D 12/15/2008

### Columbia County Building Permit

This Permit Must Be Prominently Posted on Premises During Construction

PERMIT 000027521

APPLICANT DAN NICKELSON	PHONE 867-5616
ADDRESS P.O. BOX 3631	LAKE CITY FL 32056
OWNER DAN NICKELSON	PHONE 867-5616
ADDRESS 440 SW EMORYWOOD GLEN	LAKE CITY FL 32024
CONTRACTOR DAN NICKELSON	PHONE 867-5616
LOCATION OF PROPERTY 47S, TL LITTLE RD., TL	EMORYWOOD, 4TH LOT ON RIGHT
TYPE DEVELOPMENT SFD,UTILITY	ESTIMATED COST OF CONSTRUCTION 308800.00
HEATED FLOOR AREA 3600.00 TOTA	AL AREA 6176.00 HEIGHT STORIES 2
FOUNDATION CONC WALLS FRAMED	ROOF PITCH 12/12 FLOOR SLAB
LAND USE & ZONING A-3	MAX. HEIGHT 39
Minimum Set Back Requirments: STREET-FRONT	30.00 REAR 25.00 SIDE 25.00
NO. EX.D.U. 0 FLOOD ZONE X PP	DEVELOPMENT PERMIT NO.
PARCEL ID 01-5S-16-03397-107 SUBD	DIVISION COVE AT ROSE CREEK
	NIT TOTAL ACRES 5.01
000001695	Dulghur
Culvert Permit No. Culvert Waiver Contractor's Lices CULVERT 08-0753 Bb	2007 ★ 1 ★ 100 ×
	& Zoning checked by Approved for Issuance New Resident
	as assuming should be a second of the second
COMMENTS: ELEVATION CONFIRMATION LETTER REQ	QUIRED AT SLAB,MITE AT 80.3
NOC ON FILE	Check # or Cash 12386
FOR RIVIN DING 9	ZONING DEDARTMENT ONLY
	(Tooler Stab)
Temporary Power Foundation  date/app. by	date/app. by Monolithic date/app. by
Under slab rough-in plumbing	Slab Sheathing/Nailing
date/app. by	date/app. by date/app. by
	mbing above slab and below wood floor
date/app. by	date/app. by
Electrical rough-in Heat & Air D	
date/app. by	date/app. by
Permanent power C.O. Final date/app. by	date/app. by Culvert date/app. by
M/H tie downs, blocking, electricity and plumbing	Pool
	date/app. by date/app. by
Reconnection Pump pole	Utility Pole
date/app. by M/H Pole Travel Trailer	date/app. by  Re-roof
date/app. by	date/app. by date/app. by
BUILDING PERMIT FEE \$ 1545.00 CERTIFICAT	TION FEE \$ 30.88 SURCHARGE FEE \$ 30.88
MISC. FEES \$ 0.00 ZONING CERT. FEE \$	50.00 FIRE FEE \$ 0.00 WASTE FEE \$
FLOOD DEVELOPMENT FEE \$ FLOOD ZONE FEE	25.00 CULVERT FEE \$ 25.00 TOTAL FEE 1706.76
INSPECTORS OFFICE	CLERKS OFFICE

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED TO BE IN ACTIVE PROGESS WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

existing well- no well letter

Page 1 of 2 (Both Pages must be submitted together.)

Columbia	County	Building	Permit	Application
Columbia	County	Dunumy	Leimir	Application

/
For Office Use Only Application # 08/2-09 Date Received 12-5-08 By CH Permit # 1695/2752
Zoning Official But Date 2-12-08 Flood Zone Control Land Use A-3 Zoning A-3
FEMA Map # NA Elevation NA MFE 80.5 River NA Plans Examiner Date 12/15/0
Comments Elevation Consignation Letto required at state
NOC FEN Deed or PA Site Plan - State Road Info - Parent Parcel #
= Dev Permit # = In Floodway :: Letter of Auth. from Contractor = F W Comp. letter
IMPACT FEES: EMS 29.88 Fire 78.63 Corr 409.16 Road/Code 1,046.41/210
School_ 41,500.00 = TOTAL_ 43,063.67
Septic Permit No. 08-0753 Fax
Name Authorized Person Signing Permit Dan Nickelson builder Phone 867-5616
Address P.O. Box 3631 Cake GYPL 32056
Owners Name Dan & Gail Midelson Phone 867-5616
911 Address 440 SW Emorywood Glen Lake GYPL 32024
Contractors Name Owner Builder Dan Nidlelson Phone 867-5616
Address P.D. Box 3631 Lale City FL 320S6
Fee Simple Owner Name & Address
Bonding Co. Name & Address NH
Architect/Engineer Name & Address Daniel Shahen Mick Geisler
Mortgage Lenders Name & Address Columbia Bank
Circle the correct power company – FL Power & Light – Clay Elec. – Suwannee Valley Elec. – Progress Energy
Property ID Number 0-55-16-03397-107 Estimated Cost of Construction 350 K
Subdivision Name Core at Rose Creek Lot 7 Block Unit Phase  Driving Directions State Road 47 S. L on Little Road, Lon Emorywood
Driving Directions State Road 47 S. L on Little Road, Lon Emorywood
Glem Lot on R 4th lot on Right
Number of Existing Dwellings on Property
Construction of Single Family duelling Total Acreage 5.01 Lot Size 5.01
Do you need a Culvert Permit or Culvert Waiver or Have an Existing Drive Total Building Height 39
Actual Distance of Structure from Property Lines - Front 20 Side 200 Side 289 Rear 220
Number of Stories 2 Heated Floor Area 3600 Total Floor Area 6176 Roof Pitch 12-12
Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

Columbia County Building Permit Application

infrastructures and facilities has been corrected.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment

According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your properly, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE: YOU ARE HEREBY NOTIFIED as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public

OWNERS CERTIFICATION: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning. I further understand the above written responsibilities in Columbia County for obtaining this Building Permit.

Owners Signature	Lest	NOTARY PUBLIC-STATE OF FLORIDA Linda R. Roder Commission # DD755608 Expires: MAR. 24, 2012 BONDED THRU ATLANTIC BONDING CO., INC.
CONTRACTORS AFFIDAVIT: By my signature	I understand	and agree that I have informed and provided this
written statement to the owner of all the ab	ove written r	esponsibilities in Columbia County for obtaining
this Building Permit.		190
Contractor's Signature (Permitee)	<u> </u>	Contractor's License Number CBC / 2539 Columbia County Competency Card Number
		bscribed before me this 4 day of Dec 2008
Personally known or Produced Identificat	ion	
State of Florida Notary Signature (For the Contr		NOTARY PUBLIC-STATE OF FLORIDA  Linda R. Roder  Commission # DD755608  Expires: MAR. 24, 2012
Page 2 of 2 (Both Pages must be submitted tog	ether.)	BONDED THRU ATLANTIC BONDING CO., INC. Revised 11-30-07

FROM : COLUMBIA CO BUILDING + ZONING

FAX NO. :386-758-2160

Nov. 30 2007 10:23AM P1



### COLUMBIA COUNTY BUILDING DEPARTMENT

135 NE Hernando Ave., Suite B-21

Lake City, FL 32055

Office: 386-758-1008 Fax: 386-758-2160

NOTARIZED DISCLOSURE STATEMENT

FOR OWNER/BUILDER WHEN ACTING AS THER OWN CONTRACTOR AND CLAIMING EXEMPTION OF CONTRACTOR LICENSING REQUIREMENTS IN ACCORDANCE WITH FLORIDA STATUTES, ss. 489.103(7).

State law requires construction to be done by licensed contractors. You have applied for a permit under an exemption to that law. The exemption allows you, as the pwner of your property, to act as your own contractor with certain restrictions even though you do not have a license. You must provide direct, onsite supervision of the construction yourself. You may build or improve a one-family or two-family residence or a farm outbuilding. You may also build or improve a commercial building, provided your costs do not exceed \$75,000. The building or residence must be for your own use or occupancy. It may not be built or substantially improved for sale or lease. If you sell or lease a building you have built or substantially improved for yourself within 1 year after the construction is complete, the law will presume that you built or substantially improved it for sale or lease, which is a violation of this exemption. You may not hire an unlicensed person to act as your contractor or to supervise people working on your building. It is your responsibility to make sure that people employed by you have licenses required by state law and by county or municipal licensing ordinances. You may not delegate the responsibility for supervising work to a licensed contractor who is not licensed to perform the work being done. Any person working on your building who is not licensed must work under your direct supervision and must be employed by you, which means that you must deduct F. I.C.A. and withholding tax and provide workers' compensation for that employee, all as prescribed by law. Your construction must comply with all applicable laws, ordinances, building codes, and zoning regulations.

I understand that if I am not physically doing the work or physically supervising free labor from friends or relatives, that I must hire licensed contractors, i.e. electrician, plumber, mechanical (heating & air conditioning), etc. I further understand that the violation of not physically doing the work, and the use of unlicensed contractors at the construction site, will cause the project to be shut down by the inspection staff of the Columbia County Building Department. Additionally, state statutes allows for additional penalties. I also understand that if this violation does occur, that in order for the job to proceed, I will have a licensed contractor come in and obtain a new permit as taking the job over. I understand that if I hire subcontractors under a contract price, that they must be licensed to work in Columbia County, i.e. masonry, drywall, carpentry. Contractors licensed by the Columbia County Contractor Licensing Section or the State of Florida are required to have worker's compensation and liability coverage.

	TYPE OF	CONSTRUCTION		
M Single Family Dwelling	() Two-Farr	ily Residence	() Farm Outbe	uilding
() Other		Alteration, Modificat		37
	( ) Additions	riteration, mountain	don or other improv	cincin
1 Daniel Wickelson	, have	been advised of the a	bove disclosure state	ement for exemption
from contractor licensing as an owner/build	er. I agree to d	omply with all require	ements provided for	in Florida Statutes
ss.489.103(7) allowing this exception for the				
Permit Number	Construction	ser will control	County building	1.1.
COME HOUNDED		clark	(ollie	12/4/08
				1-11100
		Owner Builder Si	gnature	Date
FLORIDA NOTARY				<b>9 %</b>
The above signer is personally known to me	or produced is	lentification	gg. Asperson.	·
Notary Signature Links feel	7 Date	12-4-08	NOTARY PURITC-STAT	E OF FLORIDA
				R. Roder
			5 B . W.	#DD755608
FOR BUILDING DEPARTMENT USE ONLY				FAR. 24, 2012
I hereby certify that the above listed owner/	builder has be	en notified of the disc	closure statement in	5kgrideoStatutes
55000 FACE SECTION (1997) 1997 1997 1997 1997 1997 1997 1997		epresentative		
Bui	THIS CHICION	ebi eselitative		

Dan & Gail Nickelson Lot 7 Cove at Rose Creek 61-55-16-03397-107 358.39 S.W. Emarywood Soot 359 38' S)es 120 90'-2" 220 5791 dring الر 00 h2-4501

Z

PREPARED BY AND RETURN TO:

TERRY McDAVID POST OFFICE BOX 1328 LAKE CITY, FL 32056-1328

Property Appraiser's Identification Number R03397-018 Inst:2006023777 Date:10/04/2006 Time:13:13

Doc Stamp\_Deed: 875.00

\_\_\_\_\_DC,P.DeWitt Cason,Columbia County B:1098 P:537

TM File No: 06-419

### WARRANTY DEED

This Warranty Deed, made this 3rD day of October, 2006, BETWEEN WESTFIELD INVESTMENT GROUP, LLLP, a Florida Limited Liability Limited Partnership, f/k/a WESTFIELD GROUP, LTD., whose post office address is P.O. Box 3566, Lake City, FL 32056, of the County of Columbia, State of Florida, grantor\*, and DANIEL JAY NICKELSON AND GAIL F. NICKELSON, Husband and Wife whose post office address is P.O. Box 3631, Lake City, Florida 32056, of the County of Columbia, State of Florida, grantee\*.

(Whenever used herein the terms "grantor" and "grantee" include all the parties to this instrument and the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporations, trusts and trustees)

Witnesseth: that said grantor, for and in consideration of the sum of Ten Dollars (\$10.00), and other good and valuable considerations to said grantor in hand paid by said grantee, the receipt whereof is hereby acknowledged, has granted, bargained and sold to the said grantee, and grantee's heirs and assigns forever, the following described land, situate, lying and being in Columbia County, Florida, to-wit:

Lot 7, Cove at Rose Creek, a subdivision according to the plat thereof recorded in Plat Book 8, Page 107-109, public records, Columbia County, Florida.

Together with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

To Have and to Hold, the same in fee simple forever.

And subject to taxes for the current year and later years and all valid easements and restrictions of record, if any, which are not hereby reimposed; and also subject to any claim, right, title or interest arising from any recorded instrument reserving, conveying, leasing, or otherwise alienating any interest in the oil, gas and other minerals. And grantor does warrant the title to said land and will defend the same against the lawful claims of all persons whomsoever, subject only to the exceptions set forth herein.

In Witness Whereof, grantor has hereunto set grantor's hand and seal the day and year first above written.

Signed, sealed and delivered WESTFIELD INVESTMENT GROUP, in our presence: LLLP Scott D. Stewart, General Partner Charles S Sparks, Partner

(Typed Name of Second Witness)

STATE OF FLORIDA COUNTY OF COLUMBIA

The foregoing instrument was acknowledged before me this day of October, 2006, by Scott D. Stewart and Charles S. Sparks, General Partners of Westfield Investment Group, LLLP, a Florida Limited Liability Limited Partnership, on behalf of said partnership, who is/are personally known to me or who has/have as identification and who did not take produced an oath.

My Commission Expires:

Notary Public Printed, typed, or stamped name:

(SEAL)

General



Inst:2006023777 Date:10/04/2006 Time:13:13

Doc Stamp-Deed: 875.00

\_DC,P.Dewitt Cason,Columbia County B:1098 P:538

Inst. Number: 200812021952 Book: 1163 Page: 1197 Date: 12/5/2008 Time: 4: 7:00 PM Page 1 of 1 Prepared By & Rehen To: MATT ROCCA 0812-09 Sierra Title, LLC 619 SW Baya Dr., Ste 102 Lake City, FL 32025 #08-0504 Permit Number: Tax Folio Number: 01-55-State of: Florida County of: Columbia File Number: 08-0504 NOTICE OF COMMENCEMENT The undersigned hereby gives notice that improvement will be made to certain real property, and, in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement. Description of Property: All of Lot 7, Cove at Rose Creek, a subdivision according to the plat thereof as recorded in Plat Book 8, Pages 107 through 109, of the Public Records of Columbia County, Florida. 2. General Description of Improvements: Construction of Single Family Residence 3. Name and Address: Daniel Jay Nickelson and Gail F. Nickelson PO Box 3631, Lake City, FL 32056 Interest in property: Fee Simple ь. Names and address of fee simple title holder (if other than owner): Daniel J. Nickelson, PO Box 3631, Lake City, FL 32056 Contractor: 5. Surety: Lender: Columbia Bank, PO Box 1609, Lake City, Florida 32056 Persons within the State of Plorida designated by Owner upon whom notices or other documents may be served as provided by Section 713.13(1) (a)7., Florida Statutes. In addition to himself, Owner designates the following persons to receive a copy of the Lienor's Notice as provided in Section 713.13(1)(b), Florida Statutes. 9. Expiration date of Notice of Commencement (the expiration date is 1 year from date of recording unless a different date is specified): . Daniel Jay Nickelson Gail F. Nickelson Sworn to and subscribed before me December 5, 2008 by Daniel Jay Nickelson and Gail F. Nickelson who are personally known to me or who did provide Notary Public My Commission Expires: . 43578349

Project Name:

**Dan and Gail Nickelson** 

Builder: Nickelson .

### FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs Residential Whole Building Performance Method A

Address: City, State:	SW Emory Woo Lake City, FI		Permitting Office: (o /u Permit Number: 275	21
Owner: Climate Zone:	Dan and Gail Nic North	ckelson	Jurisdiction Number: 2	221000
New constructi	on or existing	New	12. Cooling systems	
2. Single family o	r multi-family	Single family	a. Central Unit	Cap: 36.0 kBtu/hr
<ol><li>Number of unit</li></ol>	s, if multi-family	1 _		SEER: 14.00
<ol><li>Number of Bed</li></ol>	lrooms	3 _	b. N/A	_
<ol><li>Is this a worst of</li></ol>		Yes _		
<ol><li>Conditioned flo</li></ol>		3600 ft <sup>2</sup>	c. N/A	_
	d area: (Label reqd. by 13-1	104.4.5 if not default)		
<ul><li>a. U-factor:</li></ul>	Г	Description Area	<ol><li>Heating systems</li></ol>	100 mm
(or Single or D	ouble DEFAULT) 7a(Sng	gle Default) 511.6 ft <sup>2</sup>	a. LP Gas Heat Pump	Cap: 36.0 kBtu/hr
b. SHGC:				COP: 2.00
	int DEFAULT) 7b.	(Clear) 511.6 ft <sup>2</sup>	b. N/A	_
<ol><li>Floor types</li></ol>	3 (A)			
a. Raised Wood, A		R=3.5, 150.0ft <sup>2</sup>	c. N/A	_
b. Slab-On-Grade	Edge Insulation	R=0.0, 339.0(p) ft		_
c. N/A		-	14. Hot water systems	
9. Wall types	The same the same		a. Electric Resistance	Cap: 50.0 gallons
a. Frame, Wood, I		R=11.0, 1116.5 ft <sup>2</sup>	(4)	EF: 0.92
b. Frame, Wood, I	Exterior	R=0.0, 2397.9 ft <sup>2</sup>	b. N/A	_
c. N/A		· -	as see the same some services and the same services are same services and the same services and the same services are same services are same services are same services are same services and the same services are sa	-
d. N/A		_	c. Conservation credits	-
e. N/A		_	(HR-Heat recovery, Solar	
<ol> <li>Ceiling types</li> <li>Under Attic</li> </ol>		D 20 0 2050 0 03	DHP-Dedicated heat pump)	
b. N/A		R=30.0, 2958.0 ft <sup>2</sup>	15. HVAC credits	MZ-C, PT, CF, MZ-
c. N/A			(CF-Ceiling fan, CV-Cross ventilation,	
11. Ducts		ş.—	HF-Whole house fan,	
	Unc. AH: Garage	Sup. R=6.0, 125.0 ft	PT-Programmable Thermostat,	
b. N/A	One. All. Galage	5up. K-0.0, 123.0 Il	MZ-C-Multizone cooling, MZ-H-Multizone heating)	
*****		-	wiz-n-wuitizone neating)	
		÷—		
Gla	ass/Floor Area: 0.1	Total as-built ¡ Total base ¡	points: 43190 points: 43308 PASS	

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy

PREPARED BY: Mora & DATE: 12/4/08

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code

**OWNER/AGENT:** 

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908

Florida Statutes.

BUILDING OFFICIAL: \_

DATE:

### **SUMMER CALCULATIONS**

### Residential Whole Building Performance Method A - Details

ADDRESS: SW Emory Wood Glen, Lake City, FI,

PERMIT #:

BASE		AS-BUILT								
GLASS TYPES										
.18 X Conditioned X BSPM = Points Floor Area	Type/SC		rhang Len	Hgt	Area X	SPM X	SOF	= Points		
.18 3600.0 18.59 12046.0	1.Single, Clear	Е	0.0	0.0	12.0	47.92	1.00	575.0		
	2.Single, Clear	E		0.0		47.92	1.00	638.0		
	3.Single, Clear	E		0.0		47.92	1.00	838.0		
	4.Single, Clear	E		0.0		47.92	1.00	1437.0		
2	5.Single, Clear	Ξ		0.0	100000000	47.92	1.00	766.0		
	6.Single, Clear	E		0.0		47.92	1.00	790.0		
	7.Single, Clear	E		0.0		47.92	1.00	718.0		
	8.Single, Clear	E		0.0		47.92	1.00	1437.0		
a t	9.Single, Clear 10.Single, Clear	E E		0.0		47.92	1.00	766.0 790.0		
	11.Single, Clear	E	0.0	0.0		47.92 47.92	1.00	1629.0		
	12.Single, Clear	w	0.0	0.0	C. 178 C. School Co.	43.84	1.00	394.0		
	13.Single, Clear	w	0.0	0.0		43.84	1.00	3013.0		
	14.Single, Clear	w	0.0	0.0		43.84	1.00	2104.0		
	15.Single, Clear	W	0.0	0.0		43.84	1.00	2893.0		
	16.Single, Clear	N	0.0	0.0		21.73	1.00	260.0		
	17.Single, Clear	N	0.0	0.0		21.73	1.00	325.0		
	18.Single, Clear	s	0.0	0.0		40.81	1.00	1305.0		
	19.Single, Clear	S	0.0	0.0	44.0	40.81	1.00	1795.0		
	As-Built Total:				511.6			22473.0		
WALL TYPES Area X BSPM = Points	Туре		R-	Value	Area	X SP	M =	Points		
Adjacent 0.0 0.00 0.0	1. Frame, Wood, Exterior			11.0	1116.5	1.70	)	1898.1		
Exterior 3514.4 1.70 5974.5	2. Frame, Wood, Exterior			0.0	2397.9	5.50		13188.4		
Base Total: 3514.4 5974.5	As-Built Total:				3514.4			15086.5		
DOOR TYPES Area X BSPM = Points	Туре				Area	X SP	M =	Points		
Adjacent 0.0 0.00 0.0	1.Exterior Wood				48.0	6.10	)	292.8		
Exterior 208.0 6.10 1268.8	2.Exterior Wood				64.0	6.10	)	390.4		
	3.Exterior Wood				24.0	6.10	1	146.4		
	4.Exterior Wood				48.0	6.10	ì	292.8		
	5.Exterior Wood				24.0	6.10	ĺ	146.4		
Base Total: 208.0 1268.8	As-Built Total:				208.0			1268.8		
CEILING TYPES Area X BSPM = Points	Туре	F	R-Valu	ie A	Area X S	SPM X S	CM =	Points		
Under Attic 2958.0 1.73 5117.3	1. Under Attic		ş	30.0	2958.0 1	1.73 X 1.00		5117.3		
Base Total: 2958.0 5117.3	As-Built Total:				2958.0			5117.3		

### **SUMMER CALCULATIONS**

### Residential Whole Building Performance Method A - Details

ADDRESS: SW Emory Wood Glen, Lake City, FI, PERMIT #:

BASI		AS-BUILT							
FLOOR TYPES Area	X BSPM = Points	Туре	R-Value Area	X SPM	= Points				
Slab 339.0(p) Raised 150.0	-37.0 -12543.0 -3.99 -598.5	Raised Wood, Adjacent     Slab-On-Grade Edge Insulation	3.5 150.0 0.0 339.0(p	1.50 -41.20	225.0 -13966.8				
Base Total:	-13141.5	As-Built Total:	489.0		-13741.8				
INFILTRATION Area	X BSPM = Points		Area	X SPM	= Points				
3600.0	10.21 36756.0		3600.	0 10.21	36756.0				
Summer Base Poin	ts: 48021.1	Summer As-Built Points: 66959.8							
Total Summer X System Points Multiple		Component Ratio Mu	Duct X System > ultiplier Multiplier DSM x AHU)	Credit = Multiplier	Cooling Points				
48021.1 0.32	50 15606.9		FF(14.0) Ducts:Unc(S),Unc(R) x 1.147 x 1.00)	0.857	) 17497.8 <b>17497.8</b>				

### WINTER CALCULATIONS

### Residential Whole Building Performance Method A - Details

ADDRESS: SW Emory Wood Glen, Lake City, FI,

PERMIT #:

BASE					AS-	BUI	LT			
GLASS TYPES .18 X Conditioned X B Floor Area	WPM = P	oints	Type/SC	Ove Ornt	rhang Len	Hgt	Area X	WPM X	wo	F = Points
.18 3600.0	20.17	13070.0	1.Single, Clear	Е	0.0	0.0	12.0	26.41	1.00	316.0
			2.Single, Clear	E	0.0	0.0	13.3	26.41	1.00	352.0
			3.Single, Clear	E	0.0	0.0	17.5	26.41	1.00	462.0
			4.Single, Clear	Е	0.0	0.0	30.0	26.41	1.00	792.0
			5.Single, Clear	Е	0.0	0.0	16.0	26.41	1.00	422.0
			6.Single, Clear	Е	0.0	0.0	16.5	26.41	1.00	435.0
			7.Single, Clear	E	0.0	0.0	15.0	26.41	1.00	396.0
			8.Single, Clear	E	0.0	0.0	30.0	26.41	1.00	792.0
			9.Single, Clear	E	0.0	0.0	16.0	26.41	1.00	422.0
ľ			10.Single, Clear	E	0.0	0.0	16.5	26.41	1.00	435.0
			11.Single, Clear 12.Single, Clear	E W	0.0	0.0	34.0	26.41	1.00	897.0
			13.Single, Clear	W	0.0	0.0	9.0 68.8	28.84 28.84	1.00	259.0 1982.0
			14.Single, Clear	W	0.0	0.0	48.0	28.84	1.00	1384.0
			15.Single, Clear	w	0.0	0.0	66.0	28.84	1.00	1903.0
			16.Single, Clear	N	0.0	0.0	12.0	33.22	1.00	398.0
			17.Single, Clear	N	0.0	0.0	15.0	33.22	1.00	498.0
			18.Single, Clear	S	0.0	0.0	32.0	20.24	1.00	647.0
			19.Single, Clear	S	0.0	0.0	44.0	20.24	1.00	890.0
			As-Built Total:				511.6			13682.0
WALL TYPES Area X	BWPM =	Points	Туре		R-	Value	Area	X · WPN	1 =	Points
Adjacent 0.0	0.00	0.0	1. Frame, Wood, Exterior		8	11.0	1116.5	3.70		4131.1
Exterior 3514.4	3.70	13003.3	2. Frame, Wood, Exterior			0.0	2397.9	11.10		26616.7
Base Total: 3514.4		13003.3	As-Built Total:				3514.4	8		30747.7
DOOR TYPES Area X	BWPM =	Points	Туре				Area	X WPM	1 =	Points
Adjacent 0.0	0.00	0.0	1.Exterior Wood				48.0	12.30		590.4
Exterior 208.0	12.30	2558.4	2.Exterior Wood				64.0	12.30		787.2
			3.Exterior Wood				24.0	12.30		295.2
			4.Exterior Wood				48.0	12.30		590.4
		Southern Committee Committee	5.Exterior Wood				24.0	12.30		295.2
Base Total: 208.0		2558.4	As-Built Total:				208.0			2558.4
CEILING TYPES Area X	BWPM =	Points	Туре	R-	Value	Ar	ea X W	PM X WC	= M	Points
Under Attic 2958.0	2.05	6063.9	1. Under Attic		;	30.0	2958.0 2	2.05 X 1.00		6063.9
Base Total: 2958.0		6063.9	As-Built Total:				2958.0			6063.9

### WINTER CALCULATIONS

### Residential Whole Building Performance Method A - Details

ADDRESS: SW Emory Wood Glen, Lake City, FI, PERMIT #:

	BASE			AS-BUILT						
FLOOR TYPE	S Area X	BWPM	= Points	Type R-Value Area X	WPM	=	Points			
Slab Raised	339.0(p) 150.0	8.9 0.96	3017.1 144.0	1. Raised Wood, Adjacent 3.5 150.0 2. Slab-On-Grade Edge Insulation 0.0 339.0(p	7.40 18.80		1110.0 6373.2			
Base Total:			3161.1	As-Built Total: 489.0			7483.2			
INFILTRATION	N Area X	BWPM	= Points	Area X	WPM	=	Points			
	3600.0	-0.59	-2124.0	3600.0	-0.59		-2124.0			
Winter Base	Points:	3	5732.7	Winter As-Built Points: 58411.2						
Total Winter X Points	System Multiplier		nting Points		redit : ultiplier		leating Points			
35732.7	0.5540	0 1	9795.9		nc(R),Gar 0.902 <b>).902</b>	1	R6.0 7786.9 <b>786.9</b>			

### **WATER HEATING & CODE COMPLIANCE STATUS**

Residential Whole Building Performance Method A - Details

ADDRESS: SW Emory Wood Glen, Lake City, FI, PERMIT #:

	BASE	AS-BUILT												
WATER HEATING  Number of X Multiplier = Total  Bedrooms					Tank Volume	EF	EF Number of X Tank X Multiplier X Cred Bedrooms Ratio Multip					= To	ital	
3		2635.00		7905.0	50.0	0.92	3		1.00	2635.00	3	1.00	790	05.0
					As-Built To	otal:							790	05.0

	CODE COMPLIANCE STATUS												
8	BASE							AS-BUILT					
Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points	Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points
15607 19796 7905 43308 17498 17787 7905 43									43190				

**PASS** 



### **Code Compliance Checklist**

### Residential Whole Building Performance Method A - Details

ADDRESS: SW Emory Wood Glen, Lake City, FI,

PERMIT #:

### 6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members.  EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3		
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

### 6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked cir breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610.  Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation 604.1, 602.1 Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides.  Common ceiling & floors R-11.			

# ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

### ESTIMATED ENERGY PERFORMANCE SCORE\* = 89.8

The higher the score, the more efficient the home.

Dan and Gail Nickelson, SW Emory Wood Glen, Lake City, FI,

1. 2. 3. 4. 5.	New construction or existing Single family or multi-family Number of units, if multi-family Number of Bedrooms Is this a worst case? Conditioned floor area (ft²)	New Single family 1 3 Yes 3600 ft <sup>2</sup>	=	<ul><li>12. Cooling systems</li><li>a. Central Unit</li><li>b. N/A</li><li>c. N/A</li></ul>	Cap: 36.0 kBtu/hr SEER: 14.00	
7. a. b. 8. a. b. c. 9. a. b. c. d.	Glass type <sup>1</sup> and area: (Label reqd. b U-factor: (or Single or Double DEFAULT) SHGC:	by 13-104.4.5 if not default)  Description Area		a. LP Gas Heat Pump b. N/A c. N/A 14. Hot water systems a. Electric Resistance b. N/A c. Conservation credits	Cap: 36.0 kBtu/hr COP: 2.00 Cap: 50.0 gallons EF: 0.92	
10. a. b. c. 11. a.	Ceiling types Under Attic N/A N/A N/A Ducts Sup: Unc. Ret: Unc. AH: Garage N/A	R=30.0, 2958.0 ft <sup>2</sup> Sup. R=6.0, 125.0 ft	=	(HR-Heat recovery, Solar DHP-Dedicated heat pump)  5. HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)	MZ-C, PT, CF, MZ-	2 <b>3—</b> -2
Con in th base Buil	rtify that this home has complied struction through the above energis home before final inspection and on installed Code compliant for Signature:	rgy saving features which . Otherwise, a new EPL I features.	h will be	installed (or exceeded) Card will be completed	COD WE THIS STATE OF THE STATE	AUNIDA

\*NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.

<sup>1</sup> Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4. EnergyGauge® (Version: FLRCPB v4.5.2)

## **Columbia County Building Department Culvert Permit**

### Culvert Permit No.

000001695

DATE $12/1$	5/2008 PARCEL ID	# <u>01-5S-16-03397-107</u>	
APPLICANT	DAN NICKELSON	PHONE	867-5616
ADDRESS _	P.O. BOX 3631	LAKE CITY	FL 32056
OWNER DA	AN & GAIL NICKELSON	PHONE	867-5616
ADDRESS 4	40 SW EMORYWOOD GLEN	LAKE CITY	FL 32024
CONTRACTO	R DAN NICKELSON	PHONE	867-5616
LOCATION O	F PROPERTY 47S, TL ON LITTLE RO	AD, TL ON EMORYWOOD, 4	TH LOT ON
SUBDIVISION	/LOT/BLOCK/PHASE/UNIT COVE	AT ROSE CREEK	7
SIGNATURE	Signature o	nrile	
X	INSTALLATION REQUIREMENT Culvert size will be 18 inches in diamodriving surface. Both ends will be mit thick reinforced concrete slab.  INSTALLATION NOTE: Turnouts with a majority of the current and exist b) the driveway to be served will be a Turnouts shall be concrete or payon concrete or payed driveway, which current and existing payed or concrete o	eter with a total lenght of 3 ered 4 foot with a 4 : 1 slop  Il be required as follows: sting driveway turnouts are paved or formed with conved a minimum of 12 feet with ever is greater. The width	pe and poured with a 4 inch paved, or; crete. wide or the width of the
	Culvert installation shall conform to	the approved site plan sta	ndards.
	Department of Transportation Permi	t installation approved star	ndards.
	Other	*	
			The second secon

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED DURING THE INSTALATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21 Lake City, FL 32055

Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00



# COLUMBIA COUNTY BUILDING DEPARTMENT RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR THE FLORIDA RESIDENTIAL BUILDING CODE 2004 with 2005 & 2006 Supplements and One (1) and Two (2) Family Dwellings

ALL REQUIREMENTS ARE SUBJECT TO CHANGE

ALL BUILDING PLANS MUST INDICATE COMPLIANCE with the Current FLORIDA BUILDING CODES and the Current FLORIDA RESIDENTIAL CODE. ALL PLANS OR DRAWING SHALL PROVIDED CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS.

FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEEDS ARE PER FIGURE R301.2(4) of the Residential Code (Florida Wind speed map) SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

- 1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ----- 100 MPH
- 2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE -----110 MPH
- 3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

### **GENERAL REQUIREMENTS:**

- Two (2) complete sets of plans containing the following:
- All drawings must be clear, concise and drawn to scale, details that are not used shall be marked void
- Condition space (Sq. Ft.) and total (Sq. Ft.) under roof shall be shown on the plans.
- Designers name and signature shall be on all documents and a licensed architect or engineer, signature and official embossed seal shall be affixed to the plans and documents per FBC 106.1.

### Site Plan information including:

- O Dimensions of lot or parcel of land
- Dimensions of all building set backs
- Location of all other structures (include square footage of structures) on parcel, existing or proposed well and septic tank and all utility easements.
- Provide a full legal description of property.

### Wind-load Engineering Summary, calculations and any details required:

- Plans or specifications must meet state compliance with FRC Chapter 3
- The following information must be shown as per section FRC
- Basic wind speed (3-second gust), miles per hour
- Wind importance factor and nature of occupancy
- Wind exposure if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated
- The applicable internal pressure coefficient, Components and Cladding The design wind pressure in terms of psf (kN/m²), to be used for the design of exterior component and cladding materials not specifally designed by the registered design professional.

### **Elevations Drawing including:**

- All side views of the structure
- Roof pitch
- Overhang dimensions and detail with attic ventilation
  - Location, size and height above roof of chimneys
- Location and size of skylights with Florida Product Approval
- Number of stories
- e) Building height from the established grade to the roofs highest peak

F	oor Plan including:
0/	Dimensioned area plan showing rooms, attached garage, breeze ways, covered porches, deck,
1	balconies and raised floor surfaces located more than 30 inches above the floor or grade
6	All exterior and interior shear walls indicated
6	Shear wall opening shown (Windows, Doors and Garage doors
0	Emergency escape and rescue opening in each bedroom (net clear opening shown)
6	Safety glazing of glass where needed
0	Fireplaces types (gas appliance) (vented or non-vented) or wood burning with Hearth (see chapter 10 of FRC)
0	Stairs with dimensions (width, tread and riser and total run) details of guardrails, Handrails (see FRC 311)
0	Plans must show and identify accessibility of bathroom (see FRC 322)
Al	materials placed within opening or onto/into exterior shear walls, soffits or roofs shall have Florida
pro	educt approval number and mfg, installation information submitted with the plans (see Florida product
ap	proval form)
<u>F</u>	oundation Plans Per FRC 403:
9	a) Location of all load-bearing walls footings indicated as standard, monolithic, dimensions, size and
	type of reinforcing.
0	b) All posts and/or column footing including size and reinforcing
6	c) Any special support required by soil analysis such as piling.
0	d) Assumed load-bearing valve of soil (psf)
0	e) Location of horizontal and vertical steel, for foundation or walls (include # size and type)
	, , , , , , , , , , , , , , , , , , ,
C	ONCRETE SLAB ON GRADE Per FRC R506
0	Show Vapor retarder (6mil. Polyethylene with joints lapped 6 inches and sealed)
0	Show control joints, synthetic fiber reinforcement or welded fire fabric reinforcement and Supports
	simple and supports
р	ROTECTION ACAINST TERMITES DOWED C 220.
	ROTECTION AGAINST TERMITES Per FRC 320:
<u>P</u>	Indicate on the foundation plan if soil treatment is used for subterranean termite prevention or submit
	ROTECTION AGAINST TERMITES Per FRC 320:  Indicate on the foundation plan if soil treatment is used for subterranean termite prevention or submit other approved termite protection methods. Protection shall be provided by registered termiticides
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M:	Indicate on the foundation plan if soil treatment is used for subterranean termite prevention or submit other approved termite protection methods. Protection shall be provided by registered termiticides  asonry Walls and Stem walls (load bearing & shear Walls) FRC Section R606  Show all materials making up walls, wall height, and Block size, mortar type  Show all Lintel sizes, type, spans and tie-beam sizes and spacing of reinforcement
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Me En	Indicate on the foundation plan if soil treatment is used for subterranean termite prevention or submit other approved termite protection methods. Protection shall be provided by registered termiticides  asonry Walls and Stem walls (load bearing & shear Walls) FRC Section R606  Show all materials making up walls, wall height, and Block size, mortar type Show all Lintel sizes, type, spans and tie-beam sizes and spacing of reinforcement tal frame shear wall and roof systems shall be designed, signed and sealed by Florida Prof.  sincer or Architect  Floor truss package shall including layout and details, signed and sealed by Florida Registered
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Me En	Indicate on the foundation plan if soil treatment is used for subterranean termite prevention or submit other approved termite protection methods. Protection shall be provided by registered termiticides  asonry Walls and Stem walls (load bearing & shear Walls) FRC Section R606  Show all materials making up walls, wall height, and Block size, mortar type Show all Lintel sizes, type, spans and tie-beam sizes and spacing of reinforcement tal frame shear wall and roof systems shall be designed, signed and sealed by Florida Prof.  gineer or Architect  Floor truss package shall including layout and details, signed and sealed by Florida Registered Professional Engineer Show conventional floor joist type size, span, spacing and attachment to load bearing walls, stem walls and/or priers
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Me En	Indicate on the foundation plan if soil treatment is used for subterranean termite prevention or submit other approved termite protection methods. Protection shall be provided by registered termiticides  asonry Walls and Stem walls (load bearing & shear Walls) FRC Section R606  Show all materials making up walls, wall height, and Block size, mortar type Show all Lintel sizes, type, spans and tie-beam sizes and spacing of reinforcement tal frame shear wall and roof systems shall be designed, signed and sealed by Florida Prof.  gineer or Architect  Floor truss package shall including layout and details, signed and sealed by Florida Registered Professional Engineer Show conventional floor joist type size, span, spacing and attachment to load bearing walls, stem walls and/or priers Girder type, size and spacing to load bearing walls, stem wall and/or priers Attachment of joist to girder Wind load requirements where applicable
Me En	Indicate on the foundation plan if soil treatment is used for subterranean termite prevention or submit other approved termite protection methods. Protection shall be provided by registered termiticides  asonry Walls and Stem walls (load bearing & shear Walls) FRC Section R606  Show all materials making up walls, wall height, and Block size, mortar type Show all Lintel sizes, type, spans and tie-beam sizes and spacing of reinforcement tal frame shear wall and roof systems shall be designed, signed and sealed by Florida Prof.  gineer or Architect  Floor truss package shall including layout and details, signed and sealed by Florida Registered Professional Engineer Show conventional floor joist type, size, span, spacing and attachment to load bearing walls, stem walls and/or priers Girder type, size and spacing to load bearing walls, stem wall and/or priers Attachment of joist to girder Wind load requirements where applicable Show required under-floor crawl space
ME En Skobb	Indicate on the foundation plan if soil treatment is used for subterranean termite prevention or submit other approved termite protection methods. Protection shall be provided by registered termiticides  asonry Walls and Stem walls (load bearing & shear Walls) FRC Section R606 Show all materials making up walls, wall height, and Block size, mortar type Show all Lintel sizes, type, spans and tie-beam sizes and spacing of reinforcement tal frame shear wall and roof systems shall be designed, signed and sealed by Florida Prof.  gineer or Architect  Floor truss package shall including layout and details, signed and sealed by Florida Registered Professional Engineer Show conventional floor joist type size, span, spacing and attachment to load bearing walls, stem walls and/or priers Girder type, size and spacing to load bearing walls, stem wall and/or priers Attachment of joist to girder Wind load requirements where applicable Show required under-floor crawl space Show required amount of ventilation opening for under-floor spaces
ME En	Indicate on the foundation plan if soil treatment is used for subterranean termite prevention or submit other approved termite protection methods. Protection shall be provided by registered termiticides  asonry Walls and Stem walls (load bearing & shear Walls) FRC Section R606  Show all materials making up walls, wall height, and Block size, mortar type Show all Lintel sizes, type, spans and tie-beam sizes and spacing of reinforcement tal frame shear wall and roof systems shall be designed, signed and sealed by Florida Prof.  aron Framing System: First and/or second story Floor truss package shall including layout and details, signed and sealed by Florida Registered Professional Engineer Show conventional floor joist type size, span, spacing and attachment to load bearing walls, stem walls and/or priers Girder type, size and spacing to load bearing walls, stem wall and/or priers Attachment of joist to girder Wind load requirements where applicable Show required under-floor crawl space Show required amount of ventilation opening for under-floor spaces Show required covering of ventilation opening.
ME En S S S S S S S S S S S S S S S S S S	Indicate on the foundation plan if soil treatment is used for subterranean termite prevention or submit other approved termite protection methods. Protection shall be provided by registered termiticides  asonry Walls and Stem walls (load bearing & shear Walls) FRC Section R606  Show all materials making up walls, wall height, and Block size, mortar type Show all Lintel sizes, type, spans and tie-beam sizes and spacing of reinforcement tal frame shear wall and roof systems shall be designed, signed and sealed by Florida Prof.  gineer or Architect  Floor truss package shall including layout and details, signed and sealed by Florida Registered Professional Engineer Show conventional floor joist type, size, span, spacing and attachment to load bearing walls, stem walls and/or priers  Girder type, size and spacing to load bearing walls, stem wall and/or priers  Attachment of joist to girder Wind load requirements where applicable Show required under-floor crawl space Show required amount of ventilation opening.  Show the required access opening to access to under-floor spaces
ME En	Indicate on the foundation plan if soil treatment is used for subterranean termite prevention or submit other approved termite protection methods. Protection shall be provided by registered termiticides  asonry Walls and Stem walls (load bearing & shear Walls) FRC Section R606  Show all materials making up walls, wall height, and Block size, mortar type Show all Lintel sizes, type, spans and tie-beam sizes and spacing of reinforcement tal frame shear wall and roof systems shall be designed, signed and sealed by Florida Prof.  gineer or Architect  Floor truss package shall including layout and details, signed and sealed by Florida Registered Professional Engineer Show conventional floor joist type size, span, spacing and attachment to load bearing walls, stem walls and/or priers Girder type, size and spacing to load bearing walls, stem wall and/or priers Attachment of joist to girder Wind load requirements where applicable Show required under-floor crawl space Show required amount of ventilation opening for under-floor spaces Show trequired access opening to access to under-floor spaces Show the sub-floor structural panel sheathing type, thickness and fastener schedule on the edges &
ME En	Indicate on the foundation plan if soil treatment is used for subterranean termite prevention or submit other approved termite protection methods. Protection shall be provided by registered termiticides  asonry Walls and Stem walls (load bearing & shear Walls) FRC Section R606  Show all materials making up walls, wall height, and Block size, mortar type Show all Lintel sizes, type, spans and tie-beam sizes and spacing of reinforcement tal frame shear wall and roof systems shall be designed, signed and sealed by Florida Prof.  gineer or Architect  NOT Framing System: First and/or second story  Floor truss package shall including layout and details, signed and sealed by Florida Registered Professional Engineer Show conventional floor joist type size, span, spacing and attachment to load bearing walls, stem walls and/or priers  Girder type, size and spacing to load bearing walls, stem wall and/or priers  Attachment of joist to girder  Wind load requirements where applicable Show required under-floor crawl space Show required amount of ventilation opening for under-floor spaces Show the required access opening to access to under-floor spaces Show the required access opening to access to under-floor spaces Show the sub-floor structural panel sheathing type, thickness and fastener schedule on the edges & intermediate of the areas structural panel sheathing
ME En S S S S S S	Indicate on the foundation plan if soil treatment is used for subterranean termite prevention or submit other approved termite protection methods. Protection shall be provided by registered termiticides  asonry Walls and Stem walls (load bearing & shear Walls) FRC Section R606  Show all materials making up walls, wall height, and Block size, mortar type Show all Lintel sizes, type, spans and tie-beam sizes and spacing of reinforcement tal frame shear wall and roof systems shall be designed, signed and sealed by Florida Prof.  gineer or Architect  NOT Framing System: First and/or second story  Floor truss package shall including layout and details, signed and sealed by Florida Registered Professional Engineer Show conventional floor joist type size, span, spacing and attachment to load bearing walls, stem walls and/or priers  Girder type, size and spacing to load bearing walls, Attachment of joist to girder  Wind load requirements where applicable Show required under-floor crawl space Show required amount of ventilation opening for under-floor spaces Show the required access opening to access to under-floor spaces Show the sub-floor structural panel sheathing Show Draft stopping, Fire caulking and Fire blocking
ME En	Indicate on the foundation plan if soil treatment is used for subterranean termite prevention or submit other approved termite protection methods. Protection shall be provided by registered termiticides  asonry Walls and Stem walls (load bearing & shear Walls) FRC Section R606  Show all materials making up walls, wall height, and Block size, mortar type Show all Lintel sizes, type, spans and tie-beam sizes and spacing of reinforcement tal frame shear wall and roof systems shall be designed, signed and sealed by Florida Prof.  gineer or Architect  NOT Framing System: First and/or second story  Floor truss package shall including layout and details, signed and sealed by Florida Registered Professional Engineer Show conventional floor joist type size, span, spacing and attachment to load bearing walls, stem walls and/or priers  Girder type, size and spacing to load bearing walls, stem wall and/or priers  Attachment of joist to girder  Wind load requirements where applicable Show required under-floor crawl space Show required amount of ventilation opening for under-floor spaces Show the required access opening to access to under-floor spaces Show the required access opening to access to under-floor spaces Show the sub-floor structural panel sheathing type, thickness and fastener schedule on the edges & intermediate of the areas structural panel sheathing

### WOOD WALL FRAMING CONSTRUCTION FRC CHAPTER 6

- Stud type, grade, size, wall height and oc spacing for all load bearing or shear walls.
- Fastener schedule for structural members per table R602.3 (1) are to be shown.
- Show wood structural panel's sheathing attachment to studs, joist, trusses, rafters and structural members, showing fastener schedule attachment on the edges & intermediate of the areas structural panel sheathing
  - Show all required connectors with a max uplift rating and required number of connectors and oc spacing for continuous connection of structural walls to foundation and roof trusses or rafter systems.
- Show sizes, type, span lengths and required number of support jack studs, king studs for shear wall opening and girder or header per FRC Table R502 5 (1)
- Indicate where pressure treated wood will be placed.
- Show all wall structural panel sheathing, grade, thickness and show fastener schedule for structural panel sheathing edges & intermed ate areas
- A detail showing gable truss bracing, wall balloon framing details or/ and wall hinge bracing detail

### **ROOF SYSTEMS:**

- Truss design drawing shall meet section FRC R802.10 Wood trusses. Include a layout and truss details and be signed and sealed by Fl. Pro. Eng.
- Show types of connector's assemblies' and resistance uplift rating for all trusses and rafters
- Show gable ends with rake beams showing reinforcement or gable truss and wall bracing details
- Provide dead load rating of trusses

### Conventional Roof Framing Layout Per FRC 802:

- Rafter and ridge beams sizes, span, species and spacing
- Connectors to wall assemblies' include assemblies' resistance to uplift rating.
- Valley framing and support details
- Provide dead load rating of rafter system.

### ROOF SHEATHING FRC Table R602,3(2) FRC 803

Include all materials which will make up the roof decking, identification of structural panel sheathing, grade, thickness and show fastener schedule for structural panel sheathing on the edges & intermediate areas

### **ROOF ASSEMBLIES FRC Chapter 9**

Include all materials which will make up the roof assembles covering; with Florida Product Approval numbers for each component of the roof assembles covering.

### FCB Chapter 13 Florida Energy Efficiency Code for Building Construction

- Residential construction shall comply with this code by using the following compliance methods in the FBC Subchapter 13-6, Residential buildings compliance methods. Two of the required forms are to be submitted, showing dimensions condition area equal to the total condition living space area
- Show the insulation R value for the following areas of the structure: Attic space, Exterior wall cavity and Crawl space (if applicable)

### **HVAC** information shown

- Manual J sizing equipment or equivalent computation
- Exhaust fans locations in bathrooms

### Plumbing Fixture layout shown

All fixtures waste water lines shall be shown on the foundation plan

### Electrical layout shown including:

- Switches, outlets/receptacles, lighting and all required GFCI outlets identified
- Ceiling fans
- Smoke detectors
- Service panel, sub-panel, location(s) and total ampere ratings

- On the electrical plans identify the electrical service overcurrent protection device for the main electrical service. This device shall be installed on the exterior of structures to serve as a disconnecting means for the utility company electrical service. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground. Indicate if the utility company service entrance cable will be of the overhead or underground type.
- Appliances and HVAC equipment and disconnects
  Arc Fault Circuits (AFCI) in bedrooms
- Notarized Disclosure Statement for Owner Builders
- Notice of Commencement Recorded (in the Columbia County Clerk Office) Notice
  Of Commencement is required to be filed with the building department Before Any
  Inspections Will Be Done.

### **Private Potable Water**

- Size of pump motor
- Size of pressure tank
- Cycle stop valve if used

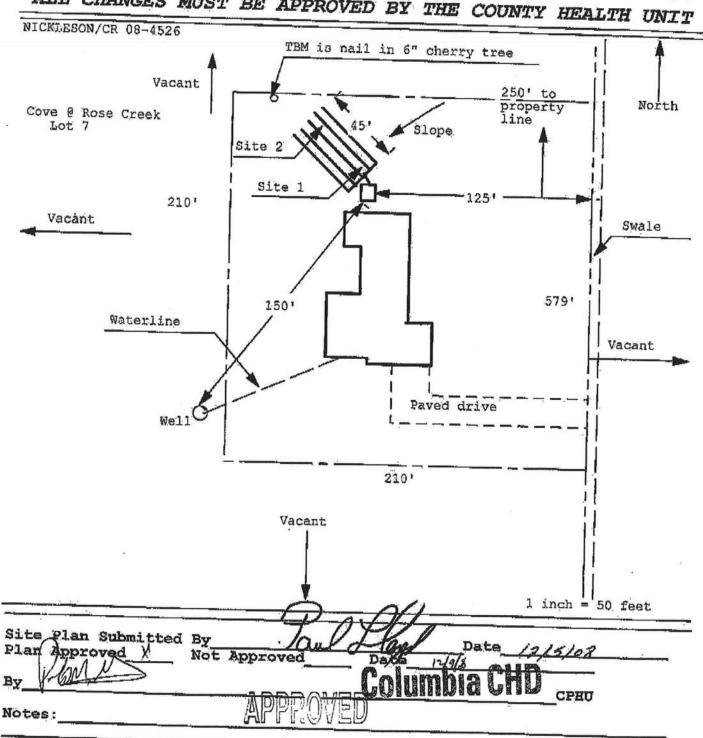
### THE FOLLOWING ITEMS MUST BE SUBMITTED WITH BUILDING PLANS

- Building Permit Application: A current Building Permit Application form is to be completed and submitted for all residential projects.
- Parcel Number: The parcel number (Tax ID number) from the Property Appraiser (386) 758-1084 is required. A copy of property deed is also requested.
- Environmental Health Permit or Sewer Tap Approval: A copy of the Environmental Health permit, existing septic approval or sewer tap approval is required before a building permit can be issued. (386) 758-1058 (Toilet facilities shall be provided for construction workers)
- City Approval: If the project is to be located within the city limits of the Town of Fort White, prior approval is required. The Town of Fort White approval letter is required to be submitted by the owner or contractor to this office when applying for a Building Permit. (386) 497-2321
- Flood Information: All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting application to this office. Any project located within a flood zone where the base flood elevation (100 year flood) has been established shall meet the requirements of Section 8.8 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.7 of the Columbia County Land Development Regulations. CERTIFIED FINISHED FLOOR ELEVATIONS WILL BE REQUIRED ON ANY PROJECT WHERE THE BASE FLOOD ELEVATION (100 YEAR FLOOD) HAS BEEN ESTABLISHED. A development permit will also be required. The permit cost is \$50.00.
- Driveway Connection: If the property does not have an existing access to a public road, then an application for a culvert permit (\$25.00) must be made. If the applicant feels that a culvert is not needed, they may apply for a culvert waiver (\$50.00). All culvert waivers are sent to the Columbia County Public Works Department for approval or denial.
- 911 Address: If the project is located in an area where the 911 address has been issued, then the proper Paper work from the 911 Addressing Departments must be submitted. (386) 758-1125

ALL REQUIRED INFORMATION IS TO BE SUBMITTED FOR REVIEW. NOTIFICATION WILL BE GIVEN WHEN THE APPLICATION AND PLANS ARE APPROVED AND READY TO PERMIT.

Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan Permit Application Number:

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT



STATE OF FLORIDA DEPARTMENT OF HEALTH AND REHABILITATIVE SERVICES ONSITE SEWAGE DISPOSAL SYSTEM APPLICATION FOR CONSTRUCTION PERMIT Authority: Chapter 381, FS & Chapter 10D-6, FAC PERMIT # DATE PAID FEE PAID \$ RECEIPT # CR. #

APPLICATION FOR:  [X] New System [ ] Existing System [ ] Holding Tank [ ] Repair [ ] Abandonment [ ] Other (Specify)	[ ] Temporary/Experimental System
APPLICANT: DANIEL & GAIL NICKELSON	TELEPHONE: 386-752-2281
AGENT: NORTH FLORIDA PERMIT SERVICE - Fax 752-2282	
	TY STATE: FL ZIP: 32024
TO BE COMPLETED BY APPLICANT OR APPLICANT'S AUTHORIZED AGENT. SITE FAN SHOWING PERTINENT FEATURES REQUIRED BY CHAPTER 10D-	ATTACH BUILDING PLAN AND TO-SCALE 6, FLORIDA ADMINISTRATIVE CODE.
PROPERTY INFORMATION [IF LOT IS NOT IN A RECORDED SUBDIVISION	(77)
LOT: 7 BLOCK: SUBDIVISION: COVE AT F	ROSE CREEK DATESUBD: 16
PROPERTY ID #: 01-58-16-03397-107 [Section/Township	/Range/Parcel] ZONING:
PROPERTY SIZE: 5.01 ACRES [Sqft/43560] PROPERTY WATER	SUPPLY: [X] PRIVATE [ ] PUBLIC
PROPERTY STREET ADDRESS: SW EMORYWOOD GLEN	V-11
DIRECTIONS TO PROPERTY: STATE ROAD 47 SOUTH, TL ON LITTLE ROAD RIGHT	, TL ON EMORYWOOD GLEN, LOT ON
BUILDING INFORMATION [X] RESIDENTIAL [] COM	MERCIAL
	ersons Business Activity ved For Commercial Only
1 house 3 4234	4
2	The second secon
3	
4	
[N] Garbage Grinders/Disposals [N] Spas/Hot Tubs. [N] Ultra-low Volume Flush Toilets [N] Other (Specify	[N] Floor/Equipment Drains
APPLICANT'S SIGNATURE: June Holl	DATE: 12408
HRS-H Form 4015 March 1992 (Obsoletes Previous Editions Which	May Not Be Used) Page 1 of 3

APPROVED BY:

DATE ISSUED: 12/5/8

BRS-E Form 4016 March 1992 (Obsoletes Previous Editions Which May Not Be Used) Page 1 of 2

TITLE: 65 I

EXPIRATION DATE: 6/9/10

COLUMBIA. CPRU

Atn: Webbie

### Columbia County Building Department Culvert Waiver

Phone: 386-758-1008 Fax: 386-758-2160

Culvert Waiver No. 000001695

DATE: 07/29/2009 BUILDING PERMIT	NO. 27521
APPLICANT DAN NICKELSON	PHONE <u>867-5616</u>
ADDRESS P.O. BOX 3631	LAKE CITY FL 32056
OWNER DAN & GAIL NICKELSON	PHONE 867-5616
ADDRESS 440 SW EMORYWOOD GLEN	LAKE CITY FL 32024
CONTRACTOR DAN NICKELSON	PHONE 867-5616
RIGHT 47S, TL ON LITTLE ROAI	D, TL ON EMORYWOOD, 4TH LOT ON
SUBDIVISION/LOT/BLOCK/PHASE/UNITCOVE AT R	ROSE CREEK 7
PARCEL ID # 01-5S-16-03397-107	
A SEPARATE CHECK IS REQUIRED MAKE CHECKS PAYABLE TO BCC	
PUBLIC WORKS DEPAR	RTMENT USE ONLY
I HEREBY CERTIFY THAT I HAVE EXAMINED THIS APPL CULVERT WAIVER IS:	ICATION AND DETERMINED THAT THE
APPROVED	NOT APPROVED - NEEDS A CULVERT PERMIT
COMMENTS: PUT LONGRETTE DRIVENAY. OR PUT 15 X ZI CULVERT.	Put invert in PRIVEWAY.
SIGNED: Willie Mills	DATE: 8-3-09
ANY QUESTIONS PLEASE CONTACT THE PUBLIC WORKS	S DEPARTMENT AT 386-752-5955.

27521

### **WILLIAM N. KITCHEN**

PROFESSIONAL SURVEYOR AND MAPPER 152 N. MARION AVENUE LAKE CITY, FLORIDA 32055 PHONE (386) 755-7786 FAX (386) 755-5506 E-MAIL BSSK@BELLSOUTH.NET



DATE: 1/8/2009

To Whom It May Concern:

RE: DANIEL NICKELSON LOT 7, COVE AT ROSE CREEK

SUBJECT Parcel: 01-5S-16-03397-107

SUBJECT PARCEL IS NOT IN A FLOOD ZONE ACCORDING TO FEMA FLOOD INSURANCE RATE MAP NO. 12023C383C DATED FEBUARY 4, 2009. AND THE TOP OF STEM WALLS = ELEVATION 90.25 FEET. LOT 7 PER PLAT SHOWS A MINIMUM FLOOR ELEVATION OF 80.5 FEET.

Thank you,

WILLIAM N. KITCHEN PSM # 5490

William N. Kether 1-8-2009

### **New Construction Subterranean Termite Soil Treatment Record**

OMB Approval No. 2502-0525

This form is completed by the licensed Pest Control Company.

Public reporting burden for this collection of information is estimated to average 15 minutes per response, including the time for reviewing instructions, searching existing data sources, gathering and maintaining the data needed, and completing and reviewing the collection of information. This information is mandatory and is required to obtain benefits. HUD may not collect this information, and you are not required to complete this form, unless it displays a currently valid OMB control number.

Section 24 CFR 200.926d(b)(3) requires that the sites for HUD insured structures must be free of termite hazards. This information collection requires the builder to certify that an authorized Pest Control company performed all required treatment for termites, and that the builder guarantees the treated area against infestation for one year. Builders, pest control companies, mortgage lenders, homebuyers, and HUD as a record of treatment for specific homes will use the information collected. The information is not considered confidential.

This report is submitted for informational purposes to the builder on proposed (new) construction cases when soil treatment for prevention of subterranean termite infestation is specified by the builder, architect, or required by the lender, architect, FHA, or VA.

All contracts for services are between the Pest Control Operator and builder, unless stated otherwise.

