

| REVISIONS | |
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SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBC 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS TO TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END, 2X6 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE)

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, $F_c = 3000$ PSI.

WELDED WIRE REINFORCED SLAB: 6" x 8" W1.4 x W1.4, $F_y = 85$ KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.R.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT, FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT W.W.R. OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, $F_y = 60$ KSI. ALL LAP SPLICES 48" DB (30" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 318-86, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, $F_b = 2.4$ ksi, $E = 1800$ ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALC.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, STAGGERED, FASTENED WITH 8d COMMON NAILS (131), 6" OC PANEL EDGES, 12" OC INTERMEDIATE MEMBERS; GABLE ENDS AND DIAPHRAGM BOUNDARY, 4" OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED CONCRETE OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBC 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION, IF YOU BELIEVE THE PLAN OMIT'S A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBC 2004, SECTION 1609 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2004 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

DESIGN DATA

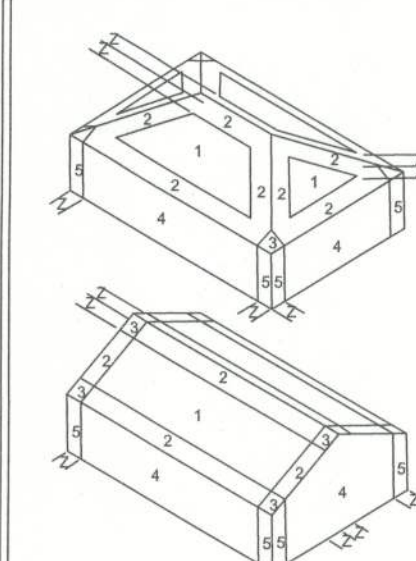
WIND LOADS PER FLORIDA BUILDING CODE 2004, SECTION 1609

(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 50FT IN EXP. B, 30FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

1. BASIC WIND SPEED = 110 MPH
2. WIND EXPOSURE = B
3. WIND IMPORTANCE FACTOR = 1.0
4. BUILDING CATEGORY = II
5. ROOF ANGLE = 10-45 DEGREES
6. MEAN ROOF HEIGHT = <30 FT
7. INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING, 1609.6)
8. COMPONENTS AND CLADDING DESIGN WIND PRESSURES (FBC TABLE 1609.6 B&C)



| Zone | Effective Wind Area (ft ²) | | |
|-------|--|-------|------|
| 1 | 19.9 | -21.8 | 18.1 |
| 2 | 19.9 | -25.5 | 18.1 |
| 2.07g | -40.6 | -40.6 | |
| 3 | 19.9 | -25.5 | 18.1 |
| 3.07g | -68.3 | -42.4 | |
| 4 | 21.8 | -23.6 | 18.5 |
| 5 | 21.8 | -29.1 | 18.5 |

| Doors & Windows | Worst Case (Zone 5, 10 ft ²) | 21.8 | -29.1 |
|------------------|--|-------|-------|
| 8x7 Garage Door | 19.5 | -22.9 | |
| 16x7 Garage Door | 18.5 | -21.0 | |

DESIGN LOADS

| | |
|---------------------------------------|--|
| FLOOR | 40 PSF (ALL OTHER DWELLING ROOMS) |
| | 30 PSF (SLEEPING ROOMS) |
| | 30 PSF (ATTICS WITH STORAGE) |
| | 10 PSF (ATTICS WITHOUT STORAGE, <3:12) |
| ROOF | 20 PSF (FLAT OR <4:12) |
| | 16 PSF (4:12 TO <12:12) |
| | 12 PSF (12:12 AND GREATER) |
| STAIRS | 40 PSF (ONE & TWO FAMILY DWELLINGS) |
| SOIL BEARING CAPACITY | 1000PSF |
| NOT IN FLOOD ZONE (BUILDER TO VERIFY) | |

WINDLOAD ENGINEER: Mark Disoway,
PE No. 53915, POB 868, Lake City, FL
32056, 386-754-5419

DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section 1609, Florida building code 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOWAY
P.E. 53915

Mark Disoway
1600005
SEAL

Isaac Construction

Cheshire Model
Lot 9 Cobblestone S/D

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Lake City, Florida 32056
Phone: (386) 754 - 5419
Fax: (386) 269 - 4871

PRINTED DATE:
November 16, 2005
ENGINEERING DRAWN BY:
David Disoway
DESIGNED BY:

FINALS DATE:
16 / Nov / 05

JOB NUMBER:
510102

DRAWING NUMBER

S-1
OF 3 SHEETS

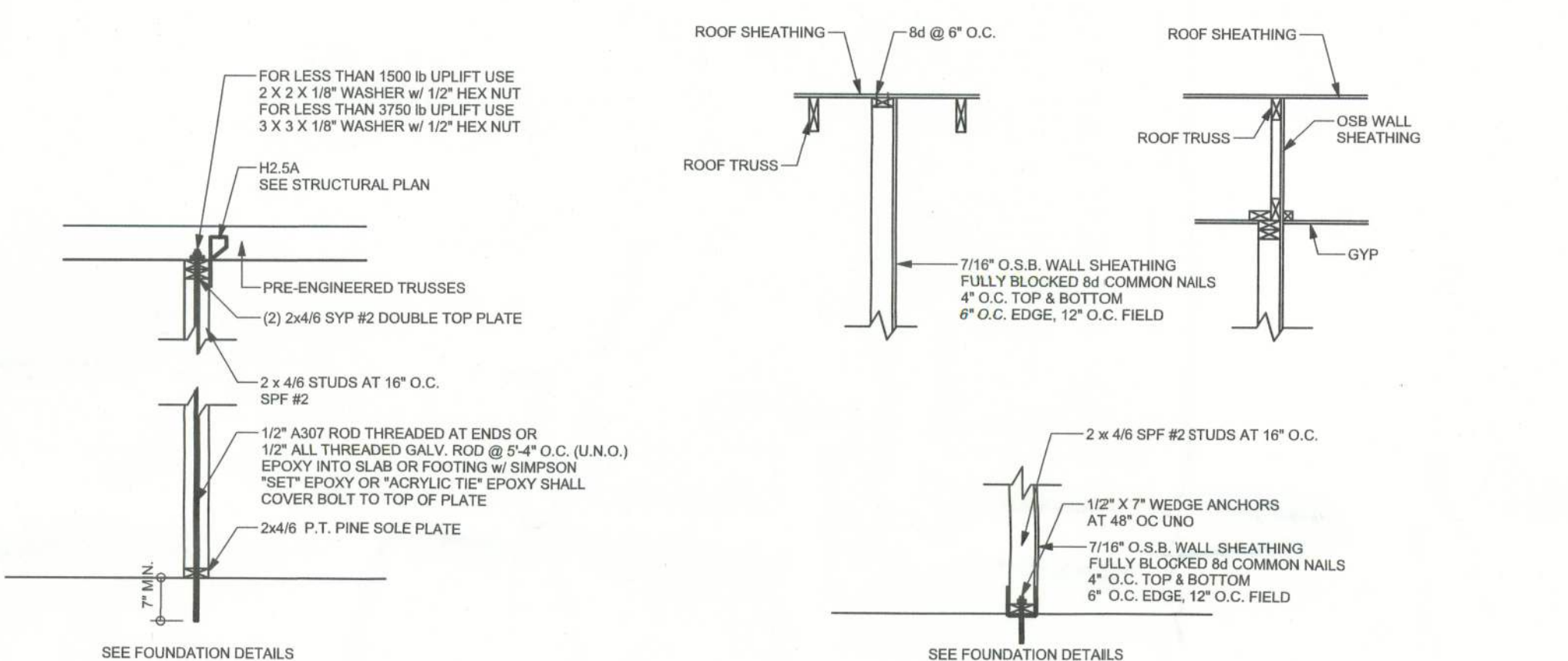
ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

