11/02/06
BC DL	10.0 PSF	DRW	HCUSR487 06306019
BC LL	0.0 PSF	HC-ENG	DAB/AF
TOT.LD.	40.0 PSF	SEQN-	36482
DUR.FAC.	1.25		
SPACING	24.0"	JBFF -	1T1Z487_201

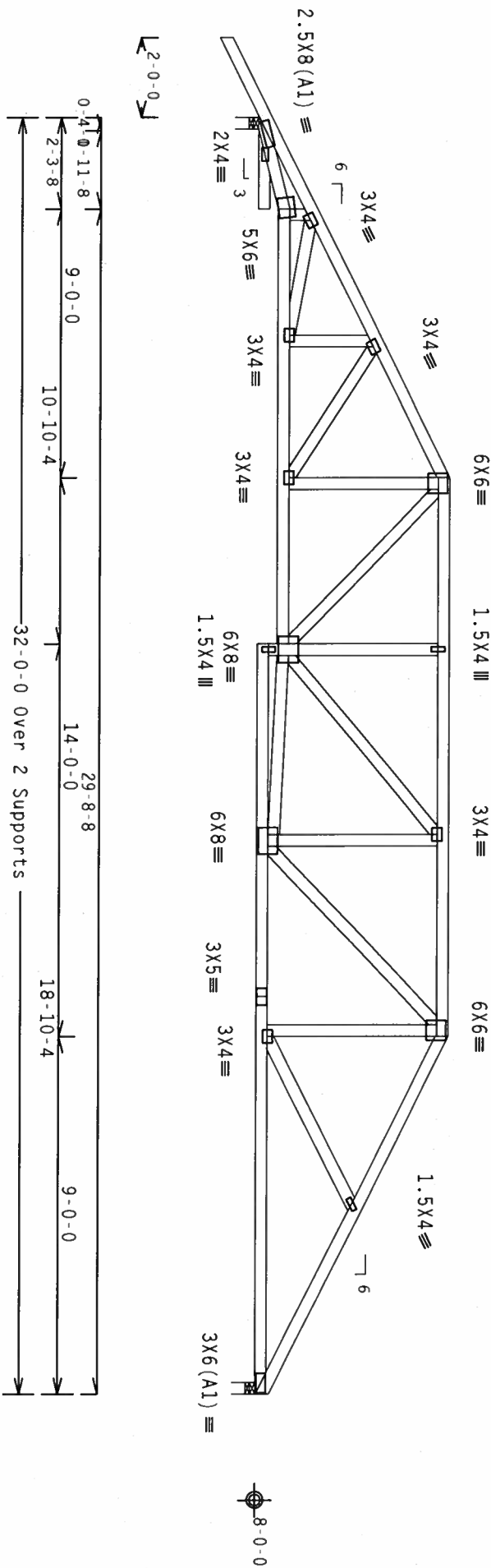
Top chord 2x4 SP #2 Dense  
Bot chord 2x4 SP #2 Dense  
Webs 2x4 SP #3  
Filler 2x4 SP #3

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

See DWGS BCFILLER1103 for Bottom Chord filler details.  
Laterally brace BC above filler @24" O.C.  
INCLUDING A LATERAL BRACE AT CHORD ENDS.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Wind reactions based on MMFRS pressures.



R=1458 U=180 W=3.5"

R=1313 U=180 W=3.5"

PLT TYP. Wave

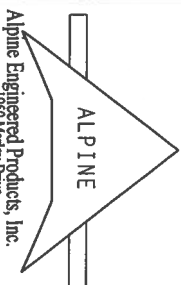
Design Crit: TPI-2002(STD)/FBC  
Cq/RT=1.00(1.25)/10(0) 7.24.1

FL/-/4/-/R/-

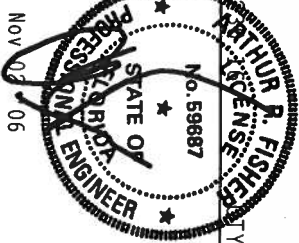
Scale = .25"/ft.

\*\*WARNING\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSTI (BUILDING COMPONENT SAFETY INFORMATION) FOR THE LATEST EDITION OF THE 6300 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA 22314, AND WICA (WOOD TRUSS CONNECT) UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

\*\*IMPORTANT\*\* FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AEP) AND TPI. ALPINE PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 1604.2. MINIMUM INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI 1-2002 SEC.3. A SEAL ON THIS DESIGN SHOWS THE SUITABILITY OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



Alpine Engineered Products, Inc.  
1950 Marley Drive  
Haines City, FL 33844  
Certificate # 24000



TC LL	20.0 PSF	REF R487-- 56346
TC DL	10.0 PSF	DATE 11/02/06
BC DL	10.0 PSF	DRW HCUSR487 06306020
BC LL	0.0 PSF	HC-ENG DAB/AF
TOT.LD.	40.0 PSF	SEQN- 36398
DUR.FAC.	1.25	
SPACING	24.0"	JRFF- 1T17487_201

Top chord 2x4 SP #2 Dense  
Bot chord 2x4 SP #2 Dense  
Webs 2x4 SP #3 : W8 2x4 SP #2 Dense:  
Filler 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Wind reactions based on MWFRS pressures.

See DWGS BC FILLER 1103 for Bottom Chord filler details.  
Laterally brace BC above filler @24" O.C.  
INCLUDING A LATERAL BRACE AT CHORD ENDS.

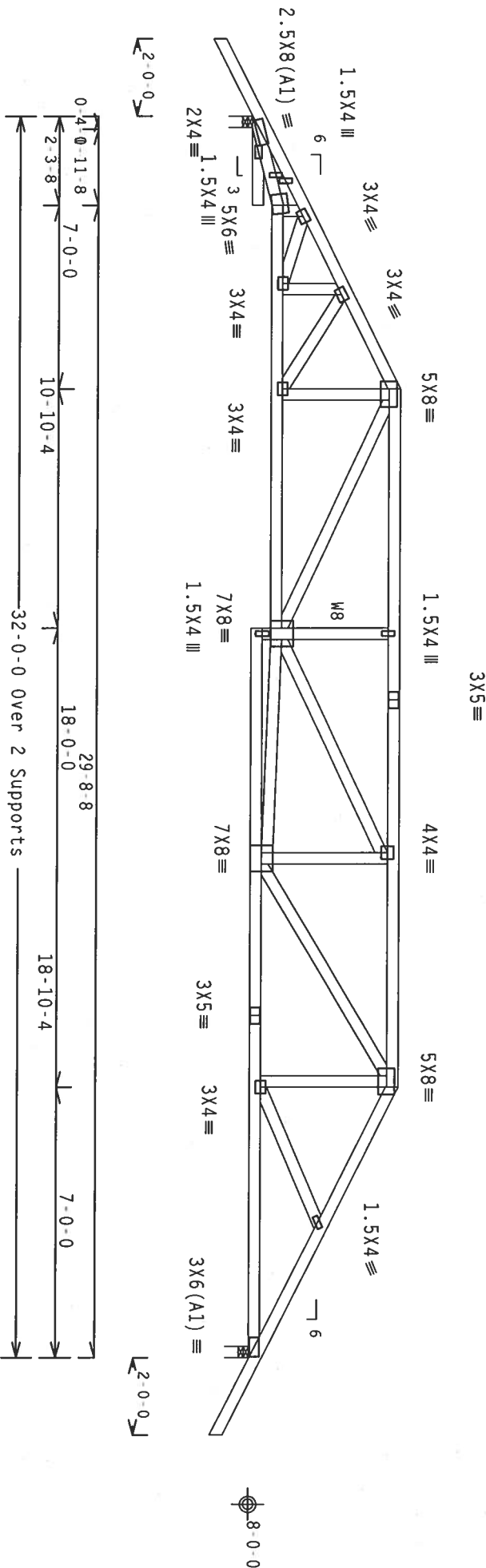
## 2 COMPLETE TRUSSES REQUIRED

Nailing Schedule: (10d Common (0.148"x3", min.), nails)

Top Chord: 1 Row @12.00" O.C.  
Bot Chord: 1 Row @12.00" O.C.  
Webs : 1 Row @ 4" O.C.  
Use equal spacing between rows and stagger nails in each row to avoid splitting.

#1 hip supports 7'-0" jacks with no webs.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



R=2720 U-246 W=3.5"

R=2720 U-246 W=3.5"

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC  
Cq/RT=1.00(1.25)/10(0)

QTY: 1 FL/-/4/-/R/-

Scale = .25"/ft.

\*\*WARNING\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 (BUILDING COMPONENTS) PART 1, (FORMS 22314), AND W800 TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA 22304, FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

\*\*IMPORTANT\*\* FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR.

PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTIONS ARE MADE OF 20/18/16GA (W/H/SS/K) ASTM A653 GRADE 40/60 (W. K/H/SS) GALV. STEEL. APPLY PLATES EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z.

CONNECTIONS OF PLATES LOCATED BY (1) SHALL BE PER DIMENSIONS AS OF TPI1 2002 SEC.3. A SEAL ON THIS DRAWING INDICATE THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

Alpine Engineered Products, Inc.  
Haines City, FL 33844  
Certificate # 24000 # 11

ALPINE



TC LL	20.0 PSF	REF	R487--	56347
TC DL	10.0 PSF	DATE	11/02/06	
BC DL	10.0 PSF	DRW	HCUSR487	06306021
BC LL	0.0 PSF	HC-ENG	DAB/AF	
TOT.LD.	40.0 PSF	SEQN-	36419	REV
DUR.FAC.	1.25			
SPACING	24.0"	QRFF-	1T17487_201	

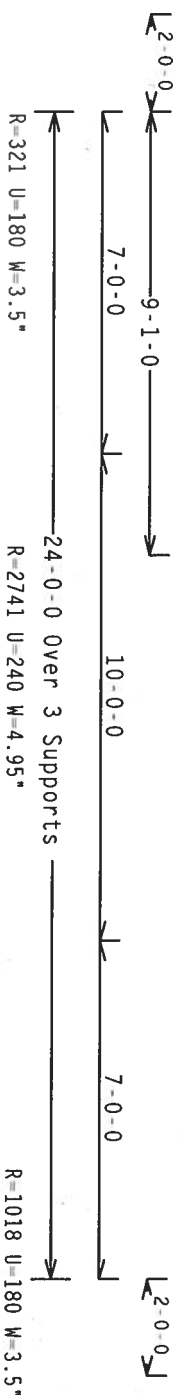
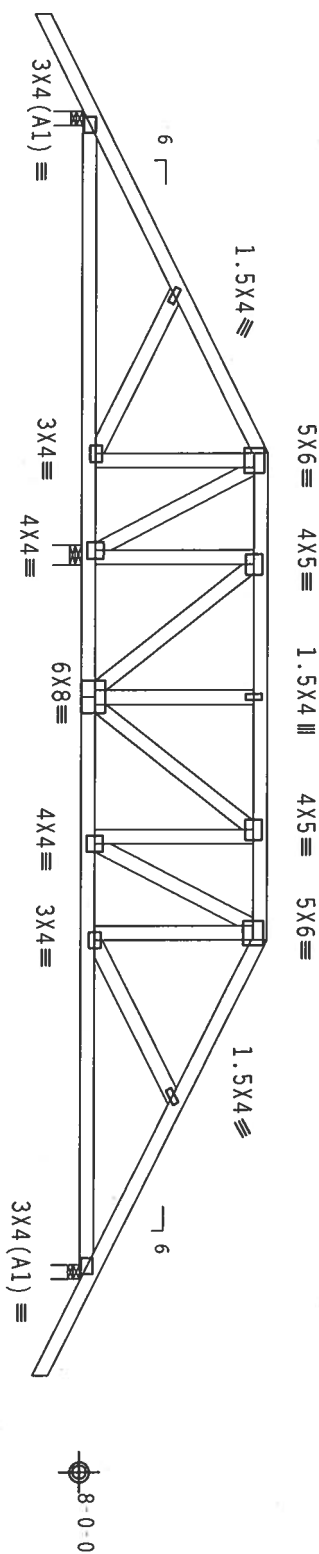
Top chord 2x4 SP #2 Dense  
Bot chord 2x4 SP #2 Dense  
Webs 2x4 SP #3

Wind reactions based on MMFRS pressures.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

#1 hip supports 7-0-0 jacks with no webs.



PLT TYP. Wave

Design Cmt: TPI-2002(STD)/FBC  
Cq/RT=1.00(1.25)/10(0)

QUANTITY: 1 FL/-/4/-/1/R/-

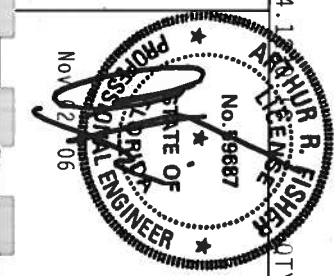
Scale = .25"/ft.

\*\*\*WARNING\*\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFERENCE TO THE TPI-2002 (STD) TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 300, MILWAUKEE, WI 53219, FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

\*\*\*IMPORTANT\*\*\* FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI-2002 (STD) TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 300, MILWAUKEE, WI 53219, FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

ALPINE

Alpine Engineered Products, Inc.  
1950 Marley Drive  
Haines City, FL 33844  
Telephone # 888-333-3333



TC LL	20.0 PSF	REF	R487--	56348
TC DL	10.0 PSF	DATE	11/02/06	
BC DL	10.0 PSF	DRW	HCUSR487	06306022
BC LL	0.0 PSF	HC-ENG	DAB/AF	
TOT.LD.	40.0 PSF	SEQN-	36427	
DUR.FAC.	1.25			
SPACING	24.0"			

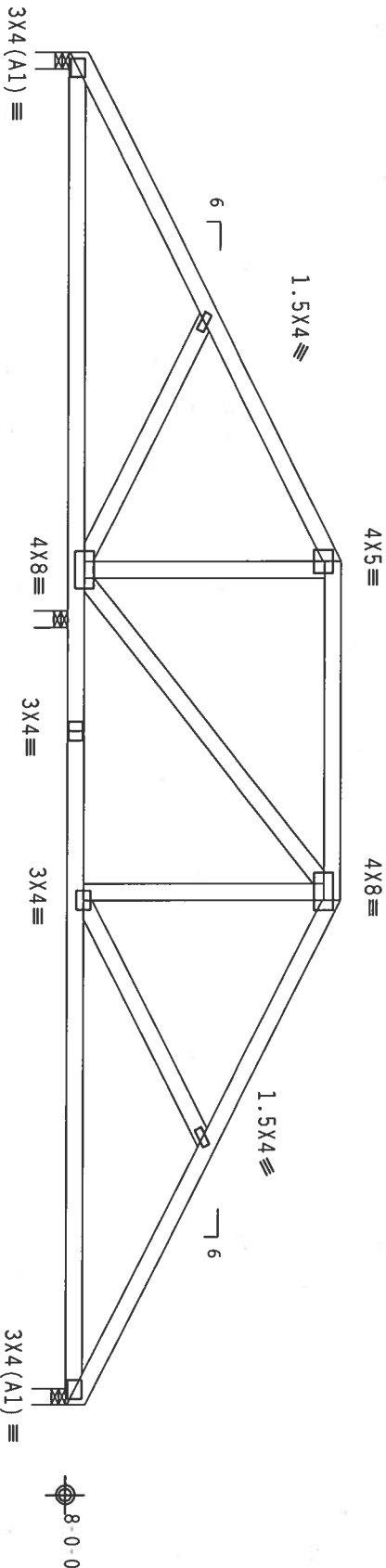
DRWF- 1117487\_201

Top chord 2x4 SP #2 Dense  
Bot chord 2x4 SP #2 Dense  
Webs 2x4 SP #3

Wind reactions based on MMFRS pressures.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



10'-0'-4" 9'-0'-0" 6'-0'-0" 9'-0'-0" 24'-0'-0" Over 3 Supports  
R-773 U=180 W=3.5" R-369 U=180 W=3.5" R-835 U=180 W=3.5"

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC  
Cq/RT=1.00(1.25)/10(0)

7.24.1

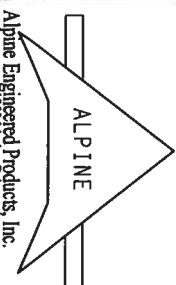
FL/-/4/-/-/R/-

Scale = .3125"/ft.

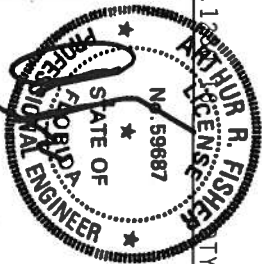
**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. (GULF COAST COMPONENTS, INC., 2100 NORTH LEE STREET, SUITE 100, TAMPA, FL 33604) (GULF COAST COMPONENTS, INC., 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR.

ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIA/PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 2018/1664 (W/H/55/K) ASTM A653 GRADE 40/60 (W. K/H/55) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DESIGN INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT BUILDING DESIGNER PER ANNEX 1 SEC. 2.

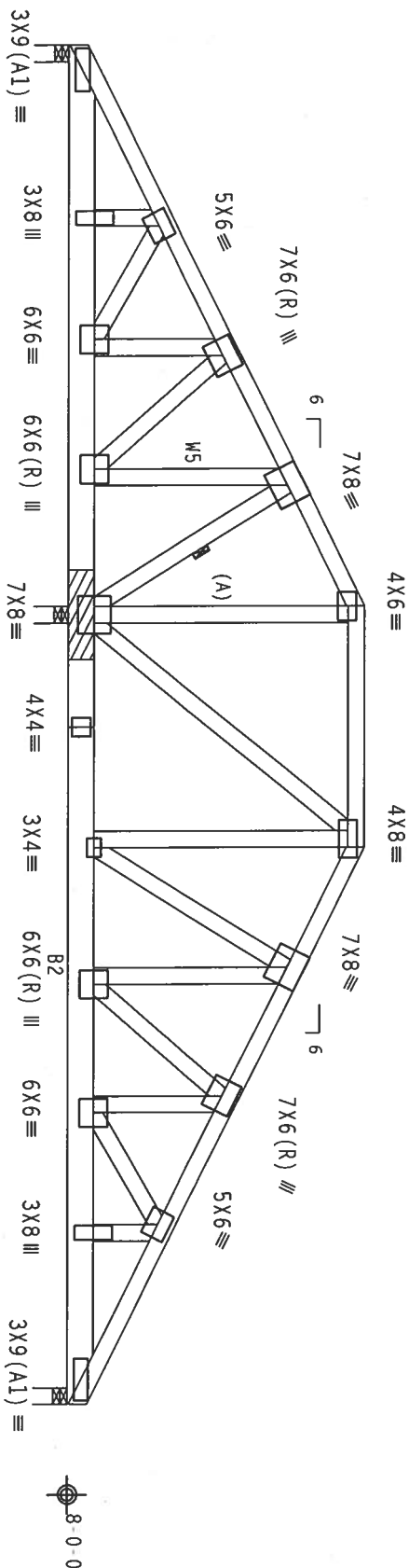


Alpine Engineered Products, Inc.  
1990 Marley Drive  
Haines City, FL 33944  
Certificate #



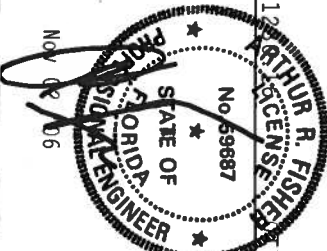
TC LL	20.0 PSF	REF	R487--	56349
TC DL	10.0 PSF	DATE	11/02/06	
BC DL	10.0 PSF	DRW	HCUSR487	06306023
BC LL	0.0 PSF	HC-ENG	DAB/AF	
TOT.LD.	40.0 PSF	SEQN-	36396	
DUR.FAC.	1.25			
SPACING	24.0"			
		JBEE-1117487-201		

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

 $R = 91 \text{ U} = 180 \text{ W} = 3.5^{\circ}$ 

Scale = .3125"/Ft.

## Conclusion



TC LL	20.0 PSF	REF	R487 - 56350
TC DL	10.0 PSF	DATE	11/02/06
BC DL	10.0 PSF	DRW	HCUSR487 06306024
BC LL	0.0 PSF	HC-ENG	DAB/AF
TOT.LD.	40.0 PSF	SEQN -	36487
DUR.FAC.	1.25		
SPACING	24.0"	JRFF -	1T17487_201

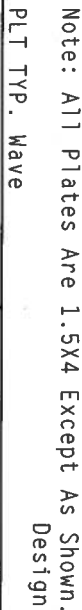
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 6.06 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Wind reactions based on MMFRS pressures.

Dead loads are stated on projected horizontal area basis.

Truss spaced at 24.0" OC designed to support 1-4-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

Deflection meets  $L/360$  live and  $L/240$  total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC  
Cq/RT=1.00(1.25)

TY:1 FL/-/4/-/4/R/-

Scale = .25" / Ft.

**\*\*WARNING\*\***  
 ISSUES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, AND BRACING  
 REFER TO GC51 (BUILDING COMPONENT SAFETY INFORMATION). PUBLISHED BY IPT (STEEL PAPER INSTITUTE, 218  
 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA 22314) AND AISC (AMERICAN INSTITUTE OF STEEL CONSTRUCTION, INC., 500  
 ENTERPRISE LANE, MOISTON, IL 61859) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS  
 OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE  
 PROPERLY ATTACHED RIGID CEILING.

FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR

TRUSS IN CONFORMANCE WITH TPI, OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&A) AND TPI. CONNECTOR PLATES ARE MADE OF 304/316 STAINLESS STEEL. ALL STEEL PARTS ARE ALUMINUM.

ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TP11-2002 SEC.3. A SEAL ON THIS

DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT

DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT

DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT IS THE RESPONSIBILITY OF THE DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT

DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE

DESIGN SHOWN. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC 2

BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

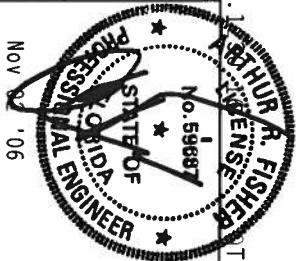
BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

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# THE

1950 Marley Drive  
Haines City, FL 33844  
Certificate  
Registration # 5



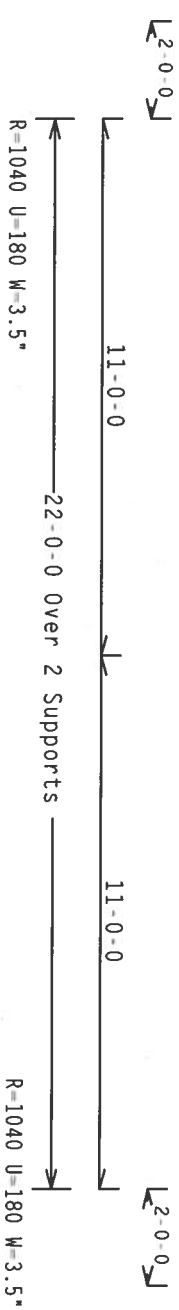
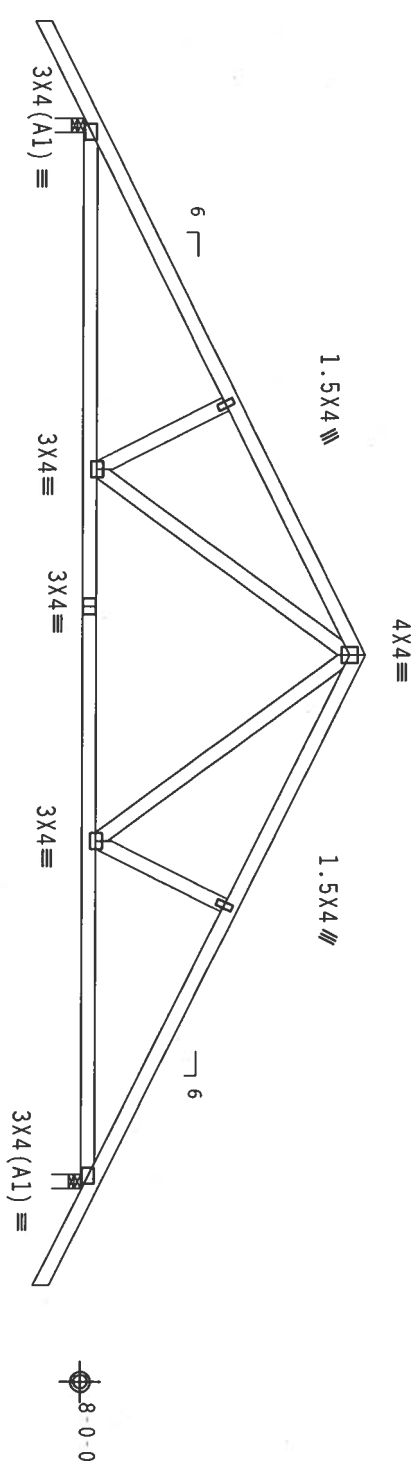
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TC DL	10.0 PSF	DATE	11/02/06
BC DL	10.0 PSF	DRW	HCUSR487 06306025
BC LL	0.0 PSF	HC-ENG	DAB/AF
TOT.LD.	40.0 PSF	SEQN -	36395 REV
DUR.FAC.	1.25		
SPACING	24.0"	JREF -	1T1Z487 201

Top chord 2x4 SP #2 Dense  
Bot chord 2x4 SP #2 Dense  
Webs 2x4 SP #3

Wind reactions based on MFRS pressures.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located  
anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0  
psf.

Deflection meets L/360 live and L/240 total load. Creep increase  
factor for dead load is 1.50.



PLT TYP. Wave

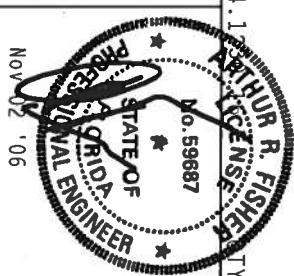
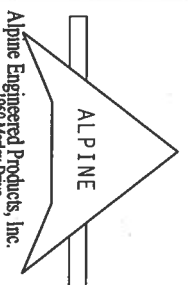
Design Crit: TP1-2002(STD)/FBC  
Cq/RT=1.00(1.25)/10(0)

7.24.11

Scale = .25\"/>

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO SPEC 1000 FOR CONSTRUCTION DETAILS. THE TRUSS PLATE INSTALLATION, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA 22314, AND AT LEAST TWO THUS THESE FUNCTIONS, UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TP1 OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TP1. ALPINE CONNECTION PLATES ARE MADE OF 2010/1604 (W.H/55/7) ASTM A553 GRADE 40/60 (W. K/H.55) GALV. STEEL. APPLY ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEK 43 OF TP11-2002 SEC.3. A SEAL ON THIS DESIGN SHOWS ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT BUILDING DESIGNER PER ANSI/TP1 1 SEC. 2.



TC LL	20.0 PSF	REF	R487--	56352
TC DL	10.0 PSF	DATE	11/02/06	
BC DL	10.0 PSF	DRW	HCUSR487	06306001
BC LL	0.0 PSF	HC-ENG	DAB/AF	*
TOT. LD.	40.0 PSF	SEQN-	36401	
DUR. FAC.	1.25			
SPACING	24.0"			

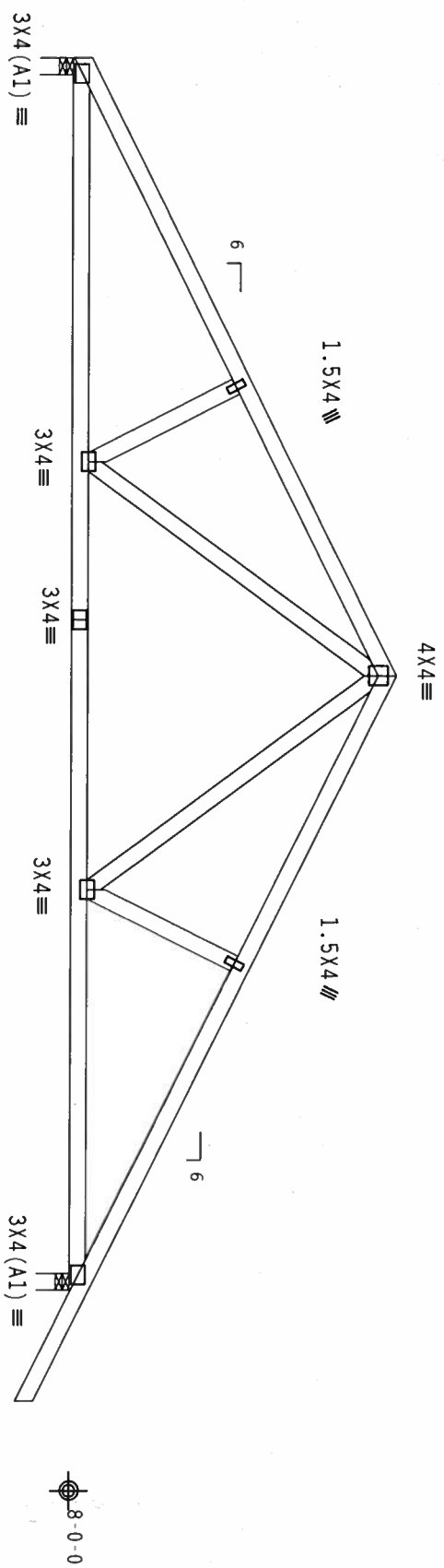


Top chord 2x4 SP #2 Dense  
Bot chord 2x4 SP #2 Dense  
Webs 2x4 SP #3

Wind reactions based on MFRS pressures.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



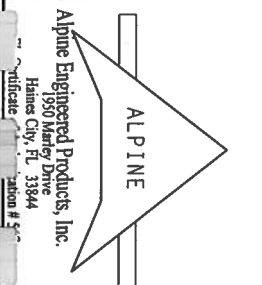
PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC  
Cq/RT=1.00(1.25)/10(0)



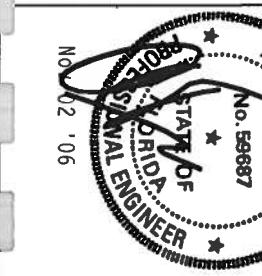
FL / 4 / - / - / R / -

Scale = .3125" / Ft.



**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO THE MANUFACTURER'S INSTRUCTIONS FOR THE PROPER HANDLING AND BRACING OF THE TRUSSES. THE TRUSSES ARE NOT TO BE USED IN ANY MANNER OTHER THAN THAT SPECIFIED IN THE INSTRUCTIONS. THE TRUSSES ARE NOT TO BE USED IN ANY MANNER OTHER THAN THAT SPECIFIED IN THE INSTRUCTIONS. THE TRUSSES ARE NOT TO BE USED IN ANY MANNER OTHER THAN THAT SPECIFIED IN THE INSTRUCTIONS.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIA/ASA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W/N/S/V) ASTM A553 GRADE 40/60 (W, K/H, SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEK AS OF TPI-11-2002 SEC.3. A SEAL ON THIS DESIGN SHOWS THE SIGNATURE OF THE DESIGNER. THE SEAL IS THE PROPERTY OF ALPINE ENGINEERED PRODUCTS, INC. THE SEAL IS THE PROPERTY OF ALPINE ENGINEERED PRODUCTS, INC. THE SEAL IS THE PROPERTY OF ALPINE ENGINEERED PRODUCTS, INC.



TC LL	20.0 PSF	REF R487 - 56353
TC DL	10.0 PSF	DATE 11/02/06
BC DL	10.0 PSF	DRW HCUSR487 06306002
BC LL	0.0 PSF	HC-ENG DAB/AF *
TOT. LD.	40.0 PSF	SEQN - 36399
DUR. FAC.	1.25	
SPACING	24.0"	
DRWF	11/7487_201	

## 2 COMPLETE TRUSSES REQUIRED

Bot Chord: I Row @ 3.50" o.c.

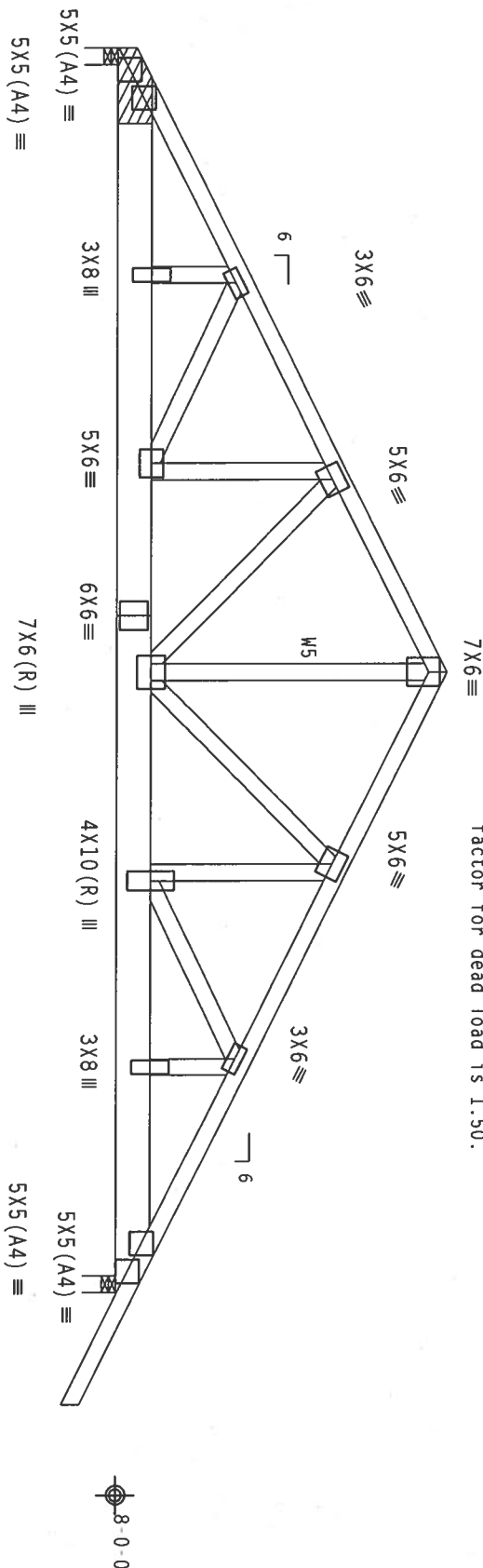
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Bearing bckcks: Nail type: 10d Common (0.448"x3", min.) nail
BRG X-LOC #BLOCKS LENGTH/BLK #NAILS/BLK WALL PLATE
1 0.000" 1 16" 17 Match Truss
Bearing block to be same size and species as bottom chord.
Refer to drawing CNBRGBLK1103 for additional information.