Section 1: General Information (Treating Company Information)	
Company Name: Aspen Pest Control, Inc.	
Company Address: P.O. Box 1798	City Lake City State Zip 32055
MOLANIA COM	Company Phone No. 388-755-3811 352-494-5751
FHA/VA Case No. (if any)	
Section 2: Builder Information	
Company Name: Mn Nickelson	Company Phone No. <u>867 - 5616</u>
Section 3: Property Information	
Location of Structure(s) Treated (Street Address or Legal Description, (	City, State and Zip) Dan Nichelson 440 SW Emorywood Glen
Type of Construction (More than one box may be checked) Slab Approximate Depth of Footing: Outside	Basement Crawl Other Inside 3 Type of Fill
Date(s) of Treatment(s)  Brand Name of Product(s) Used	
< facetonic	STABARRA
Name of Applicator(s)	Certification No. (if required by State law)
The applicator has used a product in accordance with the product label and stafederal regulations.  Authorized Signature	ate requirements. All treatment materials and methods used comply with state an Date

Warning: HUD will prosecute false claims and statements. Conviction may result in criminal and/or civil penalties. (18 U.S.C. 1001, 1010. 1012; 31 U.S.C. 3729, 3802)

Form NPCA-99-B may still be used



# Department of Building and Zoning Inspection **COLUMBIA COUNTY, FLORIDA**

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Building permit No. 000027521

77.00

Fire:

Parcel Number 01-5S-16-03397-107

Use Classification SFD,UTILITY

Permit Holder DAN NICKELSON

Owner of Building DAN NICKELSON

278.00

Total:

Waste: 201.00

Location: 440 SW EMORYWOOD GLEN, LAKE CITY, FL

Date: 10/28/2009

**Building Inspector** 

POST IN A CONSPICUOUS PLACE (Business Places Only)

Dan Nickelson

Missels (abalas

### PRODUCT APPROVAL SPECIFICATION SHEET

### Location:

### **Project Name:**

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and the product approval number(s) on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit on or after April 1, 2004. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. More information about statewide product approval can be obtained at <a href="https://www.floridabuilding.org">www.floridabuilding.org</a>

Category/Subcategory	Manufacturer	Product Description	Approval Number(s)
A. EXTERIOR DOORS			
1. Swinging	Anderson	outswing door	FL 1097
2. Sliding			
3. Sectional	i.		
4. Roll up	Gerenal America	n garage door	FL 2868
5. Automatic			
6. Other			8
B. WINDOWS			
Single hung	MI	window	FL 5108
Horizontal Slider			
3. Casement	,		
4. Double Hung			
5. Fixed	MI	fixed window	FL 125-R1
6. Awning			
7. Pass -through			
8. Projected			
9. Mullion			
10. Wind Breaker			
11 Dual Action			
12. Other			
C. PANEL WALL			
1. Siding			
2. Soffits	Ashley Alumbrum	n Aluminum 804775	FL 406
3. EIFS	1		
4. Storefronts			
5. Curtain walls		N. Control of the Con	
6. Wall louver			
7. Glass block			
8. Membrane			
9. Greenhouse			
10. Other			
D. ROOFING PRODUCTS			
Asphalt Shingles	Tamko	30-year asphault	FL 673
2. Underlayments			
3. Roofing Fasteners			
4. Non-structural Metal Rf			
5. Built-Up Roofing			
6. Modified Bitumen			
7. Single Ply Roofing Sys			
8. Roofing Tiles			
Roofing Insulation			
10. Waterproofing			

Dan Midelson

Contractor or Contractor's Authorized Agent Signature

ory/Subcategory (cont.)	Manufacturer	Product Description	Approval Number(s)
13. Liquid Applied Roof Sys			
14. Cements-Adhesives -		and the second second	
Coatings			
15. Roof Tile Adhesive			
16. Spray Applied Polyurethane Roof			
17. Other			
. SHUTTERS			
1. Accordion			
2. Bahama			···
3. Storm Panels		i i	
4. Colonial			·
5. Roll-up	-, -		
6. Equipment			
7. Others			<u> </u>
. SKYLIGHTS			
1. Skylight			
2. Other			
S. STRUCTURAL			
COMPONENTS			
Wood connector/ancho	r		
2. Truss plates			
Engineered lumber			
4. Railing			
5. Coolers-freezers			
6. Concrete Admixtures			
7. Material	1		
8. Insulation Forms			
9. Plastics			
10. Deck-Roof			
11. Wall	1		
12. Sheds			
13. Other			
H. NEW EXTERIOR			
ENVELOPE PRODUCTS			
1.			
	-		
time of inspection of these jobsite; 1) copy of the prod and certified to comply with	products, the f luct approval, 2 n, 3) copy of the	trate product approval at plan review following information must be availang the performance characteristics when applicable manufacturers installating the removed if approval cannot be designed.	hich the product was tested on requirements.
understand these produc	is may have to	De Temoved ii approvai dariilot bo d	
1 0 lede		Linda Ro	oder 17-5-01

Print Name

Date

## VENT-FREE

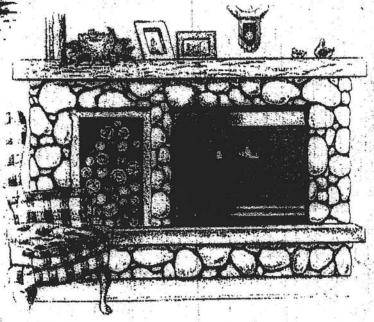
This unit is A.G.A. certified as a heater with 99% heat efficiency No chimney or flue system required Wide selection of factory installed options offered

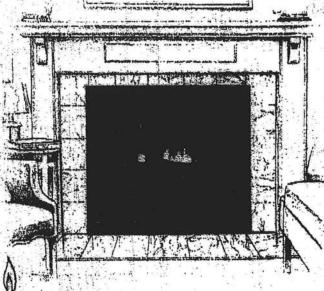
### VF-4000

- 14,000 25,000 Bru/hr with manual control valve
- . 19,500 25,000 Beu/hr with millivolt control valve
- · Fully assembled and ready to install
- Artractive wood surrounds available
- 15" x 30" fixed or operable screen opening

### VF-5000

- · 25,000 Btu/hr millivolt variable heat output
- 15" X 30" glass or screen viewing area
- · Clean burning, safe and easy to install
- Realistic charted oak logs with glowing embers



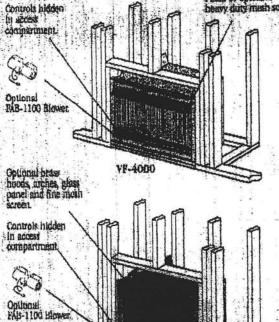


### VF-6000

- 32,000 Bru/hr millivolt variable heat output
- · Beautiful 20" X 34" glass or screen viewing area
- · Will operate during a power failure
- · Designed for large rooms



# VP-6000 shows. Controls hidden have substituted in access conjustriment.



### SURROUNDS

Millitoh controls and piezo lgalilon operate during a power fallure.

The Charleston Poplar Surround is hand crafted using a combination of solid Poplar and Poplar veneer. Using the unique wood type of Poplar allows you the option to paint or stain this elegantly detailed surround. The surround is constructed using easy to assemble cam locks, and available in comer and wall units.

VF-5000/6000





Distributed by:



The state of the s

Refractory tan brick panels



Gas Dux liner kit.





Brass Louver Kit (For VF-4 only)







Glass door kit. (For VF-5 & VF6 only)



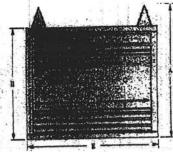




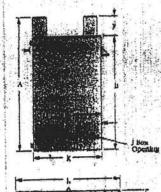
Wall switch or optional wireless remote systable.
(For VF-4MV, VF-5 & VP-6)



Front View



Left Side View



Top View

### Vent-Free Product Dimensions

1 VII-4000/4000C	VF-6000C
MINOR OF THE PARTY	42-1/8*
Marie Company of the Company	36-5/8"
per programme de la compressión de la compressió	20°
1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	Э. Э.
	alex er 40°
750 (100 100 100 100 100 100 100 100 100 1	5.1/2"
3. 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	THE RESERVE OF THE PARTY OF THE
R 38/4	3-3/4"
	8-1/3
	3
是这个自己的。我就是是这个	19.1/9
£ 27°	28-1/2"

### Btu Chart

Model	Natural	Propane
	14,000 - 25,000	14,000 - 25,000
VE-4000/5000 milliwok	19,500 - 25,000	19,500 - 25,000
VR-6000	25,000 - 32,000	25,000 - 32,000

### Framing Dimension

-	Model	Width	Height	Depth
-	VF-4000/5000	37"	37-1/4"	15-1/2"
-	V9-6000	41"	42-3/81	19-1/2"

NOTE: Diagrams and disstrations are not to scale. Product designs, materials, dimensions, specifications, colors and prices subject to change or discontinuation without notice. Bull: to ANSI 221.11.2 standard and approved by A.G.A. (report # 12970017).

Consult your claimbuter for local fireplace code information.



www.lenhoxNearthProducts.com

Printed in U.S.A. © 2001 Lennox Hearth Products • 1110 West Taft Ave., Orange, CA 92865-4150 ionnox reacts reacts blood tent house and appliances include a 20-year limited warring.

P/N 903464 HEV R 2/00

FRX NO. :+386 758 4735

EROM : LAKE CITY INDUSTRIES



Project Information:

Truss Design Information:

Builder: Cash Account

Model: custom

Builders FirstSource Job #: L288061F

Street: 440 SW EMERYWOOD

City: LAKE CITY County: COLUMBIA Building Code: FBC2004/TPI2002

Roof: 32 psf Total

Floor: 55 psf Total

Computer Program Used: MiTek 6.3

**Gravity Loads** 

Wind Wind Standard: ASCE 7-02

Wind Speed: 110 mph

Mean Roof Ht: 20 ft

Builders FirstSource

Lake City, FL 32055

2525 E. Duval St.

JULIUS LEE'S CONSULT. ENGR INC 1455 SW 4TH AVE , DELRAY BEACH

FLORIDA. 3344 Exposure: B

design criteria, truss geometry, lumber, and plate information.

**Design Professional Information:** Design Professional Of Record: BUILDER
Delegated Truss Engineer: Julius Lee

Note: Refer to individual truss design drawings for special loading conditions,

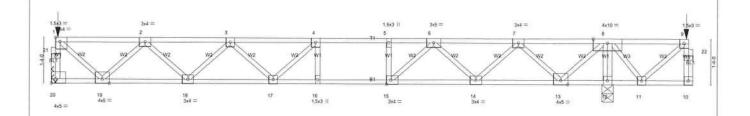
License #:

License #: 34869

This truss specification package consists of this index sheet and 16 truss design drawings. This signed and sealed index sheet indicates acceptance of my professional engineering responsibility solely for listed truss design drawings. The suitability and use of each truss component for any particular building is the responsibility of the building designer per TPI.

Truss #	Truss Label	Drawing #	Seal Date	Truss #	Truss Label	Drawing #	Seal Date	Truss #	Truss Label	Drawing #	Seal Date
1	F01	L288061F001	10/29/2008								
2	F02	L288061F002	10/29/2008								
3	F03	L288061F003	10/29/2008				1				
4	F04	L288061F004	10/29/2008								
5	F05	L288061F005	10/29/2008								
6	F06	L288061F006	10/29/2008								
7	F07	L288061F007	10/29/2008								
8	F07A	L288061F008	10/29/2008								
9	F08	L288061F009	10/29/2008								
10	F09	L288061F010	10/29/2008								
11	F10	L288061F011	10/29/2008								
12	F11	L288061F012	10/29/2008								
13	F11A	L288061F013	10/29/2008								
14	F12	L288061F014	10/29/2008								
15	FGT	L288061F015	10/29/2008								
16	FGT3	L288061F016	10/29/2008								
									-		
					3411-4-5-12						
$\neg$											
$\neg$							-				
-											

lob	Truss	Truss Type	Qty	Ply	NICKELSON / FLOOR
288061F	F01	FLOOR	7	1	L288061F001 Job Reference (optional)
Builders FirstSource,	Loke City EL 220EE			6 37	0 s Apr 19 2006 MiTek Industries, Inc. Wed Oct 29 08:52:15 2008 Page
bunders raistadurce,	Lake City, FI 32055			0.30	to slaping 2006 wither industries, Inc., wed Oct 29 08:52:15 2008, Page
0-1-8	Lake City, Pt 32055			6.30	to's Apr 19 2006 will ex industries, Inc.   Wed Oct 29 08:52:15 2008 Page



1-6-0	4-0-0	6-6-0	ř	12-5-8			14-11-8	16-4-0	17-4-0	18-10-0
1-6-0	2-6-0	2-6-0		5-11-8			2-6-0	1-4-8	1-0-0	1-6-0
Plate Offsets (X,Y): [1	Edge,0-1-8], [15:0-1-8,Ed	ge], [20:Edge,0-1	-8]							(3) (3)(5)
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0	SPACING Plates Increase Lumber Increase	e 1.00	CSI TC 0.79 BC 0.92	DEFL Vert(LL) Vert(TL)	in (loc) -0.18 16-17 -0.28 16-17	l/defl >999 >683	L/d 360 240	PLATES MT20	GRIP 244/190	
BCDL 5.0	Rep Stress Incr Code FBC2004/		WB 0.47 (Matrix)	Horz(TL)	0.04 12	n/a	n/a	Weight: 100	) lb	

LUMBER TOP CHORD 4 X 2 SYP No.2 BOT CHORD 4 X 2 SYP No.1D WEBS 4 X 2 SYP No.3 RRACING TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 6-0-0 oc bracing: 12-13,11-12,10-11.

REACTIONS (Ib/size) 20=1467/Mechanical, 12=1304/0-4-0 Max Grav 20=1493(load case 2), 12=1304(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

20-21=-148970, 1-21=-148770, 10-22=0/8, 9-22=0/8, 1-2=-905/0, 2-3=-2150/0, 3-4=-2784/0, 4-5=-2887/0, 5-6=-2887/0, 6-7=-2044/0, 7-8=-785/169, 8-9=0/245
19-20=0/77, 18-19=0/1672, 17-18=0/2608, 16-17=0/2887, 15-16=0/2887, 14-15=0/2529, 13-14=0/1561, 12-13=-433/0, 11-12=-437/0, 10-11=-0/0
8-12=-1275/0, 1-19=0/1126, 8-13=0/1182, 2-19=-1066/0, 7-13=-1094/0, 2-18=0/665, 7-14=0/710, 3-18=-637/0, 6-14=-725/0, 3-17=0/356, 6-15=0/764, 4-17=-396/194, 4-16=-227/86, 5-15=-321/0, 9-11=-332/0, 8-11=0/302 TOP CHORD BOT CHORD

WEBS

NOTES (9)

Unbalanced floor live loads have been considered for this design.
 All plates are 3x3 MT20 unless otherwise indicated.

2) his places are 3.5 km or others unless unless unless that was inticated.

3) Refer to girder(s) for truss to truss connections.

4) Load case(s) 1, 2, 3, 4, 5 has/have been modified. Building designer must review loads to verify that they are correct for the intended use of this truss.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

6) CAUTION, Do not erect truss backwards.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 120 lb down at 18-7-12, and 620 lb down at 0-2-4 on

top chord. The design/selection of such connection device(s) is the responsibility of others.

8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
1) Floor: Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (plf) Vert: 10-20=-10, 1-9=-100

Concentrated Loads (lb) Vert: 1=-620(F) 9=-120(F)

1st unbalanced Floor: Lumber Increase=1.00, Plate Increase=1.00
 Uniform Loads (plf)

Vert: 10-20=-10, 1-8=-100, 8-9=-20 Concentrated Loads (lb)

Vert: 1=-620(F) 9=-33(F)
3) 2nd unbalanced Floor; Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (plf) Vert: 10-20=-10, 1-8=-20, 8-9=-100

Concentrated Loads (lb) Vert: 1=-620(F) 9=-120(F)

4) 1st chase Floor: Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (plf)

Vert: 10-20=-10, 1-5=-100, 5-8=-20, 8-9=-100 Concentrated Loads (lb)

Vert: 1=-620(F) 9=-120(F)

Continued on page 2

Job	Truss	Truss Type		Qty	Ply	NICKELSON / FLOOR
L288061F	F02	FLOOR		2	1	
Builders FirstSource	, Lake City, FI 32055				6.30	Job Reference (optional) 000 s Apr 19 2006 MiTek Industries, Inc. Wed Oct 29 08:52:16 2008 Page
0-1-8						
H   1-2-8	1-3-0		1-	11-8		1-1-4 0 <sub>7</sub> 1 <sub>6</sub> 8
111						Scale + 1
1.5x3 = , 4x4 =	**		1,5x3: ()	1.5x3.11		4x4 =
1 1	2	191	4	71 6	6	7 6150 =
1 100	NO CON	43 W	W3 W1		wa/	W3 W3 W4
4 1	(// `	\			//	
	<b>M</b>	- M	73	81	0	
16	15 4x4 =	14	13	12		10 464 =
	4x4 =					484 =

OADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) I/defl L/d	PLATES GRIP
CLL 40.0	Plates Increase 1.00	TC 0.48	Vert(LL) -0.13 13-14 >999 360	MT20 244/190
CDL 10.0	Lumber Increase 1.00	BC 0.62	Vert(TL) -0.19 13-14 >944 240	100.000 No.00000
CLL 0.0	Rep Stress Incr YES	WB 0.41	Horz(TL) 0.04 9 n/a n/a	
ICDL 5.0	Code FBC2004/TPI2002	(Matrix)		Weight: 78 lb

15-0-4 15-0-4

LUMBER TOP CHORD 4 X 2 SYP No.2 BOT CHORD 4 X 2 SYP No.2 WEBS 4 X 2 SYP No.3

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

### REACTIONS (lb/size) 16=806/Mechanical, 9=806/0-3-8

FORCES (ib) - Maximum Compression/Maximum Tension
TOP CHORD
TOP CH

- NOTES (5)

  1) Unbalanced floor live loads have been considered for this design.

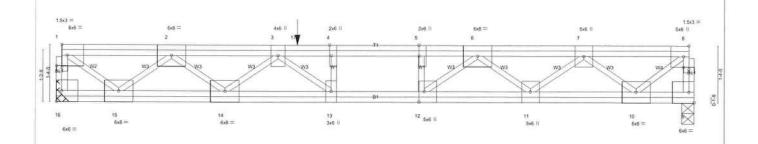
  2) All plates are 3x3 MT20 unless otherwise indicated.

  3) Refer to girder(s) for truss to truss connections.

  4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their cutes make or restrained by other preserve. outer ends or restrained by other means.
  5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

ob	Truss	Truss Type	Qty	Ply	NICKELSON / FLOOR
288061F	F03	FLOOR	3	1	L288061F003 Job Reference (optional)
Builders FirstSource, La	ake City, FI 32055			6.30	0 s Apr 19 2006 MiTek Industries, Inc. Wed Oct 29 08:52:18 2008 Page 1
0-1-8					
H   1-2-8	1-3-0	<b>—</b>	1-11-8		1-1-4 0-1-8



15-0-4 15-0-4 Plate Offsets (X.Y): [1:0-1-8.0-0-8]. [4:0-3-0 Edgel. [5:0-3-0.0-0-0]. [8:0-1-8.0-0-8]. [8:0-3-0 Edgel. [12:0-3-0 Edgel.

LOADING (psf)	SPACING 2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES GRIP
TCLL 40.0	Plates Increase 1.00	TC 0.88	Vert(LL)	-0.21 13-14	>853	360	MT20 244/190
TCDL 10.0	Lumber Increase 1.00	BC 0.93	Vert(TL)	-0.32 13-14	>550	240	
BCLL 0.0	Rep Stress Incr NO	WB 0.81	Horz(TL)	0.04 9	n/a	n/a	General Francis
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)	100000000000000000000000000000000000000				Weight: 118 lb

LUMBER TOP CHORD 4 X 2 SYP No.2 BOT CHORD 4 X 2 SYP No.1D WEBS 4 X 2 SYP No.3

BRACING

TOP CHORD **BOT CHORD**  Structural wood sheathing directly applied or 5-7-1 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 16=1412/Mechanical, 9=1173/0-3-8

FORCES (Ib) - Maximum Compression/Maximum Tension TOP CHORD

BOT CHORD

1-16=-1392/0, 8-9=-1178/0, 1-2=-1587/0, 2-3=-4295/0, 3-17=-5174/0, 4-17=-5174/0, 4-5=-5174/0, 5-6=-5174/0, 6-7=-3236/0, 7-8=-1242/0
15-16=-0/0, 14-15=0/3066, 13-14=0/5357, 12-13=0/5174, 11-12=0/4163, 10-11=0/2413, 9-10=0/0
8-10=0/1640, 1-15=0/2028, 7-10=-1551/0, 2-15=-1959/0, 7-11=0/1089, 2-14=0/1625, 6-11=-1227/0, 3-14=-1406/0, 6-12=0/1554, 3-13=-531/390, 4-13=-230/15, 5-12=-625/0 WEBS

NOTES (6)

1) Unbalanced floor live loads have been considered for this design.

- 2) Refer to girder(s) for truss to truss connections.
  3) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their
- outer ends or restrained by other means.
  4) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 960 lb down at 5-8-0 on top chord. The design/selection of such connection device(s) is the responsibility of others.

  5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

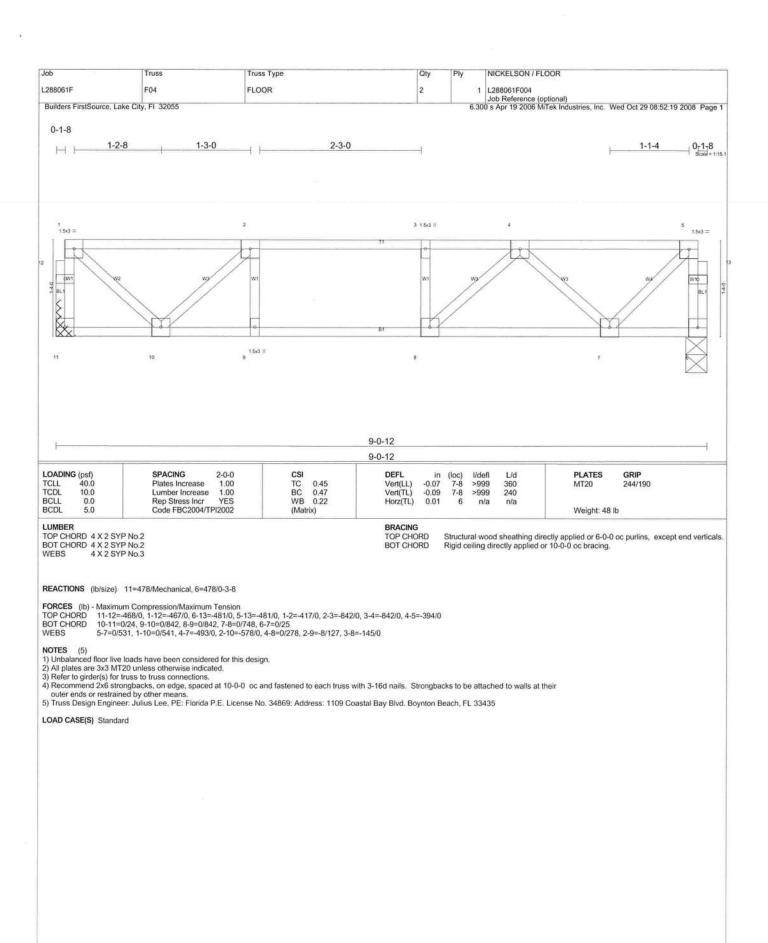
  6) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

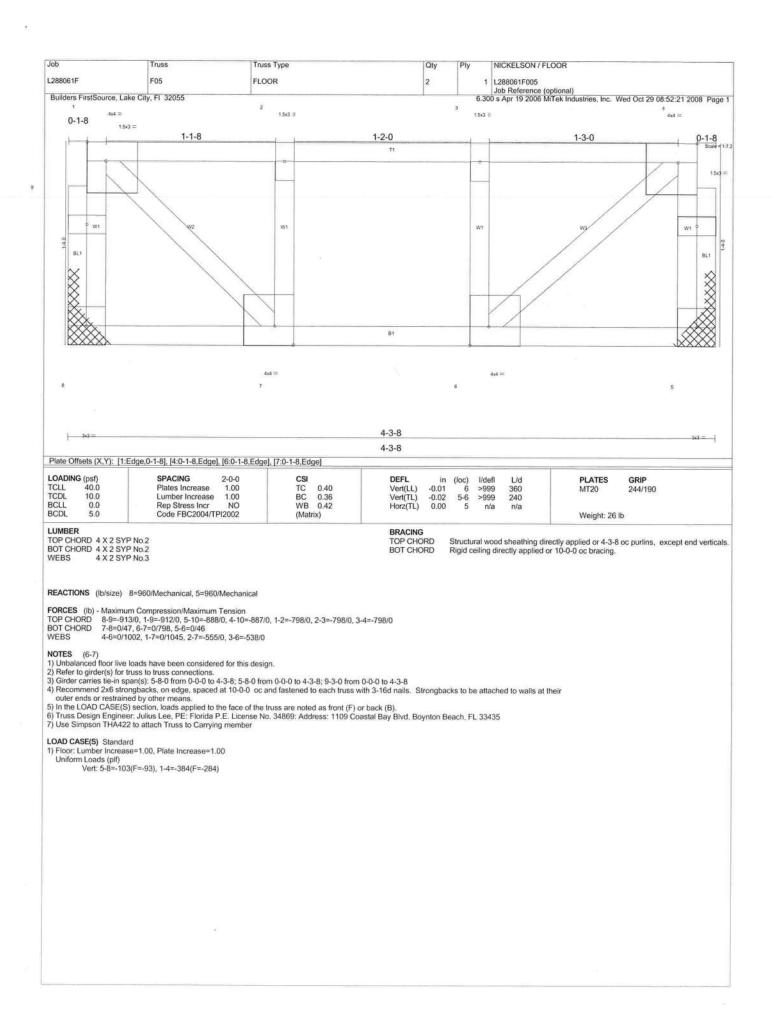
LOAD CASE(S) Standard

Floor: Lumber Increase=1.00, Plate Increase=1.00
 Uniform Loads (plf)

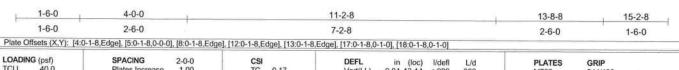
Vert: 9-16=-10, 1-8=-100 Concentrated Loads (lb)

Vert: 17=-960(F)





L288061F F05A FLOOR 1 3 Job Reference (optional) Builders FirstSource, Lake City, FI 32055 5 5.300 s Apr 19 2006 MiTek Ind	
Builders Piristource, Lake City, PI 3/2055 6.300 s Apr 19 2006 MiTek Ind	
	ustries, Inc. Wed Sep 17 12:47:40 2008 Page 1
0-1-8	
H   1-3-0   1-11-8	0-1-8 scale 1-28
	Scale = 125
2x4 = 2x4    1 2 3 4 52x4    6	7 8 24 =
	79 79



TCLL 40.0 TCDL 10.0 BCLL 0.0	SPACING 2-0-0 Plates Increase 1.00 Lumber Increase 1.00 Rep Stress Incr NO	CSI TC 0.17 BC 0.23 WB 0.14	DEFL         in (loc)         l/defl         L/d           Vert(LL)         -0.04         13-14         >999         360           Vert(TL)         -0.06         13-14         >999         240           Horz(TL)         0.01         9         n/a         n/a	PLATES GRIP MT20 244/190
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)	TOTAL TOTAL AND THE	Weight: 236 lb

LUMBER TOP CHORD 4 X 2 SYP No.2 BOT CHORD 4 X 2 SYP No.2 WEBS 4 X 2 SYP No.3

BRACING TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 16=816/0-6-0, 9=816/Mechanical, 9=816/Mechanical

FORCES (ib) - Maximum Compression/Maximum Tension

TOP CHORD
BOT CHORD
WEBS

TOP CHORD
BOT CHORD
BOT CHORD
BOT CHORD
WEBS

TOP CHORD
BOT CHORD
BOT

### NOTES (6)

- NOTES (6)

  1) Trusses to be fastened together to act as a single unit. All loads to be distributed equally over the 3 plies.

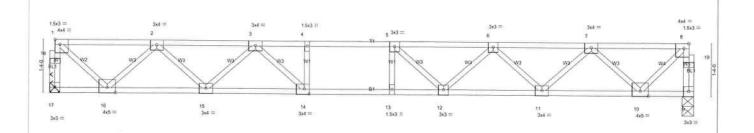
  2) Unbalanced floor live loads have been considered for this design.

  3) All plates are 3x5 MT20 unless otherwise indicated.

  4) Refer to girder(s) for truss to truss connections.

  5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means. 6) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

Job	Truss	Truss Type	Qty	Ply	NICKELSON / FLOOR
L288061F	F06	FLOOR	6	1	L288061F006 Job Reference (optional)
	Lake City, FI 32055			6.30	0 s Apr 19 2006 MiTek Industries, Inc. Wed Oct 29 08:52:23 2008 Page 1
0-1-8					
H <del>- 1-2-8</del>	1-3-0	ļ2-	0-8		1-1-4 0-11-8 Scale = 127.5



16-4-4 16-4-4 Plate Offsets (X Y): [1:Edge 0-1-8] [8:0-1-8 Edge] [14:0-1-8 Edge]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (lo	) //defi	L/d	PLATES	GRIP
TCLL 40.0 TCDL 10.0 BCLL 0.0	Plates Increase 1.00 Lumber Increase 1.00 Rep Stress Incr YES	TC 0.61 BC 0.78 WB 0.45	Vert(LL) -0.18 12- Vert(TL) -0.28 12- Horz(TL) 0.05	3 >999	360 240 n/a		244/190
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)	110/2(12) 0.00	o ma	IIId	Weight: 84 lb	

LUMBER

TOP CHORD 4 X 2 SYP No.2 BOT CHORD 4 X 2 SYP No.1D WEBS 4 X 2 SYP No.3

BRACING TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 17=879/Mechanical, 9=879/0-3-8

FORCES (ib) - Maximum Compression/Maximum Tension

TOP CHORD

BOT CHORD

OF CHORD

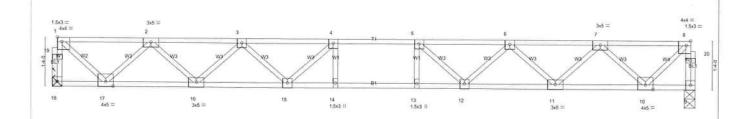
OF

NOTES (4)

1) Unbalanced floor live loads have been considered for this design.

2) Refer to girder(s) for truss to truss connections.
3) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
4) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

Job	Truss	Truss Type		Qty	Ply	NICKELSON / FLOOR
L288061F	F07	FLOOR		1	1	L288061F007
Builders FirstSource	e, Lake City, FI 32055	0.007.4000.00			6.30	Job Reference (optional) 0 s Apr 19 2006 MiTek Industries, Inc. Wed Oct 29 08:52:24 2008 Page 1
0-1-8						
H   1-2-8	1-3-0		2-1-8	ł		1-1-4 0-1-8 Solar = 129.8



17-8-4 17-8-4 Plate Offects (Y V): [1:Edgo 0.1.9] [9:0.1.9 Edgo]

TCLL 40.0 Plates Increase 1.00 TC 0.47 Vert(LL) -0.20 14-15 >999 360 MT20 244/190 TCDL 10.0 Lumber Increase 1.00 BC 0.79 Vert(TL) -0.31 14-15 >684 240	LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) I/defl L/d	PLATES GRIP
TCDL 10.0 Lumber Increase 1.00 BC 0.79 Vert(TL) -0.31 14-15 >684 240	FCLL 40.0	Plates Increase 1.00			
	TCDL 10.0				W120 244/190
BCLL 0.0 Rep Stress Incr YES WB 0.49 Horz(TL) 0.06 9 n/a n/a	BCLL 0.0	Rep Stress Incr YES	WB 0.49		

LUMBER

TOP CHORD 4 X 2 SYP No.2 BOT CHORD 4 X 2 SYP No.1D WEBS 4 X 2 SYP No.3

BRACING TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 18=953/Mechanical, 9=953/0-3-8

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

18-19=-948/0, 1-19=-947/0, 9-20=-948/0, 8-20=-947/0, 1-2=-938/0, 2-3=-2353/0, 3-4=-3165/0, 4-5=-3431/0, 5-6=-3142/0, 6-7=-2305/0, 7-8=-870/0 17-18=0/49, 16-17=0/1791, 15-16=0/2888, 14-15=0/3431, 13-14=0/3431, 12-13=0/3431, 11-12=0/2853, 10-11=0/1793, 9-10=0/49 8-10=0/1182, 1-17=0/1226, 7-10=-1196/0, 2-17=-1187/0, 7-11=0/800, 2-16=0/782, 6-11=-761/0, 3-16=-744/0, 6-12=0/486, 3-15=0/473, 5-12=-623/0, 4-15=-602/2, 4-14=-168/192, 1-12=0/2853, 10-11=0/1793, 1-12=0/486, 3-15=0/473, 5-12=-623/0, 4-15=-602/2, 4-14=-168/192, 1-12=0/2853, 10-11=0/1793, 1-12=0/486, 3-15=0/473, 5-12=-623/0, 4-15=-602/2, 4-14=-168/192, 1-12=0/2853, 10-11=0/1793, 1-12=0/486, 3-15=0/473, 5-12=-623/0, 4-15=-602/2, 4-14=-168/192, 1-12=0/2853, 10-11=0/1793, 1-12=0/486, 3-15=0/473, 5-12=-623/0, 4-15=-602/2, 4-14=-168/192, 1-12=0/2853, 10-11=0/1793, 1-12=0/486, 3-15=0/473, 5-12=-623/0, 4-15=-602/2, 4-14=-168/192, 1-12=0/2853, 10-11=0/1793, 1-12=0/2863, 10-11=0/1793, 1-12=0/286 WEBS

NOTES (5)

1) Unbalanced floor live loads have been considered for this design.

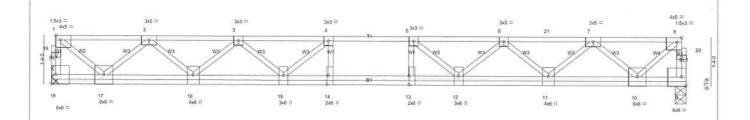
2) All plates are 3x3 MT20 unless otherwise indicated.

3) Refer to girder(s) for truss to truss connections.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

Job	Truss	Truss Type	Qty	Ply	NICKELSON / FLOOR
L288061F	F07A	FLOOR	1	- 4	L288061F008
6 01 6 16		De contrata de la contrata del contrata de la contrata del contrata de la contrata del contrata de la contrata de la contrata de la contrata del contrata de la contrata del la contr			Job Reference (optional)
builders FirstSource	e, Lake City, FI 32055			6.3	00 s Apr 19 2006 MiTek Industries, Inc. Wed Oct 29 08:52:26 2008 Page 1
0-1-8					
0-1-0					



17-8-4 17-8-4 Plate Offsets (X,Y): [1:Edge,0-1-8], [8:0-1-8.Edge], [13:0-3-0.0-0-0], [14:0-3-0.Edge]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL	in (loc)	I/defi	L/d	PLATES GRIP
TCLL 40.0	Plates Increase 1.00	TC 0.65	Vert(LL) -0	19 13	>999	360	MT20 244/190
TCDL 10.0	Lumber Increase 1.00	BC 0.76		30 13	>696	240	2.1.6.100
BCLL 0.0	Rep Stress Incr NO	WB 0.61		04 9	n/a	n/a	
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)			17.11.55	100	Weight: 114 lb

LUMBER TOP CHORD 4 X 2 SYP No.2 BOT CHORD 4 X 2 SYP No.2 WEBS 4 X 2 SYP No.3

BRACING TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 18=991/Mechanical, 9=1288/0-3-8

FORCES (Ib) - Maximum Compression/Maximum Tension

TOP CHORD
BOT CHORD
WEBS

TOP CHORD
BOT CHORD
BOT CHORD
WEBS

TOP CHORD
BOT CHORD
BOT CHORD
WEBS

TOP CHORD
BOT CHORD
WEBS

TOP CHORD
BOT CHORD
BOT CHORD
WEBS

Maximum Compression/Maximum Tension

18-19=-984/0, 1-19=-982/0, 9-20=-1282/0, 8-20=-1280/0, 1-2=-1031/0, 2-3=-2605/0, 3-4=-3539/0, 4-5=-3904/0, 5-6=-3678/0, 6-21=-2875/0, 7-21=-2875/0, 7-21=-2875/0, 7-8=-1159/0

17-18=0/54, 16-17=0/1973, 15-16=0/3205, 14-15=0/3904, 13-14=0/3904, 11-12=0/3418, 10-11=0/2306, 9-10=0/70

8-10=0/1526, 1-17=0/1316, 7-10=-1556/0, 2-17=-1278/0, 7-11=0/772, 2-16=0/858, 6-11=-737/0, 3-16=-813/0, 6-12=0/422, 3-15=0/528, 5-12=-548/97, 4-15=-742/0, 4-14=-146/284, 3-15=0/1526, 1-17=0/1316, 7-10=-1556/0, 2-17=-1278/0, 7-11=0/772, 2-16=0/858, 6-11=-737/0, 3-16=-813/0, 6-12=0/422, 3-15=0/528, 5-12=-548/97, 4-15=-742/0, 4-14=-146/284, 3-15=0/1526, 1-17=0 5-13=-246/184

NOTES (6)
1) Unbalanced floor live loads have been considered for this design.

2) Refer to girder(s) for truss to truss connections.
3) Girder carries tie-in span(s): 5-4-4 at 13-10-8 to 6-8-10 at 17-10-8.

- 3) Glober Carries lie-in span(s): 3-4-4 at 15-10-6 to 6-10 at 17-10-6.
  4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

Outer this or restained by other means.

5) In the LOAD CASE(5) section, loads applied to the face of the truss are noted as front (F) or back (B).

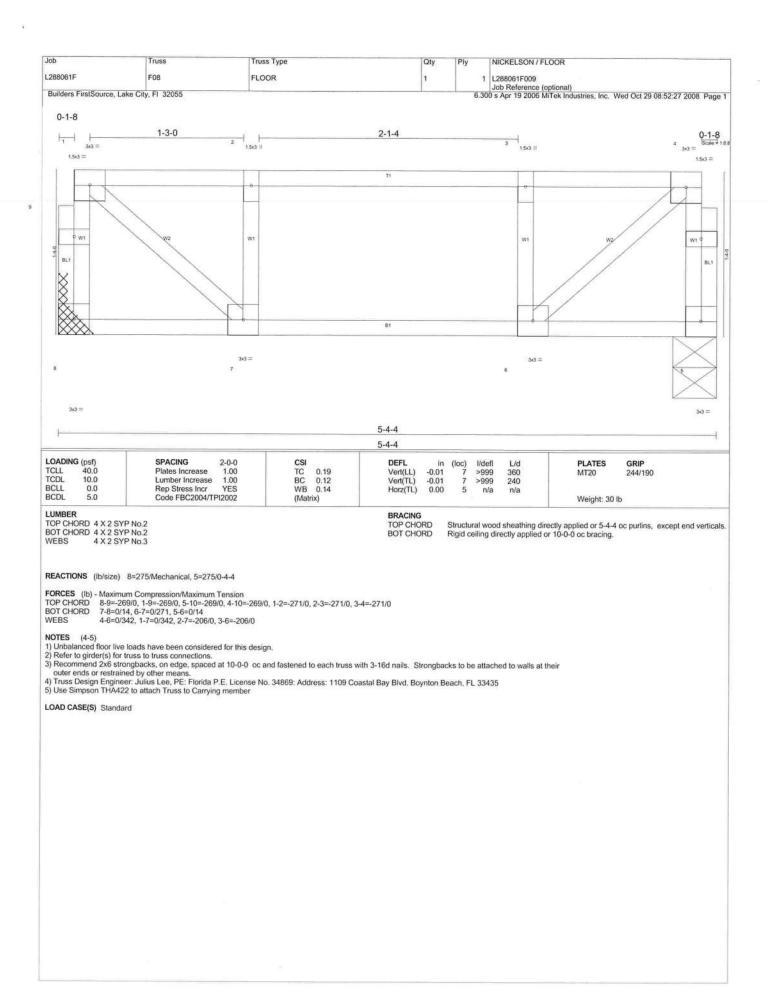
6) Truss Design Engineer. Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

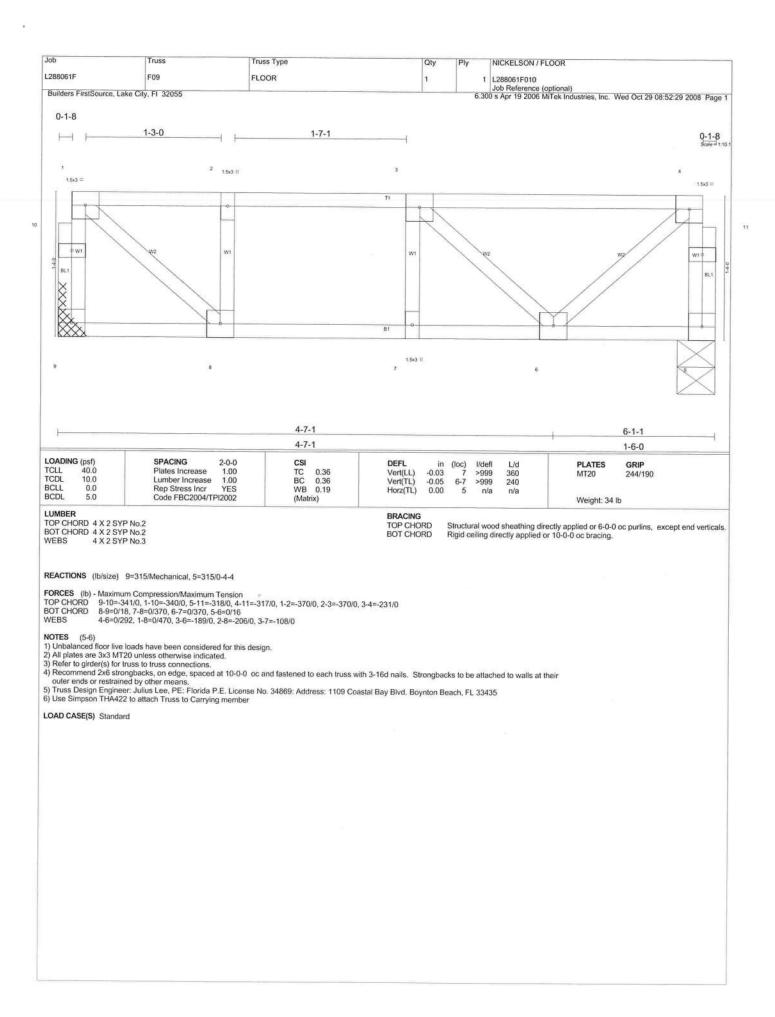
LOAD CASE(S) Standard

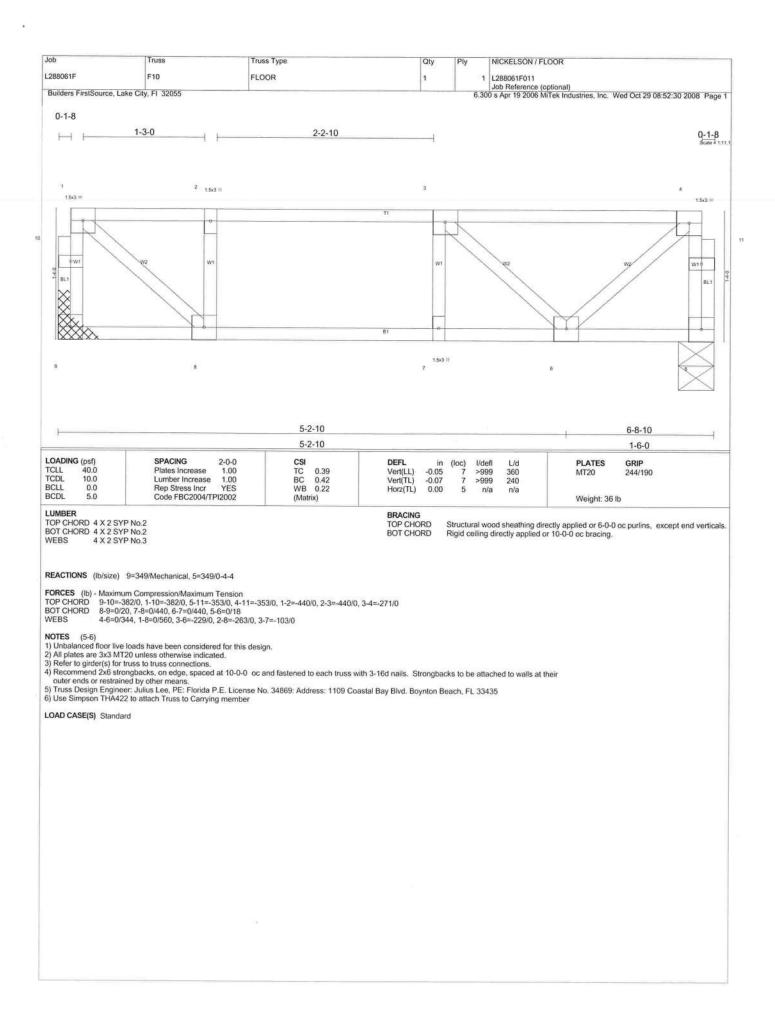
Floor: Lumber Increase=1.00, Plate Increase=1.00
 Uniform Loads (plf)

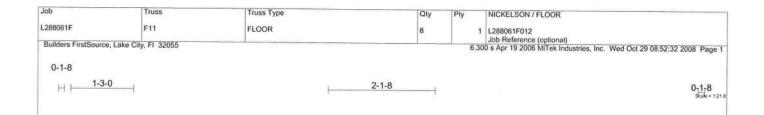
Vert; 9-18=-10, 1-21=-100

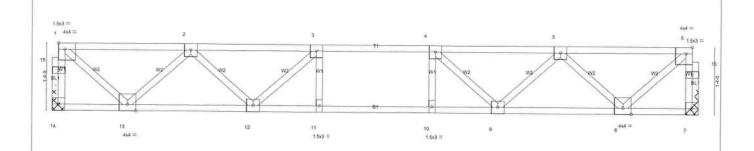
Trapezoidal Loads (plf) Vert: 21=-184(F=-84)-to-8=-222(F=-122)











1-6-0	4-0-0	8-10-8 4-10-8			11-4-8	12-10-8
1-6-0	2-6-0				2-6-0	1-6-0
Plate Offsets (X,Y): [1:E	dge,0-1-8], [6:0-1-8,Edge]					
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING 2-0-0 Plates Increase 1.00 Lumber Increase 1.00 Rep Stress Incr YES	CSI TC 0.38 BC 0.61 WB 0.34	DEFL         in (loc)         l/defl           Vert(LL)         -0.09 11-12         >999           Vert(TL)         -0.12 11-12         >999           Horz(TL)         0.02         7         n/a	L/d 360 240 n/a	PLATES MT20	GRIP 244/190
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)			Weight: 67	lb

LUMBER	
TOP CHORD	4 X 2 SYP No.2
BOT CHORD	4 X 2 SYP No.2
WEBS	4 X 2 SYP No.3

BRACING TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing. BOT CHORD

Weight: 67 lb

REACTIONS (lb/size) 14=688/Mechanical, 7=688/Mechanical

FORCES (ib) - Maximum Compression/Maximum Tension

TOP CHORD
BOT CHORD
WEBS

TOP CHORD
BOT CHORD
BOT CHORD
BOT CHORD
WEBS

TOP CHORD
BOT CHORD
BOT CHORD
BOT CHORD
BOT CHORD
WEBS

TOP CHORD
BOT CHO

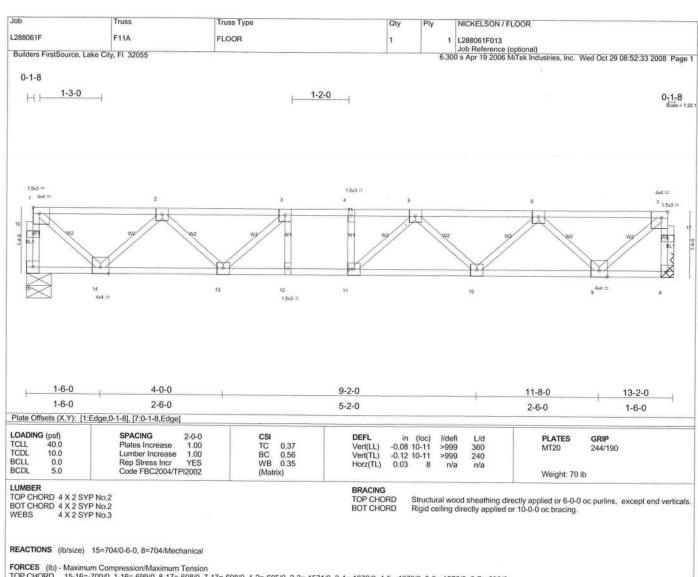
NOTES (5-6)

Unbalanced floor live loads have been considered for this design.
 All plates are 3x3 MT20 unless otherwise indicated.

2) All places and on the second of the secon outer ends or restrained by other means.

5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435 6) Use Simpson THA422 to attach Truss to Carrying member

(Matrix)



TOP CHORD 15-16-700/0, 1-16-699/0, 8-17-698/0, 7-17-698/0, 1-2-685/0, 2-3-1571/0, 3-4-1872/0, 4-5-1872/0, 5-6-1576/0, 6-7-683/0 14-15-0/36, 13-14-0/1278, 12-13-0/1872, 11-12-0/1872, 10-11-0/1835, 9-10-0/1281, 8-9-0/36

WEBS 7-9=0/879, 1-14=0/882, 6-9=-832/0, 2-14=-825/0, 6-10=0/410, 2-13=0/414, 5-10=-360/0, 3-13=-474/0, 5-11=-138/280, 3-12=-64/133, 4-11=-114/17

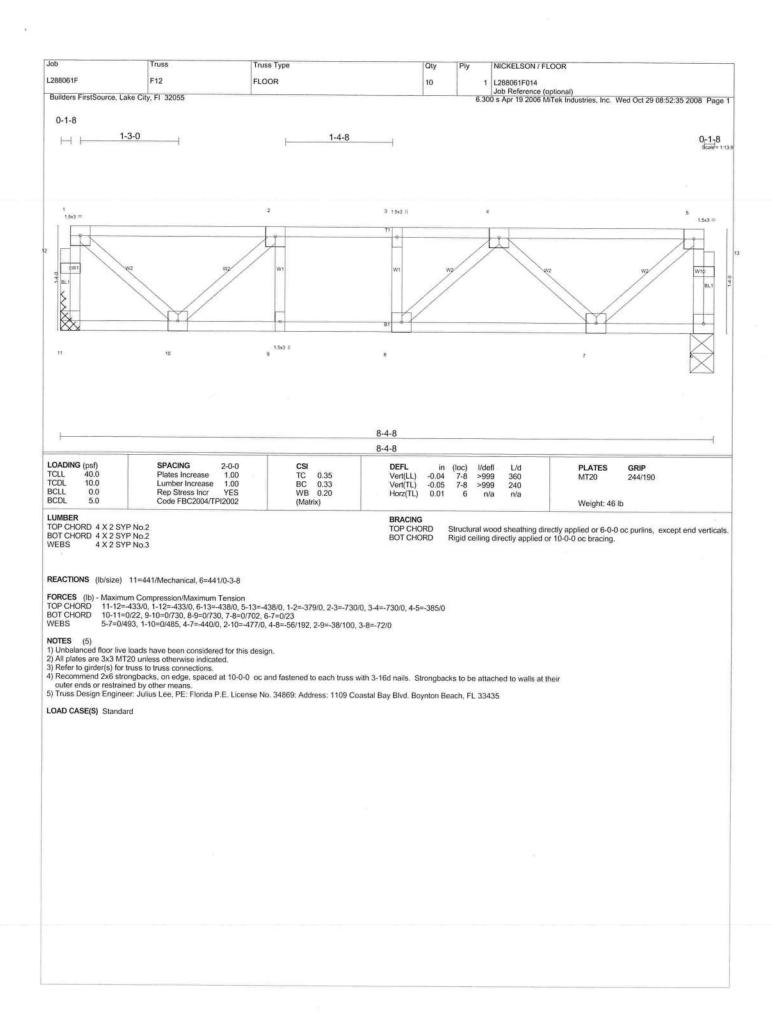
### NOTES (5-6)

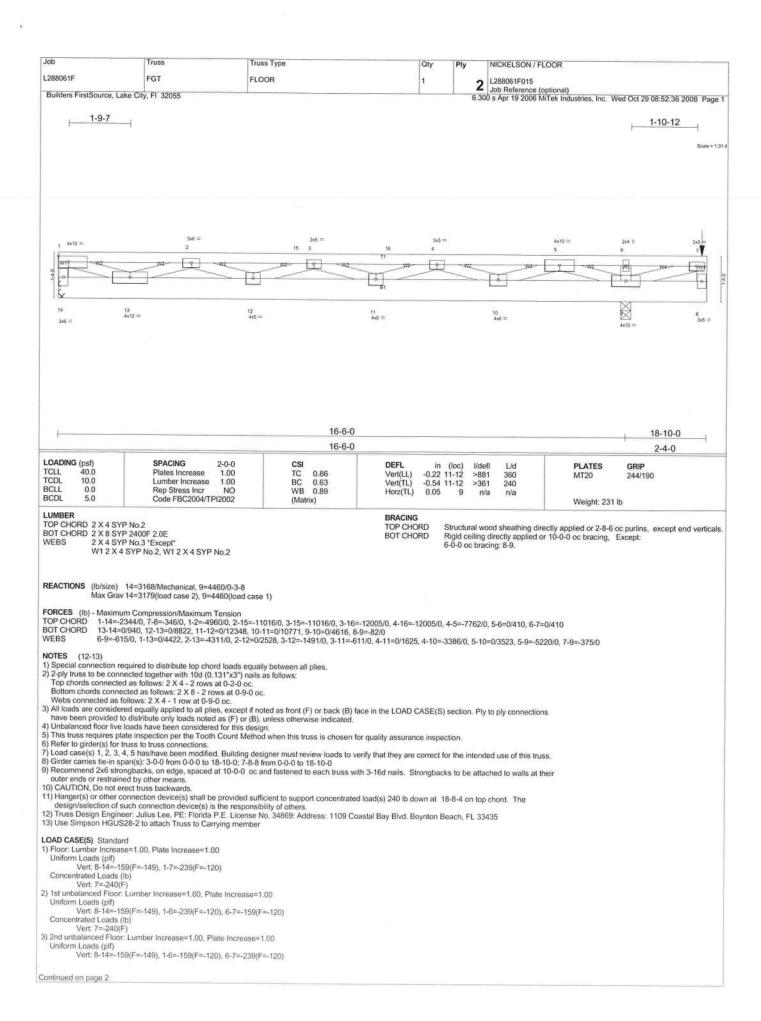
1) Unbalanced floor live loads have been considered for this design.
2) All plates are 3x3 MT20 unless otherwise indicated.
3) Refer to girder(s) for truss to truss connections.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

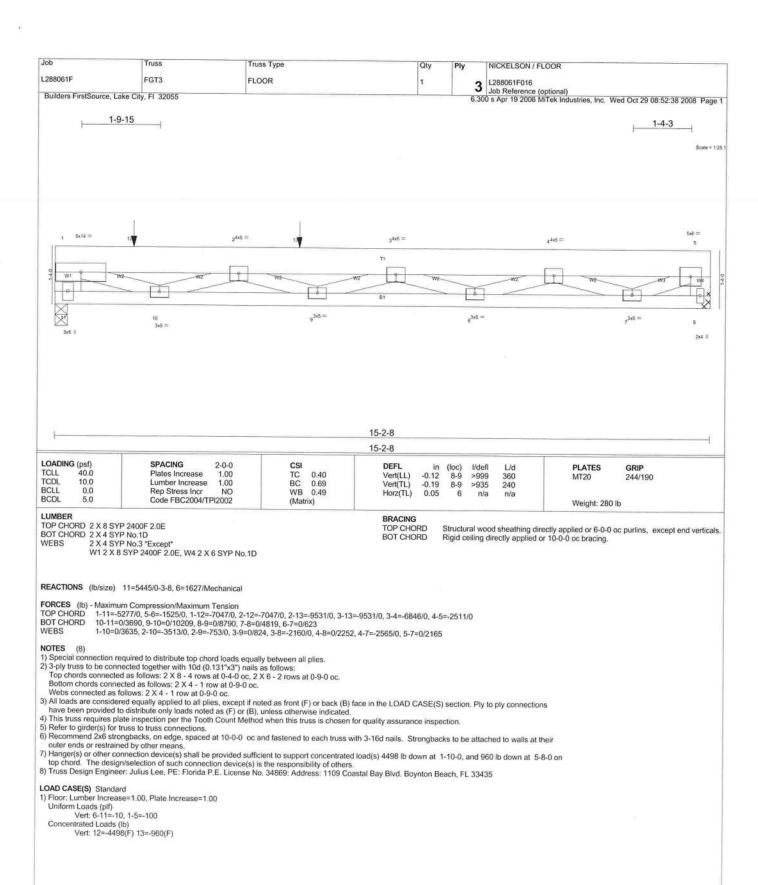
5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

6) Use Simpson THA422 to attach Truss to Carrying member

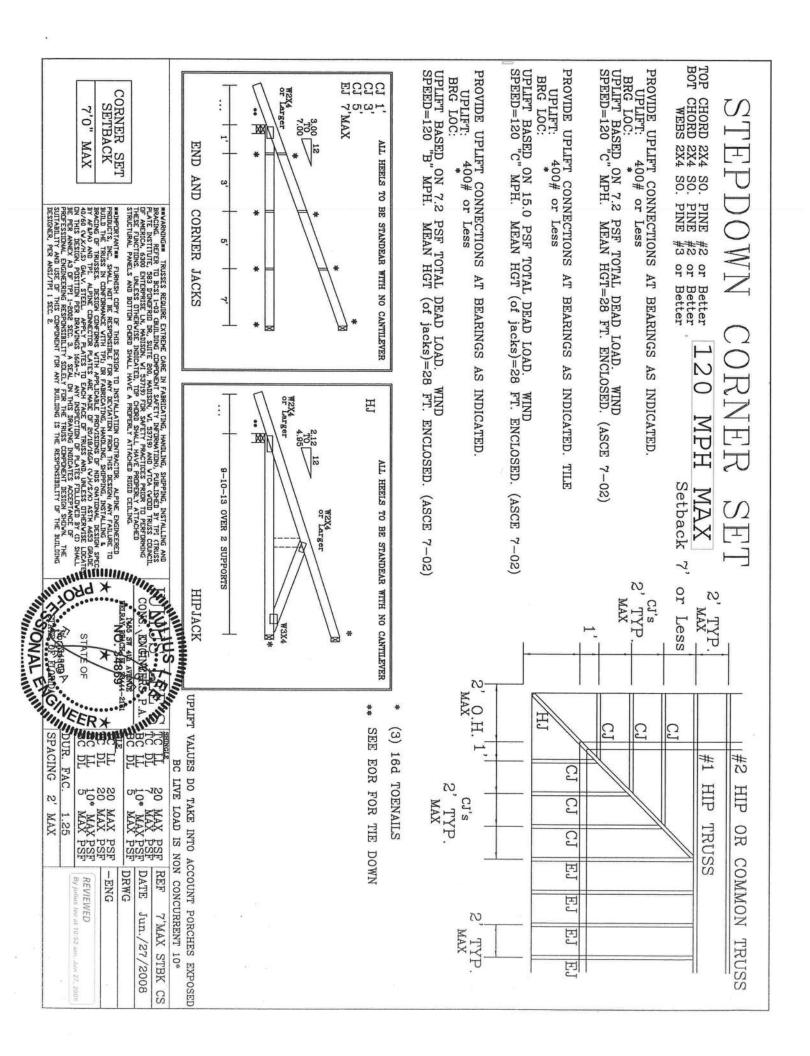




b	Truss	Truss Type	Qty	Ply	NICKELSON / FLOOR
88061F	FGT	FLOOR	1		L288061F015 Job Reference (optional) .300 s Apr 19 2006 MiTek Industries, Inc. Wed Oct 29 08:52:36 2008 Pa
ilders FirstSource, L	ake City, FI 32055			6.	3.300 s Apr 19 2006 MiTek Industries, Inc. Wed Oct 29 08:52:36 2008 Pa
AD CASE(S) Stand Concentrated Loads Vert: 7=:240 Ist chase Floor: Lur Juiform Loads (plf) Vert: 8-14=- Concentrated Loads Vert: 7=:240 Inform Loads (plf)	ard (b) (b) (F) (F) (F) (F) (F) (F) (F) (F) (F) (F	20), 6-16=-159(F=-120), 6-7=-239(F=-120)			3 Apr 19 2000 WITEK INDUSTRES, INC. WED OCT 29 06:52:36 2008 PS



b	Truss	Truss Type	Qty	Ply	NICKELSON / FLOOR
288061F	F01	FLOOR	7	1	L288061F001
ilders FirstSource,	Lake City, FI 32055			6.300	Job Reference (optional) 0 s Apr 19 2006 MiTek Industries, Inc. Wed Oct 29 08:52:15 2008 Page
	707M-0000A				
AD CASE(S) Star	ndard Lumber Increase=1.00 Plate	ncrease=1 00			
Uniform Loads (pl	Lumber Increase=1.00, Plate of the figure of	1.00			
Vert: 10-2 Concentrated Loa	u=-10, 1-4=-20, 4-8=-100, 8-9 ds (lb)	=-20			
Vert: 1=-6	20(F) 9=-33(F)				



### ASCE 7-02: 130 MPH WIND SPEED, 15 MEAN HEIGHT, ENCLOSED, Н 11 1.00, EXPOSURE 0

GROUP SPECIES

AND

GRADES:

GROUP

A: #3

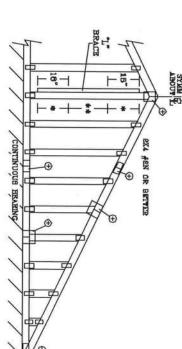
HIM-PIR

STANDARD

OUTS E#3 SWIG NEEKINGS

STANDARD

	_																												
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		1	2	31		0	. (	3			1	6	<b>33</b>		0	.(	3	•0		2	4	71		O	۱,۱	С		SPACING	GABL
	Superior Section	LHL	1 1	V.	2	TIT	H	DI'I	CD I			1	7.7	ì	TIT	ij	OI I	TOD	100 000 000 000		1	V.	j	TIT	ij	DEL	CIT	SPACING SPECIES	GABLE VERTICAL
	STANDARD	STUD	<b>‡</b> 3	#2	<b>+</b> 1	STANDARD	STUD	<b>#</b> 3	\$1 / #B	STANDARD	STUD	<b>†</b> 3	#23	#1	STANDARD	STUD	#3	£1 / #2	Ð	STUD	<b>‡</b> 3	#23	<b>4</b> 1	STANDARD	STUD	₽¥	£1 / #2	GRADE	BRACE
	4' 3"	4. 4.	4' 4"	4' 7"	4 8	4, 2,	4. 2	4. 2	4.	3' 10"			4. 27	4. 3.	8	3, 8,	3' 8"	3. 10		3' 8"	3' 6'	- 5	3' 8"	3. 3.	3' 3"		3' 4"	BRACES	Š
	8 1	7' 1"	7 2	7. 4.	7' 4"	6' 11°	6' 11°	6, 11,	- 1	5, 3,	8 1	100	8,	10.5	D)	8,0,	8 0	8.8	4, 3,	5.0	5. 0,	6° 10"	5' 10"	4' 2"	4' 11"	4' 11"	6" 10"	GROUP A	(1) 1X4 °L"
	6° 1°	7' 1"	7, 50	7' 11"	7' 11"	6' 11°	6' 11"	6 11°	7. 7.	5' 3"	g, 1,		7' 2"		6. 5.	6, 0,		6. 10.	4, 3,	5' O"		6' 3°	6' 3"	4, 2,	4' 11"	4' 11"	6, D.	GROUP B	L" BRACE .
	B' 0"	8, 9,	8, 8,	B' 9"	8, 8,	7' 10"	_	g, 8,		6' 11"	7' 11"	7' 11"	7' 11"	7' 11"	6. 10_	7' 11"	P' 11"	7' 11"	5' B*		6. 8,			5. 6,	6' 5°	6. 6.	6, 11.	GROUP	(1) 2X4 ·
STANKE IN	8' 0"	- 3	9, 5,	9' 5"	8, 2,	7' 10"		g, 8,		6' 11"	B' 1*	1.5	8' 8"	3.77	6. 10_	7' 11"	7' 11"	8 1	5' 8"	6, 2,	8. 9.	7' 5*		5. 6.	6, 5,	6' 6"	7' 1"	A GROUP B	"L" BRACE .
5	10' 5"	10' 6"	10' 5"			10' 5"		10' 5"		9' 4"					8. 8.							8 2,		7' 6-		B' 3*		GROUP A	(2) 2X4 "L"
		10' 11"	10' 11"	11, 5,		10' 6"		10' 5"		9' 4"	8' 11"	9. 11	10' 2"	10' 2"	9. 2.	9' 5"	8, 2,	9. 8 <b>-</b>	7' B'	8' 8"	8, 8,		B' 11°	7' 6"	8 3*	8' 3	- 7	GROUP B	BRACE **
	12' 6"		13' 8"	13. 8.	13' 8"	12, 3,	18' 8"	13' 8"	13' 8"	10' 10"	12 5		12' 6"		10. 7.				8' 10°		10. 4.			8. 8,		10, 1,	10' 10'	GROUP	(1) 2Xe
	12' 6"	14. 0,	14' 0"	14' 0°	14, 0,	12' 3"	13' 8"	13' 8"	14. 0	10' 10"	12, 8,	12. 8.	18' 5"	13' 5"	10. 7.	12' 4"	12' 4"	12, 8,		10' 3"	10. 4.				10' 0"		11. 2.	A GROUP B	T' BRACE .
	14' 0"	14. 0	14' 0"	14' 0"	14. 0	14' 0*	14' 0"	14 0	14. 0	14' 0"	14 0	14. 0	14' 0"	14 0	14. 0.	14' 0"	14' 0"	14. 0	12' 0"	12' 11"	12. 11.	12' 11"	12' 11"	11. 8.	12' 11"	0.1	12. 11.	B GROUP A	(2) ZXB 'L' HRACE
	14' 0"	14. 0	14' 0"	14' 0"	14 0	14' 0"	14' 0"	14 0	14. 0	14' 0"	14 0	14 0	14' D°	14' D"	14. 0	14' D°	14' D"	14. 0.	12' 0"	13' 7"	13' 7"	13' 11*		11. 8.	12' 11"	12' 11"	13′ 3.	GROUP B	BRACE -
CADE SALL SALL SOUTH STATE OF THE PARTY PA	on partitions of the same of t	CONTROLLE UPLANT CONNECTION	THE REAL PROPERTY AND PARTY OF THE PARTY OF	MAN MOLECUL DAGE DAGE	CABLE IKUSS DI				8	15	SOUTHING PINE	-	#1 & H	A-WEH	GROOT			Di not broad	THE THE	13	DOUGLAS FIR-LARCH	21.00	34	SPRUCE-DUC-JIB	GROOM		BRACING GROUP SPE		



DIAGONAL BRACES OPTION:
VENTICAL LENGTH MAX BE
DOUBLED WICHN DIAGONAL
HRACE IS USBED. CONNECT
HRACE IS USBED. CONNECT
HRACE IS USBED. WAS BEAGE
AT RACH YELD. MAX WEB
TOTAL LENGTH IS 14°.