| UPLIFT LBS. SYP | UPLIFT LBS. SPF | TRUSS CONNECTOR* | TO PLATES | TO RAFTER/TRUSS | TO STUDS |
|------------------------|-----------------|-----------------------|----------------|-----------------|-----------------------------------|
| < 420 | < 245 | H5A | 3-8d | 3-8d | |
| < 455 | < 265 | H5 | 4-8d | 4-8d | |
| < 360 | < 235 | H4 | 4-8d | 4-8d | |
| < 455 | < 320 | H3 | 4-8d | 4-8d | |
| < 415 | < 365 | H2.5 | 5-8d | 5-8d | |
| < 600 | < 535 | H2.5A | 5-8d | 5-8d | |
| < 950 | < 820 | H6 | 8-8d | 8-8d | |
| < 745 | < 565 | H9 | 5-10d, 1 1/2" | 5-10d, 1 1/2" | |
| < 1465 | < 1050 | H14-1 | 13-8d | 12-8d, 1 1/2" | |
| < 1465 | < 1050 | H14-2 | 15-8d | 12-8d, 1 1/2" | |
| < 990 | < 850 | H10-1 | 8-8d, 1 1/2" | 8-8d, 1 1/2" | |
| < 780 | < 655 | H10-2 | 6-10d | 6-10d | |
| < 1470 | < 1285 | H16-1 | 10-10d, 1 1/2" | 2-10d, 1 1/2" | |
| < 1470 | < 1285 | H16-2 | 10-10d, 1 1/2" | 2-10d, 1 1/2" | |
| < 1000 | < 860 | MTS24C | 7-10d 1 1/2" | 7-10d 1 1/2" | |
| < 1450 | < 1245 | HTS24 | 12-10d 1 1/2" | 12-10d 1 1/2" | |
| < 2900 | < 2490 | 2 - HTS24 | | | |
| < 2050 | < 1785 | LG2 | 14-16d | 14-16d | |
| HEAVY GIRDER TIEDOWNS* | | | | | TO FOUNDATION |
| < 3965 | < 3330 | MG | | 22-10d | 1-5/8" THREADED ROD 12" EMBEDMENT |
| < 10980 | < 6485 | HGT-2 | | 16-10d | 2-5/8" THREADED ROD 12" EMBEDMENT |
| < 10530 | < 9035 | HGT-3 | | 16-10d | 2-5/8" THREADED ROD 12" EMBEDMENT |
| < 9250 | < 9250 | HGT-4 | | 16-10d | 2-5/8" THREADED ROD 12" EMBEDMENT |
| STUD STRAP CONNECTOR* | | | | | TO STUDS |
| < 435 | < 435 | SSP DOUBLE TOP PLATE | 3-10d | | 4-10d |
| < 455 | < 420 | SSP SINGLE SILL PLATE | 1-10d | | 4-10d |
| < 825 | < 825 | DSP DOUBLE TOP PLATE | 6-10d | | 8-10d |
| < 825 | < 600 | DSP SINGLE SILL PLATE | 2-10d | | 8-10d |
| < 885 | < 760 | SP4 | | | 6-10d, 1 1/2" |
| < 1240 | < 1065 | SPH4 | | | 10-10d, 1 1/2" |
| < 885 | < 760 | SP6 | | | 6-10d, 1 1/2" |
| < 1240 | < 1065 | SPH6 | | | 10-10d, 1 1/2" |
| < 1235 | < 1165 | LSTA18 | 14-10d | | |
| < 1235 | < 1235 | LSTA21 | 16-10d | | |
| < 1030 | < 1030 | CS20 | 16-8d | | |
| < 1705 | < 1705 | CS16 | 28-8d | | |
| STUD ANCHORS* | | | TO STUDS | | TO FOUNDATION |
| < 1350 | < 1305 | LTT19 | 8-16d | | 1/2" AB |
| < 2310 | < 2310 | LTT31 | 18-10d, 1 1/2" | | 1/2" AB |
| < 2775 | < 2570 | HD2A | 2-5/8" BOLTS | | 5/8" AB |
| < 4175 | < 3695 | HTT16 | 18-16d | | 5/8" AB |
| < 1400 | < 1400 | PAHD42 | 16-16d | | |
| < 3335 | < 3335 | HPAH422 | 16-16d | | |
| < 2200 | < 2200 | ABU44 | 12-16d | | 1/2" AB |
| < 2300 | < 2300 | ABU66 | 12-16d | | 1/2" AB |
| < 2320 | < 2320 | ABU88 | 18-16d | | 2-5/8" AB |