Wind reactions based on MMFRS pressures.

```

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



$R=8118$  U=728 W=3.5"  
 11-0-0  
 22-0-0 Over 2 Supports  
 11-0-0  
 $R=5797$  U=552 W=3.5"  
 2-0-0

Design Crit: TPI-2002(STD)/FBC  
Cq/RT=1.00(1.25)

7.24.1 QTY:1 FL/-/4/-/R/-

Scale = .3125"/Ft.

\*\*WARNING\*\*  
 BUILDERS REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING  
 (BULBOSOME COMPONENT SAVE INFORMATION). PUBLISHED BY IPI (TRUSS PAPER INSTITUTE, 218  
 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WICA (WOOD ROSS COUNCIL OF AMERICA, 6300  
 ENTERPRISE LANE, MAJORS, IN 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS  
 OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE  
 PROPERLY ATTACHED RIDGE CEILING.


**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR.

TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING TRUSS CONFORMANCE WITH ADDITIONAL PROVISIONS OF THE NATIONAL DESIGN CODE BY STANDARD AND FOR

PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN POSITION PER DRAWINGS 160A-2 CONNECTION PLATES WERE MADE OF 2010/1800A (W. H. 55/K) ASHIN A653 GRADE 40/60 (W. K/H. 55) GALV. STEEL. APPLY

DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT

BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

[illegible]

**ALPINE**

**Alpine Engineered Products, Inc.**  
 1950 Marler Drive  
 Gaines City, FL 33844  
 Telephone # 508-885-1111

TC LL	20.0 PSF	REF	R487 - 56354
TC DL	10.0 PSF	DATE	11/02/06
BC DL	10.0 PSF	DRW	HCUSR487 06306026
BC LL	0.0 PSF	HC-ENG	DAB/AF
TOT.LD.	40.0 PSF	SEQN -	36436
DUR.FAC.	1.25		
SPACING	24.0"	JRFF -	1117487 201

Top chord 2x4 SP #2 Dense  
Bot chord 2x4 SP #2 Dense  
Webs 2x4 SP #3

SPECIAL LOADS

(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)  
TC - From 84 PLF at 9.15 to 84 PLF at 18.29  
BC - From 4 PLF at 2.00 to 4 PLF at 0.00  
BC - From 20 PLF at 0.00 to 20 PLF at 10.00  
BC - From 20 PLF at 10.00 to 20 PLF at 18.29

See DWGS A11015EE0405 & GBLLETIN0405 for more requirements.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

+ Member to be laterally braced for horizontal wind loads.  
Bracing system to be designed and furnished by others.

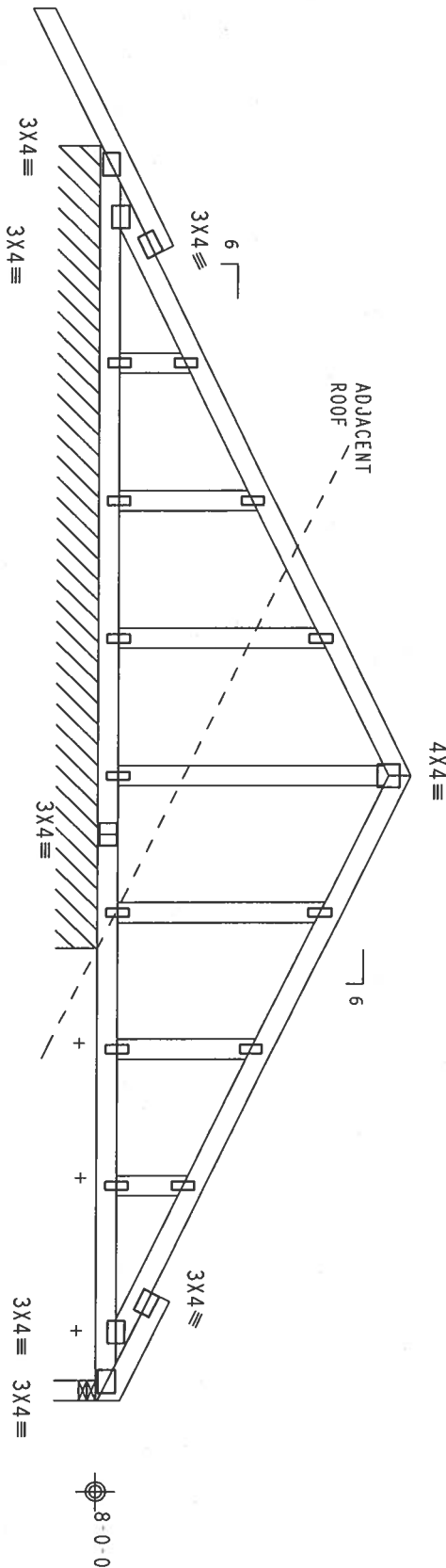
110 mph wind, 15.00 ft mean hgt. ASCE 7-02, CLOSED bldg, not located within 6.06 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Wind reactions based on MMFRS pressures.

Dead loads are stated on projected horizontal area basis.

Truss spaced at 24.0" OC designed to support 1-4-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

The building designer is responsible for the design of the roof and ceiling diaphragms, gable end shear walls, and supporting shear walls. Shear walls must provide continuous lateral restraint to the gable end. All connections to be designed by the building designer.



2-0-0-0  
0-6-81-0-7  
7-6-13  
18-3-8 Over 2 Supports  
7-6-13  
1-0-70-6-8  
R-176 PLF U=47 PLF W=11-8-0  
R-269 U=180 W=3.5

Note: All Plates Are 1.5X4 Except As Shown.

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC  
Cq/RT=1.00(1.25)/10(0)

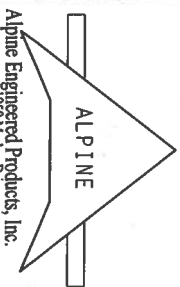
7.24.1

FL/-/4/-/R/-

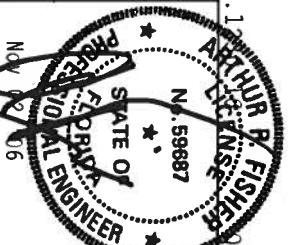
Scale = .375"/ft.

\*\*WARNING\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSTI (BUILDING COMPONENTS TESTING INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA 22304) AND WOOD JOINT TRUSS DESIGN SPECIFICATION (WJTD), UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

\*\*IMPORTANT\*\* FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY ASAP) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (N/A/55/K) ASTM A653 GRADE 40/60 (N. K/H/55) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A, Z. THE POSITION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI 2002 SEC.3. A SEAL ON THIS DRAWING INDICATES THE SEALANT OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOCIETY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SEALANT OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TP1 1 SEC. 2.



Alpine Engineered Products, Inc.  
1950 Marney Drive  
Haines City, FL 33844  
Telephone # 888-333-3333



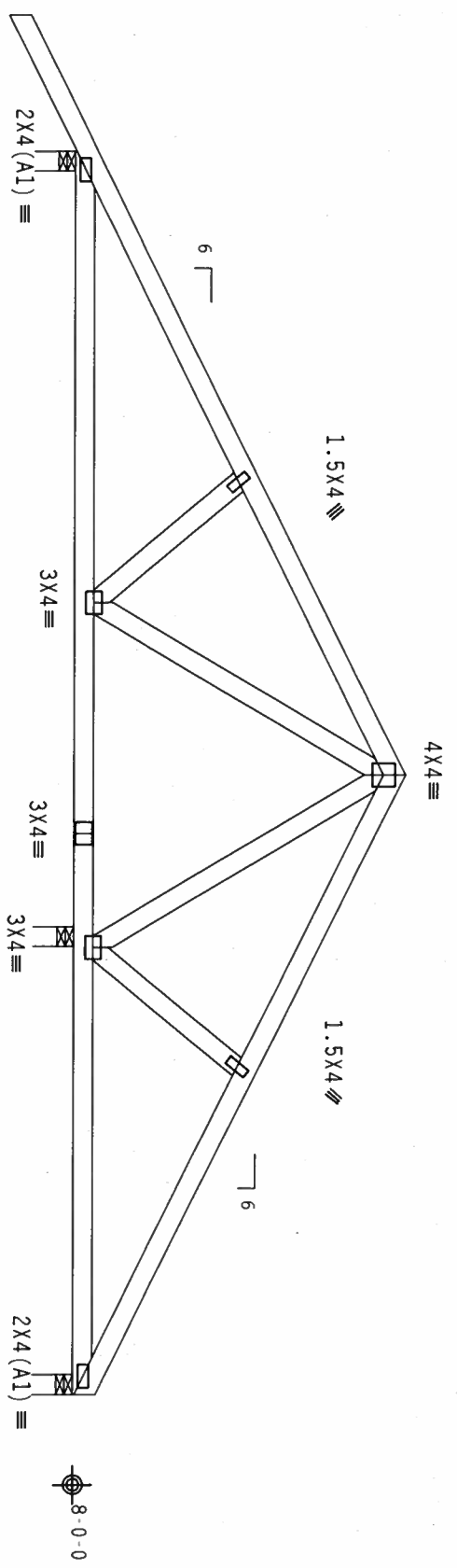
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TC DL	10.0 PSF	DATE	11/02/06	
BC DL	10.0 PSF	DRW	HCUSR487	06306027
BC LL	0.0 PSF	HC-ENG	DAB/AF	
TOT.LD.	40.0 PSF	SEQN-	36408	REV
DUR.FAC.	1.25			
SPACING	24.0"	JRFF-	1T17487_201	

Top chord 2x4 SP #2 Dense  
Bot chord 2x4 SP #2 Dense  
Webs 2x4 SP #3

Wind reactions based on MWFRS pressures.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



2-0-0  
9-1-12 11-6-4 9-1-12  
18-3-8 Over 3 Supports  
R=636 U=180 W=3.5"  
R=705 U=180 W=3.5"  
R=299 U=180 W=3.5"

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC

Cq/RT=1.00(1.25)/10(0)

7.24.1

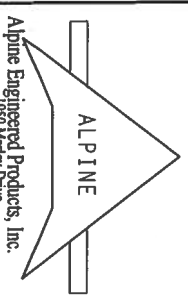
FL/-/4/-/R/-

Scale = .375"/ft.

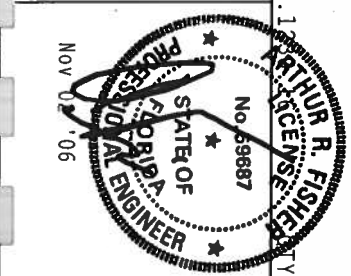
\*\*WARNING\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSTI (BUILDING COMPONENT SAFETY INFORMATION) PUBLISHED BY THE NATIONAL TRUSS ASSOCIATION, 6300 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA 22314, AND WICKA (WOOD TRUSS CONNECT) PUBLISHED BY THE NATIONAL TRUSS ASSOCIATION, 6300 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA 22314, FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

\*\*IMPORTANT\*\* FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AREA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W/H/55/X) ASTM A653 GRADE 40/60 (W, K/H/55) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2.

INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AREA AS OF TPI-1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES THE SUITABILITY OF THE DESIGN. THE SEAL IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/771.1 SEC. 2.



Alpine Engineered Products, Inc.  
Haines City, FL 33844  
1990 Marley Drive  
Certificate of Registration # 1111

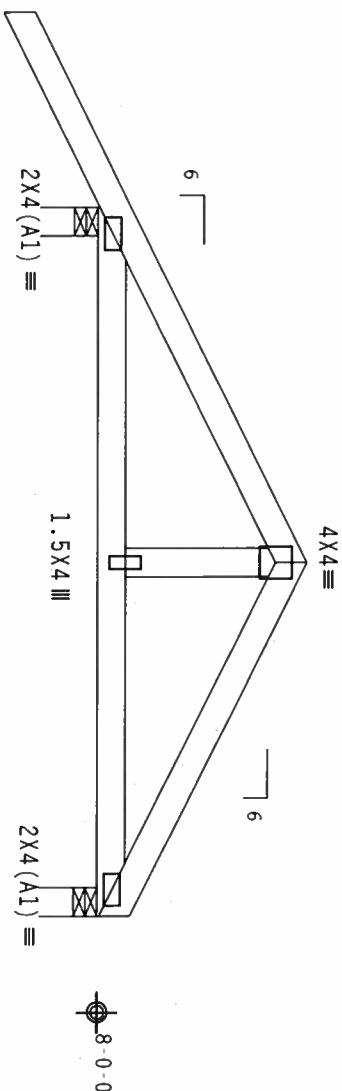


TC LL	20.0 PSF	REF	R487--	56356
TC DL	10.0 PSF	DATE	11/02/06	
BC DL	10.0 PSF	DRW	HCUSR487	06306028
BC LL	0.0 PSF	HC-ENG	DAB/AF	
TOT.LD.	40.0 PSF	SEON-	36414	
DUR.FAC.	1.25			
SPACING	24.0"	JRFF-	1T17487	201



110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



2-0-0

3-7-12

3-7-12

R=456 U=180 W=3.5"

R=278 U=180 W=3.5

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC

$$Cq/RT=1.00(1.25)/10(0)$$

7.24.1

QTY:1 FL/-/4/-/-/R/-

Scale = .5"/Ft.

\*\*\*WARNING\*\*\* THESE REQUIRE EXTREME CARE IN PARTICULATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO GC#1 (BUILDING COMPONENT SPECIFICATIONS INFORMATION), PUBLISHED BY TPI, TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND WCA (WOOD JOINTS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MOJOSON, WY 83139) FOR SPECIFIC PRACTICES PRIOR TO PERFORMING THESE CONNECTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.


**\*\* IMPORTANT \*\***  
FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR

TRUSS IN CONFORMANCE WITH TP1; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING

CONNECTOR PLATES ARE MADE OF 20/18/16GA (M. H/SS/K) ASTM A653 GRADE 40/60 (M. K/H.SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF BEAMS AND TIMBER JOINTS LOCATED ON THIS DESIGN POSITION FOR DRAWINGS 1604-2

ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TP11-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT

BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



**ALPINE**

**Alpine Engineered Products, Inc.**  
Haines City, FL 33844  
(800) 368-7262

1950 Marley Drive  
Haines City, FL 33844  
[redacted] zation # [redacted]  
[redacted] certificate

**Ratio #**

1000000

Professional Engineer Seal for Arthur R. Fisher, State of Oregon, No. E6687, dated Nov 22, 1966.

FL/-4/-1/R/-		Scale =.5"/Ft.
TC LL	20.0 PSF	REF R487-- 56358
TC DL	10.0 PSF	DATE 11/02/06
BC DL	10.0 PSF	DRW HCURS487 06306003
BC LL	0.0 PSF	HC-ENG DAB/AF *
TOT.LD.	40.0 PSF	SEON- 36412
DUR.FAC.	1.25	
SPACING	24.0"	JREF - 1T17487_201

## SPECIAL LOADS

TC - From	84 PLF at 3.65 to	84 PLF at 7.29
BC - From	4 PLF at -2.00 to	4 PLF at 0.00 to
BC - From	20 PLF at 0.00 to	20 PLF at 7.29

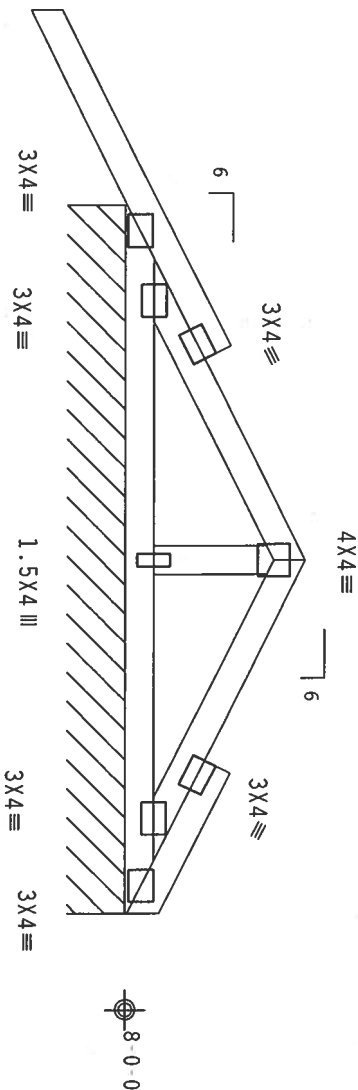
Wind reactions based on MWFRS pressures.

Dead loads are stated on projected horizontal area basis.

See DWGS A11015EE0405 & GBLLETIN0405 for more requirements.

Truss spaced at 24.0" OC designed to support 1-4-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



R=124 PLF U=100 PLF W=7-3-8

PLT TYP. Wave

Design Crit:  $TPI - 2002(STD) / FBC$ 
$$Cq/RT=1.00(1.25)/10(0)$$

SCIENCE

TY:1 FL/-/4/-/-/R/-

Scale = .5" / Ft.

