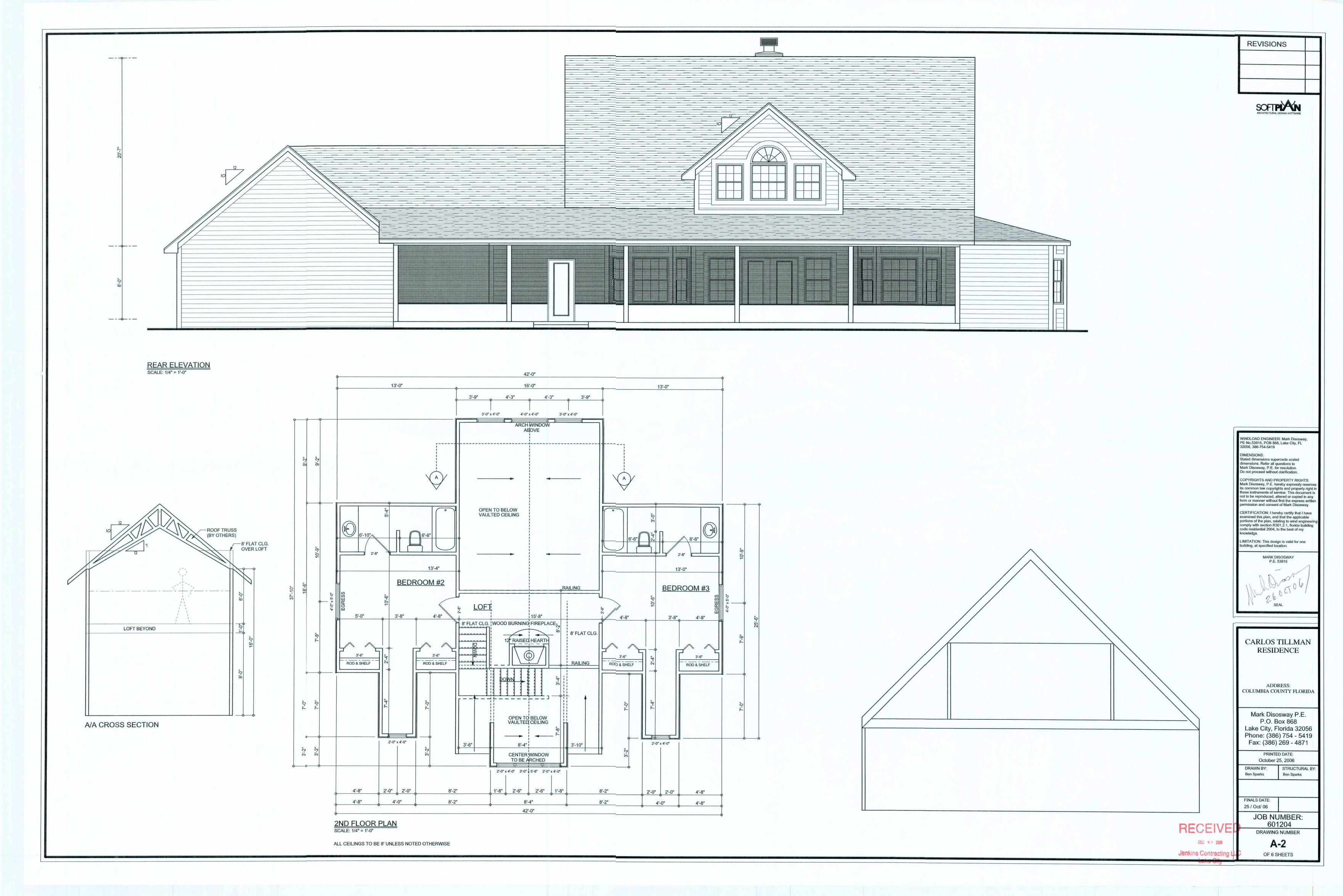
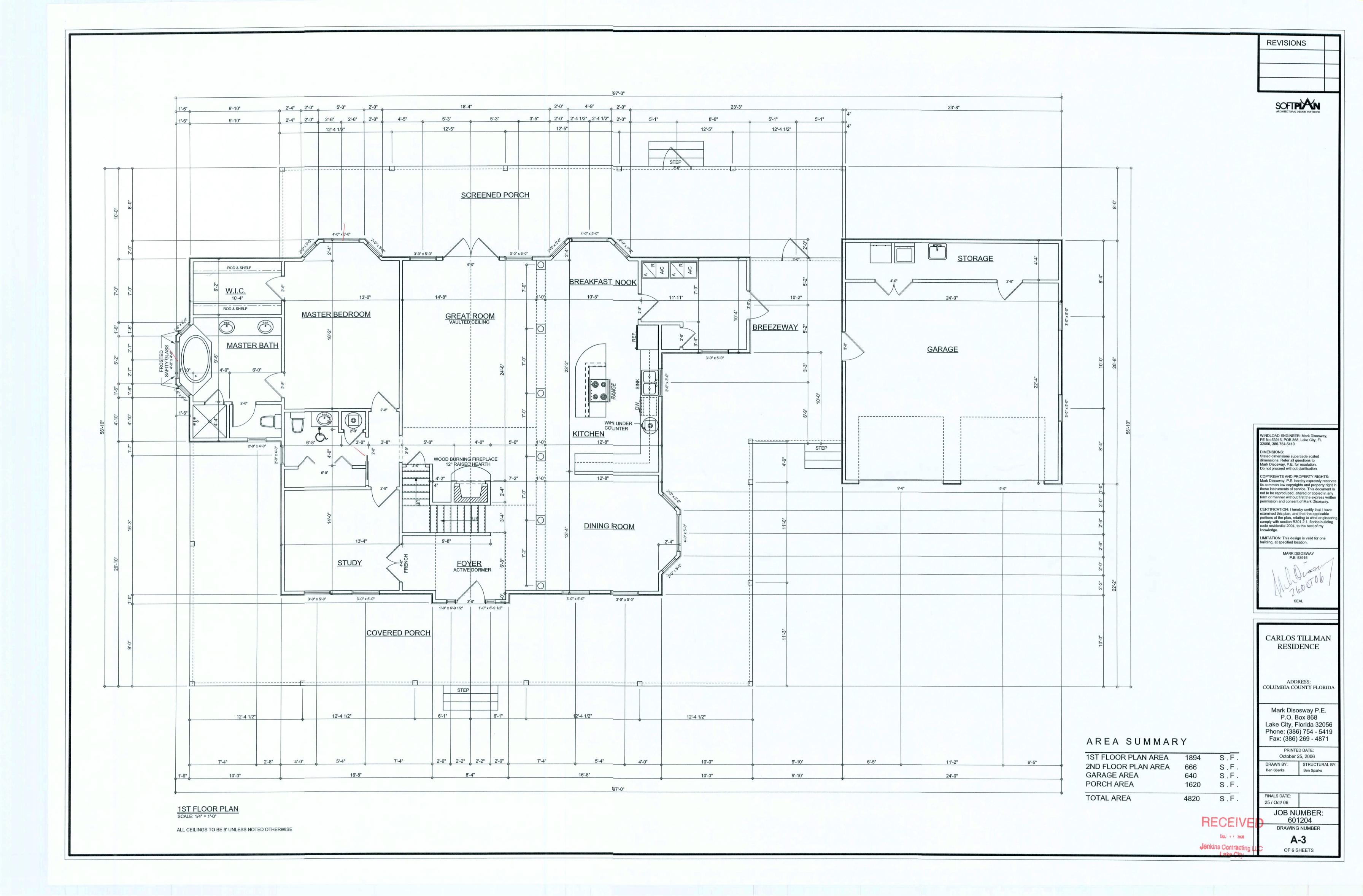
RIGHT ELEVATION
SCALE: 1/4" = 1'-0"

