| | | | |
|---|---|---|---|
| **Project Information for:** | L221086 | | |
| Builder: | BRYAN ZECHER | Date: | 1/3/2007 |
| Lot: | N/A | Start Number: | **1018** |
| Subdivision: | 639 CR 349 | SEI Ref: | L221086 |
| County or City: | COLUMBIA COUNTY | | |
| Truss Page Count: | 72 | | |

**Truss Design Load Information (UNO)** Design Program: MiTek

| Gravity | | Wind | | Building Code: | FBC2004 |
|---|---|---|---|---|---|
| Roof (psf): | **42** | Wind Standard: | **ASCE 7-02** | | |
| Floor (psf): | 55 | Wind Speed (mph): | **110** | | |

Note: See individual truss drawings for special loading conditions

**Building Designer, responsible for Structural Engineering: (See attached)**

ZECHER, BRYAN C. CBC 054575
Address: PO BOX 815
LAKE CITY,FLORIDA 32056 Designer: 199

**Truss Design Engineer:** Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987
Company: Structural Engineering and Inspections, Inc. EB 9196
Address 16105 N. Florida Ave, Ste B, Lutz, FL 33549 Phone: 813-849-5769

Notes:

1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
4. Trusses designed for veritcal loads only, unless noted otherwise.
5. Where hangers are shown, Carried Member hanger capacity per Simpson C-2006 (SYP/Full Nailing Value) as an individual component. Building Designer shall verify the suitablity and use of Carrying Member hanger capacity.

| # | Truss ID | Dwg. # | Seal Date | # | Truss ID | Dwg. # | Seal Date |
|---|---|---|---|---|---|---|---|
| 1 | CJ1 | 0103071018 | 1/3/2007 | 41 | T12 | 0103071058 | 1/3/2007 |
| 2 | CJ3 | 0103071019 | 1/3/2007 | 42 | T13 | 0103071059 | 1/3/2007 |
| 3 | CJ3T | 0103071020 | 1/3/2007 | 43 | T14 | 0103071060 | 1/3/2007 |
| 4 | CJ4 | 0103071021 | 1/3/2007 | 44 | T15 | 0103071061 | 1/3/2007 |
| 5 | CJ5 | 0103071022 | 1/3/2007 | 45 | T16 | 0103071062 | 1/3/2007 |
| 6 | CJ5T | 0103071023 | 1/3/2007 | 46 | T17 | 0103071063 | 1/3/2007 |
| 7 | CJ5TA | 0103071024 | 1/3/2007 | 47 | T18 | 0103071064 | 1/3/2007 |
| 8 | EJ2 | 0103071025 | 1/3/2007 | 48 | T19 | 0103071065 | 1/3/2007 |
| 9 | EJ2A | 0103071026 | 1/3/2007 | 49 | T20 | 0103071066 | 1/3/2007 |
| 10 | EJ2B | 0103071027 | 1/3/2007 | 50 | T21 | 0103071067 | 1/3/2007 |
| 11 | EJ2C | 0103071028 | 1/3/2007 | 51 | T22 | 0103071068 | 1/3/2007 |
| 12 | EJ2D | 0103071029 | 1/3/2007 | 52 | T22 | 0103071069 | 1/3/2007 |
| 13 | EJ5A | 0103071030 | 1/3/2007 | 53 | T23 | 0103071070 | 1/3/2007 |
| 14 | EJ5B | 0103071031 | 1/3/2007 | 54 | T23 | 0103071071 | 1/3/2007 |
| 15 | EJ6 | 0103071032 | 1/3/2007 | 55 | T23A | 0103071072 | 1/3/2007 |
| 16 | EJ7 | 0103071033 | 1/3/2007 | 56 | T23B | 0103071073 | 1/3/2007 |
| 17 | EJ7G | 0103071034 | 1/3/2007 | 57 | T24 | 0103071074 | 1/3/2007 |
| 18 | EJ7GA | 0103071035 | 1/3/2007 | 58 | T25 | 0103071075 | 1/3/2007 |
| 19 | EJ7T | 0103071036 | 1/3/2007 | 59 | T26 | 0103071076 | 1/3/2007 |
| 20 | EJ7TA | 0103071037 | 1/3/2007 | 60 | T27 | 0103071077 | 1/3/2007 |
| 21 | EJ7TB | 0103071038 | 1/3/2007 | 61 | T28 | 0103071078 | 1/3/2007 |
| 22 | HJ2 | 0103071039 | 1/3/2007 | 62 | T29 | 0103071079 | 1/3/2007 |
| 23 | HJ8 | 0103071040 | 1/3/2007 | 63 | T30 | 0103071080 | 1/3/2007 |
| 24 | HJ9 | 0103071041 | 1/3/2007 | 64 | T31 | 0103071081 | 1/3/2007 |
| 25 | HJ9T | 0103071042 | 1/3/2007 | 65 | T32 | 0103071082 | 1/3/2007 |
| 26 | HJ9TA | 0103071043 | 1/3/2007 | 66 | T33 | 0103071083 | 1/3/2007 |
| 27 | T01 | 0103071044 | 1/3/2007 | 67 | T34 | 0103071084 | 1/3/2007 |
| 28 | T02 | 0103071045 | 1/3/2007 | 68 | T35 | 0103071085 | 1/3/2007 |
| 29 | T03 | 0103071046 | 1/3/2007 | 69 | T36 | 0103071086 | 1/3/2007 |
| 30 | T04 | 0103071047 | 1/3/2007 | 70 | T37 | 0103071087 | 1/3/2007 |
| 31 | T04 | 0103071048 | 1/3/2007 | 71 | T38 | 0103071088 | 1/3/2007 |
| 32 | T05 | 0103071049 | 1/3/2007 | 72 | T39 | 0103071089 | 1/3/2007 |
| 33 | T06 | 0103071050 | 1/3/2007 | | | | |
| 34 | T07 | 0103071051 | 1/3/2007 | | | | |
| 35 | T08 | 0103071052 | 1/3/2007 | | | | |
| 36 | T09 | 0103071053 | 1/3/2007 | | | | |
| 37 | T09A | 0103071054 | 1/3/2007 | | | | |
| 38 | T10 | 0103071055 | 1/3/2007 | | | | |
| 39 | T10A | 0103071056 | 1/3/2007 | | | | |
| 40 | T11 | 0103071057 | 1/3/2007 | | | | |

JAN 0 3 2007

FILE COPY

# LATERAL TOE-NAIL DETAIL

ST-TOENAIL

MiTek Industries, Chesterfield, MO



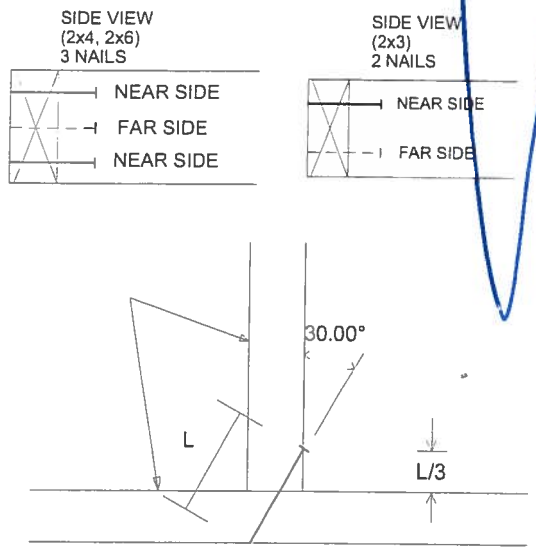
NOTES:
1. TOE-NAILS SHALL BE DRIVEN AT AN ANGLE OF 30 DEGREES WITH THE MEMBER AND STARTED 1/3 THE LENGTH OF THE NAIL FROM THE MEMBER END AS SHOWN.
2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
3. ALLOWABLE VALUE SHALL BE THE LESSER VALUE OF THE BOTTOM CHORD SPECIES FOR MEMBERS OF DIFFERENT SPECIES.

TOE-NAIL SINGLE SHEAR VALUES PER NDS 2001 (lb/nail)

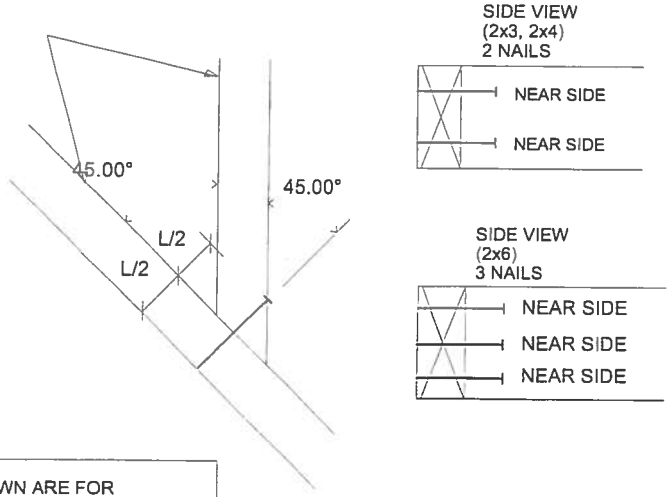
| | DIAM. | SYP |
|---|---|---|
| 3.5" LONG | .131 | 83.3 |
| | .135 | 89.6 |
| | .162 | 118.3 |
| 3.25" LONG | .128 | 80.5 |
| | .131 | 83.3 |
| | .148 | 102.1 |
| 3.0" LONG | .120 | 70.5 |
| | .128 | 80.5 |
| | .131 | 83.3 |
| | .148 | 102.1 |

VALUES SHOWN ARE CAPACITY PER TOE-NAIL.
APPLICABLE DURATION OF LOAD INCREASES MAY BE APPLIED.

## SQUARE CUT




## 45 DEGREE ANGLE BEVEL CUT




VIEWS SHOWN ARE FOR ILLUSTRATION PURPOSES ONLY

This detail may only be applied to Pre-engineered truss drawings signed and sealed by Structural Engineering and Inspections Inc.

JAN 03 2007

Dwg.#0103071018

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | CJ1 | JACK | 18 | 1 | Job Reference (optional) |

Builders FirstSource, Lake City, FL 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 03 09:47:30 2007 Page 1

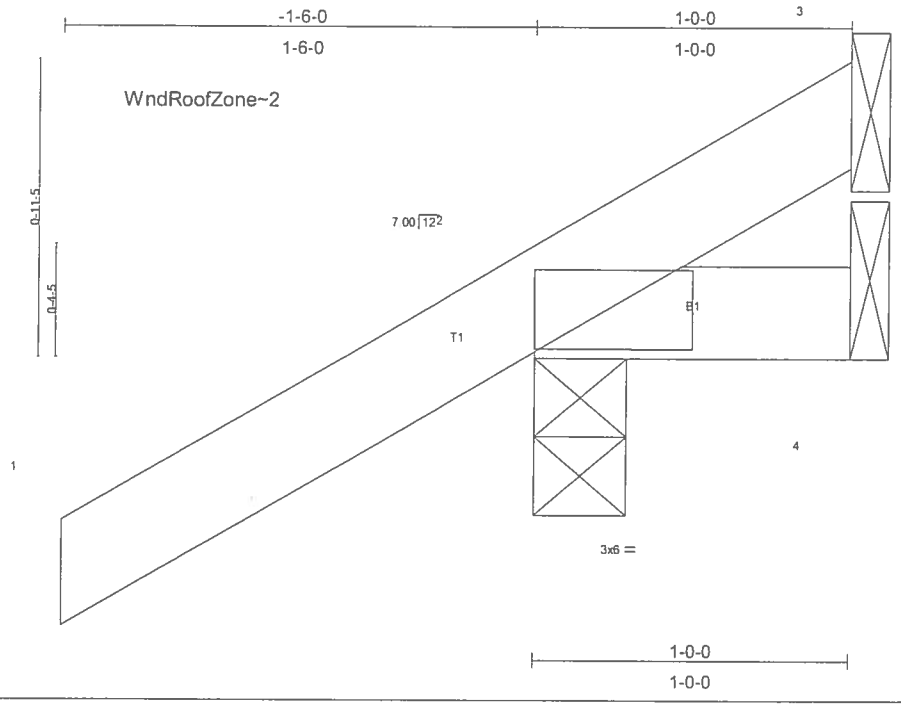



Plate Offsets (X,Y): [2:0-3-3,0-1-8]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.15 | Vert(LL) | -0.00 | 2 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.01 | Vert(TL) | -0.00 | 2 | >999 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.00 | Horz(TL) | 0.00 | 3 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 6 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 2=189/0-3-8, 4=14/Mechanical, 3=-40/Mechanical
Max Horz 2=82(load case 5)
Max Uplift2=-196(load case 5), 4=-11(load case 3), 3=-40(load case 1)
Max Grav 2=189(load case 1), 4=14(load case 1), 3=64(load case 5)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-50/41
BOT CHORD 2-4=0/0

**JOINT STRESS INDEX**
2 = 0.10

**NOTES**
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 196 lb uplift at joint 2, 11 lb uplift at joint 4 and 40 lb uplift at joint 3.

**LOAD CASE(S)** Standard

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549

Dwg.#0103071019

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | CJ3 | JACK | 14 | 1 | Job Reference (optional) |

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 03 09:47:31 2007 Page 1

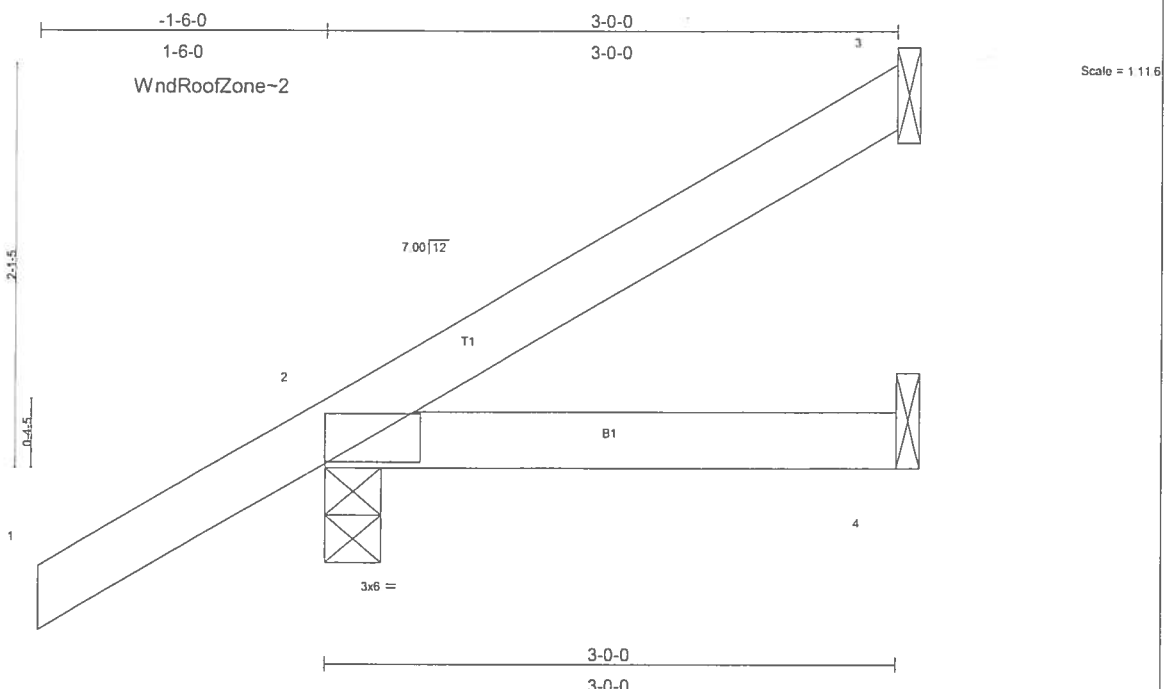



Plate Offsets (X,Y): [2:0-3-3,0-1-8]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.16 | Vert(LL) | 0.01 | 2-4 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.08 | Vert(TL) | -0.01 | 2-4 | >999 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.00 | Horz(TL) | -0.00 | 3 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 12 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 3=49/Mechanical, 2=232/0-3-8, 4=42/Mechanical
Max Horz 2=135(load case 5)
Max Uplift3=-43(load case 5), 2=-181(load case 5), 4=-33(load case 3)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-54/18
BOT CHORD 2-4=0/0

**JOINT STRESS INDEX**
2 = 0.10

**NOTES**
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 43 lb uplift at joint 3, 181 lb uplift at joint 2 and 33 lb uplift at joint 4.

**LOAD CASE(S)** Standard

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0103071020

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | CJ3T | SPECIAL | 2 | 1 | Job Reference (optional) |

Builders FirstSource, Lake City, FI 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 03 09:47:32 2007 Page 1

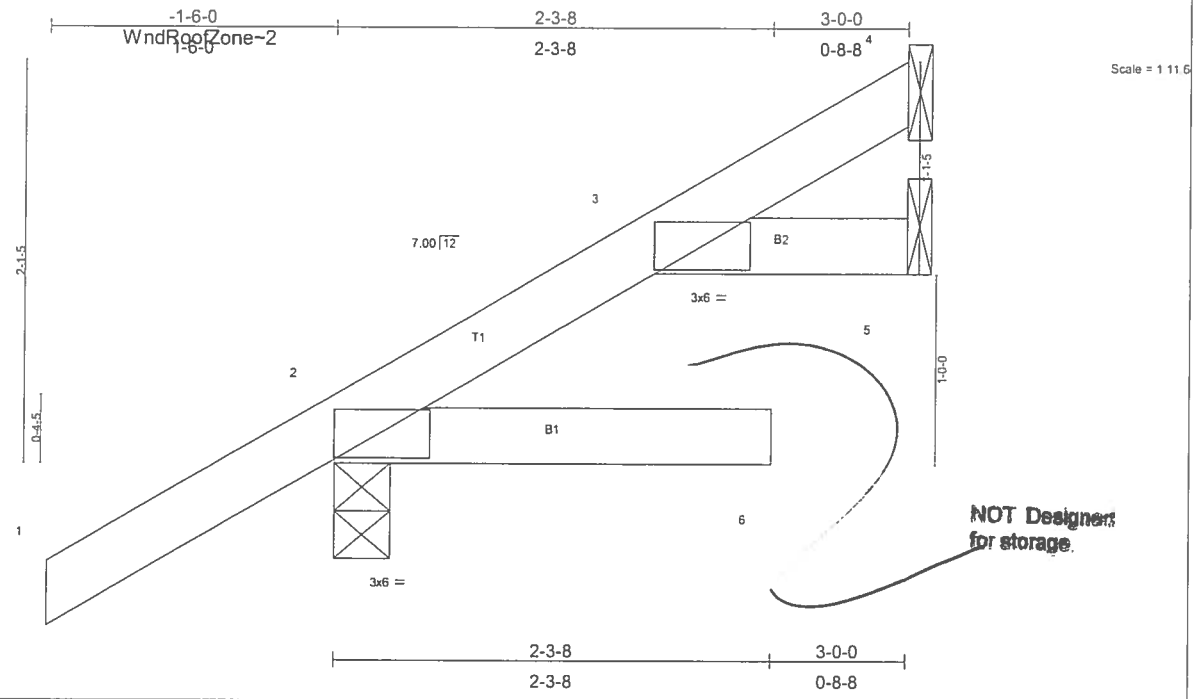



Plate Offsets (X,Y) [2:0-3-4,0-1-8]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.18 | Vert(LL) | -0.03 | 6 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.13 | Vert(TL) | -0.05 | 6 | >652 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.00 | Horz(TL) | 0.01 | 5 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 13 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 4=84/Mechanical, 2=239/0-3-8, 5=19/Mechanical
Max Horz 2=135(load case 5)
Max Uplift4=-45(load case 5), 2=-152(load case 5), 5=-15(load case 3)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-42/14, 3-4=-37/40
BOT CHORD 2-6=0/0, 3-5=0/0

**JOINT STRESS INDEX**
2 = 0.56 and 3 = 0.01

**NOTES**
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 45 lb uplift at joint 4, 152 lb uplift at joint 2 and 15 lb uplift at joint 5.

**LOAD CASE(S)** Standard

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0103071021

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | CJ4 | SPECIAL | 1 | 1 | Job Reference (optional) |

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 03 09:47:33 2007 Page 1

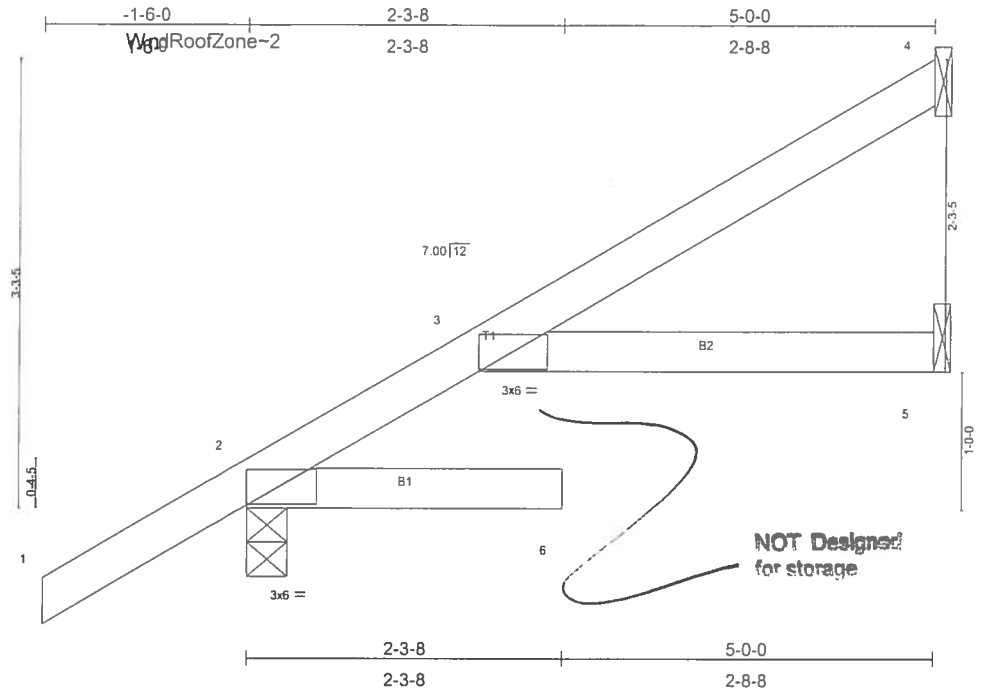



Plate Offsets (X,Y): [2 0-3-3,0-1-8]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.35 | Vert(LL) | 0.14 | 6 | >420 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.13 | Vert(TL) | -0.20 | 6 | >288 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.00 | Horz(TL) | 0.07 | 5 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 20 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 4=144/Mechanical, 2=317/0-3-8, 5=49/Mechanical
Max Horz 2=189(load case 5)
Max Uplift4=-118(load case 5), 2=-173(load case 5), 5=-38(load case 3)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-85/20, 3-4=-89/64
BOT CHORD 2-6=0/0, 3-5=0/0

**JOINT STRESS INDEX**
2 = 0.58 and 3 = 0.04

**NOTES**
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 118 lb uplift at joint 4, 173 lb uplift at joint 2 and 38 lb uplift at joint 5.

**LOAD CASE(S)** Standard

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549

Dwg.#0103071022

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | CJ5 | JACK | 12 | 1 | Job Reference (optional) |

Builders FirstSource, Lake City, FI 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 03 09:47:34 2007 Page 1

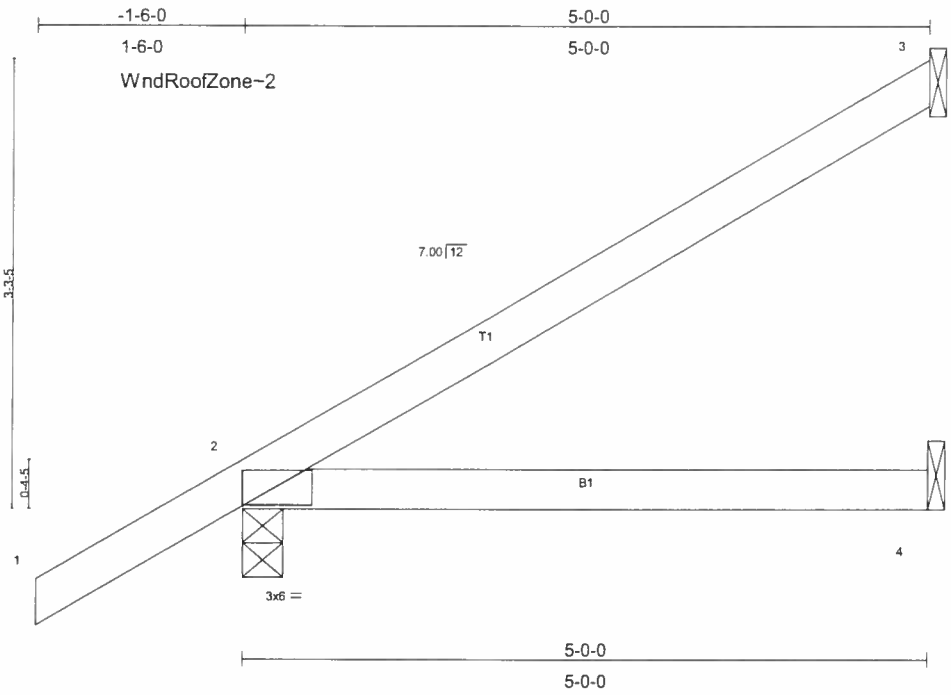



Plate Offsets (X,Y): [2:0-3-3,0-1-8]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.22 | Vert(LL) | 0.09 | 2-4 | >663 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.24 | Vert(TL) | 0.07 | 2-4 | >774 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.00 | Horz(TL) | -0.00 | 3 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 19 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 3=114/Mechanical, 2=305/0-3-8, 4=72/Mechanical
Max Horz 2=189(load case 5)
Max Uplift3=-111(load case 5), 2=-208(load case 5), 4=-56(load case 3)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-85/46
BOT CHORD 2-4=0/0

**JOINT STRESS INDEX**
2 = 0.12

**NOTES**
1) Wind: ASCE 7-02; 110mph (3-second gust), h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 111 lb uplift at joint 3, 208 lb uplift at joint 2 and 56 lb uplift at joint 4.

**LOAD CASE(S)** Standard

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE E. LUTZ, FL 33549

Dwg.#0103071023

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | CJ5T | SPECIAL | 1 | 1 | Job Reference (optional) |

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 03 09:47:35 2007 Page 1

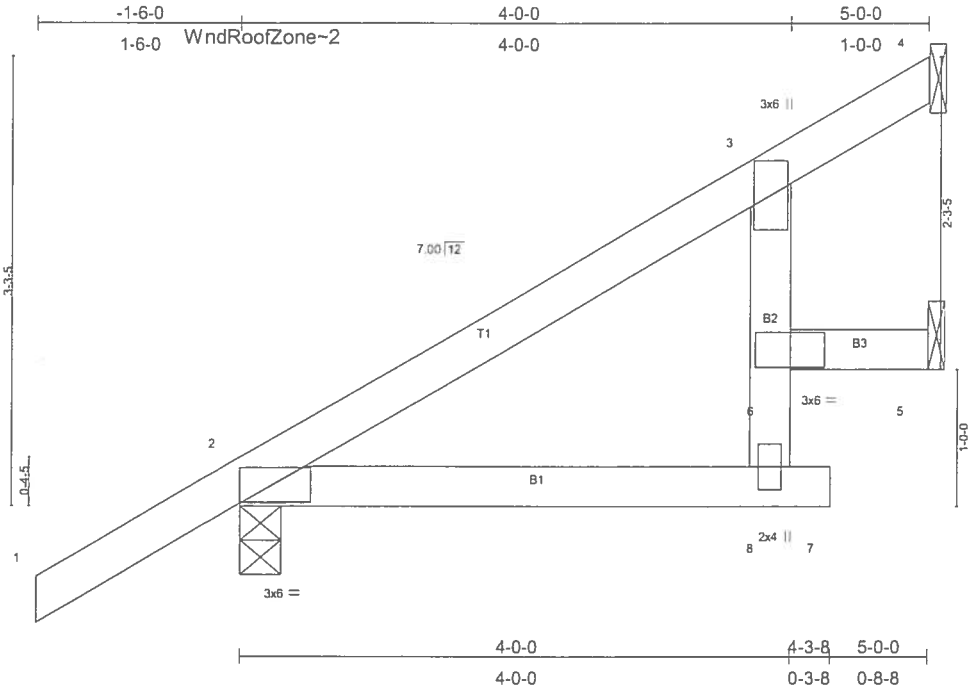



Plate Offsets (X,Y): [2:0-3-3,0-1-8]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.16 | Vert(LL) | 0.02 | 2-8 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.28 | Vert(TL) | -0.01 | 7 | >999 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.00 | Horz(TL) | -0.01 | 5 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 22 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 4=71/Mechanical, 2=308/0-3-8, 5=126/Mechanical
Max Horz 2=214(load case 5)
Max Uplift4=-57(load case 5), 2=-205(load case 5), 5=-100(load case 5)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-181/59, 3-4=-50/33
BOT CHORD 2-8=-203/106, 7-8=0/0, 6-8=-111/77, 3-6=-47/44, 5-6=0/0

**JOINT STRESS INDEX**
2 = 0.30, 3 = 0.37, 6 = 0.45 and 8 = 0.63

**NOTES**
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 57 lb uplift at joint 4, 205 lb uplift at joint 2 and 100 lb uplift at joint 5.

**LOAD CASE(S)** Standard

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877. BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549

Dwg.#0103071024

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | CJ5TA | SPECIAL | 2 | 1 | Job Reference (optional) |

Builders FirstSource, Lake City, FI 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 03 09:47:35 2007 Page 1

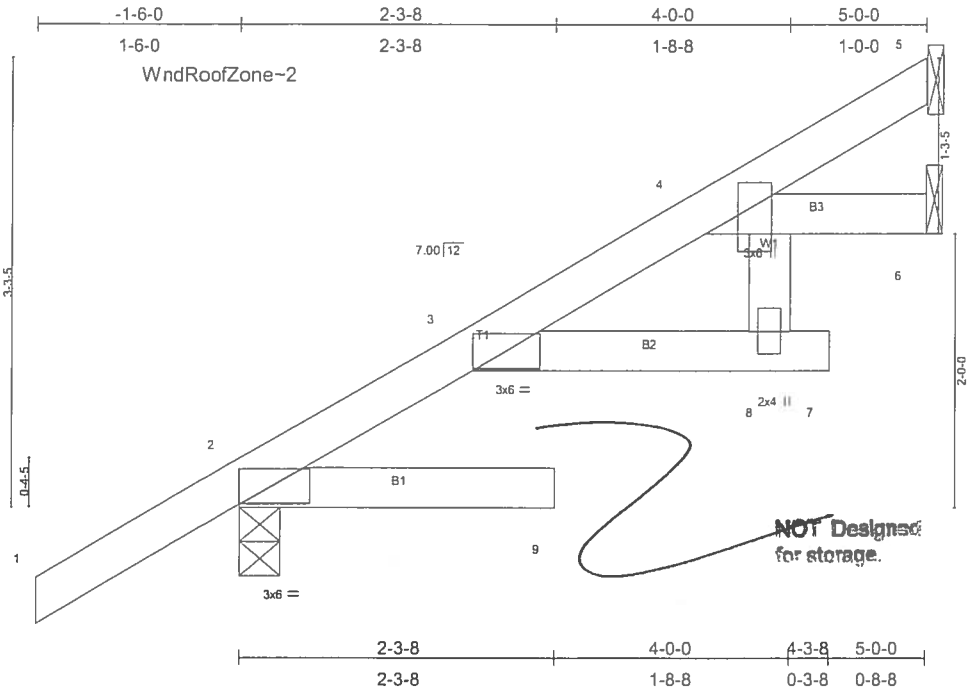



Plate Offsets (X,Y): [2:0-3-3,0-1-8], [4:0-1-8,0-2-12]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.44 | Vert(LL) | -0.14 | 9 | >417 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.13 | Vert(TL) | -0.23 | 9 | >250 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.02 | Horz(TL) | 0.09 | 6 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 22 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

**REACTIONS** (lb/size) 5=191/Mechanical, 2=323/0-3-8, 6=24/Mechanical
Max Horz 2=189(load case 5)
Max Uplift5=-96(load case 5), 2=-144(load case 5)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-88/2, 3-4=-71/33, 4-5=-61/92
BOT CHORD 2-9=0/0, 3-8=-24/0, 7-8=0/0, 4-6=0/0
WEBS 4-8=0/53

**JOINT STRESS INDEX**
2 = 0.58, 3 = 0.02, 4 = 0.08 and 8 = 0.04

**NOTES**
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 96 lb uplift at joint 5 and 144 lb uplift at joint 2.

**LOAD CASE(S)** Standard

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60967
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE STE E. LUTZ, FL 33549

Dwg.#0103071025

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | EJ2 | JACK | 2 | 1 | Job Reference (optional) |

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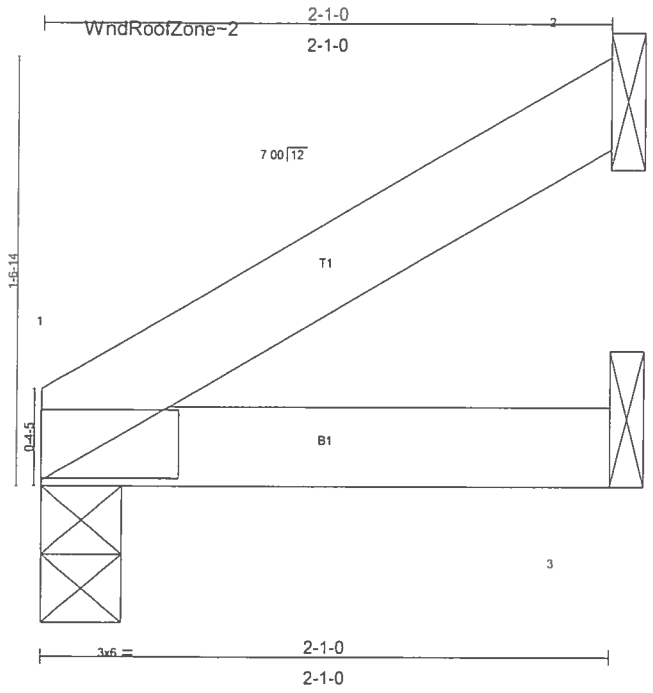


Plate Offsets (X,Y): [1:0-3-3,0-1-8]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.04 | Vert(LL) | -0.00 | 1 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.04 | Vert(TL) | -0.00 | 1-3 | >999 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.00 | Horz(TL) | -0.00 | 2 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 7 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 2-1-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 1=79/0-3-8, 2=51/Mechanical, 3=28/Mechanical
Max Horz 1=56(load case 5)
Max Uplift1=-35(load case 5), 2=-54(load case 5), 3=-22(load case 3)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-41/22
BOT CHORD 1-3=0/0

**JOINT STRESS INDEX**
1 = 0.03

**NOTES**
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 35 lb uplift at joint 1, 54 lb uplift at joint 2 and 22 lb uplift at joint 3.