\*\*\*WARNING\*\*\* THESE REQUIRE EXTREME CARE IN PARTICIPATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCG1 (BOLTING COMPONENT SAFETY INFORMATION), PUBLISHED BY IPI (TRUSS PLATE INSTITUTE), 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314, AND WICA (WOOD JOINT COUNCIL OF AMERICA), 6300 ENTERPRISE LANE, MALDEN, MI 48159 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

**\*\* IMPORTANT \*\***  
FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR.

TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF THE NATIONAL DESIGN SPEC. BY AREA AND JOI. ATTENDING

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., BY AF&PA) AND TPI.

PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z.

ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF 1P112002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT

BUILDING DESIGNER PER ANSI/TP1 1 SEC. 2.

Alpine Engineered Products, Inc.

1950 Marley Drive  
Haines City, FL 33844

ИЗДАНИЕ

Professional Engineer Seal for Arthur R. Fisher, State of Florida, No. 59887, dated 02/06.

TC LL	20.0 PSF	REF	R487 - 56359
TC DL	10.0 PSF	DATE	11/02/06
BC DL	10.0 PSF	DRW	HCUSR487 06306030
BC LL	0.0 PSF	HC-ENG	DAB/AF
TOT.LD.	40.0 PSF	SEQN-	36413 REV
DUR.FAC.	1.25		
SPACING	24.0"	JRFF -	1T1Z487_201

JRFF- 1T17487\_201

Top chord 2x4 SP #2 Dense  
Bot chord 2x4 SP #2 Dense  
Webs 2x4 SP #3

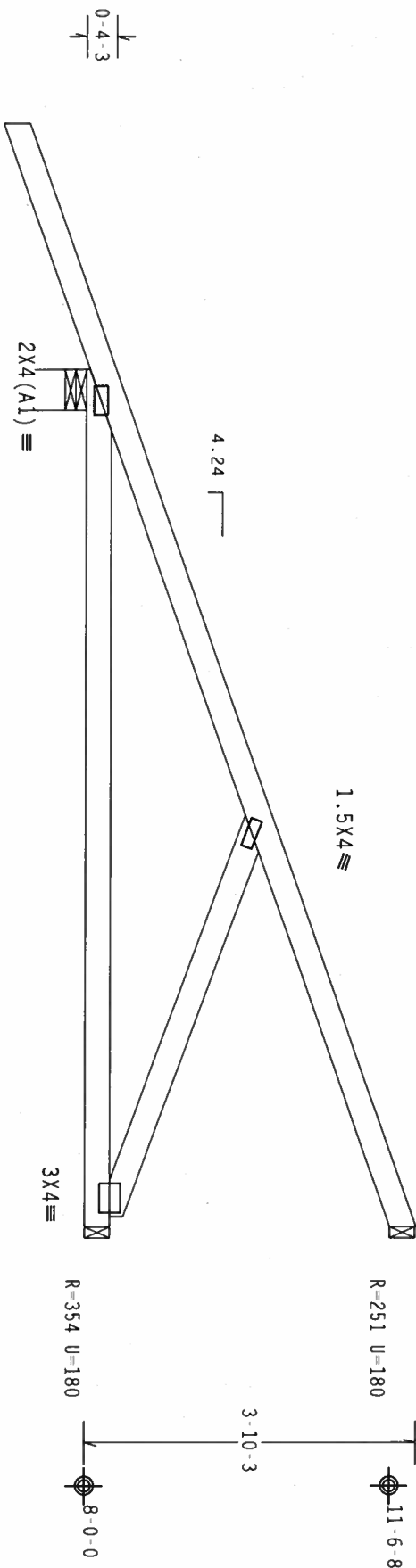
Wind reactions based on MWFRS pressures.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind 15.00 ft mean hgt. ASCE 7-02, CLOSED bldg. located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Hipjack supports 7-0-0 setback jacks with no webs.

Provide ( 2 ) 16d common nails(0.162"x3.5"), toe nailed at Top chord.  
Provide ( 3 ) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.



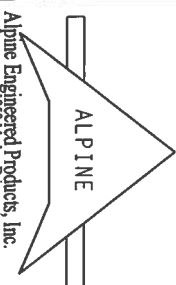
PLT TYP. Wave

Design Cr't: TPI-2002(STD)/FBC

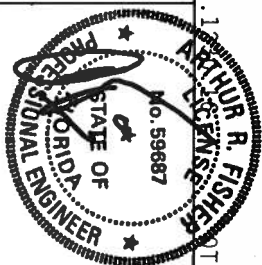
Cq/RT=1.00(1.25)/10(0) 7.24.1

\*\*WARNING\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST (BUILDING CODES) FOR TRUSS DESIGN. 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA 22304. AND WOOD TRUSS DESIGN. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

\*\*IMPORTANT\*\* FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W/H/SS/XX) ASTM A653 GRADE 40/60 (W. K/H/SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A Z. FINAL INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMEX AS OF TPI 2002 SEC.3. A SEAL ON THIS DESIGN SHOWS THE DESIGNER'S USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AMI/TPI 1 SEC. 2.



Alpine Engineered Products, Inc.  
190 Marley Drive  
Haines City, FL 33844  
Phone # 888-244-2444  
Fax # 888-244-2444



Scale = .5"/Ft.

TC LL	20.0 PSF	REF	R487--	56360
TC DL	10.0 PSF	DATE	11/02/06	
BC DL	10.0 PSF	DRW	HCUSR487	06306031
BC LL	0.0 PSF	HC-ENG	DAB/AF	
TOT.LD.	40.0 PSF	SEQN-	36417	
DUR.FAC.	1.25			
SPACING	24.0"	JRFF-	1TJ7487	201

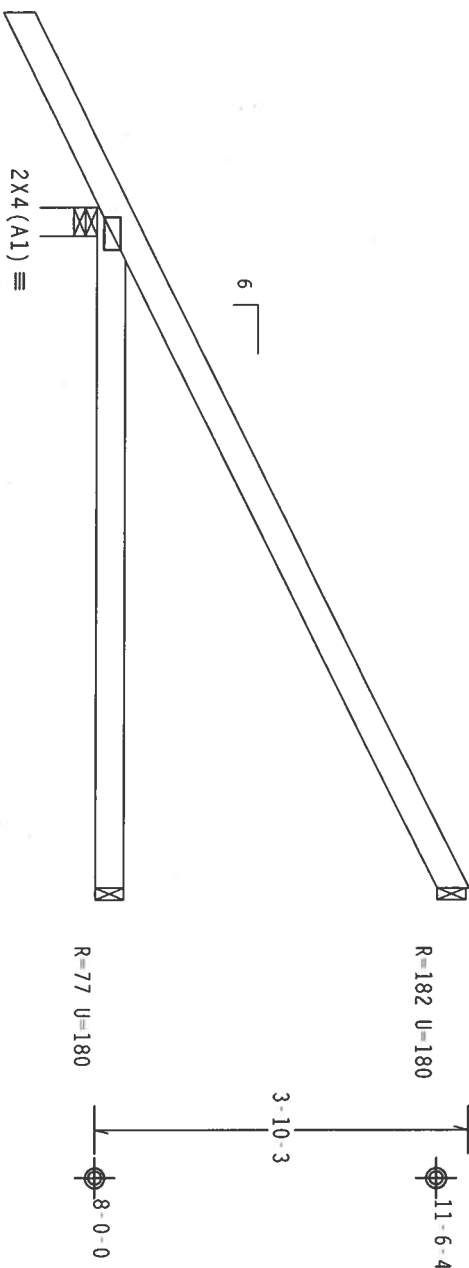


Top chord 2x4 SP #2 Dense  
Bot chord 2x4 SP #2 Dense

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Provide ( 2 ) 16d common nails(0.162"x3.5"), toe nailed at Top chord. Provide ( 2 ) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC

Cq/RT=1.00(1.25)/10(0)

7.24.1

1.25 FL/-/4/-/R/-

Scale = .5"/ft.

\*\*WARNING\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENTS COMPANY, 1000 N. 10TH STREET, SUITE 312, ALEXANDRIA, VA 22314, AND WICHITA TRUSS COMPANY, 6308 ENTERPRISE LANE, MADISON, WI 53719, FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

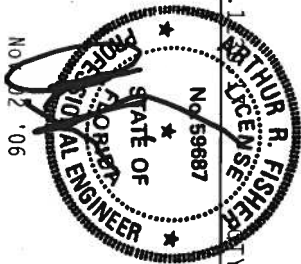
\*\*IMPORTANT\*\* FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR.

ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIA/AS) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (M/4/55/K) ASTM A653 GRADE 40/60 (K/4/55) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWING 160A-2.

AN INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMERAS AS OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES THE SIGNATURE OF A PROFESSIONAL ENGINEERING RESPONSIBILITY SOCIETY FOR THE TRUSS COMPONENT DESIGN SHOULD THE SIGNATURE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ALPINE ENGINEERED PRODUCTS, INC.

Haines City, FL 33844  
1950 Marley Drive  
Certification #



TC LL	20.0 PSF	REF R487--	56361
TC DL	10.0 PSF	DATE	11/02/06
BC DL	10.0 PSF	DRW HCUSR487	06306001
BC LL	0.0 PSF	HC-ENG TCE/AF	*
TOT.LD.	40.0 PSF	SEQN-	34917
DUR.FAC.	1.25		
SPACING	24.0"		
		DRFF- 1T17487_201	

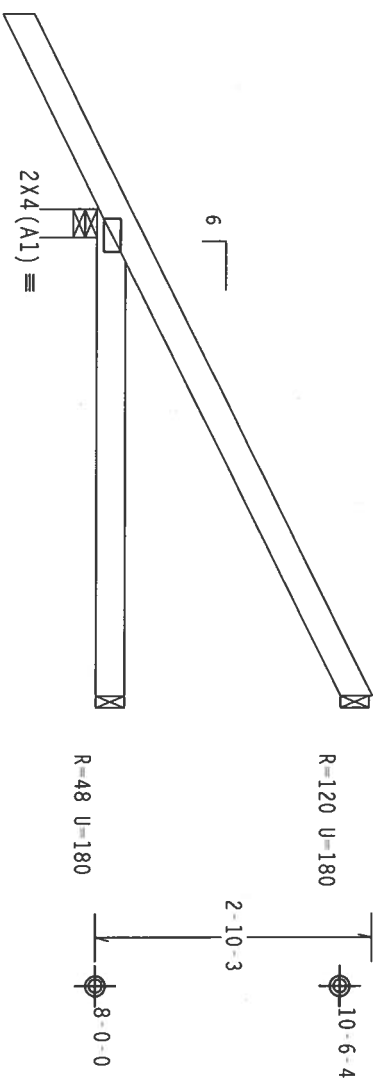
Top chord 2x4 SP #2 Dense  
Bot chord 2x4 SP #2 Dense

Wind reactions based on MWFRS pressures.

Provide { 2 } 16d common nails(0.162"x3.5") toe nailed at Top chord.  
Provide { 2 } 16d common nails(0.162"x3.5") toe nailed at Bot chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



2-0-0

5-0-0 Over 3 Supports  
R=377 U=180 W=3.5"

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC  
Cq/RT=1.00(1.25)/10(0)

QTY: 8

FL/-/4/-/R/-

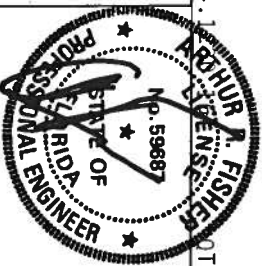
Scale = .5"/ft.

\*\*WARNING\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST (BUILDING COMPONENT SAFETY INFORMATION) PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 6300 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314, AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

\*\*IMPORTANT\*\* FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE PLATES ON EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. UNLESS OTHERWISE NOTED, ALL DIMENSIONS SHALL BE IN INCHES AND ALL ANGLES SHALL BE IN DEGREES. A SEAL ON THIS DRAWING INDICATES THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ALPINE

Alpine Engineered Products, Inc.  
1950 Marley Drive  
Haines City, FL 33844

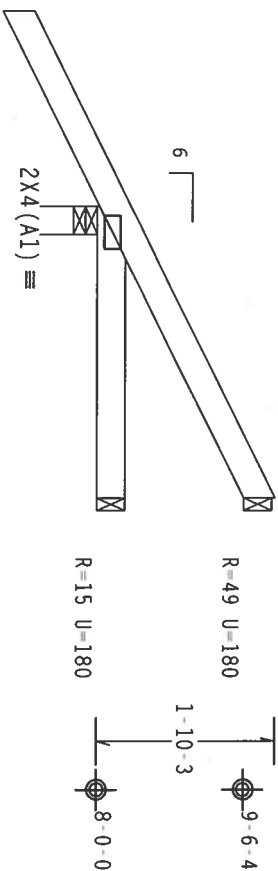


TC LL	20.0 PSF	REF R487-- 56362
TC DL	10.0 PSF	DATE 11/02/06
BC DL	10.0 PSF	DRW HCUSR487 06306004
BC LL	0.0 PSF	HC-ENG DAB/AF *
TOT.LD.	40.0 PSF	SEON- 36391
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 111Z487_201

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets  $L/360$  live and  $L/240$  total load. Creep increase factor for dead load is 1.50.

Deflection meets  $L/360$  live and  $L/240$  total load. Creep increase factor for dead load is 1.50.



3:0:0 Over 3 Supports

R=317 U=180 W=3.5"

Design Crit: TPI-2002(STD)/FBC

$$Cq/RT=1.00(1.25)/10(0)$$

7.24.12

TY:8 FL/-/4/-/-/R/

Scale = .5"/Ft.

\*\*\*\*\*WARNING\*\*\*\*\* THESE ARE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING  
REFER TO BC31 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY IP1 (TRUSS PAPER INSTITUTE), 218  
NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA 22314, AND WCA (WOOD JOINT COUNCIL OF AMERICA), 6500  
ENTERPRISE LANE, MANASSAS, VA 20108 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS  
OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE  
PROPERLY ATTACHED RIGID CEILING.

**\*\* IMPORTANT \*\***  
FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR

PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN: ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC NATIONAL DESIGN SPEC. BY AREA, AND THE FOLLOWING:

CONNECTION PLATES ARE MADE OF 201/81/16GA (N.H/55/K) ASTM A653 GRADE 40/60 (N. K.H/55) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE SPECIFIED ON THIS DESIGN, POSITION PER DRAGINGS 160A-7.

ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A.3 OF TP11-2002 SEC.3, A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT

DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

Alpine Engineered Products, Inc.

1950 Marley Drive  
Haines City, FL 33844

**STILLBORN**

**ZANDON H**

FL/-/4/-/-/R/-	Scale = .5"/ft.	
TC LL	20.0 PSF	REF R487-- 56363
TC DL	10.0 PSF	DATE 11/02/06
BC DL	10.0 PSF	DRW HCUSR487 06306005
BC LL	0.0 PSF	HC-ENG DAB/AF *
TOT.LD.	40.0 PSF	SEON- 36404
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1T17487-201

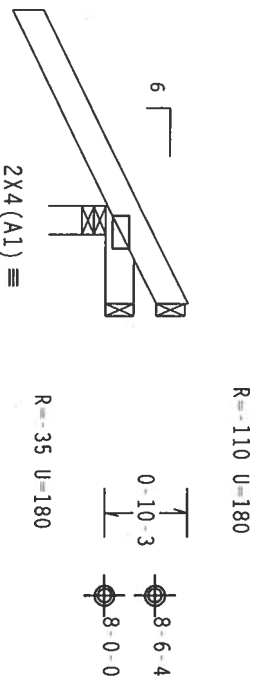
Top chord 2x4 SP #2 Dense  
Bot chord 2x4 SP #2 Dense

Wind reactions based on MWFRS pressures.

Provide { 2 } 16d common nails (0.162"x3.5"), toe nailed at Top chord.  
Provide { 2 } 16d common nails (0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, Wind TC DL=5.0 psf, Wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



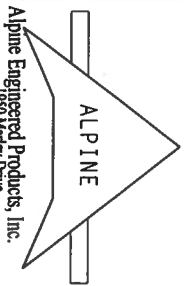
R=361 U=180 W=3.5"

PLT TYP. Wave

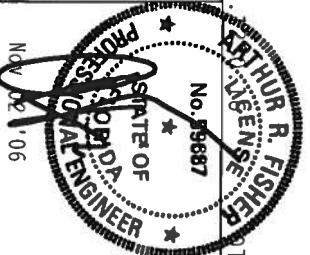
Design Crit: TPI-2002(STD)/FBC  
Cq/RT=1.00(1.25)/10(0) 7.24

\*\*WARNING\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO RCSTI (BUILDING COMPONENT SAFETY INFORMATION) PUBLISHED BY THE NATIONAL INSTITUTE OF STANDARDS AND TECHNOLOGY, 1016 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA 22304, FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