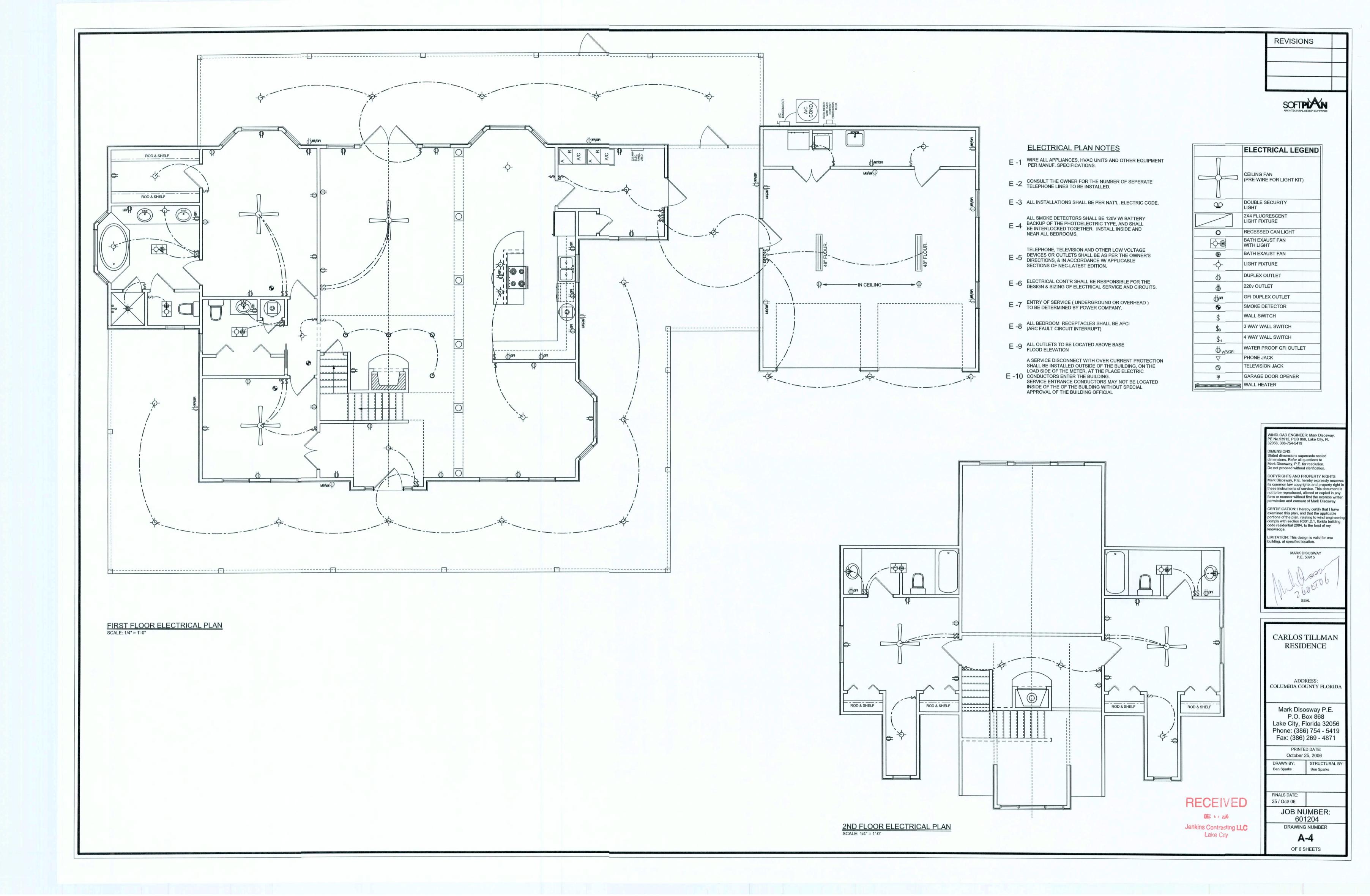
Jenkins Contracting LLC
Lake City

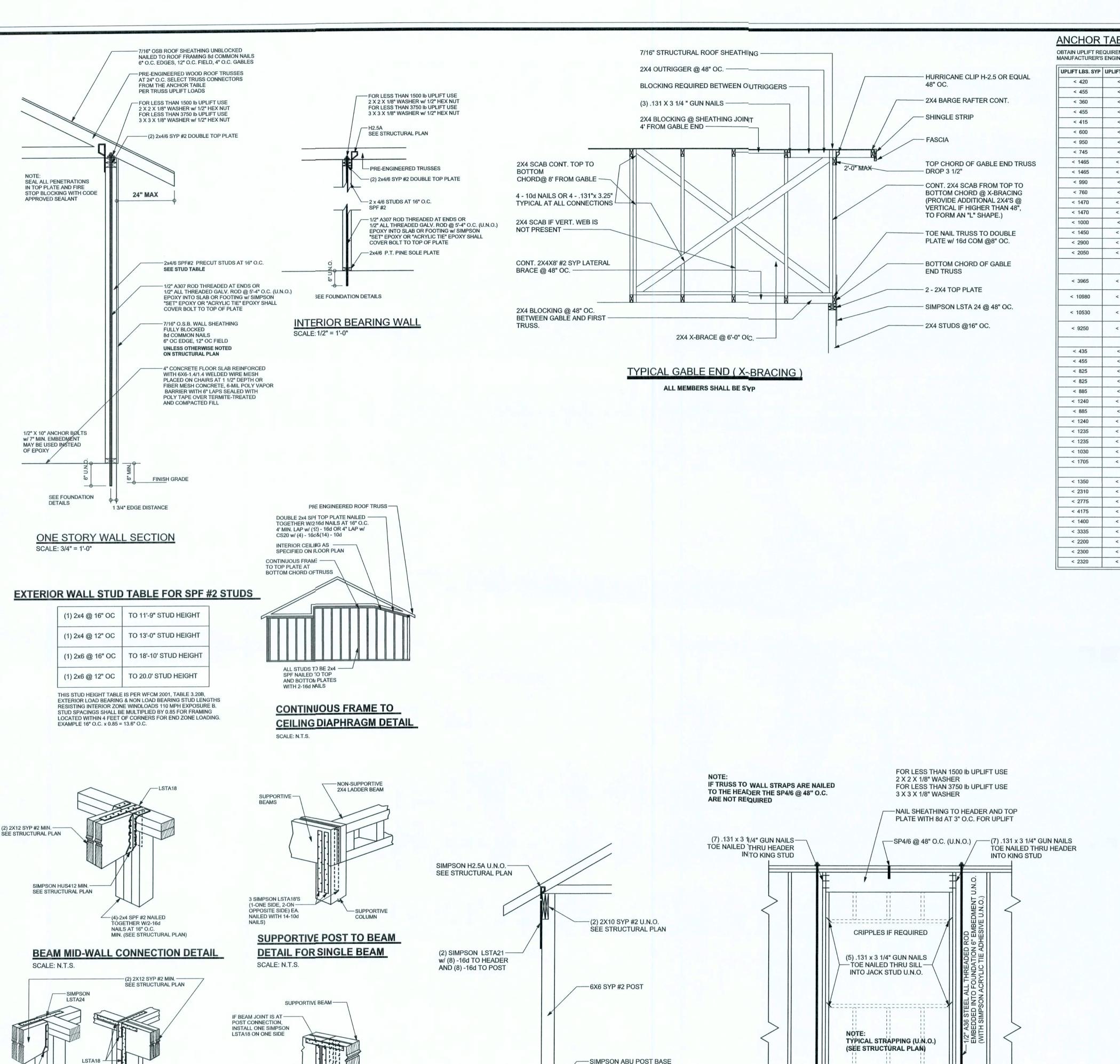
DRAWING NUMBER

OF 6 SHEETS









w/ (12) - 16d & 5/8" x 10"

-SEE FOOTING DETAILS

(1) 2X6 SPF #2 SILL UP TO 7'-6" U.N.O.

(2) 2X4 SPF #2 SILL UP TO 7'-8" U.N.O.

(1) 2X4 SPF #2 SILL UP TO 5'-1" U.N.O.

TYPICAL 1 STORY HEADER STRAPING DETAIL

SCALE: 1/2" = 1'-0"

(FOR: 120 MPH, 10'-0" WALL HEIGHT U.N.O.)

ANCHOR BOLT

TYPICAL PORCH POST DETAIL SCALE: 1/2" = 1'-0"

NAIL THRU 2x4 INTO

BEAM MAY BE ATTACHED IN

BEAM CORNER CONNECTION. DETAIL

4-SIMPSON LSTA18 -

SUPPORTIVE CENTER POST TO BEAM DETAIL

(2-ONE SIDE, 2-ON

OTHER SIDE)

BEAM W/4-16d

SIMPSON HUS412 MIN

SCALE: N.T.S.

SEE STRUCTURAL PLAN

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS

UPLIFT LBS. SYP UPLIFT LBS. SPF		TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d 3-8d		
< 455	< 265	H5	4-8d 4-8d		
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d 8-8d		
< 745	< 565	H8	5-10d, 1 1/2" 5-10d, 1 1/2"		
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d 12-8d, 1 1/2"		
< 990	< 850	H10-1	8-8d, 1 1/2" 8-8d, 1		
< 760	< 655	H10-2	6-10d 6-10d		
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LGT2	14 -16d	14 -16d	
		HEAVY GIRDER TIEDOWNS*			TO FOUNDATION
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
		STUD STRAP CONNECTOR*			TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		·
< 1705	< 1705	CS16	28-8d		
		STUD ANCHORS*	TO STUDS		TO FOUNDATION
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTTI31	18-10d, 1 1/2"	7	1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 - 16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		
< 3335	< 3335	HPAHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		1/2" AB
< 2320	< 2320	ABU88	18 - 16d		2-5/8" AB

GRADE & SPECIES TABLE

SYP #2

SYP #2

24F-V3 SP

MICROLAM

PARALAM