**LOAD CASE(S)** Standard

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ FL 33549

Dwg.#0103071026

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | EJ2A | SPECIAL | 1 | 1 | Job Reference (optional) |

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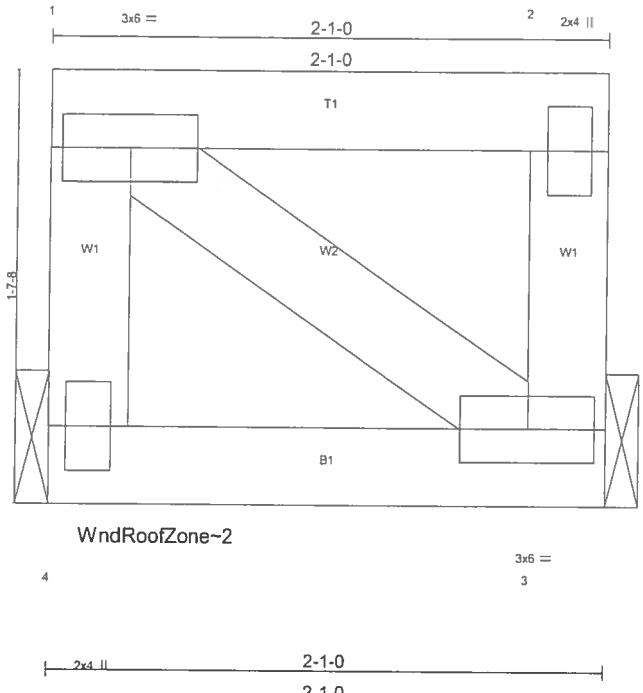


| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.04 | Vert(LL) | -0.00 | 4 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.03 | Vert(TL) | -0.00 | 4 | >999 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.00 | Horz(TL) | -0.00 | 3 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 12 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 2-1-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 4=75/Mechanical, 3=75/Mechanical
Max Uplift4=-51(load case 3), 3=-51(load case 3)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-4=-48/58, 1-2=0/0, 2-3=-48/58
BOT CHORD 3-4=-0/0
WEBS 1-3=-0/0

**JOINT STRESS INDEX**
1 = 0.03, 2 = 0.03, 3 = 0.03 and 4 = 0.03

**NOTES**
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 51 lb uplift at joint 4 and 51 lb uplift at joint 3.

**LOAD CASE(S)** Standard

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0103071027

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | EJ2B | SPECIAL | 1 | 1 | Job Reference (optional) |

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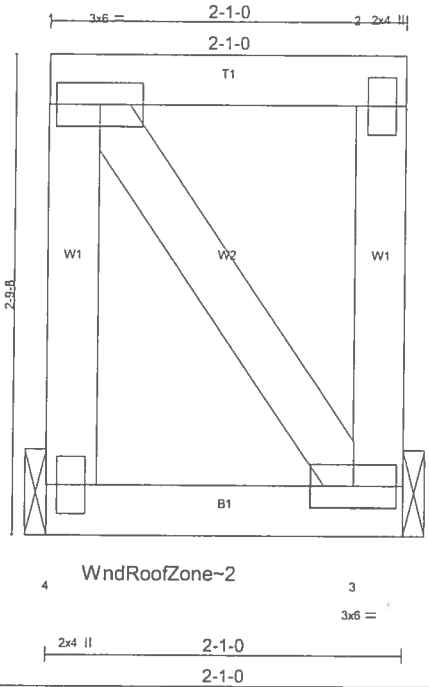



| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.04 | Vert(LL) | -0.00 | 4 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.03 | Vert(TL) | -0.00 | 4 | >999 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.00 | Horz(TL) | 0.00 | 3 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 17 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 2-1-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

**REACTIONS** (lb/size) 4=75/Mechanical, 3=75/Mechanical
Max Uplift4=-51(load case 3), 3=-51(load case 3)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-4=-48/58, 1-2=0/0, 2-3=-48/58
BOT CHORD 3-4=-0/0
WEBS 1-3=-0/0

**JOINT STRESS INDEX**
1 = 0.03, 2 = 0.03, 3 = 0.03 and 4 = 0.03

**NOTES**
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 51 lb uplift at joint 4 and 51 lb uplift at joint 3.

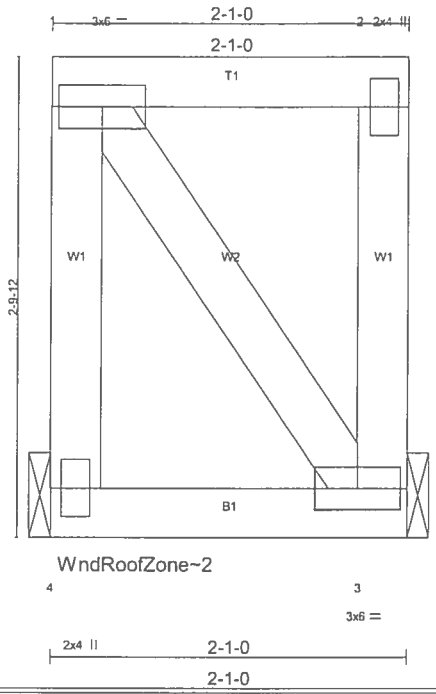
**LOAD CASE(S)** Standard

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0103071028

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | EJ2C | SPECIAL | 1 | 1 | Job Reference (optional) |

Builders FirstSource, Lake City, FI 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 03 09:47:40 2007 Page 1




Scale = 1:13.0

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.04 | Vert(LL) | -0.00 | 4 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.03 | Vert(TL) | -0.00 | 4 | >999 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.00 | Horz(TL) | 0.00 | 3 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 17 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 2-1-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 4=75/Mechanical, 3=75/Mechanical
Max Uplift4=-51(load case 3), 3=-51(load case 3)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-4=-48/58, 1-2=0/0, 2-3=-48/58
BOT CHORD 3-4=-0/0
WEBS 1-3=-0/0

**JOINT STRESS INDEX**
1 = 0.03, 2 = 0.03, 3 = 0.03 and 4 = 0.03

**NOTES**
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 51 lb uplift at joint 4 and 51 lb uplift at joint 3.

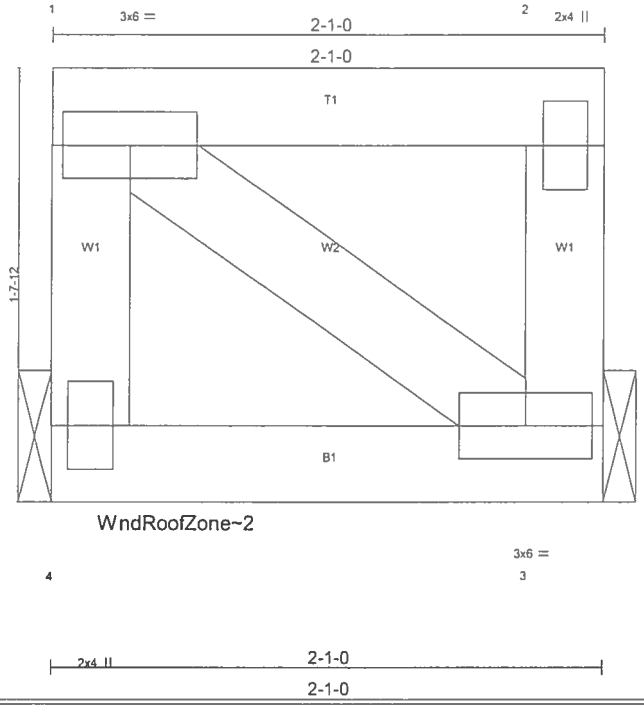
**LOAD CASE(S)** Standard

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0103071029

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | EJ2D | SPECIAL | 1 | 1 | Job Reference (optional) |

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 03 09:47:40 2007 Page 1




| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.04 | Vert(LL) | -0.00 | 4 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.03 | Vert(TL) | -0.00 | 4 | >999 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.00 | Horz(TL) | 0.00 | 3 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 12 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 2-1-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 4=75/Mechanical, 3=75/Mechanical
Max Uplift4=-51(load case 3), 3=-51(load case 3)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-4=-48/58, 1-2=0/0, 2-3=-48/58
BOT CHORD 3-4=-0/0
WEBS 1-3=-0/0

**JOINT STRESS INDEX**
1 = 0.03, 2 = 0.03, 3 = 0.03 and 4 = 0.03

**NOTES**
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 51 lb uplift at joint 4 and 51 lb uplift at joint 3.

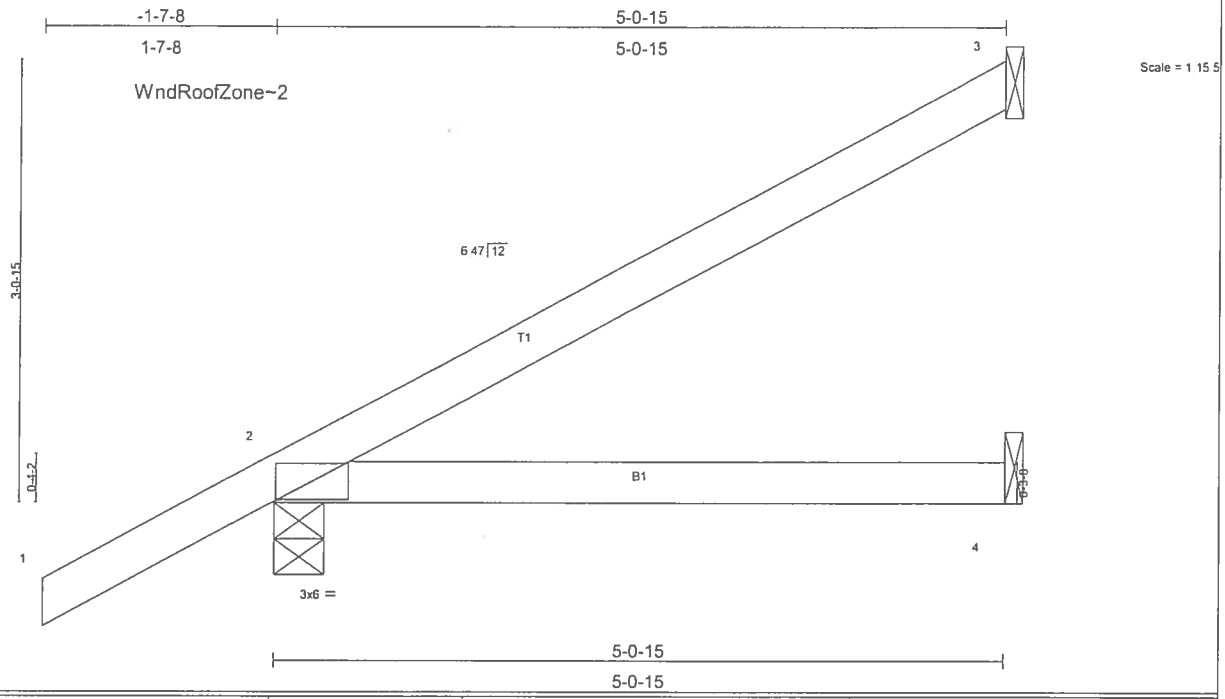
**LOAD CASE(S)** Standard

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877. BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS. INC. EB 9196
16105 N. FLORIDA AVE. STE E. LUTZ, FL 33549

Dwg.#0103071030

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | EJ5A | MONO TRUSS | 2 | 1 | Job Reference (optional) |

Builders FirstSource, Lake City, FL 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 03 09:47:42 2007 Page 1




| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.22 | Vert(LL) | 0.09 | 2-4 | >638 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.28 | Vert(TL) | 0.08 | 2-4 | >702 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | NO | WB | 0.00 | Horz(TL) | -0.00 | 3 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 19 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 5-0-15 oc purlins
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

**REACTIONS** (lb/size) 3=95/Mechanical, 2=205/0-4-2, 4=62/Mechanical
Max Horz 2=118(load case 5)
Max Uplift3=-77(load case 5), 2=-164(load case 5), 4=-59(load case 3)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/41, 2-3=-57/31
BOT CHORD 2-4=0/0

**JOINT STRESS INDEX**
2 = 0.08

**NOTES**
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 77 lb uplift at joint 3, 164 lb uplift at joint 2 and 59 lb uplift at joint 4.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

**LOAD CASE(S)** Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-2(F=26, B=26)-to-3=-69(F=-7, B=-7), 2=0(F=15, B=15)-to-4=-38(F=-4, B=-4)

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877. BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE E. LUTZ, FL 33549

Dwg.#0103071031

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | EJ5B | JACK | 2 | 1 | Job Reference (optional) |

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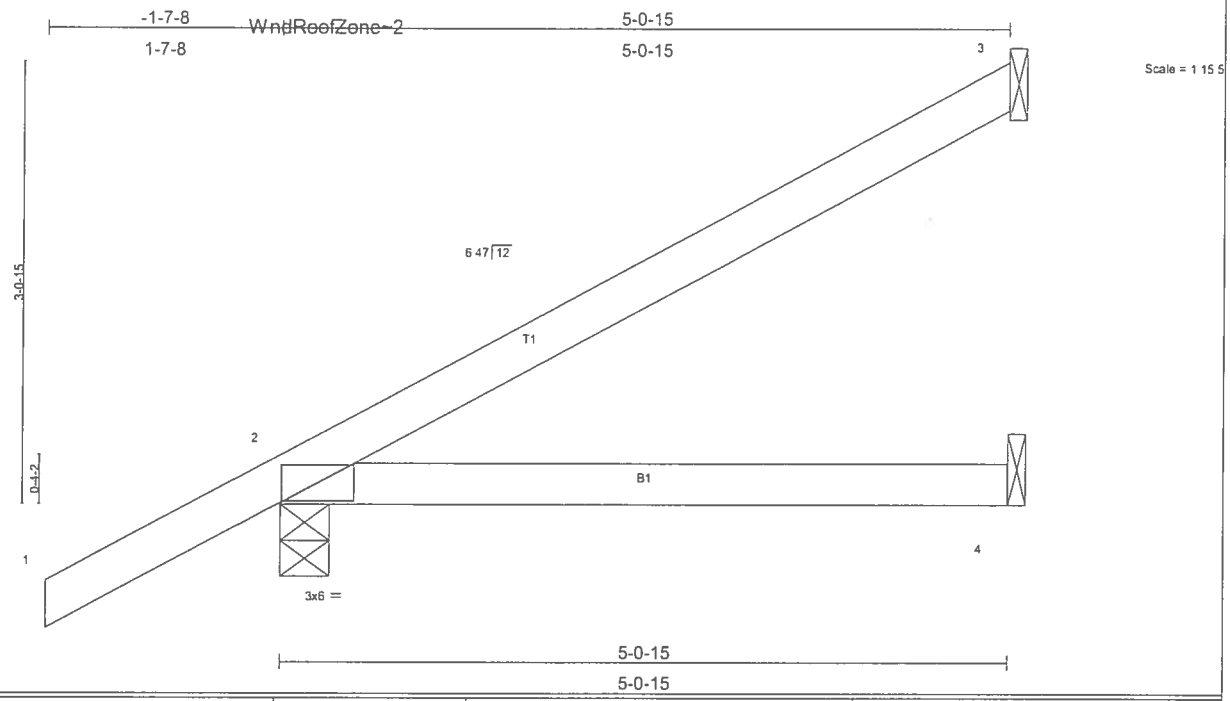


| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.22 | Vert(LL) | 0.09 | 2-4 | >642 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.28 | Vert(TL) | 0.08 | 2-4 | >707 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | NO | WB | 0.00 | Horz(TL) | -0.00 | 3 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 19 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 5-0-15 oc purlins
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 3=95/Mechanical, 2=205/0-4-2, 4=62/Mechanical
Max Horz 2=118(load case 5)
Max Uplift3=-77(load case 5), 2=-164(load case 5), 4=-59(load case 3)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/41, 2-3=-56/31
BOT CHORD 2-4=0/0

**JOINT STRESS INDEX**
2 = 0.08

**NOTES**
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 77 lb uplift at joint 3, 164 lb uplift at joint 2 and 59 lb uplift at joint 4.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B)

**LOAD CASE(S)** Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-2(F=26, B=26)-to-3=-69(F=-7, B=-7), 2=0(F=15, B=15)-to-4=-38(F=-4, B=-4)

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549

Dwg.#0103071032

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | EJ6 | JACK | 4 | 1 | Job Reference (optional) |

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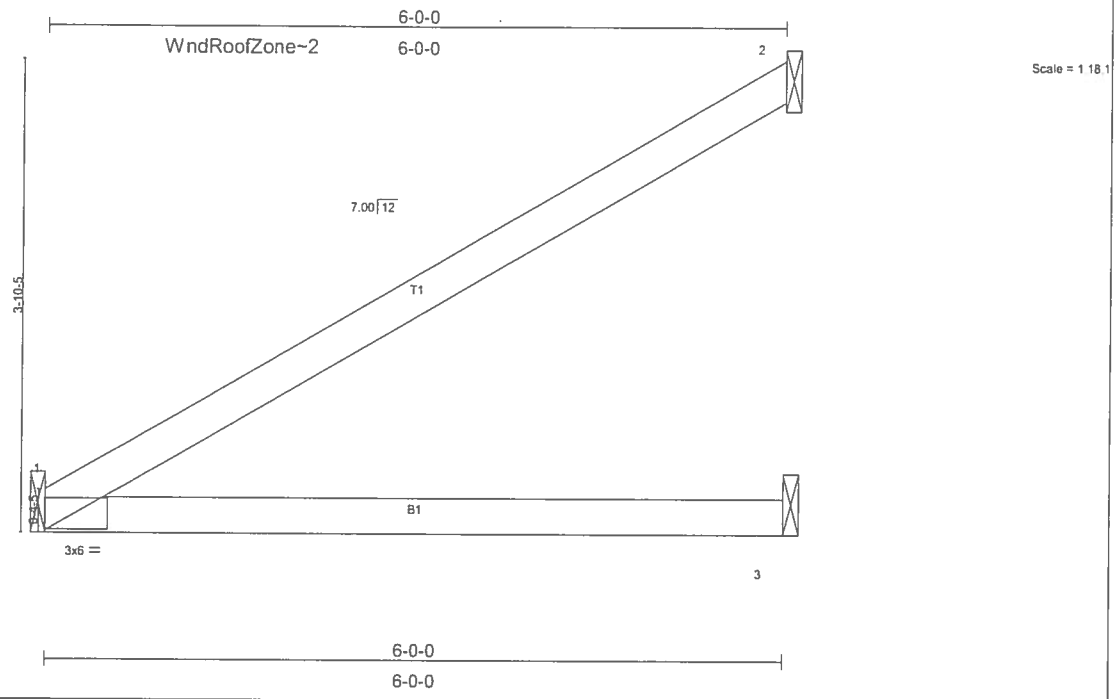


Plate Offsets (X,Y): [1:0-3-3,0-1-8]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.42 | Vert(LL) | 0.20 | 1-3 | >361 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.36 | Vert(TL) | 0.17 | 1-3 | >422 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.00 | Horz(TL) | -0.00 | 2 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 20 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No 2
BOT CHORD 2 X 4 SYP No.2

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 1=247/Mechanical, 2=159/Mechanical, 3=88/Mechanical
Max Horz 1=162(load case 5)
Max Uplift1=-119(load case 5), 2=-159(load case 5), 3=-69(load case 3)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-113/65
BOT CHORD 1-3=0/0

**JOINT STRESS INDEX**
1 = 0.08

**NOTES**
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 119 lb uplift at joint 1, 159 lb uplift at joint 2 and 69 lb uplift at joint 3.

**LOAD CASE(S)** Standard

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549

Dwg.#0103071033

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | EJ7 | MONO TRUSS | 19 | 1 | Job Reference (optional) |

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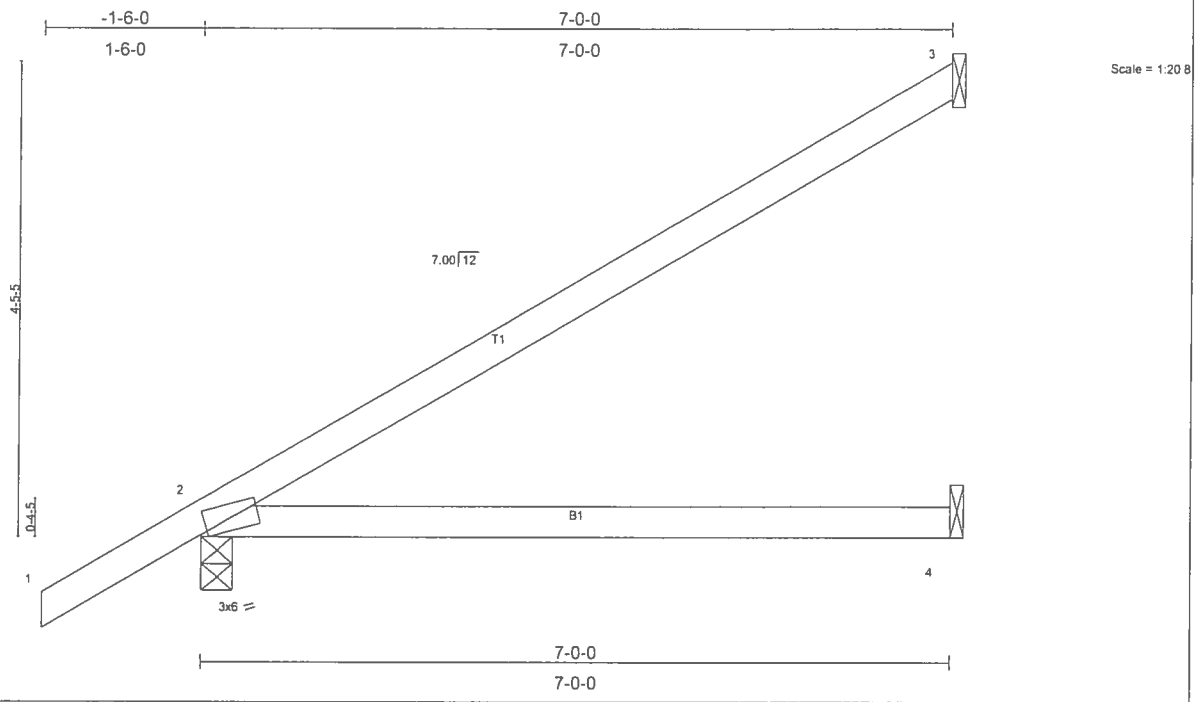


Plate Offsets (X,Y): [2:0-0-11,Edge]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.46 | Vert(LL) | -0.14 | 2-4 | >599 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.38 | Vert(TL) | -0.23 | 2-4 | >361 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.00 | Horz(TL) | -0.00 | 3 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 25 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 3=165/Mechanical, 2=385/0-3-8, 4=109/Mechanical
Max Horz 2=242(load case 5)
Max Uplift3=-151(load case 5), 2=-158(load case 5)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-107/66
BOT CHORD 2-4=0/0

**JOINT STRESS INDEX**
2 = 0.75

**NOTES**
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 151 lb uplift at joint 3 and 158 lb uplift at joint 2.

**LOAD CASE(S)** Standard

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549

Dwg.#0103071034

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | EJ7G | MONO TRUSS | 1 | 2 | Job Reference (optional) |

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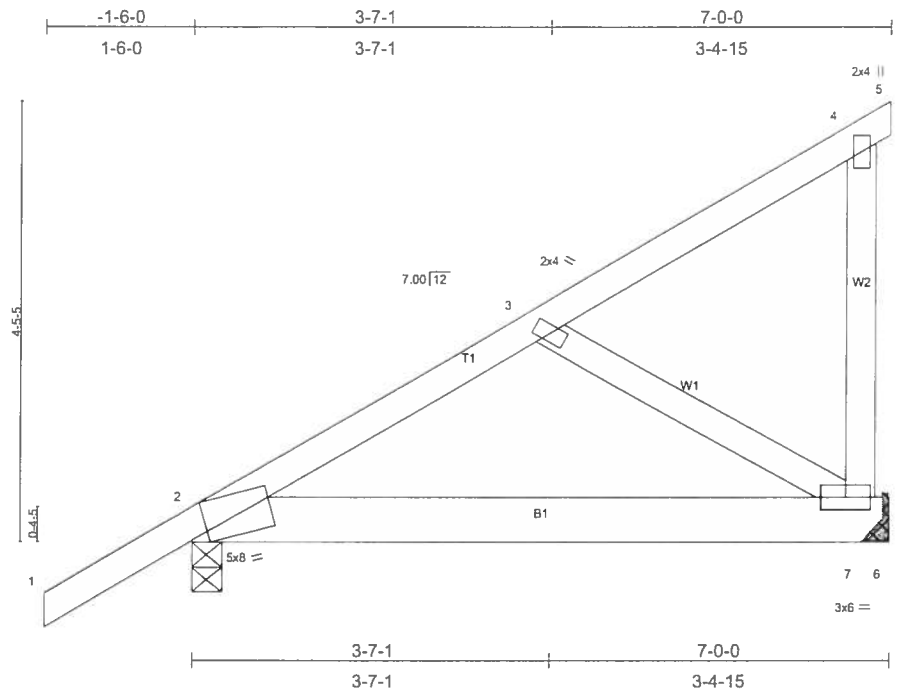


Plate Offsets (X,Y): [2:0-2-1,Edge]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.74 | Vert(LL) | -0.16 | 2-7 | >489 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.85 | Vert(TL) | -0.26 | 2-7 | >304 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | NO | WB | 0.03 | Horz(TL) | 0.00 | 7 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 83 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 2=1754/0-3-8, 7=1660/Mechanical
Max Horz 2=70(load case 4)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/43, 2-3=-199/0, 3-4=-30/0, 4-5=-8/0
BOT CHORD 2-7=0/306, 6-7=0/0
WEBS 4-7=0/44, 3-7=-357/0

**JOINT STRESS INDEX**
2 = 0.66, 3 = 0.08, 4 = 0.01 and 7 = 0.05

**NOTES**
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc
Bottom chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Girder carries tie-in span(s): 24-10-8 from 0-0-0 to 7-0-0

**LOAD CASE(S)** Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 4-5=-54, 2-7=-504(F=-474)

**JANUARY 03, 2007 TRUSS DESIGN ENGINEER:**
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**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549**

Dwg.#0103071035

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | EJ7GA | MONO TRUSS | 1 | 1 | Job Reference (optional) |

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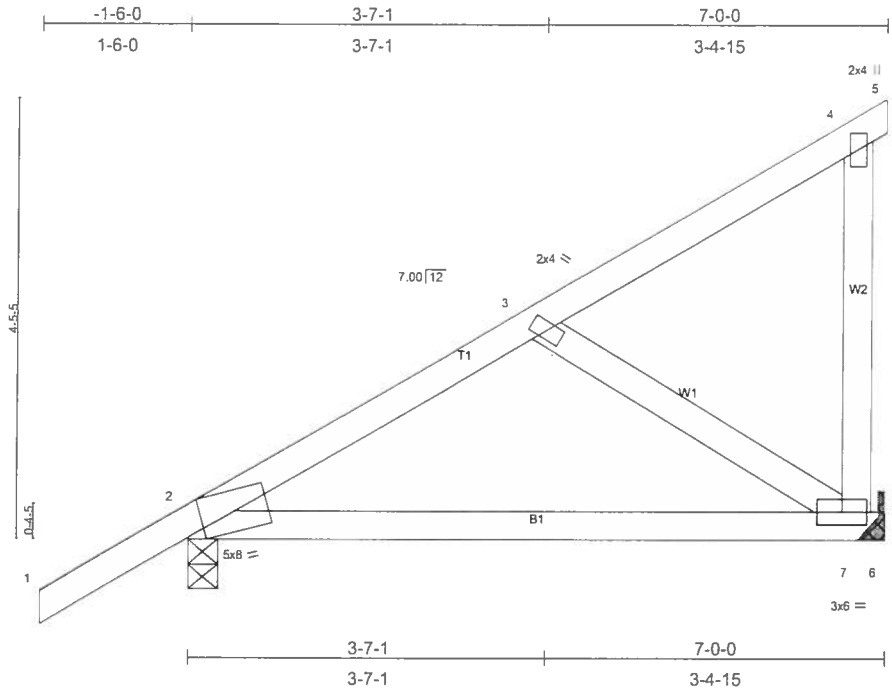


Plate Offsets (X,Y): [2:0-2-1,Edge]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.46 | Vert(LL) | -0.19 | 2-7 | >411 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.64 | Vert(TL) | -0.31 | 2-7 | >250 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | NO | WB | 0.04 | Horz(TL) | 0.00 | 7 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 36 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.1D
BOT CHORD 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 2=454/0-3-8, 7=359/Mechanical
Max Horz 2=65(load case 4)
Max Uplift2=-8(load case 4)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-75/0, 3-4=-20/0, 4-5=-8/0
BOT CHORD 2-7=0/120, 6-7=0/0
WEBS 4-7=0/36, 3-7=-141/0

**JOINT STRESS INDEX**
2 = 0.75, 3 = 0.06, 4 = 0.02 and 7 = 0.04

**NOTES**
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 8 lb uplift at joint 2.
4) Girder carries tie-in span(s): 6-0-0 from 0-0-0 to 7-0-0
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

**LOAD CASE(S)** Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 4-5=-54, 2-7=-108(F=-78)

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549

Dwg.#0103071036

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | EJ7T | SPECIAL | 12 | 1 | Job Reference (optional) |

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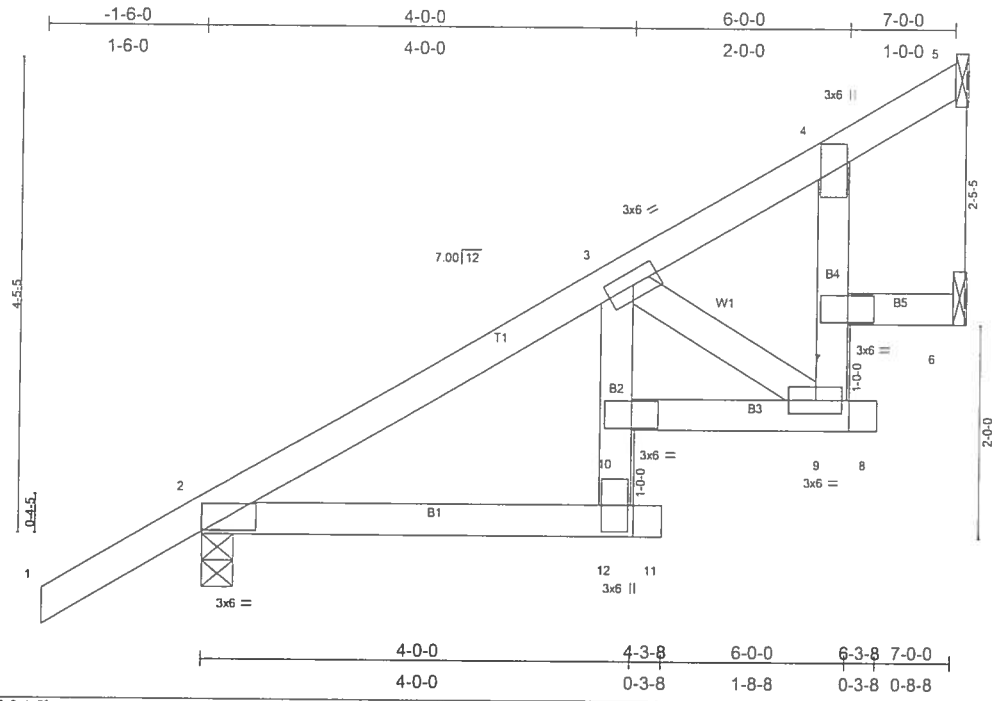


Plate Offsets (X,Y): [2:0-3-3,0-1-8]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.21 | Vert(LL) | -0.03 | 11 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.59 | Vert(TL) | -0.04 | 11 | >999 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.05 | Horz(TL) | 0.02 | 6 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 36 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.3, B4 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 5=133/Mechanical, 2=392/0-3-8, 6=160/Mechanical
Max Horz 2=242(load case 5)
Max Uplift5=-76(load case 5), 2=-156(load case 5), 6=-70(load case 5)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-357/2, 3-4=-180/17, 4-5=-43/64
BOT CHORD 2-12=-121/252, 11-12=0/0, 10-12=0/92, 3-10=0/58, 9-10=-165/337, 8-9=0/0, 7-9=-95/249, 4-7=-19/121, 6-7=0/0
WEBS 3-9=-260/125

**JOINT STRESS INDEX**
2 = 0.20, 3 = 0.38, 4 = 0.40, 7 = 0.58, 9 = 0.56, 10 = 0.33 and 12 = 0.35

**NOTES**
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 76 lb uplift at joint 5, 156 lb uplift at joint 2 and 70 lb uplift at joint 6.

**LOAD CASE(S)** Standard

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS. INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ. FL 33549

Dwg.#0103071037

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | EJ7TA | SPECIAL | 5 | 1 | Job Reference (optional) |

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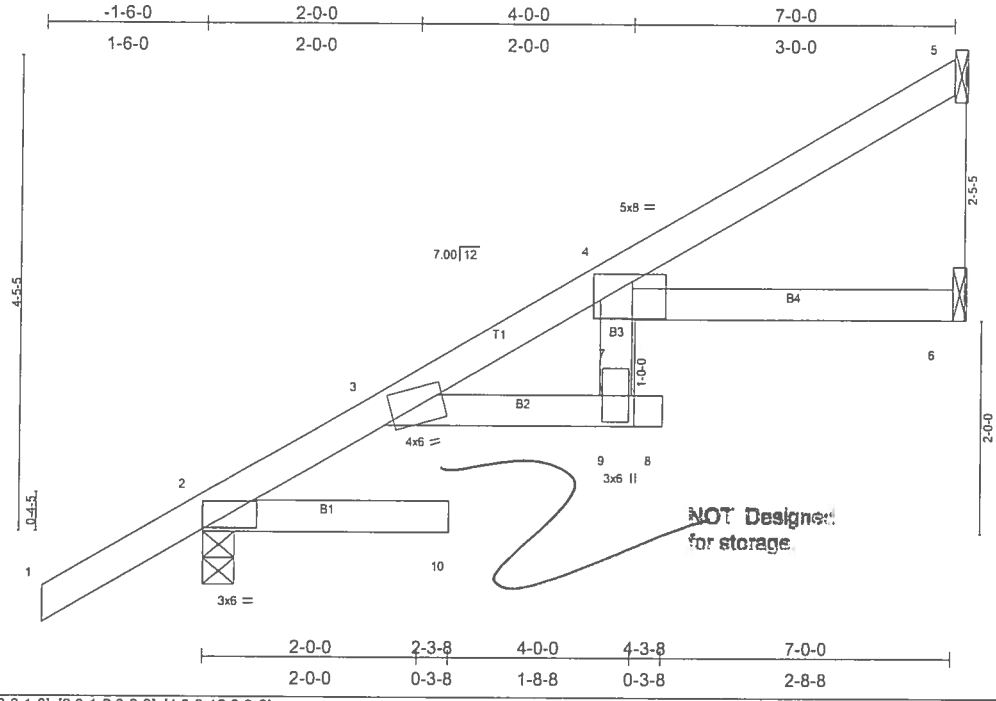


Plate Offsets (X,Y): [2:0-3-3,0-1-8], [3:0-1-2,0-3-3], [4:0-3-12,0-3-6]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.58 | Vert(LL) | -0.17 | 10 | >469 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.63 | Vert(TL) | -0.29 | 10 | >284 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.00 | Horz(TL) | 0.13 | 6 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 28 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 5=147/Mechanical, 2=404/0-3-8, 6=141/Mechanical
Max Horz 2=242(load case 5)
Max Uplift5=-104(load case 5), 2=-154(load case 5), 6=-44(load case 5)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-132/0, 3-4=-541/124, 4-5=-64/65
BOT CHORD 2-10=0/0, 3-9=-219/456, 8-9=0/0, 7-9=-66/213, 4-7=-3/165, 6-7=0/0

**JOINT STRESS INDEX**
2 = 0.61, 3 = 0.87, 4 = 0.96, 7 = 0.00 and 9 = 0.60

**NOTES**
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 104 lb uplift at joint 5, 154 lb uplift at joint 2 and 44 lb uplift at joint 6.

**LOAD CASE(S)** Standard

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549

Dwg.#0103071038

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | EJ7TB | SPECIAL | 4 | 1 | Job Reference (optional) |

Builders FirstSource, Lake City, FI 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 03 09:47:49 2007 Page 1

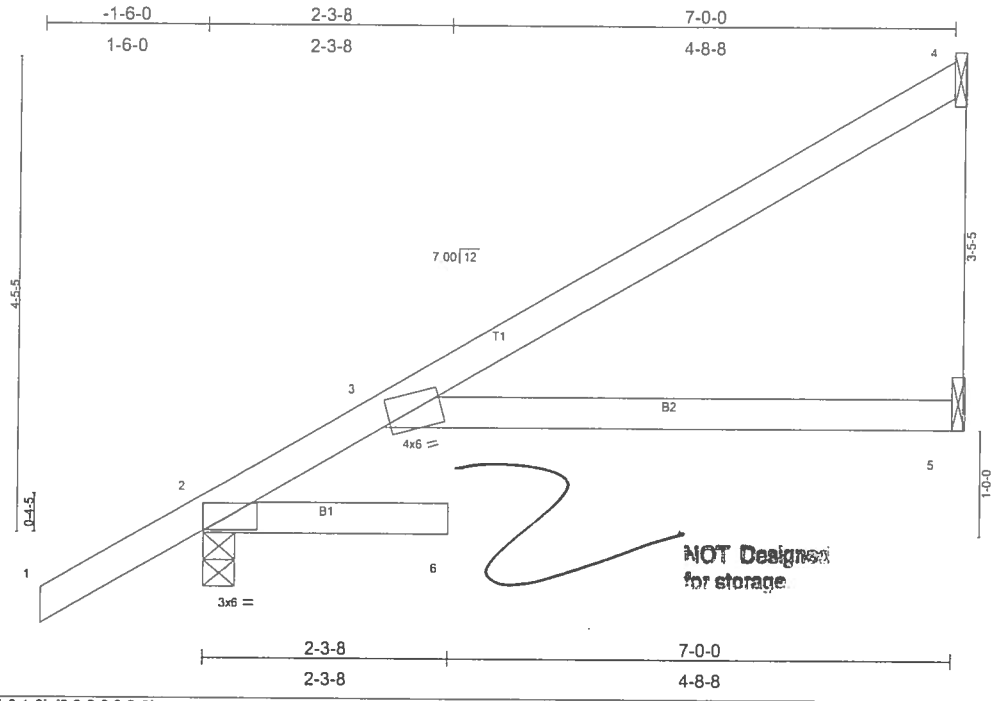


Plate Offsets (X,Y): [2:0-3-4,0-1-8], [3:0-2-8,0-2-0]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.56 | Vert(LL) | -0.22 | 6 | >370 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.46 | Vert(TL) | -0.36 | 6 | >226 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.00 | Horz(TL) | 0.14 | 5 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 26 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 4=159/Mechanical, 2=398/0-3-8, 5=120/Mechanical
Max Horz 2=242(load case 5)
Max Uplift4=-131(load case 5), 2=-155(load case 5), 5=-18(load case 5)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-129/0, 3-4=-89/66
BOT CHORD 2-6=0/0, 3-5=0/0

**JOINT STRESS INDEX**
2 = 0.61 and 3 = 0.89

**NOTES**
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 131 lb uplift at joint 4, 155 lb uplift at joint 2 and 18 lb uplift at joint 5.

**LOAD CASE(S)** Standard

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549

Dwg.#0103071039

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | HJ2 | JACK | 1 | 1 | Job Reference (optional) |

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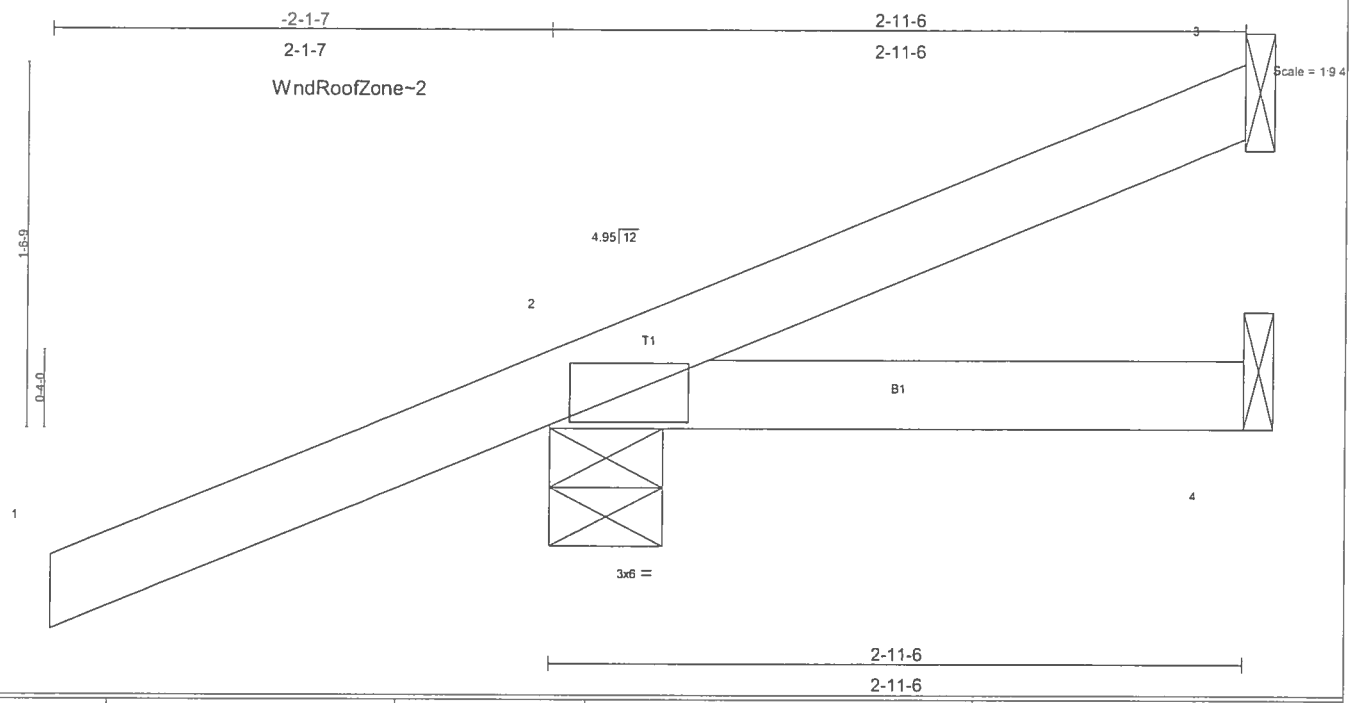


| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.32 | Vert(LL) | -0.00 | 2 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.03 | Vert(TL) | -0.00 | 2-4 | >999 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | NO | WB | 0.00 | Horz(TL) | -0.00 | 3 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 13 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 2-11-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 3=-20/Mechanical, 2=214/0-5-11, 4=19/Mechanical
Max Horz 2=71(load case 4)
Max Uplift3=-20(load case 1), 2=-233(load case 4), 4=-29(load case 2)
Max Grav 3=50(load case 6), 2=214(load case 1), 4=19(load case 1)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/43, 2-3=-32/20
BOT CHORD 2-4=0/0

**JOINT STRESS INDEX**
2 = 0.09

**NOTES**
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) All bearings are assumed to be SYP No 2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 20 lb uplift at joint 3, 233 lb uplift at joint 2 and 29 lb uplift at joint 4.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

**LOAD CASE(S)** Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=25, B=25)-to-3=-40(F=7, B=7), 2=-0(F=15, B=15)-to-4=-22(F=4, B=4)

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549

Dwg.#0103071040

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | HJ8 | MONO TRUSS | 1 | 1 | Job Reference (optional) |

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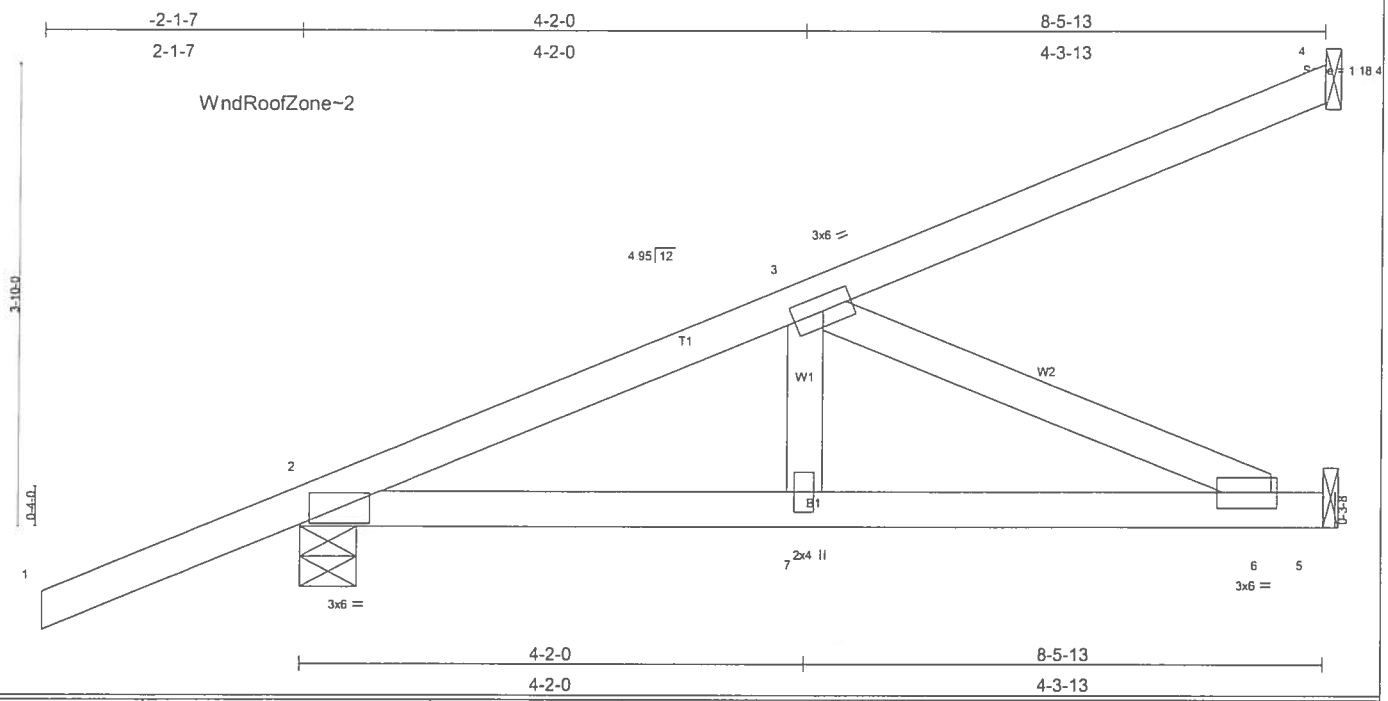


| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.32 | Vert(LL) | -0.04 | 6-7 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.37 | Vert(TL) | -0.07 | 6-7 | >999 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | NO | WB | 0.18 | Horz(TL) | 0.01 | 5 | n/a | n/a | | |
| BCDL | 5.0 | Code | FBC2004/TPI2002 | (Matrix) | | | | | | | Weight: 38 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 4=181/Mechanical, 2=398/0-5-11, 5=291/Mechanical
Max Horz 2=230(load case 4)
Max Uplift4=-166(load case 4), 2=-295(load case 4), 5=-166(load case 4)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/43, 2-3=-524/194, 3-4=-85/51
BOT CHORD 2-7=-338/465, 6-7=-338/465, 5-6=0/0
WEBS 3-7=-88/152, 3-6=-510/371

**JOINT STRESS INDEX**
2 = 0.57, 3 = 0.16, 6 = 0.14 and 7 = 0.11

**NOTES**
1) Wind: ASCE 7-02; 110mph (3-second gust), h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 166 lb uplift at joint 4, 295 lb uplift at joint 2 and 166 lb uplift at joint 5.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

**LOAD CASE(S)** Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=25, B=25)-to-4=-115(F=-30, B=-30), 2=-0(F=15, B=15)-to-5=-64(F=-17, B=-17)

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549

Dwg.#0103071041

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | HJ9 | MONO TRUSS | 5 | 1 | Job Reference (optional) |

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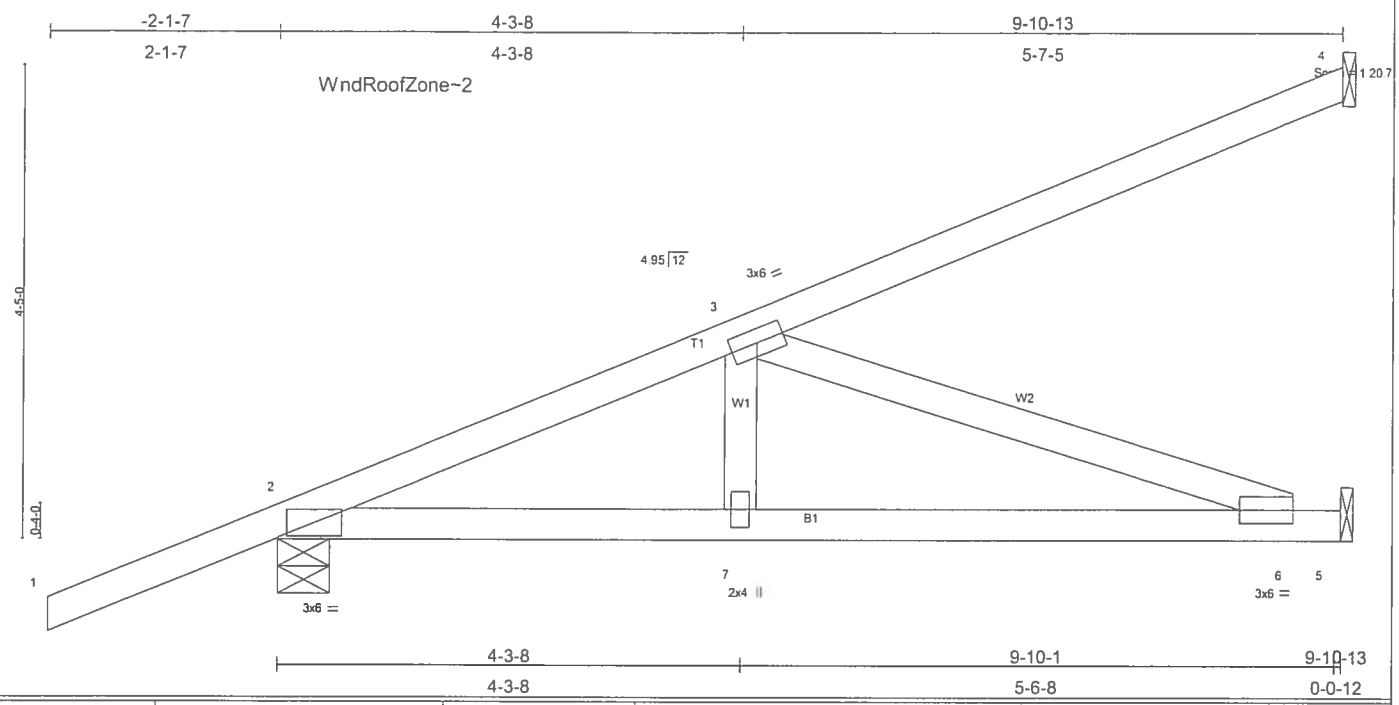


| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.60 | Vert(LL) | -0.10 | 6-7 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.59 | Vert(TL) | -0.17 | 6-7 | >667 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | NO | WB | 0.44 | Horz(TL) | 0.01 | 5 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 44 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-2-5 oc bracing.