GABLE TRUBS

VERTICAL INIGTH SHOWN IN TABLE ABOVE.

ZX4 SP #2N, DF-L #2,
SPF #/#Z, DR BETTER
DIAGONAL BRADT;
SINGLE OR DOUBLE
CUT (AS SELTEN) AT
TUDDING PAIN

UPPIN RND.

CABLE
TRUSS
DETAIL
NOTES:

DOUGLAS FIR-LARCH

HEM-PIR GROUP B:

PLYWOOD OVERHANG. CONTINUOUS BEARING (6 PSF TC DEAD LOAD). VE LOAD DEPLECTION CRITERIA IS L/240. BLE END SUPPORTS LOAD FROM 4' 0"

ATTACH EACH "L" BRACE WITH 104 NAILS.

# FOR (1) "L" BRACE; SPACE NAILS AT 2" O.C.

# FOR (2) "L" BRACES; SPACE NAILS AT 3" O.C.

IN 16" END ZONES AND 6" O.C. BETWEEN ZONES. T. BRACING MUST BE A MINIMUM OF 80% OF WEB

MEMBER LENGTH.

DEBIGN FOR	PEAK, SPLICE, AND HEEL FLATES
2.5X4	EB THAN 11'
ZX4	GREATER THAN 4' D', BUT
1X4 OR EXS	IRSS THAN 4' 0"
NO BPLICE	VERTICAL LENGTH
PLATE SIZES	GABLE VERTICAL PLAT

7-02-CAB13015

STD CABLE 15 E HT

REVIEWED  By julius loe at 12:00 pm, Jun 11, 2008  TRUSSES REQUIRE EXTREME CARE IN FARRICATION, HANDLING, SUPPDM, INSTALLING AND CONS. TRUSSES REQUIRE EXTREME CARE IN FARRICATION, POSITION POSITION, POSITION FOR ANY OF THE ACTION THAT ACCOUNTS THAT THE ACCOUNTS THAT THE ACTION THAT THE ACTION THAT THE FORMAN INCESS OF THE ACTION THAT ACCOUNTS THAT THE ACCOUNTS THAT THE ACTION THAT ACCOUNTS T
JULIUS LEERS P.A. THE CREASES GED AND CONS. ENGINEERS P.A. DELIAN BEACH, R. 3344-2161.  No. 34869  STATE OF FLORIDA
MAX. TOT. LD. 60 PSF

### ASCE 7-02: 130 MPH WIND SPEED, 30, MEAN HEIGHT, ENCLOSED, II 1.00, EXPOSURE 0

BRACING GROUP SPECIES

AND GRADES:

GROUP

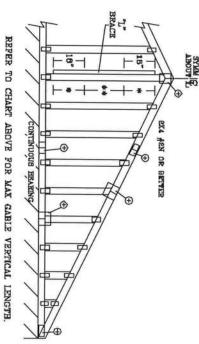
A.

#2 STANDARD

SOUTHERN PINE

STANDARD

	Γ		M	Α	X		(	7/	Δ	2		E	1	V	F	'F	ףי	T	С	۸	T		Τ.	F	אי	](	ğΓ	ТН	
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		1	2	71		0	. (	C .			1	6	"	j	0	.(	3			2	4	. "		C	۱, ۱	C		ACING	CARLE
	E 250 SE 2		1	7	2	TIT	I I	J. I	201			1	7.	2	111	T T	CI I	Z D T	20 044 EC 44 05 CT	しずし	1	7.	j	TIT	I I	OFF	CDE	SPACING SPECIES	CABLE VERTICAL
	STANDARD	STUD	*3	#2	<b>1</b> 1	STANDARD	STUD	£3	£1 / #2	STANDARD	STUD	<b>†</b> 3	#23	<b>4</b> 1	STANDARD	STUD	#8	£1 / #2	STANDARD	STUD	<b>t</b> 3	#23	<b>4</b> 1	STANDARD	STUD	₽\$	2# / IF	GRADE	BRACE
	4. 0.	4. M	4' 2"	4. 4.	4 5"			3 11"	4. 0.	3' 6,	3 8	3. 8.	3' 11"	4, 0,	3. 7.	8, 7,	3. 7.	3.	8 0	3. 3.	3. 3.	3' 6"	3' 6"	2' 11'	3' 1"	3' 1"	si,	BRACES	Z S
	Q.	6' 4'	8 6	6' 11"		5' 4"	8 3	φ. α <u>,</u>	6. 11	4' 9"	5 6	5° 7"	8' 4"	B 4*	4. 8	5, 6,	U	6.4		4 8		5' 6"	5' 8"	3,	4' 6"	4' 5	5. 6,	GROUP A	(1) 1X4 ·
	5' B"	6' 4"	6, 20	7' 8"	7' 8"	5' 4"	6. 3.	8, 3,	77. 22.	4' 9"	5, 8,	6. 7.	6' 10"	B' 10*	4. B.	6, 6.	5, 5	6. 6.	3' 10"	4' 8"	4' 8"	5' 11"	5' 11"	3, 9.	4' 5°	4' 5"	6' 8"	GROUP H	"L" BRACE .
	7' 3"	8' 3"	8, 3,	B' 3"		7' 1"	83	8' 3"	8° 3°	6" 3"	7' 3"	7. 4-		7' B"	6. 5.	7' 2"	7' 2"	7' 8"	5' 1"	5' 11"	6. 0,	6, 8,	6, B	6. 0.		6. 10	8' 8"	GROUP A GROUP	$\vdash$
PULVE I	7' 3"	8. 6.	B' 6"	8' 11"		7' 1"	8' 3"	8' 3"	8. 6.	6' 3°	7' 3*	7' 4"	B' 1"	7	6. 2	7' 2"	7 2	7' 8"	6' 1"	5' 11"	8. 0.	7' 0"	7' 00	5.0	5' 10°	5' 10"	8. 8.	GROUP B	(1) 2X4 "L" BRACE .
2	B. 8.	9' 10"	9' 10"	8, 10,	1.73	හ <sup>7</sup> ල "	9' 10"	9' 10"	1000	6. 5.	B' 11"	8. 11	B' 11"		8. 3.	8' 11"	B' 11"	B. 11.	8° 11"	7' 10"	7' 10"	7° 10°	- 1	6, 9,	7' 10"	7' 10"	7' 10"	GROUP A	(2) 2X4 "
	8, 8,	10′ 4″	10' 4"	10, 2,	10' 7"	හු හ	9' 10"			B' 5°	8, 2,	9. 6.	8, 4,	B, 2,	8. 3.	B' 11"	B' 11"	9. 23	6' 11"	8' O"	-	- 1	8,5		7' 10"	7' 10"	8.0.	GROUP B	(2) 2X4 "L" BRACE **
	11' 4"	12' 11"	- 1	200			18, 10,,	12' 11"	12' 11"	8, 8,	11, 4,	11. 6.	11, 9,		9, 3-			11. 9.		8, 3,	9. 4.	10' 3"	10' 3"	7' 10"	9' 1"		10" 3"	GROUP A	(1) exe 'L'
	11' 4"	13" 1"	18' 3"	13' 11"	13' 11"	11' 1"	12' 10"	12' 11"	15' 4"	9° 9"	11' 4"	11. 6.	12' B"	12' B"	8. 4.	11, 1,	11, 5,	12' 1"	8, 0.	9 3	9 4	11' 1"	11, 1,	7. 10-	9, 1,	9 1	10' 7"	GROUP B	BRACE *
	14' 0"	14. 0	14' 0"	14' 0"	14 0	14' 0"	14' 0"	14 0	14. 0	13' 9"	14 0	14. 0-	14' 0"	14, 0	12. 11.	14' 0"	14. 0	14. 0	10' 10"	12' 3	12. 3,	12' 3"	12' 3"	10. 7-	12' B*	12' 3"	12' 3"	B GROUP A GROUP B	.T. axz (z)
	14' 0"	14. 0	14' 0"	14 0	14 0	14' 0"	14' 0"	14 00	14. 0"	13' 3"	14.0	14. 0-	14' D°		12 11	14' 0"	14' 0"	14. 0.	10' 10"	12' 6"	12' 8"	19' 2°	13' 2"	10. 7.	12' 3"	12' 3"	12' 7"	GROUP B	HRACE ***
CARL END BUFFORTS LOAD	O PRINCES COOK IN THE PRINCES	PROVIDE UPLIFT CONNECTION	man and and an an annual states of the state	INC. NOISALIDBU UVOI GALI	GAHLE TRUSS DI				180	#3	SOUTHING PINE		#1 & 1	HEW-F	GROUI			STATUMENT	M.IS	25	DOUGLAS FIR-LARCH	2100	12	BUCB-	GROU		BRACING GROUP SPE		



DIAGONAL BRACE OPTION:
VERTICAL LENGTH MAY BE
DOUBLED WINN DIAGONAL
HRACE IS USED. CONNECT
HRACEANAL BRACE TOR BEOF
AT RACE MID. MAX WEB
TOTAL LENGTH IS 14\*.

GABLE THUSS

ZX4 SP OR

III-L #2 OH

BETTER DIAGONAL

FRACE: SINGLE

OR DOUBLE

CUT (AS SHOWN)

AT UPPER END

CABLE TRUSS DETAIL NOTES:

DOUGLAS FIR-LARCH

HEM-PIR #1 & BIR GROUP B:

ASLE END SUPPORTS LOAD FROM 4' 0" DVILLIDHORS WITH 2' 0" DVILRHANG, DR 12" PLYWOOD OVERHANG. OVIDE UPLIT CONNECTIONS FOR 180 FLF OVER CONTINUOUS BEARING (6 PSF TC DEAD LOAD). TE LOAD DEPLECTION CRITERIA IS L/240.

ATTACH EACH 'L' ERACE WITH 104 NAILS.

\* FOR (1) 'L' BRACES SPACE NAILS AF 2° O.C.

\* FOR (2) 'L' BRACES: SPACE NAILS AF 3° O.C.

BY 18° END ZONES AND 4° O.C. BETWEEN ZONES.

N 18° END ZONES AND 6° O.C. BETWEEN ZONES.

MIMBIR LENGTH. T. BRACING MUST BE A MINIMUM OF 80% OF WEB

N 4' 0' IX4 OR EX	R TO COMMON THURS DESIGN FOR	MUMON 11 6 11 6 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	8 1 3
	THAN 11' 6" 2.5X4	THAN 4' D', BU	,XZ
THAN 4' D', BUT ZX4		THAN 11' 6"	2.5X

DELZAY BLACH, FL	THE COMME
1466 BW 4th DELEAY BEACE, FL	REVIEWED  STATE OF  REVIEWED  By julius lee a
COLID. MICH	NO. \$4869 ANDURA WALL SALE TAKETING.
WASDELY TRUSSES REQUIRE EXTREME CARE IN FARRICATING, HANDLING, SHIPPING, INSTALLING AND $JUUIUUUUU$ $UUUUuuuuuuuuuuuuuuuuuuuuuuuuuuuu$	C E MONTH TRAS

60

PSF

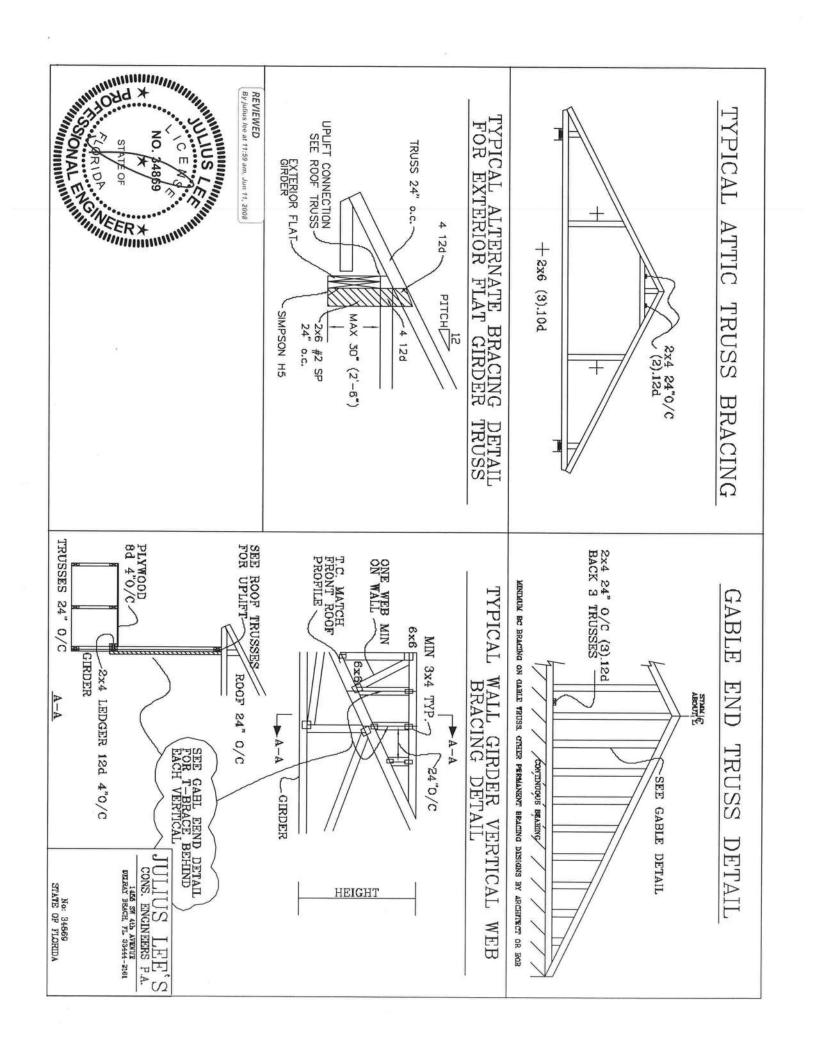
DATE REF

11/26/03

ASCE7-02-CAB13030

DWG MIASK 24D CYBIE 30, 5 HJ

24.0"



BOT CHORD 2X4 2X4 ななな 222 BETTER BETTER BETTER

# PIGGYBACK DETAIL

TYPE

SPANS

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30,

34

88 5

52

TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTION CHORD MAY BE OMITTED. TRUSS TOP CHORD WITH 1.5X3 PLATE. ATTACH VERTICAL WEBS TO

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

110 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BIDG, LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST CAT I, EXP C. WIND TO DI=5 PSF, WIND BC DI=5 PSF

110 MPH WIND, 30' MBAN HGT, FEG ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TO DL-5 PSF, WIND BC DL-5 PSF

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS: 130 MPH WIND, 30' MEAN HCT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C. WIND TC DL=6 PSF, WIND HC DL=6 PSF

> D C Ħ

5X4 EX3

**5X6** 

**6X**5

.5X4

1.5X4

1.5X4 5X6

OR SX6 TRULOX AT 4' HOTATED VERTICALLY

00,

**4**X8 284

5X6

**6X8** 

**БХ**8 3X5

2.5X4

2.6X4

FRONT FACE (B,\*) FLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX. EITHER PLATE LOCATION IS ACCEPTABLE XX V 12 20' FLAT TOP CHORD MAX SPAN TYP. Ħ 图 MAX SIZE OF ZXIZ B C-TYP. 英 D-SPIJOR 

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WEB LENGTH	REQUIRED BRACING
0' TO 7'9"	NO BRACING
7'9" TO 10'	1x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 8d NAILS AT 4" OC.
10' TO 14'	ZX4 "I" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d NAILS AT 4" OC.

	$\mathcal{C}$	(	$\subset$	ATTACH I PABRICAT (4) 0.120 PIGGYBAC SPACE 4
(	)	(	C	TEETH TO TION AT TION AT OK SPECIA OC OR I
۰	0	٥		O THE PIGGYBACK AT THE TYACH TO SUPPORTING TRU 75" NAILS PER FACE PER P. TAL PLATE TO EACH TRUSS LESS.
٥	a	0	٥	CYBACK SUPPORT BER FACI
0	٥	0	0	CE PER
0	a	٥	٥	THE TIME TRUSS ER PLY.
L	a	0	٥	TIME OF ISS WITH LY. APPLY FACE AND
-	b	4	-	K

PIGGYBACK WITH 3X8 TRUICX OR ALPINE PIGGYBACK SPECIAL PLATE.  THIS DRAWING REPLACES DRAWINGS 634,018 634,018 634,017 & 647,045  THIS DRAWING REPLACES DRAWINGS 634,018 634,018 634,017 & 647,045  THIS DRAWING REPLACES DRAWINGS 634,018 634,018 634,017 & 647,045  THIS DRAWING REPLACES DRAWINGS 634,018 634,018 634,017 & 647,045  THIS DRAWING REPLACES DRAWINGS 634,018 634,018 634,017 & 647,045  THIS DRAWING REPLACES DRAWINGS 634,018 634,018 634,017 & 647,045  THIS DRAWING REPLACES DRAWINGS 634,018 634,018 634,017 & 647,045  THIS DRAWING REPLACES DRAWING REPLACES DRAWING REPLACES DRAWINGS 634,018 634,018 634,017 & 647,045  THIS DRAWING REPLACES DRAWING REPLACES DRAWING REPLACES DRAWINGS 634,018 634,018 634,017 & 647,045  THIS DRAWING REPLACES DRAWING REPLACES DRAWINGS 634,018 634,018 634,017 & 647,045  THIS DRAWING REPLACES DRAWING RE	WALL COUNTY	STATE OF THE PROPERTY OF	*	NO. 34869	- 10 - MA	The state of the s	STILL STILL	
JULIUS CONS. ENGINE  1466 SW 456 AV  DERAY BEACH, 71.  No: 94866 STATE OF FL		REVIEWED  By julius lee at 11:59 am, Jun 11, 2008	SELECTURAL PARELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGHD CELLING.	WERICA, 63th ENTERPRISE LY, NATISSEN, VI 137199 FOR SAFETY PRACTICES PRIOR TO PERSONAGE TO PERSO	REFER TO BOST 1-03 GUILDING COMPONE ASTITUTE 383 O'UNDERSO DR. SUITE 210		PIGGYBACK WITH 3X8 TRULOX OR ALPINE PIGGYBACK SPECIAL PLATE.	
MAX LOADING 55 PSF AT 1.33 DUR. FAC. 50 PSF AT 1.25 DUR. FAC. 47 PSF AT 1.15 DUR. FAC. SPACING 24.0*	STATE OF FLORIDA			DELEGATION AND DESCRIPTION OF THE CASE OF	CONS. ENGINEERS P.A.	O,HHI SIIIIIII	THIS DRAWI	}
		47 PSF AT 1.15 DUR. FAC.	50 PSF AT	1.33 DUR. FAC.	55 PSF AT	MAX LOADING	NG REPLACES DRAWINGS	

### VALLEY TRUSS DETAIL

TOP CHORD BOT CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER. 2X3(\*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER. 2X4 SP #3 OR BETTER.

- ZX3 MAY BE RIPPED FROM A ZX6 (PITCHED OR SQUARE).
- \*\* ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH: FHC 2004 110 MPH, ASCE 7-02 110 MPH WIND OR (3) 16d ASCE 7-02 130 MPH WIND. 15' MEAN HEIGHT, ENCLOSED BUILDING, EXP. C. RESIDENTIAL, WIND TC DL=5 PSF. (2) 18d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR FOR

CUT FROM 2X6 OR LARGER AS REQ'D

12 MAX.

W2X4

12

4-0-0 MAX

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".

MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0"

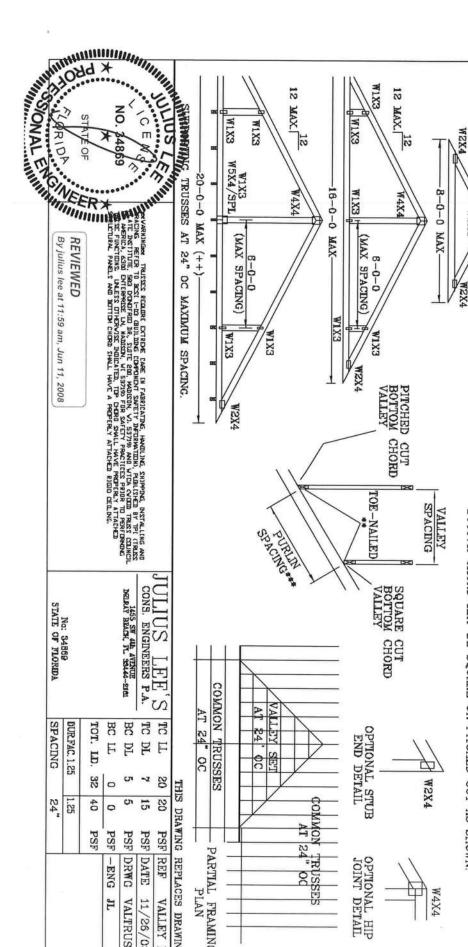
TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH: PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY INSTALLATION TRUSS

PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN

\* NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD. ENGINEERS' SEALED DESIGN. BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON

++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0"

BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN



FRAMING

PEPI ACES DRAWING

			ED RIGO CELLING.	TICES PRIOR TO PERFORMS	S, SHIPPING, DISTALLING AND	J1	
STATE OF PLORIDA	No: 34869			DELEAT BEACH, IL SSA44-21CL	CONS. ENGINEERS P.A.	JULIUS LEE'S	
SPACING	DURFAC, 1.25	TOT. LD.	BC II	BC DL	TC DL	TC II	
	EX	32 40	0	Ü	~z	20 20	
24"	1.25	40	0	Ç	15	20	1
		PSF	PSF	PSF	PSF	PSF	San Mark
			PSF -ENG JL	DRWG	PSF DATE	PSF REF	ACTUAL VALUE
			IL	VALTRUSS1103	11/26/03	VALLEY DETAIL	THE PARTY AND THE PARTY NAMED IN TAXABLE PARTY

## TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

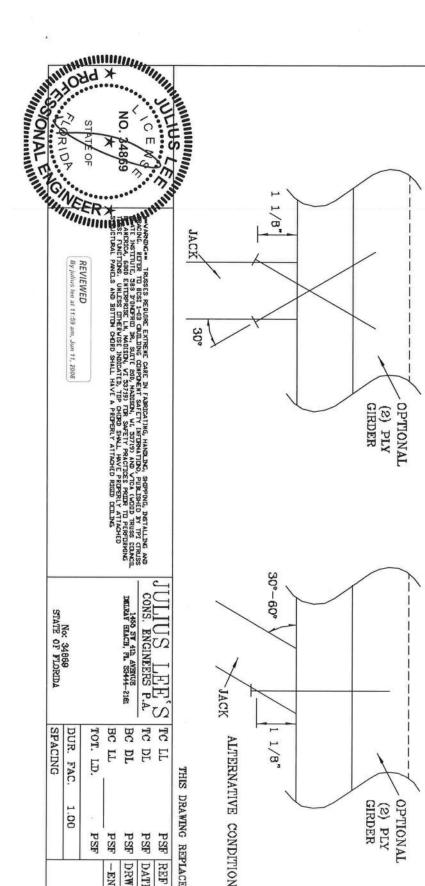
PER ANSI/AF&PA NDS-2001 SECTION 12.4.1 — EDGE DISTANCE, END DISTANCE, SPACING: "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

MAXIMUM VERTICAL RESISTANCE OF 16d (0.162"X3.5") COMMON TOE-NAILS

ALL VALUE	4 n		ယ	N	NUMBER OF TOE-NAILS			
IS MAY BI	493#	394#	296#	187#	1 PLY	SOUTHE		
ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR	639#	611#	383#	256#	2 PLIES 1 PLY	SOUTHERN PINE		
ID BY APP	452#	361#	271#	181#	1 PLY	DOUGLAS		
ROPRIATE	585#	468#	351#	234#	2 PLIES	DOUGLAS FIR-LARCH		
DURATION	390#	312#	234#	156#	1 PLY			
OF LOAD	507#	406#	304#	203#	2 PLIES	HEM-FIR		
FACTOR.	384#	307#	230#	154#	1 PLY	SPRUCE		
	498#	397#	298#	189#	2 PLIES	SPRUCE PINE FIR		



1/B"

THIS DRAWING REPLACES DRAWING 784040

	By julius lee at 11:59 am, Jun 11, 2008	REVIEWED	DICTURAL PANELS AND BUTTON CHORD SHALL HAVE A PROPERLY ATTACHED REED CELLING	ANTE INSTITUTE, 383 D'ONDERED DR., SUITE 200, NADE ANERICA, 6300 ENTERPRISE LN., NADES, VICTOR 7179 ANERICA, 6300 ENTERPRISE LN., NADES, VICTOR 7179	AVARADAG. REFER TO BISSI 1-43 GRADING COMPONENT SAFETY (HFDRWATING, PUBLISHED BY TPI CIRUSS BACONE. REFER TO BISSI 1-43 GRADING COMPONENT SAFETY (HFDRWATING, PUBLISHED BY TPI CIRUSS BACONE. REFER TO BISSI 1-43 GRADING COMPONENT SAFETY (HFDRWATING). PUBLISHED BY TPI CIRUSS	
			A PROPERLY ATTACHED RIGID CILING	, 383 YONDRO DR., SUITE 200, HADISON, HI 337(9) AND YTCA (HOUD TRUSS COLNCIL) BRITCHPRISE LM, MADISON, HI 337(9) FOR SAFETY PRACTICES PRIER TO PERFORMING		
STATE OF FLORIDA	No: 34889			DELRAY BEACH, FL 33444-2161	CONS. ENGINEERS P.A.	JULIUS LEE'S
SPACING	DUR. FAC.	TOT. LD.	BC LL	BC DL	TC DL	TC LL
	1.00	PSF	PSF	PSF	PSF	PSF REF
			F -ENG JL	DRWG	DATE	REF
			IL	CNTONAIL1103	09/12/07	TOE-NAIL

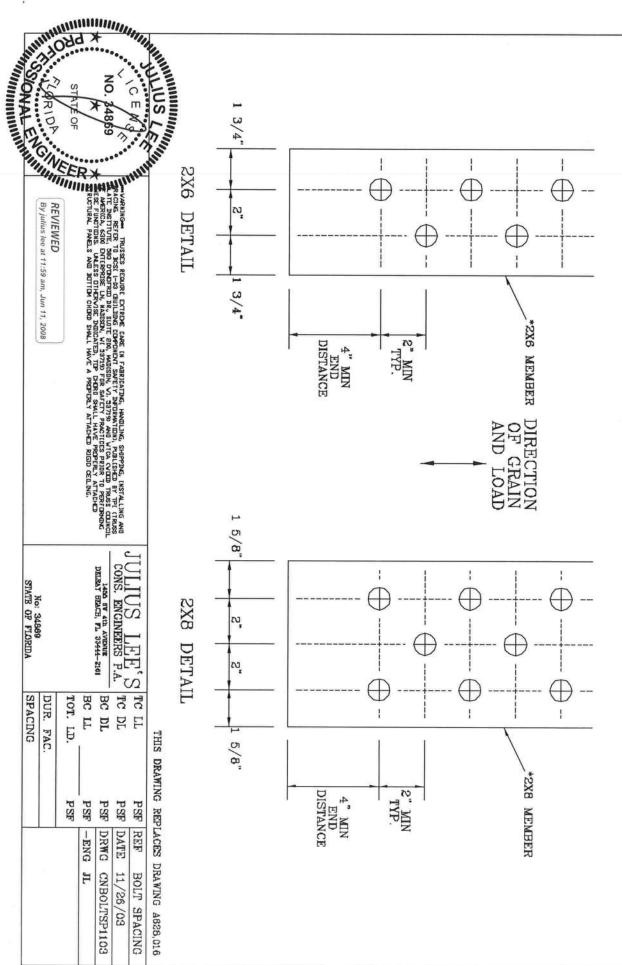
# DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN

\* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN.

BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. BOLT QUANTITIES AS NOTED ON SEALED DESIGN MUST BE APPLIED IN ONE OF THE PATTERNS SHOWN BELOW.

WASHERS REQUIRED UNDER BOLT HEAD AND NUT



# TRULOX CONNECTION I

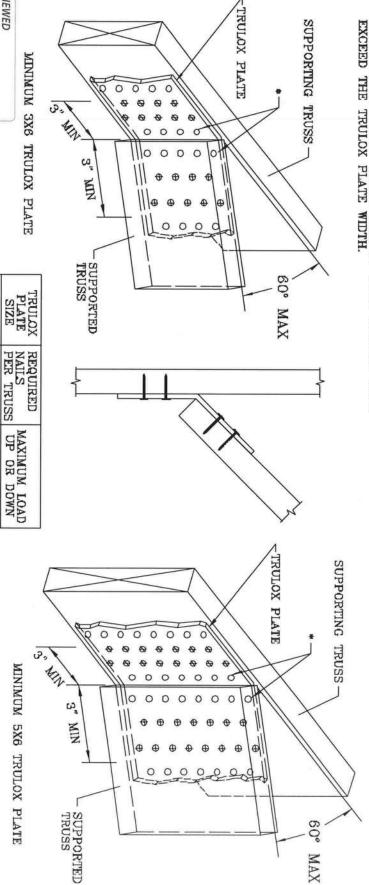
11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (\( \phi \)).

NAILS MAY BE OMITTED FROM THESE ROWS

THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST

TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN



NO. 34869

Gem TRISSES ROURE EXTROME CARE IN FARRICATING, HANGLING, SHAPPAG, INSTALLING AND RETER TO 1851 THE GIBLIOUND COMPRISHED SHETT NERWATION, PUBLISHED BY THE FIT (TRISSE STITUTE, 583 YONGFROM DR., SUITE BIO, HANGSDY, VI. 1872% AND VITCA CVITIM TRUSS COUNCIL, ASSOCIATED SHE SHETT PRACTICES PROJECT IN EXERCISHING CONTINUES. UNLESS OTHERVISE MODIFACED, TIP CADDS SHALL HAVE PROJECTLY ATTACHED AND THE SHAPPAG AND THE CONTINUES.

REVIEWED

3X6 6X6

16 9

#088 350#

THIS DRAWING REPLACES DRAWINGS 1,158,989 1,158,989/R 1,154,944 1,152,217 1,152,017 1,159,154 & 1,151,524

REF DATE DRWG -ENG

> CNTRULOX1103 11/26/03 TRULOX

JULIUS LEE'S CONS. ENGINEERS P.A. DELEVAL BEVOY, LT. 32444-5100

No: 34869 STATE OF FLORIDA

### NO. 34889 By julius lee at 11:58 am, Jun 11, 2008 REVIEWED TO BEARING TO BEARING ADD 2x4 #2 SP ONE FACE 10'-0" O/C MAX SYSTEM-42 OR FLAT TRUSS ALTERNATE DETAIL FOR STRONG BACK WITH VERTICAL STRONG (3)10d-10'-0" O/C MAX NOT LINING UP BACK DETAIL -(3)10d 2x6 #2 SP 2x6 #2 SP JULIUS LEE'S cons. ENGINEERS P.A. DZZRAY BEACH, FL 33444-2161 No: 34869 STATE OF FLORIDA

### MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

### Maximum Uniform Load Applied to Either Outside Member (PLF)

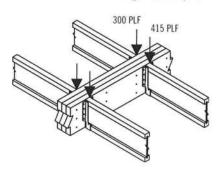
			Connector Pattern									
		Connector On-Center Spacing	Assembly A	Assembly B	Assembly C	Assembly D	Assembly E	Assembly F				
Connector Type	Number of Rows		2" 134"	13/4"	13/4" 31/2"	134" 31/2" 13/4"	31/2"	2 2 134"				
			3½" 2-ply	51/4" 3-ply	51/4" 2-ply	7" 3-ply	7" 2-ply	7" 4-ply				
10d (0.128" x 3")	2	12"	370	280	280	245						
Nail <sup>(1)</sup>	3	12"	555	415	415	370						
1/2" A307		24"	505	380	520	465	860	340				
Through Bolts(2)(4)	2	19.2"	635	475	655	580	1,075	425				
· · · · · · · · · · · · · · · · · · ·		16"	760	570	785	695	1,290	505				
THE STATE OF THE S	2	24"	680	510	510	455						
SDS 1/4" x 31/2"(4)		19.2"	850	640	640	565						
		16"	1,020	765	765	680						
		24"				455	465	455				
SDS 1/4" x 6"(3)(4)	2	19.2"				565	580	565				
		16"				680	695	680				
		24"	480	360	360	320						
USP WS35 (4)	2	19.2"	600	450	450	400						
		16"	715	540	540	480						
		24"				350	525	350				
USP WS6 (3)(4)	2	19.2"				440	660	440				
3 No. 5 APRIL 12 (1975) 1974 (1975) 1974 (1975)		16"				525	790	525				
		24"	635	475	475	425						
33/8" TrussLok <sup>(4)</sup>	2	19.2"	795	595	595	530						
HUSSEUK		16"	955	715	715	635						
123		24"	00.0020	500	500	445	480	445				
5" Trucklah(4)	2	19.2"		625	625	555	600	555				
TrussLok(4)		16"		750	750	665	725	665				
74.10		24"				445	620	445				
63/4"	2	19.2"				555	770	555				
TrussLok(4)		16"				665	925	665				

<sup>(1)</sup> Nailed connection values may be doubled for 6" on-center or tripled for 4" on-center nail snacing

### General Notes

- Connections are based on NDS® 2005 or manufacturer's code report.
- Use specific gravity of 0.5 when designing lateral connections.
- Values listed are for 100% stress level. Increase 15% for snow-loaded roof conditions or 25% for non-snow roof conditions, where code allows.
- Bold Italic cells indicate Connector Pattern must be installed on both sides.
   Stagger fasteners on opposite side of beam by ½ the required Connector Spacing.
- Verify adequacy of beam in allowable load tables on pages 16–33.
- 7" wide beams should be side-loaded only when loads are applied to both sides
  of the members (to minimize rotation).
- Minimum end distance for bolts and screws is 6".
- Beams wider than 7" require special consideration by the design professional.

### Uniform Load Design Example



First, check the allowable load tables on pages 16-33 to verify that three pieces can carry the total load of 715 plf with proper live load deflection criteria. Maximum load applied to either outside member is 415 plf. For a 3-ply 134" assembly, two rows of 10d (0.128" x 3") nails at 12" on-center is good for only 280 plf. Therefore, use three rows of 10d (0.128" x 3") nails at 12" on-center (good for 415 plf).

### Alternates:

Two rows of 1/2" bolts or SDS 1/4" x 31/2" screws at 19.2" on-center.

<sup>(2)</sup> Washers required. Bolt holes to be 1/16" maximum.

<sup>(3) 6&</sup>quot; SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.

<sup>(4) 24&</sup>quot; on-center bolted and screwed connection values may be doubled for 12" on-center spacing.

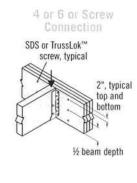
### Connector Pattern Assembly A Assembly B Assembly C Assembly D Assembly E Assembly F T Number of **Connector Type** Connectors 1 2" 31/2" 13/4" 134" 13/4" 31/2" 31/5" 31/2 51/4 51/4 2-ply 3-ply 2-ply 3-ply 2-ply 4-ply 6 1.110 835 835 740 10d (0.128" x 3") 12 2,225 1,670 1,670 1,485 18 3.335 2.505 2 505 2,225 24 4,450 3,335 3,335 2,965 4 1.915 1,860(2) 1,405(2) **SDS Screws** 1,435(4) 1.435 1,275 1/4" x 31/2" or WS35 2,870 2,150 (4) 2.150 1,915 2.785(2) 2.110(2) 1/4" x 6" or WS6(1) 2,870 (4) 8 3,825 2,870 2,550 3,715(2) 2,810(2) 4 2.545 1.910 (4) 1.910 1.695 1,925(3) 1.775(3) 33/8" or 5" 2,665(3) 6 3,815 2,860 (4) 2,860 2,545 2,890(3) TrussLok™ 5.090 3,815 (4) 3,855(3) 3,815 3,390 3,550(3)

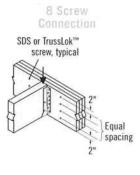
### Point Load—Maximum Point Load Applied to Either Outside Member (lbs)

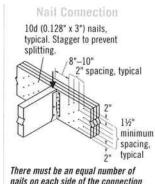
- (1) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.
- (2) 6" long screws required.
- (3) 5" long screws required.
- (4) 3½" and 3¾" long screws must be installed on both sides.

### See General Notes on page 38

### Connections







### Point Load Design Example



First, verify that a 3-ply 1¾" x 14" beam is capable of supporting the 3,000 lb point load as well as all other loads applied. The 3,000 lb point load is being transferred to the beam with a face mount hanger. For a 3-ply 1¾" assembly, eight 3¾" TrussLok™ screws are good for 3,815 lbs with a face mount hanger.

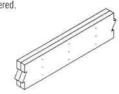
### MULTIPLE-MEMBER CONNECTIONS FOR TOP-LOADED BEAMS

### 13/4" Wide Pieces

- Minimum of three rows of 10d (0.128" x 3") nails at 12" on-center.
- Minimum of four rows of 10d (0.128" x 3") nails at 12" on-center for 14" or deeper.
- If using 12d-16d (0.148"-0.162" diameter) nails, the number of nailing rows may be reduced by one.
- Minimum of two rows of SDS, WS, or TrussLok™ screws at 16" on-center. Use 3¾" minimum length with two or three plies; 5" minimum for 4-ply members. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. For 3- or 4-ply members, connectors must be installed
- on both sides. Stagger fasteners on opposite side of beam by ½ of the required connector spacing.
- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.

### 31W With Please

- Minimum of two rows of SDS, WS, or TrussLok™ screws, 5" minimum length, at 16" on-center. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. Connectors must be installed on both sides. Stagger fasteners on opposite side of beam by ½ of the required connector spacing.
- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.
- Minimum of two rows of ½" bolts at 24" on-center staggered.



Multiple pieces can be nailed or bolted together to form a header or beam of the required size, up to a maximum width of 7"



**Project Information:** 

Builder: Cash Account

Model: custom

Builders FirstSource Job #: L288061

Street: 440 SW Emerywood Glen

City: Lake City County: Columbia

Building Code: FBC2004/TPI2002

Computer Program Used: MiTek 6.3

**Gravity Loads** 

Roof: 32 psf Total

Floor: 55 psf Total

Wind

Wind Standard: ASCE 7-02 Wind Speed: 110 mph

Mean Roof Ht: 20 ft

Builders FirstSour 2525 E. Duval St. Lake City, FL 32055

Exposure: B

FLORIDA. 33444

JULIUS LEE'S CONSULT. ENGR INC 1455 SW 4TH AVE , DELRAY BEACH

Note: Refer to individual truss design drawings for special loading conditions, design criteria, truss geometry, lumber, and plate information.

**Design Professional Information:** 

Truss Design Information:

Design Professional Of Record: BUILDER

Delegated Truss Engineer: Julius Lee

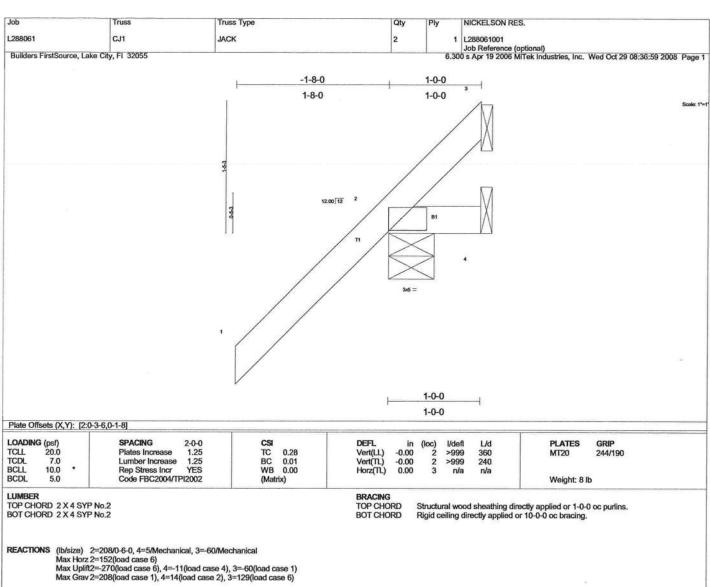
License #:

License #: 34869

This truss specification package consists of this index sheet and 99 truss design drawings. This signed and sealed index sheet indicates acceptance of my professional engineering responsibility solely for listed truss design drawings. The suitability and use of each truss component for any particular building is the responsibility of the building designer per TPI.

Truss	Truss	Drawing	Seal	Truss	Truss	Drawing	Seal	Truss	Truss	Drawing	Seal
#	Label	#	Date	#	Label	#	Date	#	Label	#	Date
1	CJ1	L288061001	10/29/2008	31	PB1H	L288061031	10/29/2008	61	T05	L288061061	10/29/2008
2	CJ1A	L288061002	10/29/2008	32	PB2	L288061032	10/29/2008	62	T06	L288061062	10/29/2008
3	CJ3	L288061003	10/29/2008	33	PB2G	L288061033	10/29/2008	63	T07	L288061063	10/29/2008
4	EJ11	L288061004	10/29/2008	34	PB3	L288061034	10/29/2008	64	T08	L288061064	10/29/2008
5	EJ11A	L288061005	10/29/2008	35	PB3G	L288061035	10/29/2008	65	T09	L288061065	10/29/2008
6	EJ11B	L288061006	10/29/2008	36	PB4	L288061036	10/29/2008	66	T10	L288061066	10/29/2008
7	EJ11C	L288061007	10/29/2008	37	PB4A	L288061037	10/29/2008	67	T11	L288061067	10/29/2008
8	EJ11D	L288061008	10/29/2008	38	PB5	L288061038	10/29/2008	68	T12	L288061068	10/29/2008
9	EJ11E	L288061009	10/29/2008	39	PB5A	L288061039	10/29/2008	69	T13	L288061069	10/29/2008
10	EJ2	L288061010	10/29/2008	40	PB5B	L288061040	10/29/2008	70	T14	L288061070	10/29/2008
11	EJ2A	L288061011	10/29/2008	41	PB5C	L288061041	10/29/2008	71	T15	L288061071	10/29/2008
12	EJ2B	L288061012	10/29/2008	42	PB5D	L288061042	10/29/2008	72	T16	L288061072	10/29/2008
13	EJ3	L288061013	10/29/2008	43	PB6	L288061043	10/29/2008	73	T16A	L288061073	10/29/2008
14	EJ4	L288061014	10/29/2008	44	PB6A	L288061044	10/29/2008	74	T16B	L288061074	10/29/2008
15	EJ4G	L288061015	10/29/2008	45	PB7	L288061045	10/29/2008	75	T16C	L288061075	10/29/2008
16	EJ5	L288061016	10/29/2008	46	PB7A	L288061046	10/29/2008	76	T16D	L288061076	10/29/2008
17	EJ5A	L288061017	10/29/2008	47	PB7B	L288061047	10/29/2008	77	T17	L288061077	10/29/2008
18	EJ5B	L288061018	10/29/2008	48	PB8	L288061048	10/29/2008	78	T17A	L288061078	10/29/2008
19	EJ5C	L288061019	10/29/2008	49	PB8A	L288061049	10/29/2008	79	T17B	L288061079	10/29/2008
20	HJ2A	L288061020	10/29/2008	50	PB9	L288061050	10/29/2008	80	T17C	L288061080	10/29/2008
21	HJ7	L288061021	10/29/2008	51	T01G	L288061051	10/29/2008	81	T17D	L288061081	10/29/2008
22	PB1	L288061022	10/29/2008	52	T02	L288061052	10/29/2008	82	T18	L288061082	10/29/2008
23	PB10	L288061023	10/29/2008	53	T02A	L288061053	10/29/2008	83	T19	L288061083	10/29/2008
24	PB10G	L288061024	10/29/2008	54	T02B	L288061054	10/29/2008	84	T20	L288061084	10/29/2008
25	PB1A	L288061025	10/29/2008	55	T02G	L288061055	10/29/2008	85	T20A	L288061085	10/29/2008
26	PB1B	L288061026	10/29/2008	56	T03	L288061056	10/29/2008	86	T20B	L288061086	10/29/2008
27	PB1C	L288061027	10/29/2008	57	T03G	L288061057	10/29/2008	87	T20C	L288061087	10/29/2008
28	PB1D	L288061028	10/29/2008	58	T04	L288061058	10/29/2008	88	T20D	L288061088	10/29/2008
29	PB1E	L288061029	10/29/2008	59	T04A	L288061059	10/29/2008	89	T21	L288061089	10/29/2008
30	PB1F	L288061030	10/29/2008	60	T04B	L288061060	10/29/2008	90	T22	L288061090	10/29/2008

Project Information:				Builder: Cash Account  Model: custom						Builders FirstSource 2525 E. Duval St.				
			Builde	ers FirstSource Job #: L288061					Lake City, FL 32055					
This si respon compo TPI.	igned and se sibility sole onent for any	ation package con aled index sheet in ly for listed truss of particular buildin	ndicates accept lesign drawing: g is the respon	ance of s. The s	my profess uitability a	ional engineering nd use of each tru								
91	T22G	L288061091	10/29/2008			-		-						
92	T23	L288061092	10/29/2008					-						
93	T23A T23G	L288061093 L288061094	10/29/2008				_							
95	T24	L288061094 L288061095	10/29/2008			-	-	-						
96	T24A	L288061095 L288061096	10/29/2008	-	-		-	-	_					
97	T24A	L288061096	10/29/2008	_		+		-	_	-				
98	T24G	L288061097	10/29/2008					$\vdash$	-					
99	T25	L288061099	10/29/2008											



FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/63, 2-3=-83/113 BOT CHORD 2-4=0/0

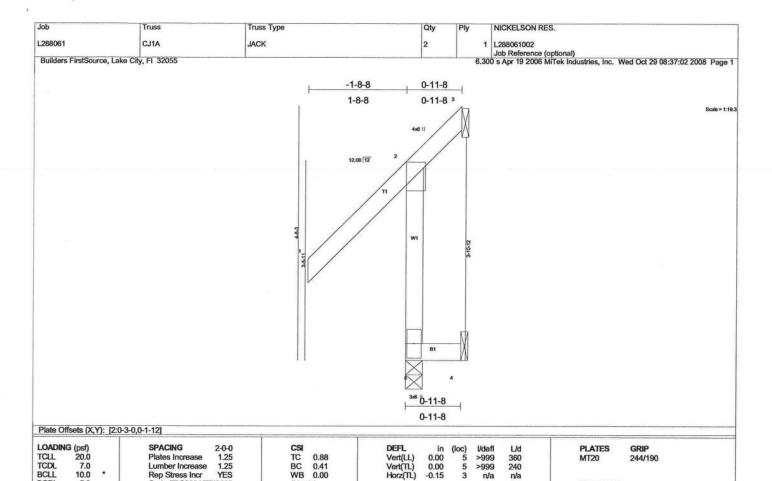
- NOTES (5)

  1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

  2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 270 lb uplift at joint 2, 11 lb uplift at joint 4 and 60 lb uplift at joint 3.

5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435



BCDL

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

10.0

BRACING TOP CHORD

Horz(TL)

-0.15

BOT CHORD

Structural wood sheathing directly applied or 1-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

Weight: 12 lb

n/a

n/a

REACTIONS (lb/size) 5=242/0-3-8, 4=-8/Mechanical, 3=-83/Mechanical Max Horz 5=157(load case 6)

Max Uplift4=-314(load case 6), 3=-108(load case 6)

Max Grav 5=272(load case 6), 4=9(load case 2)

Rep Stress Incr

Code FBC2004/TPI2002

FORCES (b) - Maximum Compression/Maximum Tension TOP CHORD 2-5=-226/38, 1-2=0/71, 2-3=-89/0 BOT CHORD 4-5=0/0

NOTES (5)

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

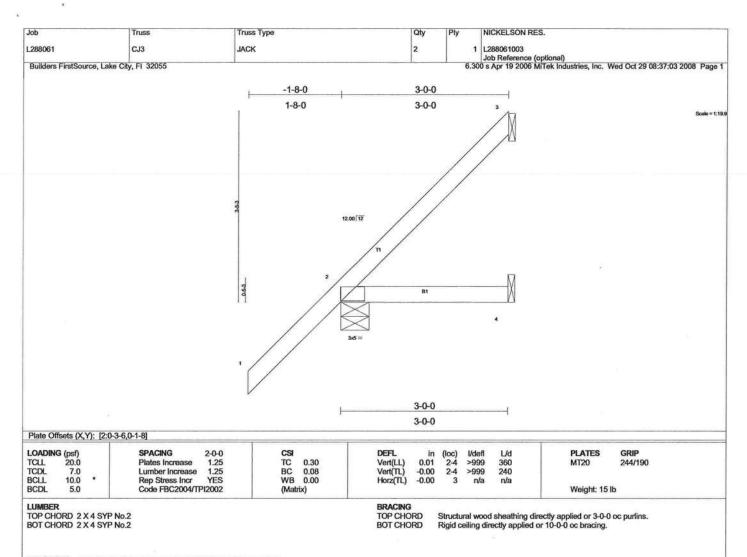
members and forces, and for MWFRS for reactions specified.

2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) Refer to girder(s) for truss to truss connections.

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 314 lb uplift at joint 4 and 108 lb uplift at joint 3.

5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435



REACTIONS (lb/size) 3=36/Mechanical, 2=226/0-6-0, 4=13/Mechanical

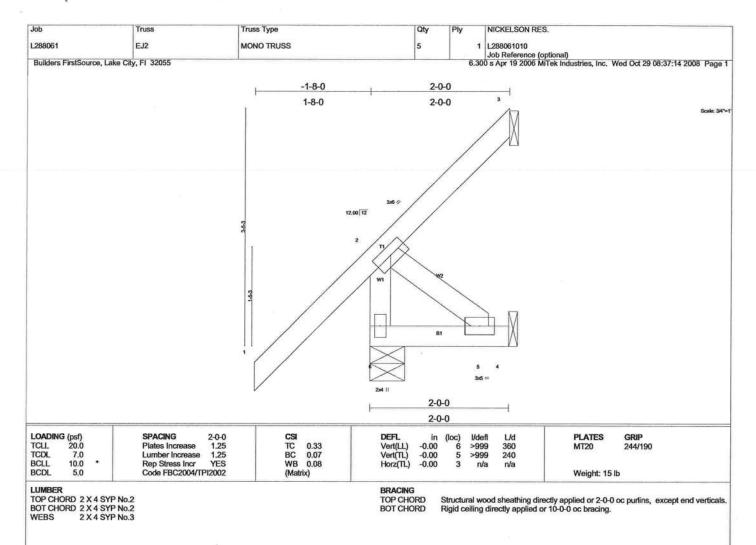
Max Horz 2=242(load case 6) Max Uplift3=63(load case 7), 2=174(load case 6), 4=32(load case 4) Max Grav 3=38(load case 4), 2=226(load case 1), 4=40(load case 2)

FORCES (Ib) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/66, 2-3=-82/27 BOT CHORD 2-4=0/0

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf, BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

  2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 3) Refer to girder(s) for truss to truss connections.
  4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 63 lb uplift at joint 3, 174 lb uplift at joint 2 and 32 lb uplift at
- joint 4.
  5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435



REACTIONS (lb/size) 6=205/0-6-0, 4=9/Mechanical, 3=2/Mechanical

Max Horz 6=230(load case 6) Max Uplift6=-52(load case 6), 4=-138(load case 6), 3=-13(load case 7) Max Grav 6=205(load case 1), 4=28(load case 2), 3=39(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/69, 2-3=-69/27, 2-6=-196/34 BOT CHORD 5-6=-256/0, 4-5=0/0 WEBS 2-5=0/326

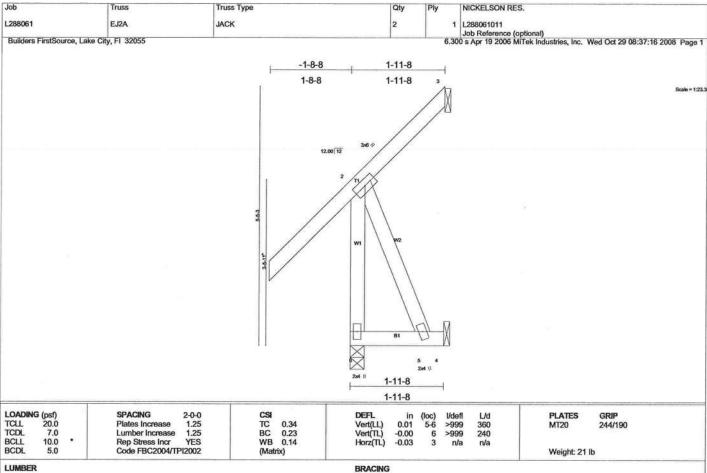
- NOTES (5)

  1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

- 2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

  3) Refer to girder(s) for truss to truss connections.