2x6 SYP #2 GARAGE DOOR BUCK ATTACHMENT

ATTACH GARAGE DOOR BUCK TO STUD PACK AT

EACH SIDE OF DOOR OPENING WITH 3/8"x4" LAG

SCREWS w/ 1" WASHER LAG SCREWS MAY BE

COUNTERSUNK. HORIZONTAL JAMBS DO NOT

TRANSFER LOAD. CENTER LAG SCREWS OR STAGGER 16d NAILS OR (2) ROWS OF .131 x 3 1/4" GN PER TABLE BELOW:

DOOR WIDTH 3/8" x 4" LAG STAGGER

24" O.C.

11' - 15' 18" O.C. 4" O.C.

16' - 18' 16" O.C. 3" O.C.

2x6SYP #2 DOOR BUCK-

BRACKET. -

5" O.C.

「IMBERSTRAND | 1700

Fb (psi) | E (10⁶ psi) |

1.6

1.6

1.6

1.8

2.0

(2) ROWS OF

.131 x 3 1/4" GN

5" O.C.

4" O.C.

3" O.C.

1200

1050

975

2400

2900

2900

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS

VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN \$IZING CALCS. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAIVINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY F&C TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

CONFIRM SITE CO	IDITIONS, FOUNDATION BEARING CAPACITY	, GRADE AND
BACKFILL HEIGHT	WIND SPEED AND DEBRIS ZONE, AND FLOO	D ZONE.
PROVIDE MATERIA	LS AND CONSTRUCTION TECHNIQUES, WHI	CH COMPLY WITH FBCR 2004
REQUIREMENTS F	OR THE STATED WIND VELOCITY AND DESIG	GN PRESSURES.
BELIEVE THE PLAI	IUOUS LOAD PATH FROM TRUSSES TO FOU OMITS A CONTINUOUS LOAD PATH CONNE GINEER IMMEDIATELY.	
DESIGN, PLACEME	MANUFACTURER'S SEALED ENGINEERING NT PLANS, TEMPORARY AND PERMANENT B CONNECTIONS, AND UPLIFT AND REACTION NS.	RACING DETAILS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

DESIGN LOADS

FLOOR 40 PSF (ALL OTHER DWELLING ROOMS)

10 PSF (ATTICS WITHOUT STORAGE, <3:12)

30 PSF (SLEEPING ROOMS) 30 PSF (ATTICS WITH STORAGE)

ROOF 20 PSF (FLAT OR <4:12)

SOIL BEARING CAPACITY 1000PSF

16 PSF (4:12 TO <12:12)

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

12 PSF (12:12 AND GREATER)

STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)