**REACTIONS** (lb/size) 4=266/Mechanical, 2=486/0-5-11, 5=389/Mechanical
Max Horz 2=297(load case 4)
Max Uplift4=-246(load case 4), 2=-329(load case 4), 5=-206(load case 4)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/43, 2-3=-821/328, 3-4=-125/74
BOT CHORD 2-7=-538/746, 6-7=-538/746, 5-6=0/0
WEBS 3-7=-96/211, 3-6=-790/570

**JOINT STRESS INDEX**
2 = 0.68, 3 = 0.24, 6 = 0.22 and 7 = 0.15

**NOTES**
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 246 lb uplift at joint 4, 329 lb uplift at joint 2 and 206 lb uplift at joint 5.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

**LOAD CASE(S)** Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=-0(F=15, B=15)-to-5=-74(F=-22, B=-22)

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS. INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ. FL 33549

Dwg.#0103071042

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | HJ9T | SPECIAL | 1 | 1 | Job Reference (optional) |

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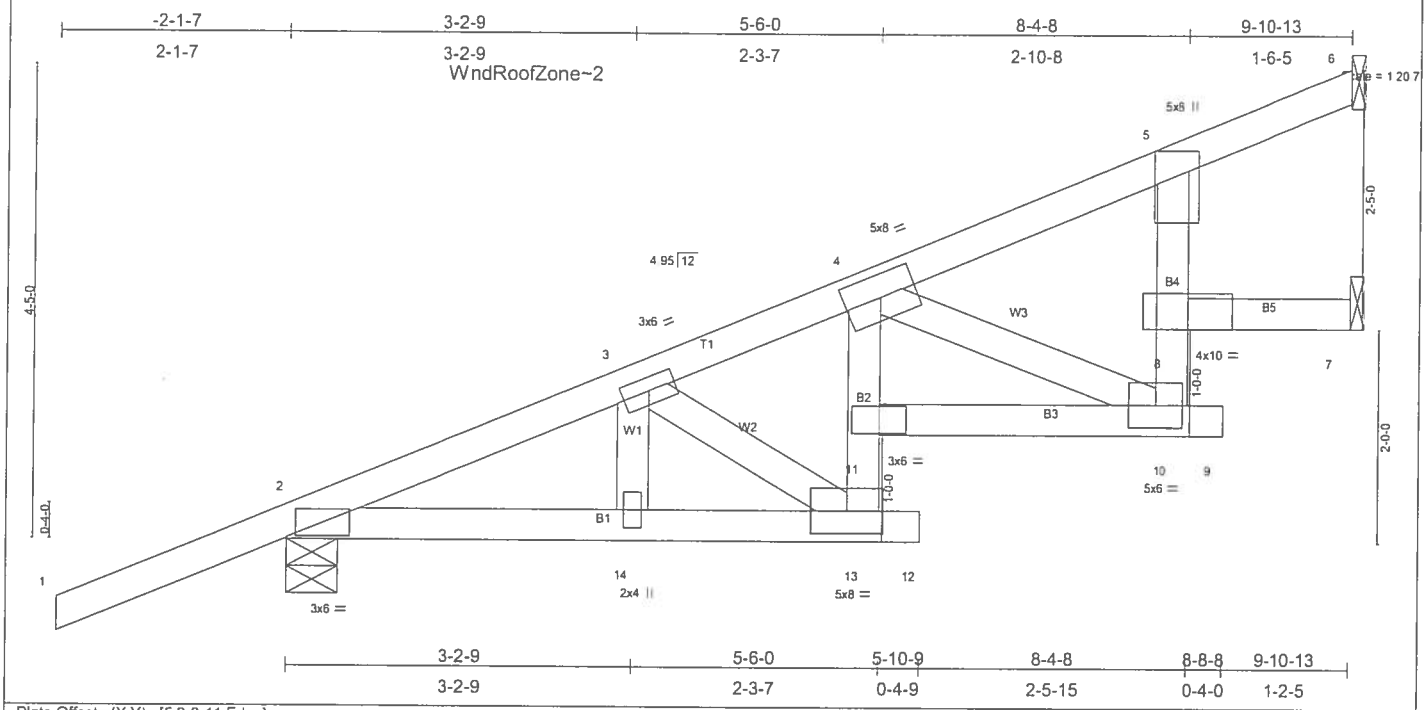


| | | | | | |
|---|---|---|---|---|---|
| 3-2-9 | 5-6-0 | 5-10-9 | 8-4-8 | 8-8-8 | 9-10-13 |
| 3-2-9 | 2-3-7 | 0-4-9 | 2-5-15 | 0-4-0 | 1-2-5 |

Plate Offsets (X,Y): [5:0-3-11,Edge]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.71 | Vert(LL) | 0.12 | 12 | >942 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.90 | Vert(TL) | -0.16 | 12 | >708 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | NO | WB | 0.13 | Horz(TL) | 0.05 | 7 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 50 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B5 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-3-10 oc bracing.

**REACTIONS** (lb/size) 6=312/Mechanical, 2=500/0-5-11, 7=382/Mechanical
Max Horz 2=397(load case 4)
Max Uplift6=-225(load case 4), 2=-315(load case 4), 7=-230(load case 4)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/43, 2-3=-662/162, 3-4=-694/217, 4-5=-490/155, 5-6=-97/111
BOT CHORD 2-14=-473/588, 13-14=-473/588, 12-13=0/0, 11-13=-99/171, 4-11=-58/144, 10-11=-691/953, 9-10=0/0, 8-10=-250/443, 5-8=-38/170, 7-8=0/0
WEBS 3-14=-125/96, 3-13=-39/21, 4-10=-605/415

**JOINT STRESS INDEX**
2 = 0.67, 3 = 0.06, 4 = 0.75, 5 = 0.61, 8 = 0.96, 10 = 0.68, 11 = 0.62, 13 = 0.86 and 14 = 0.05

**NOTES**
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 225 lb uplift at joint 6, 315 lb uplift at joint 2 and 230 lb uplift at joint 7.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

**LOAD CASE(S)** Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=25, B=25)-to-6=-134(F=-40, B=-40), 2=-0(F=15, B=15)-to-13=-40(F=-5, B=-5), 13=-40(F=-5, B=-5)-to-12=-44(F=-7, B=-7), 11=-40(F=-5, B=-5)-to-10=-62(F=-16, B=-16), 10=-62(F=-16, B=-16)-to-9=-66(F=-18, B=-18), 8=-62(F=-16, B=-16)-to-7=-74(F=-22, B=-22)

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE E. LUTZ, FL 33549

Dwg.#0103071043

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | HJ9TA | SPECIAL | 1 | 1 | Job Reference (optional) |

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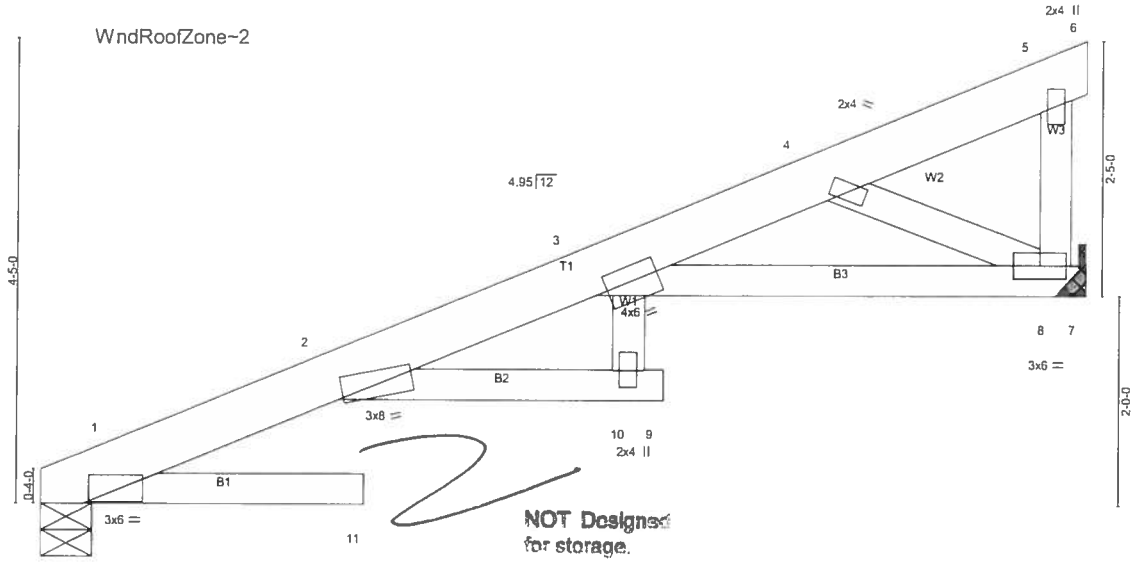
-2-1-7 | 3-0-10 | 5-6-9 | 9-10-13
2-1-7 | 3-0-10 | 2-5-15 | 4-4-4

Scale = 1:21.1




3-0-10 | 5-6-9 | 5-10-9 | 9-10-13
3-0-10 | 2-5-15 | 0-4-0 | 4-0-4

Plate Offsets (X,Y): [1:0-2-8,0-1-8], [2:0-4-0,0-1-7], [3:0-1-5,0-1-12]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.83 | Vert(LL) | -0.23 | 9 | >467 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.44 | Vert(TL) | -0.37 | 9 | >290 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | NO | WB | 0.20 | Horz(TL) | 0.17 | 8 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 47 lb | |

**LUMBER**
TOP CHORD 2 X 6 SYP No.1D
BOT CHORD 2 X 4 SYP No.2 *Except*
B3 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 5-8-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

**REACTIONS** (lb/size) 1=353/0-5-11, 8=687/Mechanical
Max Horz 1=247(load case 4)
Max Uplift1=-49(load case 4), 8=-354(load case 4)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-202/0, 2-3=-188/0, 3-4=-925/359, 4-5=-74/14, 5-6=-10/0
BOT CHORD 1-11=0/0, 2-10=-51/11, 9-10=0/0, 3-8=-560/1029, 7-8=0/0
WEBS 5-8=-92/129, 4-8=-1119/610, 3-10=-33/148

**JOINT STRESS INDEX**
1 = 0.32, 2 = 0.82, 3 = 0.47, 4 = 0.65, 5 = 0.07, 8 = 0.32 and 10 = 0.10

**NOTES**
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4 2psf; BCDL=3 0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 49 lb uplift at joint 1 and 354 lb uplift at joint 8.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

**LOAD CASE(S)** Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Trapezoidal Loads (plf)
Vert: 1=-9(F=23, B=23)-to-5=-130(F=-38, B=-38), 5=-90(F=-38, B=-38)-to-6=-94(F=-40, B=-40), 1=-3(F=13, B=13)-to-11=-22(F=4, B=4), 2=-20(F=5, B=5)-to-9=-43(F=-7, B=-7), 3=-38(F=-4, B=-4)-to-7=-74(F=-22, B=-22)

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549

Dwg.#0103071044

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | T01 | HIP | 1 | 1 | Job Reference (optional) |

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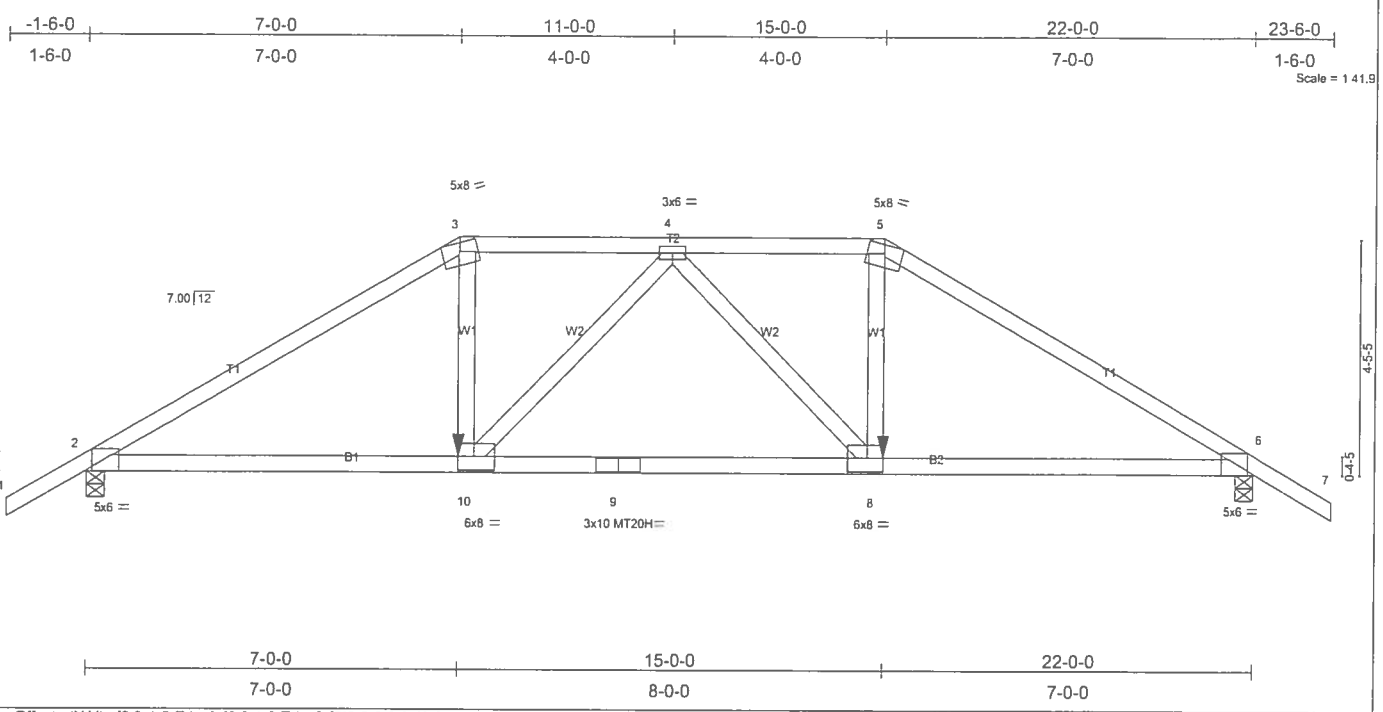


Plate Offsets (X,Y): [2:0-1-3,Edge], [6:0-1-3,Edge], [8:0-3-8,0-3-0], [10:0-3-8,0-3-0]

| LOADING (psf) | SPACING 2-0-0 | CSI | DEFL in (loc) l/defl L/d | PLATES GRIP |
|---|---|---|---|---|
| TCLL 20.0 | Plates Increase 1.25 | TC 0.60 | Vert(LL) -0.25 8-10 >999 240 | MT20 244/190 |
| TCDL 7.0 | Lumber Increase 1.25 | BC 0.77 | Vert(TL) -0.41 8-10 >629 180 | MT20H 187/143 |
| BCLL 10.0 | Rep Stress Incr NO | WB 0.39 | Horz(TL) 0.09 6 n/a n/a | |
| BCDL 5.0 | Code FBC2004/TPI2002 | (Matrix) | | Weight: 102 lb |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 3-1-13 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-8-10 oc bracing.

**REACTIONS** (lb/size) 2=1926/0-4-0, 6=1926/0-4-0
Max Horz 2=148(load case 3)
Max Uplift2=-1283(load case 4), 6=-1283(load case 5)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-3184/2057, 3-4=-2688/1845, 4-5=-2688/1845, 5-6=-3184/2057, 6-7=0/40
BOT CHORD 2-10=-1820/2648, 9-10=-2018/2873, 8-9=-2018/2873, 6-8=-1680/2648
WEBS 3-10=-841/1203, 4-10=-382/454, 4-8=-382/454, 5-8=-841/1203

**JOINT STRESS INDEX**
2 = 0.81, 3 = 0.74, 4 = 0.37, 5 = 0.74, 6 = 0.81, 8 = 0.33, 9 = 0.89 and 10 = 0.33

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1283 lb uplift at joint 2 and 1283 lb uplift at joint 6.
7) Girder carries hip end with 7-0-0 end setback.
8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 374 lb up at 15-0-0, and 539 lb down and 374 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

**LOAD CASE(S)** Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-5=-117(F=-63), 5-7=-54, 2-10=-30, 8-10=-65(F=-35), 6-8=-30
Concentrated Loads (lb)
Vert: 10=-539(F) 8=-539(F)


JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0103071045

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | T02 | HIP | 1 | 1 | Job Reference (optional) |

Builders FirstSource, Lake City, FI 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 03 09:47:57 2007 Page 1

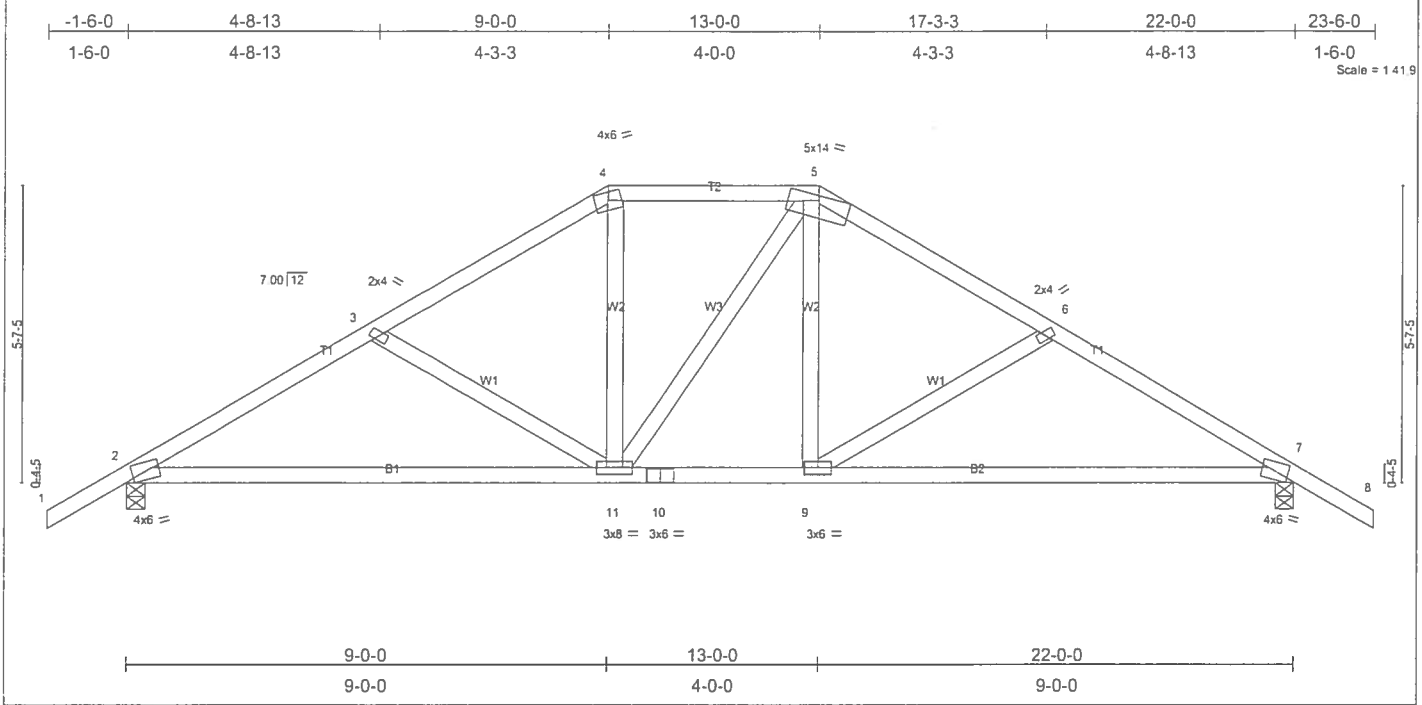


Plate Offsets (X,Y): [2:0-1-10,Edge], [7:0-1-10,Edge]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.37 | Vert(LL) | 0.40 | 7-9 | >643 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.44 | Vert(TL) | 0.33 | 7-9 | >787 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.21 | Horz(TL) | 0.04 | 7 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 114 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 5-0-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-1-9 oc bracing.

**REACTIONS** (lb/size) 2=1000/0-4-0, 7=1000/0-4-0
Max Horz 2=-189(load case 3)
Max Uplift2=-650(load case 5), 7=-650(load case 6)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-1360/1134, 3-4=-1129/1049, 4-5=-927/948, 5-6=-1128/1048, 6-7=-1360/1134, 7-8=0/40
BOT CHORD 2-11=-879/1134, 10-11=-700/926, 9-10=-700/926, 7-9=-879/1134
WEBS 3-11=-253/250, 4-11=-413/319, 5-11=-123/121, 5-9=-413/320, 6-9=-254/251

**JOINT STRESS INDEX**
2 = 0.75, 3 = 0.34, 4 = 0.34, 5 = 0.46, 6 = 0.34, 7 = 0.75, 9 = 0.35, 10 = 0.55 and 11 = 0.58

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 650 lb uplift at joint 2 and 650 lb uplift at joint 7.

**LOAD CASE(S)** Standard

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0103071046

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | T03 | COMMON | 2 | 1 | Job Reference (optional) |

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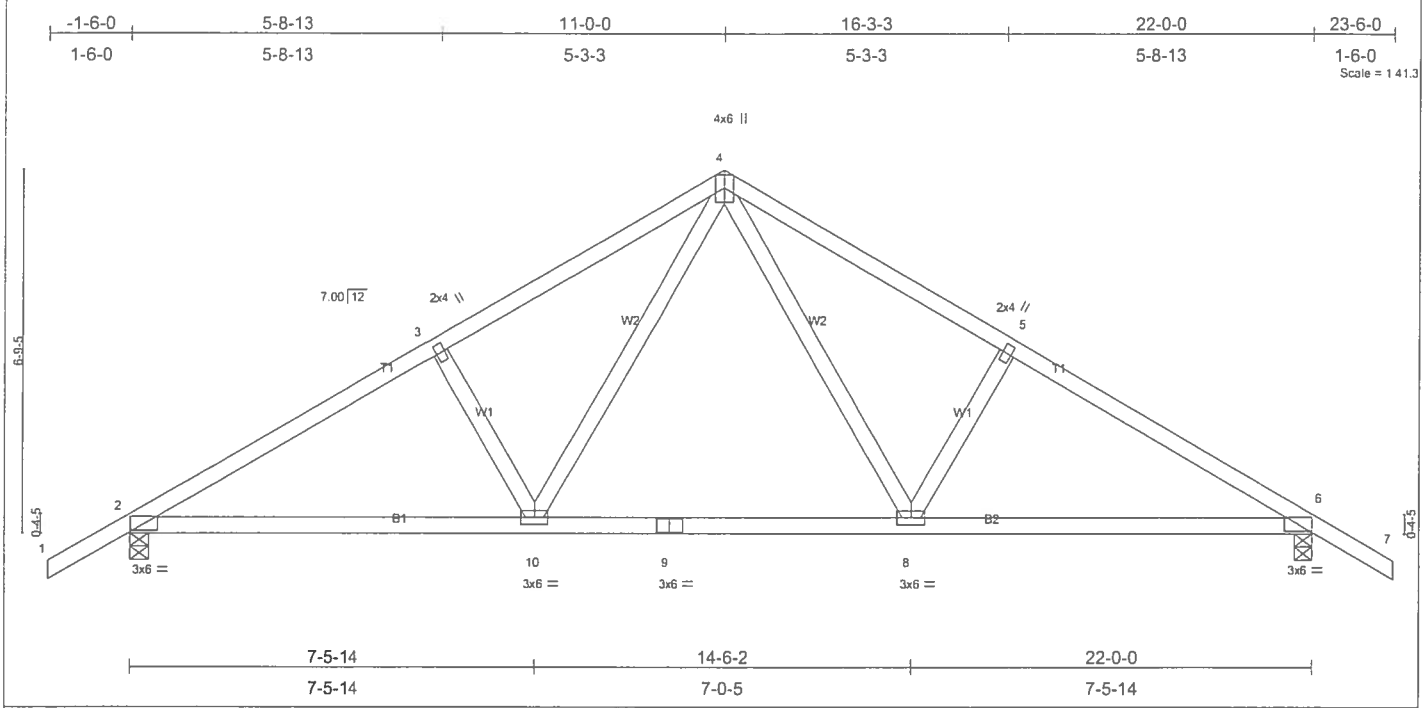


Plate Offsets (X,Y): [2:0-6-1,0-0-6], [6:0-6-1,0-0-6]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.32 | Vert(LL) | 0.20 | 2-10 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.38 | Vert(TL) | -0.16 | 2-10 | >999 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.57 | Horz(TL) | 0.04 | 6 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 108 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 5-1-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-5-4 oc bracing.

**REACTIONS** (lb/size) 2=1000/0-4-0, 6=1000/0-4-0
Max Horz 2=-229(load case 3)
Max Uplift2=-661(load case 5), 6=-661(load case 6)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-1385/1159, 3-4=-1240/1189, 4-5=-1240/1189, 5-6=-1385/1159, 6-7=0/40
BOT CHORD 2-10=-875/1126, 9-10=-497/763, 8-9=-497/763, 6-8=-875/1126
WEBS 3-10=-266/256, 4-10=-596/526, 4-8=-596/526, 5-8=-266/256

**JOINT STRESS INDEX**
2 = 0.72, 3 = 0.34, 4 = 0.51, 5 = 0.34, 6 = 0.72, 8 = 0.45, 9 = 0.32 and 10 = 0.45

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 661 lb uplift at joint 2 and 661 lb uplift at joint 6.

**LOAD CASE(S)** Standard

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE E. LUTZ, FL 33549

Dwg.#0103071047

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | T04 | SPECIAL | 1 | 3 | Job Reference (optional) |

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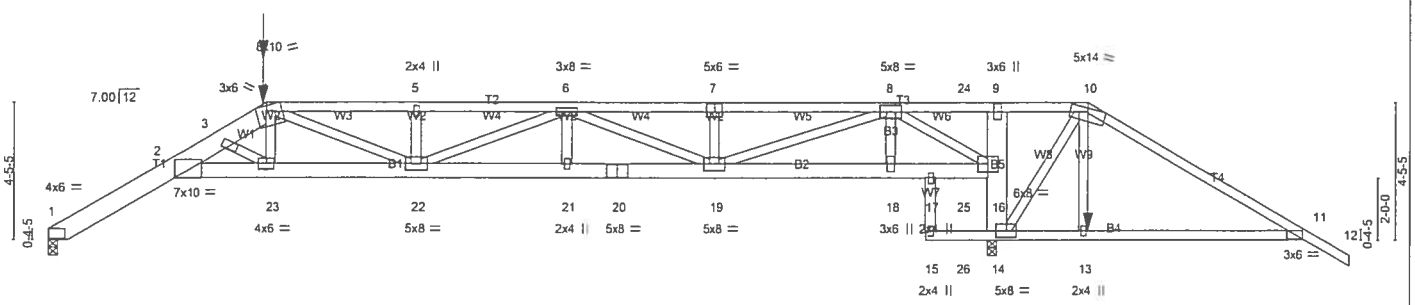
1-6-0 2-3-8 4-3-8 7-0-0 11-9-14 16-7-12 21-5-10 26-3-8 28-3-8 33-7-0 40-7-0 42-1-0

1-6-0 2-3-8 2-0-0 2-8-8 4-9-14 4-9-14 4-9-14 4-9-14 2-0-0 5-3-8 7-0-0 1-6-0

Scale = 1:71.9




28-3-8 30-5-4

2-3-8 4-3-8 7-0-0 11-9-14 16-7-12 21-5-10 26-3-8 27-0-0 30-3-8 33-7-0 40-7-0

2-3-8 2-0-0 2-8-8 4-9-14 4-9-14 4-9-14 4-9-14 0-8-8 2-0-0 3-1-12 7-0-0

1-3-8 0-1-12

Plate Offsets (X,Y): [2:0-0-3,0-0-0], [4:6-5-13,3-9-8], [4:0-4-1,Edge], [7:0-3-0,0-3-0], [11:0-3-4,0-1-8]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.95 | Vert(LL) | -0.49 | 22 | >747 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.93 | Vert(TL) | -0.77 | 22 | >475 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | NO | WB | 0.58 | Horz(TL) | 0.49 | 14 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 708 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2 *Except*
T1 2 X 8 SYP 2400F 2.0E
BOT CHORD 2 X 6 SYP No.1D *Except*
B3 2 X 4 SYP No.3, B5 2 X 8 SYP 2400F 2.0E, B4 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. Except:
6-0-0 oc bracing: 13-14,11-13.
JOINTS 1 Brace at Jt(s): 16

**REACTIONS** (lb/size) 1=2474/0-3-8, 14=5021/0-3-8
Max Horz 1=347(load case 3)
Max Uplift1=-1064(load case 3), 14=-2459(load case 5)
Max Grav 1=2554(load case 8), 14=5021(load case 1)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1222/509, 2-3=-10746/5124, 3-4=-9392/4482, 4-5=-9729/4629, 5-6=-9722/4631, 6-7=-7658/3567, 7-8=-7658/3567, 8-24=-2009/2592, 9-24=-2009/2592, 9-10=-1118/1668, 10-11=-513/754, 11-12=0/39
BOT CHORD 2-23=-5473/10828, 22-23=-4291/8459, 21-22=-4900/9936, 20-21=-4900/9936, 19-20=-4900/9936, 18-19=-1421/2787, 17-18=-1212/2467, 17-25=-1212/2467, 16-25=-1212/2467, 8-18=-23/232, 14-16=-3159/1715, 9-16=-459/445, 15-26=0/0, 14-26=0/0, 13-14=-529/553, 11-13=-560/574
WEBS 4-23=-1497/3227, 4-22=-779/1363, 5-22=-201/198, 6-22=-236/178, 6-21=-189/599, 6-19=-2665/1220, 7-19=-429/313, 8-19=-2510/5446, 10-14=-1944/1384, 10-13=-589/889, 8-16=-4708/2251, 15-17=-165/162, 3-23=-3030/1502

**JOINT STRESS INDEX**
2 = 0.81, 3 = 0.33, 4 = 0.65, 4 = 0.00, 5 = 0.34, 6 = 0.57, 7 = 0.31, 8 = 0.84, 9 = 0.17, 10 = 0.46, 11 = 0.25, 13 = 0.34, 14 = 0.49, 15 = 0.34, 16 = 0.77, 17 = 0.34, 18 = 0.49, 19 = 0.84, 20 = 0.72, 21 = 0.34, 22 = 0.29 and 23 = 0.49

**NOTES**
1) 3-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 8 - 2 rows at 0-9-0 oc, 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc, 2 X 4 - 1 row at 0-9-0 oc, 2 X 8 - 2 rows at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever right exposed ; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1064 lb uplift at joint 1 and 2459 lb uplift at joint 14.
9) Girder carries tie-in span(s): 5-6-1 from 7-0-0 to 29-5-8; 4-0-15 from 7-0-0 to 29-5-8
10) Girder carries hip end with 7-0-0 right side setback, 29-5-8 left side setback, and 7-0-0 end setback.
11) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 312 lb down and 148 lb up at 7-0-0 on top chord, and 539 lb down and 374 lb up at 33-7-0, and 382 lb down and 181 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

**LOAD CASE(S)** Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Continued on page 2

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549

Dwg.#0103071048

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | T04 | SPECIAL | 1 | 3 | Job Reference (optional) |



**LOAD CASE(S)** Standard

Uniform Loads (plf)

Vert: 1-2=-65, 2-4=-54, 4-24=-91(F=-37), 10-24=-117(F=-63), 10-12=-54, 2-23=-30, 18-23=-98(F=-68), 17-18=-98(F=-68), 17-25=-68(F), 16-25=-35(F), 15-26=-98(F=-68), 14-26=-65(F=-35), 13-14=-65(F=-35), 11-13=-30

Concentrated Loads (lb)

Vert: 4=-312(F) 23=-382(F) 13=-539(F)

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0103071049

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | T05 | SPECIAL | 1 | 1 | Job Reference (optional) |

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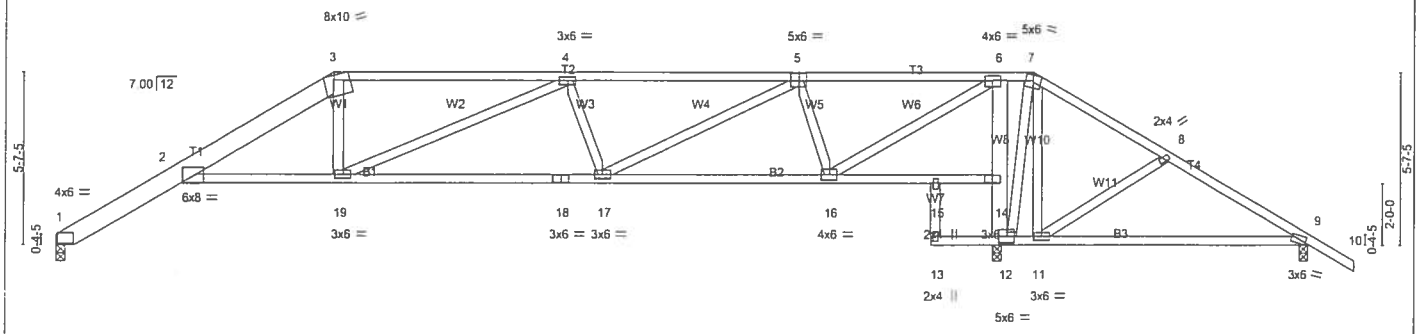
1-6-0 2-3-8 4-3-8 9-0-0 16-6-0 24-0-0 31-7-0 35-10-3 40-7-0 42-1-0

1-6-0 2-3-8 2-0-0 4-8-8 7-6-0 7-6-0 7-7-0 4-3-3 4-8-13 1-6-0

Scale = 1:71.9




30-5-4

2-3-8 4-3-8 9-0-0 17-7-12 25-0-0 28-3-8 30-3-8 31-7-0 40-7-0

2-3-8 2-0-0 4-8-8 8-7-12 7-4-4 3-3-8 2-0-0 1-1-12 9-0-0

0-1-12

Plate Offsets (X,Y): [2:0-0-12,0-0-0], [3:8-5-6,4-11-2], [3:0-4-1,Edge], [5:0-3-0,0-3-0], [9:0-1-2,0-0-7]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.97 | Vert(LL) | -0.64 | 2-19 | >572 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.93 | Vert(TL) | -1.04 | 2-19 | >350 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.90 | Horz(TL) | 0.41 | 12 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 223 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.1D *Except*
T1 2 X 8 SYP 2400F 2.0E, T2 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.1D *Except*
B2 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W8 2 X 6 SYP No.1D

**BRACING**
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied or 5-11-11 oc bracing.
JOINTS 1 Brace at Jt(s): 14

**REACTIONS** (lb/size) 1=1077/0-3-8, 9=77/0-3-8, 12=2247/0-3-8
Max Horz 1=-198(load case 3)
Max Uplift1=-351(load case 4), 9=-379(load case 6), 12=-945(load case 4)
Max Grav 1=1078(load case 9), 9=138(load case 10), 12=2247(load case 1)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-493/278, 2-3=-2374/956, 3-4=-2285/995, 4-5=-2198/984, 5-6=-848/584, 6-7=-317/707, 7-8=-330/742, 8-9=-360/653, 9-10=0/40
BOT CHORD 2-19=-1001/2266, 18-19=-1090/2317, 17-18=-1090/2317, 16-17=-543/1113, 15-16=-1064/460, 14-15=-1064/460, 12-13=0/0, 11-12=-646/288, 9-11=-524/297
WEBS 3-19=-134/403, 4-19=-237/421, 4-17=-365/305, 5-17=-505/1259, 5-16=-990/578, 6-16=-976/2166, 7-11=-472/317, 8-11=-311/262, 7-12=-537/397, 12-14=-1654/891, 6-14=-1621/888, 13-15=-56/28

**JOINT STRESS INDEX**
2 = 0.71, 3 = 0.49, 3 = 0.00, 4 = 0.49, 5 = 0.63, 6 = 0.89, 7 = 0.31, 8 = 0.34, 9 = 0.87, 11 = 0.35, 12 = 0.36, 13 = 0.34, 14 = 0.31, 15 = 0.34, 16 = 0.89, 17 = 0.72, 18 = 0.77 and 19 = 0.35

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 351 lb uplift at joint 1, 379 lb uplift at joint 9 and 945 lb uplift at joint 12.