\*\*IMPORTANT\*\* FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIA/PN) AND TPI. ALPINE PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A, Z. INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEA AS OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES THE SUITABILITY OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



Alpine Engineered Products, Inc.  
1990 Marley Drive  
Haines City, FL 33844  
Certificate of Registration # 647



TC LL	20.0 PSF	REF	R487--	56364
TC DL	10.0 PSF	DATE	11/02/06	
BC DL	10.0 PSF	DRW	HCUSR487	06306032
BC LL	0.0 PSF	HC-ENG	DAB/AF	
TOT.LD.	40.0 PSF	SEQN-	36393	
DUR.FAC.	1.25			
SPACING	24.0"			

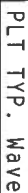
Scale = .5"/ft.

ИЗДАТЕЛЬСТВО «НАУКА» (ЛЕНИНГРАДСКОЕ ОТДЕЛЕНИЕ) 1981

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Hipjack supports 7-0-0 setback jacks with no webs.

Provide ( 2 ) 16d common nails(0.162"x3.5"), toe nailed at Top chord  
Provide ( 3 ) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

 $Cq/RT=1.00(1.25)/10(0)$ 

FL/-/4/-/-/R/-

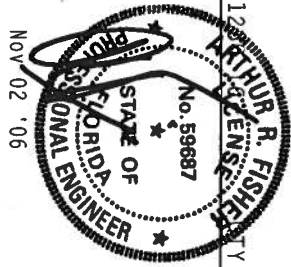
Scale = .5" / Ft.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR.

Alpine Engineered Products, Inc.

1950 Marley Drive  
Haines City, FL 33844

**\*IMPORTANT\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: (ON FABRICATING, HANDLING, SHIPPING, INSTALLING A BRACING OF TRUSSES, THE DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (ANALYSIS DESIGN SPEC. BY ASEA) AND TPI. TRUSSES, CONNECTION PLATES ARE MADE OF 2018/1664 (4H/55/57) ASTM A653 GRADE 40/60 (4H/47/55) GALV. STEEL. APPLY A PLATES TO EACH FACE OF TRUSS AND, GUESSE OVERLAP LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 1604-Z. AN INSPECTION OF PLATES PROCEEDED BY (1) SHALL BE PER AMEX 43 OR TPI 1 2002 SEC.3. A SEAL ON THIS DESIGN INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF R487-- 56365
TC DL	10.0 PSF	DATE 11/02/06
BC DL	10.0 PSF	DRW HCUR487 06306033
BC LL	0.0 PSF	HC-ENG DAB/AF
TOT.LD.	40.0 PSF	SEQN- 36418
DUR.FAC.	1.25	
SPACING	24.0"	JRFF- 1117487_201

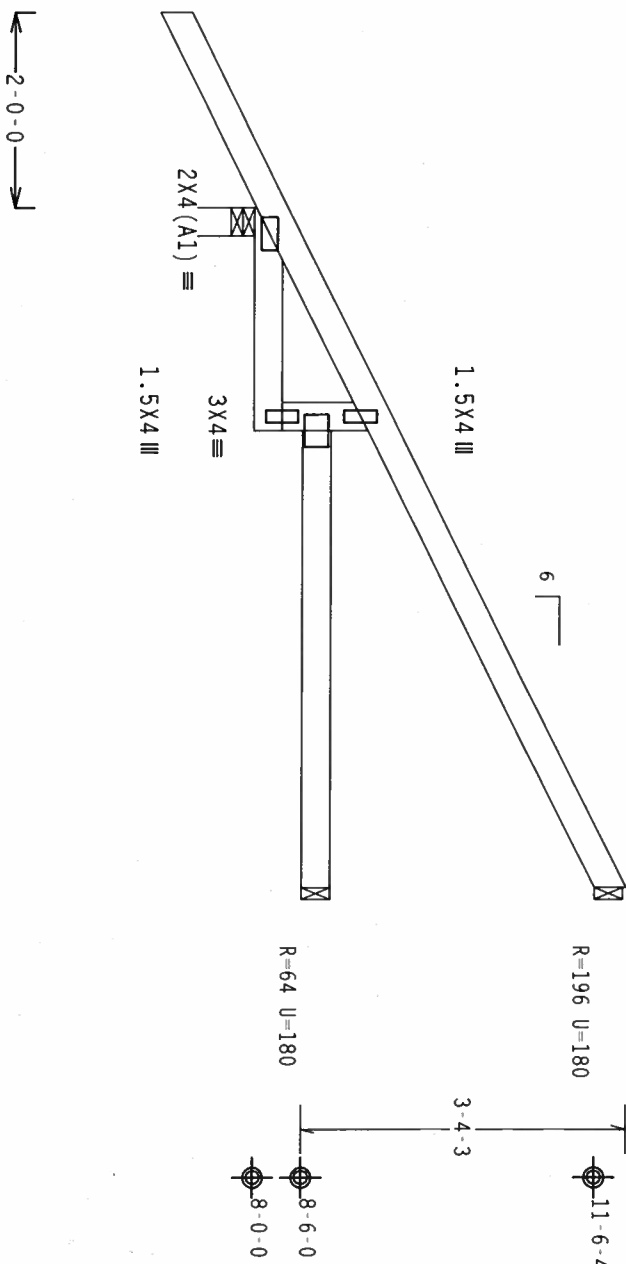
Top chord 2x4 SP #2 Dense  
Bot chord 2x4 SP #2 Dense  
Webs 2x4 SP #3

Wind reactions based on MWFRS pressures.

Provide ( 2 ) 16d common nails(0.162"x3.5"), toe nailed at Top chord.  
Provide ( 2 ) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located  
within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind  
BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase  
factor for dead load is 1.50.



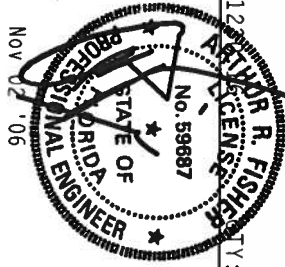
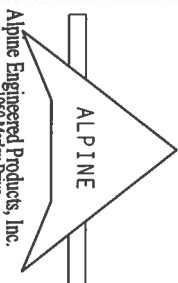
PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC

Cq/RT=1.00(1.25)/10(0) 7.24.12

\*\*WARNING\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING.  
REFER TO BC&I (OR) ENGINEERING DEPARTMENT, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA 22304, AND LUGED WOOD TRUSS COMPANY, 4653  
ENTERPRISE LANE, MADISON, WI 53719, FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS  
OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE  
A PROPERLY ATTACHED RIGID CEILING.

\*\*IMPORTANT\*\* FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED  
PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE  
TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES,  
DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY ASEP) AND TPI. ALPINE  
PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A.2.  
DRAWING SPECIFICATIONS OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT  
DESIGN SHOWN. THE SUITABILITY OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE  
BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TY: 4 FL: /4: /- /R: -

Scale = .5"/ft.

TC LL	20.0 PSF	REF	R487 - 56366
TC DL	10.0 PSF	DATE	11/02/06
BC DL	10.0 PSF	DRW	HCUSR487 06306006
BC LL	0.0 PSF	HC-ENG	DAB/AF
TOT.LD.	40.0 PSF	SEQN-	36397
DUR.FAC.	1.25		
SPACING	24.0"	JRFF-	1T1Z487_201

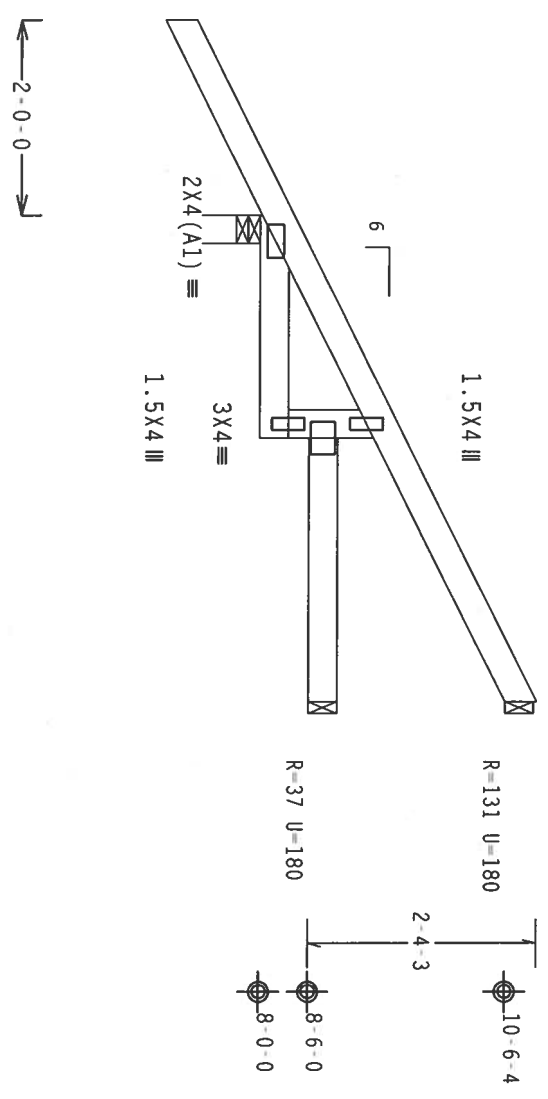
Top chord 2x4 SP #2 Dense  
Bot chord 2x4 SP #2 Dense  
Webs 2x4 SP #3

Wind reactions based on MWFRS pressures.

Provide ( 2 ) 16d common nails (0.162"x3.5"), toe nailed at Top chord.  
Provide ( 2 ) 16d common nails (0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located  
within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind  
BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase  
factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC

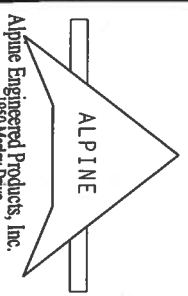
Cq/RT=1.00(1.25)/10(0)

FL/-/4/-/R/-

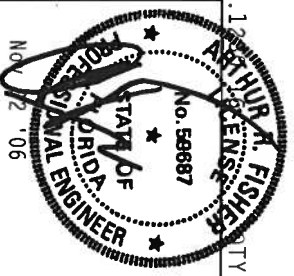
Scale = .5"/ft.

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST (BUILDING COMPONENTS) FOR TRUSS MANUFACTURING INSTRUCTIONS. 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA 22314. AND HIGH MOOD TRUSS INSTRUCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. CONNECTOR PLATES ARE MADE OF 20/18/16GA (W/V/S/S) ASTM A653 GRADE 40/60 (K, K/H-55) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A, Z. DRAWING INDICATES THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



Alpine Engineered Products, Inc.  
1950 Marley Drive  
Haines City, FL 33844  
Phone # 888-242-2424  
Fax # 888-242-2424

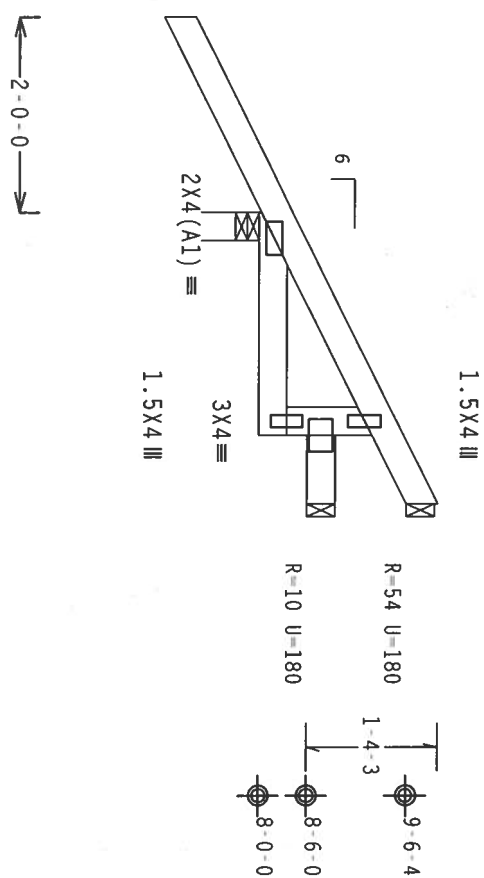


TC LL	20.0 PSF	REF	R487--	56367
TC DL	10.0 PSF	DATE	11/02/06	
BC DL	10.0 PSF	DRW	HCUSR487	06306007
BC LL	0.0 PSF	HC-ENG	DAB/AF	*
TOT.LD.	40.0 PSF	SEQN-	36410	
DUR.FAC.	1.25			
SPACING	24.0"			

Top chord 2x4 SP #2 Dense  
Bot chord 2x4 SP #2 Dense  
Webs 2x4 SP #3

Wind reactions based on MFRS pressures.  
Provide ( 2 ) 16d common nails(0.162"x3.5"), toe nailed at Top chord.  
Provide ( 2 ) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located  
anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0  
psf.  
Deflection meets L/360 live and L/240 total load. Creep increase  
factor for dead load is 1.50.



PLT TYP. Wave

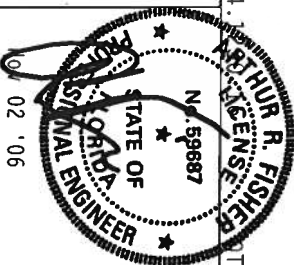
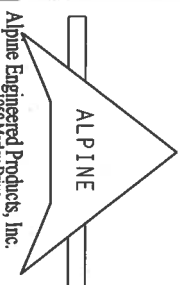
Design Crit: TP1-2002(STD)/FBC  
Cq/RT=1.00(1.25)/10(0)

7.24.1

Scale =.5"/ft.

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. ALL TRUSSES MUST BE DESIGNED AND MANUFACTURED IN ACCORDANCE WITH THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION, INC. (AISC) 360-10, 360-11, 360-12, 360-13, 360-14, 360-15, 360-16, 360-17, 360-18, 360-19, 360-20, 360-21, 360-22, 360-23, 360-24, 360-25, 360-26, 360-27, 360-28, 360-29, 360-30, 360-31, 360-32, 360-33, 360-34, 360-35, 360-36, 360-37, 360-38, 360-39, 360-40, 360-41, 360-42, 360-43, 360-44, 360-45, 360-46, 360-47, 360-48, 360-49, 360-50, 360-51, 360-52, 360-53, 360-54, 360-55, 360-56, 360-57, 360-58, 360-59, 360-60, 360-61, 360-62, 360-63, 360-64, 360-65, 360-66, 360-67, 360-68, 360-69, 360-70, 360-71, 360-72, 360-73, 360-74, 360-75, 360-76, 360-77, 360-78, 360-79, 360-80, 360-81, 360-82, 360-83, 360-84, 360-85, 360-86, 360-87, 360-88, 360-89, 360-90, 360-91, 360-92, 360-93, 360-94, 360-95, 360-96, 360-97, 360-98, 360-99, 360-100. OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

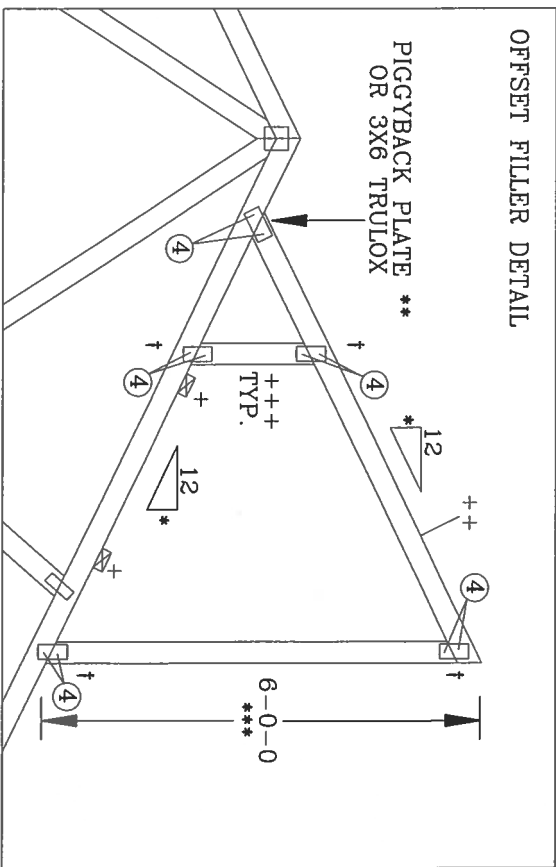
**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC. BY AISC) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 2018/1604 (W.H/55/Y) ASTM A653 GRADE 40/60 (W. K/H/55) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A.Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT BUILDING DESIGNER PER ANSI/TP1 1 SEC. 2.



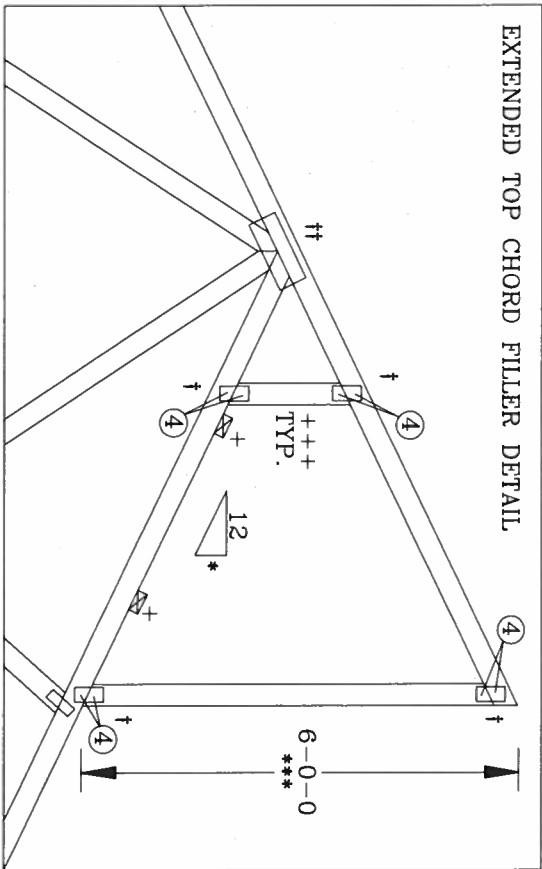
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TC DL	10.0 PSF	DATE	11/02/06	
BC DL	10.0 PSF	DRW	HCUSR487	06306008
BC LL	0.0 PSF	HC-ENG	DAB/AF	*
TOT.LD.	40.0 PSF	SEQN-	36403	
DUR.FAC.	1.25			
SPACING	24.0"			



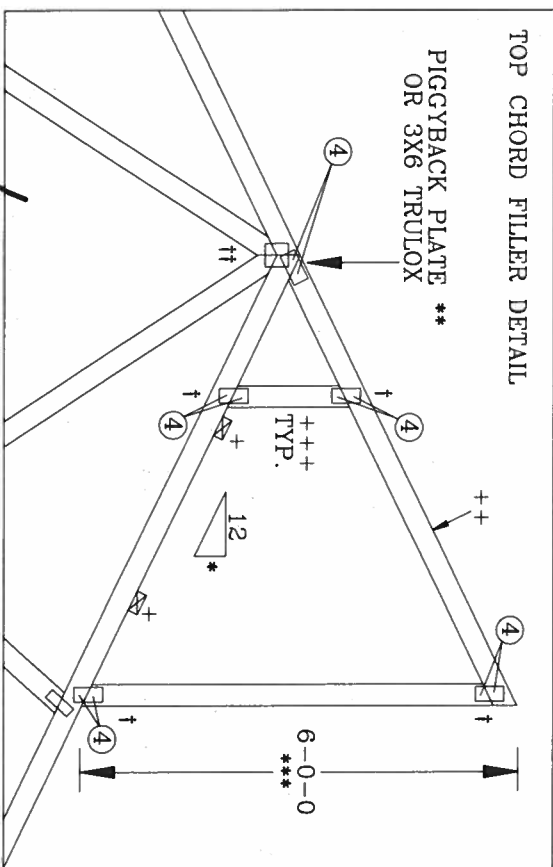
+ 2X4 CONTINUOUS LATERAL BRACING AT 24" OC MAXIMUM  
 SPACING. ATTACH TO EACH TOP CHORD WITH (2) 16d NAILS.  
 BRACING MATERIAL TO BE SUPPLIED AND ATTACHED AT BOTH  
 ENDS TO A SUITABLE SUPPORT BY ERECTION CONTRACTOR.  
 ++ 2X4 SO. PINE #2 N OR SPF #1/#2 FILLER TOP CHORD.  
 +++ 2X4 SO. PINE #3 OR SPF #1/#2 VERTICAL WEBS SPACED  
 48" OC MAXIMUM.  
 \* 8/12 MAXIMUM PITCH.  
 \*\* 2X8.25 PIGGYBACK SPECIAL PLATE. SEE DRAWING PIGBACKB0699  
 FOR PIGGYBACK SPECIAL PLATE INFORMATION.  
 \*\*\* 6'0" MAXIMUM HEIGHT.  
 † W2X4 OR 3X6 TRULOX.  
 †† REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS  
 DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT  
 SHOWN.  
 11 GAUGE (0.120)"X1.375" NAILS REQUIRED FOR TRULOX PLATE  
 ATTACHMENT. NAILS SPECIFIED IN CIRCLES MUST BE APPLIED TO  
 EACH FACE OF EACH TRUSS PLY. SEE DWG 160TL FOR NAILING AND  
 TRULOX PLATE REQUIREMENTS.



### OFFSET FILLER DETAIL




EXTENDED TOP CHORD FILLER DETAIL