TYPICAL GABLE END (X-BRACING)

ALL MEMBERS SHALL BE SYP



INTERIOR BEARING WALL

SCALE: 1/2" = 1'-0"

INTERIOR SHEAR WALL DETAIL

SCALE: 1/2" = 1'-0"

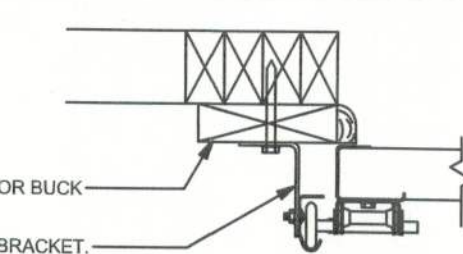
GRADE & SPECIES TABLE

| | | Fb (psi) | E (10 ⁶ psi) |
|------|--------------|----------|-------------------------|
| 2x8 | SYP #2 | 1200 | 1.6 |
| 2x10 | SYP #2 | 1050 | 1.6 |
| 2x12 | SYP #2 | 975 | 1.6 |
| GLB | 24F-V3 SP | 2400 | 1.8 |
| LSL | TIMBERSTRAND | 1700 | 1.7 |
| LVL | MICROLAM | 2900 | 2.0 |
| PSL | PARALAM | 2900 | 2.0 |

2x6 SYP #2 GARAGE DOOR BUCK ATTACHMENT

ATTACH GARAGE DOOR BUCK TO STUD PACK AT EACH SIDE OF DOOR OPENING WITH 3/8"x4" LAG SCREWS w/ 1" WASHER LAG SCREWS MAY BE COUNTERBURN. HORIZONTAL JAMBS DO NOT TRANSFER LOAD, CENTER LAG SCREWS OR STAGGER 16d NAILS OR (2) ROWS OF 131 x 3 1/4" ON PER TABLE BELOW.

| DOOR WIDTH | 3/8" x 4" LAG | 16d STAGGER | (2) ROWS OF 131 x 3 1/4" CN |
|------------|---------------|-------------|-----------------------------|
| 8' - 10' | 24" O.C. | 6" O.C. | 5" O.C. |
| 11' - 15' | 18" O.C. | 4" O.C. | 4" O.C. |
| 16' - 18' | 16" O.C. | 3" O.C. | 3" O.C. |