**LOAD CASE(S)** Standard

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

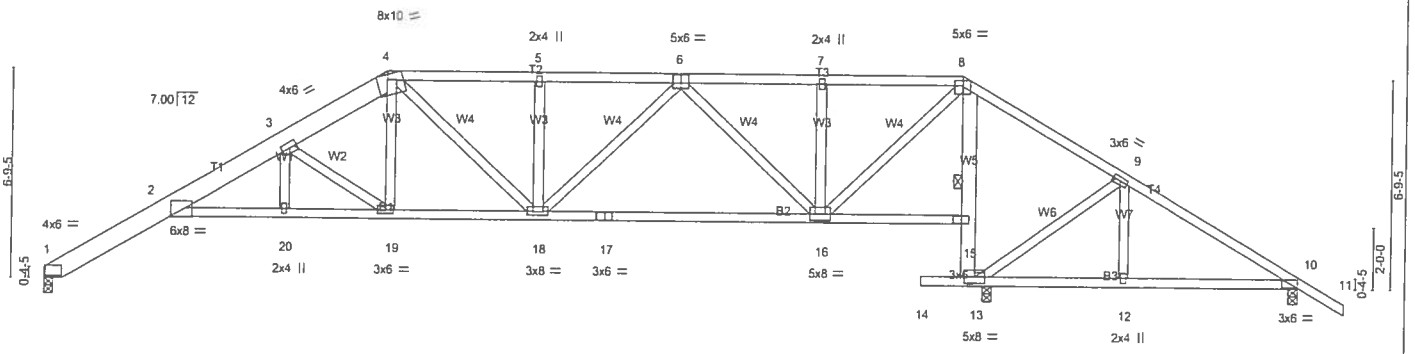
Dwg.#0103071050

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | T06 | SPECIAL | 1 | 1 | Job Reference (optional) |

Builders FirstSource, Lake City, FI 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 03 09:48:03 2007 Page 1

| 2-3-8 | 4-3-8 | 11-0-0 | 15-10-6 | 20-5-4 | 25-0-2 | 28-3-8 | 29-7-0 | 34-10-3 | 40-7-0 | 42-1-0 |
|---|---|---|---|---|---|---|---|---|---|---|
| 2-3-8 | 2-0-0 | 6-8-8 | 4-10-6 | 4-6-14 | 4-6-14 | 3-3-6 | 1-3-8 | 5-3-3 | 5-8-13 | 1-6-0 |

Scale = 1:71.9




| 2-3-8 | 4-3-8 | 11-0-0 | 15-10-6 | 20-5-4 | 25-0-2 | 28-3-8 | 29-7-0 | 34-10-3 | 40-7-0 |
|---|---|---|---|---|---|---|---|---|---|
| 2-3-8 | 2-0-0 | 6-8-8 | 4-10-6 | 4-6-14 | 4-6-14 | 3-3-6 | 1-3-8 | 4-4-15 | 5-8-13 |

30-5-4

0-10-4

Plate Offsets (X,Y): [2:0-0-8,0-0-0], [4:10-5-5,6-1-4], [4:0-4-1,Edge], [6:0-3-0,0-3-0], [8:0-3-0,0-1-12], [10:0-3-3,0-1-8]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.94 | Vert(LL) | -0.51 | 2-20 | >703 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.94 | Vert(TL) | -0.83 | 2-20 | >431 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.75 | Horz(TL) | 0.32 | 13 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 236 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2 *Except* T1 2 X 8 SYP 2400F 2.0E
BOT CHORD 2 X 4 SYP No.1D *Except* B2 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except* W5 2 X 6 SYP No.1D

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 1-5-5 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 8-13
JOINTS 1 Brace at Jt(s): 20

**REACTIONS** (lb/size) 1=1041/0-3-8, 10=100/0-3-8, 13=2306/0-3-8
Max Horz 1=-239(load case 3)
Max Uplift1=-354(load case 5), 10=-419(load case 6), 13=-822(load case 4)
Max Grav 1=1045(load case 9), 10=179(load case 10), 13=2306(load case 1)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-478/285, 2-3=-2785/1012, 3-4=-1988/803, 4-5=-1634/712, 5-6=-1631/715, 6-7=-468/447, 7-8=-468/446, 8-9=-348/885, 9-10=-280/590, 10-11=0/40
BOT CHORD 2-20=-1140/2755, 19-20=-1140/2756, 18-19=-717/1627, 17-18=-574/1157, 16-17=-574/1157, 15-16=-808/322, 13-14=0/0, 12-13=-485/238, 10-12=-485/238
WEBS 4-19=-290/847, 4-18=-269/160, 9-13=-464/425, 9-12=-225/184, 3-19=-1404/555, 13-15=-1995/793, 8-15=-1878/768, 6-18=-210/668, 6-16=-1030/503, 8-16=-751/1709, 5-18=-221/203, 7-16=-238/222, 3-20=-34/261

**JOINT STRESS INDEX**
2 = 0.74, 3 = 0.31, 4 = 0.48, 4 = 0.00, 5 = 0.34, 6 = 0.36, 7 = 0.34, 8 = 0.65, 9 = 0.42, 10 = 0.28, 12 = 0.34, 13 = 0.31, 15 = 0.22, 16 = 0.81, 17 = 0.48, 18 = 0.66, 19 = 0.55 and 20 = 0.34

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 354 lb uplift at joint 1, 419 lb uplift at joint 10 and 822 lb uplift at joint 13.

**LOAD CASE(S)** Standard

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549

Dwg.#0103071051

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | T07 | SPECIAL | 1 | 1 | Job Reference (optional) |

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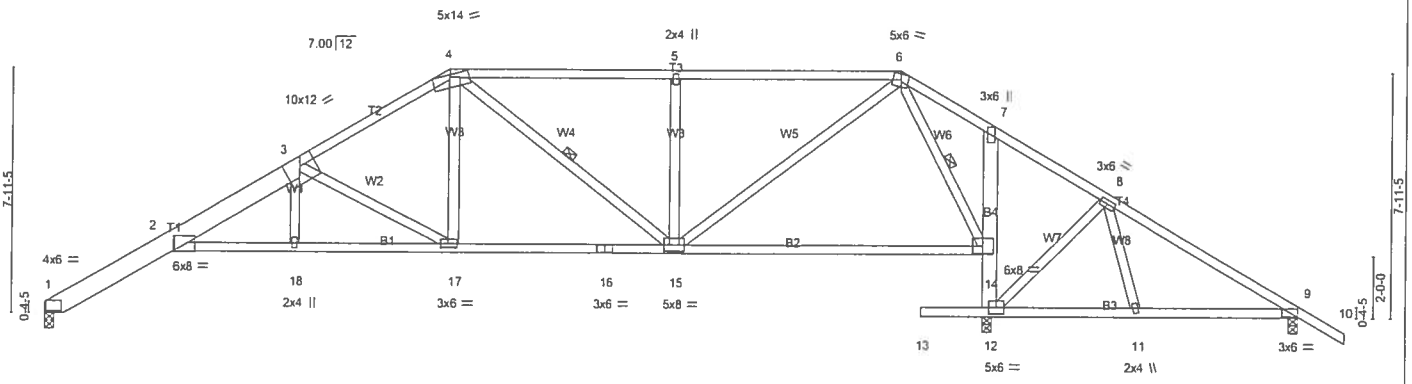
| 4-3-8 | 8-0-0 | 13-0-0 | 20-3-8 | 27-7-0 | 34-2-7 | 40-7-0 | 42-1-0 |
|---|---|---|---|---|---|---|---|
| 4-3-8 | 3-8-8 | 5-0-0 | 7-3-8 | 7-3-8 | 6-7-7 | 6-4-9 | 1-6-0 |

Scale = 1:71.9




30-5-4

| 2-3-8 | 4-3-8 | 8-0-0 | 13-0-0 | 20-3-8 | 28-3-8 | 30-3-8 | 35-3-5 | 40-7-0 |
|---|---|---|---|---|---|---|---|---|
| 2-3-8 | 2-0-0 | 3-8-8 | 5-0-0 | 7-3-8 | 8-0-0 | 2-0-0 | 4-10-1 | 5-3-11 |

0-1-12

Plate Offsets (X,Y): [2:0-0-12,0-0-0], [3:7-7-5,4-4-11], [9:0-3-3,0-1-8]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.97 | Vert(LL) | -0.57 | 2-18 | >639 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.90 | Vert(TL) | -0.93 | 2-18 | >392 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.88 | Horz(TL) | 0.40 | 12 | n/a | n/a | | |
| BCDL | 5.0 | Code | FBC2004/TPI2002 | (Matrix) | | | | | | | Weight: 231 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2 *Except*
T1 2 X 8 SYP 2400F 2.0E
BOT CHORD 2 X 4 SYP No.2 *Except*
B1 2 X 4 SYP No.1D, B4 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except*
W2 2 X 4 SYP No.2

**BRACING**
TOP CHORD Structural wood sheathing directly applied
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-15, 6-14

**REACTIONS** (lb/size) 12=2285/0-3-8, 9=106/0-3-8, 1=1078/0-3-8
Max Horz 1=-285(load case 3)
Max Uplift12=-600(load case 4), 9=-419(load case 6), 1=-376(load case 5)
Max Grav 12=2285(load case 1), 9=203(load case 10), 1=1078(load case 1)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-493/309, 2-3=-2632/885, 3-4=-1705/655, 4-5=-1137/542, 5-6=-1137/542, 6-7=-418/1141, 7-8=-243/846, 8-9=-157/543, 9-10=0/40
BOT CHORD 2-18=-1035/2548, 17-18=-1033/2532, 16-17=-620/1409, 15-16=-620/1409, 14-15=-110/110, 12-14=-1911/562, 7-14=-165/138, 12-13=0/0, 11-12=-510/170, 9-11=-446/131
WEBS 3-17=-1290/574, 4-17=-228/758, 4-15=-348/159, 5-15=-424/363, 6-15=-585/1452, 8-12=-427/353, 8-11=-235/262, 6-14=-1978/850, 3-18=-28/276

**JOINT STRESS INDEX**
2 = 0.75, 3 = 0.35, 3 = 0.00, 4 = 0.96, 5 = 0.34, 6 = 0.65, 7 = 0.81, 8 = 0.38, 9 = 0.41, 11 = 0.34, 12 = 0.71, 14 = 0.86, 15 = 0.68, 16 = 0.51, 17 = 0.50 and 18 = 0.34

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 600 lb uplift at joint 12, 419 lb uplift at joint 9 and 376 lb uplift at joint 1.

**LOAD CASE(S)** Standard

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549

Dwg.#0103071052

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | T08 | SPECIAL | 1 | 1 | Job Reference (optional) |

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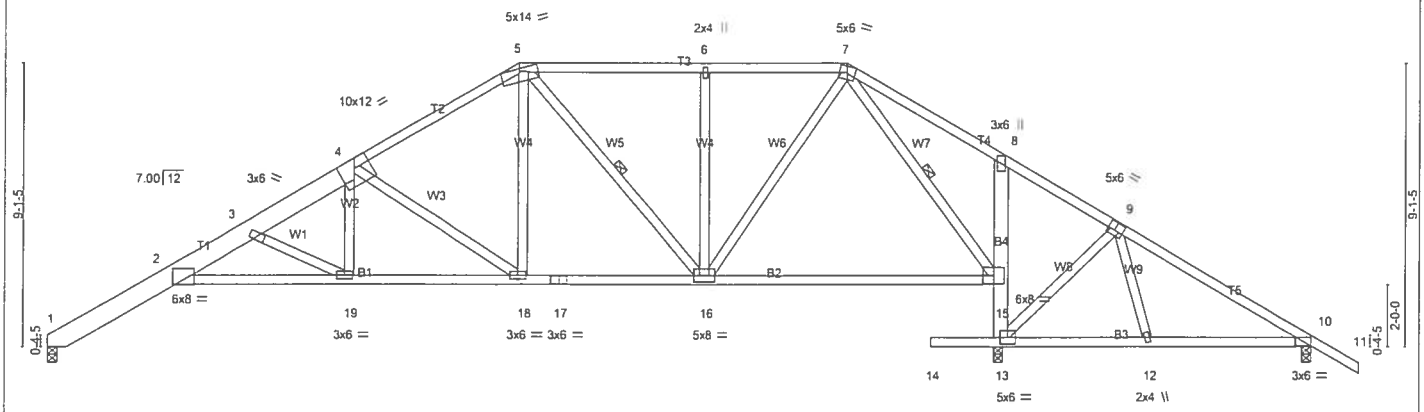
| 2-3-8 | 4-3-8 | 6-8-0 | 9-7-12 | 15-0-0 | 21-0-0 | 25-7-0 | 30-3-8 | 34-2-7 | 40-7-0 | 42-1-0 |
|---|---|---|---|---|---|---|---|---|---|---|
| 2-3-8 | 2-0-0 | 2-4-8 | 2-11-12 | 5-4-4 | 6-0-0 | 4-7-0 | 4-8-8 | 3-10-15 | 6-4-9 | 1-6-0 |

Scale = 1:71.4




30-5-4

| 2-3-8 | 4-3-8 | 9-7-12 | 15-0-0 | 21-0-0 | 28-3-8 | 30-3-8 | 35-3-5 | 40-7-0 |
|---|---|---|---|---|---|---|---|---|
| 2-3-8 | 2-0-0 | 5-4-4 | 5-4-4 | 6-0-0 | 7-3-8 | 2-0-0 | 4-10-1 | 5-3-11 |

0-1-12

Plate Offsets (X,Y): [9:0-2-12,0-3-0], [10:0-3-3,0-1-8]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 1.00 | Vert(LL) | -0.52 | 2-19 | >697 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.77 | Vert(TL) | -0.85 | 2-19 | >427 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.81 | Horz(TL) | 0.38 | 13 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 247 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2 *Except*
T1 2 X 8 SYP 2400F 2.0E
BOT CHORD 2 X 4 SYP No.2 *Except*
B1 2 X 4 SYP No.1D, B4 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. Except:
1 Row at midpt 2-19
WEBS 1 Row at midpt 5-16, 7-15

**REACTIONS** (lb/size) 1=1099/0-3-8, 13=2198/0-3-8, 10=171/0-3-8
Max Horz 1=-325(load case 3)
Max Uplift1=-390(load case 5), 13=-592(load case 5), 10=-427(load case 6)
Max Grav 1=1099(load case 1), 13=2198(load case 1), 10=267(load case 10)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-503/325, 2-3=-3347/1186, 3-4=-2502/878, 4-5=-1468/545, 5-6=-923/454, 6-7=-923/454, 7-8=-295/910, 8-9=-179/712, 9-10=-98/426, 10-11=0/40
BOT CHORD 2-19=-1324/3485, 18-19=-791/2079, 17-18=-492/1207, 16-17=-492/1207, 15-16=-162/372, 13-15=-1820/407, 8-15=-183/191, 13-14=0/0, 12-13=-403/119, 10-12=-341/81
WEBS 4-19=-257/883, 4-18=-1067/504, 5-18=-250/737, 5-16=-444/197, 6-16=-298/269, 7-16=-402/1033, 7-15=-1859/662, 9-13=-417/348, 9-12=-233/251, 3-19=-1519/630

**JOINT STRESS INDEX**
2 = 0.79, 3 = 0.52, 4 = 0.42, 5 = 0.95, 6 = 0.34, 7 = 0.52, 8 = 0.43, 9 = 0.58, 10 = 0.41, 12 = 0.34, 13 = 0.63, 15 = 0.85, 16 = 0.51, 17 = 0.42, 18 = 0.48 and 19 = 0.58

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 390 lb uplift at joint 1, 592 lb uplift at joint 13 and 427 lb uplift at joint 10.

**LOAD CASE(S)** Standard

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549

Dwg.#0103071053

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | T09 | SPECIAL | 1 | 1 | Job Reference (optional) |

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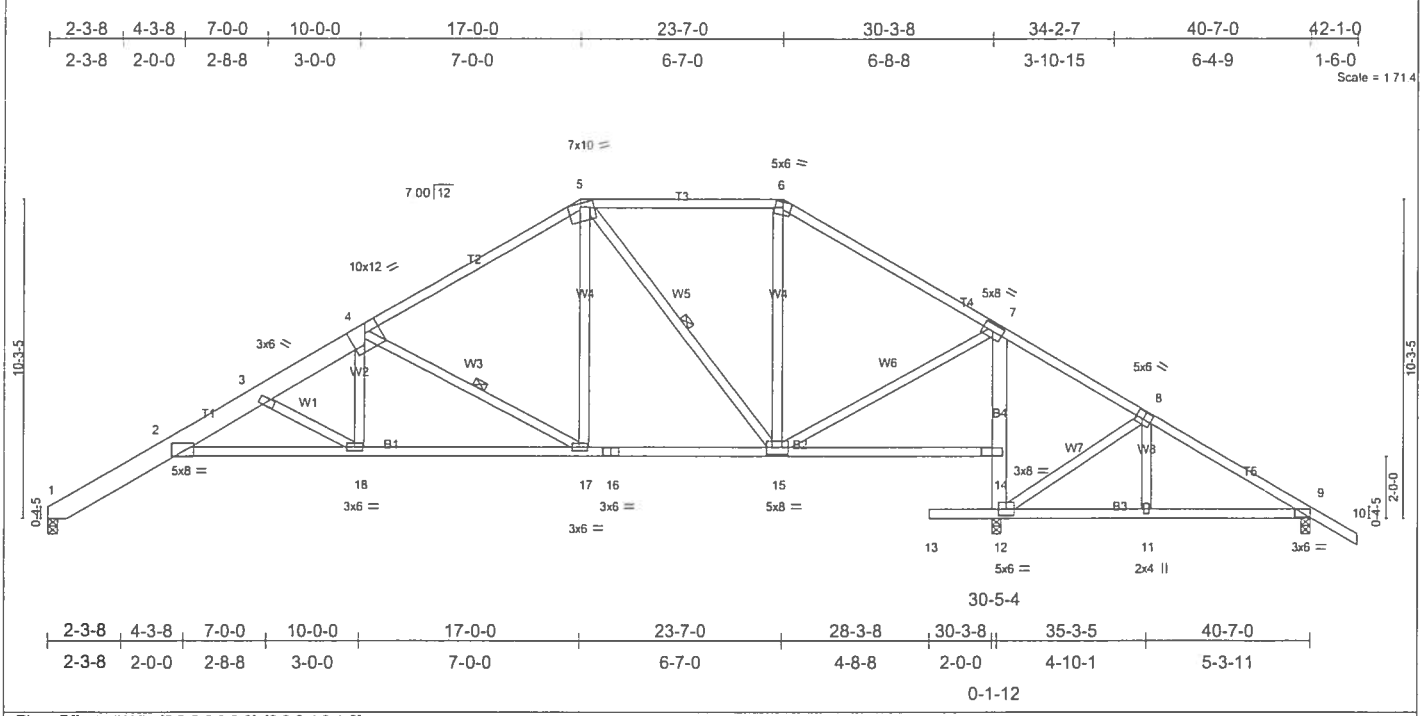


Plate Offsets (X,Y): [8:0-3-0,0-3-0], [9:0-3-4,0-1-8]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.99 | Vert(LL) | -0.55 | 2-18 | >668 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.87 | Vert(TL) | -0.89 | 2-18 | >409 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.52 | Horz(TL) | 0.38 | 12 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 244 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2 *Except*
T1 2 X 8 SYP 2400F 2.0E
BOT CHORD 2 X 4 SYP No.2 *Except*
B4 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied or 5-8-0 oc bracing.
WEBS 1 Row at midpt 4-17, 5-15

**REACTIONS** (lb/size) 12=2217/0-3-8, 9=157/0-3-8, 1=1094/0-3-8
Max Horz 1=-366(load case 3)
Max Uplift12=-631(load case 5), 9=-424(load case 6), 1=-394(load case 5)
Max Grav 12=2217(load case 1), 9=273(load case 10), 1=1094(load case 1)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-501/344, 2-3=-3163/1144, 3-4=-2430/885, 4-5=-1244/479, 5-6=-565/403, 6-7=-749/380, 7-8=-186/752, 8-9=-111/474, 9-10=0/40
BOT CHORD 2-18=-1179/3261, 17-18=-679/2020, 16-17=-350/994, 15-16=-350/994, 14-15=-775/285, 12-14=-1879/467, 7-14=-1747/479, 12-13=0/0, 11-12=-348/96, 9-11=-345/94
WEBS 4-18=-246/897, 4-17=-1182/592, 5-17=-242/765, 5-15=-737/307, 6-15=-84/119, 7-15=-459/1521, 8-12=-459/380, 8-11=-214/233, 3-18=-1370/572

**JOINT STRESS INDEX**
2 = 0.74, 3 = 0.43, 4 = 0.41, 5 = 0.79, 6 = 0.62, 7 = 0.50, 8 = 0.40, 9 = 0.27, 11 = 0.34, 12 = 0.66, 14 = 0.66, 15 = 0.70, 16 = 0.40, 17 = 0.50 and 18 = 0.59

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 631 lb uplift at joint 12, 424 lb uplift at joint 9 and 394 lb uplift at joint 1.

**LOAD CASE(S)** Standard

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549

Dwg.#0103071054

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | T09A | SPECIAL | 1 | 1 | Job Reference (optional) |

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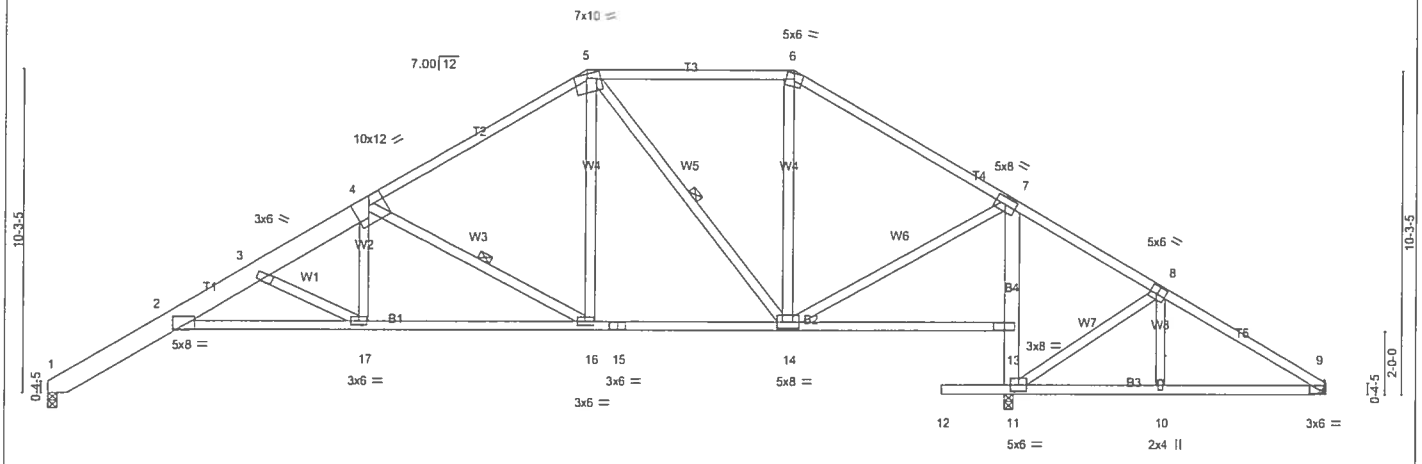
2-3-8 | 4-3-8 | 6-10-4 | 10-0-0 | 17-0-0 | 23-7-0 | 30-3-8 | 35-3-5 | 40-7-0 | 42-1-0

2-3-8 | 2-0-0 | 2-6-12 | 3-1-12 | 7-0-0 | 6-7-0 | 6-8-8 | 4-11-13 | 5-3-11 | 1-6-0

Scale = 1:70.5




30-5-4

2-3-8 | 4-3-8 | 6-10-4 | 10-0-0 | 17-0-0 | 23-7-0 | 28-3-8 | 30-3-8 | 35-3-5 | 40-7-0

2-3-8 | 2-0-0 | 2-6-12 | 3-1-12 | 7-0-0 | 6-7-0 | 4-8-8 | 2-0-0 | 4-10-1 | 5-3-11

0-1-12

Plate Offsets (X,Y): {8:0-3-0,0-3-0}, {9:0-3-4,0-1-8}

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.99 | Vert(LL) | -0.54 | 2-17 | >673 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.88 | Vert(TL) | -0.88 | 2-17 | >412 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.53 | Horz(TL) | 0.38 | 11 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 241 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2 *Except*
T1 2 X 8 SYP 2400F 2.0E
BOT CHORD 2 X 4 SYP No.2 *Except*
B4 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied or 5-6-6 oc bracing.
WEBS 1 Row at midpt 4-16, 5-14

**REACTIONS** (lb/size) 11=2223/0-3-8, 9=61/Mechanical, 1=1095/0-3-8
Max Horz 1=348(load case 4)
Max Uplift11=-652(load case 5), 9=-326(load case 3), 1=-390(load case 5)
Max Grav 11=2223(load case 1), 9=177(load case 10), 1=1095(load case 1)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-501/317, 2-3=-3214/1169, 3-4=-2420/880, 4-5=-1244/475, 5-6=-566/384, 6-7=-750/357, 7-8=-204/751, 8-9=-124/456
BOT CHORD 2-17=-1238/3327, 16-17=-694/2020, 15-16=-362/994, 14-15=-362/994, 13-14=-772/286, 11-13=-1876/483, 7-13=-1744/495, 11-12=0/0, 10-11=-341/91, 9-10=-339/89
WEBS 4-17=-240/869, 4-16=-1182/596, 5-16=-244/765, 5-14=-736/311, 6-14=-84/124, 7-14=-471/1521, 8-11=-482/414, 8-10=-233/246, 3-17=-1417/602

**JOINT STRESS INDEX**
2 = 0.75, 3 = 0.48, 4 = 0.40, 5 = 0.79, 6 = 0.62, 7 = 0.49, 8 = 0.44, 9 = 0.39, 10 = 0.34, 11 = 0.67, 13 = 0.66, 14 = 0.70, 15 = 0.40, 16 = 0.50 and 17 = 0.57

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 652 lb uplift at joint 11, 326 lb uplift at joint 9 and 390 lb uplift at joint 1.

**LOAD CASE(S)** Standard

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549

Dwg.#0103071055

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | T10 | SPECIAL | 1 | 1 | Job Reference (optional) |

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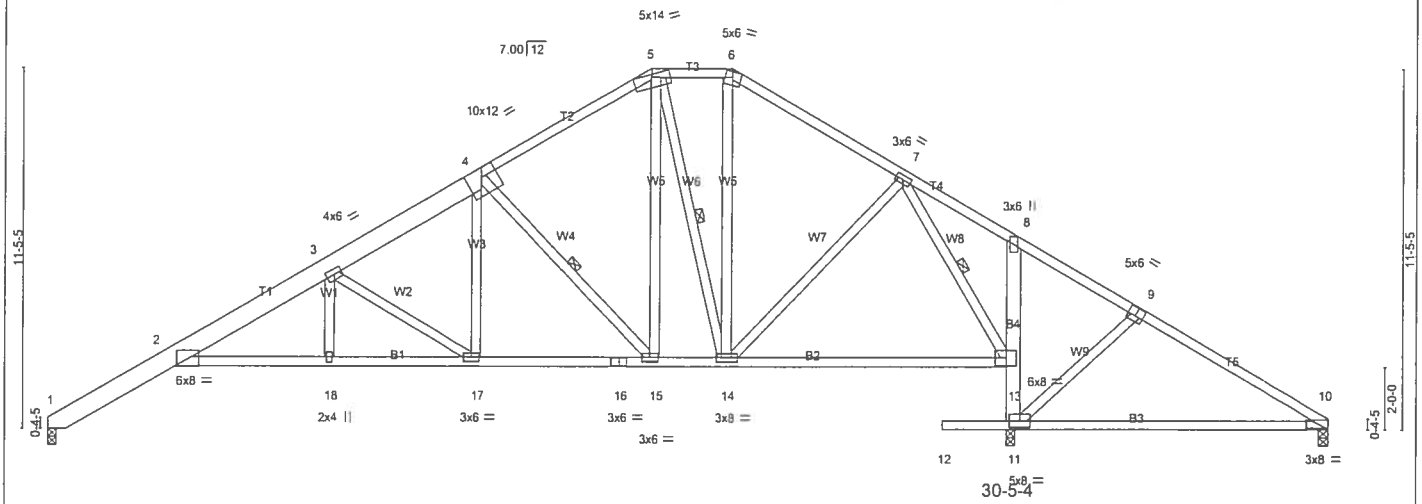
| 2-3-8 | 4-3-8 | 8-10-4 | 13-6-0 | 19-0-0 | 21-7-0 | 27-0-0 | 30-3-8 | 34-2-7 | 40-7-0 |
|---|---|---|---|---|---|---|---|---|---|
| 2-3-8 | 2-0-0 | 4-6-12 | 4-7-12 | 5-6-0 | 2-7-0 | 5-5-0 | 3-3-8 | 3-10-15 | 6-4-9 |

Scale = 1:70.4




| 2-3-8 | 4-3-8 | 8-10-4 | 13-6-0 | 19-0-0 | 21-7-0 | 28-3-8 | 30-3-8 | 35-3-5 | 40-7-0 |
|---|---|---|---|---|---|---|---|---|---|
| 2-3-8 | 2-0-0 | 4-6-12 | 4-7-12 | 5-6-0 | 2-7-0 | 6-8-8 | 2-0-0 | 4-10-1 | 5-3-11 |
| | | | | | | | 0-1-12 | | |

Plate Offsets (X,Y): [2:0-0-8,0-0-0], [9:0-3-0,0-3-0], [10:0-8-0,0-0-10]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 1.00 | Vert(LL) | -0.62 | 2-18 | >591 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.84 | Vert(TL) | -1.01 | 2-18 | >362 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.70 | Horz(TL) | 0.42 | 11 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 266 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2 *Except*
T1 2 X 8 SYP 2400F 2.0E
BOT CHORD 2 X 4 SYP No.2 *Except*
B1 2 X 4 SYP No.1D, B4 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-15, 5-14, 7-13

**REACTIONS** (lb/size) 11=2172/0-3-8, 10=100/0-3-8, 1=1107/0-3-8
Max Horz 1=388(load case 4)
Max Uplift11=-636(load case 5), 10=-389(load case 3), 1=-405(load case 5)
Max Grav 11=2172(load case 1), 10=226(load case 10), 1=1107(load case 1)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-507/341, 2-3=-2649/973, 3-4=-1800/709, 4-5=-1003/471, 5-6=-672/436, 6-7=-850/433, 7-8=-157/864, 8-9=-64/592, 9-10=-102/494
BOT CHORD 2-18=-970/2573, 17-18=-970/2573, 16-17=-436/1422, 15-16=-436/1422, 14-15=-237/798, 13-14=-76/263, 11-13=-1742/415, 8-13=-85/89, 11-12=0/0, 10-11=-379/61
WEBS 4-17=-297/876, 4-15=-925/482, 5-15=-347/752, 5-14=-567/282, 6-14=-126/242, 7-14=-192/676, 9-11=-308/293, 7-13=-1796/485, 3-18=-28/259, 3-17=-1347/629

**JOINT STRESS INDEX**
2 = 0.73, 3 = 0.31, 4 = 0.31, 5 = 0.61, 6 = 0.35, 7 = 0.62, 8 = 0.52, 9 = 0.71, 10 = 0.73, 11 = 0.81, 13 = 0.82, 14 = 0.69, 15 = 0.49, 16 = 0.54, 17 = 0.57 and 18 = 0.34

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 636 lb uplift at joint 11, 389 lb uplift at joint 10 and 405 lb uplift at joint 1.

**LOAD CASE(S)** Standard

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE E. LUTZ, FL 33549

Dwg.#0103071056

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | T10A | SPECIAL | 1 | 1 | Job Reference (optional) |

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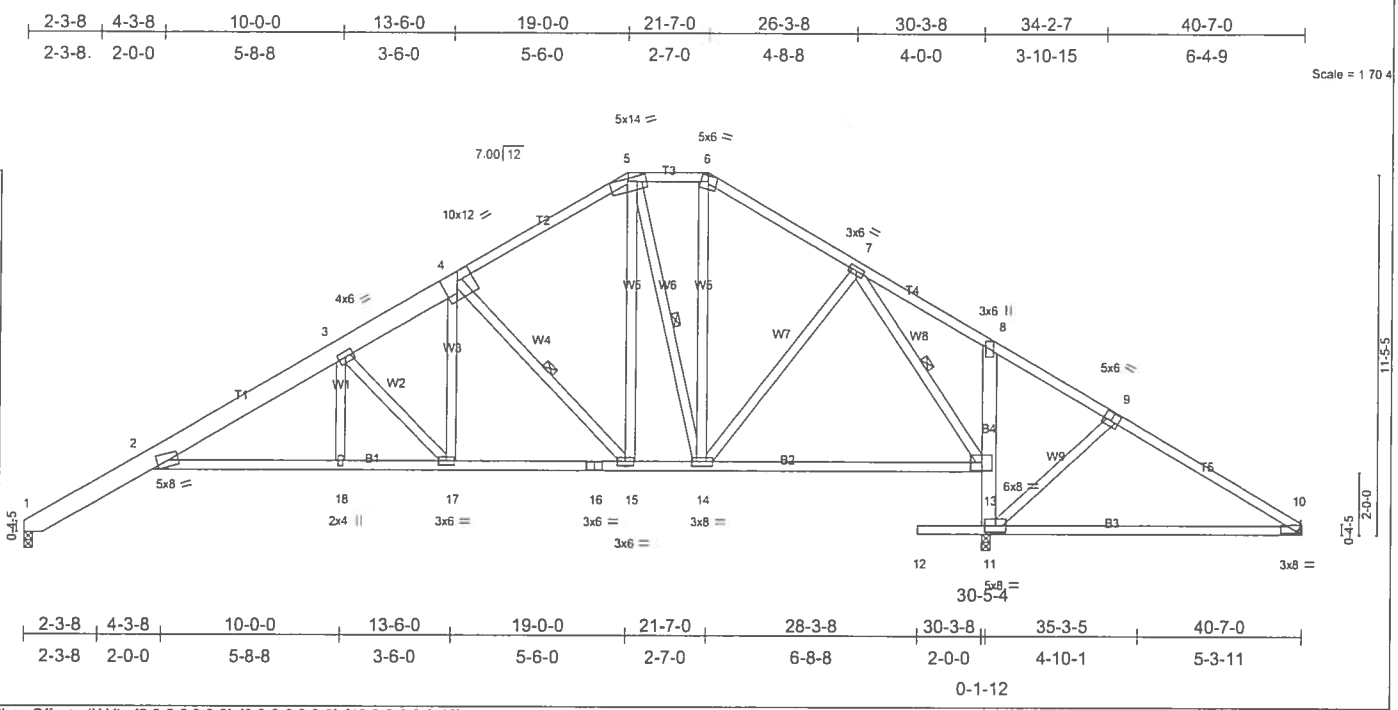


Plate Offsets (X,Y): [2:0-2-6,0-0-9], [9:0-3-0,0-3-0], [10:0-8-0,0-0-10]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 1.00 | Vert(LL) | -0.69 | 2-18 | >527 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.77 | Vert(TL) | -1.13 | 2-18 | >323 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.55 | Horz(TL) | 0.46 | 11 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 267 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2 *Except*
T1 2 X 8 SYP 2400F 2.0E
BOT CHORD 2 X 4 SYP No.2 *Except*
B1 2 X 4 SYP No.1D, B4 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-15, 5-14, 7-13

**REACTIONS** (lb/size) 11=2192/0-3-8, 10=85/Mechanical, 1=1102/0-3-8
Max Horz 1=388(load case 4)
Max Uplift11=-647(load case 5), 10=-392(load case 3), 1=-402(load case 5)
Max Grav 11=2192(load case 1), 10=219(load case 10), 1=1102(load case 1)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-505/341, 2-3=-2405/870, 3-4=-1830/743, 4-5=-992/467, 5-6=-658/429, 6-7=-825/436, 7-8=-179/893, 8-9=-75/621, 9-10=-113/514
BOT CHORD 2-18=-845/2306, 17-18=-845/2306, 16-17=-429/1406, 15-16=-429/1406, 14-15=-233/790, 13-14=-47/316, 11-13=-1764/427, 8-13=-123/132, 11-12=0/0, 10-11=-405/70
WEBS 4-17=-378/1019, 4-15=-915/478, 5-15=-345/754, 5-14=-581/281, 6-14=-146/244, 7-14=-188/628, 9-11=-306/291, 7-13=-1834/483, 3-17=-1270/590, 3-18=-32/264

**JOINT STRESS INDEX**
2 = 0.83, 3 = 0.33, 4 = 0.36, 5 = 0.63, 6 = 0.26, 7 = 0.63, 8 = 0.51, 9 = 0.70, 10 = 0.73, 11 = 0.80, 13 = 0.84, 14 = 0.69, 15 = 0.49, 16 = 0.54, 17 = 0.67 and 18 = 0.34

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 647 lb uplift at joint 11, 392 lb uplift at joint 10 and 402 lb uplift at joint 1.