### TOP CHORD FILLER DETAIL

PIGGYBACK PLATE \*\*  
OR 3X6 TRULOX



ALPINE ENGINEERED PRODUCTS, INC.  
POMPANO BEACH, FLORIDA

■ **WARNING** ■ TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING, AND BRACING. REFER TO BCST 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 963 DUNEDRID RD., SUITE 200, MADISON, WI 53719, AND AISC A600 TRUSS CONJUGAL DESIGN, 6500 ENTERPRISE DRIVE, MADISON, WI 53717, FOR DETAILED PRACTICES RELATIVE TO PERFORMING THESE TASKS. THE TPI TRUSS PLATE INSTITUTE'S TRUSS CONJUGAL DESIGN SPECIFICATIONS FOR JOINTED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

■ **IMPORTANT** ■ FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF THE TRUSSES. DESIGN CONTRACTORS WITH APPLICABLE PROVISIONS OF NJS CONTRACTUAL DESIGN SPEC. 17-2.10.1 AND 17-2.10.2 SHALL BE RESPONSIBLE FOR THE DESIGN, FABRICATION, AND INSTALLATION OF THE TRUSS ON THIS DESIGN. POSITION PER DRAWINGS 1604-2. AN INSPECTION OF PLATES FOLLOWED BY CD SHALL BE PER ANNEX A3 OF TPI 1-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF THE PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSI/TPI 1 SEC. 2.

TC LL	MAX 30 PSF	REF	TC-FILLER
TC DL	MAX 15 PSF	DATE	11/26/03
BC DL	MAX 10 PSF	DRWG	TCFILLER1103
BC LL	0 PSF	-ENG	SUP/KAR
TOT. LD.	MAX 55 PSF		
DUR. FAC.	1.15 OR 1.33		
SPACING	24.0"		

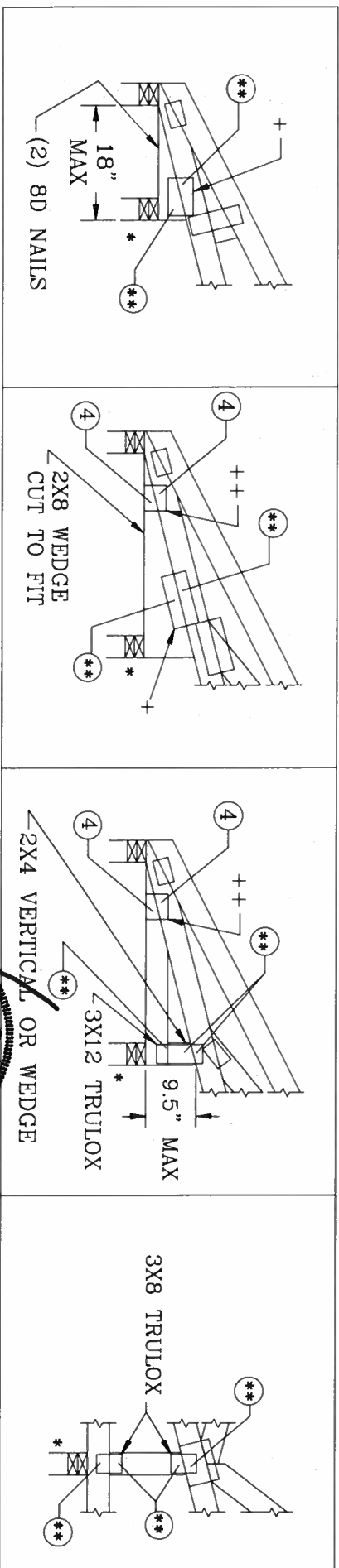
BOTTOM CHORD FILLER DETAIL.

- ++ 2X4 WAVE OR 3X6 TRULOX  
+ 3X4 WAVE OR 4X8 TRULOX

REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS  
DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT  
SHOWN.

ALL TRULOX PLATES SHOWN ARE MINIMUMS. LARGER PLATES MAY BE REQUIRED TO ACCOMMODATE REQUIRED NAILS (\*\*)

FILLER BOTTOM CHORD OR WEDGE SPECIES	MAXIMUM REACTION		MINIMUM BEARING AREA	** REQUIRED NAILS PER FACE WITH TRULOX PLATES				
	DOWNWARD	UPLIFT		1.00 D.O.L.	1.15 D.O.L.	1.25 D.O.L.	1.33 D.O.L.	1.60 D.O.L.
DOUGLAS FIR-LARCH	3281#	1656#	1.5" X 3.5"	12	11	10	9	8
HEM-FIR	2126#	1095#	1.5" X 3.5"	9	8	7	7	6
SPRUCE-PINE-FIR	2231#	1192#	1.5" X 3.5"	10	9	8	8	6
SOUTHERN PINE DENSE	3465#	1791#	1.5" X 3.5"	12	11	10	9	8
SOUTHERN PINE	2966#	1492#	1.5" X 3.5"	10	9	8	8	7
SOUTHERN PINE NON-DENSE	2520#	1343#	1.5" X 3.5"	9	8	7	7	6



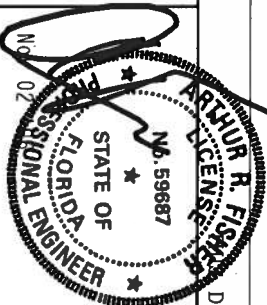
DRAWING REPLACES DRAWINGS A115 A115/R & 884.132

TC LL	—	PSF	REF	BC FILLER
TC DL	—	PSF	DATE	11/26/03
BC DL	10.0	PSF	DRWG	BCFILLER103
BC LL	—	PSF	—ENG	DLJ/KAR
TOT. LD.	—	PSF		
DUR. FAC. 1.0/1.15/1.25/1.33				
SPACING 24.0"				



**ALPINE ENGINEERED PRODUCTS, INC.  
POMPANO BEACH, FLORIDA**

\*\*\*WARNING\*\*\* TRUSSES REQUIRING EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND  
 \*\*\*WARNING\*\*\* REFERENCE TO BECI-1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS  
 PLATE INSTITUTE, 583 DUNFORD DR., SUITE 200, MADISON, VI, 53719) AND VITA CYCLOD TRUSS COUNCIL  
 OF AMERICA, 6500 ENTERPRISE LN, MADISON, VI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING  
 THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TID CHORD SHALL HAVE PROPERLY ATTACHED  
 STRUCTURAL PANELS AND OTHER CHORD SHALL HAVE A SPECIAL ATTACHED RIGID CEILING.  
 \*\*\*IMPORTANT\*\*\* FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ALPINE ENGINEERED  
 PRODUCTS, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO  
 BUILD THE TRUSSES IN CONFORMANCE WITH TPI, OR FABRICATING, HANDLING, SHIPPING, INSTALLING &  
 BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS NATIONAL DESIGN SPEC,  
 BY ALPINE AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 2018/1664 C4H5/50 A57A A653 GRADE  
 40/60 DESIGN CLASS (STEEL). APPLY PLATES TO EACH FACE OF TRUSS AND UNLESS OTHERWISE LOCATED  
 OR NOTED, DESIGN CLASS 40/60 STEEL. THIS DESIGN CONFORMS WITH THE BUILDING CODES AND SHALL  
 BE PER ALPINE A3 OF TPI 1-2002 SEC. 16.06.5.4. THE DESIGNING ENGINEER ASSUMES RESPONSIBILITY  
 OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN, THE  
 SUITABILITY AND USE OF THIS DESIGN FOR THE BUILDING. THE RESPONSIBILITY OF THE BUILDING  
 DESIGNER, PER ANSI/TPI 1 SEC. 2.



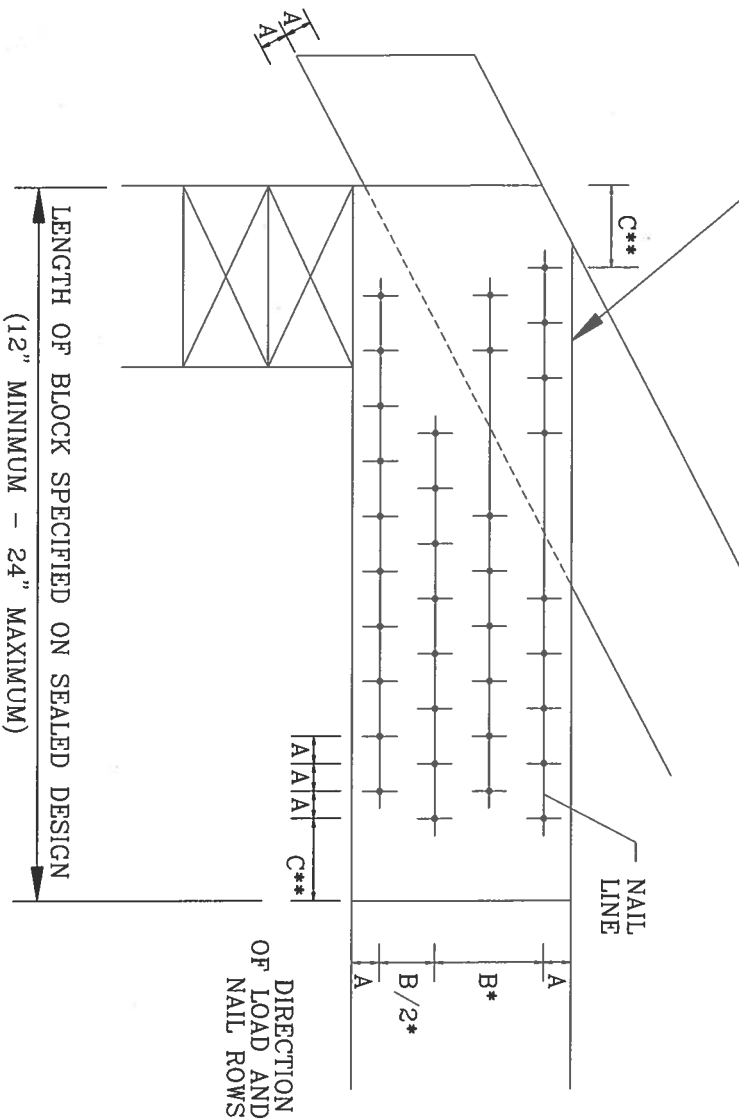
# BEARING BLOCK NAIL SPACING DETAIL

MINIMUM SPACING FOR SINGLE BEARING BLOCK IS SHOWN. DOUBLE NAIL SPACINGS AND STAGGER NAILING FOR TWO BLOCKS. GREATER SPACING MAY BE REQUIRED TO AVOID SPLITTING.

- A - EDGE DISTANCE AND SPACING BETWEEN STAGGERED ROWS OF NAILS (6 NAIL DIAMETERS)
- B - SPACING OF NAILS IN A ROW (12 NAIL DIAMETERS)
- C - END DISTANCE (15 NAIL DIAMETERS)

IF NAIL HOLES ARE PREBORED, SOME SPACING MAY BE REDUCED BY THE AMOUNTS GIVEN BELOW:  
 • SPACING MAY BE REDUCED BY 50%  
 • SPACING MAY BE REDUCED BY 33%

BEARING BLOCK TO BE SAME SIZE AND SPECIES AS BOTTOM CHORD. BLOCKS MAY BE ANY GRADE WITHIN THE SPECIES, PROVIDED THE COMPRESSION PERPENDICULAR TO GRAIN VALUE ( $F_c$ -perp) IS AT LEAST THAT OF THE CHORD.



## MAXIMUM NUMBER OF NAIL LINES PARALLEL TO GRAIN

NAIL TYPE	CHORD SIZE					
	2X4	2X6	2X8	2X10	2X12	
8d BOX (0.113"x2.5")	3	6	9	12	15	
10d BOX (0.128"x3")	3	5	7	10	12	
12d BOX (0.128"x3.25")	3	5	7	10	12	
16d BOX (0.135"x3.5")	3	5	7	10	12	
20d BOX (0.148"x4")	2	4	5	6	8	
8d COMMON (0.131"x2.5")	3	5	7	10	12	
10d COMMON (0.148"x3")	2	4	6	8	10	
12d COMMON (0.148"x3.25")	2	4	6	8	10	
16d COMMON (0.162"x3.5")	2	4	6	8	10	
0.120"x2.5" GUN	3	6	8	11	14	
0.131"x2.5" GUN	3	5	7	10	12	
0.120"x3.0" GUN	3	6	8	11	14	
0.131"x3.0" GUN	3	5	7	10	12	

## MINIMUM NAIL SPACING DISTANCES

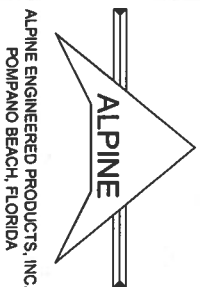
NAIL TYPE	DISTANCES			
	A	B*	C**	
8d BOX (0.113"x2.5")	3/4"	1 3/8"	1 3/4"	
10d BOX (0.128"x3")	7/8"	1 5/8"	2"	
12d BOX (0.128"x3.25")	7/8"	1 5/8"	2"	
16d BOX (0.135"x3.5")	7/8"	1 5/8"	2 1/8"	
20d BOX (0.148"x4")	1"	1 7/8"	2 1/4"	
8d COMMON (0.131"x2.5")	7/8"	1 5/8"	2"	
10d COMMON (0.148"x3")	1"	1 7/8"	2 1/4"	
12d COMMON (0.148"x3.25")	1"	1 7/8"	2 1/4"	
16d COMMON (0.162"x3.5")	1"	2"	2 1/2"	
0.120"x2.5" GUN	3/4"	1 1/2"	1 7/8"	
0.131"x2.5" GUN	7/8"	1 5/8"	2"	
0.120"x3.0" GUN	3/4"	1 1/2"	1 7/8"	
0.131"x3.0" GUN	7/8"	1 5/8"	2"	

DIRECTION OF LOAD AND NAIL ROWS

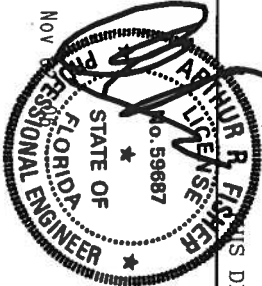
LENGTH OF BLOCK SPECIFIED ON SEALED DESIGN  
 (12" MINIMUM - 24" MAXIMUM)

\*\*\*WARNING\*\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC31-1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 DOWNSIDE DR., SUITE 200, MADISON, WI 53719) AND VITCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

\*\*\*IMPORTANT\*\*\* FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI, OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA C/H/S/XO ASTM A653 GRADE 40/50 C/H/S/XO GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (C) SHALL BE PER ANNEK AS OF TPI 1-2008 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF THE BUILDING DESIGN. THE SEALING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY OF THE TRUSS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSI/TPI 1 SEC. 2.



ALPINE ENGINEERED PRODUCTS, INC.  
 POMPANO BEACH, FLORIDA



THIS DRAWING REPLACES DRAWING B139 AND CNBRCBLK0699

REF	BEARING BLOCK
DATE	11/26/03
DRWG	CNBRGBLK1103
-ENG	SJP/KAR

# CLB WEB BRACE SUBSTITUTION

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON AN ALPINE TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

## NOTES:

THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB BRACING.

ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE. FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE BRACING.

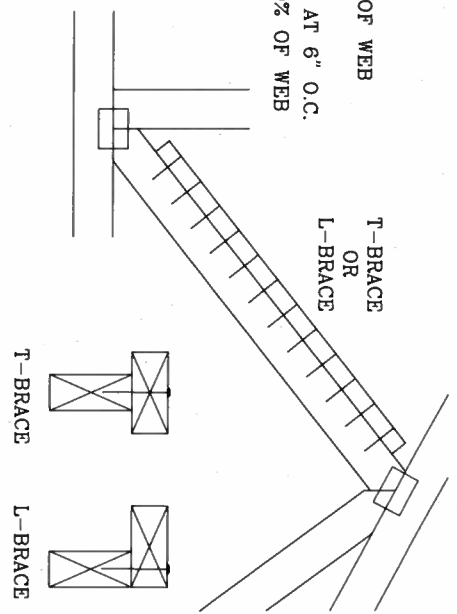
WEB MEMBER SIZE	SPECIFIED CLB BRACING	T OR L-BRACE	ALTERNATIVE BRACING SCAB BRACE
2X3 OR 2X4	1 ROW	2X4	1-2X4
2X3 OR 2X4	2 ROWS	2X6	2-2X4
2X6	1 ROW	2X4	1-2X6
2X6	2 ROWS	2X6	2-2X4(*)
2X8	1 ROW	2X6	1-2X8
2X8	2 ROWS	2X6	2-2X6(*)

T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

(\*) CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH FACE OF WEB.

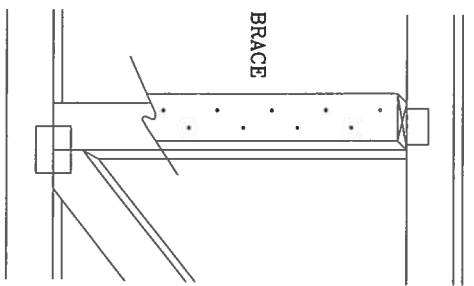
## T-BRACING OR L-BRACING:

APPLY TO EITHER SIDE OF WEB NARROW FACE  
ATTACH WITH 16d NAILS AT 6" O.C.  
BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH



## SCAB BRACING:

APPLY SCAB(S) TO WIDE FACE OF WEB.  
NO MORE THAN (1) SCAB PER FACE.  
ATTACH WITH 10d OR .128"x3" GUN NAILS AT 6" O.C. BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH



THIS DRAWING REPLACES DRAWING 579,640

ALPINE ENGINEERED PRODUCTS, INC.  
POMPAHO BEACH, FLORIDA

\*\*\*WARNING\*\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC31-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 963 DUNDON RD. DR. SUITE 200, HANSDEN, VI, 53719) AND VITA (WOOD TRUSS COUNCIL, 1000 W. 10TH AVE., SUITE 100, DENVER, CO 80202) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. THE USER OF THIS DRAWING SHALL BE RESPONSIBLE FOR THE PROPER ATTACHMENT OF STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