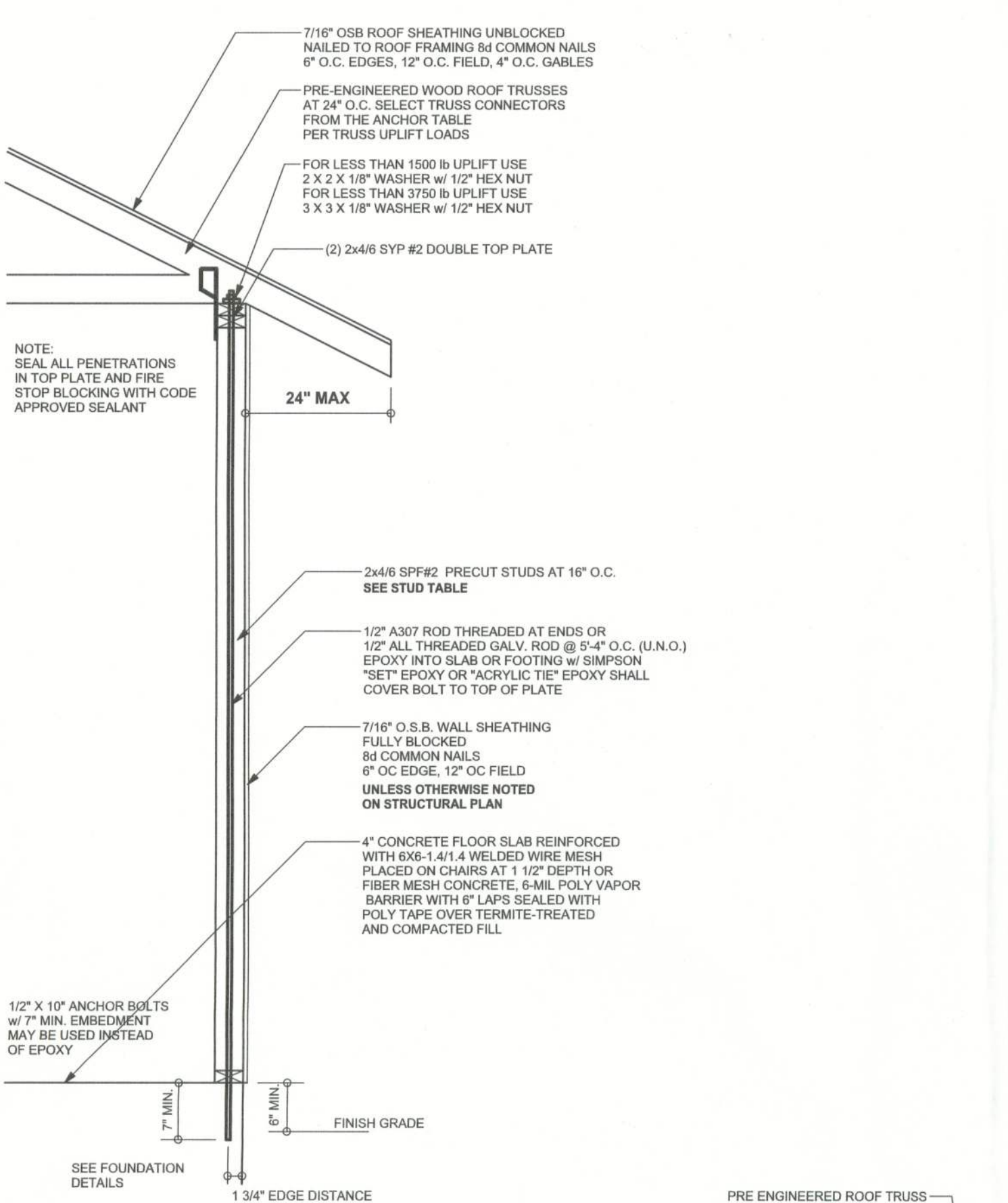
- (1) 2x6 SPF #2 SILL UP TO 7'-6" U.N.O.
- (2) 2x4 SPF #2 SILL UP TO 7'-8" U.N.O.
- (1) 2x4 SPF #2 SILL UP TO 5'-1" U.N.O. (FOR: 120 MPH, 10'-0" WALL HEIGHT U.N.O.)

TYPICAL 1 STORY HEADER STRAPING DETAIL

SCALE: 1/2" = 1'-0"

TYPICAL PORCH POST DETAIL

SCALE: 1/2" = 1'-0"