**LOAD CASE(S)** Standard

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ. FL 33549

Dwg.#0103071057

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | T11 | SPECIAL | 1 | 1 | Job Reference (optional) |

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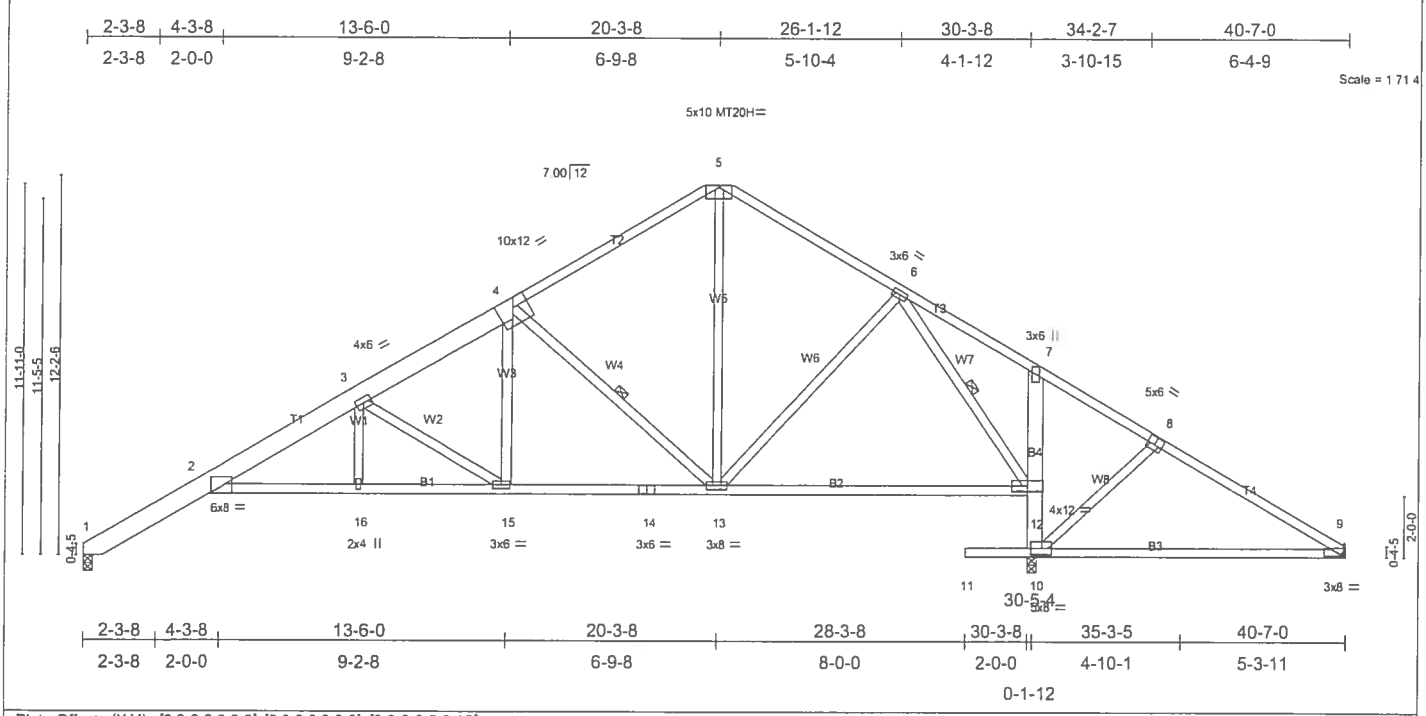


Plate Offsets (X,Y): [2:0-0-8,0-0-0], [8:0-3-0,0-3-0], [9:0-8-0,0-0-10]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 1.00 | Vert(LL) | -0.62 | 2-16 | >592 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.84 | Vert(TL) | 0.33 | 9-10 | >361 | 180 | MT20H | 187/143 |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.69 | Horz(TL) | 0.42 | 10 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 244 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2 *Except*
T1 2 X 8 SYP 2400F 2.0E
BOT CHORD 2 X 4 SYP No.2 *Except*
B1 2 X 4 SYP No.1D, B4 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-13, 6-12

**REACTIONS** (lb/size) 10=2177/0-3-8, 9=96/Mechanical, 1=1106/0-3-8
Max Horz 1=414(load case 4)
Max Uplift10=-652(load case 5), 9=-398(load case 3), 1=-407(load case 5)
Max Grav 10=2177(load case 1), 9=226(load case 10), 1=1106(load case 1)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-507/353, 2-3=-2643/983, 3-4=-1799/726, 4-5=-912/466, 5-6=-883/460, 6-7=-128/852, 7-8=-42/599, 8-9=-79/512
BOT CHORD 2-16=-991/2566, 15-16=-991/2566, 14-15=-468/1428, 13-14=-468/1428, 12-13=-50/327, 10-12=-1747/433, 7-12=-122/129, 10-11=0/0, 9-10=-385/42
WEBS 4-15=-297/861, 4-13=-966/534, 6-13=-169/600, 8-10=-308/291, 5-13=-255/479, 6-12=-1797/489, 3-15=-1332/615, 3-16=-26/262

**JOINT STRESS INDEX**
2 = 0.73, 3 = 0.31, 4 = 0.30, 5 = 0.77, 6 = 0.61, 7 = 0.41, 8 = 0.71, 9 = 0.72, 10 = 0.80, 12 = 0.97, 13 = 0.60, 14 = 0.51, 15 = 0.56 and 16 = 0.34

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All plates are MT20 plates unless otherwise indicated.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 652 lb uplift at joint 10, 398 lb uplift at joint 9 and 407 lb uplift at joint 1.

**LOAD CASE(S)** Standard

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877. BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS. INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ. FL 33549

Dwg.#0103071058

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | T12 | MONO HIP | 1 | 1 | Job Reference (optional) |

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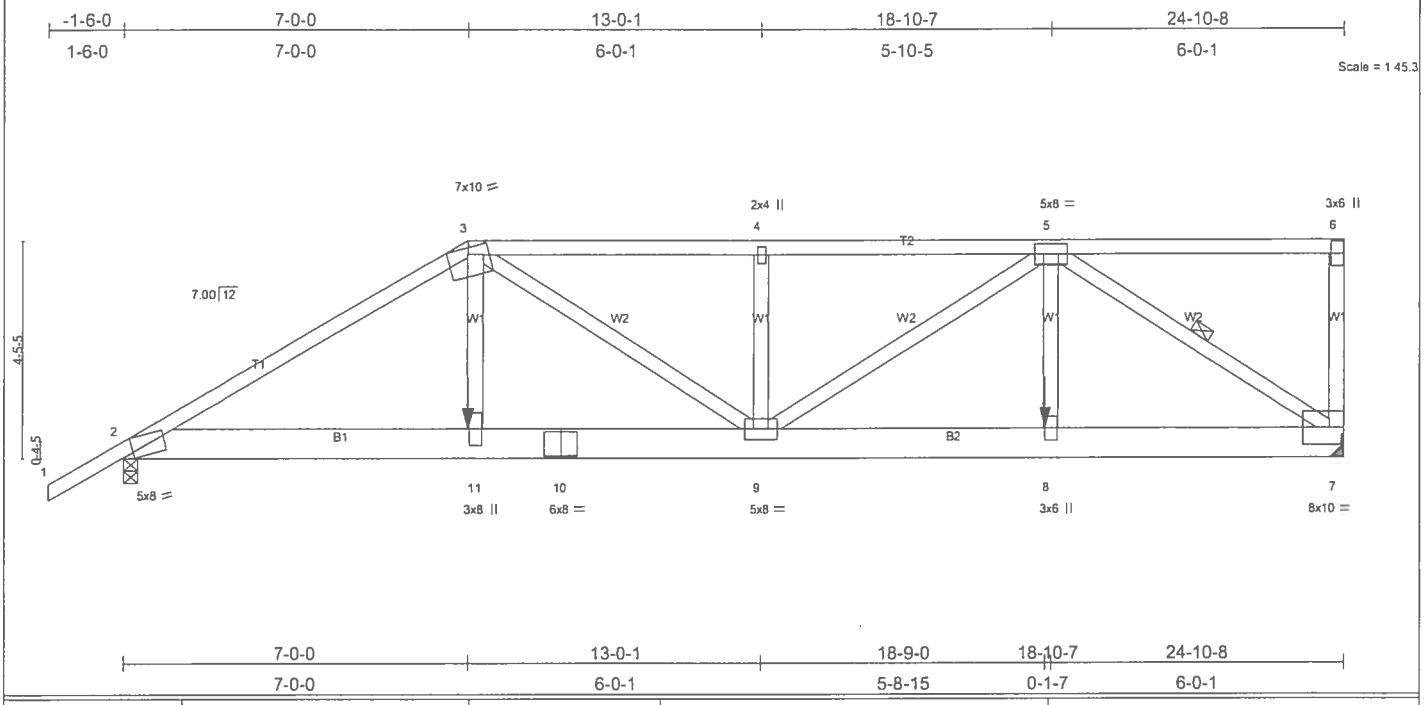


| LOADING (psf) | SPACING 2-0-0 | CSI | DEFL in (loc) l/defl L/d | PLATES GRIP |
|---|---|---|---|---|
| TCLL 20.0 | Plates Increase 1.25 | TC 0.79 | Vert(LL) -0.16 9 >999 240 | MT20 244/190 |
| TCDL 7.0 | Lumber Increase 1.25 | BC 0.20 | Vert(TL) -0.26 9-11 >999 180 | |
| BCLL 10.0 | Rep Stress Incr NO | WB 0.85 | Horz(TL) 0.05 7 n/a n/a | |
| BCDL 5.0 | Code FBC2004/TPI2002 | (Matrix) | | Weight: 169 lb |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 2-9-3 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 9-2-7 oc bracing.
WEBS 1 Row at midpt 5-7

**REACTIONS** (lb/size) 7=2060/Mechanical, 2=2150/0-3-8
Max Horz 2=249(load case 4)
Max Uplift7=-1089(load case 2), 2=-1070(load case 4)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-3681/1843, 3-4=-3735/2030, 4-5=-3734/2030, 5-6=-48/29, 6-7=-134/106
BOT CHORD 2-11=-1645/3110, 10-11=-1657/3141, 9-10=-1657/3141, 8-9=-1460/2724, 7-8=-1460/2724
WEBS 3-11=-323/866, 3-9=-562/711, 4-9=-720/795, 5-9=-723/1223, 5-8=-103/629, 5-7=-3238/1732

**JOINT STRESS INDEX**
2 = 0.74, 3 = 0.96, 4 = 0.46, 5 = 0.80, 6 = 0.29, 7 = 0.55, 8 = 0.20, 9 = 0.57, 10 = 0.61 and 11 = 0.28

**NOTES**
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1089 lb uplift at joint 7 and 1070 lb uplift at joint 2.
5) Girder carries hip end with 6-1-8 right side setback, 7-0-0 left side setback, and 7-0-0 end setback.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 374 lb up at 7-0-0, and 359 lb down and 170 lb up at 18-9-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

**LOAD CASE(S)** Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-5=-117(F=-63), 5-6=-54, 2-11=-30, 8-11=-65(F=-35), 7-8=-30
Concentrated Loads (lb)
Vert: 11=-539(F) 8=-359(F)

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0103071059

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | T13 | MONO HIP | 1 | 1 | Job Reference (optional) |

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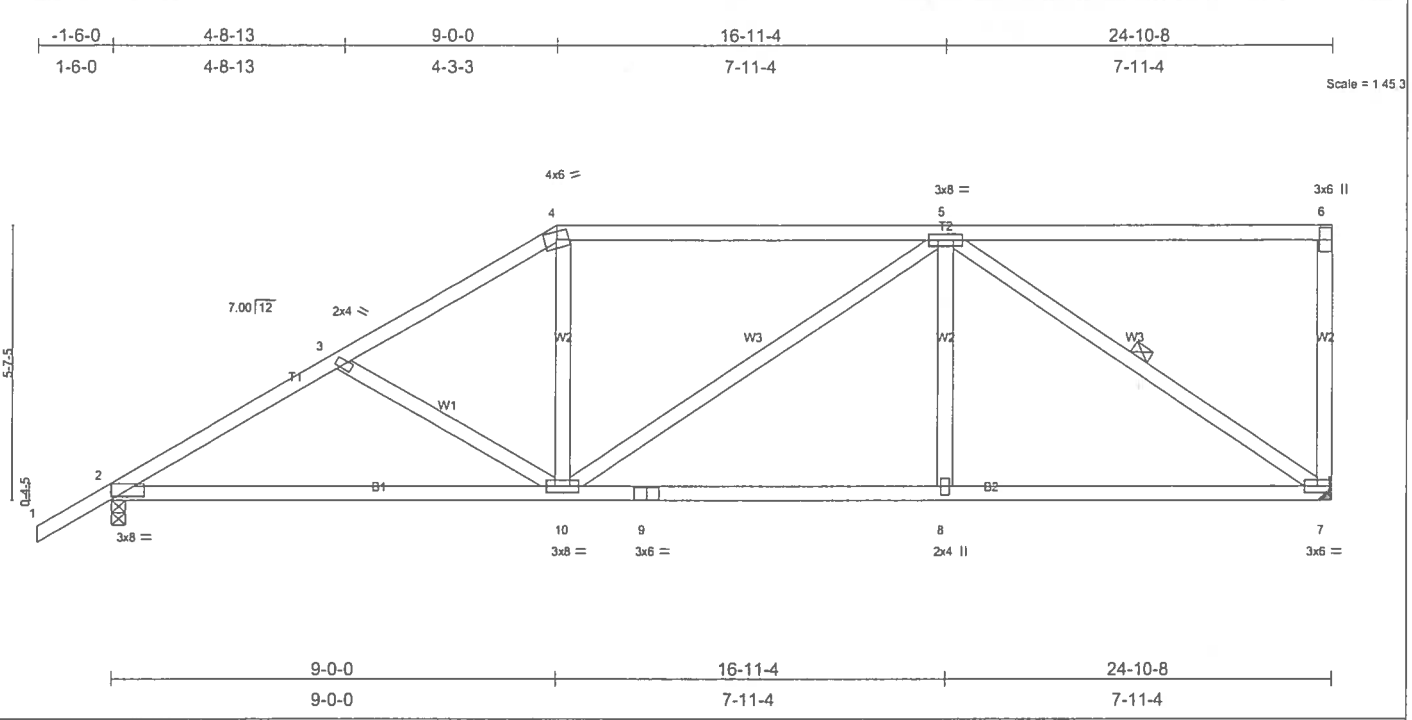


Plate Offsets (X,Y): [2:0-8-1,0-0-14]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.66 | Vert(LL) | -0.16 | 2-10 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.49 | Vert(TL) | -0.28 | 2-10 | >999 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.54 | Horz(TL) | 0.05 | 7 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 136 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 4-7-10 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-6-2 oc bracing.
WEBS 1 Row at midpt 5-7

**REACTIONS** (lb/size) 7=1030/Mechanical, 2=1124/0-3-8
Max Horz 2=299(load case 5)
Max Uplift7=-394(load case 3), 2=-389(load case 5)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-1600/448, 3-4=-1392/399, 4-5=-1170/382, 5-6=-58/25, 6-7=-192/157
BOT CHORD 2-10=-550/1334, 9-10=-400/1114, 8-9=-400/1114, 7-8=-400/1114
WEBS 3-10=-201/203, 4-10=-2/333, 5-10=-89/166, 5-8=0/227, 5-7=-1279/459

**JOINT STRESS INDEX**
2 = 0.73, 3 = 0.34, 4 = 0.74, 5 = 0.61, 6 = 0.51, 7 = 0.66, 8 = 0.34, 9 = 0.42 and 10 = 0.57

**NOTES**
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 394 lb uplift at joint 7 and 389 lb uplift at joint 2.

**LOAD CASE(S)** Standard

**JANUARY 03, 2007 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

Dwg.#0103071060

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | T14 | MONO HIP | 1 | 1 | Job Reference (optional) |

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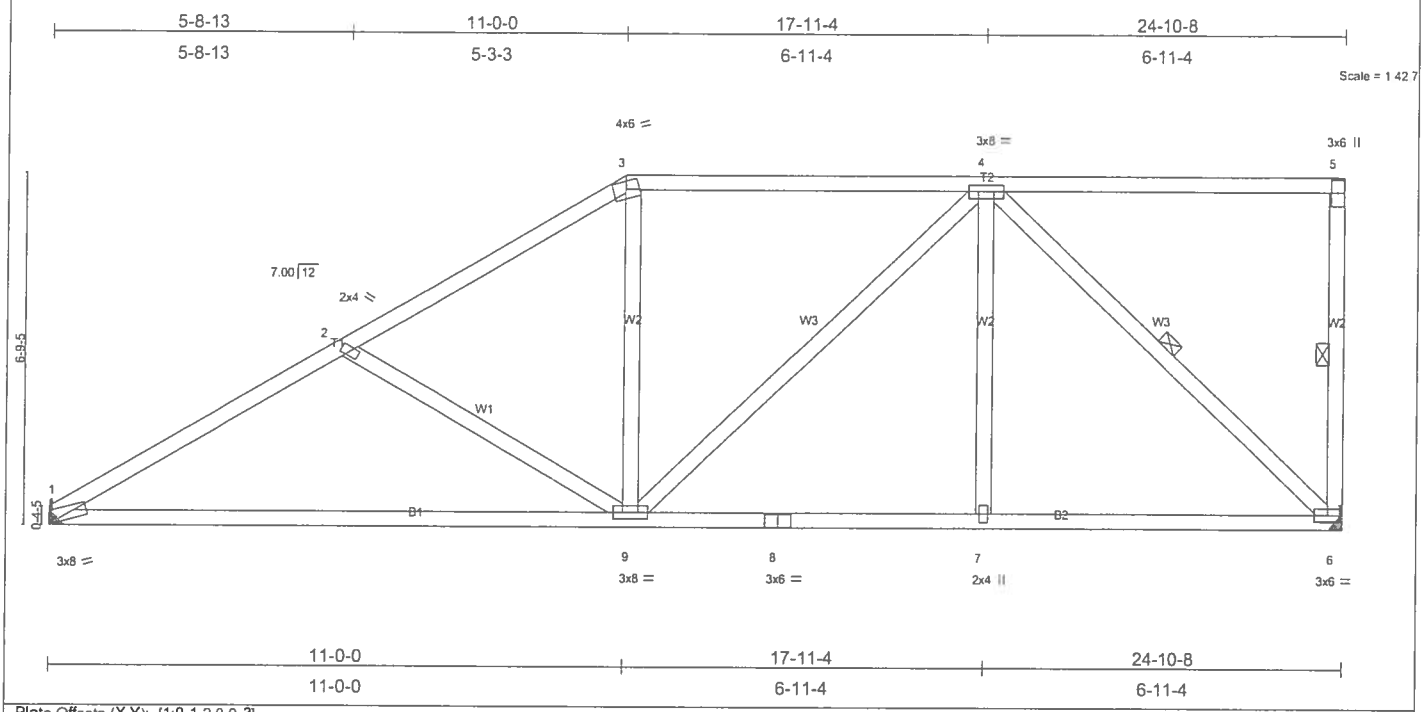


Plate Offsets (X,Y): [1:0-1-2,0-0-3]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.47 | Vert(LL) | -0.36 | 1-9 | >817 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.66 | Vert(TL) | -0.62 | 1-9 | >478 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.46 | Horz(TL) | 0.04 | 6 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 140 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 4-2-9 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-10-1 oc bracing.
WEBS 1 Row at midpt 5-6, 4-6

**REACTIONS** (lb/size) 1=1033/Mechanical, 6=1033/Mechanical
Max Horz 1=299(load case 5)
Max Uplift1=-297(load case 5), 6=-365(load case 4)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1559/472, 2-3=-1265/354, 3-4=-1033/366, 4-5=-34/13, 5-6=-165/133
BOT CHORD 1-9=-620/1308, 8-9=-292/832, 7-8=-292/832, 6-7=-292/832
WEBS 2-9=-329/294, 3-9=0/284, 4-9=-157/278, 4-7=0/170, 4-6=-1102/388

**JOINT STRESS INDEX**
1 = 0.84, 2 = 0.34, 3 = 0.59, 4 = 0.57, 5 = 0.35, 6 = 0.51, 7 = 0.34, 8 = 0.32 and 9 = 0.57

**NOTES**
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 297 lb uplift at joint 1 and 365 lb uplift at joint 6.

**LOAD CASE(S)** Standard

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0103071061

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | T15 | MONO HIP | 1 | 1 | Job Reference (optional) |

Builders FirstSource, Lake City, FI 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 03 09:48:18 2007 Page 1

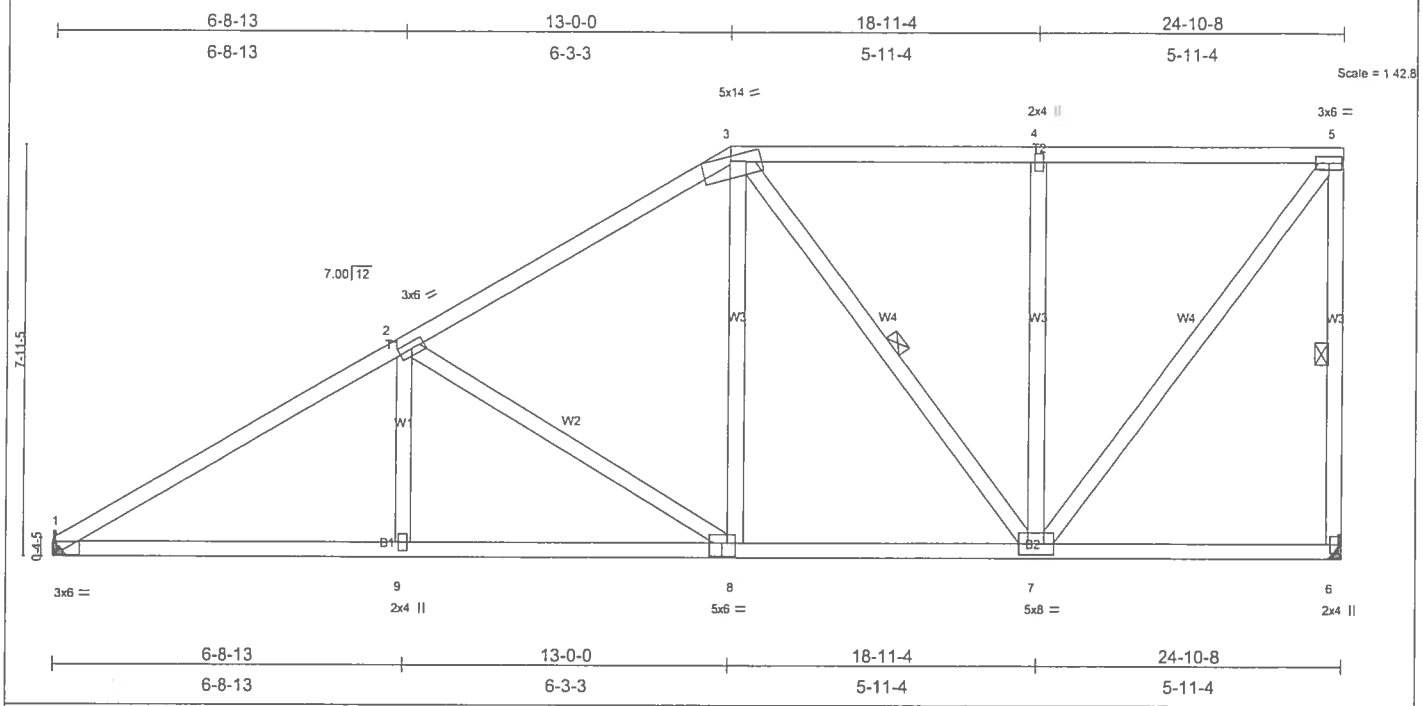


Plate Offsets (X,Y): [1:0-3-3,0-1-8], [8:0-2-12,0-3-0]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.37 | Vert(LL) | -0.10 | 1-9 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.49 | Vert(TL) | -0.17 | 1-9 | >999 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.57 | Horz(TL) | 0.04 | 6 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 154 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 4-6-8 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-7-10 oc bracing.
WEBS 1 Row at midpt 5-6, 3-7

**REACTIONS** (lb/size) 1=1033/Mechanical, 6=1033/Mechanical
Max Horz 1=353(load case 5)
Max Uplift1=-296(load case 5), 6=-354(load case 4)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1646/439, 2-3=-1078/317, 3-4=-631/214, 4-5=-630/214, 5-6=-949/365
BOT CHORD 1-9=-632/1342, 8-9=-632/1342, 7-8=-343/862, 6-7=-8/19
WEBS 2-9=0/233, 2-8=-576/343, 3-8=-140/477, 3-7=-377/220, 4-7=-332/313, 5-7=-344/1013

**JOINT STRESS INDEX**
1 = 0.69, 2 = 0.42, 3 = 0.92, 4 = 0.34, 5 = 0.74, 6 = 0.71, 7 = 0.50, 8 = 0.53 and 9 = 0.34

**NOTES**
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 296 lb uplift at joint 1 and 354 lb uplift at joint 6.

**LOAD CASE(S)** Standard

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ. FL 33549

Dwg.#0103071062

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | T16 | MONO HIP | 1 | 1 | Job Reference (optional) |

Builders FirstSource, Lake City, FI 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 03 09:48:19 2007 Page 1

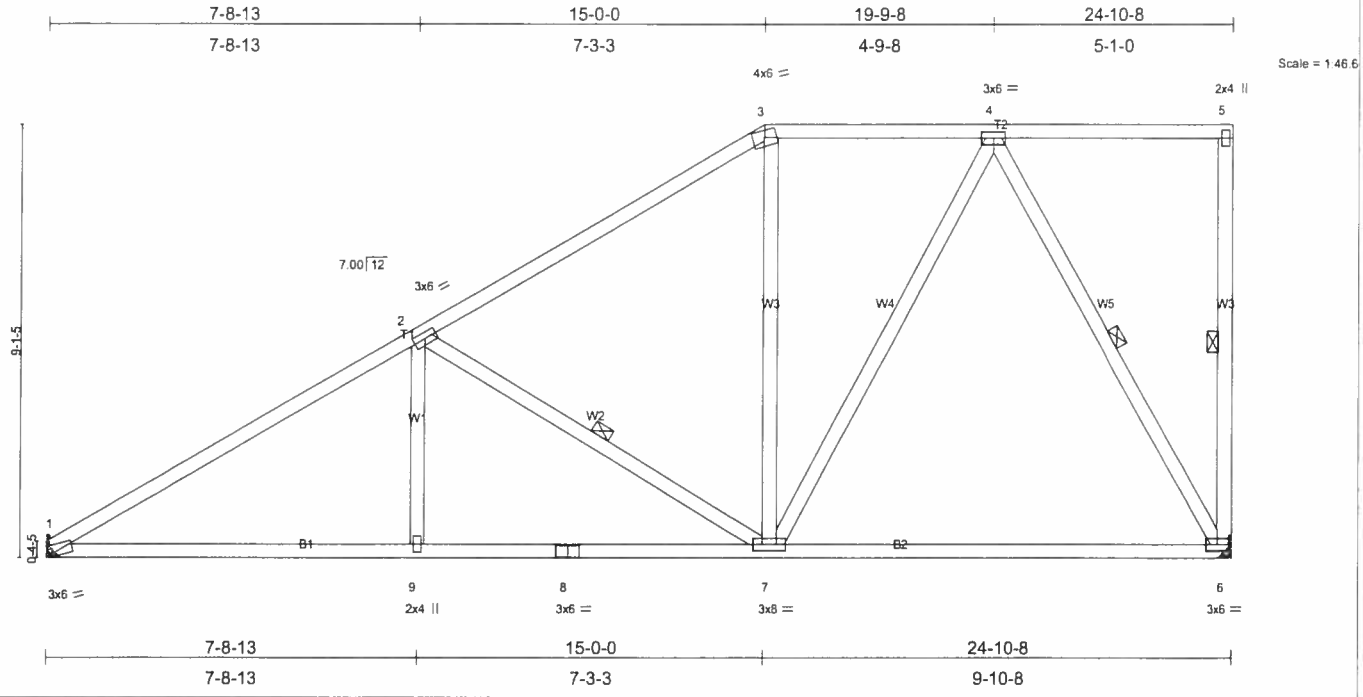


Plate Offsets (X,Y): [1:0-0-11,Edge]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.53 | Vert(LL) | -0.20 | 6-7 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.61 | Vert(TL) | -0.34 | 6-7 | >865 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.46 | Horz(TL) | 0.04 | 6 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 151 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 4-4-13 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-4-12 oc bracing.
WEBS 1 Row at midpt 5-6, 2-7, 4-6

**REACTIONS** (lb/size) 1=1033/Mechanical, 6=1033/Mechanical
Max Horz 1=406(load case 5)
Max Uplift1=-290(load case 5), 6=-353(load case 5)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1596/412, 2-3=-947/262, 3-4=-726/302, 4-5=-26/1, 5-6=-133/91
BOT CHORD 1-9=-649/1292, 8-9=-649/1292, 7-8=-649/1292, 6-7=-180/453
WEBS 2-9=0/249, 2-7=-666/406, 3-7=0/99, 4-7=-260/584, 4-6=-874/369

**JOINT STRESS INDEX**
1 = 0.87, 2 = 0.42, 3 = 0.70, 4 = 0.48, 5 = 0.91, 6 = 0.70, 7 = 0.65, 8 = 0.47 and 9 = 0.34

**NOTES**
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 290 lb uplift at joint 1 and 353 lb uplift at joint 6.

**LOAD CASE(S)** Standard

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549

Dwg.#0103071063

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | T17 | SPECIAL | 1 | 1 | Job Reference (optional) |

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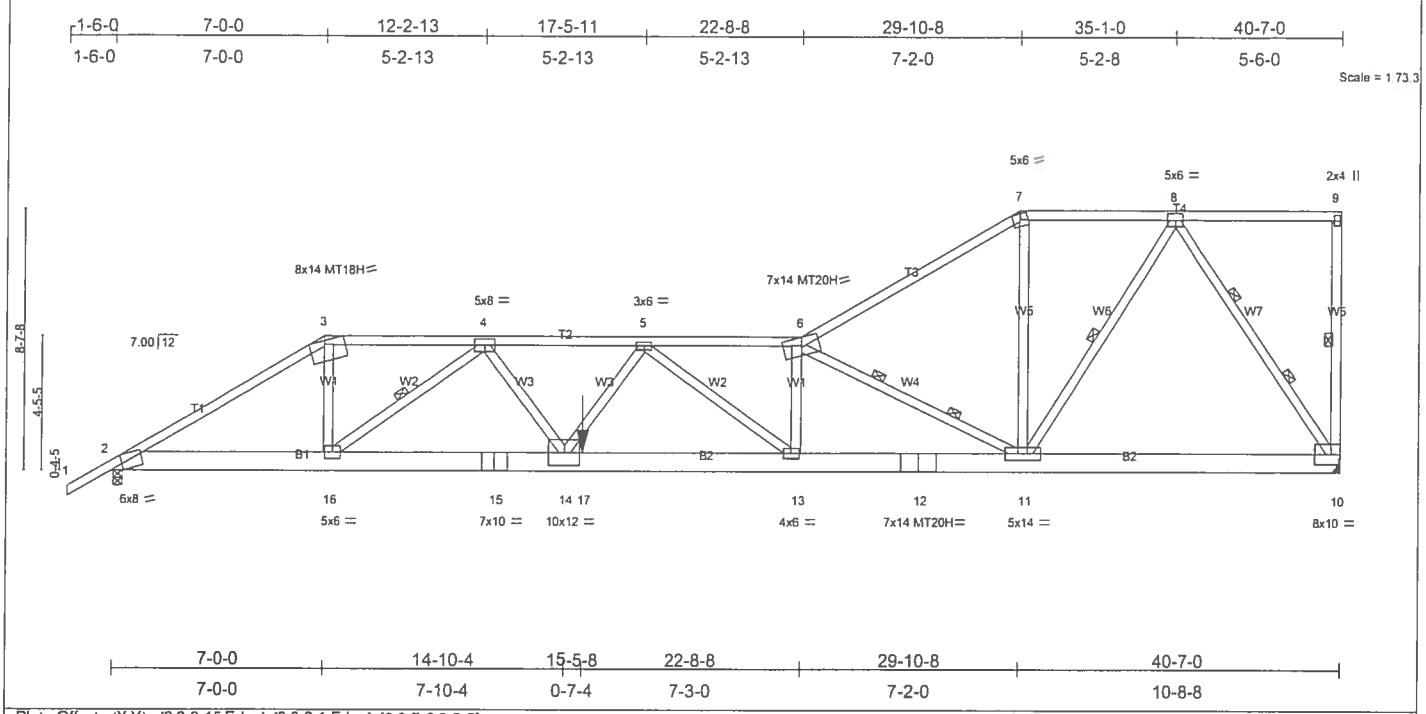



Plate Offsets (X,Y): [2:0-3-15,Edge], [3:0-6-1,Edge], [6:0-7-0,0-3-6]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.94 | Vert(LL) | -0.57 | 13-14 | >852 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.51 | Vert(TL) | -0.91 | 13-14 | >532 | 180 | MT20H | 187/143 |
| BCLL | 10.0 | Rep Stress Incr | NO | WB | 1.00 | Horz(TL) | 0.13 | 10 | n/a | n/a | MT18H | 244/190 |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 296 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3 *Except*
W4 2 X 4 SYP No.1D

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 1-7-12 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-10-2 oc bracing.
WEBS 1 Row at midpt 9-10, 4-16, 8-11
2 Rows at 1/3 pts 6-11, 8-10

**REACTIONS** (lb/size) 10=2321/Mechanical, 2=2812/0-3-8
Max Horz 2=441(load case 4)
Max Uplift10=-794(load case 2), 2=-1065(load case 4)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-4936/1720, 3-4=-4264/1585, 4-5=-7793/2925, 5-6=-7064/2440, 6-7=-2916/933, 7-8=-2457/894, 8-9=-19/7, 9-10=-140/102
BOT CHORD 2-16=-1755/4187, 15-16=-2643/6704, 14-15=-2643/6704, 14-17=-2929/7656, 13-17=-2929/7656, 12-13=-2625/7037, 11-12=-2625/7037, 10-11=-492/1356
WEBS 3-16=-746/2082, 4-16=-3102/1387, 4-14=-723/1987, 5-14=-141/250, 5-13=-754/622, 6-13=-390/728, 6-11=-5237/1976, 7-11=-383/1055, 8-11=-763/2089, 8-10=-2440/887

**JOINT STRESS INDEX**
2 = 0.92, 3 = 0.85, 4 = 0.82, 5 = 0.41, 6 = 0.77, 7 = 0.77, 8 = 0.82, 9 = 0.85, 10 = 0.37, 11 = 0.98, 12 = 0.65, 13 = 0.33, 14 = 0.31, 15 = 0.94 and 16 = 0.73

**NOTES**
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf, BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide adequate drainage to prevent water ponding.
3) All plates are MT20 plates unless otherwise indicated.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 794 lb uplift at joint 10 and 1065 lb uplift at joint 2.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1660 lb down and 788 lb up at 15-5-8 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

**LOAD CASE(S)** Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-6=-54, 6-7=-54, 7-9=-54, 2-10=-30
Concentrated Loads (lb)
Vert: 17=-1660(F)

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0103071064

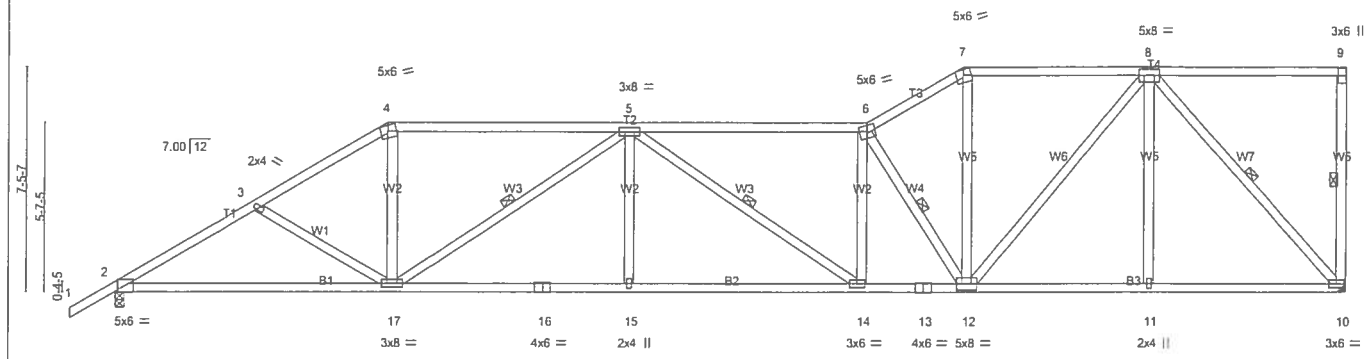
| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | T18 | SPECIAL | 1 | 1 | Job Reference (optional) |

Builders FirstSource, Lake City, FI 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 03 09:48:22 2007 Page 1

1-6-0 | 4-8-13 | 9-0-0 | 16-10-4 | 24-8-8 | 27-10-8 | 34-1-0 | 40-7-0

1-6-0 | 4-8-13 | 4-3-3 | 7-10-4 | 7-10-4 | 3-2-0 | 6-2-8 | 6-6-0

Scale = 1:73.3




9-0-0 | 16-10-4 | 24-8-8 | 27-10-8 | 34-1-0 | 40-7-0

9-0-0 | 7-10-4 | 7-10-4 | 3-2-0 | 6-2-8 | 6-6-0

Plate Offsets (X,Y): [2:0-1-3,Edge]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.55 | Vert(LL) | -0.35 | 14-15 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.79 | Vert(TL) | -0.57 | 14-15 | >845 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.84 | Horz(TL) | 0.16 | 10 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 250 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 3-0-9 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-8-8 oc bracing.
WEBS 1 Row at midpt 9-10, 5-17, 5-14, 6-12, 8-10

**REACTIONS** (lb/size) 10=1690/Mechanical, 2=1783/0-3-8
Max Horz 2=384(load case 5)
Max Uplift10=-624(load case 3), 2=-677(load case 5)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-2854/992, 3-4=-2664/930, 4-5=-2287/871, 5-6=-3105/1028, 6-7=-2399/787, 7-8=-2047/711, 8-9=-25/11, 9-10=-155/125
BOT CHORD 2-17=-1097/2401, 16-17=-1227/3188, 15-16=-1227/3188, 14-15=-1227/3188, 13-14=-1112/3098, 12-13=-1112/3098, 11-12=-463/1306, 10-11=-463/1306
WEBS 3-17=-165/172, 4-17=-223/973, 5-17=-1096/543, 5-15=0/230, 5-14=-102/139, 6-14=-40/232, 6-12=-2022/769, 7-12=-303/961, 8-12=-453/1147, 8-11=0/200, 8-10=-1930/682

**JOINT STRESS INDEX**
2 = 0.73, 3 = 0.34, 4 = 0.72, 5 = 0.57, 6 = 0.68, 7 = 0.53, 8 = 0.56, 9 = 0.30, 10 = 0.65, 11 = 0.34, 12 = 0.56, 13 = 0.90, 14 = 0.35, 15 = 0.34, 16 = 0.98 and 17 = 0.57

**NOTES**
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 624 lb uplift at joint 10 and 677 lb uplift at joint 2.

**LOAD CASE(S)** Standard

**JANUARY 03, 2007 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

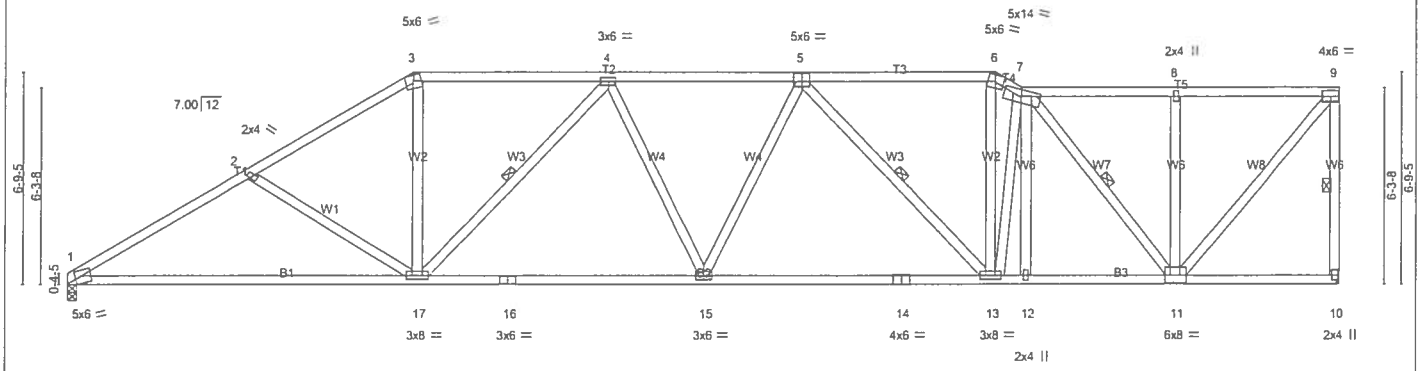
Dwg.#0103071065

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | T19 | SPECIAL | 1 | 1 | Job Reference (optional) |

Builders FirstSource, Lake City, FI 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 03 09:48:23 2007 Page 1

| 5-8-13 | 11-0-0 | 17-2-5 | 23-4-11 | 29-7-0 | 30-5-0 | 35-4-4 | 40-7-0 |
|---|---|---|---|---|---|---|---|
| 5-8-13 | 5-3-3 | 6-2-5 | 6-2-5 | 6-2-5 | 0-10-0 | 4-11-4 | 5-2-12 |

Scale = 1:70.8




| 11-0-0 | 20-3-8 | 29-7-0 | 30-5-0 | 35-4-4 | 40-7-0 |
|---|---|---|---|---|---|
| 11-0-0 | 9-3-8 | 9-3-8 | 0-10-0 | 4-11-4 | 5-2-12 |

Plate Offsets (X,Y): [1:0-3-6,Edge], [5:0-3-0,0-3-0]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.59 | Vert(LL) | -0.43 | 1-17 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.84 | Vert(TL) | -0.72 | 1-17 | >674 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.91 | Horz(TL) | 0.14 | 10 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 253 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 2-11-8 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-11-0 oc bracing.
WEBS 1 Row at midpt 9-10, 4-17, 5-13, 7-11

**REACTIONS** (lb/size) 1=1692/0-3-8, 10=1692/Mechanical
Max Horz 1=283(load case 5)
Max Uplift1=-554(load case 4), 10=-749(load case 4)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2824/1024, 2-3=-2547/980, 3-4=-2155/895, 4-5=-2642/1133, 5-6=-2145/915, 6-7=-2359/991, 7-8=-1287/564, 8-9=-1287/564, 9-10=-1616/756
BOT CHORD 1-17=-1013/2386, 16-17=-1145/2584, 15-16=-1145/2584, 14-15=-1150/2576, 13-14=-1150/2576, 12-13=-922/2132, 11-12=-922/2130, 10-11=-10/22
WEBS 2-17=-301/287, 3-17=-286/949, 4-17=-724/449, 4-15=-29/199, 5-15=-14/218, 5-13=-717/341, 6-13=-263/819, 7-13=-48/195, 7-12=-84/0, 7-11=-1328/563, 8-11=-308/260, 9-11=-857/1956

**JOINT STRESS INDEX**
1 = 0.80, 2 = 0.34, 3 = 0.55, 4 = 0.46, 5 = 0.52, 6 = 0.64, 7 = 0.61, 8 = 0.34, 9 = 0.92, 10 = 0.79, 11 = 0.73, 12 = 0.34, 13 = 0.59, 14 = 0.87, 15 = 0.46, 16 = 0.86 and 17 = 0.57

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 554 lb uplift at joint 1 and 749 lb uplift at joint 10.