WIND LOADS PER FLORIDA BUILDING CO	DDE 2004 RESIDENTIAL, SECTION R301.2.1						
MEAN ROOF HEIGHT NOT EXCEEDING LE ON UPPER HALF OF HILL OR ESCARPME	NGS WITH FLAT, HIPPED, OR GABLE ROC EAST HORIZONTAL DIMENSION OR 60 FT; NT 60FT IN EXP. B, 30FT IN EXP. C AND > OR 50x HEIGHT OR 1 MILE WHICHEVER IS						
BUILDING IS NOT IN THE HIGH VELOCITY	HURRICANE ZONE						
BUILDING IS NOT IN THE WIND-BORNE D	EBRIS REGION						
1.) BASIC WIND SPEED = 110 MPH							
2.) WIND EXPOSURE = B							
) WIND IMPORTANCE FACTOR = 1.0							
) BUILDING CATEGORY = II							
ROOF ANGLE = 10-45 DEGREES							
6.) MEAN ROOF HEIGHT = <30 FT							
7.) INTERNAL PRESSURE COEFFICIENT	= N/A (ENCLOSED BUILDING)						
	IGN WIND PRESSURES (TABLE R301.2(2)						
**	Zone Effective Wind Area (ft						
	1 19.9 -21.8 18.1 -18.						
3 2	2 19.9 -25.5 18.1 -21.						
1 7	2 O'hg -40.6 -40						
2 2 2 2 5	3 19.9 -25.5 18.1 -21						
4	3 O'hg -68.3 -42						
	4 21.8 -23.6 18.5 -20						
2	5 21.8 -29.1 18.5 -22.						
The state of the s	Doors & Windows 21.8 -29						
	Worst Case						
2/ 2	(Zone 5, 10 ft2)						
	31 32 31 32 31 32 32 33 33 34 34 35 34 35 34 34 34 34 34 34 34 34 34 34 34 34 34						
3 2 2 5 5 2 1 33 S	8x7 Garage Door 19.5 -22						

VINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419 DIMENSIONS:

REVISIONS

SOFTPLAN

dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification. COPYRIGHTS AND PROPERTY RIGHTS Mark Disosway, P.E. hereby expressly reserv ts common law copyrights and property right in these instruments of service. This document is

not to be reproduced, altered or copied in any form or manner without first the express written permission and consent of Mark Disosway. CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineeri comply with section R301.2.1, florida building

LIMITATION: This design is valid for one

code residential 2004, to the best of my

building, at specified location. P.E. 53915

CARLOS TILLMAN RESIDENCE

ADDRESS: COLUMBIA COUNTY FLORIDA

Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

PRINTED DATE: October 25, 2006 DRAWN BY: STRUCTURAL BY Ben Sparks Ben Sparks

FINALS DATE:

25 / Oct/ 06 JOB NUMBER: 601204

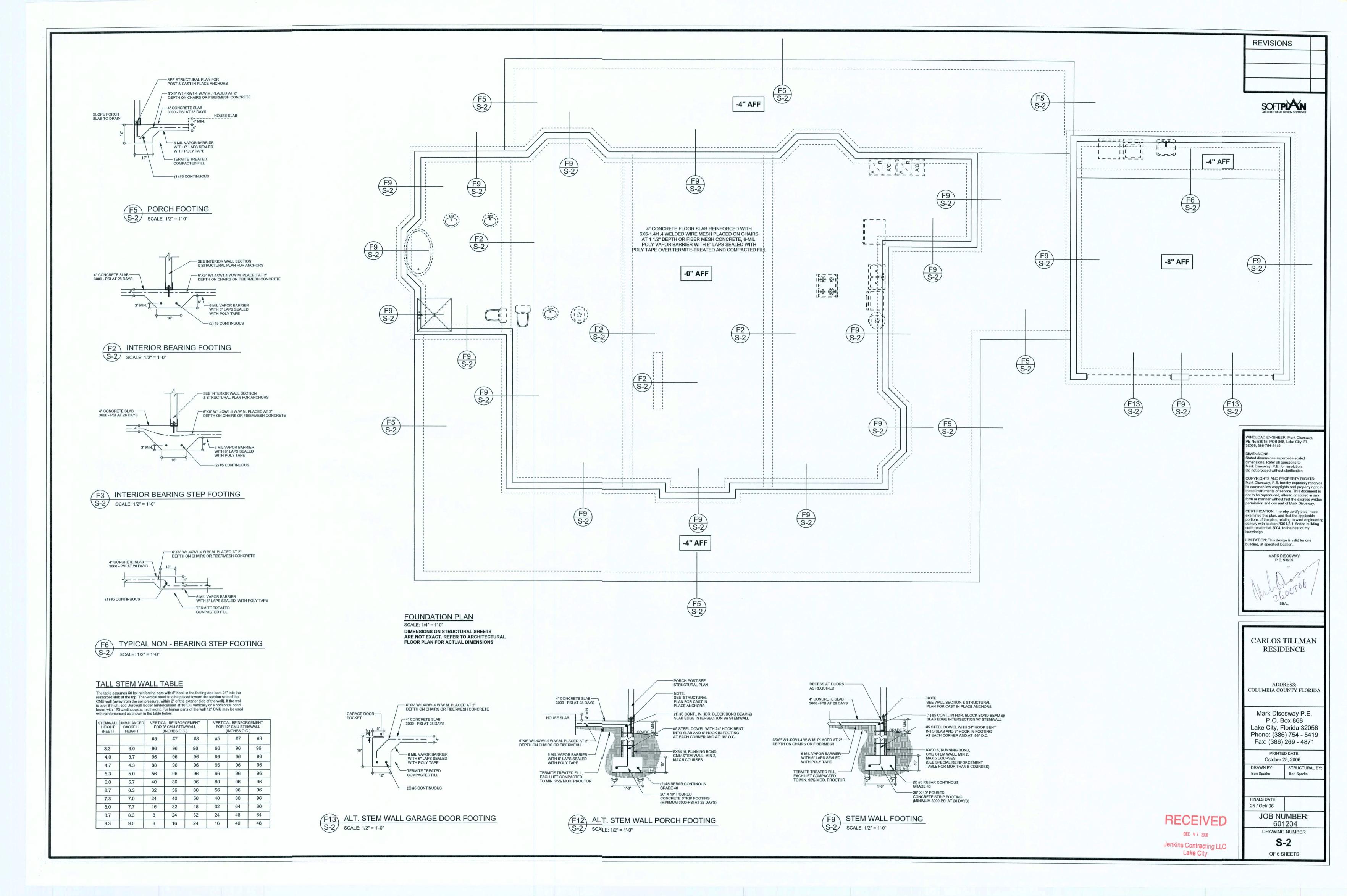
> DRAWING NUMBER **S-1**

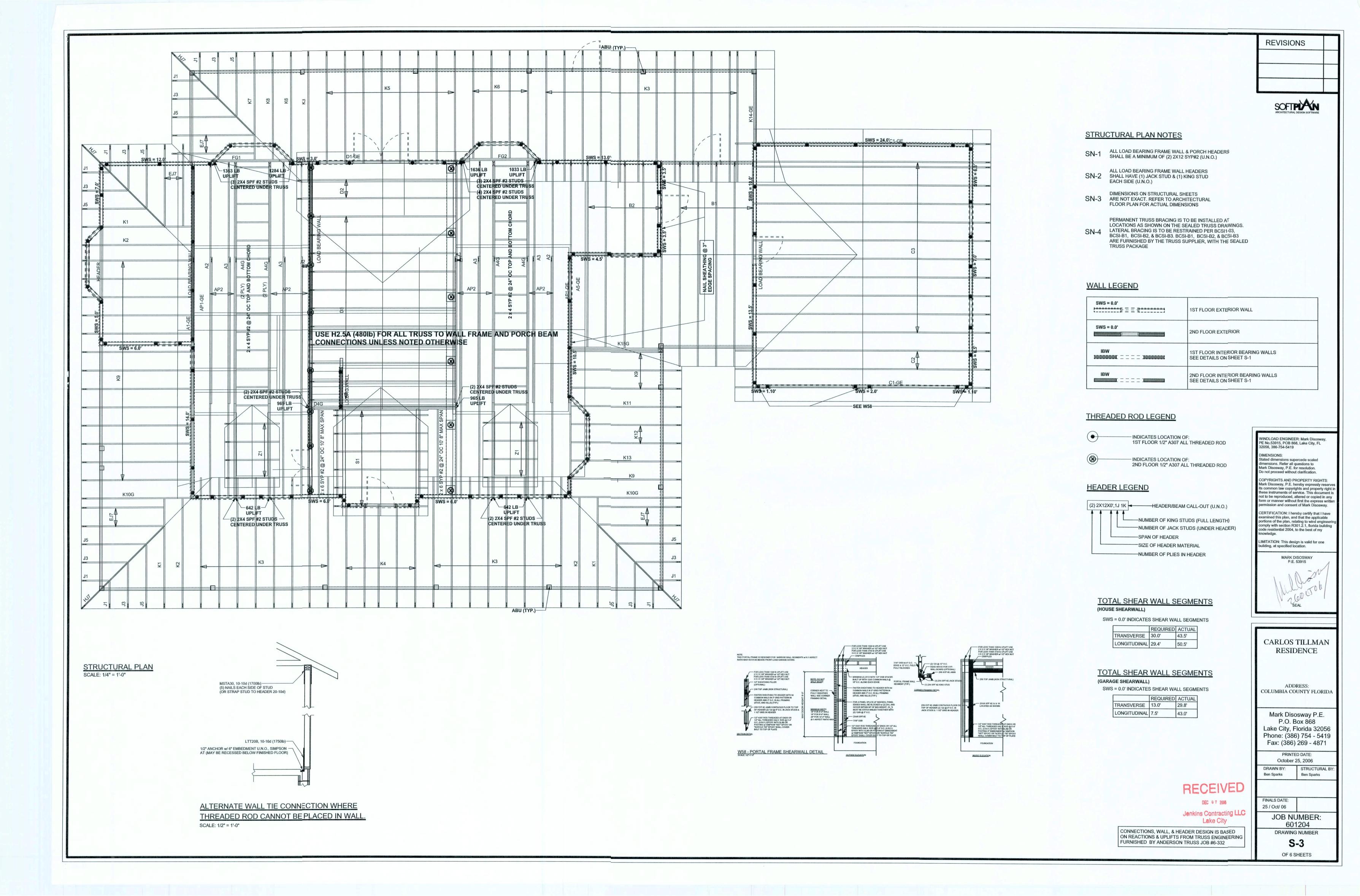
> > OF 6 SHEETS

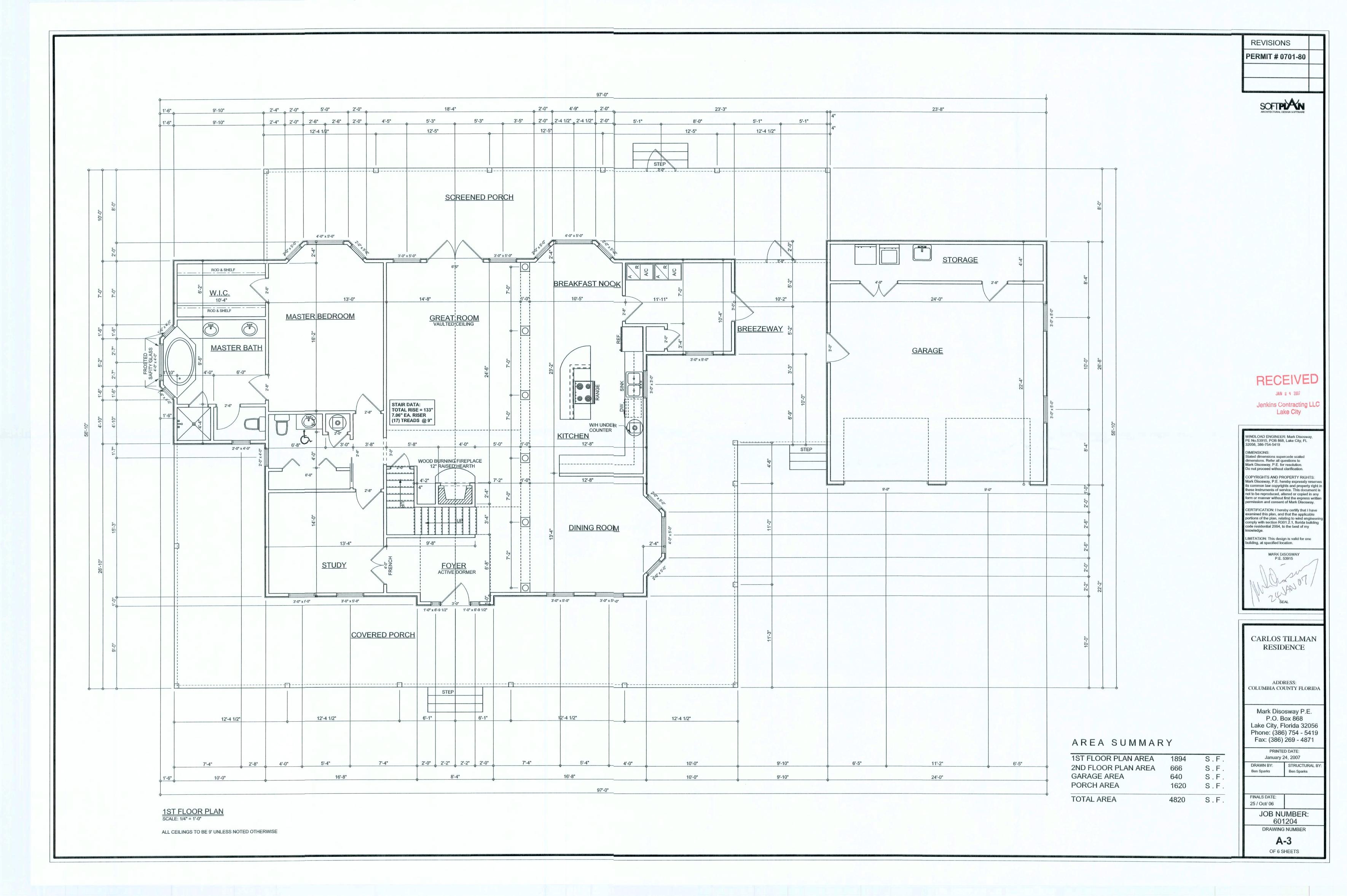
onkins Contracting LL

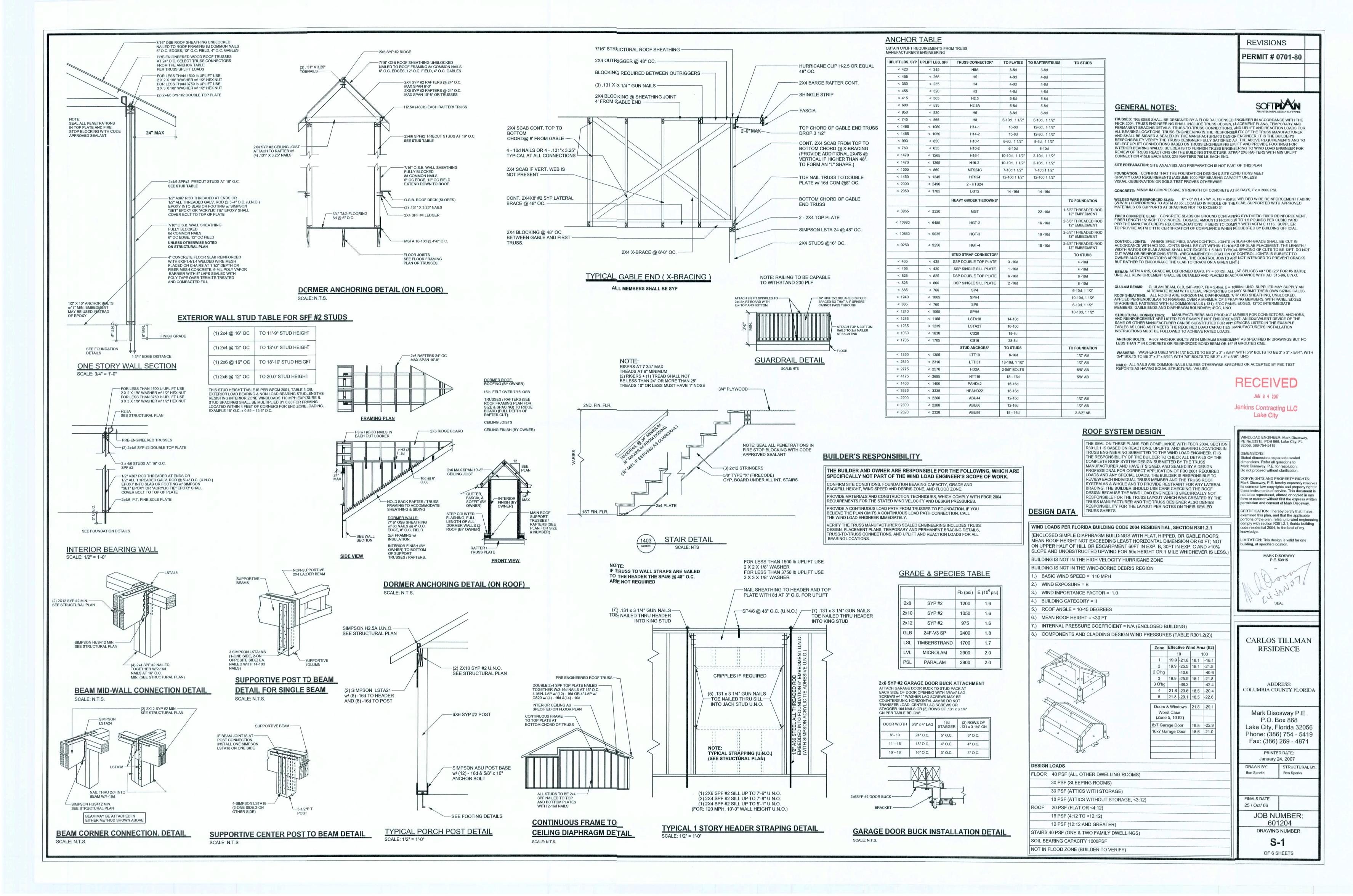
Lake City

GARAGE DOOR BUCK INSTALLATION DETAIL









NOTE: PROVIDE MONOLYTHIC FOOTINGS AT PERIMETER OF ALL FUTURE CARPORT AREAS TO SUPPORT