\*\*\*IMPORTANT\*\*\* FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS NATIONAL DESIGN SPEC. BY A500 AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20V18/16GA (V/H/S/Y) ASTM A653 GRADE 50. ALL DESIGN PROVISIONS OF TPI 1-2002 SEC 3. ON THIS DRAWING INDICATED ACCEPTED BY THE PROFESSIONAL ENGINEERING RESPONSIBILITY SOCIETY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSI/TPI 1 SEC. 2.

RECEIVED  
R. FISHER  
No. 59687  
FLORIDA  
ENGINEER

TC LL	PSF	REF	CLB SUBST.
TC DL	PSF	DATE	11/26/03
BC DL	PSF	DRWG	BRCLBSUB1103
BC LL	PSF	-ENG	MLH/KAR
TOT. LD.	PSF		
DUR. FAC.			
SPACING			

CABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS  $L/240$

PROVIDE UPLIFT CONNECTIONS FOR 80 PLF OVER  
CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

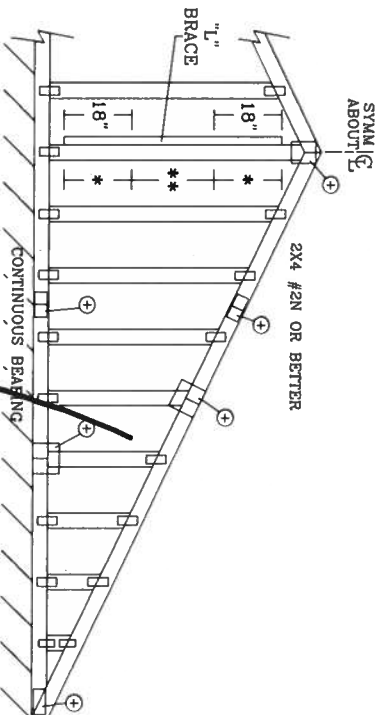
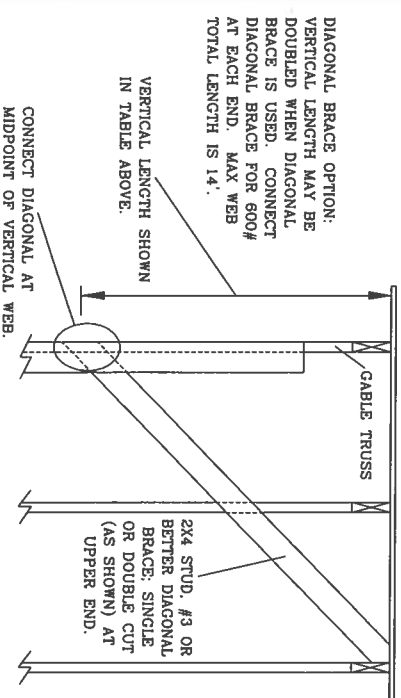
CABLE END SUPPORTS LOAD FROM 4' 0"  
OUTLOOKERS WITH 2' 0" OVERHANG, OR 12"  
PLYWOOD OVERHANG.

\* ATTACH EACH "L" BRACE WITH 10d NAILS.  
\* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C.  
IN 18" END ZONES AND 4" O.C. BETWEEN ZONES  
\*\* FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C.  
IN 18" END ZONES AND 6" O.C. BETWEEN ZONES

CABLE VERTICAL PLATE SIZES	
VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4
GREATER THAN 11' 6"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR  
PEAK, SPLICE, AND HEEL PLATES.

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.



REFER TO CHART ABOVE FOR MAX CABLE VERTICAL LENGTH



**ALPINE ENGINEERED PRODUCTS, INC.**  
**POMPANO BEACH, FLORIDA**

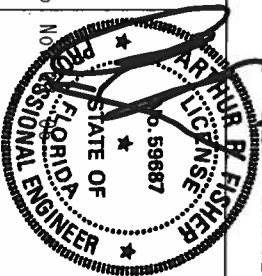
DIAGONAL BRACE OPTION:  
VERTICAL LENGTH MAY BE  
DOUBLED WHEN DIAGONAL  
BRACE IS USED. CONNECT  
DIAGONAL BRACE FOR 600#  
AT EACH END. MAX WEB  
TOTAL LENGTH IS 14'.

VERTICAL LENGTH SHOWN  
IN TABLE ABOVE.

CONNECT DIAGONAL AT  
MIDPOINT OF VERTICAL WEB

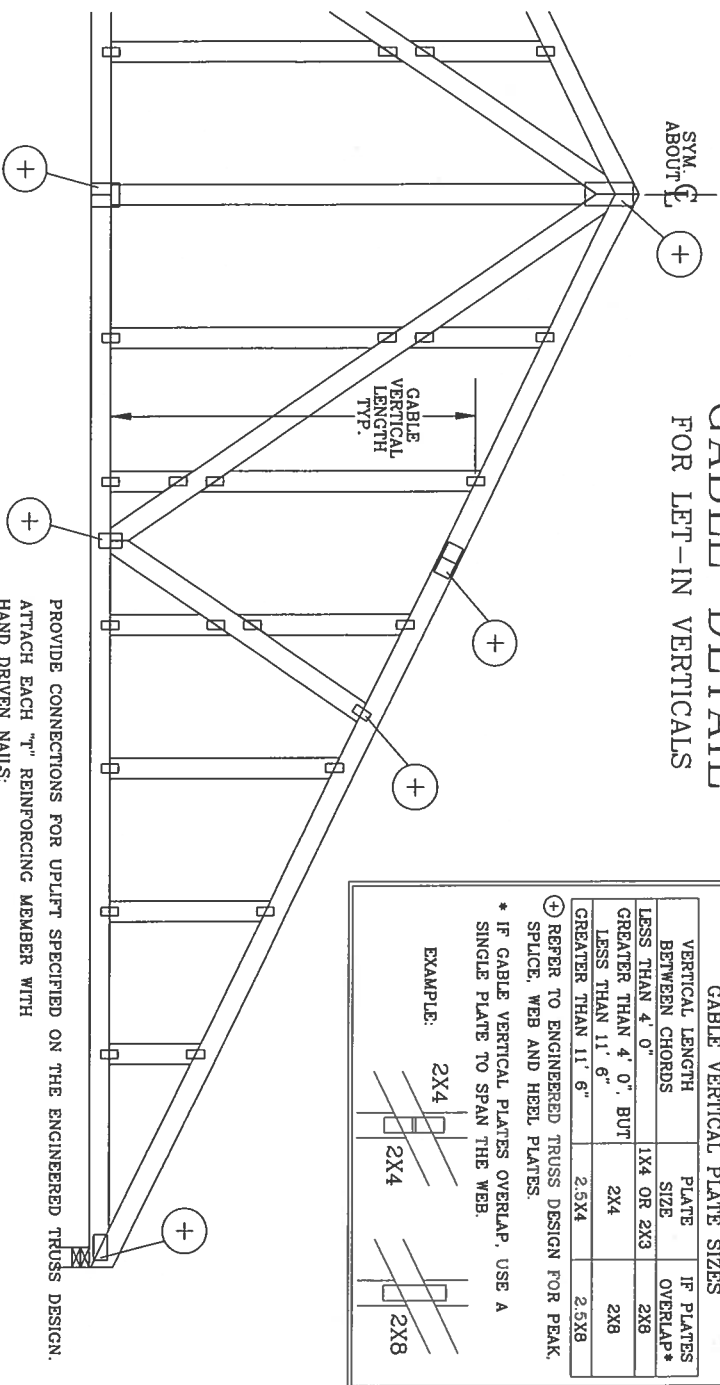
■ **WARNING:** THESE PROCESSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST-1-03 KILLING BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI CROSS LAMINATE INSTITUTE, 583 DUNDAS RD. E., SUITE 200, MISSISSAUGA, ONT. L4X 1L3, CANADA AND VITA (WOOD TRUSS CONSULTING OF AMERICA, 6300 ENTERPRISE IN MADISON, WI 53719) FOR SAFETY PRACTICES PERTAINING TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT NOTE: THE DESIGN COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR, ALPINE ENGINEERING & ARCHITECT, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI, OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONTRACTORS WITH APPLICABLE PROVISIONS OF NDS NATIONAL DESIGN SPECIFICATION (NDS) AND TPI, ALPINE CONNECTOR PLATES ARE MADE OF 2018/6184 (V4/V5/V6) ASTM A663 GRADE 302-1 ON 24 GA. AND 302-2 ON 24 GA. OF TRUSS AND, UNLESS OTHERWISE SPECIFIED, SHALL BE USED IN ALL TRUSSES. (D) SHALL BE PER ANNEX A3 OF TPI 1-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF THE PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN, THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER. PER ANNEX 1/TP1 1 SEC. 2.



REF	ASCE7-02-CAB11015
DATE	04/15/05
DRWG	A11015EE0405
-ENG	
MAX. TOT. LD. 60 PSF	
MAX. SPACING 24.0"	

# CABLE DETAIL FOR LET-IN VERTICALS

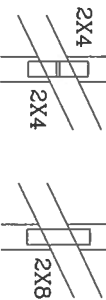


CABLE VERTICAL PLATE SIZES			
VERTICAL LENGTH BETWEEN CHORDS	PLATE SIZE	IF PLATES OVERLAP*	
LESS THAN 4' 0"	1X4 OR 2X3	2X6	
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4	2X6	
GREATER THAN 11' 6"	2.5X4	2.5X6	

\* REFER TO ENGINEERED TRUSS DESIGN FOR PEAK SPLICE, WEB AND HEEL PLATES.

\* IF CABLE VERTICAL PLATES OVERLAP, USE A SINGLE PLATE TO SPAN THE WEB.

EXAMPLE:



PROVIDE CONNECTIONS FOR UPLIFT SPECIFIED ON THE ENGINEERED TRUSS DESIGN.  
ATTACH EACH "T" REINFORCING MEMBER WITH  
HAND DRIVEN NAILS:  
10d COMMON (0.148" X 3.1" MIN) TOENAILS AT 4" O.C. PLUS  
(4) 16d COMMON (0.162" X 3.5" MIN) TOENAILS IN TOP AND BOTTOM CHORD.  
GUN DRIVEN NAILS:  
8d COMMON (0.131" X 2.5" MIN) TOENAILS AT 4" O.C. PLUS  
(4) TOENAILS IN TOP AND BOTTOM CHORD.

THIS DETAIL TO BE USED WITH THE APPROPRIATE ALPINE CABLE DETAIL FOR ASCE OR SBCCI WIND LOAD.

- ASCE 7-93 GABLE DETAIL DRAWINGS  
A11015EN1103, A10015EN1103, A08015EN1103, A07015EN1103  
A11030EN1103, A10030EN1103, A09030EN1103, A08030EN1103, A07030EN1103  
ASCE 7-98 GABLE DETAIL DRAWINGS  
A13015EC1103, A12015EC1103, A11015EC1103, A08515EC1103  
A13030EC1103, A12030EC1103, A11030EC1103, A08530EC1103  
ASCE 7-02 GABLE DETAIL DRAWINGS  
A13015EC0405, A12015EC0405, A11015EC0405, A08515EC0405, A13030EC0405, A12030EC0405, A11030EC0405, A08530EC0405

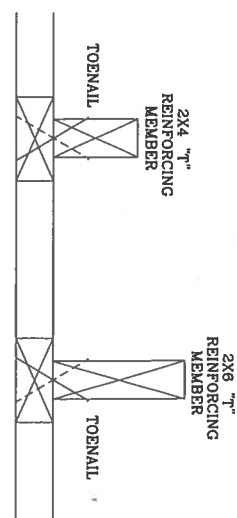
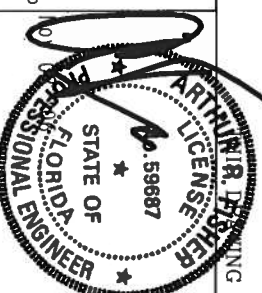
SEE APPROPRIATE ALPINE CABLE DETAIL (ASCE OR SBCCI WIND LOAD) FOR MAXIMUM UNREINFORCED CABLE VERTICAL LENGTH.

\*\*\*WARNING\*\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 593 DOWNSIDE DR., SUITE 200, MADISON, WI 53719 AND VICA C4000 TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

\*\*\*IMPORTANT\*\*\* FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS NATIONAL DESIGN SPEC. BY AEP(94) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/1664 CV/H/S/40 ASTM A653 GRADE 40/60 CV/H/S GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PERFECTNESS AS OF TPI 1-2008 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PERFECTNESS AND CONSTRUCTION OF THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND PERFORMANCE OF THE TRUSS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSI/TPI 1 SEC. 2.

ALPINE

ALPINE ENGINEERED PRODUCTS, INC.  
POMPANO BEACH, FLORIDA



TO CONVERT FROM "L" TO "T" REINFORCING MEMBERS, MULTIPLY "T" FACTOR BY LENGTH (BASED ON CABLE VERTICAL SPECIES, GRADE AND SPACING) FOR (1) 2X4 "L" BRACE, GROUP A, OBTAINED FROM THE APPROPRIATE ALPINE CABLE DETAIL FOR ASCE OR SBCCI WIND LOAD.

MAXIMUM ALLOWABLE "T" REINFORCED CABLE VERTICAL LENGTH IS 14' FROM TOP TO BOTTOM CHORD.

## WEB LENGTH INCREASE W/ "T" BRACE

WIND SPEED AND MRH	"T" REINF. MBR. SIZE	SBCCI	ASCE
110 MPH	2x4	10 %	10 %
15 FT	2x6	40 %	50 %
110 MPH	2x4	10 %	10 %
30 FT	2x6	50 %	50 %
100 MPH	2x4	10 %	10 %
15 FT	2x6	30 %	50 %
100 MPH	2x4	10 %	10 %
30 FT	2x6	40 %	40 %
90 MPH	2x4	20 %	40 %
15 FT	2x6	10 %	10 %
90 MPH	2x4	10 %	10 %
30 FT	2x6	30 %	50 %
80 MPH	2x4	10 %	20 %
15 FT	2x6	10 %	30 %
80 MPH	2x4	20 %	10 %
30 FT	2x6	0 %	40 %
70 MPH	2x4	0 %	20 %
15 FT	2x6	0 %	20 %
70 MPH	2x4	10 %	20 %
30 FT	2x6	10 %	30 %

EXAMPLE:  
ASCE WIND SPEED = 100 MPH  
MEAN ROOF HEIGHT = 30 FT  
CABLE VERTICAL = 24" O.C. SP #3  
"T" REINFORCING MEMBER SIZE = 2X4  
(1) "T" BRACE INCREASE (FROM ABOVE) = 10% = 1.10  
(1) 2X4 "L" BRACE LENGTH = 6' 7"  
MAXIMUM "T" REINFORCED CABLE VERTICAL LENGTH  
1.10 x 6' 7" = 7' 3"

REPLACES DRAWINGS GAB98117 876,719 & HC26294035

MAX TOT. LD. 60 PSF	REF LET-IN VERT
DUR. FAC. ANY	DATE 04/14/05
MAX SPACING 24.0"	DRWG GBLTETIN0405
	-ENG DJL/KAR



# FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

## Residential Component Prescriptive Method B

NORTH 1 2 3

Compliance with Method B Chapter 6 of the Florida Energy Efficiency Code may be demonstrated by the use of Form 600B for single and multifamily residences of 3 stories or less in height, and additions to existing residential buildings. To comply, a building must meet or exceed all of the energy efficiency prescriptives in any one of the prescriptive component packages and comply with the prescriptive measures listed in Table 6B-1 of this form. An alternative method is provided for additions of 600 square feet or less by use of Form 600C. If a building does not comply with this method, it may still comply under other sections in Chapter 6 of the Code.

PROJECT NAME: AND ADDRESS:	SPARKS 165 SW Wise Dr. Lake City	BUILDER:	Mike Todd
OWNER:	Mrs. Mrs. D.C. Sparks	PERMITTING OFFICE:	Columbia Co.
		PERMIT NO.:	25241
		CLIMATE ZONE:	1 <input type="checkbox"/> 2 <input type="checkbox"/> 3 <input type="checkbox"/>
		JURISDICTION NO.:	221000

## GENERAL DIRECTIONS

1. New construction including additions which incorporates any of the following features cannot comply using this method: steel stud walls, single assembly roof/ceiling construction, or skylights or other non-vertical roof glass.
2. Choose one of the component packages "A" through "E" from Table 6B-1 by which you intend to comply with the Code. Circle the column of the package you have chosen.
3. Fill in all the applicable spaces of the "To Be Installed" column on Table 6B-1 with the information requested. All "To Be Installed" values must be equal to or more efficient than the required levels.
4. Complete page 1 based on the "To Be Installed" column information.