**LOAD CASE(S)** Standard

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549

Dwg.#0103071066

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | T20 | SPECIAL | 1 | 1 | Job Reference (optional) |

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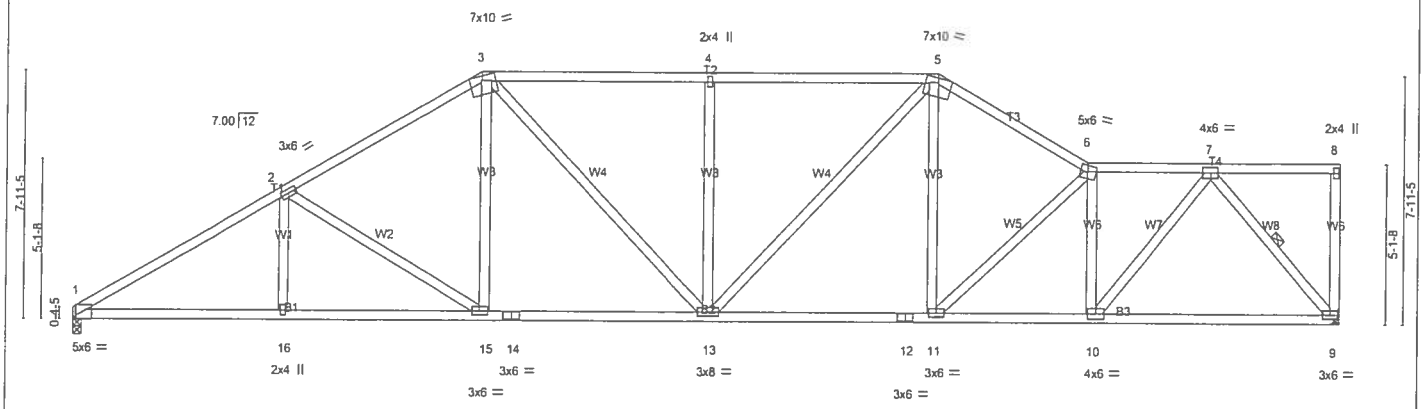
| 6-8-13 | 13-0-0 | 20-3-8 | 27-7-0 | 32-5-0 | 36-4-4 | 40-7-0 |
|---|---|---|---|---|---|---|
| 6-8-13 | 6-3-3 | 7-3-8 | 7-3-8 | 4-10-0 | 3-11-4 | 4-2-12 |

Scale = 1:71.0




| 6-8-13 | 13-0-0 | 20-3-8 | 27-7-0 | 32-5-0 | 40-7-0 |
|---|---|---|---|---|---|
| 6-8-13 | 6-3-3 | 7-3-8 | 7-3-8 | 4-10-0 | 8-2-0 |

Plate Offsets (X,Y): [1:0-1-3,Edge]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.47 | Vert(LL) | -0.23 | 11-13 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.68 | Vert(TL) | -0.37 | 11-13 | >999 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.66 | Horz(TL) | 0.13 | 9 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 245 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 3-3-5 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-6-13 oc bracing.
WEBS 1 Row at midpt 7-9

**REACTIONS** (lb/size) 1=1692/0-3-8, 9=1692/Mechanical
Max Horz 1=262(load case 5)
Max Uplift1=-489(load case 5), 9=-571(load case 4)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2915/812, 2-3=-2370/775, 3-4=-2279/820, 4-5=-2279/820, 5-6=-2328/719, 6-7=-2252/658, 7-8=-42/2, 8-9=-110/75
BOT CHORD 1-16=-871/2427, 15-16=-871/2427, 14-15=-729/1981, 13-14=-729/1981, 12-13=-631/1977, 11-12=-631/1977, 10-11=-669/2284, 9-10=-438/1278
WEBS 2-16=0/225, 2-15=-539/333, 3-15=-131/480, 3-13=-349/559, 4-13=-415/369, 5-13=-291/560, 5-11=-64/479, 6-11=-443/138, 6-10=-1043/378, 7-10=-424/1578, 7-9=-1914/676

**JOINT STRESS INDEX**
1 = 0.73, 2 = 0.42, 3 = 0.69, 4 = 0.34, 5 = 0.67, 6 = 0.57, 7 = 0.75, 8 = 0.77, 9 = 0.72, 10 = 0.75, 11 = 0.36, 12 = 0.66, 13 = 0.57, 14 = 0.71, 15 = 0.35 and 16 = 0.34

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 489 lb uplift at joint 1 and 571 lb uplift at joint 9.

**LOAD CASE(S)** Standard

**JANUARY 03, 2007 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549**

Dwg.#0103071067

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | T21 | SPECIAL | 1 | 1 | Job Reference (optional) |

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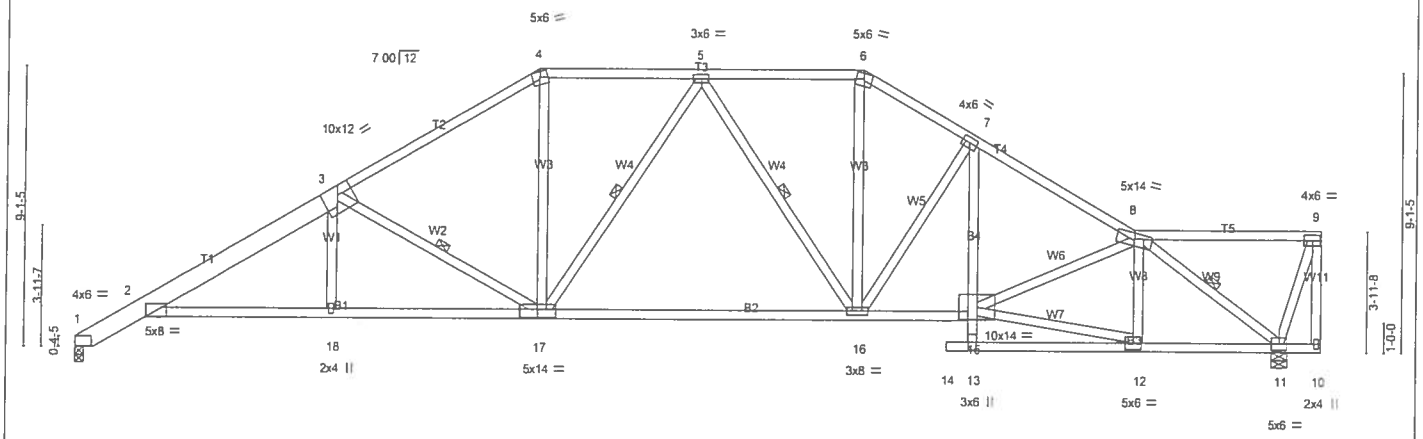
| 2-3-8 | 8-4-0 | 15-0-0 | 20-3-8 | 25-7-0 | 28-3-8 | 34-5-0 | 40-7-0 |
|---|---|---|---|---|---|---|---|
| 2-3-8 | 6-0-8 | 6-8-0 | 5-3-8 | 5-3-8 | 2-8-8 | 6-1-8 | 6-2-0 |

Scale = 1 72.2




| 2-3-8 | 8-4-0 | 15-0-0 | 20-3-8 | 25-7-0 | 28-3-8 | 29-0-0 | 34-5-0 | 39-2-12 | 40-7-0 |
|---|---|---|---|---|---|---|---|---|---|
| 2-3-8 | 6-0-8 | 6-8-0 | 5-3-8 | 5-3-8 | 2-8-8 | 0-8-8 | 5-5-0 | 4-9-12 | 1-4-4 |

Plate Offsets (X,Y): [3:7-11-6,4-7-6], [17:0-7-0,0-3-0]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.88 | Vert(LL) | 0.85 | 16-17 | >552 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.85 | Vert(TL) | -0.87 | 16-17 | >540 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.88 | Horz(TL) | 0.41 | 11 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 265 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2 *Except*
T1 2 X 8 SYP 2400F 2.0E
BOT CHORD 2 X 4 SYP No.2 *Except*
B1 2 X 4 SYP No.1D, B4 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3 *Except*
W11 2 X 4 SYP No.2

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins, except end verticals
BOT CHORD Rigid ceiling directly applied or 3-10-10 oc bracing.
WEBS 1 Row at midpt 3-17, 5-17, 5-16, 8-11

**REACTIONS** (lb/size) 11=1762/0-6-7, 1=1607/0-3-8
Max Horz 1=368(load case 4)
Max Uplift11=-976(load case 4), 1=-851(load case 4)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-700/278, 2-3=-3273/2465, 3-4=-2436/1904, 4-5=-1988/1674, 5-6=-1936/1620, 6-7=-2266/1820, 7-8=-2655/2002, 8-9=-9/10, 9-10=-31/48
BOT CHORD 2-18=-2442/2950, 17-18=-2432/2941, 16-17=-1643/2053, 15-16=-1788/2225, 13-15=-102/125, 7-15=-345/430, 13-14=0/0, 12-13=-160/223, 11-12=-1492/1955, 10-11=-44/44
WEBS 3-18=-264/256, 3-17=-1104/940, 4-17=-819/915, 5-17=-273/307, 5-16=-332/305, 6-16=-772/867, 7-16=-560/445, 12-15=-1368/1782, 8-15=-184/330, 8-12=-255/144, 8-11=-2458/1860, 9-11=-153/145

**JOINT STRESS INDEX**
2 = 0.78, 3 = 0.52, 3 = 0.00, 4 = 0.54, 5 = 0.43, 6 = 0.48, 7 = 0.37, 8 = 0.68, 9 = 0.61, 10 = 0.36, 11 = 0.56, 12 = 0.59, 13 = 0.45, 15 = 0.96, 16 = 0.59, 17 = 0.64 and 18 = 0.34

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 976 lb uplift at joint 11 and 851 lb uplift at joint 1.

**LOAD CASE(S)** Standard

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549

Dwg.#0103071068

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | T22 | SPECIAL | 1 | 2 | Job Reference (optional) |

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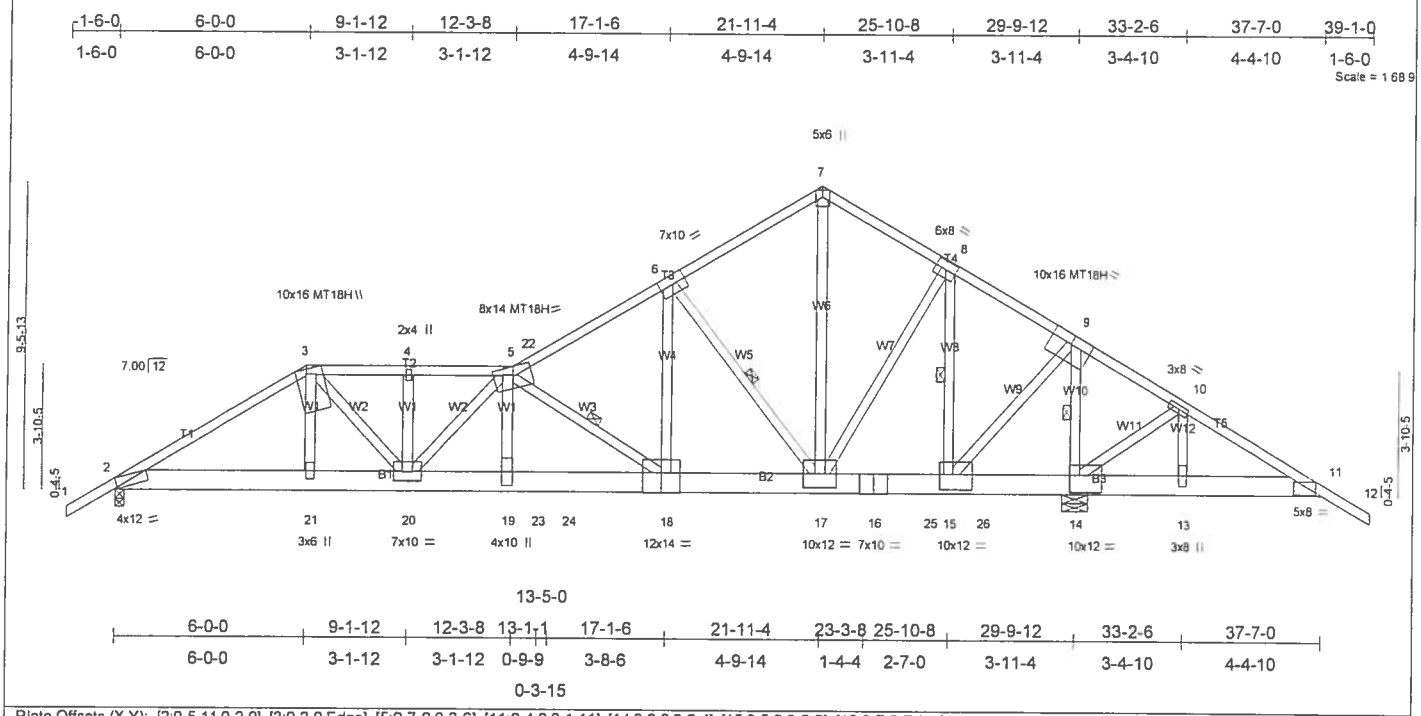


Plate Offsets (X,Y): [2:0-5-11,0-2-0], [3:0-2-0,Edge], [5:0-7-0,0-3-6], [11:0-4-0,0-1-11], [14:0-3-8,0-6-4], [15:0-5-0,0-5-8], [18:0-7-0,Edge]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.85 | Vert(LL) | -0.46 | 18-19 | >768 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.84 | Vert(TL) | -0.74 | 18-19 | >480 | 180 | MT18H | 244/190 |
| BCLL | 10.0 | Rep Stress Incr | NO | WB | 0.98 | Horz(TL) | 0.08 | 14 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 587 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3 *Except*
W4 2 X 4 SYP No.2, W9 2 X 4 SYP No.1D, W10 2 X 4 SYP No.2

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 3-1-1 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 5-18, 6-17, 8-15, 9-14

**REACTIONS** (lb/size) 2=5919/0-3-8, 14=21833/1-0-14 (input: 0-9-11)
Max Horz 2=349(load case 3)
Max Uplift2=-2912(load case 4), 14=-10256(load case 5)
Max Grav 2=5935(load case 8), 14=21833(load case 1)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-11140/5378, 3-4=-13762/6703, 4-5=-13762/6702, 5-22=-11772/5556, 6-22=-11728/5573, 6-7=-6196/2931, 7-8=-6118/2946, 8-9=-3311/1555, 9-10=-3688/7450, 10-11=-2220/4354, 11-12=0/45
BOT CHORD 2-21=-4691/9564, 20-21=-4702/9592, 19-20=-8701/18069, 19-23=-8621/17901, 23-24=-8621/17901, 18-24=-8621/17901, 17-18=-4699/10141, 16-17=-1122/2824, 16-25=-1122/2824, 15-25=-1122/2824, 15-26=-6147/3285, 14-26=-6147/3285, 13-14=-3728/2042, 11-13=-3728/2042
WEBS 3-21=-259/648, 3-20=-3076/6140, 4-20=-220/361, 5-20=-6342/3024, 5-19=-2055/3923, 5-18=-9494/4799, 6-18=-4251/8742, 6-17=-7886/3949, 7-17=-2816/5951, 8-17=-2299/5019, 8-15=-5916/2828, 9-15=-6200/13526, 9-14=-15783/7336, 10-14=-3297/1680, 10-13=-1631/3308

**JOINT STRESS INDEX**
2 = 0.81, 3 = 0.68, 4 = 0.34, 5 = 0.97, 6 = 0.80, 7 = 0.64, 8 = 0.84, 9 = 0.81, 10 = 0.98, 11 = 0.51, 13 = 0.54, 14 = 0.39, 15 = 0.99, 16 = 0.70, 17 = 0.63, 18 = 0.99, 19 = 0.48, 20 = 0.76 and 21 = 0.16

**NOTES**
1) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 2060 lb down and 977 lb up at 13-5-0, 2321 lb down and 1101 lb up at 23-3-8, and 1543 lb down and 1071 lb up at 13-1-1, and 378 lb down and 262 lb up at 6-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
2) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-3-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 2060 lb down and 977 lb up at 13-5-0, 2321 lb down and 1101 lb up at 23-3-8, and 1543 lb down and 1071 lb up at 13-1-1, and 378 lb down and 262 lb up at 6-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
3) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
4) Unbalanced roof live loads have been considered for this design.
5) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever right exposed ; Lumber DOL=1.60 plate grip DOL=1.60.
6) Provide adequate drainage to prevent water ponding.
7) All plates are MT20 plates unless otherwise indicated.
8) WARNING: Required bearing size at joint(s) 14 greater than input bearing size.
9) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2912 lb uplift at joint 2 and 10256 lb uplift at joint 14
11) Girder carries tie-in span(s): 24-10-8 from 13-5-0 to 23-3-8; 40-7-0 from 25-3-8 to 37-7-0; 40-7-0 from 23-3-8 to 26-3-8, 10-0-0 from 33-0-0 to 37-7-0
12) Girder carries hip end with 24-5-15 right side setback, 6-0-0 left side setback, and 6-0-0 end setback
13) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 2060 lb down and 977 lb up at 13-5-0, 2321 lb down and 1101 lb up at 23-3-8, and 1543 lb down and 1071 lb up at 13-1-1, and 378 lb down and 262 lb up at 6-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

Continued on page 2

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0103071069

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | T22 | SPECIAL | 1 | 2 | Job Reference (optional) |

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**LOAD CASE(S)** Standard
1) Regular: Lumber Increase=1 25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-5=-103(F=-50), 5-22=-104(F=-49), 7-22=-54, 7-12=-54, 2-21=-30, 21-23=-58(F=-28), 23-24=-30, 16-24=-504(F=-474), 16-25=-834(F=-804), 25-26=-1638(F=-1608), 13-26=-834(F=-804), 11-13=-996(F=-966)
Concentrated Loads (lb)
Vert: 21=-378(F) 16=-2321(F) 23=-1543(F) 24=-2060(F)

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549

Dwg.#0103071070

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | T23 | SPECIAL | 1 | 3 | Job Reference (optional) |

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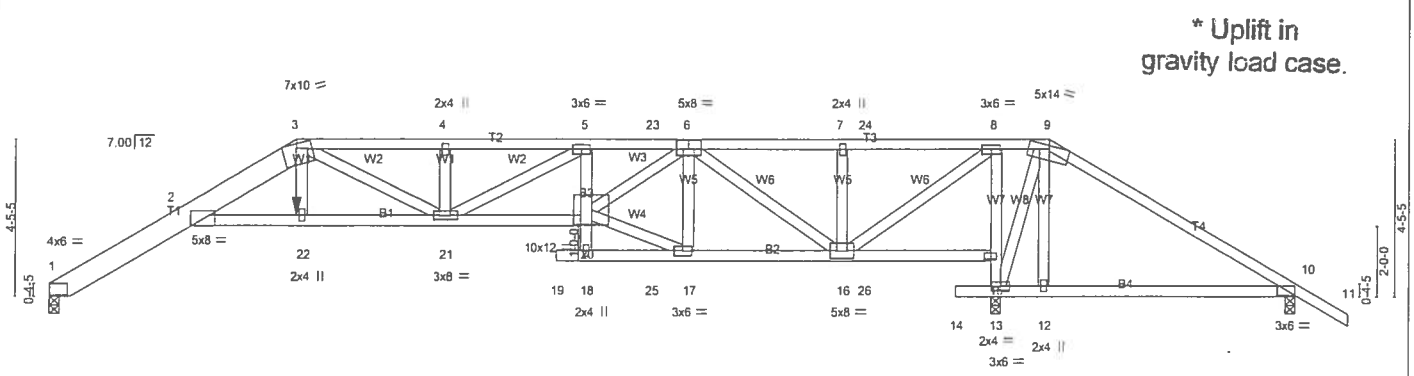
1-6-0 2-3-8 4-3-8 7-0-0 11-1-12 15-0-0 18-0-11 22-4-11 26-8-12 28-3-0 35-3-0 36-9-0
1-6-0 2-3-8 2-0-0 2-8-8 4-1-12 3-10-4 3-0-11 4-4-0 4-4-1 1-6-4 7-0-0 1-6-0

Scale = 1:62.7




28-3-0

2-3-8 4-3-8 7-0-0 11-1-12 14-3-8 15-0-0 18-0-11 22-4-11 25-7-0 26-8-12 35-3-0
2-3-8 2-0-0 2-8-8 4-1-12 3-1-12 0-8-8 3-0-11 4-4-0 3-2-5 1-1-12 1-6-4 7-0-0

Plate Offsets (X,Y): [2:0-7-12,0-0-0], [3:6-5-12,3-9-8], [6:0-4-0,0-3-0], [10:0-3-4,0-1-8]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.76 | Vert(LL) | -0.36 | 2-22 | >877 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.72 | Vert(TL) | -0.59 | 2-22 | >543 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | NO | WB | 0.98 | Horz(TL) | 0.27 | 13 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 578 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2 *Except*
T1 2 X 8 SYP 2400F 2.0E
BOT CHORD 2 X 4 SYP No.1D *Except*
B3 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3 *Except*
W7 2 X 4 SYP No.2, W7 2 X 4 SYP No.2

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

**REACTIONS** (lb/size) 1=2066/0-3-8, 10=-284/0-3-8, 13=3563/0-3-8
Max Horz 1=256(load case 3)
Max Uplift1=-838(load case 3), 10=-518(load case 8), 13=-1499(load case 3)
Max Grav 1=2066(load case 1), 10=396(load case 3), 13=3563(load case 1)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-956/377, 2-3=-6415/2956, 3-4=-6887/3220, 4-5=-6878/3221, 5-23=-6817/3205, 6-23=-6817/3205, 6-7=-1256/737, 7-24=-1256/737, 8-24=-1256/737, 8-9=-660/1397, 9-10=-716/1321, 10-11=0/40
BOT CHORD 2-22=-3118/6348, 21-22=-3155/6437, 20-21=-3397/7048, 18-20=-67/228, 5-20=-191/166, 18-19=0/0, 18-25=-161/343, 17-25=-161/343, 16-17=-1642/3498, 16-26=-1839/867, 15-26=-1839/867, 13-14=0/0, 12-13=-1096/615, 10-12=-1104/613
WEBS 3-22=-526/1262, 3-21=-433/630, 4-21=-169/178, 5-21=-255/195, 17-20=-1603/3414, 6-20=-1883/3995, 6-17=-1065/573, 6-16=-2815/1332, 7-16=-295/239, 8-16=-1769/3785, 9-12=0/260, 13-15=-2683/1368, 8-15=-2636/1381, 9-13=-1104/355

**JOINT STRESS INDEX**
2 = 0.73, 3 = 0.55, 3 = 0.00, 4 = 0.34, 5 = 0.35, 6 = 0.77, 7 = 0.34, 8 = 0.74, 9 = 0.53, 10 = 0.25, 12 = 0.34, 13 = 0.51, 15 = 0.45, 16 = 0.59, 17 = 0.65, 18 = 0.54, 20 = 0.72, 21 = 0.57 and 22 = 0.49

**NOTES**
1) 3-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 8 - 2 rows at 0-9-0 oc, 2 X 4 - 1 row at 0-9-0 oc
Bottom chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi.
7) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 838 lb uplift at joint 1, 518 lb uplift at joint 10 and 1499 lb uplift at joint 13.
9) Girder carries tie-in span(s): 4-6-1 from 7-0-0 to 17-0-0; 4-11-4 from 7-0-0 to 17-0-0; 4-9-8 from 17-0-0 to 23-0-0; 4-5-4 from 17-0-0 to 23-0-0
10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 687 lb down and 326 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

**LOAD CASE(S)** Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Continued on page 2

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ. FL 33549

Dwg.#0103071071

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | T23 | SPECIAL | 1 | 3 | Job Reference (optional) |



**LOAD CASE(S)** Standard

Uniform Loads (plf)

Vert: 1-2=-65, 2-3=-54, 3-23=-101(F=-47), 23-24=-107(F=-53), 9-24=-54, 9-11=-54, 2-22=-30, 20-22=-86(F=-56), 18-19=-86(F=-56), 18-25=-86(F=-56), 25-26=-75(F=-45), 15-26=-30, 10-14=-30

Concentrated Loads (lb)

Vert: 22=-687(F)

**JANUARY 03, 2007 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549**

Dwg.#0103071072

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | T23A | MONO HIP | 1 | 1 | Job Reference (optional) |

Builders FirstSource, Lake City, FL 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 03 09:48:30 2007 Page 1

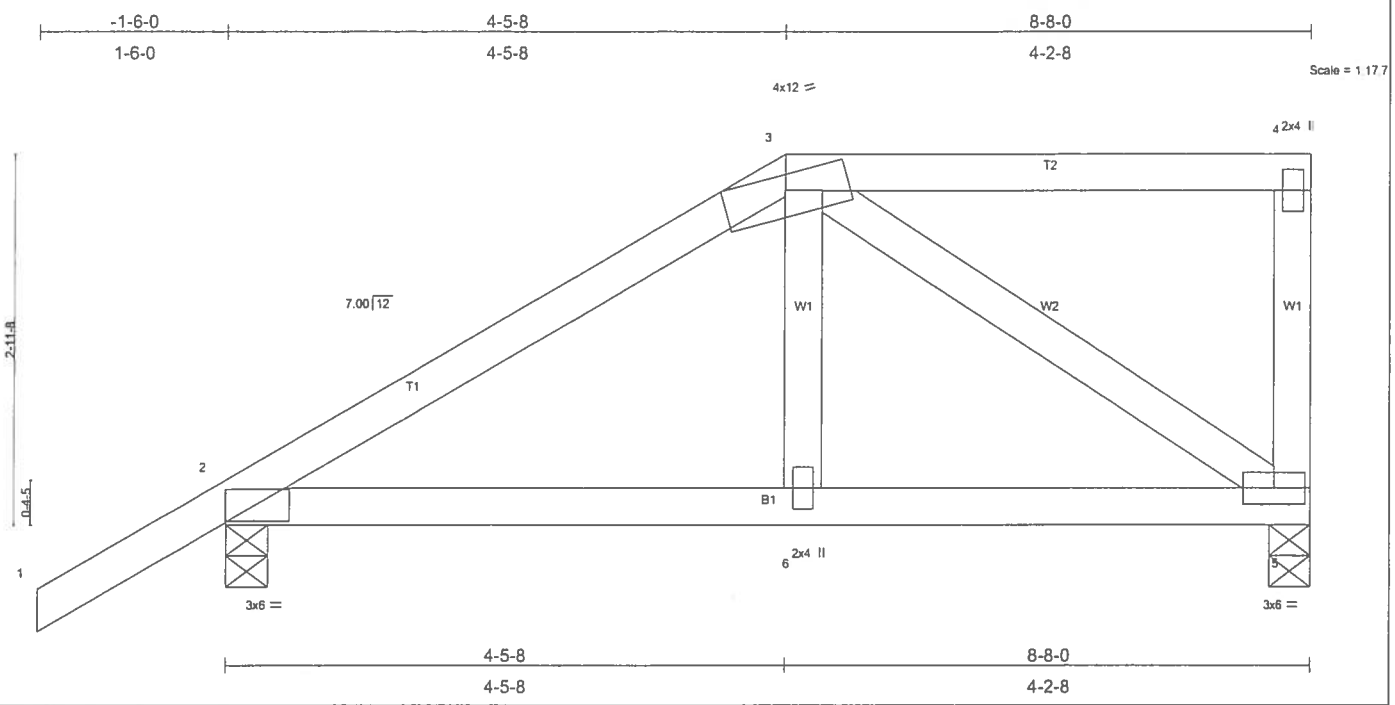



Plate Offsets (X,Y): [2:0-3-3,0-1-8]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.18 | Vert(LL) | -0.01 | 2-6 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.11 | Vert(TL) | -0.02 | 2-6 | >999 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.13 | Horz(TL) | 0.00 | 5 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 43 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 5=342/0-4-0, 2=450/0-4-0
Max Horz 2=177(load case 5)
Max Uplift5=-119(load case 4), 2=-205(load case 5)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-419/82, 3-4=-31/23, 4-5=-100/102
BOT CHORD 2-6=-111/308, 5-6=-109/316
WEBS 3-6=0/134, 3-5=-341/125

**JOINT STRESS INDEX**
2 = 0.28, 3 = 0.39, 4 = 0.51, 5 = 0.17 and 6 = 0.10

**NOTES**
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 119 lb uplift at joint 5 and 205 lb uplift at joint 2.

**LOAD CASE(S)** Standard

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0103071073

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | T23B | MONO HIP | 1 | 1 | Job Reference (optional) |

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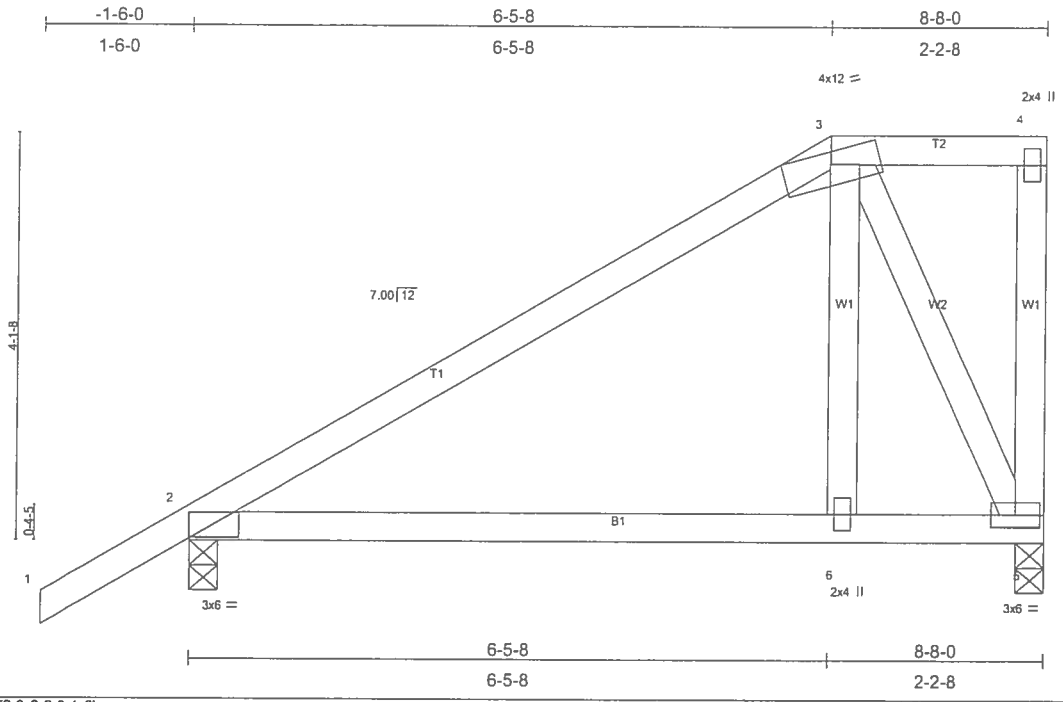



Plate Offsets (X,Y): [2:0-3-3,0-1-8]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.33 | Vert(LL) | -0.06 | 2-6 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.27 | Vert(TL) | -0.10 | 2-6 | >999 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.16 | Horz(TL) | 0.00 | 5 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 46 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 5=343/0-3-8, 2=449/0-3-8
Max Horz 2=230(load case 5)
Max Uplift5=-136(load case 5), 2=-193(load case 5)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-330/32, 3-4=-6/9, 4-5=-58/77
BOT CHORD 2-6=-103/213, 5-6=-103/221
WEBS 3-6=0/218, 3-5=-485/230

**JOINT STRESS INDEX**
2 = 0.38, 3 = 0.80, 4 = 0.26, 5 = 0.21 and 6 = 0.16

**NOTES**
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 136 lb uplift at joint 5 and 193 lb uplift at joint 2.

**LOAD CASE(S)** Standard

**JANUARY 03, 2007 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B. LUTZ. FL 33549**

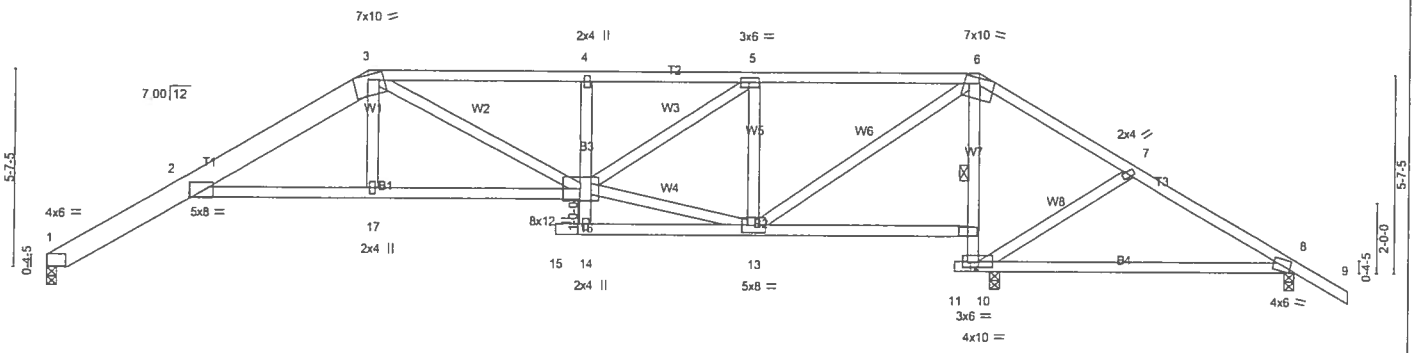
Dwg.#0103071074

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | T24 | SPECIAL | 1 | 1 | Job Reference (optional) |

Builders FirstSource, Lake City, FI 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 03 09:48:33 2007 Page 1

| 1-6-0 | 2-3-8 | 4-3-8 | 9-0-0 | 15-0-0 | 19-11-4 | 25-7-0 | 26-3-0 | 30-6-3 | 35-3-0 | 36-9-0 |
|---|---|---|---|---|---|---|---|---|---|---|
| 1-6-0 | 2-3-8 | 2-0-0 | 4-8-8 | 6-0-0 | 4-11-4 | 5-7-12 | 0-8-0 | 4-3-3 | 4-8-13 | 1-6-0 |

Scale = 1:62.7




| 2-3-8 | 4-3-8 | 9-0-0 | 14-3-8 | 15-0-0 | 19-11-4 | 25-7-0 | 26-8-12 | 35-3-0 |
|---|---|---|---|---|---|---|---|---|
| 2-3-8 | 2-0-0 | 4-8-8 | 5-3-8 | 0-8-8 | 4-11-4 | 5-7-12 | 1-1-12 | 8-6-4 |

Plate Offsets (X,Y): [2:0-7-4,0-0-0], [3:8-5-10,4-11-5], [8:0-1-10,Edge]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.75 | Vert(LL) | 0.42 | 8-10 | >251 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.81 | Vert(TL) | 0.33 | 8-10 | >314 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.93 | Horz(TL) | 0.30 | 10 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 191 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2 *Except* T1 2 X 8 SYP 2400F 2.0E
BOT CHORD 2 X 4 SYP No.2 *Except* B3 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3 *Except* W7 2 X 4 SYP No.2

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 4-9-13 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-7-15 oc bracing.
WEBS 1 Row at midpt 6-10

**REACTIONS** (lb/size) 1=850/0-3-8, 8=-127/0-3-8, 10=2271/0-3-8
Max Horz 1=-198(load case 3)
Max Uplift1=-287(load case 5), 8=-355(load case 9), 10=-851(load case 4)
Max Grav 1=856(load case 9), 8=167(load case 4), 10=2271(load case 1)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-389/247, 2-3=-1686/645, 3-4=-1351/611, 4-5=-1319/595, 5-6=-467/427, 6-7=-459/1170, 7-8=-501/1027, 8-9=0/40
BOT CHORD 2-17=-702/1617, 16-17=-702/1629, 14-16=0/85, 4-16=-240/226, 14-15=0/0, 13-14=-62/56, 12-13=-1107/439, 10-11=0/0, 8-10=-842/416
WEBS 3-17=-2/265, 3-16=-327/151, 13-16=-193/424, 5-16=-450/1075, 5-13=-922/531, 6-13=-804/1837, 7-10=-278/259, 10-12=-2167/869, 6-12=-1914/813

**JOINT STRESS INDEX**
2 = 0.74, 3 = 0.43, 3 = 0.00, 4 = 0.44, 5 = 0.63, 6 = 0.94, 7 = 0.34, 8 = 0.86, 10 = 0.49, 12 = 0.18, 13 = 0.85, 14 = 0.39, 16 = 0.49 and 17 = 0.34

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 287 lb uplift at joint 1, 355 lb uplift at joint 8 and 851 lb uplift at joint 10.

**LOAD CASE(S)** Standard

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ. FL 33549

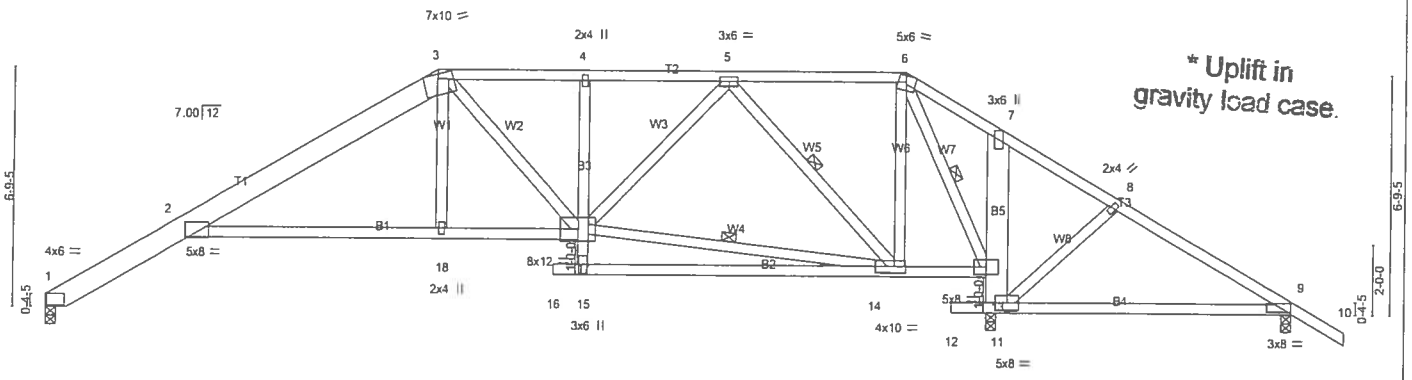
Dwg.#0103071075

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | T25 | SPECIAL | 1 | 1 | Job Reference (optional) |

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 03 09:48:35 2007 Page 1

| 1-6-0 | 2-3-8 | 4-3-8 | 11-0-0 | 15-0-0 | 19-3-4 | 24-3-0 | 26-8-12 | 30-2-3 | 35-3-0 | 36-9-0 |
|---|---|---|---|---|---|---|---|---|---|---|
| 1-6-0 | 2-3-8 | 2-0-0 | 6-8-8 | 4-0-0 | 4-3-4 | 4-11-12 | 2-5-12 | 3-5-7 | 5-0-13 | 1-6-0 |

Scale = 1:62.7




| 2-3-8 | 4-3-8 | 11-0-0 | 14-3-8 | 15-0-0 | 24-3-0 | 25-7-0 | 26-8-12 | 35-3-0 |
|---|---|---|---|---|---|---|---|---|
| 2-3-8 | 2-0-0 | 6-8-8 | 3-3-8 | 0-8-8 | 9-3-0 | 1-4-0 | 0-1-12 | 8-6-4 |

26-7-0

1-0-0

Plate Offsets (X,Y): [3:10-5-9,6-1-9], [9:0-8-1,0-0-10]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.65 | Vert(LL) | -0.55 | 2-18 | >583 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.64 | Vert(TL) | -0.90 | 2-18 | >355 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.61 | Horz(TL) | 0.41 | 11 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 221 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2 *Except*
T1 2 X 8 SYP 2400F 2.0E
BOT CHORD 2 X 4 SYP No.2 *Except*
B3 2 X 4 SYP No.3, B5 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-9-14 oc bracing.
WEBS 1 Row at midpt 14-17, 5-14, 6-13

**REACTIONS** (lb/size) 1=758/0-3-8, 11=2829/0-3-8, 9=-569/0-3-8
Max Horz 1=243(load case 4)
Max Uplift1=-239(load case 5), 11=-902(load case 4), 9=-787(load case 9)
Max Grav 1=758(load case 9), 11=2829(load case 1), 9=322(load case 4)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-344/236, 2-3=-1090/370, 3-4=-707/460, 4-5=-702/457, 5-6=-280/675, 6-7=-949/2411, 7-8=-729/1940, 8-9=-759/1822, 9-10=0/40
BOT CHORD 2-18=-458/1005, 17-18=-458/1014, 15-17=0/161, 4-17=-139/178, 15-16=0/0, 14-15=-7/218, 13-14=-846/444, 11-13=-2486/828, 7-13=-169/211, 11-12=0/0, 9-11=-1526/640
WEBS 3-18=-1/275, 3-17=-561/255, 14-17=-265/165, 5-17=-322/890, 5-14=-1175/587, 6-14=-377/1099, 6-13=-2759/1068, 8-11=-270/230

**JOINT STRESS INDEX**
2 = 0.52, 3 = 0.35, 3 = 0.00, 4 = 0.34, 5 = 0.57, 6 = 0.74, 7 = 0.39, 8 = 0.34, 9 = 0.73, 11 = 0.66, 13 = 0.86, 14 = 0.53, 15 = 0.67, 17 = 0.47 and 18 = 0.34

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust), h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi.
5) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 239 lb uplift at joint 1, 902 lb uplift at joint 11 and 787 lb uplift at joint 9.