  4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 52 lb uplift at joint 6, 138 lb uplift at joint 4 and 13 lb uplift at
- joint 3.
  5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435



TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.2 \*Except\*
W2 2 X 4 SYP No.3

TOP CHORD **BOT CHORD**  Structural wood sheathing directly applied or 2-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 6=209/0-3-8, 4=9/Mechanical, 3=-3/Mechanical Max Horz 6=203(load case 6) Max Uplift4=-363(load case 6), 3=-11(load case 9) Max Grav 6=209(load case 1), 4=27(load case 2), 3=41(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 2-6=278/0, 1-2=0/71, 2-3=-71/33 BOT CHORD 5-6=-228/0, 4-5=0/0

WEBS 2-5=0/573

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

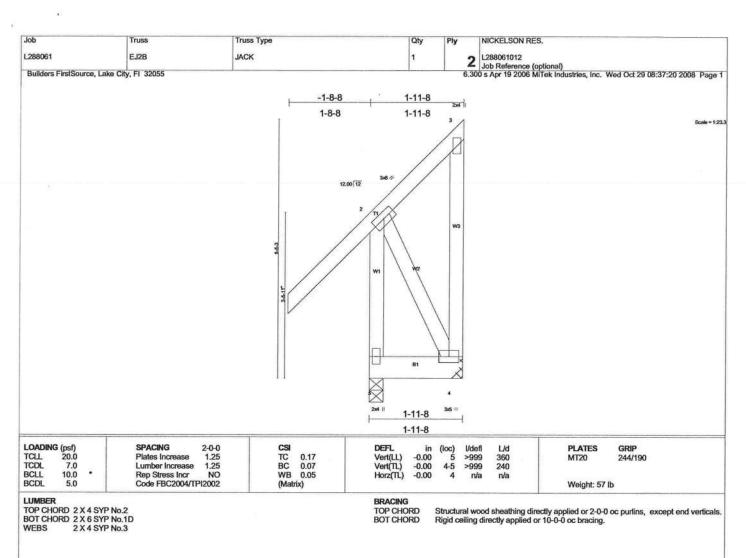
MWPRS for reactions specified.

2) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) Refer to girder(s) for truss to truss connections.

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 363 lb uplift at joint 4 and 11 lb uplift at joint 3.

5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435



REACTIONS (lb/size) 5=677/0-3-8, 4=465/Mechanical Max Horz 5=197(load case 5)

Max Uplift5=-223(load case 3), 4=-556(load case 5)

FORCES (Ib) - Maximum Compression/Maximum Tension TOP CHORD 2:5=-201/0, 1-2=0/71, 2-3=-71/38 BOT CHORD 4-5=-197/0 WEBS 3-4=-55/44, 2-4=0/424

3-4=-55/44, 2-4=0/424

NOTES (8)

1) 2-ply truss to be connected together with 10d (0.131\*x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 6 - 2 rows at 0-7-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

3) Unbalanced roof live loads have been considered for this design.

4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60.

exposed; Lumber DOL=1.60 plate grip DOL=1.60.

5) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

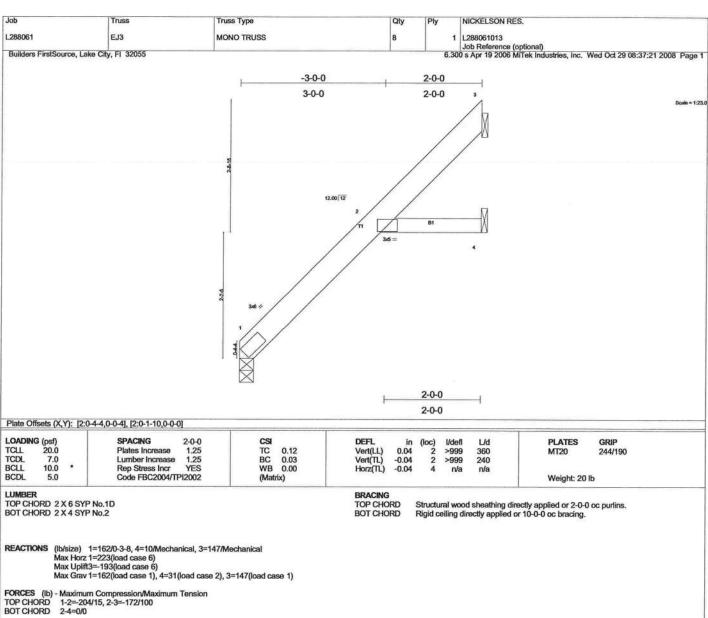
6) Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 223 lb uplift at joint 5 and 556 lb uplift at joint 4.

8) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

### LOAD CASE(S) Standard

1) Regular. Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf) Vert: 1-2=54, 2-3=54, 4-5=571(F=-561)



NOTES (6)

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf, BCDL=3.0psf, Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

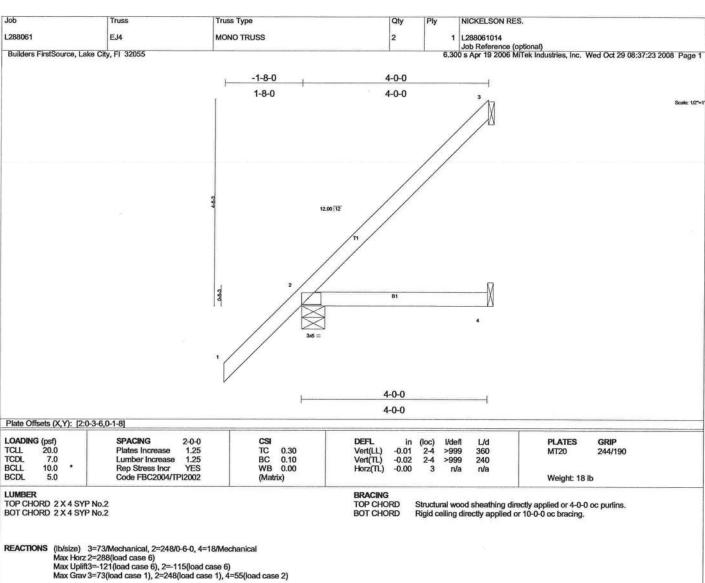
2) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) Refer to girder(s) for truss to truss connections.

4) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 193 lb uplift at joint 3.

6) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address; 1109 Coastal Bay Blvd. Boynton Beach, FL 33435



FORCES (Ib) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/66, 2-3=-120/39 BOT CHORD 2-4=0/0

NOTES (5)

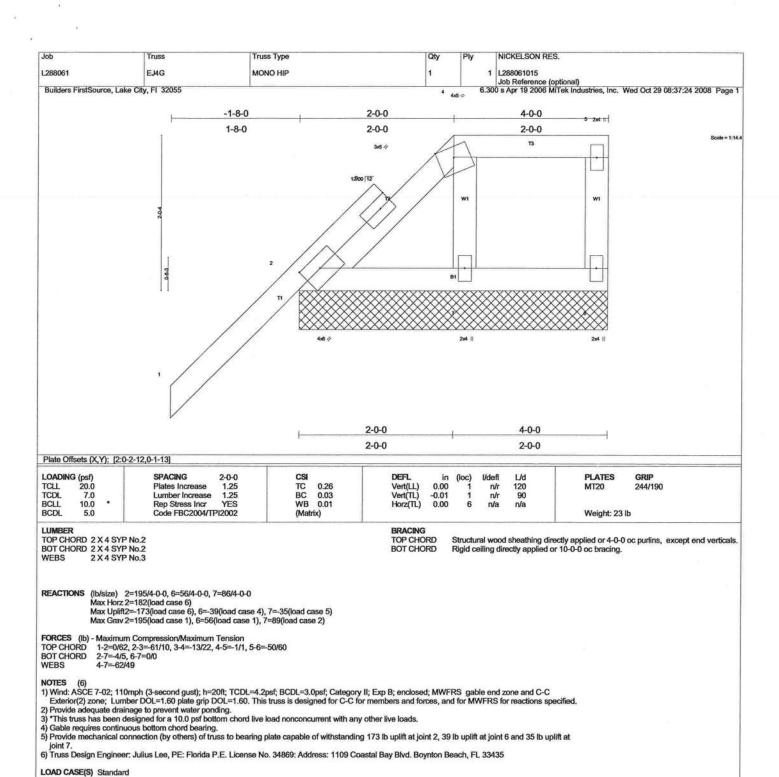
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

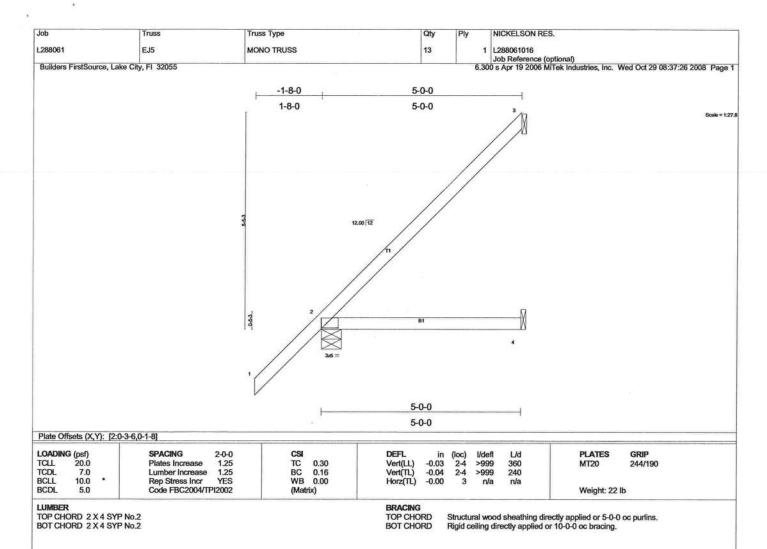
2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) Refer to girder(s) for truss to truss connections.

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 121 lb uplift at joint 3 and 115 lb uplift at joint 2.

5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435





REACTIONS (lb/size) 3=105/Mechanical, 2=275/0-6-0, 4=23/Mechanical

Max Horz 2=334(load case 6)
Max Uplift3=173(load case 6), 2=97(load case 6)
Max Grav 3=105(load case 1), 2=275(load case 1), 4=70(load case 2)

FORCES (b) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/66, 2-3=-168/59 BOT CHORD 2-4=0/0

NOTES (5)

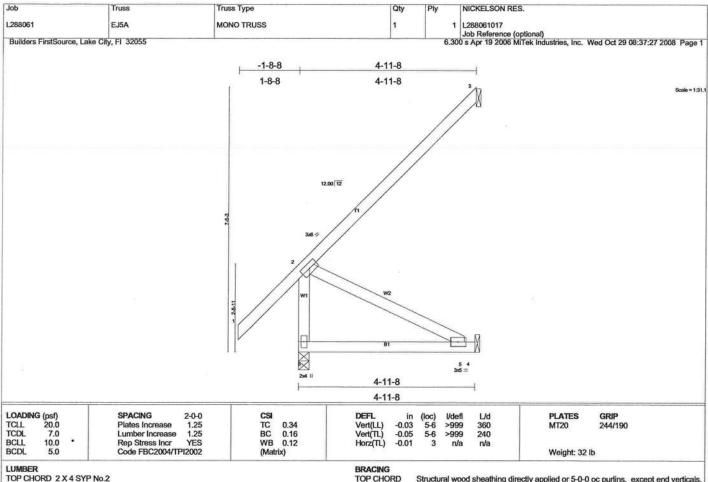
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) Refer to girder(s) for truss to truss connections.

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 173 lb uplift at joint 3 and 97 lb uplift at joint 2.

5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435



TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3

BOT CHORD

Structural wood sheathing directly applied or 5-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 9-6-2 oc bracing.

REACTIONS (lb/size) 3=109/Mechanical, 6=272/0-3-8, 4=24/Mechanical Max Horz 6=391 (load case 6) Max Uplift3=-155 (load case 6), 4=-160 (load case 6) Max Grav 3=109 (load case 1), 6=272 (load case 1), 4=71 (load case 2)

FORCES (Ib) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/71, 2-3=-149/59, 2-6=-248/13 BOT CHORD 5-6=-432/11, 4-5=0/0

2-5-12/484

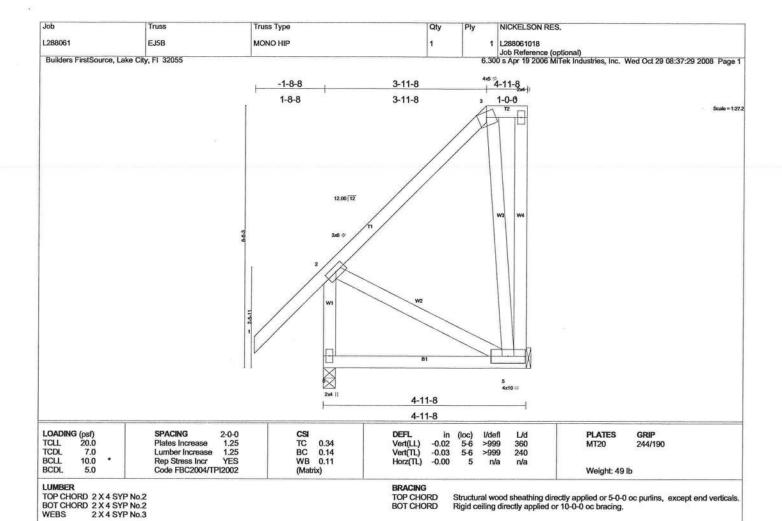
WEBS

- NOTES (5)

  1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

  2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 3) Refer to girder(s) for truss to truss connections.
  4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 155 lb uplift at joint 3 and 160 lb uplift at joint 4.
  5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435



REACTIONS (lb/size) 5=135/Mechanical, 6=261/0-3-8

Max Horz 6=351 (load case 6) Max Uplift5=268 (load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/71, 2-3=-101/30, 3-4=-2/2, 4-5=-23/28, 2-6=-239/11 BOT CHORD 5-6=-388/5

3-5=-94/136, 2-5=-3/424

- NOTES (6)

  1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for Extendit(2) 2016; entit vehicla feit exposed, Lumber 2012–1.00 piate grip 2012–1.00. This truss is designed for 2016 members and it. MWFRS for reactions specified.

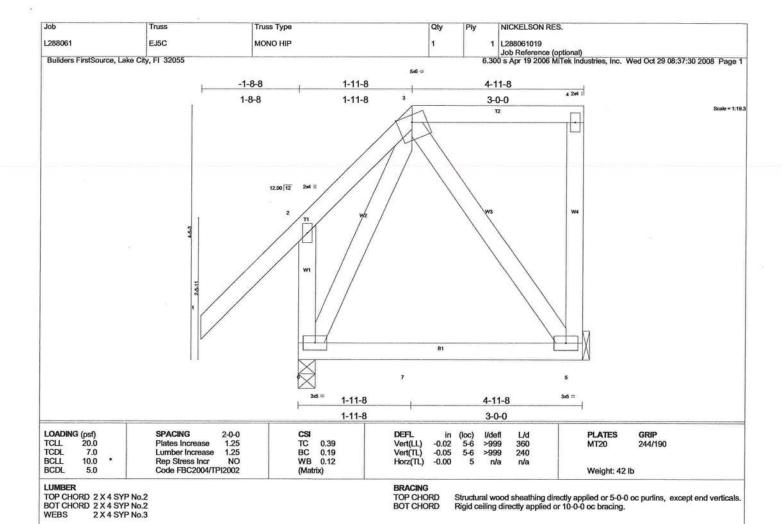
  2) Provide adequate drainage to prevent water ponding.

  3) "This truss has been designed for a 10.0 pef bottom chord live load nonconcurrent with any other live loads.

  4) Refer to girder(s) for truss to truss connections.

  5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 268 lb uplift at joint 5.

  6) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435



REACTIONS (lb/size) 5=189/Mechanical, 6=296/0-3-8 Max Horz 6=260(load case 5)

Max Uplift5=220(load case 5), 6=118(load case 5)

FORCES (Ib) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/71, 2-3=-71/355, 3-4=-2/1, 4-5=-113/132, 2-6=-200/445
BOT CHORD 67=97/30, 5-7=-97/30
WEBS 3-5=-52/169, 3-6=-339/57

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; end vertical left 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20tt; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; enexposed; Lumber DOL=1.60 plate grip DOL=1.60.
  2) Provide adequate drainage to prevent water ponding.
  3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  4) Refer to girder(s) for truss to truss connections.
  5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 220 lb uplift at joint 5 and 118 lb uplift at joint 6.
  6) Girder carries hip end with 0-0-0 right side setback, 1-11-8 left side setback, and 4-0-0 end setback.
  7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
  8) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

## LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (pif) Vert: 1-2=54, 2-3=54, 3-4=-79(F=-25), 6-7=-10, 5-7=-15(F=-5)

Job Truss Type NICKELSON RES. Truss Plv Qtv L288061 EJ11 MONO TRUSS 1 L288061004 Job Reference (optional) 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Oct 29 08:37:05 2008 Page 1 Builders FirstSource, Lake City, FI 32055 -1-8-8 5-5-8 10-11-8 1-8-8 5-5-8 5-6-0 Scale = 1:60.5 4x10 = 5-5-8 10-11-8 5-5-8 5-6-0 LOADING (psf) TCLL 20.0 SPACING DEFL in (loc) 26 7-8 44 7-8 I/defl **PLATES** L/d TCLL TC BC 244/190 Plates Increase 1.25 0.98 Vert(LL) Vert(TL) -0.26 -0.44 >495 >287 360 240 MT20 7.0 Lumber Increase 0.40 Rep Stress Incr YES Code FBC2004/TPI2002 WB 0. (Matrix) BCI I 10.0 0.22 Horz(TL) -0.00 n/a BCDL 5.0 Weight: 89 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3 Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

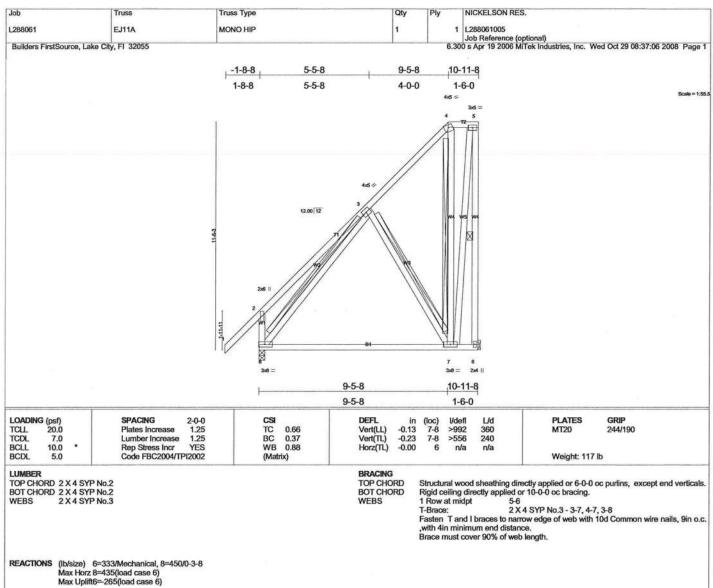
1 Row at midpt
4-7
1-Brace: 2 X 4 SYP No.3 - 3-7, 3-8
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c. with 4in minimum and distance. TOP CHORD BOT CHORD ,with 4in minimum end distance. Brace must cover 90% of web length. REACTIONS (lb/size) 8=446/0-3-8, 7=335/Mechanical Max Horz 8=480(load case 6) Max Uplift7=335(load case 6) FORCES (ib) - Maximum Compression/Maximum Tension
TOP CHORD 28=305/397, 1-2=0/71, 2-3=-287/287, 3-4=-159/77, 4-5=-3/0, 4-7=-122/183
BOT CHORD 7-8=-262/131, 6-7=0/0
WEBS 3-7=-202/426, 3-8=-509/205 10) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions vertical left exposed; Lumber DOL=1.00 prate grip DOL=1.00. This does a designed to the specified.

2) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) Refer to girder(s) for truss to truss connections.

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 335 lb uplift at joint 7.

5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435 LOAD CASE(S) Standard



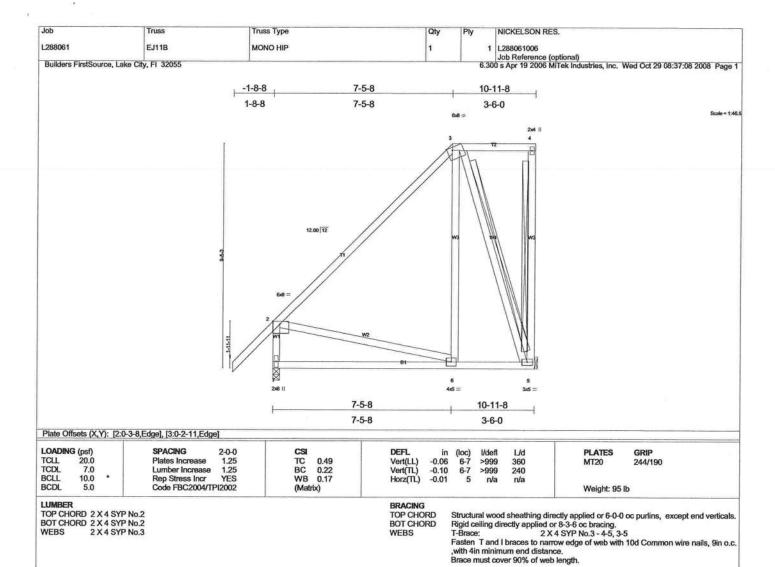
FORCES (b) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/71, 2-3=-203/352, 3-4=-148/8, 4-5=-40/46, 5-6=-370/397, 2-8=-309/451
BOT CHORD 7-8=-246/132, 6-7=-4/2
WEBS 3-7=176/382, 4-7=-106/162, 5-7=-397/360, 3-8=-510/86

NOTES (6)

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) Provide adequate drainage to prevent water ponding.
3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) Refer to girder(s) for truss to truss connections.

4) Neter to guide (s) for the six of the second of trues to bearing plate capable of withstanding 265 lb uplift at joint 6.
6) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435



REACTIONS (lb/size) 5=333/Mechanical, 7=450/0-3-8 Max Horz 7=371(load case 6)

Max Uplift5=-186(load case 6), 7=-41(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/71, 2-3=-296/10, 3-4=-3/2, 4-5=-75/63, 2-7=-409/129
BOT CHORD 6-7=-586/114, 5-6=-145/108
WEBS 3-6=-64/199, 3-5=-315/427, 2-6=-50/454

NOTES (7)

1) Unbalanced roof live loads have been considered for this design.

1) United ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4,2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions ventual iet exposed, curried bot.—I.ob plate grip bot.—I.ob. This truss is designed for C-C for members a specified.

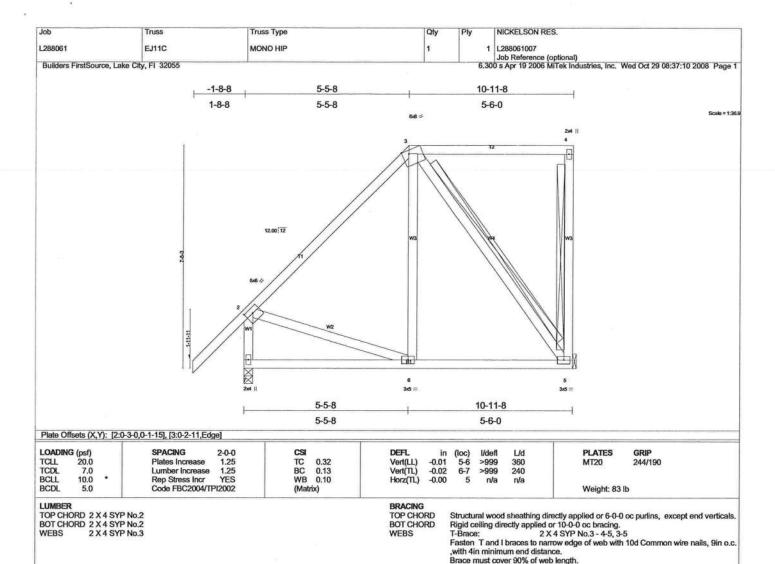
3) Provide adequate drainage to prevent water ponding.

4) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 186 lb uplift at joint 5 and 41 lb uplift at joint 7.

7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435



REACTIONS (lb/size) 5=333/Mechanical, 7=450/0-3-8 Max Horz 7=307(load case 6)

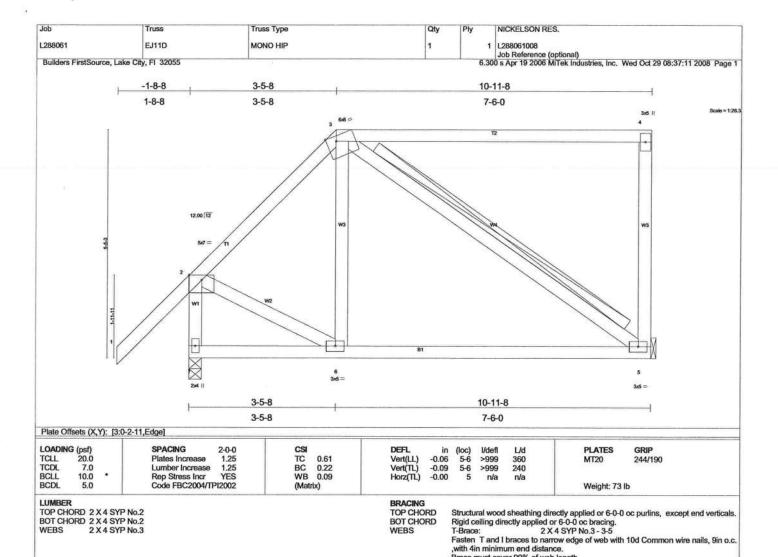
Max Uplift5=136(load case 5), 7=81(load case 6)

FORCES (ib) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/71, 2-3=-303/97, 3-4=-13/6, 4-5=-128/105, 2-7=-422/204
BOT CHORD 6-7=-397/41, 5-6=-181/145
WEBS 3-6=-33/154, 3-5=-222/290, 2-6=-50/228

- NOTES (6)

  1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
   \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
   Refer to girder(s) for truss to truss connections.

- 4) reterior in grades for the score of the s



Brace must cover 90% of web length.

REACTIONS (Ib/size) 5=333/Mechanical, 7=450/0-3-8 Max Horz 7=202(load case 6) Max Uplift5=-115(load case 5), 7=-107(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/71, 2-3=-288/156, 3-4=-45/30, 4-5=-194/173, 2-7=-443/268
BOT CHORD 67=-190/18, 5-6=-198/185
WEBS 3-6=-64/123, 3-5=-171/204, 2-6=-67/267

NOTES (6)

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

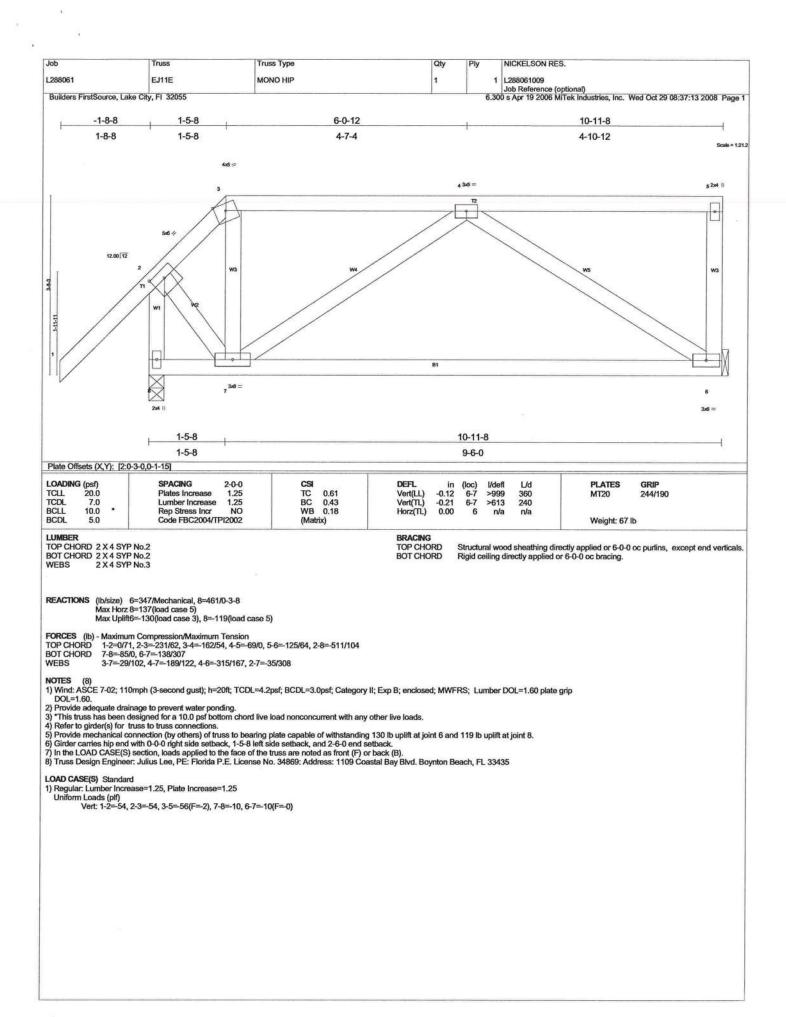
2) Provide adequate drainage to prevent water ponding.

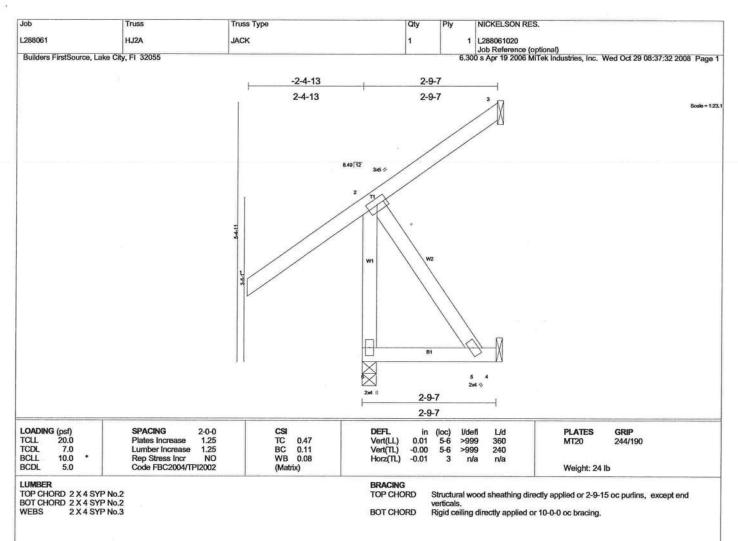
3) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 115 lb uplift at joint 5 and 107 lb uplift at joint 7.

6) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435





REACTIONS (lb/size) 6=237/0-3-8, 3=-17/Mechanical, 4=9/Mechanical Max Horz 6=218(load case 5) Max Uplift6=-78(load case 3), 3=-17(load case 1), 4=-218(load case 5) Max Grav 6=237(load case 1), 3=76(load case 3), 4=89(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 2-6=-233/92, 1-2=0/79, 2-3=-55/43 BOT CHORD 5-6=-180/68, 4-5=0/0 WEBS 2-5=-117/312

- NOTES (6)

  1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60.

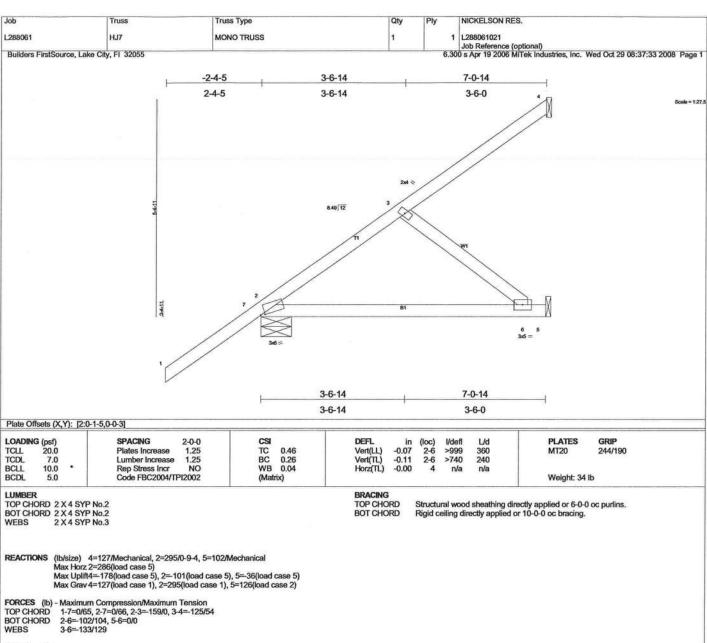
  2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Refer to girder(s) for truss to truss connections.
  4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 78 lb uplift at joint 6, 17 lb uplift at joint 3 and 218 lb uplift at
- joint 4.
- joint 4.
  5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
  6) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf) Vert: 1-2=-54

Trapezoidal Loads (plf)

Vert: 2=-3(F=25, B=25)-to-3=-57(F=-2, B=-2), 6=0(F=5, B=5)-to-4=-11(F=-0, B=-0)



- NOIES (6)

  1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.

  2) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

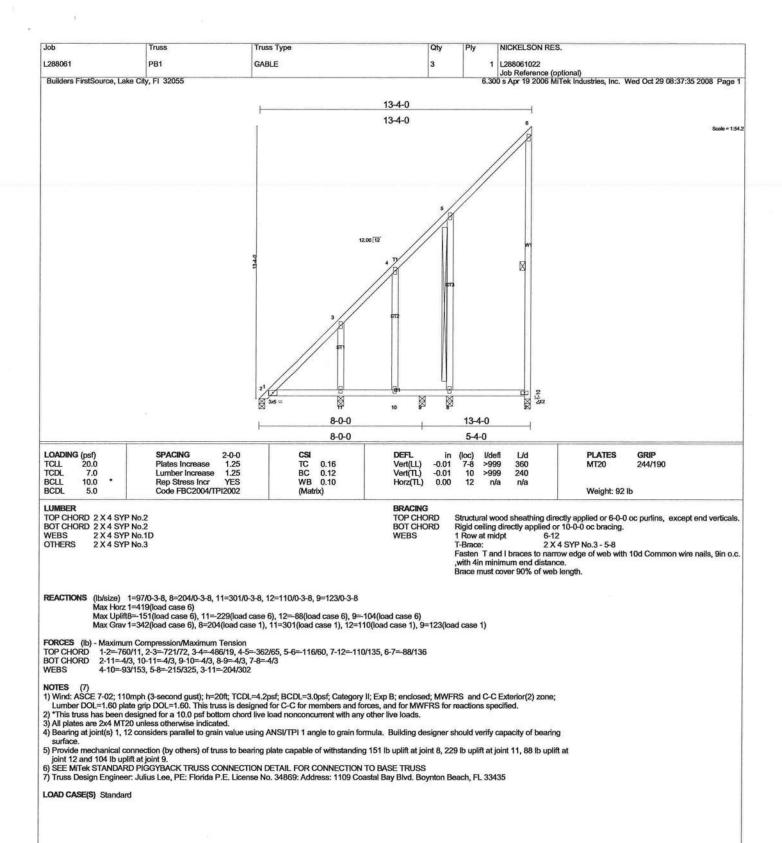
  3) Refer to grider(s) for truss to truss connections.

  4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 178 lb uplift at joint 4, 101 lb uplift at joint 2 and 36 lb uplift at loads.
- joint 5.
  5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
  6) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf) Vert: 1-7=-54

Trapezoidal Loads (plf)
Vert: 7=0(F=27, B=27)-to-4=95(F=21, B=-21), 2=-1(F=5, B=5)-to-5=-18(F=4, B=-4)



Job NICKELSON RES. Truss Type Qty 1.288061 PB1A GABLE 1 L288061025 Job Reference (optional) 6.300 s Apr 19 2006 MTek Industries, Inc. Wed Oct 29 08:37:40 2008 Page 1 Builders FirstSource, Lake City, FI 32055 12-0-0 13-4-0 12-0-0 1-4-0 415 > 12.00 12 M X X 12 8-0-0 13-4-0 8-0-0 5-4-0 LOADING (psf) TCLL 20.0 SPACING CSI DEFL in (loc) 0.02 12-13 l/defi >999 PLATES GRIP 244/190 Plates Increase 1.25 TC 0.13 Vert(LL) 360 MT20 Lumber Increase 12 TCDL 1.25 BC Vert(TL) -0.01 >999 240 BCLL 10.0 Rep Stress Incr YES WB 0.09 Horz(TL) -0.01 n/a BCDL Code FBC2004/TPI2002 (Matrix) Weight: 107 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.2 Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

1 Row at midp 7-14

T-Brace: 2 X 4 SYP No.3 - 6-9, 5-10 TOP CHORD BOT CHORD WEBS OTHERS 2 X 4 SYP No.3 Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c. .with 4in minimum end distance. Brace must cover 90% of web length. (lb/size) 1=97/0-3-8, 10=210/0-3-8, 13=304/0-3-8, 14=111/0-3-8, 11=115/0-3-8 Max Horz 1=382(load case 6) Max Uplift10=-148(load case 6), 13=233(load case 6), 14=45(load case 6), 11=97(load case 6) Max Grav 1=307(load case 6), 10=210(load case 1), 13=304(load case 1), 14=111(load case 1), 11=115(load case 1) REACTIONS FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2-688/8, 2-3-648/71, 3-4-414/19, 4-5-280/60, 5-6-74/18, 6-7-3/2, 8-14-111/100, 7-8-53/41 BOT CHORD 2-13-4/4, 12-13-4/4, 11-12-4/4, 10-11-4/4, 9-10-4/4, 8-9-3/3 WEBS 6-9-55/93, 4-12-100/167, 5-10-189/274, 3-13-203/300 NOTES (8)

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DCL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

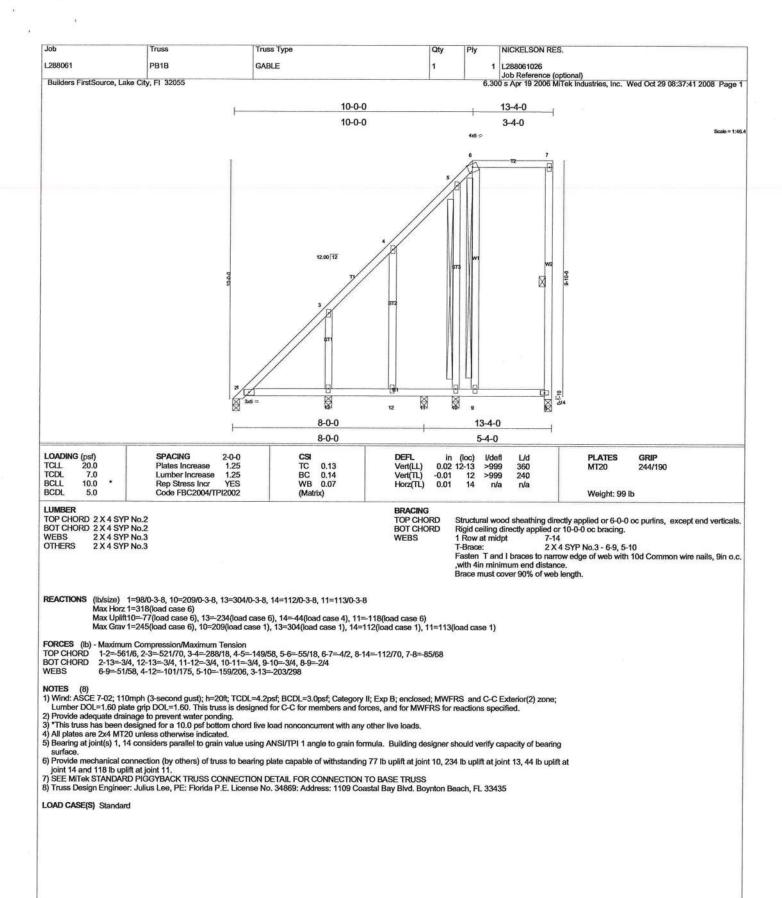
2) Provide adequate drainage to prevent water ponding.

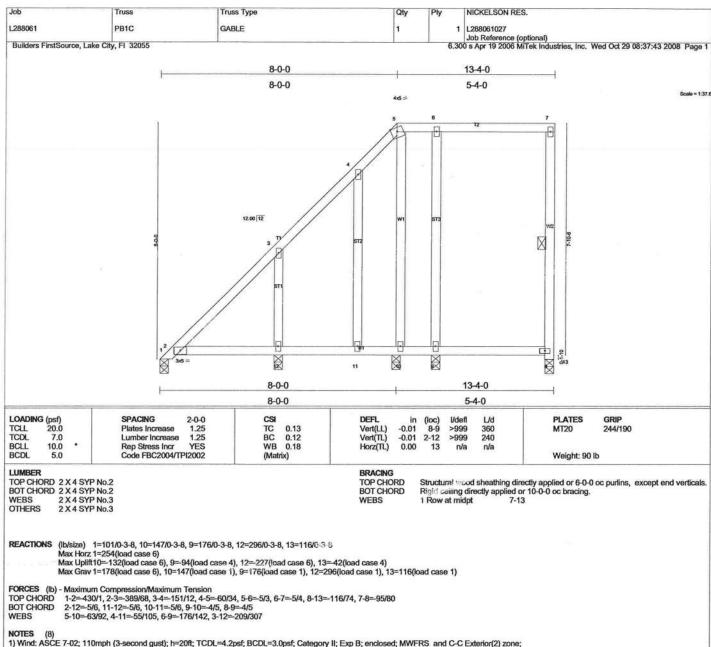
3) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) All plates are 2x4 MT20 unless otherwise indicated.

5) Bearing at joint(s) 1, 14 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface. surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 148 lb uplift at joint 10, 233 lb uplift at joint 13, 45 lb uplift at joint 14 and 97 lb uplift at joint 11.

7) SEE MITEK STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS 8) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

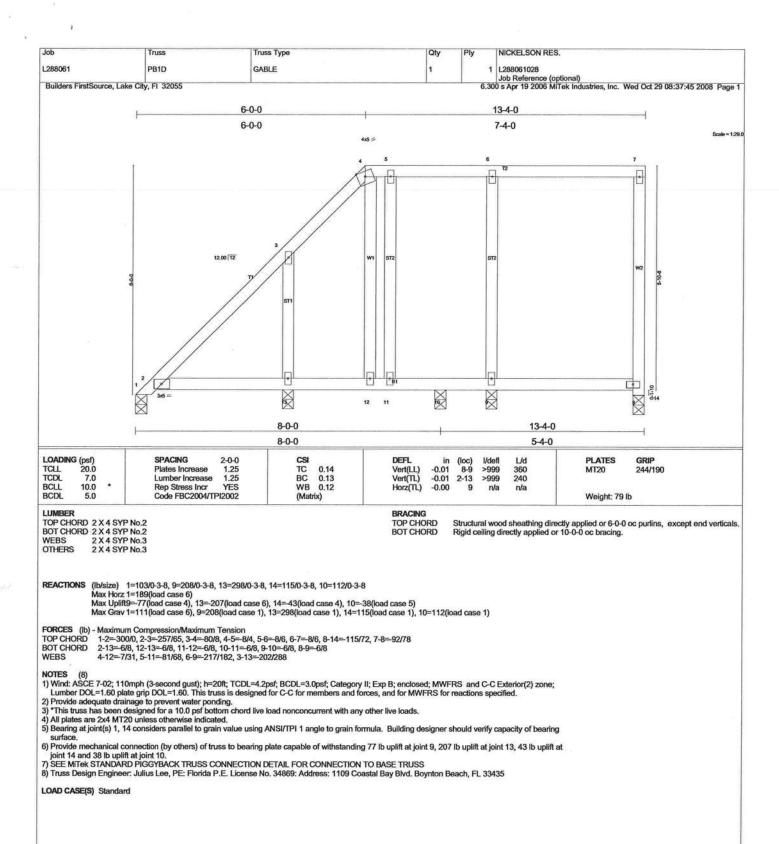


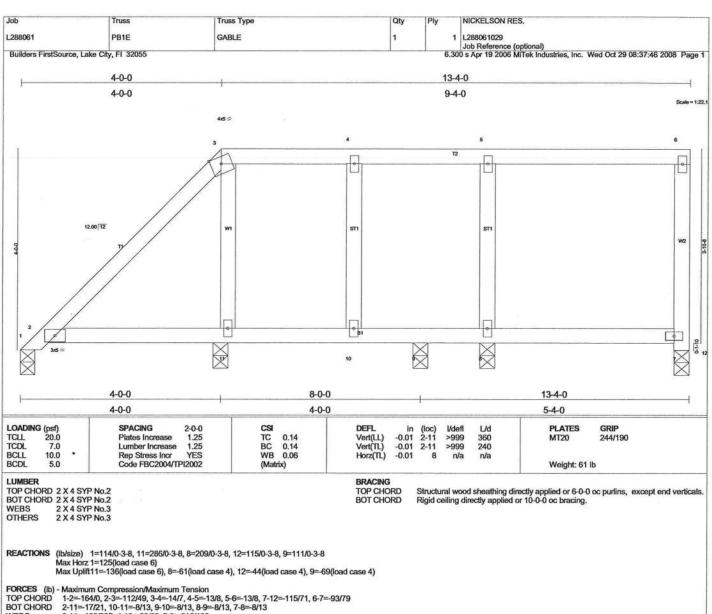


1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) Provide adequate drainage to prevent water ponding.
3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All plates are 2x4 MT20 unless otherwise indicated.

- 5) Bearing at joint(s) 1, 13 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing
- 50 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 132 lb uplift at joint 10, 94 lb uplift at joint 9, 227 lb uplift at joint 12 and 42 lb uplift at joint 13.
   7) SEE MITEK STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS
   8) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435





WEBS 3-11=-189/235, 4-10=-88/68, 5-8=-216/180

- NOTES (8)

  1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

  2) Provide adequate drainage to prevent water ponding.

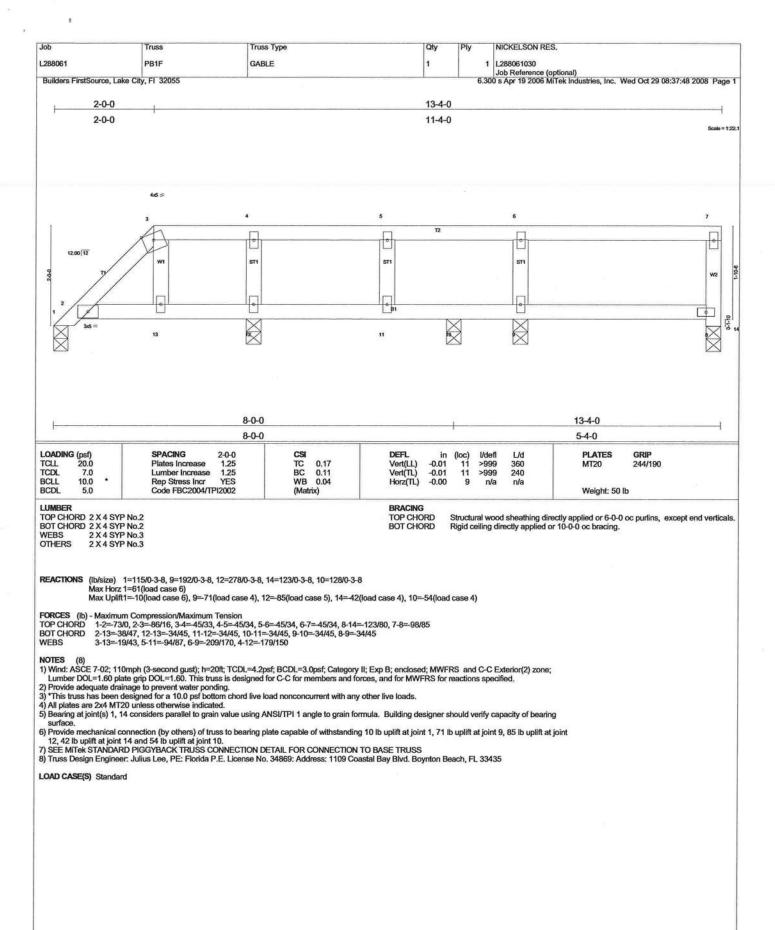
  3) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

  4) All plates are 2x4 MT20 unless otherwise indicated.

- 5) Be aring at joint(s) 1, 12 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 136 lb uplift at joint 11, 61 lb uplift at joint 8, 44 lb uplift at joint 12 and 69 lb uplift at joint 9.

  7) SEE MITek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

  8) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435



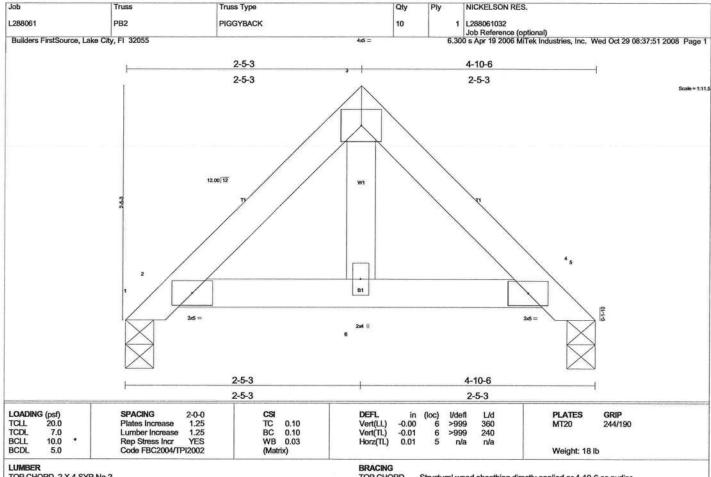
Job NICKELSON RES. Truss Type Ply 1.288061 PB1H VALLEY 1 L288061031 Job Reference (optional) 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Oct 29 08:37:49 2008 Page 1 Builders FirstSource, Lake City, FI 32055 10-8-0 10-8-0 26 12.00 12 4-0-0 8-0-0 10-8-0 4-0-0 4-0-0 2-8-0 Plate Offsets (X,Y): [11:0-10-8,0-0-0] LOADING (psf) SPACING DEFL in (loc) 0.10 10-11 **PLATES** GRIP TC BC WB 20.0 TCLL 0.79 >486 244/190 Plates Increase 1.25 Vert(LL) MT20 360 TCDL 7.0 Lumber Increase Rep Stress Incr 0.86 0.15 1.25 Vert(TL) -0.02 >999 240 MT20H 187/143 YES -0.04Horz(TL) n/a BCDL 5.0 Code FBC2004/TPI2002 Weight: 93 lb BRACING TOP CHORD LUMBER LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.1D \*Except\*
W2 2 X 4 SYP No.3 Structural wood sheathing directly applied or 5-1-15 oc purlins, except end verticals. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 4-5-10 oc bracing: 10-11. OTHERS WEBS 2 X 4 SYP No.3 1 Row at midpt 2 X 4 SYP No.3 - 4-8 T-Brace: Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c. ,with 4in minimum end distance. Brace must cover 90% of web length. REACTIONS (lb/size) 10=309/0-3-8, 12=96/0-3-8, 13=85/0-3-8, 7=175/0-3-8 (ID/SIZE) 10-303/10-30, 12-303/10-30, 12-303/10-30, 12-303/10-300/10-300/10-300/10-300/10-300/10-300/10-300/10-300 FORCES (lb) - Maximum Compression/Maximum Tension 11-12-763/0, 1-11-492/0, 1-2-681/79, 2-3-268/2, 3-4-283/55, 4-5-127/66, 6-13-85/186, 5-6-96/160 10-11-13/3, 9-10-13/3, 8-9-13/3, 7-8-13/3, 6-7-13/3 TOP CHORD BOT CHORD WEBS 3-9-34/48, 4-8-193/210, 2-10-225/552 NOTES (7)

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified. 2) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) All plates are MT20 plates unless otherwise indicated.

4) All plates are 2x4 MT20 unless otherwise indicated.

5) Bearing at joint(s) 12, 13 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 595 lb uplift at joint 10 and 123 lb uplift at joint 13.
7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435 LOAD CASE(S) Standard



TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3

TOP CHORD Structural wood sheathing directly applied or 4-10-6 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing. BOT CHORD

REACTIONS (lb/size) 1=148/0-3-8, 5=148/0-3-8 Max Horz 1=64(load case 5)

Max Uplift1=26(load case 6), 5=-26(load case 7)

FORCES (ib) - Maximum Compression/Maximum Tension TOP CHORD 1-2-94/58, 2-3-171/98, 3-4-171/98, 4-5-94/58 BOT CHORD 2-6-22/111, 4-6-22/111

WEBS 3-6=-35/95

NOTES (7)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4,2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

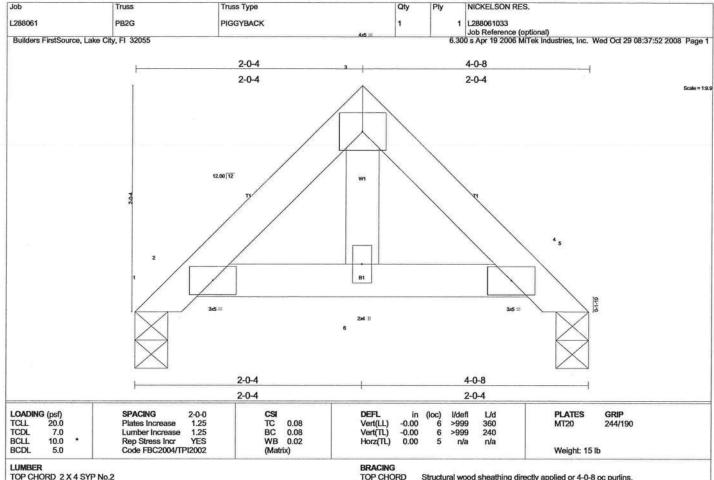
4) Bearing at joint(s) 1, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing

4) Dearing at joint(s) 1, 3 considers parallel to grant value using revolution 1 angle to grant learning. Solutions.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 26 lb uplift at joint 1 and 26 lb uplift at joint 5.

6) SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435



TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 4-0-8 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=121/0-3-8, 5=121/0-3-8 Max Horz 1=-65(load case 4) Max Uplift1=-46(load case 6), 5=-46(load case 7)

FORCES (ib) - Maximum Compression/Maximum Tension TOP CHORD 1-2-77/54, 2-3-141/80, 3-4-141/80, 4-5-77/48 BOT CHORD 2-6-28/94, 4-6-28/94

WEBS 3-6=-30/77

NOTES (7)

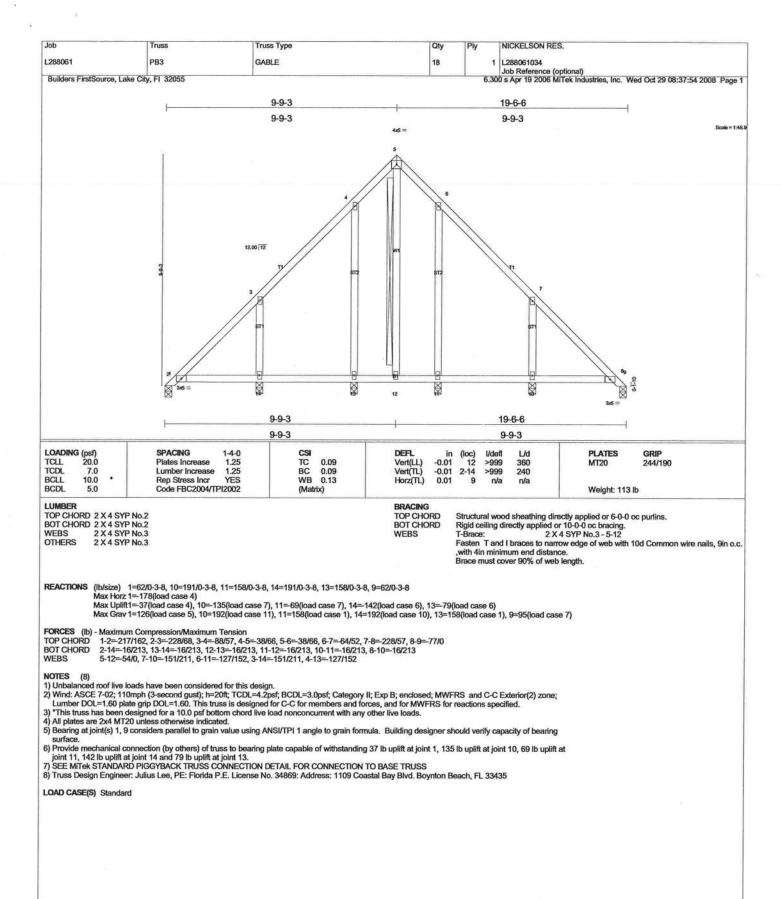
1) Unbalanced roof live loads have been considered for this design.