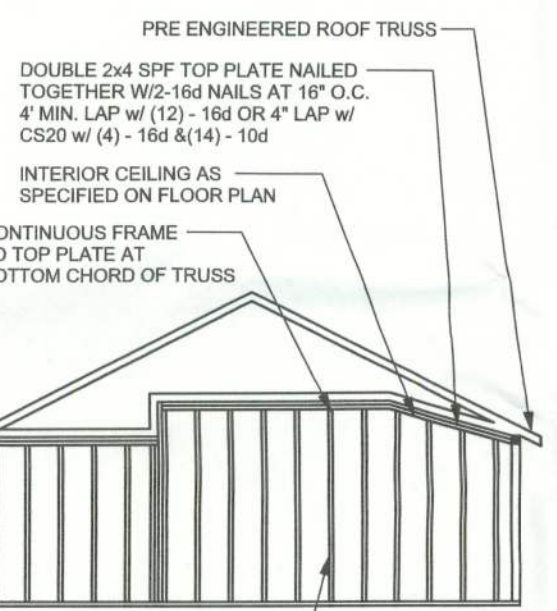


ONE STORY WALL SECTION

SCALE: 3/4" = 1'-0"

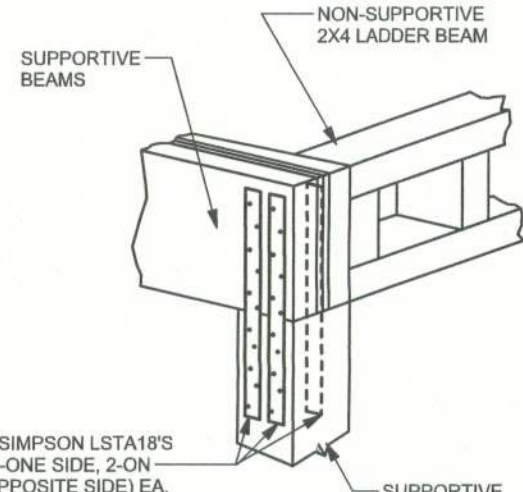
EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

| | |
|------------------|------------------------|
| (1) 2x4 @ 16" OC | TO 10'-9" WALL HEIGHT |
| (1) 2x4 @ 12" OC | TO 13'-0" WALL HEIGHT |
| (1) 2x6 @ 16" OC | TO 18'-10" WALL HEIGHT |
| (1) 2x6 @ 12" OC | TO 20'-0" WALL HEIGHT |



CONTINUOUS FRAME TO CEILING DIAPHRAGM DETAIL

SCALE: N.T.S.

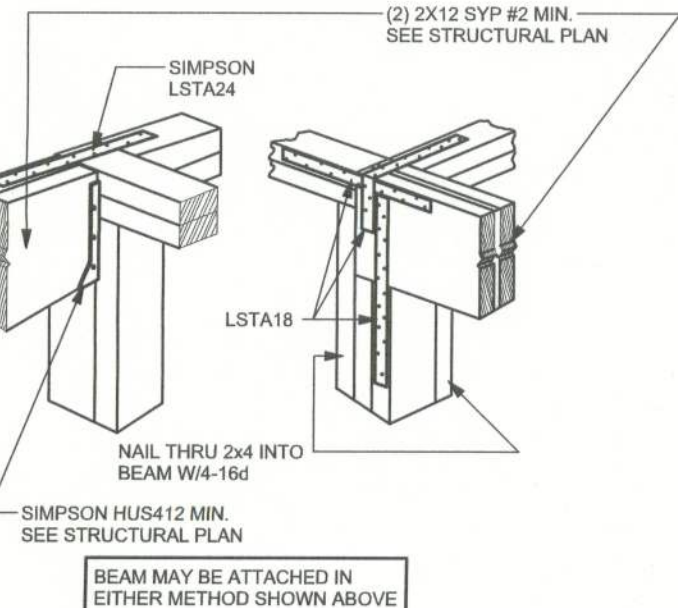


SUPPORTIVE POST TO BEAM DETAIL FOR SINGLE BEAM

SCALE: N.T.S.

BEAM MID-WALL CONNECTION DETAIL

SCALE: N.T.S.



BEAM CORNER CONNECTION DETAIL

SCALE: N.T.S.

SUPPORTIVE CENTER POST TO BEAM DETAIL

SCALE: N.T.S.