5. Read "Minimum Requirements for All Packages", Table 6B-2 and check each box to indicate your intent to comply with all applicable items.
6. Read, sign and date the "Prepared By" certification statement at the bottom of page 1. The owner or owner's agent must also sign and date the form.

1. Compliance package chosen (A-F)
2. New construction or addition
3. Single family detached or Multifamily attached
4. If Multifamily—No. of units covered by this submission
5. Is this a worst case? (yes / no)
6. Conditioned floor area (sq. ft.)
7. Predominant eave overhang (ft.)
8. Glass type and area :
  - a. Clear glass
  - b. Tint, film or solar screen
9. Percentage of glass to floor area
10. Floor type, area or perimeter, and insulation:
  - a. Slab on grade (R-value)
  - b. Wood, raised (R-value)
  - c. Wood, common (R-value)
  - d. Concrete, raised (R-value)
  - e. Concrete, common (R-value)
11. Wall type, area and insulation:
  - a. Exterior: 1. Masonry (Insulation R-value)
  2. Wood frame (Insulation R-value)
  - b. Adjacent: 1. Masonry (Insulation R-value)
  2. Wood frame (Insulation R-value)
12. Ceiling type, area and insulation:
  - a. Under attic (Insulation R-value)
  - b. Single assembly (Insulation R-value)
13. Air Distribution System: Duct insulation, location  
Test report (attach if required)
14. Cooling system  
(Types: central, room unit, package terminal A.C., gas, none)
15. Heating system:  
(Types: heat pump, elec. strip, nat. gas, L.P. gas, gas h.p., room or PTAC, none)
16. Hot water system:  
(Types: elec., nat. gas, L.P. gas, solar, heat rec., ded. heat pump, other, none)

Please Print

CK

1.	A	
2.	New	
3.	Single family	
4.		
5.	NO	
6.	1698	
7.	2'	
	Single Pane	Double Pane
8a.	sq. ft.	191 sq. ft.
8b.	sq. ft.	sq. ft.
9.	11 %	
10a.	R= 0	lin. ft.
10b.	R=	sq. ft.
10c.	R=	sq. ft.
10d.	R=	sq. ft.
10e.	R=	sq. ft.
11a-1	R=	sq. ft.
11a-2	R= 13	sq. ft.
11b-1	R=	sq. ft.
11b-2	R=	sq. ft.
12a.	R= 30	1698 sq. ft.
12b.	R=	sq. ft.
13.	R= 6	
14a.	Type: Central	
14b.	SEER/EER: 13.0	
14c.	Capacity: 3 TON	
15a.	Type: Heat Pump	
15b.	HSPF/COP/AFUE:	
15c.	Capacity: 36K	
16a.	Type: Elect.	
16b.	EF: 1.89	

I hereby certify that the plans and specifications covered by the calculation are in compliance with the Florida Energy Code.

PREPARED BY: Mike Todd

DATE: 10/30/06

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER AGENT: [Signature]

DATE: 10/30/06

Review of plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed, this building will be inspected for compliance in accordance with Section 553.908, F.S.

BUILDING OFFICIAL: \_\_\_\_\_

DATE: \_\_\_\_\_

TABLE 6B-1

## MINIMUM REQUIREMENTS

Climate Zones 1 2 3

COMPONENTS		PACKAGES FOR NEW CONSTRUCTION				
GLASS	Max. % of glass to Floor Area	A	B	C	D	E
	Type	Double Clear (DC)	Double Clear (DC)	Double Clear (DC)	Double Clear (DC)	Double Tint (DT)
	Overhang	1'4"	2'	2'	2'	2'
WALLS	Masonry	EXTERIOR AND ADJACENT MASONRY WALLS R-5 COMMON MASONRY WALLS R-3 EACH SIDE.				
	Wood Frame	EXTERIOR, ADJACENT, AND COMMON WOOD FRAME WALLS R-11				
CEILING		R-30	R-30	R-30	R-30	R-30
		(NO SINGLE ASSEMBLY CEILING ALLOWED)				
FLOORS	Slab-On-Grade	R-0				
	Raised Wood	R-19 (ONLY STEM WALL CONSTRUCTION ALLOWED EXCEPT PACKAGE C)				
	Raised Concrete	R-7				
DUCTS		R-6	R-6	R-6, TESTED	R-6	R-6, TESTED
SPACE COOLING (SEER)		12.0	10.5	12.0	11.0	12.0
HEAT	Elect. (HSPF)	7.9	7.1	7.4	7.4	7.4
	Gas/Oil (AFUE)	MINIMUM OF .73 (Direct heating) or .78 (Central)				
HOT WATER SYSTEM	Electric Resistance**	EF .88	NOT ALLOWED (SEE BELOW)	EF .91	NOT ALLOWED (SEE BELOW)	EF .91
	Gas & Oil **	MINIMUM EF OF .54				NATURAL GAS ONLY (SEE BELOW)
	Other	Any of the following are allowed: dedicated heat pump, heat recovery unit or solar system.				

\* Single package units minimum SEER=9.7, HSPF = 6.6.

\*\* Minimum efficiencies for gas and electric hot water systems apply to 40 gallon water heaters. Refer to Table 6-12 for minimum Code efficiencies for oil water heaters and other sizes.

## DESCRIPTION OF BUILDING COMPONENTS LISTED

Percent of Glass to Floor Area: This percentage is calculated by dividing the total of all glass areas by the total conditioned floor area.

Overhang: The overhang is the distance the roof or soffit projects out horizontally from the face of the glass. All glass areas shall be under an overhang of at least the prescribed length with the following exceptions:

1) glass on the gabled ends of a house and 2) the glass in the lower stories of a multi-story house.

Wall, Ceiling and Floor Insulation Values: The R-values indicated represent the minimum acceptable insulation level added to the structural components of the wall, ceiling or floor. The R-value of the structural building materials shall not be included in this calculation. "Common" components are those separating conditioned tenancies in a multifamily building. "Adjacent" components separate conditioned space from unconditioned but enclosed space.

"Exterior" components separate conditioned space from unconditioned and unenclosed space.

Floor: Slab-on-grade floors without edge insulation are acceptable. Raised wood floors shall have continuous stem walls with insulation placed on the stem wall or under the floor except Package C.

Ducts: "TESTED" shall mean the ducts have less than 5% leakage based on a certified test report by a State-approved tester.

Space Cooling System: Cooling systems shall have a Seasonal Energy Efficiency Ratio (SEER) for central units or Energy Efficiency Ratio (EER) for room units or PTAC's equal to or greater than the prescribed value.

Electric Space Heating Option: Heat pump systems shall be rated with a Heating Seasonal Performance Factor (HSPF) equal to or greater than the prescribed HSPF. Heat pump systems may contain electric strip backups meeting the criteria of section 608.1 ABC.3.2.1.2. No electric resistance space heat is allowed for these packages.

Electric Resistance Hot Water Option: For packages designated "Not Allowed", an electric resistance hot water system may be installed only in conjunction with one of the "Other Hot Water System Options". See below.

Other Hot Water System Options: Any dedicated heat pump, heat recovery unit, or solar hot water system may be installed. Solar systems must have an EF of 1.5 or higher. Electric resistance systems having an EF of .88 or greater, or natural gas systems with EF .54 or greater may be used in conjunction with these systems.

TO BE INSTALLED	
DC: <input checked="" type="checkbox"/>	DT: <input type="checkbox"/>
FEET	
EXT: R =	
ADJ: R =	
COM: R =	
EXT: R = 13	
ADJ: R =	
COM: R =	
UNDER ATTIC: R = 30	
COMMON: R =	
R = 0	
R =	
R =	
R = 4 COND. <input type="checkbox"/>	
SEER = 13	
COP = 2.5	
AFUE =	
EF = 88	
EF =	
DHP: <input checked="" type="checkbox"/>	EF =
HRU: <input type="checkbox"/>	
SOLAR: <input type="checkbox"/>	EF =

TABLE 6B-2 MINIMUM REQUIREMENTS FOR ALL PACKAGES			
COMPONENTS	SECTION	REQUIREMENTS	CHECK
Exterior Joints & Cracks	606.1	To be caulked, gasketed, weather-stripped or otherwise sealed.	<input checked="" type="checkbox"/>
Exterior Windows & Doors	606.1	Max .3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	<input checked="" type="checkbox"/>
Sole & Top Plates	606.1	Sole plates and penetrations through top plates of exterior walls must be sealed.	<input checked="" type="checkbox"/>
Recessed Lighting	606.1	Type IC rated with no penetrations (two alternatives allowed).	<input checked="" type="checkbox"/>
Multi-story Houses	606.1	Air barrier on perimeter of floor cavity between floors.	<input checked="" type="checkbox"/>
Exhaust Fans	606.1	Exhaust fans vented to unconditioned space shall have dampers, except for combustion devices with integral exhaust ductwork.	<input checked="" type="checkbox"/>
Water Heaters	612.1	Comply with efficiency requirements in Table 6-12. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required for vertical pipe risers.	<input checked="" type="checkbox"/>
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have minimum thermal efficiency of 78%.	<input checked="" type="checkbox"/>
Hot Water Pipes	612.1	Insulation is required for hot water circulating systems (Including heat recovery units).	<input checked="" type="checkbox"/>
Shower Heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	<input checked="" type="checkbox"/>
HVAC Duct Construction, Insulation & Installation	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated and installed in accordance with the criteria of Section 610.1. Ducts in attics must be insulated to a minimum of R-6.	<input checked="" type="checkbox"/>
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	<input checked="" type="checkbox"/>