CARPORT ROOF ON COLUMNS WITH MOMENT CONNECTION TO FOOTING AND BEAM WITH EPOXY SET ANCHORS

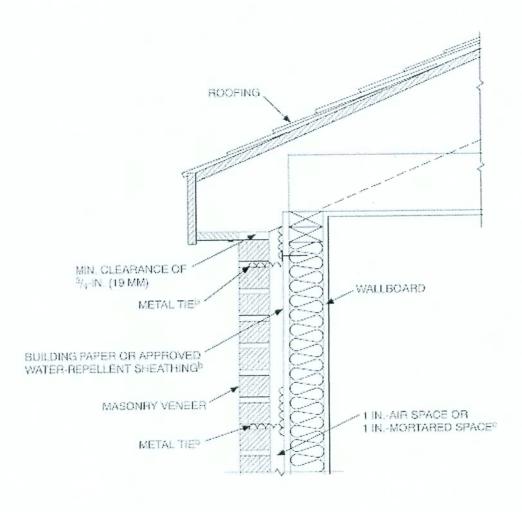
FUTURIZ FUTURE COLUMNS 2000 PSI CONCRETE - FUTUREEPOXEY SET ANCHORS (4)-45 CONTINUOUS 2" COVER TOP 3" COVER SIDES + BOTTOM TERMITETREATED COMPACTEDIFILL L 6MIN POLY VAPOR BARRER

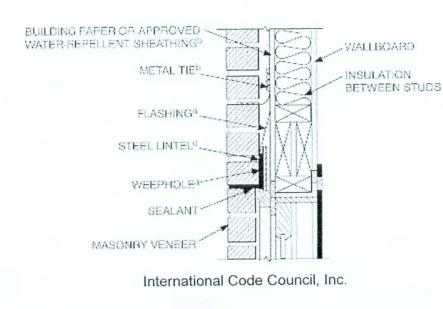
6"LAPS SEALED WITH POLY TAPE 36" X 14" POURED .