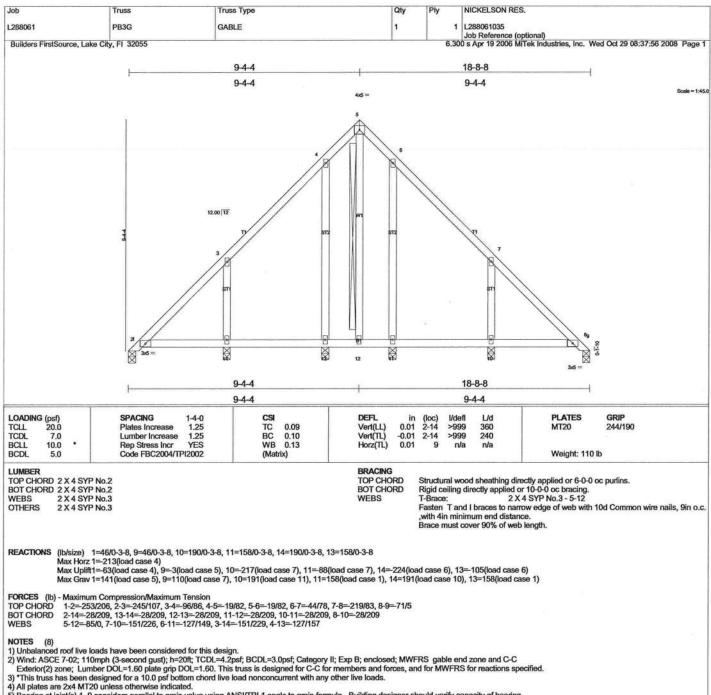
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) Bearing at joint(s) 1, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 46 lb uplift at joint 1 and 46 lb uplift at joint 5.
6) SEE MITEK STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS
7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435



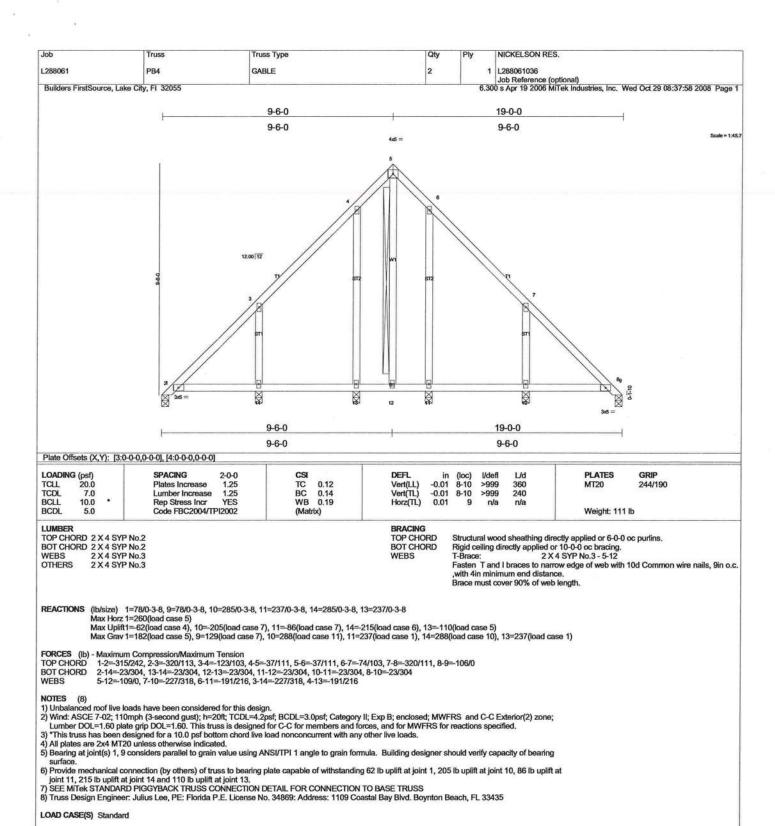


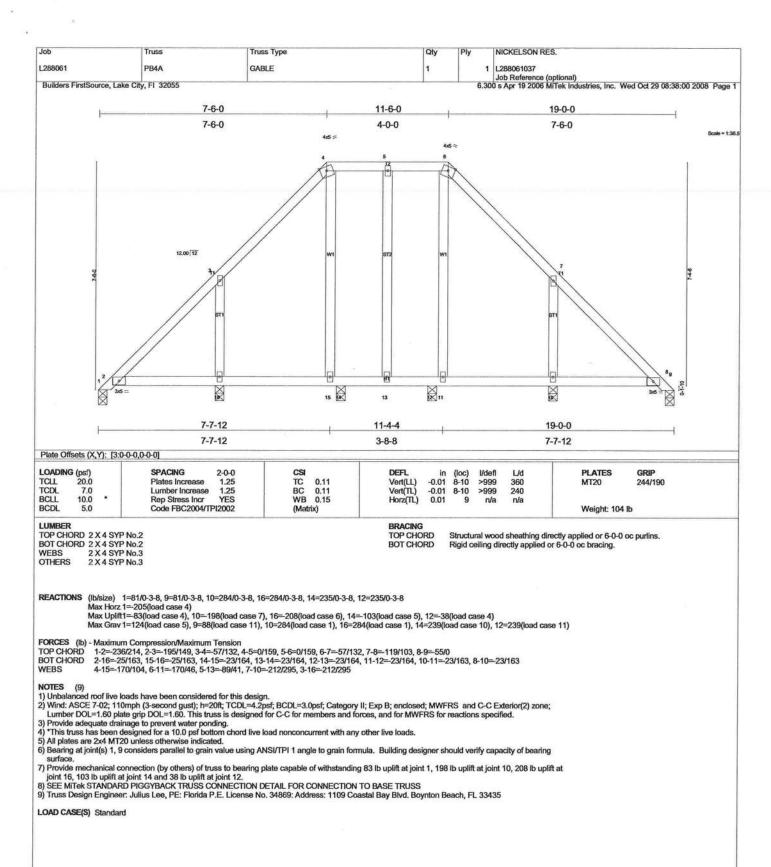
5) Bearing at joint(s) 1, 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

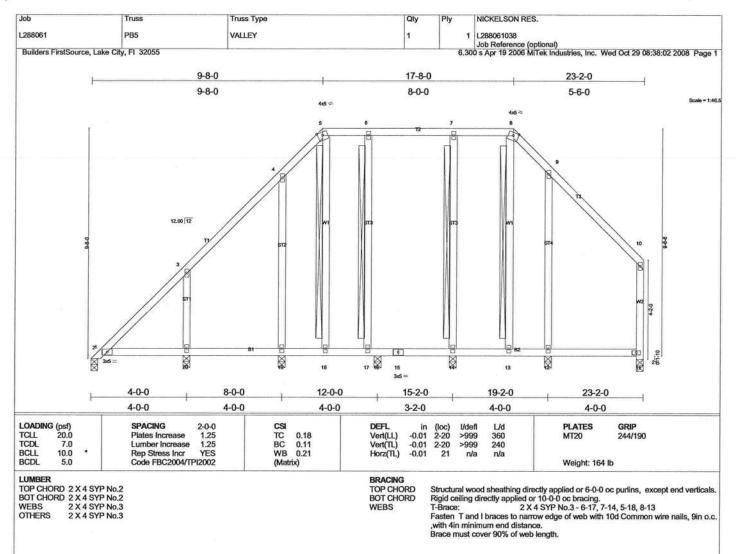
Surface.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 63 lb uplift at joint 1, 3 lb uplift at joint 9, 217 lb uplift at joint 10, 88 lb uplift at joint 11, 224 lb uplift at joint 14 and 105 lb uplift at joint 13, 7 SEE MITER STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

8) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435







REACTIONS (lb/size) 1=112/0-3-8, 12=249/0-3-8, 14=223/0-3-8, 20=286/0-3-8, 19=247/0-3-8, 21=135/0-3-8, 16=213/0-3-8

Max Horz 1=260(load case 5)

Max Uplift1=-130(load case 4), 12=-60(load case 7), 14=-87(load case 5), 20=-238(load case 6), 19=-154(load case 5), 21=-108(load case 7), 16=-86(load case 5)

Max Grav 1=203(load case 5), 12=249(load case 1), 14=225(load case 10), 20=286(load case 1), 19=254(load case 10), 21=136(load case 11), 16=218(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=329/206, 2-3=311/235, 3-4=161/230, 4-5=90/295, 5-6=17/249, 6-7=17/249, 7-8=17/249, 8-9=88/299, 9-10=95/144,

11-21=-136/159, 10-11=-117/160

2-20-11/17, 19-20-11/17, 18-19-11/17, 17-18-13/17, 16-17-13/17, 15-16-13/17, 14-15-13/17, 13-14-13/17, 12-13-11/17, 11-12-11/17 BOT CHORD

WEBS 6-17=-182/100, 9-12=-199/206, 7-14=-197/107, 3-20=-226/340, 4-19=-201/198, 5-18=-105/18, 8-13=-100/22

# NOTES (9)

(9)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

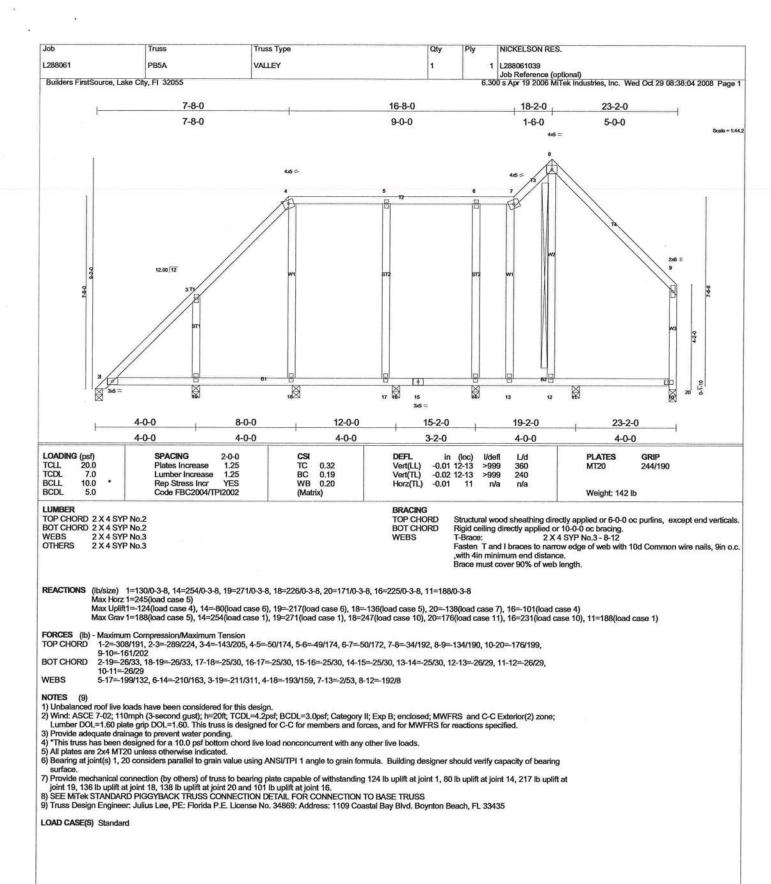
5) All plates are 2x4 MT20 unless otherwise indicated.

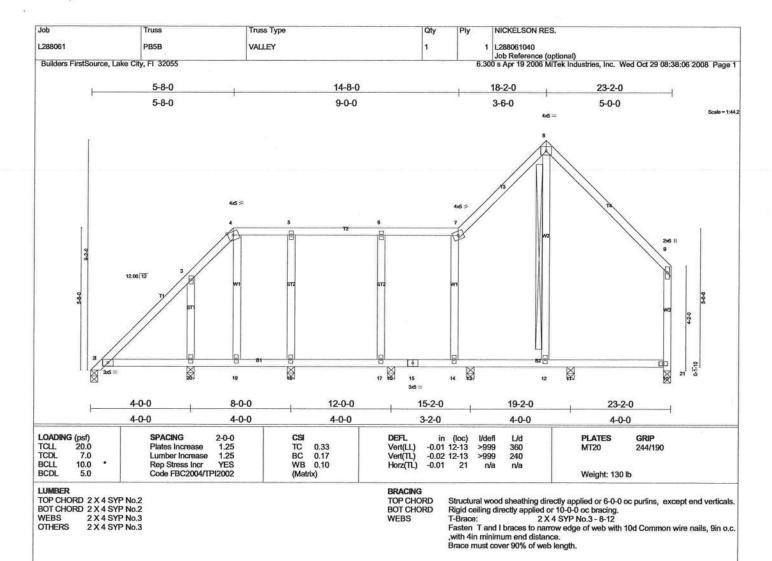
- aring at joint(s) 1, 21 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface
- suitace.

  7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 130 lb uplift at joint 1, 60 lb uplift at joint 12, 87 lb uplift at joint 14, 238 lb uplift at joint 20, 154 lb uplift at joint 19, 108 lb uplift at joint 21 and 86 lb uplift at joint 16.

  8) SEE MITEK STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435





REACTIONS (lb/size) 1=123/0-3-8, 20=268/0-3-8, 18=242/0-3-8, 21=176/0-3-8, 11=170/0-3-8, 13=249/0-3-8, 16=238/0-3-8

Max Horz 1=245(load case 5)

Max Uplift1=-123(load case 4), 20=-215(load case 5), 18=-99(load case 5), 21=-131(load case 7), 11=-18(load case 5), 13=-72(load case 6), 16=-159(load case 4)

Max Grav 1=194(load case 5), 20=278(load case 10), 18=254(load case 10), 21=184(load case 11), 11=170(load case 1), 13=249(load case 1), 16=238(load case 1)

TOP CHORD

FORCES (lb) - Maximum Compression/Maximum Tension 1-2=312/191, 2-3=287/214, 3-4=142/170, 4-5=102/138, 5-6=101/138, 6-7=101/137, 7-8=102/198, 8-9=140/178, 10-21=184/190,

9-10=-167/191

2-20-14/30, 19-20-14/30, 18-19-14/30, 17-18-14/30, 16-17-14/30, 15-16-14/30, 14-15-14/30, 13-14-20/31, 12-13-20/31, 11-12-20/31, 10-11-20/31

WEBS 6-17=179/146, 3-20=191/242, 5-18=190/110, 7-14=197/221, 4-19=53/75, 8-12=175/56

### NOTES (9)

BOT CHORD

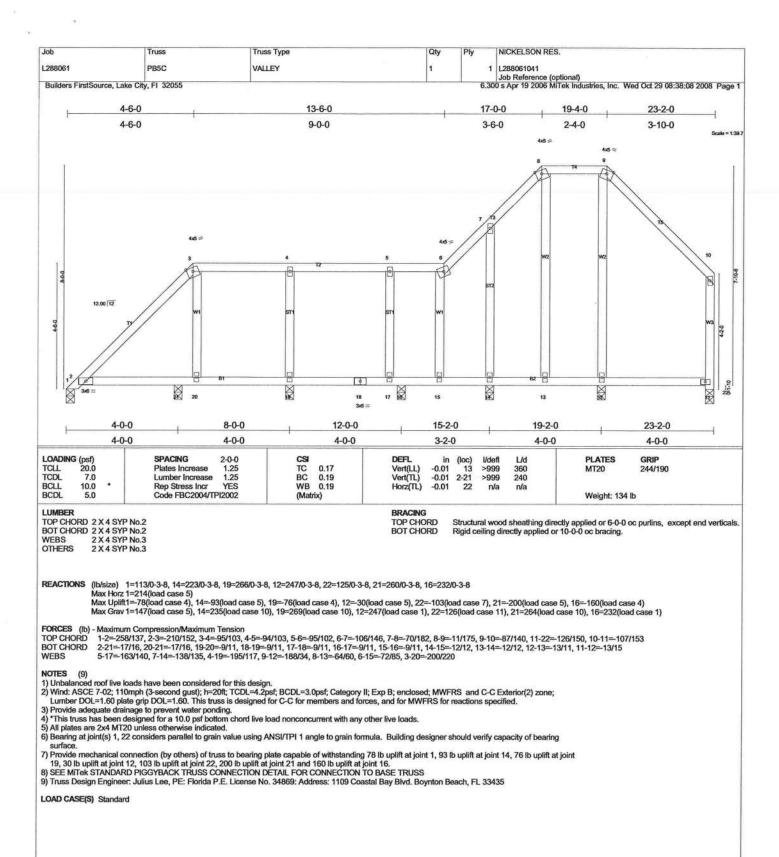
- 1) Unbalanced roof live loads have been considered for this design.
  2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
  3) Provide adequate drainage to prevent water ponding.

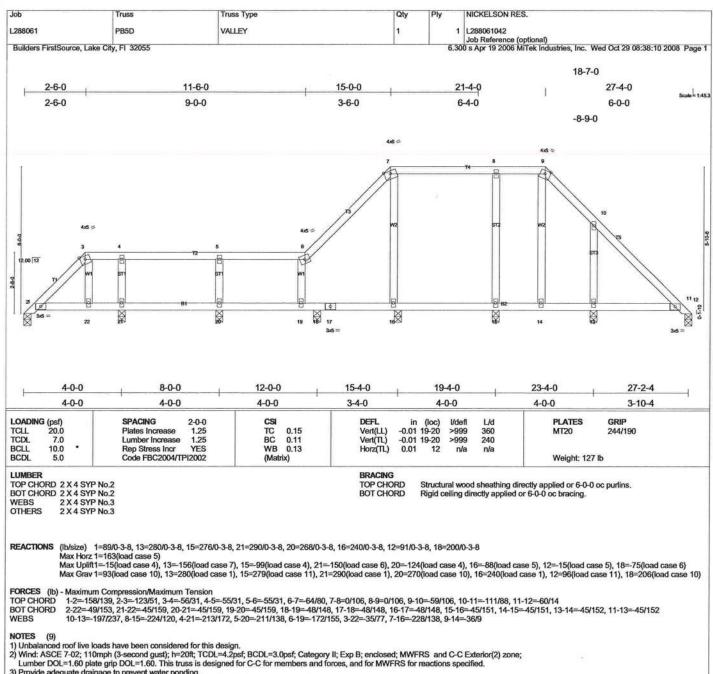
4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 2x4 MT20 unless otherwise indicated.

- 6) Bearing at joint(s) 1, 21 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 123 lb uplift at joint 1, 215 lb uplift at joint 20, 99 lb uplift at joint 18, 131 lb uplift at joint 21, 18 lb uplift at joint 11, 72 lb uplift at joint 13 and 159 lb uplift at joint 16.

  8) SEE MITER STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435



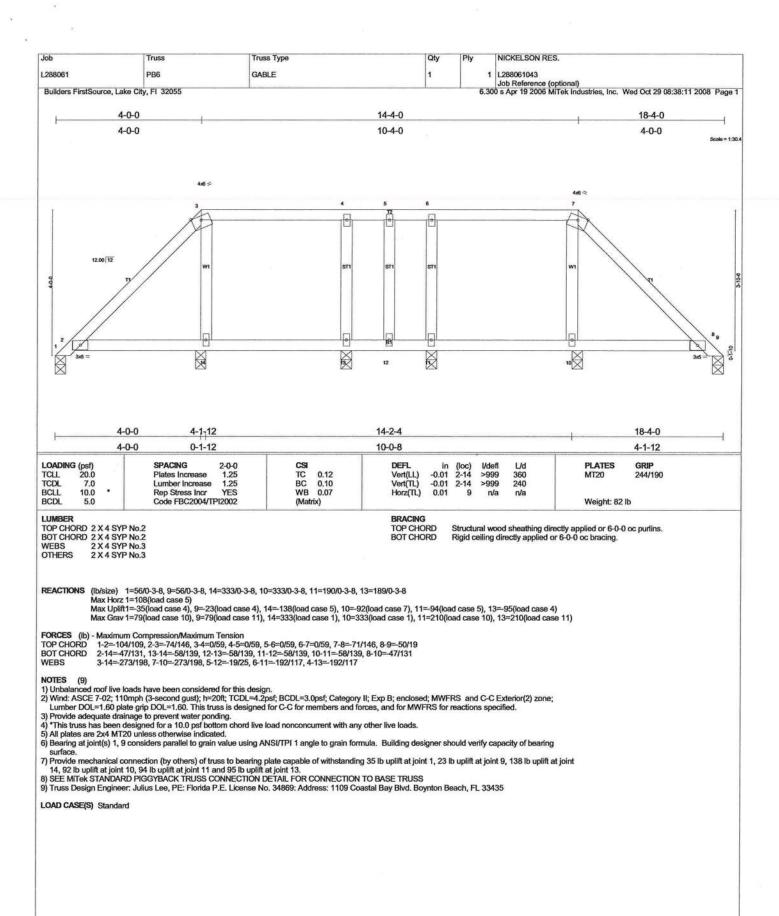


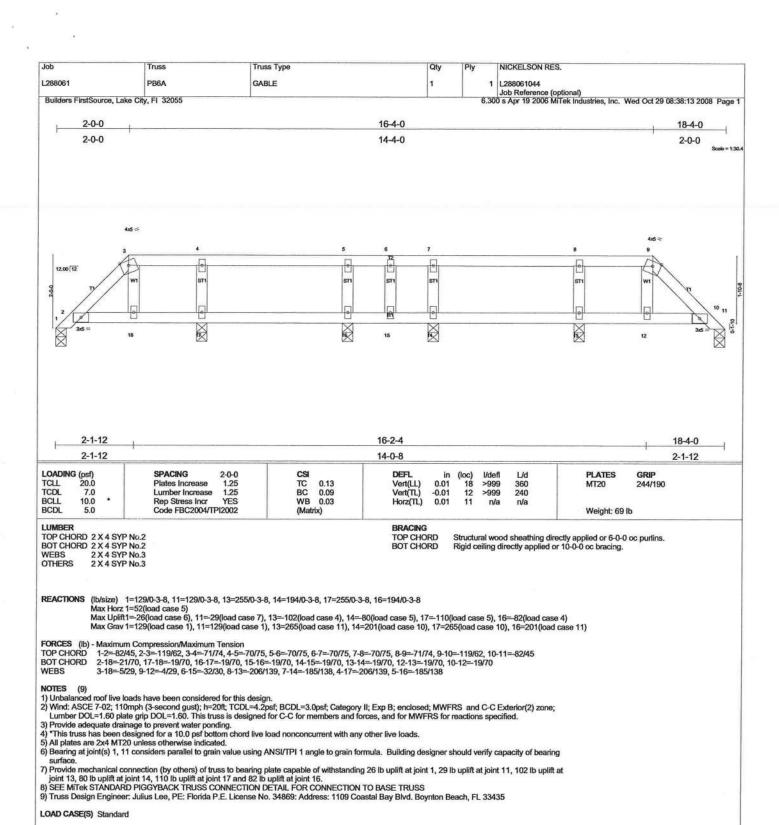
Provide adequate drainage to prevent water ponding.
 \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 All plates are 2x4 MT20 unless otherwise indicated.

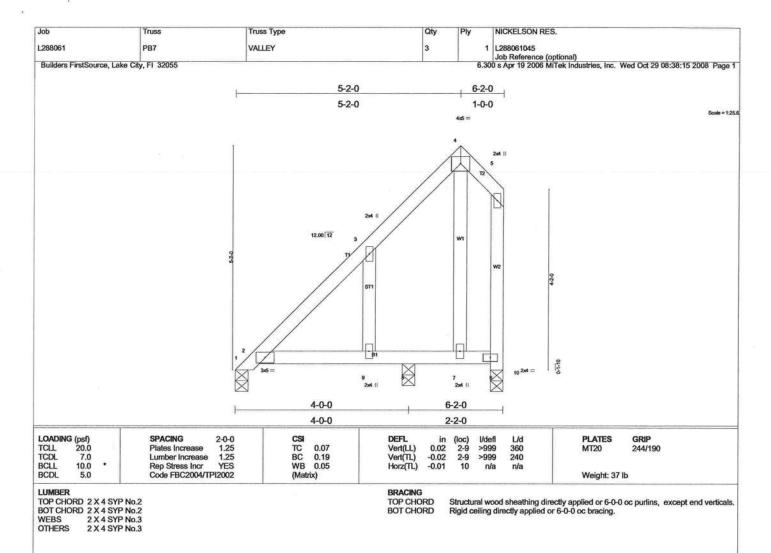
- 6) Bearing at joint(s) 1, 12 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 15 lb uplift at joint 1, 156 lb uplift at joint 13, 99 lb uplift at joint 15, 150 lb uplift at joint 21, 124 lb uplift at joint 20, 88 lb uplift at joint 16, 15 lb uplift at joint 12 and 75 lb uplift at joint 18.

  8) SEE MITEK STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435







REACTIONS (lb/size) 1=103/0-3-8, 10=23/0-3-8, 8=251/0-3-8

Max Horz 1=144(load case 6)

Max Uplift1=-1(load case 4), 10=-5(load case 10), 8=-176(load case 6)

Max Grav 1=103(load case 1), 10=37(load case 11), 8=251(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2-209/27, 2-3-200/58, 3-4-49/44, 4-5-25/39, 6-10-37/5, 5-6-31/31 BOT CHORD 2-9-2/4, 8-9-2/4, 7-8-2/4, 6-7-2/4 WEBS 3-9-132/217, 4-7-115/133

# NOTES (7)

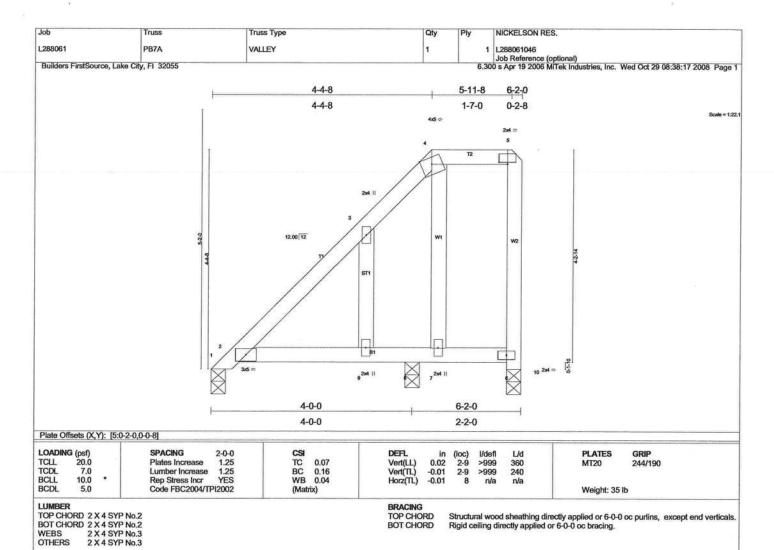
1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) Bearing at joint(s) 1, 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

- surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1 lb uplift at joint 1, 5 lb uplift at joint 10 and 176 lb uplift at joint 8.
  6) SEE MITek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS
  7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435



REACTIONS (lb/size) 1=103/0-3-8, 10=23/0-3-8, 8=252/0-3-8

Max Horz 1=137(load case 6)

Max Uplift10=-37(load case 4), 8=-168(load case 6)

Max Grav 1=103(load case 1), 10=25(load case 6), 8=252(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=217/0, 2-3=196/49, 3-4=67/35, 4-5=4/2, 6-10=25/37, 5-6=-21/32
BOT CHORD 2-9=-3/2, 8-9=-3/2, 7-8=-3/2, 6-7=-3/3
WEBS 3-9=89/160, 4-7=-119/168

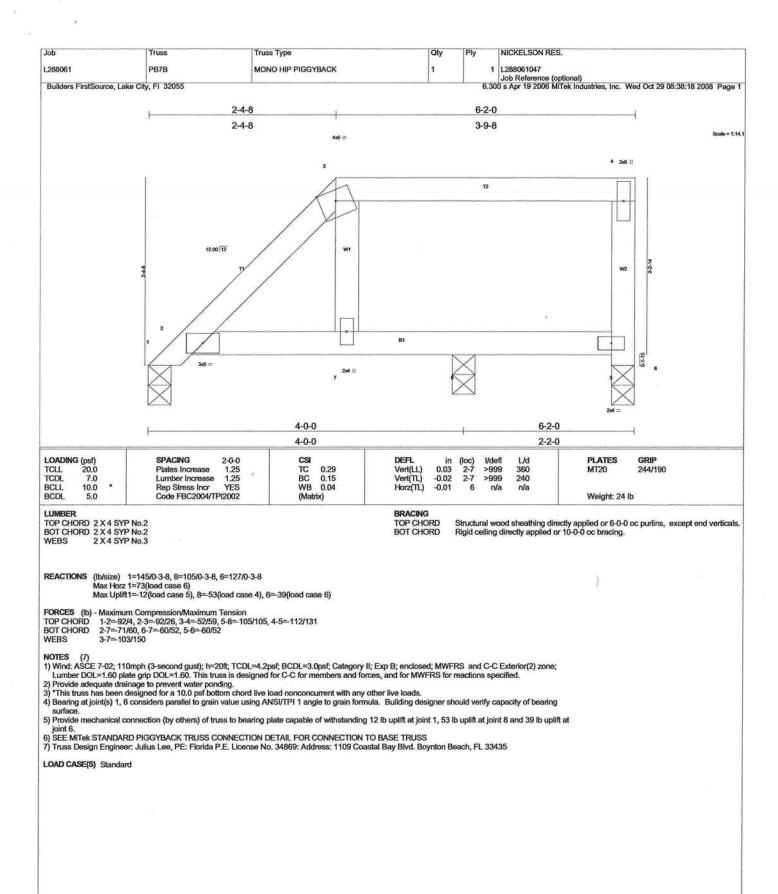
- NOTES (7)

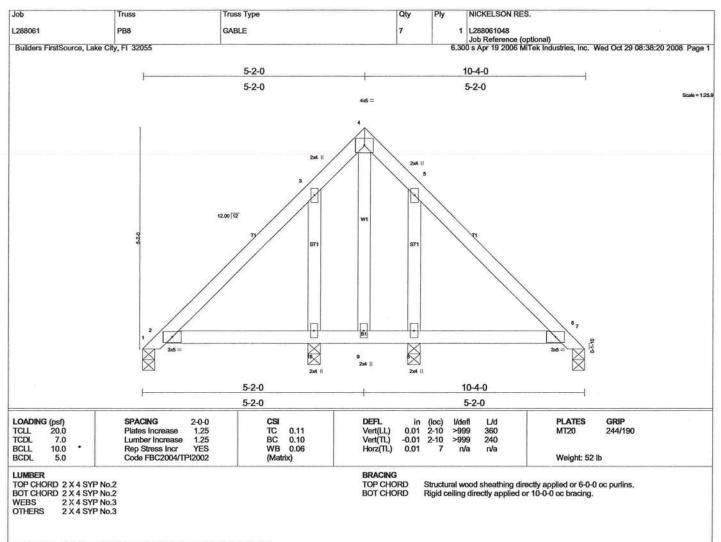
  1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DDL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

  2) Provide adequate drainage to prevent water ponding.

  3) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

  4) Bearing at joint(s) 1, 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing entrange.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 37 lb uplift at joint 10 and 168 lb uplift at joint 8.
  6) SEE MITek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS
  7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435





REACTIONS (lb/size) 1=33/0-3-8, 7=33/0-3-8, 8=290/0-3-8, 10=290/0-3-8

Max Horz 1=-139(load case 4)
Max Uplift1=-25(load case 4), 7=-8(load case 10), 8=-143(load case 7), 10=-165(load case 6)
Max Grav 1=79(load case 5), 7=54(load case 11), 8=290(load case 1), 10=290(load case 1)

FORCES (Ib) - Maximum Compression/Maximum Tension
TOP CHORD
1-2=157/127, 2-3=205/179, 3-4=-9/82, 4-5=-18/82, 5-6=-205/179, 6-7=-52/12
BOT CHORD
VEBS
4-9=-83/5, 5-8=-199/253, 3-10=-199/253

NOTES (7)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

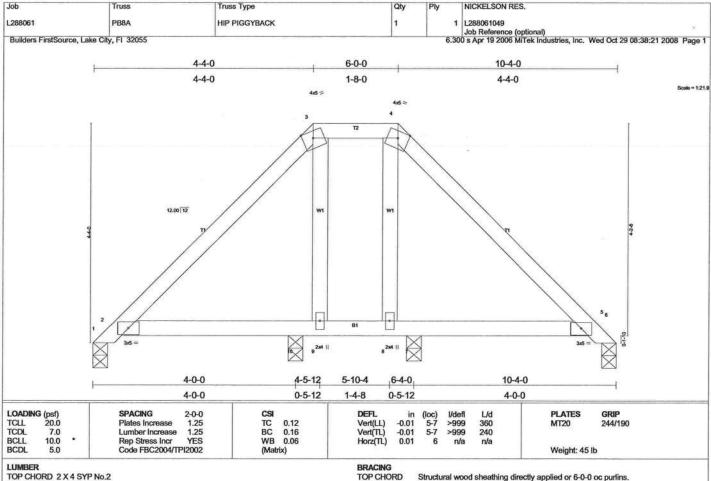
3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) Bearing at joint(s) 1, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 25 lb uplift at joint 1, 8 lb uplift at joint 7, 143 lb uplift at joint 8 and 165 lb uplift at joint 10.

6) SEE MITCH STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435



TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 6-0-0 oc bracing. BOT CHORD

REACTIONS (lb/size) 1=68/0-3-8, 6=68/0-3-8, 10=255/0-3-8, 7=255/0-3-8

Max Horz 1=-117(load case 4)

Max Uplift1=-33(load case 4), 6=-11(load case 4), 10=-126(load case 5), 7=-90(load case 7)

Max Grav 1=84(load case 10), 6=84(load case 11), 10=255(load case 1), 7=255(load case 1)

FORCES (Ib) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=114/115, 2-3=68/128, 3-4=0/51, 4-5=67/128, 5-6=53/11
BOT CHORD 2-10=-47/148, 9-10=-47/148, 8-9=-51/154, 7-8=-47/148, 5-7=-47/148
WEBS 3-9=-202/157, 4-8=-202/157

## NOTES (8)

NOTES (8)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

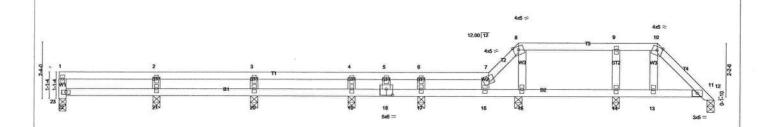
3) Provide adequate drainage to prevent water ponding.

4) Which have been designed for a 40.0 are better short live load proposurement with any other live loads.

- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
   Bearing at joint(s) 1, 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 33 lb uplift at joint 1, 11 lb uplift at joint 6, 126 lb uplift at joint 10 and 90 lb uplift at joint 7.
  7) SEE MITER STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

8) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

Job	Truss	Truss Type	Qty	Ply	NICKELSON R	ES.	
L288061	PB9	GABLE	1	1	L288061050 Job Reference	(ontional)	
Builders FirstSource	ce, Lake City, FI 32055			6.30		MiTek Industries, Inc. Wed	Oct 29 08:38:23 2008 F
-		17-7-4			18-10-0	24-6-0	26-10-0
1.5*		17-7-4			1-2-12	5-8-0	2-4-0



26-10-0 26-10-0 Plate Offsets (X,Y): [18:0-3-0.0-3-0]

LOADIN	G (psf)	- 1	SPACING	1-4-0	CSI		DEFL	in	(loc)	<b>V</b> defi	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.09	Vert(LL)	0.01	13	>999	360	MT20	244/190
TCDL	7.0		Lumber Increase	1.25	BC	0.06	Vert(TL)	-0.01	11-13	>999	240	100000000	
BCLL	10.0		Rep Stress Incr	YES	WB	0.03	Horz(TL)	0.00	12	n/a	n/a		
BCDL	5.0		Code FBC2004/TI	PI2002	(Mati	rix)	115-2000-1000					Weight: 93	lb

LUMBER TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3 OTHERS 2 X 4 SYP No.3

BRACING TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 12=72/0-3-8, 14=178/0-3-8, 17=155/0-3-8, 21=183/0-3-8, 20=174/0-3-8, 19=137/0-3-8, 15=166/0-3-8, 23=68/0-3-8

Max Horz 23=41(load case 4)

Max Uplift12=18(load case 7), 14=65(load case 4), 17=54(load case 4), 21=59(load case 4), 20=65(load case 4), 19=50(load case 4), 15=51(load case 5), 23=31(load case 4)

Max Grav 12=73(load case 11), 14=184(load case 11), 17=155(load case 1), 21=185(load case 10), 20=174(load case 1), 19=139(load case 10), 15=172(load case 10), 23=68(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

22-23=68/37, 1-22=61/45, 1-2=23/15, 2-3=23/15, 3-4=23/15, 4-5=23/15, 5-6=23/15, 6-7=23/15, 7-8=10/30, 8-9=16/38, 9-10=15/38, 10-11=44/22, 11-12=46/26

BOT CHORD

BOT CHORD

21-22=15/28, 20-21=15/28, 19-20=15/28, 18-19=15/28, 17-18=15/28, 16-17=15/28, 15-16=16/24, 14-15=11/27, 13-14=11/27, 14-1

11-13=-11/26 WEBS

5-18=0/1, 9-14=140/89, 6-17=-119/84, 2-21=-151/106, 3-20=-147/106, 4-19=-120/86, 8-15=-131/78, 7-16=-24/29, 10-13=-11/9

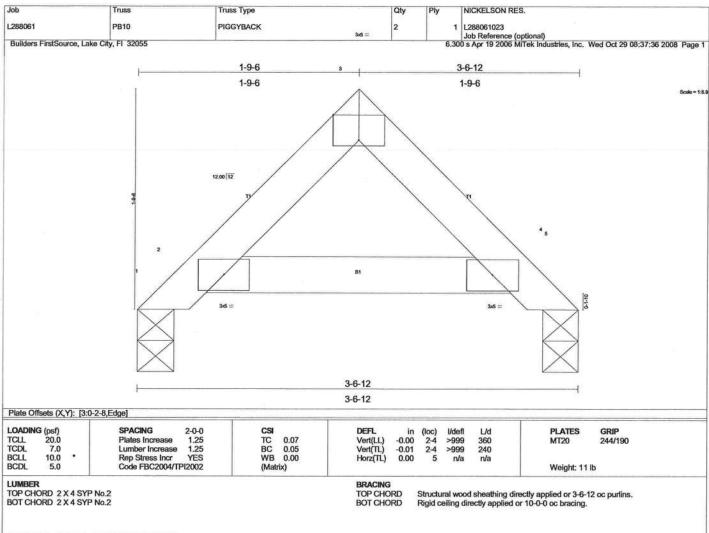
NOTES (8)

Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust): h=20t; TCDL=4,2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.
4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 2x4 MT20 unless otherwise indicated.
6) Bearing at joint(s) 12, 23 considers parallel to grain value using ANS//TPI 1 angle to grain formula. Building designer should verify capacity of bearing

6) Bearing at Joint(s) 12, 23 Consider published by Bearing plate capable of withstanding 18 lb uplift at joint 12, 65 lb uplift at joint 14, 54 lb uplift at joint 17, 59 lb uplift at joint 21, 65 lb uplift at joint 20, 50 lb uplift at joint 19, 51 lb uplift at joint 15 and 31 lb uplift at joint 23.
8) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435



REACTIONS (lb/size) 1=106/0-3-8, 5=106/0-3-8 Max Horz 1=-45(load case 4) Max Uplift1=-19(load case 6), 5=-19(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=67/42, 2-3=97/56, 3-4=97/56, 4-5=67/42 BOT CHORD 2-4=17/71

NOTES (7)

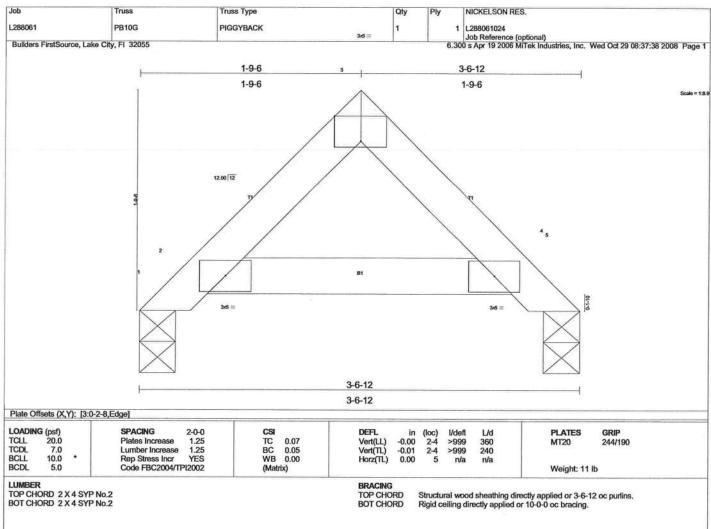
1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) Bearing at joint(s) 1, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 19 lb uplift at joint 1 and 19 lb uplift at joint 5.
6) SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS
7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435



REACTIONS (lb/size) 1=106/0-3-8, 5=106/0-3-8 Max Horz 1=-57(load case 4) Max Uplift1=-40(load case 6), 5=-40(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=67/47, 2-3=97/56, 3-4=97/56, 4-5=67/42 BOT CHORD 2-4=24/71

NOTES (7)

I) Unbalanced roof live loads have been considered for this design.

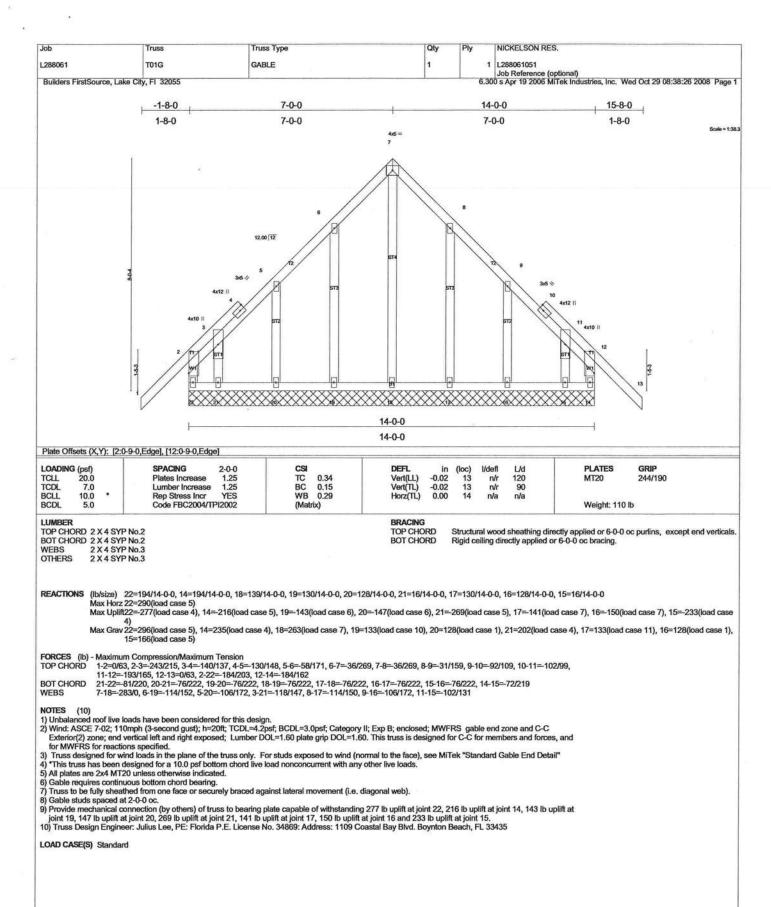
1) Unbalanced roof live loads have been considered for this design.

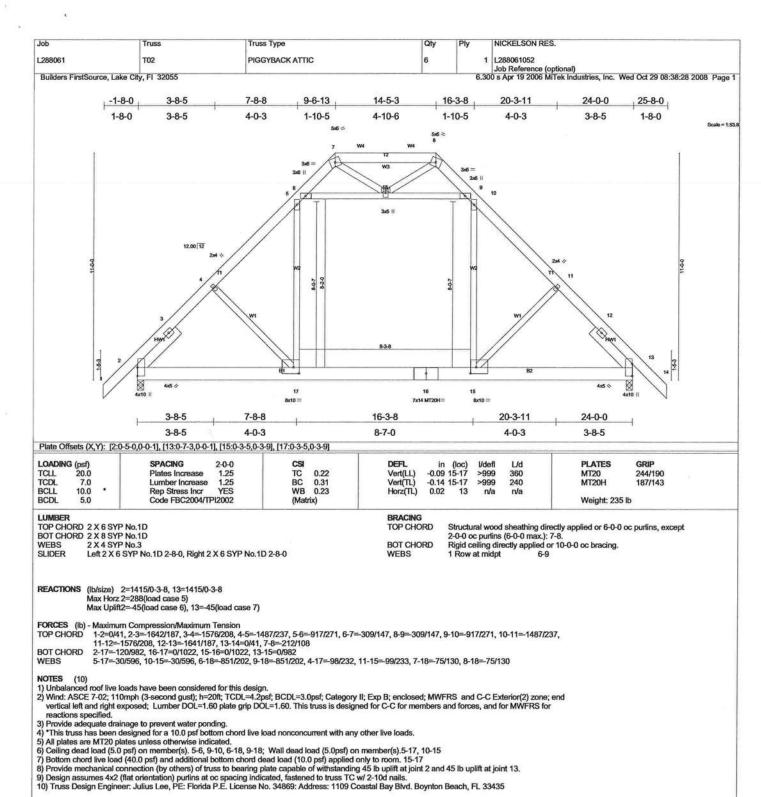
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

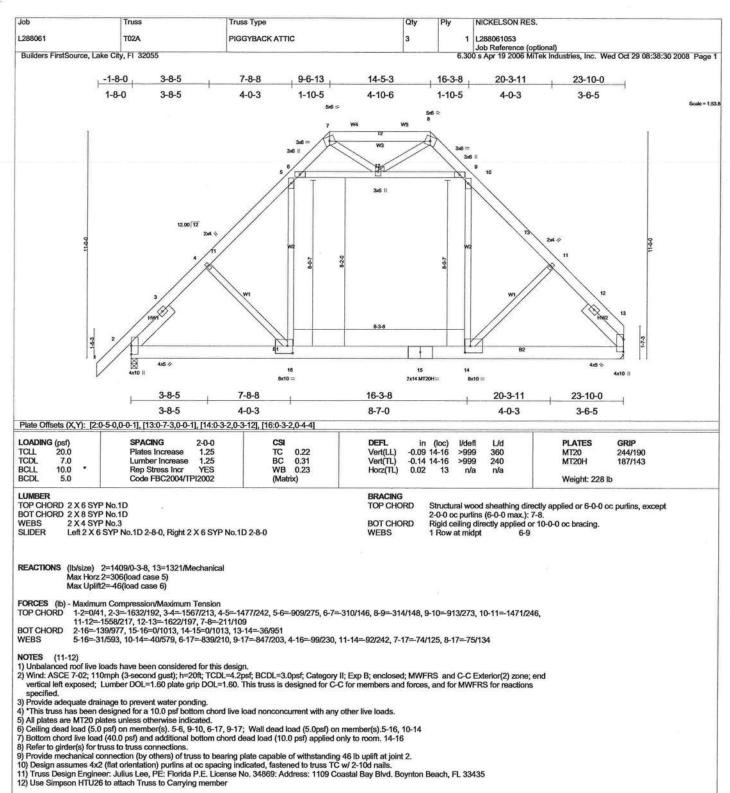
3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

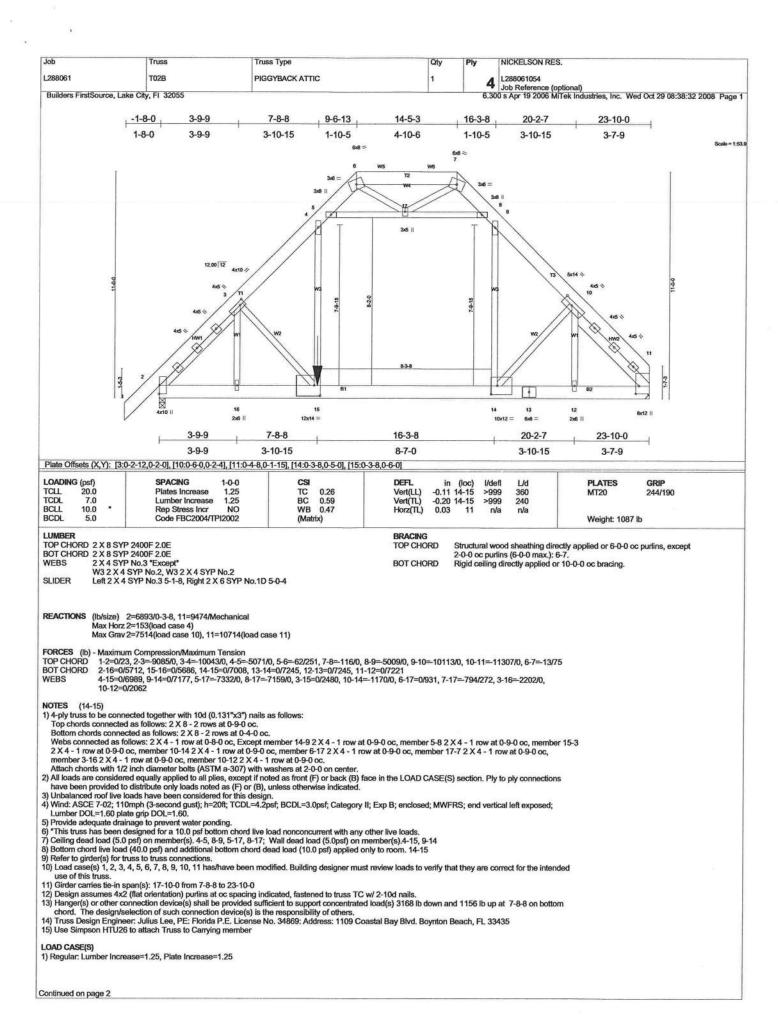
4) Bearing at joint(s) 1, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 40 lb uplift at joint 1 and 40 lb uplift at joint 5.
6) SEE MITek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS
7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

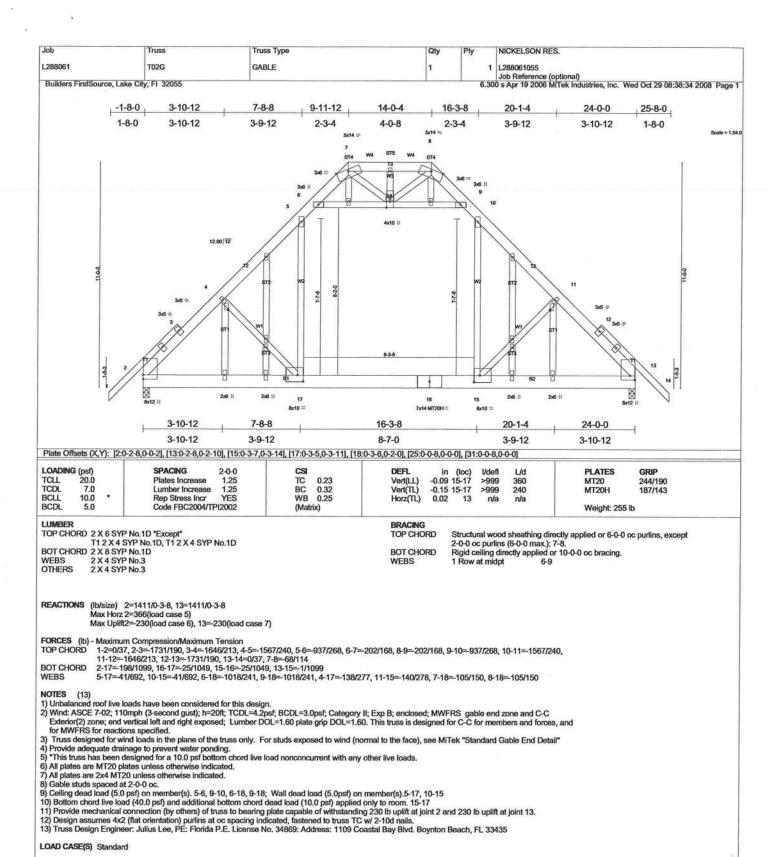


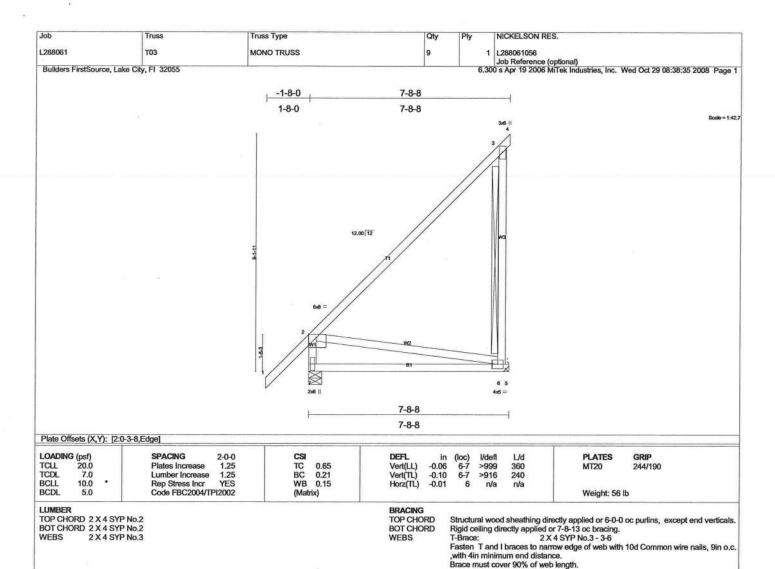






	-			123	
Job	Truss	Truss Type	Qty	Ply	NICKELSON RES.
L288061	T02B	PIGGYBACK ATTIC	1	4	L288061054 Job Reference (optional)
Builders FirstSource, Lake Cit	y, FI 32055			6.30	0 s Apr 19 2006 MiTek Industries, Inc. Wed Oct 29 08:38:32 2008 Page 2
Drag: 4-15=-5, 9-1 Concentrated Loads (lb) Vert: 15=-3168(F) 2) IBC BC Live: Lumber Incre Uniform Loads (plf)	4=-5 ease=1.25, Plate Increase=1.25	=-734), 1-4=-27, 4-5=-32, 5-6=-27, 7-8=-27 5 -12, 5-6=-7, 7-8=-7, 8-9=-12, 9-11=-7, 6-7=		7, 6-7=-27	5-85
Drag: 4-15=-5, 9-14 Concentrated Loads (lb)	4=5				
Vert: 15=-1188(F) 3) MWFRS Wind Left: Lumbe	er Increase=1.60, Plate Increas	e=1.60			
Horz: 1-2=10, 2-6- Drag: 4-15=5, 9-14 Concentrated Loads (lb)	=1, 7-11=13	=-320), 1-2=6, 2-4=-5, 4-5=-8, 5-6=-5, 7-8=	9, 8-9=6, 9-11=9,	6-7=15, 5-6	<b>⇒</b> 3
Vert: 15=1156(F) 4) MWFRS Wind Right: Lumb	per Increase=1.60, Plate Increa	use=1.60			
Horz: 1-2=9, 2-6= Drag: 4-15=5, 9-14 Concentrated Loads (lb)	-13, 7-11=-1	=-320), 1-2=5, 2-4=9, 4-5=6, 5-6=9, 7-8=-5	, 8-9=-8, 9-11=-5,	6-7=15, 5-6	<b>⊨</b> 3
Vert: 15=1156(F)	Lumber Increase=1.60, Plate	Increase=1 60			
Uniform Loads (plf)	5=329(F=320), 11-14=323(F =16, 7-11=10	=320), 1-2=21, 2-4=12, 4-5=9, 5-6=12, 7-8	3=6, 8-9=3, 9-11=6	, 6-7=6, 5-1	3=3
Vert: 15=876(F)					
<ol><li>6) MWFRS 2nd Wind Parallel Uniform Loads (plf)</li></ol>	l: Lumber Increase=1.60, Plate	Increase=1.60			
	10, 7-11=16	=320), 1-2=3, 2-4=6, 4-5=3, 5-6=6, 7-8=12	2, 8-9=9, 9-11=12,	6-7=6, 5-8	-3
Concentrated Loads (lb)					
Vert: 15=876(F) 7) MWFRS 3rd Wind Parallel:	: Lumber Increase=1.60, Plate	Increase=1.60			
Horz: 1-2=-25, 2-6=	-16, 7-11=10	=320), 1-2=21, 2-4=12, 4-5=9, 5-6=12, 7-8	3=6, 8-9=3, 9-11=6	, 6-7=6, 5-8	3=3
Drag: 4-15=5, 9-14 Concentrated Loads (lb) Vert: 15=876(F)	j=-5				
8) MWFRS 4th Wind Parallel:	Lumber Increase=1.60, Plate	Increase=1.60			
Uniform Loads (plf) Vert: 2-15=-3, 14-1: Horz: 1-2=-7, 2-6=- Drag: 4-15=-5, 9-14	10, 7-11=16	=320), 1-2=3, 2-4=6, 4-5=3, 5-6=6, 7-8=12	2, 8-9=9, 9-11=12,	6-7=6, 5-8=	=3
Concentrated Loads (lb)					
Vert: 15=876(F) 9) Attic Floor: Lumber Increas	e=1.00, Plate Increase=1.00				
Uniform Loads (plf)	5000/E95/\\ 11-1/950/E	=854), 1-4=-7, 4-5=-12, 5-6=-7, 7-8=-7, 8-9	- 12 0.11- 7 67	-750-6	
Drag: 4-15=5, 9-14 Concentrated Loads (lb) Vert: 15=-1188(F)	=5	-001), 14-1,40-12,00-1,10-1,00	-12, 5-11-7, 6-7		·
10) 1st unbalanced Regular: L	umber Increase=1.25, Plate In	crease=1.25			
Drag: 4-15=-5, 9-1		F=854), 1-4=27, 4-5=32, 5-6=27, 7-8=7,	8-9=12, 9-11=7,	6-7=-27, 5	8=-5
Concentrated Loads (lb) Vert: 15=-3168(F)					
11) 2nd unbalanced Regular:	Lumber Increase=1.25, Plate In	ncrease=1.25			
Drag: 4-15=-5, 9-1		F=854), 1-4=7, 4-5=12, 5-6=7, 7-8=-27, 8	8-9-32, 9-11-27,	6-7=-27, 5	-8=-5
Concentrated Loads (lb) Vert: 15=3168(F)					





REACTIONS (lb/size) 6=228/Mechanical, 7=343/0-6-0 Max Horz 7=361 (load case 6) Max Uplift6=-229(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/69, 2-3=-206/95, 3-4=-3/0, 3-6=-160/258, 2-7=-308/44 BOT CHORD 6-7=-639/178, 5-6=0/0

- NOTES (5)

  1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions
- specified.

  2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 229 lb uplift at joint 6.
5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

NICKELSON RES. Job Truss Type Qty Truss L288061 T03G GABLE 1 L288061057 Job Reference (optional) 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Oct 29 08:38:37 2008 Page 1 Builders FirstSource, Lake City, FI 32055 -1-8-0 7-8-8 1-8-0 7-8-8 Scale = 1:41.8 214 2s4 7-8-8 7-8-8 Plate Offsets (X,Y): [2:0-6-0,Edge] LOADING (psf) SPACING PLATES 244/190 TCLL 20.0 Plates Increase 1.25 1.25 TC BC 0.48 Vert(LL) -0.05 -0.08 7-8 7-8 >999 360 240 MT20 7.0 Lumber Increase Vert(TL) >999 Rep Stress Incr YES Code FBC2004/TPI2002 WB 0.27 (Matrix) BCLL 10.0 -0.01 BCDL Weight: 75 lb 5.0 LUMBER BRACING

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

2 X 4 SYP No.3 \*Except\* WEBS W1 2 X 6 SYP No.1D OTHERS 2 X 4 SYP No.3

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 7-1-1 oc bracing.

T-Brace: 2 X 4 SYP No.3 - 4-7

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c. ,with 4in minimum end distance. Brace must cover 90% of web length.

REACTIONS (lb/size) 7=224/Mechanical, 8=346/3-0-0

Max Horz 8=468(load case 6) Max Uplift7=332(load case 6), 8=9(load case 6)

FORCES (b) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/68, 2-3=-202/0, 3-4=-167/82, 4-5=-3/0, 4-7=-137/231, 2-8=-312/72
BOT CHORD 7-8=-766/312, 6-7=0/0

NOTES (7)

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"

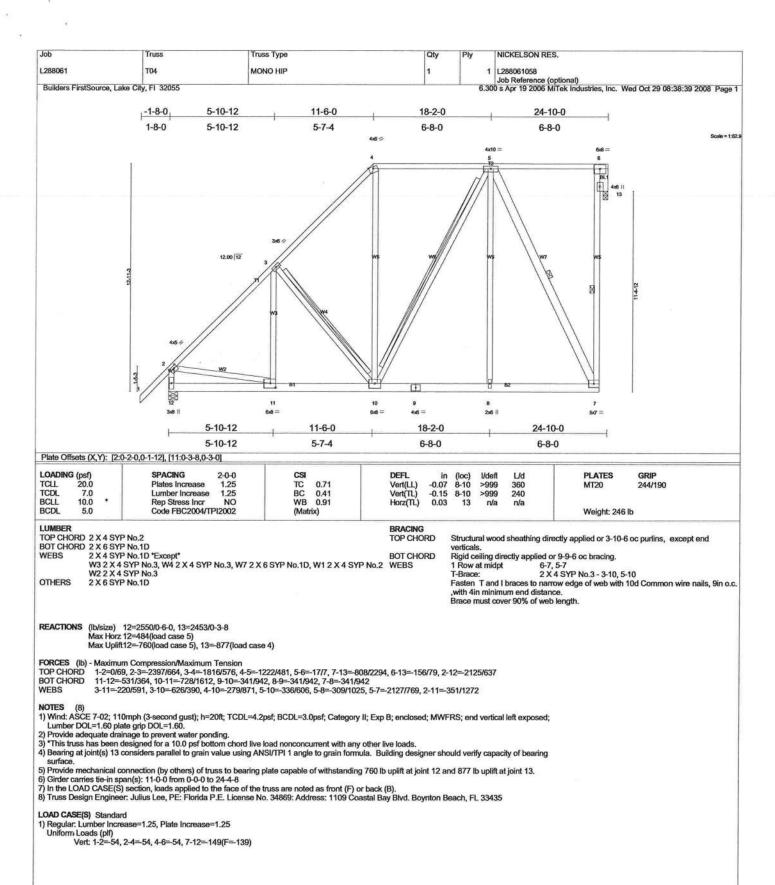
3) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

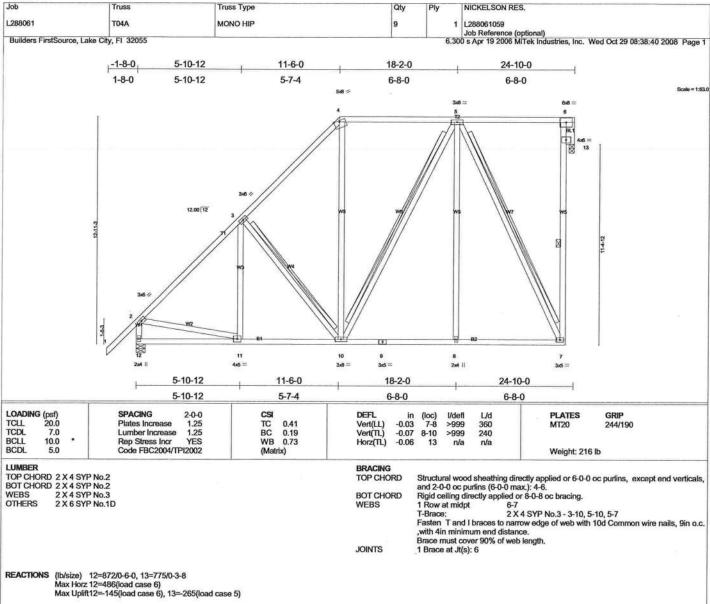
4) Gable studs spaced at 2-0-0 oc.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 332 lb uplift at joint 7 and 9 lb uplift at joint 8.