**LOAD CASE(S)** Standard

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

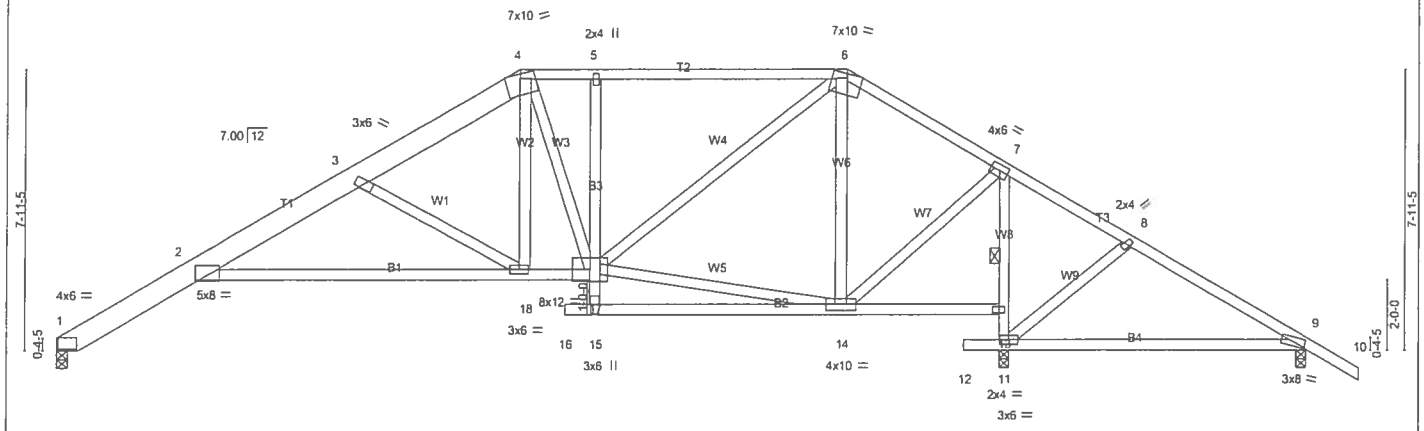
Dwg.#0103071076

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | T26 | SPECIAL | 1 | 1 | Job Reference (optional) |

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 03 09:48:37 2007 Page 1

| -1-6-0 | 2-3-8 | 4-3-8 | 8-7-12 | 13-0-0 | 14-3-8 | 22-3-0 | 25-7-0 | 30-2-3 | 35-3-0 | 36-9-0 |
|---|---|---|---|---|---|---|---|---|---|---|
| 1-6-0 | 2-3-8 | 2-0-0 | 4-4-4 | 4-4-4 | 1-3-8 | 7-11-8 | 3-4-0 | 4-7-3 | 5-0-13 | 1-6-0 |

Scale = 1:62.7




15-0-0

| 2-3-8 | 4-3-8 | 13-0-0 | 14-3-8 | 22-3-0 | 25-7-0 | 26-8-12 | 35-3-0 |
|---|---|---|---|---|---|---|---|
| 2-3-8 | 2-0-0 | 8-8-8 | 1-3-8 | 7-3-0 | 3-4-0 | 1-1-12 | 8-6-4 |

0-8-8

Plate Offsets (X,Y): [4:12-5-8,7-3-5], [9:0-0-14,0-0-3]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.89 | Vert(LL) | -0.61 | 2-18 | >521 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.77 | Vert(TL) | -1.01 | 2-18 | >315 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.89 | Horz(TL) | 0.37 | 11 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 222 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2 *Except*
T1 2 X 8 SYP 2400F 2.0E
BOT CHORD 2 X 4 SYP No.2 *Except*
B3 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3 *Except*
W8 2 X 4 SYP No.2

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. Except:
1 Row at midpt 2-18
WEBS 1 Row at midpt 7-11

**REACTIONS** (lb/size) 1=981/0-3-8, 9=192/0-3-8, 11=1841/0-3-8
Max Horz 1=-279(load case 3)
Max Uplift1=-339(load case 5), 9=-383(load case 6), 11=-510(load case 5)
Max Grav 1=981(load case 1), 9=273(load case 10), 11=1841(load case 1)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-448/292, 2-3=-2227/755, 3-4=-1518/494, 4-5=-1109/447, 5-6=-1120/457, 6-7=-541/324, 7-8=-157/516, 8-9=-181/405, 9-10=0/40
BOT CHORD 2-18=-843/2245, 17-18=-432/1200, 15-17=0/132, 5-17=-322/289, 15-16=0/0, 14-15=0/196, 13-14=-511/196, 11-12=0/0, 9-11=-311/141
WEBS 3-18=-1232/587, 4-18=-264/970, 4-17=-293/70, 14-17=-146/221, 6-17=-406/928, 6-14=-647/332, 7-14=-414/1235, 11-13=-1528/338, 7-13=-1481/348, 8-11=-267/235

**JOINT STRESS INDEX**
2 = 0.51, 3 = 0.37, 4 = 0.40, 4 = 0.00, 5 = 0.54, 6 = 0.62, 7 = 0.61, 8 = 0.34, 9 = 0.86, 11 = 0.53, 13 = 0.37, 14 = 0.62, 15 = 0.44, 17 = 0.45 and 18 = 0.63

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 339 lb uplift at joint 1, 383 lb uplift at joint 9 and 510 lb uplift at joint 11.

**LOAD CASE(S)** Standard

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0103071077

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | T27 | SPECIAL | 1 | 1 | Job Reference (optional) |

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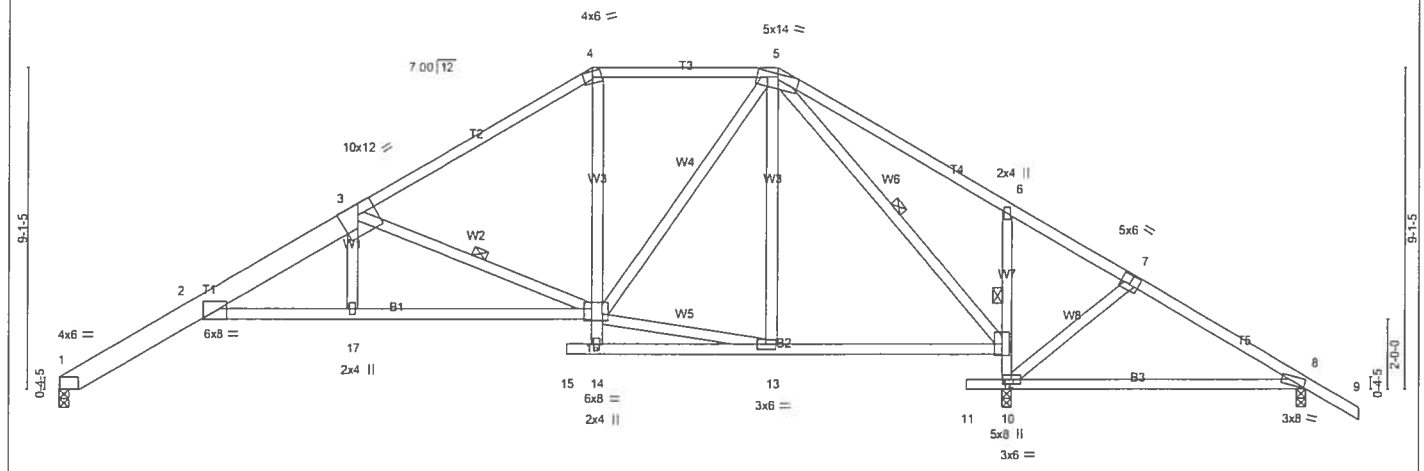
1-6-0 | 2-3-8 | 4-3-8 | 8-5-0 | 15-0-0 | 20-3-0 | 26-8-12 | 30-2-3 | 35-3-0 | 36-9-0

1-6-0 | 2-3-8 | 2-0-0 | 4-1-8 | 6-7-0 | 5-3-0 | 6-5-12 | 3-5-7 | 5-0-13 | 1-6-0

Scale = 1:62.7




2-3-8 | 4-3-8 | 8-5-0 | 14-3-8 | 15-0-0 | 20-3-0 | 25-7-0 | 26-8-12 | 35-3-0

2-3-8 | 2-0-0 | 4-1-8 | 5-10-8 | 0-8-8 | 5-3-0 | 5-4-0 | 1-1-12 | 8-6-4

Plate Offsets (X,Y): [2:0-0-8,0-0-0], [3:7-10-7,4-6-14], [7:0-3-0,0-3-0], [8:0-0-14,0-0-3], [16:0-2-8,0-2-0]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.88 | Vert(LL) | -0.51 | 2-17 | >630 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.82 | Vert(TL) | 0.30 | 8-10 | >338 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.79 | Horz(TL) | 0.38 | 10 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 211 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2 *Except*
T1 2 X 8 SYP 2400F 2.0E
BOT CHORD 2 X 4 SYP No.2 *Except*
B1 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except*
W3 2 X 4 SYP No.1D, W3 2 X 4 SYP No.1D, W7 2 X 4 SYP No.2

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 3-16, 5-12, 6-10

**REACTIONS** (lb/size) 8=206/0-3-8, 10=1819/0-3-8, 1=983/0-3-8
Max Horz 1=-325(load case 3)
Max Uplift8=-366(load case 6), 10=-546(load case 5), 1=-344(load case 5)
Max Grav 8=293(load case 10), 10=1819(load case 1), 1=983(load case 1)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-449/309, 2-3=-2311/762, 3-4=-1208/425, 4-5=-940/434, 5-6=-192/612, 6-7=-126/481, 7-8=-151/364, 8-9=0/40
BOT CHORD 2-17=-718/2230, 16-17=-718/2213, 14-15=0/0, 13-14=-3/27, 12-13=-128/544, 10-11=0/0, 8-10=-277/115
WEBS 3-16=-1379/638, 13-16=-126/529, 5-13=0/129, 5-12=-1564/416, 14-16=0/80, 4-16=-69/279, 5-16=-297/684, 10-12=-1529/388, 6-12=-251/274, 7-10=-229/203, 3-17=0/319

**JOINT STRESS INDEX**
2 = 0.75, 3 = 0.30, 3 = 0.00, 4 = 0.63, 5 = 0.29, 6 = 0.34, 7 = 0.42, 8 = 0.84, 10 = 0.53, 12 = 0.45, 13 = 0.35, 14 = 0.34, 16 = 0.70 and 17 = 0.34

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 366 lb uplift at joint 8, 546 lb uplift at joint 10 and 344 lb uplift at joint 1.

**LOAD CASE(S)** Standard

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0103071078

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | T28 | SPECIAL | 1 | 1 | Job Reference (optional) |

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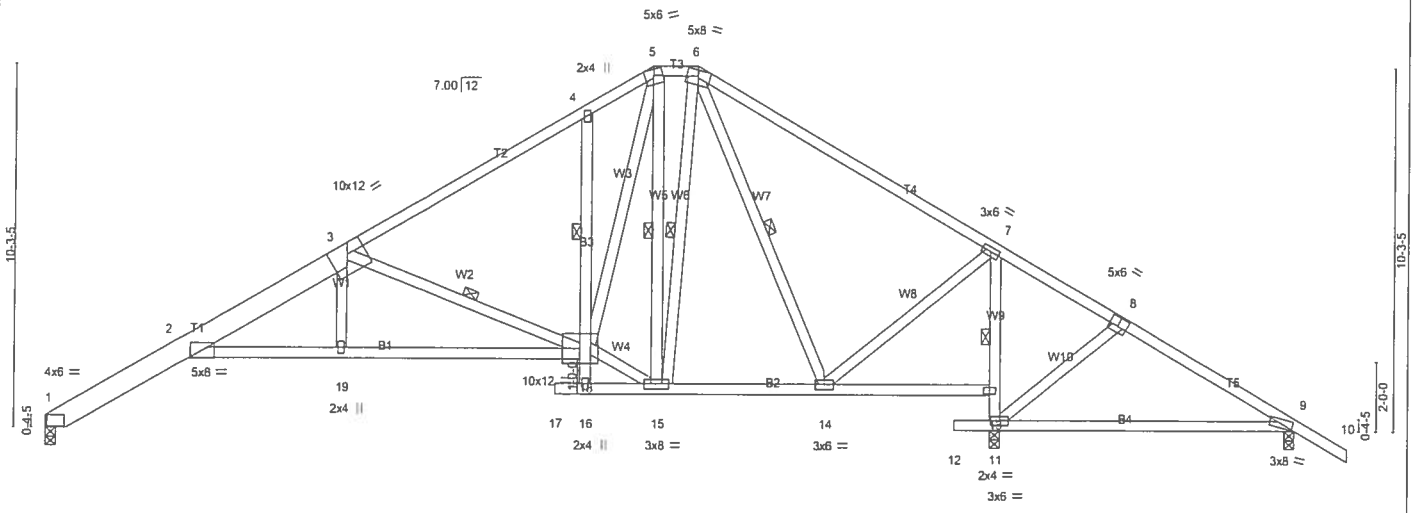
1-6-0 | 2-3-8 | 4-3-8 | 8-5-0 | 14-3-8 | 17-0-0 | 18-3-0 | 21-11-0 | 30-2-3 | 35-3-0 | 36-9-0

1-6-0 | 2-3-8 | 2-0-0 | 4-1-8 | 5-10-8 | 2-8-8 | 1-3-0 | 3-8-0 | 8-3-3 | 5-0-13 | 1-6-0

Scale = 1:62.7




17-0-0

2-3-8 | 4-3-8 | 8-5-0 | 14-3-8 | 15-0-0 | 18-3-0 | 21-11-0 | 25-7-0 | 26-8-12 | 35-3-0

2-3-8 | 2-0-0 | 4-1-8 | 5-10-8 | 0-8-8 | 2-0-0 | 1-3-0 | 3-8-0 | 3-8-0 | 1-1-12 | 8-6-4

Plate Offsets (X,Y): [2:0-0-4,0-0-0], [3:7-10-9,4-6-13], [8:0-3-0,0-3-0], [9:0-0-14,0-0-3]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.87 | Vert(LL) | -0.51 | 2-19 | >622 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.98 | Vert(TL) | 0.30 | 9-11 | >336 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.89 | Horz(TL) | 0.38 | 11 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 231 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2 *Except*
T1 2 X 8 SYP 2400F 2.0E
BOT CHORD 2 X 4 SYP No.2 *Except*
B3 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3 *Except*
W9 2 X 4 SYP No.2

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing. Except:
1 Row at midpt 4-18
WEBS 1 Row at midpt 5-15, 6-14, 7-11, 6-15, 3-18

**REACTIONS** (lb/size) 9=168/0-3-8, 11=1873/0-3-8, 1=973/0-3-8
Max Horz 1=-365(load case 3)
Max Uplift9=-358(load case 6), 11=-585(load case 5), 1=-345(load case 5)
Max Grav 9=270(load case 10), 11=1873(load case 1), 1=973(load case 1)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-444/329, 2-3=-2265/776, 3-4=-1178/423, 4-5=-1100/570, 5-6=-607/371, 6-7=-523/337, 7-8=-118/551, 8-9=-139/412, 9-10=0/40
BOT CHORD 2-19=-750/2193, 18-19=-750/2178, 16-18=0/34, 4-18=-248/285, 16-17=0/0, 15-16=-65/19, 14-15=-88/584, 13-14=-515/218, 11-12=0/0, 9-11=-319/105
WEBS 15-18=-66/695, 5-18=-494/1237, 5-15=-600/295, 6-14=-608/192, 11-13=-1600/440, 7-13=-1542/450, 7-14=-222/1131, 8-11=-198/169, 6-15=-243/452, 3-19=-3/290, 3-18=-1366/652

**JOINT STRESS INDEX**
2 = 0.77, 3 = 0.30, 3 = 0.00, 4 = 0.72, 5 = 0.73, 6 = 0.75, 7 = 0.81, 8 = 0.38, 9 = 0.83, 11 = 0.55, 13 = 0.38, 14 = 0.68, 15 = 0.66, 16 = 0.34, 18 = 0.60 and 19 = 0.34

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 358 lb uplift at joint 9, 585 lb uplift at joint 11 and 345 lb uplift at joint 1.

**LOAD CASE(S)** Standard

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549

Dwg.#0103071079

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | T29 | SPECIAL | 1 | 1 | Job Reference (optional) |

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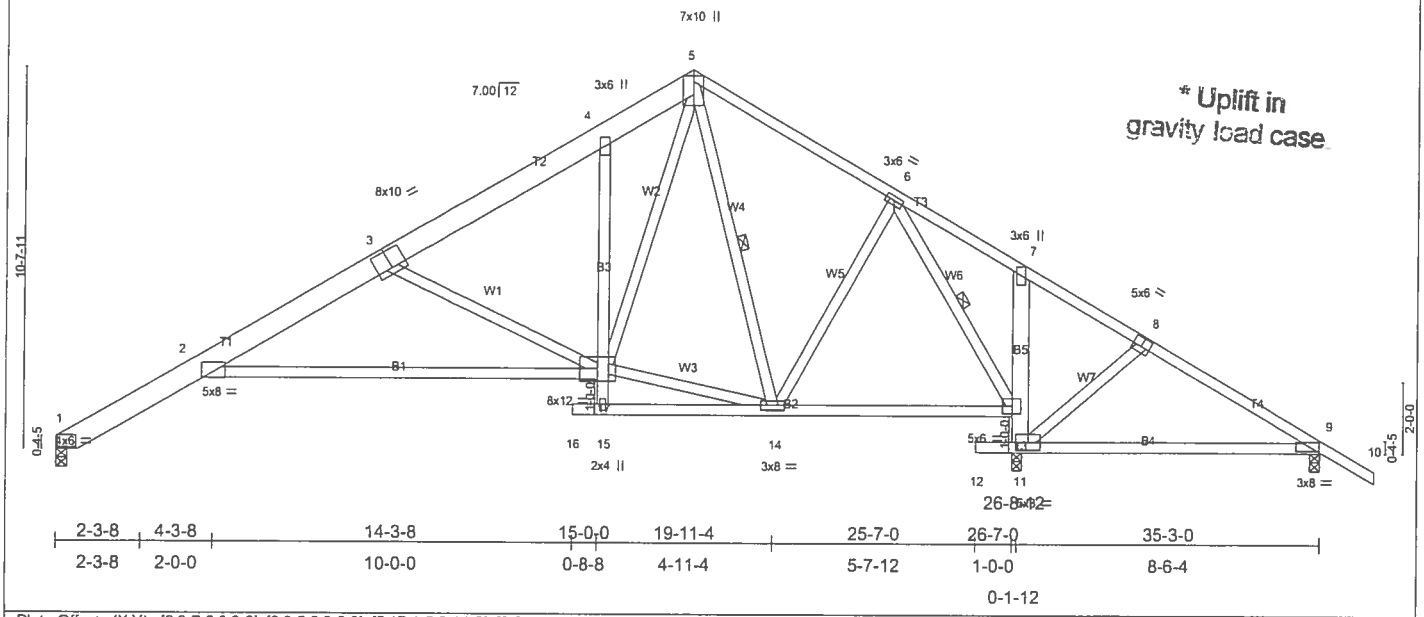
| 1-6-0 | 2-3-8 | 4-3-8 | 9-3-8 | 14-3-8 | 17-7-8 | 25-7-0 | 30-2-3 | 35-3-0 | 36-9-0 |
|---|---|---|---|---|---|---|---|---|---|
| 1-6-0 | 2-3-8 | 2-0-0 | 5-0-0 | 5-0-0 | 3-4-0 | 7-11-8 | 4-7-3 | 5-0-13 | 1-6-0 |

Scale = 1:61.9




| 2-3-8 | 4-3-8 | 14-3-8 | 15-0-0 | 19-11-4 | 25-7-0 | 26-7-0 | 35-3-0 |
|---|---|---|---|---|---|---|---|
| 2-3-8 | 2-0-0 | 10-0-0 | 0-8-8 | 4-11-4 | 5-7-12 | 1-0-0 | 8-6-4 |
| | | | | | | 0-1-12 | |

Plate Offsets (X,Y): [2:0-7-8,0-0-0], [3:0-5-0,0-6-0], [5:17-1-2,9-11-3], [8:0-3-0,0-3-0], [9:0-8-1,0-0-10]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.78 | Vert(LL) | -0.67 | 2-17 | >474 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.85 | Vert(TL) | -1.13 | 2-17 | >284 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.89 | Horz(TL) | 0.40 | 11 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 235 lb | |

**LUMBER**
TOP CHORD 2 X 8 SYP 2400F 2.0E *Except* T3 2 X 4 SYP No.2, T4 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except* B3 2 X 4 SYP No.3, B5 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 4-1-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 5-14, 6-13

**REACTIONS** (lb/size) 11=2419/0-3-8, 9=-252/0-3-8, 1=848/0-3-8
Max Horz 1=-375(load case 3)
Max Uplift11=-778(load case 5), 9=-467(load case 3), 1=-303(load case 5)
Max Grav 11=2419(load case 1), 9=53(load case 4), 1=848(load case 1)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-386/336, 2-3=-1638/612, 3-4=-918/356, 4-5=-698/407, 5-6=-306/343, 6-7=-400/1650, 7-8=-315/1344, 8-9=-284/1172, 9-10=0/40
BOT CHORD 2-17=-584/1652, 15-17=0/99, 4-17=-29/165, 15-16=0/0, 14-15=-213/0, 13-14=-324/203, 11-13=-2098/600, 7-13=-84/101, 11-12=0/0, 9-11=-966/294
WEBS 3-17=-1130/608, 14-17=0/594, 5-17=-376/916, 5-14=-776/223, 6-14=-192/876, 8-11=-250/225, 6-13=-2244/676

**JOINT STRESS INDEX**
2 = 0.71, 3 = 0.35, 4 = 0.23, 5 = 0.44, 5 = 0.00, 6 = 0.77, 7 = 0.52, 8 = 0.55, 9 = 0.65, 11 = 0.55, 13 = 0.71, 14 = 0.89, 15 = 0.53 and 17 = 0.71

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 778 lb uplift at joint 11, 467 lb uplift at joint 9 and 303 lb uplift at joint 1.

**LOAD CASE(S)** Standard

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ. FL 33549

Dwg.#0103071080

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | T30 | SPECIAL | 1 | 1 | Job Reference (optional) |

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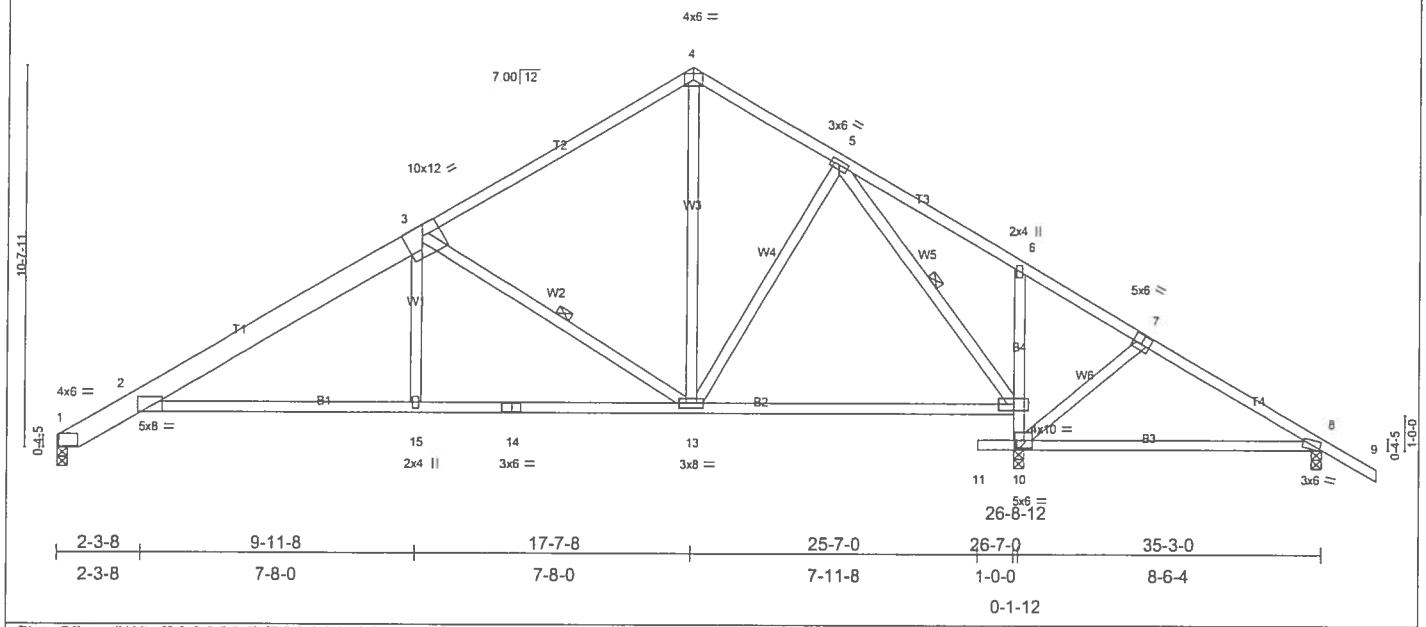
| 1-6-0 | 2-3-8 | 9-11-8 | 17-7-8 | 25-7-0 | 30-2-3 | 35-3-0 | 36-9-0 |
|---|---|---|---|---|---|---|---|
| 1-6-0 | 2-3-8 | 7-8-0 | 7-8-0 | 7-11-8 | 4-7-3 | 5-0-13 | 1-6-0 |

Scale = 1:61.9




| 2-3-8 | 9-11-8 | 17-7-8 | 25-7-0 | 26-7-0 | 35-3-0 |
|---|---|---|---|---|---|
| 2-3-8 | 7-8-0 | 7-8-0 | 7-11-8 | 1-0-0 | 8-6-4 |
| | | | | 0-1-12 | |

Plate Offsets (X,Y): [3:9-6-9,5-6-6], [7:0-3-0,0-3-0], [8:0-0-11,Edge]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.54 | Vert(LL) | -0.36 | 2-15 | >884 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.75 | Vert(TL) | -0.59 | 2-15 | >542 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.48 | Horz(TL) | 0.17 | 10 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 203 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2 *Except*
T1 2 X 8 SYP 2400F 2.0E
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 3-13, 5-12

**REACTIONS** (lb/size) 10=1768/0-3-8, 8=239/0-3-8, 1=1012/0-3-8
Max Horz 1=-378(load case 3)
Max Uplift10=-498(load case 5), 8=-415(load case 6), 1=-373(load case 5)
Max Grav 10=1768(load case 1), 8=321(load case 10), 1=1012(load case 1)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-438/326, 2-3=-1648/589, 3-4=-851/429, 4-5=-767/436, 5-6=0/472, 6-7=0/405, 7-8=-57/426, 8-9=0/40
BOT CHORD 2-15=-515/1445, 14-15=-516/1436, 13-14=-516/1436, 12-13=-74/448, 10-12=-1455/329, 6-12=-231/217, 10-11=0/0, 8-10=-300/16
WEBS 3-15=0/278, 3-13=-961/530, 4-13=-262/438, 5-13=-114/416, 7-10=-244/214, 5-12=-1323/258

**JOINT STRESS INDEX**
2 = 0.41, 3 = 0.40, 3 = 0.00, 4 = 0.62, 5 = 0.45, 6 = 0.34, 7 = 0.44, 8 = 0.89, 10 = 0.64, 12 = 0.98, 13 = 0.60, 14 = 0.51 and 15 = 0.34

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 498 lb uplift at joint 10, 415 lb uplift at joint 8 and 373 lb uplift at joint 1.

**LOAD CASE(S)** Standard

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ. FL 33549

Dwg.#0103071081

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | T31 | SPECIAL | 2 | 1 | Job Reference (optional) |

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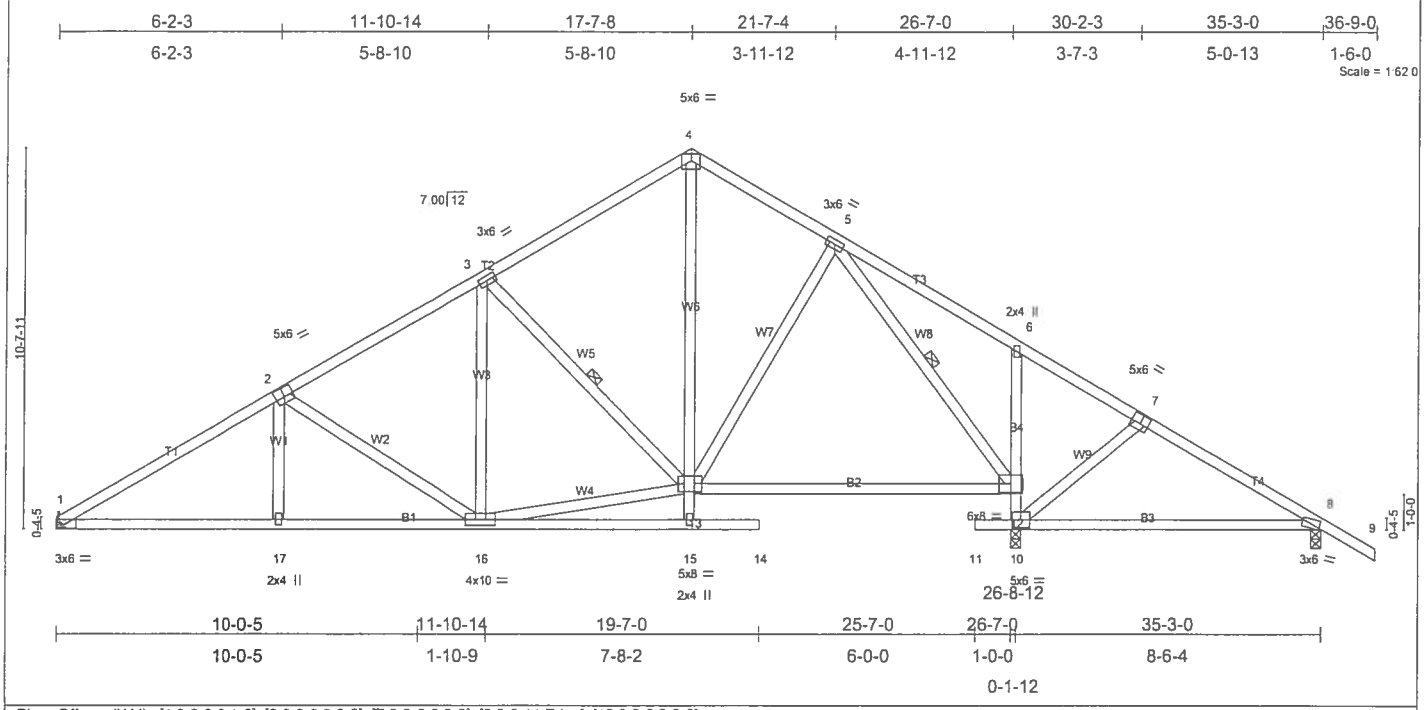


Plate Offsets (X,Y): [1:0-3-3,0-1-8], [2:0-3-0,0-3-0], [7:0-3-0,0-3-0], [8:0-0-11,Edge], [13:0-2-8,0-2-8]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.26 | Vert(LL) | 0.22 | 8-10 | >459 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.82 | Vert(TL) | 0.17 | 8-10 | >584 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.41 | Horz(TL) | 0.03 | 8 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 217 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B4 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 4-4-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 3-13, 5-12

**REACTIONS** (lb/size) 1=1107/Mechanical, 10=1641/0-3-8, 8=366/0-3-8
Max Horz 1=-378(load case 3)
Max Uplift1=-395(load case 5), 10=-393(load case 5), 8=-454(load case 6)
Max Grav 1=1107(load case 1), 10=1641(load case 1), 8=382(load case 10)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1809/636, 2-3=-1321/542, 3-4=-900/455, 4-5=-878/487, 5-6=-34/495, 6-7=0/408, 7-8=-183/451, 8-9=0/40
BOT CHORD 1-17=-575/1483, 16-17=-576/1479, 15-16=-0/34, 14-15=0/0, 12-13=-137/595, 10-12=-1316/220, 6-12=-249/221, 10-11=0/0, 8-10=-280/123
WEBS 2-17=0/205, 3-13=-533/331, 13-15=0/147, 4-13=-307/553, 5-13=-72/328, 7-10=-248/213, 13-16=-341/1054, 3-16=-64/286, 2-16=-490/288, 5-12=-1114/134

**JOINT STRESS INDEX**
1 = 0.75, 2 = 0.53, 3 = 0.42, 4 = 0.33, 5 = 0.44, 6 = 0.64, 7 = 0.45, 8 = 0.75, 10 = 0.64, 12 = 0.76, 13 = 0.55, 15 = 0.34, 16 = 0.47 and 17 = 0.34

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 395 lb uplift at joint 1, 393 lb uplift at joint 10 and 454 lb uplift at joint 8.

**LOAD CASE(S)** Standard

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ. FL 33549

Dwg.#0103071082

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | T32 | SPECIAL | 1 | 1 | Job Reference (optional) |

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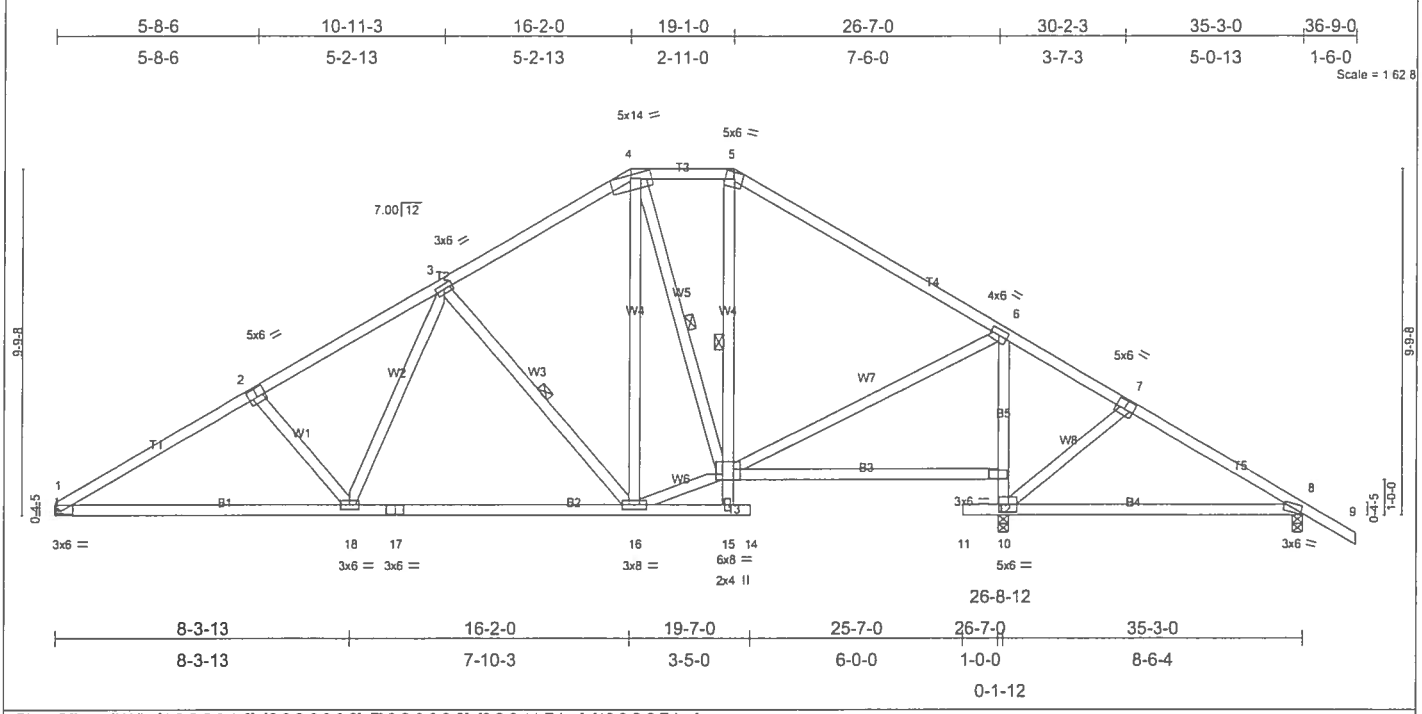


Plate Offsets (X,Y): [1:0-3-3,0-1-8], [2:0-3-0,0-3-0], [7:0-3-0,0-3-0], [8:0-0-11,Edge], [13:0-2-8,Edge]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.39 | Vert(LL) | 0.22 | 8-10 | >466 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.71 | Vert(TL) | 0.17 | 8-10 | >593 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.29 | Horz(TL) | 0.02 | 8 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 216 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B5 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 4-3-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
8-0-13 oc bracing: 1-18
6-0-0 oc bracing: 12-13.
WEBS 1 Row at midpt 3-16, 5-15, 4-13

**REACTIONS** (lb/size) 1=1108/Mechanical, 10=1562/0-3-8, 8=406/0-3-8
Max Horz 1=-348(load case 3)
Max Uplift1=-397(load case 5), 10=-370(load case 5), 8=-469(load case 6)
Max Grav 1=1108(load case 1), 10=1562(load case 1), 8=421(load case 10)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1763/652, 2-3=-1588/639, 3-4=-939/476, 4-5=-721/462, 5-6=-943/459, 6-7=-58/435, 7-8=-252/471, 8-9=0/40
BOT CHORD 1-18=-587/1469, 17-18=-366/1112, 16-17=-366/1112, 15-16=-0/6, 14-15=0/0, 12-13=-182/60, 10-12=-1250/205, 6-12=-1107/230, 10-11=0/0, 8-10=-295/180
WEBS 2-18=-262/251, 3-18=-166/517, 3-16=-573/361, 4-16=-184/377, 6-13=-100/779, 7-10=-221/187, 13-15=0/21, 5-13=-121/237, 13-16=-240/791, 4-13=-224/113

**JOINT STRESS INDEX**
1 = 0.73, 2 = 0.46, 3 = 0.42, 4 = 0.55, 5 = 0.73, 6 = 0.69, 7 = 0.28, 8 = 0.73, 10 = 0.67, 12 = 0.82, 13 = 0.50, 15 = 0.34, 16 = 0.73, 17 = 0.49 and 18 = 0.48

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 397 lb uplift at joint 1, 370 lb uplift at joint 10 and 469 lb uplift at joint 8.

**LOAD CASE(S)** Standard

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549

Dwg.#0103071083

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | T33 | SPECIAL | 1 | 1 | Job Reference (optional) |

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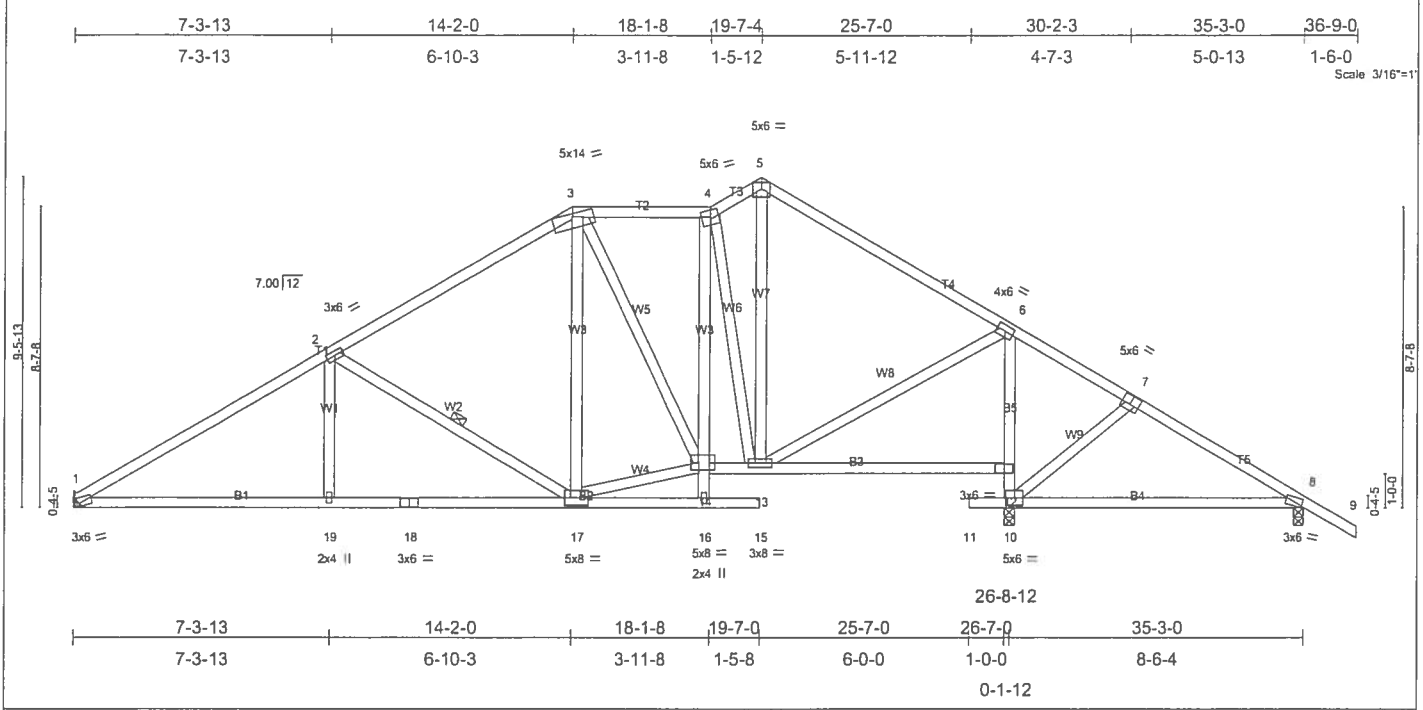


Plate Offsets (X,Y): [1:0-0-11,Edge], [7:0-3-0,0-3-0], [8:0-0-11,Edge], [14:0-5-8,0-2-8]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.40 | Vert(LL) | 0.22 | 8-10 | >464 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.61 | Vert(TL) | 0.17 | 8-10 | >595 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.81 | Horz(TL) | 0.02 | 8 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 225 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B5 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 4-2-12 oc purlins
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 2-17

**REACTIONS** (lb/size) 1=1120/Mechanical, 10=1574/0-3-8, 8=414/0-3-8
Max Horz 1=-337(load case 3)
Max Uplift1=-408(load case 5), 10=-414(load case 5), 8=-457(load case 6)
Max Grav 1=1120(load case 1), 10=1574(load case 1), 8=437(load case 10)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1793/646, 2-3=-1163/514, 3-4=-908/480, 4-5=-802/440, 5-6=-970/458, 6-7=-88/408, 7-8=-285/448, 8-9=0/40
BOT CHORD 1-19=-556/1463, 18-19=-556/1463, 17-18=-556/1463, 16-17=-2/8, 15-16=0/0, 13-14=-202/900, 12-13=-152/67, 10-12=-1261/246, 6-12=-1144/262, 10-11=0/0, 8-10=-276/208
WEBS 2-19=0/260, 2-17=-644/370, 3-17=-84/249, 3-14=-126/83, 14-16=0/94, 4-14=-148/401, 7-10=-228/197, 14-17=-250/949, 5-13=-233/588, 4-13=-820/334, 6-13=-75/844

**JOINT STRESS INDEX**
1 = 0.84, 2 = 0.42, 3 = 0.97, 4 = 0.25, 5 = 0.58, 6 = 0.52, 7 = 0.30, 8 = 0.75, 10 = 0.66, 12 = 0.60, 13 = 0.80, 14 = 0.49, 16 = 0.34, 17 = 0.47, 18 = 0.53 and 19 = 0.34

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 408 lb uplift at joint 1, 414 lb uplift at joint 10 and 457 lb uplift at joint 8.

**LOAD CASE(S)** Standard

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0103071084

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | T34 | SPECIAL | 1 | 1 | Job Reference (optional) |

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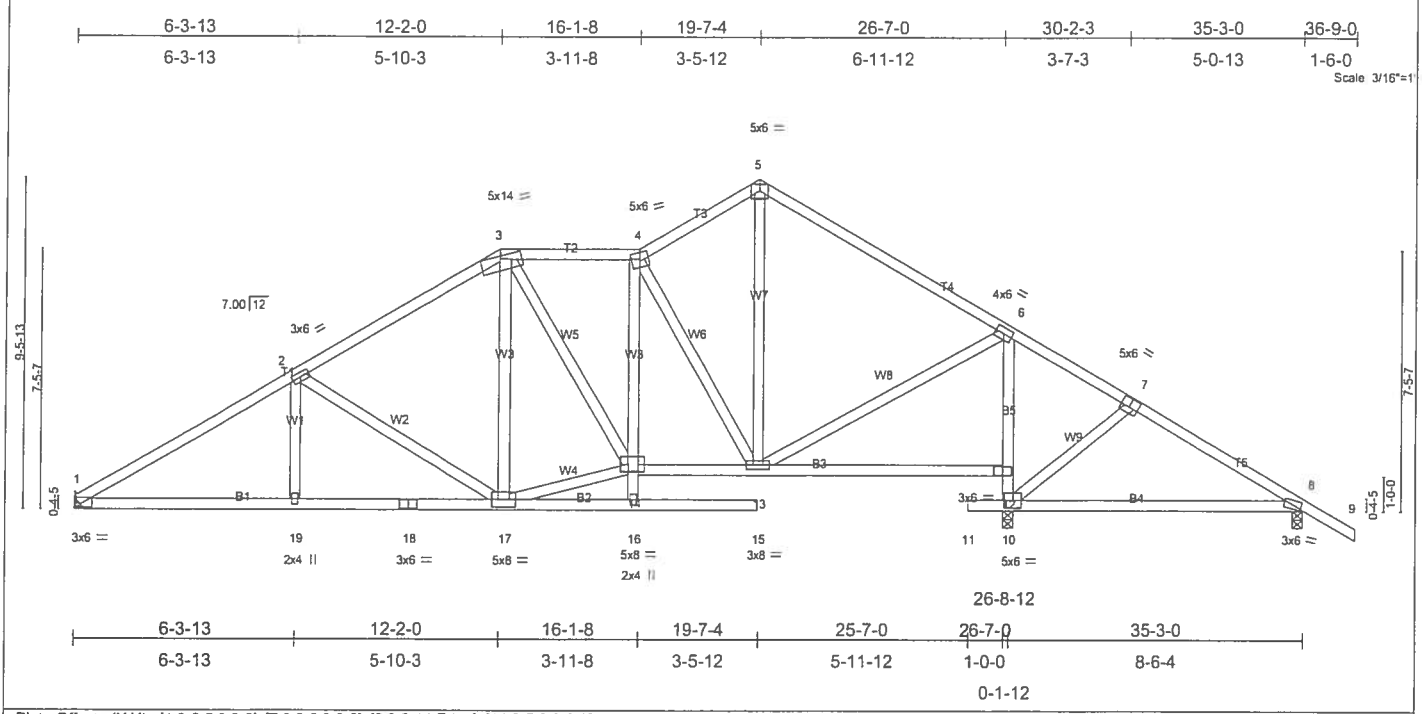


Plate Offsets (X,Y): [1:0-6-5,0-0-6], [7:0-3-0,0-3-0], [8:0-0-11,Edge], [14:0-5-8,0-2-8]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.35 | Vert(LL) | 0.22 | 8-10 | >460 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.66 | Vert(TL) | 0.17 | 8-10 | >592 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.87 | Horz(TL) | 0.03 | 8 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 220 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except* B5 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 4-3-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

**REACTIONS** (lb/size) 1=1137/Mechanical, 10=1635/0-3-8, 8=396/0-3-8
Max Horz 1=-338(load case 3)
Max Uplift1=-402(load case 5), 10=-413(load case 5), 8=-454(load case 6)
Max Grav 1=1137(load case 1), 10=1635(load case 1), 8=424(load case 10)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1870/651, 2-3=-1332/543, 3-4=-1253/541, 4-5=-900/447, 5-6=-990/445, 6-7=-63/401, 7-8=-259/440, 8-9=0/40
BOT CHORD 1-19=-573/1536, 18-19=-573/1536, 17-18=-573/1536, 16-17=-4/15, 15-16=0/0, 13-14=-318/1249, 12-13=-146/46, 10-12=-1324/245, 6-12=-1204/261, 10-11=0/0, 8-10=-269/186
WEBS 2-19=0/228, 2-17=-545/311, 3-17=-67/81, 3-14=-73/332, 14-16=0/209, 4-14=-64/312, 6-13=-71/896, 7-10=-226/195, 5-13=-258/617, 14-17=-318/1105, 4-13=-994/396

**JOINT STRESS INDEX**
1 = 0.79, 2 = 0.42, 3 = 0.74, 4 = 0.29, 5 = 0.51, 6 = 0.54, 7 = 0.29, 8 = 0.77, 10 = 0.66, 12 = 0.62, 13 = 0.85, 14 = 0.70, 16 = 0.34, 17 = 0.52, 18 = 0.64 and 19 = 0.34

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 402 lb uplift at joint 1, 413 lb uplift at joint 10 and 454 lb uplift at joint 8.

**LOAD CASE(S)** Standard

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549

Dwg.#0103071085

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | T35 | SPECIAL | 1 | 1 | Job Reference (optional) |

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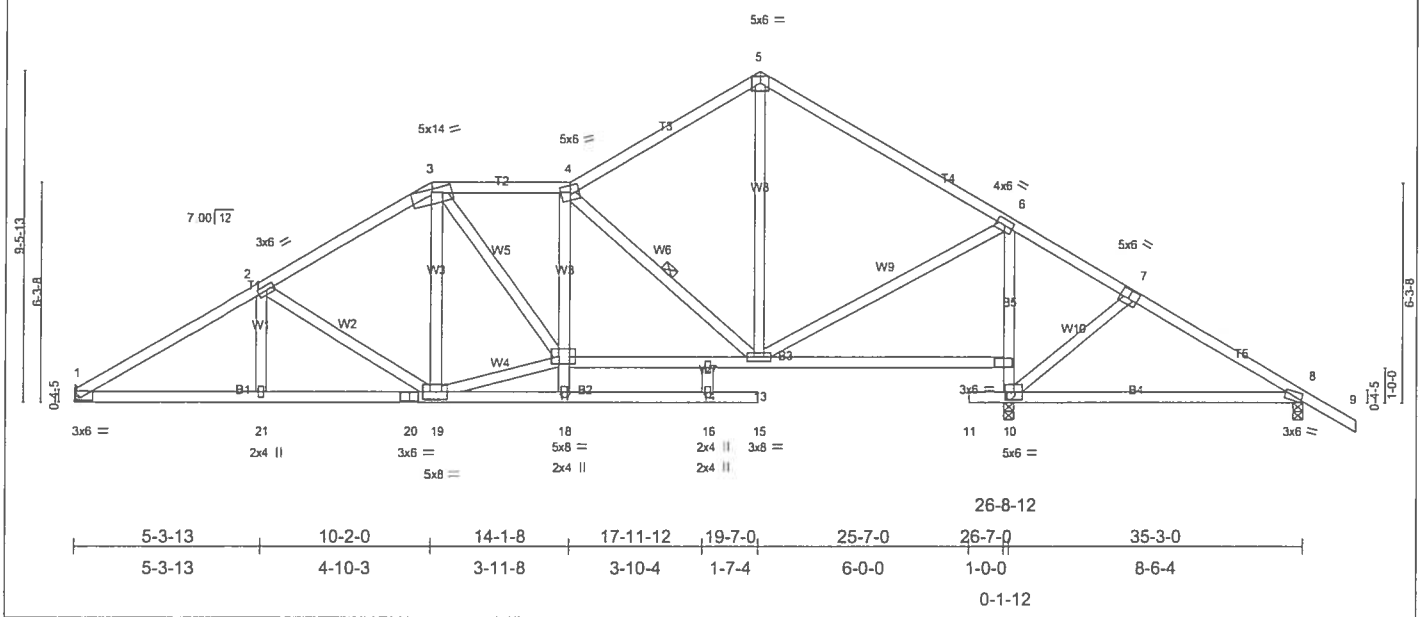


Plate Offsets (X,Y): [1:0-6-5,0-0-6], [7:0-3-0,0-3-0], [8:0-0-11,Edge], [17:0-5-8,0-2-8]

| LOADING (psf) | SPACING 2-0-0 | CSI | DEFL in (loc) l/defl L/d | PLATES GRIP |
|---|---|---|---|---|
| TCLL 20.0 | Plates Increase 1.25 | TC 0.32 | Vert(LL) 0.22 8-10 >456 240 | MT20 244/190 |
| TCDL 7.0 | Lumber Increase 1.25 | BC 0.64 | Vert(TL) -0.20 8-10 >506 180 | |
| BCLL 10.0 | Rep Stress Incr YES | WB 0.38 | Horz(TL) 0.03 8 n/a n/a | |
| BCDL 5.0 | Code FBC2004/TPI2002 | (Matrix) | | Weight: 217 lb |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B5 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 4-5-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-13
JOINTS 1 Brace at Jt(s): 14

**REACTIONS** (lb/size) 1=1111/Mechanical, 10=1605/0-3-8, 8=388/0-3-8
Max Horz 1=-338(load case 3)
Max Uplift1=-404(load case 5), 10=-432(load case 5), 8=-449(load case 6)
Max Grav 1=1111(load case 1), 10=1605(load case 1), 8=423(load case 10)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1856/671, 2-3=-1426/585, 3-4=-1499/661, 4-5=-909/441, 5-6=-945/447, 6-7=-60/388, 7-8=-255/427, 8-9=0/40
BOT CHORD 1-21=-603/1531, 20-21=-603/1531, 19-20=-603/1531, 18-19=-8/22, 16-18=0/0, 15-16=0/0, 14-17=-495/1503, 13-14=-495/1502, 12-13=-136/43, 10-12=-1294/263, 6-12=-1177/276, 10-11=0/0, 8-10=-259/183
WEBS 2-21=0/178, 2-19=-431/256, 3-19=-31/50, 3-17=-171/536, 17-18=0/114, 4-17=-20/119, 6-13=-96/876, 7-10=-227/195, 17-19=-395/1196, 5-13=-235/527, 4-13=-1043/497, 14-16=0/107

**JOINT STRESS INDEX**
1 = 0.78, 2 = 0.42, 3 = 0.51, 4 = 0.42, 5 = 0.52, 6 = 0.53, 7 = 0.41, 8 = 0.78, 10 = 0.65, 12 = 0.61, 13 = 0.85, 14 = 0.34, 16 = 0.34, 17 = 0.84, 18 = 0.34, 19 = 0.57, 20 = 0.51 and 21 = 0.34

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 404 lb uplift at joint 1, 432 lb uplift at joint 10 and 449 lb uplift at joint 8.

**LOAD CASE(S)** Standard

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549

Dwg.#0103071086

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | T36 | SPECIAL | 1 | 1 | Job Reference (optional) |

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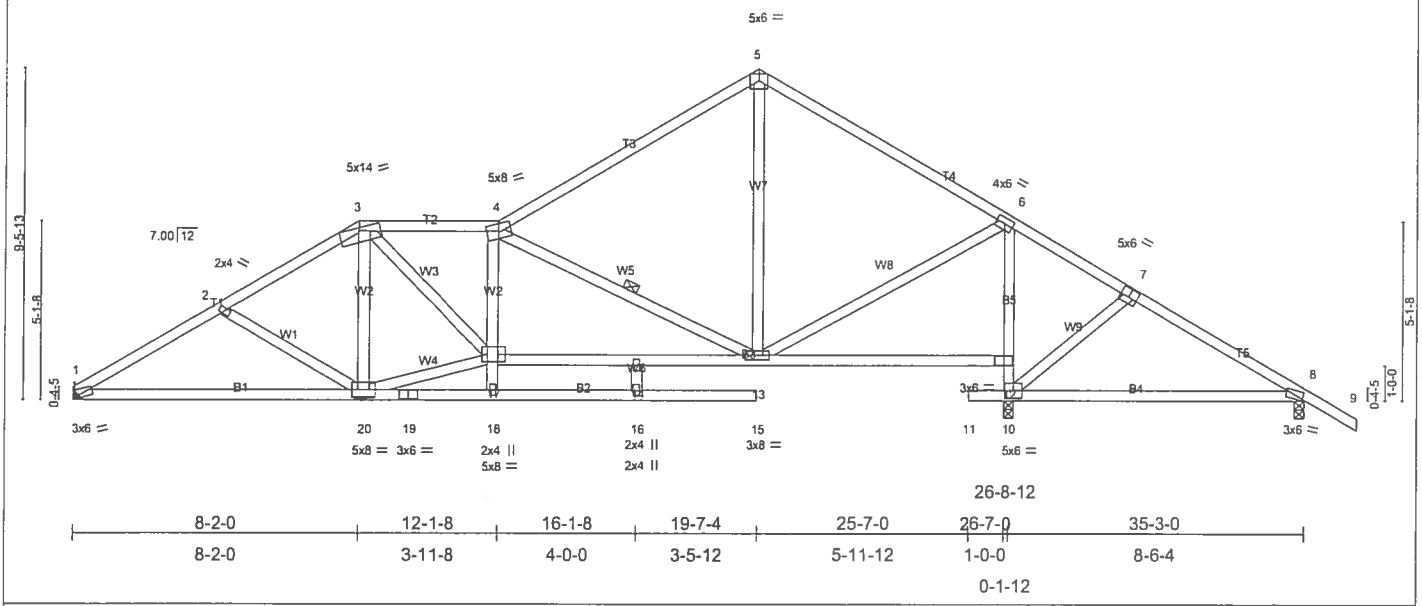
| 4-3-13 | 8-2-0 | 12-1-8 | 19-7-4 | 26-7-0 | 30-2-3 | 35-3-0 | 36-9-0 |
|---|---|---|---|---|---|---|---|
| 4-3-13 | 3-10-3 | 3-11-8 | 7-5-12 | 6-11-12 | 3-7-3 | 5-0-13 | 1-6-0 |

Scale 3/16"=1'




26-8-12

| 8-2-0 | 12-1-8 | 16-1-8 | 19-7-4 | 25-7-0 | 26-7-0 | 35-3-0 |
|---|---|---|---|---|---|---|
| 8-2-0 | 3-11-8 | 4-0-0 | 3-5-12 | 5-11-12 | 1-0-0 | 8-6-4 |

0-1-12

Plate Offsets (X,Y): [1:0-1-2,0-0-7], [7:0-3-0,0-3-0], [8:0-0-11,Edge], [17:0-6-0,0-2-8]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.39 | Vert(LL) | 0.22 | 8-10 | >450 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.97 | Vert(TL) | -0.89 | 15 | >360 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.51 | Horz(TL) | 0.03 | 8 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 211 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B5 2 X 4 SYP No.3, B2 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 4-1-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
7-11-2 oc bracing: 1-20
6-0-0 oc bracing: 12-13.
WEBS 1 Row at midpt 4-13
JOINTS 1 Brace at Jt(s): 14

**REACTIONS** (lb/size) 1=1131/Mechanical, 10=1648/0-3-8, 8=384/0-3-8
Max Horz 1=-338(load case 3)
Max Uplift1=-395(load case 5), 10=-445(load case 5), 8=-437(load case 6)
Max Grav 1=1131(load case 1), 10=1648(load case 1), 8=431(load case 10)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1839/682, 2-3=-1629/597, 3-4=-2042/794, 4-5=-1007/412, 5-6=-1002/424, 6-7=-74/361, 7-8=-268/402, 8-9=0/40
BOT CHORD 1-20=-626/1548, 19-20=-13/37, 18-19=-13/37, 16-18=0/0, 15-16=0/0, 14-17=-689/2059, 13-14=-689/2058, 12-13=-134/45, 10-12=-1338/273, 6-12=-1247/280, 10-11=0/0, 8-10=-238/194
WEBS 2-20=-233/215, 3-20=-58/55, 17-18=0/80, 4-17=-138/166, 6-13=-121/971, 7-10=-229/199, 5-13=-178/528, 4-13=-1426/646, 17-20=-445/1367, 3-17=-336/972, 14-16=0/194

**JOINT STRESS INDEX**
1 = 0.82, 2 = 0.34, 3 = 0.46, 4 = 0.64, 5 = 0.64, 6 = 0.48, 7 = 0.42, 8 = 0.81, 10 = 0.64, 12 = 0.45, 13 = 0.92, 14 = 0.34, 16 = 0.34, 17 = 0.92, 18 = 0.34, 19 = 0.26 and 20 = 0.65

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 395 lb uplift at joint 1, 445 lb uplift at joint 10 and 437 lb uplift at joint 8.

**LOAD CASE(S)** Standard

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549

Dwg.#0103071087

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | T37 | SPECIAL | 1 | 2 | Job Reference (optional) |

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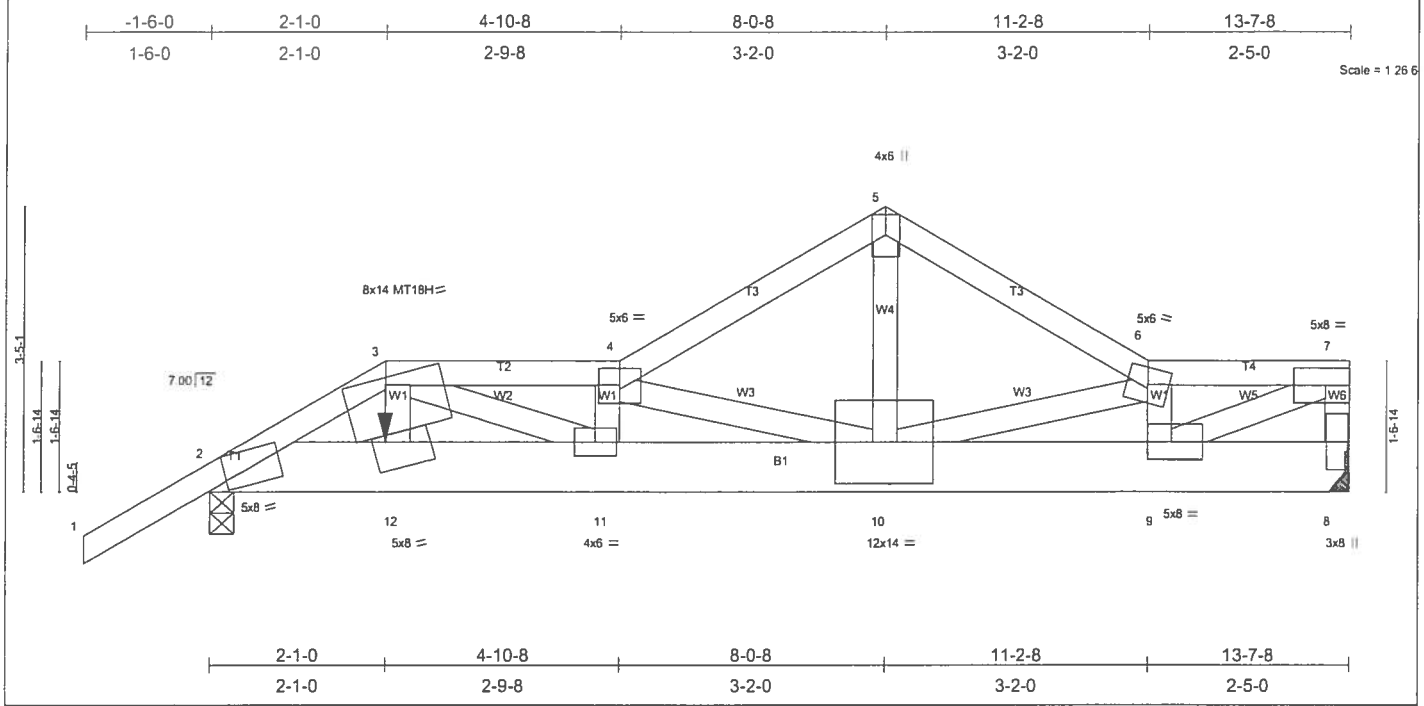


Plate Offsets (X,Y): [3:0-6-1,0-1-2], [9:0-3-8,0-2-8], [12:0-1-14,0-4-2]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.63 | Vert(LL) | -0.17 | 10-11 | >949 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.60 | Vert(TL) | -0.27 | 10-11 | >593 | 180 | MT18H | 244/190 |
| BCLL | 10.0 | Rep Stress Incr | NO | WB | 0.62 | Horz(TL) | 0.03 | 8 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 182 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3 *Except*
W6 2 X 4 SYP No.2, W4 2 X 4 SYP No.2, W5 2 X 4 SYP No.2

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 3-4-5 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 8=5154/Mechanical, 2=5272/0-3-8
Max Horz 2=141(load case 4)
Max Uplift8=-2327(load case 4), 2=-2480(load case 4)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-8787/3965, 3-4=-11427/5208, 4-5=-7017/3188, 5-6=-7023/3203, 6-7=-6675/3020, 7-8=-3392/1532
BOT CHORD 2-12=-3496/7625, 11-12=-3646/7965, 10-11=-5325/11619, 9-10=-3199/7066, 8-9=-358/780
WEBS 3-12=-1178/2627, 3-11=-1723/3756, 4-11=-1479/700, 4-10=-5847/2708, 5-10=-3003/6609, 6-10=-1057/490, 6-9=-3010/1373, 7-9=-3020/6671

**JOINT STRESS INDEX**
2 = 0.88, 3 = 0.98, 4 = 0.83, 5 = 0.77, 6 = 0.79, 7 = 0.73, 8 = 0.66, 9 = 0.75, 10 = 0.58, 11 = 0.75 and 12 = 0.68

**NOTES**
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-7-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.
6) All plates are MT20 plates unless otherwise indicated.
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2327 lb uplift at joint 8 and 2480 lb uplift at joint 2.
9) Girder carries tie-in span(s): 35-3-0 from 0-0-0 to 13-7-8
10) Girder carries hip end with 0-0-0 right side setback, 2-1-0 left side setback, and 2-1-0 end setback.
11) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 17 lb down and 12 lb up at 2-1-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

**LOAD CASE(S)** Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-4=-52(F=2), 4-5=-52(F=2), 5-6=-52(F=2), 6-7=-52(F=2), 2-12=-723(F=-693), 8-12=-722(F=-692)
Concentrated Loads (lb)
Vert: 12=-17(F)

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549

Dwg.#0103071088

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | T38 | HIP | 1 | 1 | Job Reference (optional) |

Builders FirstSource, Lake City, FI 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 03 09:48:55 2007 Page 1

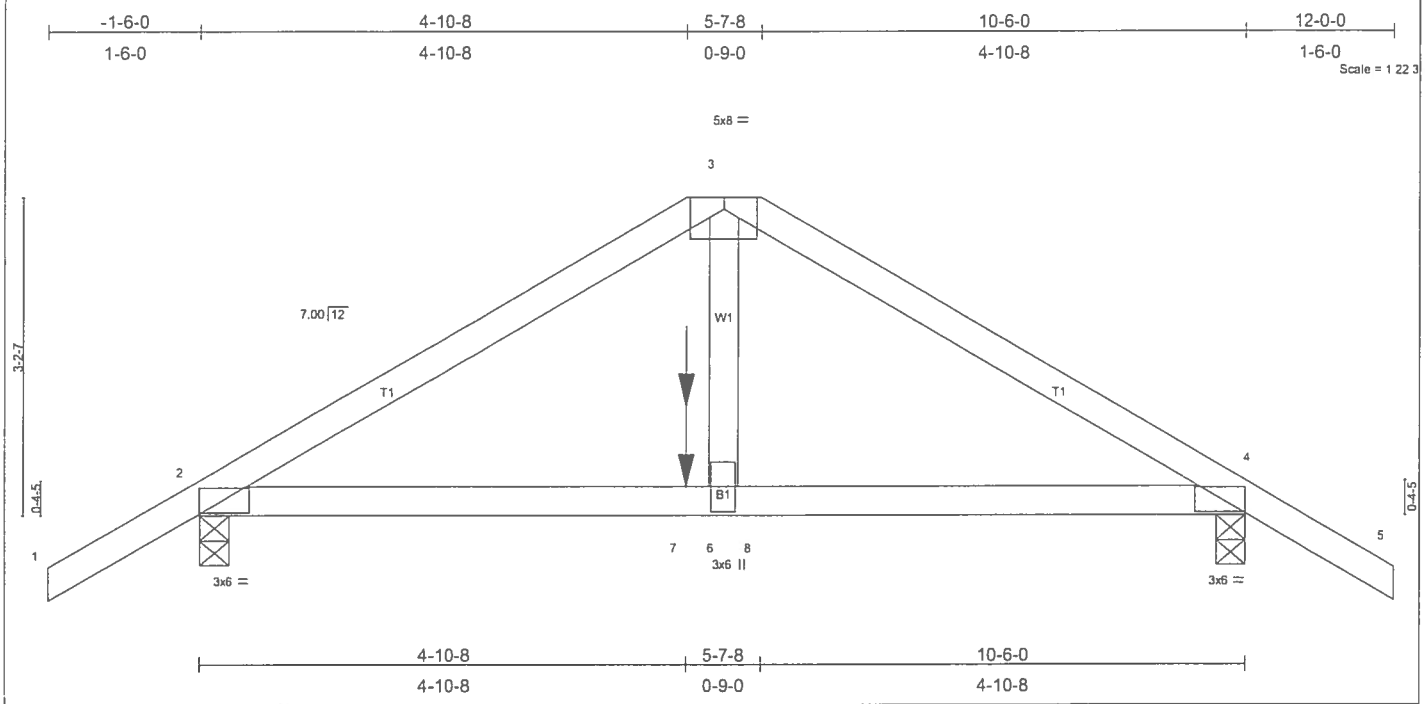


Plate Offsets (X,Y): [2:0-3-3,0-1-8], [4:0-3-3,0-1-8]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.21 | Vert(LL) | -0.04 | 2-6 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.50 | Vert(TL) | -0.06 | 2-6 | >999 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | NO | WB | 0.32 | Horz(TL) | 0.02 | 4 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 43 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 5-4-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 2=928/0-3-8, 4=928/0-3-8
Max Horz 2=-112(load case 2)
Max Uplift2=-391(load case 4), 4=-391(load case 5)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-1307/443, 3-4=-1307/443, 4-5=0/40
BOT CHORD 2-7=-302/1062, 6-7=-302/1062, 6-8=-302/1062, 4-8=-302/1062
WEBS 3-6=-278/987

**JOINT STRESS INDEX**
2 = 0.57, 3 = 0.50, 4 = 0.57 and 6 = 0.32

**NOTES**
1) Unbalanced roof live loads have been considered for this design
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 391 lb uplift at joint 2 and 391 lb uplift at joint 4.
5) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 410 lb down and 155 lb up at 4-10-8, and 410 lb down and 155 lb up at 5-7-8 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

**LOAD CASE(S)** Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-5=-54, 2-4=-30
Concentrated Loads (lb)
Vert: 7=-410(F) 8=-410(F)


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16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549

Dwg.#0103071089

| Job | Truss | Truss Type | Qty | Ply | ZECHER ADDITION |
|---|---|---|---|---|---|
| L221086 | T39 | MONO HIP | 2 | 1 | Job Reference (optional) |

Builders FirstSource, Lake City, FI 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 03 09:48:56 2007 Page 1

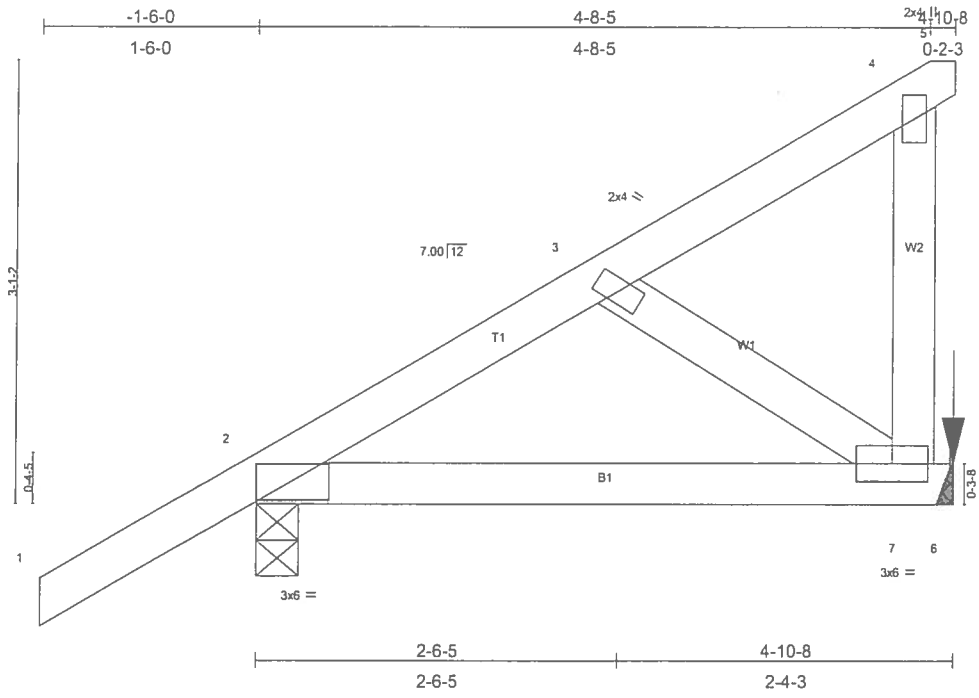


Plate Offsets (X,Y): [2:0-3-3,0-1-8]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.16 | Vert(LL) | -0.02 | 2-7 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.17 | Vert(TL) | -0.04 | 2-7 | >999 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | NO | WB | 0.02 | Horz(TL) | 0.00 | 7 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 26 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 4-10-8 oc purlins
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 2=292/0-3-8, 7=410/Mechanical
Max Horz 2=184(load case 4)
Max Uplift2=-144(load case 4), 7=-234(load case 4)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-137/2, 3-4=-43/21, 4-5=-1/0
BOT CHORD 2-7=-68/76, 6-7=0/0
WEBS 3-7=-91/81, 4-7=-57/65

**JOINT STRESS INDEX**
2 = 0.11, 3 = 0.04, 4 = 0.03 and 7 = 0.04

**NOTES**
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 144 lb uplift at joint 2 and 234 lb uplift at joint 7.
4) Girder carries hip end with 0-0-0 right side setback, 4-10-0 left side setback, and 5-0-0 end setback.
5) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 237 lb down and 138 lb up at 4-10-8 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

**LOAD CASE(S)** Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-5=-14, 2-7=-30
Concentrated Loads (lb)
Vert: 7=-237(F)

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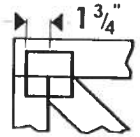
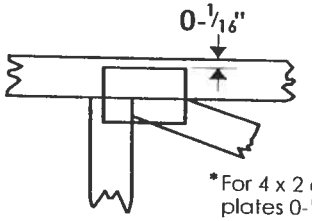
# Symbols

## PLATE LOCATION AND ORIENTATION




*Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and securely seat.




*For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

*This symbol indicates the required direction of slots in connector plates.

***Plate location details available in MiTek 20/20 software or upon request.**

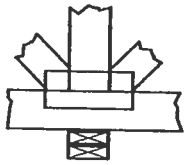
## PLATE SIZE

4 x 4 — The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

## LATERAL BRACING

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

## BEARING

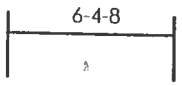


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

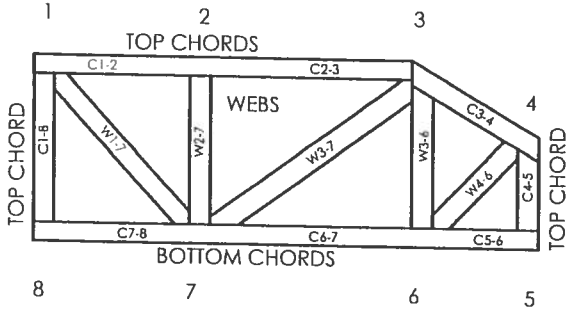
**Industry Standards:**

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI1: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

# Numbering System




dimensions shown in ft-in-sixteenths




**JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.**

**CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.**

## CONNECTOR PLATE CODE APPROVALS

| | |
|---|---|
| BOCA | 96-31, 95-43, 96-20-1, 96-67, 84-32 |
| ICBO | 4922, 5243, 5363, 3907 |
| SBCCI | 9667, 9730, 9604B, 9511, 9432A |

MiTek®

**MiTek Engineering Reference Sheet: MII-7473**

# General Safety Notes

**Failure to Follow Could Cause Property Damage or Personal Injury**

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI1.
2. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
3. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
4. Cut members to bear tightly against each other.
5. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI1.
6. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI1.
7. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
8. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
9. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
10. Plate type, size, orientation and location dimensions shown indicate minimum plating requirements.
11. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
12. Top chords must be sheathed or purlins provided at spacing shown on design.
13. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
14. Connections not shown are the responsibility of others.
15. Do not cut or alter truss member or plate without prior approval of a professional engineer.
16. Install and load vertically unless indicated otherwise.

25467

THIS INSTRUMENT WAS PREPARED BY:
FIRST FEDERAL SAVINGS BANK OF FLORIDA
4705 WEST U.S. HIGHWAY 90
P.O. BOX 2029
LAKE CITY, FLORIDA 32056

PERMIT NO._____ TAX FOLIO NO._____

## NOTICE OF COMMENCEMENT

STATE OF FLORIDA
COUNTY OF Columbia

The undersigned hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

1. Description of property: Lots 4 and 5 of SWEET GUM ESTATES, according to the Plat thereof as recorded in Plat Book 6, Page 43, of the public records of Columbia County, Florida.

2. General description of improvement: Construction of Dwelling

3. Owner information:
   a. Name and address: BRYAN C. ZECHER and MELISSA W. ZECHER
   639 SW CR 349, Lake City, FL 32024

   b. Interest in property: Fee Simple

   c. Name and address of fee simple title holder (if other than Owner): NONE

4. Contractor (name and address): Bryan Zecher Construction, Inc.
   P.O. Box 815, Lake City, FL 32056

5. Surety:
   a. Name and address: _____

   b. Amount of bond: _____

6. Lender: FIRST FEDERAL SAVINGS BANK OF FLORIDA
   4705 WEST U.S. HIGHWAY 90
   P. O. BOX 2029
   LAKE CITY, FLORIDA 32056

7. Persons within the State of Florida designated by Owner upon whom notices or other document may be served as provided by Section 713.13 (1) (a) 7., Florida Statutes: NONE

8. In addition to himself, Owner designates PAULA HACKER of FIRST FEDERAL SAVINGS BANK OF FLORIDA, 4705 West U.S. Highway 90 / P. O. Box 2029, Lake City, Florida 32056 to receive a copy of the Lienor's Notice as provided in Section 713.13 (1) (b), Florida Statutes.

9. Expiration date of notice of commencement (the expiration date is 1 year from the date of recording unless a different date is specified).

Borrower Name

Melissa W. Zecher
Co-Borrower Name

The foregoing instrument was acknowledged before me this 29th day of January, 2007 by Bryan C. & Melissa W. Zecher, who is personally known to me or who has produced driver's license for identification.

Notary Public
My Commission Expires:

TERRY MCDAVID
MY COMMISSION # DD 500788
EXPIRES: January 16, 2010
Bonded Thru Notary Public Underwriters