CONCRETE STRIP FOOTING

(MINIMUM 3000-PSI AT 28 DAYS)

Florida Building Code, 2004 Complete Collection



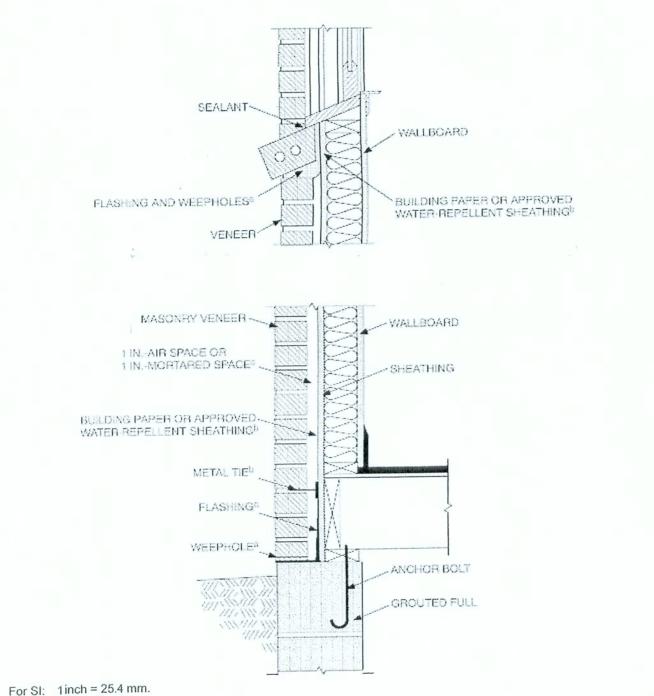


-6"X6" W1.4XW1.4 W.W.M. PLACED AT 2" DEPTH ON CHAIRS OR FIBERMESH CONCRETE -4" CONCRETE SLAB ----6 MIL VAPOR BARRIER WITH POLY TAPE TERMITE TREATED COMPACTED FILL (2) #5 CONTINUOUS

MONOLITHIC FOOTING SCALE: 1/2" = 1'-0"

USE THISFOOTING FOR MOND SLABS FOR FUTURE CARPORTS OR PORCHES THAT WE DON'T KNOW WHAT THEY ARE THAT WON'T REQUIRE MOMENTCONPECTIONS FOR THE COLUMNS,

Florida Building Code, 2004 Complete Collection



International Code Council, Inc.

/---(3) 6" STD RISERS

Residential 2004 / CHAPTER 7 WALL COVERING / SECTION R703 EXTERIOR COVERING / R703.7 Stone and masonry veneer, general. / R703.7.3 Lintels.

R703.7.3 Lintels.

Masonry veneer shall not support any vertical load other than the dead load of the veneer above. Veneer above openings shall be supported on lintels of non-combustible materials and the allowable span shall not exceed the values set forth in Table R703.7.3. The lintels shall have a length of bearing of not less than 4 inches (102 mm).

TABLE R703.7.3

ALLOWABLE SPANS FOR LINTELS SUPPORTING MASONRY VENEERa,b,c

SIZE OF STEEL ANGLE ^{a,c} (inches)	NO STORY ABOVE	ONE STORY ABOVE	TWO STORIES ABOVE	NO. OF ½" OR EQUIVALENT REINFORCING BARS ^C
3 × 3 × 1/4	6′-0″	4'-6"	3'-0"	1
4 × 3 × 1/4	8'-0"	6′-0″	4'-6"	1
5 × 3½ × 5/16	10'-0"	8'-0"	6'-0"	2
6 × 3½ × ⁵ / ₁₆	14'-0"	9'-6"	7'-0"	2
2-6 × 3½ × ⁵ / ₁₆	20'-0"	12'-0"	9′-6″	4

- For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm.
- a. Long leg of the angle shall be placed in a vertical position. b. Depth of reinforced lintels shall not be less than 8 inches and all cells of hollow masonry lintels shall be grouted
- sollid. Reinforcing bars shall extend not less than 8 inches into the support.
- c. Streel members indicated are adequate typical examples; other steel members meeting structural design requirements may be used.

R703.7.4 Anchorage.

YS)

Masonry veneer shall be anchored to the supporting wall with corrosion-resistant metal ties. Where veneer is anchored to wood backings through the use of corrugated sheet metal ties, the distance separating the veneer from the sheathing material shall be a maximum of 1 inch (25.4 mm). Where the veneer is anchored to wood backings through the use of metal strand wire ties, the distance separating the veneer from the sheathing material shall be a Imaximum of 41/2 inches (114 mm). Where the veneer is anchored to cold-formed steel backings, adjustable metal strand wire ties shall be used. Where veneer is anchored to cold-formed steel backings, the distance separating the veneer from the sheathing material shall be a maximum of 4.5 inches (114 mm).

R703.7.4.1 Size and spacing.

Veneer ties, if strand wire, shall not be less in thickness than No. 9 U.S. gage wire and shall have a hood embedded in the mortar joint, or if sheet metal, shall be not less than No. 22 U.S. gage by 7/8 inch (22.3 mm) corrugated. Each tie shall be spaced not more than 24 inches (610 mm) on center horizontally and vertically and shall support not more than 2.67 square feet (0.248 m2) of wall area.

Exception: Where the wind pressure exceeds 30 pounds per square foot pressure (1.44 kN/m²), each tie shall support not more than 2 square feet (0.186 m²) of wall

R703.7.4.1.1 Veneer ties around wall openings. Veneer ties around wall openings. Additional metal ties shall be provided around all wall openings greater than 16 inches (406 mm) in either dimension. Metal ties around the perimeter of openings shall be spaced not more than 3 feet (9144 mm) on center and placed within 12 inches (305 mm) of the wall opening.

R703.7.4.2 Air space.

The veneer shall be separated from the sheathing by an air space of a minimum of 1 inch (25.4 mm) but not more than 4.5 inches (114 mm). The weather-resistant membrane or asphalt-saturated felt required by Section R703.2 is not required over water-repellent sheathing materials.

Residential 2004 / CHAPTER 7 WALL COVERING / SECTION R703 EXTERIOR COVERING / R703.7 Stone and masonry veneer, general. / R703.7.2 Exterior veneer support.

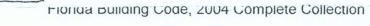
R703.7.2 Exterior veneer support

Exterior masonry veneers having an installed weight of 40 pounds per square foot (195 kg/m²) or less shall be permitted to be supported on wood or cold-formed steel construction. When masonry veneer supported by wood or cold-formed steel construction adjoins masonry veneer supported by the foundation, there shall be a movement joint between the veneer supported by the wood or cold-formed steel construction and the veneer supported by the foundation. The wood or cold-formed steel construction supporting the masonry veneer shall be designed to limit the deflection to $^{1}/_{600}$ of the span for the supporting members. The design of the wood or cold-formed steel construction shall consider the weight of the veneer and any other loads.

R703.7.2.1 Support by steel angle.

A minimum 6 inches by 4 inches by $\frac{5}{16}$ inch (152 mm by 102 mm by 8 mm) steel angle, with the long leg placed vertically, shall be anchored to double 2 inches by 4 inches (51 mm by 102 mm) wood studs at a maximum on center spacing of 16 inches (406 mm). Anchorage of the steel angle at every double stud spacing shall be a minimum of two 7/16 inch (11.1 mm) diameter by 4 inches (102 mm) lag screws. The steel angle shall have a minimum clearance to underlying construction of 1/16 inch (1.6 mm). A minimum of two-thirds the width of the masonry veneer thickness shall bear on the steel angle. Flashing and weep holes shall be located in the masonry veneer wythe in accordance with Figure R703.7.1. The maximum height of masonry veneer above the steel angle support shall be 12 feet, 8 inches (3861 mm). The air space separating the masonry veneer from the wood backing shall be in accordance with R703.7.4 and R703.7.4.2. The method of support for the masonry veneer on wood construction shall be constructed in accordance with Figure R703.7.2.1.