7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435





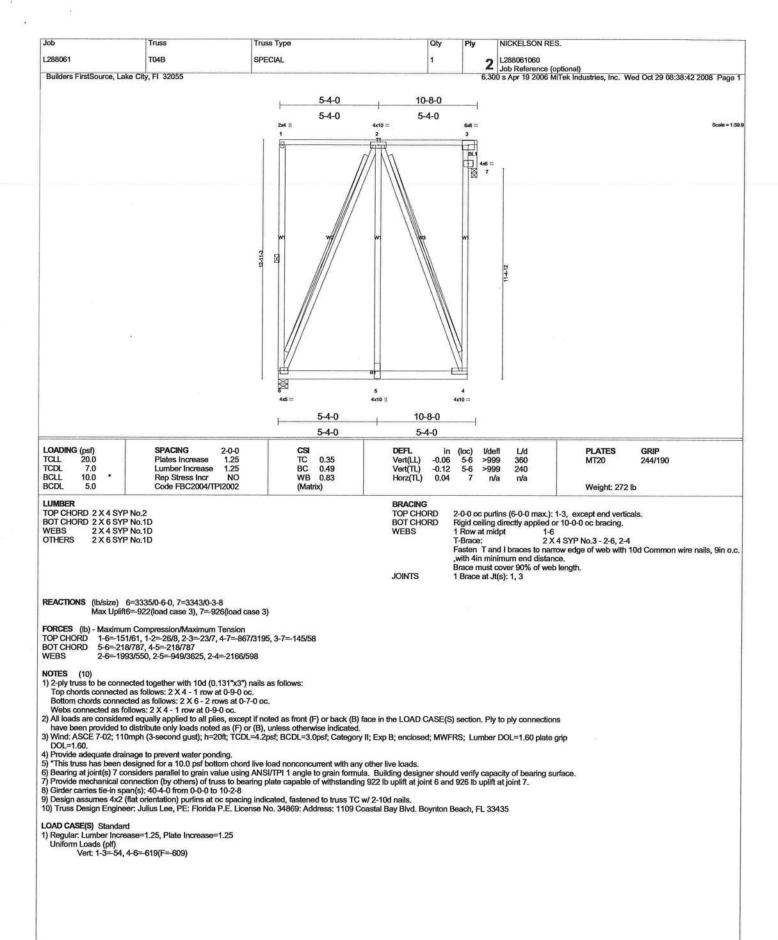
FORCES (ib) - Maximum Compression/Maximum Tension
TOP CHORD
TOP CHORD
11-2=0/69, 2-3=813/242, 34=-638/316, 4-5=378/330, 5-6=8/5, 7-13=425/627, 6-13=147/107, 2-12=840/345
BOT CHORD
WEBS
41-2=621/83, 10-11=532/496, 9-10=214/294, 8-9=214/294, 7-8=214/294
3-11=37/114, 3-10=187/316, 4-10=0/172, 5-10=256/183, 5-8=0/204, 5-7=-660/483, 2-11=25/422

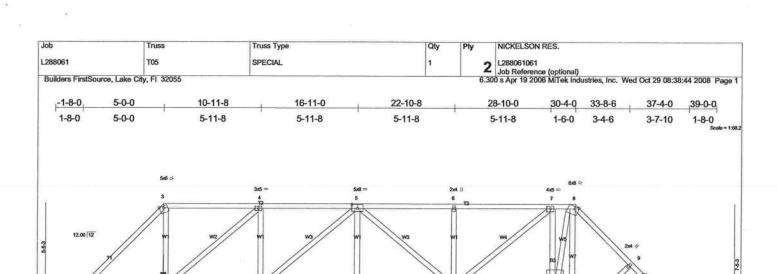
NOTES (7)

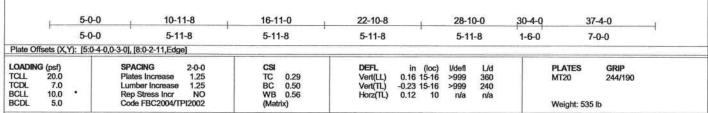
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions

Provide adequate drainage to prevent water ponding.
 \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 Bearing at joint(s) 13 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 145 lb uplift at joint 12 and 265 lb uplift at joint 13. 6) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.
7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435







653

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 6 SYP No.1D \*Except\*

B3 2 X 4 SYP No.3

WEBS 2 X 4 SYP No.3

BRACING TOP CHORD

**BOT CHORD** 

Structural wood sheathing directly applied or 6-0-0 oc purfins. Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 6-0-0 oc bracing: 13-14.

24 11 548 =

3

4x10 =

REACTIONS (lb/size) 2=2093/0-6-0, 10=2146/0-6-0 Max Horz 2=309(load case 4)

Max Uplift2=1112(load case 4), 10=1025(load case 3)

19

18

17 4x10 =

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/68, 2-3=-2624/1482, 3-4=-1797/1111, 4-5=-3149/1871, 5-6=-3596/2059, 6-7=-3596/2059, 7-8=-2524/1458, 8-9=-2433/1348,

9-10=-2563/1327, 10-11=0/68 **BOT CHORD** 

2-19-1231/1754, 18-19-2012/3149, 17-18-2311/3754, 16-17-2311/3754, 15-16-2311/3754, 14-15-1459/2584, 13-14-169/30,

2-19-12311734, 16-19-20123146, 17-10-22-110-75, 10-12-110-WEBS

12-14-1285/2722, 8-14-2032/3473, 8-12-1851/1077, 9-12-73/99

### NOTES (11)

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc, 2 X 4 - 1 row at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

3) Unbalanced roof live loads have been considered for this design.

4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip

5) Provide adequate drainage to prevent water ponding.

\*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1112 lb uplift at joint 2 and 1025 lb uplift at joint 10. 8) Girder carries tie-in span(s): 5-0-0 from 28-10-0 to 37-4-0

8) Girder cames the in span(s): 5-0-0 from 28-10-0 to 37-4-0
 9) Girder cames the in span(s): 5-0-0 from 28-10-0 to 37-4-0
 9) Girder cames the in with 7-0-0 right side setback, 5-0-0 left side setback, and 5-0-0 end setback.
 10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 187 lb down and 178 lb up at 5-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
 11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

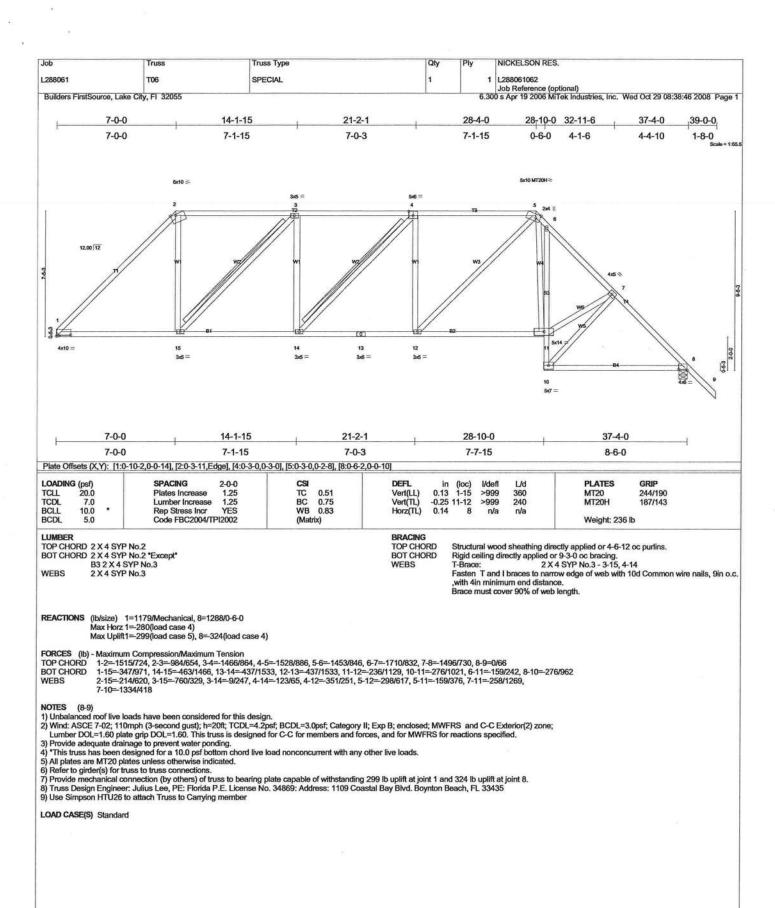
LOAD CASE(S) Standard

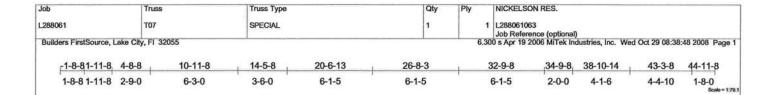
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

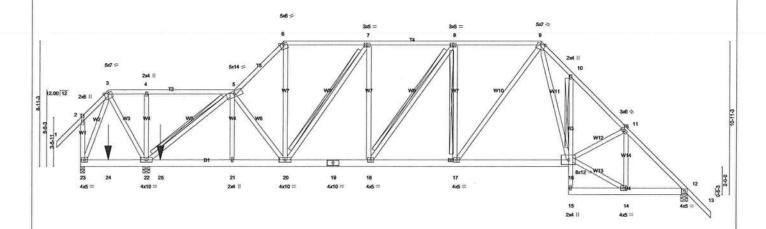
Uniform Loads (pif)
Vert: 1-3=54, 3-8=93(F=39), 8-11=54, 2-19=10, 14-19=17(F=7), 12-13=57(F=47), 10-12=50(F=40)

Concentrated Loads (lb)

Vert: 19=187(F)







	4-8-8		10-11-8	14-5-8	20-6-13	26-8-3	34-9-8	38-10-14	43-3-8
-		4-8-8	6-3-0	3-6-0	6-1-5	6-1-5	8-1-5	4-1-6	4-4-10
LOADIN	G (psf)		SPACING	2-0-0	CSI	DEFL in (	loc) I/defl L/d	PLATES	GRIP
TCLL	20.0 7.0		Plates Increase Lumber Increase	1.25 1.25	TC 0.52 BC 0.24	Vert(IL) -0.07 Vert(TL) -0.15 16	(012) (1125) HELE	MT20	244/190
BCLL BCDL	10.0 5.0	•	Rep Stress Incr Code FBC2004/TP	NO 12002	WB 0.66 (Matrix)	Horz(TL) 0.06	12 n/a n/a	Weight: 361	lb

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 6 SYP No.1D \*Except\* B3 2 X 4 SYP No.3

WEBS 2 X 4 SYP No.3 BRACING

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 4-9-5 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 6-0-0 oc bracing: 22-23,14-15.

WEBS

T-Brace: 2 X 4 SYP No.3 - 10-16
T-Brace: 2 X 4 SYP No.3 - 5-22, 7-20, 8-18, 8-17
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c. with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 12=1238/0-6-0, 22=2826/0-6-0, 23=-620/0-3-8 Max Horz 23=-331(load case 3) Max Upit12=301(load case 3), 22=1082(load case 3), 23=-653(load case 10) Max Grav 12=1238(load case 1), 22=2826(load case 1), 23=178(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD

1.2=0/71, 2-3=-1/217, 3-4=-148/714, 4-5=-148/713, 5-6=-1112/347, 6-7=-750/273, 7-8=-1099/387, 8-9=-1184/425, 9-10=-1553/526, 10-11=-1641/448, 11-12=-1439/365, 12-13=0/68, 2-23=-132/278

23-24-293/378, 22-24-293/378, 22-25-271/690, 21-25-271/690, 20-21-270/688, 19-20-328/1099, 18-19-328/1099, 17-18-300/1183, 16-17-174/965, 15-16-0/57, 10-16-95/159, 14-15-16/32, 12-14-153/933, 3-22-873/281, 4-22-343/192, 5-22-1799/497, 5-21-17/159, 5-20-0/202, 6-20-177/484, 7-20-652/255, 7-18-18/254, 8-18-184/67, BOT CHORD

8-17=274/222, 9-17=216/448, 9-16=194/532, 14-16=154/1008, 11-16=97/211, 11-14=447/101, 3-23=338/742

NOTES

WEBS

NOTES (9)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; end vertical left exposed; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60.

3) Provide adequate drainage to prevent verter ponding.

1) This has been designed force 10.0 per bottom choose the load population and the load population of the loads.

4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 301 lb uplift at joint 12, 1082 lb uplift at joint 22 and 653 lb uplift at joint 23.

Girder carries hip end with 38-5-4 right side setback, 1-11-8 left side setback, and 2-6-0 end setback.

7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 465 lb down and 384 lb up at 5-9-0, and 21 lb down and 20 lb up at 2-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

8) In the LOAD CASE(s) section, loads applied to the face of the truss are noted as front (F) or back (B).

9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

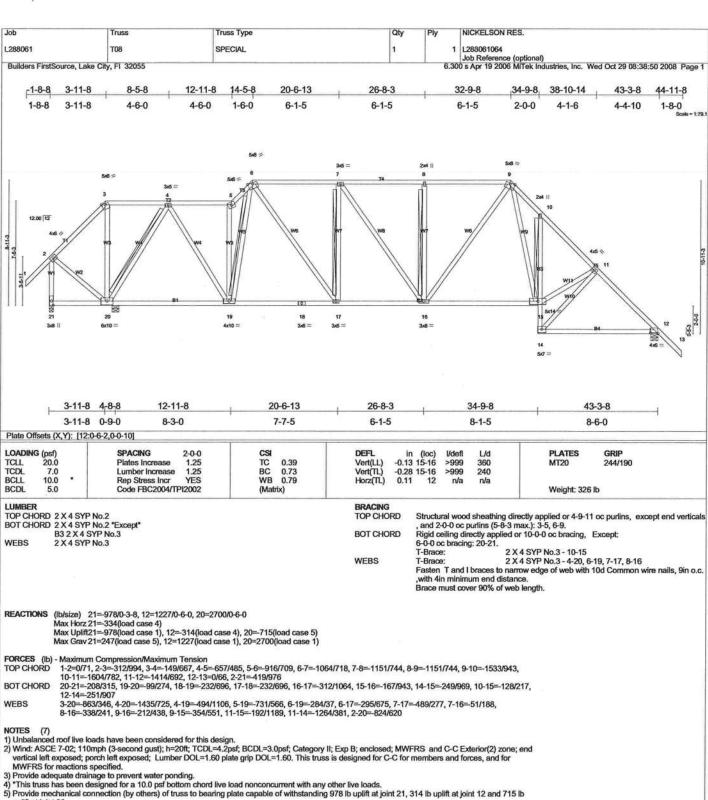
LOAD CASE(S) Standard

Uniform Loads (pif)

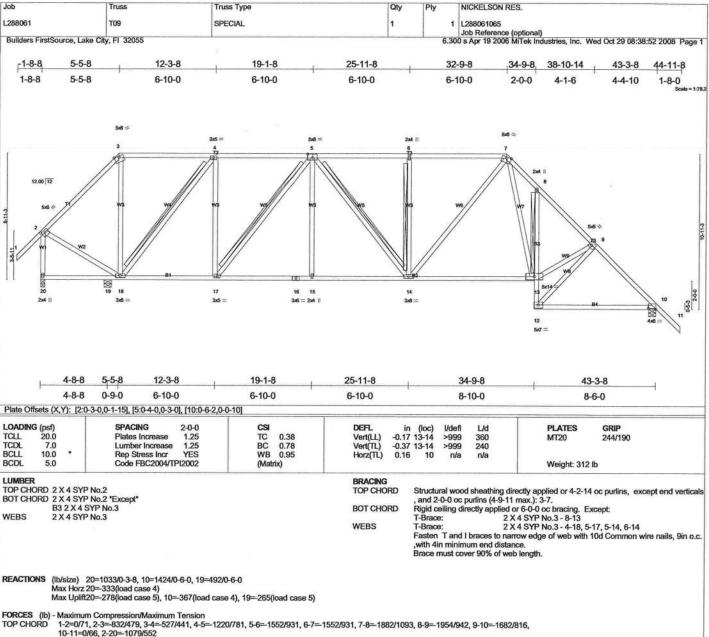
Vert: 1-2=54, 2-3=54, 3-4=57(F=3), 4-5=54, 5-6=54, 6-9=54, 9-13=54, 23-24=10, 22-24=11(F=1), 16-22=10, 12-15=10

Concentrated Loads (lb)

Vert: 24=21(F) 25=465(F)



besign assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.
 Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435



BOT CHORD

19-20-209/333, 18-19-209/333, 17-18-423/1220, 16-17-491/1550, 15-16-491/1550, 14-15-491/1550, 13-14-249/1180, 12-13-333/1161, 8-13-115/205, 10-12-335/1089 3-18-102/262, 4-18-1155/555, 4-17-179/527, 5-17-543/263, 5-15-0/182, 5-14-64/94, 6-14-380/270, 7-14-306/664, 7-13-362/593, 9-13-315/1469, 9-12-1525/496, 2-18-182/635 WEBS

NOTES (7)

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical left exposed; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for WWTRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

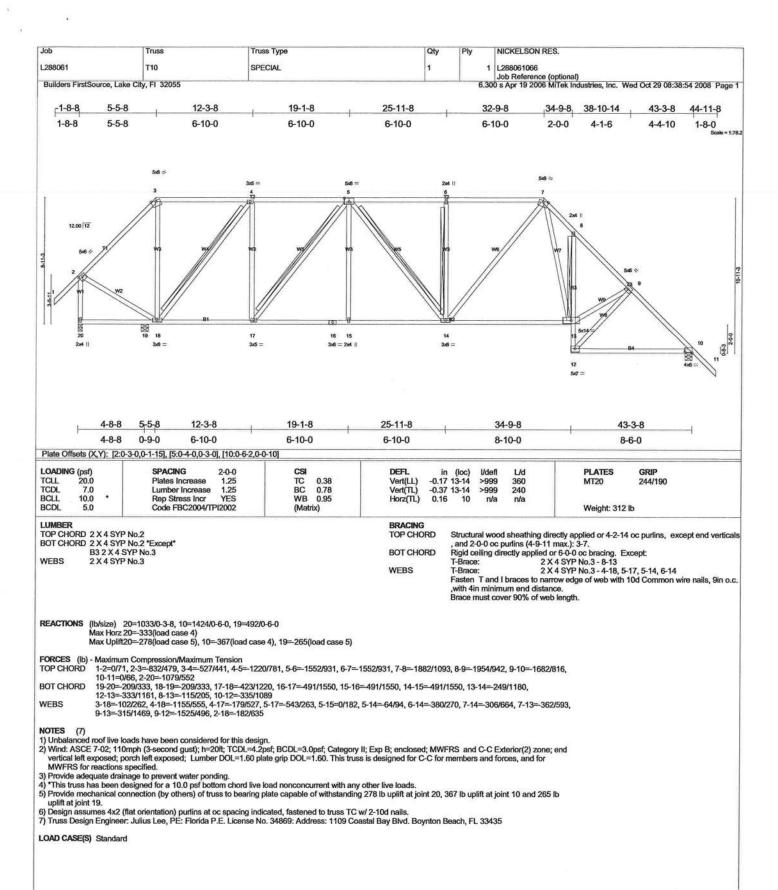
4) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

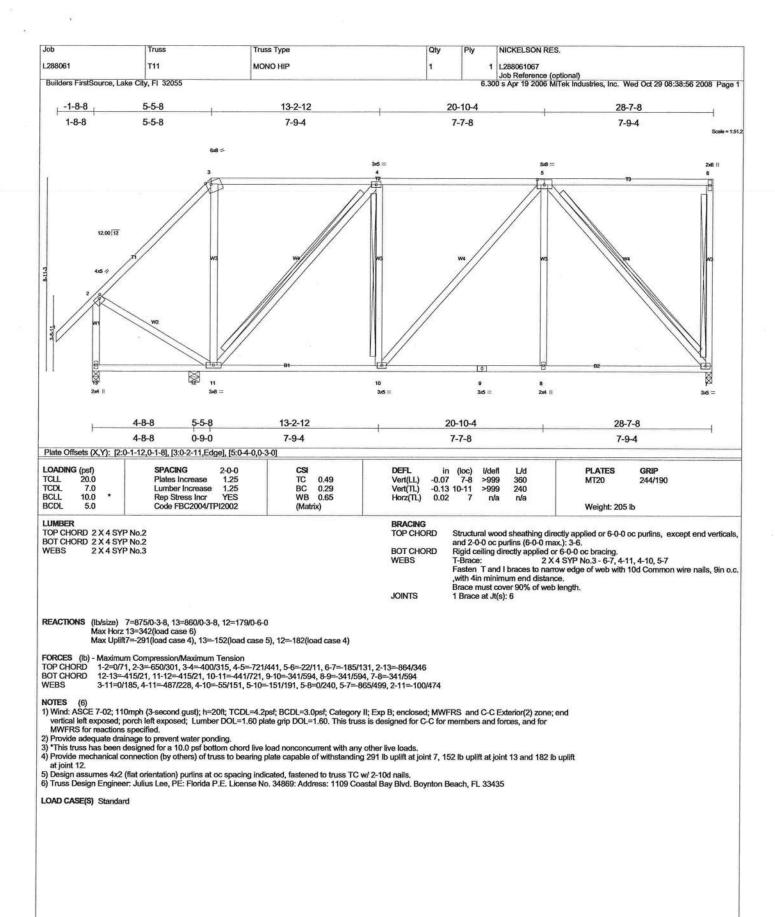
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 278 lb uplift at joint 20, 367 lb uplift at joint 10 and 265 lb

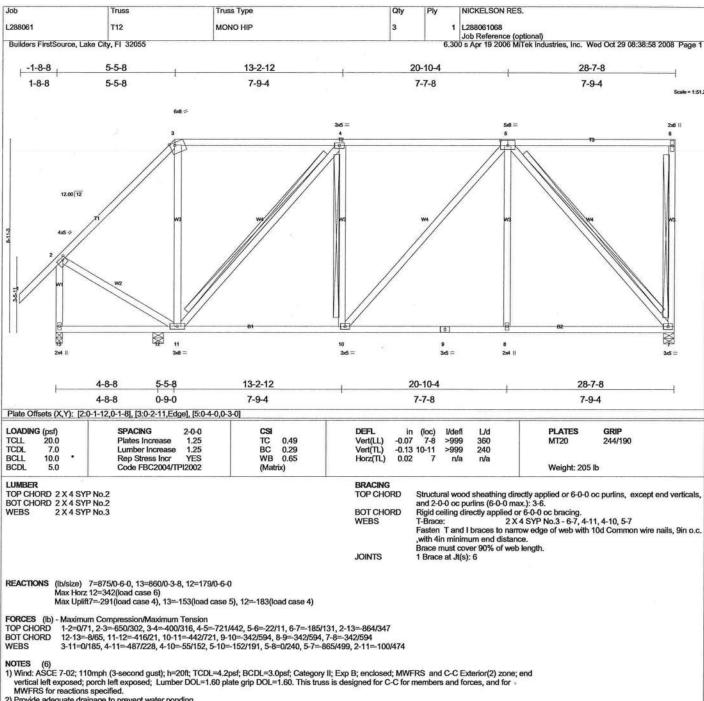
opinit as Joint 15.

6) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435







- Provide adequate drainage to prevent water ponding.
   \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
   Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 291 lb uplift at joint 7, 153 lb uplift at joint 13 and 183 lb uplift
- Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.
   Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

NICKELSON RES. Job Truss Truss Type Qty L288061 T13 SPECIAL 1 1 L288061069 Job Reference (optional) 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Oct 29 08:38:59 2008 Page 1 Builders FirstSource, Lake City, FI 32055 5-4-12 10-6-0 14-9-0 19-0-0 24-2-0 5-4-12 5-1-4 4-3-0 3-8-7 1-5-9 4-3-0 Scale = 1:61.5 12.00 12 8x12 = 5-4-12 10-6-0 19-0-0 22-8-7 5-4-12 5-1-4 8-6-0 3-8-7 Plate Offsets (X,Y): [6:0-7-0,Edge], [12:0-0-0,0-2-8] LOADING (psf) SPACING DEFL PLATES TCLL 20.0 Plates Increase 1.25 1.25 TC BC 0.87 Vert(LL) -0.19 6-8 >999 360 240 MT20 244/190 0.92 Vert(TL) >817 7.0 Lumber Increase -0.356-8 Rep Stress Incr YES Code FBC2004/TPI2002 BCLL WB 0.81 0.55 BCDL (Matrix) Weight: 198 lb 5.0 LUMBER BRACING Structural wood sheathing directly applied or 2-2-0 oc purlins, except end verticals, and 2-0-0 oc purlins (5-4-7 max.): 1-5. Rigid ceiling directly applied or 9-11-7 oc bracing. TOP CHORD 2 X 4 SYP No.2 \*Except\* T2 2 X 6 SYP No.1D BOT CHORD 2 X 4 SYP No.2 \*Except\* BOT CHORD B2 2 X 4 SYP No.1D 2 X 4 SYP No.3 \*Except 1-12 2 X 4 SYP No.3 - 1-11, 2-11 **WEBS** 1 Row at midpt WEBS T-Brace: W1 2 X 4 SYP No.1D, W2 2 X 4 SYP No.1D, W1 2 X 4 SYP No.1D Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c. with 4in minimum end distance. Brace must cover 90% of web length. **JOINTS** 1 Brace at Jt(s): 1 REACTIONS (lb/size) 12=764/0-6-0, 7=771/0-3-8 Max Horz 12=-166(load case 7) Max Uplift12=-257(load case 5), 7=-114(load case 4) FORCES (lb) - Maximum Compression/Maximum Tension 1-12=735495, 1-2-256/170, 2-3-1326/431, 3-4=1344/423, 4-5=1015/390, 5-6=1234/374, 6-7=510/206 11-12=6/223, 10-11=2/4, 9-10=0/61, 3-9=253/180, 8-9=401/1276, 6-8=230/993 1-11=438/657, 2-11=1038/403, 9-11=182/539, 2-9=421/1299, 4-9=31/149, 4-8=344/216, 5-8=111/430 TOP CHORD BOT CHORD WEBS NOTES (7)

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) Provide adequate drainage to prevent water ponding.

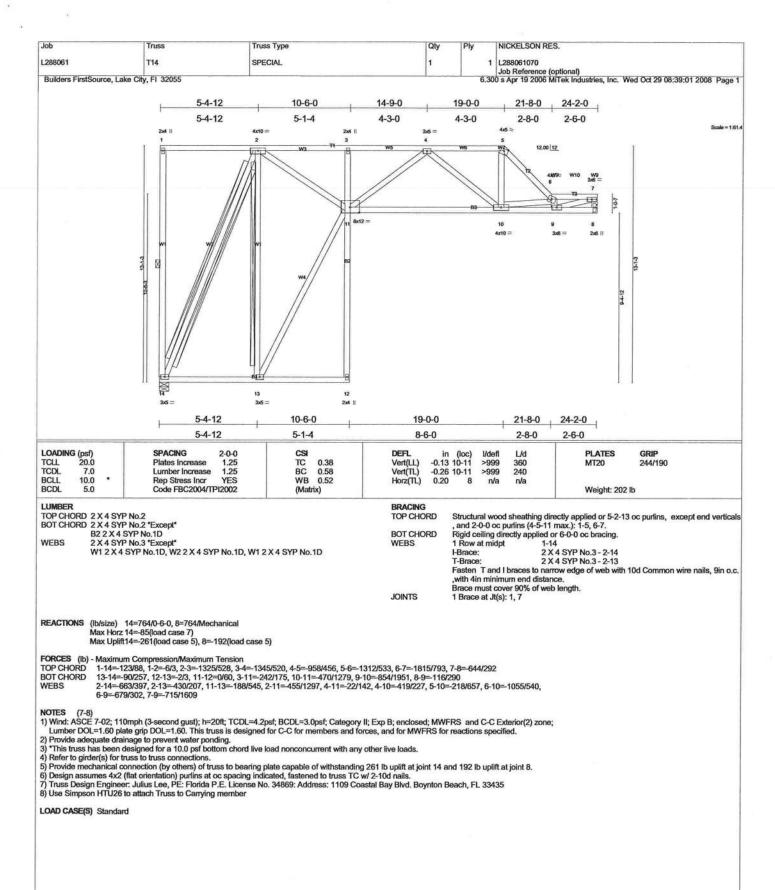
3) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

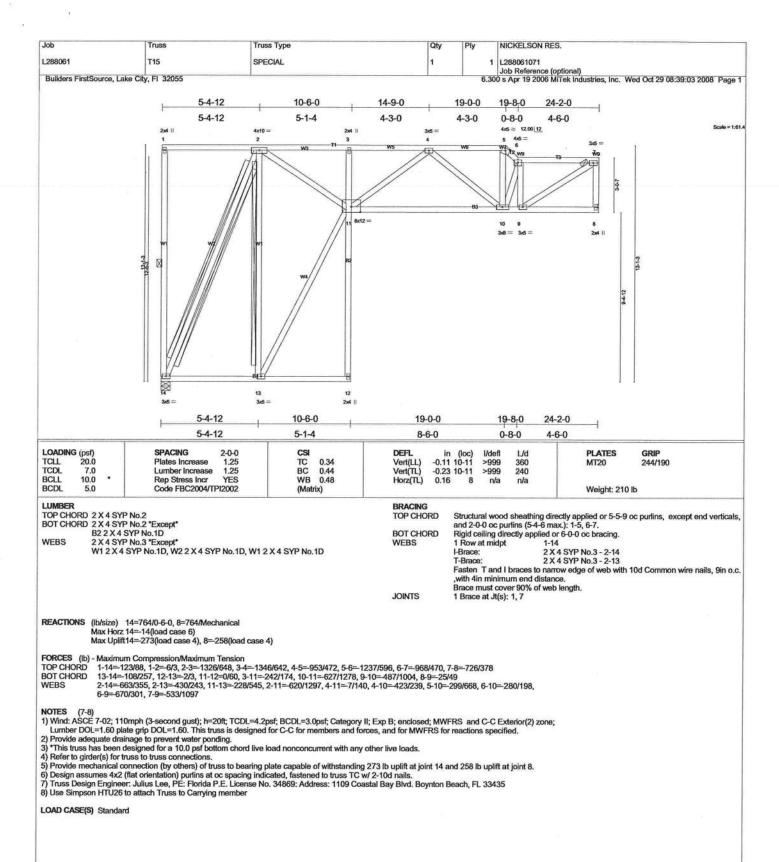
4) Bearing at joint(s) 7 considers parallel to grain value using ANS/ITPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

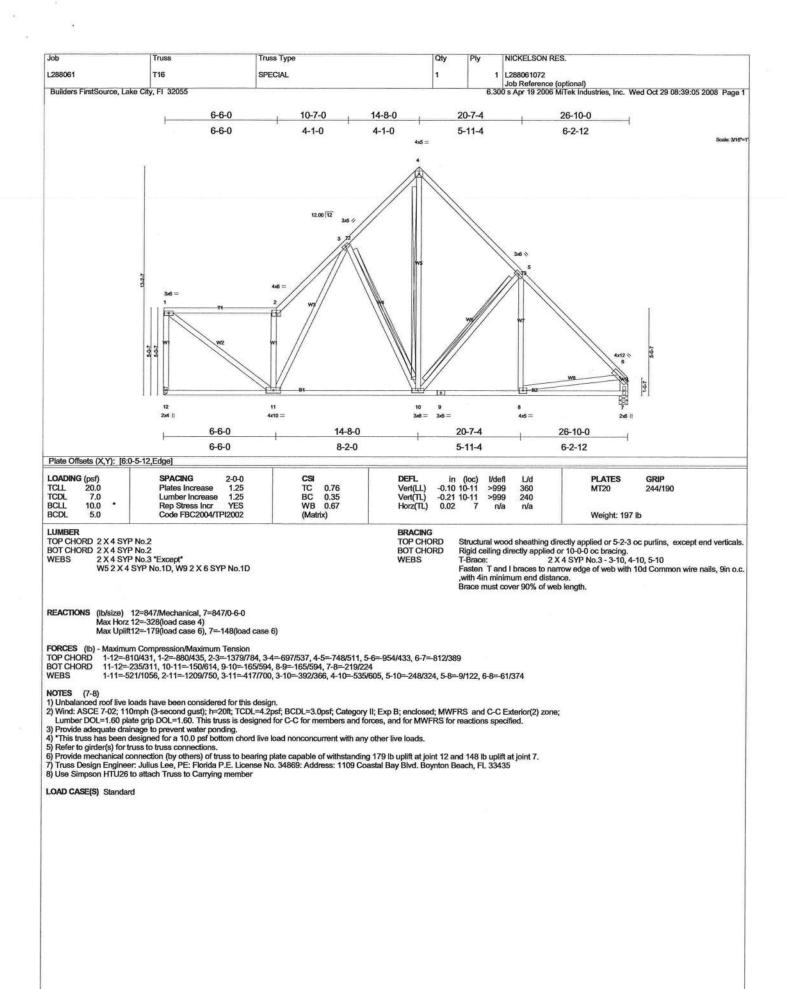
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 257 ib uplift at joint 12 and 114 ib uplift at joint 7.

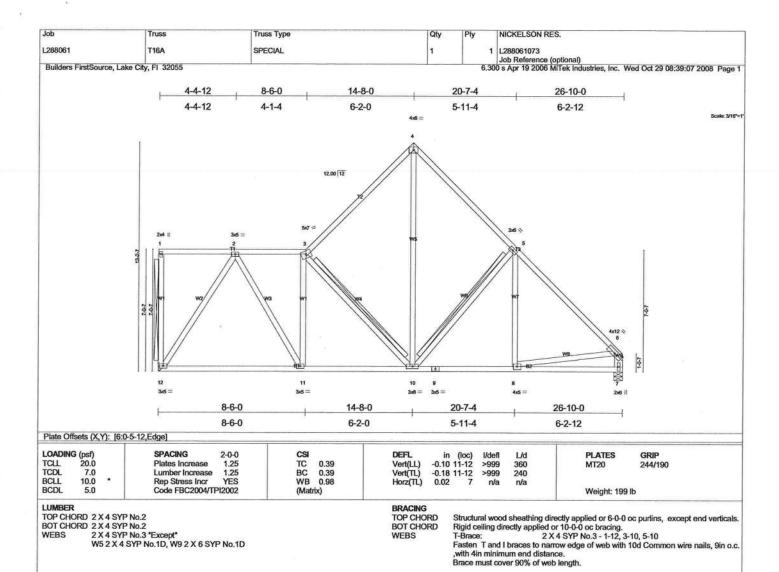
6) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

7) Truss Design Engineer. Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435 LOAD CASE(S) Standard









REACTIONS (lb/size) 12=847/Mechanical, 7=847/0-6-0 Max Horz 12=325(load case 4) Max Uplift12=227(load case 4), 7=135(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

1-12=110/80, 1-2=-22/2, 2-3=-738/401, 3-4=-743/475, 4-5=-743/487, 5-6=-955/411, 6-7=-814/371 11-12=-170/453, 10-11=-154/746, 9-10=-147/593, 8-9=-147/593, 7-8=-229/226 2-12=-824/460, 2-11=-304/561, 3-11=-408/313, 3-10=-441/310, 4-10=-432/553, 5-10=-252/311, 5-8=0/141, 6-8=-38/371 TOP CHORD

BOT CHORD

WEBS

NOTES (7)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

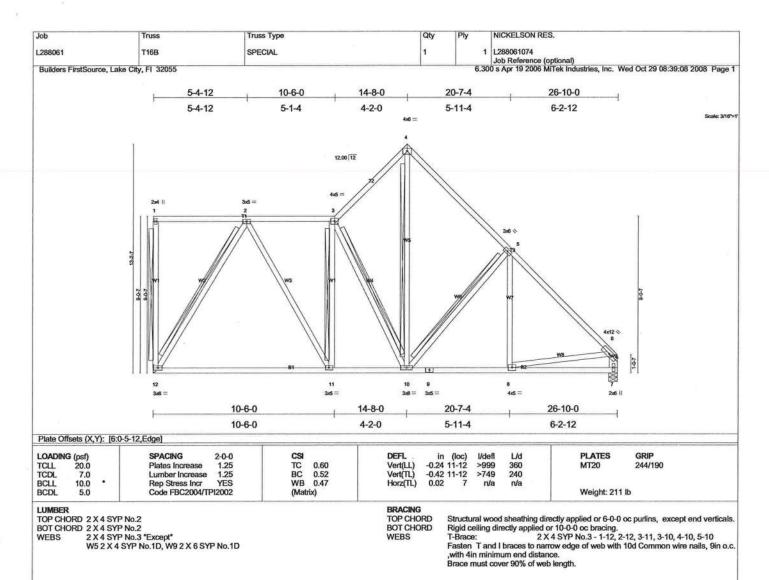
3) Provide adequate drainage to prevent water ponding.

4) \*This truss has been designed for a 10.0 pef bottom chord live load nonconcurrent with any other live loads.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 227 lb uplift at joint 12 and 135 lb uplift at joint 7.

7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435



REACTIONS (lb/size) 12=847/Mechanical, 7=847/0-6-0 Max Horz 12=323(load case 4) Max Uplift12=277(load case 4), 7=-121(load case 7)

FORCES (ib) - Maximum Compression/Maximum Tension
TOP CHORD
TOP CHORD
1-12=127/86, 1-2=-25/0, 2-3=623/363, 3-4=-691/482, 4-5=-743/461, 5-6=-956/383, 6-7=-815/348
BOT CHORD
1-12=127/417, 10-11=-86/627, 9-10=-130/596, 8-9=-130/596, 7-8=-220/217
WEBS
2-12=-782/490, 2-11=-226/416, 3-11=-275/254, 3-10=-432/252, 4-10=-456/598, 5-10=-255/324, 5-8=0/153, 6-8=-31/383

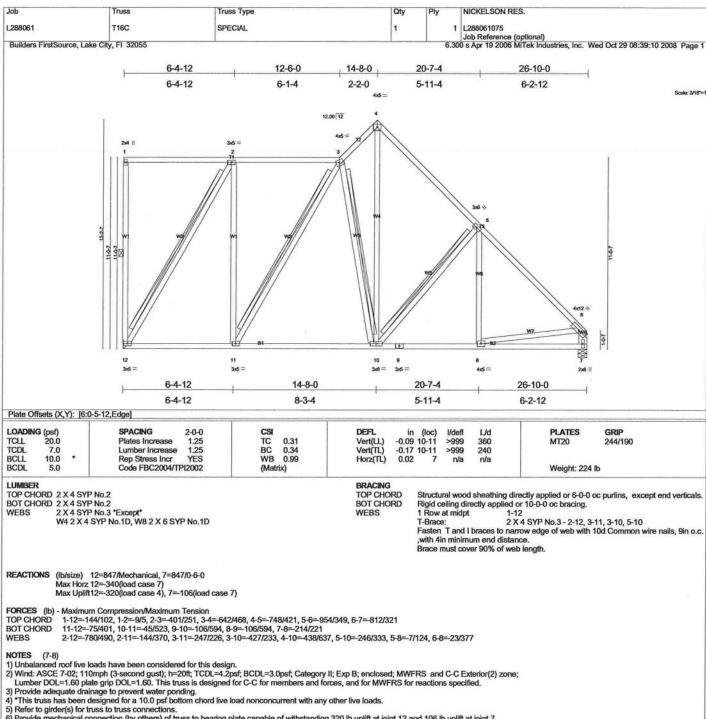
NOTES (7-8)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Provide adequate drainage to prevent water ponding.
 This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

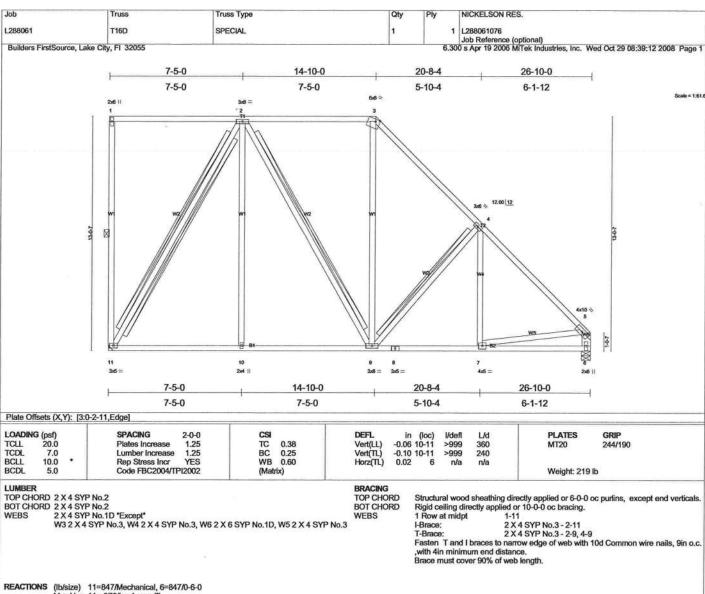
4) \*This truss has been designed for a 10.0 psi bottom chord live load horizonculrent with any other live loads.
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 277 lb uplift at joint 12 and 121 lb uplift at joint 7.
7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
8) Use Simpson HTU26 to attach Truss to Carrying member



6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 320 lb uplift at joint 12 and 106 lb uplift at joint 7.

7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

8) Use Simpson HTU26 to attach Truss to Carrying member



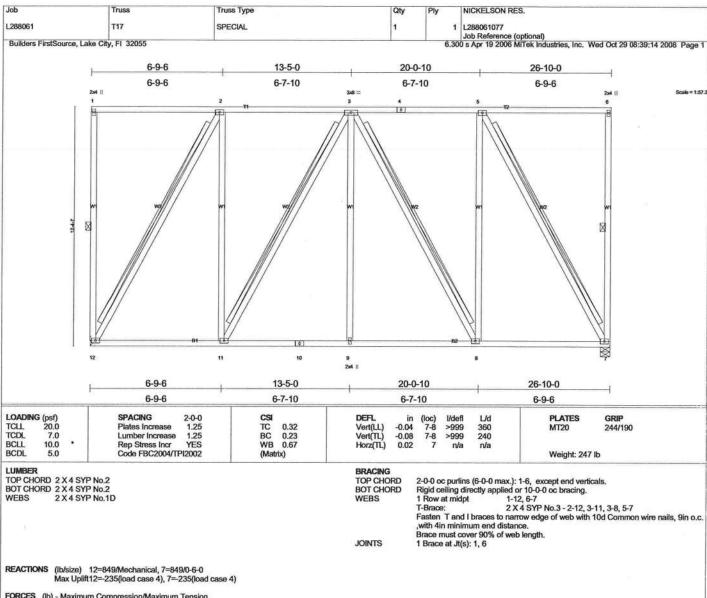
Max Horz 11=379(load case 7) Max Uplift11=278(load case 4), 6=88(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-11-170/123, 1-2-12/6, 2-3-453/376, 3-4-746/373, 4-5-954/306, 5-6-813/286
BOT CHORD 10-11-93/376, 9-10-93/376, 8-9-78/593, 7-8-78/593, 6-7-210/223
WEBS 2-11-735/517, 2-10-0/236, 2-9-231/153, 3-9-7/216, 4-9-215/327, 4-7-5/131, 5-7-34/374

NOTES (6-7)

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) Provide adequate drainage to prevent water ponding.
3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 278 lb uplift at joint 11 and 88 lb uplift at joint 6.
6) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
7) Use Simpson HTU26 to attach Truss to Carrying member



FORCES (lb) - Maximum Compression/Maximum Tension

BOT CHORD

1-12=157/112, 1-2=-11/5, 2-3=-372/191, 3-4=-372/191, 4-5=-372/191, 5-6=-11/5, 6-7=-157/112 11-12=-191/372, 10-11=-248/484, 9-10=-248/484, 8-9=-248/484, 7-8=-191/372 2-12=-750/385, 2-11=-57/354, 3-11=-234/120, 3-9=0/192, 3-8=-234/120, 5-8=-57/354, 5-7=-750/385

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) Provide adequate drainage to prevent water ponding.

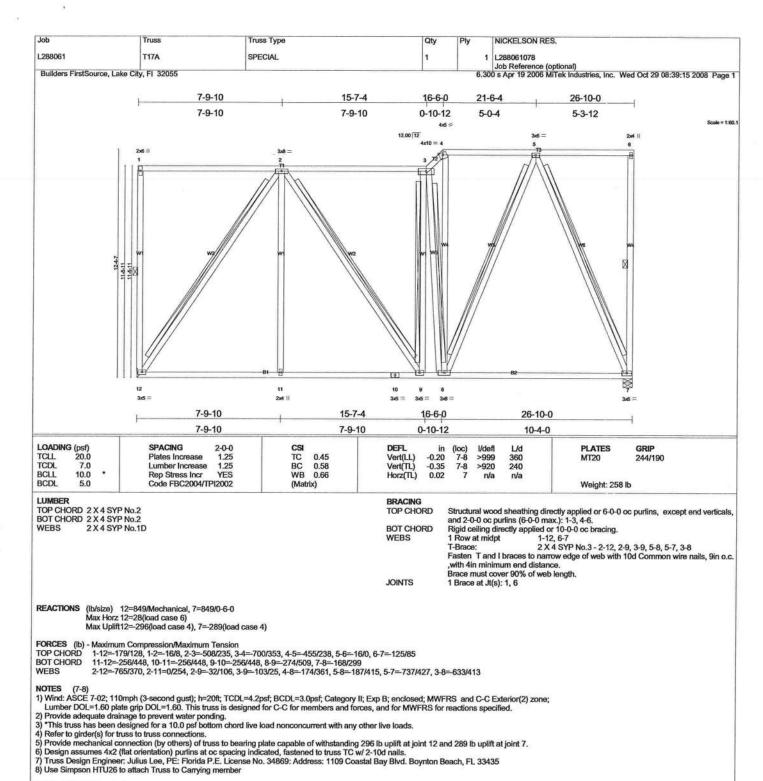
3) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

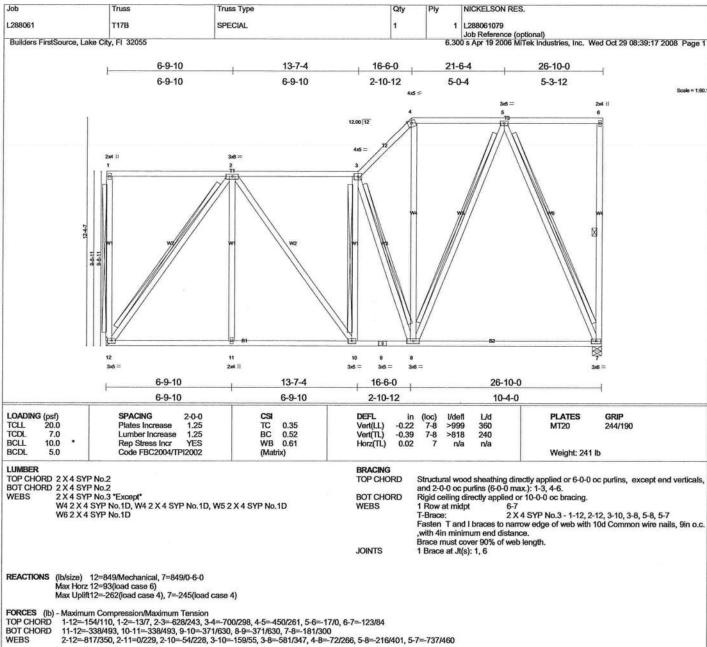
4) All plates are 3x5 MT20 unless otherwise indicated.

5) Pafer to girther(4) for trues to true second-girther and trues to true to the second-girther and trues to the second-girther and true to the second-girther and trues to t

5) Refer to girder(s) for truss to truss connections.

For to girder(s) for truss to truss connections.
 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 235 lb uplift at joint 12 and 235 lb uplift at joint 7.
 Design assumes 4x2 (flat orientation) purins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.
 Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
 Use Simpson HTU26 to attach Truss to Carrying member





WEBS

NOTES (7-8)

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) Provide adequate drainage to prevent water ponding.

3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

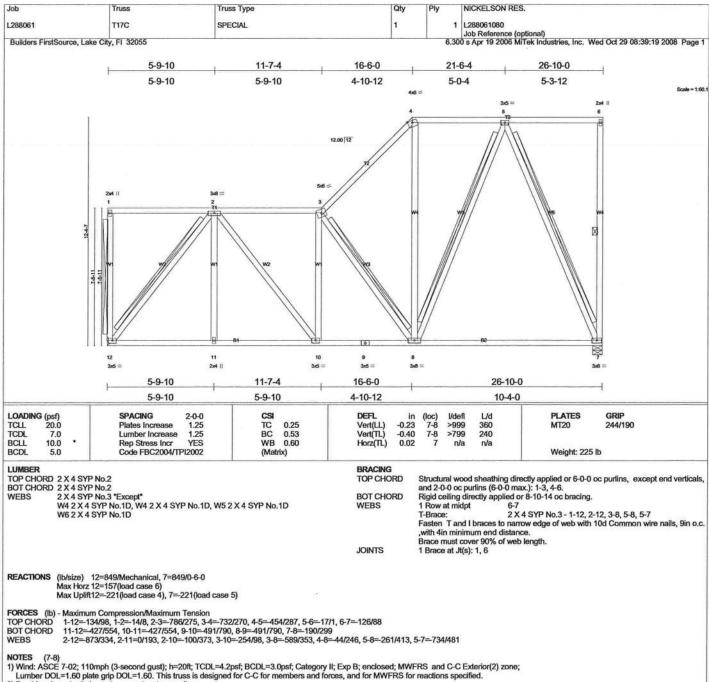
4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 262 lb uplift at joint 12 and 245 lb uplift at joint 7.

6) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

8) Use Simpson HTU26 to attach Truss to Carrying member



Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) Provide adequate drainage to prevent water ponding.

3) 'This truss has been designed for a 10.0 pef bottom chord live load nonconcurrent with any other live loads.

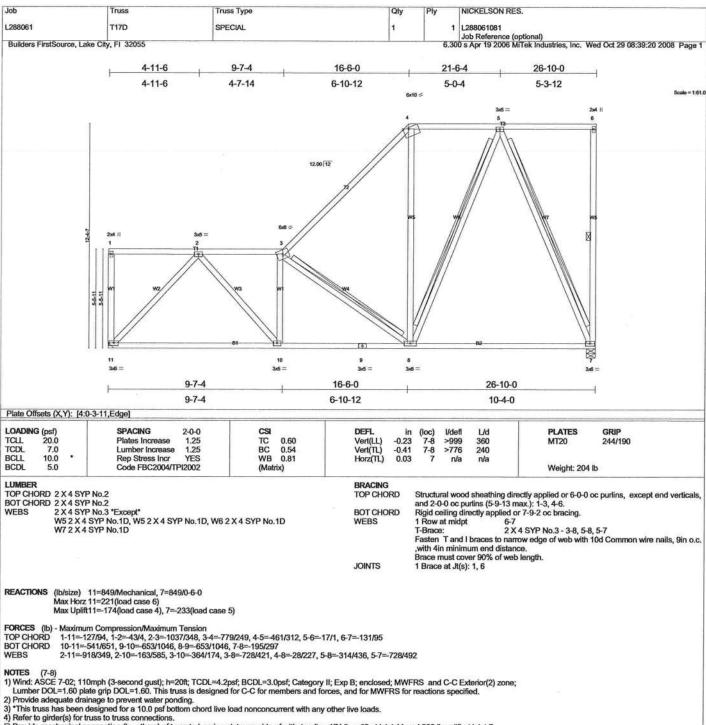
4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 221 lb uplift at joint 12 and 221 lb uplift at joint 7.

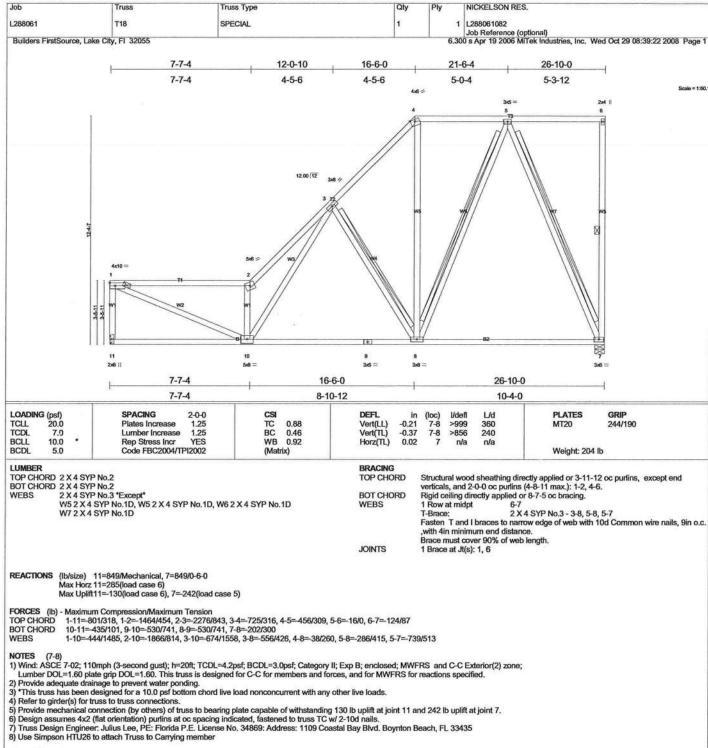
6) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

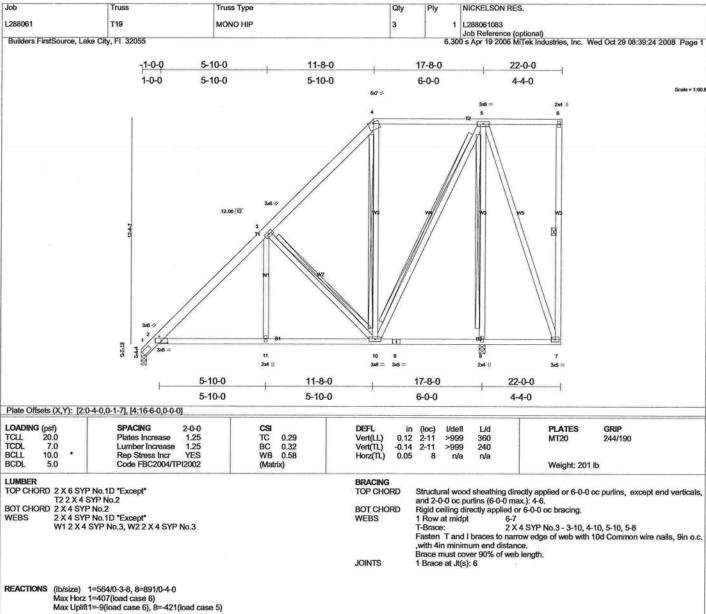
7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

8) Use Simpson HTU26 to attach Truss to Carrying member



4) Refer to girder(s) for truss to truss connections.
 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 174 lb uplift at joint 11 and 233 lb uplift at joint 7.
 6) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.
 7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
 8) Use Simpson HTU26 to attach Truss to Carrying member





FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

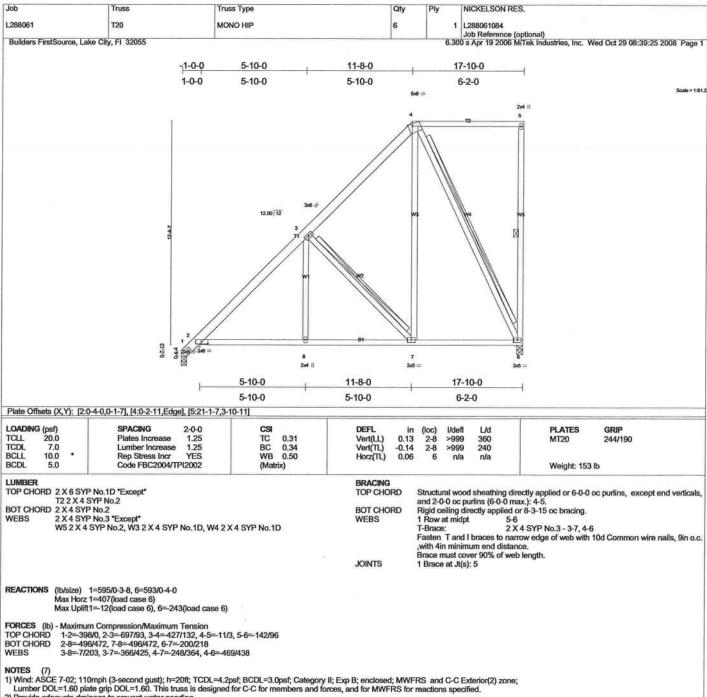
BOT CHORD

1-2=422/0, 2-3=651/31, 3-4=382/70, 4-5=180/147, 5-6=1/4, 6-7=80/62 2-11=452/439, 10-11=452/440, 9-10=-31/51, 8-9=31/51, 7-8=31/51 3-11=7/206, 3-10=371/427, 4-10=121/171, 5-10=457/477, 5-8=827/777, 5-7=165/97 **WEBS** 

NOTES (7)

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions

specified.
2) Provide adequate drainage to prevent water ponding.
3) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) Bearing at joint(s) 1 considers parallel to grain value using ANS/ITPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 9 lb upfit at joint 1 and 421 lb upfit at joint 8.
6) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.
7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435



Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) Provide adequate drainage to prevent water ponding.

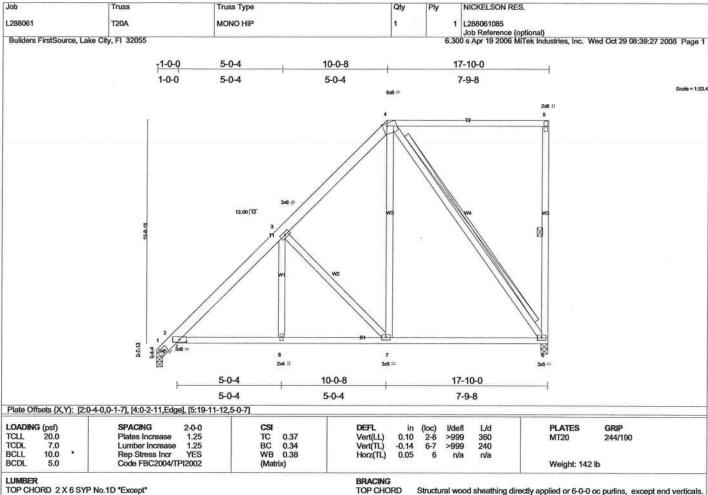
3) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) Bearing at joint(s) 1 considers parallel to grain value using ANS/ITPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 12 lb uplift at joint 1 and 243 lb uplift at joint 6.

6) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435



T2 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEBS

2 X 4 SYP No.1D \*Except\* W1 2 X 4 SYP No.3, W2 2 X 4 SYP No.3, W4 2 X 4 SYP No.2

TOP CHORD BOT CHORD WEBS

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 8-4-5 oc bracing.

1 Row at midpt

5-6

T-Brace: 2 X 4 SYP No.3 - 4-6
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.

with 4in minimum end distance. Brace must cover 90% of web length.

REACTIONS (lb/size) 1=595/0-3-8, 6=593/0-4-0 Max Horz 1=354(load case 6) Max Uplift1=-39(load case 6), 6=-208(load case 5)

FORCES (Ib) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=347/0, 2-3=722/176, 3-4=485/201, 4-5=21/8, 5-6=186/130
BOT CHORD 2-8=501/497, 7-8=501/497, 6-7=246/287
WEBS 3-8=19/163, 3-7=303/366, 4-7=200/358, 4-6=452/399

NOTES (6)

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

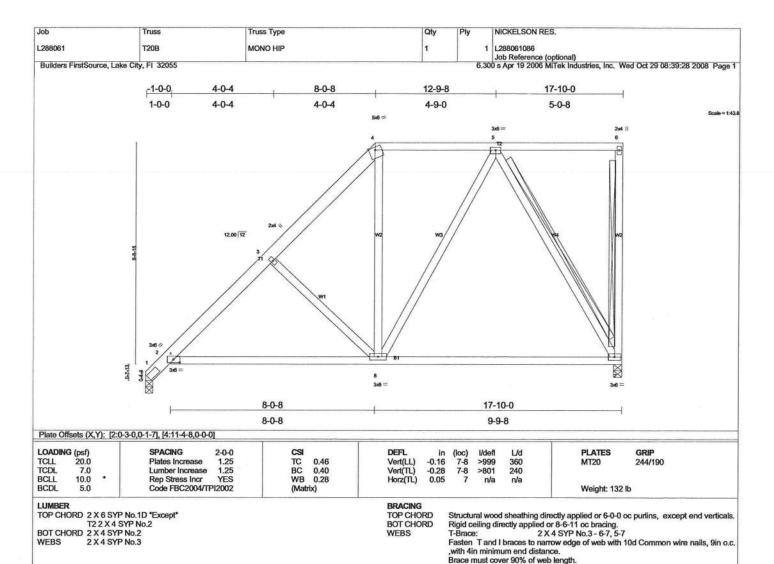
2) Provide adequate drainage to prevent water ponding.

3) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 39 lb uplift at joint 1 and 208 lb uplift at joint 6.

6) Truss Design Engineer. Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435



REACTIONS (lb/size) 1=595/0-3-8, 7=593/0-4-0

Max Horz 1=290(load case 6) Max Uplift1=63(load case 6), 7=194(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=347/0, 2-3=-743/261, 3-4=-590/284, 4-5=-349/270, 5-6=-20/0, 6-7=-118/80
BOT CHORD 2-8=-515/539, 7-8=-197/262
WEBS 3-8=-267/335, 4-8=-44/212, 5-8=-157/206, 5-7=-500/392

NOTES (6)

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

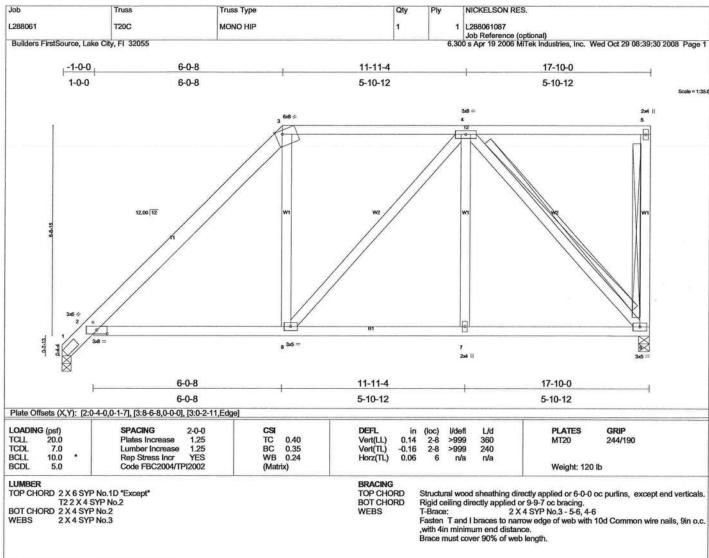
2) Provide adequate drainage to prevent water ponding.

3) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 63 lb uplift at joint 1 and 194 lb uplift at joint 7.

6) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435



REACTIONS (lb/size) 1=595/0-3-8, 6=593/0-4-0 Max Horz 1=226(load case 6) Max Uplift1=78(load case 5), 6=-182(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=347/10, 2-3=680/263, 3-4=453/339, 4-5=18/10, 5-6=143/104 BOT CHORD 2-8=342/449, 7-8=245/389, 6-7=245/389

3-8-7/159, 4-8-143/91, 4-7=0/171, 4-6-558/352

NOTES (6)

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.6.0. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

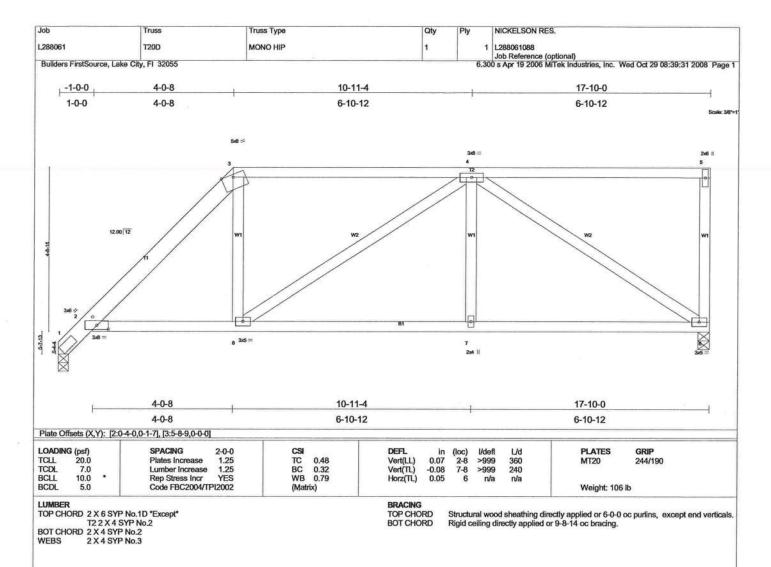
2) Provide adequate drainage to prevent water ponding.

3) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 78 lb uplift at joint 1 and 182 lb uplift at joint 6.

6) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435



REACTIONS (lb/size) 1=595/0-3-8, 6=593/0-4-0

Max Horz 1=162(load case 6) Max Uplift1=106(load case 5), 6=191(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=347/39, 2-3=785/352, 3-4=559/377, 4-5=41/22, 5-6=168/120
BOT CHORD 2-8=378/551, 7-8=362/627, 6-7=362/627
WEBS 3-8=14/234, 4-8=85/90, 4-7=0/212, 4-6=702/407

Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

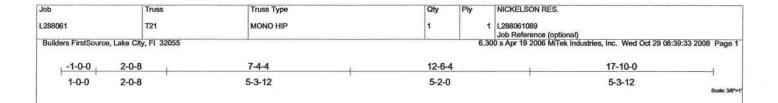
2) Provide adequate drainage to prevent water ponding.

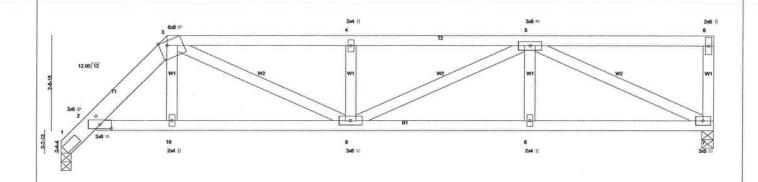
3) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 106 lb uplift at joint 1 and 191 lb uplift at joint 6.

6) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435





L	2-0-8	5-3-1	2		5-2-0				5-3-1	12
Plate Offsets (X,Y):	2:0-4-0,0-1-7], [3:2-10-10,0-	0-0], [3:0-2-11,E	dge]							
TCLL 20.0 TCDL 7.0 BCLL 10.0	SPACING Plates Increase Lumber Increase Rep Stress Incr	2-0-0 1.25 1.25 NO	CSI TC 0.41 BC 0.41 WB 0.66	DEFL Vert(LL) Vert(TL) Horz(TL)	in -0.06 -0.12 0.06	(loc) 8-9 8-9 7	l/defl >999 >999 n/a	L/d 360 240 n/a	PLATES MT20	GRIP 244/190
BCDL 5.0	Code FBC2004/		(Matrix)	·ioiz(iz)	2.00				Weight: 97 II	b

### LUMBER

TOP CHORD 2 X 6 SYP No.1D \*Except\* T2 2 X 4 SYP No.2

2-0-8

BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3

### BRACING

12-6-4

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 5-3-8 oc purlins, except end verticals. Rigid ceiling directly applied or 9-0-11 oc bracing.

17-10-0

### REACTIONS (lb/size) 1=670/0-3-8, 7=696/0-4-0 Max Horz 1=99(load case 5)

Max Uplift1=203(load case 4), 7=306(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-391/115, 2-3=-1028/365, 3-4=-1350/569, 4-5=-1346/571, 5-6=-66/29, 6-7=-159/101
BOT CHORD 2-10=-3180/787, 9-10=-3180/785, 8-9=-486/1116, 7-8=-486/1116
WEBS 3-10=0/188, 3-9=-304/604, 4-9=-299/203, 5-9=-102/255, 5-8=0/165, 5-7=-1163/505

NOTES (8)

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.

DOL=1.60.

2) Provide adequate drainage to prevent water ponding.

3) 'This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 203 lb uplift at joint 1 and 306 lb uplift at joint 7.

6) Girder carries hip end with 0-0-0 right side setback, 2-0-8 left side setback, and 3-0-0 end setback.

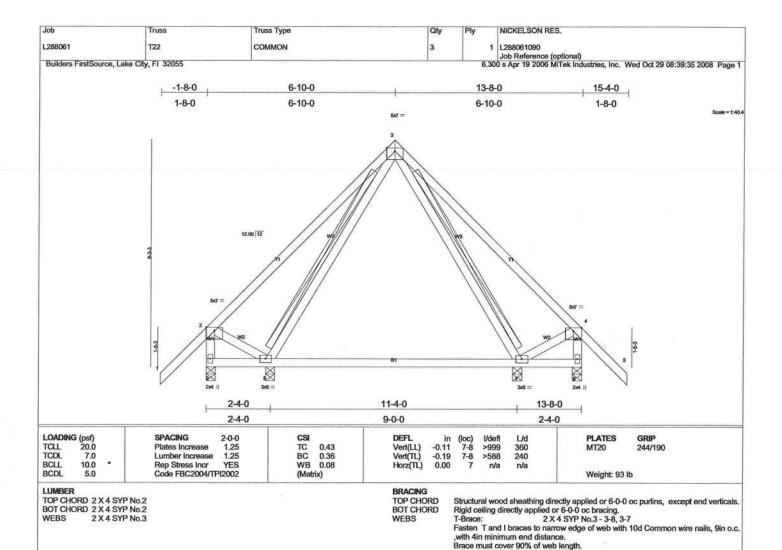
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

8) Truss Design Engineer. Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

7-4-4

### LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf) Vert: 1-2=66, 2-3=54, 3-6=64(F=10), 2-10=10, 7-10=12(F=2)



REACTIONS (lb/size) 9=296/0-4-0, 6=296/0-4-0, 8=230/0-4-0, 7=230/0-4-0
Max Horz 9=183(load case 5)
Max Uplift9=-119(load case 4), 6=-117(load case 7), 8=-106(load case 6), 7=-95(load case 7)
Max Grav 9=296(load case 1), 6=296(load case 1), 8=302(load case 2), 7=302(load case 2)

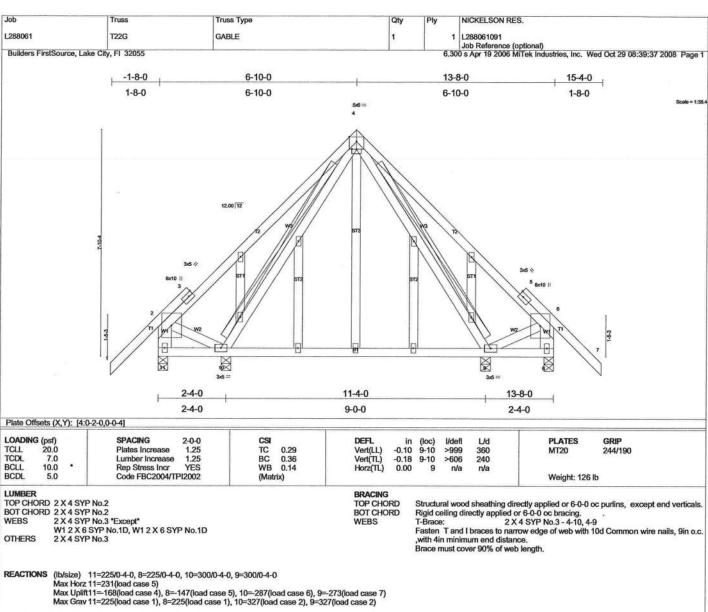
FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD BOT CHORD 1-2=0/69, 2-3=201/140, 3-4=-201/140, 4-5=0/69, 2-9=-313/148, 4-6=-313/148 8-9=-336/287, 7-8=-28/164, 6-7=-156/113

WEBS

3-8-151/46, 3-7-151/52, 2-8-134/329, 4-7-127/329

- 1) Unbalanced roof live loads have been considered for this design.
  2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions
- 3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 119 lb uplift at joint 9, 117 lb uplift at joint 6, 106 lb uplift at
- joint 8 and 95 lb uplift at joint 7.
  5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435



FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/68, 2-3=-157/62, 3-4=-133/99, 4-5=-133/77, 5-6=-157/42, 6-7=0/68, 2-11=-256/111, 6-8=-256/93 10-11=-470/361, 9-10=-46/216, 8-9=-246/177 TOP CHORD

BOT CHORD WEBS

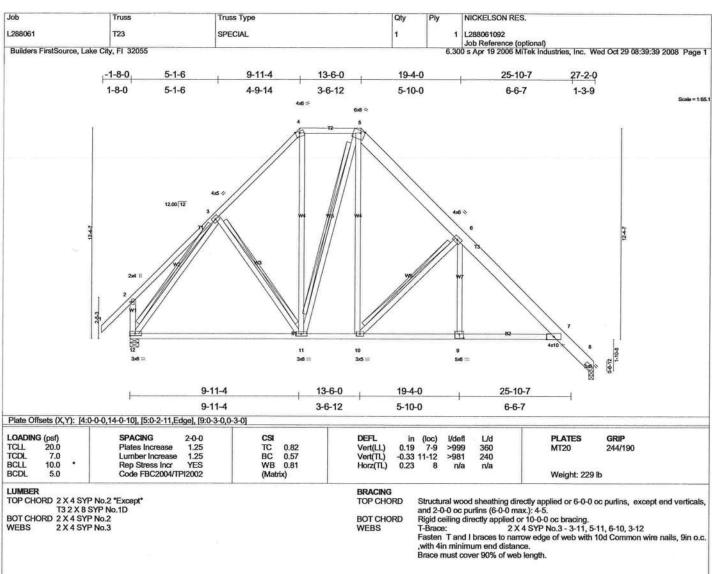
4-10=161/80, 4-9=161/80, 2-10=204/543, 6-9=199/538

### NOTES

- 1) Unbalanced roof live loads have been considered for this design.
  2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail" 4) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  5) All plates are 2x4 MT20 unless otherwise indicated.
  6) Gable studs spaced at 2-0-0 oc.

- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 168 lb uplift at joint 11, 147 lb uplift at joint 8, 287 lb uplift at joint 10 and 273 lb uplift at joint 9.

  8) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435



REACTIONS (lb/size) 8=863/0-3-8, 12=961/0-6-0 Max Horz 12=-377(load case 4) Max Uplift8=-184(load case 7), 12=-216(load case 6)

FORCES (Ib) - Maximum Compression/Maximum Tension
TOP CHORD
TOP CHORD
TOP CHORD
11-2=0/69, 2-3=197/333, 3-4=736/544, 4-5=458/476, 5-6=854/566, 6-7=1157/525, 7-8=554/281, 2-12=290/416
BOT CHORD
11-12=251/480, 10-11=99/482, 9-10=-235/893, 7-9=235/891
WEBS
3-11=100/258, 4-11=211/257, 5-11=185/176, 5-10=311/412, 6-10=581/485, 6-9=0/232, 3-12=752/140

### NOTES (8)

10) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

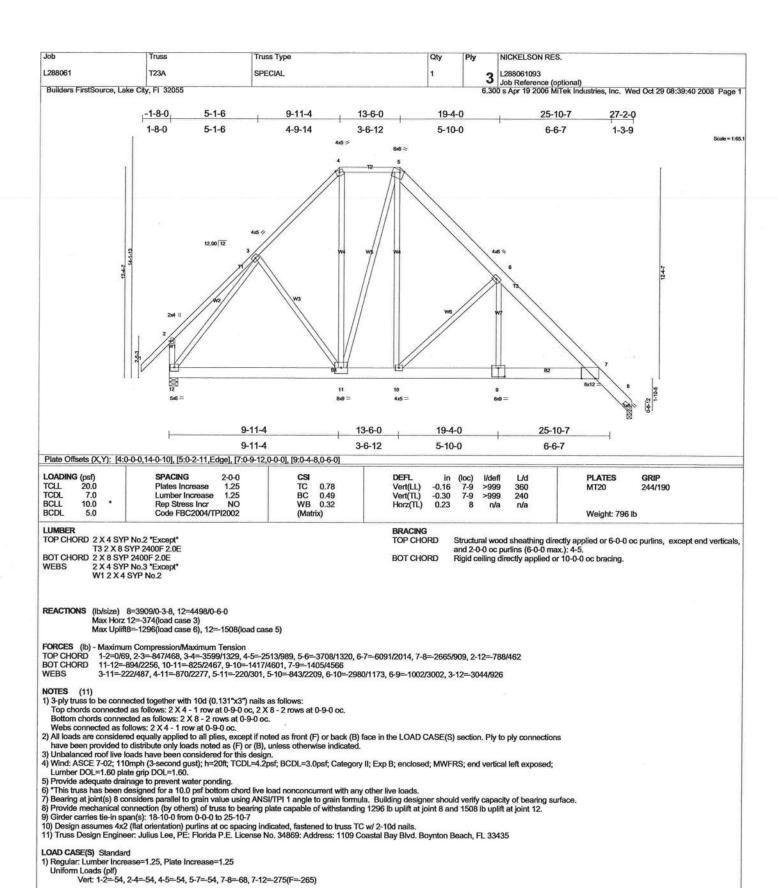
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

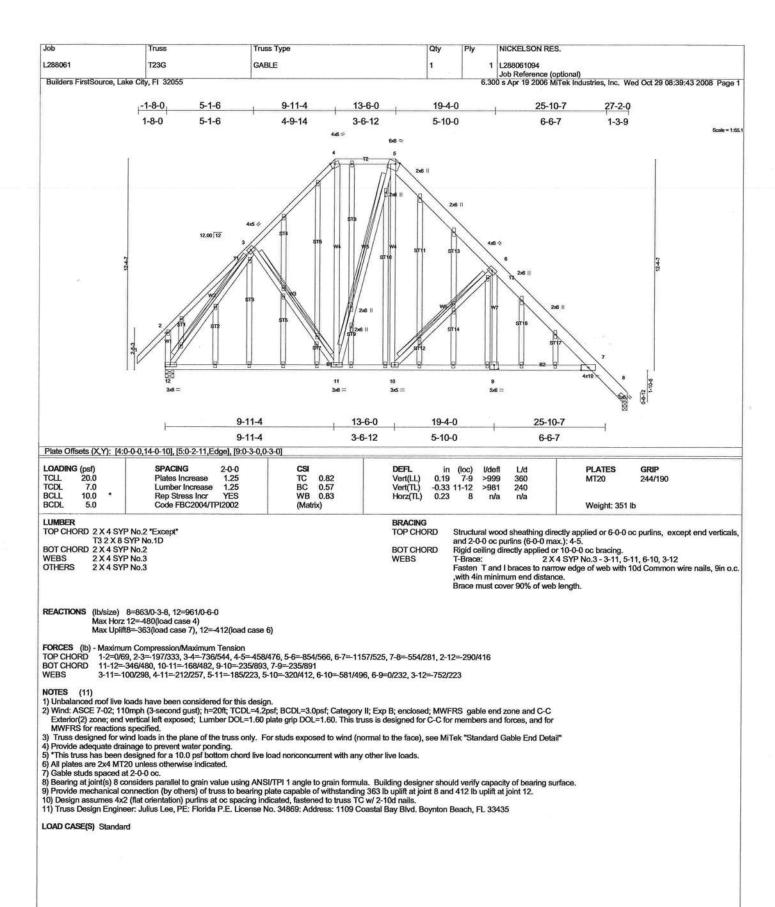
  5) Bearing at joint(s) 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

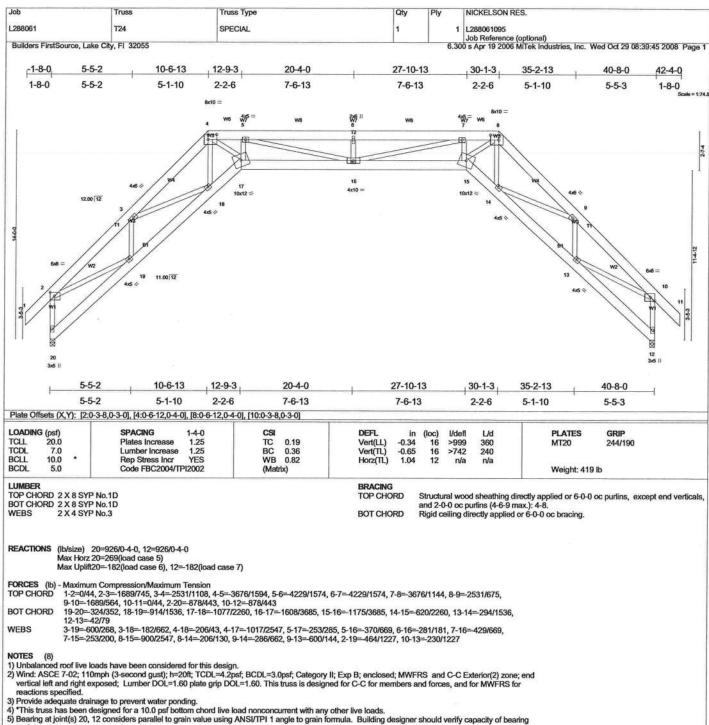
  6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 184 lb uplift at joint 8 and 216 lb uplift at joint 12.

  7) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-104 nails.

  8) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435





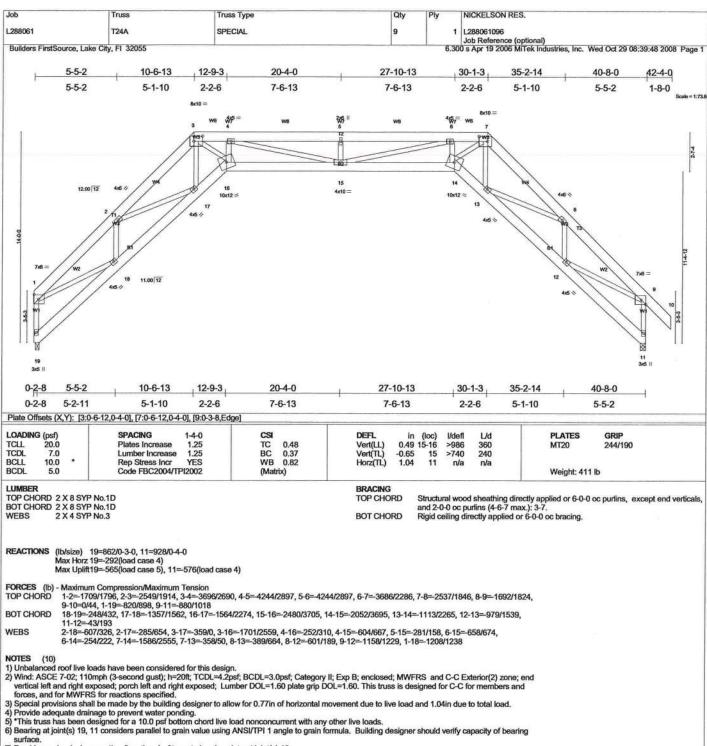


- 5) Bearing at joint(s) 20, 12 Chistoff parallel to grain value bearing 1415 and 182 is uplift at joint 20 and 182 is uplift at joint 12.

  6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 182 is uplift at joint 20 and 182 is uplift at joint 12.

  7) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

  8) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

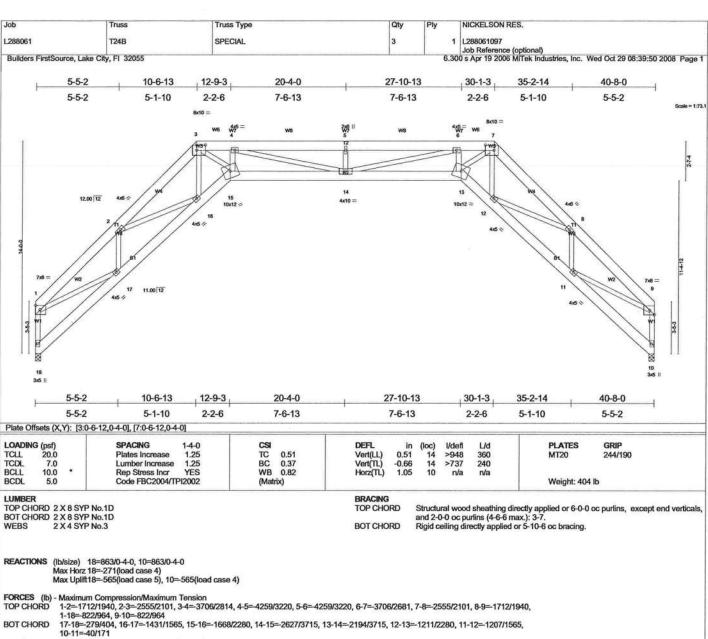


7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 19.

8) Provide mechanical connection (by others) of truss to bearing plate applied withstanding 565 lb uplift at joint 19 and 576 lb uplift at joint 11.

9) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435.



2-17=609/343, 2-16=313/656, 3-16=335/0, 3-15=1789/2567, 4-15=253/303, 4-14=606/673, 5-14=281/164, 6-14=665/673, 6-13=253/217, 7-13=1673/2567, 7-12=335/58, 8-12=418/656, 8-11=609/235, 1-17=1319/1240, 9-11=1319/1240

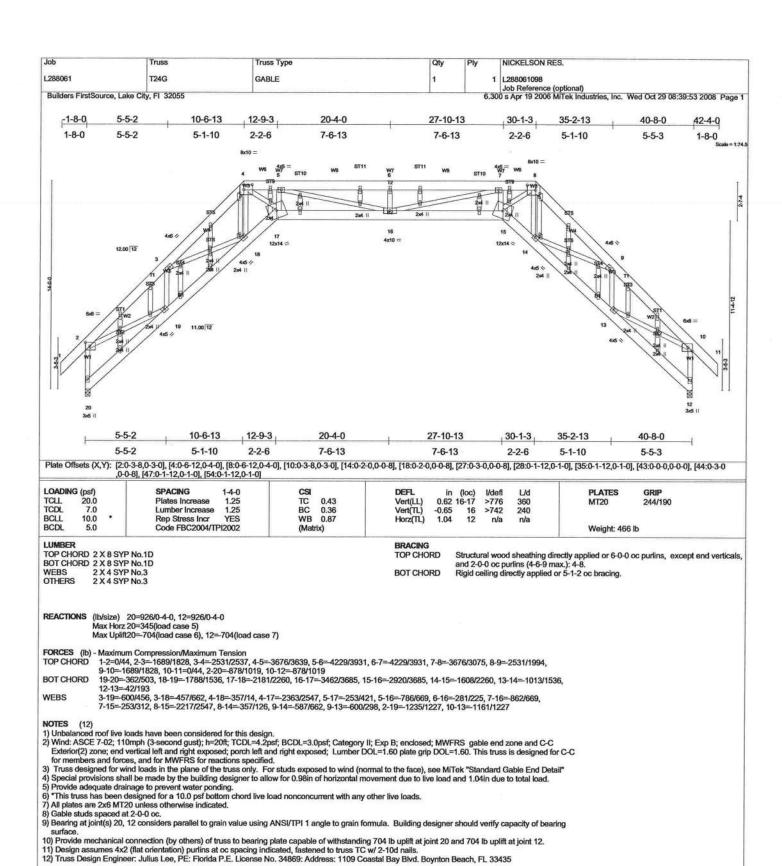
### NOTES (9)

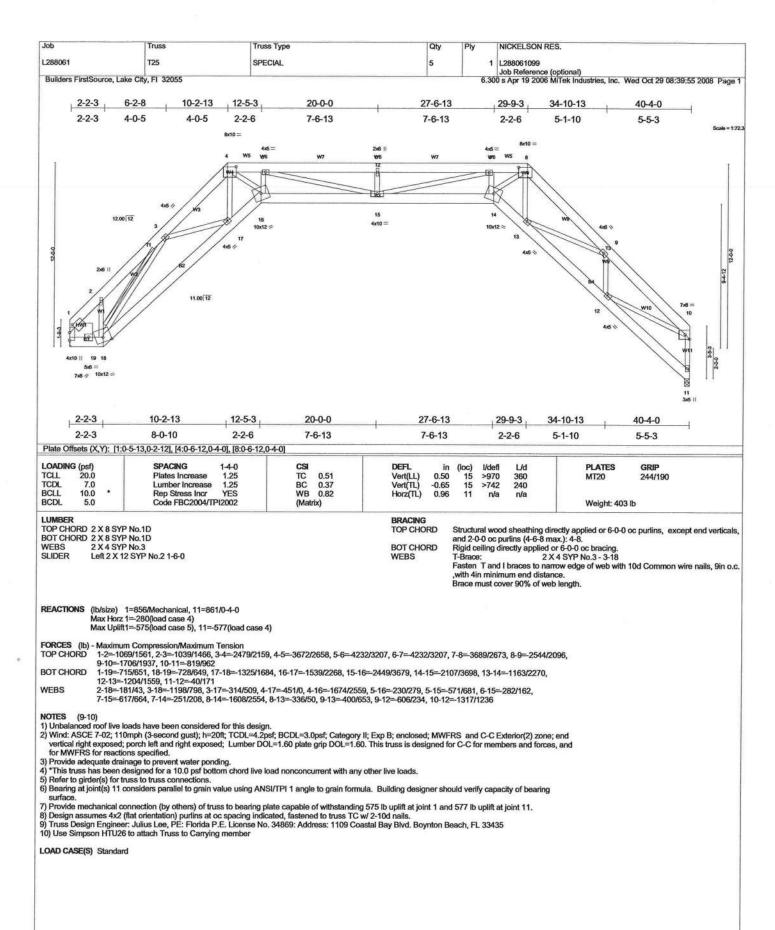
WEBS

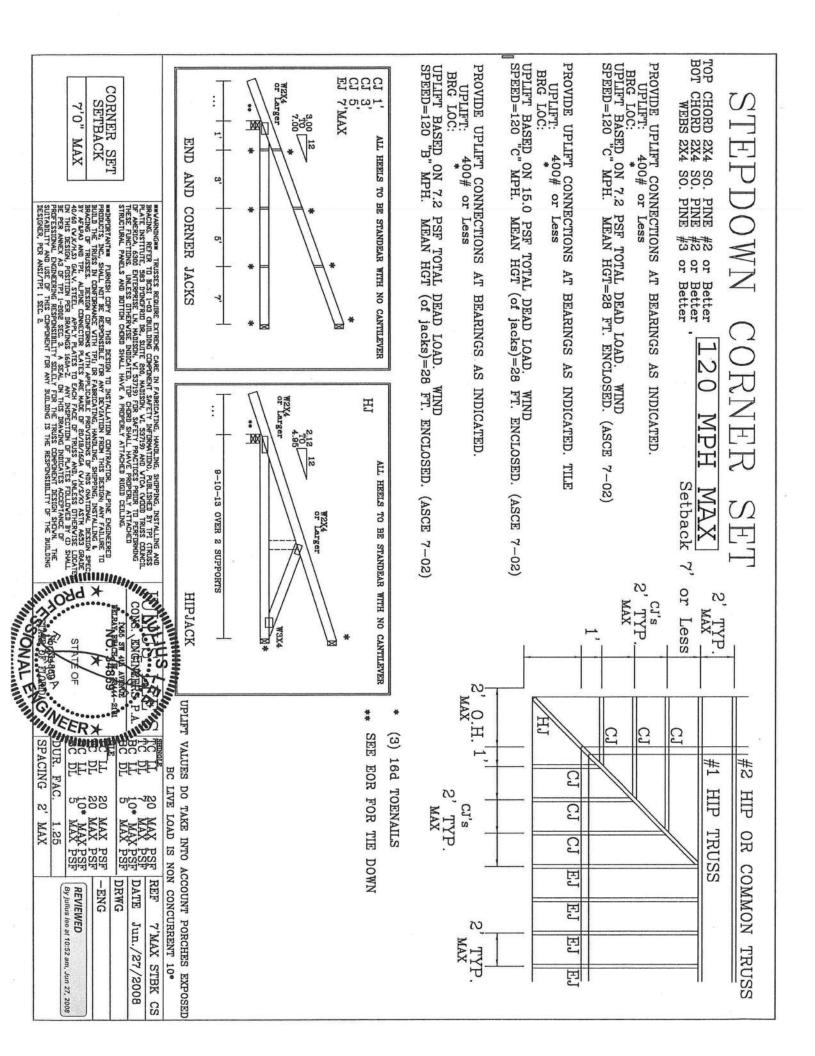
- 1) Unbalanced roof live loads have been considered for this design.
  2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical left and right exposed; porch left and right exposed; burner DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Special provisions shall be made by the building designer to allow for 0.80in of horizontal movement due to live load and 1.05in due to total load.

- Provide adequate drainage to prevent water ponding.
   This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
   Bearing at joint(s) 18, 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 565 lb uplift at joint 18 and 565 lb uplift at joint 10. 8) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

  9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435







### ASCE 7-02: 130 MPH WIND SPEED, 15 MEAN HEIGHT, ENCLOSED, Н II 1.00, EXPOSURE C

		M	Α	X		C	i	A.I	3]	L	E		V	E	R	r	Ί	C	A	L		L	Æ	N	10	¥ .	ГН	
	1	2	91		0	.(	3.			1	6	33	9	0	.(	3			2	4	33	1	0	. (	C	•	PACING	GABLE
		1 1 1	7	2	TIT	H	J. J.	CDT			1	77	)	TIT	I I	OFF	CDT			1	77	j	TIL	H	STI	TTT	SPACING   SPECIES	GABLE VERTICAL
STANDARD	STUD	#3	#22	41	STANDARD	STUD	#3	£1 / #2	STANDARD	STUD	<b>†</b> 3	#23	+1	STANDARD	STUD	**	和 / #2	STANDARD	STUD	<b>1</b> 3	#23	+1	STANDARD	STUD	₽\$	\$1 / #2	GRADE	BRACE
4. 9		4' 4"	4. 7.		4.	4. 2.	4.	4.	3, 10,	4' 0"	4. 0.	4' 2"	4 3"	3' 8'	3' 8"	3' 8"	3, 10,		3' 8"					3' 3"	3' 3"	3' 4'	BRACES	Z O
6 1	7' 1"	2	7' 4"	7' 4"	6' 11°	6' 11°	6' 11"		5' 3"	6' 1°	6	B' 8"	B' B"	ςŋ N	8' 0"		8 8	4' 3"	U. 0.	5, 0,	6' 10"	5' 10°		4' 11"	4' 11"	6' 10"	GROUP A	,T, PXI [1]
6. 1.	7' 1"	7, 5,	7' 11"	7' 11"	6' 11°	6' 11.	6' 11°		5' 3"	6' 1"	6. 5.	7' 2"	7' 2"	6, 5,	6′0"	8'0"	6, 10,	4' 3"	5. 0.	6′0"	6' 3°	6' 3"	4' 2"	4' 11"	4' 11"	6° 0"	GROUP H	* BRACE +
B. 0.		B, 8,		8	7' 10"	B' 9"		B' 8"	6' 11°	7' 11"		7' 11"		6, 10,	7' 11"		7' 11"	5' 8*	8, 2,	13455	6′ 11°	6' 11"		6′ 5″	8. 8,	6′ 11″	GROUP A	'T' 1X6 (1)
8. 0.		8,		8, 2,	7' 10"		ଟ' ୫"		6' 11°	B' 1°	B' 2"	B' 8"	B' 6"			7' 11"	8' 1"	5' 8"			7' 5"	7' 50		6, 5,	e, 9,	7' 1"	GROUP B	" BRACE *
10' 5"		10' 5"		10' 5°		10' 5"	10' 6"	10' 6"	9' 4"	8, 2,	9' 6"			80,	8' 5*	100	100	7' B*		B' 3"	1		1	œ.	8 3	8' 3"	GROUP A	(2) 2X4 "L"
10' 8"		10' 11"		11' 2"	10' 6"	10' 5"		10. 8 <sub>4</sub>	9, 4,	8, 11,,		10′ 2°	10' 2"	8, 5,	g, 5"	9, 2,	9. 9.	10.5	8' 8"			B' 11°	7′ 5*		8' 3"	8, 8,	GROUP B	BRACE **
122		13' 8"		13' 8"				13' 8"	10' 10"	12' 5"	12' 6'	12' 6"	12' 5"	10. 2.		12' 4"	12, 6,		10' 3"	DI		10, 10,	8,	10' D"	10' 1"	10' 10'	GROUP A	,T, 9XC (1)
123	14' 0"	14' 0"	14' 0"	14'0"	12, 3,	19' 6"	13' 6"	14' 0"	10' 10"	12, 6,	18' 8"	18' 5"	19' 5"	10' 7"	12' 4"	12' 4"	12, 8,	B' 10"	10' 3"	10' 4'	11' 8"	11' 8"	8, 8,	10' 0"	10, 1,	11, 2,	GROUP B	BRACE .
14' 0"	14. O.	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14. 0.	14' 0"	- 1	14. 0.	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14. 0.		12' 11"	12, 11,	12' 11"	12' 11°			12' 11"	12' 11"	GROUP B GROUP A GROUP	,T, 8XZ (2)
14' 0"	14. 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14. 0"	14' 0"	14 0	14. 0.	14' 0"	14' 0"	14. 0.	14' 0"	14' 0"		12' 0"	13' 7"	13' 7"	13' 11"	13' 11'				13′ 3"	GROUP B	HRACE **

DOUGLAS FIR-LARCH

SOUTHEEN POUE STANDARD

GROUP B: FLEX-PIR

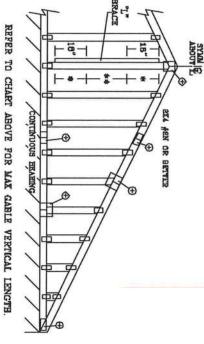
SPRUCE-PINE-INB
#1 / #2 STANDARD
#3 STUD

STANDARD

BRACING GROUP SPECIES

AND GRADES:

GROUP A:



DIAGONAL BHACE OPTION:
YESTICAL LENGTH MAY BE
DOUBLED WICHN DIAGONAL
HRACE IS USED. CONNECT
HIAGONAL BHACE TOR SAGE
AT EAGH YES
TOTAL LENGTH IS 14\*.

SEDEL THREE

VERTICAL LENGTH SHOWN

ZX4 SF #ZN, DF-L #Z,
SPF #/#Z, DR BETTER
DIAGONAL BRACE;
SINCLE OR DOUBLE
CUT (AS SERWA) AT

UPPER END.

BLE
TRUSS
DETAIL
NOTES:

SOUTHERN PINE

ABLE END SUPPOSIS LOAD FROM 4' 0" PROVIDE UPLET CONNECTIONS FUR 136 FLE OVER CONTINUOUS BEARING (6 PSP TC DEAD LOAD). IVE LOAD DEPLECTION CHITERIA IS L/240. PLYWOOD OVERHAMG.

ATTACH EAGH 'L' ERACE WITH 104 NAIS.

\* FOR (1) 'L' BRACE; SPACE WALES AF 2' O.C.

\* FUR (2) 'L' BRACES; SPACE WALES AF 3' O.C.

BY 18' END ZONES AND 6' O.C. BETWEEN ZONES.

BY 18' END ZONES AND 6' O.C. BETWEEN ZONES. I" BRACING MUST BE A MINIMUM OF BOX OF WEB

MINBER LENGTH.

PLATES.	PEAK, SPLICE, AND HEEL I
2.5X4	GREATER THAN 11' 6"
2344	CREATER THAN 4' D', BUT
DX4 OR EXS	LESS THAN 4' 0"
NO SPLICE	ARRINCYT CENCIH
E SIZES	GABLE VERTICAL PLATE

THE CONAL FROM	Odd STATE OF VEEK	*/	NO. 34869	THE PART OF THE PA	SA AMERICA	CONNECT DIAGONAL AT ALL	\
	REVIEWED  By Julius lee at 12:00 pm, Jun 11, 2008		MERICA, 6300 ENIDORRISE LM, MAJISON, VJ 337/9) FOR SAFETY PRACTICES PRIDE TO R 32 FUNCTIONS. UNLESS OTHERVISE (KOICATED, TOP GADES SHALL HAVE PROPERLY ATTA UCTURAL PARELS AND EDITINY CHRON SHALL HAVE A PROPER Y ATTACHEN RITHIN CRITICAL INC.	CING. REFER TO 3CS1 1-G3 (BUILLING CONFIDENT SAFETY INFORMATION), TE (NSTITUTE, 583 D'UNDFRID DR., SUITE 200, MADISON, VI. 53715) AND V	ARMINER TRINSS'S REDITOR FYTRENE DASE IN EABOYCATING HAND THE	7 4 A REFE	
			PROPORLY ATTACHED	ALCY (ADDO LEARS CONCOL)	~	REFER TO CHART ABOVE FOR	
No: 34869 STATE OF FLORIDA			DELRAY BEACH, FL S3444-2161	CONS. ENGINEERS P.A.		OR MAX GABLE VERTICAL LENGTH.	
NAX. SPACING 24.0"	MAX. TOT. LD. 60 PSF					NGTH.	*
24.0"	) PSF	-ENG	DRWG	DATE	REF		FRAN, SPEILE, MILL

WG MIES SID GABLE 15 E HI

11/26/09 ASCEY-02-GAB13015

### ASCE 7-02: 130 MPH WIND SPEED, 30 MEAN HEIGHT, ENCLOSED, I 11 1.00, EXPOSURE 0

BRACING GROUP SPECIES AND GRADES:

GROUP A:

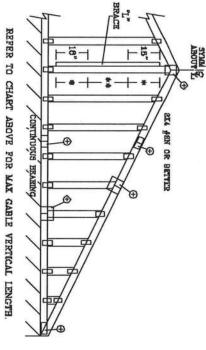
OUGLAS FIR-LARCE

SOUTHERN PORE

SPRUCE-PINE-IVE

STANDARD

	35	M	A	X		(	i	4]	3	L	E		V	F	R	ľ	ľ	C	A	L		L	E	N	10	¥ .	ГН	
	1	2	"	9	0	. (	3.			1	6	33		0	.(	3		0.55	2	4	<b>.</b>		0	١.(	С		SPACING	GARLI
	しばし	ブライ	טַ	2	TIT	H	CLL	ロロコ	No. Office of	ヒュー	1	7	)	TIT	H	OL L	CDT		ヒゖ.	1	7	) j	TTT.	L L	OLL	TTO	SPECIES	CABLE VERTICAL
STANDARD	COLE	¥3	#2	*1	STANDARD	STUD	<b>*</b> 3	£1 / #2	STANDARD	STUD	ż	#23	41	STANDARD	CUTS	₽\$	£1 / #2	STANDARD	STUD	<b>t</b> 3	#23	14	STANDARD	STUD	#8	打 / #2	GRADE	BRACE
4.0	44	4.	4. 4.	4. 5,	3' 11'	3' 11"	3' 11"		3' 8"	3. 8,	3, 8,	3' 11"	4. 0,		3' 7"	3' 7"		3' 0"	3.	3. 3.	3' 6"	3' 6"	2' 11'	3' 1"	3' 1"	3, 5,	BRACES	Z O
5.6	1	6, 6,			5' 4"	6' 3"	6. 3°	8' 11"	4' 9"	5' 6"	5. 3.	8' 4"	8' 4"	4. 8.	5, 6,	5.	8' 4"	3' 10"	4' 8"	4' 6'	5' 6"			4' 6"	4' 5"	5' 6"	GROUP A	"T. PXT (T)
5. 6.	1	6, 5,	1		5 4,			7. 2.		5, 8,	6. 7.	8' 10"	8' 10"	4' B"	6, 5,	5, 5,	6' 6"	3' 10"	4' 8"	4' 6"	5' 11'	5' 11"		4' 5"	4' 5"	6' 8"	GROUP H	BRACE .
7. 3	1	8, 3,	8' 3"					8' 3"	6" 3"		7' 4"	7' 8"		6' 2"	7' 2"	7' 2"			5' 11"	6, 0,	6, 8,		6' 0"	Same	6, 10,	8' 6"	GROUP A	(1) 2X4 "L"
7: 3-			8' 11"		7' 1"	e 3"	B' 3"	8. 8.	6' 3"	7' 3"	7' 4"	8' 1"	B' 1°		7' 2"	7' 20	7' 8"	5' 1"	5' 11"		7' 0"	7' 0"	5' 0"	5' 10"	5' 10"	8' 9"	GROUP B	L" BRACE *
8.8	1	9, 10,	8, 10,		9, 6,,	9' 10"	100	9' 10"	A: 5"	8, 11,	8' 11"	8' 11"	B' 11"	8, 3,	8' 11"	8, 11,	8' 11"	8' 11"	7' 10"	7' 10"	7' 10"	7' 10"	6, 9,	7' 10"	7' 10"	7' 10"	GROUP A	(2) 2X4 "L"
8, 8,		10′ 4″	10' 7"	10' 7"	9, 8,	9' 10"	8' 10"	10, 1,	8' 5"	9, 9,	8. 6.	8, 4,	8, 2,	8. 3.		B' 11"			8' 0"		8, 2,	8, 2,		7' 10"	7' 10"	8, 0,	GROUP B	BRACE **
11' 4"	12. 11.		12, 11,		11' 1"	18, 10,	12' 11"	12' 11"	8, 8,		11. 2.		11, 8,	8, 2,,	11, 1,	11, 5,		B, 0,		9' 4"	10′ 3″	10' 3"	7' 10"	9' 1"	8, 1,	10' 3"	GROUP A	(1) 2X6 T.
11' 4"	13. 1.	18' 3"		13' 11"	11' 1"	12' 10"	12' 11"	13' 4"	8, 8,	11, 4,	11' 6"			8, 4,	- 21	11, 2,	12' 1"			9. 4.	11, 1,		7' 10"	9, 1,	9' 1"	10′ 7″	GROUP B	BRACE *
14' 0"	14. O.	14' 0"	14' 0"	14' O"	14' 0"	14' 0"	14' O'	14. O.	13, 3,	14. 0	T4. 0"	14' 0"	14' Q*	18, 11,		14' 0"	14' Q"		12' 3"	12' 3'	12, 3,	12' 3"	10' 7"	12, 8,	12' 3"	12, 3,	UP B GROUP A GROUP	(2) 200 "L"
14' 0"	14. 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14. 00	14. 0.	13' 3"	14. 0	14. 0.	14" 0"	14' D"	12' 11.	14' D°		14. 0.	10' 10"	12' 6"	12, 8,	13' 2"	13' 2"	10' 7"		12' 3"	12, 7,	CROUP B	HRACE -



DIAGONAL BHACE OPTION:
VERTICAL LENGTH MAY BE
DOUBLED WHEN DIAGONAL
BRACE IS USED. CONDECT
DIAGONAL BRACE FOR SBUJ
AT EACH END. MAY WEB

CABLE TRUSS

TOTAL LEXICIH IS 14".

VERTICAL LENGTH SHOWN IN TABLE ABOVE.

ZK4 9P OR DIT-L #2 OB BETTIK DIAGONAL BRACE; SINGLE OR DOUBLE

AT UPPER END

TOTAL PROPERTY AND ADDRESS OF THE PARTY AND AD

EE
RUS
ETAIL
NOTE

SOUTHERN PINE

FIEL & BIR GROUP B:

PLYWOOD OVERHANG. CONTINUOUS BEADING (6 PSF TC DEAD LOAD). TE LOAD DEPLECTION CRETERIA IS L/240. BUT END SUPPORTS LOAD FROM 4, 0,

ATIMOE EACH "L" BRACE WITH 104 KAIES.

\$ FOR (1) "L" BRACES, SPACE WAIES AF 2" O.C.

\$ FOR (2) "L" BRACES; SPACE WAIES AT 3" O.C.

BY 18" END ZONES AND 4" O.C. BETWEEN ZONES.

BY 18" END ZONES AND 6" O.C. BETWEEN ZONES. T." BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

+ REFER TO COMMON THUSS DESIG	GREATER THAN 11' 6" 2.1	CREATER THAN 11' B' BUT Z	I ESS THAN 4' 0" IX4 C	ARRINCYT CENCIE NO 2	CUDIC APPLICATE LIVE OF
S. FOR	5X4	7XZ	OR EXTS	NOTICE.	Cathorical

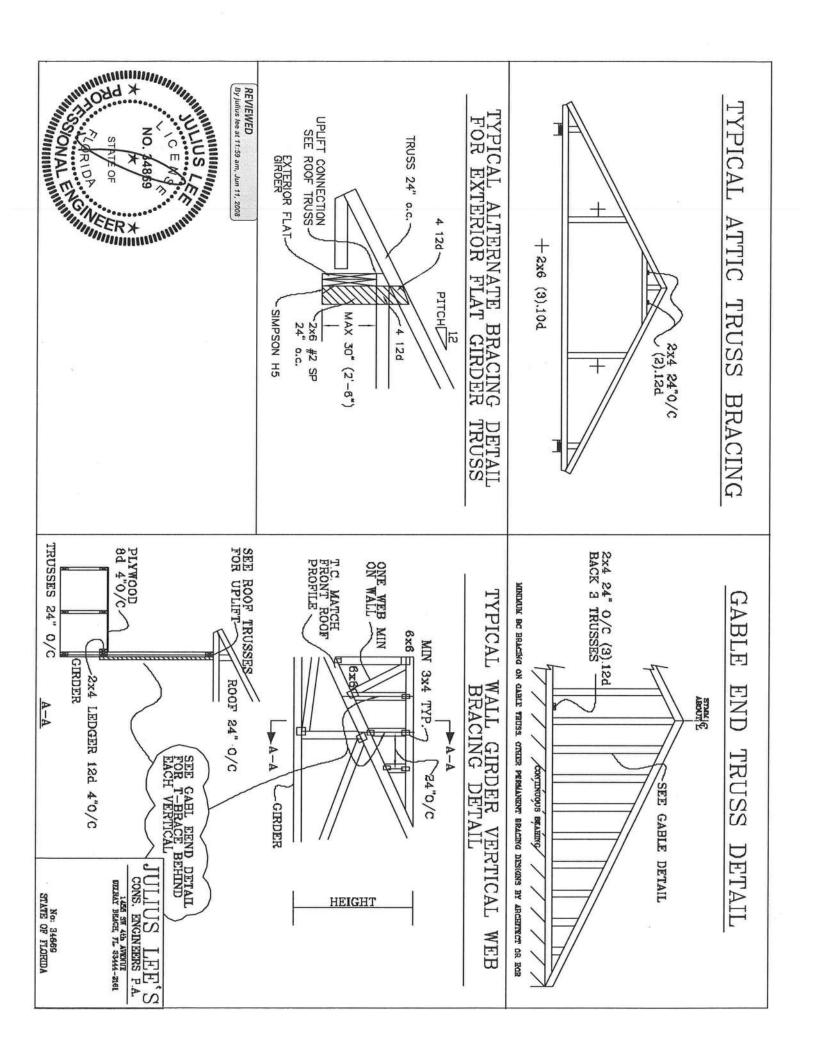
NO. 44869  NO. 44869  NO. 44869  NO. 44869  NO. 44869  NO. 44869	ORLEAY BENCH FL. 3344-2161				
STATE OF REVIEWED		MAX.	MAX. TOT. LD. 60 PSF	ID. 61	0
El DR 10 P Sy julius lee at 12:00 pm, Jun 11, 2008	VI. 0.050				
TO COMPANY	STATE OF ILDRIDA	AVA	SPACI	MAX. SPACING 24.0"	24

DATE REF

11/26/03 ASCE7-02-CAB13030

-ENG

DWC MIXER SED CARLE SO, E HI



BOT CHORD CHORD 2X4 2X4 202 222 BETTER BETTER BETTER

## PIGGYBACK DETAIL

TAPL

SPANS

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30

34

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52

REFER TO SEALED DESIGN FOR DASHED PLATES

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. TRUSS TOP CHORD WITH 1.5X3 PLATE. ATTACH VERTICAL WEBS TO

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BRNEATH THE TOP CHORD OF SUPPORTING TRUSS

REFER TO ENCINEER'S SEALED DESIGN FOR REQUIRED FURLIN SPACING

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

110 MPH WIND, 30' MEAN HGT, ASCE 7-03, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST CAT I, EXP C, WIND TO DL=5 PSF, WIND BC DL=5 PSF

130 MFH WIND, 30' MEAN HGT, ASCE 7-03, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=6 PSF, WIND HG DL=6 PSF

b

584

5

C

EXC.

1.5X4 5X6

1.6X4

1.5X4 PXG

ш

**4**X8

9X9

**6X8** 

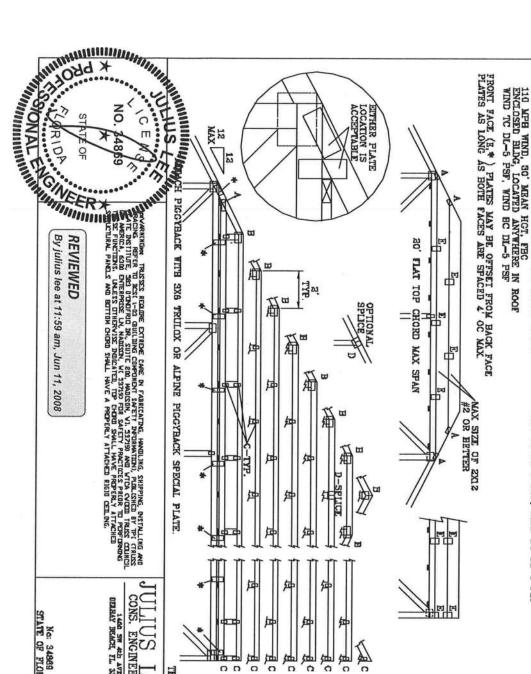
9XG

284

2.5X4

2.6X4

BXE



ATTACH TRULOX PLATES WITH (8) 0.120" X 1.375" NAILS, EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER BE CONNECTED. REFER TO DRAWING 160 TL FOR TRULOX INFORMATION 28 28

4XB OR SX8 TRULOX AT 4'
HOTATED VEHTICALLY

20,

	7'9" TO 10'	0' TO 7'9" ]	WEB LENGIH	
ZX4 T BRACE, SAME GRADE, SPECIES AS WEB	x4 "T" BRACE. SAME GRADE, SPECIES AS WEB ADMHER, OR HETTER, AND 80% LENGTH OF WEH ADMHER. ATTACH WITH 8d NAILS AT 4" OC.	O BRACING	REQUIRED BRACING	WEB BRACING CHART

\* PIGGYBACK SPECIAL PLATE

ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS STREAM (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS.

8 1/4" M

THIS DRAWING REPLACES DRAWINGS 634,016 834,017 & 847,045

CONS. ENGINEERS P.A. DENEMA BEACH, IL. 33444-2181 1.25 DUR. 55 PSF AT 1.33 DUR. FAC. MAX LOADING 50 PSF 47 PSF AT FAC. FAC DATE REF DRWG MITEK STD PIGG H 09/12/07 PIGGYBACK

C

No: 34869 STATE OF FLORIDA

SPACING

1.15 DUR.

### VALLEYTRUSS DETAIL

TOP CHORD SEEA 2X4 SP #2 OR SPF #1/#2 OR BETTER.
2X3(\*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER.
2X4 SP #3 OR BETTER.

- ZX3 MAY BE RIPPED FROM A ZX6 (PITCHED OR SQUARE).
- \* ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH ASCE 7-02 130 MPH WIND. 15' MEAN HEIGHT, ENCI FEC 2004 110 MPH, ASCE 7-02 110 MPH WIND ASCE 7-02 130 MPH WIND. 15' MEAN HEIGHT, (2) 18d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR OR (3) 16d FOR ENCLOSED

LARGER AS REQ'D

4-0-0

MAX

12 MAX.

EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9" UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.6") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING,

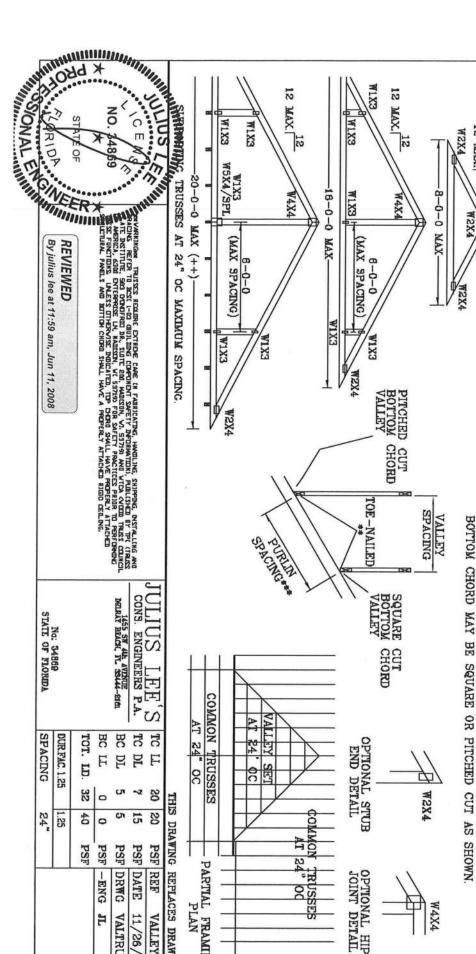
MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0".

TOP CHORD OF TRUSS BENEATH VALLEY SET NUST BE BRACED WITH: PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS INSTALLATION

PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN

BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON ENGINEERS' SEALED DESIGN

HENEATH THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS HENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD. ++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0".



PARTIAL FRAMING PLAN

THIS DRAWING REPLACES DRAWING A105

			ATTACHED RIGIO CEILING.	PRACTICES PRIOR TO PERFORMS	GLING, SHIPPING, INSTALLING AND		
STATE OF FLORIDA	No: 34869			DELRAY BEACK, I'L SSAAA-SICH	CONS. ENGINEERS P.A.	JULIUS LEE'S	
SPA	DUR	TOT	BC II	BC	TC	TC	١
SPACING	UR.FAC. 1.25	TOT. LD.	F	DI	DL	F	١
	i.ii	32	0	U	~2	20	l
24.	1.25	40	0	Ç1	15	20	١
		PSF	PSF	PSF	PSF	PSF	
	X-00 W		PSF -ENG JL	DRWG	DATE	PSF REF	
			II.	VALTRUSS1103	11/26/09	VALLEY DETAIL	

## TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AF&PA NDS-2001 SECTION 12,4.1 — EDGE DISTANCE, END DISTANCE, SPACING: "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

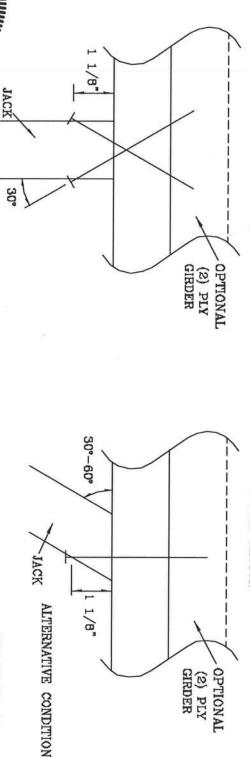
THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE, PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

## MAXIMUM VERTICAL RESISTANCE OF 16d (0.162"X3.5") COMMON TOE-NAILS

NUMBER OF	SOUTH	SOUTHERN PINE	DOUGLAS	DOUGLAS FIR-LARCH		HEM-FIR	SPRUCE PINE FIR	PINE
OE-NAILS	1 PLY	2 PLIES	1 PLY	2 PLIES	1 PLY	2 PLIES	1 PLY	2 PLIES
N	197#	256#	181#	234#	156#	203#	154#	189#
ω	296#	383#	271#	351#	234#	304#	230#	#88S
4.	394#	611#	361#	468#	312#	406#	307#	397#
σ	493#	639#	452#	585#	390#	507#	384#	496#
ALL VALUE	TO WAY F	HIGHLIN AN	DO AN AN	STATE OF STATE	NOTE	ALL VALUES WAY BE WILLIAMINED BY VEDENDERALE DISTRICTION OF LOVE ENCOUND	A CALL	

TI TIVIT IVINITE MOTIVITOR 5 TOAL PACTOR



WILLIAM STATE OF THE STATE OF T	THE CONTRACTOR	MO. 2000	STATE OF	<u>***</u>	NO. 34869	CERG	Will Control of the C	WHITHINININININI
		By Julius lee at 11:59 am, Jun 11, 2008	REVIEWED	2	MACTIO	WARDIG: TRUSSES REGURE EXTREME CARE IN FARRICATING, HANDLING, SUPPINIE, INSTALLING AND RECOVER. RETER TO BEST 1-43 CHILLING COMPONENT SAFETY (MEDINATION), PUBLISHED BY TPI CIRLISS		JACK
	STATE OF FLORIDA	No: 34869			DELRAY SEACH, FL SH444-2161	CONS. ENGINEERS P.A.	S, HHT SN	
	SPACING	DUR. FAC.	TOT. LD.	BC LL	BC DL	TC DL	TC LL	THIS DR
		1.00	PSF	PSF	PSF	PSF	PSF REF	AWING REI
				-ENG JL	DRWG CNTONAIL1103	DATE 09/12/07	REF TOE-NAIL	THIS DRAWING REPLACES DRAWING 784040

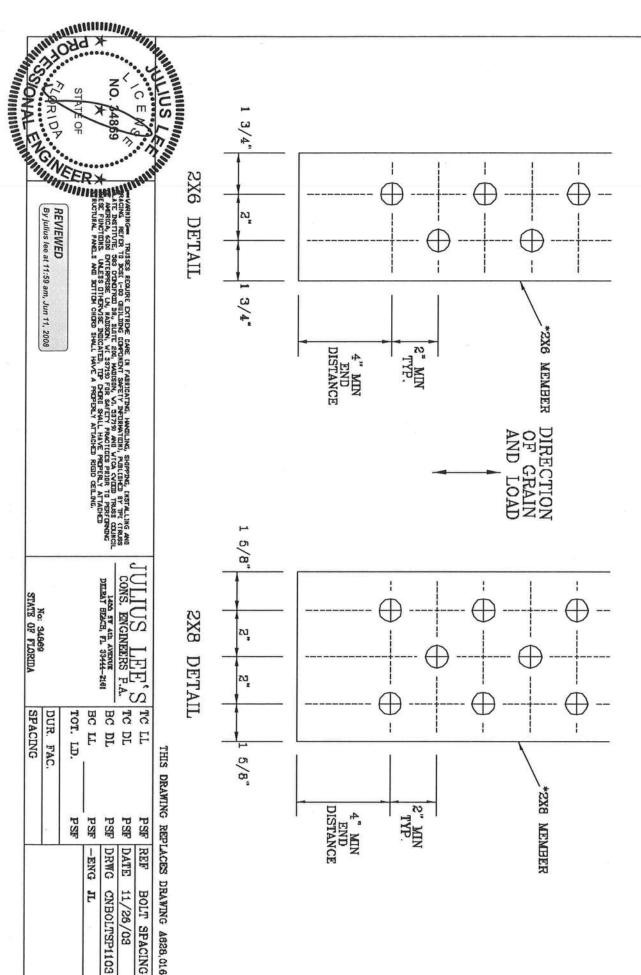
## DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN.

\* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN

BOLT HOLES SHALL BE A MINIMUM OF 1/S2" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. BOLT QUANTITIES AS NOTED ON SEALED DESIGN MUST BE APPLIED IN ONE OF THE PAITTERNS SHOWN BELOW.

WASHERS REQUIRED UNDER BOLT HEAD AND NUT



# TRULOX CONNECTION DETAI

11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (\( \phi \)).

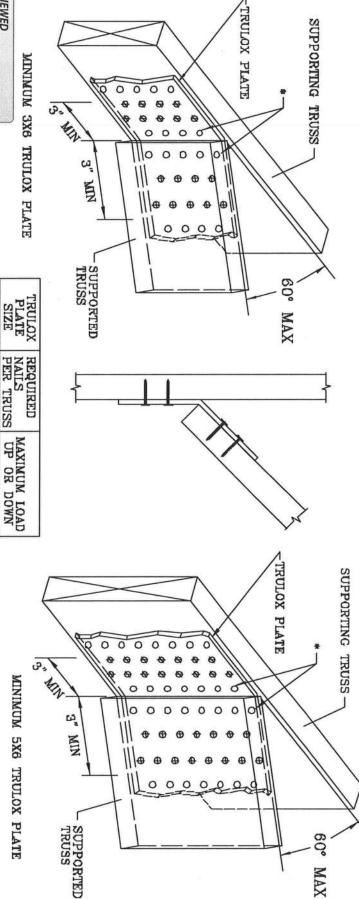
NAILS MAY BE OMITTED FROM THESE ROWS

THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE HICH TSUM

TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN

MAX



NO. 34869

WARRINGS PRUSSES REQUIRE EXTREME CARE IN FABRUANTING, HANDLING, SUPPENG, INSTALLING AND COMMENT OF THE REPORT OF THE PROPERTY IN THE SETTING OF THE SETING OF THE SETTING OF THE SET

By julius lee at 11:58 am, Jun 11, 2008

3X6

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SOLTOR

EE'S

1,154,844 1,152,217 1,152,017 1,159,154 & 1,151,524 THIS DRAWING REPLACES DRAWINGS 1,158,989 1,158,989/R

CONS. ENGINEERS P.A. DETRYL BEYCH, LT. 384444—5160

DRWG DATE REF

CNTRULOX1103

11/26/09 TRULOX

-ENG

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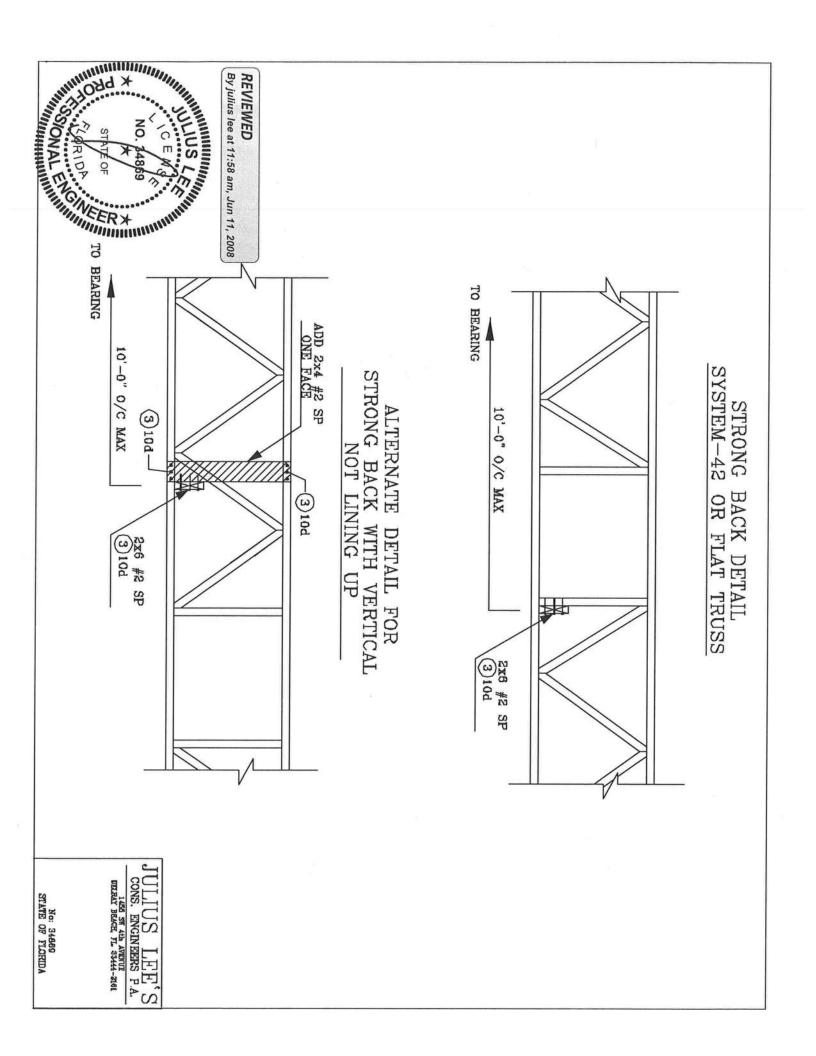
No: 34869 STATE OF FLORIDA

PER TRUSS

MAXIMUM LOAD
UP OR DOWN 350#

MINIMUM 5X6 TRULOX PLATE

REVIEWED



### MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

### Maximum Uniform Load Applied to Either Outside Member (PLF)

Connector Type Nu	Powe 0		Connector Pattern							
		Connector On-Center Spacing	Assembly A  2" 134"	Assembly B	Assembly C	Assembly D	Assembly E	Assembly F		
			3½" 2-ply	51/4" 3-ply	51/4" 2-ply	7"	7"	7" 4-ply		
10d (0.128" x 3")	2	12"	370	280	280	245				
Nail <sup>(1)</sup>	3	12"	555	415	415	370				
1/# 4007	THE PARTY OF	24"	505	380	520	465	860	340		
72" A3U/ Through Rolte(2)(4)	2	19.2"	635	475	655	580	1,075	425		
im ough boits		16"	760	570	785	695	2" 3½"  7" 2-ply  860 1,075 1,290  465 580 695  525 660 790  480 600 725 620	505		
		24"	680	510	510	455				
SDS 1/4" x 31/2"(4)	2	19.2"	850	640	640	565				
		16"	1,020	765	765	680				
		24"				455	465	455		
SDS 1/4" x 6"(3)(4)	2	19.2"				565	580	565		
		16"				680	695	680		
	S 1/4" x 31/2"(4) 2 S 1/4" x 6"(3)(4) 2 SP WS35 (4) 2	24"	480	360	360	320				
USP WS35 (4)	2	19.2"	600	450	450	400				
		16"	715	540	540	480				
	40.4	24"				350		350		
USP WS6 (3)(4)	2	19.2"				440	660	440		
		16"				525	2"   1   31/2"	525		
21/8		24"	635	475	475	425				
33/8" TrussLok <sup>(4)</sup>	2	19.2"	795	595	595	530				
II nasrok	DAUGUE AR	16**	955	715	715	635	1 2" 1 3½"  7" 2-ply  860 1,075 1,290  465 580 695  525 660 790  480 600 725 620 770			
-		24"		500	500	445	480	445		
5" TrussLok(4)	2	19.2"		625	625	555	600	555		
II USSLUK."		16"		750	750	665	2" 1 3½"  7" 2-ply  860 1,075 1,290  465 580 695  525 660 790  480 600 725 620 770	665		
01/8		24"			LINE STATE	445	620	445		
63/4" TrussLok <sup>(4)</sup>	2	19.2"				555		555		
II USSLUK.		16"				7" 3-ply 245 370 465 580 695 455 565 680 455 565 680 320 400 480 350 440 525 425 530 635 445 555 6665 445	925	665		

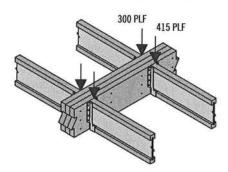
Nailed connection values may be doubled for 6" on-center or tripled for 4" on-center nail spacing.

- (2) Washers required. Bolt holes to be 9/16" maximum.
- (3) 6\* SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.
- (4) 24" on-center bolted and screwed connection values may be doubled for 12" on-center spacing.

### **General Notes**

- Connections are based on NDS® 2005 or manufacturer's code report.
- Use specific gravity of 0.5 when designing lateral connections.
- Values listed are for 100% stress level. Increase 15% for snow-loaded roof conditions or 25% for non-snow roof conditions, where code allows.
- Bold Italic cells indicate Connector Pattern must be installed on both sides.
   Stagger fasteners on opposite side of beam by ½ the required Connector Spacing.
- Verify adequacy of beam in allowable load tables on pages 16–33.
- 7" wide beams should be side-loaded only when loads are applied to both sides
  of the members (to minimize rotation).
- Minimum end distance for bolts and screws is 6".
- Beams wider than 7" require special consideration by the design professional.

### **Uniform Load Design Example**



First, check the allowable load tables on pages 16–33 to verify that three pieces can carry the total load of 715 plf with proper live load deflection criteria. Maximum load applied to either outside member is 415 plf. For a 3-ply  $1\frac{1}{4}$ " assembly, two rows of 10d (0.128" x 3") nails at 12" on-center is good for only 280 plf. Therefore, use three rows of 10d (0.128" x 3") nails at 12" on-center (good for 415 plf).

### Alternates:

Two rows of 1/2" bolts or SDS 1/4" x 31/2" screws at 19.2" on-center.

### Point Load—Maximum Point Load Applied to Either Outside Member (lbs)

		Connector Pattern						
Connector Type	Number of Connectors	Assembly A  2" 1 2" 134"	Assembly B	Assembly C	Assembly D	Assembly E  2°  1  2"  2"  31/3"	Assembly F	
		31/2" 2-ply	51/4" 3-ply	51/4" 2-ply	7" 3-ply	7" 2-ply	7" 4-ply	
	6	1,110	835	835	740			
10d (0.128" x 3")	12	2,225	1,670	1,670	1,485			
Nail	18	3,335	2,505	2,505	2,225			
	24	4,450	3,335	3,335	2,965			
SDS Screws	4	1,915	1,435(4)	1,435	1,275	1,860(2)	1,405(2)	
/4" x 31/2" or WS35	6	2,870	2,150 (4)	2,150	1,915	2,785(2)	2,110(2)	
1/4" x 6" or WS6(1)	8	3,825	2,870 (4)	2,870	2,550	3,715(2)	2,810(2)	
02/8 58	4	2,545	1,910 (4)	1,910	1,695	1,925(3)	1,775(3)	
33/8" or 5" TrussLok™	6	3,815	2,860 (4)	2,860	2,545	2,890 <sup>(3)</sup>	2,665(3)	
HUSSEUK	8	5,090	3,815 (4)	3,815	3,390	3,855(3)	3,550(3)	

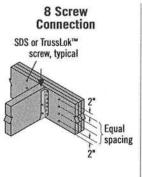
(1) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.

See General Notes on page 38

- (2) 6" long screws required.
- (3) 5" long screws required.
- (4) 31/2" and 35/8" long screws must be installed on both sides.

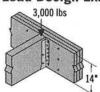
### Connections

## 4 or 6 or Screw Connection SDS or TrussLok™ screw, typical 2", typical top and bottom ½ beam depth



## Nail Connection 10d (0.128" x 3") nails, typical. Stagger to prevent splitting. 8"-10" 2" spacing, typical 2" 1½" minimum spacing, 2" typical There must be an equal number of nails on each side of the connection

### Point Load Design Example



First, verify that a 3-ply 1¾" x 14" beam is capable of supporting the 3,000 lb point load as well as all other loads applied. The 3,000 lb point load is being transferred to the beam with a face mount hanger. For a 3-ply 1¾" assembly, eight 3¾" TrussLok™ screws are good for 3,815 lbs with a face mount hanger.

### **MULTIPLE-MEMBER CONNECTIONS FOR TOP-LOADED BEAMS**

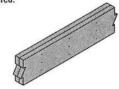
### 134" Wide Pieces

- Minimum of three rows of 10d (0.128" x 3") nails at 12" on-center.
- Minimum of four rows of 10d (0.128" x 3") nails at 12" on-center for 14" or deeper.
- If using 12d-16d (0.148"-0.162" diameter) nails, the number of nailing rows may be reduced by one.
- Minimum of two rows of SDS, WS, or TrussLok™ screws at 16" on-center. Use 3¾" minimum length with two or three plies; 5" minimum for 4-ply members. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. For 3- or 4-ply members, connectors must be installed
- on both sides. Stagger fasteners on opposite side of beam by ½ of the required connector spacing.
- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded heares.

### 31/2" Wide Pieces

■ Minimum of two rows of SDS, WS, or TrussLok™ screws, 5" minimum length, at 16" on-center. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. Connectors must be installed on both sides. Stagger fasteners on opposite side of beam by ½ of the required connector spacing.

- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.
- Minimum of two rows of ½" bolts at 24" on-center staggered.



Multiple pieces can be nailed or bolted together to form a header or beam of the required size, up to a maximum width of 7"

### **Residential System Sizing Calculation**

Summary

Dan and Gail Nickelson SW Emory Wood Glen Lake City, Fl Project Title: Dan and Gail Nickelson

Code Only Professional Version Climate: North

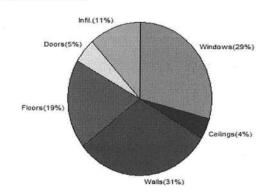
12/4/2008

				12/4/200	U
Location for weather data: Gaine	sville - Defa	aults: Latifu	ude(29) Altitude(152 ft.) Temp Ran	ige(M)	
Humidity data: Interior RH (50%					
Winter design temperature	33	F	Summer design temperature	92	F
Winter setpoint	70	F	Summer setpoint	75	F
Winter temperature difference	37	F	Summer temperature difference	17	F
Total heating load calculation	81822	Btuh	Total cooling load calculation	73469	Btuh
Submitted heating capacity	% of calc	Btuh	Submitted cooling capacity	% of calc	Btuh
Total (LP Gas Heat Pump)	44.0	36000	Sensible (SHR = 0.75)	39.8	27000
Heat Pump + Auxiliary(0.0kW)	44.0	36000	Latent	161.5	9000
The control of the co	910,000		Total (LP Gas Heat Pump)	49.0	36000

### WINTER CALCULATIONS

Winter Heating Load (for 3600 sqft)

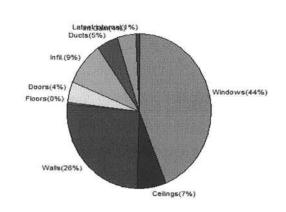
Load component			Load	
Window total	512	sqft	24039	Btuh
Wall total	3514	sqft	25212	Btuh
Door total	208	sqft	4156	Btuh
Ceiling total	2958	sqft	3486	Btuh
Floor total	See detail rep	oort	15694	Btuh
Infiltration	228	cfm	9235	Btuh
Duct loss			0	Btuh
Subtotal			81822	Btuh
Ventilation	0	cfm	0	Btuh
<b>TOTAL HEAT LOSS</b>			81822	Btuh



### **SUMMER CALCULATIONS**

Summer Cooling Load (for 3600 sqft)

Load component			Load	
Window total	512	sqft	32157	Btuh
Wall total	3514	sqft	19166	Btuh
Door total	208	sqft	3145	Btuh
Ceiling total	2958	sqft	4899	Btuh
Floor total			290	Btuh
Infiltration	120	cfm	2233	Btuh
Internal gain			3090	Btuh
Duct gain			2918	Btuh
Sens. Ventilation	0	cfm	0	Btuh
Total sensible gain			67898	Btuh
Latent gain(ducts)			586	Btuh
Latent gain(infiltration)			4385	Btuh
Latent gain(ventilation)			0	Btuh
Latent gain(internal/occup	ants/othe	r)	600	Btuh
Total latent gain		(500)	5571	Btuh
TOTAL HEAT GAIN			73469	Btuh



Version 8
For Florida residences only

EnergyGauge® System Sizing
PREPARED BY: 1000 J. 10114
DATE: 12408

### **System Sizing Calculations - Winter**

### Residential Load - Whole House Component Details

Dan and Gail Nickelson SW Emory Wood Glen Lake City, FI Project Title: Dan and Gail Nickelson

Code Only Professional Version Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F

12/4/2008

### Component Loads for Whole House

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
1	1, Clear, Metal, 1.27	E	12.0	47.0	564 Btul
2	1, Clear, Metal, 1.27	E	13.3	47.0	626 Btu
3	1, Clear, Metal, 1.27	E	17.5	47.0	822 Btu
4	1, Clear, Metal, 1.27	E E E	30.0	47.0	1410 Btu
5 6	1, Clear, Metal, 1.27	E	16.0	47.0	752 Btu
6	1, Clear, Metal, 1.27	E	16.5	47.0	775 Btul
7	1, Clear, Metal, 1.27	Ε.	15.0	47.0	705 Btul
8	1, Clear, Metal, 1.27	E	30.0	47.0	1410 Btu
9	1, Clear, Metal, 1.27	E	16.0	47.0	752 Btu
10	1, Clear, Metal, 1.27	E	16.5	47.0	775 Btu
11	1, Clear, Metal, 1.27	E	34.0	47.0	1598 Btu
12	1, Clear, Metal, 1.27	W	9.0	47.0	423 Btu
13	1, Clear, Metal, 1.27	W	68.8	47.0	3231 Btul
14	1, Clear, Metal, 1.27	W	48.0	47.0	2256 Btul
15	1, Clear, Metal, 1.27	W	66.0	47.0	3101 Btul
16	1, Clear, Metal, 1.27	N	12.0	47.0	564 Btul
17	1, Clear, Metal, 1.27	N	15.0	47.0	705 Btul
18	1, Clear, Metal, 1.27	S	32.0	47.0	1504 Btul
19	1, Clear, Metal, 1.27	S	44.0	47.0	2068 Btul
	Window Total	Ü	512(sqft)	11.0	24039 Btul
Walls	Туре	R-Value	Area X	HTM=	Load
1	Frame - Wood - Ext(0.09)	11.0	1117	3.5	3919 Btul
2	Frame - Wood - Ext(0.24)	0.0	2398	8.9	21293 Btul
	Wall Total	0.0	3514	0.0	25212 Btul
Doors	Туре	-	Area X	HTM=	Load
1	Wood - Exterior		48	20.0	959 Btul
2	Wood - Exterior		64	20.0	1279 Btul
3	Wood - Exterior		24	20.0	480 Btul
4	Wood - Exterior		48	20.0	959 Btul
5	Wood - Exterior		24	20.0	480 Btul
	Door Total		208	20.0	4156Btul
Ceilings	Type/Color/Surface	R-Value	Area X	HTM=	Load
1	Vented Attic/D/Shin	30.0	2958	1.2	3486 Btul
39.5	Ceiling Total	00.0	2958	1.2	3486Btul
Floors	Type	R-Value	Size X	HTM=	Load
1	Raised Wood - Adj	3.5	150.0 sqft	6.0	893 Btul
2	Slab On Grade	0	339.0 ft(p)	43.7	14801 Btul
~	Floor Total	J	489	70.7	15694 Btul
	1 Iool Total		403		13094 Blui
			Envelope Su	btotal:	72587 Btul
Infiltration	Туре	ACH X Vol	ume(cuft) walls(sqft)	) CFM=	
	Natural	0.38	36000 3514	228.0	9235 Btul

## **Manual J Winter Calculations**

Residential Load - Component Details (continued)
Project Title: Cod

Dan and Gail Nickelson SW Emory Wood Glen Lake City, FI

Dan and Gail Nickelson

Code Only Professional Version Climate: North

12/4/2008

Ductload	(DLM of 0.000)	0 Btuh
All Zones	Sensible Subtotal All Zones	81822 Btuh

WHOLE HOUSE TOTA	LS	
	Subtotal Sensible Ventilation Sensible Total Btuh Loss	81822 Btuh 0 Btuh 81822 Btuh

#### **EQUIPMENT**

1. LP Gas Heat Pump # 36000 Btuh	1. LP Gas Heat Pump	#	36000 Btuh
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Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint) (Frame types - metal, wood or insulated metal) (U - Window U-Factor or 'DEF' for default)

(HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types )



# **System Sizing Calculations - Winter**

# 

Dan and Gail Nickelson SW Emory Wood Glen Lake City, FI

Dan and Gail Nickelson

Code Only Professional Version

Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F

12/4/2008

#### Component Loads for Zone #1: 2nd Floor

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
7	1, Clear, Metal, 1.27	E	15.0	47.0	705 Btuh
8	1, Clear, Metal, 1.27	E	30.0	47.0	1410 Btuh
9	1, Clear, Metal, 1.27	E	16.0	47.0	752 Btuh
10	1, Clear, Metal, 1.27	E E E	16.5	47.0	775 Btuh
11	1, Clear, Metal, 1.27	E	34.0	47.0	1598 Btuh
	Window Total		112(sqft)		5239 Btuh
Walls	Туре	R-Value	Area X	HTM=	Load
1	Frame - Wood - Ext(0.09)	11.0	1117	3.5	3919 Btuh
£1	Wall Total		1117		3919 Btuh
Doors	Туре		Area X	HTM=	Load
4	Wood - Exterior		48	20.0	959 Btuh
	Door Total		48		959Btuh
Floors	Туре	R-Value	Size X	HTM=	Load
1	Raised Wood - Adj	3.5	150.0 sqft	6.0	893 Btuh
	Floor Total		150		893 Btuh
	-	11010 Btuh			
Infiltration	Туре	ACH X Vol	ume(cuft) walls(sq	ft) CFM=	
	Natural	0.38	6420 1117	72.4	2934 Btuh
Ductload	Average sealed, Supply(R6	0-Attic), Retur	n(R6.0-Attic) (D	LM of 0.000)	0 Btuh
Zone #1	10	Sens	sible Zone Subt	otal	13944 Btuh

## **Manual J Winter Calculations**

Residential Load - Component Details (continued)

Project Title: Cod

Glen Dan and Gail Nickelson Project

Dan and Gail Nickelson SW Emory Wood Glen Lake City, FI

Code Only Professional Version Climate: North

12/4/2008

#### Component Loads for Zone #2: 1st Floor

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load				
1	1, Clear, Metal, 1.27	Е	12.0	47.0	564 Btuh				
2	1, Clear, Metal, 1.27	E	13.3	47.0	626 Btuh				
3	1, Clear, Metal, 1.27	E	17.5	47.0	822 Btuh				
4	1, Clear, Metal, 1.27	E	30.0	47.0	1410 Btuh				
5	1, Clear, Metal, 1.27	E	16.0	47.0	752 Btuh				
6	1, Clear, Metal, 1.27	E	16.5	47.0	775 Btuh				
12	1, Clear, Metal, 1.27	W	9.0	47.0	423 Btuh				
13	1, Clear, Metal, 1.27	W	68.8	47.0	3231 Btuh				
14	1, Clear, Metal, 1.27	W	48.0	47.0	2256 Btuh				
15	1, Clear, Metal, 1.27	W	66.0	47.0	3101 Btuh				
16	1, Clear, Metal, 1.27	N	12.0	47.0	564 Btuh				
17	1, Clear, Metal, 1.27	N	15.0	47.0	705 Btuh				
18	1, Clear, Metal, 1.27	S	32.0	47.0	1504 Btuh				
19	1, Clear, Metal, 1.27	S	44.0	47.0	2068 Btuh				
	Window Total		400(sqft)		18800 Btuh				
Walls	Туре	R-Value	Area X	HTM=	Load				
2	Frame - Wood - Ext(0.24)	0.0	2398	8.9	21293 Btuh				
	Wall Total		2398	10000000	21293 Btuh				
Doors	Туре		Area X	HTM=	Load				
1	Wood - Exterior		48	20.0	959 Btuh				
2	Wood - Exterior		64	20.0	1279 Btuh				
3	Wood - Exterior		24	20.0	480 Btuh				
5	Wood - Exterior		24	20.0	480 Btuh				
	Door Total		160	0.000000000000000000000000000000000000	3197Btuh				
Ceilings	Type/Color/Surface	R-Value	Area X	HTM=	Load				
1	Vented Attic/D/Shin	30.0	2958	1.2	3486 Btuh				
	Ceiling Total		2958	-117400077	3486Btuh				
Floors	Туре	R-Value	Size X	HTM=	Load				
2	Slab On Grade	0	339.0 ft(p)	43.7	14801 Btuh				
	Floor Total		339	E	14801 Btuh				
	Zone Envelope Subtotal: 6								
Infiltration	Туре	ACH X Vol	ume(cuft) walls(sqf	t) CFM=					
	Natural	0.38	29580 2398	155.6	6301 Btuh				
Ductload	Average sealed, Supply(R6.	0-Attic), Retur	n(R6.0-Attic) (D	LM of 0.000)	0 Btuh				
Zone #2		Sens	sible Zone Subt	otal	67878 Btuh				

## **Manual J Winter Calculations**

Residential Load - Component Details (continued)
Project Title: Continued

Dan and Gail Nickelson SW Emory Wood Glen Lake City, FI

Dan and Gail Nickelson

Code Only Professional Version Climate: North

12/4/2008

WHOLE HOUSE TOTAL	.s	
		0.4000 Pt 1
	Subtotal Sensible Ventilation Sensible	81822 Btuh 0 Btuh
	Total Btuh Loss	81822 Btuh

#### **EQUIPMENT**

1. LP Gas Heat Pump	#	36000 Btuh
		1

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint) (Frame types - metal, wood or insulated metal) (U - Window U-Factor or 'DEF' for default) (HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types )



# **System Sizing Calculations - Summer**

## Residential Load - Whole House Component Details

Dan and Gail Nickelson SW Emory Wood Glen Lake City, Fl Project Title: Dan and Gail Nickelson Code Only Professional Version Climate: North

Reference City: Gainesville (Defaults)

Summer Temperature Difference: 17.0 F

12/4/2008

#### **Component Loads for Whole House**

	Type*		Over	hang	Win	dow Area	a(sqft)	H	HTM	Load	
Window	Pn/SHGC/U/InSh/ExSh/IS	Ornt	Len	Hgt	Gross	Shaded	Unshaded	Shaded	Unshaded		
1	1, Clear, 1.27, B-M, N,N	Е	Oft.	Oft.	12.0	0.0	12.0	29	70	845	Btuh
2	1, Clear, 1.27, B-M, N,N	E	Oft.	Oft.	13.3	0.0	13.3	29	70	938	Btuh
3	1, Clear, 1.27, B-M, N,N	E	Oft.	Oft.	17.5	0.0	17.5	29	70	1232	Btuh
4	1, Clear, 1.27, B-M, N,N	E	Oft.	Oft.	30.0	0.0	30.0	29	70	2112	Btuh
5	1, Clear, 1.27, B-M, N,N	Е	Oft.	Oft.	16.0	0.0	16.0	29	70		Btuh
6	1, Clear, 1.27, B-M, N,N	E	Oft.	Oft.	16.5	0.0	16.5	29	70		Btuh
7	1, Clear, 1.27, B-M, N,N	E	Oft.	Oft.	15.0	0.0	15.0	29	70	1056	
8	1, Clear, 1.27, B-M, N,N	E	Oft.	Oft.	30.0	0.0	30.0	29	70		Btuh
9	1, Clear, 1.27, B-M, N,N	E	Oft.	Oft.	16.0	0.0	16.0	29	70	1126	
10	1, Clear, 1.27, B-M, N,N	Ē	Oft.	Oft.	16.5	0.0	16.5	29	70		Btuh
11	1, Clear, 1.27, B-M, N,N	E	Oft.	Oft.	34.0	0.0	34.0	29	70	2394	
12	1, Clear, 1.27, B-M, N,N	w	Oft.	Oft.	9.0	0.0	9.0	29	70	634	
13	1, Clear, 1.27, B-M, N,N	W	Oft.	Oft.	68.8	0.0	68.8	29	70	4840	
14			Oft.	Oft.	48.0	0.0			70		
15	1, Clear, 1.27, B-M, N,N	W					48.0	29	1000000	3379	
	1, Clear, 1.27, B-M, N,N	W	Oft.	Oft.	66.0	0.0	66.0	29	70	4647	
16	1, Clear, 1.27, B-M, N,N	N	Oft.	Oft.	12.0	0.0	12.0	29	29	351	Btuh
17	1, Clear, 1.27, B-M, N,N	N	Oft.	Oft.	15.0	0.0	15.0	29	29	439	
18	1, Clear, 1.27, B-M, N,N	S	Oft.	Oft.	32.0	0.0	32.0	29	34	1096	
19	1, Clear, 1.27, B-M, N,N	S	Oft.	Oft.	44.0	0.0	44.0	29	34		Btuh
	Window Total				512 (	sqft)				32157	Btuh
Walls	Туре	R-V			-Value	e Area(sqft)			HTM	Load	
1	Frame - Wood - Ext			11.0/	0.09	111	16.5		2.5	2765	Btuh
2	Frame - Wood - Ext			0.0/0.24		2397.9			6.8		Btuh
<del>177</del> .2	Wall Total			0.07	3514 (sqft)						Btuh
Doors	Type					Area (sqft)			НТМ	Load	Dian
							NEW 100 150				D: 1
1	Wood - Exterior						3.0		15.1	726	Btuh
2	Wood - Exterior						1.0		15.1	968	Btuh
3	Wood - Exterior						1.0		15.1	363	Btuh
4	Wood - Exterior						3.0		15.1	726	Btuh
5	Wood - Exterior						1.0		15.1		Btuh
	Door Total					20	8 (sqft)			3145	Btuh
Ceilings	Type/Color/Surface		R-Va	alue		Area(sqft)		HTM		Load	
1	Vented Attic/DarkShingle			30.0		2958.0			1.7		Btuh
	Ceiling Total			00.0		2958 (sqft)		1.7		4899	
Floors	The state of the s		R-Va	duo				НТМ			Diun
	Туре		N-Ve				ze		and the second	Load	
1	Raised Wood - Adj			3.5			50 (saft)		1.9	290	Btuh
2	Slab On Grade			0.0	339 (ft(p))				0.0		Btuh
	Floor Total					489.0 (sqft)					Btuh
						E	nvelope	Subtotal	l:	59656	Btuh
filtration	Туре		Α	СН	Volum	e(cuft)	wall area	(sqft)	CFM=	Load	
	SensibleNatural			0.20		36000	3514		228.0	2233	Btuh
Internal		(	Оссир			Btuh/oc	cupant	1	Appliance	Load	
gain				3		X 23			2400	3090	Btuh
							ensible E	nvelope		64980	
ouct load							(DGI	M of 0.0	45)	2918	Btuh
			Ener	avGau	ne® FI	<sub>CDB</sub> S,er	sible Lo	oad All	Zones	67898	Btuh <sub>c</sub>

Residential Load - Component Details (continued)
Project Title:

Dan and Gail Nickelson SW Emory Wood Glen Lake City, FI

Dan and Gail Nickelson

Code Only Professional Version Climate: North

12/4/2008

#### WHOLE HOUSE TOTALS

9	Sensible Envelope Load All Zones	64980	Btuh
	Sensible Duct Load	2918	Btuh
	Total Sensible Zone Loads	67898	Btuh
	Sensible ventilation	0	Btuh
	Blower	0	Btuh
Whole House	Total sensible gain	67898	Btuh
<b>Totals for Cooling</b>	Latent infiltration gain (for 54 gr. humidity difference)	4385	Btuh
	Latent ventilation gain	0	Btuh
	Latent duct gain	586	Btuh
	Latent occupant gain (3 people @ 200 Btuh per person)	600	Btuh
	Latent other gain	0	Btuh
	Latent total gain	5571	Btuh
4	TOTAL GAIN	73469	Btuh

EQUIPMENT		
1. Central Unit	#	36000 Btuh

\*Key: Window types (Pn - Number of panes of glass)

(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint) (U - Window U-Factor or 'DEF' for default)

(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R))

(ExSh - Exterior shading device: none(N) or numerical value)

(BS - Insect screen: none(N), Full(F) or Half(H))

(Ornt - compass orientation)



# **System Sizing Calculations - Summer**

# Residential Load - Room by Room Component Details Nickelson Project Title: Code C

Dan and Gail Nickelson SW Emory Wood Glen Lake City, FI

Dan and Gail Nickelson

Code Only **Professional Version** Climate: North

Reference City: Gainesville (Defaults)

Summer Temperature Difference: 17.0 F

12/4/2008

#### Component Loads for Zone #1: 2nd Floor

	Type*		Over	hang	Win	dow Area	a(sqft)	H	HTM	Load	
Window	Pn/SHGC/U/InSh/ExSh/IS	Ornt	Len	Hgt	Gross	Shaded	Unshaded	Shaded	Unshaded		
7 8	1, Clear, 1.27, B-M, N,N 1, Clear, 1.27, B-M, N,N	E E	Oft.	Oft.	15.0 30.0	0.0	15.0 30.0	29 29	70 70	1056 2112	
9	1, Clear, 1.27, B-M, N,N	F	Oft.	Oft.	16.0	0.0	16.0	29	70	1126	100000000000000000000000000000000000000
10	1, Clear, 1.27, B-M, N,N	E	Oft.	Oft.	16.5	0.0	16.5	29	70	1162	
11	1, Clear, 1.27, B-M, N,N	Ē	Oft.	Oft.	34.0	0.0	34.0	29	70	2394	
	Window Total				112 (	sqft)				7850	Btuh
Walls	Туре		R-Va	alue/L	I-Value		(sqft)		HTM	Load	
1	Frame - Wood - Ext			11.0/	0.09	111	16.5		2.5	2765	Btuh
	Wall Total					111	7 (sqft)		0.25*	2765	Btuh
Doors	Туре						(sqft)		HTM	Load	
4	Wood - Exterior					48.0			15.1		Btuh
	Door Total					48 (sqft)				726	Btuh
Floors	Туре		R-Va	alue		Size			HTM	Load	
1	Raised Wood - Adj			3.5	150 (sqft)			1.9	290	Btuh	
	Floor Total				150.0 (sqft)				290	Btuh	
			Zone Envelope Subtotal:							11630	Btuh
Infiltration	Type SensibleNatural		A	CH 0.20	Volume(cuft) wall area		(sqft)	CFM= 38.1	Load 710	Btuh	
Internal		(	Occup	oants		Btuh/oc	cupant	-	Appliance	Load	
gain				3		X 23			2400	3090	Btuh
						S	ensible E	nvelope	e Load:	15429	Btuh
Duct load	Average sealed, Supply	(R6.0-/	Attic),	Retu	n(R6.0	-Attic)		(DGM c	of 0.045)	693	Btuh
							Sensib	le Zone	Load	16122	Btuh

Residential Load - Component Details (continued)

Dan and Gail Nickelson SW Emory Wood Glen Lake City, FI

Dan and Gail Nickelson

Code Only Professional Version Climate: North

12/4/2008

#### Component Loads for Zone #2: 1st Floor

	Type*		Over	hang	Win	dow Area	(sqft)	H	HTM	Load	
Window	Pn/SHGC/U/InSh/ExSh/IS	Ornt	Len	Hqt	Gross		Unshaded	Shaded	Unshaded		
1	1, Clear, 1.27, B-M, N,N	Е	Oft.	Oft.	12.0	0.0	12.0	29	70	845	Btuh
2	1, Clear, 1.27, B-M, N,N	E	Oft.	Oft.	13.3	0.0	13.3	29	70	938	Btuh
3	1, Clear, 1.27, B-M, N,N	E	Oft.	Oft.	17.5	0.0	17.5	29	70	1232	
4	1, Clear, 1.27, B-M, N,N	E	Oft.	Oft.	30.0	0.0	30.0	29	70	2112	Btuh
5	1, Clear, 1.27, B-M, N,N	E	Oft.	Oft.	16.0	0.0	16.0	29	70	1126	Btuh
6	1, Clear, 1.27, B-M, N,N	E	Oft.	Oft.	16.5	0.0	16.5	29	70	1162	Btuh
12	1, Clear, 1.27, B-M, N,N	W	Oft.	Oft.	9.0	0.0	9.0	29	70	634	Btuh
13	1, Clear, 1.27, B-M, N,N	W	Oft.	Oft.	68.8	0.0	68.8	29	70	4840	Btuh
14	1, Clear, 1.27, B-M, N,N	W	Oft.	Oft.	48.0	0.0	48.0	29	70	3379	Btuh
15	1, Clear, 1.27, B-M, N,N	W	Oft.	Oft.	66.0	0.0	66.0	29	70	4647	Btuh
16	1, Clear, 1.27, B-M, N,N	N	Oft.	Oft.	12.0	0.0	12.0	29	29	351	Btuh
17	1, Clear, 1.27, B-M, N,N	N	Oft.	Oft.	15.0	0.0	15.0	29	29	439	Btuh
18	1, Clear, 1.27, B-M, N,N	S	Oft.	Oft.	32.0	0.0	32.0	29	34	1096	Btuh
19	1, Clear, 1.27, B-M, N,N	S	Oft.	Oft.	44.0	0.0	44.0	29	34	1507	Btuh
	Window Total				400 (	sqft)				24307	Btuh
Walls	Туре		R-Va	alue/U	-Value	Area(	sqft)		HTM	Load	
2	Frame - Wood - Ext			0.0/	0.24	239			6.8	16402	Btuh
	Wall Total					2398 (sqft)			0.0	16402	
Doors	Туре					Area			НТМ	Load	Dian
1	Wood - Exterior										D
2	Wood - Exterior					48.			15.1	726	Btuh
3	Wood - Exterior					64.			15.1	968	Btuh
5	Wood - Exterior					24. 24.			15.1	363	Btuh
	Door Total								15.1	2419	Btuh
Ceilings	Type/Color/Surface		R-Va	due	160 (sqft)		НТМ		Load	Diun	
1	Vented Attic/DarkShingle		11-06			Area(sqft)			24-252-447-200-1		
· ·				30.0		2958.0			1.7	4899	The second of
	Ceiling Total						3 (sqft)		2	4899	Btuh
Floors	Туре		R-Va	llue		Siz	e		HTM	Load	
2	Slab On Grade			0.0		33	9 (ft(p))	0.0		0	Btuh
	Floor Total					339.0	(sqft)			0	Btuh
						Zone Envelope Subtotal:					Btuh
nfiltration	Туре		Α	СН	Volum	e(cuft) w	all area	(sqft)	CFM=	Load	
	SensibleNatural			0.20		29580	2398		81.9	1524	Btuh
Internal		(	Occup	ants		Btuh/oc	cupant	P	ppliance	Load	
gain				0		X 230			0	0	Btuh
	Sensible Envelope Load								Load:	49551	Btuh
uct load	Average sealed, Supply	(R6.0-A	Attic),	Retur	n(R6.0-	Attic)		(DGM o	f 0.045)	2225	Btuh
							Sensib	le Zone	Load	51776 E	Btuh

Residential Load - Component Details (continued)

Relson Project Title: Cod

Glen Dan and Gail Nickelson Prof

Dan and Gail Nickelson SW Emory Wood Glen Lake City, FI

Code Only Professional Version Climate: North

12/4/2008

Residential Load - Component Details (continued)
Project Title: Continued

Dan and Gail Nickelson SW Emory Wood Glen Lake City, FI

Dan and Gail Nickelson

Code Only **Professional Version** Climate: North

12/4/2008

#### WHOLE HOUSE TOTALS

		1	
	Sensible Envelope Load All Zones	64980	Btuh
	Sensible Duct Load	2918	Btuh
	Total Sensible Zone Loads	67898	Btuh
	Sensible ventilation	0	Btuh
	Blower	0	Btuh
Whole House	Total sensible gain	67898	Btuh
<b>Totals for Cooling</b>	Latent infiltration gain (for 54 gr. humidity difference)	4385	Btuh
	Latent ventilation gain	0	Btuh
	Latent duct gain	586	Btuh
	Latent occupant gain (3 people @ 200 Btuh per person)	600	Btuh
	Latent other gain	0	Btuh
	Latent total gain	5571	Btuh
	TOTAL GAIN	73469	Btuh

EQUIPMENT								
1. Central Unit	#	36000 Btuh						

\*Key: Window types (Pn - Number of panes of glass)

(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint) (U - Window U-Factor or 'DEF' for default) (InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R)) (ExSh - Exterior shading device: none(N) or numerical value) (BS - Insect screen: none(N), Full(F) or Half(H))

(Ornt - compass orientation)



# **Residential Window Diversity**

### MidSummer

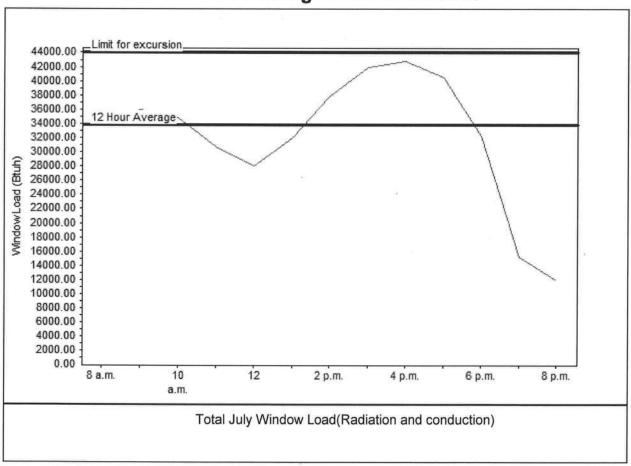
Dan and Gail Nickelson SW Emory Wood Glen Lake City, Fl Project Title: Dan and Gail Nickelson

Code Only Professional Version Climate: North

12/4/2008

Weather data for: Gainesville - Defaults							
Summer design temperature	92	Ę	Average window load for July	33862 Btu			
Summer setpoint	75	F	Peak window load for July	42797 Btu			
Summer temperature difference	17	F	Excusion limit(130% of Ave.)	44020 Btu			
Latitude	29	North	Window excursion (July)	None			

#### **WINDOW Average and Peak Loads**



The midsummer window load for this house does not exceed the window load excursion limit. This house has adequate midsummer window diversity.

EnergyGauge® System Sizing for Florida residences only

PREPARED BY:

12/4/08

ACC MANUAL J

EnergyGauge® FLRCPB v4.5.2



II DECEMBER 2008

JOE HALTIWANGER, BUILDING OFFICIAL COLUMBIA COUNTY, BUILDING DEPT. COLUMBIA COUNTY COURTHOUSE ANNEX LAKE CITY, FLORIDA 32055

RE: DAN NICKELSON RESIDENCE PERMIT Nr.:

DEAR SIR:

PLEASE BE ADVISED OF THE FOLLOWING CLARIFICATIONS FOR THE ABOVE REFERENCED RESIDENTIAL PROJECT:

- I. DORMERS SHALL BE CONSTRUCTED IN THE MANNER AS INDICATED ON THE ATTACHED DETAIL.
- 2. THE REAR BALCONY WAS PLANNED TO BE FRAMED WITH STEPPED FLOOR TRUSSES, HOWEVER THE TRUSS MANUFACTURER DID NOT INCLUDE THIS AREA IN THEIR FRAMING PLANS. THE BALCONY FLOOR SHALL BE CONSTRUCTED WITH 2XIØ LUMBER JOISTS @ 16" O.C., WITH A CURVED, BACK-SAWN, EDGE BAND AND A CONTINUOUS LEDGER AT THE BEARING HEADER OF THE FLOOR TRUSSES. THE HEADER SHALL BE ANCHORED TO THE SUPPORTING STRUCTURE W/ 1/2" & LAG SCREWS & 1 1/2" & WASHERS @ 24" O.C. THE JOISTS SHALL BE ATTACHED TO THE LEDGER WITH "SIMPSON" LUS28 HANGERS AND WITH "SIMPSON" H2.58 STRAPS AT THE CURVED EXTERIOR BEARING WALL, BELOW.
- 3. THE DECK AT THE REAR BALCONY SHALL BE 3/4" T&G OSB SHEATHING, WITH A TORCH APPLIED MODIFIED BITUMEN ROOFING, COVERED WITH MUD SET FLOOR TILES.
- 4. THE SAFEGUARD RAILING AT THE REAR BALCONY SHALL BE AS PER THE MANUFACTURERS SHOP DRAWINGS INCLUDING THE FOLLOWING FEATURES:
  HEIGHT ABOVE WALKING SURFACE SHALL BE NOT LESS THAN 42"
  ALL OPENINGS SHALL REJECT A 4"\$ BALL
  HANDRAIL SHALL MEET OR EXCEED ALL CODE MANDATED LOADING
- 5. THE WINDOWS AT THE DORMERS AND THE CENTER WINDOW OF THE WINDOW GROUP IN BEDROOM Nr.2 AT THE 2nd FLOOR SHALL BE DESIGNATED AS "EGRESS" WINDOWS AND SHALL MEET OR EXCEED ALL CODE MANDATED SIZE REQUIREMENTS FOR THE EGRESS OPENING.

- 6. MISSING FOUNDATION SIZES AT THE FRONT ENTRY AND THE DECORATIVE COLUMNS AS WELL AS THE WING WALL EXTENSION SHALL BE AS PER THE ATTACHED PARTIAL FOUNDATION PLAN.
- 1. THE CURVED BEAM ABOVE THE EXTERIOR WALL FORMING THE BREAKFAST NOOK SHALL BE BACK-SAWN 2XIO FACE MEMBERS ALONG WITH A 1/2" PLYWOOD FLITCH PLATE. THE FACE MEMBERS SHALL BE GLUED AND NAILED 12" O.C., STAGGERED, TOP AND BOTTOM, EACH SIDE W/ 160 NAILS.

SHOULD YOU HAVE ANY FURTHER QUESTIONS WITH THIS, PLEASE CALL FOR ASSISTANCE.

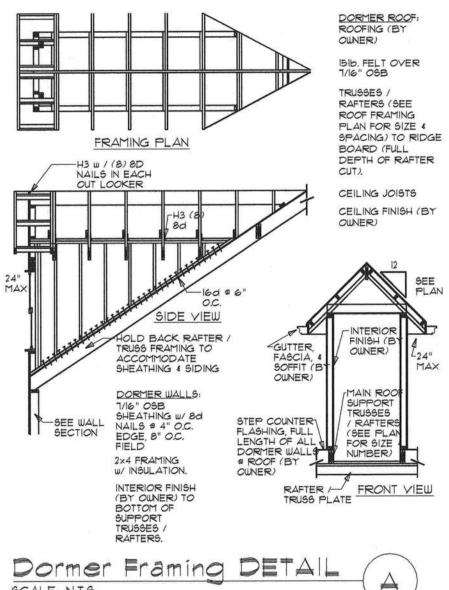
YOURS TRULY, NICHOLAS PAUL GEISLER, ARCHITECT AR0001005



ARCHITECT 1758 NW Brown Road Lake City, FL 32055 N.C.A.R.B. Certified 386/755-9021

NOTE!

ALTERNATE TO FRAMING SHOWN AT THE ROOF: IT IS PERMISSABLE TO FRAME WITH A SINGLE MEMBER OR SHAPED 2XIO ie: CURVED ROOF



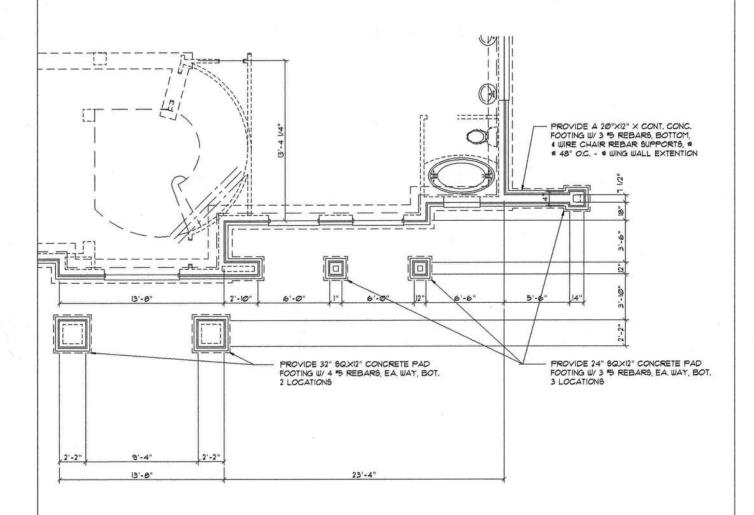
SCALE: N.T.S.

RE: DAN NICKELSON RESIDENCE

PERMIT Nr.:

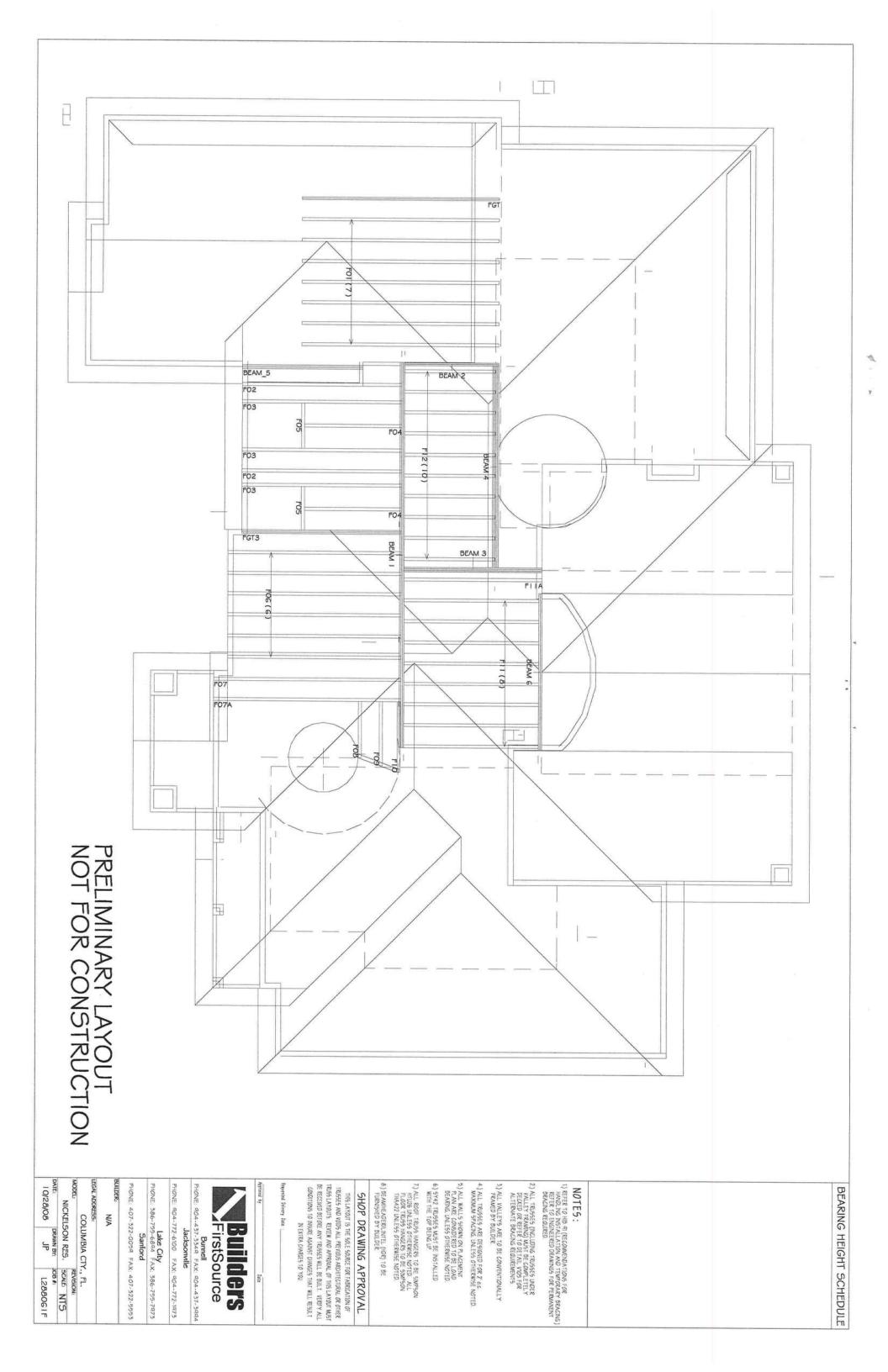
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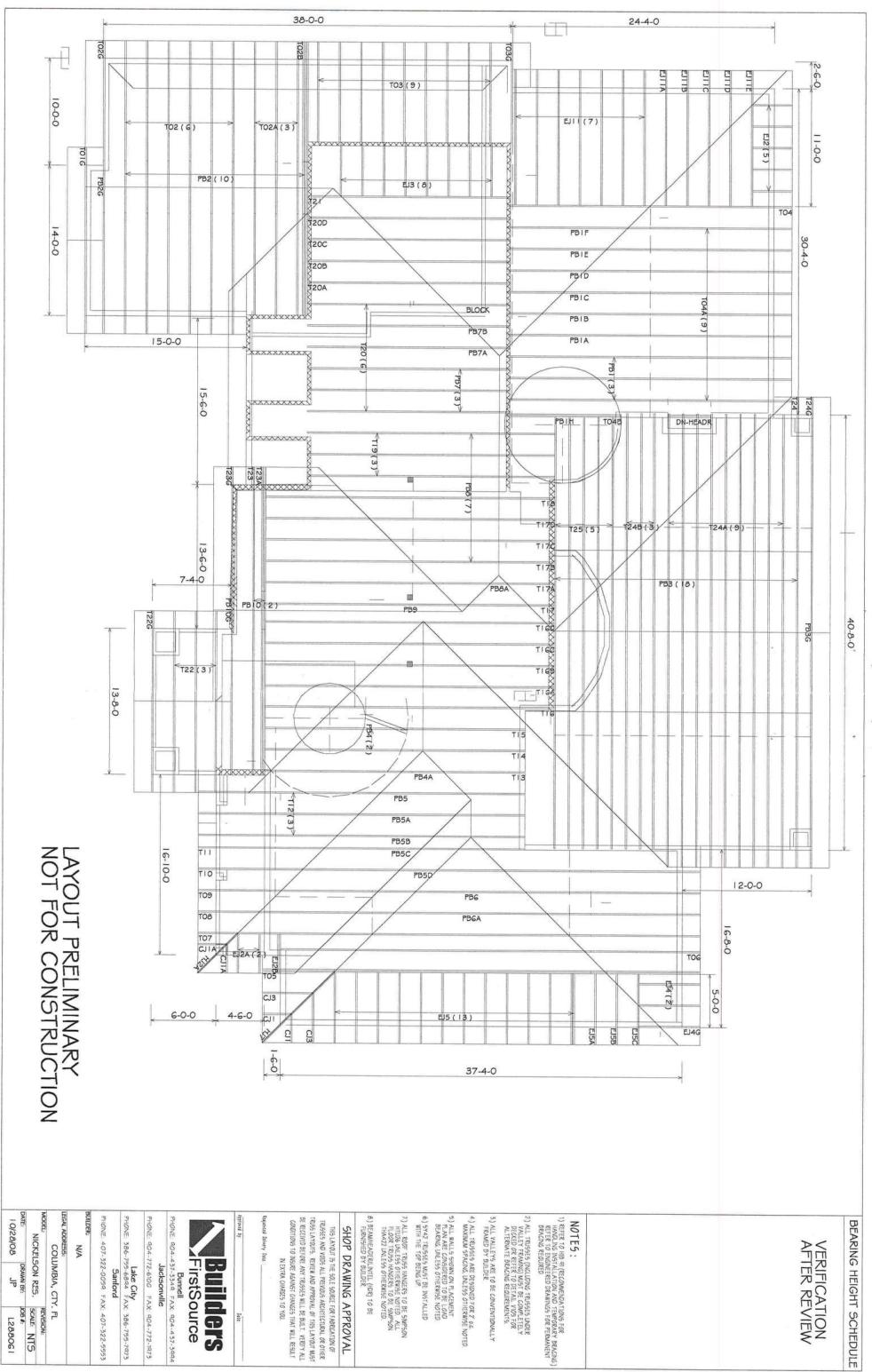




RE: DAN NICKELSON RESIDENCE PERMIT Nr.:

120-218





VERIFICATION AFTER REVIEW