The maximum slope of the roof construction without stops shall be 7:12. Roof construction with slopes greater than 7:12 but not more than 12:12 shall have stops of a minimum 3 inches by 3 inches by 1/4 inch (76 mm by 76 mm by 6 mm) steel plate welded to the angle at 24 inches (610 mm) on center along the angle or as approved by the building official.



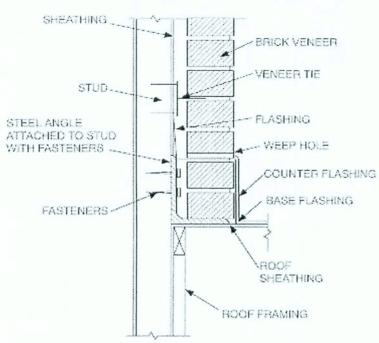


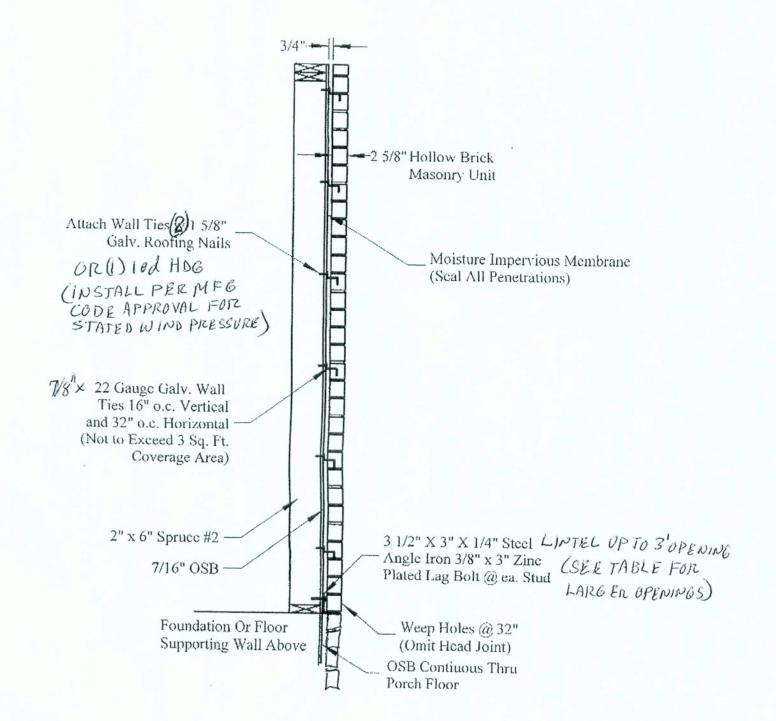
FIGURE R 703.7.2.2 EXTERIOR MASONRY VENEER SUPPORT BY STEEL ANGLES

12-09-2005 10:18 CEMENT PRECAST 3523784611

REVISIONS ADDENDUM BRICK DETAILS

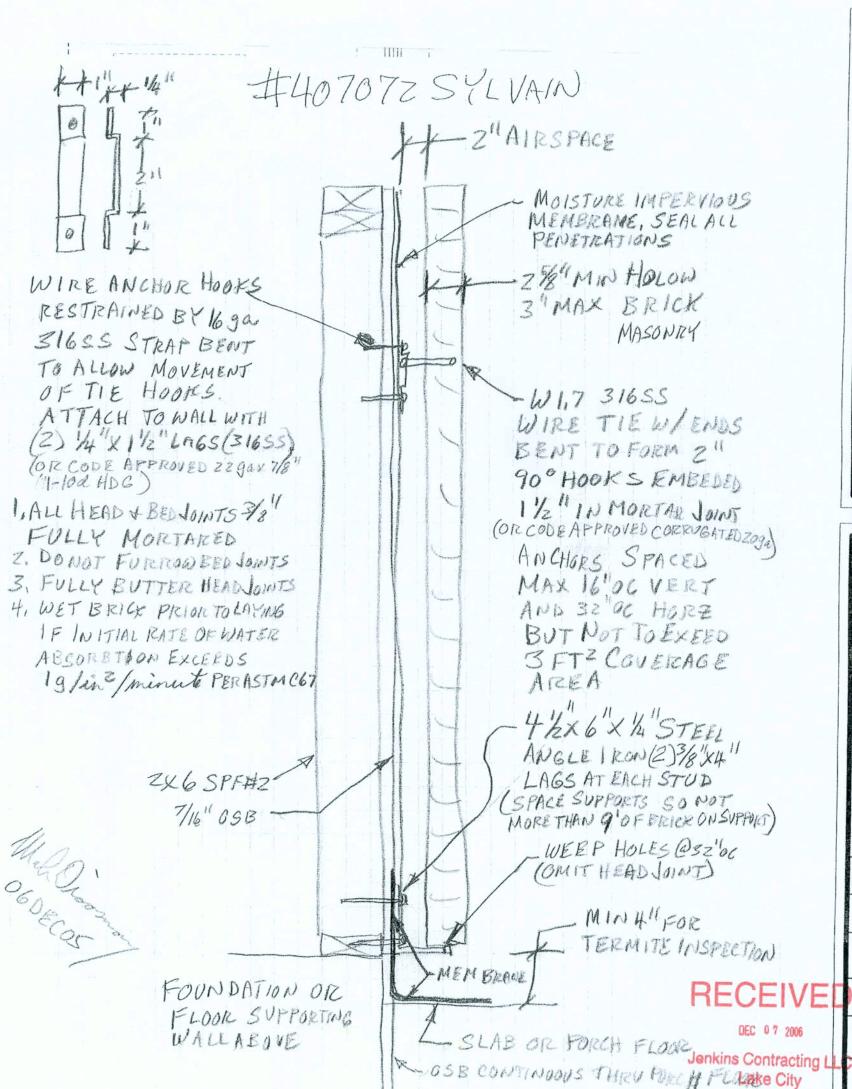
PAGE2

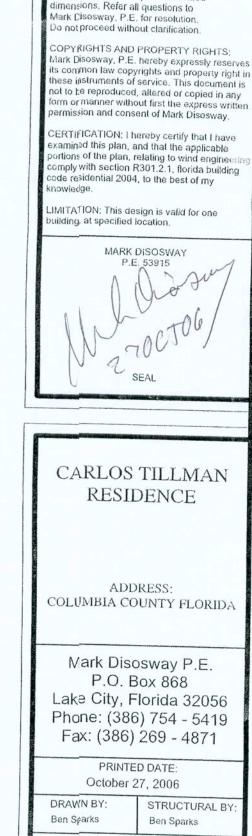
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FINALS DATE:

JOB NUMBER:

601204

DRAWING NUMBER

OF 6 SHEETS

WINDLOAD ENGINEER: Mark Disoswa PE No.53915, POB 868, Lake City, FL

32056.386-754-5419

DIMENSIONS: